

STATE	STATE PROJECT REFERENCE NO.	SHEET NO.	TOTAL SHEETS
N.C.	40057.1.1 (B-4955)	1	15

STATE OF NORTH CAROLINA
DEPARTMENT OF TRANSPORTATION
DIVISION OF HIGHWAYS
GEOTECHNICAL ENGINEERING UNIT

STRUCTURE
SUBSURFACE INVESTIGATION

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PROJ. REFERENCE NO. 40057.1.1 (B-4955) F.A. PROJ. BRSTP-1113(6)
COUNTY ALAMANCE
PROJECT DESCRIPTION BRIDGE #162 ON SR 1113 (KIMESVILLE RD.)
OVER SOUTH PRONG STINKING QUARTER CREEK

SITE DESCRIPTION BRIDGE #162 ON SR 1113 (KIMESVILLE RD.)
OVER SOUTH PRONG STINKING QUARTER CREEK

CAUTION NOTICE

THE SUBSURFACE INFORMATION AND THE SUBSURFACE INVESTIGATION ON WHICH IT IS BASED WERE MADE FOR THE PURPOSE OF STUDY, PLANNING, AND DESIGN, AND NOT FOR CONSTRUCTION OR PAY PURPOSES. THE VARIOUS FIELD BORING LOGS, ROCK CORES, AND SOIL TEST DATA AVAILABLE MAY BE REVIEWED OR INSPECTED IN RALEIGH BY CONTACTING THE N. C. DEPARTMENT OF TRANSPORTATION, GEOTECHNICAL ENGINEERING UNIT AT 1901 250-4088. NEITHER THE SUBSURFACE PLANS AND REPORTS, NOR THE FIELD BORING LOGS, ROCK CORES, OR SOIL TEST DATA ARE PART OF THE CONTRACT.

GENERAL SOIL AND ROCK STRATA DESCRIPTIONS AND INDICATED BOUNDARIES ARE BASED ON A GEOTECHNICAL INTERPRETATION OF ALL AVAILABLE SUBSURFACE DATA AND MAY NOT NECESSARILY REFLECT THE ACTUAL SUBSURFACE CONDITIONS BETWEEN BORINGS OR BETWEEN SAMPLED STRATA WITHIN THE BOREHOLE. THE LABORATORY SAMPLE DATA AND THE IN SITU (IN-PLACE) TEST DATA CAN BE RELIED ON ONLY TO THE DEGREE OF RELIABILITY INHERENT IN THE STANDARD TEST METHOD. THE OBSERVED WATER LEVELS OR SOIL MOISTURE CONDITIONS INDICATED IN THE SUBSURFACE INVESTIGATIONS ARE AS RECORDED AT THE TIME OF THE INVESTIGATION. THESE WATER LEVELS OR SOIL MOISTURE CONDITIONS MAY VARY CONSIDERABLY WITH TIME ACCORDING TO CLIMATIC CONDITIONS INCLUDING TEMPERATURES, PRECIPITATION, AND WIND, AS WELL AS OTHER NON-CLIMATIC FACTORS.

THE BIDDER OR CONTRACTOR IS CAUTIONED THAT DETAILS SHOWN ON THE SUBSURFACE PLANS ARE PRELIMINARY ONLY AND IN MANY CASES THE FINAL DESIGN DETAILS ARE DIFFERENT. FOR BIDDING AND CONSTRUCTION PURPOSES, REFER TO THE CONSTRUCTION PLANS AND DOCUMENTS FOR FINAL DESIGN INFORMATION ON THIS PROJECT. THE DEPARTMENT DOES NOT WARRANT OR GUARANTEE THE SUFFICIENCY OR ACCURACY OF THE INVESTIGATION MADE, NOR THE INTERPRETATIONS MADE, OR OPINION OF THE DEPARTMENT AS TO THE TYPE OF MATERIALS AND CONDITIONS TO BE ENCOUNTERED. THE BIDDER OR CONTRACTOR IS CAUTIONED TO MAKE SUCH INDEPENDENT SUBSURFACE INVESTIGATIONS AS HE DEEMS NECESSARY TO SATISFY HIMSELF AS TO CONDITIONS TO BE ENCOUNTERED ON THIS PROJECT. THE CONTRACTOR SHALL HAVE NO CLAIM FOR ADDITIONAL COMPENSATION OR FOR AN EXTENSION OF TIME FOR ANY REASON RESULTING FROM THE ACTUAL CONDITIONS ENCOUNTERED AT THE SITE DIFFERING FROM THOSE INDICATED IN THE SUBSURFACE INFORMATION.

PROJECT: 40057.1.1 ID: B-4955

PERSONNEL

J. K. STICKNEY

M. L. SMITH

C. L. SMITH

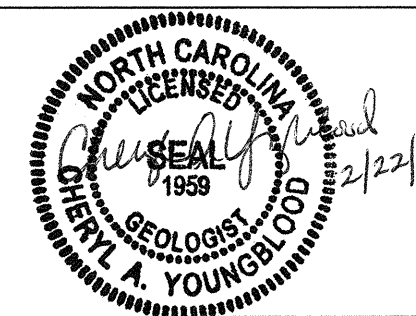
A. C. SMITH

INVESTIGATED BY C. A. YOUNGBLOOD

CHECKED BY B. D. WORLEY

SUBMITTED BY K. B. MILLER

DATE FEBRUARY 2011



DRAWN BY: C. E. BURRIS

NOTE - THE INFORMATION CONTAINED HEREIN IS NOT IMPLIED OR GUARANTEED BY THE N. C. DEPARTMENT OF TRANSPORTATION AS BEING ACCURATE NOR IT IS CONSIDERED TO BE PART OF THE PLANS, SPECIFICATIONS, OR CONTRACT FOR THE PROJECT.

NOTE - BY HAVING REQUESTED THIS INFORMATION THE CONTRACTOR SPECIFICALLY WAIVES ANY CLAIMS FOR INCREASED COMPENSATION OR EXTENSION OF TIME BASED ON DIFFERENCES BETWEEN THE CONDITIONS INDICATED HEREIN AND THE ACTUAL CONDITIONS AT THE PROJECT SITE.

NORTH CAROLINA DEPARTMENT OF TRANSPORTATION
DIVISION OF HIGHWAYS
GEOTECHNICAL ENGINEERING UNIT

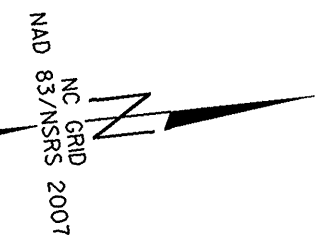
PROJECT REFERENCE NO. 40057.I.I (B-4955)	SHEET NO. 2
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SUBSURFACE INVESTIGATION

SOIL AND ROCK LEGEND, TERMS, SYMBOLS, AND ABBREVIATIONS

SOIL DESCRIPTION				GRADATION				ROCK DESCRIPTION				TERMS AND DEFINITIONS			
SOIL IS CONSIDERED TO BE THE UNCONSOLIDATED, SEMI-CONSOLIDATED, OR WEATHERED EARTH MATERIALS THAT CAN BE PENETRATED WITH A CONTINUOUS FLIGHT POWER AUGER, AND YIELD LESS THAN 100 BLOWS PER FOOT ACCORDING TO STANDARD PENETRATION TEST (ASTM D-1586). SOIL CLASSIFICATION IS BASED ON THE AASHTO SYSTEM. BASIC DESCRIPTIONS GENERALLY SHALL INCLUDE: CONSISTENCY, COLOR, TEXTURE, MOISTURE, AASHTO CLASSIFICATION, AND OTHER PERTINENT FACTORS SUCH AS MINERALOGICAL COMPOSITION, ANGULARITY, STRUCTURE, PLASTICITY, ETC. EXAMPLE: <i>VERY STIFF, GRAY, SILTY CLAY, MOST WITH INTERBEDDED FINE SAND LAYERS, HIGH PLASTIC, A-7-6</i>				WELL GRADED - INDICATES A GOOD REPRESENTATION OF PARTICLE SIZES FROM FINE TO COARSE. UNIFORM - INDICATES THAT SOIL PARTICLES ARE ALL APPROXIMATELY THE SAME SIZE. ALSO POORLY GRADED GAP-GRADED - INDICATES A MIXTURE OF UNIFORM PARTICLES OF TWO OR MORE SIZES.				HARD ROCK IS NON-COASTAL PLAIN MATERIAL THAT IF TESTED, WOULD YIELD SPT REFUSAL. AN INFERRED ROCK LINE INDICATES THE LEVEL AT WHICH NON-COASTAL PLAIN MATERIAL WOULD YIELD SPT REFUSAL. SPT REFUSAL IS PENETRATION BY A SPLIT SPOON SAMPLER EQUAL TO OR LESS THAN 60 BLOWS PER FOOT IN NON-COASTAL PLAIN MATERIAL. THE TRANSITION BETWEEN SOIL AND ROCK IS OFTEN REPRESENTED BY A ZONE OF WEATHERED ROCK. ROCK MATERIALS ARE TYPICALLY DIVIDED AS FOLLOWS:				ALLUVIUM (ALLUV.) - SOILS THAT HAVE BEEN TRANSPORTED BY WATER. AQUIFER - A WATER BEARING FORMATION OR STRATA. ARENACEOUS - APPLIED TO ROCKS THAT HAVE BEEN DERIVED FROM SAND OR THAT CONTAIN SAND. ARGILLACEOUS - APPLIED TO ALL ROCKS OR SUBSTANCES COMPOSED OF CLAY MINERALS, OR HAVING A NOTABLE PROPORTION OF CLAY IN THEIR COMPOSITION, AS SHALE, SLATE, ETC. ARTESIAN - GROUND WATER THAT IS UNDER SUFFICIENT PRESSURE TO RISE ABOVE THE LEVEL AT WHICH IT IS ENCOUNTERED, BUT WHICH DOES NOT NECESSARILY RISE TO OR ABOVE THE GROUND SURFACE. CALCAREOUS (CALC.) - SOILS THAT CONTAIN APPRECIABLE AMOUNTS OF CALCIUM CARBONATE. COLLUVIUM - ROCK FRAGMENTS MIXED WITH SOIL DEPOSITED BY GRAVITY ON SLOPE OR AT BOTTOM OF SLOPE. CORE RECOVERY (REC.) - TOTAL LENGTH OF ALL MATERIAL RECOVERED IN THE CORE BARREL DIVIDED BY TOTAL LENGTH OF CORE RUN AND EXPRESSED AS A PERCENTAGE. DIKE - A TABULAR BODY OF IGNEOUS ROCK THAT CUTS ACROSS THE STRUCTURE OF ADJACENT ROCKS OR CUTS MASSIVE ROCK. DIP - THE ANGLE AT WHICH A STRATUM OR ANY PLANAR FEATURE IS INCLINED FROM THE HORIZONTAL. DIP DIRECTION (DIP AZIMUTH) - THE DIRECTION OR BEARING OF THE HORIZONTAL TRACE OF THE LINE OF DIP, MEASURED CLOCKWISE FROM NORTH. FAULT - A FRACTURE OR FRACTURE ZONE ALONG WHICH THERE HAS BEEN DISPLACEMENT OF THE SIDES RELATIVE TO ONE ANOTHER PARALLEL TO THE FRACTURE. FISSILE - A PROPERTY OF SPLITTING ALONG CLOSELY SPACED PARALLEL PLANES. FLOAT - ROCK FRAGMENTS ON SURFACE NEAR THEIR ORIGINAL POSITION AND DISLODGED FROM PARENT MATERIAL. FLOOD PLAIN (FP) - LAND BORDERING A STREAM, BUILT OF SEDIMENTS DEPOSITED BY THE STREAM. FORMATION (FM) - A MAPPABLE GEOLOGIC UNIT THAT CAN BE RECOGNIZED AND TRACED IN THE FIELD. JOINT - FRACTURE IN ROCK ALONG WHICH NO APPRECIABLE MOVEMENT HAS OCCURRED. LEDGE - A SHELF-LIKE RIDGE OR PROJECTION OF ROCK WHOSE THICKNESS IS SMALL COMPARED TO ITS LATERAL EXTENT. LENS - A BODY OF SOIL OR ROCK THAT THINS OUT IN ONE OR MORE DIRECTIONS. MOTTLED (MOT.) - IRRREGULARLY MARKED WITH SPOTS OF DIFFERENT COLORS. MOTTLING IN SOILS USUALLY INDICATES POOR AERATION AND LACK OF GOOD DRAINAGE. PERCHED WATER - WATER MAINTAINED ABOVE THE NORMAL GROUND WATER LEVEL BY THE PRESENCE OF AN INTERVENING IMPERVIOUS STRATUM. RESIDUAL (RES.) SOIL - SOIL FORMED IN PLACE BY THE WEATHERING OF ROCK. ROCK QUALITY DESIGNATION (RQD) - A MEASURE OF ROCK QUALITY DESCRIBED BY TOTAL LENGTH OF ROCK SEGMENTS EQUAL TO OR GREATER THAN 4 INCHES DIVIDED BY THE TOTAL LENGTH OF CORE RUN AND EXPRESSED AS A PERCENTAGE. SAPROLITE (SAP.) - RESIDUAL SOIL THAT RETAINS THE RELIC STRUCTURE OR FABRIC OF THE PARENT ROCK. SILL - AN INTRUSIVE BODY OF IGNEOUS ROCK OF APPROXIMATELY UNIFORM THICKNESS AND RELATIVELY THIN COMPARED WITH ITS LATERAL EXTENT, THAT HAS BEEN EMPLACED PARALLEL TO THE BEDDING OR SCHISTOSITY OF THE INTRUDED ROCKS. SLICKENSIDE - POLISHED AND STRIATED SURFACE THAT RESULTS FROM FRICTION ALONG A FAULT OR SLIP PLANE. STANDARD PENETRATION TEST (PENETRATION RESISTANCE) (SPT) - NUMBER OF BLOWS IN OR BPF) OF A 140 LB. HAMMER FALLING 30 INCHES REQUIRED TO PRODUCE A PENETRATION OF 1 FOOT INTO SOIL WITH A 2 INCH OUTSIDE DIAMETER SPLIT SPOON SAMPLER. SPT REFUSAL IS PENETRATION EQUAL TO OR LESS THAN 60 BLOWS PER FOOT. STRATA CORE RECOVERY (SCREC) - TOTAL LENGTH OF STRATA MATERIAL RECOVERED DIVIDED BY TOTAL LENGTH OF STRATUM AND EXPRESSED AS A PERCENTAGE. STRATA ROCK QUALITY DESIGNATION (SROD) - A MEASURE OF ROCK QUALITY DESCRIBED BY TOTAL LENGTH OF ROCK SEGMENTS WITHIN A STRATUM EQUAL TO OR GREATER THAN 4 INCHES DIVIDED BY THE TOTAL LENGTH OF STRATA AND EXPRESSED AS A PERCENTAGE. TOPSOIL (TS.) - SURFACE SOILS USUALLY CONTAINING ORGANIC MATTER.			
SOIL LEGEND AND AASHTO CLASSIFICATION				MINERALOGICAL COMPOSITION				WEATHERING				ROCK HARDNESS			
GENERAL CLASS. GRANULAR MATERIALS (<= 35% PASSING #200) SILT-CLAY MATERIALS (> 35% PASSING #200) ORGANIC MATERIALS				MINERAL NAMES SUCH AS QUARTZ, FELDSPAR, MICA, TALC, KAOLIN, ETC. ARE USED IN DESCRIPTIONS WHENEVER THEY ARE CONSIDERED OF SIGNIFICANCE.				WEATHERED ROCK (WR) NON-COASTAL PLAIN MATERIAL THAT WOULD YIELD SPT N VALUES > 100 BLOWS PER FOOT IF TESTED. CRYSTALLINE ROCK (CR) FINE TO COARSE GRAIN IGNEOUS AND METAMORPHIC ROCK THAT WOULD YIELD SPT REFUSAL IF TESTED. ROCK TYPE INCLUDES GRANITE, GNEISS, GABBRO, SCHIST, ETC. NON-CRYSTALLINE ROCK (NCR) FINE TO COARSE GRAIN METAMORPHIC AND NON-COASTAL PLAIN SEDIMENTARY ROCK THAT WOULD YIELD SPT REFUSAL IF TESTED. ROCK TYPE INCLUDES PHYLLITE, SLATE, SANDSTONE, ETC. COASTAL PLAIN SEDIMENTARY ROCK (CP) COASTAL PLAIN SEDIMENTS CEMENTED INTO ROCK, BUT MAY NOT YIELD SPT REFUSAL. ROCK TYPE INCLUDES LIMESTONE, SANDSTONE, CEMENTED SHELL BEDS, ETC.				FRESH ROCK FRESH, CRYSTALS BRIGHT, FEW JOINTS MAY SHOW SLIGHT STAINING. ROCK RINGS UNDER HAMMER IF CRYSTALLINE. VERY SLIGHT (V SL.) ROCK GENERALLY FRESH, JOINTS STAINED, SOME JOINTS MAY SHOW THIN CLAY COATINGS IF OPEN. CRYSTALS ON A BROKEN SPECIMEN FACE SHINE BRIGHTLY. ROCK RINGS UNDER HAMMER BLOWS IF OF A CRYSTALLINE NATURE. SLIGHT (SL.) ROCK GENERALLY FRESH, JOINTS STAINED AND DISCOLORATION EXTENDS INTO ROCK UP TO 1 INCH. OPEN JOINTS MAY CONTAIN CLAY. IN GRANITOID ROCKS SOME OCCASIONAL FELDSPAR CRYSTALS ARE DULL AND DISCOLORED. CRYSTALLINE ROCKS RING UNDER HAMMER BLOWS. MODERATE (MOD.) SIGNIFICANT PORTIONS OF ROCK SHOW DISCOLORATION AND WEATHERING EFFECTS. IN GRANITOID ROCKS, MOST FELDSPARS ARE DULL AND DISCOLORED. SOME SHOW CLAY. ROCK HAS DULL SOUND UNDER HAMMER BLOWS AND SHOWS SIGNIFICANT LOSS OF STRENGTH AS COMPARED WITH FRESH ROCK. MODERATELY SEVERE (MOD. SEV.) ALL ROCK EXCEPT QUARTZ DISCOLORED OR STAINED. IN GRANITOID ROCKS, ALL FELDSPARS DULL AND DISCOLORED AND A MAJORITY SHOW KAOLINIZATION. ROCK SHOWS SEVERE LOSS OF STRENGTH AND CAN BE EXCAVATED WITH A GEOLOGIST'S PICK. ROCK GIVES "CLUNK" SOUND WHEN STRUCK. IF TESTED, WOULD YIELD SPT REFUSAL SEVERE (SEV.) ALL ROCK EXCEPT QUARTZ DISCOLORED OR STAINED. ROCK FABRIC CLEAR AND EVIDENT BUT REDUCED IN STRENGTH TO STRONG SOIL. IN GRANITOID ROCKS ALL FELDSPARS ARE KAOLINIZED TO SOME EXTENT. SOME FRAGMENTS OF STRONG ROCK USUALLY REMAIN. IF TESTED, YIELDS SPT N VALUES > 100 BPF VERY SEVERE (V SEV.) ALL ROCK EXCEPT QUARTZ DISCOLORED OR STAINED. ROCK FABRIC ELEMENTS ARE DISCERNIBLE BUT THE MASS IS EFFECTIVELY REDUCED TO SOIL STATUS, WITH ONLY FRAGMENTS OF STRONG ROCK REMAINING. SAPROLITE IS AN EXAMPLE OF ROCK WEATHERED TO A DEGREE SUCH THAT ONLY MINOR VESTIGES OF THE ORIGINAL ROCK FABRIC REMAIN. IF TESTED, YIELDS SPT N VALUES < 100 BPF COMPLETE ROCK REDUCED TO SOIL. ROCK FABRIC NOT DISCERNIBLE, OR DISCERNIBLE ONLY IN SMALL AND SCATTERED CONCENTRATIONS. QUARTZ MAY BE PRESENT AS DIKES OR STRINGERS. SAPROLITE IS ALSO AN EXAMPLE.			
COMPRESSIBILITY				PERCENTAGE OF MATERIAL				GROUND WATER				MISCELLANEOUS SYMBOLS			
SLIGHTLY COMPRESSIBLE LIQUID LIMIT LESS THAN 31 MODERATELY COMPRESSIBLE LIQUID LIMIT EQUAL TO 31-50 HIGHLY COMPRESSIBLE LIQUID LIMIT GREATER THAN 50				ORGANIC MATERIAL GRANULAR SILT - CLAY OTHER MATERIAL TRACE OF ORGANIC MATTER 2 - 3% 3 - 5% TRACE 1 - 10% LITTLE ORGANIC MATTER 3 - 5% 5 - 12% LITTLE 10 - 20% MODERATELY ORGANIC 5 - 10% 12 - 20% SOME 20 - 35% HIGHLY ORGANIC >10% >20% HIGHLY 35% AND ABOVE				WATER LEVEL IN BORE HOLE IMMEDIATELY AFTER DRILLING STATIC WATER LEVEL AFTER 24 HOURS PERCHED WATER, SATURATED ZONE, OR WATER BEARING STRATA SPRING OR SEEP				ROADWAY EMBANKMENT (RE) WITH SOIL DESCRIPTION SOIL SYMBOL ARTIFICIAL FILL (AF) OTHER THAN ROADWAY EMBANKMENT INFERRED SOIL BOUNDARY INFERRED ROCK LINE ALLUVIAL SOIL BOUNDARY DIP & DIP DIRECTION OF ROCK STRUCTURES TEST BORING W/ CORE SPT N-VALUE SPT REFUSAL MONITORING WELL PIEZOMETER INSTALLATION SLOPE INDICATOR INSTALLATION CONE PENETROMETER TEST SOUNDING ROD			
TEXTURE OR GRAIN SIZE				ABBREVIATIONS				EQUIPMENT USED ON SUBJECT PROJECT				FRACTURE SPACING			
U.S. STD. SIEVE SIZE OPENING (MM) 4 10 40 60 200 270 4.76 2.00 0.42 0.25 0.075 0.053				AR - AUGER REFUSAL BT - BORING TERMINATED CL - CLAY CPT - CONE PENETRATION TEST CSE - COARSE DMT - DILATOMETER TEST DPT - DYNAMIC PENETRATION TEST e - VOID RATIO F - FINE FOSS - FOSSILIFEROUS FRAC. - FRACTURED, FRACTURES FRAGS. - FRAGMENTS HI - HIGHLY MED. - MEDIUM MICA - MICACEOUS MOD. - MODERATELY NP - NON PLASTIC ORG. - ORGANIC PNT - PRESSUREMETER TEST SAP. - SAPROLITIC SD. - SAND, SANDY SL. - SILT, SILTY SLI. - SLIGHTLY TCR - TRICONE REFUSAL W - MOISTURE CONTENT V - VERY VST - VANE SHEAR TEST WEA. - WEATHERED ? - UNIT WEIGHT ? _d - DRY UNIT WEIGHT				DRILL UNITS: MOBILE B- BK-51 CME-45C CME-550 PORTABLE HOIST ADVANCING TOOLS: CLAY BITS 6" CONTINUOUS FLIGHT AUGER 8" HOLLOW AUGERS HARD FACED FINGER BITS TUNG-CARBIDE INSERTS CASING w/ ADVANCER TRICONE STEEL TEETH TRICONE 2 15/16 TUNG-CARB. CORE BIT HAMMER TYPE: AUTOMATIC MANUAL CORE SIZE: B N NXBWL H HAND TOOLS: POST HOLE DIGGER HAND AUGER SOUNDING ROD VANE SHEAR TEST				VERY HARD CANNOT BE SCRATCHED BY KNIFE OR SHARP PICK. BREAKING OF HAND SPECIMENS REQUIRES SEVERAL HARD BLOWS OF THE GEOLOGIST'S PICK. HARD CAN BE SCRATCHED BY KNIFE OR PICK ONLY WITH DIFFICULTY. HARD HAMMER BLOWS REQUIRED TO DETACH HAND SPECIMEN. MODERATELY HARD CAN BE SCRATCHED BY KNIFE OR PICK. GOUGES OR GROOVES TO 0.25 INCHES DEEP CAN BE EXCAVATED BY HARD BLOW OF A GEOLOGIST'S PICK. HAND SPECIMENS CAN BE DETACHED BY MODERATE BLOWS. MEDIUM HARD CAN BE GROUDED OR GOUGED 0.05 INCHES DEEP BY FIRM PRESSURE OF KNIFE OR PICK POINT. CAN BE EXCAVATED IN SMALL CHIPS TO PEICES 1 INCH MAXIMUM SIZE BY HARD BLOWS OF THE POINT OF A GEOLOGIST'S PICK. SOFT CAN BE GROUDED OR GOUGED READILY BY KNIFE OR PICK. CAN BE EXCAVATED IN FRAGMENTS FROM CHIPS TO SEVERAL INCHES IN SIZE BY MODERATE BLOWS OF A PICK POINT. SMALL, THIN PIECES CAN BE BROKEN BY FINGER PRESSURE. VERY SOFT CAN BE CARVED WITH KNIFE. CAN BE EXCAVATED READILY WITH POINT OF PICK. PIECES 1 INCH OR MORE IN THICKNESS CAN BE BROKEN BY FINGER PRESSURE. CAN BE SCRATCHED READILY BY FINGER NAIL.			
SOIL MOISTURE - CORRELATION OF TERMS				SOIL MOISTURE SCALE (ATTERBERG LIMITS)				FRACURE SPACING				BEDDING			
SOIL MOISTURE SCALE (ATTERBERG LIMITS) FIELD MOISTURE DESCRIPTION GUIDE FOR FIELD MOISTURE DESCRIPTION				SATURATED (SAT.) USUALLY LIQUID; VERY WET, USUALLY FROM BELOW THE GROUND WATER TABLE WET (W) SEMISOLID; REQUIRES DRYING TO ATTAIN OPTIMUM MOISTURE MOIST (M) SOLID; AT OR NEAR OPTIMUM MOISTURE DRY (D) REQUIRES ADDITIONAL WATER TO ATTAIN OPTIMUM MOISTURE				TERM SPACING VERY WIDE MORE THAN 10 FEET WIDE 3 TO 10 FEET MODERATELY CLOSE 1 TO 3 FEET CLOSE 0.16 TO 1 FEET VERY CLOSE LESS THAN 0.16 FEET				TERM THICKNESS VERY THICKLY BEDDED > 4 FEET THICKLY BEDDED 1.5 - 4 FEET THINLY BEDDED 0.16 - 1.5 FEET VERY THINLY BEDDED 0.03 - 0.16 FEET THICKLY LAMINATED 0.008 - 0.03 FEET THINLY LAMINATED < 0.008 FEET			
PLASTICITY				PLASTICITY INDEX (PI)				INDURATION				NOTES:			
NONPLASTIC LOW PLASTICITY MED. PLASTICITY HIGH PLASTICITY				PLASTICITY INDEX (PI) DRY STRENGTH 0-5 VERY LOW 6-15 SLIGHT 16-25 MEDIUM 26 OR MORE HIGH				FOR SEDIMENTARY ROCKS, INDURATION IS THE HARDENING OF THE MATERIAL BY CEMENTING, HEAT, PRESSURE, ETC. FRIABLE RUBBING WITH FINGER FREES NUMEROUS GRAINS; GENTLE BLOW BY HAMMER DISINTEGRATES SAMPLE. MODERATELY INDURATED GRAINS CAN BE SEPARATED FROM SAMPLE WITH STEEL PROBE; BREAKS EASILY WHEN HIT WITH HAMMER. INDURATED GRAINS ARE DIFFICULT TO SEPARATE WITH STEEL PROBE; DIFFICULT TO BREAK WITH HAMMER. EXTREMELY INDURATED SHARP HAMMER BLOWS REQUIRED TO BREAK SAMPLE; SAMPLE BREAKS ACROSS GRAINS.				BENCH MARK: BL-4 STA. 13+94.54 -L- 13.74 LT N 807278.3101 E 1843044.0256 ELEVATION: 554.14 FT. CAR = CASING ADVANCER REFUSAL			
DESCRIPTIONS MAY INCLUDE COLOR OR COLOR COMBINATIONS (TAN, RED, YELLOW-BROWN, BLUE-GRAY). MODIFIERS SUCH AS LIGHT, DARK, STREAKED, ETC. ARE USED TO DESCRIBE APPEARANCE.															

PROPOSED SKEW ANGLE: 75°



BM#2
-BL- STATION 4I+05.00
210' LEFT
ELEV. 542.02'

-BL-4

N 3° 03' 27.0" E

N 5° 56' 48.5" E

N 6° 00' 50.4" E

BRIDGE #162
WD & STEEL

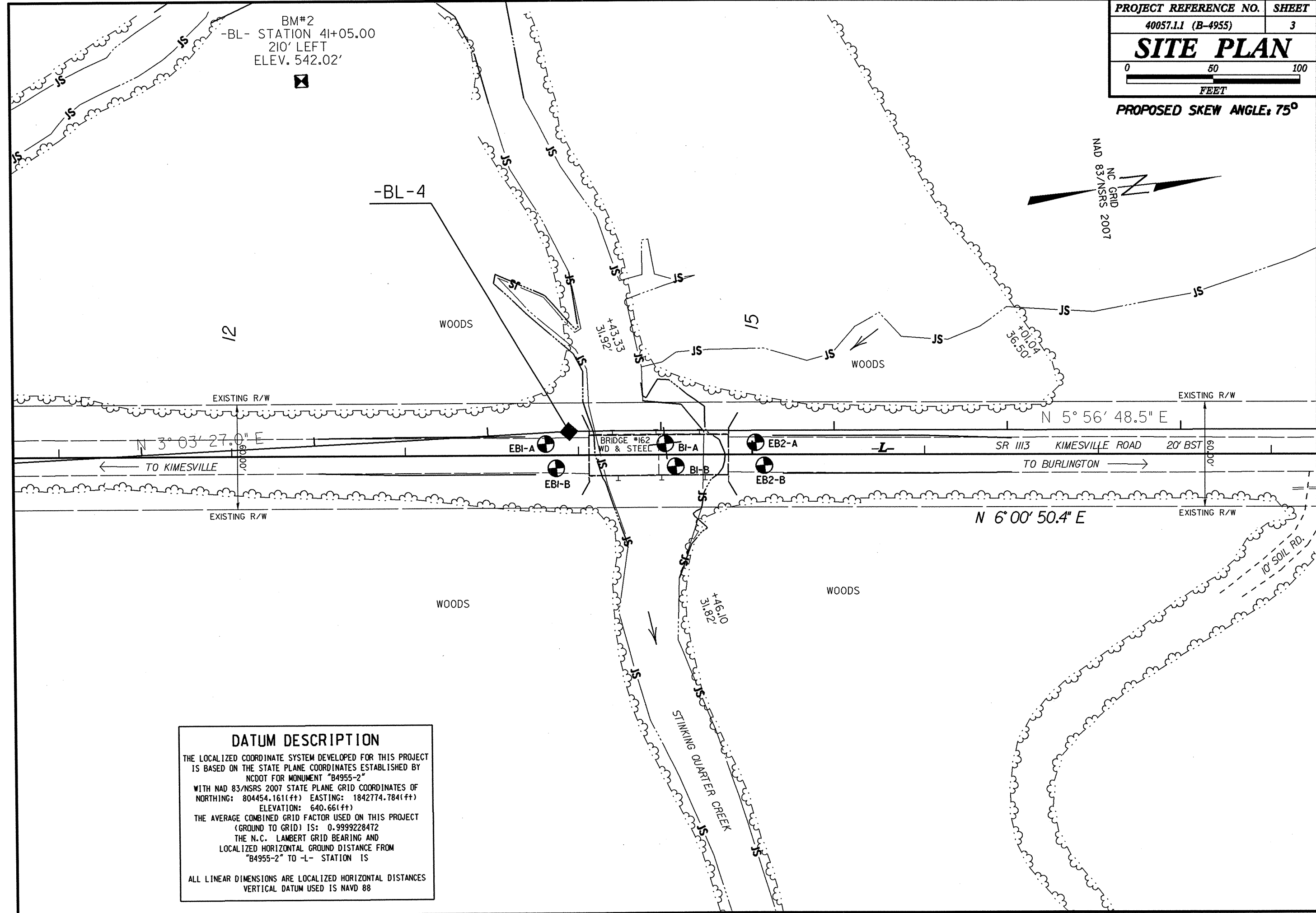
SR III3 KIMESVILLE ROAD 20' BST

← TO KIMESVILLE

TO BURLINGTON →

DATUM DESCRIPTION

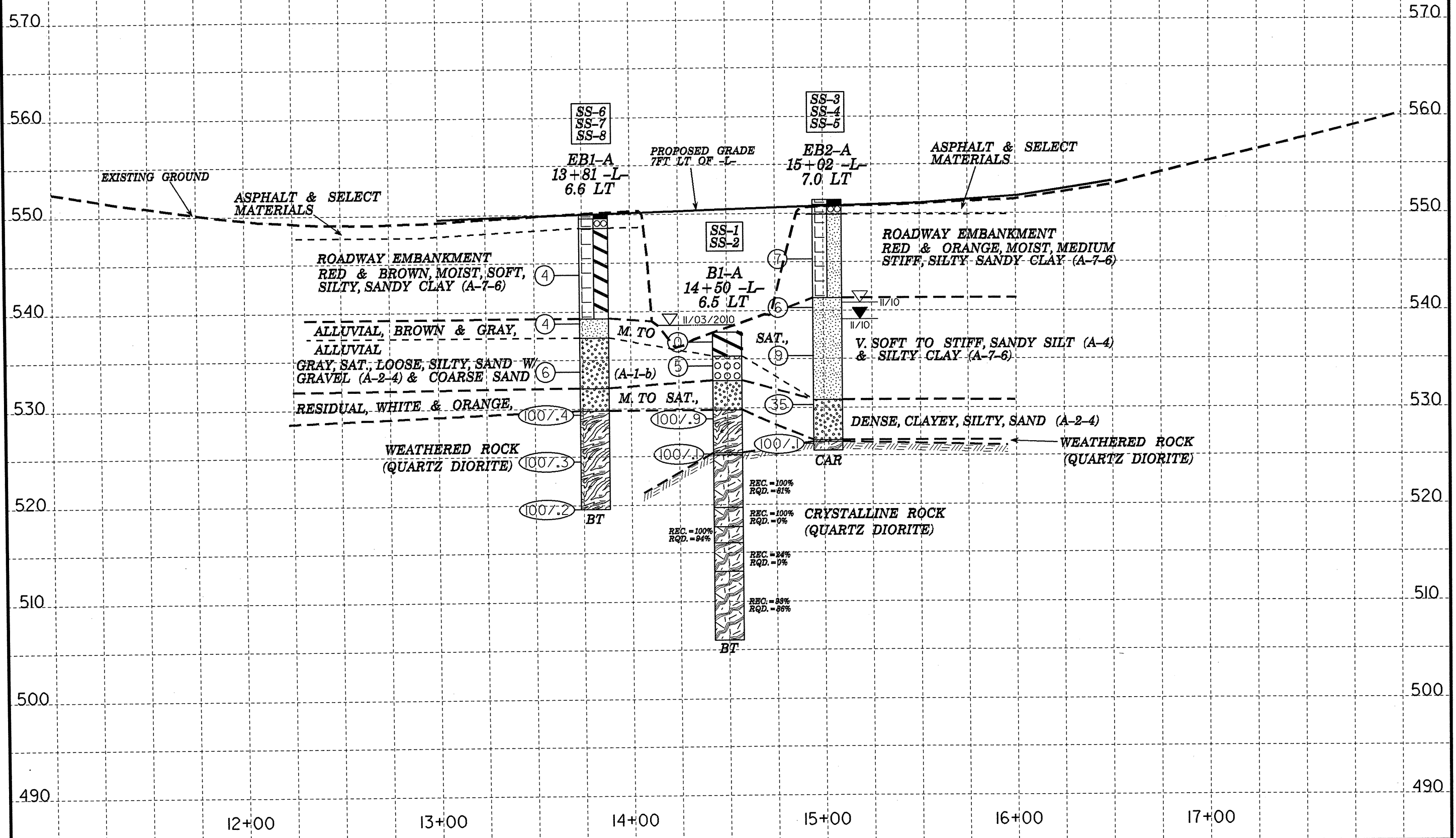
THE LOCALIZED COORDINATE SYSTEM DEVELOPED FOR THIS PROJECT IS BASED ON THE STATE PLANE COORDINATES ESTABLISHED BY NCDOT FOR MONUMENT "B4955-2" WITH NAD 83/NSRS 2007 STATE PLANE GRID COORDINATES OF NORTHING: 804454.161(±) EASTING: 1842774.784(±) ELEVATION: 640.66(±) THE AVERAGE COMBINED GRID FACTOR USED ON THIS PROJECT (GROUND TO GRID) IS: 0.9999228472 THE N.C. LAMBERT GRID BEARING AND LOCALIZED HORIZONTAL GROUND DISTANCE FROM "B4955-2" TO -L- STATION IS ALL LINEAR DIMENSIONS ARE LOCALIZED HORIZONTAL DISTANCES VERTICAL DATUM USED IS NAVD 88

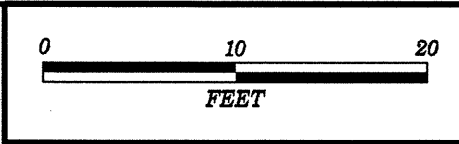




PROJECT REFERENCE NO.	SHEET
40057.1.1 (B-4955)	4
Profile 7ft left of -L- Bridge No. 162 on -L- (Sta. 14+43.5) VE=5:1	

-L-





PROJECT REFERENCE NO.	SHEET
40057.1.1 (B-4955)	5
SECTION THROUGH EB-1 STA. 13+82.27 -L- SKEW = 75° 00' 00"	

580 -L- 580

570 570

560 560

550 550

540 540

530 530

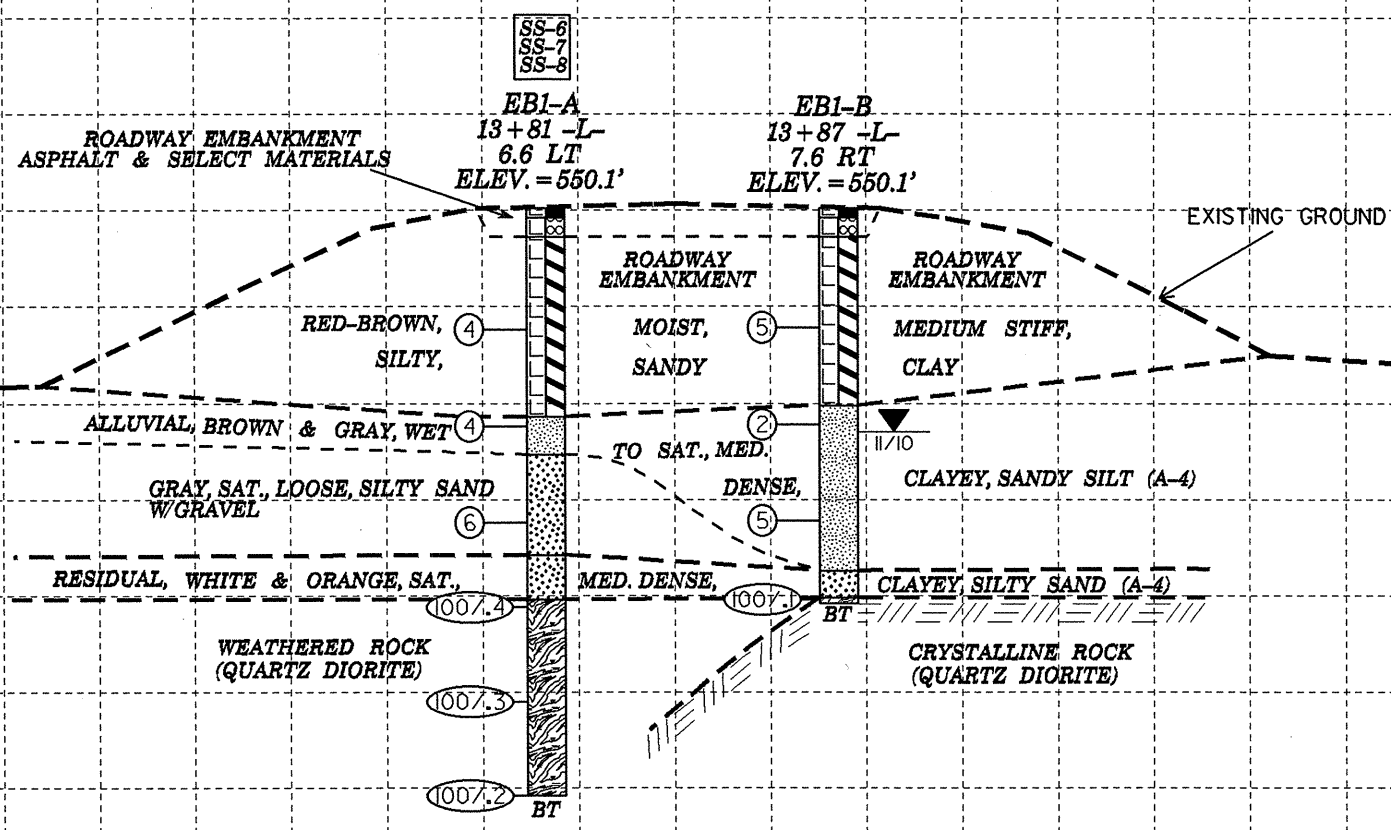
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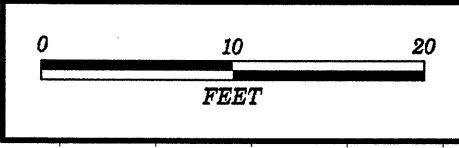
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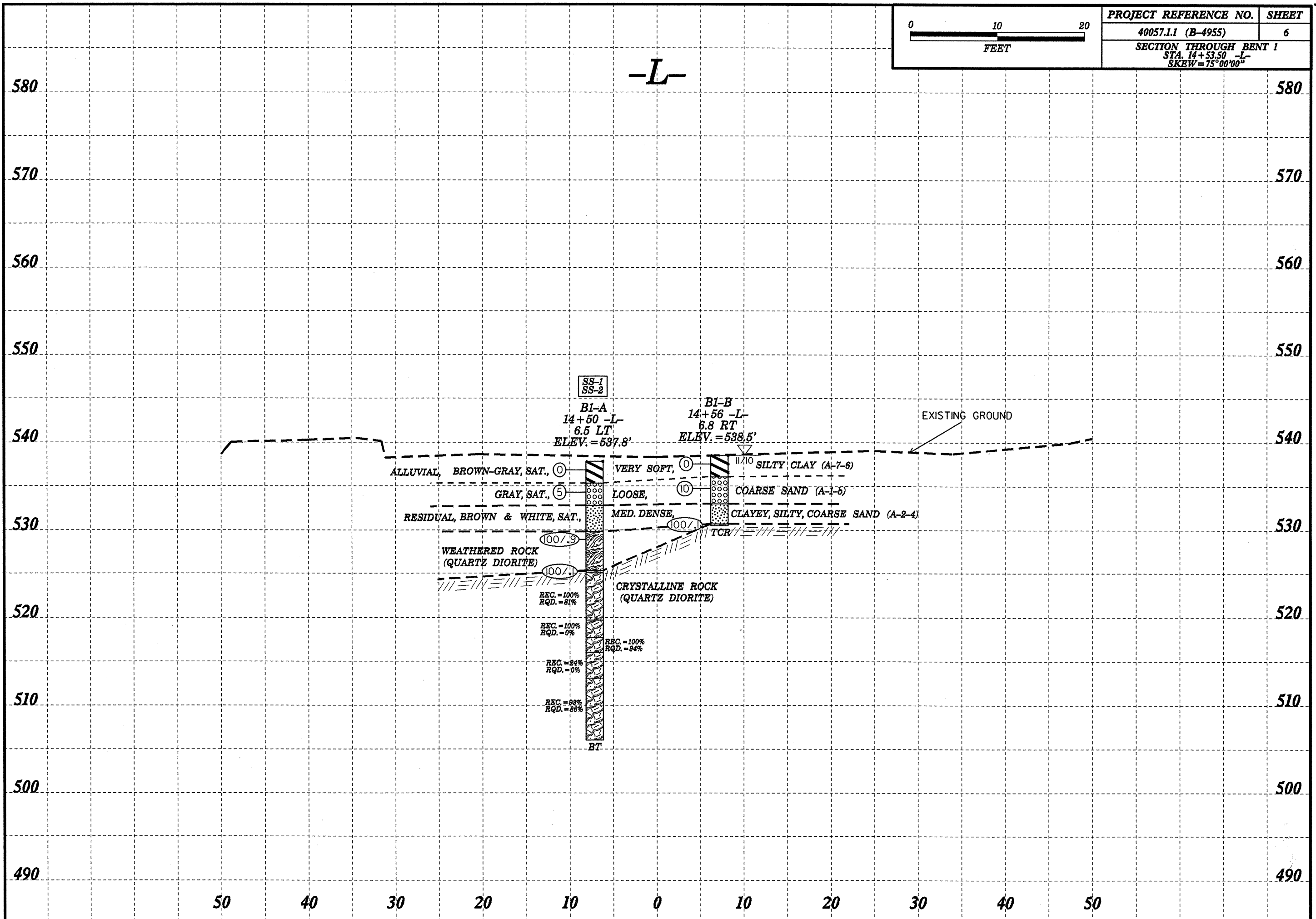
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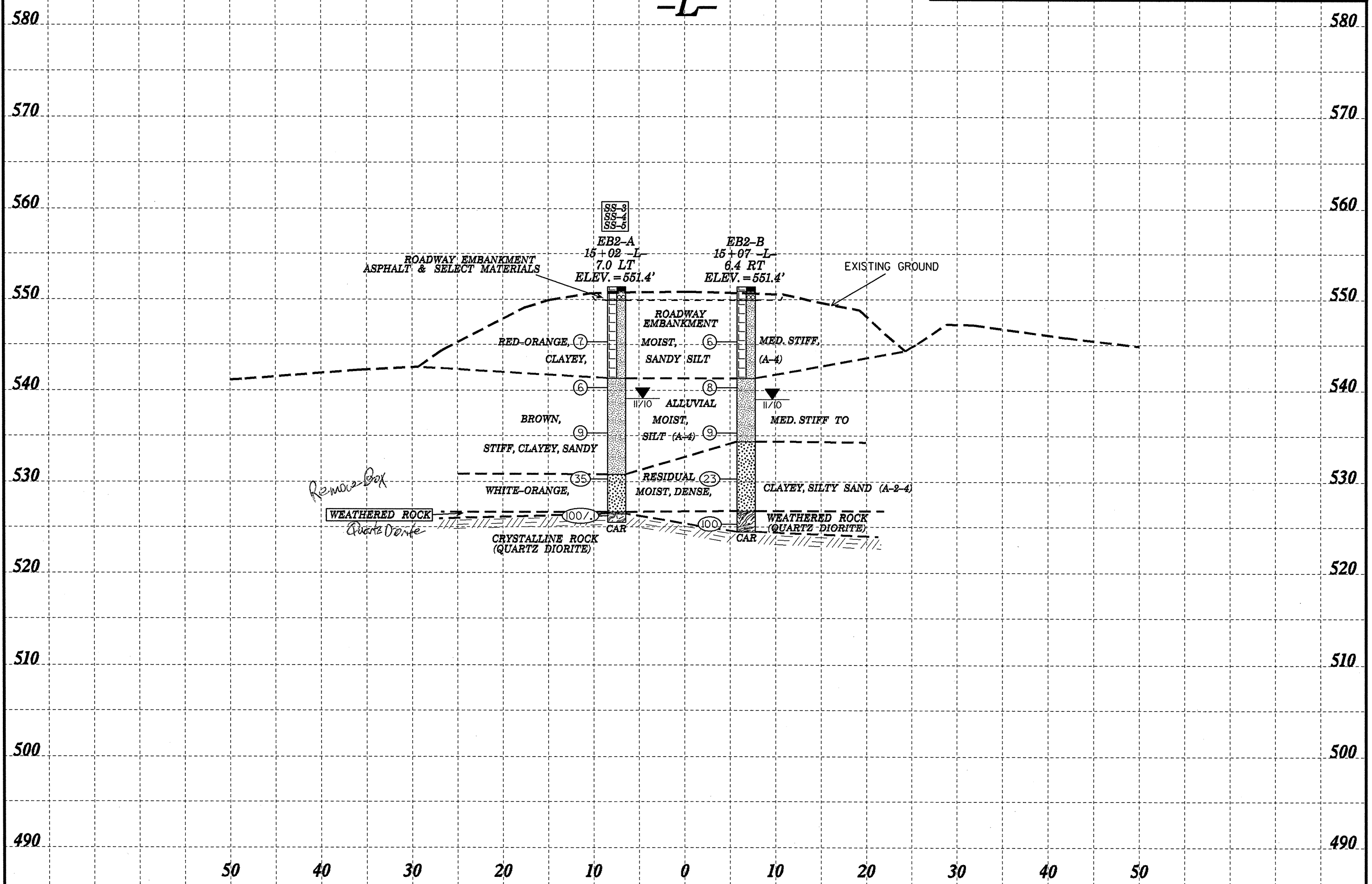
50 40 30 20 10 0 10 20 30 40 50





PROJECT REFERENCE NO.	SHEET
40057.1.1 (B-4955)	6
SECTION THROUGH BENT 1	
STA. 14+53.50 -L-	
SKEW = 75°00'00"	





WBS 40057.1.1	TIP B-4955	COUNTY ALAMANCE	GEOLOGIST Stickney, J. K.
SITE DESCRIPTION Bridge No. 162 on SR 1113 (Kimesville Rd.) over S. prong Stinking Quarter Creek			GROUND WTR (ft)
BORING NO. EB1-A	STATION 13+81	OFFSET 7 ft LT	ALIGNMENT -L-
COLLAR ELEV. 550.1 ft	TOTAL DEPTH 30.5 ft	NORTHING 807,264	EASTING 1,843,050
DRILL RIG/HAMMER EFF./DATE HFO0072 CME-550 89% 09/02/2009		DRILL METHOD NW Casing w/ SPT	HAMMER TYPE Automatic
DRILLER Smith, M. L.	START DATE 11/02/10	COMP. DATE 11/02/10	SURFACE WATER DEPTH N/A

ELEV (ft)	DRIVE ELEV (ft)	DEPTH (ft)	BLOW COUNT			BLOWS PER FOOT					SAMP. NO.	LOG	SOIL AND ROCK DESCRIPTION	DEPTH (ft)
			0.5ft	0.5ft	0.5ft	0	25	50	75	100				
555														
550													GROUND SURFACE 0.0 ROADWAY EMBANKMENT 1.5 ASPHALT SELECT MATERIALS RED, BROWN SILTY SANDY CLAY	
545	544.8	5.3	3	2	2						SS-6	M		
540	539.8	10.3	2	2	2						SS-7	W		
535	534.8	15.3	2	2	4						SS-8	Sat.		
530	529.8	20.3	100/4										RESIDUAL WHITE, ORANGE CLAYEY SILTY SAND WEATHERED ROCK (QUARTZ DIORITE)	20.3
525	524.8	25.3	100/3											
520	519.8	30.3	100/2											
													Boring Terminated at Elevation 519.6 ft in weathered rock (Quartz diorite)	30.5

WBS 40057.1.1	TIP B-4955	COUNTY ALAMANCE	GEOLOGIST Stickney, J. K.
SITE DESCRIPTION Bridge No. 162 on SR 1113 (Kimesville Rd.) over S. prong Stinking Quarter Creek			GROUND WTR (ft)
BORING NO. EB1-B	STATION 13+87	OFFSET 8 ft RT	ALIGNMENT -L-
COLLAR ELEV. 550.1 ft	TOTAL DEPTH 20.5 ft	NORTHING 807,269	EASTING 1,843,064
DRILL RIG/HAMMER EFF./DATE HFO0072 CME-550 89% 09/02/2009		DRILL METHOD NW Casing w/ SPT	HAMMER TYPE Automatic
DRILLER Smith, M. L.	START DATE 11/02/10	COMP. DATE 11/02/10	SURFACE WATER DEPTH N/A

ELEV (ft)	DRIVE ELEV (ft)	DEPTH (ft)	BLOW COUNT			BLOWS PER FOOT					SAMP. NO.	LOG	SOIL AND ROCK DESCRIPTION	DEPTH (ft)
			0.5ft	0.5ft	0.5ft	0	25	50	75	100				
555														
550													GROUND SURFACE 0.0 ROADWAY EMBANKMENT 1.5 ASPHALT SELECT MATERIALS RED, BROWN SILTY SANDY CLAY	
545	544.9	5.2	2	3	2							M		
540	539.9	10.2	3	1	1								ALLUVIAL BROWN, GRAY CLAYEY SANDY SILT	10.2
535	534.9	15.2	1	2	3							Sat.		
530	529.9	20.2	100/1										RESIDUAL WHITE & ORANGE CLAYEY SILTY SAND WEATHERED ROCK (QUARTZ DIORITE)	20.2
													Boring Terminated at Elevation 529.6 ft in crystalline rock (Quartz Diorite)	20.5

NCDOT BORE DOUBLE B4955_GEO_BRDG162_ALAMANCE.GPJ_NC_DOT_GDT_02/22/11

WBS 40057.1.1		TIP B-4955		COUNTY ALAMANCE		GEOLOGIST Stickney, J. K.										
SITE DESCRIPTION Bridge No. 162 on SR 1113 (Kimesville Rd.) over S. prong Stinking Quarter Creek							GROUND WTR (ft)									
BORING NO. B1-A		STATION 14+50		OFFSET 7 ft LT		ALIGNMENT -L-										
COLLAR ELEV. 537.8 ft		TOTAL DEPTH 31.8 ft		NORTHING 807,333		EASTING 1,843,057										
DRILL RIG/HAMMER EFF./DATE HFO0072 CME-550 89% 09/02/2009		DRILL METHOD NW Casing W/SPT & Core		HAMMER TYPE Automatic												
DRILLER Smith, M. L.		START DATE 11/01/10		COMP. DATE 11/01/10		SURFACE WATER DEPTH 0.3ft										
ELEV (ft)	DRIVE ELEV (ft)	DEPTH (ft)	BLOW COUNT			BLOWS PER FOOT					SAMP. NO.	MOI	LOG	SOIL AND ROCK DESCRIPTION		
			0.5ft	0.5ft	0.5ft	0	25	50	75	100				ELEV. (ft)	DEPTH (ft)	
540																
	537.8	0.0														537.8 WATER SURFACE (11/01/10) 0.0
535	535.3	2.5	4	2	3											535.3 ALLUVIAL BROWN, GRAY SILTY CLAY 2.5
			4	2	3											532.8 GRAY COARSE SAND 5.0
530	530.3	7.5	19	32	68/4											529.8 RESIDUAL BROWN, WHITE CLAYEY SILTY COARSE SAND 8.0
			19	32	68/4											525.3 WEATHERED ROCK (QUARTZ DIORITE) 12.5
525	525.3	12.5	100/1													525.1 CRYSTALLINE ROCK (QUARTZ DIORITE) 12.7
			100/1													519.7 (QUARTZ DIORITE) 18.1
520																517.7 (QUARTZ DIORITE) 20.1
																516.0 (QUARTZ DIORITE) 21.8
515																513.1 (QUARTZ DIORITE) 24.7
510																506.0 Boring Terminated at Elevation 506.0 ft in crystalline rock (Quartz Diorite) 31.8

NCDOT BORE DOUBLE B4955_GEO_BRDG162_ALAMANCE.GPJ NC_DOT.GDT 2/24/11

WBS 40057.1.1		TIP B-4955		COUNTY ALAMANCE		GEOLOGIST Stickney, J. K.						
SITE DESCRIPTION Bridge No. 162 on SR 1113 (Kimesville Rd.) over S. prong Stinking Quarter Creek							GROUND WTR (ft)					
BORING NO. B1-A		STATION 14+50		OFFSET 7 ft LT		ALIGNMENT -L-						
COLLAR ELEV. 537.8 ft		TOTAL DEPTH 31.8 ft		NORTHING 807,333		EASTING 1,843,057						
DRILL RIG/HAMMER EFF./DATE HFO0072 CME-550 89% 09/02/2009		DRILL METHOD NW Casing W/SPT & Core		HAMMER TYPE Automatic								
DRILLER Smith, M. L.		START DATE 11/01/10		COMP. DATE 11/01/10		SURFACE WATER DEPTH 0.3ft						
ELEV (ft)	RUN ELEV (ft)	DEPTH (ft)	RUN (ft)	DRILL RATE (Min/ft)	TOTAL RUN 19.1 ft		SAMP. NO.	STRATA		LOG	DESCRIPTION AND REMARKS	
					REC. (%)	RQD (%)		REC. (%)	RQD (%)		ELEV. (ft)	DEPTH (ft)
525												
	525.1	12.7	4.1	3.66/1.1 3.50/1.0 3.31/1.0 3.45/1.0	(4.1) 100%	(3.2) 78%		(5.4) 100%	(4.4) 81%			Begin Coring @ 12.7 ft
520	521.0	16.8	5.0	2.07/1.0 1.93/1.0 2.30/1.0 2.10/1.0 1.11/1.0	(5.0) 100%	(2.9) 58%	RS-1					525.1 VERY SLIGHTLY WEATHERED, HARD, MODERATELY CLOSE TO V. CLOSELY FRACTURED, WHITE, GRAY & BLACK QUARTZ DIORITE 12.7
												R1=7, R2=17, R3=10, R4=12, R5=7. RMR=53 ROCK TYPE = E
												519.7 MODERATELY SEVERELY WEATHERED, MEDIUM HARD, V. CLOSELY FRACTURED, WHITE, GRAY, BROWN & BLACK QUARTZ DIORITE 18.1
												517.7 MODERATELY SEVERELY WEATHERED, MEDIUM HARD, V. CLOSELY FRACTURED, WHITE, GRAY, BROWN & BLACK QUARTZ DIORITE 20.1
515	516.0	21.8	5.0	1.04/1.0 1.23/1.0 1.50/1.0 2.00/1.0 2.09/1.0	(3.3) 66%	(2.8) 56%		(2.0) 100%	(0.0) 0%			516.0 VERY SLIGHTLY WEATHERED, HARD, CLOSELY FRACTURED, WHITE, GRAY & BLACK, QUARTZ DIORITE 21.8
												513.1 MODERATELY TO SEVERELY WEATHERED, MODERATELY HARD TO SOFT, CLOSE TO VERY CLOSELY FRACTURED, WHITE, GRAY, BROWN & BLACK, QUARTZ DIORITE 24.7
510	511.0	26.8	5.0	2.10/1.0 3.22/1.0 3.32/1.0 3.17/1.0 3.01/1.0	(4.5) 90%	(4.0) 80%		(1.7) 100%	(1.6) 94%			513.1 MODERATELY TO SEVERELY WEATHERED, MODERATELY HARD TO SOFT, CLOSE TO VERY CLOSELY FRACTURED, WHITE, GRAY, BROWN & BLACK, QUARTZ DIORITE 24.7
												506.0 VERY SLIGHTLY WEATHERED, HARD, MODERATELY CLOSE TO VERY CLOSELY FRACTURED, WHITE, GRAY & BLACK, QUARTZ DIORITE 31.8
												Boring Terminated at Elevation 506.0 ft in crystalline rock (Quartz Diorite)

NCDOT CORE DOUBLE B4955_GEO_BRDG162_ALAMANCE.GPJ NC_DOT.GDT 2/24/11



WBS 40057.1.1		TIP B-4955		COUNTY ALAMANCE		GEOLOGIST Stickney, J. K.									
SITE DESCRIPTION Bridge No. 162 on SR 1113 (Kimesville Rd.) over S. prong Stinking Quarter Creek							GROUND WTR (ft)								
BORING NO. B1-B		STATION 14+56		OFFSET 7 ft RT		ALIGNMENT -L-									
COLLAR ELEV. 538.5 ft		TOTAL DEPTH 8.0 ft		NORTHING 807,337		EASTING 1,843,071									
DRILL RIG/HAMMER EFF./DATE HFO0072 CME-550 89% 09/02/2009		DRILL METHOD NW Casing w/ SPT		HAMMER TYPE Automatic											
DRILLER Smith, M. L.		START DATE 11/01/10		COMP. DATE 11/01/10		SURFACE WATER DEPTH 0.3ft									
ELEV (ft)	DRIVE ELEV (ft)	DEPTH (ft)	BLOW COUNT			BLOWS PER FOOT					SAMP. NO.	LOG	SOIL AND ROCK DESCRIPTION	DEPTH (ft)	
			0.5ft	0.5ft	0.5ft	0	25	50	75	100					
540															
	538.5	0.0												538.5	0.0
	535.7	2.8	3	4	6								Sat.	536.0	2.5
													Sat.	533.0	5.5
	530.7	7.8												530.7	7.8
			100/1											530.5	8.0

NCDOT BORE SINGLE B4955_GEO_BRDG162_ALAMANCE.GPJ NC_DOT_GDT_02/24/11

WATER SURFACE (11/01/10) 0.0

Sat. 536.0 ALLUVIAL BROWN, GRAY SILTY CLAY 2.5

Sat. 533.0 GRAY COARSE SAND 5.5

RESIDUAL BROWN, WHITE CLAYEY SILTY COARSE SAND 7.8

CRYSTALLINE ROCK (QUARTZ DIORITE) 8.0

Boring Terminated with Tri-Cone Roller Refusal at Elevation 530.5 ft in crystalline rock (Quartz Diorite)

NCDOT GEOTECHNICAL ENGINEERING UNIT
BORELOG REPORT

WBS 40057.1.1	TIP B-4955	COUNTY ALAMANCE	GEOLOGIST Stickney, J. K.
SITE DESCRIPTION Bridge No. 162 on SR 1113 (Kimesville Rd.) over S. prong Stinking Quarter Creek			GROUND WTR (ft)
BORING NO. EB2-A	STATION 15+02	OFFSET 7 ft LT	ALIGNMENT -L-
COLLAR ELEV. 551.4 ft		TOTAL DEPTH 25.9 ft	NORTHING 807,384
DRILL RIG/HAMMER EFF./DATE HFO0072 CME-550 89% 09/02/2009		DRILL METHOD NW Casing w/ SPT	HAMMER TYPE Automatic
DRILLER Smith, M. L.	START DATE 11/02/10	COMP. DATE 11/02/10	SURFACE WATER DEPTH N/A

ELEV (ft)	DRIVE ELEV (ft)	DEPTH (ft)	BLOW COUNT			BLOWS PER FOOT					SAMP. NO.	LOG	SOIL AND ROCK DESCRIPTION	
			0.5ft	0.5ft	0.5ft	0	25	50	75	100				
555														GROUND SURFACE ELEV. 551.4
550														ROADWAY EMBANKMENT ASPHALT ELEV. 549.9
545	546.3	5.1	3	3	4							SS-3	M	SELECT MATERIALS RED, ORANGE CLAYEY SANDY SILT
540	541.3	10.1	1	2	4							SS-4		ALLUVIAL BROWN, TAN CLAYEY SANDY SILT
535	536.3	15.1	3	3	6							M		
530	531.3	20.1	5	15	20							SS-5	M	RESIDUAL WHITE, ORANGE CLAYEY SILTY SAND
	526.3	25.1												WEATHERED ROCK (QUARTZ DIORITE) ELEV. 526.4
														CRYSTALLINE ROCK (QUARTZ DIORITE) ELEV. 525.5

Boring Terminated with Casing Advancer Refusal at Elevation 525.5 ft in crystalline rock (Quartz Diorite)

WBS 40057.1.1	TIP B-4955	COUNTY ALAMANCE	GEOLOGIST Stickney, J. K.
SITE DESCRIPTION Bridge No. 162 on SR 1113 (Kimesville Rd.) over S. prong Stinking Quarter Creek			GROUND WTR (ft)
BORING NO. EB2-B	STATION 15+07	OFFSET 6 ft RT	ALIGNMENT -L-
COLLAR ELEV. 551.4 ft		TOTAL DEPTH 27.0 ft	NORTHING 807,388
DRILL RIG/HAMMER EFF./DATE HFO0072 CME-550 89% 09/02/2009		DRILL METHOD NW Casing w/ SPT	HAMMER TYPE Automatic
DRILLER Smith, M. L.	START DATE 11/02/10	COMP. DATE 11/02/10	SURFACE WATER DEPTH N/A

ELEV (ft)	DRIVE ELEV (ft)	DEPTH (ft)	BLOW COUNT			BLOWS PER FOOT					SAMP. NO.	LOG	SOIL AND ROCK DESCRIPTION	
			0.5ft	0.5ft	0.5ft	0	25	50	75	100				
555														GROUND SURFACE ELEV. 551.4
550														ROADWAY EMBANKMENT PAVEMENT ELEV. 549.9
545	546.3	5.1	2	2	4							M		SELECT MATERIALS RED, ORANGE CLAYEY SANDY SILT
540	541.3	10.1	2	3	5									ALLUVIAL BROWN, TAN CLAYEY SANDY SILT
535	536.3	15.1	3	4	5							M		RESIDUAL WHITE, ORANGE CLAYEY SILTY SAND
530	531.3	20.1	8	8	15							M		
525	526.3	25.1	79	21/5								D		WEATHERED ROCK (QUARTZ DIORITE) ELEV. 526.7
														Boring Terminated with Casing Advancer Refusal at Elevation 524.4 ft on crystalline rock Quartz Diorite)

Boring Terminated with Casing Advancer Refusal at Elevation 524.4 ft on crystalline rock Quartz Diorite)

NCDOT BORE DOUBLE B4955 GEO BRDGI162_ALAMANCE.GPJ_NC_DOT.GDT 02/22/11

TEST RESULTS

PROJECT: 40057.1.1 (B-4955)

COUNTY: ALAMANCE

SITE DESCRIPTION: BRIDGE NO. 162 ON SR 1113 (KIMESVILLE RD.) OVER SOUTH PRONG STINKING QUARTER CREEK

SOIL SAMPLE RESULTS

SAMPLE NO.	OFFSET	STATION	DEPTH INTERVAL	AASHTO CLASS	N	L.L.	P.I.	% BY WEIGHT				% PASSING SIEVES			% MOISTURE	% ORGANIC	UNIT WT. (d)	VOID RATIO
								C. SAND	F. SAND	SILT	CLAY	10	40	200				
EB1-A																		
SS-6	6.6 LT	13+81	5.3-6.8	A-7-6(10)	4	47	24	26.5	13.9	19.2	40.4	87	69	54				
SS-7	6.6 LT	13+81	10.3-11.8	A-4(0)	4	23	5	27.7	31.1	23.0	18.2	98	87	44				
SS-8	6.6 LT	13+81	15.3-16.8	A-2-4(0)	6	20	NP	39.6	42.2	12.1	6.1	92	69	21				
B1-A																		
SS-1	6.5 LT	14+50	0.5-1.5	A-7-6(15)	0	41	17	5.5	13.9	44.2	36.4	98	95	83				
SS-2	6.5 LT	14+50	3.0-4.0	A-1-b(0)	5	28	5	67.5	14.5	9.9	8.1	80	36	16				
EB2-A																		
SS-3	7.0 LT	15+02	5.1-6.6	A-4(2)	7	30	10	30.5	18.2	25.1	26.3	89	68	50				
SS-4	7.0 LT	15+02	10.1-11.6	A-4(3)	6	26	9	11.3	34.3	26.1	28.3	100	95	62				
SS-5	7.0 LT	15+02	20.6-21.6	A-2-4(0)	35	27	3	49.5	28.5	13.9	8.1	86	56	23				

ROCK SAMPLE RESULTS

SAMPLE NO.	OFFSET	STATION	DEPTH INTERVAL	RQD	UNIT WT lb/ft3	Q (ksf)	E @ 40% (MPsi)	DIAMETER in	AREA in2	SPECIMEN HT (in)	H/D RATIO
B1-A											
RS-1 Top	6.5 LT.	14+50	15.2-16.3	81%	167.2	2026.08	3.68	1.868	2.7406	4.07	2.18
RS-1 Bottom	6.5 LT.	14+50	15.2-16.3	81%	166.5	2286.72	3.2	1.868	2.7406	3.98	2.13



FIELD SCOUR REPORT

WBS: 40057.1.1 TIP: B-4955 COUNTY: Alamance

DESCRIPTION(1): Bridge No. 162 on SR 1113 (Kimesville Rd.) Over South Prong Stinking Quarter Creek

EXISTING BRIDGE

Information from: Field Inspection Microfilm _____ (reel _____ pos: _____)
 Other (explain) _____

Bridge No.: 162 Length: 80.7 Total Bents: 3 Bents in Channel: 2 Bents in Floodplain: 3
 Foundation Type: Concrete Abutments, Timber Pile, Timber Deck on Steel Girders

EVIDENCE OF SCOUR(2)

Abutments or End Bent Slopes: Not Evident

Interior Bents: Some erosion at corners of footing

Channel Bed: Not Evident

Channel Bank: Some undercutting, trees lean toward channel

EXISTING SCOUR PROTECTION

Type(3): None observed

Extent(4): N/A

Effectiveness(5): N/A

Obstructions(6): N/A

INSTRUCTIONS

- 1 Describe the specific site's location, including route number and body of water crossed.
- 2 Note scour evidence at existing end bents or abutments (e.g. undermining, sloughing, degradations).
- 3 Note existing scour protection (e.g. rip rap).
- 4 Describe extent of existing scour protection.
- 5 Describe whether or not the scour protection appears to be working.
- 6 Note obstructions such as dams, fallen trees, debris at bents, etc.
- 7 Describe the channel bed material based on observation and/or samples. Include any lab results with report.
- 8 Describe the channel bank material based on observation and/or samples. Include any lab results with report.
- 9 Describe the material covering the banks (e.g. grass, trees, rip rap, none).
- 10 Determine the approximate floodplain width from field observation or a topographic map.
- 11 Describe the material covering the floodplain (e.g. grass, trees, crops).
- 12 Use professional judgement to specify if the stream is degrading, aggrading, or static.
- 13 Describe potential and direction of the stream to migrate laterally during the bridge's life (approx. 100 years).
- 14 Give the design scour elevation (DSE) expected over the life of the bridge (approx. 100 years). This elevation can be given as a range across the site, or for each bent. Discuss the relationship between the Hydraulics Unit theoretical scour and the DSE. If the DSE is dependent on scour counter measures, explain (e.g. rip rap armoring on slopes). The DSE is based on the erodability of materials, giving consideration to the influence of joints, foliation, bedding characteristics, % core recovery, % RQD, differential weathering, shear strength, observations at existing structures, other tests deemed appropriate, and overall geologic conditions at the site.

DESIGN INFORMATION

Channel Bed Material(7): Silt, Sand, and Gravel

Channel Bank Material(8): Silt and Sand

Channel Bank Cover(9): Mature trees, grass, and shrubs

Floodplain Width(10): 400 Feet +/-

Floodplain Cover(11): Mature trees, grass, and shrubs

Stream is(12): Aggrading Degrading _____ Static _____

Channel Migration Tendency(13): Moderate

Observations and Other Comments: Active beaver site

DESIGN SCOUR ELEVATIONS(14)

Feet Meters _____

BENTS

	B1	B2	B3	B4						
529.8										

Comparison of DSE to Hydraulics Unit theoretical scour:
 Hydraulics Unit did not supply a theoretical scour until the geotechnical investigation was complete and GASE was calculated for this project.

SOIL ANALYSIS RESULTS FROM CHANNEL BED AND BANK MATERIAL

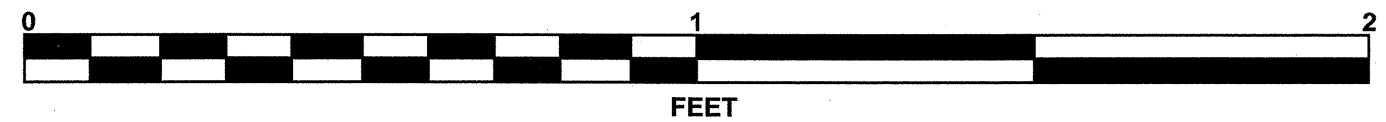
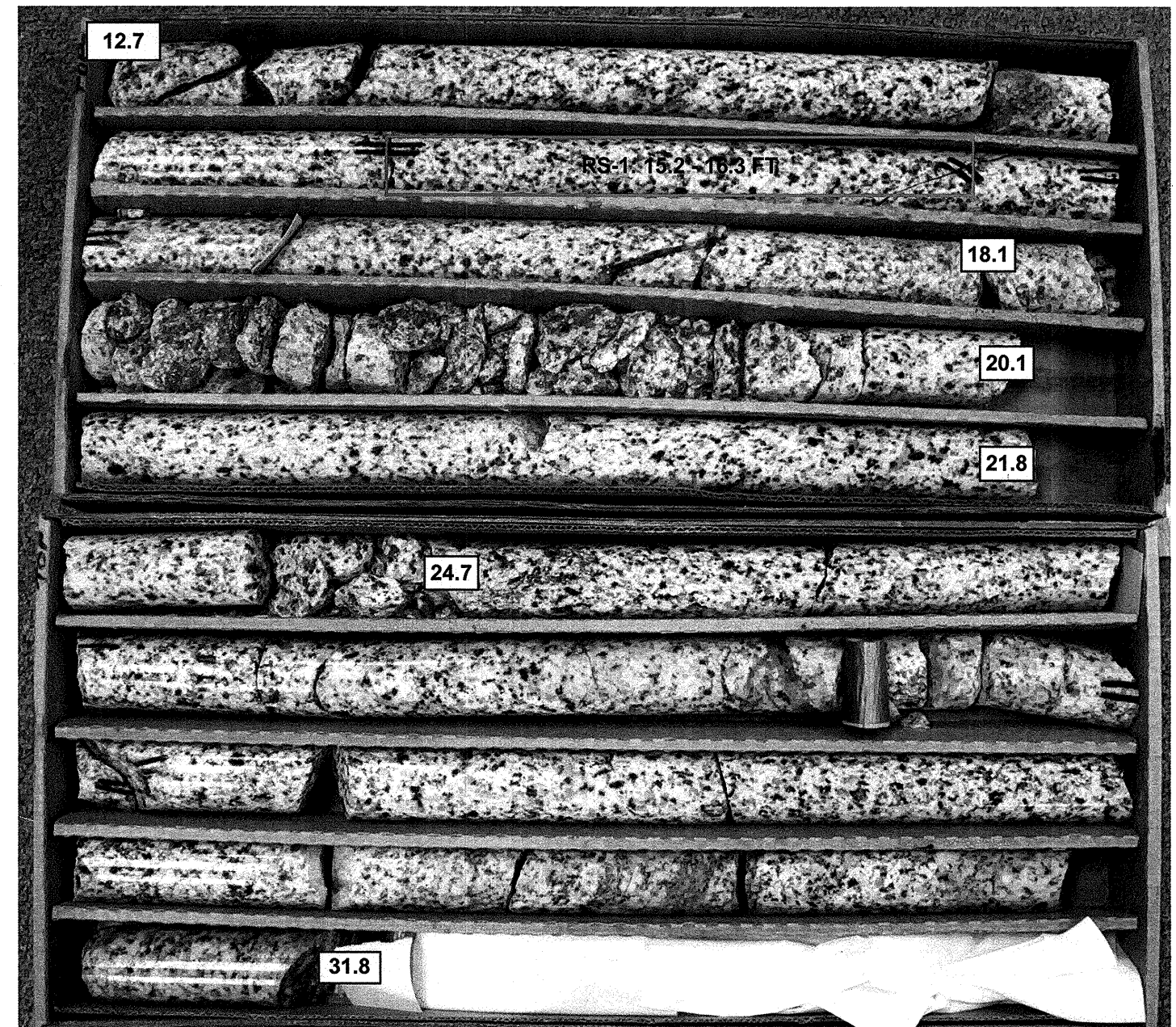
Bed or Bank	Bed									
Sample No.	SS-1									
Retained #4										
Passed #10	98									
Passed #40	95									
Passed #200	83									
Coarse Sand	5.5									
Fine Sand	13.9									
Silt	44.2									
Clay	36.4									
LL	41									
PI	17									
AASHTO	A-7-6(15)									
Station	14+50									
Offset	6.5 LT									
Depth	0.5 - 1.5									

Reported by: *Cheryl A. Youngblood*
 Cheryl A. Youngblood

Date: 12/11/2011

CORE PHOTOGRAPHS

B1-A
BOXES 1 & 2: 12.7 - 31.8 FEET



SITE PHOTOGRAPH

Bridge No. 162 on SR 1113 (Kimesville Rd.) Over South Prong Stinking Quarter Creek

