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STATE OF NORTH CAROLINA

DEPARTMENT OF TRANSPORTATION
DIVISION OF HIGHWAYS
GEOTECHNICAL ENGINEERING UNIT

STRUCTURE SUBSURFACE INVESTIGATION

PROJ. REFERENCE NO. 38509.1.1 (B-4736)

F.A. PROJ. *BRSTP-1001 (40*)

COUNTY **BLADEN/COLUMBUS**

PROJECT DESCRIPTION BRIDGE NO. 233 ON SR 1001 (HALLSBORO RD.)

OVER SLADES SWAMP.

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CAUTION NOTICE

THE SUBSURFACE INFORMATION AND THE SUBSURFACE INVESTIGATION ON WHICH IT IS BASED WERE MADE FOR THE PURPOSE OF STUDY, PLANNING, AND DESIGN, AND NOT FOR CONSTRUCTION OR PAY PURPOSES. THE VARIOUS FIELD BORING LOGS, ROCK CORES, AND SOL TEST DATA AVAILABLE MAY BE REVIEWED OR INSPECTED IN RALEIGH BY CONTACTING THE N. C. DEPARTMENT OF TRANSPORTATION, GOOTECHNICAL ENGINEERING UNIT AT (919) 250-4088. NEITHER THE SUBSURFACE PLANS AND REPORTS, NOR THE FIELD BORING LOGS, ROCK CORES, OR SOIL TEST DATA ARE PART OF THE CONTRACT.

GENERAL SOIL AND ROCK STRATA DESCRIPTIONS AND INDICATED BOUNDARIES ARE BASED ON A GEOTECHNICAL INTERPRETATION OF ALL AVAILABLE SUBSURFACE DATA AND MAY NOT NECESSARILY REFLECT THE ACTUAL SUBSURFACE CONDITIONS BETWEEN BORINGS OR BETWEEN SAMPLED STRATA WITHIN THE BOREHOLE. THE LABORATORY SAMPLE DATA AND THE IN STIU UN-PLACE/TEST DATA CAN BE RELIED ON ONLY TO THE DEGREE OF RELIABILITY INHERENT IN THE STANDARD TEST METHOD. THE OBSERVED WATER LEVELS OR SOIL MOISTURE CONDITIONS INDICATED IN THE SUBSURFACE INVESTIGATIONS ARE AS RECORDED AT THE TIME OF THE INVESTIGATION. THESE WATER LEVELS OR SOIL MOISTURE CONDITIONS AND VARY CONSIDERABLY WITH THE ACCORDING TO CLIMATIC CONDITIONS INCLUDING TEMPERATURES, PRECIPITATION, AND WIND, AS WELL AS OTHER NON-CLIMATIC FACTORS.

THE BIDDER OR CONTRACTOR IS CAUTIONED THAT DETAILS SHOWN ON THE SUBSURFACE PLANS ARE PRELIMINARY ONLY AND IN MANY CASES THE FINAL DESIGN DETAILS ARE DIFFERENT. FOR BIDDING AND CONSTRUCTION PURPOSES, REFER TO THE CONSTRUCTION PLANS AND DOCUMENTS FOR FINAL DESIGN INFORMATION ON THIS PROJECT. THE DEPARTMENT DOES NOT MARRANT OR GUARANTEE THE SUFFICIENCY OR ACCURACY OF THE INVESTICATION MADE, NOR THE INTERPRETATIONS MADE, OR OPINION OF THE DEPARTMENT AS TO THE TYPE OF MATERIALS AND CONDITIONS TO BE ENCOUNTERED. THE BIDDER OR CONTRACTOR IS CAUTIONED TO MAKE SUCH INDEPENDENT SUBSURFACE INVESTIGATIONS AS HE DEEMS NECESSARY TO SATISFY HIMSELF AS TO CONDITIONS TO BE ENCOUNTERED ON THIS PROJECT. THE CONTRACTOR SHALL HAVE NO CLAIM FOR ADDITIONAL COMPENSATION OR FOR AN EXTENSION OF TIME FOR ANY REASON RESULTING FROM THE ACTUAL CONDITIONS ENCOUNTERED AT THE SITE DIFFERING FROM THOSE INDICATED.

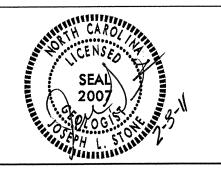
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INVESTIGATED BY J.L. STONE

CHECKED BY D.N. ARGENBRIGHT

SUBMITTED BY D.N. ARGENBRIGHT

DATE FEBUARY 2011



DRAWN BY: C.R. SUMNER

NOTE - THE INFORMATION CONTAINED HEREIN IS NOT IMPLIED OR GUARANTEED BY THE N.C. DEPARTMENT
OF TRANSPORTATION AS BEING ACCURATE NOR IT IS CONSIDERED TO BE PART OF THE PLANS,
SSECIENCATIONS, OR CONTRACT FOR THE PROJECT.

NOTE - BY HAVING REQUESTED THIS INFORMATION THE CONTRACTOR SPECIFICALLY WAIVES ANY CLAIMS FOR INCREASED COMPENSATION OR EXTENSION OF TIME BASED ON DIFFERENCES BETWEEN THE CONDITIONS INDICATED HEREIN AND THE ACTUAL CONDITIONS AT THE PROJECT SITE.

NORTH CAROLINA DEPARTMENT OF TRANSPORTATION

DIVISION OF HIGHWAYS

GEOTECHNICAL ENGINEERING UNIT

SUBSURFACE INVESTIGATION

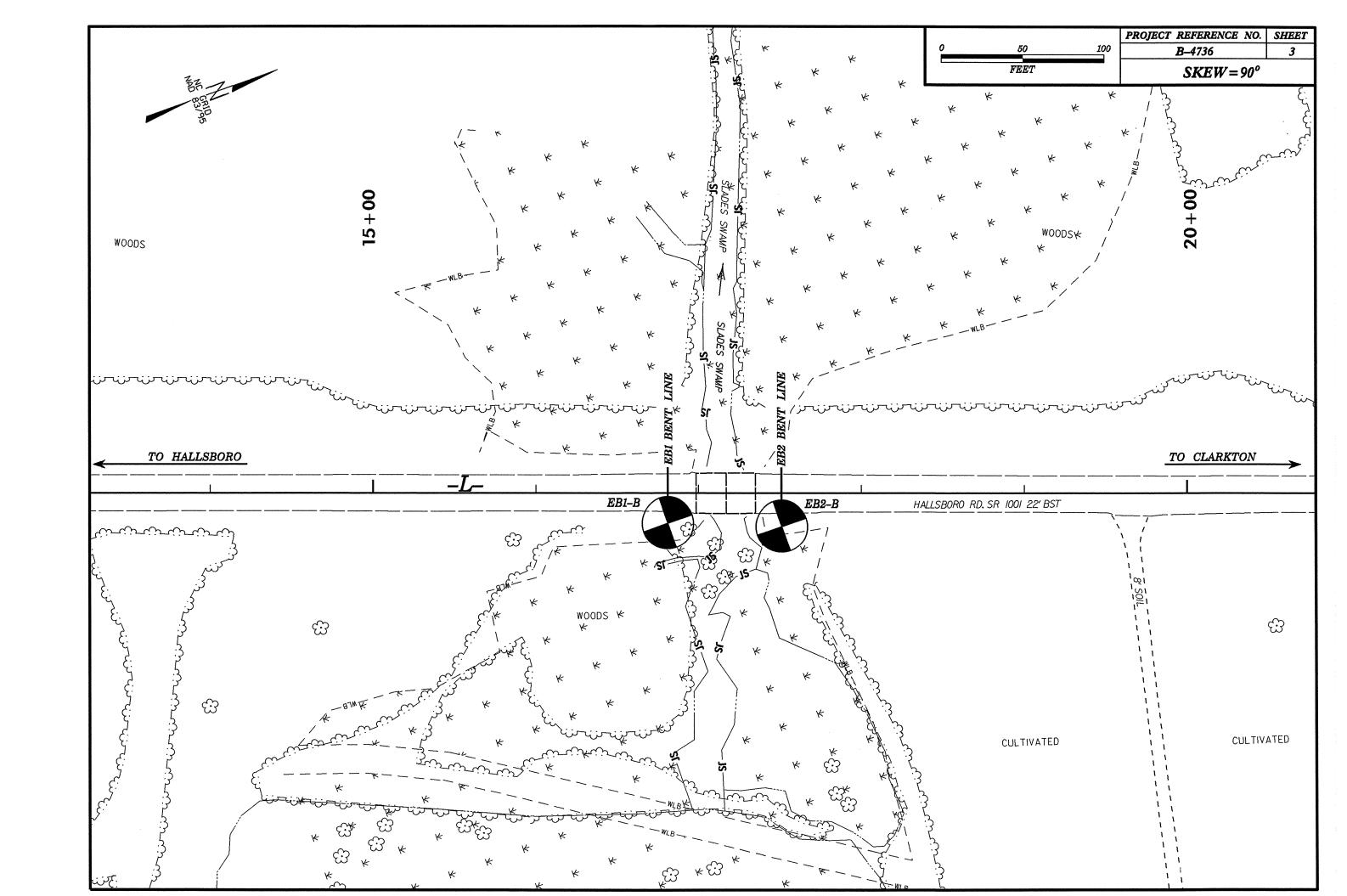
	SOIL AND ROCK LEGEND, TERM	IS, SYMBOLS, AND ABBREVIATIONS	
SOIL DESCRIPTION	GRADATION	ROCK DESCRIPTION	TERMS AND DEFINITIONS
SOIL IS CONSIDERED TO BE THE UNCONSOLIDATED, SEMI-CONSOLIDATED, OR WEATHERED EARTH MATERIALS	NELL GRADED - INDICATES A GOOD REPRESENTATION OF PARTICLE SIZES FROM FINE TO COARSE. UNIFORM - INDICATES THAT SOIL PARTICLES ARE ALL APPROXIMATELY THE SAME SIZE. (ALSO	HARD ROCK IS NON-COASTAL PLAIN MATERIAL THAT IF TESTED, WOULD YIELD SPT REFUSAL, AN INFERRED ROCK LINE INDICATES THE LEVEL AT WHICH NON-COASTAL PLAIN MATERIAL WOULD YIELD SPT REFUSAL.	ALLUVIUM (ALLUV.) - SOILS THAT HAVE BEEN TRANSPORTED BY WATER.
THAT CAN BE PENETRATED WITH A CONTINUOUS FLIGHT POWER AUGER, AND YIELD LESS THAN 100 BLOWS PER FOOT ACCORDING TO STANDARD PENETRATION TEST (AASHTO T206, ASTM D-1586). SDIL	POORLY GRADED) GAP-GRADED - INDICATES A MIXTURE OF UNIFORM PARTICLES OF TWO OR MORE SIZES.	SPT REFUSAL IS PENETRATION BY A SPLIT SPOON SAMPLER EQUAL TO OR LESS THAN &1 FOOT PER 60 BLOWS. IN NON-COASTAL PLAIN MATERIAL. THE TRANSITION BETWEEN SOIL AND ROCK IS OFTEN REPRESENTED BY A ZONE	AQUIFER - A WATER BEARING FORMATION OR STRATA.
CLASSIFICATION IS BASED ON THE AASHTO SYSTEM, BASIC DESCRIPTIONS GENERALLY SHALL INCLUDE: CONSISTENCY, COLOR, TEXTURE, MOISTURE, AASHTO CLASSIFICATION, AND OTHER PERTINENT FACTORS SUCH	ANGULARITY OF GRAINS	OF WEATHERED ROCK, ROCK MATERIALS ARE TYPICALLY DIVIDED AS FOLLOWS:	AMENACEDUS - APPLIED TO ROCKS THAT HAVE BEEN DERIVED FROM SAND OR THAT CONTAIN SAND.
AS MINERALOGICAL COMPOSITION, ANGULARITY, STRUCTURE, PLASTICITY, ETC. EXAMPLE: VERY STAT, GRASSITY CLAS, MOST WITH WITERBEDGED FINE SAMD LATERS, MICH. 47-6	THE ANGULARITY OR ROUNDNESS OF SOIL GRAINS IS DESIGNATED BY THE TERMS: ANGULAR, SUBROUNDED, OR ROUNDED.	WEATHERED NON-COASTAL PLAIN MATERIAL THAT WOULD VIELD SPT N VALUES > 100	ARGILLACEOUS - APPLIED TO ALL ROCKS OR SUBSTANCES COMPOSED OF CLAY MINERALS, OR HAVING A NOTABLE PROPORTION OF CLAY IN THEIR COMPOSITION, AS SHALE, SLATE, ETC.
SOIL LEGEND AND AASHTO CLASSIFICATION	MINERALOGICAL COMPOSITION	ROCK (WR) BLOWS PER FOOT IF TESTED.	ARTESIAN - GROUND WATER THAT IS UNDER SUFFICIENT PRESSURE TO RISE ABOVE THE LEVEL
GENERAL GRANULAR MATERIALS SILT-CLAY MATERIALS ODGANIC MATERIALS	MINERAL NAMES SUCH AS QUARTZ, FELDSPAR, MICA, TALC, KAOLIN, ETC. ARE USED IN DESCRIPTIONS	CRYSTALLINE ROCK (CR) FINE TO COARSE GRAIN IGNEOUS AND METAMORPHIC ROCK THAT WOULD YIELD SPT REFUSAL IF TESTED. ROCK TYPE INCLUDES GRANITE,	AT WHICH IT IS ENCOUNTERED, BUT WHICH DOES NOT NECESSARILY RISE TO OR ABOVE THE GROUND SURFACE.
CLASS. (\$35% PASSING *200) (>35% PASSING *200)	WHENEVER THEY ARE CONSIDERED OF SIGNIFICANCE.	GNEISS, GABBRO, SCHIST, ETC.	CALCAREOUS (CALC.) - SOILS THAT CONTAIN APPRECIABLE AMOUNTS OF CALCIUM CARBONATE.
GROUP A-1 A-3 A-2 A-4 A-5 A-6 A-7 A-1, A-2 A-4, A-5 CLASS. A-1-a A-1-b A-2-4 A-2-5 A-2-6 A-2-7 A-3 A-6, A-7	COMPRESSIBILITY SLIGHTLY COMPRESSIBLE LIQUID LIMIT LESS THAN 31	ROCK THAT WOULD YELD SPT REFUSAL IF TESTED, ROCK TYPE	COLLUVIUM - ROCK FRAGMENTS MIXED WITH SOIL DEPOSITED BY GRAVITY ON SLOPE OR AT BOTTOM OF SLOPE.
SYMBOL BOOOGOOG	MODERATELY COMPRESSIBLE LIQUID LIMIT EQUAL TO 31-50	COASTAL PLAIN COASTAL PLAIN SEDIMENTS CEMENTED INTO ROCK, BUT MAY NOT YIELD	CORE RECOVERY (REC.) - TOTAL LENGTH OF ALL MATERIAL RECOVERED IN THE CORE BARREL DIVIDED BY TOTAL
V PASSING	HIGHLY COMPRESSIBLE LIQUID LIMIT GREATER THAN 50 PERCENTAGE OF MATERIAL	SEDIMENTARY ROCK SPT REFUSAL, ROCK TYPE INCLUDES LIMESTONE, SANDSTONE, CEMENTED SHELL BEDS, ETC.	LENGTH OF CORE RUN AND EXPRESSED AS A PERCENTAGE.
# 10 50 MX GRANULAR SILT- MUCK,	ORGANIC MATERIAL GRANULAR SILT - CLAY	WEATHERING	DIKE - A TABULAR BODY OF IGNEOUS ROCK THAT CUTS ACROSS THE STRUCTURE OF ADJACENT ROCKS OR CUTS MASSIVE ROCK.
# 40 30 MX 50 MX 51 MN PEAT SOILS PEAT SOILS PEAT SOILS PEAT	TRACE OF ORGANIC MATTER 2 - 3% 3 - 5% TRACE 1 - 10%	FRESH ROCK FRESH, CRYSTALS BRIGHT, FEW JOINTS MAY SHOW SLIGHT STAINING, ROCK RINGS UNDER HAMMER IF CRYSTALLINE.	DIP - THE ANGLE AT WHICH A STRATUM OR ANY PLANAR FEATURE IS INCLINED FROM THE
LIQUID LIMIT 48 MX 41 MN 48 MX 41 MN 48 MX 41 MN 48 MX 41 MN SOILS WITH	LITTLE ORGANIC MATTER 3 - 5% 5 - 12% LITTLE 10 - 20% MODERATELY ORGANIC 5 - 10% 12 - 20% SOME 20 - 35%	VERY SLIGHT ROCK GENERALLY FRESH, JOINTS STAINED, SOME JOINTS MAY SHOW THIN CLAY COATINGS IF OPEN.	HORIZONTAL.
PLASTIC INDEX 6 MX NP 10 MX 10 MX 11 MN 11 MN 10 MX 10 MX 11 MN 11 MN LITTLE OR HIGHLY	HIGHLY ORGANIC >10% >20% HIGHLY 35% AND ABOVE	(V SLI.) CRYSTALS ON A BROKEN SPECIMEN FACE SHINE BRIGHTLY. ROCK RINGS UNDER HAMMER BLOWS IF OF A CRYSTALLINE NATURE.	DIP DIRECTION ODE AZIMUTH) - THE DIRECTION OR BEARING OF THE HORIZONTAL TRACE OF THE LINE OF DIP, MEASURED CLOCKWISE FROM NORTH.
GROUP INDEX 6 0 6 4 MX 8 MX 12 MX 16 MX No MX MODERATE AMOUNTS OF SOILS	GROUND WATER	SLIGHT ROCK GENERALLY FRESH, JOINTS STAINED AND DISCOLORATION EXTENDS INTO ROCK UP TO	FAULT - A FRACTURE OR FRACTURE ZONE ALONG WHICH THERE HAS BEEN DISPLACEMENT OF THE
USUAL TYPES STONE FRAGS. FINE SILTY OR CLAYEY SILTY CLAYEY ORGANIC OF AUDR SAND SAND SOILS SOILS MATTER	WATER LEVEL IN BORE HOLE IMMEDIATELY AFTER DRILLING	(SLI,) 1 INCH, OPEN JOINTS MAY CONTAIN CLAY, IN GRANITOID ROCKS SOME OCCASIONAL FELDSPAR CRYSTALS ARE DULL AND DISCOLORED, CRYSTALLINE ROCKS RING UNDER HAMMER BLOWS,	SIDES RELATIVE TO ONE ANOTHER PARALLEL TO THE FRACTURE. FISSILE - A PROPERTY OF SPLITTING ALONG CLOSELY SPACED PARALLEL PLANES.
MATERIALS SAND SHIPD DIVIVEE HIPD SHIPD SOILS SOILS	STATIC WATER LEVEL AFTER 24 HOURS	MODERATE SIGNIFICANT PORTIONS OF ROCK SHOW DISCOLORATION AND WEATHERING EFFECTS. IN	FLOAT - ROCK FRAGMENTS ON SURFACE NEAR THEIR ORIGINAL POSITION AND DISLODGED FROM
AS A EXCELLENT TO GOOD FAIR TO POOR FAIR TO POOR UNSUITABLE	∇PW PERCHED WATER SATURATED ZONE, OR WATER BEARING STRATA	(MOD.) GRANITOID ROCKS, MOST FELDSPARS ARE DULL AND DISCOLORED, SOME SHOW CLAY, ROCK HAS DULL SOUND UNDER HAMMER BLOWS AND SHOWS SIGNIFICANT LOSS OF STRENGTH AS COMPARED	PARENT MATERIAL.
SUBGRADE PI OF A-7-5 SUBGROUP IS ≤ LL - 30 ; PI OF A-7-6 SUBGROUP IS > LL - 30	SPRING OR SEEP	WITH FRESH ROCK.	FLOOD PLAIN (FP) - LAND BORDERING A STREAM, BUILT OF SEDIMENTS DEPOSITED BY THE STREAM.
CONSISTENCY OR DENSENESS	MISCELLANEOUS SYMBOLS	MODERATELY SEVERE AND DISCOLORED AND A MAJORITY SHOW KAOLINIZATION, ROCK SHOWS SEVERE LOSS OF STRENGTH	FORMATION (FM.) - A MAPPABLE GEOLOGIC UNIT THAT CAN BE RECOGNIZED AND TRACED IN
PRIMARY SOIL TYPE COMPACTNESS OR RANGE OF STANDARD RANGE OF UNCONFINED COMPRESSIVE STRENGTH	CDT	(MOD. SEV.) AND CAN BE EXCAVATED WITH A GEOLOGIST'S PICK. ROCK GIVES "CLUNK" SOUND WHEN STRUCK. IF TESTED, WOULD YIELD SPT REFUSAL	THE FIELD.
CUNSISTENCY (N-VALUE) (TONS/FT ²)	WITH SOIL DESCRIPTION VST PMT W/ CORE	SEVERE ALL ROCK EXCEPT QUARTZ DISCOLORED OR STAINED. ROCK FABRIC CLEAR AND EVIDENT BUT REDUCED	JOINT - FRACTURE IN ROCK ALONG WHICH NO APPRECIABLE MOVEMENT HAS OCCURRED.
GENERALLY VERY LOOSE <4 GRANNII AR LOOSE 4 TO 10	SOIL SYMBOL AUGER BORING SPT N-VALUE	(SEV.) IN STRENGTH TO STRONG SOIL. IN GRANITOID ROCKS ALL FELDSPARS ARE KAOLINIZED TO SOME	LEDGE - A SHELF-LIKE RIDGE OR PROJECTION OF ROCK WHOSE THICKNESS IS SMALL COMPARED TO
MATERIAL MEDIUM DENSE 10 TO 30 N/A	ARTIFICIAL FILL (AF) OTHER - CORE BORING (REF)— SPT REFUSAL	EXTENT. SOME FRAGMENTS OF STRONG ROCK USUALLY REMAIN. IF TESTED, YIELDS SPT N VALUES > 100 BPF	LENS - A BODY OF SOIL OR ROCK THAT THINS OUT IN ONE OR MORE DIRECTIONS.
(NON-COHESIVE) DENSE 30 TO 50 VERY DENSE >50	THAN ROADWAY EMBANKMENT	VERY SEVERE ALL ROCK EXCEPT QUARTZ DISCOLORED OR STAINED, ROCK FABRIC ELEMENTS ARE DISCERNIBLE BUT	MOTTLED (MOT.) - IRREGULARLY MARKED WITH SPOTS OF DIFFERENT COLORS. MOTTLING IN SOILS USUALLY INDICATES POOR AERATION AND LACK OF GOOD DRAINAGE.
VERY SOFT (2 (0.25	INFERRED SOIL BOUNDARY OMONITORING WELL	(V SEV.) THE MASS IS EFFECTIVELY REDUCED TO SOIL STATUS, WITH ONLY FRAGMENTS OF STRONG ROCK REMAINING. SAPROLITE IS AN EXAMPLE OF ROCK WEATHERED TO A DEGREE SUCH THAT ONLY MINOR	PERCHED WATER - WATER MAINTAINED ABOVE THE NORMAL GROUND WATER LEVEL BY THE PRESENCE OF AN
GENERALLY SOFT 2 TO 4 0.25 TO 0.50 SILT-CLAY MEDIUM STIFF 4 TO 8 0.5 TO 1.0	INFERRED ROCK LINE A PIEZOMETER INSTALLATION	VESTIGES OF THE ORIGINAL ROCK FABRIC REMAIN. <u>IF TESTED, YIELDS SPT N VALUES < 100 BPF</u>	INTERVENING IMPERVIOUS STRATUM.
MATERIAL STIFF 8 TO 15 1 TO 2	SLOPE INDICATOR	COMPLETE ROCK REDUCED TO SOIL. ROCK FABRIC NOT DISCERNIBLE, OR DISCERNIBLE ONLY IN SMALL AND SCATTERED CONCENTRATIONS. QUARTZ MAY BE PRESENT AS DIKES OR STRINGERS, SAPROLITE IS	RESIDUAL (RES.) SOIL - SOIL FORMED IN PLACE BY THE WEATHERING OF ROCK.
(COHESIVE) VERY STIFF 15 TO 30 2 TO 4 HARD >30 >4	25/025 DIP & DIP DIRECTION OF	ALSO AN EXAMPLE.	ROCK QUALITY DESIGNATION (ROD) - A MEASURE OF ROCK QUALITY DESCRIBED BY TOTAL LENGTH OF ROCK SEGMENTS EQUAL TO OR GREATER THAN 4 INCHES DIVIDED BY THE TOTAL LENGTH OF CORE RUN AND
TEXTURE OR GRAIN SIZE	ROCK STRUCTURES CONE PENETROMETER TEST	ROCK HARDNESS	EXPRESSED AS A PERCENTAGE.
U.S. STD. SIEVE SIZE 4 10 40 60 200 270	SOUNDING ROD	VERY HARD CANNOT BE SCRATCHED BY KNIFE OR SHARP PICK. BREAKING OF HAND SPECIMENS REDUIRES SEVERAL HARD BLOWS OF THE GEOLOGIST'S PICK.	SAPROLITE (SAP.) - RESIDUAL SOIL THAT RETAINS THE RELIC STRUCTURE OR FABRIC OF THE PARENT ROCK.
OPENING (MM) 4.76 2.00 0.42 0.25 0.075 0.053	ABBREVIATIONS	HARD CAN BE SCRATCHED BY KNIFE OR PICK ONLY WITH DIFFICULTY, HARD HAMMER BLOWS REQUIRED	SILL - AN INTRUSIVE BODY OF IGNEOUS ROCK OF APPROXIMATELY UNIFORM THICKNESS AND
BOULDER COBBLE GRAVEL COARSE FINE SILT CLAY	AR - AUGER REFUSAL HI HIGHLY # MOISTURE CONTENT	TO DETACH HAND SPECIMEN.	RELATIVELY THIN COMPARED WITH ITS LATERAL EXTENT, THAT HAS BEEN EMPLACED PARALLEL TO THE BEDDING OR SCHISTOSITY OF THE INTRUDED ROCKS.
(BLDR.) (COB.) (GR.) (CSE. SD.) (F SD.) (SL.) (CL.) GRAIN MM 305 75 2.0 8.25 0.05 0.005	BT - BORING TERMINATED MED MEDIUM V - VERY CL CLAY MICA MICACEOUS VST - VANE SHEAR TEST	MODERATELY CAN BE SCRATCHED BY KNIFE OR PICK. GOUGES OR GROOVER TO 0.25 INCHES DEEP CAN BE HARD EXCAVATED BY HARD BLOW OF A GEOLOGIST'S PICK. HAND SPECIMENS CAN BE DETACHED BY MODERATE BLOWS.	SLICKENSIDE - POLISHED AND STRIATED SURFACE THAT RESULTS FROM FRICTION ALONG A FAULT OR SLIP PLANE.
SIZE IN. 12 3	CPT - CONE PENETRATION TEST MOD MODERATELY WEA WEATHERED CSE COARSE NP - NON PLASTIC 7 - UNIT WEIGHT	MEDIUM CAN BE GROOVED OR GOUGED 0.05 INCHES DEEP BY FIRM PRESSURE OF KNIFE OR PICK POINT.	STANDARD PENETRATION TEST (PENETRATION RESISTANCE) (6PT) - NUMBER OF BLOWS (N OR BPF) OF A 140 LB, HAMMER FALLING 30 INCHES REQUIRED TO PRODUCE A PENETRATION OF 1 FOOT INTO SOIL WITH
SOIL MOISTURE - CORRELATION OF TERMS	DMT - DILATOMETER TEST ORG ORGANIC 7d - DRY UNIT WEIGHT DPT - DYNAMIC PENETRATION TEST PMT - PRESSUREMETER TEST	HARD CAN BE EXCAVATED IN SMALL CHIPS TO PEICES 1 INCH MAXIMUM SIZE BY HARD BLOWS OF THE POINT OF A GEOLOGIST'S PICK.	A 2 INCH DUTSIDE DIAMETER SPLIT SPOON SAMPLER, SPT REFUSAL IS PENETRATION EQUAL TO DR LESS
SOIL MOISTURE SCALE FIELD MOISTURE GUIDE FOR FIELD MOISTURE DESCRIPTION GUIDE FOR FIELD MOISTURE DESCRIPTION	e - VOID RATIO SAP SAPROLITIC	SOFT CAN BE GROVED OR GOUGED READILY BY KNIFE OR PICK. CAN BE EXCAVATED IN FRAGMENTS	THAN 0.1 FOOT PER 60 BLOWS.
- SATURATED - USUALLY LIQUID; VERY WET, USUALLY	F - FINE SD SAND, SANDY FOSS FOSSILIFEROUS SL SILT, SILTY	FROM CHIPS TO SEVERAL INCHES IN SIZE BY MODERATE BLOWS OF A PICK POINT. SMALL, THIN PIECES CAN BE BROKEN BY FINGER PRESSURE.	STRATA CORE RECOVERY (SREC.) - TOTAL LENGTH OF STRATA MATERIAL RECOVERED DIVIDED BY TOTAL LENGTH OF STRATUM AND EXPRESSED AS A PERCENTAGE.
(SAT.) FROM BELOW THE GROUND WATER TABLE	FRAC FRACTURED, FRACTURES SLI SLIGHTLY FRAGS FRAGMENTS TCR - TRICONE REFUSAL	VERY CAN BE CARVED WITH KNIFE. CAN BE EXCAVATED READILY WITH POINT OF PICK. PIECES 1 INCH	STRATA ROCK QUALITY DESIGNATION (SROD) - A MEASURE OF ROCK QUALITY DESCRIBED BY TOTAL LENGTH OF ROCK SEGMENTS WITHIN A STRATUM EQUAL TO OR GREATER THAN 4 INCHES DIVIDED BY THE
PLASTIC CENTED ID PEDUTOES DOVING TO	TON THOME THE OAK	SOFT OR MORE IN THICKNESS CAN BE BROKEN BY FINGER PRESSURE. CAN BE SCRATCHED READILY BY FINGERNAIL.	TOTAL LENGTH OF STRATA AND EXPRESSED AS A PERCENTAGE.
ATTAIN OPTIMUM MOISTURE	EQUIPMENT USED ON SUBJECT PROJECT	FRACTURE SPACING BEDDING	<u>IOPSOIL (TS.)</u> - SURFACE SOILS USUALLY CONTAINING ORGANIC MATTER.
PLL + PLASTIC LIMIT	DRILL UNITS: ADVANCING TOOLS: HAMMER TYPE:	TERM SPACING TERM THICKNESS VERY HIDE HOSE THAN IN SECTION OF THE PROPERTY OF	BENCH MARK: BL-I -BL- STA. II+71
OM OPTIMUM MOISTURE - MOIST - (M) SOLID; AT OR NEAR OPTIMUM MOISTURE	CLAY BITS X AUTOMATIC MANUAL	WINE MIDE MURE IMAIN 10 FEET THICKLY BEDDED 1.5 - 4 FEET	F. CHATTON 70.7
SL SHRINKAGE LIMIT	1 La 1800EL 0 [7]	MODERATELY CLOSE 1 TO 3 FEET THINLY BEDDED 0.16 - 1.5 FEET VERY THINLY BEDDED 0.03 - 0.16 FEET	ELEVATION: 78.3 FT.
- DRY - (D) REQUIRES ADDITIONAL WATER TO ATTAIN OPTIMUM MOISTURE	l l pv.Ei	VERY CLOSE LESS THAN 0.16 FEET THICKLY LAMINATED 0.008 - 0.03 FEET THINLY LAMINATED 4.008 FEET	NOTES:
PLASTICITY		INDURATION (BLOOD FEET	
PLASTICITY INDEX (PI) DRY STRENGTH		FOR SEDIMENTARY ROCKS, INDURATION IS THE HARDENING OF THE MATERIAL BY CEMENTING, HEAT, PRESSURE, ETC.	
NONPLASTIC 8-5 VERY LOW LOW PLASTICITY 6-15 SLIGHT	CME-750 TUNG,-CARBIDE INSERTS X CASING W/ ADVANCER	FRIABLE RUBBING WITH FINGER FREES NUMEROUS GRAINS; GENTLE BLOW BY HAMMER DISINTEGRATES SAMPLE.	
MED. PLASTICITY 16-25 MEDIUM	HAND TOOLS:		
HIGH PLASTICITY 26 OR MORE HIGH		MODERATELY INDURATED URAINS CAN BE SEPARATED FROM SAMPLE WITH STEEL PROBE; BREAKS EASILY WHEN HIT WITH HAMMER.	
COLOR	L TRICONE TUNG,-CARB. HAND AUGER CORE BIT SOUNDING ROD	INDURATED GRAINS ARE DIFFICULT TO SEPARATE WITH STEEL PROBE;	
DESCRIPTIONS MAY INCLUDE COLOR OR COLOR COMBINATIONS (TAN, RED, YELLOW-BROWN, BLUE-GRAY). MODIFIERS SUCH AS LIGHT, DARK, STREAKED, ETC. ARE USED TO DESCRIBE APPEARANCE.		DIFFICULT TO BREAK WITH HAMMER.	
TOOLING SOUTH OF EASTING DIRING STREENED, ETG. HINE USED TO DESCRIBE HEFEMINANCE.	VANE SHEAR TEST	EXTREMELY INDURATED SHARP HAMMER BLOWS REQUIRED TO BREAK SAMPLE; SAMPLE BREAKS ACROSS GRAINS.	

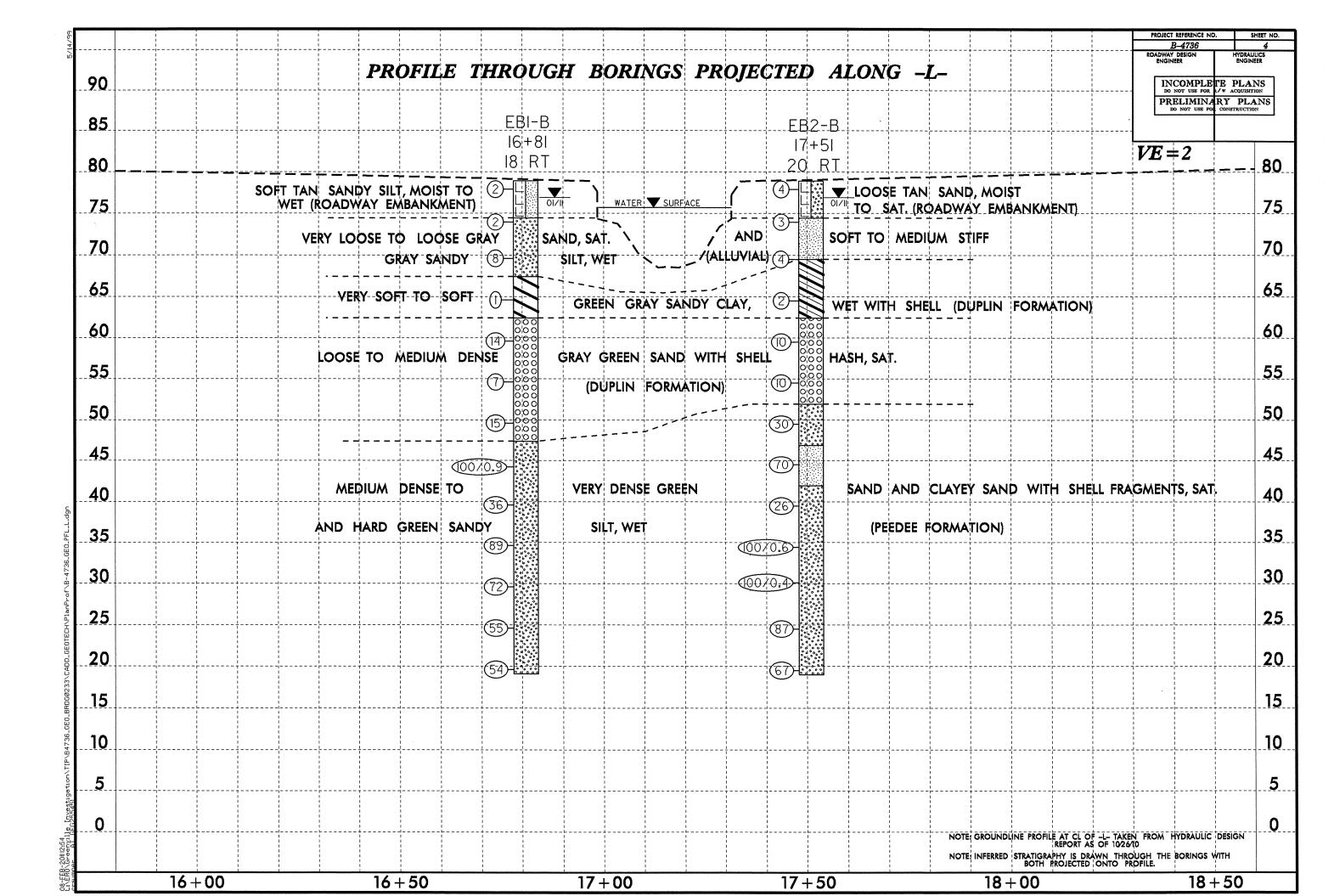
PROJECT REFERENCE NO.

B-4736

SHEET NO.

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TE DES	CRIP	TION	BRI	DGE	NO.	233	ON -L-	(SR	1001) OV	/ER S	LAC	DES	SWA	MP					GROUND WTR (ft	SITI	E DES	CRIPT	ION	BRID	GE N	O. 233	3 ON -L	- (SR	1001)	OVER	SLAD	DES SV	WAM	IP							GROUND W
ORING N	10.	EB1-B	3			STA	TION	16+	81			0	FFS	ET	18 ft	RT			ALIGNMENT -L-	OHR. N/A	BOF	RING	NO. E	B2-B			ST	ATION	17+	51		С	FFSET	T 20	ft RT			ALIG	NMENT	L-		0 HR.
DLLAR E	ELEV.	78.9	9 ft			TOT	AL DE	PTH	59.8	8 ft		N	ORT	THING	25	1,78	9		EASTING 2,119,064	24 HR. 2.0	COL	LAR	ELEV.	78.9	ft		то	TAL DE	EPTH	59.9	ft	N	ORTH	ING	251,8	55		EAST	TING 2	,119,089)	24 HR.
ILL RIG/I	HAMM	ER EF	F./DA	TE (FO00	62 CI	/IE-45B	83%	12/12	/2005					DRII	LL ME	THO) Mu	id Rotary HAMM	MER TYPE Automatic	DRIL	L RIG	/HAMMI	ER EFF	/DATE	GFC	00062 (CME-45B	83%	12/12/2	005				DRILL M	ETHOD) Mu	ud Rotary	,		HAM	MER TYPE Auto
RILLER		th, R.	E.			STA	RT DA	TE	01/18	3/11		С	OM	P. DA					SURFACE WATER DEPTH N	I/A	DRI	LLER	Smit	h, R. I	Ξ.		ST	ART DA	ATE	01/18/ ⁻	11	С	OMP.	DATE	E 01/1	8/11		SURF	FACE V	/ATER D	EPTH	V/A
DRIV ELE) (ft)	VE DE	PTH_ (ft)	BLC 0.5ft	0.5ft		ft)	25 1		S PEI	R FOC	75 75	;	100	1	.	MOI	0	SOIL AND ROCK DES	CRIPTION DEPTH (ELEV (ft)	DRI ELE (ft	EV 105		BLOW).5ft (0	25	BLOWS	PER FC 50	OT 75	j 1	11	SAMP. NO.	моі	L O G		S	OIL AND F	ROCK DE	SCRIPTION
78.9	9 +	0.0	1	1	1			. T		. 1				• •	SS	£-7			78.9 GROUND SURF		80	78	9 1	0.0	2	2	2	1			T .						_	78.9		GRO!	UND SUR	
5 74.9	9 +	4.0	2	1	1			- 1		.		-					•		TAN SANDY SILT, MOI	ST TO WET	5 75	74	9 7 4	1.0	2		2	¶.	-		: :				00.1			- 74.4	TAI	N SAND, M	OIST TO	SATURATED
70.4	6 1	8.3	2	4	4					1		- 1	: :	• •					ALLUVIAL GRAY SAND, SATU	JRATED	70	_70	5 = 8	3.4	4			Ψ3 · · · · · · · · · · · · · · · · · · ·		 				$ \cdot $	SS-1		F				ALLUVIAI SANDY SII	
	Ŧ		2	4	*		. 9 8 . 1 · · ·	:		-		-			SS	5-8		J	67.4 COASTAL PL		5]	Ţ _ Ŧ .		4	3	1	•4	:			::	: : :	:	_SS-2_/			-69.5	GREE	N GRAY S FRAC	GMENTS,	AY WITH SHELL WET
5 65.0	6 <u>†</u> 1	3.3	WOH	WOF	1		11		· · ·	$\frac{1}{1}$::	::	SS	S-9			GREEN GRAY SANDY CLA FRAGMENTS, V (DUPLIN FORMA	VET TION)	<u>65</u>	-65	5 † 1 +	3.4	1	1	1	Q2		· · · · ·	: :		: : :					62.4		·	IN FORM	
60.	6 1	8.3	5	5	9	-	: \; } 1		· · ·	-		-	• •		SS	-10		000000000000000000000000000000000000000	COASTAL PL GRAY GREEN SAND WITH SATURATED (DUPLIN F	AIN H SHELL HASH,	60	60	5 - 1	8.4	3	4	6	. 10	_		: :			\exists	SS-3		000000000000000000000000000000000000000	- -	GRAY SA	GREEN S	ASTAL PI SAND WIT (DUPLIN	AIN H SHELL HASH, FORMATION)
5 55.	6 1/2	3.3	6	4	3	_	· / · · / ·		· · ·	:			::	::				0000 0000 0000 0000	-		55	55	5 = 2	3.4	3	6	4	• 10		· · · · ·		$\cdot \cdot $		-			000000 000000 000000					
50.	6 = 2	8.3							 	:		:	::					000000000000000000000000000000000000000			50	_50	5 + 2	8.4						· · · · · · · · · · · · · · · · · · ·							0000	51.9				
	, † , † ,		3	5	10		•	15.	· · · ·	∹∔-	· · · · · · · · · · · · · · · · · · ·	:	: :						47.4COASTAL PL				#		5	16	14		:	30	1::			-	SS-4			46.9			ASTAL PI	ΔIN
45.	6 + 3	33.3	22	18	82/0	.4	· · · ·	\dashv						00/0.9	SS	-11			GREEN SAND AND CLAYI SHELL FRAGMENTS, S (PEEDEE FORMA	SATURATED	45	45	5 ‡ 3 +	3.4	18	23	47	:::		 		70	• • •		SS-5			41.9		GREEN:	SANDY S EE FORM	LT, WET
40.	6 - 3	88.3	11	18	18	-			●36		`	:	::	• •					<u>-</u>		40	40	5] 3	8.4	8	10	16		. •2	26								- -	GRE	EN SAND	ASTAL PI , SATURA ORMATIC	AIN TED (PEEDEE
35.	6 + 4		31	50	39		: : :				```` 	`.	· · ·	●89	SS	-12		+	.		35	35	5 + 4		39	89 1	1/0.1		- -			**	100/0	0.6				-				
	6 4	18.3	17	37	35		: : :			:	: : :		;/: !::	· · ·							30	30	5 <u>4</u>	8.4	00/0.4			: : :	1		::		: : :	$\begin{bmatrix} \vdots \end{bmatrix}$	SS-6 /							
25	6 + 1	53.3							 		· · · /	/. :	 									25	.5 + 5	31					-	 				<i>;</i>				•				
- 23.	Ŧ		11	24	3′		· · ·	- 1	· · ·	- 1	6 55				SS	-13			-		25		#		21	34	53		:	 			● 87	7 - - -				-				
20.	6 + 5	58.3	20	20	34			_		- -	54			• •		-			Boring Terminated at Elev		8 20	20	.5 + 5	8.4	38	34	33	<u> </u>			• •	• •/ ● 67	• • •	$\frac{\cdot}{\parallel}$				19.0	Borir			vation 19.0 ft IN
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B–4736 38509.1.1

BRIDGE NO. 233 ON SR 1001 (HALLSBORO RD.) OVER SLADES SWAMP

				\overline{SOIL} T	ES	\overline{T} .	RESU	\overline{ILTS}	EB1-	- <i>B</i>		***************************************			
SAMPLE	OFFSET	STATION	DEPTH	AASHTO	L.L.	P.I.			WEIGHT			SING (S.		%	%
NO.	011021	041111011	INTERVAL	CLASS.	2.2.	• ••	C.SAND	F.SAND	SILT	CLAY	10	40	200	MOISTURE	ORGANIC
SS-7	18 RT	16+81	1.0-1.5	A- 4(0)	19	4	2.6	50.7	26.5	20. 2	100	99	59	-	-
SS- 8	18 RT	16+81	8. 3-9. <i>8</i>	A- 2- 4(0)	23	NP	12.8	78.3	4.8	4.0	100	98	12	-	-
SS- 9	18 RT	16+81	13, 3- 14, 8	A-7-6(11)	43	21	12.9	28. 1	20.7	38.3	99	89	63	-	-
55- 10	18 RT	16 +8 1	18. 3- 19. 8	A- 1- b(0)	25	NP	70.9	18.3	3.7	7.1	82	40	10	-	-
SS- 11	18 RT	16+81	<i>33. 3- 34. 8</i>	A-2-4(0)	27	4	11.3	64.0	8.6	16. 1	100	97	32	-	-
55- 12	18 RT	16+81	43. 3-44.8	A- 2- 4(0)	22	NP	17.6	70.4	4.9	7.1	100	99	15	-	-
SS- 13	18 RT	16+81	53. 3- 54. 8	A- 2- 4(0)	32	10	13.7	54.9	7.2	24.2	100	98	33	-	-

			Å	SOIL T	ES	$oldsymbol{T}$.	RESU	VLTS	EB2-	-B					
SAMPLE OFFSET STATION DEPTH AASHTO L.L. P.I. % BY WEIGHT % PASSING (SIEVES) %														%	
NO.	*****		INTERVAL	CLASS.			C.SAND	F.SAND	SILT	CLAY	10	40	200	MOISTURE	ORGANIC
SS- 1	20 RT	17 +5 1	<i>4</i> . 5- 5. 5	A- 4(0)	17	2	2.0	58.7	25. 1	14. 1	100	100	(47)	-	-
SS-2	20 RT	17 +5 1	9. 4- 9. 9	A-6(2)	30	14	24.4	38. 1	19.3	18. 2	100	89	40	-	-
SS- 3	20 RT	17 +5 1	18. 4- 19. 9	A- 1- b(0)	26	NP	77.0	13. 3	4.6	5. 0	86	34	10	-	-
SS- 4	20 RT	17 +5 1	28. 4- 29. 9	A-2-4(0)	23	3	38.7	41.4	5.8	14. 1	96	81	21	-	-
SS- 5	20 RT	17 +5 1	33. 4- 34. 9	A- 4(0)	25	2	6.3	28. 5	57.2	8. 1	100	98	68	-	-
SS-6	20 RT	17 +5 1	48. 4- 48. 8	A-2-4(0)	25	NP	3.5	85.5	4.9	6. 1	100	100	13	-	-



FIELD SCOUR REPORT

WBS:	38509.1.1 TIP:	B-4736	COUNTY: BLADEN/	COLUMBUS
DESCRIPTION(1): BR	IDGE NO. 233 ON SR	1001 OVER SLA	DES SWAMP	
		EXISTING	BRIDGE	
Information from:	Field Inspection Other (explain)	X Micr BSR	ofilm (reel p	pos:)
Bridge No.: 233 Foundation Type: TIM	Length: 36'	Total Bents: 3	Bents in Channel: 1	Bents in Floodplain: 2
EVIDENCE OF SCO Abutments or End	DUR(2) Bent Slopes: <u>NONE N</u>	NOTED		
Interior Bents: <u>NO</u>	NE NOTED			
Channel Bed: <u>NO</u>	NE NOTED			
Channel Bank: <u>NO</u>	NE NOTED			
EXISTING SCOUR I Type(3): WC	PROTECTION DODEN END WALLS			
Extent(4):				
Effectiveness(5): EFF	FECTIVE			
Obstructions(6): NO	NE NOTED			

INSTRUCTIONS

- 1 Describe the specific site's location, including route number and body of water crossed.
- 2 Note scour evidence at existing end bents or abutments (e.g. undermining, sloughing, degradations).
- 3 Note existing scour protection (e.g. rip rap).
- 4 Describe extent of existing scour protection.
- 5 Describe whether or not the scour protection appears to be working.
- 6 Note obstructions such as dams, fallen trees, debris at bents, etc.
- 7 Describe the channel bed material based on observation and/or samples. Include any lab results with report.
- 8 Describe the channel bank material based on observation and/or samples. Include any lab results with report.
- 9 Describe the material covering the banks (e.g. grass, trees, rip rap, none).
- 10 Determine the approximate floodplain width from field observation or a topographic map.
- 11 Describe the material covering the floodplain (e.g. grass, trees, crops).
- 12 Use professional judgement to specify if the stream is degrading, aggrading, or static.
- 13 Describe potential and direction of the stream to migrate laterally during the bridge's life (approx. 100 years).
- Give the design scour elevation (DSE) expected over the life of the bridge (approx. 100 years). This elevation can be given as a range across the site, or for each bent. Discuss the relationship between the Hydraulics Unit theoritical scour and the DSE. If the DSE is dependent on scour counter measures, explain (e.g. rip rap armoring on slopes). The DSE is based on the erodability of materials, giving consideration to the influence of joints, foliation, bedding characteristics, % core recovery, % RQD, differential weathering, shear strength, observations at existing structures, other tests deemed appropriate, and overall geologic conditions at the site.

Channel Bank Material(7): NA Channel Bank Cover(9): TREES AND SANDY SILT Channel Bank Cover(9): TREES AND SHRUBS Floodplain Width(10): 250° ± Floodplain Cover(11): TREES AND SHRUBS Stream is(12): Aggrading Degrading StaticX					DES	SIGN IN	IFORM	<u>ATION</u>					
Channel Bank Cover(9): TREES AND SHRUBS Floodplain Width(10): 250' ± Floodplain Cover(11): TREES AND SHRUBS Stream is(12): Aggrading Degrading StaticX Channel Migration Tendency(13): LOW TENDENCY TO MIGRATE NORTH Observations and Other Comments: LARGE BEAVER DAM UPSTREAM OF BRIDGE DESIGN SCOUR ELEVATIONS(14) FeetX Meters CHANNEL BED CHANNEL BED COMparison of DSE to Hydraulics Unit theoretical scour: THE DSE AGREES WITH THE MAXIMUM THEORETICAL SCOUR ELEVATIONS AS OUTLINED IN THE BSR DATED 10/26/10. SOIL ANALYSIS RESULTS FROM CHANNEL BED AND BANK MATERIAL Bed or Bank Sample No	Channel	Bed Mat	erial(7):	NA									
Floodplain Width(10): 250' ± Floodplain Cover(11): TREES AND SHRUBS Stream is(12): Aggrading Degrading Static _X Channel Migration Tendency(13): LOW TENDENCY TO MIGRATE NORTH Observations and Other Comments: LARGE BEAVER DAM_UPSTREAM OF BRIDGE DESIGN SCOUR ELEVATIONS(14) Feet _X Meters CHANNEL BED Comparison of DSE to Hydraulics Unit theoretical scour: THE DSE AGREES WITH THE MAXIMUM THEORETICAL SCOUR ELEVATIONS AS OUTLINED IN THE BSR DATED 10/26/10. SOIL ANALYSIS RESULTS FROM CHANNEL BED AND BANK MATERIAL Bed or Bank Sample No.	Channel B	ank Mat	erial(8):	SAND A	ND SAN	NDY SILT	Γ						,
Stream is(12): Aggrading Degrading Static _X	Channel	Bank Co	over(9):	TREES	AND SH	IRUBS							
Stream is(12): Aggrading Degrading Static _X hannel Migration Tendency(13): LOW TENDENCY TO MIGRATE NORTH Observations and Other Comments: LARGE BEAVER DAM_UPSTREAM OF BRIDGE DESIGN SCOUR ELEVATIONS(14) Feet _X Meters CHANNEL BED 67.7	Flood	plain Wi	dth(10):	250' ±		·		,,,,					
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DESIGN SCOUR ELEVATIONS(14) Feet X Meters CHANNEL BED CHANNEL BED CHANNEL BED COMparison of DSE to Hydraulics Unit theoretical scour: THE DSE AGREES WITH THE MAXIMUM THEORETICAL SCOUR ELEVATIONS AS OUTLINED IN THE BSR DATED 10/26/10. SOIL ANALYSIS RESULTS FROM CHANNEL BED AND BANK MATERIAL Bed or Bank Sample No. Retained #4 Passed #10 Passed #10 Passed #20 Coarse Sand "Soil Test Results", Fine Sand Sitt SRS1, SS-8 (CHANNEL BANK) Clay LL PI AASHTO Station Offset		Stream	n is(12):	Ag	grading		Degr	ading		Sta	atic X	-	
CHANNEL BED CHANNEL BED 67.7 COMparison of DSE to Hydraulics Unit theoretical scour: THE DSE AGREES WITH THE MAXIMUM THEORETICAL SCOUR ELEVATIONS AS OUTLINED IN THE BSR DATED 10/26/10. SOIL ANALYSIS RESULTS FROM CHANNEL BED AND BANK MATERIAL Bed or Bank Sample No. Retained #4 Passed #10 Passed #200 Coarse Sand Fine Sand Silt Clay LL PI AASHTO Station Offset Neet X Meters Meters Meters Meters Meters See I Meters Me	hannel Migration	n Tender	ncy(13):	LOW TE	NDENC	CY TO M	IGRATE	NORTH					
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Form GEU-017e Revised 7/26/											Form GELL	-0179	Revised 7/26/2

Reported by: Date: 2/8/2011