ID: B-4122

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SHEET

ROJECT: 33475.1.1

STATE OF NORTH CAROLINA

DEPARTMENT OF TRANSPORTATION
DIVISION OF HIGHWAYS
GEOTECHNICAL ENGINEERING UNIT

STRUCTURE SUBSURFACE INVESTIGATION

	FERENCE NO.	33475.1.1 (1	B-4122)	F.A.	PROJ.	BRZ-111	7(8)
	<u>GRAHAM</u>	DD1D0E	NY 01 0N				
PROJECT	DESCRIPTION	BRIDGE	No. 81 ON	SR-1117	OVER	LONG	<u>CREEK</u>
SITE DES	CRIPTION						-

STATE	STATE PROJECT REFERENCE NO.	SHEET NO.	TOTAL
N.C.	33475.1.1 (B-A122)	1	12

CAUTION NOTICE

THE SUBSURFACE INFORMATION AND THE SUBSURFACE INVESTIGATION ON WHICH IT IS BASED WERE MADE FOR THE PURPOSE OF STUDY, PLAINENC, AND DESIGN, AND NOT FOR CONSTRUCTION OR PAY PURPOSES. THE VARIABLE MICHORY FILED BORING LOGS, ROOK COPES, AND SOIL TEST DATA AVAILABLE MAY BE REVIEWED OR INSPECTED IN RALEIGH BY CONTACTING THE N. C. DEPARTMENT OF TRANSPORTATION, CEOTECHNICAL ENGINEERING LUNT AT (91) 20-4088, NEITHER THE SUBSURFACE FLANS AND REPORTS, NOR THE FIELD BORING LOGS, ROCK COPES, OR SOIL TEST DATA ARE PART OF THE CONTRACT.

GENERAL SOIL AND ROCK STRATA DESCRIPTIONS AND INDICATED BOUNDARIES ARE BASED ON A GEOTECHNICAL INTERPRETATION OF ALL AVAILABLE SUBSURFACE DATA AND MAY NOT NECESSARILY REFLECT THE ACTUAL SUBSURFACE CONDITIONS BETWEEN BORRINGS OR BETWEEN SAMPLED STRATA WITHIN THE BORRHOLE THE LABORATORY SAMPLE DATA AND THE IN STILL WIN-PLACED TEST DATA CAN BE RELIED ON ONLY TO THE DEGREE OF RELABBLITY WHERENT IN THE STANDARD TEST METHOD. THE OBSERVED WATER LEVELS OR SOIL MOSTUME CONDITIONS MODICATED WATER LEVELS OR SOIL MOSTUME CONDITIONS TOWN AVARY CONSIDERABLY WITH TIME ACCOSMON TO CLIMATIC CONDITIONS MICH.

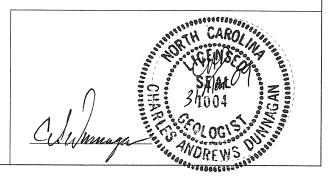
THE BUDDER OR CONTRACTOR IS CAUTIONED THAT DETAILS SHOWN ON THE SUBSUBFACE PLANS ARE PRELIMMARY ONLY AND IN MANY CASES THE FINAL DESIGN DETAILS ARE DIFFERENT, FOR BUDDING AND CONSTRUCTION PURPOSES, MEPER TO THE CONSTRUCTION PLANS AND DOCUMENTS FOR FINAL DESIGN INFORMATION ON THIS PROJECT, THE DEPAIRMENT DOES NOT WARRANT OR GUARANTEE THE SUFFICIENCY OR ACCURACY OF THE INVESTIGATION MADE, NOR THE INTERPRETATIONS MADE, OR OPINION OF THE DEPAIRMENT AS TO THE FYPE OF MATERIALS AND CONDITIONS TO BE ENCOUNTERED. THE BIDDER OR CONTINACTOR IS CAUTIONED TO MAKE SUCH INDEPENDENT SUBSURFACE INVESTIGATIONS AS HE DEEMS NECESSARY TO SATISTY HUNSLE AS TO CONDITIONS TO BE ENCOUNTERED ON THIS PROJECT, THE CONTRACTOR SHALL HAVE NO CLAM FOR ADDITIONAL COMPENSATION OR FOR AN EXTENSION OF TIME FOR ANY PRESON RESULTING FROM THE ACTUAL CONDITIONS FOR ENCOUNTERED AT THE SITE DIFFERING FROM THOSE INDICATED IN THE SUBSURFACE INFORMATION.

	M M HAGER
	L E LANGFORD
	R D CHILDERS
	_
IVESTIGATED 1	BY C A DUNNAGAN
HECKED BY	W D FRYE, Jr

SUBMITTED BY W D FRYE, Jr

MARCH 2009

PERSONNEL



NORTH CAROLINA DEPARTMENT OF TRANSPORTATION

DIVISION OF HIGHWAYS

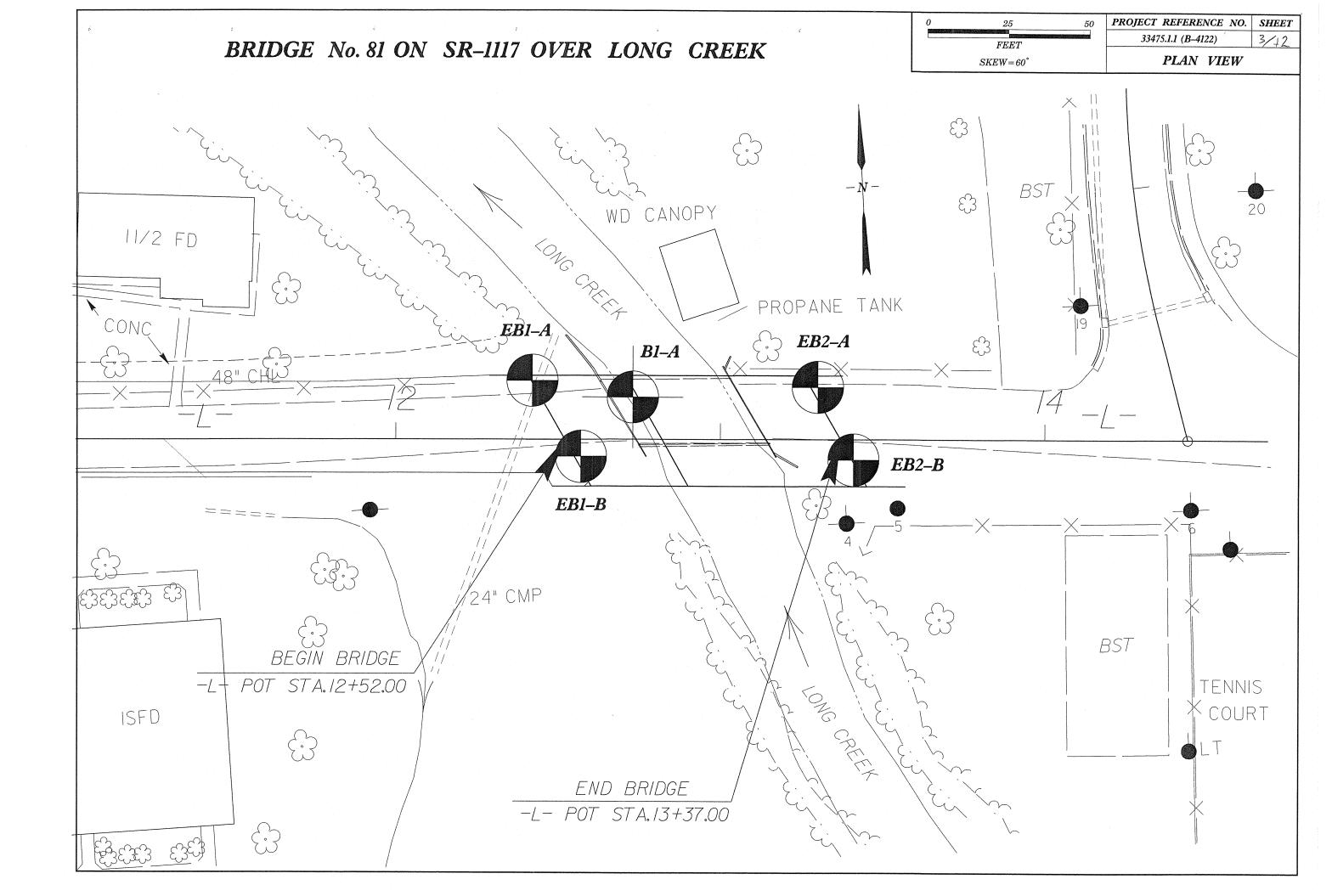
GEOTECHNICAL ENGINEERING UNIT

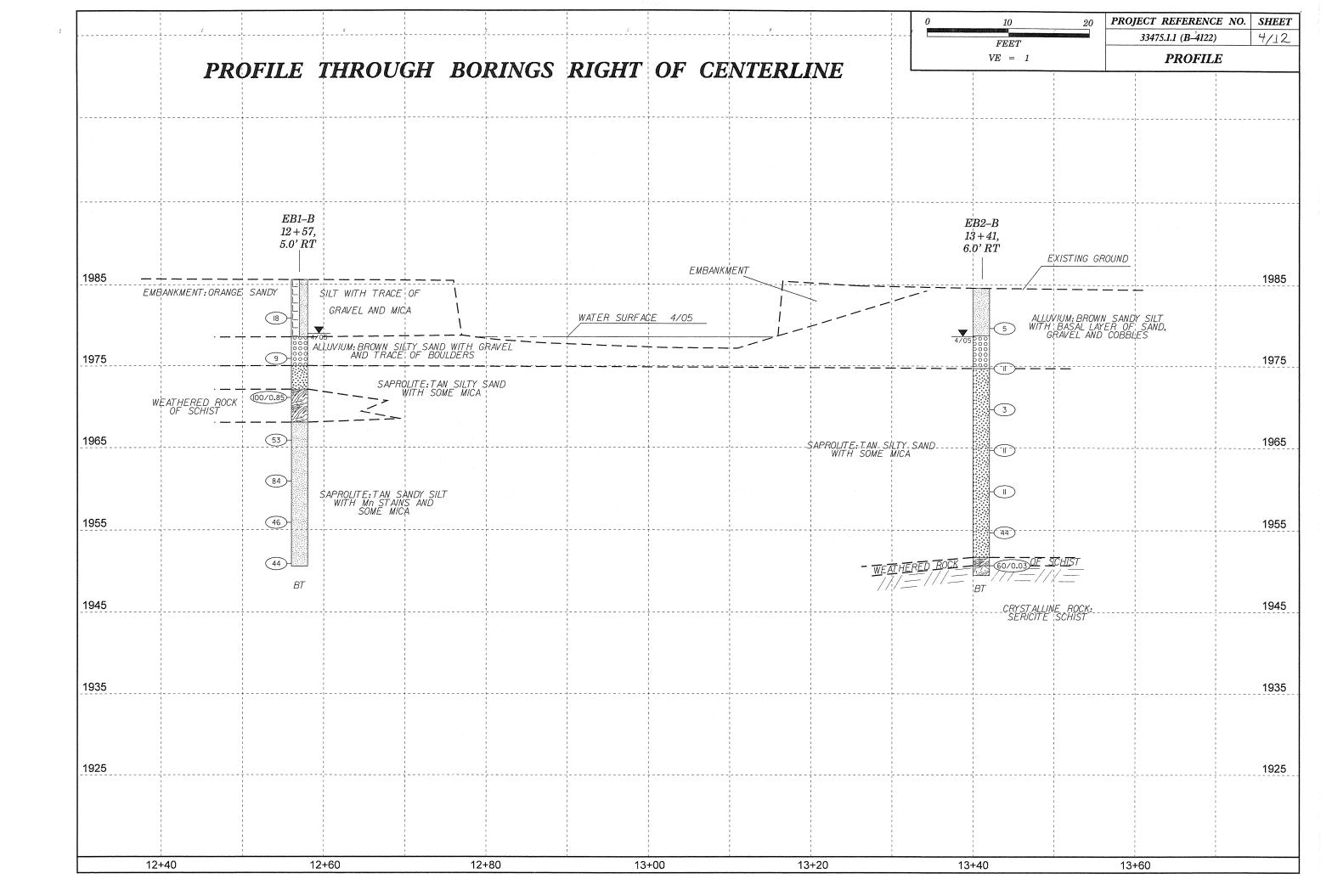
SUBSURFACE INVESTIGATION

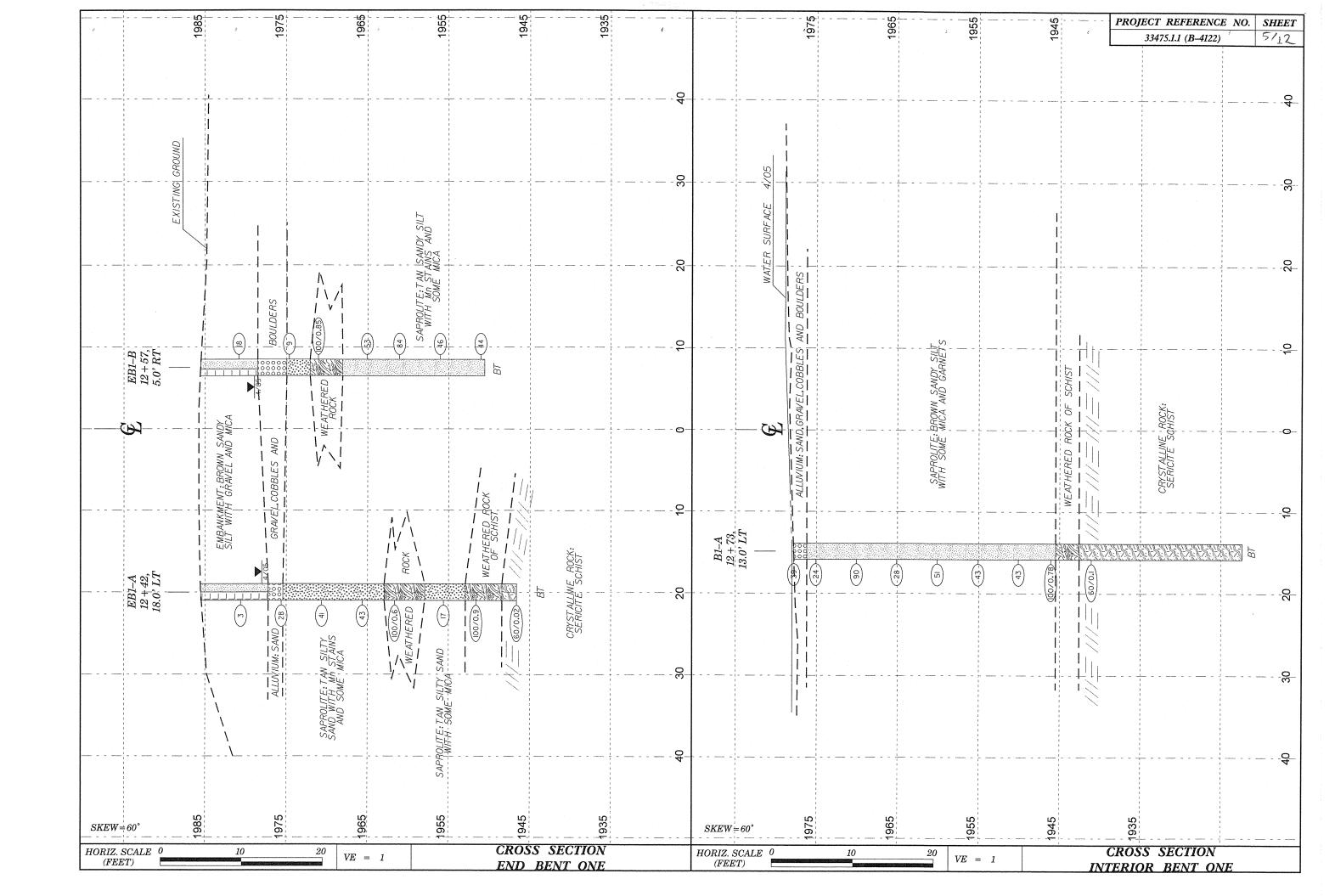
	SOIL AND RO	CK LEGEND, TERM	S, SYMBOLS,	AND ABBREVIAT	IONS	
SOIL DESCRIPTION	GRADATION	· · · · · · · · · · · · · · · · · · ·		ROCK DESC	CRIPTION	TERMS AND DEFINITIONS
SOIL IS CONSIDERED TO BE THE UNCONSOLIDATED, SEMI-CONSOLIDATED, OR WEATHERED EARTH MATERIALS	WELL GRADED - INDICATES A GOOD REPRESENTATION OF PARTICLE SIZES F UNIFORM - INDICATES THAT SOIL PARTICLES ARE ALL APPROXIMATELY THE	FROM FINE TO COARSE. E SAME SIZE.(ALSO	HARD ROCK IS NON	-COASTAL PLAIN MATERIAL THAT IF T	ESTED, WOULD YIELD SPT REFUSAL, AN INFERRED BL PLAIN MATERIAL WOULD YIELD SPT REFUSAL.	ALLUVIUM (ALLUV.) - SOILS THAT HAVE BEEN TRANSPORTED BY WATER.
THAT CAN BE PENETRATED WITH A CONTINUOUS FLIGHT POWER AUGER, AND YIELD LESS THAN 100 BLOWS PER FOOT ACCORDING TO STANDARD PENETRATION TEST (AASHTO T206, ASTM D-1586). SOIL	POORLY GRADED) GAP-GRADED - INDICATES A MIXTURE OF UNIFORM PARTICLES OF TWO OR N	MORE SIZES.	SPT REFUSAL IS P	ENETRATION BY A SPLIT SPOON SAMPL	LER EQUAL TO OR LESS THAN Ø1 FOOT PER 60 BLOWS. WEEN SOIL AND ROCK IS OFTEN REPRESENTED BY A ZON	AQUIFER - A WATER BEARING FORMATION OR STRATA.
CLASSIFICATION IS BASED ON THE AASHTO SYSTEM. BASIC DESCRIPTIONS GENERALLY SHALL INCLUDE: CONSISTENCY, COLOR, TEXTURE, MOISTURE, AASHTO CLASSIFICATION, AND OTHER PERTINENT FACTORS SUCH	ANGULARITY OF GRAINS		OF WEATHERED ROC	RE TYPICALLY DIVIDED AS FOLLOWS:	WEEN SOIL HIND ROCK IS OFTEN REPRESENTED BY A ZONI	AMENACEOUS - APPLIED TO ROCKS THAT HAVE BEEN DERIVED FROM SAND OR THAT CONTAIN SAND.
AS MINERALOGICAL COMPOSITION, ANGULARITY, STRUCTURE, PLASTICITY, ETC. EXAMPLE: VERY STIFF, GRAY, SUTY CLAY, MOST WITH INTERREDURED FINE SAND LAYERS, HIGHLY PLASTIC, A-7-6	THE ANGULARITY OR ROUNDNESS OF SOIL GRAINS IS DESIGNATED BY THE SUBANGULAR, SUBROUNDED, OR ROUNDED.	TERMS: ANGULAR.	WEATHERED	SUMBUMA	MATERIAL THAT WOULD YIELD SPT N VALUES > 100	ARGILLACEOUS - APPLIED TO ALL ROCKS OR SUBSTANCES COMPOSED OF CLAY MINERALS, OR HAVING A NOTABLE PROPORTION OF CLAY IN THEIR COMPOSITION, AS SHALE, SLATE, ETC.
SOIL LEGEND AND AASHTO CLASSIFICATION	MINERALOGICAL COMPOSITIO	∩NI	ROCK (WR)	BLOWS PER FOOT IF T	TESTED.	ARTESIAN - GROUND WATER THAT IS UNDER SUFFICIENT PRESSURE TO RISE ABOVE THE LEVEL
CENEDAL CRANIII AD MATERIAL C. CILT-CLAY MATERIAL C.	MINERAL NAMES SUCH AS QUARTZ, FELDSPAR, MICA, TALC, KAQLIN, ETC. ARE U		CRYSTALLINE ROCK (CR)	FINE TO COARSE GRAIN	N IGNEOUS AND METAMORPHIC ROCK THAT USAL IF TESTED. ROCK TYPE INCLUDES GRANITE.	AT WHICH IT IS ENCOUNTERED, BUT WHICH DOES NOT NECESSARILY RISE TO OR ABOVE THE GROUND SURFACE.
CLASS. (≤ 35% PASSING =200) (> 35% PASSING =200) ORGANIC MATERIALS	WHENEVER THEY ARE CONSIDERED OF SIGNIFICANCE.			GNEISS, GABBRO, SCHIST	T, ETC.	CALCAREOUS (CALC.) - SOILS THAT CONTAIN APPRECIABLE AMOUNTS OF CALCIUM CARBONATE.
GROUP A-1 A-3 A-2 A-4 A-5 A-6 A-7 A-1, A-2 A-4, A-5 CLASS. A-1-0 A-1-5 A-2-4 A-2-5 A-2-6 A-2-7 A-2-7 A-3 A-6, A-7	COMPRESSIBILITY		NON-CRYSTALLINE ROCK (NCR)	SEDIMENTARY ROCK THE	N METAMORPHIC AND NON-COASTAL PLAIN NAT WOULD YEILD SPT REFUSAL IF TESTED. ROCK TYPE	COLLUVIUM - ROCK FRAGMENTS MIXED WITH SOIL DEPOSITED BY GRAVITY ON SLOPE OR AT BOTTOM
0000000000	MODERATELY COMPRESSIBLE LIQUID LIMIT	LESS THAN 31 EQUAL TO 31-50	COASTAL PLAIN		ENTS CEMENTED INTO ROCK, BUT MAY NOT YIELD	OF SLOPE.
SYMBOL 8000000000000000000000000000000000000	HIGHLY COMPRESSIBLE LIQUID LIMIT PERCENTAGE OF MATERIAL	GREATER THAN 50	SEDIMENTARY ROCK (CP)	SPT REFUSAL, ROCK TY SHELL BEDS, ETC.	YPE INCLUDES LIMESTONE, SANDSTONE, CEMENTED	CORE RECOVERY (REC.) - TOTAL LENGTH OF ALL MATERIAL RECOVERED IN THE CORE BARREL DIVIDED BY TOTAL LENGTH OF CORE RUN AND EXPRESSED AS A PERCENTAGE.
# 10 50 MX GRANULAR CLAY MUCK	ORGANIC MATERIAL GRANULAR SILT - CLAY			WEATHE	RING	DIKE - A TABULAR BODY OF IGNEOUS ROCK THAT CUTS ACROSS THE STRUCTURE OF ADJACENT
# 40 30 MX 50 MX 51 MN SOILS PEAT SOILS PEAT SOILS PEAT	SULS SULS	OTHER MATERIAL ACE 1 - 10%			MAY SHOW SLIGHT STAINING. ROCK RINGS UNDER	ROCKS OR CUTS MASSIVE ROCK. DIP - THE ANGLE AT WHICH A STRATUM OR ANY PLANAR FEATURE IS INCLINED FROM THE
LIGUID LIMIT 40 MX 41 MN 49 MX 41 MN 40 MX 41 MN 40 MX 41 MN COTI C MITH	LITTLE ORGANIC MATTER 3 - 5% 5 - 12% LIT	TTLE 10 - 20%	1	R IF CRYSTALLINE.	ME JOINTS MAY SHOW THIN CLAY COATINGS IF OPEN,	HORIZONTAL.
PLASTIC INDEX 6 MX NP 10 MX 10 MX 11 MN 11 MN 10 MX 10 MX 11 MN 11 MN LITTLE OR HIGHL	HIGH Y OPCONIC	ME 20 - 35% GHLY 35% AND ABOVE	(V SLI.) CRYSTA	LS ON A BROKEN SPECIMEN FACE SHIN	NE BRIGHTLY. ROCK RINGS UNDER HAMMER BLOWS IF	DIP DIRECTION (DIP AZIMUTH) - THE DIRECTION OR BEARING OF THE HORIZONTAL TRACE OF THE LINE OF DIP, MEASURED CLOCKWISE FROM NORTH,
GROUP INDEX Ø Ø Ø 4 MX 8 MX 12 MX 16 MX No MX MODERATE ORGAN	GROUND WATER			RYSTALLINE NATURE.	D DISCOLORATION EXTENDS INTO ROCK UP TO	FAULT - A FRACTURE OR FRACTURE ZONE ALONG WHICH THERE HAS BEEN DISPLACEMENT OF THE
USUAL TYPES STONE FRAGS. FINE SILTY OR CLAYEY SILTY CLAYEY ORGANIC	WATER LEVEL IN BORE HOLE IMMEDIATELY AFTER D	DRILLING	(SLI.) 1 INCH.	OPEN JOINTS MAY CONTAIN CLAY, IN	GRANITOID ROCKS SOME OCCASIONAL FELDSPAR	SIDES RELATIVE TO ONE ANOTHER PARALLEL TO THE FRACTURE.
MATERIALS SAND SAND SAND SOILS SOILS	STATIC WATER LEVEL AFTER 24 HOURS		1		TALLINE ROCKS RING UNDER HAMMER BLOWS. LORATION AND WEATHERING EFFECTS. IN	FISSILE - A PROPERTY OF SPLITTING ALONG CLOSELY SPACED PARALLEL PLANES.
GEN. RATING AS A EXCELLENT TO GOOD FAIR TO POOR POOR UNSUITA	PERCHED WATER, SATURATED ZONE, OR WATER BEARIN	ING STRATA	(MOD.) GRANIT(DID ROCKS. MOST FELDSPARS ARE DULL	L AND DISCOLORED, SOME SHOW CLAY, ROCK HAS	FLOAT - ROCK FRAGMENTS ON SURFACE NEAR THEIR ORIGINAL POSITION AND DISLODGED FROM PARENT MATERIAL.
SUBGRADE	SPRING OR SEEP			OUND UNDER HAMMER BLOWS AND SHOW RESH ROCK.	WS SIGNIFICANT LOSS OF STRENGTH AS COMPARED	FLOOD PLAIN (FP) - LAND BORDERING A STREAM, BUILT OF SEDIMENTS DEPOSITED BY
PI OF A-7-5 SUBGROUP IS ≤ LL - 3Ø; PI OF A-7-6 SUBGROUP IS > LL - 3Ø CONSISTENCY OR DENSENESS	MISCELLANEOUS SYMBOLS		MODERATELY ALL RO	JK EXCEPT QUARTZ DISCOLORED OR S'	TAINED, IN GRANITOID ROCKS, ALL FELDSPARS DULL	THE STREAM.
RANGE OF STANDARD RANGE OF UNCONFINED			SEVERE AND DIS (MOD. SEV.) AND CAL	N BE EXCAVATED WITH A GEOLOGIST'S	LINIZATION, ROCK SHOWS SEVERE LOSS OF STRENGTH PICK, ROCK GIVES 'CLUNK' SOUND WHEN STRUCK,	FORMATION (FM.) - A MAPPABLE GEOLOGIC UNIT THAT CAN BE RECOGNIZED AND TRACED IN THE FIELD.
PRIMARY SOIL TYPE CUMPACTNESS OR PENETRATION RESISTENCE COMPRESSIVE STRENGTH (N-VALUE) (TONS/FT2)	ROADWAY EMBANKMENT (RE) WITH SOIL DESCRIPTION ROADWAY EMBANKMENT (RE) SPT CPT SP	NG DESIGNATIONS	IF TEST	TED, WOULD YIELD SPT REFUSAL		JOINT - FRACTURE IN ROCK ALONG WHICH NO APPRECIABLE MOVEMENT HAS OCCURRED.
GENERALLY VERY LOOSE <4	SOIL SYMBOL AUGER BORING	S - BULK SAMPLE	(SEV.) IN STRE	ENGTH TO STRONG SOIL. IN GRANITOID	TAINED. ROCK FABRIC CLEAR AND EVIDENT BUT REDUCED ROCKS ALL FELDSPARS ARE KAOLINIZED TO SOME	LEDGE - A SHELF-LIKE RIDGE OR PROJECTION OF ROCK WHOSE THICKNESS IS SMALL COMPARED TO
CRANULAR	ARTIFICIAL FILL (AF) OTHER	SS - SPLIT SPOON SAMPLE	EXTENT	. SOME FRAGMENTS OF STRONG ROCK (TED, YIELDS SPT N VALUES > 100 BPF	USUALLY REMAIN.	ITS LATERAL EXTENT. LENS - A BODY OF SOIL OR ROCK THAT THINS OUT IN ONE OR MORE DIRECTIONS.
(NON-COHESIVE) DENSE 30 TO 50 VERY DENSE >50	THAN ROADWAY EMBANKMENT - CORE BORING	ST - SHELBY TUBE			TAINED, ROCK FABRIC ELEMENTS ARE DISCERNIBLE BUT	MOTTLEO (MOT.) - IRREGULARLY MARKED WITH SPOTS OF DIFFERENT COLORS, MOTTLING IN
VERY SOFT <2 <0.25	INFERRED SOIL BOUNDARY	SAMPLE	(V SEV.) THE MA	SS IS EFFECTIVELY REDUCED TO SOIL	STATUS, WITH ONLY FRAGMENTS OF STRONG ROCK OCK WEATHERED TO A DEGREE SUCH THAT ONLY MINOR	SOILS USUALLY INDICATES POOR AERATION AND LACK OF GOOD DRAINAGE. PERCHED WATER - WATER MAINTAINED ABOVE THE NORMAL GROUND WATER LEVEL BY THE PRESENCE OF AN
GENERALLY SOFT 2 TO 4 0.25 TO 0.50	MONITORING WEL	RS - ROCK SAMPLE	VESTIGE	S OF THE ORIGINAL ROCK FABRIC REN	MAIN. IF TESTED, YIELDS SPT N VALUES < 100 BPF	INTERVENING IMPERVIOUS STRATUM.
MATERIAL STIFF 8 TO 15 1 TO 2	PIEZOMETER INSTALLATION	RT - RECOMPACTED TRIAXIAL	COMPLETE ROCK RE	DUCED TO SOIL. ROCK FABRIC NOT DI	ISCERNIBLE, OR DISCERNIBLE ONLY IN SMALL AND	RESIDUAL (RES.) SOIL - SOIL FORMED IN PLACE BY THE WEATHERING OF ROCK.
(COHESIVE) VERY STIFF 15 TO 30 2 TO 4 HARD >30 >4	SLOPE INDICATO	SAMPLE OR CBR - CALIFORNIA BEARING		NEXAMPLE.	PRESENT AS DIKES OR STRINGERS, SAPROLITE IS	ROCK QUALITY DESIGNATION (ROD) - A MEASURE OF ROCK QUALITY DESCRIBED BY TOTAL LENGTH OF
TEXTURE OR GRAIN SIZE	ROCK STRUCTURES	RATIO SAMPLE		ROCK HAR	RDNESS	ROCK SEGMENTS EQUAL TO OR GREATER THAN 4 INCHES DIVIDED BY THE TOTAL LENGTH OF CORE RUN AND EXPRESSED AS A PERCENTAGE.
U.S. STD. SIEVE SIZE 4 10 40 60 200 270	SOUNDING ROD REF SPT REFUSAL				PICK, BREAKING OF HAND SPECIMENS REQUIRES	SAPROLITE (SAP.) - RESIDUAL SOIL THAT RETAINS THE RELIC STRUCTURE OR FABRIC OF THE PARENT ROCK.
OPENING (MM) 4.76 2.00 0.42 0.25 0.075 0.053				AL HARD BLOWS OF THE GEOLOGIST'S I	PICK. ' WITH DIFFICULTY. HARD HAMMER BLOWS REQUIRED	SILL - AN INTRUSIVE BODY OF IGNEOUS ROCK OF APPROXIMATELY UNIFORM THICKNESS AND
BOULDER COBBLE GRAVEL COARSE FINE SILT CLAY	ABBREVIATIONS AR - AUGER REFUSAL HI HIGHLY	w - MOISTURE CONTENT		TACH HAND SPECIMEN.	WITH DIFFECCIT, MAND NAMED BLOWS REGULARD	RELATIVELY THIN COMPARED WITH ITS LATERAL EXTENT, THAT HAS BEEN EMPLACED PARALLEL TO THE BEDDING OR SCHISTOSITY OF THE INTRUDED ROCKS,
(BLDR.) (COB.) (GR.) (CSE. SD.) (F SD.) (SL.) (CL.)	BT - BORING TERMINATED MED MEDIUM	V - VERY	MODERATELY CAN BE HARD EXCAVE	SCRATCHED BY KNIFE OR PICK, GOU	GES OR GROOVES TO 0.25 INCHES DEEP CAN BE	SLICKENSIDE - POLISHED AND STRIATED SURFACE THAT RESULTS FROM FRICTION ALONG A FAULT OR
GRAIN MM 305 75 2.0 0.25 0.05 0.005	CL CLAY MICA MICACEOUS CPT - CONE PENETRATION TEST MOD MODERATELY	VST - VANE SHEAR TEST WEA WEATHERED		DERATE BLOWS.	'S PICK. HAND SPECIMENS CAN BE DETACHED	SLIP PLANE.
SIZE IN. 12 3	CSE COARSE NP - NON PLASTIC DMT - DILATOMETER TEST ORG ORGANIC	7 - UNIT WEIGHT 7 - DRY UNIT WEIGHT	MEDIUM CAN BE HARD CAN BE	GROOVED OR GOUGED 0.05 INCHES DE	DEEP BY FIRM PRESSURE OF KNIFE OR PICK POINT. CES 1 INCH MAXIMUM SIZE BY HARD BLOWS OF THE	STANDARD PENETRATION TEST (PENETRATION RESISTANCE) (SPT) - NUMBER OF BLOWS (N OR BPF) OF A 140 LB. HAMMER FALLING 30 INCHES REQUIRED TO PRODUCE A PENETRATION OF 1 FOOT INTO SOIL WITH
SOIL MOISTURE - CORRELATION OF TERMS SOIL MOISTURE SCALE FIELD MOISTURE CHARLES OF SELECTION OF TERMS	DPT - DYNAMIC PENETRATION TEST PMT - PRESSUREMETER TEST	79- OHT ONLY WEIGHT	POINT	OF A GEOLOGIST'S PICK.		A 2 INCH OUTSIDE DIAMETER SPLIT SPOON SAMPLER, SPT REFUSAL IS PENETRATION EQUAL TO OR LESS THAN 0.1 FOOT PER 60 BLOWS.
(ATTERBERG LIMITS) CENTER HOLSTONE COULDE FOR FIELD MOISTURE DESCRIPTION COULDE FOR FIELD MOISTURE DESCRIPTION	F - FINE SAP SAPROLITIC SD SAND, SANDY				IFE OR PICK, CAN BE EXCAVATED IN FRAGMENTS Y MODERATE BLOWS OF A PICK POINT, SMALL, THIN	STRATA CORE RECOVERY (SREC.) - TOTAL LENGTH OF STRATA MATERIAL RECOVERED DIVIDED BY TOTAL LENGTH
- SATURATED - USUALLY LIQUID; VERY WET, USUALLY	FOSS FOSSILIFEROUS SL SILT, SILTY FRAC FRACTURED, FRACTURES SLI SLIGHTLY		PIECES	CAN BE BROKEN BY FINGER PRESSUR	RE.	OF STRATUM AND EXPRESSED AS A PERCENTAGE.
(SAT.) FROM BELOW THE GROUND WATER TABLE	FRAGS FRAGMENTS TCR - TRICONE REFUSAL		VERY CAN BE SOFT OR MOR	CARVED WITH KNIFE. CAN BE EXCAVA- RE IN THICKNESS CAN BE BROKEN BY	ATED READILY WITH POINT OF PICK, PIECES 1 INCH FINGER PRESSURE, CAN BE SCRATCHED READILY BY	STRATA ROCK QUALITY DESIGNATION (SROD) - A MEASURE OF ROCK QUALITY DESCRIBED BY TOTAL LENGTH OF ROCK SEGMENTS WITHIN A STRATUM EQUAL TO OR GREATER THAN 4 INCHES DIVIDED BY THE
PLASTIC SEMISOLID; REQUIRES DRYING TO	EQUIDATAL HOED ON OUR LEGT E	200 1507	FINGER	NAIL.		TOTAL LENGTH OF STRATA AND EXPRESSED AS A PERCENTAGE.
(PI) PLASTIC LIMITATTAIN OPTIMUM MOISTURE	EQUIPMENT USED ON SUBJECT P	T T		RE SPACING	BEDDING TERM THICKNESS	TOPSOIL (TS.) - SURFACE SOILS USUALLY CONTAINING ORGANIC MATTER.
	DRILL UNITS: ADVANCING TOOLS:	HAMMER TYPE:	TERM VERY WIDE	SPACING MORE THAN 10 FEET	VERY THICKLY BEDDED > 4 FEET	BENCH MARK: TBM- BRIDGE NAIL IN POLE-L- STA. 12+61, 19.0' RT
OM OPTIMUM MOISTURE - MOIST - (M) SOLID; AT OR NEAR OPTIMUM MOISTUR	MOBILE B CLAY BITS	X AUTOMATIC MANUAL	WIDE	3 TO 10 FEET	THICKLY BEDDED 1.5 - 4 FEET THINLY BEDDED 0.16 - 1.5 FEET	ELEVATION: 1986,45 FT.
PECHITES ADDITIONAL WATER TO	6' CONTINUOUS FLIGHT AUGER	CORE SIZE:	MODERATELY CLOSI CLOSE	Ø.16 TO 1 FFFT	VERY THINLY BEDDED 0.03 - 0.16 FEET	NOTES:
- DRY - (D) ATTAIN OPTIMUM MOISTURE	BK-51 8* HOLLOW AUGERS	B	VERY CLOSE		THICKLY LAMINATED 0.008 - 0.03 FEET THINLY LAMINATED < 0.008 FEET	NOTES:
PLASTICITY	CME-45C HARD FACED FINGER BITS	X -N XWL		INDURAT		
PLASTICITY INDEX (PI) DRY STRENGTH	TUNGCARBIDE INSERTS	-+	FOR SEDIMENTARY ROCK		THE MATERIAL BY CEMENTING, HEAT, PRESSURE, ETC.	
NONPLASTIC Ø-5 VERY LOW LOW PLASTICITY 6-15 SLIGHT	X CME-550 X CASING X W/ ADVANCER		FRIABLE		FINGER FREES NUMEROUS GRAINS; BY HAMMER DISINTEGRATES SAMPLE.	
MED. PLASTICITY 16-25 MEDIUM HIGH PLASTICITY 26 OR MORE HIGH	PORTABLE HOIST TRICONE STEEL TEETH	HAND TOOLS: POST HOLE DIGGER	MODERATELY		SEPARATED FROM SAMPLE WITH STEEL PROBE:	
COLOR	TRICONE TUNGCARB.	HAND AUGER	MODERNIELT		WHEN HIT WITH HAMMER.	
	CORE BIT	SOUNDING ROD	INDURATED		FFICULT TO SEPARATE WITH STEEL PROBE;	
DESCRIPTIONS MAY INCLUDE COLOR OR COLOR COMBINATIONS (TAN, RED, YELLOW-BROWN, BLUE-GRAY). MODIFIERS SUCH AS LIGHT, DARK, STREAKED, ETC. ARE USED TO DESCRIBE APPEARANCE.		VANE SHEAR TEST	EVTDEMELV		BREAK WITH HAMMER. BLOWS REQUIRED TO BREAK SAMPLE;	
			EXTREMELY		S ACROSS GRAINS.	

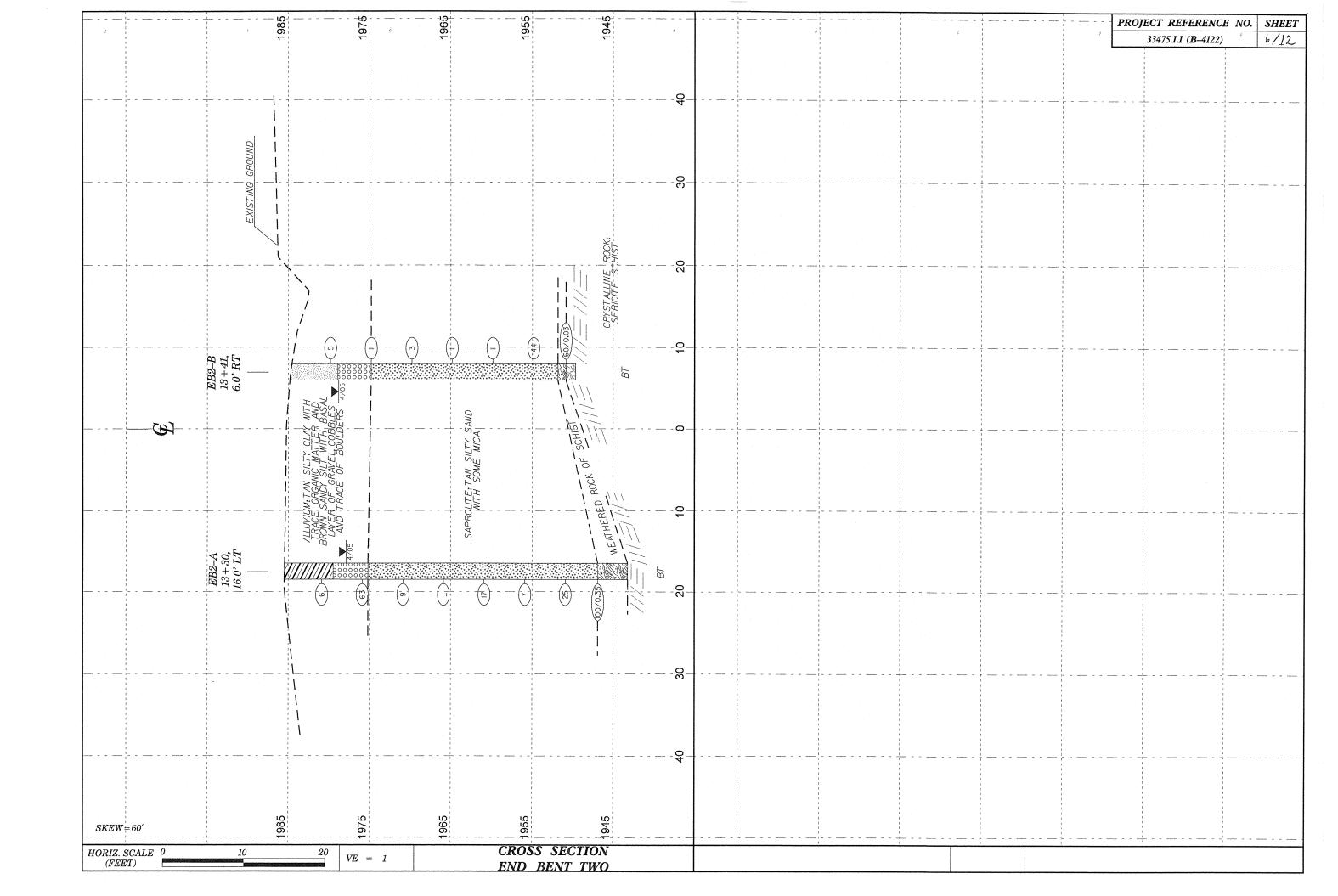
 PROJECT REFERENCE NO.
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 33475.I.I(B-4122)
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SHEET

PRO	JECT NO	O . 33	475.1.	1	ID	. В	3-4122		COUNTY	Grahan	n			GEOLOGIST Hag	ger, M. M.	
SITE	DESCR	IPTION	N Bric	ige No	o. 81	on S	SR-1117 ove	r Long Creek.							GROUND	WTR (ft)
BOR	ING NO.	EB1	-A		!	STA	ATION 12+4	2	OFFSET	18ft LT			ALIGNI	MENT -L-	0 HR.	N/A
COL	LAR ELE	E V . 1,	,985.5	ft		TOT	TAL DEPTH	38.9 ft	NORTHING	608,1	105		EASTIN	IG 565,457	24 HR.	7.5
DRIL	L MACH	IINE (CME-5	50	!	DRI	ILL METHOD	NW Casing w	/ SPT	·				HAMMER TYPE	Automatic	
STAI	RT DATE	04/0				CON	MP. DATE 0	4/06/05	SURFACE	WATER	DEP	ГН	N/A	DEPTH TO ROC	K 37.1 ft	****
ELEV (ft)	ELEV	DEPTH (ft)	'	OW CO			1	LOWS PER FOOT	75 100	SAMP.	17	0		SOIL AND ROCK DESC	CRIPTION	
(1.1)	(ft)	(1.7)	0.5π	0.5ft	0.5f	\mathbb{H}	0 25	50 	75 100	NO.	/MO	G	ELEV. (ft)			DEPTH (fi
1990	_	_											<u>. </u>			
	-	-														
1985	_		╁──		ļ	+	ļ			+	 		- 1,985.5 -	GROUND SURFA ROADWAY EMBANI	KMENT	0.0
	4.004.0	- 20										L	- -	Brown sandy si	lt.	
1980	1,981.6-	- 3.9	1	1	2	11,	3				w	L	-			
1000	-	-							1			L	-			
	1,976.6-	- 8.9				\parallel	 						1,977.2	ALLUVIAL		8.3
1975	_	-	10	16	12	-	22	8	1			200	- 1,975.4 	Tan gravel and boulders w matrix.	ith silty sand	10.1
		_						X : : : : : :					- L	SAPROLITE		
1970	1,971.6	- 13.9 -	15	17	24	\parallel		. 41		SS-8	1		- -	Tan silty sand with Mn stai mica.	ns and some	
1070	7	-							1		1		-			
	1,966.6	- - 18.9											- -			
1965	4	-	7	21	22		.	43	• • • •		ľ		-			
	1,001,0	-							1			300	1,962.9	WEATHERED RO	NCI/	22.€
1960	1,961.6	- 23.9 -	29	47	53/0.	1			100/0.6	,			-	Weathered rock of s		
1000	7	-											 1,957.9			07.0
	1,956.6	- 28.9							T			CIC	- 1,907.9	SAPROLITE		27.6
1955	1	-	5	7	10	\parallel	17		1	SS-9			-	Tan silty sand with son	ne mica.	
	1,951.6	- - - 33.9							1			7000	1,952.9	WEATHERED RO	OCK.	32.6
1950	1,951.04	- 33.9	15	67	33/0.4	4			100/0.9				-	Weathered rock of s		
	7	•											- - 1,948.4			37.1
-	1,946.6	38.9	60/0.02			Ш	<u>:::: :</u>	<u>::: ::::</u>	60/0.02				- 1,946.6	CRYSTALLINE RO Sericite schist.	OCK	38.9
1945	4	-	00/0.02						00/0.02			F	_	Boring Terminated with Penetration Test Refusal :	Standard at Elevation	
	Ŧ												-	1,946.6 ft in sericite		
1940	Ŧ											F	• •			
	T												- -			
ŀ	Ŧ											F	•			
1935	Ŧ	-										F	- 			
	Ŧ											F	•			
1930	Ŧ			.								F	•			
	Ŧ												-			
	7												•			
1925	Ŧ			l		-						F				
	Ŧ											F	•			
1920	Ŧ			ĺ								F	•			
	‡											þ	 ·			
	‡		- 1									F	•			
1915	‡	.		l								þ	-			
ĺ	‡											F				
1910	İ			1							. [Ė				



SHEET 7/12

\geq					_0	G REP	<u>ORI</u>		·							4:	/12
 	JECT NO					B-4122			COUNTY	Gra	aham)			GEOLOGIST Hag	er, M. M.	
SITE	DESCR	IPTION	Brio	dge No	. 81 o	n SR-1117 c	ver Long	Creek.								GROUND \	VTR (ft)
BOR	ING NO.	EB1-	-B		S	TATION 12	!+57		OFFSET	5ft I	RT.			ALIGNMEN	NT -L-	0 HR.	N/A
COL	LAR ELE	E V . 1,	985.6	ft	T	OTAL DEPT	H 35.0 f	t	NORTHIN	IG 6	08,0	81		EASTING	565,471	24 HR.	6.6
DRIL	L MACH	IINE (OME-5	550	D	RILL METHO	WN do	Casing w	SPT						HAMMER TYPE	Automatic	
STA	RT DATE	04/0	5/05		C	OMP. DATE	04/05/0	5	SURFAC	E WA	TER	DEPT	ΗN	I/A	DEPTH TO ROC	K N/A	
ELEV (ft)	DRIVE ELEV (ft)	DEPTH (ft)	0.5ft	0.5ft	JNT 0.5ft	0 2		PER FOOT 50	75 10	11	AMP. VO.	MOI	L O G	ELEV. (ft)	SOIL AND ROCK DESC		DEPTH (ft)
1990	-	- - -											 - - -	<u>-</u>			
1985	_			-				T		╫				- 1,985.6 	GROUND SURFA ROADWAY EMBANI		0.0
1980	1,981.9 -	3.7	5	12	6									Ora	inge sandy silt with trace mica.		7.0
1975	1,976.1	9.5	4	5	4									Bro 1,975.0	ALLUVIAL own silty sand with grave boulders.	I and trace of	10.6
	1,971.1	14.5			,								3000	1,972.1	SAPROLITE Tan silty sand with sor		13.5
1970	1,011.1	-	43	57/0.35					100/0.85					- 1,968.1	WEATHERED RO Weathered rock of s		17.5
1965	1,966.1	19.5	31	27	26			1 5 3——			S-4			Та -	SAPROLITE In sandy silt with Mn stail mica.	ns and some	
1960	1,962.1	23.5	29	42	42			: ```\`\ : : : : :	84				Ė				
1900	1,957.1	28.5							+				F	-			
1955	- -	- -	51	21	25			46	1	\parallel			ŧ	-			
	1,952.1	33.5	27	19	25								, t				
1950	-	-		,,			· · · • 4	4_ • • • •	1	╫			- -	1,950.6 - Borir	ng Terminated at Elevation hard saprolite.	on 1,950.6 ft in	35.0
1945	-	-				·							E	-			
1940	-	- - -											-				
	-	-	-										E	-			
1935	-	- - -											<u> </u>	_			
1930	- -	- - -											-	-			
1925		- -											[- -				-
	-	-												-			
1920	_	-												<u>.</u>			
1915	-	-											E	-			
1910		- - -				·							-				
	·		·	·		L											

NCDOT GEOTECHNICAL ENGINEERING UNIT

SHEET

PRO.	ECT NO). 334	175.1.1	1	ID.	В	3-4122			COUNTY	Grahai	n			GEOLOGIST Hag	ger, M. M.	
SITE	DESCR	IPTION	l Brid	lge No	. 81 o	n S	SR-1117 o	ver Long	Creek.							GROUND \	VTR (fi
BORI	NG NO.	B1-A			S	TA	TION 12	+73		OFFSET	13ft LT			ALIGNMEN	T -L-	0 HR.	N/A
COLL	AR ELE	V. 1,	977.8	ft	T	OT	AL DEPT	H 55.4 ft		NORTHIN	G 608,	098		EASTING	565,488	24 HR.	N/
DRIL	L MACH	INE C	ME-5	50	D	RIL	LL METHO	NW C	Casing w	SPT					HAMMER TYPE	Automatic	
STAF	T DATE	04/0	5/05		C	ON	IP. DATE	04/05/05	5	SURFACE	WATE	R DEPT	гн (0.2ft	DEPTH TO ROC	K 35.2 ft	
ELEV	DRIVE	DEPTH	BLC	ow co	UNT	П		BLOWS P	ER FOOT		SAME	. V/	L		SOIL AND ROCK DES	CRIPTION	
(ft)	ELEV (ft)	(ft)	0.5ft	0.5ft	0.5ft		0 2	5 5	0	75 100	NO.	МО		ELEV. (ft)			DEPTH
1980	_													_			
	1,977.8	0.0				Ш					Ш				WATER SURFACE (04/05/05)	
	1,976.1	1.7	18	18	21			39			SS-1	=	9000	1,976.2 S	ALLUVIAL and, gravel, cobbles ar	nd boulders.	
1975	-	-	14	12	12	$\ \cdot\ $		124		1	SS-2	-		Br	SAPROLITE rown sandy silt with sor		
	4 074 47	- ^-							1550	: : : :				- -	garnets.		
1970	1,971.1	6.7	32	39	51	\mathbb{L}				90-				_			
	-	_				Ш	: : : :		 سرر	1:				_			
	1,966.1	11.7				\prod		· · · · · ·		1::::				_			
965	_	-	14	14	14	$\ \cdot\ $		28-		+	+			_			
	-	F															
960	1,961.1	16.7	19	27	24	$\ \cdot\ $			51		SS-3	1		_			
	-	F						/				1		_			
	1,956.1	21.7			1									7			
955	-,,,,,,,,,,,,,,,,,,,,,,,,,,,,,,,,,,,,,,		16	22	21	11		43		<u> </u>	$\parallel \parallel$			- -			
		ļ.												- -			
1950	1,951.1	26.7	8	20	23	$\ \ $: : : :						- -			
1950	-	<u> </u>	ľ	20	23	lt		43			1	-		- -			
	1,946.1	21.7				Ш			77.55					- -			
1945	1,540.1	31.7	28	72/0.28	8	\prod			• • • •	100/0.78			770	- 1,945.5 	WEATHERED R	оск	3
		<u> </u>									11.			- - 1,942.6	Weathered rock of		3
10.40	1,941.1	36.7	60/0.1				• • • •			60/0.1				- -	CRYSTALLINE F Sericite schis		
1940	-	ţ	00/0.1	1		$\ \cdot\ $					11			- -			
		<u> </u>												-			
935	-	<u> </u>				\prod					41			_			
		ţ							: : : :					- -			
4000		ţ												-			
1930	-	<u> </u>				\mathbf{H}				 	1			-			
		<u> </u>												<u>-</u>			
1925		<u>t</u>				11]] .						
		ţ							: : : :					1,922.4			5
		<u> </u>	T		1	T							1		g Terminated at Elevat sericite schis		n
1920	-	‡										İ	l	-	33.1310 30113		
		ŧ												-			
1915		t												Ł			
		E												E			
		Ŧ												E			
1910	-	F												E			
		Ŧ											1	F			
1905		‡										Ì		F			
เฮบอ	-	‡												F			
		‡												ļ.			
900		+	1	1	1									-			



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2	U	7 7 (CO	RE B	BOR	INC	3 RE	PO	RT		8/12	2
PRO	JECT NO). 334	75.1.1	1	D. B-	4122				СО	DUNTY Graham GEOLOGIST Hager, M. M.	
	DESCRI			ge No. 8				ong Cr	eek.		GROUND WTR (f	t)
}	ING NO.				 		12+73			+	FSET 13ft LT ALIGNMENT -L- 0 HR. N/.	A
	LAR ELE				 		PTH 55		·		DRTHING 608,098 EASTING 565,488 24 HR. N/A	A
	L MACHI			50	 		HOD N		sing w	T		_
	RT DATE				 		TE 04/0			+	JRFACE WATER DEPTH 0.2ft DEPTH TO ROCK 32.3 ft	4
 	E SIZE RUN			DRILL	1	AL RU	N 17.21		ATA	DR L	RILLER Childers, R.	4
ELEV (ft)	ELEV (ft)	DEPTH (ft)	RUN (ft)	RATE (Min/ft)	REC. (ft) %	RQD (ft) %	SAMP. NO.	REC. (ft) %	RQD (ft) %	OG	DESCRIPTION AND REMARKS ELEV. (ft) DEPTH	(ft)
1939.64	1,939.6	38.2	3.2		(88.0)	(88.0)					Begin Coring @ 38.2 ft CRYSTALLINE ROCK	4
	1,936.4	•	0.2		2750%	2750%					Light grey sericite schist with garnets, biotite and staurolite. Trace of pyrite. Fresh; medium hard.	
1935	1,000.4	-	5.0		(96.0)	(96.0) 1920%					a) Occasional joints with Fe staining @ 10°. Joint spacing approx. 2.0 feet.	l
	1				132070	132076					(COMINUEU)	Ì
1930	1,931.4	46.4	5.0		(96.0)	(96.0)						
1930	‡	-			1920%	1920%						
	1,926.4	· 51.4										
1925	†	-	4.0		(93.0) 2325%	(93.0) 2325%						
	1,922.4	55.4			ļ						1,922.4 55	5.4
1920	$\frac{1}{1}$										Boring Terminated at Elevation 1,922.4 ft in sericite schist.	
	1										E	
1015	1	-										
1915	1	<u>.</u>									-	
	<u> </u>										_	
1910	#	•										Ì
	‡										-	
1905	‡	•										
1000	‡	•										
	Ŧ	•										
1900	\pm	-										
											E	
1895	+	-										l
											-	
1890	‡	•									<u>.</u> -	
1000		-									F	
	‡	•									F	
1885	‡	•									F	
	1	•									E	
1880	\perp	•									<u> </u>	
	1											
1075	‡	•									<u> </u>	
1875		-									<u>'</u>	
1870	‡	-										
	‡											
1865	‡	•									F	
	1										E .	
	I											
1860		-			L	L		<u> </u>		Ш	<u> </u>	L

NCDOT GEOTECHNICAL ENGINEERING UNIT

SHEET

PROJ	ECT NO) . 334	75.1.1	1	IE).	B-4122			COUNTY	Graham	1			GEOLOGIST Hag	ger, M. M.	
SITE	DESCR	IPTION	Brid	ge No	. 81	on	n SR-1117	over Long	Creek.							GROUND V	VTR (ft
BORII	NG NO.	EB2-	·A			ST	TATION 1	3+30		OFFSET	16ft LT			ALIGNMENT	Γ -L-	0 HR.	N/A
COLL	AR ELE	EV. 1,	983.5	ft		TC	OTAL DEP	TH 42.3 ft	·	NORTHING	608,0	98		EASTING 5	565,545	24 HR.	7.6
DRILL	. MACH	INE C	ME-5	50		DF	RILL METH	OD NW	Casing w	/ SPT					HAMMER TYPE	Automatic	
	T DATE				1	CC	OMP. DATI	≡ 04/06/0	5	SURFACE	WATER	DEPT	H N	1/A	DEPTH TO ROC	K N/A	
ELEV	DRIVE ELEV	DEPTH	BLC	W CO	UNT	П		BLOWS F	PER FOOT	-	SAMP.	V/	L		COLLAND DOOK DEC	ODIDTION	
(ft)	ELEV (ft)	(ft)	0.5ft	0.5ft	0.5	ift	0	25 5	50	75 100	NO.	мог	O G	ELEV. (ft)	SOIL AND ROCK DES		DEPTH (
													П				
1985											ł						
	-	<u> </u>	ļ			\dashv					ļ	ļ		- 1,983.5	GROUND SURF	ACE	C
	1	-				-								Tan s	silty clay with trace of	organic matter.	
1980	1,979.9	3.6	WOH	WOH	6	-		 			SS-5	w		_			
	-	-	Won	Work	"		6	<u> </u>			33-3	┨ " .		- - 1,977.5			- 6
1075	- 1,974.9	8.6							• • • •				000	- Grey g	ALLUVIAL ravel with sand and tr	ace of boulders.	
1973	1,974.9	0.0	28	47	16	3			63					1,973.2			10
į	-	L				- [: :	† 							SAPROLITE Tan silty sand with so	ma miaa	
1970	1,969.9	13.6			-		<u> · </u>	1				1		-	Tarr Sitty Saria Witt 50	ille Illica.	
	1		1	3	6	1	9	: : : :			SS-6	ł		•			
		Ŀ					: • : :	: : : :		.				•			
1965	-	F												- -			
ĺ	-					l	1 : : ; :	: : : :									
1060	1,959.9	22.6					::\:							•			
1900	1,909.9	_ 23.0	1	8	9	\neg		,			SS-7	1		-			
		_					::/::	: : : :	: : : :			1					
1955	1,954.9	28.6					. /							• •			
	-	-	1	2	5		7					ľ		•			
	-						: X; :							•			
1950	1,949.9	33.6		12	13		· · · · · · · /							• •			
-	-		5	12	'	۱ ٔ	::::	25	: : : :					• '			
40.45						-	::::	XII						•			
1945	1,944.9	38.6	100/0.3	! 5		l	 	 `- =		100/0.35			113	1,944.9	WEATHERED R		38
ľ	4	L				l		: : : :							Weathered rock of	schist.	40
1940			l		<u> </u>	\dashv			L	1	1	ļ	1	1,941.2 Borin	g Terminated with Ca	sing Advancer	42
	-	-				-							F	Refu	sal at Elevation 1,941 schist.	.2 ft in sericite	
	1					-							F	•			
1935	-													• -			
	1	-				1								•			
1000	-					İ								•			
1930	_	-				1								-			
ŀ		E											lt	•			
1925	-	Ŀ											F	-			
	-	F		·		1							F	•			
	-	-												. •			
1920		<u> </u>												• 			
	-	ţ															
	_	Ŀ											<u> </u>				
1915	4	L											F	<u>-</u>			
	1	-											F	•			
1910	1	-												•			
1910	-	<u> </u>				1											
	4	<u> </u>											E	•			
1905]					-							F				



SHEET 9/12

$\underline{\underline{\mathbb{Y}}}$	V		BO	REL	_00	i R	EP	ORT											~ ! ,	/12
PRO	JECT NO) . 334	175.1.1	<u> </u>	ID.	B-412	22			С	OUNTY	G	raham				G	EOLOGIST Hage	er, M. M.	
SITE	DESCR	IPTION	Brid	ge No.	. 81 on	SR-1	1117 c	ver Long	Creek.										GROUND W	/TR (ft)
	ING NO.						N 13				FFSET					ALIGNME			0 HR.	N/A
	LAR ELE	·····						H 35.1 f			ORTHIN	3	608,0	76		EASTING			24 HR.	5.9
	L MACH			50				DD NW										HAMMER TYPE		
	DRIVE	Γ	T	W COL		JNIP. I	DATE	04/06/0 BLOWS			URFACE	7	SAMP.	DEPT	HN	1/A		DEPTH TO ROCK	33.9 ft	
ELEV (ft)	DRIVE ELEV (ft)	DEPTH (ft)	0.5ft		0.5ft	0	2		50	75 75	100	11	NO.	моі	O G		SC	IL AND ROCK DESC		
	(-4)					†		.,,,,,,,,,,,,,,,,,,,,,,,,,,,,,,,,,,,,,,	-L	1		$\dagger \dagger$		VIVIOI	-	ELEV. (ft)				DEPTH (ft)
1985																1,984.6		GROUND SURFA	CE	
		Ē				1:	::		T :	T		H				•		ALLUVIAL		0.0
4000	1,980.7	3.9								-					E	- -	JWII :	sandy silt with trace of	i smali gravel.	
1980	_		2	2	3	5			1	\exists		$\ \ $		V		1,978.8				5.8
	4.075.7	<u> </u>				::	::		:::	:	: : : :				885 886	• •		ALLUVIAL Sand, gravel and col	obles.	
1975	1,975.7	- 8.9 -	5	5	6		11	<u></u>			• • • •					1,974.7				9.9
	-	-			ĺ	: <i>,i</i>			: : :	-			ĺ			•	T	SAPROLITE an silty sand with som	ne mica.	
1970	1,970.7	13.9	WOH	1	2	//:				-					Ī	,				
	-		WON	'	. 2	3	• •			-			1	ĺ	F	•				
	1,965.7	18.9			ŀ	17				-					E	•				
1965	-	_	2	5	6	-	11-		 	\exists					E	-				
	-	-				• 1														
1960	1,960.7_	<u> 23.9</u>	1	5	6	l i	 5 11			-	• • • •				E	-				
	-	-				. 1			: : :						t	•				
1955	1,955.7	28.9		40		• 1	<u> </u>	· · · · ·	: : :						ŧ					
1000	-	-	7	12	32			• 4	 	-					t	-				
	1,950.7	33.0				: :									1000	. 1,951.7 . 1,950.7		WEATHERED DO	<u> </u>	32.9 33.9
1950	1,900.7		60/0.03			<u> </u>			<u> </u>	\perp	60/0.03	1				-1,949.5		WEATHERED RO Weathered rock of se	chist.	35.1
													ĺ		E			CRYSTALLINE RC Sericite schist.		
1945	-	_										l	l		E	Bor Re	ring efusa	Terminated with Casi al at Elevation 1,949.5 schist.	ng Advancer of tin sericite	.
	-	-					~					1		1	þ			schist.		
1940	-	-										1	I		þ					
1340	_	-													þ	-				
	-	-										١			þ					
1935	_	-											l		F	-				
]											1			F					
1930														ĺ	E	· ·				
	-	-										1			F					1
1025	-	-											I		þ					
1925	-	-												l	þ	-				
	-	-										l			þ					
1920	-	-										1			þ	-				
	-	-										l	l		F	•				
1915													l		F					
]														F	- ·				
464-]	-													E					l
1910	-	-													F	<u>-</u>				Taribi control
	-	- -													þ					
1905		-				······································						\perp				-				

JJL

NORTH CAROLINA DEPARTMENT OF TRANSPORTATION DIVISION OF HIGHWAYS-MATERIALS AND TESTS UNIT SOILS TEST REPORT-SOILS LABORATORY

T.I.P. ID #: B-	4122	·····		·····				
				······································			· · · · · · · · · · · · · · · · · · ·	
REPORT ON SAMPL	ES OF: Soil	ls for Classifi	cation	····				
PROJECT:	33475.1.1	C	DUNTY: G	raham	O	wner:		
DATE SAMPLED:	4-5-05		RECEIVED:	4-12-05		TE REPORTE	ED: 4-22-0	05
SAMPLED FROM:	Bridge		SAMP	LED BY:	C. A. Dunna	igan		
SUBMITTED BY:	W. D. Frye				2002	STANDARD	SPECIFICA	TION
LABORATORY:	Asheville			·····				
	and the second s							
			TEST RE	ESULTS				
Project Sample No.	SS-1	SS-2	SS-3	SS-4	SS-5	SS-6	SS-7	SS-8
Lab Sample No. A	148686	148687	148688	148689	148690	148691	148692	148693
HiCAMS Sample #								
Retained #4 Sieve %								
Passing #10 Sieve %	29	100	100	98	78	100	100	98
Passing #40 Sieve %	17	85	86	85	75	91	98	85
Passing #200 Sieve %	8	47	45	45	57	27	33	25
		M	INUS #10 I	FRACTIO	N		·	
Soil Mortar - 100%								
Coarse Sand -Ret. #60	55	22	21	21	9	17	11	34
Fine Sand - Ret. #270	30	55	57	57	23	68	71	47
Silt 0.05-0.005 mm %	11	19	18	18	22	9	14	13
Clay < 0.005 mm %	4	4	4	4	46	6	4	6
Passing # 40 Sieve % Passing # 200 Sieve %								
rassing # 200 Sieve 70							L	
	· .		y				·	
Liquid Limit	23	38	36	37	38	49	35	25
Plastic Index	NP	NP	NP	NP	NP	NP	NP	NP
AASHTO Classification	A-1-a (0)	. A-4 (2)	A-4 (2)	A-4 (2)	A-6 (7)	A-2-5 (0)	A-2-4 (0)	A-2-4 (0)
Quantity	 	·				_		
Texture Station	12+86	12+86	12+86	12+70	12 144 5	12,144.2	12+44-2	12155
Hole No.	1.2780	12+80	12+80	12+/0	13+44.5	13+44.3	13+44.3	12+55
Depth (ft) From:	0.0	1.7	16.7	19.5	3.6	13.6	23.6	13.9
To:	1.5	3.2	18.2	21.0	5.1	15.0	25.1	15.4
2 01		J.4	10.2	21.0	3.1	13.1	43.1	13.4
Remarks:			**************************************	landra a valent				<u> </u>
A-148686 - 148693							······································	
CC:			***************************************		·			
C. A. Dunnagan								
File							***************************************	
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SOILS ENGINEER:				***************************************				

JJL NORTH CAROLINA DEPARTMENT OF TRANSPORTATION DIVISION OF HIGHWAYS-MATERIALS AND TESTS UNIT

REPORT ON SAMPLES OF: Soils for Classification		3011	rs irsi i	NET OKI-	SUILS LA	IDUKA I	OKI			
PROJECT: 33475.1.1 (cont.) COUNTY: Graham	T.I.P. ID #: B-	4122								
DATE SAMPLED: 4-5-05 SAMPLED BY: DATE REPORTED: 4-12-05 SAMPLED FROM: Bridge SAMPLED BY: C. A. Dunnagan SUBMITTED BY: W. D. Frye 2002 STANDARD SPECIFICATION	REPORT ON SAMPL	ES OF: Soils	for Classific	eation						
SAMPLED FROM: Bridge SAMPLED BY: C. A. Dunnagm 2002 STANDARD SPECIFICATION SUBMITTED BY: W. D. Frye 2002 STANDARD SPECIFICATION		it.) CO	UNTY: G	raham		Owner:		-		
SUBMITTED BY: W. D. Frye	DATE SAMPLED:	4-5-05	DATE I	RECEIVED:	4-12-05	I	DATE REPORT	ΓED:	4-22-05	
SUBMITTED BY: Asheville	SAMPLED FROM:	Bridge		SAMI	PLED BY:	C. A. Dui	nnagan			***************************************
TEST RESULTS Project Sample No. SS-9	SUBMITTED BY:	W. D. Frye						D SPE	CIFICATI	ON
Project Sample No. SS-9	LABORATORY:	Asheville								
Lab Sample No. A 148694 HICAMS Sample # Passing #10 Sieve % Passing #10 Sieve % 95 Passing #10 Sieve % 85 Passing #200 Sieve % 29 MINUS #10 FRACTION Soil Mortar - 100% Coarse Sand -Ret. #60 28 Fine Sand - Ret. #270 53 Sift .005-0.005 mm % 13 Clay c 0.005 mm % 6 Passing #40 Sieve % Passing #200 Sieve % Passing #200 Sieve % Passing #200 Sieve % Passing #100 Sieve % Passing #100 Sieve % Passing #100 Sieve % Passing #200 Sieve % P	· .			TEST RI	ESULTS					
HICAMS Sample #										
Retained #4 Sieve %		148694								
Passing #10 Sieve % 95 95 95 95 95 95 95										
Passing #40 Sieve % 85										
Passing #200 Sieve % 29										
MINUS #10 FRACTION Coarse Sand - Ret. #60 28										
Soil Mortar - 100% Coarse Sand -Ret. #60 28	Passing #200 Sieve %	29								~~~
Coarse Sand - Ret. #60 28		. '	MI	INUS #10]	FRACTIO	N				
Fine Sand - Ret. #270										
Silt 0.05-0.005 mm % 13	Coarse Sand -Ret. #60									
Clay < 0.005 mm % 6	Fine Sand - Ret. #270	53								
Passing # 40 Sieve %	Silt 0.05-0.005 mm %	13								
Passing # 200 Sieve %	Clay < 0.005 mm %	6								
Liquid Limit 41 Plastic Index NP AASHTO Classification A-2-5 (0) Quantity Texture Station 12+55 Hole No. Depth (ft) From: 28.9 To: 30.1 Remarks: A-148694 CCC: C. A. Dunnagan File							•			
Plastic Index	Passing # 200 Sieve %	<u></u>								· ·
Plastic Index										
AASHTO Classification	Liquid Limit	41						T		
Quantity		NP		***************************************						
Quantity	AASHTO Classification	A-2-5 (0)								
Texture	Quantity			***************************************						
Hole No. Depth (ft) From: 28.9 To: 30.1 Remarks: A-148694 CC: C. A. Dunnagan File										
Depth (ft) From: 28.9 To: 30.1 Remarks: A-148694 CC: C. A. Dunnagan File	Station	12+55								
To: 30.1	Hole No.									
Remarks: A-148694 CC: C. A. Dunnagan File	Depth (ft) From:	28.9								
A-148694 CC: C. A. Dunnagan File	To:	30.1								
A-148694 CC: C. A. Dunnagan File		<u> </u>								
CC: C. A. Dunnagan File										
C. A. Dunnagan File	A-148694							,		,
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SOILS ENGINEER:				***************************************						
SOILS ENGINEER:		· · · · · · · · · · · · · · · · · · ·	***************************************							
SOILS ENGINEER:										
	SOILS ENGINEER:									

FIELD SCOUR REPORT

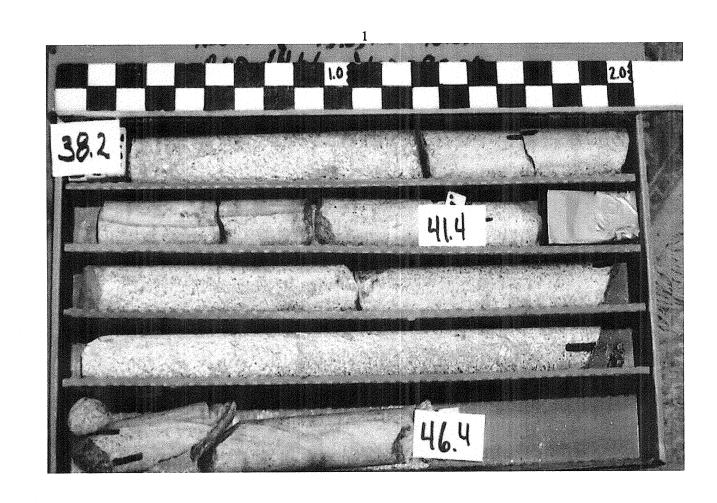
			EXISTING	BRIDGE	
Information from:	Field In Other	spection (explain)	X Micro	ofilm (reel	pos:)
Bridge No.: 81 Soundation Type:	Length:	40.7'	Total Bents: 2	Bents in Channel:0	Bents in Floodplain: 2
EVIDENCE OF SC Abutments or End	d Bent Slopes:	4,,	**		
Interior Bents: N		,			
Channel Bed: No	one noted.				
Channel Bank: No					
EXISTING SCOUR Type(3): Pi			valls.		
					0'.

INSTRUCTIONS

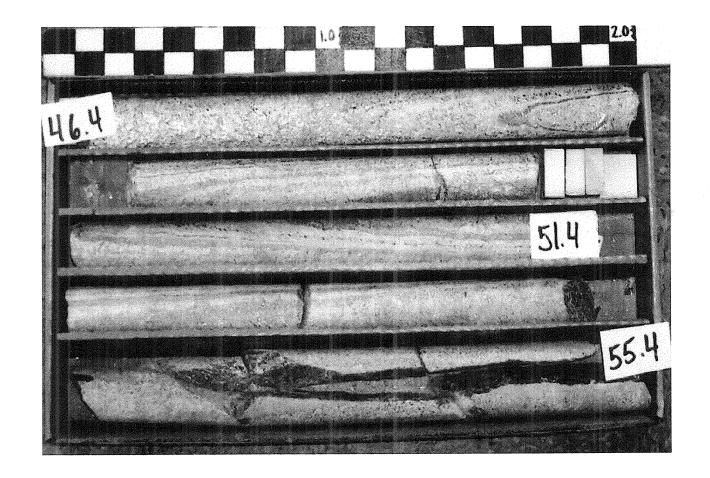
- 1 Describe the specific site's location, including route number and body of water crossed.
- 2 Note scour evidence at existing end bents or abutments (e.g. undermining, sloughing, degradations).
- 3 Note existing scour protection (e.g. rip rap).
- 4 Describe extent of existing scour protection.
- 5 Describe whether or not the scour protection appears to be working.
- 6 Note obstructions such as dams, fallen trees, debris at bents, etc.
- 7 Describe the channel bed material based on observation and/or samples. Include any lab results with report.
- 8 Describe the channel bank material based on observation and/or samples. Include any lab results with report.
- 9 Describe the material covering the banks (e.g. grass, trees, rip rap, none).
- 10 Determine the approximate floodplain width from field observation or a topographic map.
- 11 Describe the material covering the floodplain (e.g. grass, trees, crops).
- 12 Use professional judgement to specify if the stream is degrading, aggrading, or static.
- 13 Describe potential and direction of the stream to migrate laterally during the bridge's life (approx. 100 years).
- Give the design scour elevation (DSE) expected over the life of the bridge (approx. 100 years). This elevation can be given as a range across the site, or for each bent. Discuss the relationship between the Hydraulics Unit theoritical scour and the DSE. If the DSE is dependent on scour counter measures, explain (e.g. rip rap armoring on slopes). The DSE is based on the erodability of materials, giving consideration to the influence of joints, foliation, bedding characteristics, % core recovery, % RQD, differential weathering, shear strength, observations at existing structures, other tests deemed appropriate, and overall geologic conditions at the site.

Template Revised 02/07/06

DESIGN INFORMATION	
Channel Bed Material(7): Sand, gravel, cobbles and boulders.	THE PARTY OF THE P
Channel Bank Material(8): Silty sand with gravel.	
Channel Bank Cover(0): Trace and brush	
Channel Bank Cover(9): <u>Trees and brush.</u>	
Floodplain Width(10): >100' on either side.	
Floodplain Cover(11): Grass.	
Stream is(12): Aggrading Degrading X Static	
Channel Migration Tendency(13): East.	
Observations and Other Comments: Most of floodplain on EB1-B and EB2-B side probably has had	
several feet of fill placed upon it.	***************************************
	The -
Reported by: C A Dunnagan Date: 3/29/2005	-
DESIGN SCOUR ELEVATIONS(14) Feet X Meters	
· · · · · · · · · · · · · · · · · · ·	
BENTS BA	
B1 DSE 1971	
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Comparison of DSE to Hydraulics Unit theoretical scour:	
We concur with Hydraulics Unit's theoretical scour elevation FOR THE Interior Bent as presented in the Hydraulic	
Design Report dated November 2008. The End Bents will not be affected.	
DSE determined by: C A Dunnagan Date: 3/31/2009	_
SOIL ANALYSIS RESULTS FROM CHANNEL BED AND BANK MATERIAL	
Bed or Bank	1
Sample No.	
Retained #4	1
Passed #10 Passed #40	1
Passed #40 Passed #200	1
Coarse Sand	1
Fine Sand	1
Silt	
Clay	-
LL Pl	1
AASHTO	1
Station	1
Offset	1
Depth]



33475.1.1 (B-4122)
Graham County
Bridge No. 81 on SR-1117
Over Long Creek
B1-A
Box 1 of 2



33475.1.1 (B-4122)
Graham County
Bridge No. 81 on SR-1117
Over Long Creek
B1-A
Box 2 of 2