

FOUNDATION LAYOUT

DIMENSIONS LOCATING STEEL PILES ARE TO THE PILE CENTERLINE AT BOTTOM OF CAP

NOTES

FOR PILES, SEE SECTION 450 OF THE STANDARD SPECIFICATIONS.

PILES AT END BENT NO.1 ARE DESIGNED FOR A FACTORED RESISTANCE OF 90 TONS PER PILE.

DRIVE PILES AT END BENT NO.1 TO A REQUIRED DRIVING RESISTANCE OF 150 TONS PER PILE.

PILES AT END BENT NO.2 ARE DESIGNED FOR A FACTORED RESISTANCE OF 90 TONS PER PILE.

DRIVE PILES AT END BENT NO.2 TO A REQUIRED DRIVING RESISTANCE OF 150 TONS PER PILE.

PZ-27 SHEET PILES ARE TO BE PLACED IN FRONT OF HP12×53 PILES AT END BENT NO.1.

SHEET PILES ARE INSTALLED TO AN ELEVATION OF 2135.0 FT AT END BENT NO.2.

SHEET PILES ARE INSTALLED TO AN ELEVATION OF 2135.0 FT AT END BENT NO.2.

PROJECT NO. B-4987

HENDERSON COUNTY

STATION: 12+51.50 -L-

SHEET 2 OF 3

DEPARTMENT OF TRANSPORTATION
RALEIGH

GENERAL DRAWING
FOR BRIDGE ON SR 1572
OVER CLEAR CREEK BETWEEN
SR 1591 AND SR 1587

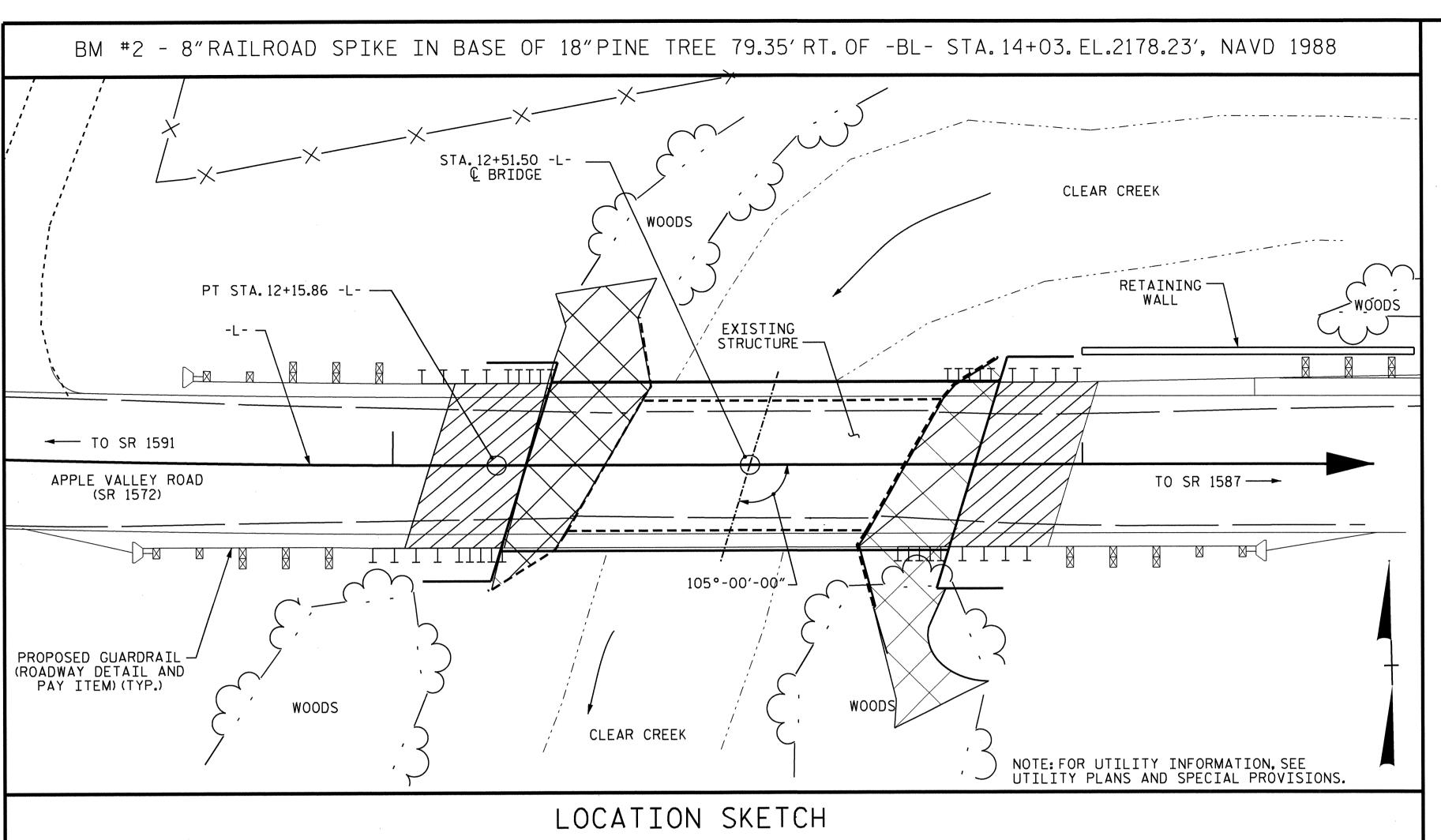
SEAL 15779

SEAL 15779

12/28/12

	SHEET NO.						
BY:	DATE:	NO.	BY:	DATE:	S-2		
		3			TOTAL SHEETS		
		4			13		

DRAWN BY: S. PEARCE DATE: 8/12
CHECKED BY: W.J. HARRIS DATE: 9/12



NOTES

ASSUMED LIVE LOAD = HL-93 OR ALTERNATE LOADING.

FOR OTHER DESIGN DATA AND GENERAL NOTES, SEE SHEET

PLANS.

FOR EROSION CONTROL MEASURES, SEE EROSION CONTROL

THIS BRIDGE HAS BEEN DESIGNED IN ACCORDANCE WITH THE AASHTO LRFD BRIDGE DESIGN SPECIFICATIONS.

THE EXISTING STRUCTURE CONSISTING OF ONE SPAN @ 44'-2" WITH A TIMBER FLOOR ON STEEL I-BEAMS SUPERSTRUCTURE WITH A CLEAR ROADWAY WIDTH OF 17'-4" ON A SUBSTRUCTURE CONSISTING OF TIMBER CAPS ON TIMBER POST AND SILLS END BENTS AND LOCATED AT THE PROPOSED STRUCTURE SHALL BE REMOVED.

REMOVAL OF THE EXISTING BRIDGE SHALL BE PERFORMED SO AS NOT TO ALLOW DEBRIS TO FALL INTO THE WATER. THE CONTRACTOR SHALL REMOVE THE BRIDGE AND SUBMIT PLANS FOR DEMOLITION IN ACCORDANCE WITH ARTICLE 402-2 OF THE STANDARD SPECIFICATIONS.

THE MATERIAL SHOWN IN THE CROSS-HATCHED AREA SHALL BE EXCAVATED FOR A DISTANCE OF 16' RIGHT AND 30' LEFT AT END BENT #1 AND 30' RIGHT AND 16' LEFT AT END BENT #2 FROM THE CENTERLINE ROADWAY AS DIRECTED BY THE ENGINEER. THIS WORK WILL BE PAID FOR AT THE CONTRACT LUMP SUM PRICE FOR UNCLASSIFIED STRUCTURE EXCAVATION. SEE SECTION 412 OF THE STANDARD SPECIFICATIONS.

THE SUBSTRUCTURE OF THE EXISTING BRIDGE INDICATED ON THE PLANS IS FROM THE BEST INFORMATION AVAILABLE. SINCE THIS INFORMATION IS SHOWN FOR THE CONVENIENCE OF THE CONTRACTOR, THE CONTRACTOR SHALL HAVE NO CLAIM WHATSOEVER AGAINST THE DEPARTMENT OF TRANSPORTATION FOR ANY DELAYS OR ADDITIONAL COST INCURRED BASED ON DIFFERENCES BETWEEN THE EXISTING BRIDGE SUBSTRUCTURE SHOWN ON THE PLANS AND THE ACTUAL CONDITIONS AT THE PROJECT SITE.

THIS STRUCTURE HAS BEEN DESIGNED IN ACCORDANCE WITH "HEC 18-EVALUATING SCOUR AT BRIDGES".

ASPHALT WEARING SURFACE IS INCLUDED IN ROADWAY QUANTITY ON ROADWAY PLANS.

FOR CRANE SAFETY, SEE SPECIAL PROVISIONS.

FOR GROUT FOR STRUCTURES, SEE SPECIAL PROVISIONS.

FOR SUBMITTAL OF WORKING DRAWINGS, SEE SPECIAL PROVISIONS.

FOR FALSEWORK AND FORMWORK, SEE SPECIAL PROVISIONS.

INASMUCH AS THE PAINT SYSTEM ON THE EXISTING STRUCTURAL STEEL CONTAINS LEAD, THE CONTRACTOR'S ATTENTION IS DIRECTED TO ARTICAL 107-1 OF THE STANDARD SPECIFICATIONS. ANY COSTS RESULTING FROM COMPLIANCE WITH APPLICABLE STATE OR FEDERAL REGULATIONS PERTAINING TO HANDLING OF MATERIALS CONTAINING LEAD BASED PAINT SHALL BE INCLUDED IN THE BID PRICE FOR "REMOVAL OF EXISTING STRUCTURE AT STATION 12+51.50 -L-"

THIS BRIDGE IS LOCATED IN SEISMIC ZONE 1.

FOR 18" STEEL SHEET PILES, SEE SPECIAL PROVISIONS.

THE CONTRACTOR SHALL PROVIDE INDEPENDENT ASSURANCE SAMPLES OF REINFORCING STEEL AS FOLLOWS: FOR PROJECTS REQUIRING UP TO 400 TONS OF REINFORCING STEEL, ONE 30 INCH SAMPLE OF EACH SIZE BAR USED, AND FOR PROJECTS REQUIRING OVER 400 TONS OF REINFORCING STEEL, TWO 30 INCH SAMPLES OF EACH SIZE BAR USED. THE BARS FROM WHICH THE SAMPLES ARE TAKEN MUST THEN BE SPLICED WITH REPLACEMENT BARS OF THE SIZE AND LENGTH OF THE SAMPLE, PLUS A MINIMUM LAP SPLICE OF THIRTY BAR DIAMETERS. PAYMENT FOR THE SAMPLES OF REINFORCING STEEL SHALL BE CONSIDERED INCIDENTAL TO VARIOUS PAY ITEMS.

	TOTAL BILL OF MATERIAL											
	REMOVAL OF EXISTING STRUCTURE	UNCLASSIFIED STRUCTURE EXCAVATION	CLASS A CONCRETE	BRIDGE APPROACH SLABS	REINFORCING STEEL		2 X 53 L PILES	VERTICAL CONCRETE BARRIER RAIL	ELASTOMERIC BEARINGS	PREST CON	X 2'-O'' TRESSED CRETE D SLABS	18" STEEL SHEET PILES
	LUMP SUM	LUMP SUM	CU. YDS.	LUMP SUM	LBS.	NO.	LIN.FT.	LIN.FT.	LUMP SUM	NO.	LIN.FT.	SQ.FT.
SUPERSTRUCTURE				LUMP SUM				130.26	LUMP SUM	9	585.00	
END BENT NO.1		LUMP SUM	19.8		2407	5	250					895
END BENT NO.2		LUMP SUM	19.8		2407	5	250					920
TOTAL	LUMP SUM	LUMP SUM	39.6	LUMP SUM	4814	10	500	130.26	LUMP SUM	9	585.00	1815

HYDRAULIC DATA

DESIGN DISCHARGE = 2,700 C.F.S
FREQUENCY OF DESIGN FLOOD = 25 YRS.
DESIGN HIGH WATER ELEVATION = 2152.9 FT.
DRAINAGE AREA = 14.0 SQ. M3
BASE DISCHARGE (Q100) = 3,880 C.F.S.
BASE HIGH WATER ELEVATION = 2154.9 FT.

OVERTOPPING FLOOD DATA

OVERTOPPING DISCHARGE _____ = 8,500 C.F.S. FREQUENCY OF OVERTOPPING FLOOD ___ = 500 YRS.+ OVERTOPPING FLOOD ELEVATION ____ = 2154.7 FT.

PROJECT NO. B-4987

HENDERSON COUNTY

STATION: 12+51.50 -L-

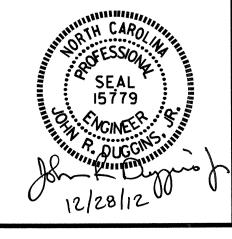
SHEET 3 OF 3

STATE OF NORTH CAROLINA

DEPARTMENT OF TRANSPORTATION

RALFIGH

GENERAL DRAWING
FOR BRIDGE ON SR 1572
OVER CLEAR CREEK BETWEEN
SR 1591 AND SR 1587



	REV:	ISIONS	5		SHEET NO
r:	DATE:	NO.	BY:	DATE:	S-3
		3			TOTAL SHEETS
		6			l 13

DRAWN BY: S. PEARCE DATE: 8/12
CHECKED BY: J.R. DUGGINS DATE: 8/12

						STRENGTH I LIMIT STATE								·	SERVICE III LIMIT STATE									
÷				· ·						MOMENT					SHEAR						MOMENT			
LEVEL		VEHICLE	WEIGHT (W) (TONS)	CONTROLLING LOAD RATING	MINIMUM RATING FACTORS (RF)	TONS = W X RF	LIVELOAD FACTORS	DISTRIBUTION FACTORS (DF)	RATING FACTOR	SPAN	GIRDER LOCATION	DISTANCE FROM LEFT END OF SPAN (ft)	DISTRIBUTION FACTORS (DF)	RATING FACTOR	SPAN	GIRDER LOCATION	DISTANCE FROM LEFT END OF SPAN (f+)	LIVELOAD FACTORS	DISTRIBUTION FACTORS (DF)	RATING FACTOR	SPAN	GIRDER LOCATION	DISTANCE FROM LEFT END OF SPAN (ft)	COMMENT NUMBER
		HL-93(Inv)	N/A	1	1.03		1.75	0.269	1.06	65′	EL	31.982	0.608	1.05	65′	EL	3.198	0.80	0.269	1.03	65′	EL	31.982	
DESIGN		HL-93(0pr)	N/A		1.362	, 	1.35	0.269	1.38	65′	EL	31.982	0.608	1.36	65′	EL	3.198	N/A						
LOAD RATING		HS-20(Inv)	36.000	2	1.296	46.666	1.75	0.269	1.36	65′	EL	31.982	0.608	1.3	65′	EL	3.198	0.80	0.269	1.32	65′	EL	31.982	
KATING		HS-20(0pr)	36.000		1.68	60.493	1.35	0.269	1.76	65′	EL	31.982	0.608	1.68	65′	EL	3.198	N/A						
		SNSH	13.500		2.898	39.127	1.4	0.269	3.74	65′	EL	31.982	0.608	3.82	65′	EL	3.198	0.80	0.269	2.90	65′	EL	31.982	
		SNGARBS2	20.000		2.194	43.878	1.4	0.269	2.83	65′	EL ·	31.982	0.608	2.73	65′	EL	3.198	0.80	0.269	2.19	65′	EL	31.982	
		SNAGRIS2	22.000		2.092	46.029	1.4	0.269	2.7	65′	EL	31.982	0.608	2.54	65′	EL	3.198	0.80	0.269	2.09	65′	EL	31.982	
		SNCOTTS3	27.250		1.443	39.328	1.4	0.269	1.86	65′	EL	31.982	0.608	1.91	65′	EL	3.198	0.80	0.269	1.44	65′	EL	31.982	
	NS [SNAGGRS4	34.925		1.219	42.576	1.4	0.269	1.57	65′	EL	31.982	0.608	1.59	65′	EL	3.198	0.80	0.269	1.22	65′	EL	31.982	
		SNS5A	35.550		1.191	42.349	1.4	0.269	1.54	65′	EL	31.982	0.608	1.62	65′	EL	3.198	0.80	0.269	1.19	65′	EL	31.982	
		SNS6A	39.950		1.098	43.884	1.4	0.269	1.42	65′	EL	31.982	0.608	1.48	65′	EL	3.198	0.80	0.269	1.10	65′	EL	31.982	
LEGAL		SNS7B	42.000		1.046	43.944	1.4	0.269	1.35	65′	EL	31.982	0.608	1.46	65′	EL	3.198	0.80	0.269	1.05	65′	EL	31.982	
LOAD RATING		TNAGRIT3	33.000		1.341	44.258	1.4	0.269	1.73	65′	EL	31.982	0.608	1.76	65′	EL	3.198	0.80	0.269	1.34	65′	EL	31.982	
NATINO		TNT4A	33.075		1.349	44.604	1.4	0.269	1.74	65′	EL	31.982	0.608	1.71	65′	EL	3.198	0.80	0.269	1.35	65′	EL	31.982	
		TNT6A	41.600		1.108	46.092	1.4	0.269	1.43	65′	EL	31.982	0.608	1.56	65′	EL	3.198	0.80	0.269	1.11	65′	EL	31.982	
	TST	TNT7A	42.000		1.116	46.888	1.4	0.269	1.44	65′	EL	31.982	0.608	1.52	65′	EL	3.198	0.80	0.269	1.12	65′	EL	31.982	
		TNT7B	42.000		1.162	48.806	1.4	0.269	1.5	65′	EL	31.982	0.608	1.42	65′	EL	3.198	0.80	0.269	1.16	65′	EL	31.982	
		TNAGRIT4	43.000		1.1	47.307	1.4	0.269	1.42	65′	EL	31.982	0.608	1.37	65′	EL	3.198	0.80	0.269	1.10	65′	EL	31.982	
		TNAGT5A	45.000		1.035	46.568	1.4	0.269	1.33	65′	EL	31.982	0.608	1.37	65′	EL	3.198	0.80	0.269	1.03	65′	EL	31.982	
	,	TNAGT5B	45.000	3	1.02	45.907	1.4	0.269	1.32	65′	EL	31.982	0.608	1.3	65′	EL	3.198	0.80	0.269	1.02	65′	EL	31.982	1

LRFR SUMMARY FOR SPAN 'A'

ASSEMBLED BY: V. NGUYEN DATE: 8/12 CHECKED BY: S. PEARCE DATE: 8/12 DRAWN BY: CVC 6/10 CHECKED BY: DNS 6/10

LOAD FACTORS:

DESIGN	LIMIT STATE	γ_{DC}	$\gamma_{\sf DW}$
LOAD RATING	STRENGTH I	1.25	1.50
FACTORS	SERVICE III	1.00	1.00

NOTES:

MINIMUM RATING FACTORS ARE BASED ON THE STRENGTH I AND SERVICE III LIMIT STATES. ALLOWABLE STRESSES FOR SERVICE III LIMIT STATE ARE AS REQUIRED FOR DESIGN.

COMMENTS:

- (#) CONTROLLING LOAD RATING
- 1 DESIGN LOAD RATING (HL-93)
- 2 DESIGN LOAD RATING (HS-20)
- 3 LEGAL LOAD RATING **
- ** SEE CHART FOR VEHICLE TYPE

GIRDER LOCATION

- I INTERIOR GIRDER
- EL EXTERIOR LEFT GIRDER
- ER EXTERIOR RIGHT GIRDER

PROJECT NO. B-4987 HENDERSON COUNTY STATION: 12+51.50 -L-

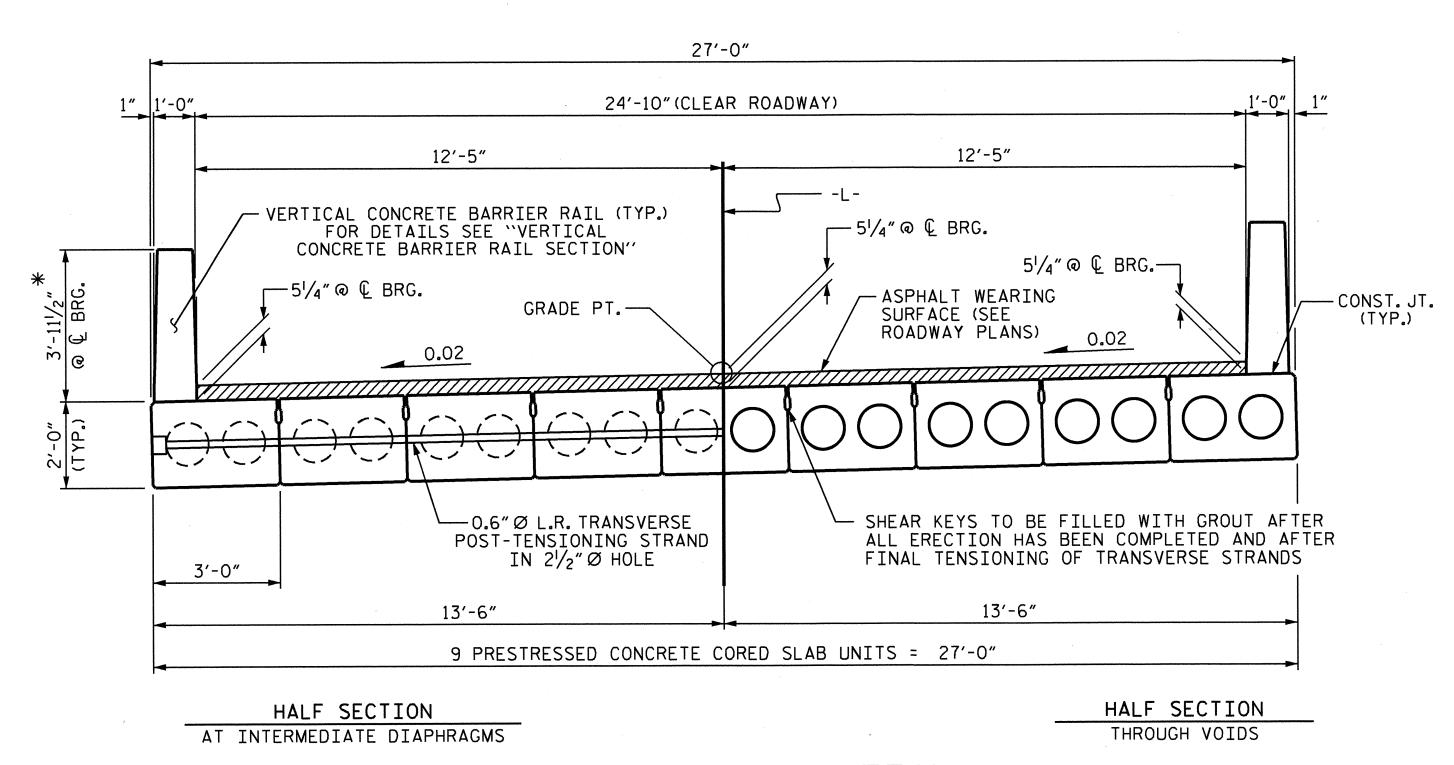
> STATE OF NORTH CAROLINA DEPARTMENT OF TRANSPORTATION
> RALEIGH

STANDARD LRFR SUMMARY FOR 65' CORED SLAB UNIT 75° SKEW & 105° SKEW

SHEET NO. S-4 REVISIONS BY:

SEAL 15779

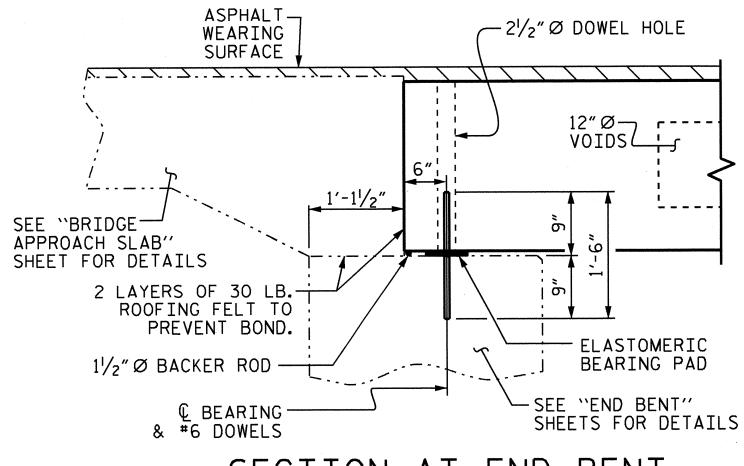
NOINEER STATES (NON-INTERSTATE TRAFFIC)



TYPICAL SECTION

* - THE MAXIMUM BARRIER RAIL HEIGHT AND ASPHALT THICKNESS IS SHOWN. THE HEIGHT OF THE BARRIER RAIL AND ASPHALT THICKNESS VARIES WHILE THE TOP OF THE BARRIER RAIL FOLLOWS THE PROFILE OF THE GUTTERLINE. FOR RAIL HEIGHT DETAILS AND ASPHALT THICKNESS, SEE THE "VERTICAL CONCRETE BARRIER RAIL SECTION" DETAIL.

FIXED END



DATE: 8/12

DATE: 8/12

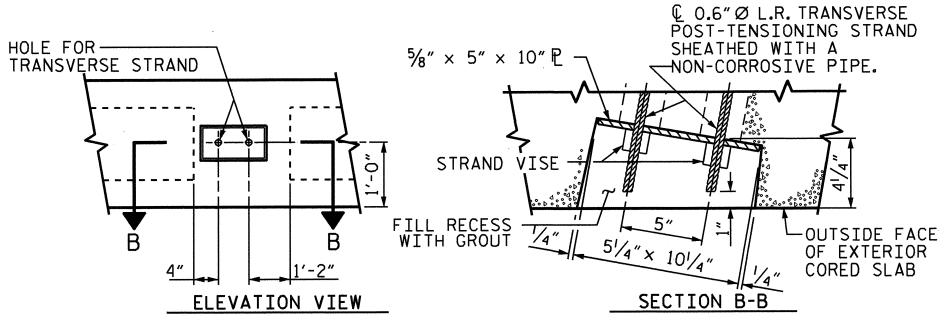
ASSEMBLED BY: V. NGUYEN

DRAWN BY: MAA 6/10 CHECKED BY : MKT 7/10

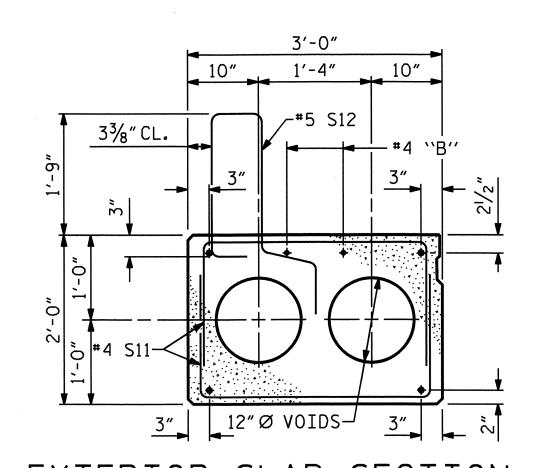
S. PEARCE

CHECKED BY :

SECTION AT END BENT

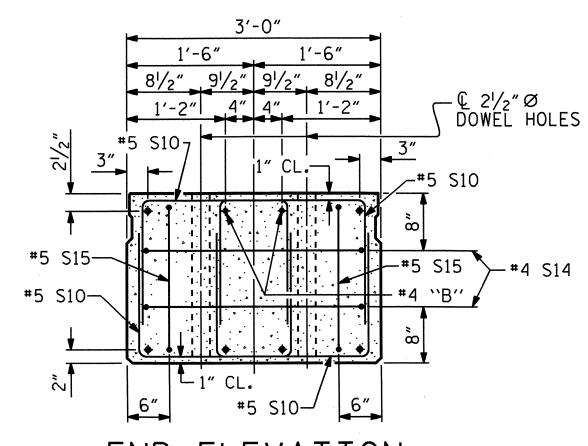


GROUTED RECESS AT END OF POST-TENSIONED STRAND CORED SLABS



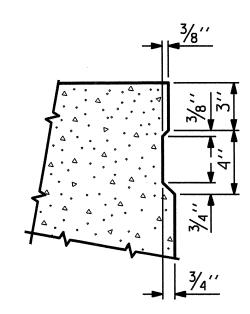
EXTERIOR SLAB SECTION

(FOR PRESTRESSED STRAND LAYOUT, SEE INTERIOR SLAB SECTION.)



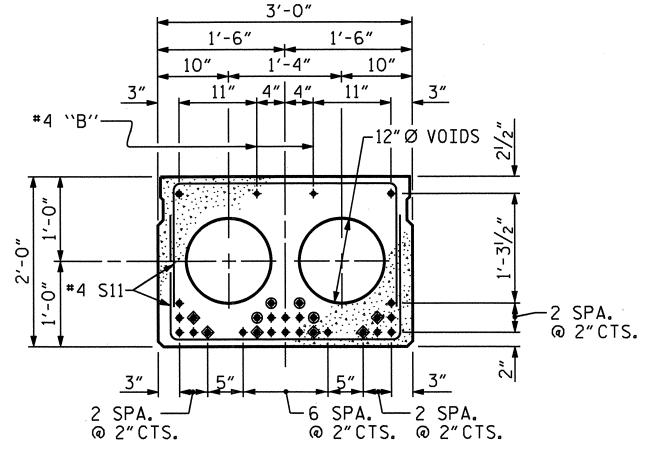
END ELEVATION

SHOWING PLACEMENT OF DOUBLE STIRRUPS AND LOCATION OF DOWEL HOLES. (STRAND LAYOUT NOT SHOWN.) INTERIOR SLAB UNIT SHOWN-EXTERIOR SLAB UNIT SIMILAR EXCEPT SHEAR KEY LOCATION.



SHEAR KEY DETAIL

NOTE: OMIT SHEAR KEY ON OUTSIDE FACE OF EXTERIOR CORED SLABS.



INTERIOR SLAB SECTION 65' UNIT (24 STRANDS REQUIRED)

0.6" Ø LOW RELAXATION STRAND LAYOUT

- BOND SHALL BE BROKEN ON THESE STRANDS FOR A DISTANCE OF 12'-0" FROM END OF CORED SLAB UNIT. SEE STANDARD SPECIFICATIONS, ARTICLE 1078-7.
- OPTIONAL FULL LENGTH DEBONDED STRANDS. THESE STRANDS ARE NOT REQUIRED. IF THE FABRICATOR CHOOSES TO INCLUDE THESE STRANDS IN THE CORED SLAB UNIT, THE STRANDS SHALL BE DEBONDED FOR THE FULL LENGTH OF THE UNIT AT NO ADDITIONAL COST. SEE STANDARD SPECIFICATIONS, ARTICLE 1078-7.

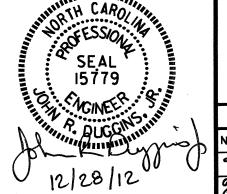
DEBONDING LEGEND

B-4987 PROJECT NO. __ HENDERSON COUNTY STATION: 12+51.50 -L-

SHEET I OF 3

STATE OF NORTH CAROLINA DEPARTMENT OF TRANSPORTATION RALEIGH STANDARD

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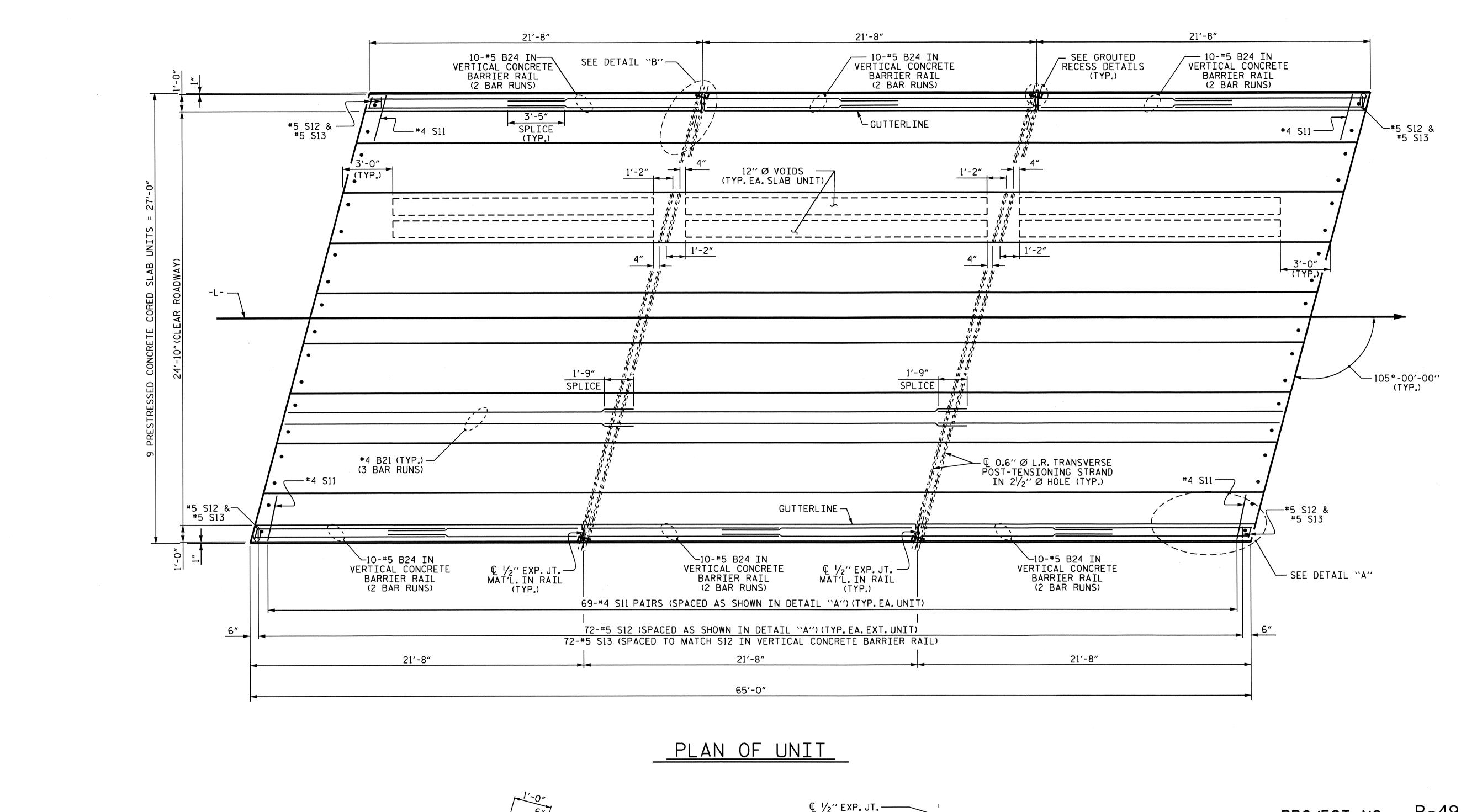


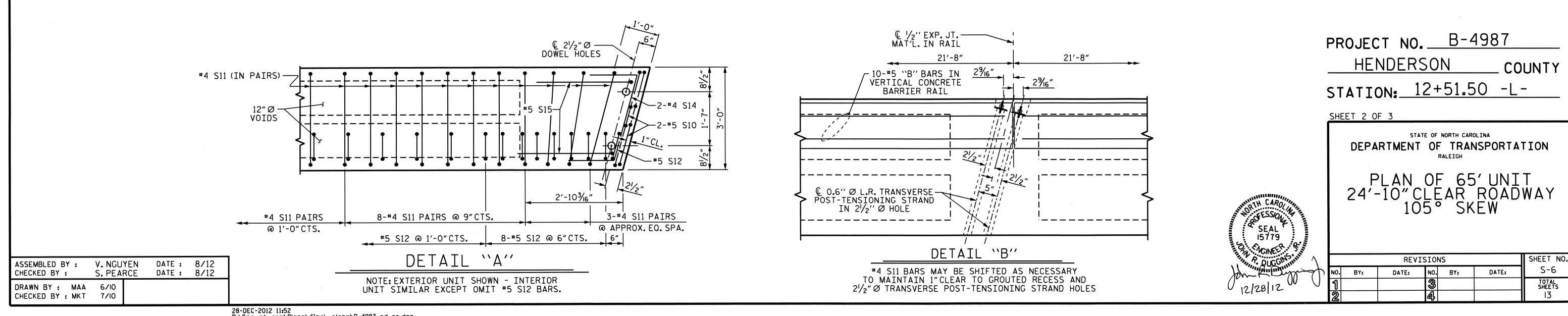
3'-0'' X 2'-0''
PRESTRESSED CONCRETE
CORED SLAB UNIT
CONED SEAD CITE

		SHEET NO.				
١0.	BY:	DATE:	NO.	BY:	DATE:	S-5
1			3			TOTAL SHEETS
2			4			13

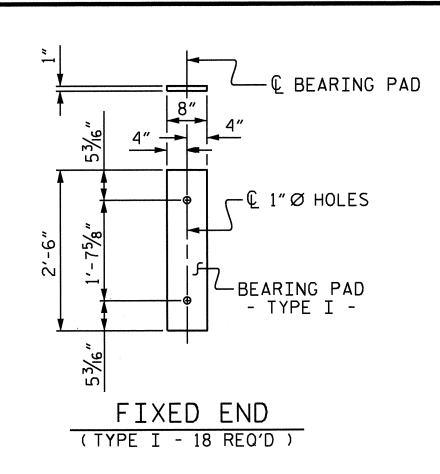
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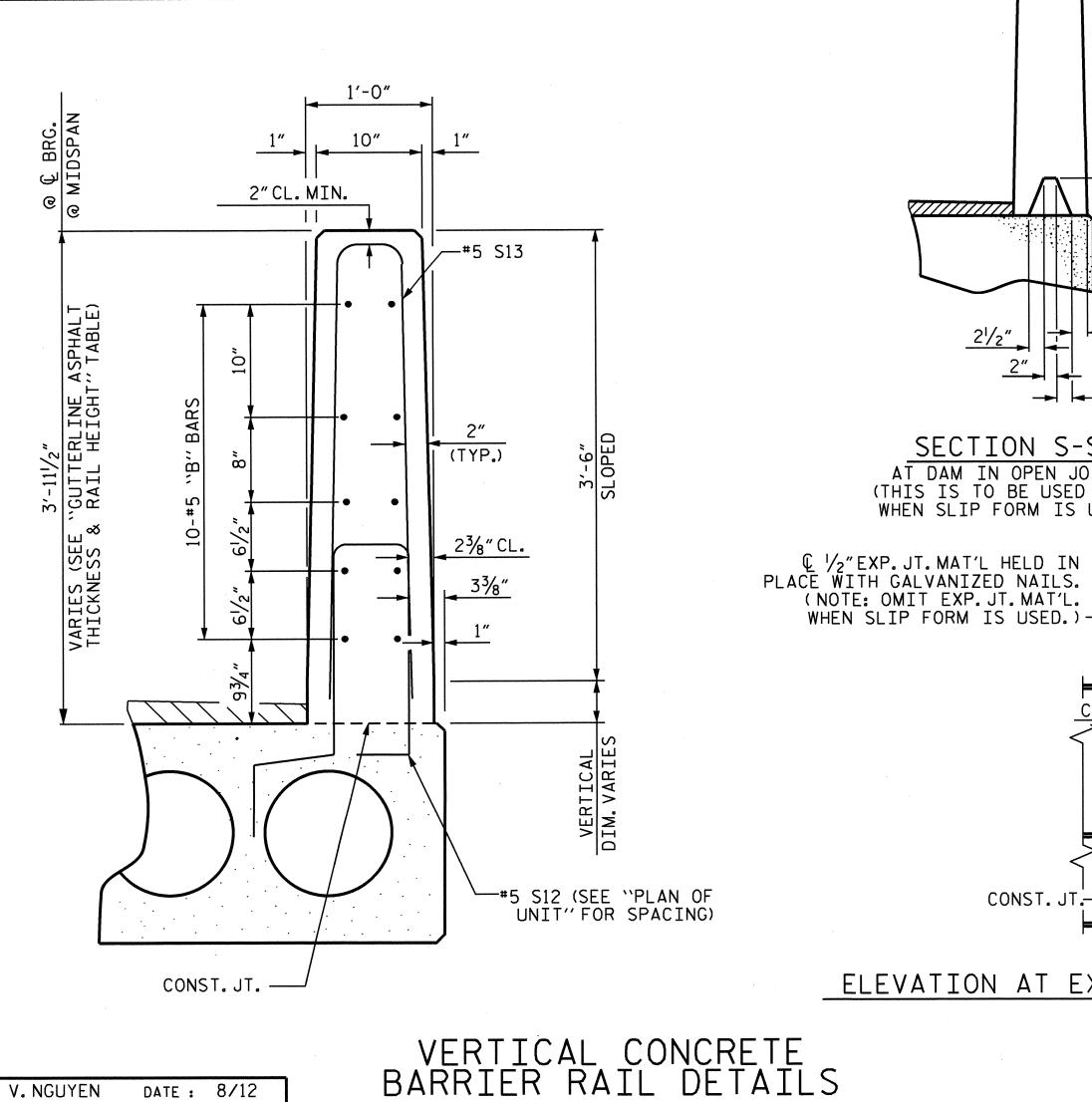
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ELASTOMERIC BEARING DETAILS

ELASTOMER IN ALL BEARINGS SHALL BE 60 DUROMETER HARDNESS.

BI	BILL OF MATERIAL FOR VERTICAL CONCRETE BARRIER RAIL										
BAR	BARS PER PAIR OF EXTERIOR UNITS	TOTAL NO.	SIZE	TYPE	LENGTH	WEIGHT					
	65' UNIT										
∗ B24	120	120	#5	STR	12′-10″	1606					
		·									
* S13	148	148	#5	2	7′-2″	1106					
* EPOX	Y COATED REINFORCING STEEL		1	LBS.		2712					
CLASS											
TOTAL	470.06										



GRADE 270 STRANDS						
	0.6"Ø L.R.					
AREA (SQUARE INCHES)	0.217					
ULTIMATE STRENGTH (LBS.PER STRAND)	58,600					
APPLIED PRESTRESS (LBS.PER STRAND)	43,950					

CORE	D	SLABS REQUIRED							
		NUMBER	LENGTH	TOTAL LENGTH					
65' UNIT	•								
EXTERIOR (C.S.	2	65′-0″	130'-0"					
INTERIOR (C.S.	7	65'-0"	455'-0"					
TOTAL		9		585'-0"					

DEAD LOAD DEFLECTION AN	ND CAMBER
	3'-0" × 2'-0"
65' CORED SLAB UNIT	O.6″∅ L.R. STRAND
CAMBER (SLAB ALONE IN PLACE)	3 ³ ⁄8″ ∮
DEFLECTION DUE TO SUPERIMPOSED DEAD LOAD**	1/2″ ♦
FINAL CAMBER	27⁄8″ ∳

21/2"

CHAMFER

CHAMFER

21/2"

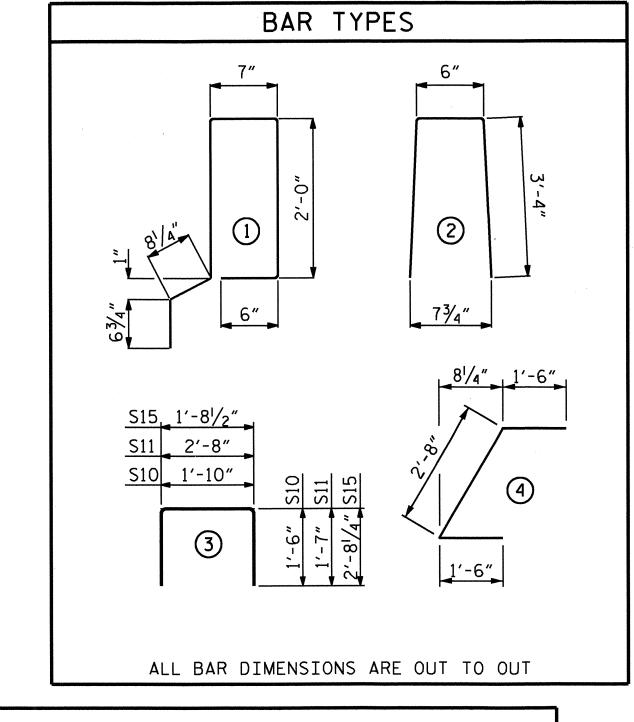
SECTION S-S

AT DAM IN OPEN JOINT (THIS IS TO BE USED ONLY WHEN SLIP FORM IS USED)

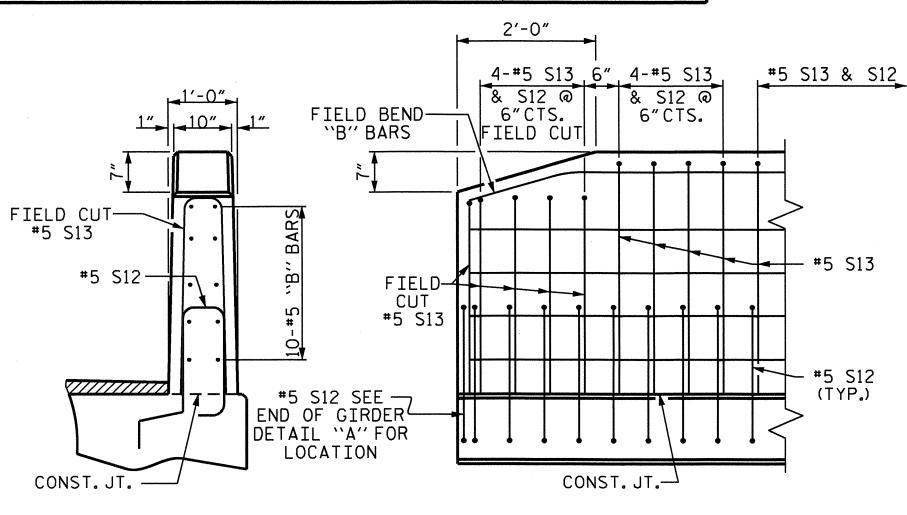
CONST. JT.

ELEVATION AT EXPANSION JOINTS

** INCLUDES FUTURE WEARING SURFACE



BILL OF MATERIAL FOR ONE 65' CORED SLAB UNIT								
EXTERIOR UNIT INTERIOR UNIT								
BAR	NUMBER	SIZE	TYPE	LENGTH	WEIGHT	LENGTH	WEIGHT	
B21	6	#4	STR	22'-10"	92	22'-10"	92	
S10	8	#5	3	4'-10"	40	4'-10"	40	
S11	138	#4	3	5′-10″	538	5′-10″	538	
* S12	74	#5	1	6′-4″	489			
S14	4	#4	4	5′-8″	15	5′-8″	15	
S15	4	#5	3	7′-1″	30	7′-1″	30	
,								
REINFORCING STEEL LBS. 715 715							715	
* EPOXY COATED								
REINFORCING STEEL LBS. 489								
6000 P.S.I. CONCRETE CU. YDS. 11.2 11.2								
0.6" Ø L.R. STRANDS No. 24 24								



END VIEW

SIDE VIEW

END OF RAIL DETAILS

GUTTERLINE ASPI	HALT THICKNESS & RAI	L HEIGHT
	ASPHALT OVERLAY THICKNESS @ MID-SPAN	RAIL HEIGHT @ MID-SPAN
65' UNITS	23/8"	3′-85⁄8″

NOTES

ALL PRESTRESSING STRANDS SHALL BE 7-WIRE LOW RELAXATION GRADE 270 STRANDS AND SHALL CONFORM TO AASHTO M203 EXCEPT FOR SAMPLING REQUIREMENTS WHICH SHALL BE IN ACCORDANCE WITH THE STANDARD SPECIFICATIONS.

ALL REINFORCING STEEL CAST WITH THE CORED SLAB SECTIONS SHALL BE GRADE 60 AND SHALL BE INCLUDED IN THE UNIT PRICE BID FOR PRESTRESSED CONCRETE CORED SLABS.

RECESSES FOR TRANSVERSE STRANDS SHALL BE GROUTED AFTER THE TENSIONING OF THE STRANDS.

THE 21/2" Ø DOWEL HOLES AT FIXED ENDS OF SLAB SECTIONS SHALL BE FILLED WITH NON-SHRINK GROUT.

THE BACKER RODS SHALL CONFORM TO THE REQUIREMENTS OF TYPE M BOND BREAKER. SEE SECTION 1028 OF THE STANDARD SPECIFICATIONS.

WHEN CORED SLABS ARE CAST, AN INTERNAL HOLD-DOWN SYSTEM SHALL BE EMPLOYED TO PREVENT VOIDS FROM RISING OR MOVING SIDEWAYS. AT LEAST SIX WEEKS PRIOR TO CASTING CORED SLABS, THE CONTRACTOR SHALL SUBMIT TO THE ENGINEER FOR REVIEW AND COMMENT, DETAILED DRAWINGS OF THE PROPOSED HOLD-DOWN SYSTEM. IN ADDITION TO STRUCTURAL DETAILS, LOCATION AND SPACING OF THE HOLD-DOWNS SHALL BE INDICATED.

THE TRANSFER OF LOAD FROM THE ANCHORAGES TO THE CORED SLAB UNIT SHALL BE DONE WHEN THE CONCRETE HAS REACHED A COMPRESSIVE STRENGTH OF NOT LESS THAN THE REQUIRED STRENGTH SHOWN IN THE "CONCRETE RELEASE STRENGTH" TABLE.

ALL REINFORCING STEEL IN VERTICAL CONCRETE BARRIER RAILS SHALL BE EPOXY COATED.

PRESTRESSING STRANDS SHALL BE CUT FLUSH WITH THE CORED SLAB UNIT ENDS.

APPLY EPOXY PROTECTIVE COATING TO CORED SLAB UNIT ENDS.

GROOVED CONTRACTION JOINTS, $\frac{1}{2}$ " IN DEPTH, SHALL BE TOOLED IN ALL EXPOSED FACES OF THE BARRIER RAIL AND IN ACCORDANCE WITH ARTICLE 825-10(B) OF THE STANDARD SPECIFICATIONS. A CONTRACTION JOINT SHALL BE LOCATED AT EACH THIRD POINT BETWEEN BARRIER RAIL EXPANSION JOINTS. ONLY ONE CONTRACTION JOINT IS REQUIRED AT MIDPOINT OF BARRIER RAIL SEGMENTS LESS THAN 20 FEET IN LENGTH AND NO CONTRACTION JOINTS ARE REQUIRED FOR THOSE SEGMENTS LESS THAN 10 FEET IN LENGTH.

TRANSVERSE POST TENSIONING OF THE CORED SLAB UNITS SHALL BE DONE IN ACCORDANCE WITH THE STANDARD SPECIFICATIONS.

MAINTAIN A SYMMETRIC TENSION FORCE BETWEEN EACH PAIR OF TRANSVERSE POST TENSIONING STRANDS IN THE DIAPHRAGM.

THE #4 S11 STIRRUPS MAY BE SHIFTED AS NECESSARY TO MAINTAIN 1" CLEAR TO THE GROUTED RECESS.

FOR GROUT FOR STRUCTURES, SEE SPECIAL PROVISIONS.

CONCRETE	RELEASE	STRENGTH
UNIT		PSI
65' UNITS		4800

<u>B-4987</u> PROJECT NO. ___ HENDERSON COUNTY STATION: 12+51.50 -L-

SHEET 3 OF 3

STATE OF NORTH CAROLINA DEPARTMENT OF TRANSPORTATION STANDARD 3'-0" X PRESTREŠSËD CONCRETE CORED SLAB UNIT

SHEET NO. REVISIONS S-7 DATE: DATE: BY: BY: TOTAL SHEETS

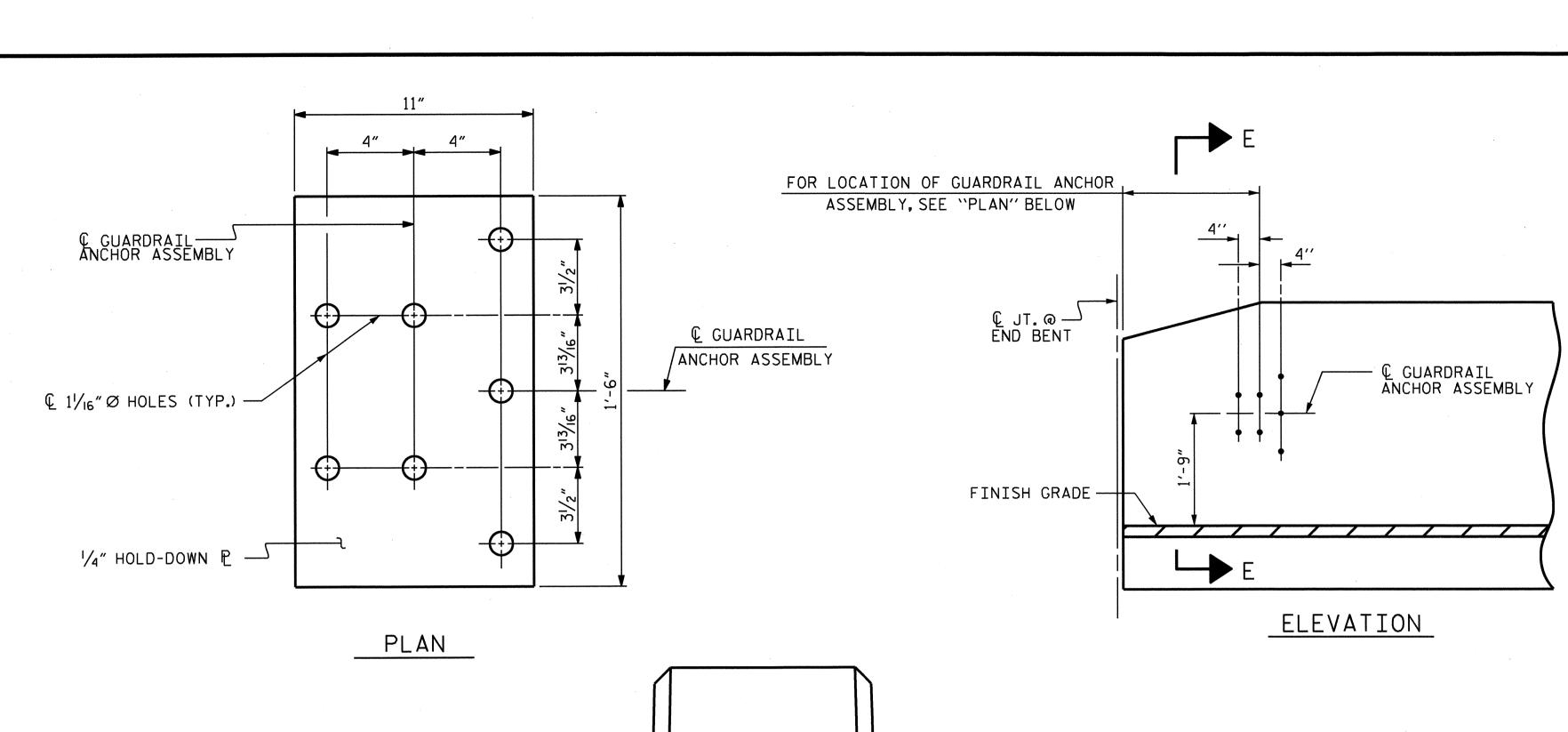
SEAL 15779

CHECKED BY : S. PEARCE DATE: 8/12 DRAWN BY : MAA 6/10 REV. 12/11 CHECKED BY : MKT 7/10

DATE: 8/12

ASSEMBLED BY: V. NGUYEN

STD. NO. 24PCS3_27_75&105S



NOTES

THE GUARDRAIL ANCHOR ASSEMBLY SHALL CONSIST OF A $\frac{1}{4}$ " HOLD DOWN PLATE AND 7 - $\frac{1}{8}$ " Ø BOLTS WITH NUTS AND WASHERS.

THE HOLD-DOWN PLATE SHALL CONFORM TO AASHTO M270 GRADE 36. AFTER FABRICATION, THE HOLD-DOWN PLATE SHALL BE HOT-DIP GALVANIZED IN ACCORDANCE WITH AASHTO M111.

BOLTS SHALL CONFORM TO THE REQUIREMENTS OF ASTM A307 AND NUTS SHALL CONFORM TO THE REQUIREMENTS OF AASHTO M291. BOLTS, NUTS AND WASHERS SHALL BE GALVANIZED. (AT THE CONTRACTOR'S OPTION, STAINLESS STEEL BOLTS, NUTS AND WASHERS MAY BE USED AS AN ALTERNATE FOR THE 1/8" Ø GALVANIZED BOLTS, NUTS AND WASHERS. THEY SHALL CONFORM TO OR EXCEED THE MECHANICAL REQUIREMENTS OF ASTM A307. THE USE OF THIS ALTERNATE SHALL BE APPROVED BY THE ENGINEER.)

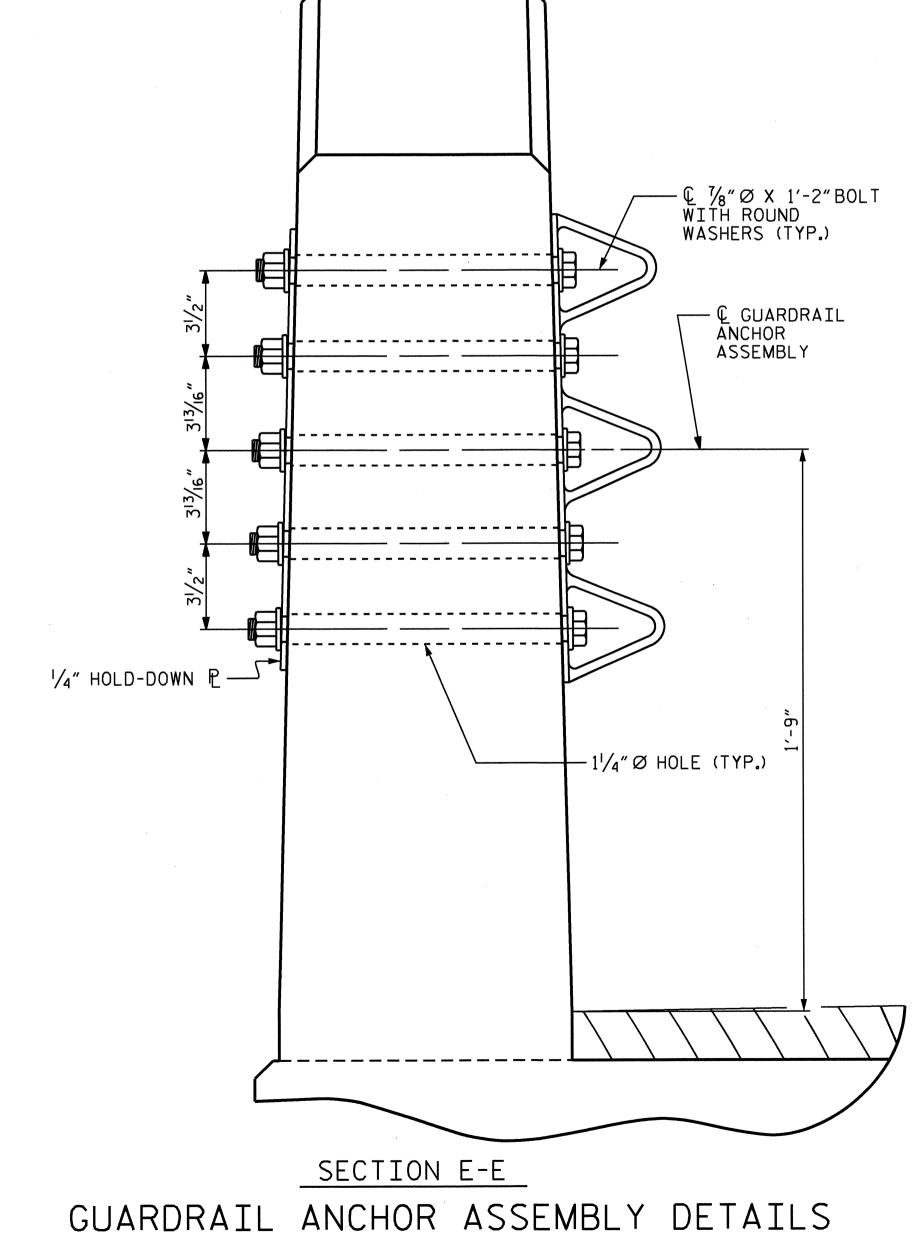
THE GUARDRAIL ANCHOR ASSEMBLY IS REQUIRED AT ALL POINTS WHERE APPROACH GUARDRAIL IS TO BE ATTACHED TO THE END OF BARRIER RAIL. FOR POINTS OF ATTACHMENT. SEE SKETCH.

AFTER INSTALLATION, THE EXPOSED THREAD OF THE BOLT SHALL BE BURRED WITH A SHARP POINTED TOOL.

THE COST OF THE GUARDRAIL ANCHOR ASSEMBLY SHALL BE INCLUDED IN THE UNIT CONTRACT PRICE BID FOR VERTICAL CONCRETE BARRIER RAIL.

THE VERTICAL REINFORCING BARS MAY BE SHIFTED SLIGHTLY IN THE VERTICAL CONCRETE BARRIER RAIL TO CLEAR ASSEMBLY BOLTS.

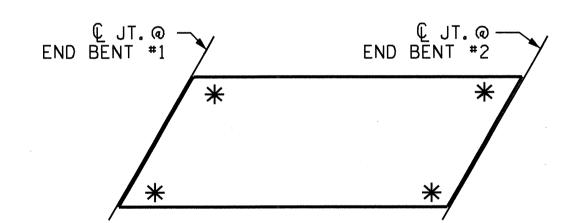
THE 1 1/4" Ø HOLES SHALL BE FORMED OR DRILLED WITH A CORE BIT. IMPACT TOOLS WILL NOT BE PERMITTED. ANY CONCRETE DAMAGED BY THIS WORK SHALL BE REPAIRED TO THE SATISFACTION OF THE ENGINEER.



€ JT.@—— END BENT 1'-10" € GUARDRAIL ANCHOR ASSEMBLY <u>PLAN</u>

> LOCATION OF ANCHORS FOR GUARDRAIL

END BENT #1 SHOWN, END BENT #2 SIMILAR.



SKETCH SHOWING POINTS OF ATTACHMENT

* DENOTES GUARDRAIL ANCHOR ASSEMBLY

PROJECT NO. B-4987

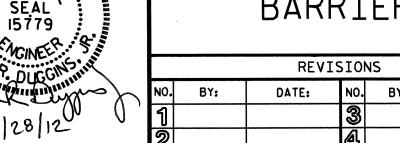
HENDERSON COUNTY

STATION: 12+51.50-L-

STATE OF NORTH CAROLINA DEPARTMENT OF TRANSPORTATION
RALEIGH

STANDARD

GUARDRAIL ANCHORAGE FOR VERTICAL CONCRETE BARRIER RAIL



			•				
			REV	ISION	S		SHEET NO.
\sim	NO.	BY:	DATE:	NO.	BY:	DATE:	S-8
) '	1			3			TOTAL SHEETS
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ASSEMBLED BY: S. PEARCE CHECKED BY: J. DUGGINS

DRAWN BY: MAA 5/10 CHECKED BY: GM 5/10

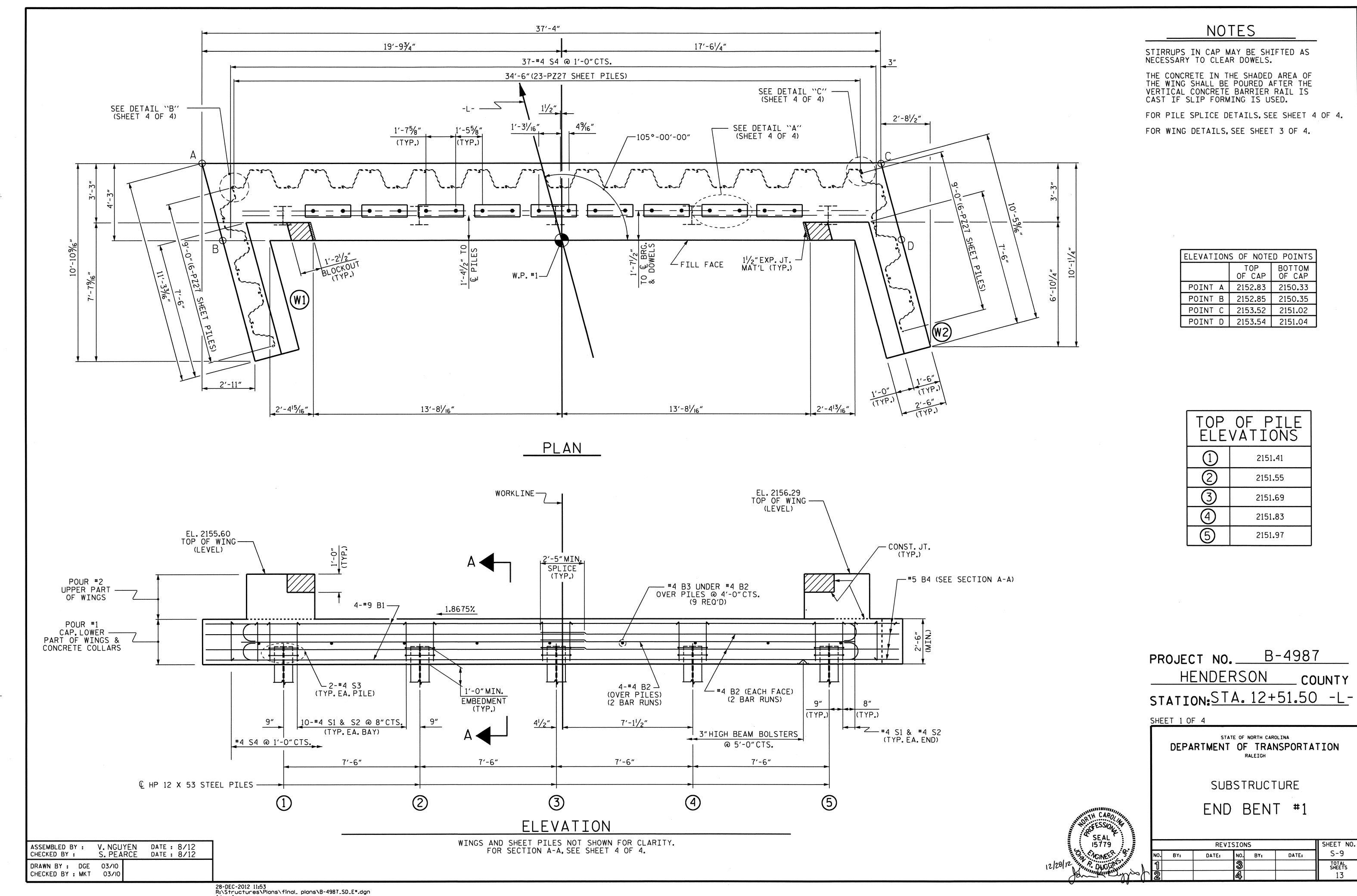
DATE: 8/12 DATE: 8/12

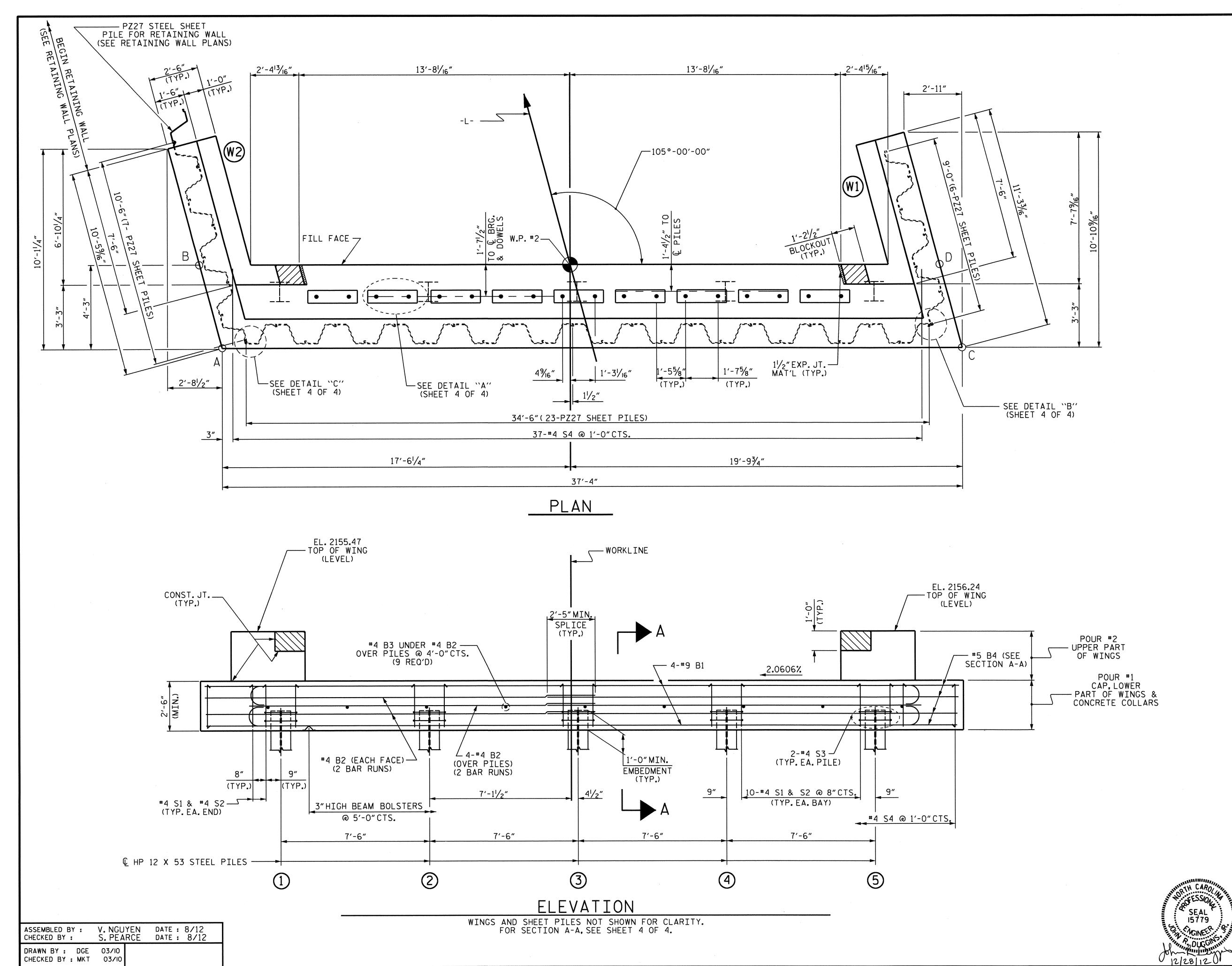
MAA/GM MAA/GM

ADDED 5/6/IO REV. IO/I/II

REV. 12/5/II

STD. NO. GRA3





NOTES

STIRRUPS IN CAP MAY BE SHIFTED AS NECESSARY TO CLEAR DOWELS.

THE CONCRETE IN THE SHADED AREA OF THE WING SHALL BE POURED AFTER THE VERTICAL CONCRETE BARRIER RAIL IS CAST IF SLIP FORMING IS USED.

FOR PILE SPLICE DETAILS, SEE SHEET 4 OF 4.

FOR WING DETAILS, SEE SHEET 3 OF 4.

ELEVAT]	ON	S OF NOTE	D POINTS
		TOP OF CAP	BOTTOM OF CAP
POINT	Α	2152.74	2150.24
POINT	В	2152.72	2150.22
POINT	С	2153.51	2151.01
POINT	D	2153.49	2150.99

TOP OF PILE ELEVATIONS				
1	2151.34			
2	2151.49			
3	2151.64			
4	2151.79			
5	2151.94			

PROJECT NO. B-4987

HENDERSON COUNTY

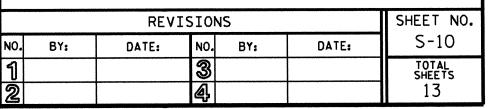
STATION: STA. 12+51.50 -L-

SHEET 2 OF 4

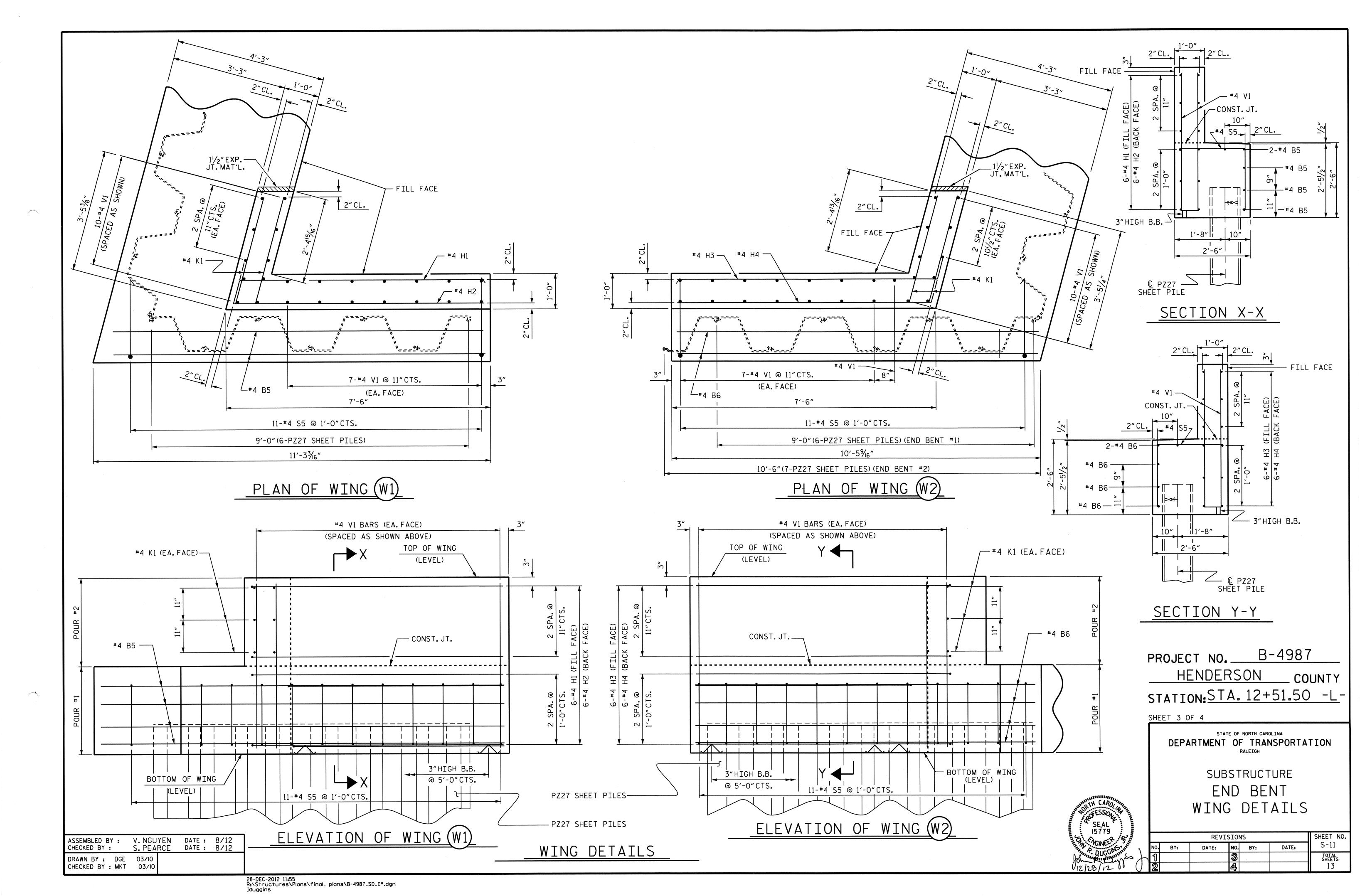
DEPARTMENT OF TRANSPORTATION
RALEIGH

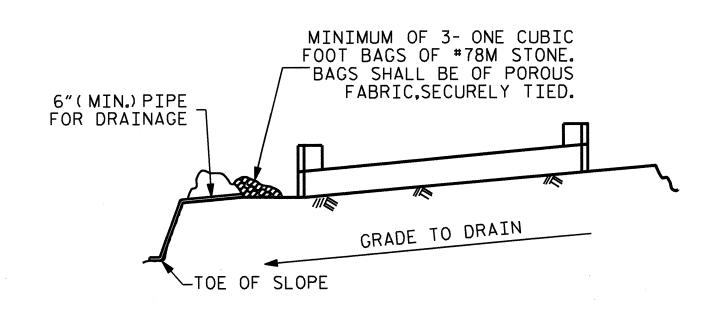
SUBSTRUCTURE

END BENT #2



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jduggins



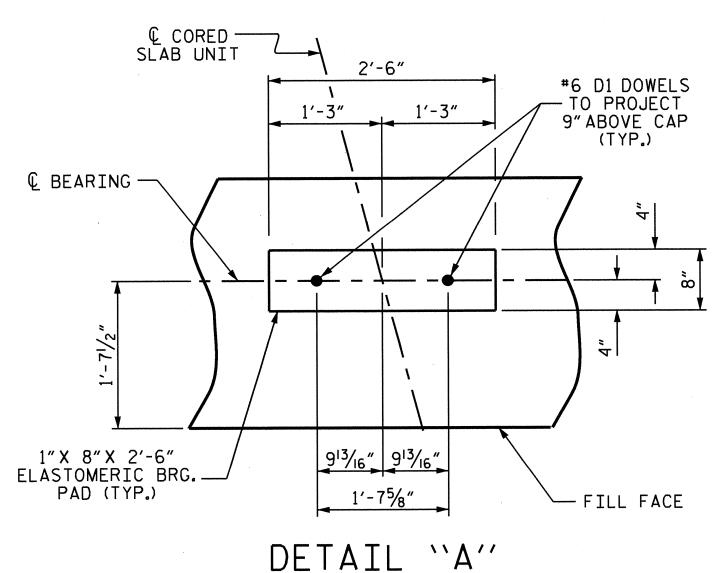


BAGGED STONE AND PIPE SHALL BE PLACED IMMEDIATELY AFTER COMPLETION OF END BENT EXCAVATION. PIPE MAY BE EITHER CONCRETE, CORRUGATED STEEL, CORRUGATED ALUMINUM ALLOY, OR CORRUGATED PLASTIC. PERFORATED PIPE WILL NOT BE ALLOWED.

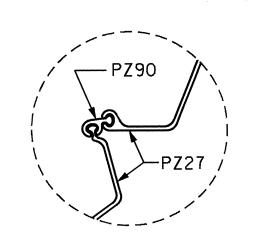
BAGGED STONE SHALL REMAIN IN PLACE UNTIL THE ENGINEER DIRECTS THAT IT BE REMOVED. THE CONTRACTOR SHALL REMOVE AND DISPOSE OF SILT ACCUMULATIONS AT BAGGED STONE WHEN SO DIRECTED BY THE ENGINEER. BAGS SHALL BE REMOVED AND REPLACED WHENEVER THE ENGINEER DETER-MINES THAT THEY HAVE DETERIORATED AND LOST THEIR EFFECTIVENESS.

NO SEPARATE PAYMENT WILL BE MADE FOR THIS WORK AND THE ENTIRE COST OF THIS WORK SHALL BE INCLUDED IN THE UNIT CONTRACT PRICE BID FOR THE SEVERAL PAY ITEMS.

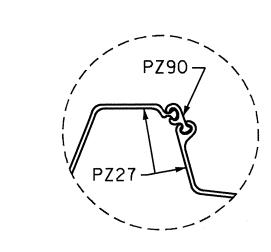
TEMPORARY DRAINAGE AT END BENT



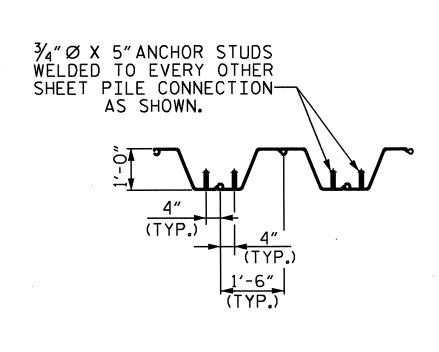
(END BENT No. 1 SHOWN, END BENT No. 2 SIMILAR BY ROTATION)



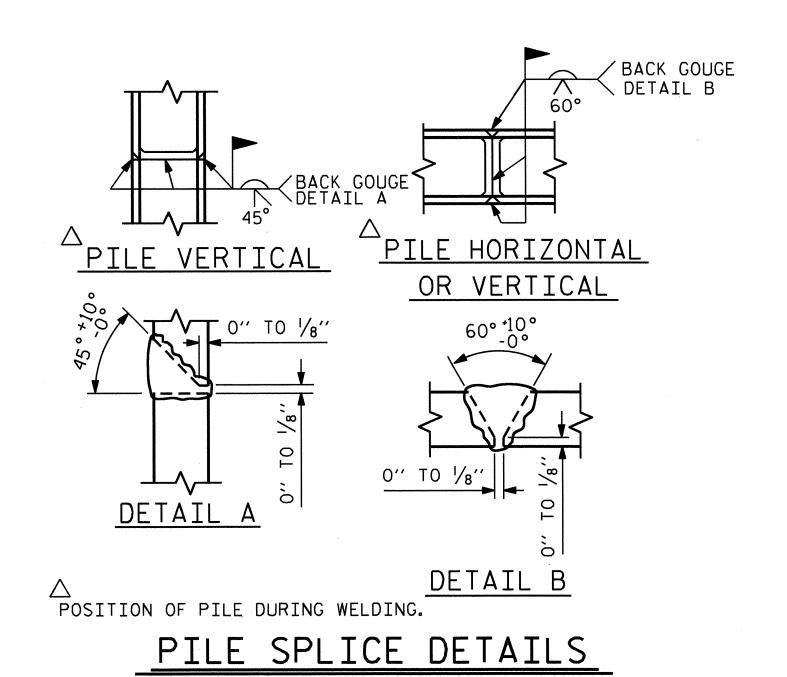
DETAIL "B"

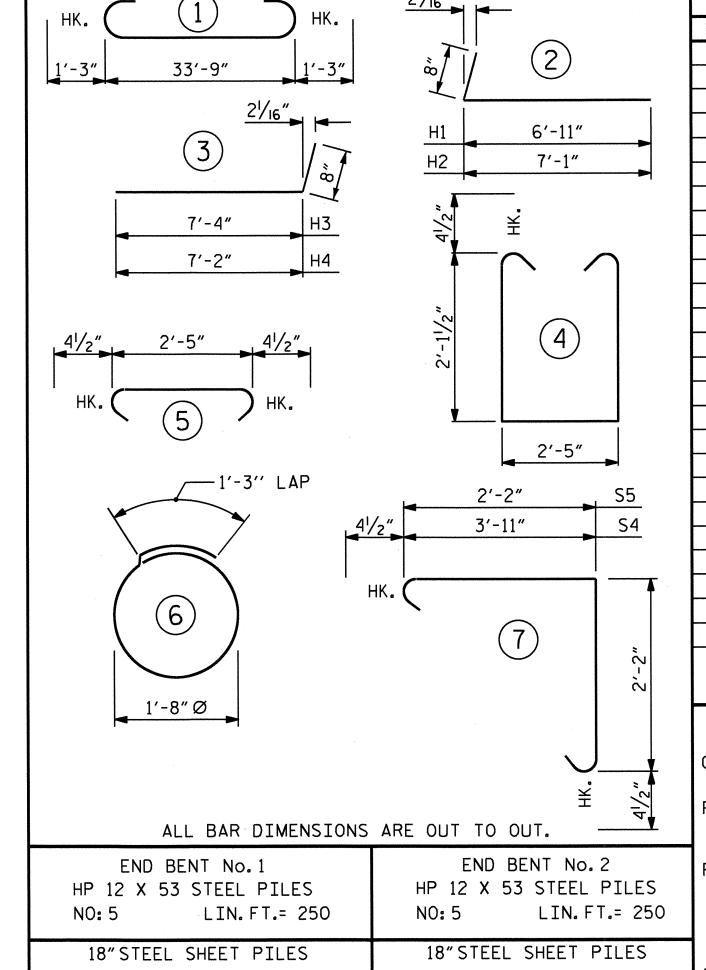


DETAIL

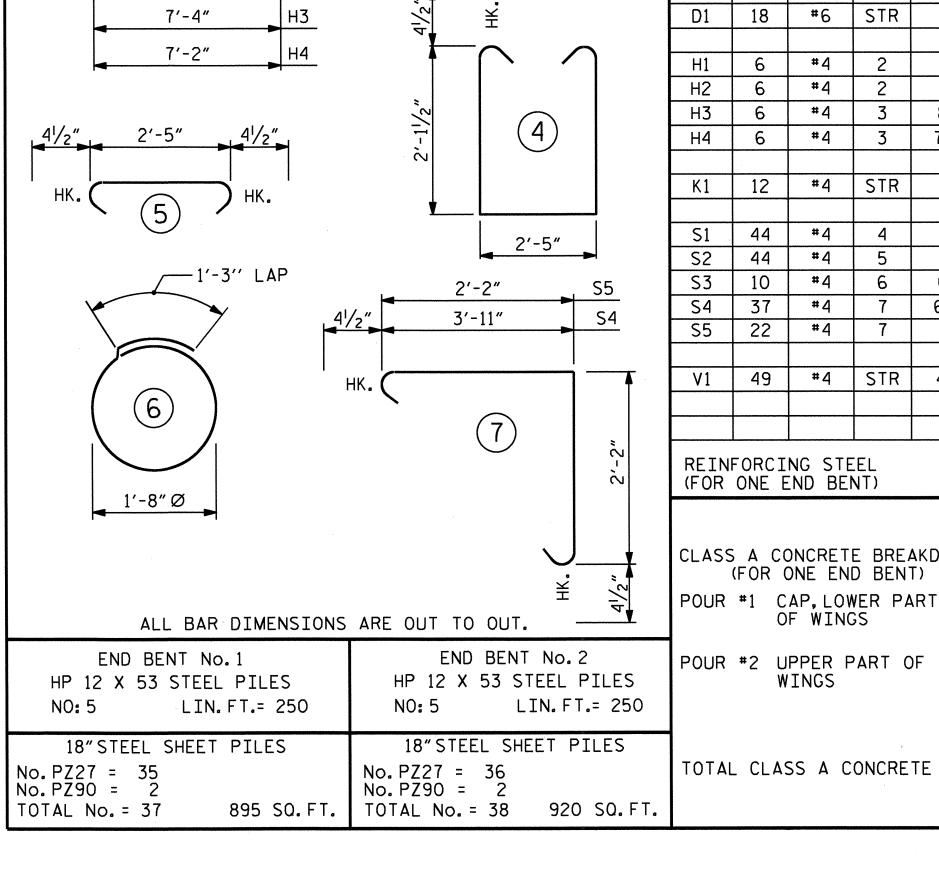


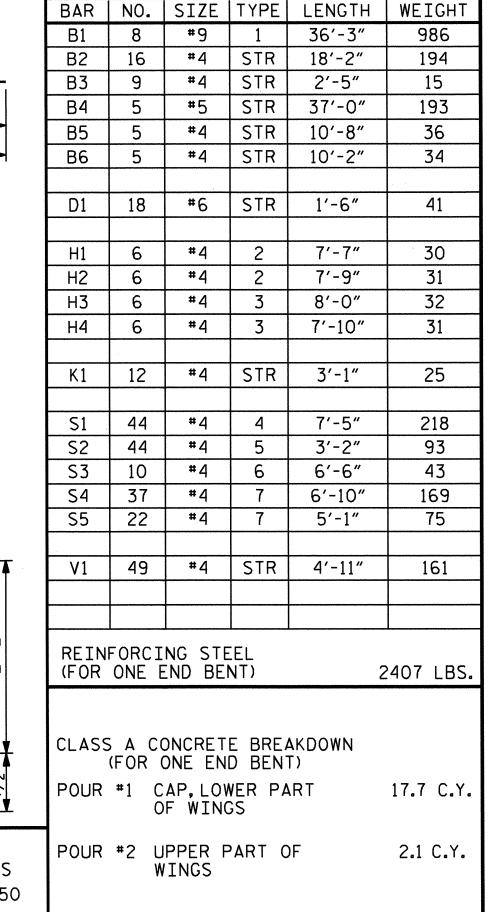
ANCHOR STUD DETAIL





BAR TYPES

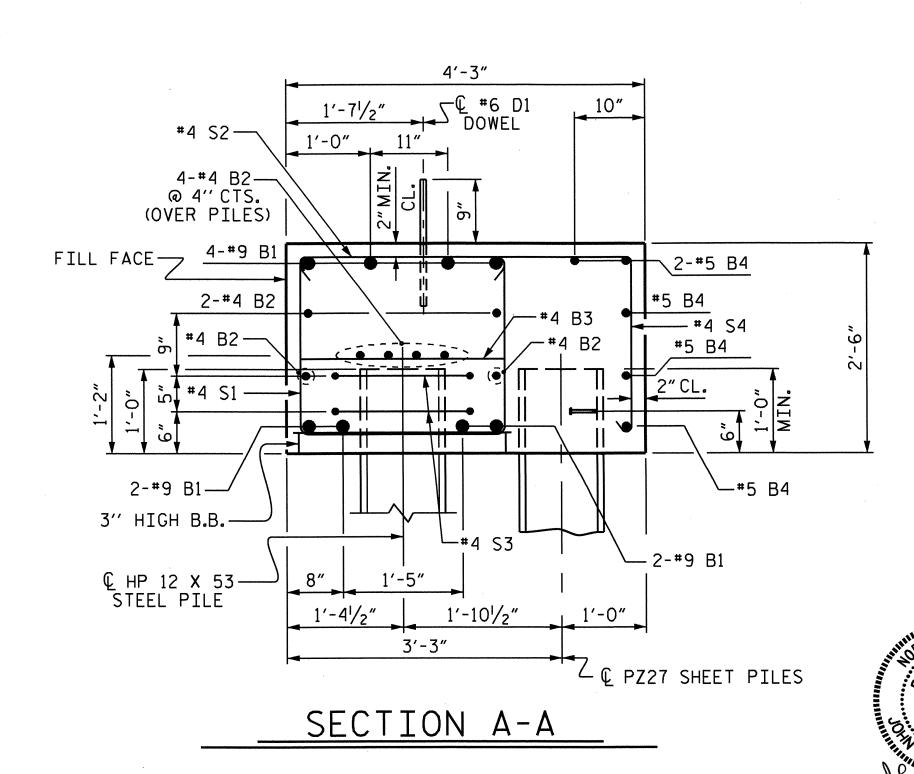




19.8 C.Y.

BILL OF MATERIAL

FOR ONE END BENT



PROJECT NO. B-4987 HENDERSON COUNTY STATION: 12+51.50 -L-

SHEET 4 OF 4

SEAL 15779

STATE OF NORTH CAROLINA DEPARTMENT OF TRANSPORTATION

SUBSTRUCTURE

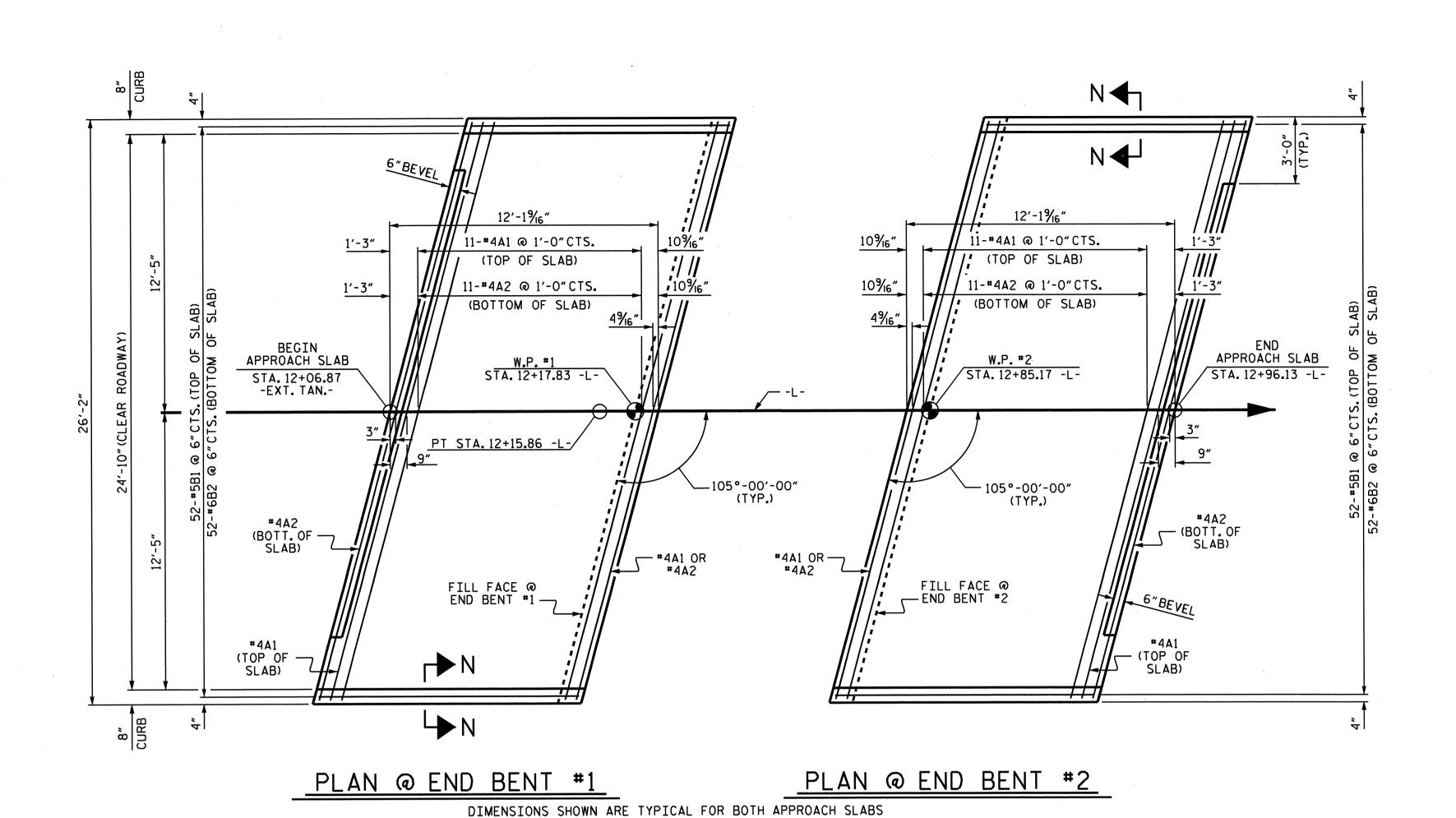
END BENT #1 & #2 DETAILS

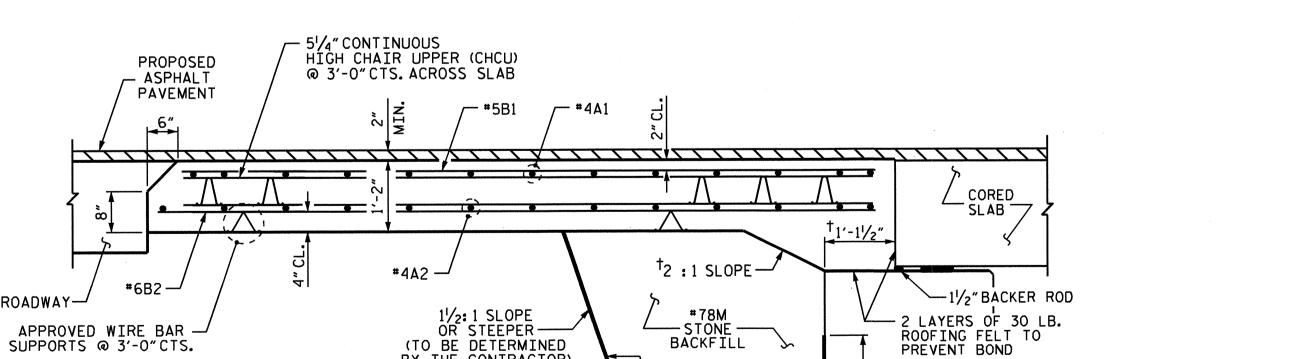
SHEET NO REVISIONS S-12 DATE: NO. BY: BY: DATE: TOTAL SHEETS 13

DATE: 8/12 DATE: 8/12 ASSEMBLED BY : CHECKED BY : V. NGUYEN S. PEARCE DRAWN BY: DGE 03/10

CHECKED BY : MKT 03/10

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 $^{\mathsf{L}}\mathsf{GEOTEXTILE}_{\frown}$

3'-0"

SPLICE LENGTHS

#5 2'-6" 2'-2"

#6 3'-10" 2'-7"

EPOXY COATED UNCOATED

2'-0" 1'-9"

NOTES

FOR BRIDGE APPROACH FILL INCLUDING GEOTEXTILE, 4"Ø DRAINAGE PIPE, AND #78M STONE BACKFILL, SEE ROADWAY PLANS.

GEOTEXTILE SHALL BE TYPE 1 IN ACCORDANCE WITH THE STANDARD SPECIFICATIONS SECTION 1056.

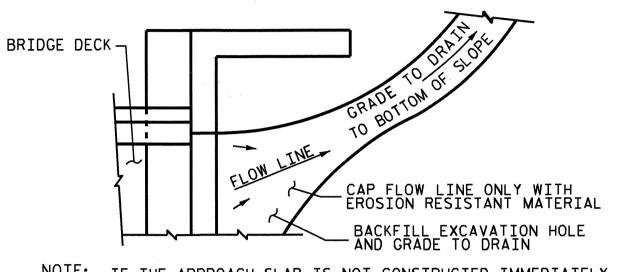
*78M STONE BACKFILL (CLASS V SELECT MATERIAL) SHALL BE IN ACCORDANCE WITH STANDARD SPECIFICATIONS SECTION 1016.

*78M STONE BACKFILL IS TO BE CONTINUOUS ALONG FILL FACE OF BACKWALL FROM OUTSIDE EDGE TO OUTSIDE EDGE OF APPROACH SLAB.

FOR THE 4" Ø DRAINAGE PIPE OUTLET(S), SEE ROADWAY STANDARD DRAWINGS.

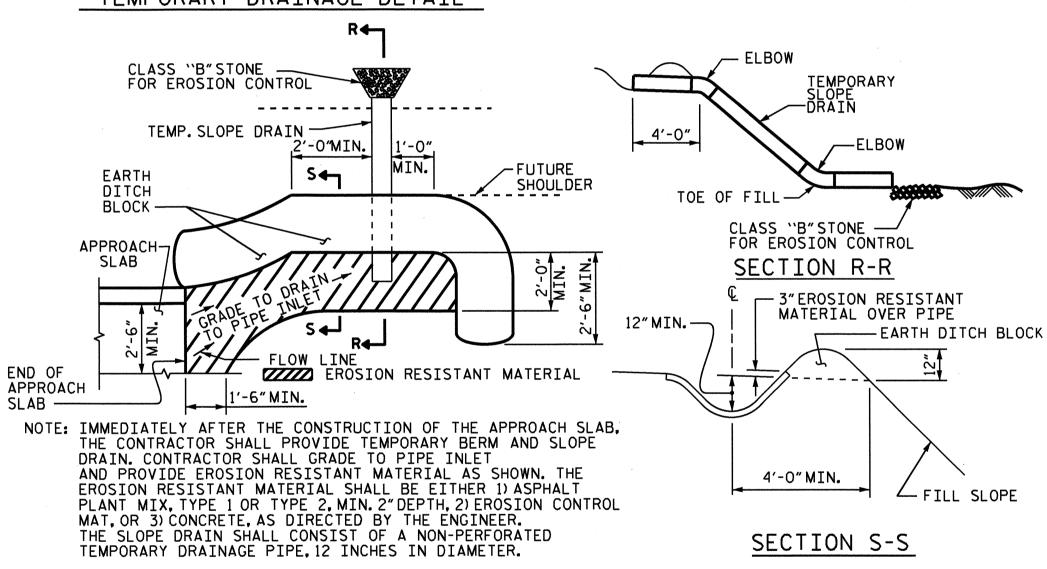
AREA BETWEEN THE WINGWALL AND APPROACH SLAB SHALL BE GRADED TO DRAIN THE WATER AWAY FROM THE FILL FACE OF THE BRIDGE AND SHALL BE PAVED. SEE ROADWAY PLANS.

APPROACH SLAB GROOVING IS NOT REQUIRED.



IF THE APPROACH SLAB IS NOT CONSTRUCTED IMMEDIATELY AFTER THE BACKFILLING OF THE END BENT EXCAVATION, GRADE TO DRAIN TO THE BOTTOM OF THE SLOPE AND PROVIDE EROSION RESISTANT MATERIAL, SUCH AS FIBERGLASS ROVING OR AS DIRECTED BY THE ENGINEER TO PREVENT SOIL EROSION AND TO PROTECT THE AREA ADJACENT TO THE STRUCTURE. THE CONTRACTOR WILL BE REQUIRED TO REMOVE THESE MATERIALS PRIOR TO CONSTRUCTION OF THE APPROACH SLAB.

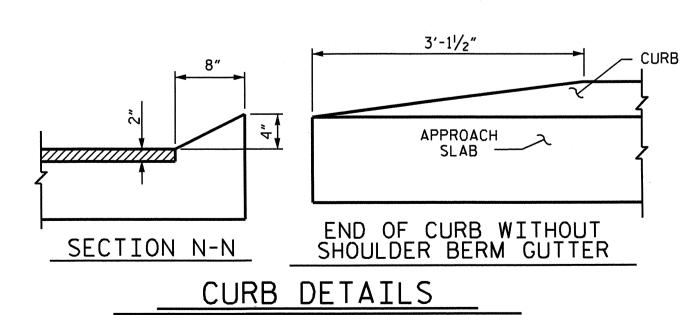
TEMPORARY DRAINAGE DETAIL



PLAN VIEW

TEMPORARY BERM AND SLOPE DRAIN DETAILS

(TO BE USED WHEN SHOULDER BERM GUTTER IS REQUIRED)



B-4987 PROJECT NO. HENDERSON COUNTY 12+51.50-L-STATION:

BILL OF MATERIAL

APPROACH SLAB AT EB #1

BAR | NO. | SIZE | TYPE | LENGTH | WEIGHT

APPROACH SLAB AT EB #2

BAR | NO. | SIZE | TYPE | LENGTH | WEIGHT

* A1 | 13 | #4 | STR | 26'-8"

*B1 | 52 | #5 | STR | 11'-1"

B2 | 52 | #6 | STR | 11'-7"

A2 | 13 | #4 | STR | 26'-8"

11'-7"

LBS.

LBS.

C. Y.

LBS.

LBS.

C.Y.

232

905

1137

232

601

905

1137

833

16.3

* A1 | 13 | #4 | STR | 26'-8"

*B1 | 52 | *5 | STR B2 | 52 | #6 | STR

REINFORCING STEEL

CLASS AA CONCRETE

REINFORCING STEEL

CLASS AA CONCRETE

REINFORCING STEEL

* EPOXY COATED

REINFORCING STEEL

* EPOXY COATED

A2 | 13 | #4 | STR | 26'-8"

STATE OF NORTH CAROLINA DEPARTMENT OF TRANSPORTATION RALEIGH STANDARD BRIDGE APPROACH SLAB

FOR PRESTRESSED CONCRETE CORED SLAB UNIT (SUB-REGIONAL TIER)

SHEET NO

S-13

TOTAL SHEETS

105° SKEW REVISIONS DATE: DATE: BY: BY:

SEAL 15779

SECTION THRU SLAB

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(TO BE DETERMINED BY THE CONTRACTOR)

4" Ø PERFORATED SCHEDULE 40 PVC PIPE

ROADWAY-

V. NGUYEN S. PEARCE

DRAWN BY : SHS/MAA 5-09 REV. 12-11

ASSEMBLED BY :

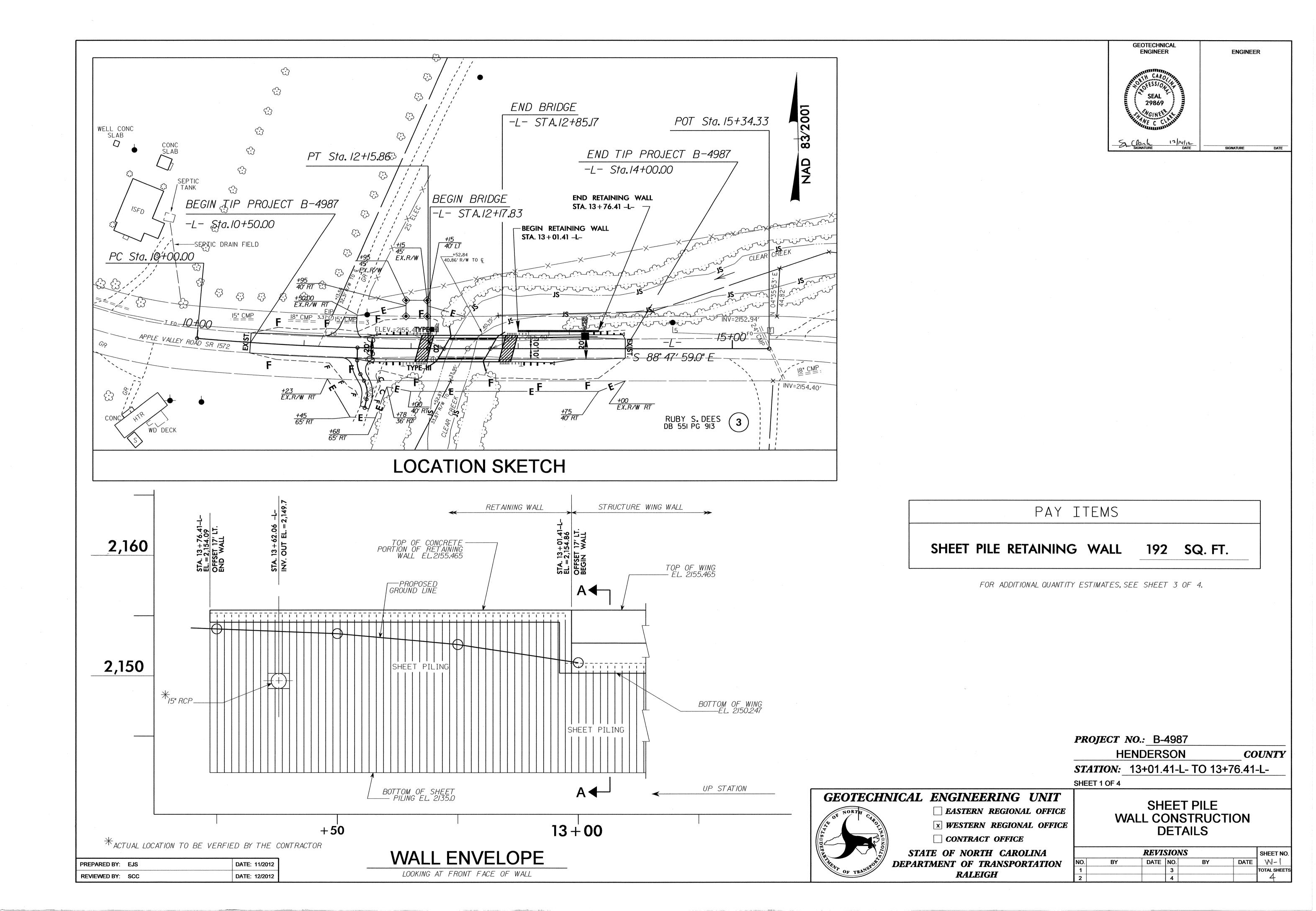
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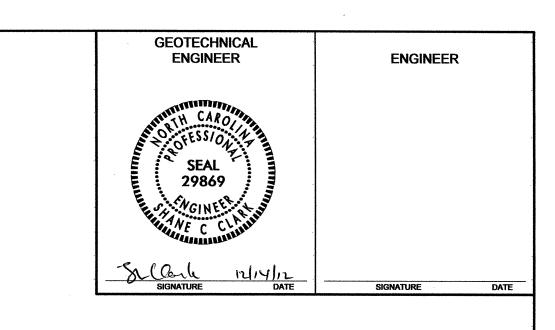
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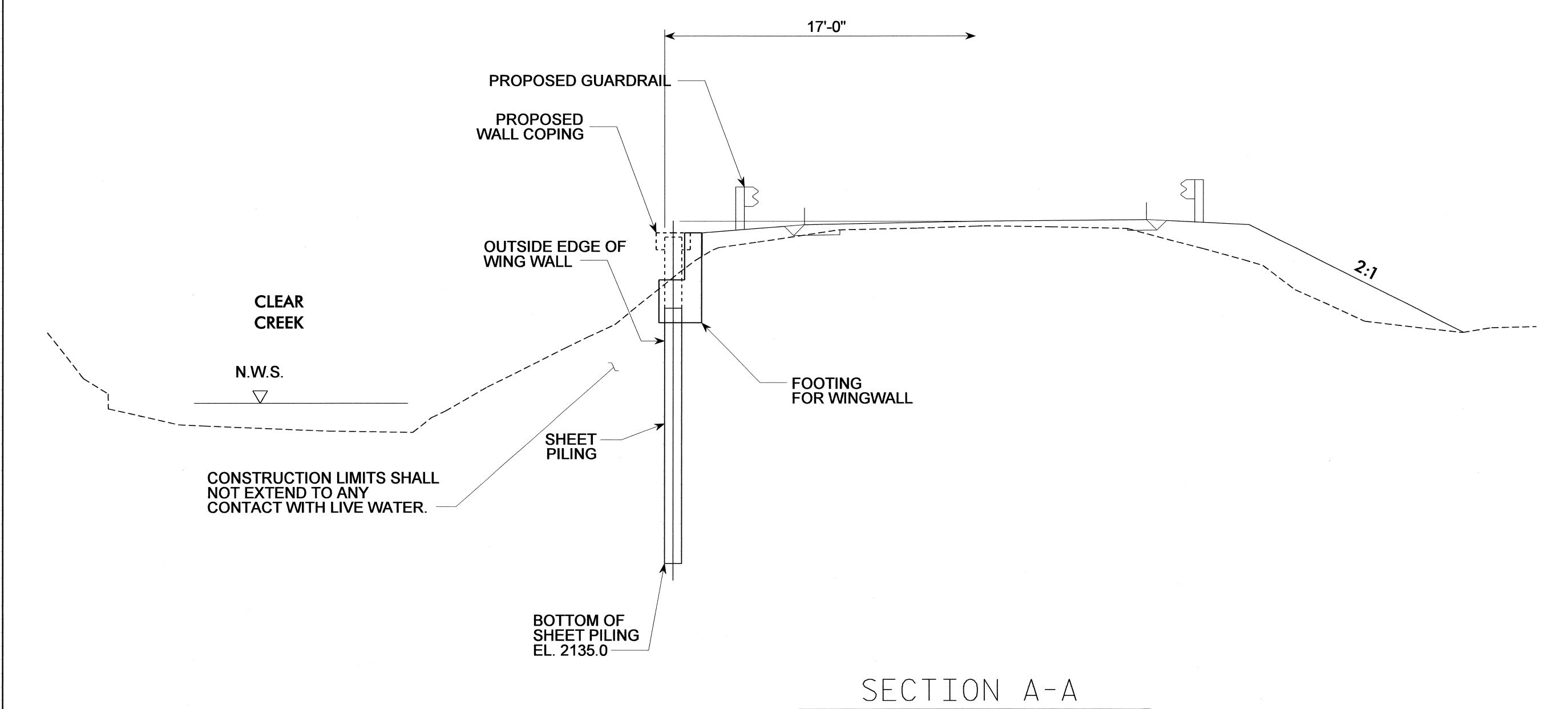
[†] NORMAL TO END BENT

DATE: 8/12 DATE: 8/12

STD. NO. BAS_27_105S







SHEET PILING RETAINING WALL

PROJECT NO.: B-4987

HENDERSON

COUNTY

STATION: 13+01.41 -L- TO 13+76.41 -L-

SHEET 2 OF 4

GEOTECHNICAL ENGINEERING UNIT

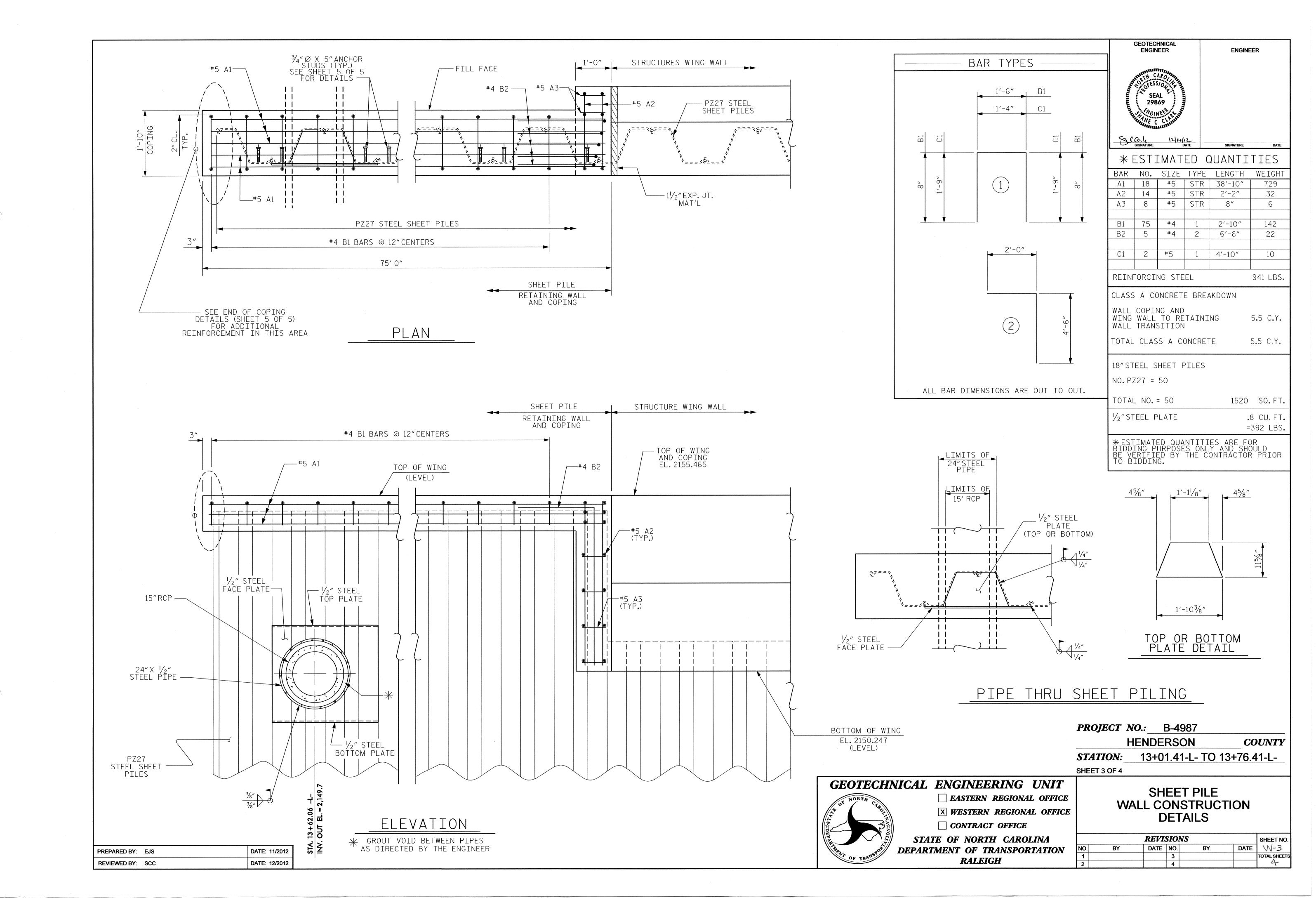
- **EASTERN REGIONAL OFFICE**
- X WESTERN REGIONAL OFFICE ☐ CONTRACT OFFICE

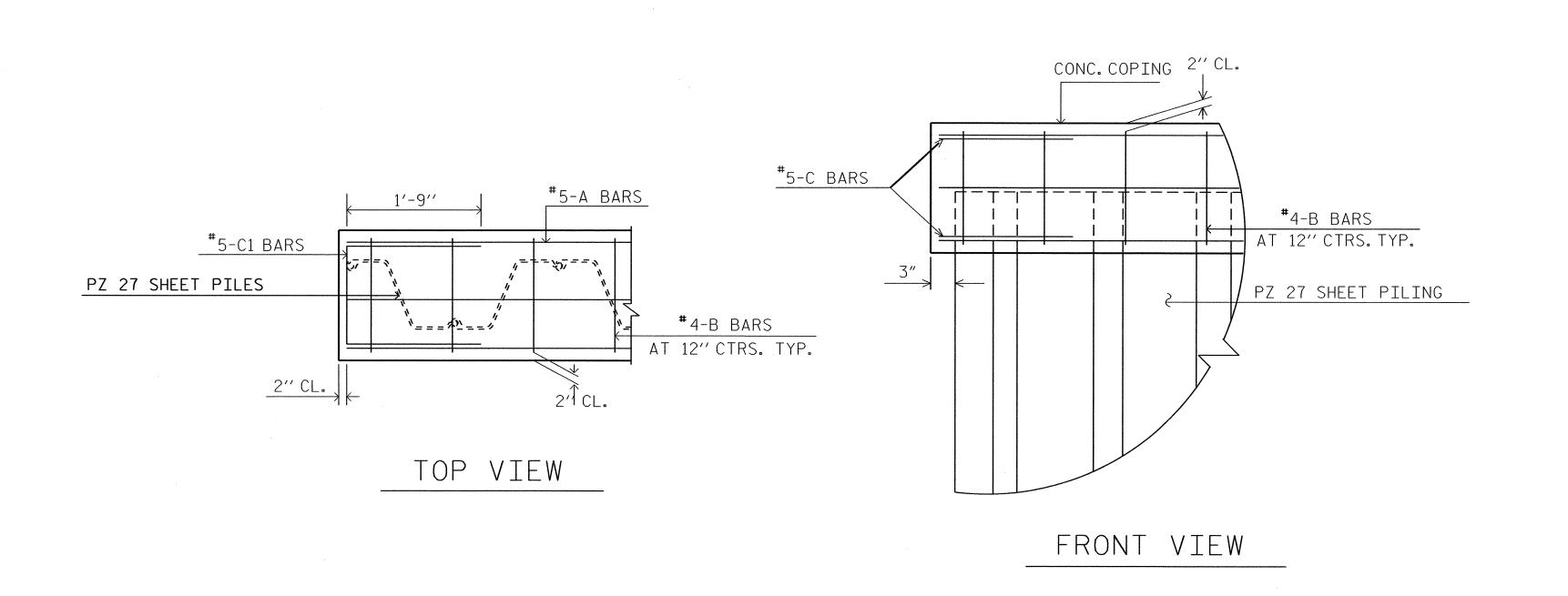
STATE OF NORTH CAROLINA DEPARTMENT OF TRANSPORTATION RALEIGH

SHEET PILE WALL CONSTRUCTION **DETAILS**

	SHEET NO.				
BY	DATE	NO.	BY	DATE	W-2
		3			TOTAL SHEETS
		4		-	4

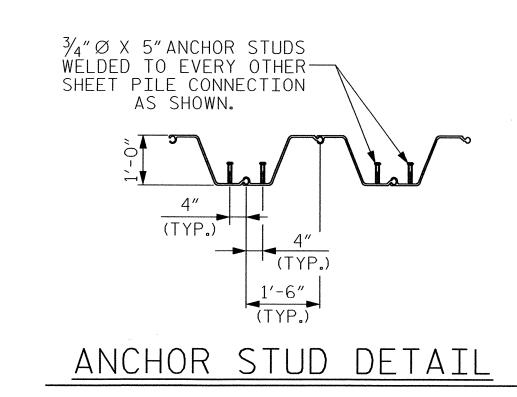
PREPARED BY: EJS DATE: 11/2012 DATE: 12/2012 REVIEWED BY: SCC





END OF COPING DETAILS

N. T. S.



GEOTECHNICAL ENGINEER **ENGINEER** Shellonk plyling DATE

NOTES

FOR SHEET PILE RETAINING WALLS, SEE SECTION 452 OF THE STANDARD SPECIFICATIONS

FIELD ADJUSTMENT OF BARS MAY BE NECESSARY FOR PLACEMENT IN COPING.

ALL REINFORCING SHALL BE GRADE 60 AS DESCRIBED IN SECTION 1070 OF THE STANDARD SPECIFICATIONS.

FOR SHEET PILE RETAINING WALLS AND CAST-IN-PLACE COPING, SEE SECTION 452 OF THE STANDARD SPECIFICATIONS.

ALL SHEET PILES TO BE PZ27

SHEET PILES ARE TO BE INSTALLED TO ELEVATIONS SHOWN IN THE TABLE. TIP ELEVATIONS BETWEEN STATIONS ARE TO BE INTERPRETED AS A STRAIGHT LINE BETWEEN STATED TIP ELEVATIONS.

SHEET PILES ARE TO BE INSTALLED PLUMB (+/- 1% OF FULL INSTALLED LENGTH) ALONG THE FRONT FACE AND LEADING EDGES.

STEEL SHEET PILES SHALL BE EMBEDDED A MINIMUM OF O'-9" INTO COPING.

A PIPE PENETRATION IS TO BE CONSTRUCTED USING $\frac{1}{2}$ " STEEL FACE PLATE, DEPENDING ON LOCATION, IS TO BE OF ADEQUATE SIZE TO SPAN ADJACENT SHEETING "FLATS" AS SHOWNN IN THE DETAILS OR AS DIRECTED BY THE ENGINEER.

THE PIPE PENTRATION IS TO BE CONSTRUCTED USING A690 STEEL COMPONENTS CONSISTING OF A 24"STEEL PIPE SLEEVE AND SHOP FABRICATED 1/2"PLATE STEEL FOR THE TOP AND BOTTOM PLATES. ALL CONECTIONS ARE TO BE WELDED AS SHOWN OR AS DIRECTED BY THE ENGINEER.

THE PIPE PENETRATION WILL BE CONSIDERED INCIDENTAL TO THE WALL. NO ADDITIONAL PAYMENT WILL BE MADE. THE SHEET PILE BID PRICE WILL INCLUDE ALL MATERIALS, FABRICATION WELDING, EXCAVATION AND BACKFILL, AS REQUIRED, AND ANY INCIDENTALS REQUIRED TO PERFORM THE WORK.

THE MINIMUM EMBEDMENT ELEVATION FOR RETAINING WALL INCLUDES EMBEDMENT FOR SCOUR.

WRAP FILL SLOPES AROUND WALL ENDS AS DIRECTED BY THE ENGINEER.

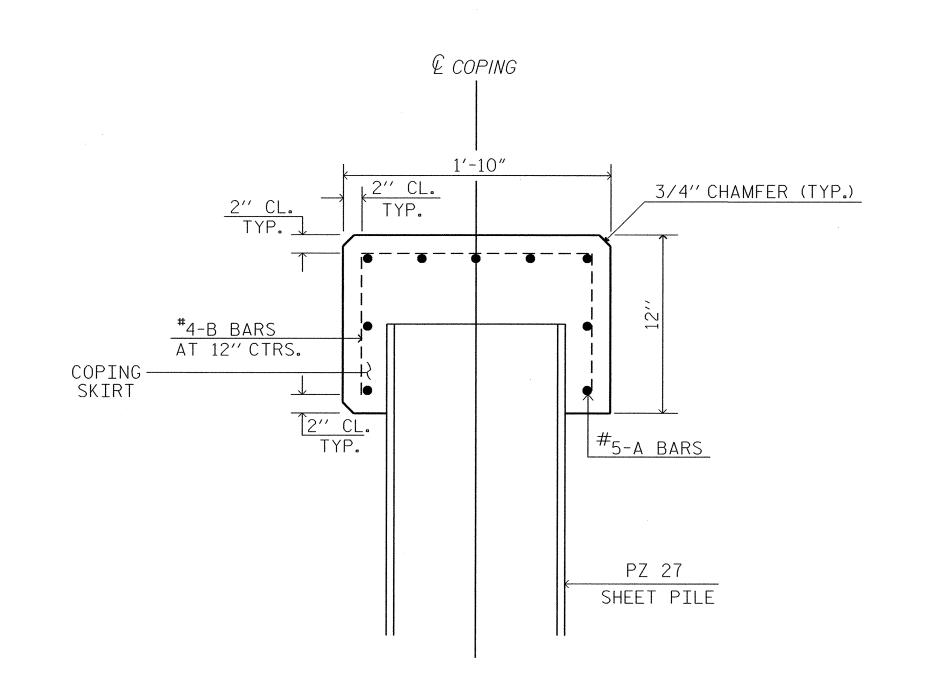
PROJECT NO.: B-4987

HENDERSON

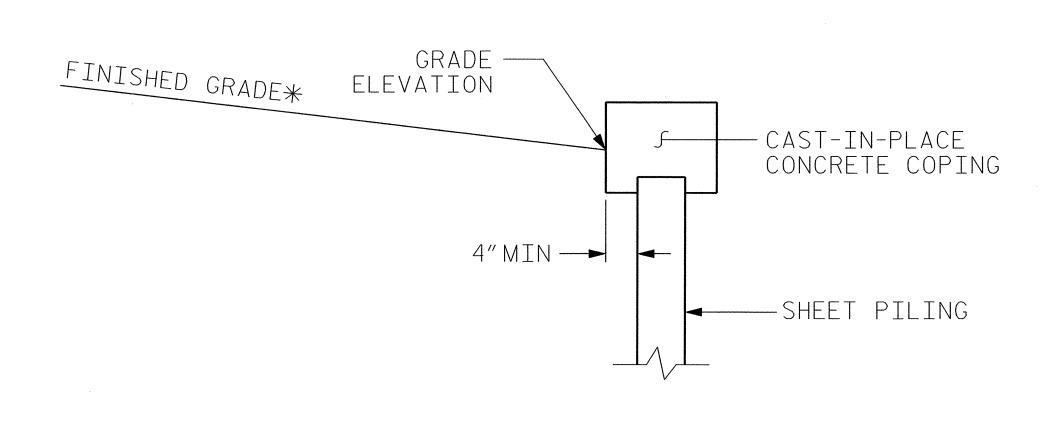
COUNTY

STATION: 13+01.41-L- TO 13+76.41-L-

SHEET 4 OF 4



FULL COPING DETAIL N. T. S.



COPING DETAILS

*SEE ROADWAY PLANS FOR FINISHED GRADE DETAILS.

GEOTECHNICAL ENGINEERING UNIT **EASTERN REGIONAL OFFICE**

X WESTERN REGIONAL OFFICE

CONTRACT OFFICE

STATE OF NORTH CAROLINA **DEPARTMENT OF TRANSPORTATION** RALEIGH

SHEET PILE WALL CONSTRUCTION **DETAILS**

REVISIONS SHEET NO. W-4 DATE TOTAL SHEETS

DATE: 11/2012 PREPARED BY: EJS REVIEWED BY: SCC DATE: 12/2012

STANDARD NOTES

DESIGN DATA:

SPECIFICATIONS	A.A.S.H.T.O. (CURRENT)
LIVE LOAD	SEE PLANS
IMPACT ALLOWANCE	SEE A.A.S.H.T.O.
STRESS IN EXTREME FIBER OF	
STRUCTURAL STEEL - AASHTO M270 GRADE 36 -	20,000 LBS. PER SQ. IN.
- AASHTO M270 GRADE 50W -	27,000 LBS. PER SQ. IN.
- AASHTO M270 GRADE 50 -	27,000 LBS. PER SQ. IN.
REINFORCING STEEL IN TENSION	
GRADE 60	24,000 LBS. PER SQ. IN.
CONCRETE IN COMPRESSION	1,200 LBS. PER SQ. IN.
CONCRETE IN SHEAR	SEE A.A.S.H.T.O.
STRUCTURAL TIMBER - TREATED OR	
UNTREATED - EXTREME FIBER STRESS	1,800 LBS. PER SQ. IN.
COMPRESSION PERPENDICULAR TO GRAIN OF TIMBER	375 LBS. PER SQ. IN.
EQUIVALENT FLUID PRESSURE OF EARTH	30 LBS. PER CU. FT.

MATERIAL AND WORKMANSHIP:

EXCEPT AS MAY OTHERWISE BE SPECIFIED ON PLANS OR IN THE SPECIAL PROVISIONS, ALL MATERIAL AND WORKMANSHIP SHALL BE IN ACCORDANCE WITH THE 2012 "STANDARD SPECIFICATIONS FOR ROADS AND STRUCTURES" OF THE N. C. DEPARTMENT OF TRANSPORTATION.

(MINIMUM)

STEEL SHEET PILING FOR PERMANENT OR TEMPORARY APPLICATIONS SHALL BE HOT ROLLED.

CONCRETE:

UNLESS OTHERWISE REQUIRED ON PLANS, CLASS A CONCRETE SHALL BE USED FOR ALL PORTIONS OF ALL STRUCTURES WITH THE EXCEPTION THAT: CLASS AA CONCRETE SHALL BE USED IN BRIDGE SUPERSTRUCTURES, ABUTMENT BACKWALLS, AND APPROACH SLABS; AND CLASS B CONCRETE SHALL BE USED FOR SLOPE PROTECTION AND RIP RAP.

CONCRETE CHAMFERS:

UNLESS OTHERWISE NOTED ON THE PLANS, ALL EXPOSED CORNERS ON STRUCTURES SHALL BE CHAMFERED 3/4"WITH THE FOLLOWING EXCEPTIONS: TOP CORNERS OF CURBS MAY BE ROUNDED TO 1-1/2"RADIUS WHICH IS BUILT INTO CURB FORMS; CORNERS OF TRANSVERSE FLOOR EXPANSION JOINTS SHALL BE ROUNDED WITH A 1/4"FINISHING TOOL UNLESS OTHERWISE REQUIRED ON PLANS; AND CORNERS OF EXPANSION JOINTS IN THE ROADWAY FACES AND TOPS OF CURBS AND SIDEWALKS SHALL BE ROUNDED TO A 1/4"RADIUS WITH A FINISHING STONE OR TOOL UNLESS OTHERWISE REQUIRED ON PLANS.

DOWELS:

DOWELS WHEN INDICATED ON PLANS AS FOR CULVERT EXTENSIONS, SHALL BE EMBEDDED AT LEAST 12" INTO THE OLD CONCRETE AND GROUTED INTO PLACE WITH 1:2 CEMENT MORTAR.

ALLOWANCE FOR DEAD LOAD DEFLECTION, SETTLEMENT, ETC. IN CASTING SUPERSTRUCTURES:

BRIDGES SHALL BE BUILT ON THE GRADE OR VERTICAL CURVE SHOWN ON PLANS. SLABS, CURBS AND PARAPETS SHALL CONFORM TO THE GRADE OR CURVE.

ALL DIMENSIONS WHICH ARE GIVEN IN SECTION AND ARE AFFECTED BY DEAD LOAD DEFLECTIONS ARE DIMENSIONS AT CENTER LINE OF BEARING UNLESS OTHERWISE NOTED ON PLANS. IN SETTING FORMS FOR STEEL BEAM BRIDGES AND PRESTRESSED CONCRETE GIRDER BRIDGES, ADJUSTMENTS SHALL BE MADE DUE TO THE DEAD LOAD DEFLECTIONS FOR THE ELEVATIONS SHOWN. WHERE BLOCKS ARE SHOWN OVER BEAMS FOR BUILDING UP TO THE SLAB, THE VERTICAL DIMENSIONS OF THE BLOCKS SHALL BE ADJUSTED BETWEEN BEARINGS TO COMPENSATE FOR DEAD LOAD DEFLECTIONS, VERTICAL CURVE ORDINATE, AND ACTUAL BEAM CAMBER. WHERE BOTTOM OF SLAB IS IN LINE WITH BOTTOM OF TOP FLANGES, DEPTH OF SLAB BETWEEN BEARINGS SHALL BE ADJUSTED TO COMPENSATE FOR DEAD LOAD DEFLECTION, VERTICAL CURVE ORDINATE, AND ACTUAL BEAM CAMBER.

IN SETTING FALSEWORK AND FORMS FOR REINFORCED CONCRETE SPANS, AN ALLOWANCE SHALL BE MADE FOR DEAD LOAD DEFLECTIONS, SETTLEMENT OF FALSEWORK, AND PERMANENT CAMBER WHICH SHALL BE PROVIDED FOR IN ADDITION TO THE ELEVATIONS SHOWN. AFTER REMOVAL OF THE FALSEWORK, THE FINISHED STRUCTURES SHALL CONFORM TO THE PROFILE AND ELEVATIONS SHOWN ON THE PLANS AND CONSTRUCTION ELEVATIONS FURNISHED BY THE ENGINEER.

DETAILED DRAWINGS FOR FALSEWORK OR FORMS FOR BRIDGE SUPERSTRUCTURE AND ANY STRUCTURE OR PARTS OF A STRUCTURE AS NOTED ON THE PLANS SHALL BE SUBMITTED TO THE ENGINEER FOR APPROVAL BEFORE CONSTRUCTION OF THE

REINFORCING STEEL:

FALSEWORK OR FORMS IS STARTED.

ALL REINFORCING STEEL SHALL BE DEFORMED. DIMENSIONS RELATIVE TO PLACEMENT OF REINFORCING ARE TO CENTERS OF BARS UNLESS OTHERWISE INDICATED IN THE PLANS. DIMENSIONS ON BAR DETAILS ARE TO CENTERS OF BARS OR ARE OUT TO OUT AS INDICATED ON PLANS.

WIRE BAR SUPPORTS SHALL BE PROVIDED FOR REINFORCING STEEL WHERE INDICATED ON THE PLANS. WHEN BAR SUPPORT PIECES ARE PLACED IN CONTINUOUS LINES, THEY SHALL BE SO PLACED THAT THE ENDS OF THE SUPPORTING WIRES SHALL BE LAPPED TO LOCK LEGS ON ADJOINING PIECES.

STRUCTURAL STEEL:

AT THE CONTRACTOR'S OPTION, HE MAY SUBSTITUTE 7/8" Ø SHEAR STUDS FOR THE 3/4" Ø STUDS SPECIFIED ON THE PLANS. THIS SUBSTITUTION SHALL BE MADE AT THE RATE OF 3 - 7/8" Ø STUDS FOR 4 - 3/4" Ø STUDS, AND STUD SPACING CHANGES SHALL BE MADE AS NECESSARY TO PROVIDE THE SAME EQUIVALENT NUMBER OF 7/8" Ø STUDS ALONG THE BEAM AS SHOWN FOR 3/4" Ø STUDS BASED ON THE RATIO OF 3 - 7/8" Ø STUDS FOR 4 - 3/4" Ø STUDS. STUDS OF THE LENGTH SPECIFIED ON THE PLANS MUST BE PROVIDED. THE MAXIMUM SPACING SHALL BE 2'-0".

EXCEPT AT THE INTERIOR SUPPORTS OF CONTINUOUS BEAMS WHERE THE COVER PLATE IS IN CONTACT WITH BEARING PLATE, THE CONTRACTOR MAY, AT HIS OPTION, SUBSTITUTE FOR THE COVER PLATES DESIGNATED ON THE PLANS COVER PLATES OF THE EQUIVALENT AREA PROVIDED THESE PLATES ARE AT LEAST 5/16"IN THICKNESS AND DO NOT EXCEED A WIDTH EQUAL TO THE FLANGE WIDTH LESS 2"OR A THICKNESS EQUAL TO 2 TIMES THE FLANGE THICKNESS. THE SIZE OF FILLET WELDS SHALL CONFORM TO THE REQUIREMENTS OF THE CURRENT ANSI/AASHTO/AWS "BRIDGE WELDING CODE". ELECTROSLAG WELDING WILL NOT BE PERMITTED.

ELECTROSLAG WELDING WILL NOT BE PERMITTED.

WITH THE SOLE EXCEPTION OF EDGES AT SURFACES WHICH BEAR ON OTHER SURFACES, ALL SHARP EDGES AND ENDS OF SHAPES AND PLATES SHALL BE SLIGHTLY ROUNDED BY SUITABLE MEANS TO A RADIUS OF APPROXIMATELY 1/16 INCH OR EQUIVALENT FLAT SURFACE AT A SUITABLE ANGLE PRIOR TO PAINTING, GALVANIZING, OR METALLIZING.

HANDRAILS AND POSTS:

METAL STANDARDS AND FACES OF THE CONCRETE END POSTS FOR THE METAL RAIL SHALL BE SET NORMAL TO THE GRADE OF THE CURB, UNLESS OTHERWISE SHOWN ON PLANS. THE METAL RAIL AND TOPS OF CONCRETE POSTS USED WITH THE ALUMINUM RAIL SHALL BE BUILT PARALLEL TO THE GRADE OF THE CURB.

METAL HANDRAILS SHALL BE IN ACCORDANCE WITH THE PLANS. RAILS SHALL BE AS MANUFACTURED FOR BRIDGE RAILING. CASTINGS SHALL BE OF A UNIFORM APPEARANCE. FINS AND OTHER DEFORMATIONS RESULTING FROM CASTING OR OTHERWISE SHALL BE REMOVED IN A MANNER SO THAT A UNIFORM COLORING OF THE COMPLETED CASTING SHALL BE OBTAINED. CASTINGS WITH DISCOLORATIONS OR OF NON-UNIFORM COLORING WILL NOT BE ACCEPTED. CERTIFIED MILL REPORTS ARE REQUIRED FOR METAL RAILS AND POSTS.

SPECIAL NOTES:

GENERALLY, IN CASE OF DISCREPANCY, THIS STANDARD SHEET OF NOTES SHALL GOVERN OVER THE SPECIFICATIONS, BUT THE REMAINDER OF THE PLANS SHALL GOVERN OVER NOTES HEREON, AND SPECIAL PROVISIONS SHALL GOVERN OVER ALL. SEE SPECIFICATIONS ARTICLE 105-4.

ENGLISH

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