

**PROJECT SPECIAL PROVISIONS****ROADWAY****CLEARING AND GRUBBING - METHOD III:**

(4-6-06) (Rev. 1-17-12)

200

SP2 R02B

Perform clearing on this project to the limits established by Method "III" shown on Standard Drawing No. 200.03 of the *2012 Roadway Standard Drawings*.

**SHOULDER AND FILL SLOPE MATERIAL:**

(5-21-02)

235, 560

SP2 R45 A

**Description**

Perform the required shoulder and slope construction for this project in accordance with the applicable requirements of Section 560 and Section 235 of the *2012 Standard Specifications*.

**Measurement and Payment**

Where the material has been obtained from an authorized stockpile or from a borrow source and *Borrow Excavation* is not included in the contract, no direct payment will be made for this work, as the cost of this work will be part of the work being paid at the contract lump sum price for *Grading*. If *Borrow Excavation* is included in this contract and the material has been obtained from an authorized stockpile or from a borrow source, measurement and payment will be as provided in Section 230 of the *2012 Standard Specifications* for *Borrow Excavation*.

**PIPE INSTALLATION:**

(11-20-12)

300

SP3 R01

Revise the *2012 Standard Specifications* as follows:

**Page 3-1, Article 300-2, Materials**, line 23-24, replace sentence with:

Provide foundation conditioning geotextile in accordance with Section 1056 for Type 4 geotextile.

**BRIDGE APPROACH FILLS:**

(10-19-10) (Rev. 1-17-12)

422

SP4 R02

**Description**

Bridge approach fills include bridge approach fills for sub regional tier bridges and reinforced bridge approach fills. Construct bridge approach fills in accordance with the contract and Standard Drawing No. 422.10 or 422.11 of the *2012 Roadway Standard Drawings*. Define "geosynthetics" as geotextiles or geomembranes.

**Materials**

Refer to Division 10 of the *2012 Standard Specifications*.

| <b>Item</b>                   | <b>Section</b> |
|-------------------------------|----------------|
| Anchor Pins                   | 1056-2         |
| Geotextiles                   | 1056           |
| Portland Cement Concrete      | 1000           |
| Select Material               | 1016           |
| Subsurface Drainage Materials | 1044           |
| Wire Staples                  | 1060-8(D)      |

For bridge approach fills for sub regional tier bridges, provide Type 1 geotextile for filtration geotextiles. For reinforced bridge approach fills, provide Type 5 geotextile for geotextile reinforcement and Type 1 geotextile and No. 78M stone for drains. Use Class B concrete for concrete pads.

Use Class III or V select material for reinforced bridge approach fills and only Class V select material (standard size No. 78M stone) for bridge approach fills for sub regional tier bridges. Provide PVC pipes, fittings and outlet pipes for subsurface drainage materials. For drains and PVC pipes behind end bents, use pipes with perforations that meet AASHTO M 278.

Use PVC, HDPE or linear low density polyethylene (LLDPE) geomembranes for reinforced bridge approach fills. For PVC geomembranes, provide grade PVC30 geomembranes that meet ASTM D7176. For HDPE and LLDPE geomembranes, use geomembranes with a nominal thickness of at least 30 mils that meet Geosynthetic Research Institute Standard Specifications GM13 or GM17, respectively. Handle and store geomembranes in accordance with Article 1056-2 of the *2012 Standard Specifications*. Provide material certifications for geomembranes in accordance with Article 1056-3 of the *2012 Standard Specifications*.

**Construction Methods**

Excavate as necessary for bridge approach fills in accordance with the contract. Notify the Engineer when foundation excavation is complete. Do not place geomembranes or filtration geotextiles until excavation dimensions and foundation material are approved. Attach geomembranes and filtration geotextiles to end bent cap back and wing walls with adhesives, tapes or other approved methods. Glue or weld geomembrane seams to prevent leakage.

For reinforced bridge approach fills, place geotextile reinforcement within 3" of locations shown in Standard Drawing No. 422.10 of the *2012 Roadway Standard Drawings* and in slight tension free of kinks, folds, wrinkles or creases. Install geotextile reinforcement with the orientation, dimensions and number of layers shown in Standard Drawing No. 422.10 of the *2012 Roadway Standard Drawings*. Place first layer of geotextile reinforcement directly on geomembranes with no void or material in between. Install geotextile reinforcement with the machine direction (MD) parallel to the roadway centerline. The MD is the direction of the length or long dimension of the geotextile roll. Do not splice or overlap geotextile reinforcement in the MD so

seams are perpendicular to the roadway centerline. Wrap geotextile reinforcement at end bent cap back and wing walls as shown in Standard Drawing No. 422.10 of the *2012 Roadway Standard Drawings* and directed by the Engineer. Extend geotextile reinforcement at least 4 ft back behind end bent cap back and wing walls into select material.

Overlap adjacent geotextiles at least 18" with seams oriented parallel to the roadway centerline. Hold geotextiles in place with wire staples or anchor pins as needed. Contact the Engineer when existing or future obstructions such as foundations, pavements, pipes, inlets or utilities will interfere with geosynthetics.

For reinforced bridge approach fills, construct one foot square drains consisting of 4" diameter continuous perforated PVC pipes surrounded by No. 78M stone wrapped in Type 1 geotextiles. Install drains in accordance with Standard Drawing No. 422.10 of the *2012 Roadway Standard Drawings*. For bridge approach fills for sub regional tier bridges, install 4" diameter continuous perforated PVC drain pipes in accordance with Standard Drawing No. 422.11 of the *2012 Roadway Standard Drawings*.

Use solvent cement to connect PVC pipes so joints do not leak. Connect perforated pipes to outlet pipes just behind wing walls. Provide drain pipes and drains with positive drainage towards outlets. Place pipe sleeves in or under wing walls for outlet pipes so positive drainage is maintained. Use sleeves that can withstand wing wall loads.

Place select material in 8" to 10" thick lifts. Use only hand operated compaction equipment to compact select material for bridge approach fills. Compact Class III select material in accordance with Subarticle 235-3(C) of the *2012 Standard Specifications*. Compact No. 78M stone with a vibratory compactor to the satisfaction of the Engineer. Do not displace or damage geosynthetics, drain pipes or drains when placing and compacting select material. End dumping directly on geosynthetics is not permitted. Do not operate heavy equipment on geosynthetics, drain pipes or drains until they are covered with at least 8" of select material. Replace any damaged geosynthetics, drain pipes or drains to the satisfaction of the Engineer.

Cover open ends of outlet pipes with rodent screens as shown in Standard Drawing No. 815.03 of the *2012 Roadway Standard Drawings*. Connect ends of outlet pipes to concrete pads or existing drainage structures as directed by the Engineer. Construct concrete pads with an Ordinary surface finish that meets Subarticle 825-6(B) of the *2012 Standard Specifications*.

### **Measurement and Payment**

*Reinforced Bridge Approach Fill, Station \_\_\_\_* will be paid at the contract lump sum price. The contract lump sum price for *Reinforced Bridge Approach Fill, Station \_\_\_\_* will be full compensation for labor, tools, equipment and reinforced bridge approach fill materials, excavating, backfilling, hauling and removing excavated materials, compacting select material, connecting outlet pipes to existing drainage structures and supplying select materials, geosynthetics, drains, pipe sleeves and outlet components and any incidentals necessary to construct all reinforced bridge approach fills at each bridge.

*Bridge Approach Fill - Sub Regional Tier, Station \_\_\_\_\_* will be paid at the contract lump sum price. The contract lump sum price for *Bridge Approach Fill - Sub Regional Tier, Station \_\_\_\_\_* will be full compensation for labor, tools, equipment and bridge approach fill materials, excavating, backfilling, hauling and removing excavated materials, compacting No. 78M stone, connecting outlet pipes to existing drainage structures and supplying No. 78M stone, filtration geotextiles, drain pipes, pipe sleeves and outlet components and any incidentals necessary to construct all bridge approach fills at each sub regional tier bridge.

Payment will be made under:

| <b>Pay Item</b>   | <b>Pay Unit</b> |
|---|-----------------|
| Reinforced Bridge Approach Fill, Station _____          | Lump Sum        |
| Bridge Approach Fill - Sub Regional Tier, Station _____ | Lump Sum        |

**ASPHALT PAVEMENTS - SUPERPAVE:**

(6-19-12)

605

SP6 R01

Revise the *2012 Standard Specifications* as follows:

**Page 6-3, Article 605-7 APPLICATION RATES AND TEMPERATURES**, replace this article, including Table 601-1, with the following:

Apply tack coat uniformly across the existing surface at target application rates shown in Table 605-1.

**TABLE 605-1  
APPLICATION RATES FOR TACK COAT**

| Existing Surface           | Target Rate (gal/sy) |
|----------------------------|----------------------|
|                            | Emulsified Asphalt   |
| New Asphalt                | 0.04 ± 0.01          |
| Oxidized or Milled Asphalt | 0.06 ± 0.01          |
| Concrete                   | 0.08 ± 0.01          |

Apply tack coat at a temperature within the ranges shown in Table 605-2. Tack coat shall not be overheated during storage, transport or at application.

**TABLE 605-2  
APPLICATION TEMPERATURE FOR TACK COAT**

| Asphalt Material                 | Temperature Range |
|----------------------------------|-------------------|
| Asphalt Binder, Grade PG 64-22   | 350 - 400°F       |
| Emulsified Asphalt, Grade RS-1H  | 130 - 160°F       |
| Emulsified Asphalt, Grade CRS-1  | 130 - 160°F       |
| Emulsified Asphalt, Grade CRS-1H | 130 - 160°F       |
| Emulsified Asphalt, Grade HFMS-1 | 130 - 160°F       |
| Emulsified Asphalt, Grade CRS-2  | 130 - 160°F       |

**Page 6-18, Article 610-1 DESCRIPTION**, lines 40-41, delete the last sentence of the last paragraph.

**Page 6-19, Subarticle 610-3(A) Mix Design-General**, line 5, add the following as the first paragraph:

Warm mix asphalt (WMA) is allowed for use at the Contractor's option in accordance with the NCDOT Approved Products List for WMA Technologies available at:  
<http://www.ncdot.org/doh/operations/materials/pdf/wma.pdf>.

**ASPHALT BINDER CONTENT OF ASPHALT PLANT MIXES:**

(11-21-00) (Rev. 7-17-12)

609

SP6 R15

The approximate asphalt binder content of the asphalt concrete plant mixtures used on this project will be as follows:

|                                      |               |      |
|--------------------------------------|---------------|------|
| Asphalt Concrete Base Course         | Type B 25.0__ | 4.4% |
| Asphalt Concrete Intermediate Course | Type I 19.0__ | 4.8% |
| Asphalt Concrete Surface Course      | Type S 4.75A  | 6.8% |
| Asphalt Concrete Surface Course      | Type SA-1     | 6.8% |
| Asphalt Concrete Surface Course      | Type SF 9.5A  | 6.7% |
| Asphalt Concrete Surface Course      | Type S 9.5__  | 6.0% |
| Asphalt Concrete Surface Course      | Type S 12.5__ | 5.6% |

The actual asphalt binder content will be established during construction by the Engineer within the limits established in the *2012 Standard Specifications*.

**ASPHALT PLANT MIXTURES:**

(7-1-95)

609

SP6 R20

Place asphalt concrete base course material in trench sections with asphalt pavement spreaders made for the purpose or with other equipment approved by the Engineer.

**PRICE ADJUSTMENT - ASPHALT BINDER FOR PLANT MIX:**

(11-21-00)

620

SP6 R25

Price adjustments for asphalt binder for plant mix will be made in accordance with Section 620 of the *2012 Standard Specifications*.

The base price index for asphalt binder for plant mix is \$ **559.33** per ton.

This base price index represents an average of F.O.B. selling prices of asphalt binder at supplier's terminals on **November 1, 2012**.

**FINAL SURFACE TESTING NOT REQUIRED:**

(5-18-04) (Rev. 5-15-12)

610

SP6 R45

Final surface testing is not required on this project.

**GUARDRAIL ANCHOR UNITS, TYPE 350:**

(4-20-04) (Rev. 8-16-11)

862

SP8 R65

**Description**

Furnish and install guardrail anchor units in accordance with the details in the plans, the applicable requirements of Section 862 of the *2012 Standard Specifications*, and at locations shown in the plans.

**Materials**

The Contractor may at his option, furnish any one of the guardrail anchor units or approved equal.

Guardrail anchor unit (ET-Plus) as manufactured by:

Trinity Industries, Inc.  
2525 N. Stemmons Freeway  
Dallas, Texas 75207  
Telephone: 800-644-7976

The guardrail anchor unit (SKT 350) as manufactured by:

Road Systems, Inc.  
3616 Old Howard County Airport  
Big Spring, Texas 79720  
Telephone: 915-263-2435

Prior to installation the Contractor shall submit to the Engineer:

- (A) FHWA acceptance letter for each guardrail anchor unit certifying it meets the requirements of NCHRP Report 350, Test Level 3, in accordance with Article 106-2 of the *2012 Standard Specifications*.
- (B) Certified working drawings and assembling instructions from the manufacturer for each guardrail anchor unit in accordance with Article 105-2 of the *2012 Standard Specifications*.

No modifications shall be made to the guardrail anchor unit without the express written permission from the manufacturer. Perform installation in accordance with the details in the plans, and details and assembling instructions furnished by the manufacturer.

**Construction Methods**

Guardrail end delineation is required on all approach and trailing end sections for both temporary and permanent installations. Guardrail end delineation consists of yellow reflective sheeting applied to the entire end section of the guardrail in accordance with Article 1088-3 of the *2012 Standard Specifications* and is incidental to the cost of the guardrail anchor unit.

**Measurement and Payment**

Measurement and payment will be made in accordance with Article 862-6 of the *2012 Standard Specifications*.

Payment will be made under:

**Pay Item**  
Guardrail Anchor Units, Type 350

**Pay Unit**  
Each

**MATERIALS:**

(2-21-12) (Rev. 12-18-12)

1000, 1005, 1080, 1081, 1092

SP10 R01

Revise the 2012 *Standard Specifications* as follows:**Page 10-1, Article 1000-1, DESCRIPTION, line 14, add the following:**

Use materials which do not produce a mottled appearance through rusting or other staining of the finished concrete surface.

**Page 10-5, Table 1000-1, REQUIREMENTS FOR CONCRETE, replace with the following:**

| TABLE 1000-1<br>REQUIREMENTS FOR CONCRETE |  |                            |                   |                            |                   |                                 |                    |                |           |              |           |
|---|--|----------------------------|-------------------|----------------------------|-------------------|---------------------------------|--------------------|----------------|-----------|--------------|-----------|
| Class of Concrete                         | Min. Comp. Strength at 28 days                   | Maximum Water-Cement Ratio |                   |                            |                   | Consistency Max. Slump          |                    | Cement Content |           |              |           |
|   |  | Air-Entrained Concrete     |                   | Non Air-Entrained Concrete |                   | Vibrated                        | Non-Vibrated       | Vibrated       |           | Non-Vibrated |           |
|   |  | Rounded Aggregate          | Angular Aggregate | Rounded Aggregate          | Angular Aggregate |                                 |                    | Min.           | Max.      | Min.         | Max.      |
| Units                                     | psi  |                            |                   |                            |                   | inch                            | inch               | lb/cy          | lb/cy     | lb/cy        | lb/cy     |
| AA  | 4,500  | 0.381                      | 0.426             | -                          | -                 | 3.5                             | -                  | 639            | 715       | -            | -         |
| AA Slip Form                              | 4,500  | 0.381                      | 0.426             | -                          | -                 | 1.5                             | -                  | 639            | 715       | -            | -         |
| Drilled Pier                              | 4,500  | -                          | -                 | 0.450                      | 0.450             | -                               | 5-7 dry<br>7-9 wet | -              | -         | 640          | 800       |
| A   | 3,000  | 0.488                      | 0.532             | 0.550                      | 0.594             | 3.5                             | 4                  | 564            | -         | 602          | -         |
| B   | 2,500  | 0.488                      | 0.567             | 0.559                      | 0.630             | 2.5                             | 4                  | 508            | -         | 545          | -         |
| B Slip Formed                             | 2,500  | 0.488                      | 0.567             | -                          | -                 | 1.5                             | -                  | 508            | -         | -            | -         |
| Sand Lightweight                          | 4,500  | -                          | 0.420             | -                          | -                 | 4                               | -                  | 715            | -         | -            | -         |
| Latex Modified                            | 3,000<br>7 day                                   | 0.400                      | 0.400             | -                          | -                 | 6                               | -                  | 658            | -         | -            | -         |
| Flowable Fill excavatable                 | 150 max. at 56 days                              | as needed                  | as needed         | as needed                  | as needed         | -                               | Flowable           | -              | -         | 40           | 100       |
| Flowable Fill non-excavatable             | 125  | as needed                  | as needed         | as needed                  | as needed         | -                               | Flowable           | -              | -         | 100          | as needed |
| Pavement                                  | 4,500 design, field<br>650 flexural, design only | 0.559                      | 0.559             | -                          | -                 | 1.5 slip form<br>3.0 hand place | -                  | 526            | -         | -            | -         |
| Precast                                   | See Table 1077-1                                 | as needed                  | as needed         | -                          | -                 | 6                               | as needed          | as needed      | as needed | as needed    | as needed |
| Prestress                                 | per contract                                     | See Table 1078-1           | See Table 1078-1  | -                          | -                 | 8                               | -                  | 564            | as needed | -            | -         |



Page 10-23, Table 1005-1, AGGREGATE GRADATION-COARSE AGGREGATE, replace with the following:

**TABLE 1005-1  
AGGREGATE GRADATION - COARSE AGGREGATE**

| Std. Size #    | Percentage of Total by Weight Passing |        |        |        |        |        |        |       |       |      |       | Remarks           |  |
|----------------|---------------------------------------|--------|--------|--------|--------|--------|--------|-------|-------|------|-------|-------------------|--|
|                | 2"                                    | 1 1/2" | 1"     | 3/4"   | 1/2"   | 3/8"   | #4     | #8    | #10   | #16  | #40   |                   | #200   |
| 4              | 100                                   | 90-100 | 20-55  | 0-15   | -      | 0-5    | -      | -     | -     | -    | -     | A                 | Asphalt Plant Mix  |
| 467M           | 100                                   | 95-100 | -      | 35-70  | -      | 0-30   | 0-5    | -     | -     | -    | -     | A                 | Asphalt Plant Mix  |
| 5              | -                                     | 100    | 90-100 | 20-55  | 0-10   | 0-5    | -      | -     | -     | -    | -     | A                 | AST, Sediment Control Stone                                |
| 57             | -                                     | 100    | 95-100 | -      | 25-60  | -      | 0-10   | 0-5   | -     | -    | -     | A                 | AST, Str. Concrete, Shoulder Drain, Sediment Control Stone |
| 57M            | -                                     | 100    | 95-100 | -      | 25-45  | -      | 0-10   | 0-5   | -     | -    | -     | A                 | AST, Concrete Pavement                                     |
| 6M             | -                                     | -      | 100    | 90-100 | 20-55  | 0-20   | 0-8    | -     | -     | -    | -     | A                 | AST  |
| 67             | -                                     | -      | 100    | 90-100 | -      | 20-55  | 0-10   | 0-5   | -     | -    | -     | A                 | AST, Str. Concrete, Asphalt Plant Mix                      |
| 78M            | -                                     | -      | -      | 100    | 98-100 | 75-100 | 20-45  | 0-15  | -     | -    | -     | A                 | Asphalt Plant Mix, AST, Str. Conc, Weep Hole Drains        |
| 14M            | -                                     | -      | -      | -      | -      | 100    | 35-70  | 5-20  | -     | 0-8  | -     | A                 | Asphalt Plant Mix, AST, Weep Hole Drains, Str. Concrete    |
| 9              | -                                     | -      | -      | -      | -      | 100    | 85-100 | 10-40 | -     | 0-10 | -     | A                 | AST  |
| ABC            | -                                     | 100    | 75-97  | -      | 55-80  | -      | 35-55  | -     | 25-45 | -    | 14-30 | 4-12 <sup>B</sup> | Aggregate Base Course, Aggregate Stabilization             |
| ABC (M)        | -                                     | 100    | 75-100 | -      | 45-79  | -      | 20-40  | -     | 0-25  | -    | -     | 0-12 <sup>B</sup> | Maintenance Stabilization                                  |
| Light-weight C | -                                     | -      | -      | -      | 100    | 80-100 | 5-40   | 0-20  | -     | 0-10 | -     | 0-2.5             | AST  |

A. See Subarticle 1005-4(A).  
 B. See Subarticle 1005-4(B).  
 C. For Lightweight Aggregate used in Structural Concrete, see Subarticle 1014-2(E)(6).

**Page 10-126, Table 1078-1, REQUIREMENTS FOR CONCRETE**, replace with the following:

| <b>Property</b>                           | <b>28 Day Design<br/>Compressive<br/>Strength<br/>6,000 psi or less</b> | <b>28 Day Design<br/>Compressive<br/>Strength<br/>greater than<br/>6,000 psi</b> |
|---|---|--|
| Maximum Water/Cementitious Material Ratio | 0.45  | 0.40   |
| Maximum Slump without HRWR                | 3.5"  | 3.5"   |
| Maximum Slump with HRWR                   | 8"  | 8"   |
| Air Content (upon discharge into forms)   | 5 + 2%  | 5 + 2%   |

**Page 10-151, Article 1080-4 Inspection and Sampling, lines 18-22**, replace (B), (C) and (D) with the following:

- (B) At least 3 panels prepared as specified in 5.5.10 of AASHTO M 300, Bullet Hole Immersion Test.
- (C) At least 3 panels of 4"x6"x1/4" for the Elcometer Adhesion Pull Off Test, ASTM D4541.
- (D) A certified test report from an approved independent testing laboratory for the Salt Fog Resistance Test, Cyclic Weathering Resistance Test, and Bullet Hole Immersion Test as specified in AASHTO M 300.
- (E) A certified test report from an approved independent testing laboratory that the product has been tested for slip coefficient and meets AASHTO M253, Class B.

**Page 10-162, Subarticle 1081-1(A) Classifications, lines 4-7**, delete the second and third sentences of the description for Type 3A.

**Page 10-162, Subarticle 1081-1(B) Requirements, lines 26-30**, replace the second paragraph with the following:

For epoxy resin systems used for embedding dowel bars, threaded rods, rebar, anchor bolts and other fixtures in hardened concrete, the manufacturer shall submit test results showing that the bonding system will obtain 125% of the specified required yield strength of the fixture. Furnish certification that, for the particular bolt grade, diameter and embedment depth required, the anchor system will not fail by adhesive failure and that there is no movement of the anchor bolt. For certification and anchorage, use 3,000 psi as the minimum Portland cement concrete compressive strength used in this test. Use adhesives that meet Section 1081.

List the properties of the adhesive on the container and include density, minimum and maximum temperature application, setting time, shelf life, pot life, shear strength and compressive strength.

**Page 10-169, Subarticle 1081-3(G) Anchor Bolt Adhesives, delete this subarticle.**

**Page 10-204, Subarticle 1092-2(A) Performance and Test Requirements, replace Table 1092-3 Minimum Coefficient of Retroreflection for NC Grade A with the following:**

**TABLE 1092-3  
MINIMUM COEFFICIENT OF RETROREFLECTION FOR NC GRADE A  
(Candelas Per Lux Per Square Meter)**

| Observation Angle, degrees | Entrance Angle, degrees | White | Yellow | Green | Red | Blue | Fluorescent Yellow Green | Fluorescent Yellow |
|----------------------------|-------------------------|-------|--------|-------|-----|------|--------------------------|--------------------|
| 0.2                        | -4.0                    | 525   | 395    | 52    | 95  | 30   | 420                      | 315                |
| 0.2                        | 30.0                    | 215   | 162    | 22    | 43  | 10   | 170                      | 130                |
| 0.5                        | -4.0                    | 310   | 230    | 31    | 56  | 18   | 245                      | 185                |
| 0.5                        | 30.0                    | 135   | 100    | 14    | 27  | 6    | 110                      | 81                 |
| 1.0                        | -4.0                    | 120   | 60     | 8     | 16  | 3.6  | 64                       | 48                 |
| 1.0                        | 30.0                    | 45    | 34     | 4.5   | 9   | 2    | 36                       | 27                 |

**SELECT MATERIAL, CLASS III, TYPE 3:**

(1-17-12)

1016, 1044

SP10 R05

Revise the 2012 Standard Specifications as follows:

**Page 10-39, Article 1016-3, CLASS III, add the following after line 14:**

**Type 3 Select Material**

Type 3 select material is a natural or manufactured fine aggregate material meeting the following gradation requirements and as described in Sections 1005 and 1006:

| Percentage of Total by Weight Passing |        |        |       |       |      |      |      |
|---------------------------------------|--------|--------|-------|-------|------|------|------|
| 3/8"                                  | #4     | #8     | #16   | #30   | #50  | #100 | #200 |
| 100                                   | 95-100 | 65-100 | 35-95 | 15-75 | 5-35 | 0-25 | 0-8  |

**Page 10-39, Article 1016-3, CLASS III, line 15, replace “either type” with “Type 1, Type 2 or Type 3”.**

**Page 10-62, Article 1044-1, line 36, delete the sentence and replace with the following:**

Subdrain fine aggregate shall meet Class III select material, Type 1 or Type 3.

**Page 10-63, Article 1044-2, line 2, delete the sentence and replace with the following:**

Subdrain coarse aggregate shall meet Class V select material.