NOTE: SEE SHEET 2A FOR PLAN SHEET LAYOUT AT TIME OF INVESTIGATION

STATE OF NORTH CAROLINA

DEPARTMENT OF TRANSPORTATION **DIVISION OF HIGHWAYS**

CONTENTS

C20295

LINE STATION **PLAN PROFILE** 13+00 TO 20+50

ROADWAY

GEOTECHNICAL ENGINEERING UNIT

SHEET **CROSS SECTIONS** STATION 15+00 TO 16+50 17+50 TO 18+50 6-7

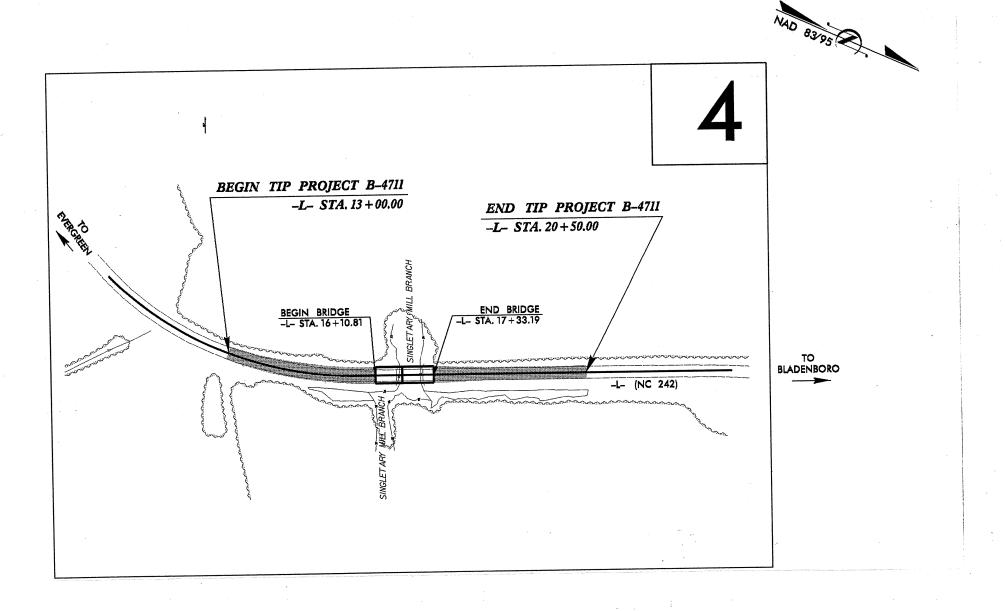
SUBSURFACE INVESTIGATION

PROJ. REFERENCE NO. 38486.1.1 (B-4711) _ F.A. PROJ. *BRSTP-242(3)* COUNTY BLADEN

PROJECT DESCRIPTION BRIDGE NO. 5 OVER SINGLETARY MILL BRANCH

ON NC 242

INVENTORY



STATE STATE PROJECT REFERENCE NO. N.C. 38486.1.1 (B-4711) STATE PROLNO. F.A.PROJ.NO. 38486.1.1 BRSTP-242(3)

CAUTION NOTICE

THE SUBSURFACE INFORMATION AND THE SUBSURFACE INVESTIGATION ON WHICH IT IS BASED WERE MADE FOR THE PURPOSE OF STUDY, PLANNING, AND DESIGNA AND NOT FOR CONSTRUCTION OR PAY PURPOSES. THE VARIOUS FIELD BORING LOGS, ROCK CORES, AND SOIL TEST DATA AVAILABLE MAY BE REVIEWED OR INSPECTED IN PALEIGH BY CONTACTING THE N. C. DEPARTMENT OF TRANSPORTATION, GEOTECHNICAL ENGINEFRING UNIT AT (919) 250-4088. NEITHER THE SUBSURFACE PLANS AND REPORTS. NOR THE FIELD BORING LOGS, ROCK CORES, OR SOIL TEST DATA ARE PART OF THE CONTRACT.

GENERAL SOIL AND ROCK STRATA DESCRIPTIONS AND INDICATED BOUNDARIES ARE BASED ON A GEOTECHNICAL INTERPRETATION OF ALL AVAILABLE SUBSURFACE DATA AND MAY NOT NECESSARILY REFLECT THE ACTUAL SUBSURFACE CONDITIONS BETWEEN BORNINGS OR BETWEEN SAMPLED STRATA WITHIN THE BOREHOLE. THE LABORATORY SAMPLE DATA AND THE IN SITU IN-PLACEJTEST DATA CAN BE RELIED ON ONLY TO THE DEGREE OF RELIGIBLITY INMERSTY IN THE STANDARD TEST METHOD. THE OBSERVED WATER LEVELS OR SOIL MOISTURE CONDITIONS INDICATED IN THE SUBSURFACE THE OSSERVEY WHERE LEVELS ON SOIL MOST SECOND OF THE INVESTIGATION. THESE WATER LEVELS OR SOIL MOISTURE CONDITIONS MAY VARY CONSIDERABLY WITH TIME ACCORDING TO CLIMATIC CONDITIONS INCLUDING TEMPERATURES, PRECIPITATION, AND WIND, AS WELL AS OTHER NON-CLIMATIC FACTORS.

THE BIDDER OR CONTRACTOR IS CAUTIONED THAT DETAILS SHOWN ON THE SUBSURFACE PLANS ARE PRELIMINARY ONLY AND IN MANY CASES THE FINAL DESIGN DETAILS ARE DIFFERENT. FOR BIDDING AND CONSTRUCTION PURPOSES, REFER TO THE CONTRUCTION PLANS AND DOCUMENTS FOR FINAL DESIGN INFORMATION ON THIS PROJECT. THE DEPARTMENT DOES NOT WARRANT OR CUARANTEE THE SUFFICIENCY OR ACCURACY OF THE INVESTIGATION MADE, NOR THE INTERPRETATIONS MADE, OR DINION OF THE DEPARTMENT AS TO THE TYPE OF MATERIALS AND CONDITIONS TO BE ENCOUNTERED. THE BIDDER OR CONTRACTOR IS CAUTIONED TO MAKE SUCH INDEPENDENT SUBSURFACE INVESTIGATIONS AS HE DEEMS NECESSARY TO SATISFY HINSELF AS TO CONDITIONS TO BE ENCOUNTERED ON THIS PROJECT. ON CLAIM FOR ADDITIONAL COMPRENATION OR FOR AN EXTENSION OF TIME FOR ANY REASON RESULTING FROM THE ACTUAL CONDITIONS ENCOUNTEED AT THE SITE DIFFERING FROM THOSE INDICATED IN THE SURPLIFEACE MEGRATION. THOSE INDICATED IN THE SUBSURFACE INFORMATION.

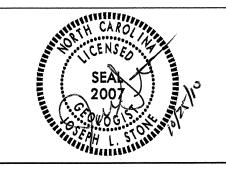
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INVESTIGATED BY J.L. STONE

D.N. ARGENBRIGHT

SUBMITTED BY____D.N. ARGENBRIGHT

OCTOBER 2010



NORTH CAROLINA DEPARTMENT OF TRANSPORTATION

PROJECT REFERENCE NO.

B-47II

SHEET NO.

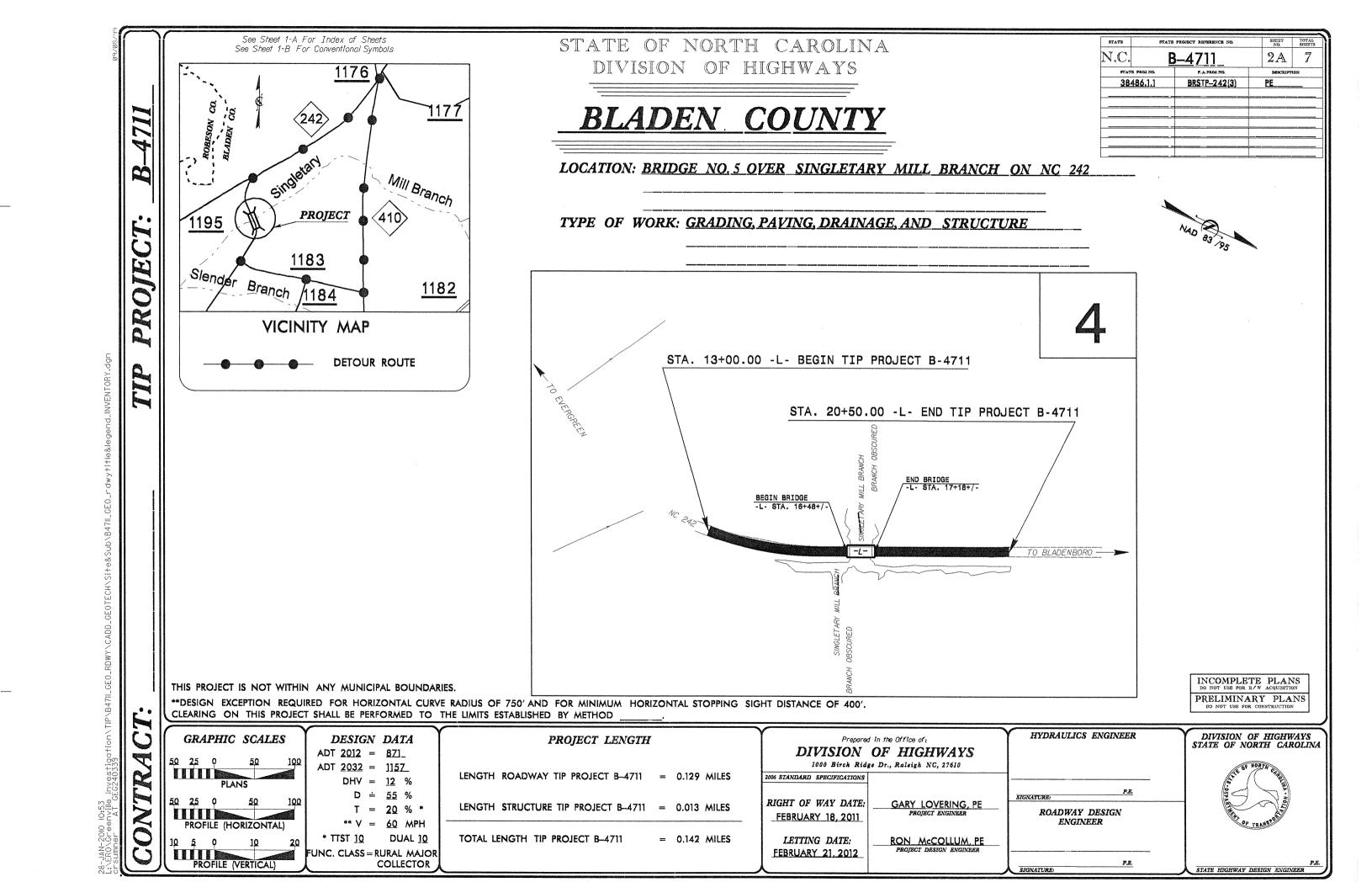
2 OF 7

DIVISION OF HIGHWAYS

GEOTECHNICAL ENGINEERING UNIT

SUBSURFACE INVESTIGATION

				501				4		
			•	SOIL AND RO	CK LEGEND, TERM	c cymroi c	ANTO ADDDICT	ZI A TIONS		
	SOIL DESCRIPTION		T	GRADATION		o, or midolo				
SOUL IS CONSIDERED TO BE THE UNCONS	SOLIDATED, SEMI-CONSOLIDATED, OR W	EATHERED FARTH MATERIALS	WELL GRADED - INDICATES A GOOD	ORADATION REPRESENTATION OF PARTICLE SIZES PARTICLES ARE ALL APPROXIMATELY TH	FROM FINE TO COARSE.	HARD ROCK IS NO	N-COASTAL PLAIN MATERIAL	K DESCRIPTION THAT IF TESTED, WOULD YIELD SPT F	REFUSAL. AN INFERRED	TERMS AND DEFINITIONS ALLUVIUM (ALLUV.) - SOILS THAT HAVE BEEN TRANSPORTED BY WATER.
THAT CAN BE PENETRATED WITH A CONT 100 BLOWS PER FOOT ACCORDING TO STA	TINUOUS FLIGHT POWER AUGER, AND YI (ANDARD PENETRATION TEST (AASHTO	TELD LESS THAN T206, ASTM D-1586). SOIL	POORLY GRADED)	RE OF UNIFORM PARTICLES OF TWO OR		SPT REFUSAL IS	PENETRATION BY A SPLIT SPO	ON-COASTAL PLAIN MATERIAL WOULD DON SAMPLER EQUAL TO OR LESS TH ITION BETWEEN SOIL AND ROCK IS O	IAN RITEONT PER 60 BLOWS	ADULTER - A WATER REARING ENRATION OR STRATA
CLASSIFICATION IS BASED ON THE AASH CONSISTENCY, COLOR, TEXTURE, MOISTURE,	AASHTO CLASSIFICATION, AND OTHER	R PERTINENT FACTORS SUCH		ANGULARITY OF GRAINS		OF WEATHERED RO	OCK. ARE TYPICALLY DIVIDED AS F		FIEN REPRESENTED BY A ZUNE	ARENACEOUS - APPLIED TO ROCKS THAT HAVE BEEN DERIVED FROM SAND OR THAT CONTAIN SAND.
AS MINERALOGICAL COMPOSITION, ANGULA VERY STIFF, GRAY, SILTY CLA	ARTIY, STRUCTURE, PLASTICITY, ETC. EX LAY, MONST WITH INTERBEDOED FINE SAND LAYERS, HIG		THE ANGULARITY OR ROUNDNESS I SUBANGULAR, SUBROUNDED, OR ROU	OF SOIL GRAINS IS DESIGNATED BY THE INDED.	TERMS: ANGULAR,	WEATHERED	NON-COASTAL	PLAIN MATERIAL THAT WOULD YIEL	.D SPT N VALUES > 100	ARGILLACEOUS - APPLIED TO ALL ROCKS OR SUBSTANCES COMPOSED OF CLAY MINERALS, OR HAVING A NOTABLE PROPORTION OF CLAY IN THEIR COMPOSITION, AS SHALE, SLATE, ETC.
SOIL LEGENE	D AND AASHTO CLASSIF	FICATION		INERALOGICAL COMPOSITION		ROCK (WR)	Language and the second	FOOT IF TESTED. RSE GRAIN IGNEOUS AND METAMORPH	IT BUCK THAT	ARTESIAN - GROUND WATER THAT IS UNDER SUFFICIENT PRESSURE TO RISE ABOVE THE LEVEL AT WHICH IT IS ENCOUNTERED, BUT WHICH DOES NOT NECESSARILY RISE TO DR ABOVE THE
GENERAL GRANULAR MATERIA CLASS. (≤35% PASSING *2			MINERAL NAMES SUCH AS QUARTZ, WHENEVER THEY ARE CONSIDERED	FELDSPAR, MICA, TALC, KAOLIN, ETC. ARE OF SIGNIFICANCE.	USED IN DESCRIPTIONS	CRYSTALLINE ROCK (CR)	WOULD YIELD	SPT REFUSAL IF TESTED. ROCK TY	PE INCLUDES GRANITE,	GROUND SURFACE. CALCAREOUS (CALC.) - SOILS THAT CONTAIN APPRECIABLE AMOUNTS OF CALCIUM CARBONATE.
0,000		A-7 A-1, A-2 A-4, A-5		COMPRESSIBILITY		NON-CRYSTALLINE ROCK (NCR)	SEDIMENTARY	RSE GRAIN METAMORPHIC AND NON-C ROCK THAT WOULD YEILD SPT REFL		COLLUVIUM - ROCK FRAGMENTS MIXED WITH SOIL DEPOSITED BY GRAVITY ON SLOPE OR AT BOTTOM
CLASS. A-1-a A-1-b A-2-4 A-2 SYMBOL 000000000000000000000000000000000000	2-5 A-2-6 A-2-7	F7-5 A-3 A-6, A-7	SLIGHTLY COMPRESSIBLE MODERATELY COMPRESSI	BLE LIQUID LIMIT	LESS THAN 31 EQUAL TO 31-50	COASTAL PLAIN	INCLUDES PH	YLLITE, SLATE, SANDSTONE, ETC. IN SEDIMENTS CEMENTED INTO ROCK	. BUT MAY NOT YIFI D	OF SLOPE. CORE RECOVERY (REC.) - TOTAL LENGTH OF ALL MATERIAL RECOVERED IN THE CORE BARREL DIVIDED BY TOTAL
% PASSING			HIGHLY COMPRESSIBLE	PERCENTAGE OF MATERIA	GREATER THAN 50	SEDIMENTARY ROCK (CP)	SHELL BEDS,		SANDSTONE, CEMENTED	LENGTH OF CORE RUN AND EXPRESSED AS A PERCENTAGE.
* 10 50 MX * 40 30 MX 50 MX 51 MN		GRANULAR SILT- CLAY PEAT		NULAR SILT - CLAY SOILS SOILS	OTHER MATERIAL			<u>IEATHERING</u>		DIKE - A TABULAR BODY OF IGNEOUS ROCK THAT CUTS ACROSS THE STRUCTURE OF ADJACENT ROCKS OR CUTS MASSIVE ROCK.
# 200 15 MX 25 MX 10 MX 35 MX 35	MX 35 MX 35 MX 36 MN 36 MN 36 MN 36	6 MN SOILS SOILS	TRACE OF ORGANIC MATTER 2	- 3% 3 - 5% TR	ACE 1 - 10% TTLE 10 - 20%		FRESH, CRYSTALS BRIGHT, FEW ER IF CRYSTALLINE.	JOINTS MAY SHOW SLIGHT STAININ	G. ROCK RINGS UNDER	<u>DIP</u> - THE ANGLE AT WHICH A STRATUM OR ANY PLANAR FEATURE IS INCLINED FROM THE HORIZONTAL.
PLASTIC INDEX 6 MX NP 10 MX 10	MN 40 MX 41 MN 40 MX 41 MN 40 MX 4: MX 11 MN 11 MN 10 MX 10 MX 11 MN 11	I MN	MODERATELY DRGANIC 5	- 10% 12 - 20% SO	ME 20 - 35%	VERY SLIGHT ROCK (V SLI.) CRYST	GENERALLY FRESH, JOINTS ST	AINED, SOME JOINTS MAY SHOW THIN FACE SHINE BRIGHTLY, ROCK RINGS	CLAY COATINGS IF OPEN,	DIP DIRECTION (DIP AZIMUTH) - THE DIRECTION OR BEARING OF THE HORIZONTAL TRACE OF
GROUP INDEX 0 0 0	4 MX 8 MX 12 MX 16 MX No	MODERATE DRGANIC	THORE FOR OTHER	GROUND WATER	SHLY 35% AND ABOVE	OF A	CRYSTALLINE NATURE.			THE LINE OF DIP, MEASURED CLOCKWISE FROM NORTH. FAULT - A FRACTURE OR FRACTURE ZONE ALONG WHICH THERE HAS BEEN DISPLACEMENT OF THE
	OR CLAYEY SILTY CLAYE IL AND SAND SOILS SOILS	EY ORGANIC SULLS	l	IN BORE HOLE IMMEDIATELY AFTER	DRILLING	(SLI.) 1 INCH	. OPEN JOINTS MAY CONTAIN	AINED AND DISCOLORATION EXTENDS CLAY, IN GRANITOID ROCKS SOME OF ED. CRYSTALLINE ROCKS RING UNDER	CCASIONAL FELDSPAR	SIDES RELATIVE TO ONE ANOTHER PARALLEL TO THE FRACTURE.
MATERIALS SAND SAND GRAVEL	L AND SAND SOILS SOILS			R LEVEL AFTER 24 HOURS		MODERATE SIGNIF	TICANT PORTIONS OF ROCK SH	OW DISCOLORATION AND WEATHERING	EFFECTS. IN	FISSILE - A PROPERTY OF SPLITTING ALONG CLOSELY SPACED PARALLEL PLANES. FLOAT - ROCK FRAGMENTS ON SURFACE NEAR THEIR ORIGINAL POSITION AND DISLODGED FROM
AS A EXCELLENT TO GOO	DOD FAIR TO POOR	FAIR TO POOR UNSUITABLE	1	ER, SATURATED ZONE, OR WATER BEAR	ING STRATA	DULL	SOUND UNDER HAMMER BLOWS	ARE DULL AND DISCOLORED, SOME S AND SHOWS SIGNIFICANT LOSS OF S		PARENT MATERIAL.
PI OF A-7-5 SUBGROUP IS	≤ LL - 30 ; PI OF A-7-6 SUI		SPRING OR SI	EEP		1	FRESH ROCK. OCK EXCEPT QUARTZ DISCOLO	RED OR STAINED. IN GRANITOID ROCE	VS ALL FELDSPARS DULL	FLOOD PLAIN (FP) - LAND BORDERING A STREAM, BUILT OF SEDIMENTS DEPOSITED BY THE STREAM.
	SISTENCY OR DENSENES			MISCELLANEOUS SYMBOLS		SEVERE AND D	ISCOLORED AND A MAJORITY	SHOW KAOLINIZATION, ROCK SHOWS S OLOGIST'S PICK, ROCK GIVES CLUNK	EVERE LOSS OF STRENGTH	FORMATION (FM.) - A MAPPABLE GEOLOGIC UNIT THAT CAN BE RECOGNIZED AND TRACED IN THE FIELD.
PRIMARY SOIL TYPE COMPACTNI CONSIST		ICE COMPRESSIVE STRENGTH (TDNS/FT ²)	ROADWAY EMBANKMEN WITH SOIL DESCRIPTI		NG TEST BORING W/ CORE	IF TE	STED, WOULD YIELD SPT REFU	SAL		JOINT - FRACTURE IN ROCK ALONG WHICH NO APPRECIABLE MOVEMENT HAS OCCURRED.
GENERALLY VERY LO			SOIL SYMBOL	AUGER BORING	SPT N-VALUE	(SEV.) IN ST	RENGTH TO STRONG SOIL. IN	RED OR STAINED ROCK FABRIC CLEA GRANITOID ROCKS ALL FELDSPARS A	R AND EVIDENT BUT REDUCED RE KAOLINIZED TO SOME	LEDGE - A SHELF-LIKE RIDGE OR PROJECTION OF ROCK WHOSE THICKNESS IS SMALL COMPARED TO
GHANULAK MEDIUM DENSE 10 TO 30 N/A ARTIFICIAL FILL (AF) OTHER ————————————————————————————————————					IT. SOME FRAGMENTS OF STRO STED, YIELDS SPT N VALUES			LENS - A BODY OF SOIL OR ROCK THAT THINS OUT IN ONE OR MORE DIRECTIONS.		
VERY DE	ENSE >50		THAN ROADWAY EMBAI	MW	11	VERY SEVERE ALL R	OCK EXCEPT QUARTZ DISCOLO	RED OR STAINED. ROCK FABRIC ELEN D TO SOIL STATUS, WITH ONLY FRAG	MENTS ARE DISCERNIBLE BUT	MOTTLED (MOT.) - IRREGULARLY MARKED WITH SPOTS OF DIFFERENT COLORS, MOTTLING IN SOILS USUALLY INDICATES POOR AERATION AND LACK OF GOOD DRAINAGE.
VERY SOFT (2 (8.25				REMAI	REMAINING, SAPROLITE IS AN EXAMPLE OF ROCK WEATHERED TO A DEGREE SUCH THAT ONLY MINOR VESTIGES OF THE ORIGINAL ROCK FABRIC REMAIN. IF IESTED, YIELDS SPT N VALUES (100 BPF INTERVENING IMPERVIOUS STRATUM.					
SILT-CLAY MEDIUM MATERIAL STIFF	8 TO 15	0.5 TO 1.0 1 TO 2	***** ALLUVIAL SOIL BOUN	INSTALLATION	0.00	COMPLETE ROCK	REDUCED TO SOIL. ROCK FABR	NC NOT DISCERNIBLE, OR DISCERNIBL	E DNLY IN SMALL AND	RESIDUAL (RES.) SOIL - SOIL FORMED IN PLACE BY THE WEATHERING OF ROCK.
(COHESIVE) VERY ST. HARD		2 TO 4	25/025 DIP & DIP DIRECTION	INSTALLATION OF	un		ERED CONCENTRATIONS. DUART AN EXAMPLE.	Z MAY BE PRESENT AS DIKES OR S	TRINGERS. SAPROLITE IS	ROCK DUALITY DESIGNATION (ROD) - A MEASURE OF ROCK QUALITY DESCRIBED BY TOTAL LENGTH OF ROCK SEGMENTS EQUAL TO OR GREATER THAN 4 INCHES DIVIDED BY THE TOTAL LENGTH OF CORE RUN AND
TE.	EXTURE OR GRAIN SIZE		ROCK STRUCTURES	CONE PENETRON	METER TEST			CK HARDNESS		EXPRESSED AS A PERCENTAGE.
U.S. STD. SIEVE SIZE		200 270		SOUNDING ROD			IOT BE SCRATCHED BY KNIFE RAL HARD BLOWS OF THE GE(OR SHARP PICK. BREAKING OF HAND OLOGIST'S PICK.	SPECIMENS REQUIRES	SAPPOLITE (SAP.) - RESIDUAL SOIL THAT RETAINS THE RELIC STRUCTURE OR FABRIC OF THE PARENT ROCK.
	4.76 2.00 0.42 0.25 0	FINE		ABBREVIATIONS			BE SCRATCHED BY KNIFE OR ETACH HAND SPECIMEN.	PICK ONLY WITH DIFFICULTY, HARD I	HAMMER BLOWS REQUIRED	SILL - AN INTRUSIVE BODY OF IGNEOUS ROCK OF APPROXIMATELY UNIFORM THICKNESS AND RELATIVELY THIN COMPARED WITH JTS LATERAL EXTENT, THAT HAS BEEN EMPLACED PARALLEL
	KAVEL SAND S	SAND (SL.) (CL.)	AR - AUGER REFUSAL BT - BORING TERMINATED	MED MEDIUM MICA MICACEOUS	VST - VANE SHEAR TEST WEA WEATHERED	MODERATELY CAN	BE SCRATCHED BY KNIFE OR	PICK. GOUGES OR GROOVES TO 0.25	INCHES DEEP CAN BE	TO THE BEDDING OR SCHISTOSITY OF THE INTRUDED ROCKS. SLICKENSIDE - POLISHED AND STRIATED SURFACE THAT RESULTS FROM FRICTION ALONG A FAULT OR
GRAIN MM 305 75	2.0 0.25	0.05 0.005	CL CLAY CPT - CONE PENETRATION TES	MOD MODERATELY T NP - NON PLASTIC	7 - UNIT WEIGHT 7 - DRY UNIT WEIGHT	HARD EXCA	IVATED BY HARD BLOW OF A 1 IODERATE BLOWS.	GEOLOGIST'S PICK, HAND SPECIMENS	CAN BE DETACHED	SLIP PLANE.
SIZE IN. 12 3	URE - CORRELATION OF	F TERMS	CSE COARSE DMT - DILATOMETER TEST	ORG ORGANIC PMT - PRESSUREMETER TEST	SAMPLE_ABBREVIATIONS			INCHES DEEP BY FIRM PRESSURE O PS TO PEICES 1 INCH MAXIMUM SIZE		STANDARD PENETRATION TEST (PENETRATION RESISTANCE) (SPT) - NUMBER OF BLOWS (N OR BPF) OF A 140 LB. HAMMER FALLING 30 INCHES REQUIRED TO PRODUCE A PENETRATION OF 1 FOOT INTO SOIL WITH
SOIL MOISTURE SCALE	FIELD MOISTURE CHIDE	FOR FIELD MOISTURE DESCRIPTION	DPT - DYNAMIC PENETRATION ' - VOID RATIO	TEST SAP SAPROLITIC SD SAND, SANDY	S - BULK SS - SPLIT SPOON	ł	T OF A GEOLOGIST'S PICK.	LY BY KNIFE OR PICK. CAN BE EXCA	NATED IN EDACHENTS	A 2 INCH DUTSIDE DIAMETER SPLIT SPOON SAMPLER, SPT REFUSAL IS PENETRATION EQUAL TO OR LESS THAN 0.1 FOOT PER 60 BLOWS.
(ATTERBERG LIMITS)	DESCRIPTION		F - FINE FOSS FOSSILIFEROUS	SL SILT, SILTY SLI SLIGHTLY	ST - SHELBY TUBE RS - ROCK	FROM		IN SIZE BY MODERATE BLOWS OF A		STRATA CORE RECOVERY (SREC.) - TOTAL LENGTH OF STRATA MATERIAL RECOVERED DIVIDED BY TOTAL LENGTH OF STRATUM AND EXPRESSED AS A PERCENTAGE.
LL LIQUID LIMIT		LY LIDUID; VERY WET, USUALLY BELOW THE GROUND WATER TABLE	FRAC FRACTURED, FRACTURES FRAGS FRAGMENTS		RT - RECOMPACTED TRIAXIAL CBR - CALIFORNIA BEARING	VERY CAN	BE CARVED WITH KNIFE. CAN	BE EXCAVATED READILY WITH POINT	OF PICK. PIECES 1 INCH	STRATA ROCK QUALITY DESIGNATION (SROD) - A MEASURE OF ROCK QUALITY DESCRIBED BY
PLASTIC	SEMISO	DLID: REQUIRES DRYING TO	HI HIGHLY	V - VERY	RATIO		ORE IN THICKNESS CAN BE B ERNAIL.	ROKEN BY FINGER PRESSURE. CAN BE	SCRATCHED READILY BY	TOTAL LENGTH OF ROCK SEGMENTS WITHIN A STRATUM EQUAL TO OR GREATER THAN 4 INCHES DIVIDED BY THE TOTAL LENGTH OF STRATA AND EXPRESSED AS A PERCENTAGE.
RANGE (PI) PLASTIC LIMIT _		OPTIMUM MOISTURE	EQUIP	MENT USED ON SUBJECT I		1	URE SPACING	BEDD TERM	ING THICKNESS	TOPSOIL (TS.) - SURFACE SOILS USUALLY CONTAINING ORGANIC MATTER.
OM OPTIMUM MOISTURE	- MOIST - (M) SOLID	O: AT OR NEAR OPTIMUM MOISTURE	DRILL UNITS:	ADVANCING TOOLS:	HAMMER TYPE: AUTOMATIC MANUAL	TERM VERY WIDE	SPACING MORE THAN 10 FEET	VERY THICKLY BEDDED	> 4 FEET	BENCH MARK:
SL SHRINKAGE LIMIT _			MOBILE B-	CLAY BITS	The contract of the contract o	WIDE MODERATELY CLO		THICKLY BEDDED THINLY BEDDED	1.5 - 4 FEET 0.16 - 1.5 FEET	ELEVATION: FT.
		RES ADDITIONAL WATER TO N OPTIMUM MOISTURE	BK-51	6° CONTINUOUS FLIGHT AUGER 8° HOLLOW AUGERS	CORE SIZE:	CLOSE VERY CLOSE	0.16 TO 1 FEET LESS THAN 0.16 FEET	VERY THINLY BEDDED THICKLY LAMINATED	0.03 - 0.16 FEET 0.008 - 0.03 FEET	NOTES:
	PLASTICITY		- CME-458	HARD FACED FINGER BITS			II	THINLY LAMINATED NDURATION	< 0.008 FEET	
	PLASTICITY INDEX (PI)	DRY STRENGTH		TUNGCARBIDE INSERTS	☐-N	FOR SEDIMENTARY RO		DENING OF THE MATERIAL BY CEMEN		
NONPLASTIC LOW PLASTICITY	0-5 6-15	VERY LOW SLIGHT	CME-550	CASING W/ ADVANCER	HAND TODLS:	FRIABLE		ING WITH FINGER FREES NUMEROUS (LE BLOW BY HAMMER DISINTEGRATES		
MED. PLASTICITY HIGH PLASTICITY	16-25 26 OR MORE	MEDIUM HIGH	PORTABLE HOIST	TRICONE STEEL TEETH	POST HOLE DIGGER	MODERATE	LY INDURATED GRAIN	NS CAN BE SEPARATED FROM SAMPLE		
	COLOR			TRICONE TUNGCARB.	X HAND AUGER	INDURATE		KS EASILY WHEN HIT WITH HAMMER. NS ARE DIFFICULT TO SEPARATE WIT	TU CTEEL DOODE	
DESCRIPTIONS MAY INCLUDE COLOR				CORE BIT	SOUNDING ROD VANE SHEAR TEST	INDUKATEL	DIFF	ICULT TO BREAK WITH HAMMER.	,	
MUDIFIERS SUCH AS LIGHT, DARK	K,STREAKED,ETC.ARE USED TO DE	ESCHIBE APPEARANCE.			X PROBE	EXTREMEL		P HAMMER BLOWS REQUIRED TO BREACLE BREAKS ACROSS GRAINS.	AK SAMPLE;	
										





STATE OF NORTH CAROLINA DEPARTMENT OF TRANSPORTATION

BEVERLY EAVES PERDUE
GOVERNOR

EUGENE A. CONTI, JR. SECRETARY

October 25, 2010

STATE PROJECT:

38486.1.1 (B-4711)

F.A. PROJECT:

BRSTP-242(3)

COUNTY:

Bladen

DESCRIPTION:

Bridge No. 5 over Singletary Mill Branch on

NC 242

SUBJECT:

Geotechnical Inventory

Project Description

This project is along NC 242 approximately 2.5 miles south of Bladenboro at the existing Singletary Mill Branch Bridge. Proposed construction consists of an in place replacement of the bridge with a slight increase in grade elevation. The associated approaches will be widened slightly to accommodate the proposed grade increase. This geotechnical investigation was confined to the limits of proposed construction.

Fieldwork for this project was conducted during December of 2009 and January 2010. Hand auger borings and soil probes were performed at various off sets from the proposed alignment. Representative soil samples were collected for visual classification in the field and for laboratory analysis by the Materials and Tests Unit.

The following alignment, totaling 750 feet was investigated. A subsurface profile and selected cross sections of this alignment is included in this report.

<u>Line</u>

-L-

Station(±) 13+00 to 20+50

MAILING ADDRESS:
NC DEPARTMENT OF TRANSPORTATION
GEOTECHNICAL ENGINEERING UNIT
1589 MAIL SERVICE CENTER

RALFIGH NC 27699-1589

TELEPHONE: 919-250-4088 FAX: 919-250-4237

WEBSITE: WWW.NCDOT.ORG/DOH

LOCATION: CENTURY CENTER COMPLEX ENTRANCE B-2 1020 BIRCH RIDGE DRIVE RALEIGH NC Sheet 3

Areas of Special Geotechnical Interest

1) The following section contains organic soils which have the potential to cause embankment stability and/or long term settlement problems:

Line	$\underline{Station(\pm)}$
-L-	14+60 to 16+40
-, I ,-	17+15 to 20+50

2) The entire project was found to exhibit seasonal high ground water.

Physiography and Geology

This project corridor is located within the Coastal Plain Physiographic Province. Topography along the project is nearly flat to gently sloping. Ground elevations range from $84\pm$ feet along the bed of Singletary Mill Branch to $101\pm$ feet along the existing NC 242 embankment.

Surficial soils in this area are generally classified as undivided coastal plain sediments and alluvial sediments.

Ground Water

Ground water data was collected in December 2009 and January 2010, during a time of above normal precipitation. Ground water elevations ranged from 92± to 93± feet above sea level.

Soils

Soils within this project area have been classified as undivided coastal plain sediments, alluvial sediments and roadway embankment.

Alluvial soils in this area are comprised of 2 to 6 or more feet of loose gray sand (A-2-4), 2 to 4 or more feet of soft silty clay (A-7-6), and 1 to 5 feet of very soft sandy silt with little organic matter. Laboratory analysis of these organic soils show organic contents ranging from 5.6% to 9.6% and natural moisture contents ranging from 36.5% to 75.2%. Vane shear analyses show a range of shear strengths of 15 psf to 146 psf.

Undivided Coastal Plain soils were encountered along the upland area at the beginning of the project. These soils are comprised of 6 or more feet of loose to medium dense silty sand (A-2-4).

Soils classified as roadway embankment were encountered along the existing NC 242 embankment. These soils are comprised of 1 to 7 feet of medium dense sand (A-2-4).

Undisturbed Samples

Undisturbed thin wall Shelby tube samples were collected at the following locations and submitted for testing.

Sheet 3A

Sample No.	Station	Depth	<u>Test</u>		
ST-1	-L- 17+50 40' RT	0.0-2.0	Consolidation		

Respectfully Submitted,

Joseph L. Stone, L.G.
Project Engineering Geologist

Earthwork Balance Sheet

Volumes in Cubic Yards

PROJECT: B-4711

COUNTY: Bladen

DATE:

5/23/2012

COMPILED BY:

B. E. HOUGH

SHEET 1 OF 1 SHEETS

STATION	STATION		EXCAVATION					EMBANKMENT				WASTE			
		TOTAL UNCLASS.	ROCK	UNDERCUT		SUITABLE UNCLASS.	13	ROCK	EARTH	EMBANK. +25%	BORROW	ROCK	SUITABLE	UNSUIT.	TOTAL
13+00.00	16+10.81	64			0110211001	64	546		546	683	619	-			
	(BEGIN BRIDGE)										017				
	·						,								
										· .					
	SUBTOTAL	64				64	546		546	683	619				
17+33.19	20 50 00	20		<u> </u>											
(END BRIDGE)	20+50.00	29				29	295		295	369	340				
(END BRIDGE)															
	SUBTOTAL	29				29	295		295	369	340				
										309	340				
(
					-										
	CHIPTOTAL														
	SUBTOTAL														
										-					
		1													
			,												
	SUBTOTAL													l l	
TOTAL		93				93	841		841	1,051	958		<u> </u>		
ADDTIONAL BORROW FOR										40	40				
RECONSTRUCTION PER DI	VISION														
LOSS DUE TO CLEARING &	GRUBBING														
ADDITIONAL UNDERCUT	TY.							MI MINING BY P. C. 1117 - 11 - 1 - 1 - 1 - 1 - 1 - 1 - 1 -							
WASTE IN LIEU OF BORRO	W														
PROJECT TOTAL		93				93	841		841	1,091	998				
EST. 5% TO REPLACE TOP S	SOIL ON BORROW PIT										50	-			
											30				
GRAND TOTAL						93	841	,	841	1,091	1,048				
							V 11		011	1,071	1,070				
SAY		100									1,050				
											7				
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