VICINITY MAP ● ● DETOUR ROUTE

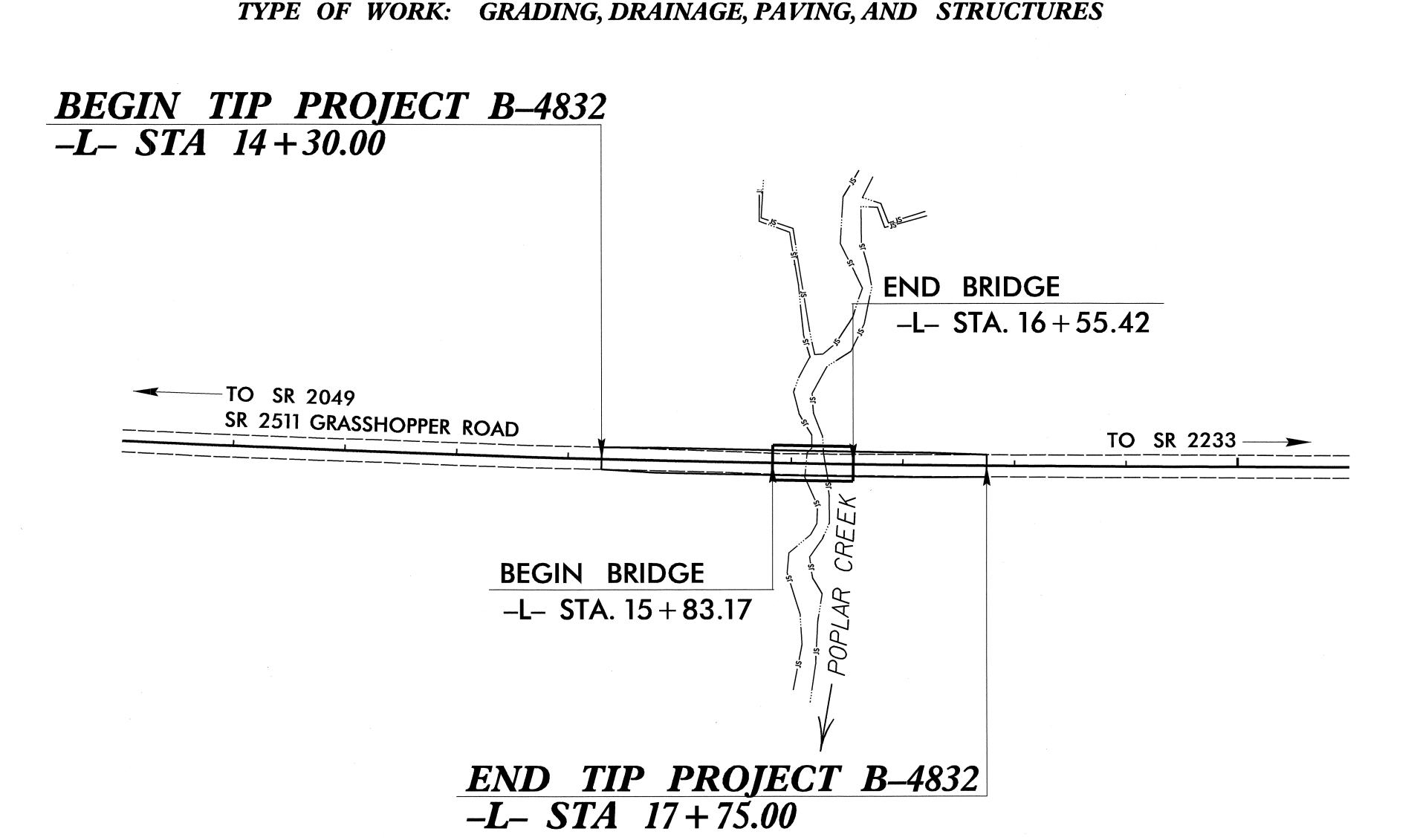
STATE OF NORTH CAROLINA DIVISION OF HIGHWAYS

WAKE COUNTY

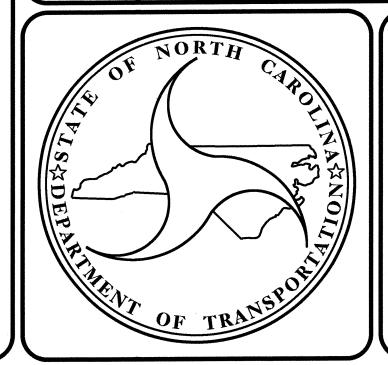
LOCATION: BRIDGE NO. 230 OVER POPLAR CREEK

ON SR 2511 (GRASSHOPPER RD.)

STATE PROJECT REPERENCE NO. B-4832 BRZ-2511(1) 38602.1.1 38602.2.1 BRZ-2511(1) R/W & UTILITIES BRZ-2511(1) CONST. 38602.3.1



STRUCTURES



DESIGN DATA

ADT 2012 = 3827

ADT 2032 = 9131

DHV = 13%

D = 65%

T = 6% *

*(TTST 1% + DUAL 5%) V = 60 MPH

CLASS = RURAL LOCAL

SUBREGIONAL TIER

PROJECT LENGTH

LENGTH ROADWAY TIP PROJECT B-4832 = 0.051 mi.

LENGTH STRUCTURE TIP PROJECT B-4832 = 0.014 mi.

TOTAL LENGTH TIP PROJECT B-4832 = 0.065 mi.

2012 STANDARD SPECIFICATIONS

Prepared in the Office of:

DIVISION OF HIGHWAYS

J. M. BAILEY, P.E.

PROJECT ENGINEER

K. W. ALFORD, P.E.

PROJECT DESIGN ENGINEER

LETTING DATE: **DECEMBER 18, 2012**

STRUCTURES MANAGEMENT UNIT 1000 BIRCH RIDGE DR. **RALEIGH, N.C. 27610**

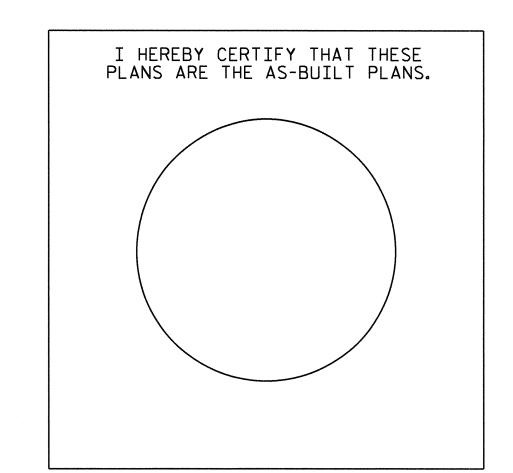
DIVISION OF HIGHWAYS STATE OF NORTH CAROLINA

STATE DESIGN ENGINEER

DEPARTMENT OF TRANSPORTATION FEDERAL HIGHWAY ADMINISTRATION

APPROVED
DIVISION ADMINISTRATOR

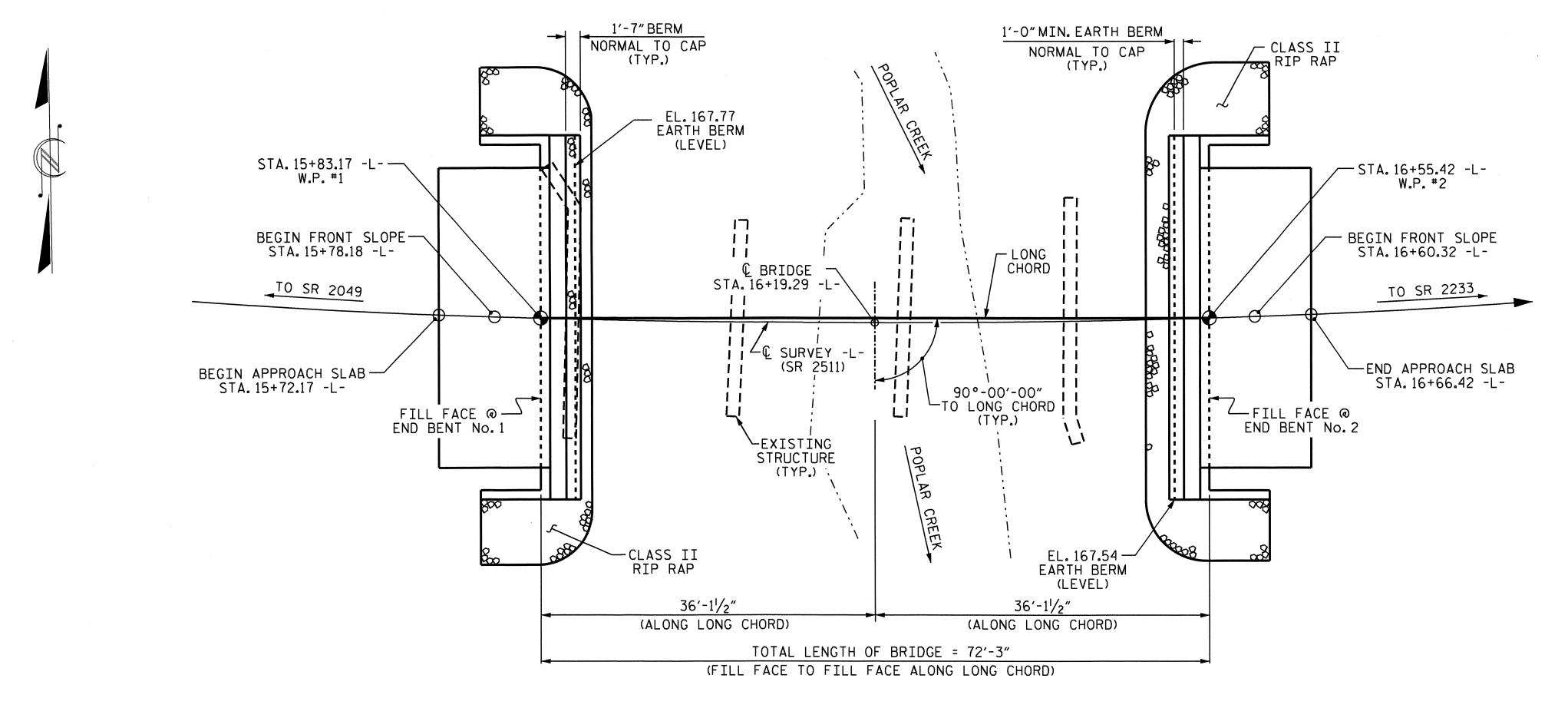
11-SEP-2012 10:26
R:\Structures\Plans\Drafting\Misc\B4832_SD_TSH.dgn
kalford



(-)2.8125% (-)0.3283% FILL FACE @ END BENT No.1 PI= 15+10.00 STA. 15+83.17 -L-GRADE PT. EL. = 173.56 EL= 173.80' SPAN "A" VC = 160' FILL FACE @ END BENT No. 2 STA.16+55.42 -L-GRADE PT.EL. = 173.32 GRADE DATA -L-HIGH WATER BEGIN FRONT SLOPE NORMAL WATER _SURFACE EL. = 175.0 ± STA.15+78.18 -L-GRADE PT.EL. = 173.59 SURFACE EL. = 163.6 — (01/2011) (1996)1'-7" BERM BEGIN FRONT SLOPE NORMAL TO CAP __ EL. 173.0 ± STA. 16+60.32 -L-GRADE PT. EL. = 173.31 EL. 173.0 ± -(TYP.) ___EL. 171.0 ± -- EXISTING STRUCTURE FIX. FIX. EL. 173.0 ± -7 (TYP.) / EXCAVATE TO EL.169.0± __GRADE TO DRAIN_/ — EL. 173.0 ± EL.171.0 ± — -170 — APPROXIMATE 1'-6" (TYP.) NATURAL GROUND EL. 167.0 ± -11'-0"MIN. EARTH BERM EL. 167.0 ± ^{\(\Delta\)}!! NORMAL TO CAP | | | **-**160 (TYP.) EL. 163.0 ± EL. 164.0 ± — CLASS II
RIP RAP. (TYP.) └─1¹/₂: 1 SLOPE (NORMAL TO CAP) HP 12 X 53 — STEEL PILES (TYP.) EXCAVATE TO -EL.168.0 ±
GRADE TO DRAIN UNCLASSIFIED STRUCTURE—>
EXCAVATION (TYP.) END BENT No. 1 END BENT No. 2

SECTION ALONG & SURVEY -L-

(SECTION AT END BENTS ARE AT RIGHT ANGLES)



PLAN

(PILES NOT SHOWN IN PLAN VIEW FOR CLARITY)

€ SURVEY -L-LONG CHORD 7 W.P.#2-∠ W. P. #1 - © BRIDGE STA.16+19.29 -L-

THE EFFECTS OF THE HORIZONTAL CURVE SHALL BE NEGLECTED IN THE CONSTRUCTION OF THIS BRIDGE. BRIDGE TO BE BUILT ALONG THE LONG CHORD BETWEEN THE WORK POINTS AT THE FILL FACES.

> B-4832 PROJECT NO. WAKE COUNTY

16+19.29 -L-STATION:_

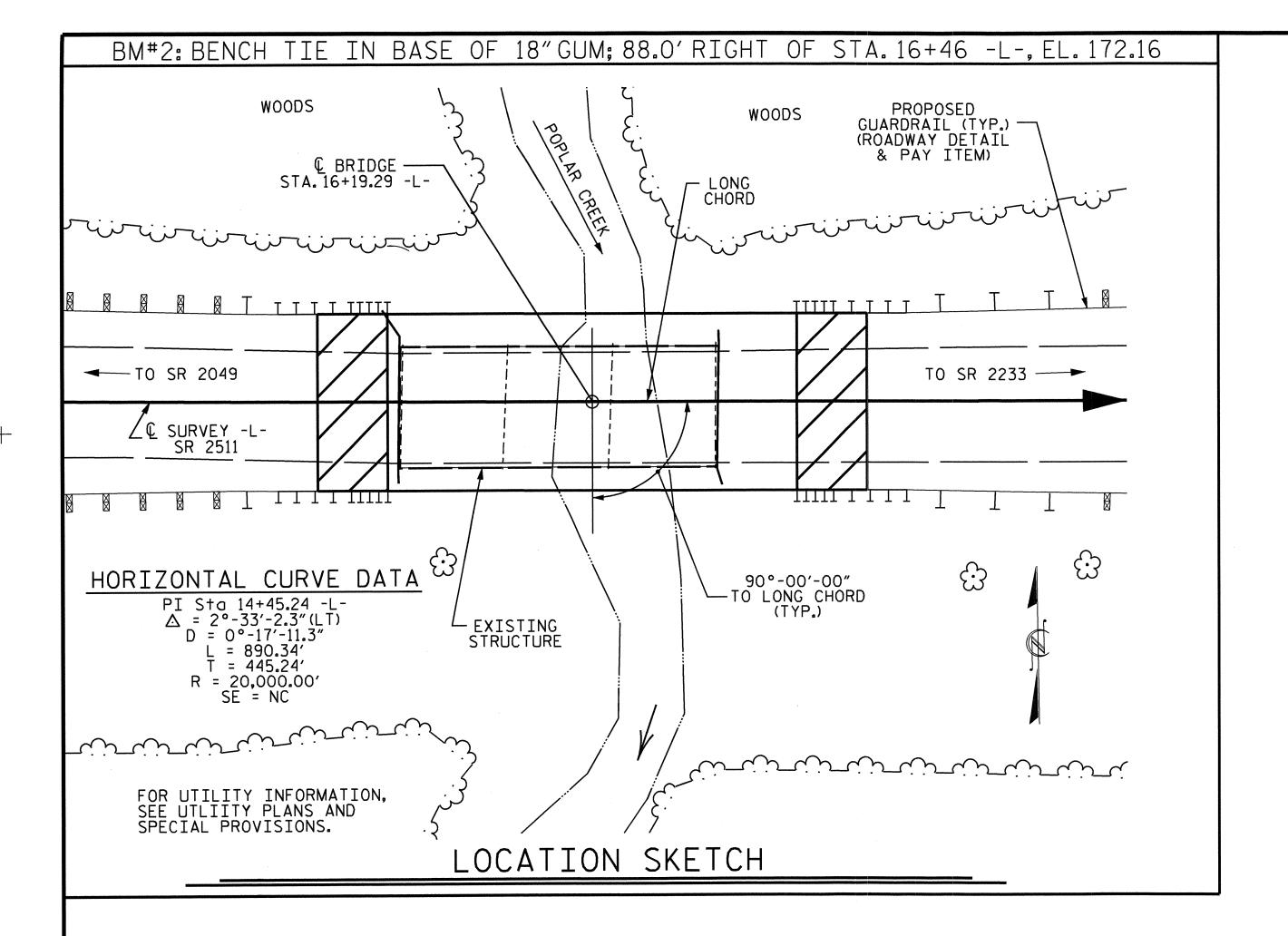
SHEET 1 OF 2 REPLACES BRIDGE No. 230 STATE OF NORTH CAROLINA

DEPARTMENT OF TRANSPORTATION GENERAL DRAWING

BRIDGE ON SR 2511 (GRASSHOPPER RD.) OVER POPLAR CREEK BETWEEN SR 2049 AND SR 2233

NO. BY: DATE: NO. BY:	DATE: S-1
	DA . L.
1 3	TOTAL SHEETS
2 4	13

J.L. LAMBERT DATE: 5/2012 CHECKED BY : S.H. SOCKWELL DATE : 5/2012



HYDRAULIC DATA

DESIGN DISCHARGE _____ 1320 CFS FREQUENCY OF DESIGN FLOOD ______ 10 YR.
DESIGN HIGH WATER ELEVATION _____ 171.53
DRAINAGE AREA ______ 7.8 SO. MI. BASE DISCHARGE (0100) _____ 3320 CFS BASE HIGH WATER ELEVATION _____ 175.04

OVERTOPPING FLOOD DATA

OVERTOPPING DISCHARGE _____ 1650 CFS FREQUENCY OF OVERTOPPING FLOOD ___ 10 YR. + OVERTOPPING FLOOD ELEVATION _____ 172.46 @ SAG STA. 18+73.44 -L-

			TOTA	L BI	LL OF	MA	ATER	IAL -					
	REMOVAL OF EXISTING STRUCTURE	UNCLASSIFIED STRUCTURE EXCAVATION	CLASS A CONCRETE	BRIDGE APPROACH SLABS	REINFORCING STEEL		12 X 53 EL PILES	VERTICAL CONCRETE BARRIER RAIL	RIP RAP CLASS II (2'-0" THICK)	GEOTEXTILE FOR DRAINAGE	ELASTOMERIC BEARINGS	PRE:)" X 2'-0" STRESSED NCRETE ED SLABS
	LUMP SUM	LUMP SUM	CU. YDS.	LUMP SUM	LBS.	NO.	LIN.FT.	LIN.FT.	TONS	SQ. YDS.	LUMP SUM	NO.	LIN.FT.
SUPERSTRUCTURE				LUMP SUM				140.25			LUMP SUM	11	770
END BENT NO. 1		LUMP SUM	14.5		2135	7	280		45	50			
END BENT NO. 2		LUMP SUM	14.5		2135	7	270		45	50			
TOTAL	LUMP SUM	LUMP SUM	29.0	LUMP SUM	4270	14	550	140.25	90	100	LUMP SUM	11	770

NOTES

THIS BRIDGE HAS BEEN DESIGNED IN ACCORDANCE WITH THE AASHTO LRFD BRIDGE DESIGN SPECIFICATIONS.

ASSUMED LIVE LOAD = HL-93 OR ALTERNATE LOADING.

FOR OTHER DESIGN DATA AND GENERAL NOTES, SEE SHEET SN.

THE EXISTING STRUCTURE CONSISTING OF SPANS OF 1 @ 18-5", 1 @ 18'-0", & 1 @ 18'-5". WITH A TIMBER DECK ON I-BEAMS AND A CLEAR ROADWAY WIDTH OF 19'-1" ON TIMBÉR CAP & PILES END BENTS AND BENTS, AND LOCATED AT THE PROPOSED STRUCTURE SHALL BE REMOVED. THE EXISTING BRIDGE IS PRESENTLY POSTED BELOW THE LEGAL LOAD LIMIT. SHOULD THE STRUCTURAL INTEGRITY OF THE BRIDGE FURTHER DETERIORATE. THIS LOAD LIMITATION MAY BE REDUCED AS FOUND NECESSARY DURING THE LIFE OF THE PROJECT. FOR REMOVAL OF EXISTING STRUCTURE, SEE SPECIAL PROVISIONS.

REMOVAL OF THE EXISTING BRIDGE SHALL BE PERFORMED SO AS NOT TO ALLOW DEBRIS TO FALL INTO THE WATER. THE CONTRACTOR SHALL REMOVE THE BRIDGE AND SUBMIT PLANS FOR DEMOLITION IN ACCORDANCE WITH ARTICLE 402-2 OF THE STANDARD SPECIFICATIONS.

THE MATERIAL SHOWN IN THE CROSS-HATCHED AREA ON SHEET S-1 SHALL BE EXCAVATED FOR A DISTANCE OF 25 FT. EACH SIDE OF CENTERLINE ROADWAY AS DIRECTED BY THE ENGINEER. THIS WORK WILL BE PAID FOR AT THE CONTRACT LUMP SUM PRICE FOR UNCLASSIFIED STRUCTURE EXCAVATION. SEE SECTION 412 OF THE STANDARD SPECIFICATIONS.

THE SUBSTRUCTURE OF THE EXISTING BRIDGE INDICATED ON THE PLANS IS FROM THE BEST INFORMATION AVAILABLE. SINCE THIS INFORMATION IS SHOWN FOR THE CONVENIENCE OF THE CONTRACTOR, THE CONTRACTOR SHALL HAVE NO CLAIM WHATSOEVER AGAINST THE DEPARTMENT OF TRANSPORTATION FOR ANY DELAYS OR ADDITIONAL COST INCURRED BASED ON DIFFERENCES BETWEEN THE EXISTING BRIDGE SUBSTRUCTURE SHOWN ON THE PLANS AND THE ACTUAL CONDITIONS AT THE PROJECT SITE.

ASPHALT WEARING SURFACE IS INCLUDED IN ROADWAY QUANTITY ON ROADWAY PLANS.

THIS STRUCTURE HAS BEEN DESIGNED IN ACCORDANCE WITH HEC 18, "EVALUATING SCOUR AT BRIDGES". MAY, 2001.

THE CONTRACTOR SHALL PROVIDE INDEPENDENT ACCORDANCE SAMPLES OF REINFORCING STEEL AS FOLLOWS: FOR PROJECTS REQUIRING UP TO 400 TONS OF REINFORCING STEEL, ONE 30 INCH SAMPLE OF EACH SIZE BAR USED, AND FOR PROJECTS REQUIRING OVER 400 TONS OF REINFORCING STEEL, TWO 30 INCH SAMPLES OF EACH SIZE BAR USED. THE BARS FROM WHICH THE SAMPLES ARE TAKEN MUST THEN BE SPLICED WITH REPLACEMENT BARS OF THE SIZE AND LENGTH OF THE SAMPLE, PLUS A MINIMUM LAP SPLICE OF THIRTY BAR DIAMETERS. PAYMENT FOR THE SAMPLES OF REINFORCING STEEL SHALL BE CONSIDERED INCIDENTAL TO VARIOUS PAY ITEMS.

INASMUCH AS THE PAINT SYSTEM ON THE EXISTING STRUCTURAL STEEL CONTAINS LEAD, THE CONTRACTOR'S ATTENTION IN CALLED TO SECTION 107-1 OF THE STANDARD SPECIFICATIONS. ANY COSTS RESULTING FROM COMPLIANCE WITH STATE OR FEDERAL REGULATIONS PERTAINING TO HANDLING OF MATERIALS CONTAINING LEAD BASED PAINT SHALL BE INCLUDED IN THE BID PRICE FOR "REMOVAL OF EXISTING STRUCTURE AT STA. 16+19.29 -L-".

FOR SUBMITTAL OF WORKING DRAWINGS, SEE SPECIAL PROVISIONS.

FOR FALSEWORK AND FORMWORK, SEE SPECIAL PROVISIONS.

FOR CRANE SAFETY, SEE SPECIAL PROVISIONS.

FOR EROSION CONTROL MEASURES, SEE EROSION CONTROL PLANS.

FOR GROUT FOR STRUCTURES, SEE SPECIAL PROVISIONS.

THIS BRIDGE IS LOCATED IN SEISMIC ZONE 1.

FOR PILES, SEE SECTION 450 OF THE STANDARD SPECIFICATIONS.

PILES AT END BENT No.1 AND END BENT No.2 ARE DESIGNED FOR A FACTORED RESISTANCE OF 85 TONS PER PILE.

DRIVE PILES AT END BENT No.1 AND END BENT No.2 TO A REQUIRED DRIVING RESISTANCE OF 145 TONS PER PILE.

> PROJECT NO. B-4832 WAKE COUNTY

STATION: 16+19.29 -L-

SHEET 2 OF 2

STATE OF NORTH CAROLINA DEPARTMENT OF TRANSPORTATION

GENERAL DRAWING BRIDGE ON SR 2511 (GRASSHOPPER RD.) OVER POPLAR CREEK BETWEEN SR 2049 AND

	SHEET NO.					
NO.	BY:	DATE:	NO.	BY:	DATE:	S-2
1			3			TOTAL SHEETS
2			4			13

CHECKED BY : S.H. SOCKWELL DATE : 5/2012

LOAD AND RESISTANCE FACTOR RATING (LRFD) SUMMARY FOR PRESTRESSED CONCRETE GIRDERS

										STRE	NGTH	I LIN	MIT ST	ГАТЕ	***************************************			SE	RVICE	III	LIMI	T STA	TE	
										MOMENT					SHEAR						MOMENT			
LEVEL		VEHICLE	WEIGHT (W) (TONS)	CONTROLLING LOAD RATING	MINIMUM RATING FACTORS (RF)	TONS = W X RF	LIVELOAD FACTORS	DISTRIBUTION FACTORS (DF)	RATING FACTOR	SPAN	GIRDER LOCATION	DISTANCE FROM LEFT END OF SPAN (ft)	DISTRIBUTION FACTORS (DF)	RATING FACTOR	SPAN	GIRDER LOCATION	DISTANCE FROM LEFT END OF SPAN (ft)	LIVELOAD FACTORS	DISTRIBUTION FACTORS (DF)	RATING FACTOR	SPAN	GIRDER LOCATION	DISTANCE FROM LEFT END OF SPAN (ft)	COMMENT NUMBER
		HL-93(Inv)	N/A	1	1.006		1.75	0.273	1.03	70′	EL	34.5	0.507	1.32	70′	EL	6.9	0.80	0.273	1.01	70′	EL	34.5	
DESIGN		HL-93(0pr)	N/A		1.341		1.35	0.273	1.34	70′	EL	34.5	0.507	1.72	70′	EL	6.9	N/A	· 					
LOAD RATING		HS-20(Inv)	36.000	2	1.306	47.02	1.75	0.273	1.34	70′	EL	34.5	0.507	1.65	70′	EL	6.9	0.80	0.273	1.31	70′	EL	34.5	
NATING		HS-20(0pr)	36.000		1.74	62.64	1.35	0.273	1.74	70′	EL	34.5	0.507	2.14	70′	EL	6.9	N/A						
		SNSH	13.500		2.917	39.379	1.4	0.273	3.75	70′	EL	34.5	0.507	4.87	70′	EL	6.9	0.80	0.273	2.92	70′	EL	34.5	
		SNGARBS2	20.000	Max Max	2.187	43.741	1.4	0.273	2.81	70′	EL	34.5	0.507	3.47	70′	EL	6.9	0.80	0.273	2.19	70′	EL	34.5	
		SNAGRIS2	22.000		2.077	45.69	1.4	0.273	2.67	70′	EL	34.5	0.507	3.23	70′	EL	6.9	0.80	0.273	2.08	70′	EL	34.5	
		SNCOTTS3	27.250		1.452	39.565	1.4	0.273	1.87	70′	EL	34.5	0.507	2.43	70′	EL	6.9	0.80	0.273	1.45	70′	EL	34.5	
	\ S	SNAGGRS4	34.925		1.218	42.554	1.4	0.273	1.57	70′	EL	34.5	0.507	2.03	70′	EL	6.9	0.80	0.273	1.22	70′	EL	34.5	
		SNS5A	35.550		1.191	42.346	1.4	0.273	1.53	70′	EL	34.5	0.507	2.06	70′	EL	6.9	0.80	0.273	1.19	70′	EL	34.5	
		SNS6A	39.950	### ####	1.095	43.747	1.4	0.273	1.41	70′	EL	34.5	0.507	1.88	70′	EL	6.9	0.80	0.273	1.10	70′	EL	34.5	
LEGAL		SNS7B	42.000	•••	1.043	43.801	1.4	0.273	1.34	70′	EL	34.5	0.507	1.85	70′	EL	6.9	0.80	0.273	1.04	70′	EL	34.5	
LOAD RATING	-	TNAGRIT3	33.000		1.336	44.087	1.4	0.273	1.72	70′	EL	34.5	0.507	2.23	70′	EL	6.9	0.80	0.273	1.34	70′	EL	34.5	
I KATINO		TNT4A	33.075	·	1.342	44.401	1.4	0.273	1.72	70′	EL	34.5	0.507	2.17	70′	EL	6.9	0.80	0.273	1.34	70′	EL	34.5	
		TNT6A	41.600		1.1	45.746	1.4	0.273	1.41	70′	EL	34.5	0.507	1.98	70′	EL	6.9	0.80	0.273	1.10	70′	EL	34.5	
	l ST	TNT7A	42.000		1.106	46.462	1.4	0.273	1.42	70′	EL	34.5	0.507	1.94	70′	EL	6.9	0.80	0.273	1.11	70′	EL	34.5	
		TNT7B	42.000		1.147	48.18	1.4	0.273	1.47	70′	EL	34.5	0.507	1.8	70′	EL	6.9	0.80	0.273	1.15	70′	EL	34.5	
		TNAGRIT4	43.000		1.089	46.838	1.4	0.273	1.4	70′	EL	34.5	0.507	1.74	70′	EL	6.9	0.80	0.273	1.09	70′	EL	34.5	
		TNAGT5A	45.000		1.026	46.175	1.4	0.273	1.32	70′	EL	34.5	0.507	1.74	70′	EL	6.9	0.80	0.273	1.03	70′	EL	34.5	
		TNAGT5B	45.000	3	1.013	45.579	1.4	0.273	1.3	70′	EL	34.5	0.507	1.66	70′	EL	6.9	0.80	0.273	1.01	70′	EL	34.5	

LOAD FACTORS:

	DESIGN LOAD RATING	LIMIT STATE	γ_{DC}	$\gamma_{\sf DW}$
		STRENGTH I	1.25	1.50
	FACTORS	SERVICE III	1.00	1.00

NOTES:

MINIMUM RATING FACTORS ARE BASED ON THE STRENGTH I AND SERVICE III LIMIT STATES.

ALLOWABLE STRESSES FOR SERVICE III LIMIT STATE ARE AS REQUIRED FOR DESIGN.

(#) CONTROLLING LOAD RATING

1 DESIGN LOAD RATING (HL-93)

2 DESIGN LOAD RATING (HS-20)

3 LEGAL LOAD RATING **

** SEE CHART FOR VEHICLE TYPE

GIRDER LOCATION

I - INTERIOR GIRDER

EL - EXTERIOR LEFT GIRDER

ER - EXTERIOR RIGHT GIRDER

B-4832 PROJECT NO.____ WAKE _ COUNTY



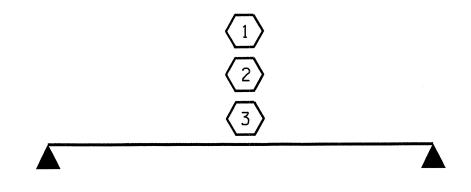
STATION: 16+19.29 -L-

STATE OF NORTH CAROLINA DEPARTMENT OF TRANSPORTATION
RALEIGH

STANDARD LRFR SUMMARY FOR 70' CORED SLAB UNIT 90° SKEW

(NON-INTERSTATE TRAFFIC)

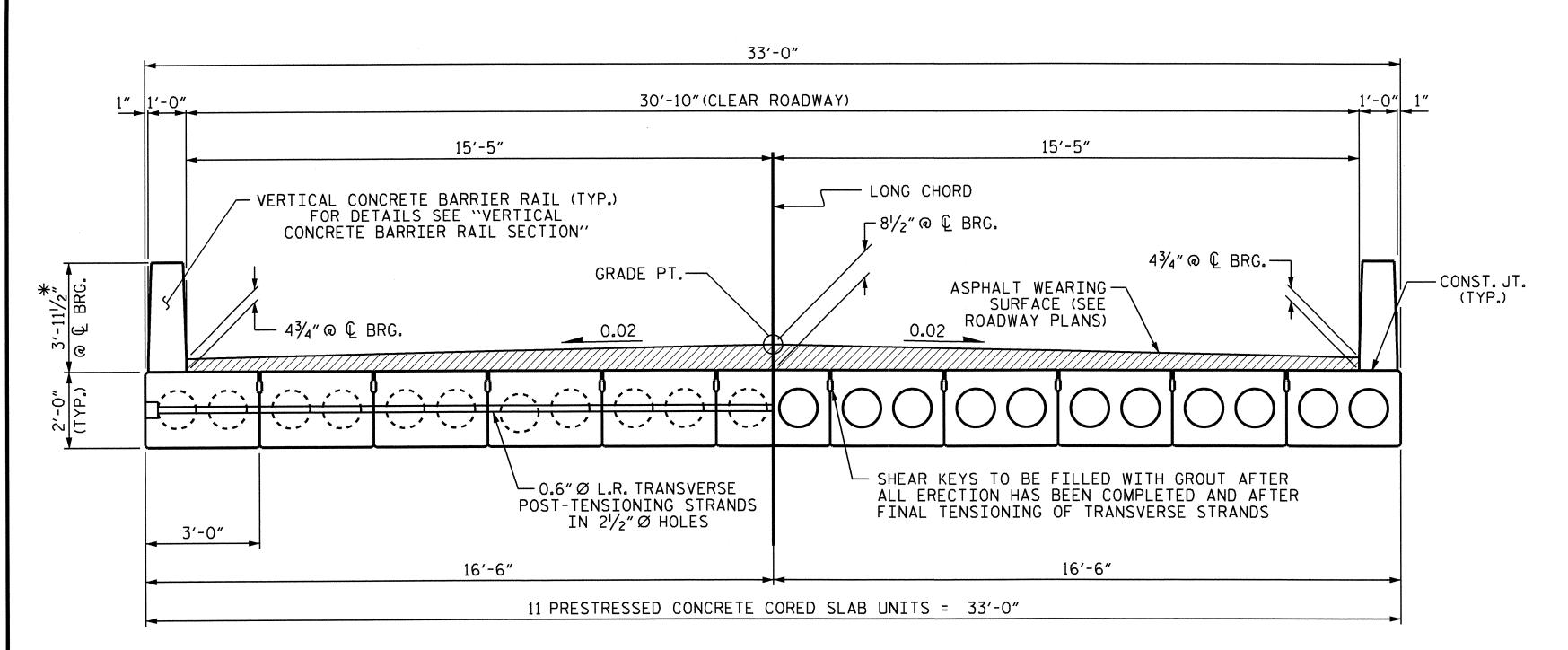
		SHEET NO.				
10.	BY:	DATE:	NO.	BY:	DATE:	S-3
1			3			TOTAL SHEETS
2			4			13



LRFR SUMMARY

FOR SPAN 'A'

ASSEMBLED BY: W.F. PARKER DATE: 6/12 CHECKED BY: K. ALFORD DATE: 6/12 DRAWN BY: CVC 6/10 CHECKED BY: DNS 6/10



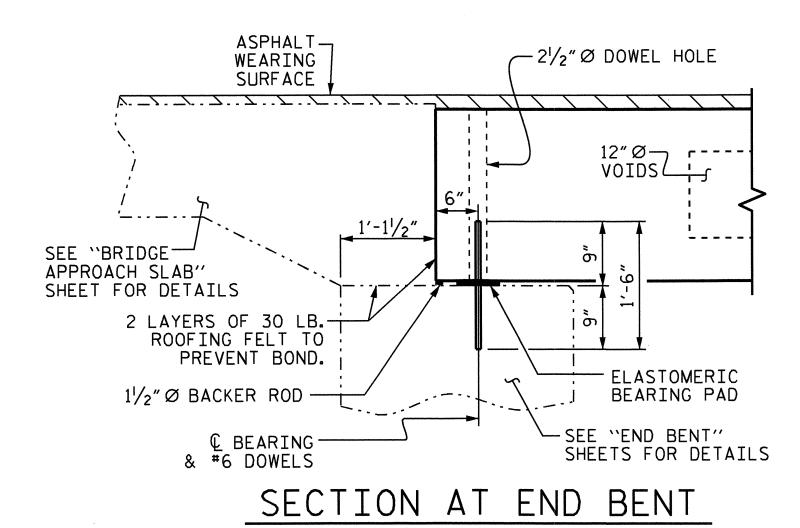
HALF SECTION AT INTERMEDIATE DIAPHRAGMS

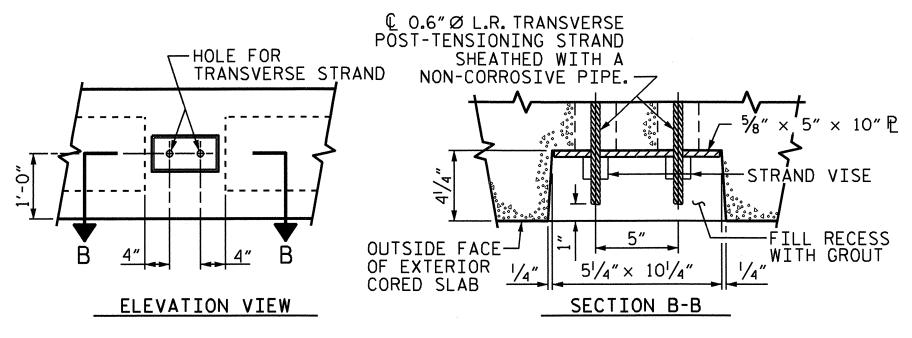
TYPICAL SECTION

HALF SECTION THROUGH VOIDS

*- THE MAXIMUM BARRIER RAIL HEIGHT AND ASPHALT THICKNESS IS SHOWN. THE HEIGHT OF THE BARRIER RAIL AND ASPHALT THICKNESS VARIES WHILE THE TOP OF THE BARRIER RAIL FOLLOWS THE PROFILE OF THE GUTTERLINE. FOR RAIL HEIGHT DETAILS AND ASPHALT THICKNESS, SEE THE "VERTICAL CONCRETE BARRIER RAIL SECTION" DETAIL.

FIXED END





ASSEMBLED BY: A. V. ROYAL DATE: 03/11 CHECKED BY: T. M. GARRISON DATE: 03/11 DRAWN BY: MAA 6/10

CHECKED BY : MKT 7/10

GROUTED RECESS AT END OF POST-TENSIONED STRAND OF CORED SLABS

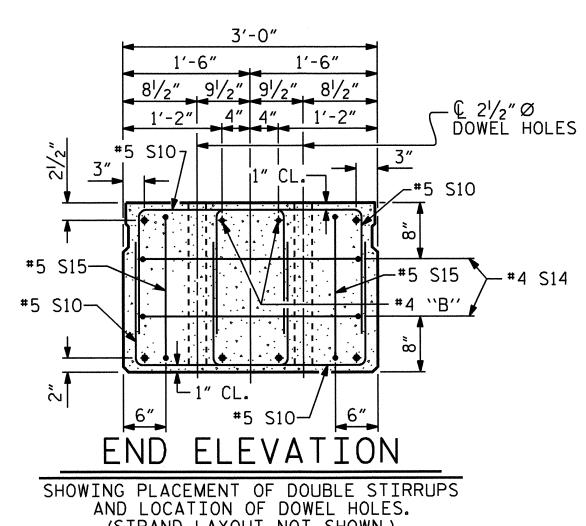
1'-4" 11" 4" 4" 11" #4 \\B''--r12"Ø VOIDS ₹ 2 SPA. @ 2"CTS. └6 SPA. └─2 SPA. 2 SPA. — @ 2"CTS. @ 2"CTS. @ 2"CTS. STRAND LAYOUT FOR 70'UNIT (28 STRANDS REQUIRED)

3'-0"

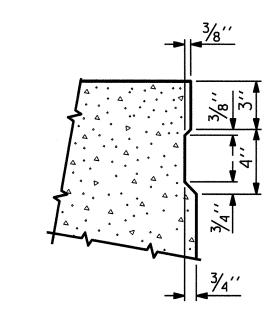
INTERIOR SLAB SECTION 0.6" Ø LOW RELAXATION

BOND SHALL BE BROKEN ON THESE STRANDS FOR A DISTANCE OF 12'-0" FROM END OF CORED SLAB UNIT. SEE STANDARD SPECIFICATIONS. ARTICLE 1078-7.

DEBONDING LEGEND

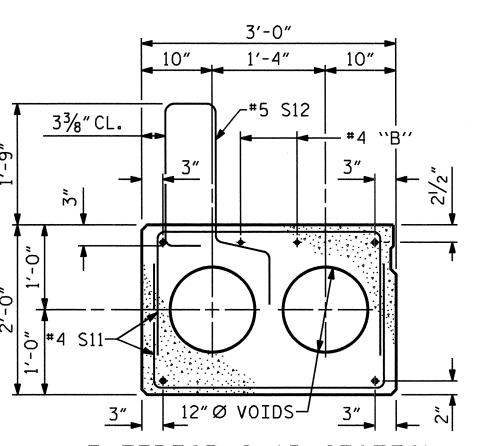


SHOWING PLACEMENT OF DOUBLE STIRRUPS
AND LOCATION OF DOWEL HOLES.
(STRAND LAYOUT NOT SHOWN.)
INTERIOR SLAB UNIT SHOWN-EXTERIOR SLAB
UNIT SIMILAR EXCEPT SHEAR KEY LOCATION.



SHEAR KEY DETAIL NOTE: OMIT SHEAR KEY ON OUTSIDE FACE

OF EXTERIOR CORED SLABS.



EXTERIOR SLAB SECTION (FOR PRESTRESSED STRAND LAYOUT, SEE INTERIOR SLAB SECTION.)

B-4832 PROJECT NO._

WAKE COUNTY

STATION: 16+19.29 -L-

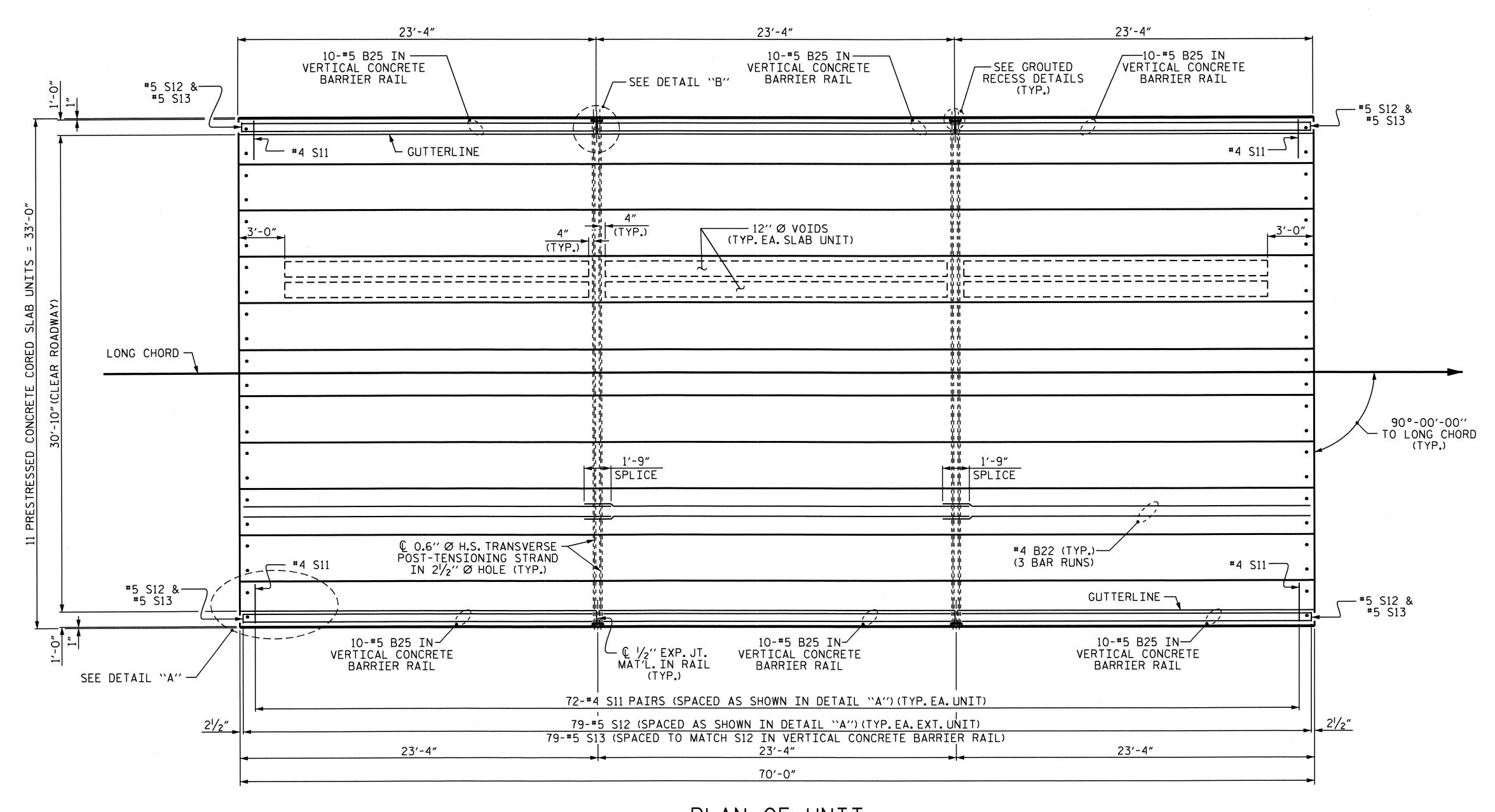
SHEET 1 OF 3

STATE OF NORTH CAROLINA DEPARTMENT OF TRANSPORTATION STANDARD

3'-0'' X 2'-0'' PRESTRESSED CONCRETE CORED SLAB UNIT 90° SKEW

SHEET NO **REVISIONS** S-4 DATE: NO. BY: BY: TOTAL SHEETS

STD. NO. 24PCS4_33_90S



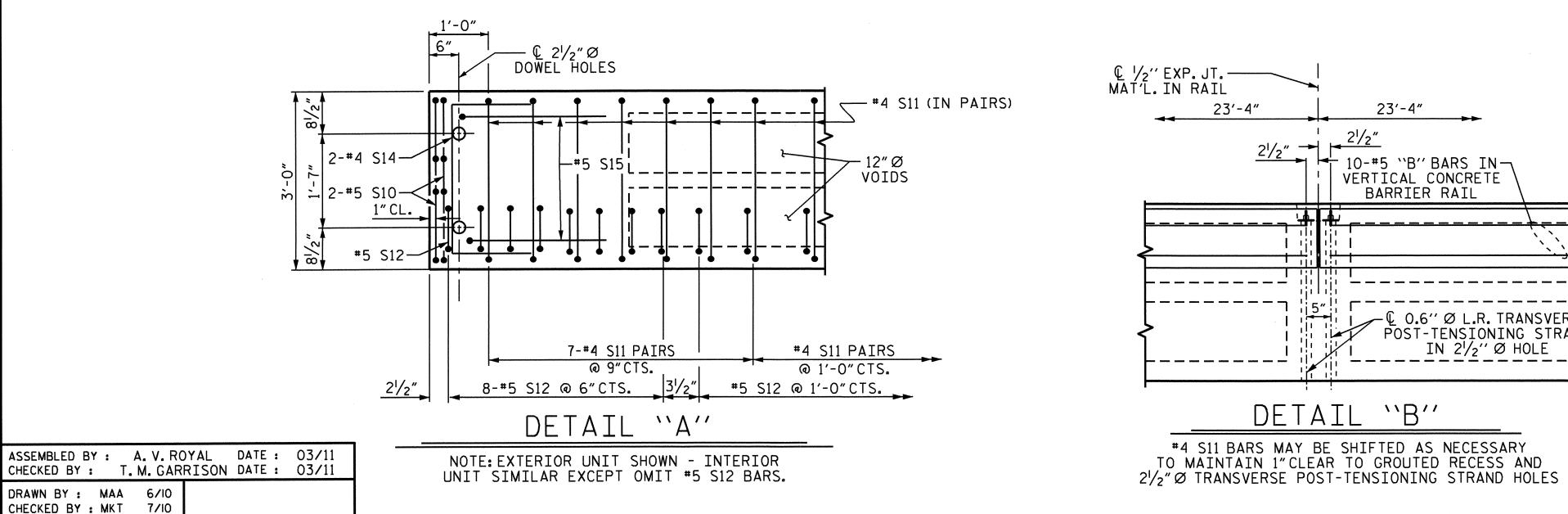
PLAN OF UNIT

23'-4"

DETAIL "B"

10-#5 "B" BARS IN-VERTICAL CONCRETE BARRIER RAIL

−Ç 0.6" Ø L.R. TRANSVERSE POST-TENSIONING STRAND IN 2½" Ø HOLE



PROJECT NO. B-4832 WAKE COUNTY STATION: 16+19.29 -L-

SHEET 2 OF 3

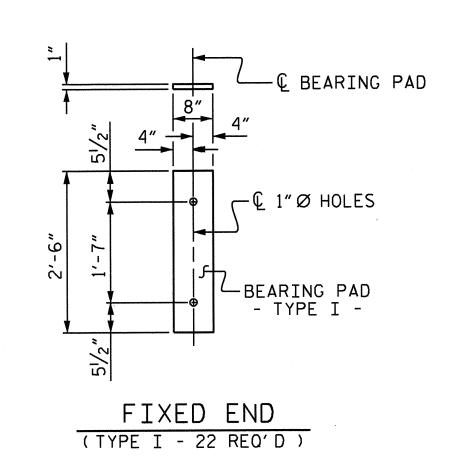
DEPARTMENT OF TRANSPORTATION

STATE OF NORTH CAROLINA

PLAN OF 70'UNIT 30'-10"CLEAR ROADWAY 90° SKEW

		SHEET NO.				
NO.	BY:	DATE:	NO.	BY:	DATE:	S-5
1			3	·		TOTAL SHEETS
2			4			13

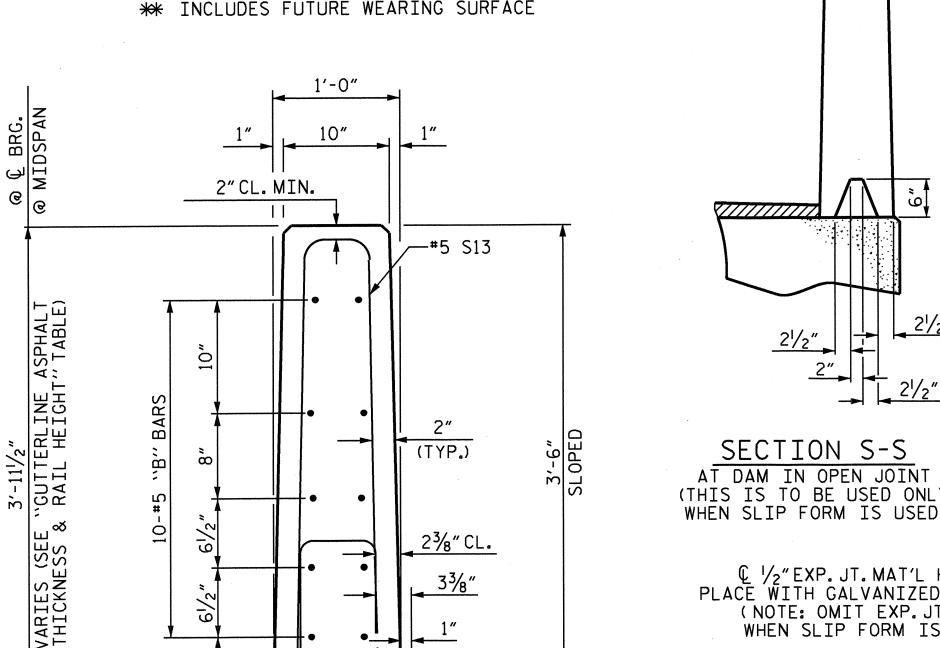
DRAWN BY: MAA 6/10 CHECKED BY: MKT 7/10



ELASTOMERIC BEARING DETAILS

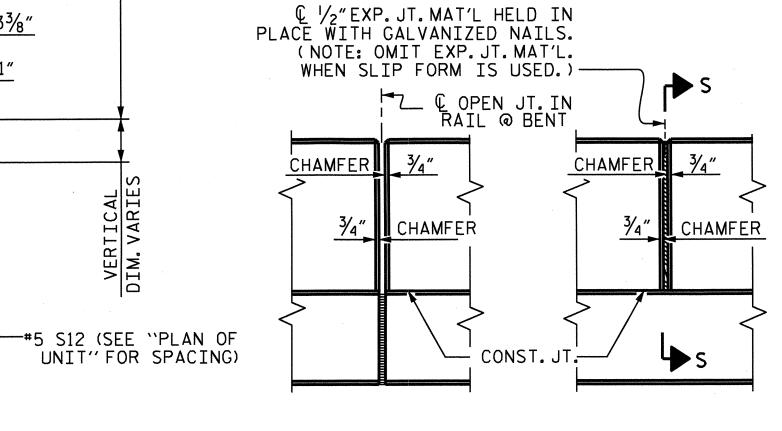
ELASTOMER IN ALL BEARINGS SHALL BE 60 DUROMETER HARDNESS.

DEAD LOAD DEFLECTION AN	ND CAMBER
	3'-0" × 2'-0"
70'CORED SLAB UNIT	0.6″Ø L.R. STRAND
CAMBER (SLAB ALONE IN PLACE)	4 ⁵ ⁄ ₁₆ ″ ∤
DEFLECTION DUE TO SUPERIMPOSED DEAD LOAD**	¹³ ⁄ ₁₆ ″ ♦
FINAL CAMBER	31/2"
* INCLUDES FUTURE WEARING SURE	ACE



23/8" CL.

SECTION S-S AT DAM IN OPEN JOINT (THIS IS TO BE USED ONLY WHEN SLIP FORM IS USED)



ELEVATION AT EXPANSION JOINTS

ASSEMBLED BY: A.V.ROYAL DATE: 03/11 CHECKED BY: T.M.GARRISON DATE: 03/11 DRAWN BY: MAA 6/10 CHECKED BY : MKT 7/10

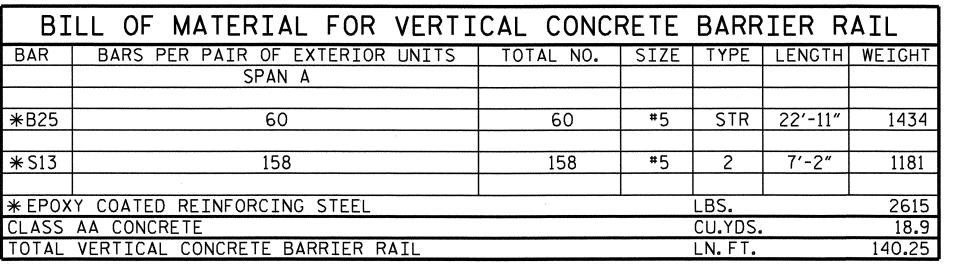
CONST. JT. —

SECTION THRU RAIL

VERTICAL CONCRETE BARRIER RAIL DETAILS

GRADE 270 STRANDS 0.6" Ø L.R. 0.217 (SQUARE INCHES) ULTIMATE STRENGT 58,600 (LBS.PER STRAND APPLIED PRESTRESS 43,950 (LBS.PER STRAND

CORED			
SPAN A	NUMBER	LENGTH	TOTAL LENGTH
EXTERIOR C.S.	2	70′-0″	140'-0"
INTERIOR C.S.	9	70′-0″	630′-0″
TOTAL	11		770′-0"



BAR TYPES

6"

\$10 \$11 \$15

ALL BAR DIMENSIONS ARE OUT TO OUT

2'-7"

2'-8"

1'-9"

3

73/4"

NOTE: S13 BARS SHALL BE FIELD CUT AS NECESSARY TO CLEAR THE TAPER IN THE BARRIER RAIL. FOR TAPER DETAILS, SEE "GUARDRAIL ANCHORAGE FOR VERTICAL BARRIER RAIL" SHEET.

BILL OF MATERIAL FOR ONE 70'CORED SLAB UNIT										
				EXTERI(OR UNIT	INTERI	OR UNIT			
BAR	NUMBER	SIZE	TYPE	LENGTH	WEIGHT	LENGTH	WEIGHT			
B22	6	#4	STR	24'-6"	98	24'-6"	98			
S10	8	#5	3	4'-9"	40	4′-9″	40			
S11	144	#4	3	5′-10″	561	5′-10″	561			
* S12	79	#5	1	6′-4″	522		·			
S14	4	#4	3	5′-7"	15	5′-7″	15			
S15	4	#5	3	7′-1″	30	7′-1″	30			
REINFO	ORCING	STEEL	LB:	S.,	744		744			
* EPOXY COATED REINFORCING STEEL LBS. 522										
7000 P.S.I. CONCRETE CU. YDS. 11.8 11.8										
0.6"Ø	L.R. STR	ANDS	No).	28		28			

	GUTTERLINE	ASPHA	ALT	THICKNESS & RA	AIL HEIGHT
- 1	SPAN A		ASPH	ALT OVERLAY THICKNESS @ MID-SPAN	RAIL HEIGHT @ MID-SPAN
	70' UNITS			11/4"	3′-8″

NOTES

ALL PRESTRESSING STRANDS SHALL BE 7-WIRE LOW RELAXATION GRADE 270 STRANDS AND SHALL CONFORM TO AASHTO M203 EXCEPT FOR SAMPLING REQUIREMENTS WHICH SHALL BE IN ACCORDANCE WITH THE STANDARD SPECIFICATIONS.

ALL REINFORCING STEEL CAST WITH THE CORED SLAB SECTIONS SHALL BE GRADE 60 AND SHALL BE INCLUDED IN THE UNIT PRICE BID FOR PRESTRESSED CONCRETE CORED SLABS.

RECESSES FOR TRANSVERSE STRANDS SHALL BE GROUTED AFTER THE TENSIONING OF THE STRANDS.

THE 21/2" Ø DOWEL HOLES AT FIXED ENDS OF SLAB SECTIONS SHALL BE FILLED WITH NON-SHRINK GROUT.

THE BACKER RODS SHALL CONFORM TO THE REQUIREMENTS OF TYPE M BOND BREAKER. SEE SECTION 1028 OF THE STANDARD SPECIFICATIONS.

WHEN CORED SLABS ARE CAST, AN INTERNAL HOLD-DOWN SYSTEM SHALL BE EMPLOYED TO PREVENT VOIDS FROM RISING OR MOVING SIDEWAYS. AT LEAST SIX WEEKS PRIOR TO CASTING CORED SLABS, THE CONTRACTOR SHALL SUBMI TO THE ENGINEER FOR REVIEW AND COMMENT, DETAILED DRAWINGS OF THE PROPOSED HOLD-DOWN SYSTEM. IN ADDITION TO STRUCTURAL DETAILS, LOCATION AND SPACING OF THE HOLD-DOWNS SHALL BE INDICATED.

THE TRANSFER OF LOAD FROM THE ANCHORAGES TO THE CORED SLAB UNIT SHALL BE DONE WHEN THE CONCRETE HAS REACHED A COMPRESSIVE STRENGTH OF NOT LESS THAN THE REQUIRED STRENGTH SHOWN IN THE "CONCRETE RELEASE STRENGTH" TABLE.

ALL REINFORCING STEEL IN VERTICAL CONCRETE BARRIER RAILS SHALL BE EPOXY COATED.

PRESTRESSING STRANDS SHALL BE CUT FLUSH WITH THE CORED SLAB UNIT ENDS.

APPLY EPOXY PROTECTIVE COATING TO CORED SLAB UNIT ENDS.

GROOVED CONTRACTION JOINTS, $\frac{1}{2}$ " IN DEPTH, SHALL BE TOOLED IN ALL EXPOSED FACES OF THE BARRIER RAIL AND IN ACCORDANCE WITH ARTICLE 825-10(B) OF THE STANDARD SPECIFICATIONS. A CONTRACTION JOINT SHALL BE LOCATED AT EACH THIRD POINT BETWEEN BARRIER RAIL EXPANSION JOINTS. ONLY ONE CONTRACTION JOINT IS REQUIRED AT MIDPOINT OF BARRIER RAIL SEGMENTS LESS THAN 20 FEET IN LENGTH AND NO CONTRACTION JOINTS ARE REQUIRED FOR THOSE SEGMENTS LESS THAN 10 FEET IN LENGTH.

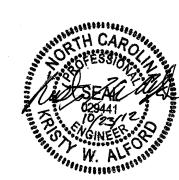
TRANSVERSE POST TENSIONING OF THE CORED SLAB UNITS SHALL BE DONE IN ACCORDANCE WITH THE STANDARD SPECIFICATIONS.

MAINTAIN A SYMMETRIC TENSION FORCE BETWEEN EACH PAIR OF TRANSVERSE POST TENSIONING STRANDS IN THE DIAPHRAGM.

THE #4 S11 STIRRUPS MAY BE SHIFTED AS NECESSARY TO MAINTAIN 1" CLEAR TO THE GROUTED RECESS.

FOR GROUT FOR STRUCTURES, SEE SPECIAL PROVISIONS.

CONCRETE	RELEASE	STRENGTH
UNIT		PSI
70'UNITS		5500



B-4832 PROJECT NO. ___ WAKE COUNTY

STATION: 16+19.29 -L-

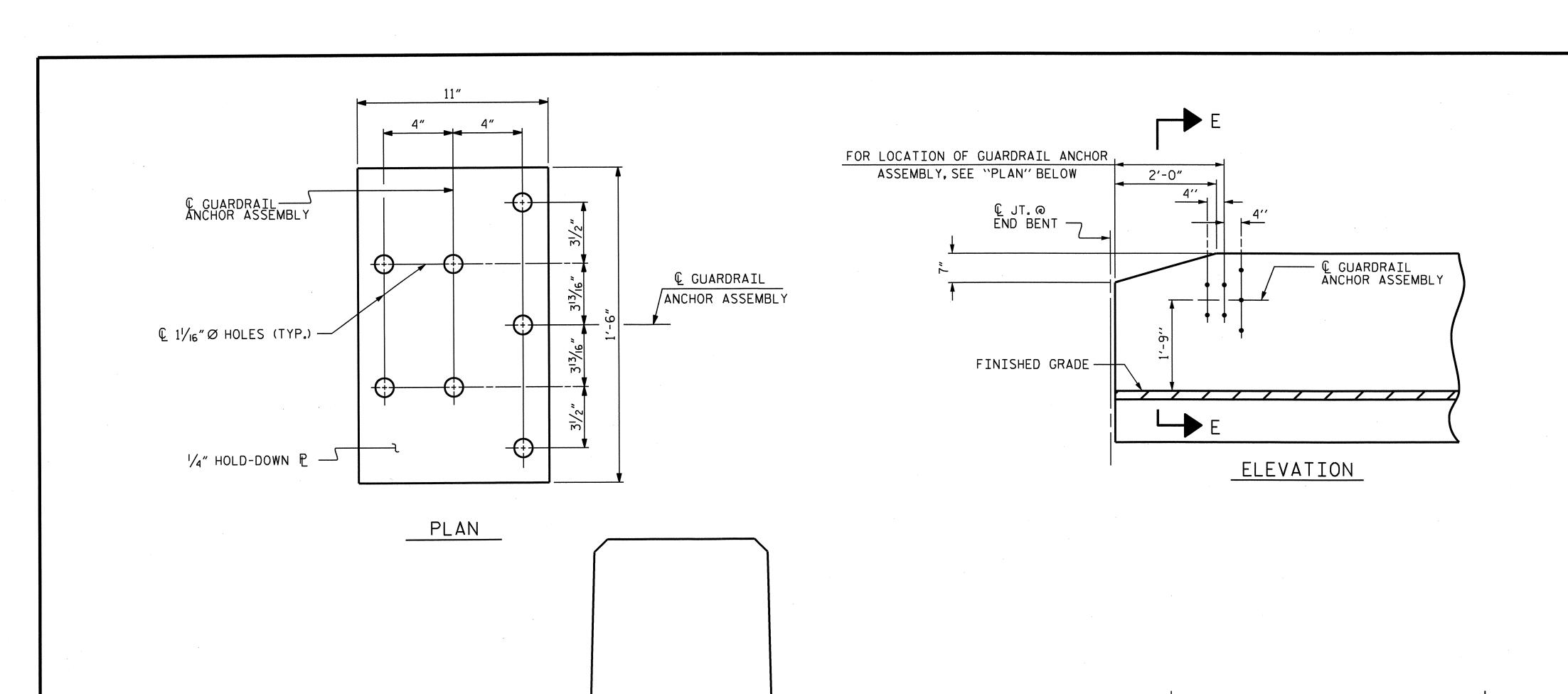
SHEET 3 OF 3

STATE OF NORTH CAROLINA DEPARTMENT OF TRANSPORTATION RALEIGH STANDARD

3'-0" X 2-0"
PRESTRESSED CONCRETE
CORED SLAB UNIT
90° SKEW

).	SHEET NO.	REVISIONS						
	S-6	DATE:	BY:	NO.	DATE:	BY:	NO.	
	TOTAL SHEETS			3			1	
	13			4		***************************************	2	

STD. NO. 24PCS3_33_90S



© 1/8"∅ X 1'-2"BOLT WITH ROUND

- ⊈ GUARDRAIL

WASHERS (TYP.)

ĀNCHOR ASSEMBLY

— 11/4" Ø HOLE (TYP.)

NOTES

THE GUARDRAIL ANCHOR ASSEMBLY SHALL CONSIST OF A $\frac{1}{4}$ " HOLD DOWN PLATE AND 7 - $\frac{7}{8}$ " Ø BOLTS WITH NUTS AND WASHERS.

THE HOLD-DOWN PLATE SHALL CONFORM TO AASHTO M270 GRADE 36. AFTER FABRICATION, THE HOLD-DOWN PLATE SHALL BE HOT-DIP GALVANIZED IN ACCORDANCE WITH AASHTO M111.

BOLTS SHALL CONFORM TO THE REQUIREMENTS OF ASTM A307 AND NUTS SHALL CONFORM TO THE REQUIREMENTS OF AASHTO M291. BOLTS, NUTS AND WASHERS SHALL BE GALVANIZED. (AT THE CONTRACTOR'S OPTION, STAINLESS STEEL BOLTS, NUTS AND WASHERS MAY BE USED AS AN ALTERNATE FOR THE 1/8" Ø GALVANIZED BOLTS, NUTS AND WASHERS. THEY SHALL CONFORM TO OR EXCEED THE MECHANICAL REQUIREMENTS OF ASTM A307. THE USE OF THIS ALTERNATE SHALL BE APPROVED BY THE ENGINEER.)

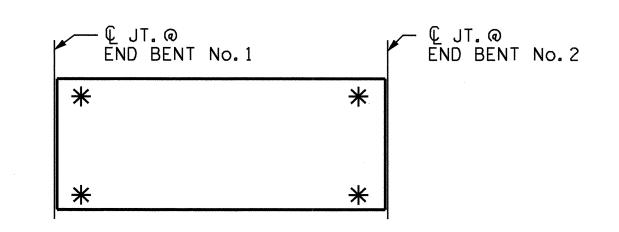
THE GUARDRAIL ANCHOR ASSEMBLY IS REQUIRED AT ALL POINTS WHERE APPROACH GUARDRAIL IS TO BE ATTACHED TO THE END OF BARRIER RAIL.FOR POINTS OF ATTACHMENT, SEE SKETCH.

AFTER INSTALLATION, THE EXPOSED THREAD OF THE BOLT SHALL BE BURRED WITH A SHARP POINTED TOOL.

THE COST OF THE GUARDRAIL ANCHOR ASSEMBLY SHALL BE INCLUDED IN THE UNIT CONTRACT PRICE BID FOR VERTICAL CONCRETE BARRIER RAIL.

THE VERTICAL REINFORCING BARS MAY BE SHIFTED SLIGHTLY IN THE VERTICAL CONCRETE BARRIER RAIL TO CLEAR ASSEMBLY BOLTS.

THE 1 $\frac{1}{4}$ " Ø HOLES SHALL BE FORMED OR DRILLED WITH A CORE BIT. IMPACT TOOLS WILL NOT BE PERMITTED. ANY CONCRETE DAMAGED BY THIS WORK SHALL BE REPAIRED TO THE SATISFACTION OF THE ENGINEER.



SKETCH SHOWING POINTS OF ATTACHMENT

* DENOTES GUARDRAIL ANCHOR ASSEMBLY

4" GUARDRAIL
ANCHOR ASSEMBLY 1'-10" © JT. @ — END BENT Z GUARDRAIL
ANCHOR ASSEMBLY PLAN

LOCATION OF ANCHORS FOR GUARDRAIL

END BENT No.1 SHOWN, END BENT No.2 SIMILAR.

B-4832 PROJECT NO._ WAKE COUNTY STATION: 16+19.29 -L-

> STATE OF NORTH CAROLINA DEPARTMENT OF TRANSPORTATION
> RALEIGH

STANDARD

GUARDRAIL ANCHORAGE FOR VERTICAL CONCRETE BARRIER RAIL

SHEET NO. REVISIONS S-7 DATE: BY:

ASSEMBLED BY: A.V.ROYAL DATE: 03/11 CHECKED BY: T.M.GARRISON DATE: 03/11

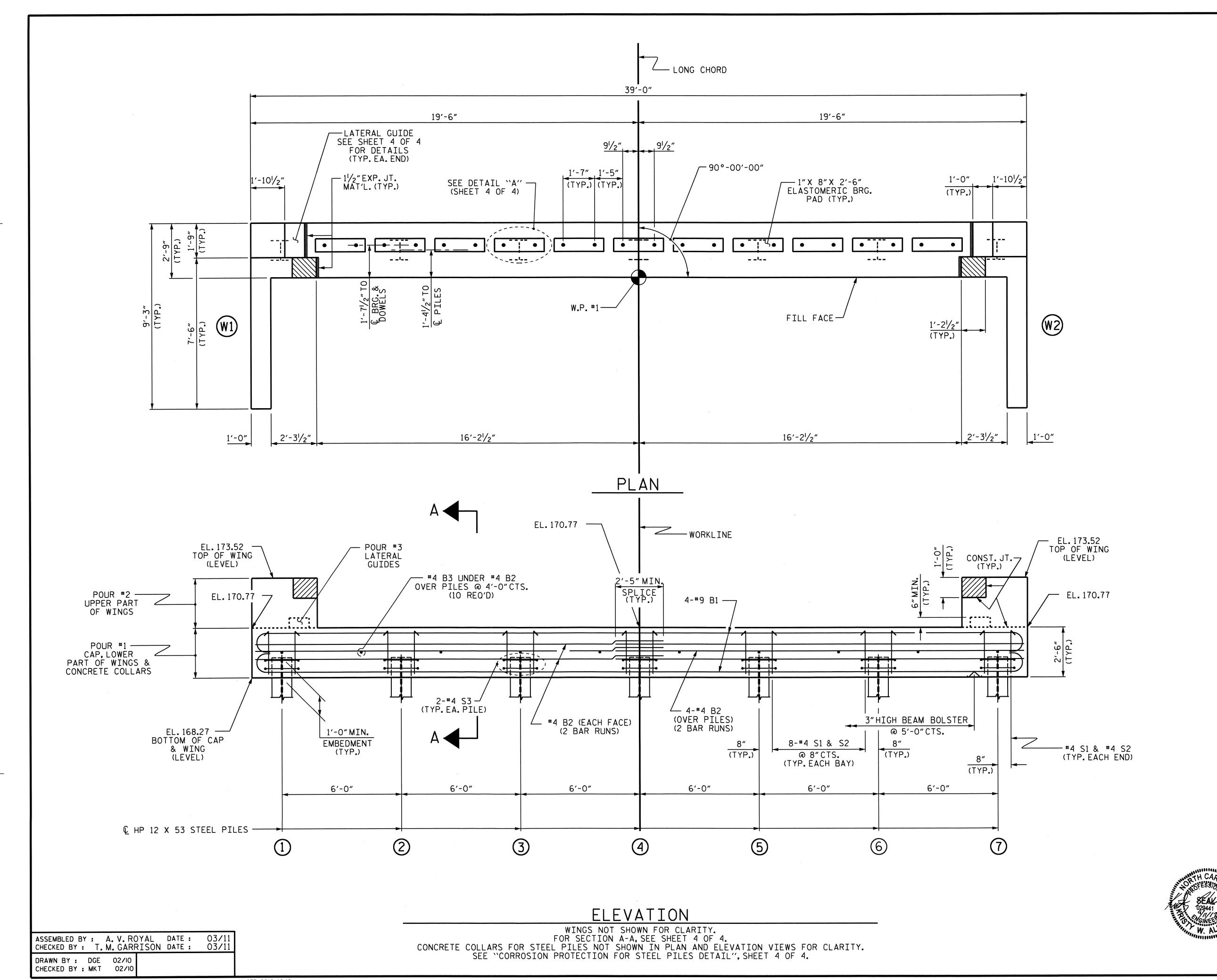
DRAWN BY : MAA 5/10 CHECKED BY : GM 5/10

23-OCT-2012 10:52
T:\Structures\Plans\Drafting\Superstructure\B-4832_SD_CS.dgn

SECTION E-E

GUARDRAIL ANCHOR ASSEMBLY DETAILS

1/4" HOLD-DOWN P -



NOTES

STIRRUPS IN CAP MAY BE SHIFTED AS NECESSARY TO CLEAR DOWELS.

THE LATERAL GUIDES ARE NOT TO BE POURED UNTIL AFTER THE CORED SLAB UNITS ARE IN PLACE.

THE CONCRETE IN THE SHADED AREA OF THE WING SHALL BE POURED AFTER THE VERTICAL CONCRETE BARRIER RAIL IS CAST IF SLIP FORMING IS USED.

FOR PILE SPLICE DETAILS, SEE SHEET 4 OF 4.

FOR WING DETAILS, SEE SHEET 3 OF 4.

THE CONTRACTOR HAS THE OPTION TO OMIT THE LATERAL GUIDE IF APPROVED BY THE ENGINEER.

PROJECT NO. B-4832

WAKE COUNTY

STATION: 16+19.29 -L-

SHEET 1 OF 4

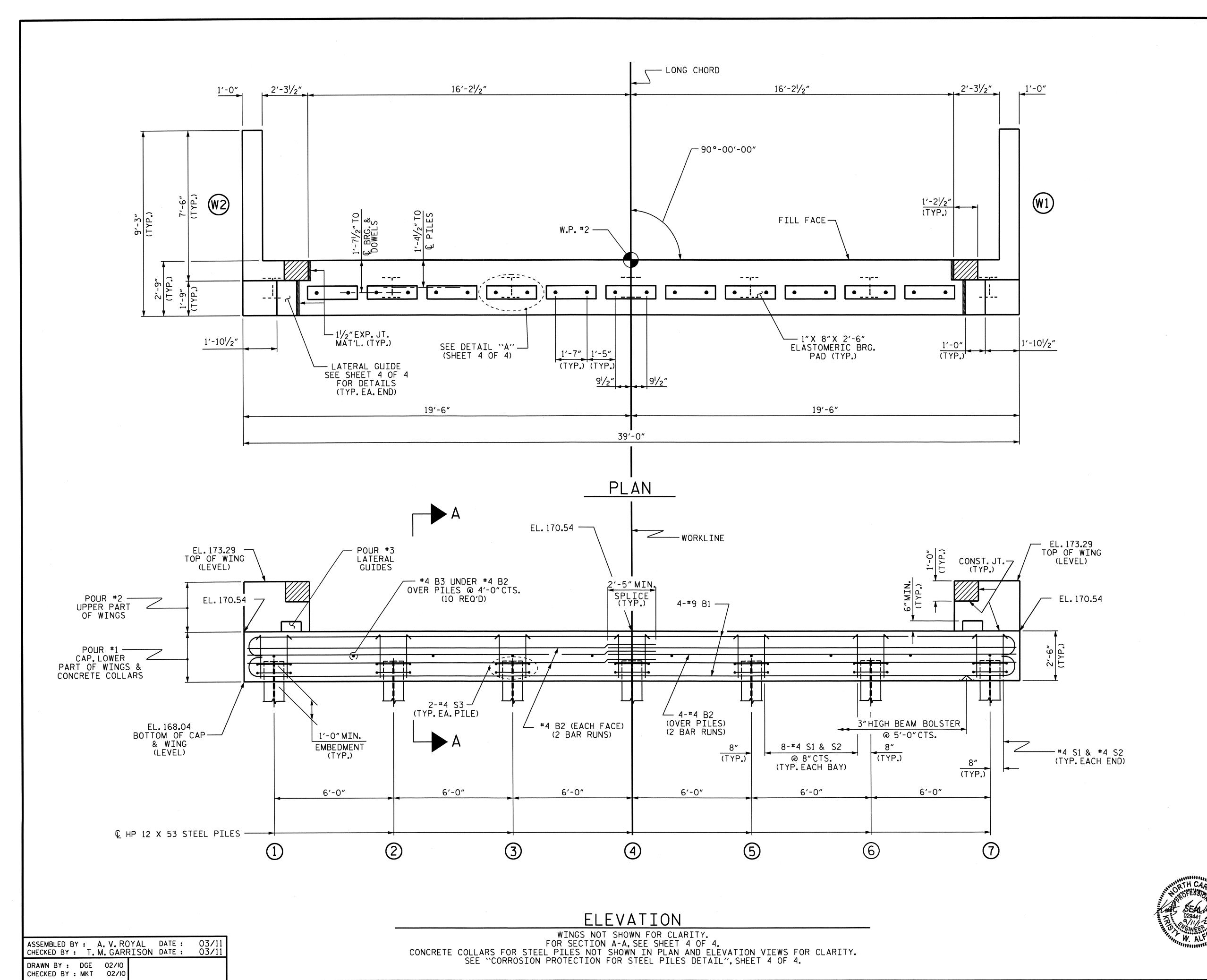
STATE OF NORTH CAROLINA

DEPARTMENT OF TRANSPORTATION

RALEIGH

SUBSTRUCTURE

END BENT No. 1



NOTES

STIRRUPS IN CAP MAY BE SHIFTED AS NECESSARY TO CLEAR DOWELS.

THE LATERAL GUIDES ARE NOT TO BE POURED UNTIL AFTER THE CORED SLAB UNITS ARE IN PLACE.

THE CONCRETE IN THE SHADED AREA OF THE WING SHALL BE POURED AFTER THE VERTICAL CONCRETE BARRIER RAIL IS CAST IF SLIP FORMING IS USED.

FOR PILE SPLICE DETAILS, SEE SHEET 4 OF 4.

FOR WING DETAILS, SEE SHEET 3 OF 4.

THE CONTRACTOR HAS THE OPTION TO OMIT THE LATERAL GUIDE IF APPROVED BY THE ENGINEER.

PROJECT NO. ______B-4832 _______WAKE _____ COUNTY STATION: _____16+19.29 -L-

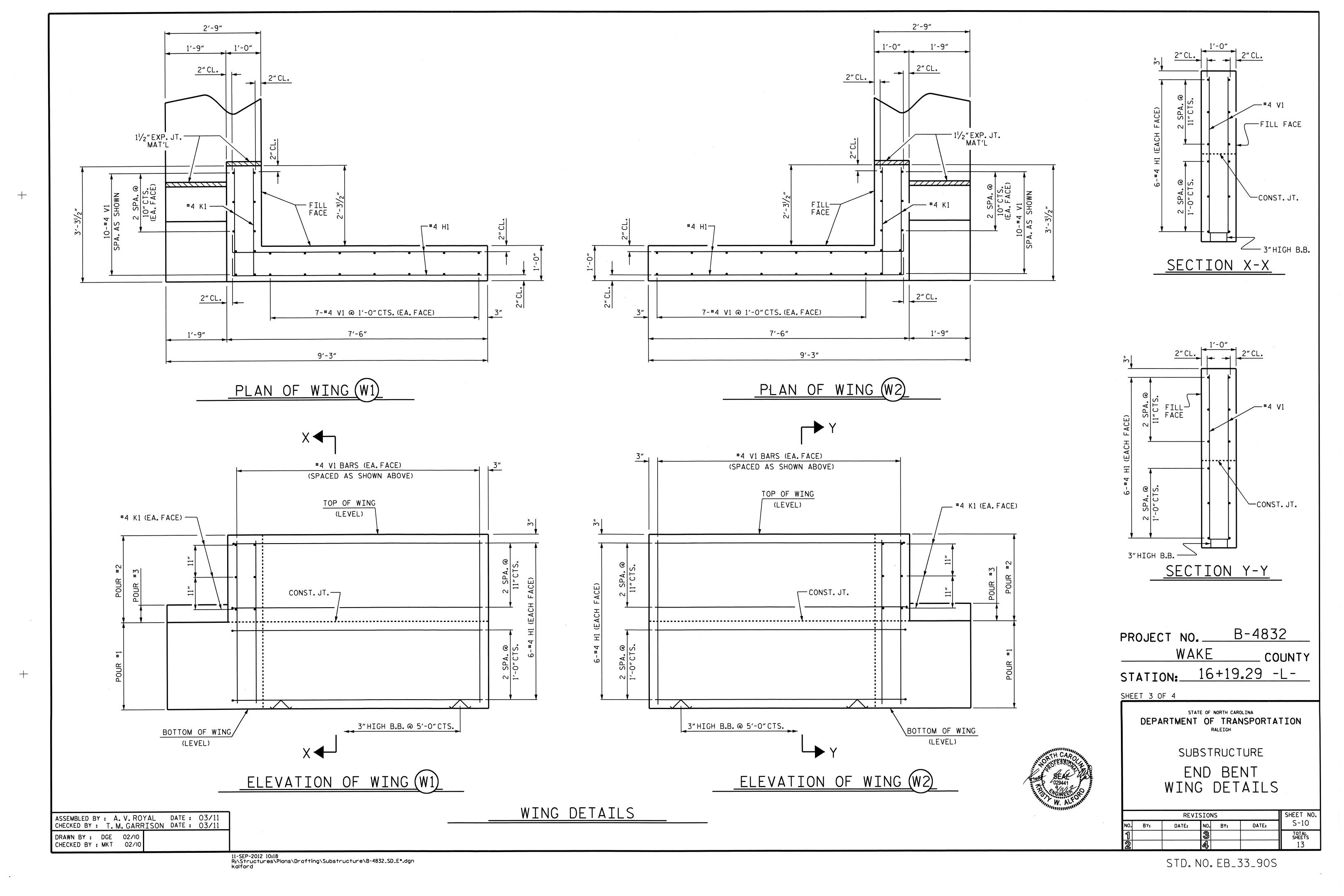
SHEET 2 OF 4

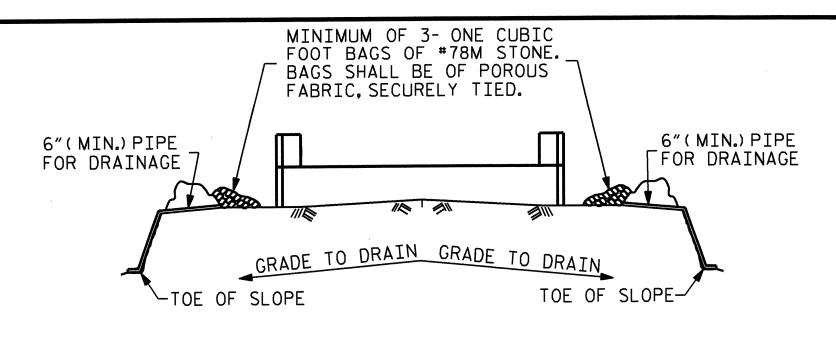
DEPARTMENT OF TRANSPORTATION
RALEIGH

SUBSTRUCTURE

END BENT No. 2

		SHEET NO.				
NO.	BY:	DATE:	NO.	BY:	DATE:	S-9
1			3			TOTAL SHEETS
2	•	,	4			13





BAGGED STONE AND PIPE SHALL BE PLACED IMMEDIATELY AFTER COMPLETION OF END BENT EXCAVATION. PIPE MAY BE EITHER CONCRETE, CORRUGATED STEEL, CORRUGATED ALUMINUM ALLOY, OR CORRUGATED PLASTIC. PERFORATED PIPE WILL NOT BE ALLOWED.

BAGGED STONE SHALL REMAIN IN PLACE UNTIL THE ENGINEER DIRECTS THAT IT BE REMOVED. THE CONTRACTOR SHALL REMOVE AND DISPOSE OF SILT ACCUMULATIONS AT BAGGED STONE WHEN SO DIRECTED BY THE ENGINEER. BAGS SHALL BE REMOVED AND REPLACED WHENEVER THE ENGINEER DETER-MINES THAT THEY HAVE DETERIORATED AND LOST THEIR EFFECTIVENESS.

NO SEPARATE PAYMENT WILL BE MADE FOR THIS WORK AND THE ENTIRE COST OF THIS WORK SHALL BE INCLUDED IN THE UNIT CONTRACT PRICE BID FOR THE SEVERAL PAY ITEMS.

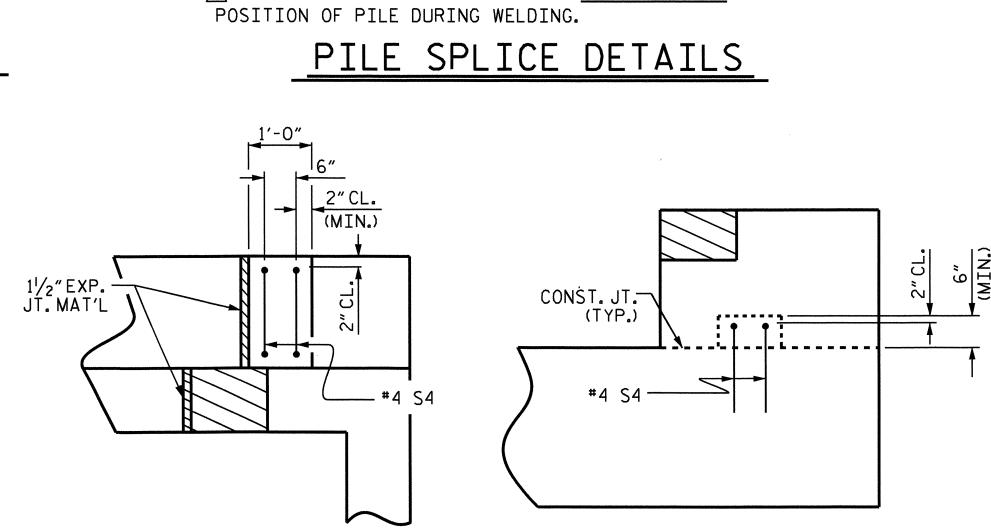
TEMPORARY DRAINAGE AT END BENT

— ℚ CORED SLAB UNIT

#6 D1 DOWELS TO PROJECT

9" ABOVE CAP

(TYP.)



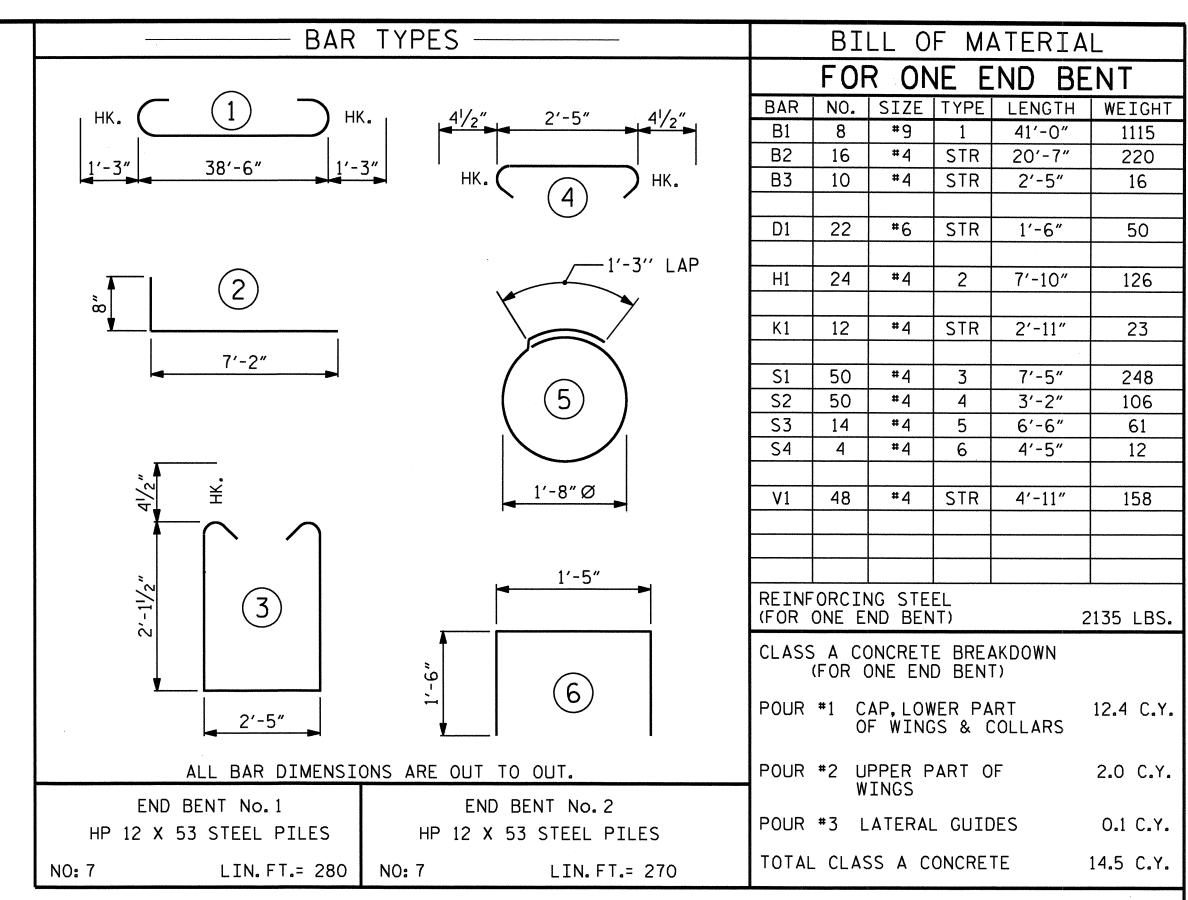
PILE VERTICAL

DETAIL A

PLAN ELEVATION

LATERAL GUIDE DETAILS

(RIGHT LATERAL GUIDE SHOWN, LEFT END SIMILAR)



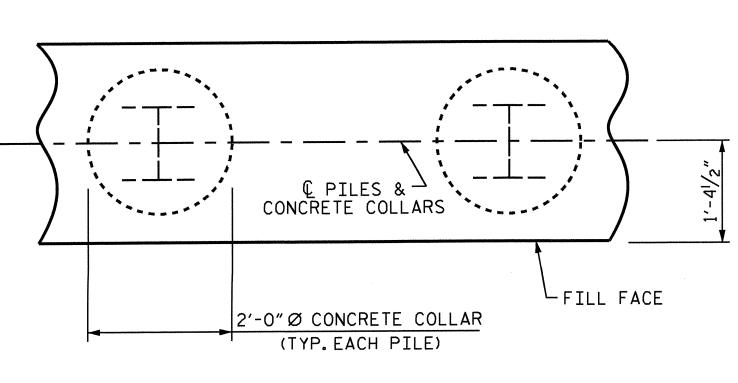
91/2" 91/2" 1" X 8" X 2'-6" — ELASTOMERIC BRG. PAD (TYP.) 1'-7" - FILL FACE DETAIL "A" (END BENT No. 1 SHOWN, END BENT No. 2 SIMILAR BY ROTATION)

2'-6"

1'-3"

€ BEARING —

DRAWN BY : DGE 02/10 CHECKED BY : MKT 02/10



2'-0" PLAN CORROSION PROTECTION FOR STEEL PILES DETAIL (END BENT No.1 SHOWN, END BENT No.2 SIMILAR BY ROTATION) ASSEMBLED BY: A.V.ROYAL DATE: 03/11 CHECKED BY: T.M.GARRISON DATE: 03/11

CONCRETE TO COLLAR BOTTOM OF CAP © HP 12 X 53 TEEL PILE ELEVATION

-€ #6 D1 DOWEL FILL FACE г#4 S2 ъ 4-#9 B1 #4 B2 (EA.FACE) #4 S1 ____ #4 B2 (EA.FACE) 2-#9 B1 2" CL. (TYP.) 2-#9 B1 --- 3" HIGH B.B. © HP 12 X 53 -STEEL PILE 1'-41/2" 1'-41/2" 2'-9"

✓ BACK GOUGE

✓ DETAIL B

PILE HORIZONTAL

OR VERTICAL

DETAIL B

0" TO 1/8"

SECTION A-A (CONCRETE COLLAR NOT SHOWN FOR CLARITY. SEE "CORROSION PROTECTION FOR STEEL PILES DETAIL.")

B-4832 PROJECT NO. WAKE COUNTY 16+19.29 -L-STATION:_

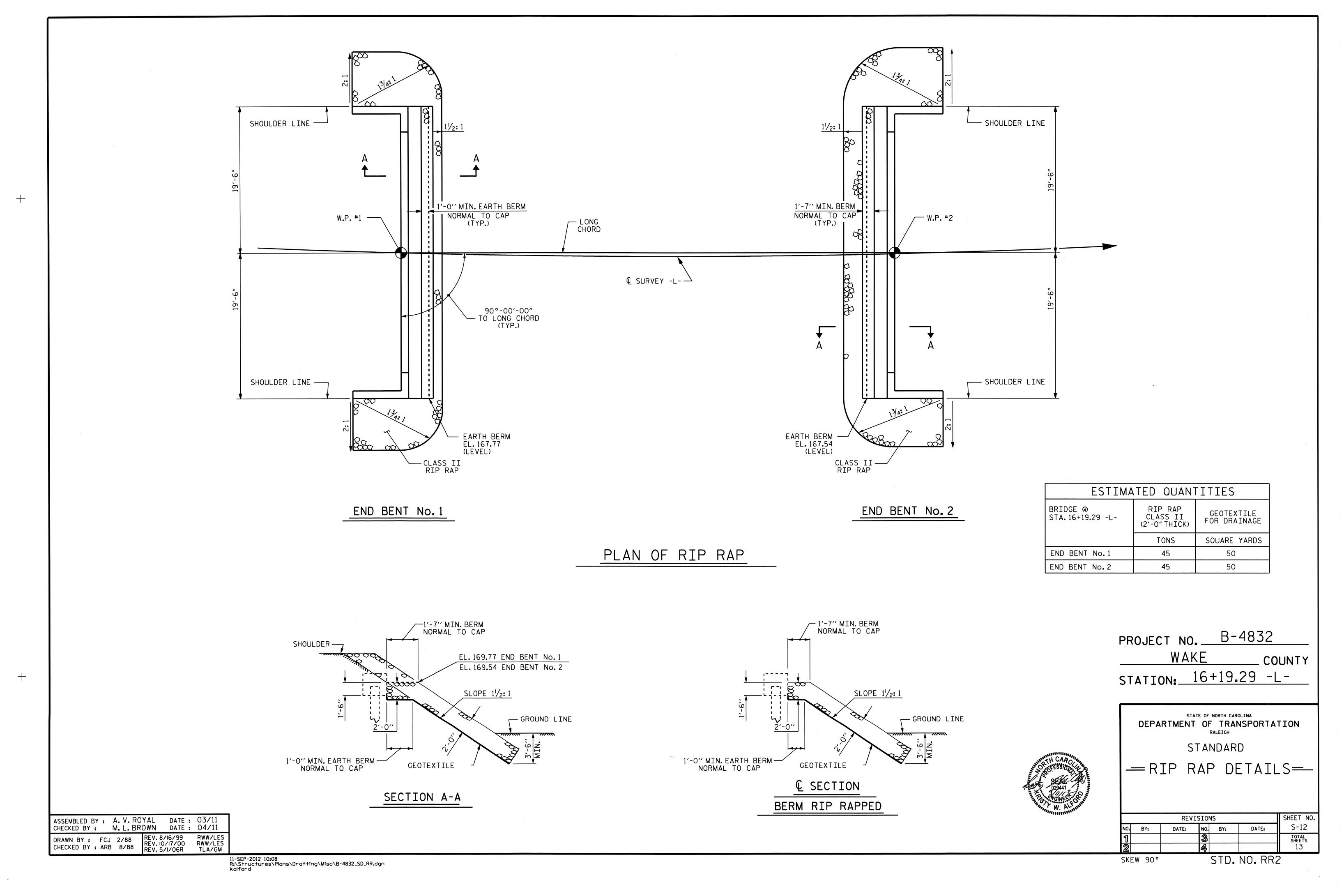
SHEET 4 OF 4

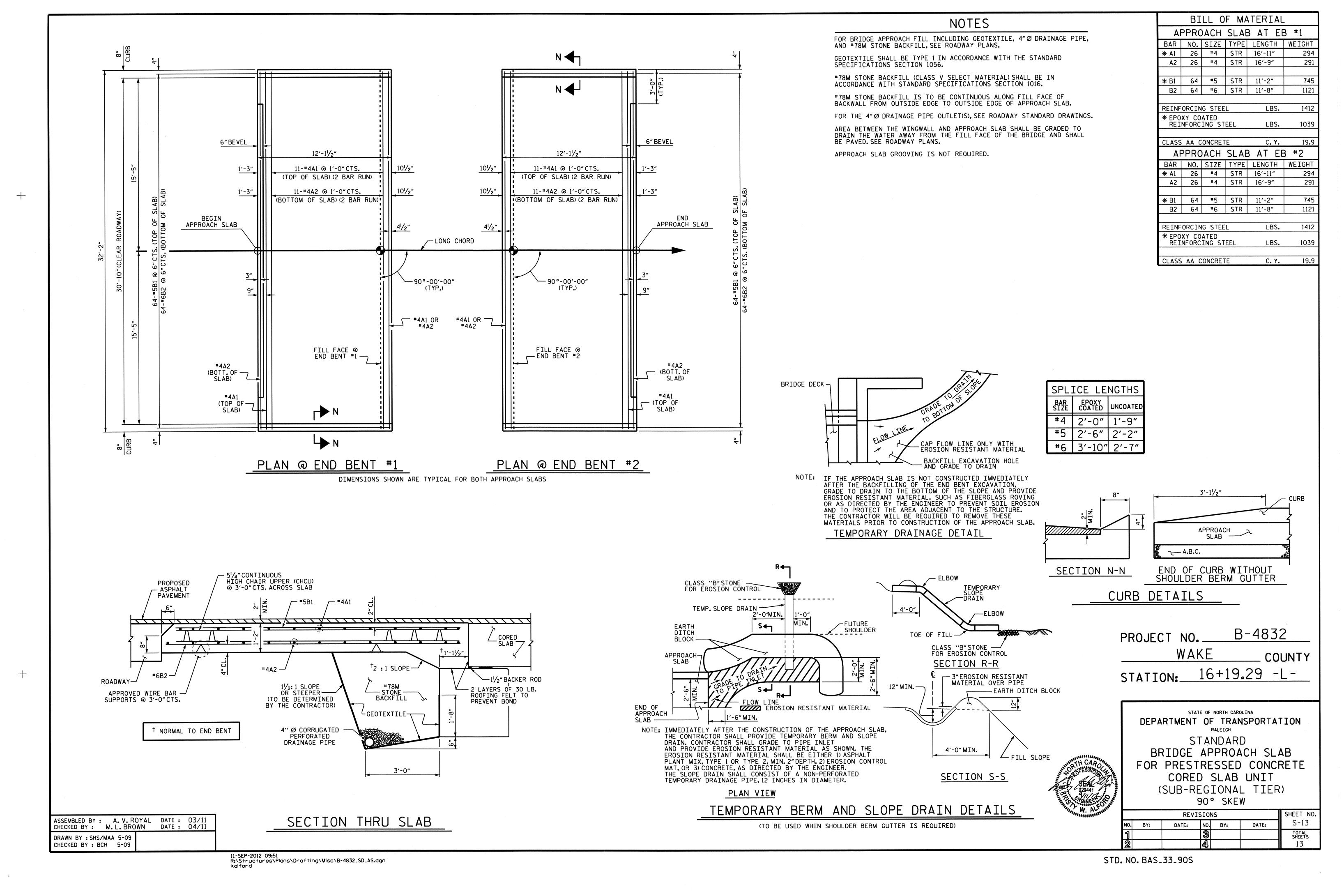
STATE OF NORTH CAROLINA DEPARTMENT OF TRANSPORTATION

SUBSTRUCTURE

END BENT No.1 & 2 DETAILS

		SHEET NO.				
).	BY:	DATE:	NO.	BY:	DATE:	S-11
			3			TOTAL SHEETS
2			4			13





STANDARD NOTES

DESIGN DATA:

STRUCTURAL TIMBER - TREATED OR

CONCRETE IN SHEAR

UNTREATED - EXTREME FIBER STRESS - - - - - 1,800 LBS. PER SQ. IN.

---- SEE A.A.S.H.T.O.

COMPRESSION PERPENDICULAR TO GRAIN

OF TIMBER ---- 375 LBS.PER SQ.IN.

EQUIVALENT FLUID PRESSURE OF EARTH

30 LBS. PER CU. FT.

IVALENT FLUID PRESSURE OF EARTH -----

(MINIMUM)

MATERIAL AND WORKMANSHIP:

EXCEPT AS MAY OTHERWISE BE SPECIFIED ON PLANS OR IN THE SPECIAL PROVISIONS, ALL MATERIAL AND WORKMANSHIP SHALL BE IN ACCORDANCE WITH THE 2012 "STANDARD SPECIFICATIONS FOR ROADS AND STRUCTURES" OF THE N. C. DEPARTMENT OF TRANSPORTATION.

STEEL SHEET PILING FOR PERMANENT OR TEMPORARY APPLICATIONS SHALL BE HOT ROLLED.

CONCRETE:

UNLESS OTHERWISE REQUIRED ON PLANS, CLASS A CONCRETE SHALL BE USED FOR ALL PORTIONS OF ALL STRUCTURES WITH THE EXCEPTION THAT: CLASS AA CONCRETE SHALL BE USED IN BRIDGE SUPERSTRUCTURES, ABUTMENT BACKWALLS, AND APPROACH SLABS; AND CLASS B CONCRETE SHALL BE USED FOR SLOPE PROTECTION AND RIP RAP.

CONCRETE CHAMFERS:

UNLESS OTHERWISE NOTED ON THE PLANS, ALL EXPOSED CORNERS ON STRUCTURES SHALL BE CHAMFERED 3/4" WITH THE FOLLOWING EXCEPTIONS: TOP CORNERS OF CURBS MAY BE ROUNDED TO 1-1/2" RADIUS WHICH IS BUILT INTO CURB FORMS; CORNERS OF TRANSVERSE FLOOR EXPANSION JOINTS SHALL BE ROUNDED WITH A 1/4" FINISHING TOOL UNLESS OTHERWISE REQUIRED ON PLANS; AND CORNERS OF EXPANSION JOINTS IN THE ROADWAY FACES AND TOPS OF CURBS AND SIDEWALKS SHALL BE ROUNDED TO A 1/4" RADIUS WITH A FINISHING STONE OR TOOL UNLESS OTHERWISE REQUIRED ON PLANS.

DOWELS:

DOWELS WHEN INDICATED ON PLANS AS FOR CULVERT EXTENSIONS, SHALL BE EMBEDDED AT LEAST 12" INTO THE OLD CONCRETE AND GROUTED INTO PLACE WITH 1:2 CEMENT MORTAR.

ALLOWANCE FOR DEAD LOAD DEFLECTION, SETTLEMENT, ETC. IN CASTING SUPERSTRUCTURES:

BRIDGES SHALL BE BUILT ON THE GRADE OR VERTICAL CURVE SHOWN ON PLANS. SLABS, CURBS AND PARAPETS SHALL CONFORM TO THE GRADE OR CURVE.

ALL DIMENSIONS WHICH ARE GIVEN IN SECTION AND ARE AFFECTED BY DEAD LOAD DEFLECTIONS ARE DIMENSIONS AT CENTER LINE OF BEARING UNLESS OTHERWISE NOTED ON PLANS. IN SETTING FORMS FOR STEEL BEAM BRIDGES AND PRESTRESSED CONCRETE GIRDER BRIDGES, ADJUSTMENTS SHALL BE MADE DUE TO THE DEAD LOAD DEFLECTIONS FOR THE ELEVATIONS SHOWN. WHERE BLOCKS ARE SHOWN OVER BEAMS FOR BUILDING UP TO THE SLAB, THE VERTICAL DIMENSIONS OF THE BLOCKS SHALL BE ADJUSTED BETWEEN BEARINGS TO COMPENSATE FOR DEAD LOAD DEFLECTIONS, VERTICAL CURVE ORDINATE, AND ACTUAL BEAM CAMBER. WHERE BOTTOM OF SLAB IS IN LINE WITH BOTTOM OF TOP FLANGES, DEPTH OF SLAB BETWEEN BEARINGS SHALL BE ADJUSTED TO COMPENSATE FOR DEAD LOAD DEFLECTION, VERTICAL CURVE ORDINATE, AND ACTUAL BEAM CAMBER.

IN SETTING FALSEWORK AND FORMS FOR REINFORCED CONCRETE SPANS, AN ALLOWANCE SHALL BE MADE FOR DEAD LOAD DEFLECTIONS, SETTLEMENT OF FALSEWORK, AND PERMANENT CAMBER WHICH SHALL BE PROVIDED FOR IN ADDITION TO THE ELEVATIONS SHOWN, AFTER REMOVAL OF THE FALSEWORK, THE FINISHED STRUCTURES SHALL CONFORM TO THE PROFILE AND ELEVATIONS SHOWN ON THE PLANS AND

CONSTRUCTION ELEVATIONS FURNISHED BY THE ENGINEER.

DETAILED DRAWINGS FOR FALSEWORK OR FORMS FOR BRIDGE SUPERSTRUCTURE
AND ANY STRUCTURE OR PARTS OF A STRUCTURE AS NOTED ON THE PLANS SHALL
BE SUBMITTED TO THE ENGINEER FOR APPROVAL BEFORE CONSTRUCTION OF THE
FALSEWORK OR FORMS IS STARTED.

REINFORCING STEEL:

ALL REINFORCING STEEL SHALL BE DEFORMED. DIMENSIONS RELATIVE TO PLACEMENT OF REINFORCING ARE TO CENTERS OF BARS UNLESS OTHERWISE INDICATED IN THE PLANS. DIMENSIONS ON BAR DETAILS ARE TO CENTERS OF BARS OR ARE OUT TO OUT AS INDICATED ON PLANS.

WIRE BAR SUPPORTS SHALL BE PROVIDED FOR REINFORCING STEEL WHERE INDICATED ON THE PLANS. WHEN BAR SUPPORT PIECES ARE PLACED IN CONTINUOUS LINES. THEY SHALL BE SO PLACED THAT THE ENDS OF THE SUPPORTING WIRES SHALL

STRUCTURAL STEEL:

BE LAPPED TO LOCK LEGS ON ADJOINING PIECES.

AT THE CONTRACTOR'S OPTION, HE MAY SUBSTITUTE 7/8"Ø SHEAR STUDS FOR THE 3/4"Ø STUDS SPECIFIED ON THE PLANS. THIS SUBSTITUTION SHALL BE MADE AT THE RATE OF 3 - 7/8"Ø STUDS FOR 4 - 3/4"Ø STUDS, AND STUD SPACING CHANGES SHALL BE MADE AS NECESSARY TO PROVIDE THE SAME EQUIVALENT NUMBER OF 7/8"Ø STUDS ALONG THE BEAM AS SHOWN FOR 3/4"Ø STUDS BASED ON THE RATIO OF 3 - 7/8"Ø STUDS FOR 4 - 3/4"Ø STUDS. STUDS OF THE LENGTH SPECIFIED ON THE PLANS MUST BE PROVIDED. THE MAXIMUM SPACING SHALL BE 2'-0".

EXCEPT AT THE INTERIOR SUPPORTS OF CONTINUOUS BEAMS WHERE THE COVER PLATE IS IN CONTACT WITH BEARING PLATE, THE CONTRACTOR MAY, AT HIS OPTION, SUBSTITUTE FOR THE COVER PLATES DESIGNATED ON THE PLANS COVER PLATES OF THE EQUIVALENT AREA PROVIDED THESE PLATES ARE AT LEAST 5/16" IN THICKNESS AND DO NOT EXCEED A WIDTH EQUAL TO THE FLANGE WIDTH LESS 2"OR A THICKNESS EQUAL TO 2 TIMES THE FLANGE THICKNESS. THE SIZE OF FILLET WELDS SHALL CONFORM TO THE REQUIREMENTS OF THE CURRENT ANSI/AASHTO/AWS "BRIDGE WELDING CODE". ELECTROSLAG WELDING WILL NOT BE PERMITTED.

WITH THE SOLE EXCEPTION OF EDGES AT SURFACES WHICH BEAR ON OTHER SURFACES, ALL SHARP EDGES AND ENDS OF SHAPES AND PLATES SHALL BE SLIGHTLY ROUNDED BY SUITABLE MEANS TO A RADIUS OF APPROXIMATELY 1/16 INCH OR EQUIVALENT FLAT SURFACE AT A SUITABLE ANGLE PRIOR TO PAINTING, GALVANIZING, OR METALLIZING.

HANDRAILS AND POSTS:

METAL STANDARDS AND FACES OF THE CONCRETE END POSTS FOR THE METAL
RAIL SHALL BE SET NORMAL TO THE GRADE OF THE CURB, UNLESS OTHERWISE SHOWN
ON PLANS. THE METAL RAIL AND TOPS OF CONCRETE POSTS USED WITH THE ALUMINUM
RAIL SHALL BE BUILT PARALLEL TO THE GRADE OF THE CURB.

METAL HANDRAILS SHALL BE IN ACCORDANCE WITH THE PLANS. RAILS SHALL BE
AS MANUFACTURED FOR BRIDGE RAILING. CASTINGS SHALL BE OF A UNIFORM APPEARANCE.
FINS AND OTHER DEFORMATIONS RESULTING FROM CASTING OR OTHERWISE SHALL BE
REMOVED IN A MANNER SO THAT A UNIFORM COLORING OF THE COMPLETED CASTING SHALL
BE OBTAINED. CASTINGS WITH DISCOLORATIONS OR OF NON-UNIFORM COLORING WILL
NOT BE ACCEPTED. CERTIFIED MILL REPORTS ARE REQUIRED FOR METAL RAILS AND POSTS.

SPECIAL NOTES:

GENERALLY, IN CASE OF DISCREPANCY, THIS STANDARD SHEET OF NOTES SHALL GOVERN OVER THE SPECIFICATIONS, BUT THE REMAINDER OF THE PLANS SHALL GOVERN OVER NOTES HEREON, AND SPECIAL PROVISIONS SHALL GOVERN OVER ALL. SEE SPECIFICATIONS ARTICLE 105-4.

ENGLISH