DEPARTMENT OF TRANSPORTATION **DIVISION OF HIGHWAYS** GEOTECHNICAL ENGINEERING UNIT

STATE OF NORTH CAROLINA

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STRUCTURE SUBSURFACE INVESTIGATION

PROJ. REFERENCE NO. 38500.1.1 (B-4726) F.A. PROJ. *BRZ-1723(7)* COUNTY CASWELL PROJECT DESCRIPTION BRIDGE NO. 5 OVER PANTHER BRANCH CREEK ON SR 1723 (JOHN OAKLEY RD.) AT -L- STATION 21+18

INVENTORY

STATE PROJECT REFERENCE NO. 38500.1.1 (B-4726)

CAUTION NOTICE

THE SUBSURFACE INFORMATION AND THE SUBSUBFACE INVESTIGATION ON WHICH IT IS BASED WERE MADE FOR THE PURPOSE OF STUDY, PLANNING, AND DESIGN, AND NOT FOR CONSTRUCTION OR PAY PURPOSES. THE VARIOUS FIELD BORNING LOGS, ROCK CORES, AND SOL TEST DATA AVAILABLE MAY BE REVIEWED OR INSPECTED IN RALEGIE BY CONTACTING THE N. C. DEPARTMENT OF TRANSPORTATION, GEOTECHNICAL ENGINEERING UNIT AT 1993 920-4088. RETHER THE SUBSUFFACE PLANS AND REPORTS, NOR THE FIELD BORNING LOGS, ROCK CORES, OR SOIL TEST DATA ARE PART OF THE CONTRACT.

GENERAL SOIL AND ROCK STRATA DESCRIPTIONS AND INDICATED BOUNDARIES ARE BASED ON A GEOTECHNICAL INTERPRETATION OF ALL AVAILABLE. SUBSURFACE DATA AND MAY NOT NECESSARILY GEOTECHNICAL INTERPRETATION OF ALL AVAILABLE SUBSURFACE DATA AND MAY NOT NECESSARILY REFLECT THE ACTUAL SUBSURFACE CONDITIONS DETWEEN BORINGS OR BETWEEN SAMPLED STRATA WITHIN THE BOREHOLE. THE LABORATORY SAMPLE DATA AND THE IN SITU (IN-PLACE) TEST DATA CAN BE RELIED ON ONLY TO THE DEGREE OF RELIABILITY INHERENT IN THE STANDARD TEST METHOD. THE OBSERVED WATER LEVELS OR SOIL MOISTURE CONDITIONS INDICATED IN THE SUBSURFACE INVESTIGATIONS ARE AS RECORDED AT THE TIME OF THE INVESTIGATION. THESE WATER LEVELS OR SOIL MOISTURE CONDITIONS AND ASY CONSIDERABLY WITH TIME ACCORDING TO CLIMATIC CONDITIONS INCLUDING TEMPERATURES. PRECIPITATION, AND WIND, AS WELL AS OTHER NON-CLIMATIC FACTORS.

THE BIDDER OR CONTRACTOR IS CAUTIONED THAT DETAILS SHOWN ON THE SUBSURFACE PLANS ARE PRELIMINARY ONLY AND IN MANY CASES THE FINAL DESIGN DETAILS ARE DIFFERENT, FOR BIDDING AND CONSTRUCTION PLANS AND DOCUMENTS FOR FINAL DESIGN INFORMATION ON THIS PROJECT. THE DEPARTMENT DOES NOT WARRANT OR CUARANTEE THE SUFFICIENCY OR ACCURACY OF THE INVESTIGATION MADE, NOR THE INTERPRETATIONS MADE, OR OPINION OF THE DEPARTMENT AS TO THE TYPE OF MATERIALS AND CONDITIONS TO BE ENCOUNTERED. THE BIDDER OR CONTRACTOR IS CAUTIONED TO MAKE SUCH INDEPENDENT SUBSURFACE INVESTIGATIONS AS HE DEEMS NECESSARY TO SATISFY HIMSELF AS TO CONDITIONS TO BE ENCOUNTERED ON THIS PROJECT. THE CONTRACTOR SHALL HAVE NO CLAIM FOR ADDITIONAL COMPENSATION OR FOR AN EXTENSION OF TIME FOR ANY BEASON RESULTING FROM THE ACTUAL CONDITIONS TO BE.

PERSONNEL J.I. MILKOVITS, JR.

J.R. MATULA

J.R. TURNAGE

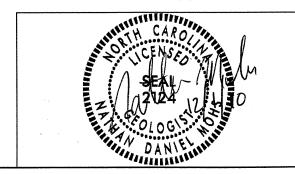
D.W. DIXON N.D. MOHS

INVESTIGATED BY N.D. MOHS

N.T. ROBERSON

SUBMITTED BY N.T. ROBERSON

DECEMBER 2010



38500.

NORTH CAROLINA DEPARTMENT OF TRANSPORTATION

DIVISION OF HIGHWAYS

GEOTECHNICAL ENGINEERING UNIT

SUBSURFACE INVESTIGATION

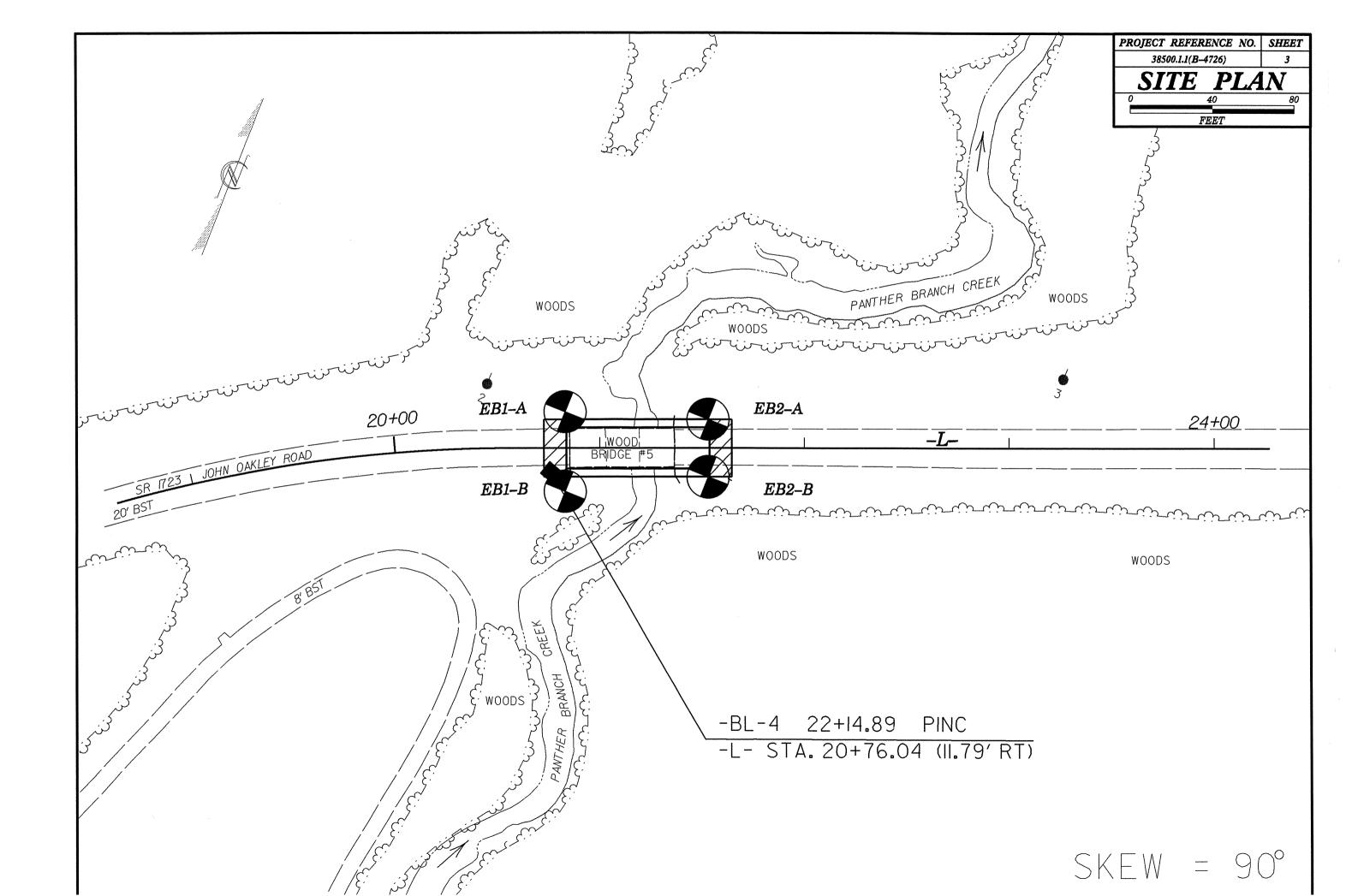
SOIL AND ROCK LEGEND, TERMS, SYMBOLS, AND ABBREVIATIONS

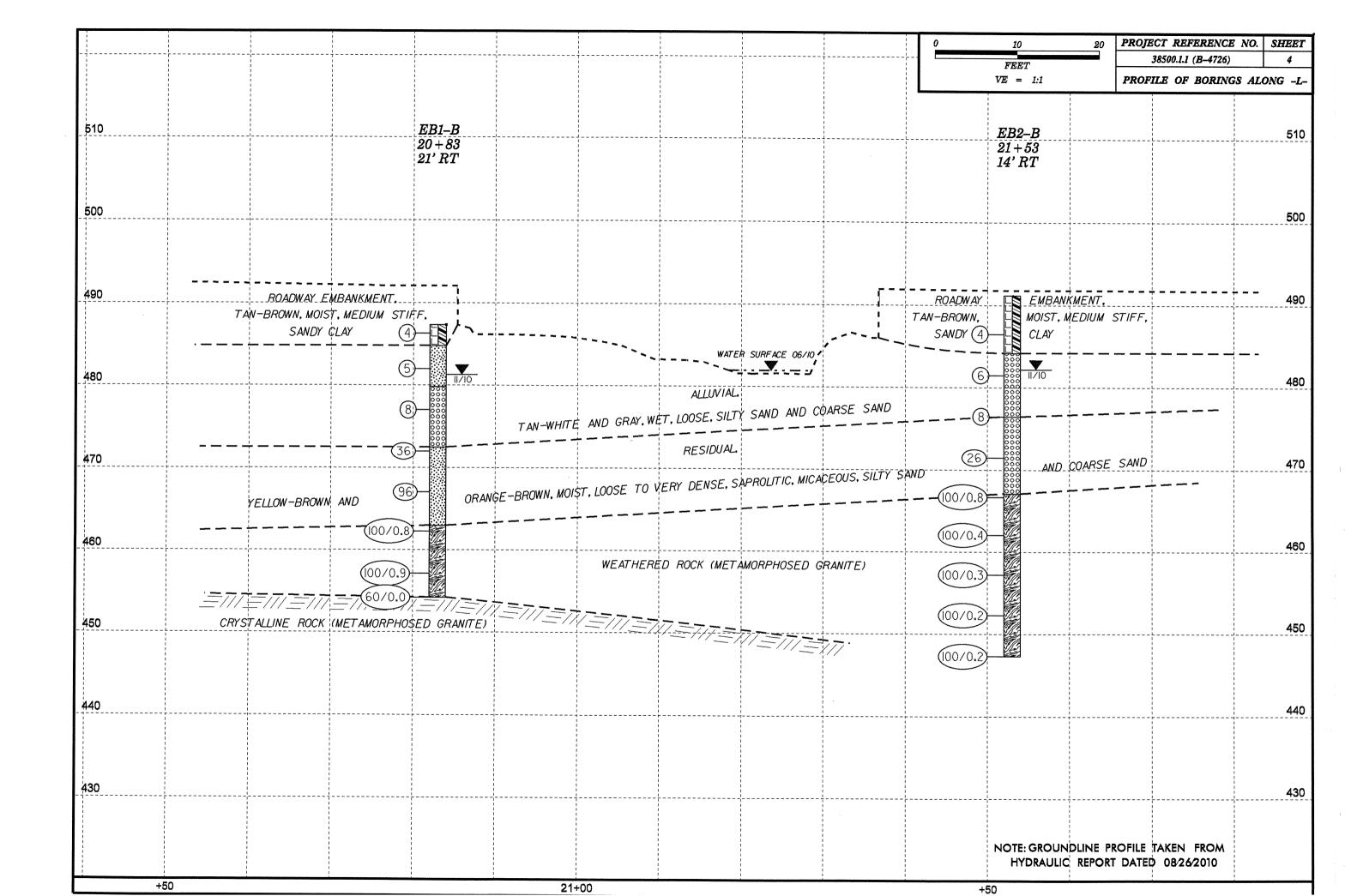
				A LEGEND, IERM	o, or Mbolo,			**************************************	
SOIL DESCRIPTION	WELL GRA	RADED - INDICATES A GOOD REPRESE	GRADATION ENTATION OF PARTICLE SIZES FR	ROM FINE TO COARSE.	HARD ROCK IS NON		DESCRIPTION AT IF TESTED, WOULD YIELD SPT REF	FUSAL, AN INFERRED	TERMS AND DEFINITIONS
SOIL IS CONSIDERED TO BE THE UNCONSOLIDATED, SEMI-CONSOLIDATED, OR WEATHERED EAR THAT CAN BE PENETRATED WITH A CONTINUOUS FLIGHT POWER AUGER, AND YIELD LESS THAT	RTH MATERIALS UNIFORM -	RADED - INDICATES A GOOD REPRESE M - INDICATES THAT SOIL PARTICLES GRADED	S ARE ALL APPROXIMATELY THE	SAME SIZE. (ALSO	ROCK LINE INDICAT	TES THE LEVEL AT WHICH NON-	COASTAL PLAIN MATERIAL WOULD YII N SAMPLER EQUAL TO DR LESS THAN	FLD SPT REFLISAL.	ALLUVIUM (ALLUV.) - SDILS THAT HAVE BEEN TRANSPORTED BY WATER.
100 BLOWS PER FOOT ACCORDING TO STANDARD PENETRATION TEST (AASHTO T206, ASTM D- CLASSIFICATION IS BASED ON THE AASHTO SYSTEM, BASIC DESCRIPTIONS GENERALLY SHALL	0-1586). SOIL GAP-GRADE	<u> ADED - INDICATES A MIXTURE DF UN</u>		DRE SIZES.	IN NON-COASTAL P OF WEATHERED RO	LAIN MATERIAL, THE TRANSITI	ION BETWEEN SOIL AND ROCK IS OFT	EN REPRESENTED BY A ZONE	ACUIFER - A WATER BEARING FORMATION OR STRATA. ARENACEOUS - APPLIED TO ROCKS THAT HAVE BEEN DERIVED FROM SAND OR THAT CONTAIN SAND.
CONSISTENCY, COLOR, TEXTURE, MOISTURE, AASHTO CLASSIFICATION, AND OTHER PERTINENT FOR MINERALOGICAL COMPOSITION, ANGULARITY, STRUCTURE, PLASTICITY, ETC. EXAMPLE:	ACTORS SUCH	ANGU NGULARITY OR ROUNDNESS OF SOIL O	JLARITY OF GRAINS	FRMS. ANGLILAR		RE TYPICALLY DIVIDED AS FOL	LOWS:		ARGILLACEOUS - APPLIED TO ALL ROCKS OR SUBSTANCES COMPOSED OF CLAY MINERALS,
VERY STIFF, BRIN, SETY CLAY, MOST WITH INTERBEDOED FINE SAID LIVERS, HIGHLY PLASTIC, A-7-6		GULAR, SUBROUNDED, OR ROUNDED.	MANO 10 DESIGNATED DI TILE I	ENTOS PROCEETS	WEATHERED ROCK (WR)		PLAIN MATERIAL THAT WOULD YIELD	SPT N VALUES > 100	DR HAVING A NOTABLE PROPORTION OF CLAY IN THEIR COMPOSITION, AS SHALE, SLATE, ETC.
SOIL LEGEND AND AASHTO CLASSIFICATION	· · · · · · · · · · · · · · · · · · ·	MINERAL	LOGICAL COMPOSITIO	N		BLOWS PER FO	UI IF TESTED. E GRAIN IGNEOUS AND METAMORPHIC	ROCK THAT	ARTESIAN - GROUND WATER THAT IS UNDER SUFFICIENT PRESSURE TO RISE ABOVE THE LEVEL AT WHICH IT IS ENCOUNTERED, BUT WHICH DOES NOT NECESSARILY RISE TO OR ABOVE THE
GENERAL GRANULAR MATERIALS SILT-CLAY MATERIALS ORG		NAMES SUCH AS DUARTZ, FELDSPAR ER THEY ARE CONSIDERED OF SIGNIF		SED IN DESCRIPTIONS	CRYSTALLINE ROCK (CR)	WOULD YIELD S GNEISS, GABBRO	PT REFUSAL IF TESTED. ROCK TYPE	INCLUDES GRANITE,	GROUND SURFACE.
CLASS. (≤ 35% PASSING *200) (> 35% PASSING *200) (SROUP A-1 A-3 A-2 A-4 A-5 A-6 A-7 A-1, A-2			OMPRESSIBILITY		NON-CRYSTALLINE	FINE TO COARS	E GRAIN METAMORPHIC AND NON-COA	STAL PLAIN	CALCAREOUS (CALC.) - SOILS THAT CONTAIN APPRECIABLE AMOUNTS OF CALCIUM CARBONATE. COLLUVIUM - ROCK FRAGMENTS MIXED WITH SOIL DEPOSITED BY GRAVITY ON SLOPE OR AT BOTTOM
CLASS. A-1-a A-1-b A-2-4 A-2-5 A-2-6 A-2-7 A-7-5 A-3	A-6, A-7	SLIGHTLY COMPRESSIBLE	LIQUID LIMIT I		ROCK (NCR)	INCLUDES PHYL	OCK THAT WOULD YEILD SPT REFUSA LITE, SLATE, SANDSTONE, ETC.		OF SLOPE.
SYMBOL DOOG GOOD CO.		MODERATELY COMPRESSIBLE HIGHLY COMPRESSIBLE		EQUAL TO 31-50 GREATER THAN 50	COASTAL PLAIN SEDIMENTARY ROCK	SPT REFUSAL. I	SEDIMENTS CEMENTED INTO ROCK, B ROCK TYPE INCLUDES LIMESTONE, SAY		CORE RECOVERY (REC.) - TOTAL LENGTH OF ALL MATERIAL RECOVERED IN THE CORE BARREL DIVIDED BY TOTAL LENGTH OF CORE RLN AND EXPRESSED AS A PERCENTAGE.
% PASSING	SILT- MUSIC		NTAGE OF MATERIAL		(CP)	SHELL BEDS, ET	c. ATHERING		DIKE - A TABULAR BODY OF IGNEOUS ROCK THAT CUTS ACROSS THE STRUCTURE OF ADJACENT
* 10 50 MX GRANULA * 40 38 MX 50 MX 51 MN SOILS	AR CLAY PEAT ORGAN	GANIC MATERIAL GRANULAR SOILS	SILT - CLAY SOILS	OTHER MATERIAL					ROCKS OR CUTS MASSIVE ROCK.
# 200 15 MX 25 MX 10 MX 35 MX 35 MX 35 MX 36 MX 36 MN 36 MN 36 MN 36 MN	TRACE OF	DF ORGANIC MATTER 2 - 3% ORGANIC MATTER 3 - 5%	3 - 5% TRAC		FRESH ROCK F HAMMEI	R IF CRYSTALLINE.	JOINTS MAY SHOW SLIGHT STAINING.	ROCK RINGS UNDER	<u>DIP</u> - THE ANGLE AT WHICH A STRATUM OR ANY PLANAR FEATURE IS INCLINED FROM THE HORIZONTAL.
LIQUID LIMIT 48 MX 41 MN 50IL	LS WITH MODERATE	TELY ORGANIC 5 - 10%	12 - 20% SOME				NED, SOME JOINTS MAY SHOW THIN C		DIP DIRECTION (DIP AZIMUTH) - THE DIRECTION OR BEARING OF THE HORIZONTAL TRACE OF
	TLE OR HIGHLY OF ORGANIC	DRGANIC >10%	>20% HIGH	ILY 35% AND ABOVE		RYSTALLINE NATURE.	ACE SHINE BRIGHTLY. ROCK RINGS UN	IDER HAMMER BLOWS IF	THE LINE OF DIP, MEASURED CLOCKWISE FROM NORTH.
HIGHAL TYPES STONE FRACS	OUNTS OF SOILS	WATER LEVEL IN DODE	GROUND WATER HOLE IMMEDIATELY AFTER DR	DILLI INC			NED AND DISCOLORATION EXTENDS IN .AY. IN GRANITOID ROCKS SOME OCCA		FAULT - A FRACTURE OR FRACTURE ZONE ALONG WHICH THERE HAS BEEN DISPLACEMENT OF THE SIDES RELATIVE TO ONE ANOTHER PARALLEL TO THE FRACTURE.
OF MAJOR GRAVEL, AND GRAVEL AND SAND SOLIS SOLIS MAT				ALLING). CRYSTALLINE ROCKS RING UNDER H		FISSILE - A PROPERTY OF SPLITTING ALONG CLOSELY SPACED PARALLEL PLANES.
MAIERIALS SANU							DISCOLORATION AND WEATHERING E		FLOAT - ROCK FRAGMENTS ON SURFACE NEAR THEIR ORIGINAL POSITION AND DISLODGED FROM
AS A EXCELLENT TO GOOD FAIR TO POOR POOR POOR	POUR UNSUITABLE	121101120 1111101110	RATED ZONE, DR WATER BEARIN	IG STRATA	DULL S	SOUND UNDER HAMMER BLOWS A	ND SHOWS SIGNIFICANT LOSS OF STE		PARENT MATERIAL. FLOOD PLAIN (FP) - LAND BORDERING A STREAM, BUILT OF SEDIMENTS DEPOSITED BY
PI OF A-7-5 SUBGROUP IS ≤ LL - 30 ; PI OF A-7-6 SUBGROUP IS ≥	>LL - 30 OM	SPRING OR SEEP				RESH ROCK. NCK EXCEPT DUARTZ DISCOLORE	D OR STAINED. IN GRANITOID ROCKS.	ALL FELDSPARS DULL	THE STREAM.
CONSISTENCY OR DENSENESS		MISC	ELLANEOUS SYMBOLS		SEVERE AND DI	SCOLORED AND A MAJORITY SH	DW KAOLINIZATION. ROCK SHOWS SEV	ERE LOSS OF STRENGTH	FORMATION (FM.) - A MAPPABLE GEOLOGIC UNIT THAT CAN BE RECOGNIZED AND TRACED IN THE FIELD.
PRIMARY SOIL TYPE CONSISTENCY PENETRATION RESISTENCE COMPE	RESSIVE STRENGTH	ROADWAY EMBANKMENT (RE)	SPT DMT TEST BORING	TEST BORING W/ CORE		TED, WOULD YIELD SPT REFUSA	.OGIST'S PICK. ROCK GIVES "CLUNK"S <u>L</u>	DUND WHEN STRUCK.	JOINT - FRACTURE IN ROCK ALONG WHICH NO APPRECIABLE MOVEMENT HAS OCCURRED.
(M-AHCOE)	(TONS/FT ²)	WITH SOIL DESCRIPTION	~	<u> </u>			D OR STAINED ROCK FABRIC CLEAR		LEDGE - A SHELF-LIKE RIDGE OR PROJECTION OF ROCK WHOSE THICKNESS IS SMALL COMPARED TO
GENERALLY VERY LOOSE 4 TO 10 GRANULAR LOOSE 4 TO 10		- SOIL SYMBOL	AUGER BORING	SPT N-VALUE	EXTEN	SOME FRAGMENTS OF STRONG	ANITOID ROCKS ALL FELDSPARS ARE G ROCK USUALLY REMAIN.	KAULINIZED TO SOME	ITS LATERAL EXTENT.
MATERIAL MEDIUM DENSE 10 TO 30	N/A B	ARTIFICIAL FILL (AF) OTHER THAN ROADWAY EMBANKMENT	- CORE BORING	REF SPT REFUSAL		STED. YIELDS SPT N VALUES >			LENS - A BODY OF SOIL OR ROCK THAT THINS OUT IN ONE OR MORE DIRECTIONS. MOTTLED (MOT.) - IRREGULARLY MARKED WITH SPOTS OF DIFFERENT COLORS, MOTTLING IN
(NON-COHESIVE) VERY DENSE >50		- INFERRED SOIL BOUNDARY	MONITORING WELL	L	(V SEV.) THE MA	ASS IS EFFECTIVELY REDUCED	D OR STAINED. ROCK FABRIC ELEMENT TO SOIL STATUS, WITH ONLY FRAGME	NTS OF STRONG ROCK	SOILS USUALLY INDICATES POOR AERATION AND LACK OF GOOD DRAINAGE.
VERY SOFT <2	<0.25		△ PIEZOMETER				E OF ROCK WEATHERED TO A DEGREE BRIC REMAIN. <i>IF TESTED, YIELDS SF</i>		PERCHED WATER - WATER MAINTAINED ABOVE THE NORMAL GROUND WATER LEVEL BY THE PRESENCE OF AN INTERVENING IMPERVIOUS STRATUM.
SILT-CLAY MEDIUM STIFF 4 TO 8	0.25 TO 0.50 0.5 TO 1.0	INFERRED ROCK LINE	INSTALLATION		1		NOT DISCERNIBLE, OR DISCERNIBLE		RESIDUAL (RES.) SOIL - SOIL FORMED IN PLACE BY THE WEATHERING OF ROCK.
MATERIAL STIFF 8 TO 15 (COHESIVE) VERY STIFF 15 TO 30	1 TO 2 2 TP++4	r ALLUVIAL SOIL BOUNDARY	SLOPE INDICATOR INSTALLATION	₹	SCATTE		MAY BE PRESENT AS DIKES OR STR		ROCK QUALITY DESIGNATION (ROD) - A MEASURE OF ROCK QUALITY DESCRIBED BY TOTAL LENGTH OF
HARD >3Ø	>4 25/925	DIP & DIP DIRECTION OF ROCK STRUCTURES	CONE PENETROME	TER TEST	HEOU P		(HARDNESS	M	ROCK SEGMENTS EQUAL TO OR GREATER THAN 4 INCHES DIVIDED BY THE TOTAL LENGTH OF CORE RUN AN EXPRESSED AS A PERCENTAGE.
TEXTURE OR GRAIN SIZE		10011 01110010120	OONE TENETHORIE	TEN TEOT	VERY HARD CANNO		SHARP PICK, BREAKING OF HAND SP	PECIMENS REQUIRES	SAPROLITE (SAP.) - RESIDUAL SOIL THAT RETAINS THE RELIC STRUCTURE OR FABRIC OF THE
U.S. STD. SIEVE SIZE 4 10 40 60 200 270 OPENING (MM) 4.76 2.00 0.42 0.25 0.075 0.05			 SOUNDING ROD 			RAL HARD BLOWS OF THE GEOLG			PARENT ROCK. SILL - AN INTRUSIVE BODY OF IGNEOUS ROCK OF APPROXIMATELY UNIFORM THICKNESS AND
OPENING (MM) 4.76 2.00 0.42 0.25 0.075 0.05 COARSE FINE			ABBREVIATIONS			BE SCRATCHED BY KNIFE OR PIC TACH HAND SPECIMEN.	CK ONLY WITH DIFFICULTY. HARD HA	MMER BLOWS REQUIRED	RELATIVELY THIN COMPARED WITH ITS LATERAL EXTENT, THAT HAS BEEN EMPLACED PARALLEL
BOULDER COBBLE GRAVEL SAND SAND			ED MEDIUM ICA MICACEOUS	VST - VANE SHEAR TEST WEA WEATHERED	i		CK. GOUGES OR GROOVES TO 0.25 IN	CHES DEEP CAN BE	TO THE BEDDING OR SCHISTOSITY OF THE INTRUDED ROCKS. SLICKENSIDE - POLISHED AND STRIATED SURFACE THAT RESULTS FROM FRICTION ALONG A FAULT OR
GRAIN MM 305 75 2.0 0.25 0.05	CL CL	CLAY	OD MODERATELY		HARD EXCAN		OLOGIST'S PICK. HAND SPECIMENS CA		SLIP PLANE.
SIZE IN. 12 3) Cr1 - t		P ~ NON PLASTIC RG ORGANIC	γ_{d} - DRY UNIT WEIGHT	MEDIUM CAN I	BE GROOVED OR GOUGED 0.05 I	NCHES DEEP BY FIRM PRESSURE OF		STANDARD PENETRATION TEST (PENETRATION RESISTANCE) (BPT) - NUMBER OF BLOWS (N OR BPF) OF A 140 LB. HAMMER FALLING 30 INCHES REQUIRED TO PRODUCE A PENETRATION OF 1 FOOT INTO SOIL WITH
SOIL MOISTURE - CORRELATION OF TERMS			MT - PRESSUREMETER TEST AP SAPROLITIC	SAMPLE ABBREVIATIONS S - BULK		BE EXCAVATED IN SMALL CHIPS OF A GEOLOGIST'S PICK.	TO PEICES 1 INCH MAXIMUM SIZE B	Y HARD BLOWS OF THE	A 2 INCH DUTSIDE DIAMETER SPLIT SPOON SAMPLER, SPT REFUSAL IS PENETRATION EQUAL TO DR LESS
SDIL MOISTURE SCALE FIELD MOISTURE GUIDE FOR FIELD MOISTURE (ATTERBERG LIMITS) DESCRIPTION		VOID RATIO SI	D SAND, SANDY	SS - SPLIT SPOON	SOFT CAN I	BE GROVED OR GOUGED READILY	BY KNIFE OR PICK. CAN BE EXCAVA		THAN 0.1 FOOT PER 60 BLOWS. STRATA CORE RECOVERY (SREC.) - TOTAL LENGTH OF STRATA MATERIAL RECOVERED DIVIDED BY TOTAL LENGTH
	F - FIN		L SILT, SILTY LI SLIGHTLY	ST - SHELBY TUBE RS - ROCK		CHIPS TO SEVERAL INCHES IN S CAN BE BROKEN BY FINGER	SIZE BY MODERATE BLOWS OF A PIG PRESSURE.	CK POINT. SMALL, THIN	OF STRATUM AND EXPRESSED AS A PERCENTAGE.
	GROUND WATER TABLE FRAC	FRACTURED, FRACTURES TO	CR - TRICONE REFUSAL - MOISTURE CONTENT	RT - RECOMPACTED TRIAXIAL CBR - CALIFORNIA BEARING			EXCAVATED READILY WITH POINT OF		STRATA ROCK QUALITY DESIGNATION (SROD) - A MEASURE OF ROCK QUALITY DESCRIBED BY TOTAL LENGTH OF ROCK SEGMENTS WITHIN A STRATUM EQUAL TO OR GREATER THAN 4 INCHES DIVIDED BY TH
PLASTIC LIOUID LIMIT	ні ні		- VERY	RATIO	SOFT OR MO		KEN BY FINGER PRESSURE. CAN BE S	SCRATCHED READILY BY	TOTAL LENGTH OF STRATA AND EXPRESSED AS A PERCENTAGE.
RANGE - WET - (W) SEMISULID; REBUIRE ATTAIN OPTIMUM MO		EQUIPMENT	USED ON SUBJECT P	ROJECT	FRACTI	JRE SPACING	BEDDIN		TOPSOIL (TS.) - SURFACE SOILS USUALLY CONTAINING ORGANIC MATTER.
PLL - PLASTIC LIMIT -	DRILL UP	UNITS: ADVANCIN	NG TOOLS:	HAMMER TYPE:	IERM	SPACING	TERM VERY THICKLY BEDDED	THICKNESS > 4 FEET	BENCH MARK: BL-4, STA. 20+76.04, IL.79' RT
OM OPTIMUM MOISTURE - MOIST - (M) SOLID; AT OR NEAR	AR OPTIMUM MOISTURE	CLA	Y RITS	AUTOMATIC MANUAL	VERY WIDE WIDE	MORE THAN 10 FEET 3 TO 10 FEET	THICKLY BEDDED	1.5 - 4 FEET	ELEVATION: 491.53 FT.
SL SHRINKAGE LIMIT		MORITE R.	ONTINUOUS FLIGHT AUGER	CODE DITE.	MODERATELY CLO CLOSE	SE 1 TO 3 FEET 0.16 TO 1 FEET	THINLY BEDDED VERY THINLY BEDDED	0.16 - 1.5 FEET 0.03 - 0.16 FEET	
- DRY - (D) REQUIRES ADDITION.		D. J. P	OLLOW AUGERS	CORE SIZE:	VERY CLOSE	LESS THAN 0.16 FEET	THICKLY LAMINATED THINLY LAMINATED	0.008 - 0.03 FEET < 0.008 FEET	NOTES:
PLASTICITY			D FACED FINGER BITS			INI	OURATION		
	STRENGTH CM	CITE-45C	GCARBIDE INSERTS		FOR SEDIMENTARY RO	CKS, INDURATION IS THE HARDE	NING OF THE MATERIAL BY CEMENTIN	NG, HEAT, PRESSURE, ETC.	
	RY LOW CM	CME-550	ING W/ ADVANCER	н	FRIABLE		G WITH FINGER FREES NUMEROUS GR		
MED. PLASTICITY 16-25 ME	IGHT DIUM		CONE STEEL TEETH	HAND TOOLS:]	204740	BLOW BY HAMMER DISINTEGRATES S CAN BE SEPARATED FROM SAMPLE V		
	HIGH LJ PO			POST HOLE DIGGER HAND AUGER	MODERATEL		EASILY WHEN HIT WITH HAMMER.	TIII SIEEL FRUBE!	
COLOR		CME-550X	CONE TUNGCARB.	SOUNDING ROD	INDURATED		ARE DIFFICULT TO SEPARATE WITH	STEEL PROBE;	
DESCRIPTIONS MAY INCLUDE COLOR OR COLOR COMBINATIONS (TAN, RED, YELLOW-B	1 []	COR	E BIT	VANE SHEAR TEST			ULT TO BREAK WITH HAMMER. HAMMER BLOWS REQUIRED TO BREAK	CAMPI E.	
MODIFIERS SUCH AS LIGHT, DARK, STREAKED, ETC. ARE USED TO DESCRIBE APPE	EHRHINUE.				EXTREMELY		E BREAKS ACROSS GRAINS.	OHITE!	

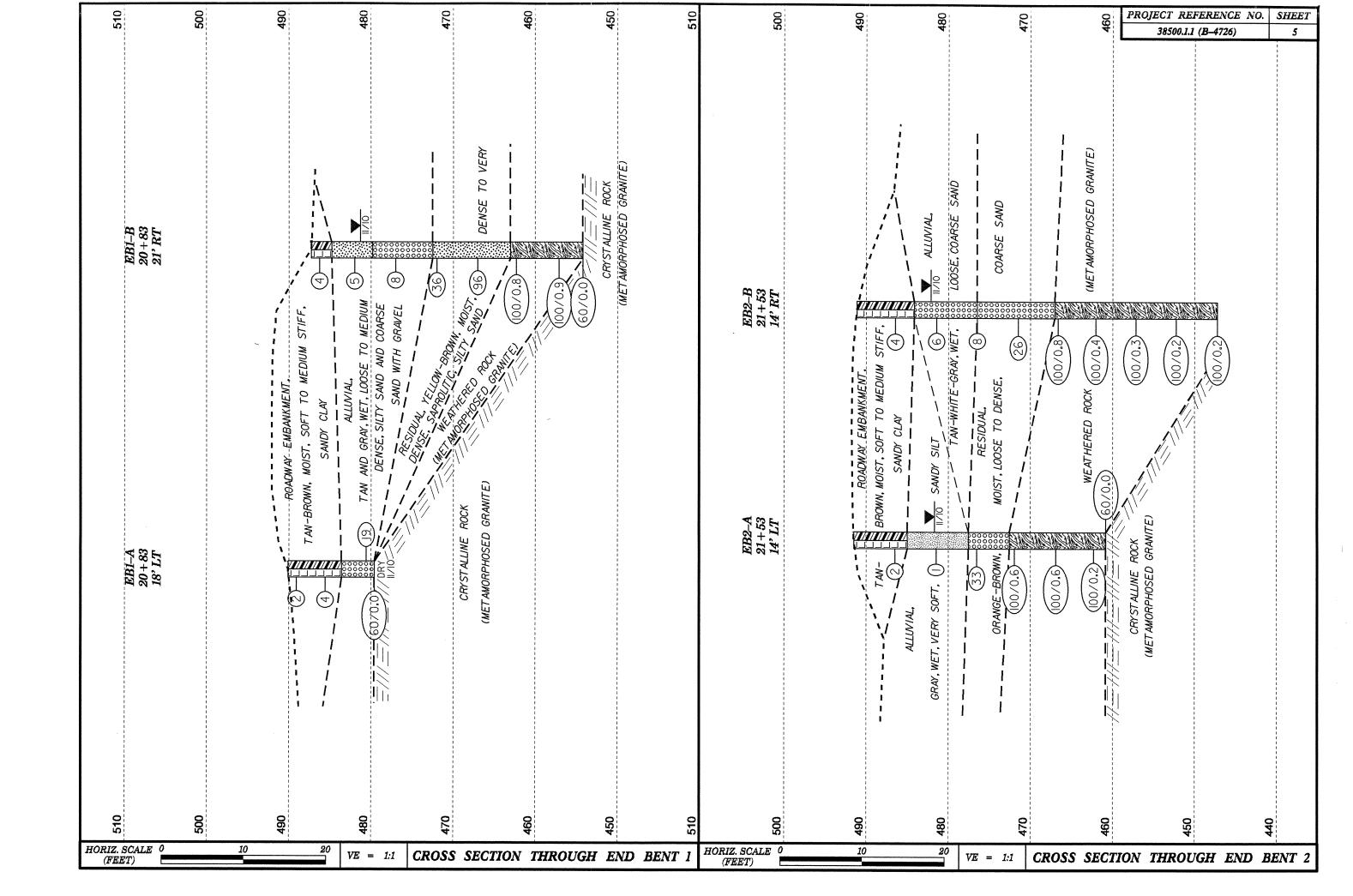
PROJECT REFERENCE NO.

38500.I.I (B-4726)

SHEET NO.

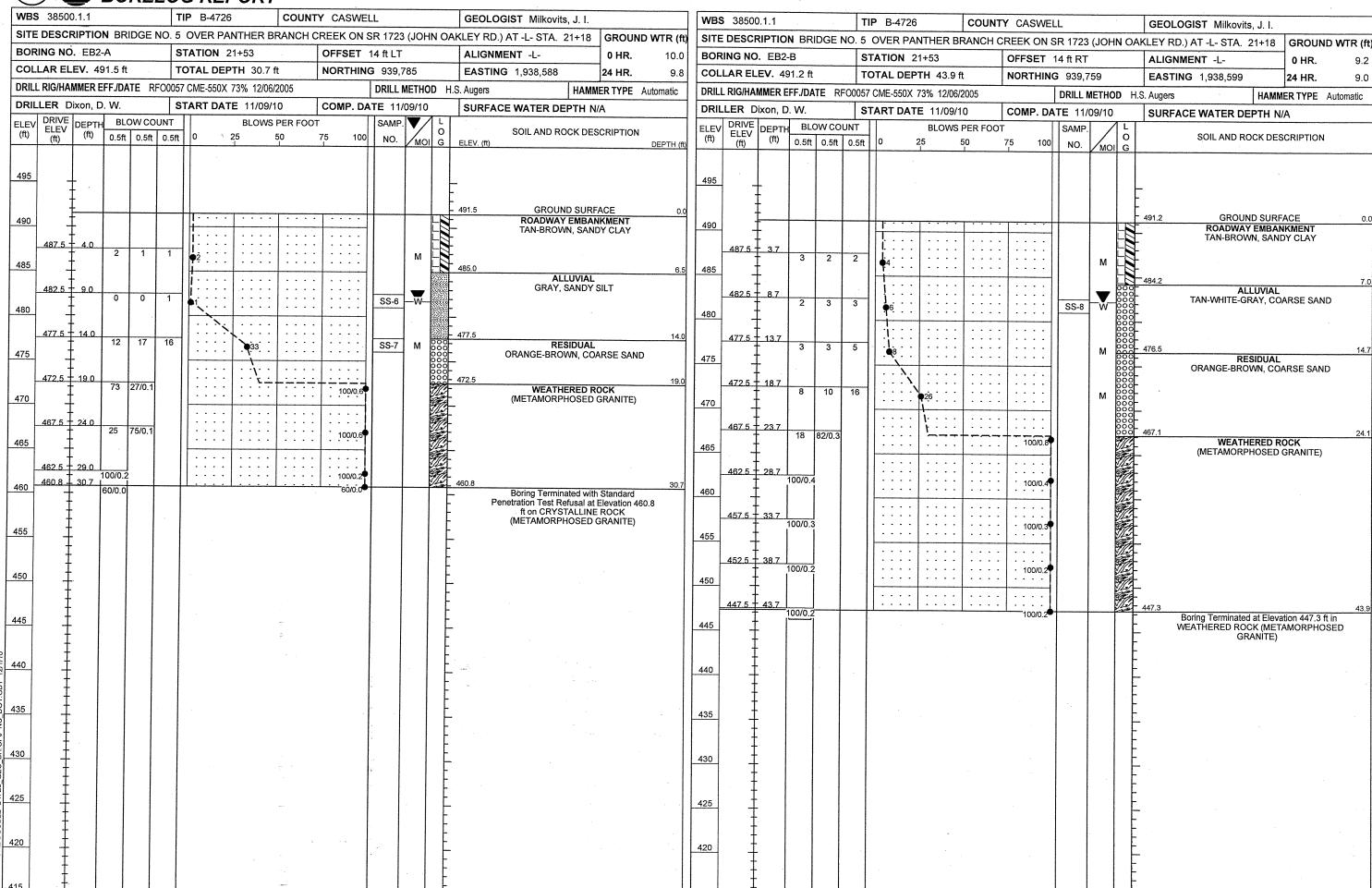






NCDOT GEOTECHNICAL ENGINEERING UNIT BORELOG REPORT

W BOREL	OG REPORT			,									
WBS 38500.1.1	<u></u>	JNTY CASWELL	GEOLOGIST Milkovits, J. I.	┨ ┣━━━	S 38500.1.1			IP B-4726	COUNTY CASW			GEOLOGIST Milkovits, J. I.	
SITE DESCRIPTION BRIDGE NO				l								AKLEY RD.) AT -L- STA. 21+18	GROUND WTR (fi
BORING NO. EB1-A	STATION 20+83	OFFSET 18 ft LT	ALIGNMENT -L- 0 HR. Dry	l	RING NO. E			TATION 20+83	OFFSET	21 ft R7	Γ	ALIGNMENT -L-	0 HR . 6.0
COLLAR ELEV. 490.1 ft	TOTAL DEPTH 10.5 ft	NORTHING 939,762	EASTING 1,938,522 24 HR. Dry	┦ ├───	LLAR ELEV.			OTAL DEPTH 33.				EASTING 1,938,537	24 HR. 6.0
DRILL RIG/HAMMER EFF./DATE RFC		DRILL METHOD H.		 				7 CME-550X 73% 12/			METHOD I		MER TYPE Automatic
DRILLER Dixon, D. W.	START DATE 11/09/10	COMP. DATE 11/09/10	SURFACE WATER DEPTH N/A	1 -	LLER Dixon			TART DATE 11/0		·		SURFACE WATER DEPTH	1/A
ELEV CHI DEPTH BLOW COUL		OOT SAMP. L O NO. MOI G	SOIL AND ROCK DESCRIPTION ELEV. (ft) DEPTH (ft)	ELEV (ft)	V DRIVE DEF	0.5ft 0.5		4 1	VS PER FOOT 50 75 10	SAMP NO.	MOI G	SOIL AND ROCK DE	SCRIPTION
495			-	495									
490 490.1 0.0	1 •2 · · · · · · · · · · ·	SS-5 M	490.1 GROUND SURFACE 0.0 ROADWAY EMBANKMENT TAN-BROWN, SANDY CLAY	490	7 Ŧ							- - -	
486.6 + 3.5	2	·· ···			487.3 7 0		2 2	4	:::::::::::::::::::::::::::::::::::::::	SS-1	M	- 487.3 GROUND SUR ROADWAY EMBA	NKMENT
485	2	M [- 483.6 6.5	485	$+$ \pm					-		TAN-BROWN, SAN ALLUVIA	4 .
1016 05			ALLUVIAL GRAY, COARSE SAND	11	483.0 4		2 3			SS-2	- w	TAN, SILTY S	- AND
	13	w ooo	-479.6 10.5	480	<u> </u>					1 00-2	-	479.8	7
479.6 10.5 60/0.0		60/0.0	Boring Terminated with Standard Penetration Test Refusal at Elevation 479.6		478.0 9	3		1 1			000	GRAY, COARSI	SAND
			ft on CRYSTALLINE ROCK (METAMORPHOSED GRANITE)		1 +	4 4	1 4	8		11000	W 0000		
475			(WETAWORFHOSED GRANTE)	475	7 I					+	000		
					473.0 14	3 9 1	5 21		6	1	1 . 600	472.5	14.
470				470	$oldsymbol{oldsymbol{oldsymbol{eta}}}$				° \	SS-4	┤‴╠	RESIDUA YELLOW-BROWN, M	IICACEOUS,
					468.0 19	3				71		SAPROLITIC, SIL	IY SAND
1					-	19 3	2 64			96	М		
465			-	465	- I					H		-	
1					463.0 24	3 40 60/	0.3			닠		463.0 WEATHERED	POCK 24.
460				460	‡	1 40 00/	0.0		1 100/0.	8		(METAMORPHOSEI	
+			-	400	7 I					1		<u> </u>	
					458.0 29	59 41/	0.4	:::: :::	100/0.	11			
455			_	455	454.2				100/0.			463.0 WEATHERED (METAMORPHOSEI	33.
					454.2 33	60/0.0			60/0.	0	3970	Boring Terminated w	ith Standard
450			_	450								Penetration Test Refusal : ft on CRYSTALLIF (METAMORPHOSEI	NE ROCK
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SHEET 8 38500.1.1 (B-4726)

EB1-A

			S	OIL T	TE.	ST	RE	SUL	TS						
SAMPLE			DEPTH	AASHTO				% BY W	EIGHT		% PAS	SING (S	IEVES)	%	%
NO.	OFFSET	STATION	INTERVAL	CLASS.	L.L.	P.I.	C.SAND	F.SAND	SILT	CLAY	10	40	200	MOISTURE	ORGANIC
SS-5	18'LT	20+83	0.0-1.5	A-6(4)	33	11	26.7	20.0	27.1	26.3	95	78	55	-	-

EB1-B

			S	OIL T	TE.	ST	RE	SUL	TS						
SAMPLE			DEPTH	AASHTO				% BY V	VEIGHT		% PAS	SING (S	SIEVES)	%	%
NO.	OFFSET	STATION	INTERVAL	CLASS.	L.L.	P.I.	C.SAND	F.SAND	SILT	CLAY	10	40	200	MOISTURE	ORGANIC
SS-1	21' RT	20+83	0.0-1.5	A-6(9)	36	12	12.3	13.3	40.0	34.3	99	90	76	-	
SS-2	21' RT	20+83	4.3-5.8	A-2-4(0)	24	NP	58.2	28.3	7.5	6.1	99	68	16	-	-
SS-3	21' RT	20+83	9.3-10.8	A-1-b(0)	22	NP	73.8	15.7	6.5	4.0	82	32	10	-	-
SS-4	21' RT	20+83	14.8-15.8	A-2-4(0)	27	NP	58.2	28.1	10.7	3.0	94	56	17	-	•

EB2-A

			S	OIL 7	TE.	\overline{ST}	RE	SUL	TS						
SAMPLE			DEPTH	AASHTO				% BY W	VEIGHT		% PAS	SING (S	IEVES)	%	%
NO.	OFFSET	STATION	INTERVAL	CLASS.	L.L.	P.I.	C.SAND	F.SAND	SILT	CLAY	10	40	200	MOISTURE	ORGANIC
SS-6	14'LT	21+53	9.0-10.5	A-4(0)	23	4	7.7	39.0	33.1	20.2	100	99	60	-	-
SS-7	14'LT	21+53	14.0-15.5	A-1-b(0)	24	NP	57.3	26.8	11.9	4.0	77	45	15	-	-

ER2-R

LD4-D															
			S	OIL T	E	ST	RE	SUL	TS						
SAMPLE			DEPTH	AASHTO				% BY W	VEIGHT		% PAS	SING (S	SIEVES)	%	%
NO.	OFFSET	STATION	INTERVAL	CLASS.	L,L.	P.I.	C.SAND	F.SAND	SILT	CLAY	10	40	200	MOISTURE	ORGANIC
SS-8	14'RT	21+53	8.7-10.2	A-1-b(0)	26	NP	72.1	21.2	4.6	2.0	94	43	8	•	•

Form GEU-017e Revised 7/26/2007

Date: 11/2/2010



FIELD SCOUR REPORT

WBS:	38500.1.1 TIP: B-472	26 COUNTY: Caswell	
DESCRIPTION(1):	Bridge No. 5 over Panther Branch (Creek on SR 1723 (John Oakley Rd.) at -L- Station 21+18	
	EXIS	TING BRIDGE	
Information from:	Field Inspection X	Microfilm (reel pos:)	
Bridge No.: Foundation Type:	5 Length: 52' Total Be	nts:4 Bents in Channel:1 Bents in Floodplain:3 tings	
EVIDENCE OF S Abutments or B			Periodications
Interior Bents:	local scouring around Bent 2 (1.0')		_
Channel Bed:	None		
Channel Bank:	Steep banks have been eroded dur	ng high water	
1	UR PROTECTION Timber abutment walls with wing wa	alls	_
Extent(4):	Wing walls: 9 +/- feet at each end o	f the bridge	
Effectiveness(5):	Effective		
Obstructions(6):	Boulders in the creek on the north s	ide of bridge	

INSTRUCTIONS

- 1 Describe the specific site's location, including route number and body of water crossed.
- 2 Note scour evidence at existing end bents or abutments (e.g. undermining, sloughing, degradations).
- 3 Note existing scour protection (e.g. rip rap).
- 4 Describe extent of existing scour protection.
- 5 Describe whether or not the scour protection appears to be working.
- 6 Note obstructions such as dams, fallen trees, debris at bents, etc.
- 7 Describe the channel bed material based on observation and/or samples. Include any lab results with report.
- 8 Describe the channel bank material based on observation and/or samples. Include any lab results with report.
- **9** Describe the material covering the banks (e.g. grass, trees, rip rap, none).
- 10 Determine the approximate floodplain width from field observation or a topographic map.
- 11 Describe the material covering the floodplain (e.g. grass, trees, crops).
- 12 Use professional judgement to specify if the stream is degrading, aggrading, or static.
- 13 Describe potential and direction of the stream to migrate laterally during the bridge's life (approx. 100 years).
- 14 Give the design scour elevation (DSE) expected over the life of the bridge (approx. 100 years). This elevation can be given as a range across the site, or for each bent. Discuss the relationship between the Hydraulics Unit theoritical scour and the DSE. If the DSE is dependent on scour counter measures, explain (e.g. rip rap armoring on slopes). The DSE is based on the erodability of materials, giving consideration to the influence of joints, foliation, bedding characteristics, % core recovery, % RQD, differential weathering, shear strength, observations at existing structures, other tests deemed appropriate, and overall geologic conditions at the site.

Channel Bed Material(7): Alluvial, tan-white-gray, loose, coarse sand (SS-3,SS-8)	
Channel Bank Material(8): Alluvial, tan, loose, silty sand (SS-2), Gray, very soft, sandy silt (SS-6)	
Channel Bank Cover(9): Grass, trees and shrubs	
Floodplain Width(10): 100'-200'	
Floodplain Cover(11): Grass, trees and shrubs	
Stream is(12): Aggrading DegradingX Static	
nannel Migration Tendency(13): Northwest toward Bent 2	
Observations and Other Comments:	
DESIGN SCOUR ELEVATIONS(14) Feet X Meters	
BENTS EB1 EB2	
473.2' 473.2'	
	1
Comparison of DSE to Hydraulics Unit theoretical scour: GASE agrees with the scour elevation indicated on the Hydraulic's Report dated August 27, 2010.	
SOIL ANALYSIS RESULTS FROM CHANNEL BED AND BANK MATERIAL Bed or Bank Sample No.	
SOIL ANALYSIS RESULTS FROM CHANNEL BED AND BANK MATERIAL Bed or Bank Sample No. Retained #4	
SOIL ANALYSIS RESULTS FROM CHANNEL BED AND BANK MATERIAL Bed or Bank Sample No. Retained #4 Passed #10 See Shoot 8	
GASE agrees with the scour elevation indicated on the Hydraulic's Report dated August 27, 2010. SOIL ANALYSIS RESULTS FROM CHANNEL BED AND BANK MATERIAL Bed or Bank Sample No. Retained #4 Passed #10 Passed #40 See Sheet 8, "Soil Toet Popults"	
SOIL ANALYSIS RESULTS FROM CHANNEL BED AND BANK MATERIAL Bed or Bank Sample No. Retained #4 Passed #10 Passed #200 Passed #200 Coarse Sand GASE agrees with the scour elevation indicated on the Hydraulic's Report dated August 27, 2010. SOIL ANALYSIS RESULTS FROM CHANNEL BED AND BANK MATERIAL See Sheet 8, "Soil Test Results", for samples:	
GASE agrees with the scour elevation indicated on the Hydraulic's Report dated August 27, 2010. SOIL ANALYSIS RESULTS FROM CHANNEL BED AND BANK MATERIAL Bed or Bank Sample No. Retained #4 Passed #10 Passed #40 Passed #200 Coarse Sand Fine Sand Soil Test Results", for samples: SS-2	
SOIL ANALYSIS RESULTS FROM CHANNEL BED AND BANK MATERIAL Bed or Bank Sample No. Retained #4 Passed #10 Passed #200 Coarse Sand Fine Sand Silt Silt Silt See Sheet 8, See Sheet	
SOIL ANALYSIS RESULTS FROM CHANNEL BED AND BANK MATERIAL Bed or Bank Sample No. Retained #4 Passed #10 Passed #200 Coarse Sand Fine Sand Silt Clay SOIL ANALYSIS RESULTS FROM CHANNEL BED AND BANK MATERIAL See Sheet 8, "Soil Test Results", for samples: SS-2 SS-3 SS-6 SS-6 SS-6 SS-6 SS-6 SS-6 SS-1	
SOIL ANALYSIS RESULTS FROM CHANNEL BED AND BANK MATERIAL Bed or Bank Sample No. Retained #4 Passed #10 Passed #200 Coarse Sand Fine Sand Silt SOIL ANALYSIS RESULTS FROM CHANNEL BED AND BANK MATERIAL See Sheet 8, "Soil Test Results", for samples: SS-2 SS-3	
GASE agrees with the scour elevation indicated on the Hydraulic's Report dated August 27, 2010. SOIL ANALYSIS RESULTS FROM CHANNEL BED AND BANK MATERIAL Bed or Bank Sample No. Retained #4 Passed #10 Passed #40 Passed #200 Coarse Sand Fine Sand Silt Clay SS-8 SOIL ANALYSIS RESULTS FROM CHANNEL BED AND BANK MATERIAL See Sheet 8, "Soil Test Results", for samples: SS-2 SS-3 SS-6 SS-6 SS-8	
GASE agrees with the scour elevation indicated on the Hydraulic's Report dated August 27, 2010. SOIL ANALYSIS RESULTS FROM CHANNEL BED AND BANK MATERIAL Bed or Bank Sample No. Retained #4 Passed #10 Passed #40 Passed #200 Coarse Sand Fine Sand Silt Clay LL PI AASHTO	
SOIL ANALYSIS RESULTS FROM CHANNEL BED AND BANK MATERIAL Bed or Bank Sample No. Retained #4 Passed #10 Passed #200 Coarse Sand Fine Sand Silt Clay LL PI AASHTO Station SOIL ANALYSIS RESULTS FROM CHANNEL BED AND BANK MATERIAL See Sheet 8, "Soil Test Results", for samples: SS-2 SS-3 SS-6 SS-8 AASHTO Station	
SOIL ANALYSIS RESULTS FROM CHANNEL BED AND BANK MATERIAL Bed or Bank Sample No. Retained #4 Passed #10 Passed #200 Coarse Sand Fine Sand Silt Clay LL PI AASHTO	

Reported by: Ath Ms

SITE PHOTOGRAPH

Bridge No. 5 over Panther Branch Creek on SR 1723 (John Oakley Rd.) at -L- Station 21+18

