

STATE OF NORTH CAROLINA
DEPARTMENT OF TRANSPORTATION
DIVISION OF HIGHWAYS
GEOTECHNICAL ENGINEERING UNIT

STRUCTURE
SUBSURFACE INVESTIGATION

PROJ. REFERENCE NO. 40107.1.1(B-4949) F.A. PROJ. BRNHS-095-2(103)40
COUNTY CUMBERLAND
SITE DESCRIPTION BRIDGE NO. 61 ON -L- (US 301/I-95 BUSINESS)
OVER CROSS CREEK

INVENTORY

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CAUTION NOTICE

THE SUBSURFACE INFORMATION AND THE SUBSURFACE INVESTIGATION ON WHICH IT IS BASED WERE MADE FOR THE PURPOSE OF STUDY, PLANNING, AND DESIGN, AND NOT FOR CONSTRUCTION OR PAY PURPOSES. THE VARIOUS FIELD BORING LOGS, ROCK CORES, AND SOIL TEST DATA AVAILABLE MAY BE REVIEWED OR INSPECTED IN FIELD BY CONTACTING THE N.C. DEPARTMENT OF TRANSPORTATION, GEOTECHNICAL ENGINEERING UNIT AT (919) 250-4088. NEITHER THE SUBSURFACE PLANS AND REPORTS, NOR THE FIELD BORING LOGS, ROCK CORES, OR SOIL TEST DATA ARE PART OF THE CONTRACT.

GENERAL SOIL AND ROCK STRATA DESCRIPTIONS AND INDICATED BOUNDARIES ARE BASED ON A GEOTECHNICAL INTERPRETATION OF ALL AVAILABLE SUBSURFACE DATA AND MAY NOT NECESSARILY REFLECT THE ACTUAL SUBSURFACE CONDITIONS BETWEEN BORINGS OR BETWEEN SAMPLED STRATA WITHIN THE BOREHOLE. THE LABORATORY SAMPLE DATA AND THE IN SITU (IN-PLACED) TEST DATA CAN BE RELIED ON ONLY TO THE DEGREE OF RELIABILITY INHERENT IN THE STANDARD TEST METHOD. THE OBSERVED WATER LEVELS OR SOIL MOISTURE CONDITIONS INDICATED IN THE SUBSURFACE INVESTIGATIONS ARE AS RECORDED AT THE TIME OF THE INVESTIGATION. THESE WATER LEVELS OR SOIL MOISTURE CONDITIONS MAY VARY CONSIDERABLY WITH TIME ACCORDING TO CLIMATIC CONDITIONS INCLUDING TEMPERATURES, PRECIPITATION, AND WIND, AS WELL AS OTHER NON-CLIMATIC FACTORS.

THE BIDDER OR CONTRACTOR IS CAUTIONED THAT DETAILS SHOWN ON THE SUBSURFACE PLANS ARE PRELIMINARY ONLY AND IN MANY CASES THE FINAL DESIGN DETAILS ARE DIFFERENT. FOR BIDDING AND CONSTRUCTION PURPOSES, REFER TO THE CONSTRUCTION PLANS AND DOCUMENTS FOR FINAL DESIGN INFORMATION ON THIS PROJECT. THE DEPARTMENT DOES NOT WARRANT OR GUARANTEE THE SUFFICIENCY OR ACCURACY OF THE INVESTIGATION MADE, NOR THE INTERPRETATIONS MADE, OR OPINION OF THE DEPARTMENT AS TO THE TYPE OF MATERIALS AND CONDITIONS TO BE ENCOUNTERED. THE BIDDER OR CONTRACTOR IS CAUTIONED TO MAKE SUCH INDEPENDENT SUBSURFACE INVESTIGATIONS AS HE DEEMS NECESSARY TO SATISFY HIMSELF AS TO CONDITIONS TO BE ENCOUNTERED ON THIS PROJECT. THE CONTRACTOR SHALL HAVE NO CLAIM FOR ADDITIONAL COMPENSATION OR FOR AN EXTENSION OF TIME FOR ANY REASON RESULTING FROM THE ACTUAL CONDITIONS ENCOUNTERED AT THE SITE DIFFERING FROM THOSE INDICATED IN THE SUBSURFACE INFORMATION.

PROJECT: 40107.1.1 ID: B-4949

PERSONNEL

O.B. OTI

D.W. DIXON

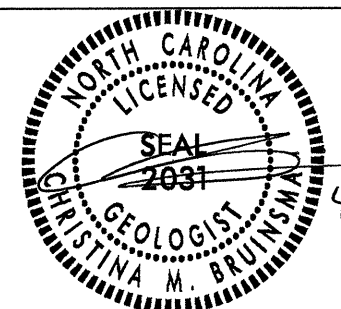
J.R. TURNAGE

INVESTIGATED BY O.B. OTI

CHECKED BY C.M. BRUINSMA

SUBMITTED BY N.T. ROBERSON

DATE APRIL 2011



DRAWN BY: T.T. WALKER, C.M. BRUINSMA

NOTE - THE INFORMATION CONTAINED HEREIN IS NOT IMPLIED OR GUARANTEED BY THE N.C. DEPARTMENT OF TRANSPORTATION AS BEING ACCURATE NOR IT IS CONSIDERED TO BE PART OF THE PLANS, SPECIFICATIONS, OR CONTRACT FOR THE PROJECT.

NOTE - BY HAVING REQUESTED THIS INFORMATION THE CONTRACTOR SPECIFICALLY WAIVES ANY CLAIMS FOR INCREASED COMPENSATION OR EXTENSION OF TIME BASED ON DIFFERENCES BETWEEN THE CONDITIONS INDICATED HEREIN AND THE ACTUAL CONDITIONS AT THE PROJECT SITE.

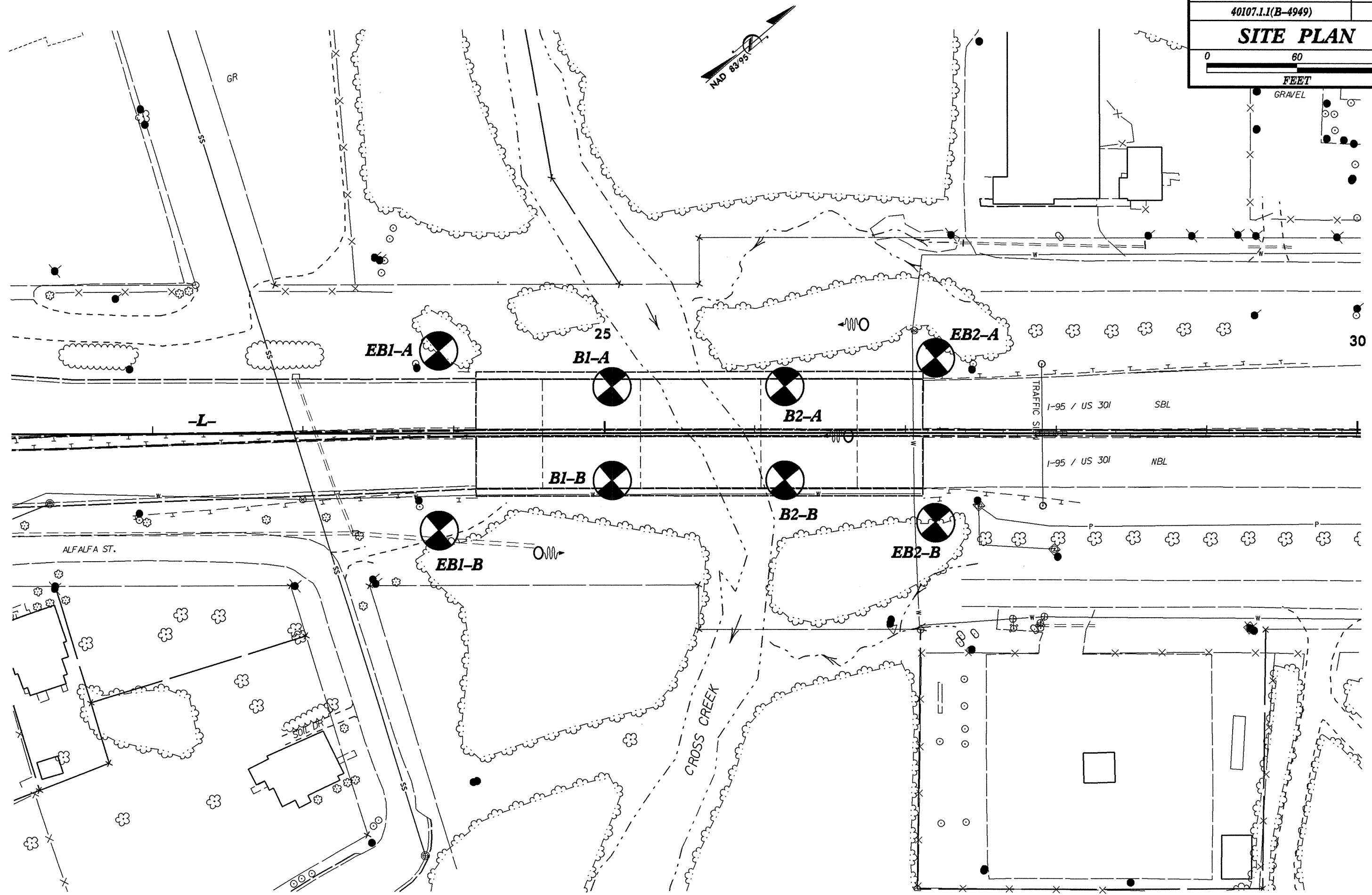
NORTH CAROLINA DEPARTMENT OF TRANSPORTATION
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PROJECT REFERENCE NO. 40107.I.(B-4949)	SHEET NO. 2
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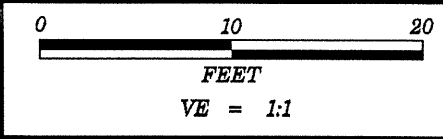
SUBSURFACE INVESTIGATION

SOIL AND ROCK LEGEND, TERMS, SYMBOLS, AND ABBREVIATIONS

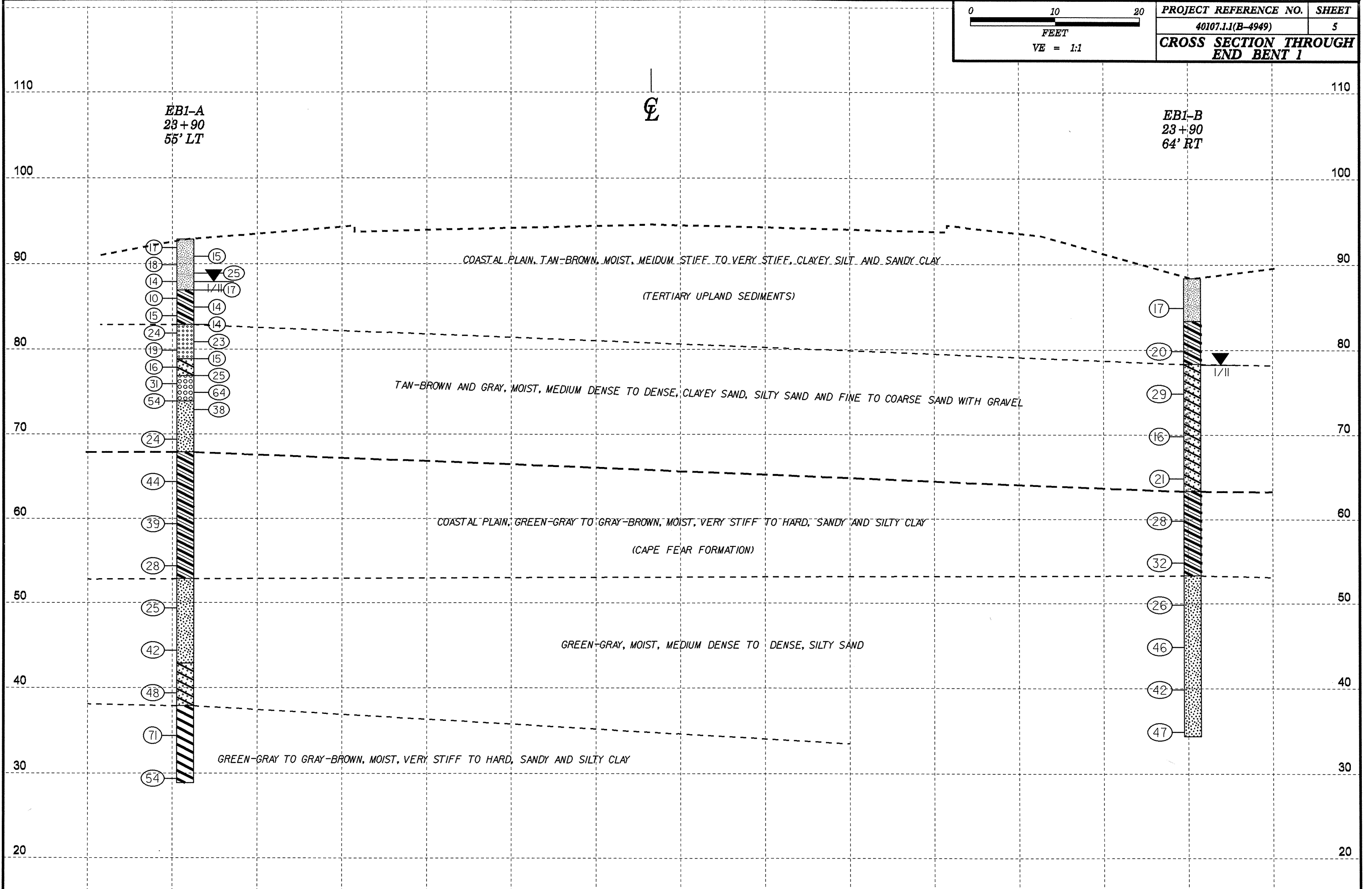
SOIL DESCRIPTION	GRADATION	ROCK DESCRIPTION	TERMS AND DEFINITIONS																																																																																																																																																																																														
<p>SOIL IS CONSIDERED TO BE THE UNCONSOLIDATED, SEMI-CONSOLIDATED, OR WEATHERED EARTH MATERIALS THAT CAN BE PENETRATED WITH A CONTINUOUS FLIGHT POWER AUGER, AND YIELD LESS THAN 100 BLOWS PER FOOT ACCORDING TO STANDARD PENETRATION TEST (AASHTO T206, ASTM D-1586). SOIL CLASSIFICATION IS BASED ON THE AASHTO SYSTEM. BASIC DESCRIPTIONS GENERALLY SHALL INCLUDE: CONSISTENCY, COLOR, TEXTURE, MOISTURE, AASHTO CLASSIFICATION, AND OTHER PERTINENT FACTORS SUCH AS MINERALOGICAL COMPOSITION, ANGULARITY, STRUCTURE, PLASTICITY, ETC. EXAMPLE:</p> <p style="text-align: center;"><i>VERY STIFF, GRAY SILTY CLAY, MOST WITH INTERBEDDED FINE SAND LAYERS, HEAVY PLASTIC, A-7-6</i></p>	<p>WELL GRADED - INDICATES A GOOD REPRESENTATION OF PARTICLE SIZES FROM FINE TO COARSE. UNIFORM - INDICATES THAT SOIL PARTICLES ARE ALL APPROXIMATELY THE SAME SIZE. (ALSO POORLY GRADED) GAP-GRADED - INDICATES A MIXTURE OF UNIFORM PARTICLES OF TWO OR MORE SIZES.</p> <p style="text-align: center;">ANGULARITY OF GRAINS</p> <p>THE ANGULARITY OR ROUNDNESS OF SOIL GRAINS IS DESIGNATED BY THE TERMS: <u>ANGULAR</u>, <u>SUBANGULAR</u>, <u>SUBROUNDED</u>, OR <u>ROUNDED</u>.</p>	<p>HARD ROCK IS NON-COASTAL PLAIN MATERIAL THAT IF TESTED, WOULD YIELD SPT REFUSAL. AN INFERRED ROCK LINE INDICATES THE LEVEL AT WHICH NON-COASTAL PLAIN MATERIAL WOULD YIELD SPT REFUSAL. SPT REFUSAL IS PENETRATION BY A SPLIT SPOON SAMPLER EQUAL TO OR LESS THAN 0.1 FOOT PER 60 BLOWS, IN NON-COASTAL PLAIN MATERIAL. THE TRANSITION BETWEEN SOIL AND ROCK IS OFTEN REPRESENTED BY A ZONE OF WEATHERED ROCK. ROCK MATERIALS ARE TYPICALLY DIVIDED AS FOLLOWS:</p>	<p>ALLUVIUM (ALLUV.) - SOILS THAT HAVE BEEN TRANSPORTED BY WATER. AQUIFER - A WATER BEARING FORMATION OR STRATA. ARENACEOUS - APPLIED TO ROCKS THAT HAVE BEEN DERIVED FROM SAND OR THAT CONTAIN SAND. ARGILLACEOUS - APPLIED TO ALL ROCKS OR SUBSTANCES COMPOSED OF CLAY MINERALS, OR HAVING A NOTABLE PROPORTION OF CLAY IN THEIR COMPOSITION, AS SHALE, SLATE, ETC. ARTESIAN - GROUND WATER THAT IS UNDER SUFFICIENT PRESSURE TO RISE ABOVE THE LEVEL AT WHICH IT IS ENCOUNTERED, BUT WHICH DOES NOT NECESSARILY RISE TO OR ABOVE THE GROUND SURFACE. CALCAREOUS (CALC.) - SOILS THAT CONTAIN APPRECIABLE AMOUNTS OF CALCIUM CARBONATE. COLLUVIUM - ROCK FRAGMENTS MIXED WITH SOIL DEPOSITED BY GRAVITY ON SLOPE OR AT BOTTOM OF SLOPE. CORE RECOVERY (REC.) - TOTAL LENGTH OF ALL MATERIAL RECOVERED IN THE CORE BARREL DIVIDED BY TOTAL LENGTH OF CORE RUN AND EXPRESSED AS A PERCENTAGE. DIKE - A TABULAR BODY OF IGNEOUS ROCK THAT CUTS ACROSS THE STRUCTURE OF ADJACENT ROCKS OR CUTS MASSIVE ROCK. DIP - THE ANGLE AT WHICH A STRATUM OR ANY PLANAR FEATURE IS INCLINED FROM THE HORIZONTAL. DIP DIRECTION (DIP AZIMUTH) - THE DIRECTION OR BEARING OF THE HORIZONTAL TRACE OF THE LINE OF DIP, MEASURED CLOCKWISE FROM NORTH. FAULT - A FRACTURE OR FRACTURE ZONE ALONG WHICH THERE HAS BEEN DISPLACEMENT OF THE SIDES RELATIVE TO ONE ANOTHER PARALLEL TO THE FRACTURE. FISSILE - A PROPERTY OF SPLITTING ALONG CLOSELY SPACED PARALLEL PLANES. FLOAT - ROCK FRAGMENTS ON SURFACE NEAR THEIR ORIGINAL POSITION AND DISLOGGED FROM PARENT MATERIAL. FLOOD PLAIN (FP) - LAND BORDERING A STREAM, BUILT OF SEDIMENTS DEPOSITED BY THE STREAM. FORMATION (FM) - A MAPPABLE GEOLOGIC UNIT THAT CAN BE RECOGNIZED AND TRACED IN THE FIELD. JOINT - FRACTURE IN ROCK ALONG WHICH NO APPRECIABLE MOVEMENT HAS OCCURRED. LEDGE - A SHELF-LIKE RIDGE OR PROJECTION OF ROCK WHOSE THICKNESS IS SMALL COMPARED TO ITS LATERAL EXTENT. LENS - A BODY OF SOIL OR ROCK THAT THINS OUT IN ONE OR MORE DIRECTIONS. MOTTLED (MOT.) - IRREGULARLY MARKED WITH SPOTS OF DIFFERENT COLORS, MOTTLING IN SOILS USUALLY INDICATES POOR AERATION AND LACK OF GOOD DRAINAGE. PERCHED WATER - WATER MAINTAINED ABOVE THE NORMAL GROUND WATER LEVEL BY THE PRESENCE OF AN INTERVENING IMPERVIOUS STRATUM. RESIDUAL (RES.) SOIL - SOIL FORMED IN PLACE BY THE WEATHERING OF ROCK. ROCK QUALITY DESIGNATION (RQD) - A MEASURE OF ROCK QUALITY DESCRIBED BY TOTAL LENGTH OF ROCK SEGMENTS EQUAL TO OR GREATER THAN 4 INCHES DIVIDED BY THE TOTAL LENGTH OF CORE RUN AND EXPRESSED AS A PERCENTAGE.</p>																																																																																																																																																																																														
<p style="text-align: center;">SOIL LEGEND AND AASHTO CLASSIFICATION</p> <table border="1" style="width: 100%; border-collapse: collapse;"> <tr> <th>GENERAL CLASS.</th> <th colspan="4">GRANULAR MATERIALS (≤ 35% PASSING #200)</th> <th colspan="4">SILT-CLAY MATERIALS (> 35% PASSING #200)</th> <th colspan="4">ORGANIC MATERIALS</th> </tr> <tr> <td>GROUP CLASS.</td> <td>A-1</td> <td>A-3</td> <td>A-2</td> <td>A-4</td> <td>A-5</td> <td>A-6</td> <td>A-7</td> <td>A-1, A-2</td> <td>A-4, A-5</td> <td>A-6, A-7</td> <td></td> <td></td> <td></td> </tr> <tr> <td>SYMBOL</td> <td></td> <td></td> <td></td> <td></td> <td></td> <td></td> <td></td> <td></td> <td></td> <td></td> <td></td> <td></td> <td></td> </tr> <tr> <td>% PASSING</td> <td>100</td> <td>100</td> <td>100</td> <td>100</td> <td>100</td> <td>100</td> <td>100</td> <td>100</td> <td>100</td> <td>100</td> <td></td> <td></td> <td></td> </tr> <tr> <td>LIQUID LIMIT</td> <td>≤ 5</td> <td>≤ 10</td> <td>≤ 15</td> <td>≤ 20</td> <td>≤ 25</td> <td>≤ 30</td> <td>≤ 35</td> <td>≤ 40</td> <td>≤ 45</td> <td>≤ 50</td> <td></td> <td></td> <td></td> </tr> <tr> <td>PLASTIC INDEX</td> <td>≤ 4</td> <td>≤ 7</td> <td>≤ 10</td> <td>≤ 13</td> <td>≤ 16</td> <td>≤ 19</td> <td>≤ 22</td> <td>≤ 25</td> <td>≤ 28</td> <td>≤ 31</td> <td></td> <td></td> <td></td> </tr> <tr> <td>GROUP INDEX</td> <td>0</td> <td>0</td> <td>0</td> <td>0</td> <td>0</td> <td>0</td> <td>0</td> <td>0</td> <td>0</td> <td>0</td> <td></td> <td></td> <td></td> </tr> <tr> <td>USUAL TYPES OF MAJOR MATERIALS</td> <td>STONE FRAGS. GRAVEL AND SAND</td> <td>FINE SAND</td> <td>SILTY OR CLAYEY GRAVEL AND SAND</td> <td>SILTY SOILS</td> <td>CLAYEY SOILS</td> <td></td> <td></td> <td></td> <td></td> <td></td> <td></td> <td></td> <td></td> </tr> <tr> <td>GEN. RATING AS A SUBGRADE</td> <td colspan="3">EXCELLENT TO GOOD</td> <td colspan="3">FAIR TO POOR</td> <td>FAIR TO POOR</td> <td>POOR</td> <td>UNSATURABLE</td> <td></td> <td></td> <td></td> <td></td> </tr> </table> <p style="text-align: center;">PI OF A-7-5 SUBGROUP IS ≤ LL - 30 ; PI OF A-7-6 SUBGROUP IS > LL - 30</p>	GENERAL CLASS.	GRANULAR MATERIALS (≤ 35% PASSING #200)				SILT-CLAY MATERIALS (> 35% PASSING #200)				ORGANIC MATERIALS				GROUP CLASS.	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ARE USED IN DESCRIPTIONS WHENEVER THEY ARE CONSIDERED OF SIGNIFICANCE.</p> <p style="text-align: center;">COMPRESSIBILITY</p> <table border="1" style="width: 100%; border-collapse: collapse;"> <tr> <td>SLIGHTLY COMPRESSIBLE</td> <td>LIQUID LIMIT LESS THAN 31</td> </tr> <tr> <td>MODERATELY COMPRESSIBLE</td> <td>LIQUID LIMIT EQUAL TO 31-50</td> </tr> <tr> <td>HIGHLY COMPRESSIBLE</td> <td>LIQUID LIMIT GREATER THAN 50</td> </tr> </table> <p style="text-align: center;">PERCENTAGE OF MATERIAL</p> <table border="1" style="width: 100%; border-collapse: collapse;"> <tr> <th>ORGANIC MATERIAL</th> <th>GRANULAR SOILS</th> <th>SILT - CLAY SOILS</th> <th>OTHER MATERIAL</th> </tr> <tr> <td>TRACE OF ORGANIC MATTER</td> <td>2 - 3%</td> <td>3 - 5%</td> <td>TRACE</td> </tr> <tr> <td>LITTLE ORGANIC MATTER</td> <td>3 - 5%</td> <td>5 - 12%</td> <td>LITTLE</td> </tr> <tr> <td>MODERATELY ORGANIC</td> <td>5 - 10%</td> <td>12 - 20%</td> <td>SOME</td> </tr> <tr> <td>HIGHLY ORGANIC</td> <td>>10%</td> <td>>20%</td> <td>HIGHLY</td> </tr> <tr> <td></td> <td></td> <td></td> <td>35% AND ABOVE</td> </tr> </table> <p style="text-align: center;">GROUND WATER</p> <p> WATER LEVEL IN BORE HOLE IMMEDIATELY AFTER DRILLING</p> <p> STATIC WATER LEVEL AFTER 24 HOURS</p> <p> PERCHED WATER, SATURATED ZONE, OR WATER BEARING STRATA</p> <p> SPRING OR SEEP</p> <p style="text-align: center;">MISCELLANEOUS SYMBOLS</p> <table border="1" style="width: 100%; border-collapse: collapse;"> <tr> <td> ROADWAY EMBANKMENT (RE) WITH SOIL DESCRIPTION</td> <td> SPT TEST BORING</td> <td> TEST BORING W/ CORE</td> </tr> <tr> <td> SOIL SYMBOL</td> <td> AUGER BORING</td> <td> SPT N-VALUE</td> </tr> <tr> <td> ARTIFICIAL FILL (AF) OTHER THAN ROADWAY EMBANKMENT</td> <td> CORE BORING</td> <td> SPT REFUSAL</td> </tr> <tr> <td> INFERRED SOIL BOUNDARY</td> <td> MONITORING WELL</td> <td></td> </tr> <tr> <td> INFERRED ROCK LINE</td> <td> PIEZOMETER INSTALLATION</td> <td></td> </tr> <tr> <td> ALLUVIAL SOIL BOUNDARY</td> <td> SLOPE INDICATOR INSTALLATION</td> <td></td> </tr> <tr> <td> DIP & DIP DIRECTION OF ROCK STRUCTURES</td> <td> CONE PENETROMETER TEST</td> <td></td> </tr> <tr> <td></td> <td> SOUNDING ROD</td> <td></td> </tr> </table>	SLIGHTLY COMPRESSIBLE	LIQUID LIMIT LESS THAN 31	MODERATELY COMPRESSIBLE	LIQUID LIMIT EQUAL TO 31-50	HIGHLY COMPRESSIBLE	LIQUID LIMIT GREATER THAN 50	ORGANIC MATERIAL	GRANULAR SOILS	SILT - CLAY SOILS	OTHER MATERIAL	TRACE OF ORGANIC MATTER	2 - 3%	3 - 5%	TRACE	LITTLE ORGANIC MATTER	3 - 5%	5 - 12%	LITTLE	MODERATELY ORGANIC	5 - 10%	12 - 20%	SOME	HIGHLY ORGANIC	>10%	>20%	HIGHLY				35% AND ABOVE	ROADWAY EMBANKMENT (RE) WITH SOIL DESCRIPTION	SPT TEST BORING	TEST BORING W/ CORE	SOIL SYMBOL	AUGER BORING	SPT N-VALUE	ARTIFICIAL FILL (AF) OTHER THAN ROADWAY EMBANKMENT	CORE BORING	SPT REFUSAL	INFERRED SOIL BOUNDARY	MONITORING WELL		INFERRED ROCK LINE	PIEZOMETER INSTALLATION		ALLUVIAL SOIL BOUNDARY	SLOPE INDICATOR INSTALLATION		DIP & DIP DIRECTION OF ROCK STRUCTURES	CONE PENETROMETER TEST			SOUNDING ROD		<p style="text-align: center;">ROCK HARDNESS</p> <table border="1" style="width: 100%; border-collapse: collapse;"> <tr> <td>VERY HARD</td> <td>CANNOT BE SCRATCHED BY KNIFE OR SHARP PICK. 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HAMMER FALLING 30 INCHES REQUIRED TO PRODUCE A PENETRATION OF 1 FOOT INTO SOIL WITH A 2 INCH OUTSIDE DIAMETER SPLIT SPOON SAMPLER. SPT REFUSAL IS PENETRATION EQUAL TO OR LESS THAN 0.1 FOOT PER 60 BLOWS.</p> <p>STRATA CORE RECOVERY (SCRC) - TOTAL LENGTH OF STRATA MATERIAL RECOVERED DIVIDED BY TOTAL LENGTH OF STRATUM AND EXPRESSED AS A PERCENTAGE.</p> <p>STRATA ROCK QUALITY DESIGNATION (SRQD) - A MEASURE OF ROCK QUALITY DESCRIBED BY TOTAL LENGTH OF ROCK SEGMENTS WITHIN A STRATUM EQUAL TO OR GREATER THAN 4 INCHES DIVIDED BY THE TOTAL LENGTH OF STRATA AND EXPRESSED AS A PERCENTAGE.</p> <p>TOPSOIL (TS.) - SURFACE SOILS USUALLY CONTAINING ORGANIC MATTER.</p>	VERY HARD	CANNOT BE SCRATCHED BY KNIFE OR SHARP PICK. BREAKING OF HAND SPECIMENS REQUIRES SEVERAL HARD BLOWS OF THE GEOLOGIST'S PICK.	HARD	CAN BE SCRATCHED BY KNIFE OR PICK ONLY WITH DIFFICULTY. HARD HAMMER BLOWS REQUIRED TO DETACH HAND SPECIMEN.	MODERATELY HARD	CAN BE SCRATCHED BY KNIFE OR PICK. 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HARD	CAN BE SCRATCHED BY KNIFE OR PICK ONLY WITH DIFFICULTY. HARD HAMMER BLOWS REQUIRED TO DETACH HAND SPECIMEN.																																																																																																																																																																																																
MODERATELY HARD	CAN BE SCRATCHED BY KNIFE OR PICK. GOUGES OR GROOVES TO 0.25 INCHES DEEP CAN BE EXCAVATED BY HARD BLOW OF A GEOLOGIST'S PICK. HAND SPECIMENS CAN BE DETACHED BY MODERATE BLOWS.																																																																																																																																																																																																
MEDIUM HARD	CAN BE GROOVED OR GOUGED 0.05 INCHES DEEP BY FIRM PRESSURE OF KNIFE OR PICK POINT. CAN BE EXCAVATED IN SMALL CHIPS TO PEICES 1 INCH MAXIMUM SIZE BY HARD BLOWS OF THE POINT OF A GEOLOGIST'S PICK.																																																																																																																																																																																																
SOFT	CAN BE GROVED OR GOUGED READILY BY KNIFE OR PICK. CAN BE EXCAVATED IN FRAGMENTS FROM CHIPS TO SEVERAL INCHES IN SIZE BY MODERATE BLOWS OF A PICK POINT. SMALL, THIN PIECES CAN BE BROKEN BY FINGER PRESSURE.																																																																																																																																																																																																
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	26 OR MORE	HIGH																																																																																																																																																																																															
PRIMARY SOIL TYPE	COMPACTNESS OR CONSISTENCY	RANGE OF STANDARD PENETRATION RESISTANCE (N-VALUE)	RANGE OF UNCONFINED COMPRESSIVE STRENGTH (TONS/FT ²)																																																																																																																																																																																														
GENERALLY GRANULAR MATERIAL (NON-COHESIVE)	VERY LOOSE LOOSE MEDIUM DENSE DENSE VERY DENSE	<4 4 TO 10 10 TO 30 30 TO 50 >50	N/A																																																																																																																																																																																														
GENERALLY SILT-CLAY MATERIAL (COHESIVE)	VERY SOFT SOFT MEDIUM STIFF STIFF VERY STIFF HARD	<2 2 TO 4 4 TO 8 8 TO 15 15 TO 30 >30	<0.25 0.25 TO 0.50 0.5 TO 1.0 1 TO 2 2 TO 4 >4																																																																																																																																																																																														



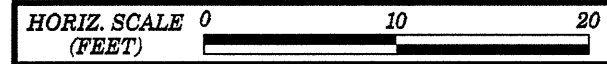
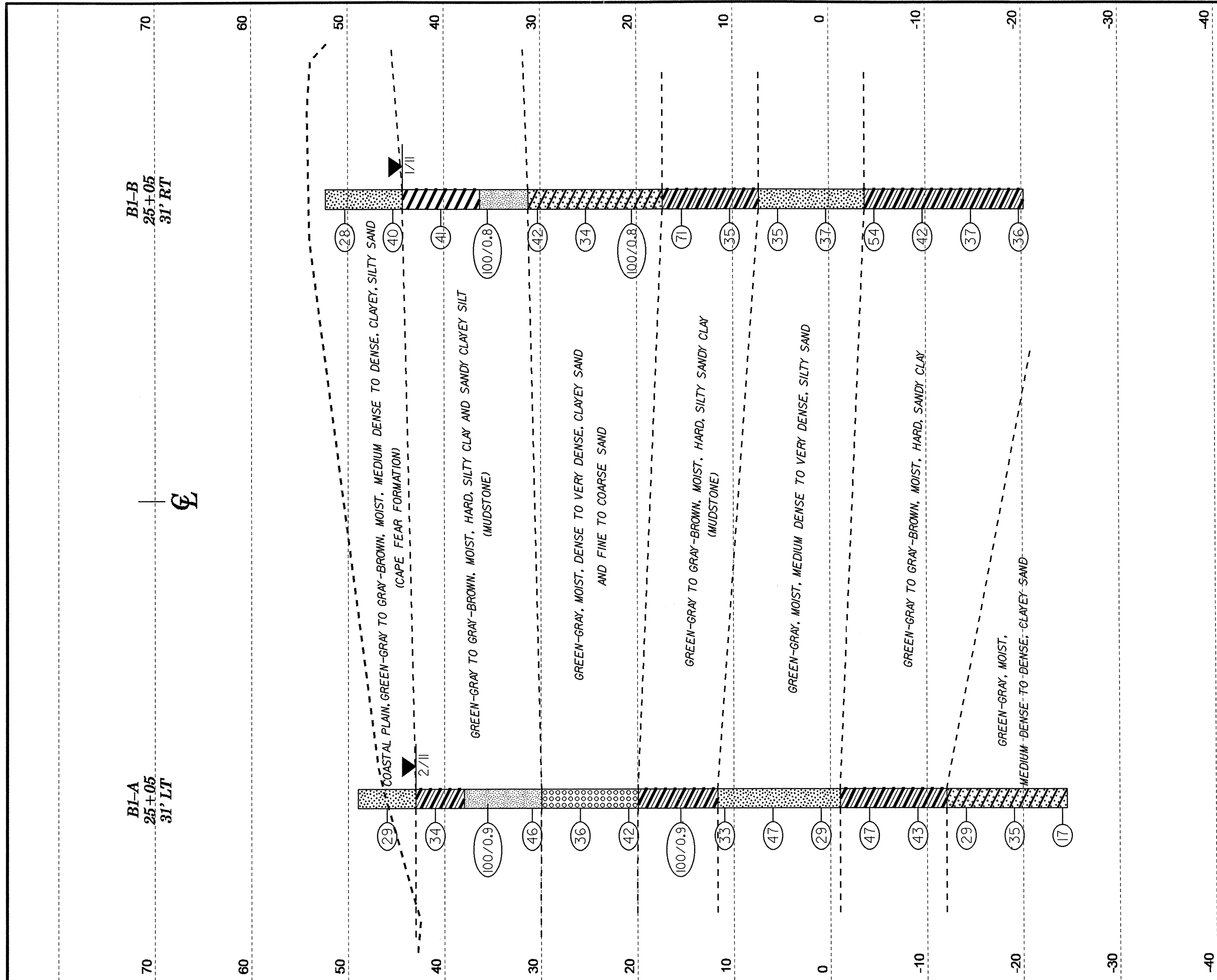
90° SKEW



PROJECT REFERENCE NO.	SHEET
40107.1.1(B-4949)	5
CROSS SECTION THROUGH END BENT 1	



NOTE: GROUNDLINE IS A COMBINATION OF DATA OBTAINED FROM THE .TIN FILE DATED 11/4/08 AND FIELD SURVEY PERFORMED ON 2/16/11.



VE = 1:1

CROSS SECTION THROUGH BENT 1

B1-B
25 ± 0.5
31' RT

B1-A
25 ± 0.5
31' LT



70

60

50

40

30

20

10

0

-10

-20

-30

-40

70

60

50

40

30

20

10

0

-10

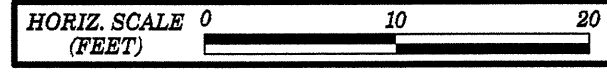
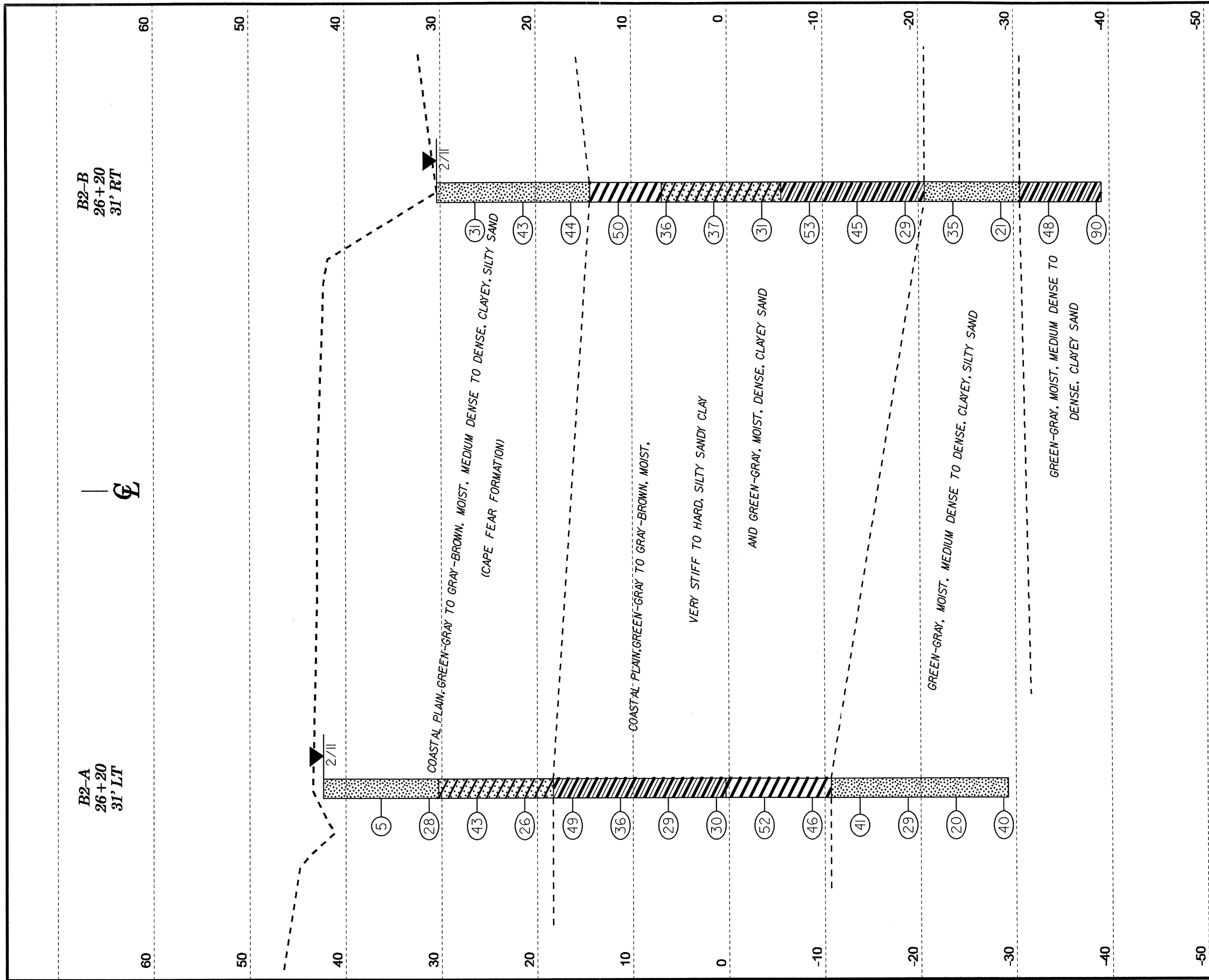
-20

-30

-40

-50

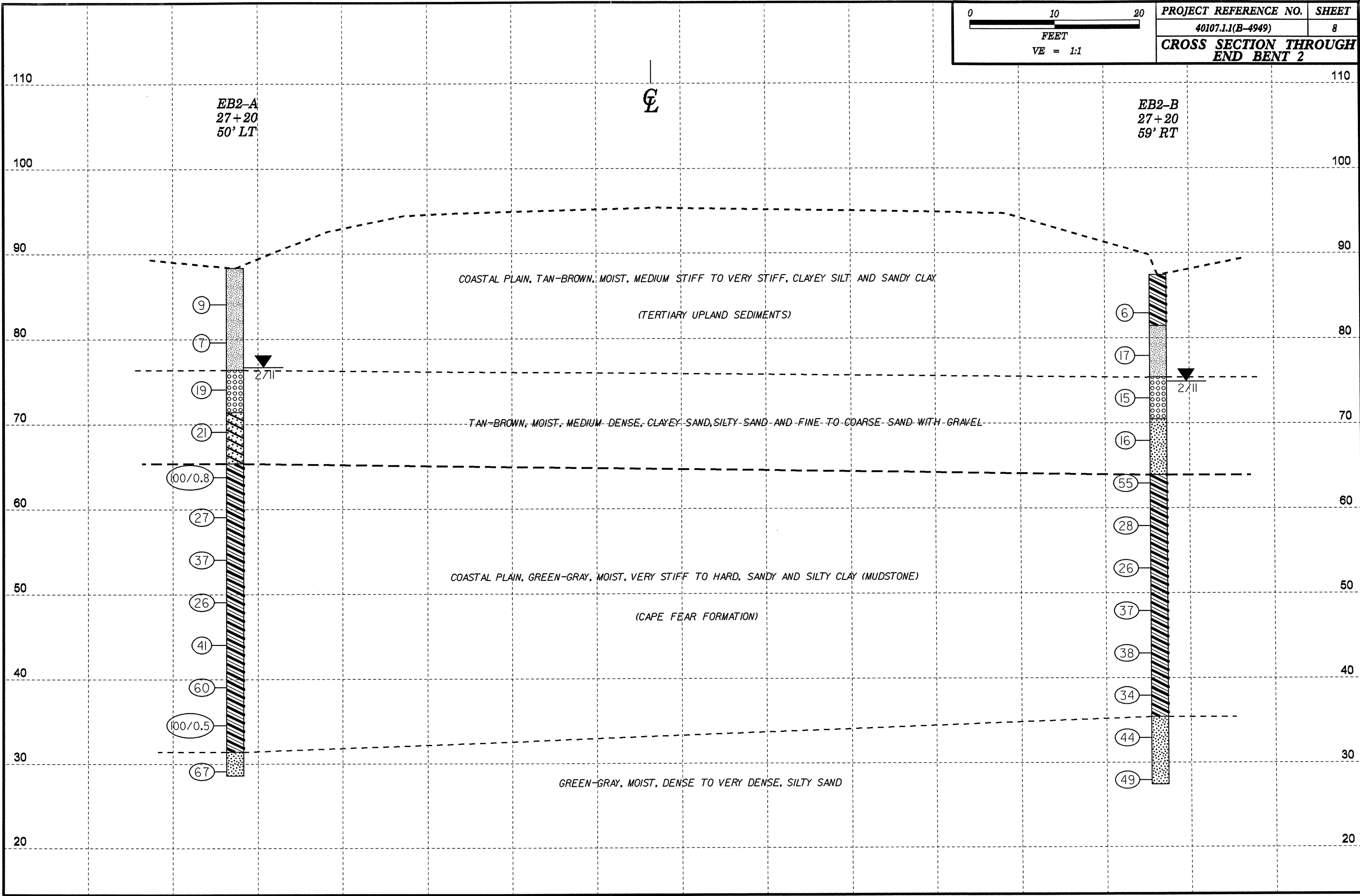
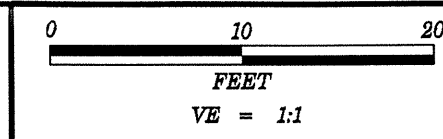
-60



VE = 1:1

CROSS SECTION THROUGH BENT 2

NOTE: GROUNDLINE IS A COMBINATION OF DATA OBTAINED FROM THE .TIN FILE DATED 11/4/08 AND FIELD SURVEY PERFORMED ON 2/16/11.

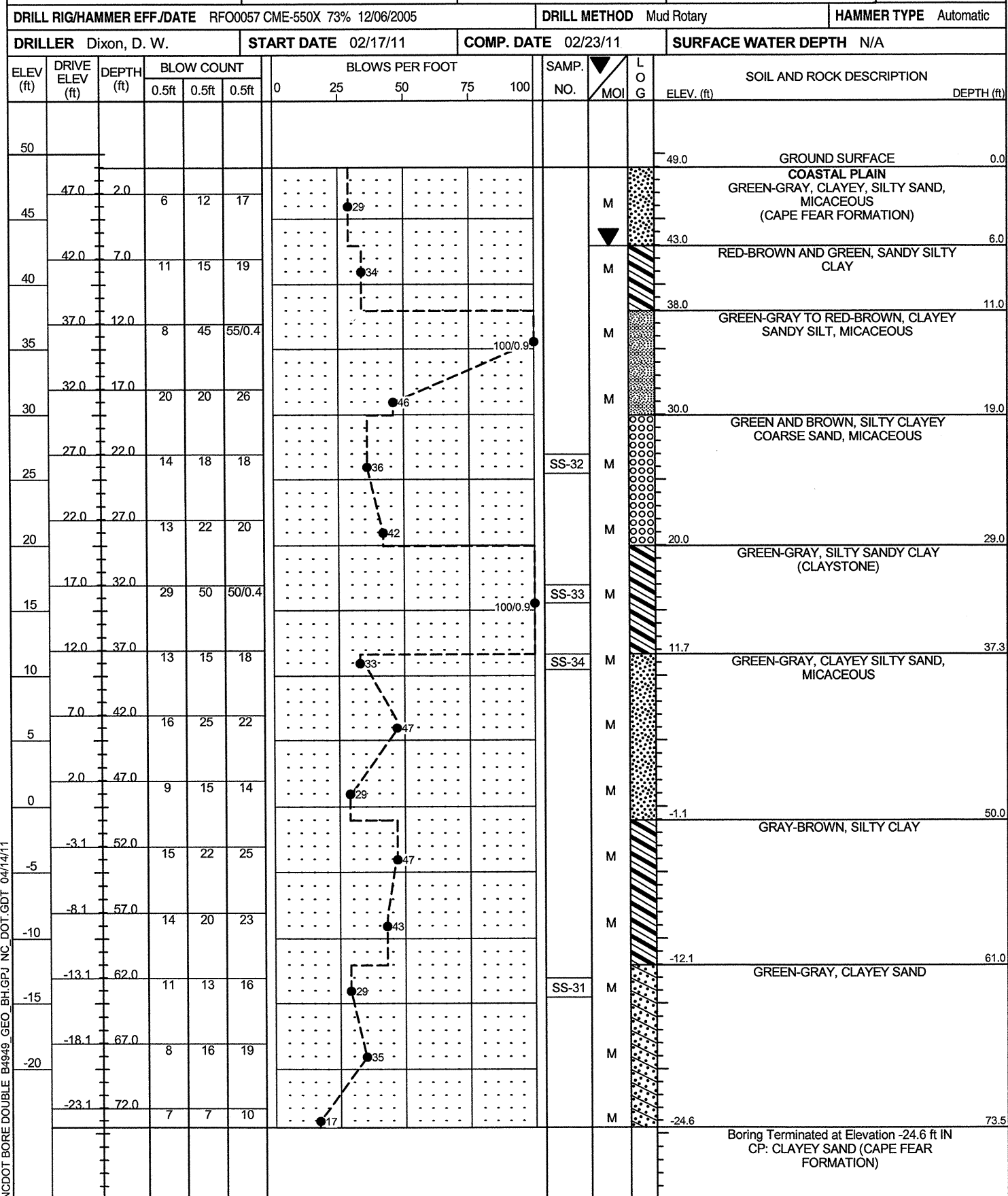




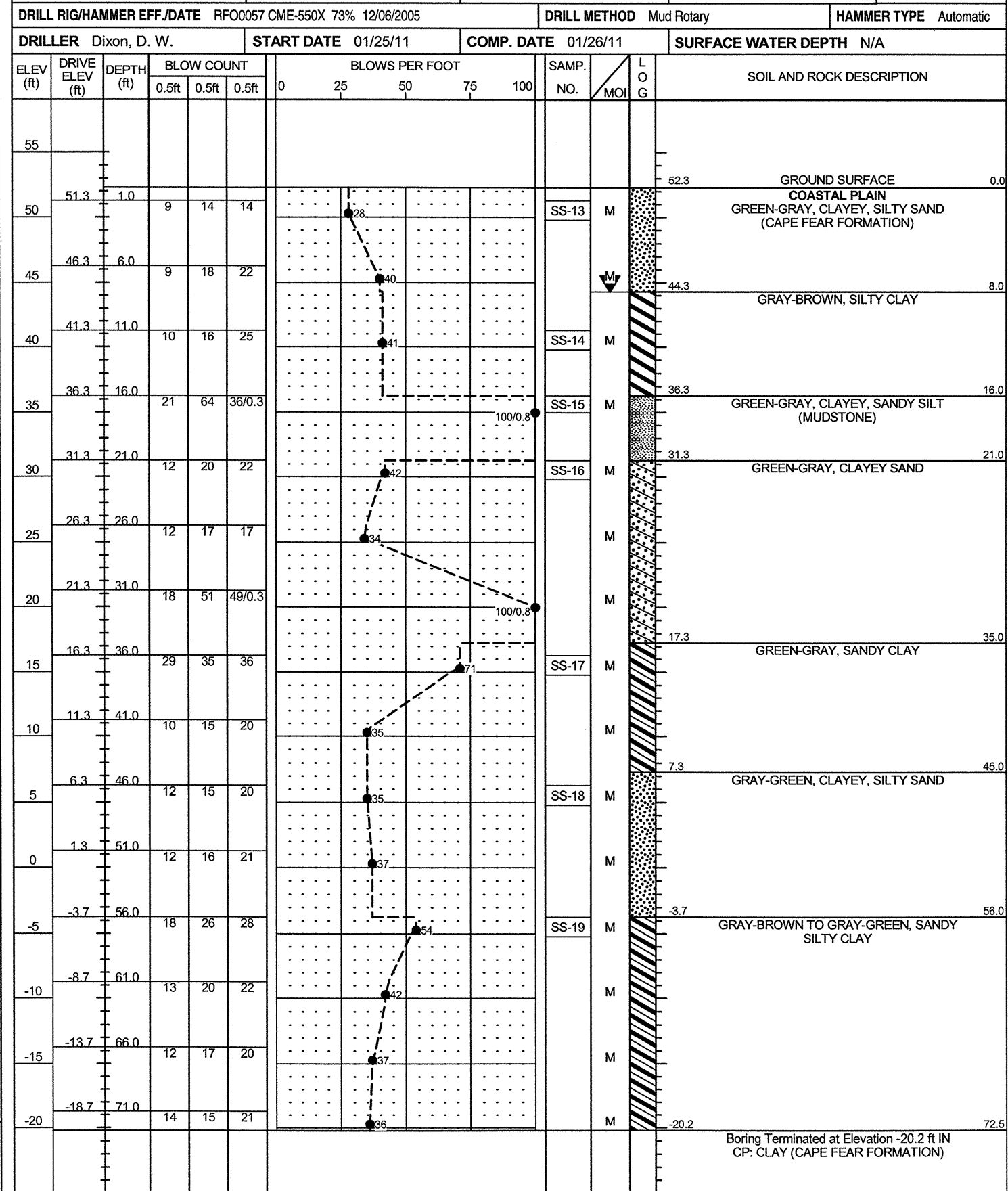
NCDOT GEOTECHNICAL ENGINEERING UNIT

BORELOG REPORT

WBS 40107.1.1	TIP B-4949	COUNTY CUMBERLAND	GEOLOGIST Oti, O. B.
SITE DESCRIPTION BRIDGE NO. 61 ON -L- (US 301/I-95 BUS.) OVER CROSS CREEK			GROUND WTR (ft)
BORING NO. B1-A	STATION 25+05	OFFSET 31 ft LT	ALIGNMENT -L-
COLLAR ELEV. 49.0 ft	TOTAL DEPTH 73.5 ft	NORTHING 474,859	EASTING 2,041,104
DRILL RIG/HAMMER EFF./DATE RFO0057 CME-550X 73% 12/06/2005		DRILL METHOD Mud Rotary	HAMMER TYPE Automatic
DRILLER Dixon, D. W.	START DATE 02/17/11	COMP. DATE 02/23/11	SURFACE WATER DEPTH N/A



WBS 40107.1.1	TIP B-4949	COUNTY CUMBERLAND	GEOLOGIST Oti, O. B.
SITE DESCRIPTION BRIDGE NO. 61 ON -L- (US 301/I-95 BUS.) OVER CROSS CREEK			GROUND WTR (ft)
BORING NO. B1-B	STATION 25+05	OFFSET 31 ft RT	ALIGNMENT -L-
COLLAR ELEV. 52.3 ft	TOTAL DEPTH 72.5 ft	NORTHING 474,819	EASTING 2,041,151
DRILL RIG/HAMMER EFF./DATE RFO0057 CME-550X 73% 12/06/2005		DRILL METHOD Mud Rotary	HAMMER TYPE Automatic
DRILLER Dixon, D. W.	START DATE 01/25/11	COMP. DATE 01/26/11	SURFACE WATER DEPTH N/A



NCDOT BORE DOUBLE B4949_GEO_BH.GPJ NC_DOT.GDT 04/14/11

Boring Terminated at Elevation -24.6 ft IN
CP: CLAYEY SAND (CAPE FEAR FORMATION)

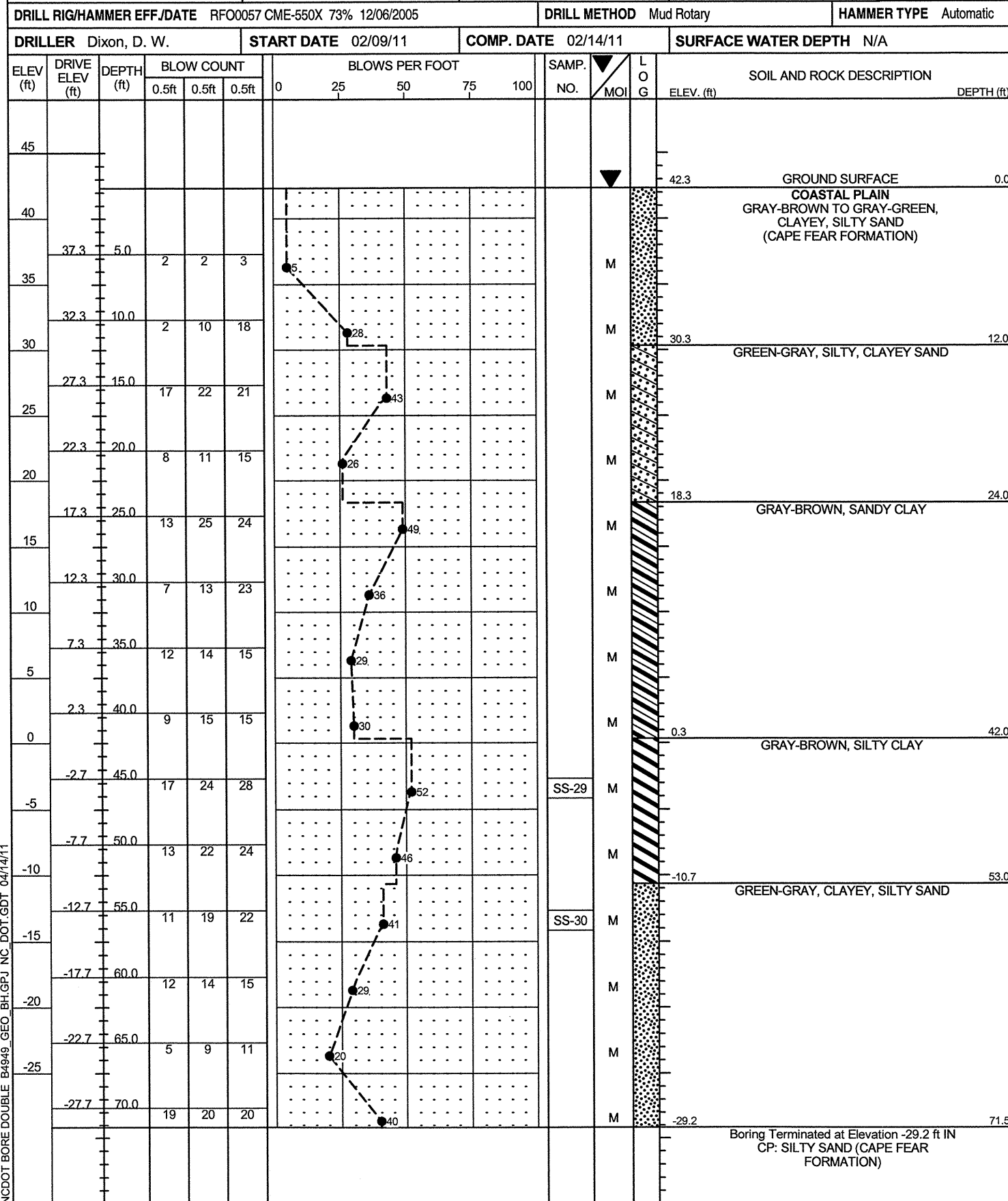
Boring Terminated at Elevation -20.2 ft IN
CP: CLAY (CAPE FEAR FORMATION)



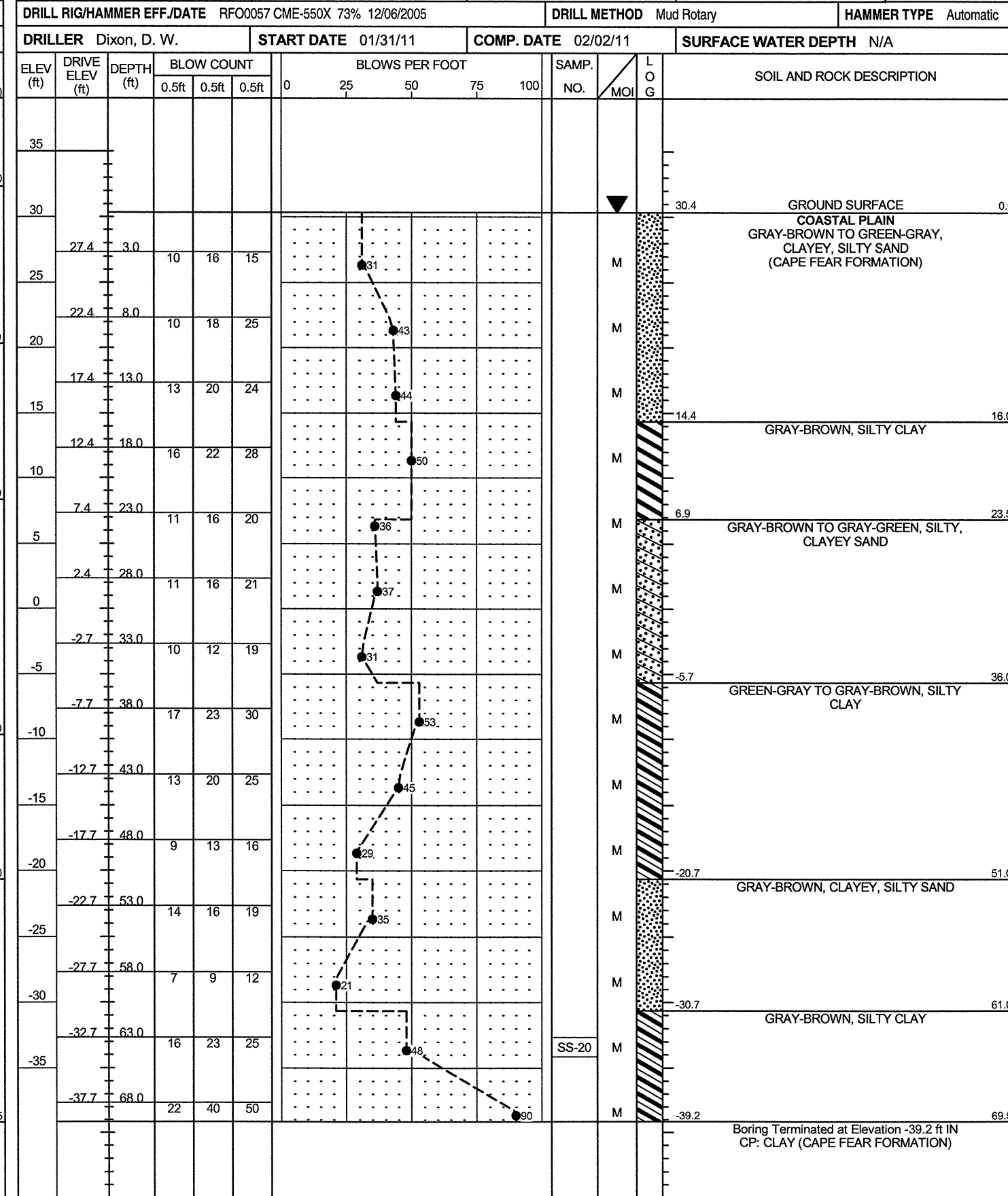
NCDOT GEOTECHNICAL ENGINEERING UNIT

BORELOG REPORT

WBS 40107.1.1	TIP B-4949	COUNTY CUMBERLAND	GEOLOGIST Oti, O. B.
SITE DESCRIPTION BRIDGE NO. 61 ON -L- (US 301/I-95 BUS.) OVER CROSS CREEK			GROUND WTR (ft)
BORING NO. B2-A	STATION 26+20	OFFSET 31 ft LT	ALIGNMENT -L-
COLLAR ELEV. 42.3 ft	TOTAL DEPTH 71.5 ft	NORTHING 474,947	EASTING 2,041,177
DRILL RIG/HAMMER EFF./DATE RFO0057 CME-550X 73% 12/06/2005		DRILL METHOD Mud Rotary	HAMMER TYPE Automatic
DRILLER Dixon, D. W.	START DATE 02/09/11	COMP. DATE 02/14/11	SURFACE WATER DEPTH N/A



WBS 40107.1.1	TIP B-4949	COUNTY CUMBERLAND	GEOLOGIST Oti, O. B.
SITE DESCRIPTION BRIDGE NO. 61 ON -L- (US 301/I-95 BUS.) OVER CROSS CREEK			GROUND WTR (ft)
BORING NO. B2-B	STATION 26+20	OFFSET 31 ft RT	ALIGNMENT -L-
COLLAR ELEV. 30.4 ft	TOTAL DEPTH 69.5 ft	NORTHING 474,907	EASTING 2,041,225
DRILL RIG/HAMMER EFF./DATE RFO0057 CME-550X 73% 12/06/2005		DRILL METHOD Mud Rotary	HAMMER TYPE Automatic
DRILLER Dixon, D. W.	START DATE 01/31/11	COMP. DATE 02/02/11	SURFACE WATER DEPTH N/A



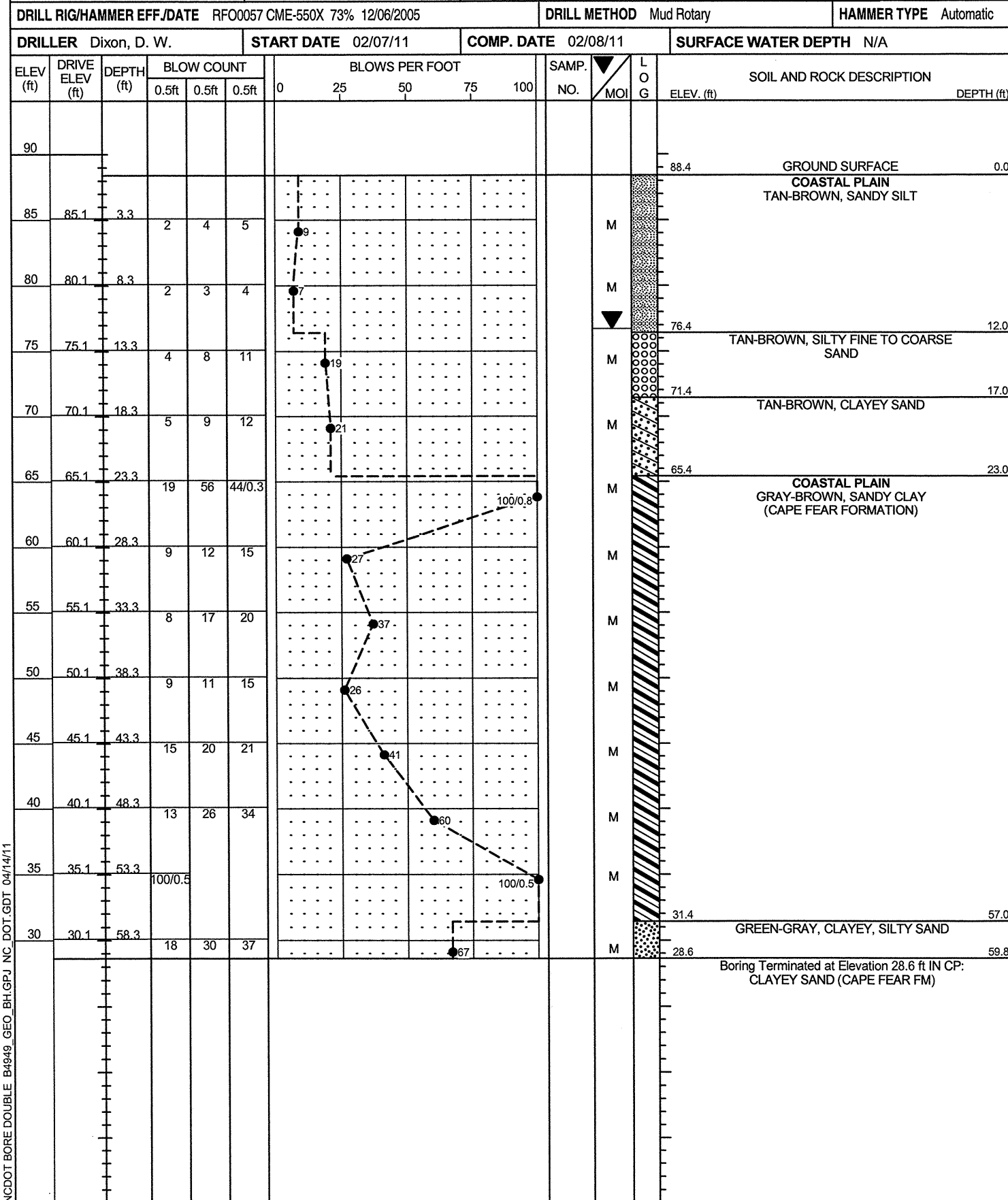
NCDOT BORE DOUBLE B4949 GEO. BH.GPJ NC_DOT.GDT 04/14/11



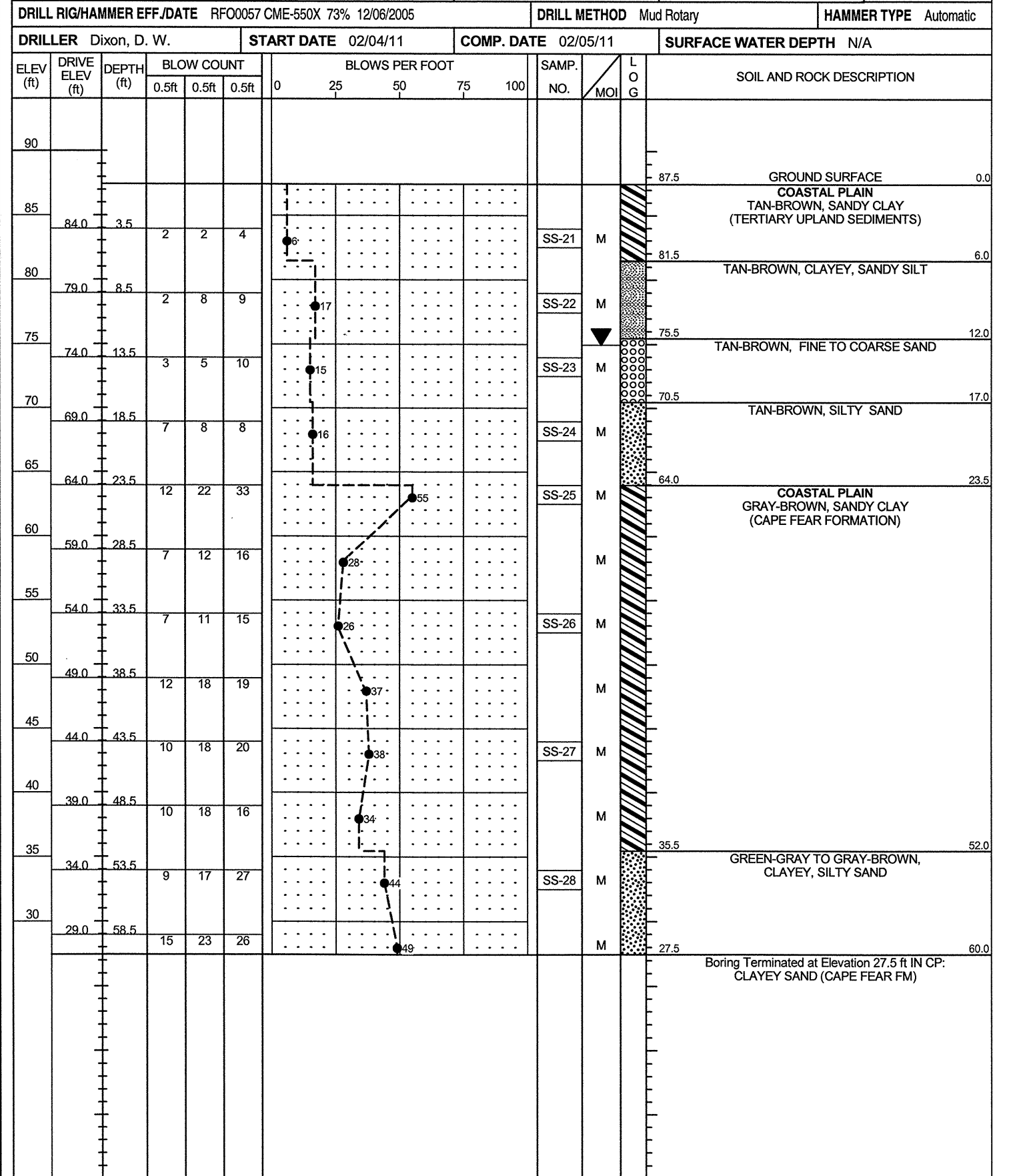
NCDOT GEOTECHNICAL ENGINEERING UNIT

BORELOG REPORT

WBS 40107.1.1	TIP B-4949	COUNTY CUMBERLAND	GEOLOGIST Oti, O. B.
SITE DESCRIPTION BRIDGE NO. 61 ON -L- (US 301/I-95 BUS.) OVER CROSS CREEK			GROUND WTR (ft)
BORING NO. EB2-A	STATION 27+20	OFFSET 50 ft LT	ALIGNMENT -L-
COLLAR ELEV. 88.4 ft	TOTAL DEPTH 59.8 ft	NORTHING 475,036	EASTING 2,041,227
DRILL RIG/HAMMER EFF./DATE RFO0057 CME-550X 73% 12/06/2005		DRILL METHOD Mud Rotary	HAMMER TYPE Automatic
DRILLER Dixon, D. W.	START DATE 02/07/11	COMP. DATE 02/08/11	SURFACE WATER DEPTH N/A



WBS 40107.1.1	TIP B-4949	COUNTY CUMBERLAND	GEOLOGIST Oti, O. B.
SITE DESCRIPTION BRIDGE NO. 61 ON -L- (US 301/I-95 BUS.) OVER CROSS CREEK			GROUND WTR (ft)
BORING NO. EB2-B	STATION 27+20	OFFSET 59 ft RT	ALIGNMENT -L-
COLLAR ELEV. 87.5 ft	TOTAL DEPTH 60.0 ft	NORTHING 474,966	EASTING 2,041,311
DRILL RIG/HAMMER EFF./DATE RFO0057 CME-550X 73% 12/06/2005		DRILL METHOD Mud Rotary	HAMMER TYPE Automatic
DRILLER Dixon, D. W.	START DATE 02/04/11	COMP. DATE 02/05/11	SURFACE WATER DEPTH N/A



NCDOT BORE DOUBLE B4949_GEO_BH.GPJ NC_DOT.GDT 04/14/11

EB1-A

SOIL TEST RESULTS															
SAMPLE NO.	OFFSET	STATION	DEPTH INTERVAL	AASHTO CLASS.	L.L.	P.I.	% BY WEIGHT				% PASSING (SIEVES)			%	%
							C.SAND	F.SAND	SILT	CLAY	10	40	200		
SS-1	50 LT	23+90	0.0-1.0	A-4(1)	25	8	12.5	40.8	22.4	24.2	97	92	53	-	-
SS-2	50 LT	23+90	2.0-3.0	A-4(2)	24	8	10.5	39.4	25.9	24.2	99	93	60	9.2	-
SS-3	50 LT	23+90	4.0-5.0	A-4(5)	27	10	3.2	35.4	35.2	26.3	100	99	73	-	-
SS-4	50 LT	23+90	6.0-7.0	A-6(10)	36	15	2.8	34.3	30.5	32.3	100	99	73	11.6	-
SS-5	50 LT	23+90	10.0-11.0	A-3(0)	19	NP	63.1	28.1	3.7	5.1	93	58	10	-	-
SS-6	50 LT	23+90	14.0-15.0	A-2-7(4)	58	44	53.5	18.1	5.2	23.2	96	64	28	19.7	-
SS-7	50 LT	23+90	16.0-17.0	A-1-b(0)	23	NP	81.4	11.5	2.0	5.1	79	31	6	-	-
SS-8	50 LT	23+90	19.0-20.0	A-2-5(0)	44	NP	62.3	27.3	6.4	4.0	98	64	13	-	-
SS-9	50 LT	23+90	27.5-29.0	A-6(2)	28	14	27.3	40.6	24.0	8.1	100	86	41	20.1	-
SS-10	50 LT	23+90	42.5-44.0	A-2-4(0)	33	NP	63.1	23.4	9.4	4.0	97	56	17	-	-
SS-11	50 LT	23+90	52.5-54.0	A-2-6(1)	33	16	36.9	36.3	19.8	7.1	98	78	33	-	-
SS-12	50 LT	23+90	57.5-59.0	A-7-6(17)	41	22	9.9	14.3	45.5	30.3	100	94	80	21.7	-

B2-B

SOIL TEST RESULTS															
SAMPLE NO.	OFFSET	STATION	DEPTH INTERVAL	AASHTO CLASS.	L.L.	P.I.	% BY WEIGHT				% PASSING (SIEVES)			%	%
							C.SAND	F.SAND	SILT	CLAY	10	40	200		
SS-20	31 RT	26+20	63.0-64.5	A-6(23)	38	24	1.0	5.6	43.3	50.1	100	99	96	-	-

EB2-B

SOIL TEST RESULTS															
SAMPLE NO.	OFFSET	STATION	DEPTH INTERVAL	AASHTO CLASS.	L.L.	P.I.	% BY WEIGHT				% PASSING (SIEVES)			%	%
							C.SAND	F.SAND	SILT	CLAY	10	40	200		
SS-21	59 RT	27+20	3.5-5.0	A-6(5)	28	12	8.0	37.8	30.1	24.0	99	96	62	-	-
SS-22	59 RT	27+20	8.5-20.0	A-4(0)	26	8	6.4	56.1	19.5	18.0	98	97	43	-	-
SS-23	59 RT	27+20	13.5-15.0	A-1-b(0)	20	NP	76.5	17.2	4.3	2.0	91	41	6	-	-
SS-24	59 RT	27+20	18.5-20.0	A-2-7(1)	41	25	66.3	13.6	6.1	14.0	96	58	20	-	-
SS-25	59 RT	27+20	0.0-0.0	A-6(6)	39	16	26.6	29.0	34.3	10.0	99	82	52	-	-
SS-26	59 RT	27+20	33.5-35.0	A-6(1)	30	12	17.8	54.5	23.7	4.0	100	98	36	-	-
SS-27	59 RT	27+20	43.5-45.0	A-6(1)	32	12	22.4	46.2	27.3	4.0	100	90	39	-	-
SS-28	59 RT	27+20	53.5-55.0	A-2-4(0)	35	NP	11.6	70.3	16.1	2.0	100	99	23	-	-

B1-A

SOIL TEST RESULTS															
SAMPLE NO.	OFFSET	STATION	DEPTH INTERVAL	AASHTO CLASS.	L.L.	P.I.	% BY WEIGHT				% PASSING (SIEVES)			%	%
							C.SAND	F.SAND	SILT	CLAY	10	40	200		
SS-31	31 LT	25+05	62.0-63.5	A-2-6(0)	32	14	53.3	29.6	15.1	2.0	100	73	22	-	-
SS-32	31 LT	25+05	22.0-23.6	A-1-b(0)	40	NP	72.1	21.0	4.9	2.0	97	47	9	-	-
SS-33	31 LT	25+05	32.0-33.5	A-6(3)	30	15	14.2	45.4	28.3	12.0	100	96	45	-	-
SS-34	31 LT	25+05	37.3-38.5	A-2-4(0)	25	9	46.6	31.2	18.1	4.0	100	77	26	-	-

B1-B

SOIL TEST RESULTS															
SAMPLE NO.	OFFSET	STATION	DEPTH INTERVAL	AASHTO CLASS.	L.L.	P.I.	% BY WEIGHT				% PASSING (SIEVES)			%	%
							C.SAND	F.SAND	SILT	CLAY	10	40	200		
SS-13	31 RT	25+05	1.0-2.5	A-2-4(0)	37	NP	60.1	29.8	10.1	0.0	98	61	13	-	-
SS-14	31 RT	25+05	11.0-12.5	A-7-6(17)	51	25	12.2	25.2	48.5	14.0	100	93	70	-	-
SS-15	31 RT	25+05	16.0-17.5	A-4(0)	21	6	28.0	38.6	31.3	2.0	100	85	42	-	-
SS-16	31 RT	25+05	21.0-22.5	A-2-6(0)	36	11	29.8	45.8	22.3	2.0	100	87	32	-	-
SS-17	31 RT	25+05	36.0-37.5	A-6(3)	32	14	9.0	52.3	30.7	8.0	100	99	46	-	-
SS-18	31 RT	25+05	46.0-47.5	A-2-4(0)	36	NP	46.6	40.6	12.7	0.0	99	75	17	-	-
SS-19	31 RT	25+05	56.0-57.5	A-6(8)	35	17	19.0	27.6	45.3	8.0	100	89	61	-	-

B2-A

SOIL TEST RESULTS															
SAMPLE NO.	OFFSET	STATION	DEPTH INTERVAL	AASHTO CLASS.	L.L.	P.I.	% BY WEIGHT				% PASSING (SIEVES)			%	%
							C.SAND	F.SAND	SILT	CLAY	10	40	200		
SS-29	31 LT	26+20	45.0-46.5	A-7-6(17)	43	20	2.0	26.4	49.5	22.0	100	99	82	-	-
SS-30	31 LT	26+20	55.0-56.5	A-2-4(0)	33	6	23.6	49.8	22.5	4.0	98	88	31	-	-



**FIELD
 SCOUR REPORT**

WBS: Q TIP: B-4949 COUNTY: CUMBERLAND

DESCRIPTION(1): BR. NO. 61 ON US 301/I-95 BUSINESS OVER CROSS CREEK

EXISTING BRIDGE

Information from: Field Inspection Microfilm _____ (reel _____ pos: _____)
 Other (explain) _____

Bridge No.: 61 Length: 297 Total Bents: 6 Bents in Channel: 2 Bents in Floodplain: 4
 Foundation Type: CONCRETE FOOTING ON PILES

EVIDENCE OF SCOUR(2)

Abutments or End Bent Slopes: GULLIES HAVE FORMED ADJACENT AND UNDERNEATH END BENTS. THESE GULLIES EXTEND FROM THE END BENT FOOTING TO THE CREEK.

Interior Bents: SOME GULLYING EXTENDS NEAR INTERIOR BENTS BUT DO NOT EXPOSE FOOTING

Channel Bed: NO SCOUR EVIDENT. SMALL TRIBUTARY/STREAM ON SOUTHEAST BANK HAS SCoured DOWN CHANNEL TO CROSS CREEK

Channel Bank: BLUFFS ADJACENT TO CREEK HAVE LARGE GULLYING DUE TO RUNOFF. SOME OF THIS MAY ALSO BE FACILITATED BY SCOUR DURING HIGH WATER EVENTS.

EXISTING SCOUR PROTECTION

Type(3): NO SIDE SLOPE PROTECTION OR RIP RAP USED ONSITE

Extent(4): N/A

Effectiveness(5): N/A

Obstructions(6): NONE

INSTRUCTIONS

- 1 Describe the specific site's location, including route number and body of water crossed.
- 2 Note scour evidence at existing end bents or abutments (e.g. undermining, sloughing, degradations).
- 3 Note existing scour protection (e.g. rip rap).
- 4 Describe extent of existing scour protection.
- 5 Describe whether or not the scour protection appears to be working.
- 6 Note obstructions such as dams, fallen trees, debris at bents, etc.
- 7 Describe the channel bed material based on observation and/or samples. Include any lab results with report.
- 8 Describe the channel bank material based on observation and/or samples. Include any lab results with report.
- 9 Describe the material covering the banks (e.g. grass, trees, rip rap, none).
- 10 Determine the approximate floodplain width from field observation or a topographic map.
- 11 Describe the material covering the floodplain (e.g. grass, trees, crops).
- 12 Use professional judgement to specify if the stream is degrading, aggrading, or static.
- 13 Describe potential and direction of the stream to migrate laterally during the bridge's life (approx. 100 years).
- 14 Give the design scour elevation (DSE) expected over the life of the bridge (approx. 100 years). This elevation can be given as a range across the site, or for each bent. Discuss the relationship between the Hydraulics Unit theoretical scour and the DSE. If the DSE is dependent on scour counter measures, explain (e.g. rip rap armoring on slopes). The DSE is based on the erodability of materials, giving consideration to the influence of joints, foliation, bedding characteristics, % core recovery, % RQD, differential weathering, shear strength, observations at existing structures, other tests deemed appropriate, and overall geologic conditions at the site.

DESIGN INFORMATION

Channel Bed Material(7): ALLUVIAL SAND AND GRAVEL OVERLYING HARD/DENSE COASTAL PLAIN SOILS

Channel Bank Material(8): COASTAL PLAIN CLAYEY, SILTY SANDS (CAPE FEAR FORMATION)

Channel Bank Cover(9): GRASS, SHRUBS, SMALL TREES

Floodplain Width(10): 125 FT

Floodplain Cover(11): GRASS, SHRUB, OCCASIONAL TREES. URBAN DEVELOPMENT NEARBY

Stream is(12): Aggrading _____ Degrading Static _____

Channel Migration Tendency(13): SOUTH

Observations and Other Comments: EVIDENCE OF SPRING AT TOP OF BLUFF ON SOUTH EAST SIDE LARGE DRAINAGE FEATURE/GULLY ON NORTHWEST SIDE AS WELL.

DESIGN SCOUR ELEVATIONS(14)

Feet Meters _____

BENTS

B1-A	B1-B	B2-A	B2-B						
30.9	30.9	33.7	30.0						

Comparison of DSE to Hydraulics Unit theoretical scour:
 The Geotechnical Engineering Unit agrees with the Hydraulic Unit's theoretical scour elevations at Interior Bent 1 and B2-A. The DSE has been lowered at B2-B by 3.7 feet.

SOIL ANALYSIS RESULTS FROM CHANNEL BED AND BANK MATERIAL

Bed or Bank									
Sample No.									
Retained #4									
Passed #10									
Passed #40									
Passed #200									
Coarse Sand									
Fine Sand									
Silt									
Clay									
LL									
PI									
AASHTO									
Station									
Offset									
Depth									

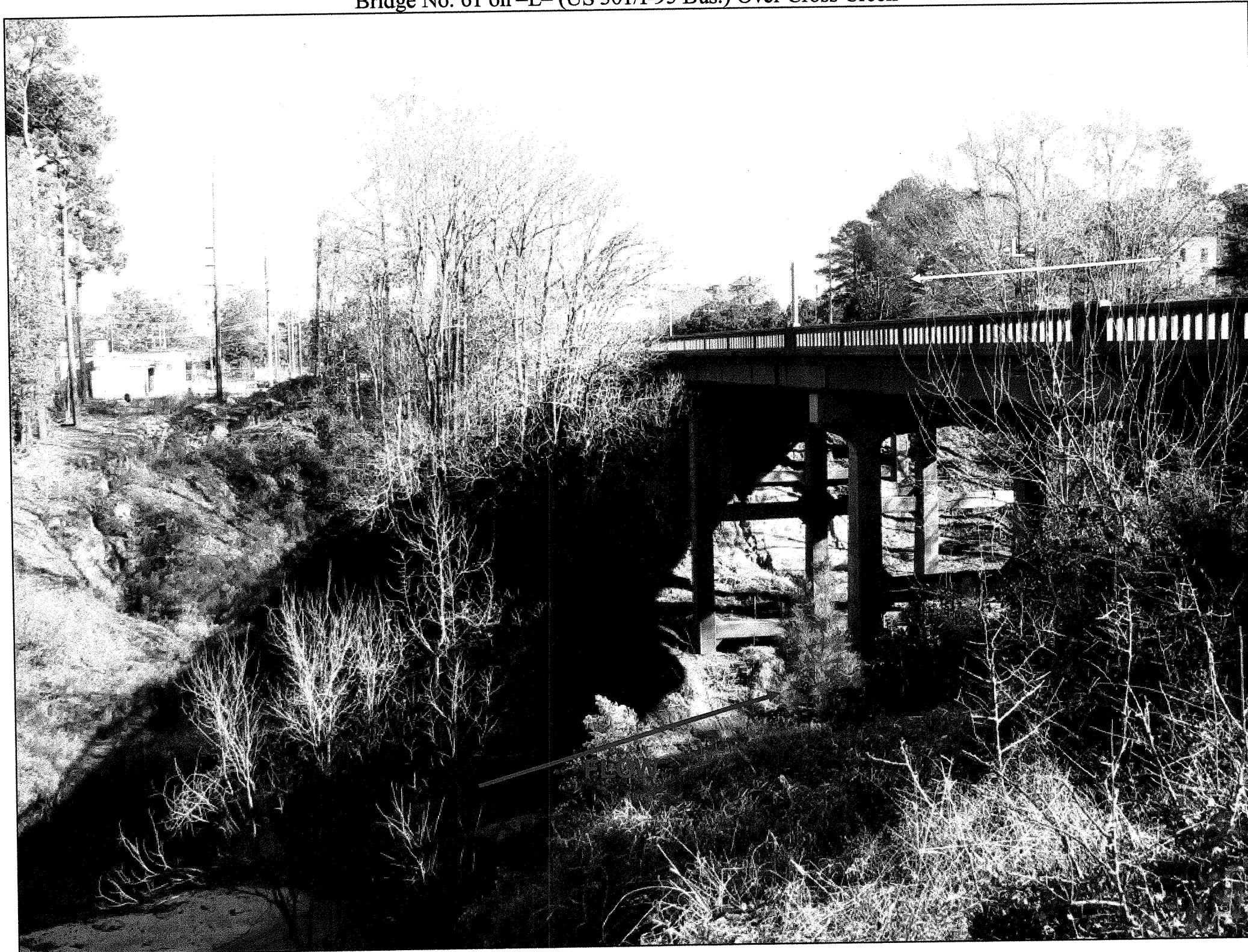
See Sheet 12,
 "Soil Test Results",
 for samples:
 SS-13 (BANK)
 NO ALLUVIAL CHANNEL
 MATERIAL TESTED

Reported by: Christina M. Bruinsma, L.G.

Date: 2/22/2011

SITE PHOTOGRAPH

Bridge No. 61 on -L- (US 301/I-95 Bus.) Over Cross Creek



Looking Northeast towards End Bent 2

STATE OF NORTH CAROLINA
DEPARTMENT OF TRANSPORTATION
DIVISION OF HIGHWAYS
GEOTECHNICAL ENGINEERING UNIT

STRUCTURE
SUBSURFACE INVESTIGATION

CONTENTS

<u>SHEET</u>	<u>DESCRIPTION</u>
1	TITLE SHEET
2	LEGEND
3	SITE PLAN AND PROFILE

PROJ. REFERENCE NO. 40107.1.1 (B-4949) F.A. PROJ. BRNHS-095-2(103)40
COUNTY CUMBERLAND
PROJECT DESCRIPTION REPLACE BR. 61 OVER CROSS CREEK
ON I-95 BUSINESSUS 301

SITE DESCRIPTION RETAINING WALL: -L- STA. 20+00

WALL INVENTORY

CAUTION NOTICE

THE SUBSURFACE INFORMATION AND THE SUBSURFACE INVESTIGATION ON WHICH IT IS BASED WERE MADE FOR THE PURPOSE OF STUDY, PLANNING, AND DESIGN, AND NOT FOR CONSTRUCTION OR PAY PURPOSES. THE VARIOUS FIELD BORING LOGS, ROCK CORES, AND SOIL TEST DATA AVAILABLE MAY BE REVIEWED OR INSPECTED IN RALEIGH BY CONTACTING THE N.C. DEPARTMENT OF TRANSPORTATION, GEOTECHNICAL ENGINEERING UNIT AT (919) 707-6850. NEITHER THE SUBSURFACE PLANS AND REPORTS, NOR THE FIELD BORING LOGS, ROCK CORES, OR SOIL TEST DATA ARE PART OF THE CONTRACT.

GENERAL SOIL AND ROCK STRATA DESCRIPTIONS AND INDICATED BOUNDARIES ARE BASED ON A GEOTECHNICAL INTERPRETATION OF ALL AVAILABLE SUBSURFACE DATA AND MAY NOT NECESSARILY REFLECT THE ACTUAL SUBSURFACE CONDITIONS BETWEEN BORINGS OR BETWEEN SAMPLED STRATA WITHIN THE BOREHOLE. THE LABORATORY SAMPLE DATA AND THE IN SITU (ON-PLACED) TEST DATA CAN BE RELED ON ONLY TO THE DEGREE OF RELIABILITY INHERENT IN THE STANDARD TEST METHOD. THE OBSERVED WATER LEVELS OR SOIL MOISTURE CONDITIONS INDICATED IN THE SUBSURFACE INVESTIGATIONS ARE AS RECORDED AT THE TIME OF THE INVESTIGATION. THESE WATER LEVELS OR SOIL MOISTURE CONDITIONS MAY VARY CONSIDERABLY WITH TIME ACCORDING TO CLIMATIC CONDITIONS INCLUDING TEMPERATURES, PRECIPITATION, AND WIND, AS WELL AS OTHER NON-CLIMATIC FACTORS.

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PROJECT: 40107.1.1 ID: B-4949

PERSONNEL

O. B. OTT

J. I. MILKOVITS

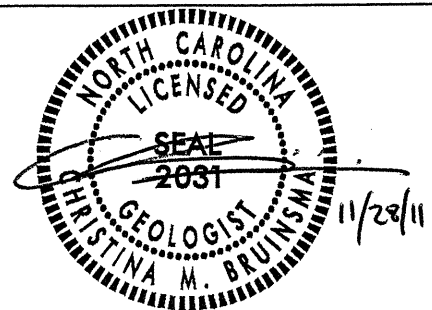
C. M. BRUINSMA

INVESTIGATED BY C.M. BRUINSMA

CHECKED BY N.T. ROBERSON

SUBMITTED BY N. T. ROBERSON

DATE NOVEMBER 2011



DRAWN BY: W. D. FIELDS

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NORTH CAROLINA DEPARTMENT OF TRANSPORTATION
DIVISION OF HIGHWAYS
GEOTECHNICAL ENGINEERING UNIT

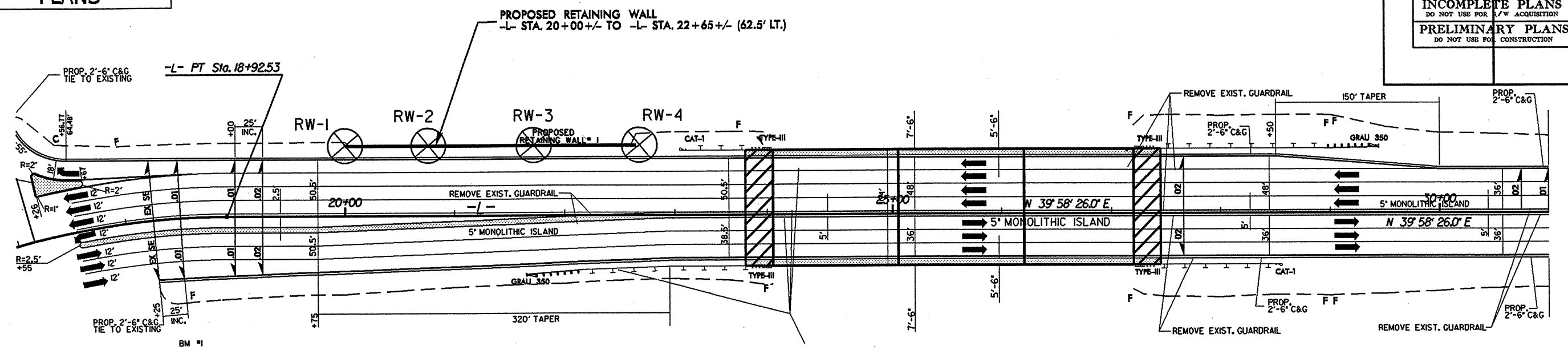
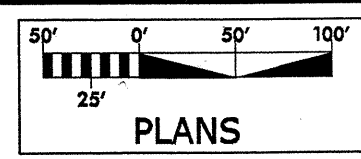
PROJECT REFERENCE NO. B-4949 (40107.I.I)	SHEET NO. 2
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SUBSURFACE INVESTIGATION

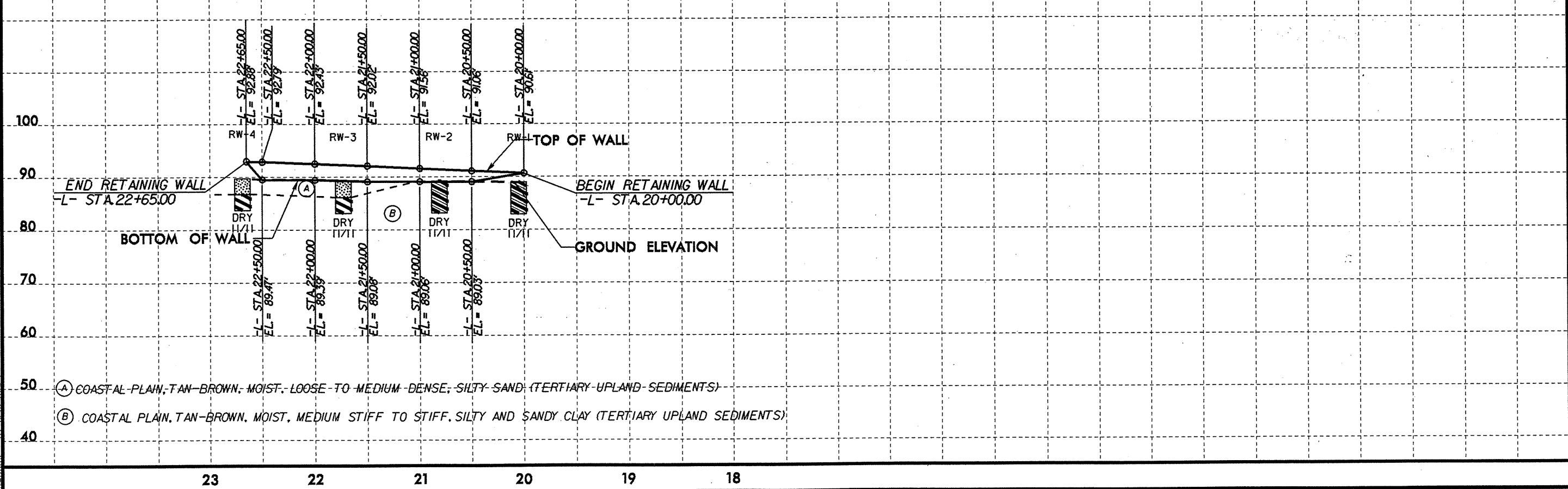
SOIL AND ROCK LEGEND, TERMS, SYMBOLS, AND ABBREVIATIONS

SOIL DESCRIPTION	GRADATION	ROCK DESCRIPTION	TERMS AND DEFINITIONS
SOIL IS CONSIDERED TO BE THE UNCONSOLIDATED, SEMI-CONSOLIDATED, OR WEATHERED EARTH MATERIALS THAT CAN BE PENETRATED WITH A CONTINUOUS FLIGHT POWER AUGER, AND YIELD LESS THAN 100 BLOWS PER FOOT ACCORDING TO STANDARD PENETRATION TEST (AASHTO T206, ASTM D-1586). SOIL CLASSIFICATION IS BASED ON THE AASHTO SYSTEM. BASIC DESCRIPTIONS GENERALLY SHALL INCLUDE: CONSISTENCY, COLOR, TEXTURE, MOISTURE, AASHTO CLASSIFICATION, AND OTHER PERTINENT FACTORS SUCH AS MINERALOGICAL COMPOSITION, ANGULARITY, STRUCTURE, PLASTICITY, ETC. EXAMPLE: <i>VERY STIFF, GRAV. SILTY CLAY, MOST WITH INTERBEDDED FINE SAND LAYERS, HIGH PLASTIC, A-7-6</i>	WELL GRADED - INDICATES A GOOD REPRESENTATION OF PARTICLE SIZES FROM FINE TO COARSE. UNIFORM - INDICATES THAT SOIL PARTICLES ARE ALL APPROXIMATELY THE SAME SIZE. (ALSO POORLY GRADED) GAP-GRADED - INDICATES A MIXTURE OF UNIFORM PARTICLES OF TWO OR MORE SIZES. THE ANGULARITY OR ROUNDNESS OF SOIL GRAINS IS DESIGNATED BY THE TERMS: ANGULAR, SUBANGULAR, SUBROUNDED, OR ROUNDED.	HARD ROCK IS NON-COASTAL PLAIN MATERIAL THAT IF TESTED, WOULD YIELD SPT REFUSAL. AN INFERRED ROCK LINE INDICATES THE LEVEL AT WHICH NON-COASTAL PLAIN MATERIAL WOULD YIELD SPT REFUSAL. SPT REFUSAL IS PENETRATION BY A SPLIT SPOON SAMPLER EQUAL TO OR LESS THAN 6.1 FOOT PER 60 BLOWS. IN NON-COASTAL PLAIN MATERIAL, THE TRANSITION BETWEEN SOIL AND ROCK IS OFTEN REPRESENTED BY A ZONE OF WEATHERED ROCK. ROCK MATERIALS ARE TYPICALLY DIVIDED AS FOLLOWS: WEATHERED ROCK (WR) NON-COASTAL PLAIN MATERIAL THAT WOULD YIELD SPT N VALUES > 100 BLOWS PER FOOT IF TESTED. CRYSTALLINE ROCK (CR) FINE TO COARSE GRAIN IGNEOUS AND METAMORPHIC ROCK THAT WOULD YIELD SPT REFUSAL IF TESTED. ROCK TYPE INCLUDES GRANITE, GNEISS, GABBRO, SCHIST, ETC. NON-CRYSTALLINE ROCK (NCR) FINE TO COARSE GRAIN METAMORPHIC AND NON-COASTAL PLAIN SEDIMENTARY ROCK THAT WOULD YIELD SPT REFUSAL IF TESTED. ROCK TYPE INCLUDES PHYLLITE, SLATE, SANDSTONE, ETC. COASTAL PLAIN SEDIMENTARY ROCK (CP) COASTAL PLAIN SEDIMENTS CEMENTED INTO ROCK, BUT MAY NOT YIELD SPT REFUSAL. ROCK TYPE INCLUDES LIMESTONE, SANDSTONE, CEMENTED SHELL BEDS, ETC.	ALLUVIUM (ALLUV.) - SOILS THAT HAVE BEEN TRANSPORTED BY WATER. AQUIFER - A WATER BEARING FORMATION OR STRATA. ARENACEOUS - APPLIED TO ROCKS THAT HAVE BEEN DERIVED FROM SAND OR THAT CONTAIN SAND. ARGILLACEOUS - APPLIED TO ALL ROCKS OR SUBSTANCES COMPOSED OF CLAY MINERALS, OR HAVING A NOTABLE PROPORTION OF CLAY IN THEIR COMPOSITION, AS SHALE, SLATE, ETC. ARTESIAN - GROUND WATER THAT IS UNDER SUFFICIENT PRESSURE TO RISE ABOVE THE LEVEL AT WHICH IT IS ENCOUNTERED, BUT WHICH DOES NOT NECESSARILY RISE TO OR ABOVE THE GROUND SURFACE. CALCAREOUS (CALC.) - SOILS THAT CONTAIN APPRECIABLE AMOUNTS OF CALCIUM CARBONATE. COLLUVIUM - ROCK FRAGMENTS MIXED WITH SOIL DEPOSITED BY GRAVITY ON SLOPE OR AT BOTTOM OF SLOPE. CORE RECOVERY (REC.) - TOTAL LENGTH OF ALL MATERIAL RECOVERED IN THE CORE BARREL DIVIDED BY TOTAL LENGTH OF CORE RUN AND EXPRESSED AS A PERCENTAGE. DIKE - A TABULAR BODY OF IGNEOUS ROCK THAT CUTS ACROSS THE STRUCTURE OF ADJACENT ROCKS OR CUTS MASSIVE ROCK. DIP - THE ANGLE AT WHICH A STRATUM OR ANY PLANAR FEATURE IS INCLINED FROM THE HORIZONTAL. DIP DIRECTION (DIP AZIMUTH) - THE DIRECTION OR BEARING OF THE HORIZONTAL TRACE OF THE LINE OF DIP, MEASURED CLOCKWISE FROM NORTH. FAULT - A FRACTURE OR FRACTURE ZONE ALONG WHICH THERE HAS BEEN DISPLACEMENT OF THE SIDES RELATIVE TO ONE ANOTHER PARALLEL TO THE FRACTURE. FISSILE - A PROPERTY OF SPLITTING ALONG CLOSELY SPACED PARALLEL PLANES. FLOAT - ROCK FRAGMENTS ON SURFACE NEAR THEIR ORIGINAL POSITION AND DISLODGED FROM PARENT MATERIAL. FLOOD PLAIN (FP) - LAND BORDERING A STREAM, BUILT OF SEDIMENTS DEPOSITED BY THE STREAM. FORMATION (FML) - A MAPPABLE GEOLOGIC UNIT THAT CAN BE RECOGNIZED AND TRACED IN THE FIELD. JOINT - FRACTURE IN ROCK ALONG WHICH NO APPRECIABLE MOVEMENT HAS OCCURRED. LEDGE - A SHELF-LIKE RIDGE OR PROJECTION OF ROCK WHOSE THICKNESS IS SMALL COMPARED TO ITS LATERAL EXTENT. LENS - A BODY OF SOIL OR ROCK THAT THINS OUT IN ONE OR MORE DIRECTIONS. MOTTLED (MOT.) - IRREGULARLY MARKED WITH SPOTS OF DIFFERENT COLORS. MOTTLING IN SOILS USUALLY INDICATES POOR AERATION AND LACK OF GOOD DRAINAGE. PERCHED WATER - WATER MAINTAINED ABOVE THE NORMAL GROUND WATER LEVEL BY THE PRESENCE OF AN INTERVENING IMPERVIOUS STRATUM. RESIDUAL (RES.) SOIL - SOIL FORMED IN PLACE BY THE WEATHERING OF ROCK. ROCK QUALITY DESIGNATION (RQD) - A MEASURE OF ROCK QUALITY DESCRIBED BY TOTAL LENGTH OF ROCK SEGMENTS EQUAL TO OR GREATER THAN 4 INCHES DIVIDED BY THE TOTAL LENGTH OF CORE RUN AND EXPRESSED AS A PERCENTAGE. SAPROLITE (SAP.) - RESIDUAL SOIL THAT RETAINS THE RELIC STRUCTURE OR FABRIC OF THE PARENT ROCK. SILL - AN INTRUSIVE BODY OF IGNEOUS ROCK OF APPROXIMATELY UNIFORM THICKNESS AND RELATIVELY THIN COMPARED WITH ITS LATERAL EXTENT, THAT HAS BEEN EMPLACED PARALLEL TO THE BEDDING OR SCHISTOSITY OF THE INTRUDED ROCKS. SLICKENSIDE - POLISHED AND STRATED SURFACE THAT RESULTS FROM FRICTION ALONG A FAULT OR SLIP PLANE. STANDARD PENETRATION TEST (PENETRATION RESISTANCE) (SPT) - NUMBER OF BLOWS (N OR BPF) OF A 140 LB. HAMMER FALLING 30 INCHES REQUIRED TO PRODUCE A PENETRATION OF 1 FOOT INTO SOIL WITH A 2 INCH OUTSIDE DIAMETER SPLIT SPOON SAMPLER. SPT REFUSAL IS PENETRATION EQUAL TO OR LESS THAN 6.1 FOOT PER 60 BLOWS. STRATA CORE RECOVERY (SREC.) - TOTAL LENGTH OF STRATA MATERIAL RECOVERED DIVIDED BY TOTAL LENGTH OF STRATUM AND EXPRESSED AS A PERCENTAGE. STRATA ROCK QUALITY DESIGNATION (SRQD) - A MEASURE OF ROCK QUALITY DESCRIBED BY TOTAL LENGTH OF ROCK SEGMENTS WITHIN A STRATUM EQUAL TO OR GREATER THAN 4 INCHES DIVIDED BY THE TOTAL LENGTH OF STRATA AND EXPRESSED AS A PERCENTAGE. TOPSOIL (TS.) - SURFACE SOILS USUALLY CONTAINING ORGANIC MATTER.
SOIL LEGEND AND AASHTO CLASSIFICATION	MINERALOGICAL COMPOSITION	WEATHERING	
GENERAL CLASS. GRANULAR MATERIALS (<= 35% PASSING #200) SILT-CLAY MATERIALS (> 35% PASSING #200) ORGANIC MATERIALS	MINERAL NAMES SUCH AS QUARTZ, FELDSPAR, MICA, TALC, KAOLIN, ETC. ARE USED IN DESCRIPTIONS WHENEVER THEY ARE CONSIDERED OF SIGNIFICANCE.	FRESH ROCK FRESH, CRYSTALS BRIGHT, FEW JOINTS MAY SHOW SLIGHT STAINING, ROCK RINGS UNDER HAMMER IF CRYSTALLINE. VERY SLIGHT (V SL.) ROCK GENERALLY FRESH, JOINTS STAINED, SOME JOINTS MAY SHOW THIN CLAY COATINGS IF OPEN, CRYSTALS ON A BROKEN SPECIMEN FACE SHINE BRIGHTLY, ROCK RINGS UNDER HAMMER BLOWS IF OF A CRYSTALLINE NATURE. SLIGHT (SL.) ROCK GENERALLY FRESH, JOINTS STAINED AND DISCOLORATION EXTENDS INTO ROCK UP TO 1 INCH. OPEN JOINTS MAY CONTAIN CLAY. IN GRANITOID ROCKS SOME OCCASIONAL FELDSPAR CRYSTALS ARE DULL AND DISCOLORED. CRYSTALLINE ROCKS RING UNDER HAMMER BLOWS. MODERATE (MOD.) SIGNIFICANT PORTIONS OF ROCK SHOW DISCOLORATION AND WEATHERING EFFECTS. IN GRANITOID ROCKS, MOST FELDSPARS ARE DULL AND DISCOLORED, SOME SHOW CLAY. ROCK HAS DULL SOUND UNDER HAMMER BLOWS AND SHOWS SIGNIFICANT LOSS OF STRENGTH AS COMPARED WITH FRESH ROCK. MODERATELY SEVERE (MOD. SEV.) ALL ROCK EXCEPT QUARTZ DISCOLORED OR STAINED. IN GRANITOID ROCKS, ALL FELDSPARS DULL AND DISCOLORED AND A MAJORITY SHOW KAOLINIZATION. ROCK SHOWS SEVERE LOSS OF STRENGTH AND CAN BE EXCAVATED WITH A GEOLOGIST'S PICK. ROCK GIVES "CLUNK" SOUND WHEN STRUCK. <i>IF TESTED, WOULD YIELD SPT REFUSAL</i> SEVERE (SEV.) ALL ROCK EXCEPT QUARTZ DISCOLORED OR STAINED. ROCK FABRIC CLEAR AND EVIDENT BUT REDUCED IN STRENGTH TO STRONG SOIL. IN GRANITOID ROCKS ALL FELDSPARS ARE KAOLINIZED TO SOME EXTENT. SOME FRAGMENTS OF STRONG ROCK USUALLY REMAIN. <i>IF TESTED, YIELDS SPT N VALUES > 100 BPF</i> VERY SEVERE (V SEV.) ALL ROCK EXCEPT QUARTZ DISCOLORED OR STAINED. ROCK FABRIC ELEMENTS ARE DISCERNIBLE BUT THE MASS IS EFFECTIVELY REDUCED TO SOIL STATUS, WITH ONLY FRAGMENTS OF STRONG ROCK REMAINING. SAPROLITE IS AN EXAMPLE OF ROCK WEATHERED TO A DEGREE SUCH THAT ONLY MINOR VESTIGES OF THE ORIGINAL ROCK FABRIC REMAIN. <i>IF TESTED, YIELDS SPT N VALUES < 100 BPF</i> COMPLETE ROCK REDUCED TO SOIL. ROCK FABRIC NOT DISCERNIBLE, OR DISCERNIBLE ONLY IN SMALL AND SCATTERED CONCENTRATIONS. QUARTZ MAY BE PRESENT AS DIKES OR STRINGERS. SAPROLITE IS ALSO AN EXAMPLE.	
COMPRESSIONIBILITY	PERCENTAGE OF MATERIAL	GROUND WATER	
SLIGHTLY COMPRESSIBLE LIQUID LIMIT LESS THAN 31 MODERATELY COMPRESSIBLE LIQUID LIMIT EQUAL TO 31-50 HIGHLY COMPRESSIBLE LIQUID LIMIT GREATER THAN 50	ORGANIC MATERIAL GRANULAR SOILS SILT-CLAY SOILS OTHER MATERIAL TRACE OF ORGANIC MATTER 2 - 3% 3 - 5% TRACE 1 - 10% LITTLE ORGANIC MATTER 3 - 5% 5 - 12% LITTLE 10 - 20% MODERATELY ORGANIC 5 - 10% 12 - 20% SOME 20 - 35% HIGHLY ORGANIC >10% >20% HIGHLY 35% AND ABOVE	WATER LEVEL IN BORE HOLE IMMEDIATELY AFTER DRILLING STATIC WATER LEVEL AFTER 24 HOURS PERCHED WATER, SATURATED ZONE, OR WATER BEARING STRATA SPRING OR SEEP	
CONSISTENCY OR DENSENESS	MISCELLANEOUS SYMBOLS	ROCK HARDNESS	
PRIMARY SOIL TYPE COMPACTNESS OR CONSISTENCY RANGE OF STANDARD PENETRATION RESISTANCE (N-VALUE) RANGE OF UNCONFINED COMPRESSIVE STRENGTH (TONS/F ²)	ROADWAY EMBANKMENT (RE) WITH SOIL DESCRIPTION SOIL SYMBOL ARTIFICIAL FILL (AF) OTHER THAN ROADWAY EMBANKMENT INFERRED SOIL BOUNDARY INFERRED ROCK LINE ALLUVIAL SOIL BOUNDARY DIP & DIP DIRECTION OF ROCK STRUCTURES	VERY HARD CANNOT BE SCRATCHED BY KNIFE OR SHARP PICK. BREAKING OF HAND SPECIMENS REQUIRES SEVERAL HARD BLOWS OF THE GEOLOGIST'S PICK. HARD CAN BE SCRATCHED BY KNIFE OR PICK ONLY WITH DIFFICULTY. HARD HAMMER BLOWS REQUIRED TO DETACH HAND SPECIMEN. MODERATELY HARD CAN BE SCRATCHED BY KNIFE OR PICK, GOUGES OR GROOVES TO 0.25 INCHES DEEP CAN BE EXCAVATED BY HARD BLOW OF A GEOLOGIST'S PICK. HAND SPECIMENS CAN BE DETACHED BY MODERATE BLOWS. MEDIUM HARD CAN BE GROOVED OR GOUGED 0.05 INCHES DEEP BY FIRM PRESSURE OF KNIFE OR PICK POINT. CAN BE EXCAVATED IN SMALL CHIPS TO PIECES 1 INCH MAXIMUM SIZE BY HARD BLOWS OF THE POINT OF A GEOLOGIST'S PICK. SOFT CAN BE GROOVED OR GOUGED READILY BY KNIFE OR PICK. CAN BE EXCAVATED IN FRAGMENTS FROM CHIPS TO SEVERAL INCHES IN SIZE BY MODERATE BLOWS OF A PICK POINT. SMALL, THIN PIECES CAN BE BROKEN BY FINGER PRESSURE. VERY SOFT CAN BE CARVED WITH KNIFE. CAN BE EXCAVATED READILY WITH POINT OF PICK. PIECES 1 INCH OR MORE IN THICKNESS CAN BE BROKEN BY FINGER PRESSURE. CAN BE SCRATCHED READILY BY FINGER NAIL.	
TEXTURE OR GRAIN SIZE	ABBREVIATIONS	FRACTURE SPACING	BEDDING
U.S. STD. SIEVE SIZE OPENING (MM) 4 10 40 60 200 270 4.76 2.00 0.42 0.25 0.075 0.053	AR - AUGER REFUSAL MED. - MEDIUM MICA - MICACEOUS BT - BORING TERMINATED MOD. - MODERATELY CL - CLAY NP - NON PLASTIC CPT - CONE PENETRATION TEST ORG. - ORGANIC CSE - COARSE PMT - PRESSUREMETER TEST DMT - DILATOMETER TEST DPT - DYNAMIC PENETRATION TEST e - VOID RATIO F - FINE FOSS - FOSSILIFEROUS FRAC. - FRACTURED, FRACTURES FRAGS. - FRAGMENTS HI. - HIGHLY VST - VANE SHEAR TEST WEA. - WEATHERED W - UNIT WEIGHT W _d - DRY UNIT WEIGHT SAMPLE ABBREVIATIONS S - BULK SS - SPLIT SPOON ST - SHELBY TUBE RS - ROCK RT - RECOMPACTED TRIAXIAL CB - CALIFORNIA BEARING RATIO	VERY WIDE MORE THAN 10 FEET WIDE 3 TO 10 FEET MODERATELY CLOSE 1 TO 3 FEET CLOSE 0.16 TO 1 FEET VERY CLOSE LESS THAN 0.16 FEET	VERY THICKLY BEDDED > 4 FEET THICKLY BEDDED 1.5 - 4 FEET THINLY BEDDED 0.15 - 1.5 FEET VERY THINLY BEDDED 0.03 - 0.16 FEET THICKLY LAMINATED 0.008 - 0.03 FEET THINLY LAMINATED < 0.008 FEET
SOIL MOISTURE - CORRELATION OF TERMS	EQUIPMENT USED ON SUBJECT PROJECT	INDURATION	NOTES:
SOIL MOISTURE SCALE (ATTERBERG LIMITS) FIELD MOISTURE DESCRIPTION GUIDE FOR FIELD MOISTURE DESCRIPTION LL - LIQUID LIMIT - SATURATED - (SAT.) USUALLY LIQUID; VERY WET, USUALLY FROM BELOW THE GROUND WATER TABLE PL - PLASTIC LIMIT - WET - (W) SEMISOLID; REQUIRES DRYING TO ATTAIN OPTIMUM MOISTURE OM - OPTIMUM MOISTURE - MOIST - (M) SOLID; AT OR NEAR OPTIMUM MOISTURE SL - SHRINKAGE LIMIT - DRY - (D) REQUIRES ADDITIONAL WATER TO ATTAIN OPTIMUM MOISTURE	DRILL UNITS: MOBILE B- BK-51 CME-45C CME-550 PORTABLE HOIST ADVANCING TOOLS: CLAY BITS 6" CONTINUOUS FLIGHT AUGER 8" HOLLOW AUGERS HARD FACED FINGER BITS TUNG-CARBIDE INSERTS CASING w/ ADVANCER TRICONE * STEEL TEETH TRICONE * TUNG-CARB. CORE BIT HAMMER TYPE: AUTOMATIC MANUAL CORE SIZE: B N H HAND TOOLS: POST HOLE DIGGER HAND AUGER SOUNDING ROD VANE SHEAR TEST	FOR SEDIMENTARY ROCKS, INDURATION IS THE HARDENING OF THE MATERIAL BY CEMENTING, HEAT, PRESSURE, ETC. FRIABLE RUBBING WITH FINGER FREES NUMEROUS GRAINS; GENTLE BLOW BY HAMMER DISINTEGRATES SAMPLE. MODERATELY INDURATED GRAINS CAN BE SEPARATED FROM SAMPLE WITH STEEL PROBE; BREAKS EASILY WHEN HIT WITH HAMMER. INDURATED GRAINS ARE DIFFICULT TO SEPARATE WITH STEEL PROBE; DIFFICULT TO BREAK WITH HAMMER. EXTREMELY INDURATED SHARP HAMMER BLOWS REQUIRED TO BREAK SAMPLE; SAMPLE BREAKS ACROSS GRAINS.	BENCH MARK: ELEVATION: FT. NOTES:
PLASTICITY			
NONPLASTIC LOW PLASTICITY MED. PLASTICITY HIGH PLASTICITY PLASTICITY INDEX (PI) DRY STRENGTH VERY LOW SLIGHT MEDIUM HIGH			
COLOR			
DESCRIPTIONS MAY INCLUDE COLOR OR COLOR COMBINATIONS (TAN, RED, YELLOW-BROWN, BLUE-GRAY). MODIFIERS SUCH AS LIGHT, DARK, STREAKED, ETC. ARE USED TO DESCRIBE APPEARANCE.			

PLAN VIEW OF RETAINING WALL AND WALL ENVELOPE



WALL 1



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STATE OF NORTH CAROLINA
DEPARTMENT OF TRANSPORTATION
DIVISION OF HIGHWAYS
GEOTECHNICAL ENGINEERING UNIT

STRUCTURE
SUBSURFACE INVESTIGATION

PROJ. REFERENCE NO. 33449.1.1(B-4091) F.A. PROJ. BRSTP-301(12)
 COUNTY CUMBERLAND
 PROJECT DESCRIPTION BRIDGE NO. 85 ON -L- (US 301/95 BUS. LOOP)
OVER -Y1- (SR 1738), -Y2- (SR 1741) AND CAPE FEAR RIVER

SITE DESCRIPTION _____

CONTENTS

<u>SHEET</u>	<u>DESCRIPTION</u>
1	TITLE SHEET
2	LEGEND
3	SITE PLAN
4	PROFILE(S)
5-12	CROSS SECTION(S)
13-27	BORE LOG & CORE REPORT(S)
28-30	SOIL AND ROCK TEST RESULTS
31	SCOUR REPORT
32-33	CORE PHOTOGRAPH(S)
34	SITE PHOTOGRAPH(S)

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PERSONNEL
C.M. BRUINSMA

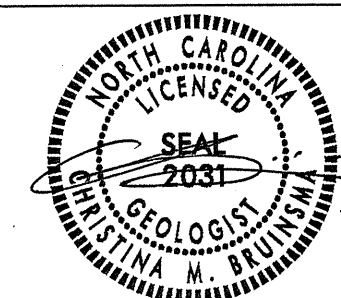
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INVESTIGATED BY **C.M. BRUINSMA**

CHECKED BY **N.T. ROBERSON**

SUBMITTED BY **N.T. ROBERSON**

DATE **AUGUST 2011**



PROJECT: 33449.1.1 ID: B-4091

DRAWN BY: **T.T. WALKER, D.W. FIELDS, C.M. BRUINSMA**

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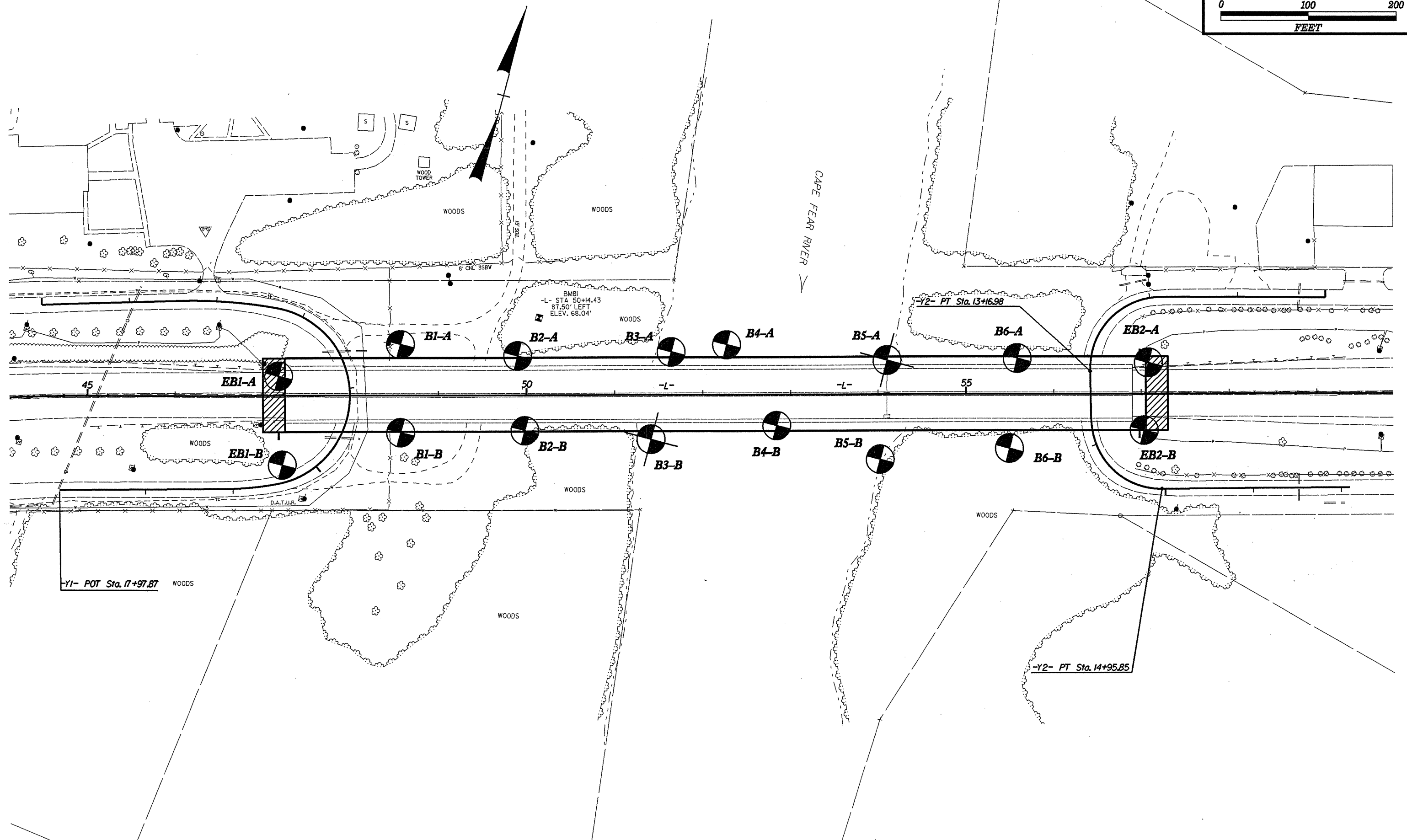
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SUBSURFACE INVESTIGATION

SOIL AND ROCK LEGEND, TERMS, SYMBOLS, AND ABBREVIATIONS

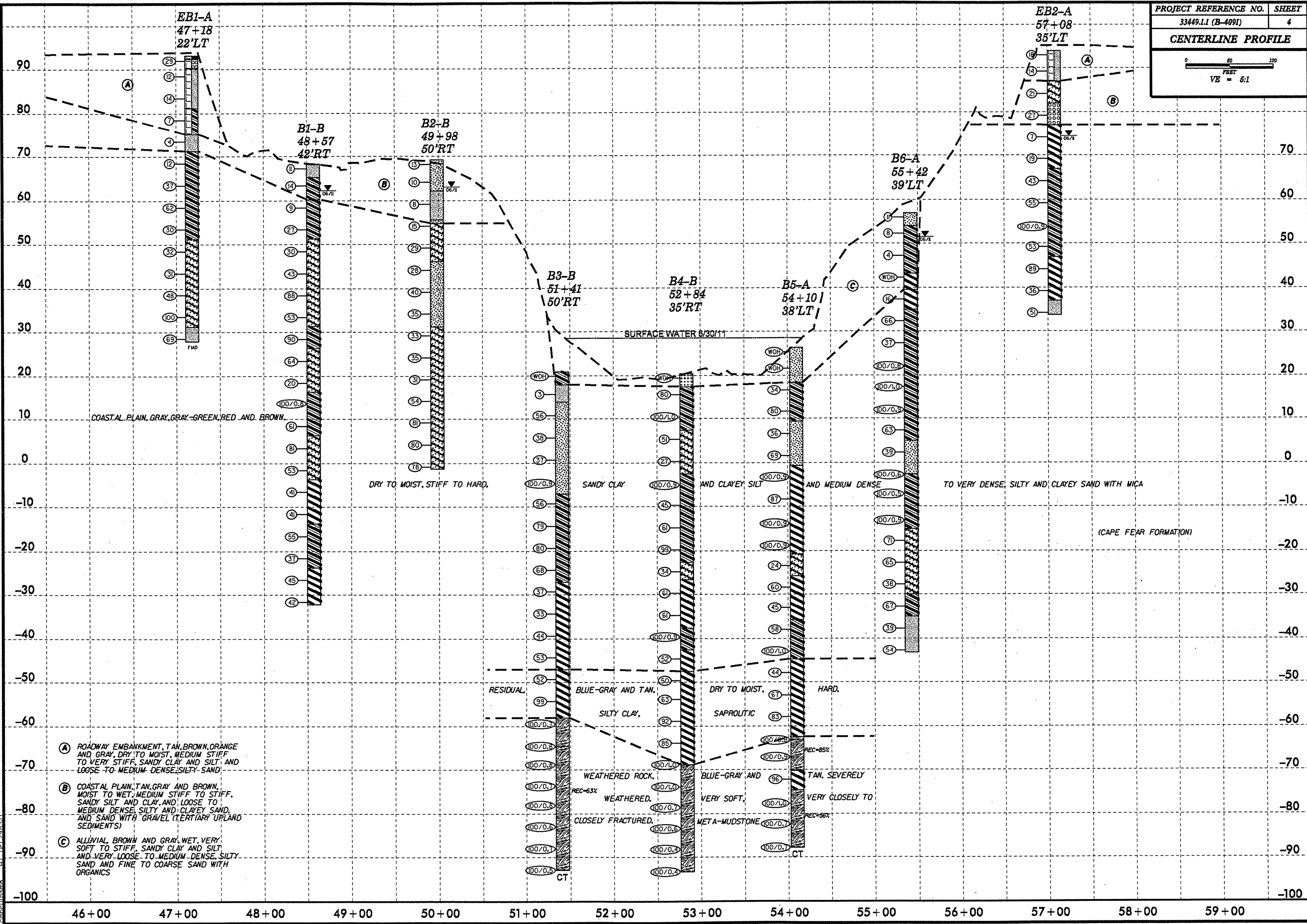
SOIL DESCRIPTION		GRADATION		ROCK DESCRIPTION		TERMS AND DEFINITIONS																																																							
<p>SOIL IS CONSIDERED TO BE THE UNCONSOLIDATED, SEMI-CONSOLIDATED, OR WEATHERED EARTH MATERIALS THAT CAN BE PENETRATED WITH A CONTINUOUS FLIGHT POWER AUGER, AND YIELD LESS THAN 100 BLOWS PER FOOT ACCORDING TO STANDARD PENETRATION TEST (ASTM D-1586). SOIL CLASSIFICATION IS BASED ON THE AASHTO SYSTEM. BASIC DESCRIPTIONS GENERALLY SHALL INCLUDE: CONSISTENCY, COLOR, TEXTURE, MOISTURE, AASHTO CLASSIFICATION, AND OTHER PERTINENT FACTORS SUCH AS MINERALOGICAL COMPOSITION, ANGULARITY, STRUCTURE, PLASTICITY, ETC. EXAMPLE:</p> <p style="text-align: center;"><i>VERY STIFF, GRN. SILTY CLAY, MOST WITH INTERBEDDED FINE SAND LAYERS, HIGH PLASTIC, A-7-6</i></p>		<p>WELL GRADED - INDICATES A GOOD REPRESENTATION OF PARTICLE SIZES FROM FINE TO COARSE. UNIFORM - INDICATES THAT SOIL PARTICLES ARE ALL APPROXIMATELY THE SAME SIZE. (ALSO POORLY GRADED)</p> <p>POORLY GRADED - INDICATES A MIXTURE OF UNIFORM PARTICLES OF TWO OR MORE SIZES.</p> <p style="text-align: center;">ANGULARITY OF GRAINS</p> <p>THE ANGULARITY OR ROUNDNESS OF SOIL GRAINS IS DESIGNATED BY THE TERMS: <u>ANGULAR</u>, <u>SUBANGULAR</u>, <u>SUBROUNDED</u>, OR <u>ROUNDED</u>.</p>		<p>HARD ROCK IS NON-COASTAL PLAIN MATERIAL THAT IF TESTED, WOULD YIELD SPT REFUSAL, AN INFERRED ROCK LINE INDICATES THE LEVEL AT WHICH NON-COASTAL PLAIN MATERIAL WOULD YIELD SPT REFUSAL. SPT REFUSAL IS PENETRATION BY A SPLIT SPOON SAMPLER EQUAL TO OR LESS THAN 0.1 FOOT PER 60 BLOWS. IN NON-COASTAL PLAIN MATERIAL, THE TRANSITION BETWEEN SOIL AND ROCK IS OFTEN REPRESENTED BY A ZONE OF WEATHERED ROCK. ROCK MATERIALS ARE TYPICALLY DIVIDED AS FOLLOWS:</p> <p>WEATHERED ROCK (WR) NON-COASTAL PLAIN MATERIAL THAT WOULD YIELD SPT N VALUES > 100 BLOWS PER FOOT IF TESTED.</p> <p>CRYSTALLINE ROCK (CR) FINE TO COARSE GRAIN IGNEOUS AND METAMORPHIC ROCK THAT WOULD YIELD SPT REFUSAL IF TESTED. ROCK TYPE INCLUDES GRANITE, GNEISS, GABBRO, SCHIST, ETC.</p> <p>NON-CRYSTALLINE ROCK (NCR) FINE TO COARSE GRAIN METAMORPHIC AND NON-COASTAL PLAIN SEDIMENTARY ROCK THAT WOULD YIELD SPT REFUSAL IF TESTED. ROCK TYPE INCLUDES PHYLLITE, SLATE, SANDSTONE, ETC.</p> <p>COASTAL PLAIN SEDIMENTARY ROCK (CP) COASTAL PLAIN SEDIMENTS CEMENTED INTO ROCK, BUT MAY NOT YIELD SPT REFUSAL. ROCK TYPE INCLUDES LIMESTONE, SANDSTONE, CEMENTED SHELL BEDS, ETC.</p>		<p>ALLUVIUM (ALLUV.) - SOILS THAT HAVE BEEN TRANSPORTED BY WATER.</p> <p>AQUIFER - A WATER BEARING FORMATION OR STRATA.</p> <p>ARENACEOUS - APPLIED TO ROCKS THAT HAVE BEEN DERIVED FROM SAND OR THAT CONTAIN SAND.</p> <p>ARGILLACEOUS - APPLIED TO ALL ROCKS OR SUBSTANCES COMPOSED OF CLAY MINERALS, OR HAVING A NOTABLE PROPORTION OF CLAY IN THEIR COMPOSITION, AS SHALE, SLATE, ETC.</p> <p>ARTESIAN - GROUND WATER THAT IS UNDER SUFFICIENT PRESSURE TO RISE ABOVE THE LEVEL AT WHICH IT IS ENCOUNTERED, BUT WHICH DOES NOT NECESSARILY RISE TO OR ABOVE THE GROUND SURFACE.</p> <p>CALCAREOUS (CALC.) - SOILS THAT CONTAIN APPRECIABLE AMOUNTS OF CALCIUM CARBONATE.</p> <p>COLLUVIUM - ROCK FRAGMENTS MIXED WITH SOIL DEPOSITED BY GRAVITY ON SLOPE OR AT BOTTOM OF SLOPE.</p> <p>CORE RECOVERY (REC.) - TOTAL LENGTH OF ALL MATERIAL RECOVERED IN THE CORE BARREL DIVIDED BY TOTAL LENGTH OF CORE RUN AND EXPRESSED AS A PERCENTAGE.</p> <p>DIKE - A TABULAR BODY OF IGNEOUS ROCK THAT CUTS ACROSS THE STRUCTURE OF ADJACENT ROCKS OR CUTS MASSIVE ROCK.</p> <p>DIP - THE ANGLE AT WHICH A STRATUM OR ANY PLANAR FEATURE IS INCLINED FROM THE HORIZONTAL.</p> <p>DIP DIRECTION (DIP AZIMUTH) - THE DIRECTION OR BEARING OF THE HORIZONTAL TRACE OF THE LINE OF DIP, MEASURED CLOCKWISE FROM NORTH.</p> <p>FAULT - A FRACTURE OR FRACTURE ZONE ALONG WHICH THERE HAS BEEN DISPLACEMENT OF THE SIDES RELATIVE TO ONE ANOTHER PARALLEL TO THE FRACTURE.</p> <p>FISSILE - A PROPERTY OF SPLITTING ALONG CLOSELY SPACED PARALLEL PLANES.</p> <p>FLOAT - ROCK FRAGMENTS ON SURFACE NEAR THEIR ORIGINAL POSITION AND DISLODGED FROM PARENT MATERIAL.</p> <p>FLOOD PLAIN (FP) - LAND BORDERING A STREAM, BUILT OF SEDIMENTS DEPOSITED BY THE STREAM.</p> <p>FORMATION (FM.) - A MAPPABLE GEOLOGIC UNIT THAT CAN BE RECOGNIZED AND TRACED IN THE FIELD.</p> <p>JOINT - FRACTURE IN ROCK ALONG WHICH NO APPRECIABLE MOVEMENT HAS OCCURRED.</p> <p>LEDGE - A SHELF-LIKE RIDGE OR PROJECTION OF ROCK WHOSE THICKNESS IS SMALL COMPARED TO ITS LATERAL EXTENT.</p> <p>LENS - A BODY OF SOIL OR ROCK THAT THINS OUT IN ONE OR MORE DIRECTIONS.</p> <p>MOTTLED (MOT.) - IRREGULARLY MARKED WITH SPOTS OF DIFFERENT COLORS. MOTTLING IN SOILS USUALLY INDICATES POOR AERATION AND LACK OF GOOD DRAINAGE.</p> <p>PERCHED WATER - WATER MAINTAINED ABOVE THE NORMAL GROUND WATER LEVEL BY THE PRESENCE OF AN INTERVENING IMPERVIOUS STRATUM.</p> <p>RESIDUAL (RES.) SOIL - SOIL FORMED IN PLACE BY THE WEATHERING OF ROCK.</p> <p>ROCK QUALITY DESIGNATION (RQD) - A MEASURE OF ROCK QUALITY DESCRIBED BY TOTAL LENGTH OF ROCK SEGMENTS EQUAL TO OR GREATER THAN 4 INCHES DIVIDED BY THE TOTAL LENGTH OF CORE RUN AND EXPRESSED AS A PERCENTAGE.</p> <p>SAPROLITE (SAP.) - RESIDUAL SOIL THAT RETAINS THE RELIC STRUCTURE OR FABRIC OF THE PARENT ROCK.</p> <p>SILL - AN INTRUSIVE BODY OF IGNEOUS ROCK OF APPROXIMATELY UNIFORM THICKNESS AND RELATIVELY THIN COMPARED WITH ITS LATERAL EXTENT, THAT HAS BEEN EMPLACED PARALLEL TO THE BEDDING OR SCHISTOSITY OF THE INTRUDED ROCKS.</p> <p>SLICKENSIDE - POLISHED AND STRIATED SURFACE THAT RESULTS FROM FRICTION ALONG A FAULT OR SLIP PLANE.</p> <p>STANDARD PENETRATION TEST (PENETRATION RESISTANCE) (SPT) - NUMBER OF BLOWS (N OR BPF) OF A 140 LB. HAMMER FALLING 30 INCHES REQUIRED TO PRODUCE A PENETRATION OF 1 FOOT INTO SOIL WITH A 2 INCH OUTSIDE DIAMETER SPLIT SPOON SAMPLER. SPT REFUSAL IS PENETRATION EQUAL TO OR LESS THAN 0.1 FOOT PER 60 BLOWS.</p> <p>STRATA CORE RECOVERY (SREC.) - TOTAL LENGTH OF STRATA MATERIAL RECOVERED DIVIDED BY TOTAL LENGTH OF STRATUM AND EXPRESSED AS A PERCENTAGE.</p> <p>STRATA ROCK QUALITY DESIGNATION (SRQD) - A MEASURE OF ROCK QUALITY DESCRIBED BY TOTAL LENGTH OF ROCK SEGMENTS WITHIN A STRATUM EQUAL TO OR GREATER THAN 4 INCHES DIVIDED BY THE TOTAL LENGTH OF STRATA AND EXPRESSED AS A PERCENTAGE.</p> <p>TOPSOIL (TS.) - SURFACE SOILS USUALLY CONTAINING ORGANIC MATTER.</p>																																																							
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ARE USED IN DESCRIPTIONS WHENEVER THEY ARE CONSIDERED OF SIGNIFICANCE.</p> <p style="text-align: center;">COMPRESSIBILITY</p> <p>SLIGHTLY COMPRESSIBLE LIQUID LIMIT LESS THAN 31 MODERATELY COMPRESSIBLE LIQUID LIMIT EQUAL TO 31-50 HIGHLY COMPRESSIBLE LIQUID LIMIT GREATER THAN 50</p> <p style="text-align: center;">PERCENTAGE OF MATERIAL</p> <table border="1" style="width: 100%; border-collapse: collapse;"> <thead> <tr> <th>ORGANIC MATERIAL</th> <th>GRANULAR SOILS</th> <th>SILT - CLAY SOILS</th> <th>OTHER MATERIAL</th> </tr> </thead> <tbody> <tr> <td>TRACE OF ORGANIC MATTER</td> <td>2 - 3%</td> <td>3 - 5%</td> <td>TRACE</td> </tr> <tr> <td>LITTLE ORGANIC MATTER</td> <td>3 - 5%</td> <td>5 - 12%</td> <td>LITTLE</td> </tr> <tr> <td>MODERATELY ORGANIC</td> <td>5 - 10%</td> <td>12 - 20%</td> <td>SOME</td> </tr> <tr> <td>HIGHLY ORGANIC</td> <td>> 10%</td> <td>> 20%</td> <td>HIGHLY</td> </tr> </tbody> </table> <p style="text-align: center;">GROUND WATER</p> <p> WATER LEVEL IN BORE HOLE IMMEDIATELY AFTER DRILLING</p> <p> STATIC WATER LEVEL AFTER 24 HOURS</p> <p> PERCHED WATER, SATURATED ZONE, OR WATER BEARING STRATA</p> <p> SPRING OR SEEP</p>		ORGANIC MATERIAL	GRANULAR SOILS	SILT - CLAY SOILS	OTHER MATERIAL	TRACE OF ORGANIC MATTER	2 - 3%	3 - 5%	TRACE	LITTLE ORGANIC MATTER	3 - 5%	5 - 12%	LITTLE	MODERATELY ORGANIC	5 - 10%	12 - 20%	SOME	HIGHLY ORGANIC	> 10%	> 20%	HIGHLY	<p>FRESH - ROCK FRESH, CRYSTALS BRIGHT, FEW JOINTS MAY SHOW SLIGHT STAINING. ROCK RINGS UNDER HAMMER IF CRYSTALLINE.</p> <p>VERY SLIGHT (V SL.) - ROCK GENERALLY FRESH, JOINTS STAINED, SOME JOINTS MAY SHOW THIN CLAY COATINGS IF OPEN. CRYSTALS ON A BROKEN SPECIMEN FACE SHINE BRIGHTLY. ROCK RINGS UNDER HAMMER BLOWS IF OF A CRYSTALLINE NATURE.</p> <p>SLIGHT (SL.) - ROCK GENERALLY FRESH, JOINTS STAINED AND DISCOLORATION EXTENDS INTO ROCK UP TO 1 INCH. OPEN JOINTS MAY CONTAIN CLAY. IN GRANITOID ROCKS SOME OCCASIONAL FELDSPAR CRYSTALS ARE DULL AND DISCOLORED. CRYSTALLINE ROCKS RING UNDER HAMMER BLOWS.</p> <p>MODERATE (MOD.) - SIGNIFICANT PORTIONS OF ROCK SHOW DISCOLORATION AND WEATHERING EFFECTS. IN GRANITOID ROCKS, MOST FELDSPARS ARE DULL AND DISCOLORED, SOME SHOW CLAY. ROCK HAS DULL SOUND UNDER HAMMER BLOWS AND SHOWS SIGNIFICANT LOSS OF STRENGTH AS COMPARED WITH FRESH ROCK.</p> <p>MODERATELY SEVERE (MOD. SEV.) - ALL ROCK EXCEPT QUARTZ DISCOLORED OR STAINED. IN GRANITOID ROCKS, ALL FELDSPARS DULL AND DISCOLORED AND A MAJORITY SHOW KAOLINIZATION. ROCK SHOWS SEVERE LOSS OF STRENGTH AND CAN BE EXCAVATED WITH A GEOLOGIST'S PICK. ROCK GIVES "CLUNK" SOUND WHEN STRUCK. <i>IF TESTED, WOULD YIELD SPT REFUSAL</i></p> <p>SEVERE (SEV.) - ALL ROCK EXCEPT QUARTZ DISCOLORED OR STAINED. ROCK FABRIC CLEAR AND EVIDENT BUT REDUCED IN STRENGTH TO STRONG SOIL. IN GRANITOID ROCKS ALL FELDSPARS ARE KAOLINIZED TO SOME EXTENT. SOME FRAGMENTS OF STRONG ROCK USUALLY REMAIN. <i>IF TESTED, YIELDS SPT N VALUES > 100 BPF</i></p> <p>VERY SEVERE (V SEV.) - ALL ROCK EXCEPT QUARTZ DISCOLORED OR STAINED. ROCK FABRIC ELEMENTS ARE DISCERNIBLE BUT THE MASS IS EFFECTIVELY REDUCED TO SOIL STATUS, WITH ONLY FRAGMENTS OF STRONG ROCK REMAINING. SAPROLITE IS AN EXAMPLE OF ROCK WEATHERED TO A DEGREE SUCH THAT ONLY MINOR VESTIGES OF THE ORIGINAL ROCK FABRIC REMAIN. <i>IF TESTED, YIELDS SPT N VALUES < 100 BPF</i></p> <p>COMPLETE - ROCK REDUCED TO SOIL. ROCK FABRIC NOT DISCERNIBLE, OR DISCERNIBLE ONLY IN SMALL AND SCATTERED CONCENTRATIONS. QUARTZ MAY BE PRESENT AS DIKES OR STRINGERS. SAPROLITE IS ALSO AN EXAMPLE.</p>	
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COLOR		INDURATION		FRACTURE SPACING		INDURATION																																																							
<p>DESCRIPTIONS MAY INCLUDE COLOR OR COLOR COMBINATIONS (TAN, RED, YELLOW-BROWN, BLUE-GRAY). MODIFIERS SUCH AS LIGHT, DARK, STREAKED, ETC. ARE USED TO DESCRIBE APPEARANCE.</p>		<p>FRAGILE</p> <p>MODERATELY INDURATED</p> <p>INDURATED</p> <p>EXTREMELY INDURATED</p>		<p>VERY WIDE</p> <p>WIDE</p> <p>MODERATELY CLOSE</p> <p>CLOSE</p> <p>VERY CLOSE</p>		<p>FRAGILE</p> <p>MODERATELY INDURATED</p> <p>INDURATED</p> <p>EXTREMELY INDURATED</p>																																																							
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<p>BENCH MARK: BL-4; -BL- STA. 23+76.45</p> <p>N: 476109.77 E: 2042852.84</p> <p>ELEVATION: 93.59 FT.</p>		<p>FRAGILE</p> <p>MODERATELY INDURATED</p> <p>INDURATED</p> <p>EXTREMELY INDURATED</p>		<p>VERY WIDE</p> <p>WIDE</p> <p>MODERATELY CLOSE</p> <p>CLOSE</p> <p>VERY CLOSE</p>		<p>FRAGILE</p> <p>MODERATELY INDURATED</p> <p>INDURATED</p> <p>EXTREMELY INDURATED</p>																																																							

SITE PLAN



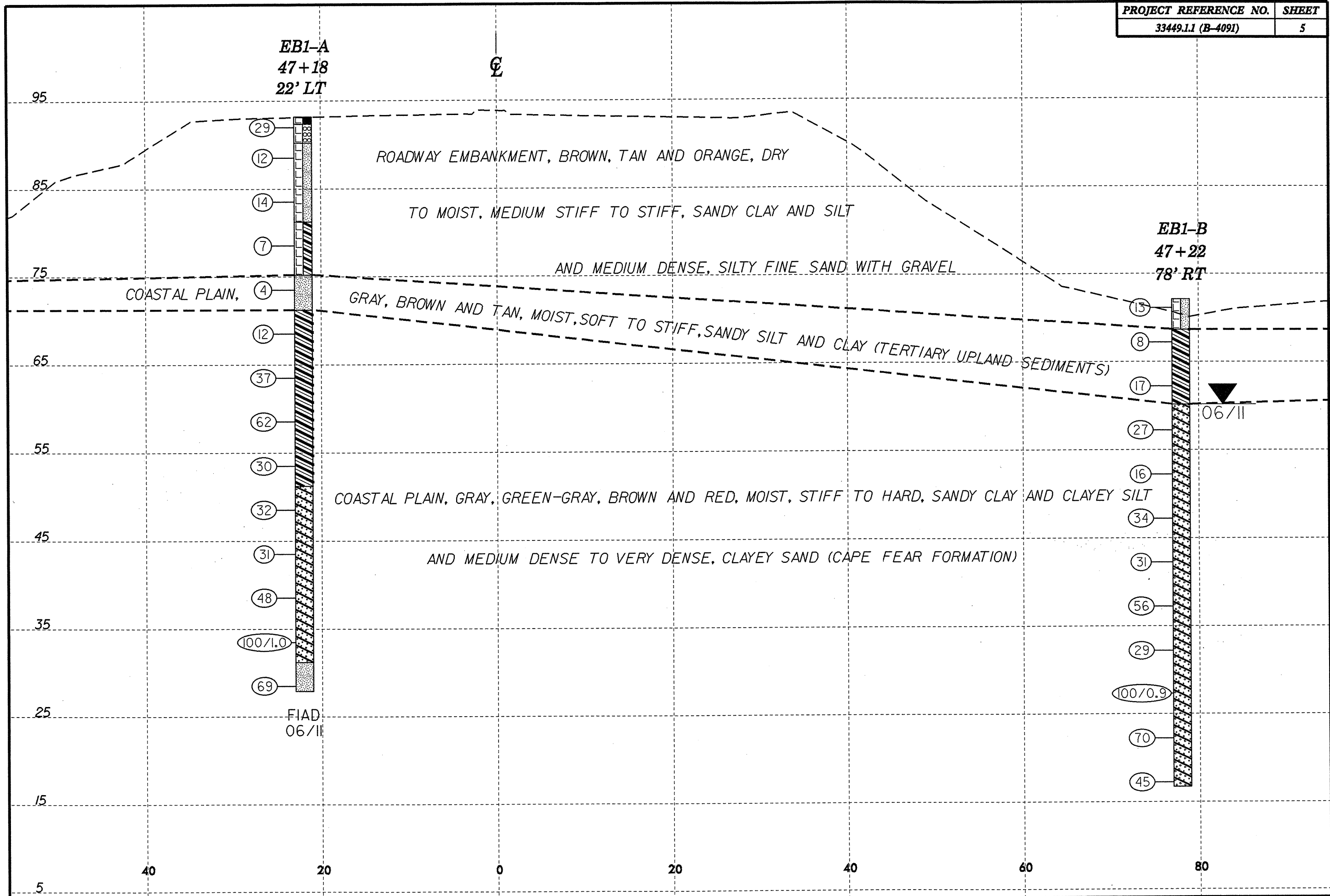
5/14/99

PROJECT REFERENCE NO.	SHEET
33449.1.1 (B-4091)	4
CENTERLINE PROFILE	



- (A) ROADWAY EMBANKMENT, TAN, BROWN, ORANGE AND GRAY, DRY TO MOIST, MEDIUM STIFF TO VERY STIFF, SANDY CLAY AND SILT AND LOOSE TO MEDIUM-DENSE, SILTY SAND
- (B) COASTAL PLAIN, TAN, GRAY AND BROWN, MOIST TO WET, MEDIUM STIFF TO STIFF, SANDY SILT AND CLAY, AND LOOSE TO MEDIUM-DENSE, SILTY AND CLAYEY SAND, AND SAND WITH GRAVEL (TERTIARY UPLAND SEDIMENTS)
- (C) ALLUVIAL, BROWN AND GRAY, WET, VERY SOFT TO STIFF, SANDY CLAY AND SILT AND VERY LOOSE TO MEDIUM-DENSE, SILTY SAND AND FINE TO COARSE SAND WITH ORGANICS

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EB1-A
47+18
22' LT

EB1-B
47+22
78' RT

ROADWAY EMBANKMENT, BROWN, TAN AND ORANGE, DRY

TO MOIST, MEDIUM STIFF TO STIFF, SANDY CLAY AND SILT

AND MEDIUM DENSE, SILTY FINE SAND WITH GRAVEL

COASTAL PLAIN,

GRAY, BROWN AND TAN, MOIST, SOFT TO STIFF, SANDY SILT AND CLAY (TERTIARY UPLAND SEDIMENTS)

COASTAL PLAIN, GRAY, GREEN-GRAY, BROWN AND RED, MOIST, STIFF TO HARD, SANDY CLAY AND CLAYEY SILT

AND MEDIUM DENSE TO VERY DENSE, CLAYEY SAND (CAPE FEAR FORMATION)

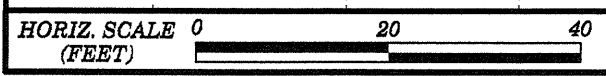
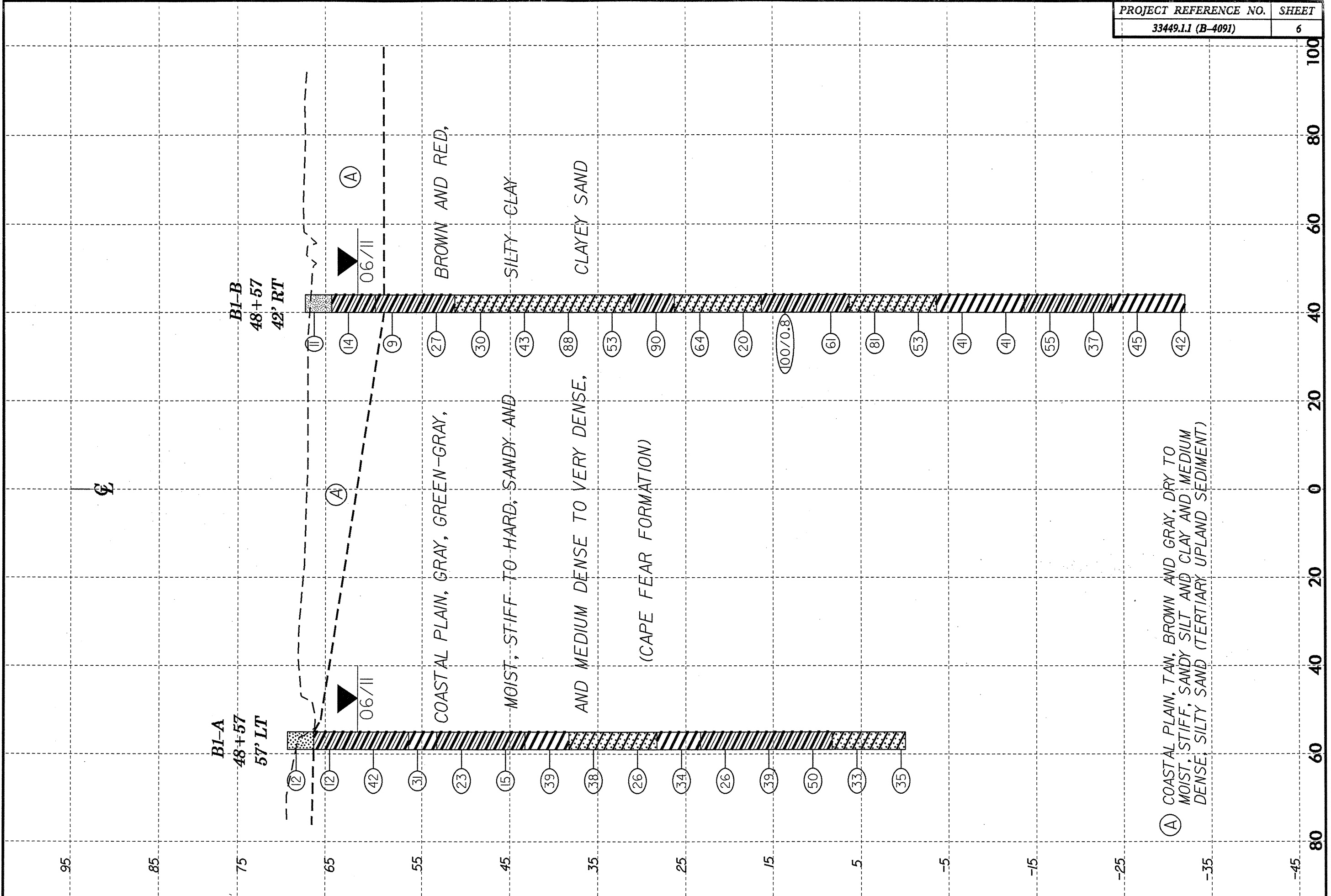
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(FEET)

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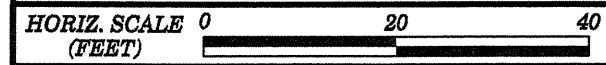
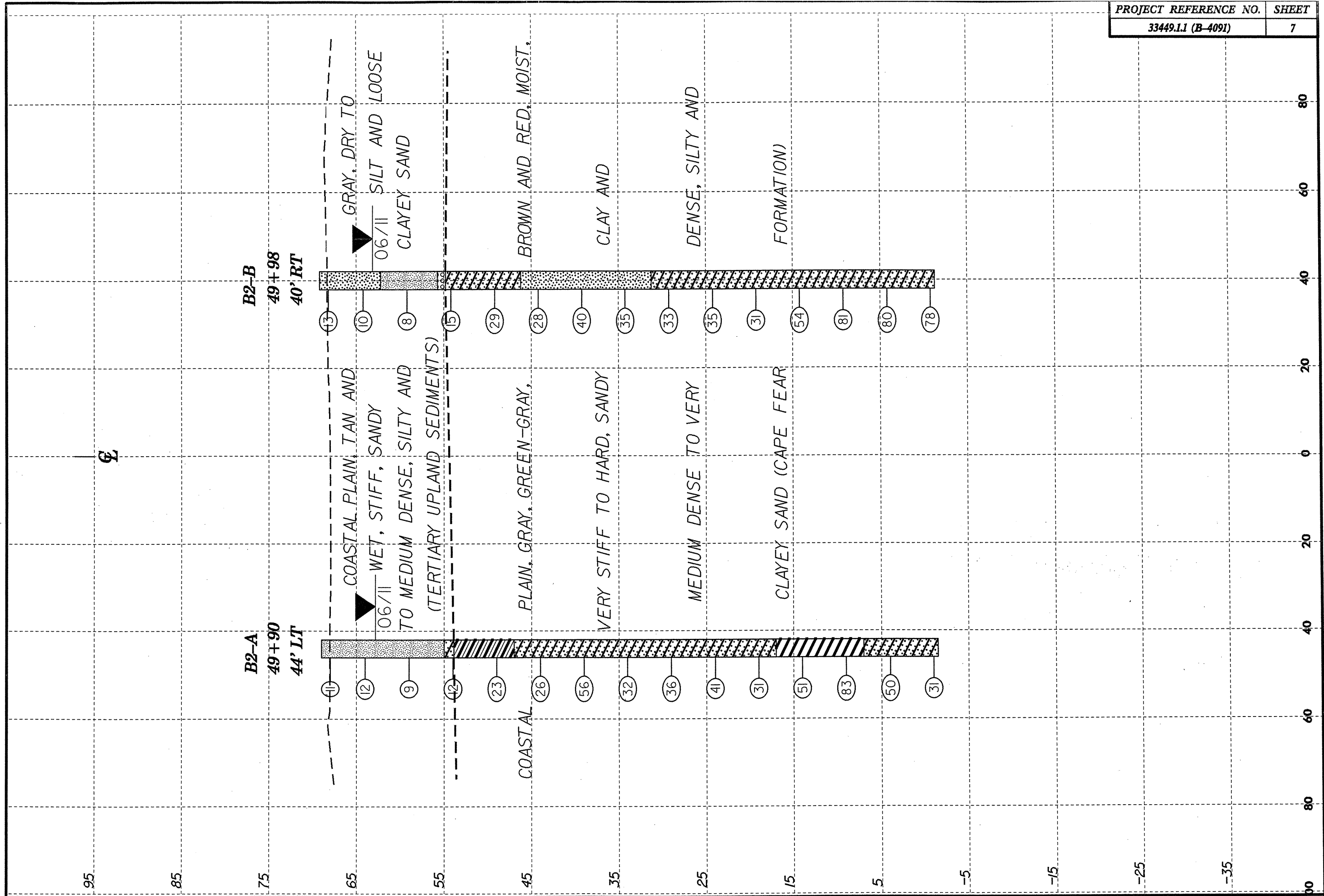
CROSS SECTION THROUGH END BENT 1



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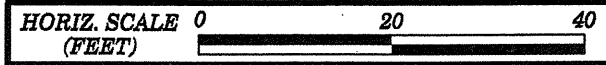
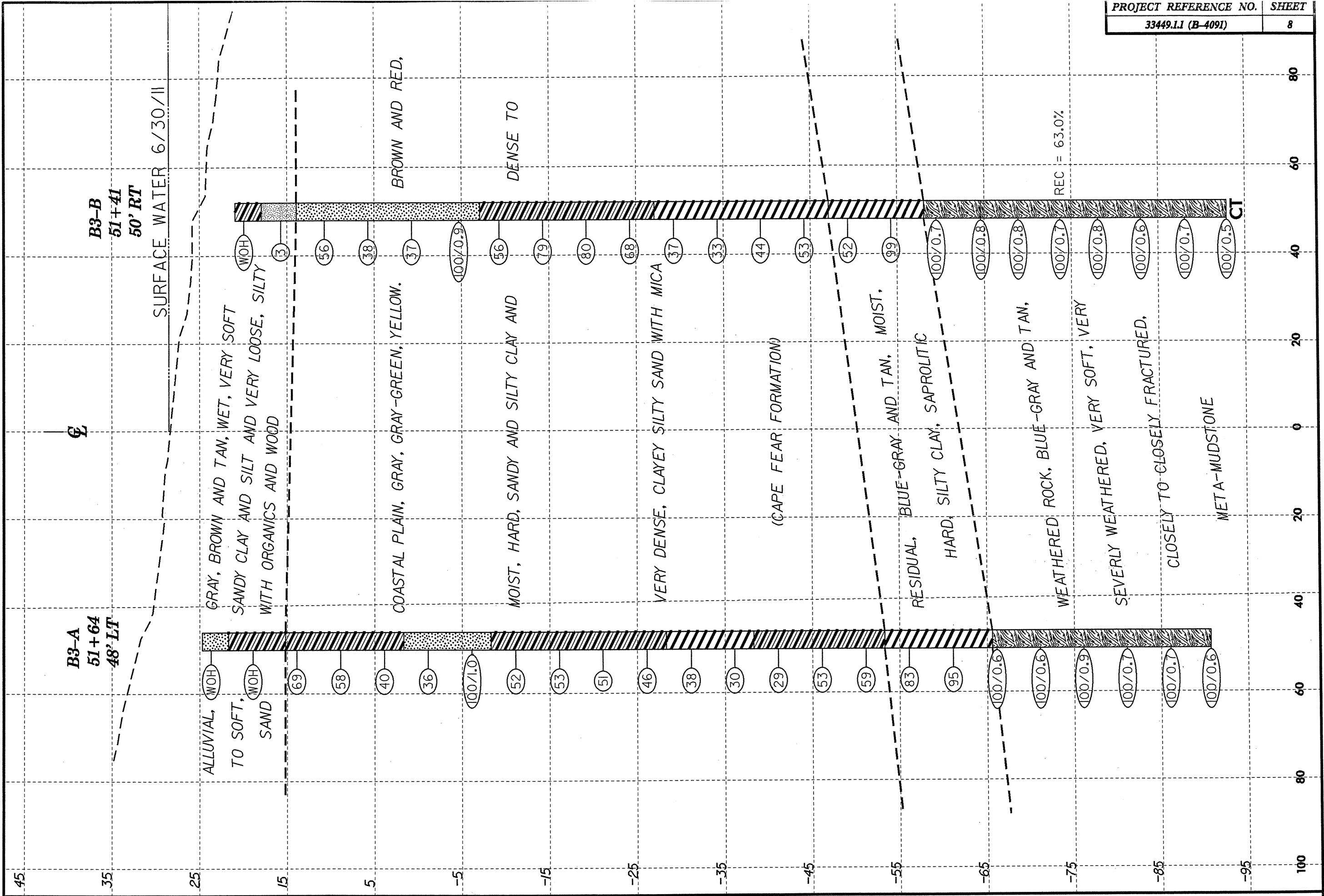
CROSS SECTION THROUGH BENT 1

Ⓐ COASTAL PLAIN, TAW, BROWN AND GRAY, DRY TO MOIST, STIFF, SANDY SILT AND CLAY AND MEDIUM DENSE, SILTY SAND (TERTIARY UPLAND SEDIMENT)



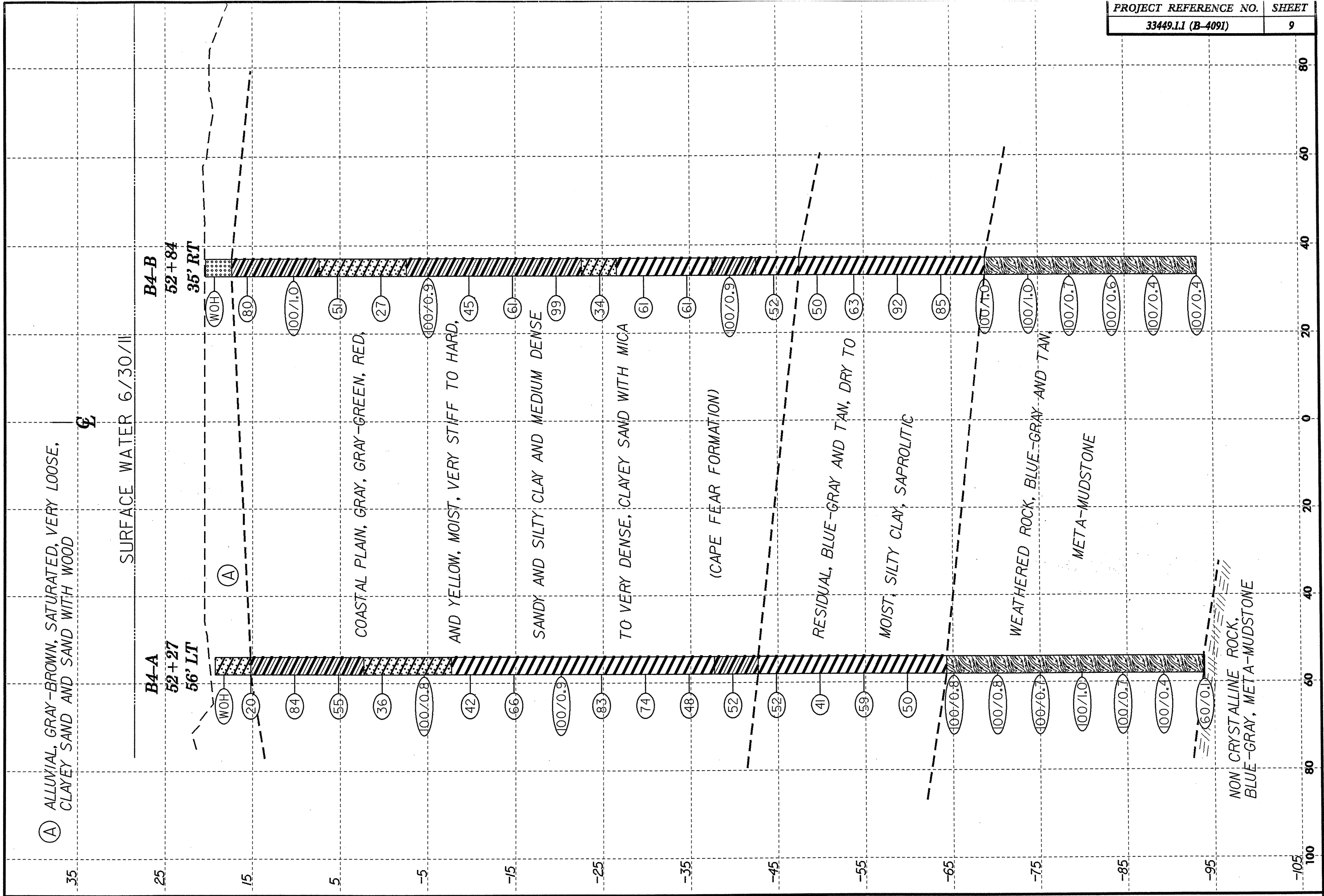
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CROSS SECTIONS THROUGH BENT 2



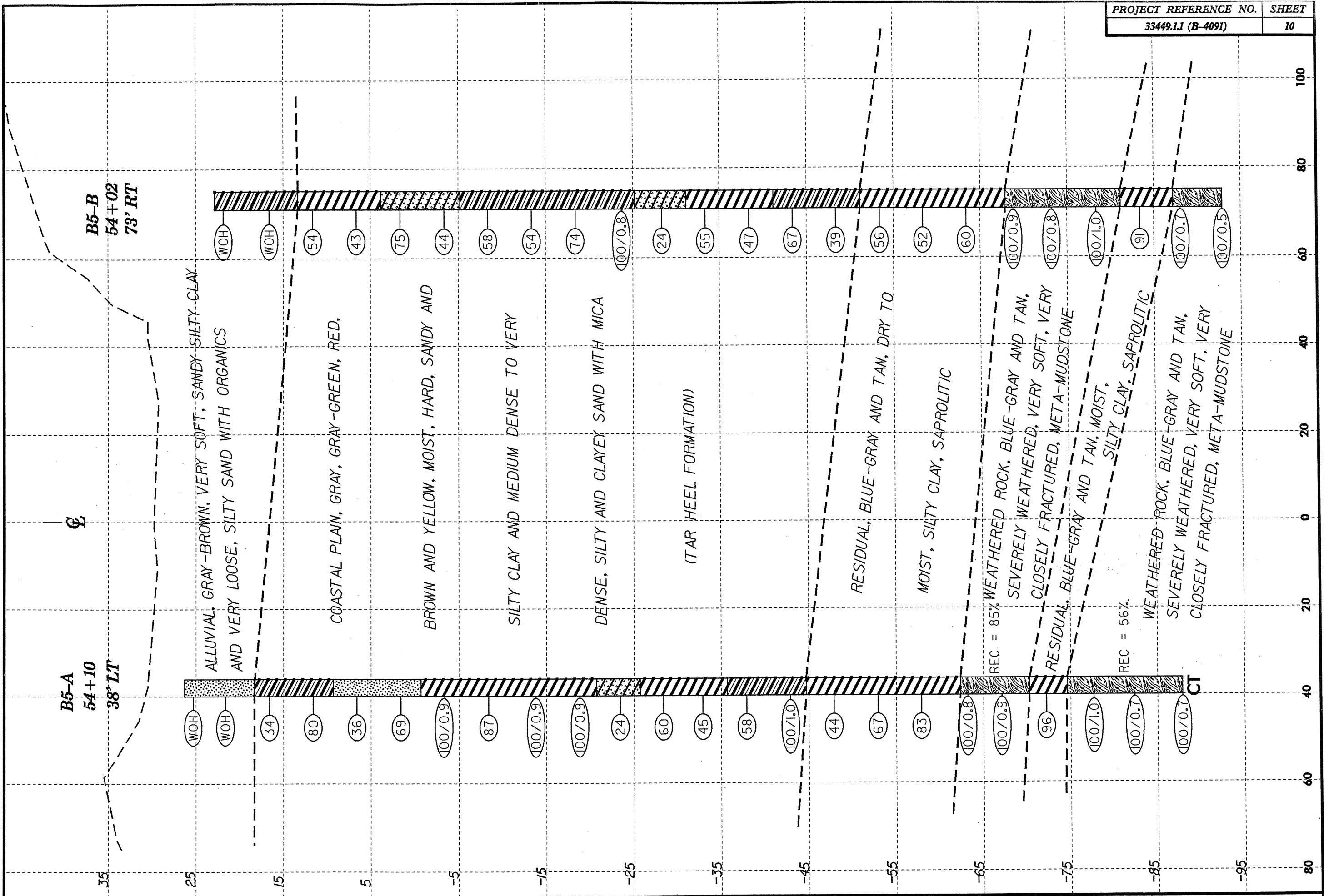
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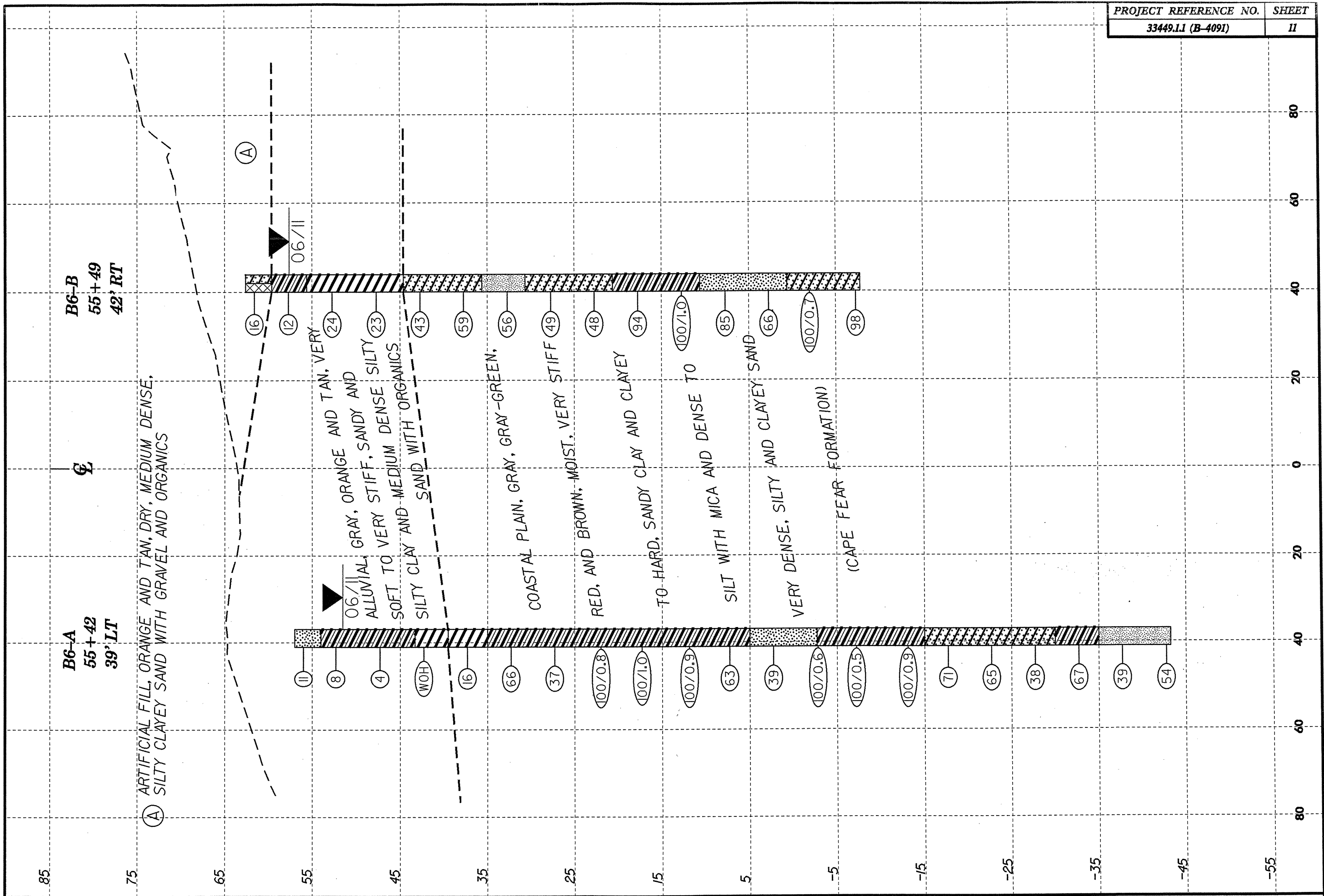
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CROSS SECTION THROUGH BENT 4



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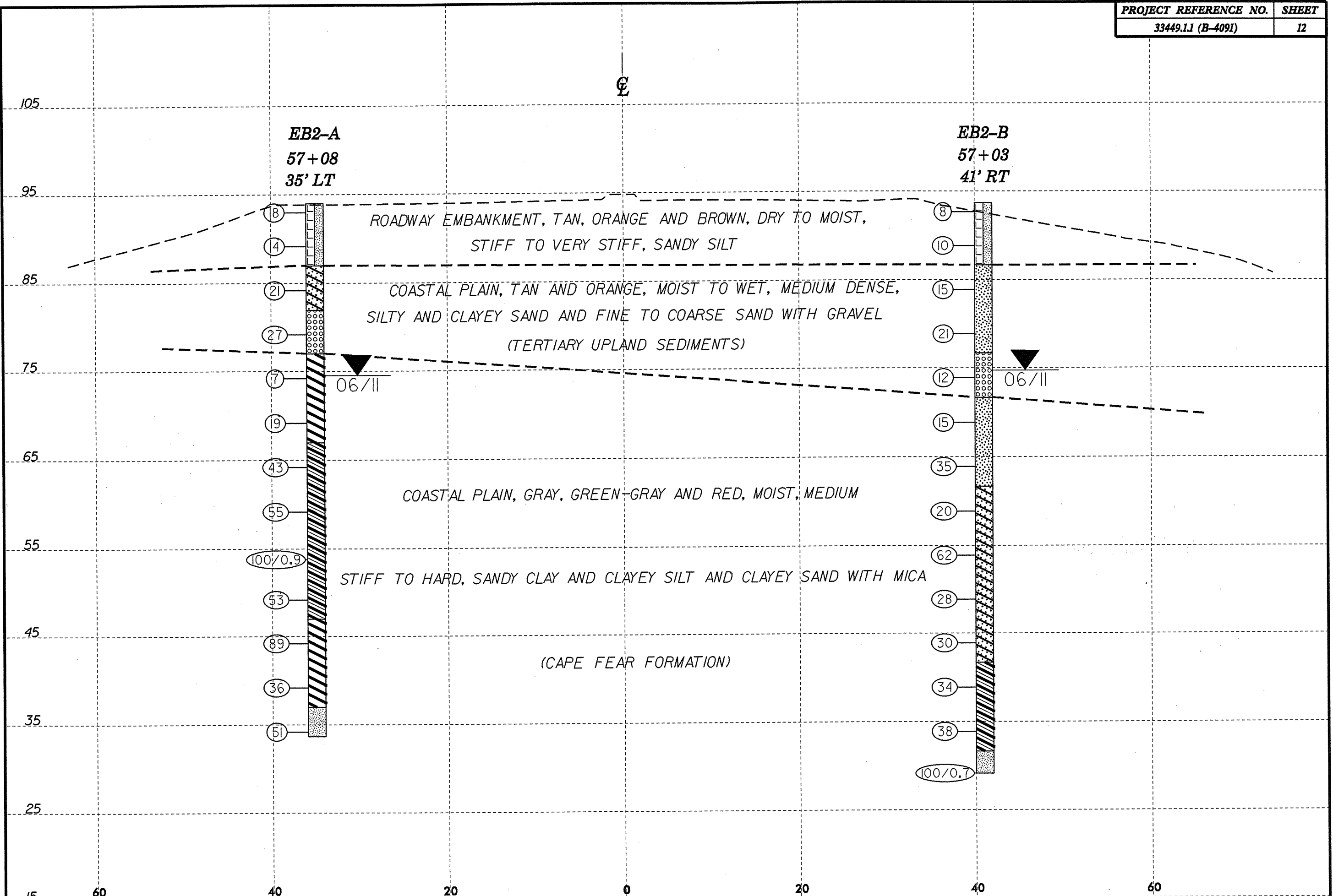
CROSS SECTION THROUGH BENT 5



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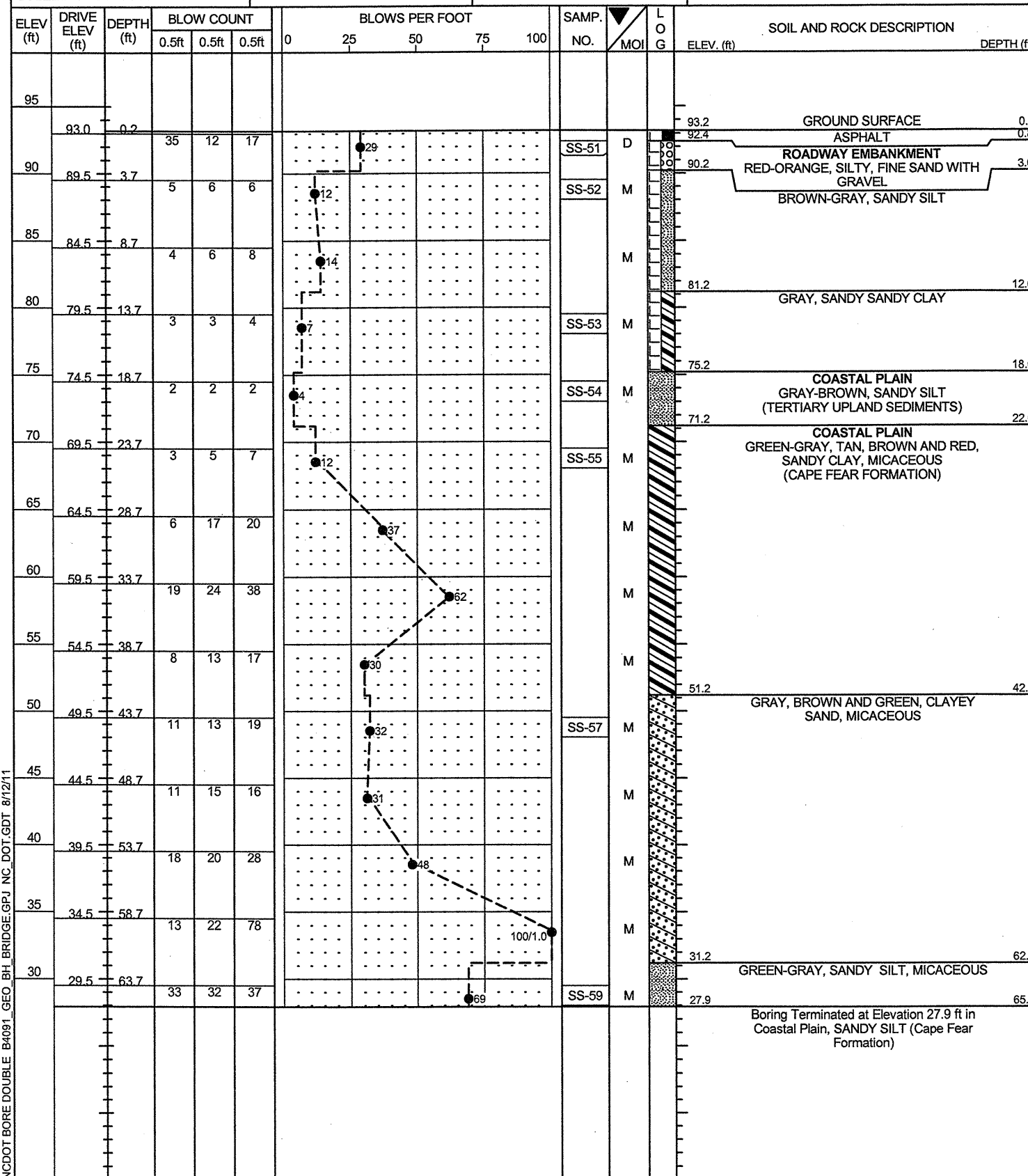
CROSS SECTION THROUGH BENT 6

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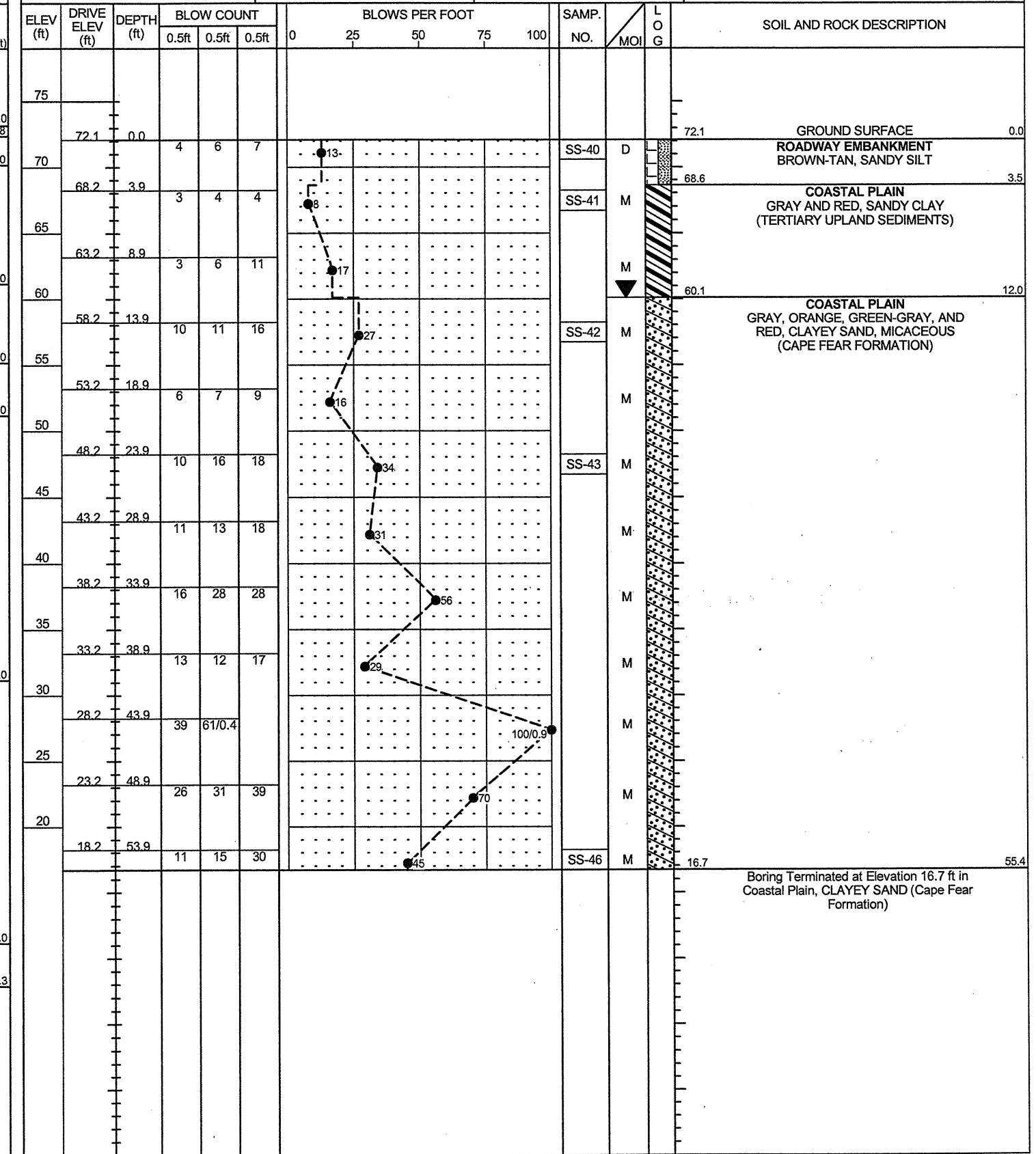


NCDOT GEOTECHNICAL ENGINEERING UNIT
BORELOG REPORT

WBS 33449.1.1	TIP B-4091	COUNTY CUMBERLAND	GEOLOGIST Howard, J.
SITE DESCRIPTION Bridge No. 85 on -L- (US 301/I-95 Bus. Loop) over -Y1- (SR 1738), -Y2- (SR 1741) and Cape Fear River			GROUND WTR (ft)
BORING NO. EB1-A	STATION 47+18	OFFSET 22 ft LT	ALIGNMENT -L-
COLLAR ELEV. 93.2 ft	TOTAL DEPTH 65.3 ft	NORTHING 476,160	EASTING 2,042,838
DRILL RIG/HAMMER EFF/DATE MAC6893 CME-55 88% 02/10/2010		DRILL METHOD Mud Rotary	HAMMER TYPE Automatic
DRILLER Contract Driller	START DATE 06/14/11	COMP. DATE 06/14/11	SURFACE WATER DEPTH N/A



WBS 33449.1.1	TIP B-4091	COUNTY CUMBERLAND	GEOLOGIST Howard, J.
SITE DESCRIPTION Bridge No. 85 on -L- (US 301/I-95 Bus. Loop) over -Y1- (SR 1738), -Y2- (SR 1741) and Cape Fear River			GROUND WTR (ft)
BORING NO. EB1-B	STATION 47+22	OFFSET 78 ft RT	ALIGNMENT -L-
COLLAR ELEV. 72.1 ft	TOTAL DEPTH 55.4 ft	NORTHING 476,065	EASTING 2,042,868
DRILL RIG/HAMMER EFF/DATE MAC6893 CME-55 88% 02/10/2010		DRILL METHOD Mud Rotary	HAMMER TYPE Automatic
DRILLER Contract Driller	START DATE 06/13/11	COMP. DATE 06/13/11	SURFACE WATER DEPTH N/A



NCDOT BORE DOUBLE B4091_GEO_BH_BRIDGE.GPJ_NC_DOT_GDT_8/12/11



NCDOT GEOTECHNICAL ENGINEERING UNIT BORELOG REPORT

WBS 33449.1.1	TIP B-4091	COUNTY CUMBERLAND	GEOLOGIST Howard, J.
SITE DESCRIPTION Bridge No. 85 on -L- (US 301/I-95 Bus. Loop) over -Y1- (SR 1738), -Y2- (SR 1741) and Cape Fear River			GROUND WTR (ft)
BORING NO. B1-A	STATION 48+57	OFFSET 57 ft LT	ALIGNMENT -L-
COLLAR ELEV. 70.3 ft	TOTAL DEPTH 70.3 ft	NORTHING 476,230	EASTING 2,042,964
DRILL RIG/HAMMER EFF./DATE MAC6893 CME-55 88% 02/10/2010		DRILL METHOD Mud Rotary	HAMMER TYPE Automatic
DRILLER Contract Driller	START DATE 06/09/11	COMP. DATE 06/09/11	SURFACE WATER DEPTH N/A

ELEV (ft)	DRIVE ELEV (ft)	DEPTH (ft)	BLOW COUNT			BLOWS PER FOOT					SAMP. NO.	LOG	SOIL AND ROCK DESCRIPTION	DEPTH (ft)	
			0.5ft	0.5ft	0.5ft	0	25	50	75	100					
75															
70	70.3	0.0	10	6	6								70.3	GROUND SURFACE	0.0
65	66.5	3.8	4	5	7	12					SS-13	D	67.3	COASTAL PLAIN TAN-YELLOW, SILTY, FINE SAND (TERTIARY UPLAND SEDIMENT)	3.0
60	61.5	8.8	17	19	23	12					SS-14	M		COASTAL PLAIN RED-BROWN, GREEN-GRAY, AND GRAY, SANDY CLAY (CAPE FEAR FORMATION)	
55	56.5	13.8	10	13	18							M	56.5	TAN AND GRAY, SILTY CLAY	13.8
50	51.5	18.8	8	11	12						SS-15	M	53.3	TAN AND GRAY, SANDY CLAY, MICACEOUS	17.0
45	46.5	23.8	7	7	8							M			
40	41.5	28.8	13	17	22						SS-16	M	43.3	TAN AND GRAY, SANDY CLAY, MICACEOUS	27.0
35	36.5	33.8	12	17	21							M			
30	31.5	38.8	9	11	15						SS-17	M	38.3	RED AND GRAY, SILTY CLAY, MICACEOUS	32.0
25	26.5	43.8	12	15	19							M			
20	21.5	48.8	11	12	14						SS-18	M	28.3	GREEN-GRAY AND BROWN, CLAYEY SAND, MICACEOUS	42.0
15	16.5	53.8	15	13	26							M			
10	11.5	58.8	17	22	28						SS-19	M	23.3	GRAY AND RED, SILTY CLAY, MICACEOUS	47.0
5	6.5	63.8	13	15	18							M			
0	1.5	68.8	12	16	19						SS-20	M	8.3	GRAY AND RED, SANDY CLAY, MICACEOUS	62.0
												M			
											SS-21	M			
												M			
											SS-22	M	0.0	LIGHT GRAY AND GREEN, CLAYEY SAND, MICACEOUS	70.3
														Boring Terminated at Elevation 0.0 ft in Coastal Plain, CLAYEY SAND (Cape Fear Formation)	

NCDOT BORE SINGLE B4091_GEO_BH_BRIDGE.GPJ NC_DOT_GDT 8/12/11

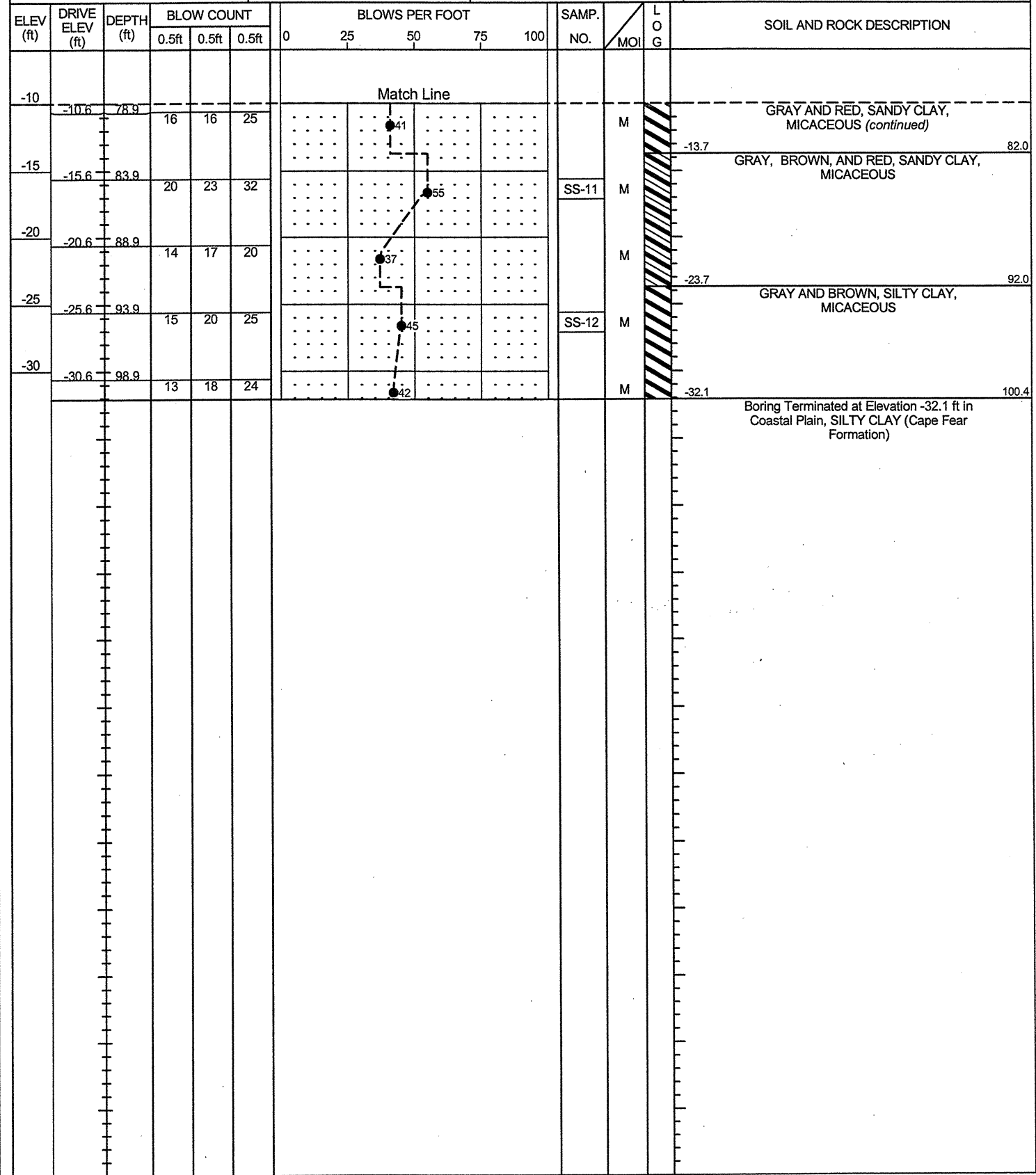
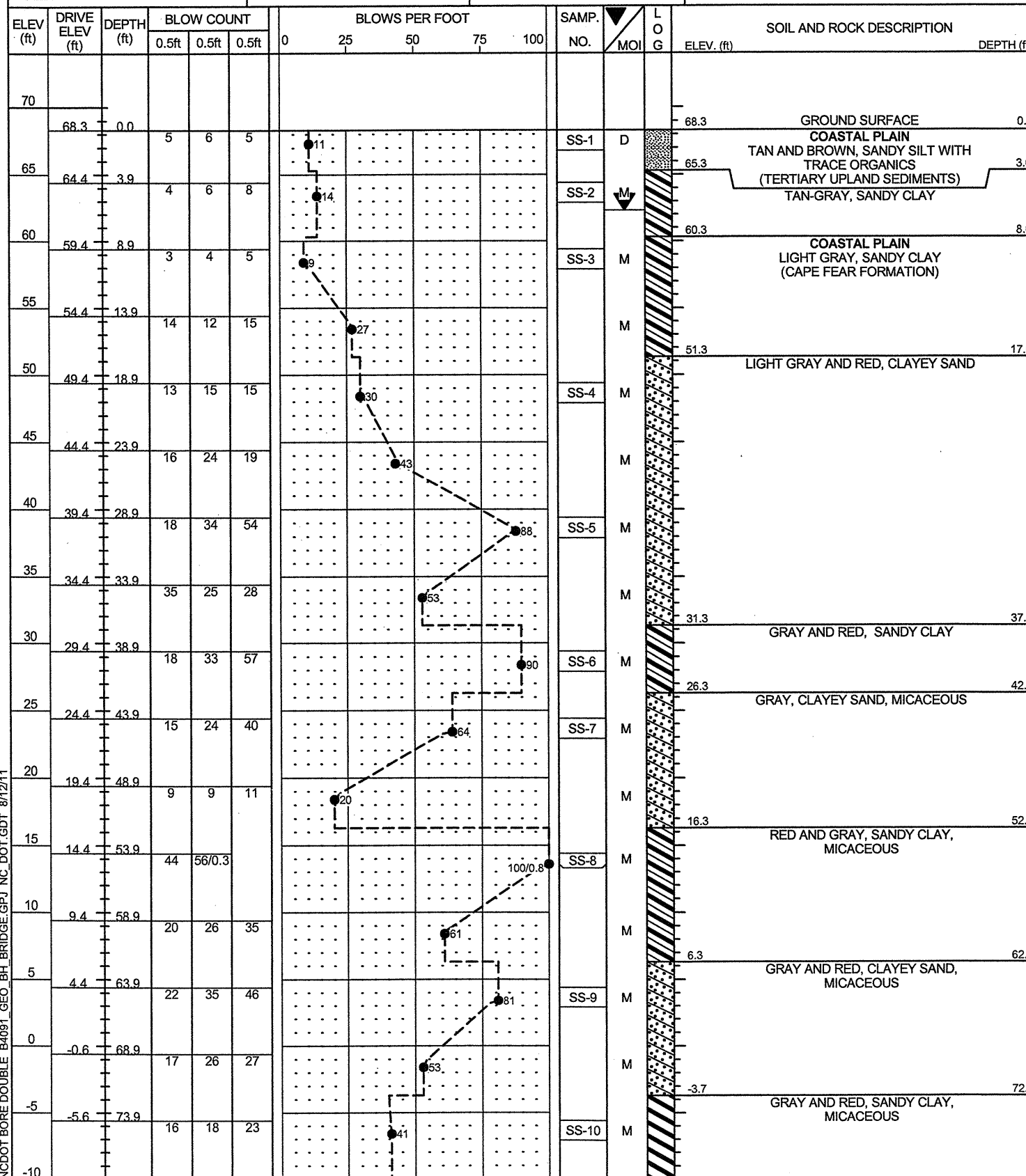


NCDOT GEOTECHNICAL ENGINEERING UNIT

BORELOG REPORT

WBS 33449.1.1	TIP B-4091	COUNTY CUMBERLAND	GEOLOGIST Howard, J.
SITE DESCRIPTION Bridge No. 85 on -L- (US 301/I-95 Bus. Loop) over -Y1- (SR 1738), -Y2- (SR 1741) and Cape Fear River			GROUND WTR (ft) 0 HR. 8.2
BORING NO. B1-B	STATION 48+57	OFFSET 42 ft RT	ALIGNMENT -L-
COLLAR ELEV. 68.3 ft	TOTAL DEPTH 100.4 ft	NORTHING 476,134	EASTING 2,042,989
DRILL RIG/HAMMER EFF./DATE MAC6893 CME-55 88% 02/10/2010		DRILL METHOD Mud Rotary	HAMMER TYPE Automatic
DRILLER Contract Driller	START DATE 06/07/11	COMP. DATE 06/08/11	SURFACE WATER DEPTH N/A

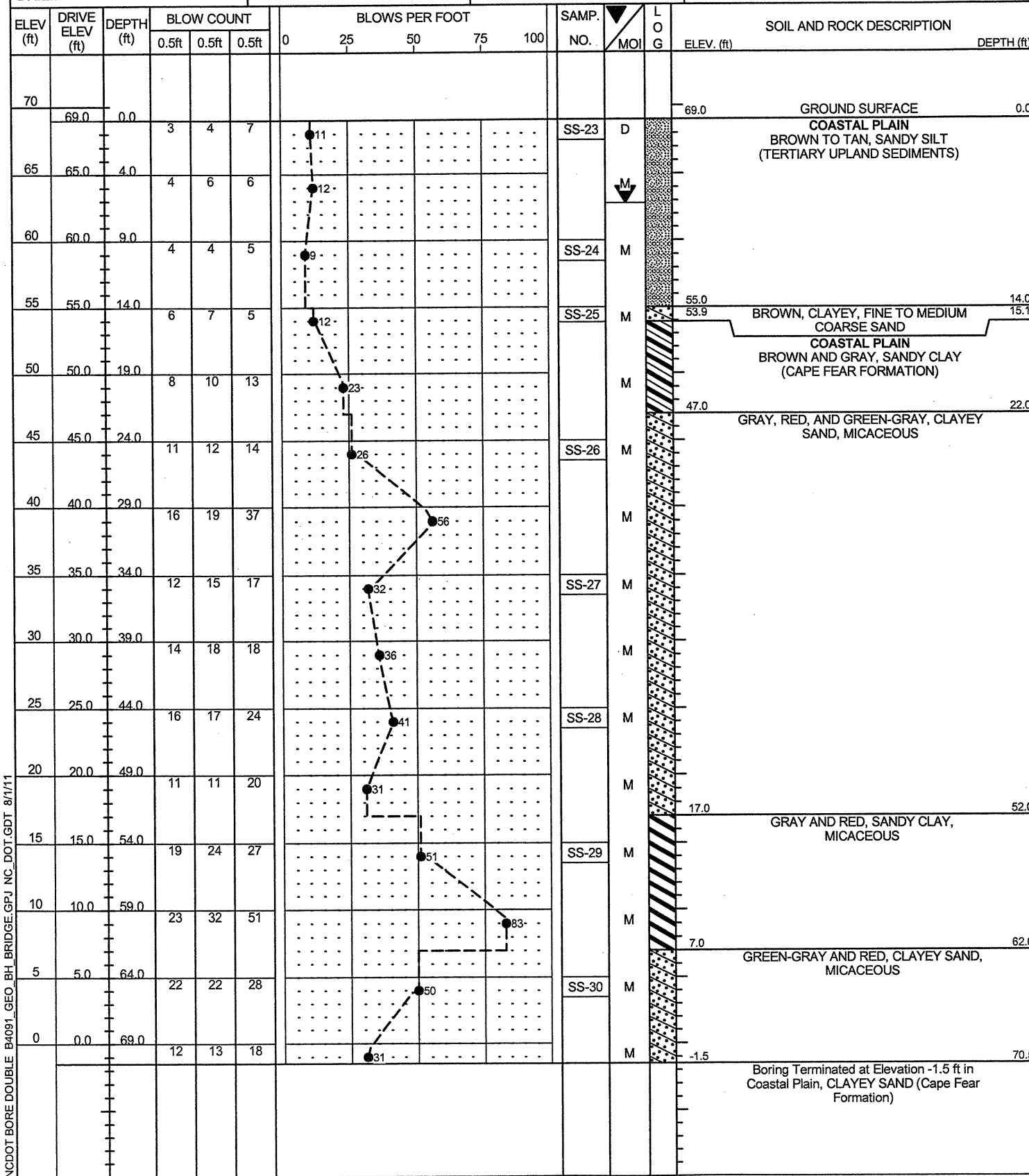
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SITE DESCRIPTION Bridge No. 85 on -L- (US 301/I-95 Bus. Loop) over -Y1- (SR 1738), -Y2- (SR 1741) and Cape Fear River			GROUND WTR (ft) 0 HR. 8.2
BORING NO. B1-B	STATION 48+57	OFFSET 42 ft RT	ALIGNMENT -L-
COLLAR ELEV. 68.3 ft	TOTAL DEPTH 100.4 ft	NORTHING 476,134	EASTING 2,042,989
DRILL RIG/HAMMER EFF./DATE MAC6893 CME-55 88% 02/10/2010		DRILL METHOD Mud Rotary	HAMMER TYPE Automatic
DRILLER Contract Driller	START DATE 06/07/11	COMP. DATE 06/08/11	SURFACE WATER DEPTH N/A



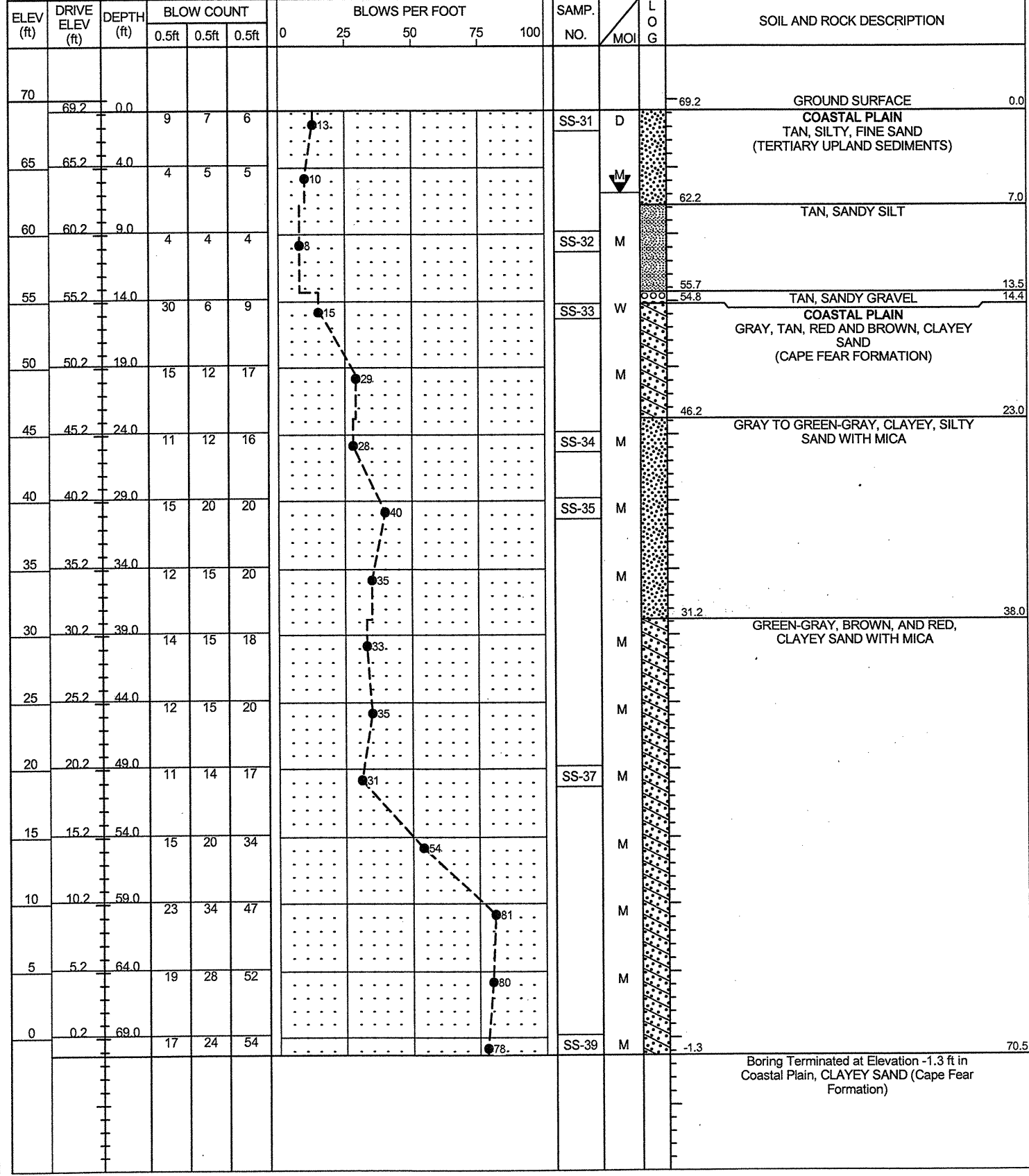
NCDOT BORE DOUBLE B4091_GEO_BH_BRIDGE.GPJ NC_DOT_GDT 8/12/11

NCDOT GEOTECHNICAL ENGINEERING UNIT
BORELOG REPORT

WBS 33449.1.1	TIP B-4091	COUNTY CUMBERLAND	GEOLOGIST Howard, J.
SITE DESCRIPTION Bridge No. 85 on -L- (US 301/I-95 Bus. Loop) over -Y1- (SR 1738), -Y2- (SR 1741) and Cape Fear River			GROUND WTR (ft)
BORING NO. B2-A	STATION 49+90	OFFSET 44 ft LT	ALIGNMENT -L-
COLLAR ELEV. 69.0 ft	TOTAL DEPTH 70.5 ft	NORTHING 476,251	EASTING 2,043,095
DRILL RIG/HAMMER EFF./DATE MAC6893 CME-55 88% 02/10/2010		DRILL METHOD Mud Rotary	HAMMER TYPE Automatic
DRILLER Contract Driller	START DATE 06/09/11	COMP. DATE 06/10/11	SURFACE WATER DEPTH N/A



WBS 33449.1.1	TIP B-4091	COUNTY CUMBERLAND	GEOLOGIST Howard, J.
SITE DESCRIPTION Bridge No. 85 on -L- (US 301/I-95 Bus. Loop) over -Y1- (SR 1738), -Y2- (SR 1741) and Cape Fear River			GROUND WTR (ft)
BORING NO. B2-B	STATION 49+98	OFFSET 40 ft RT	ALIGNMENT -L-
COLLAR ELEV. 69.2 ft	TOTAL DEPTH 70.5 ft	NORTHING 476,172	EASTING 2,043,125
DRILL RIG/HAMMER EFF./DATE MAC6893 CME-55 88% 02/10/2010		DRILL METHOD Mud Rotary	HAMMER TYPE Automatic
DRILLER Contract Driller	START DATE 06/10/11	COMP. DATE 06/10/11	SURFACE WATER DEPTH N/A



NCDOT BORE DOUBLE B4091_GEO_BH_BRIDGE.GPJ NC_DOT_GDT_8/1/11

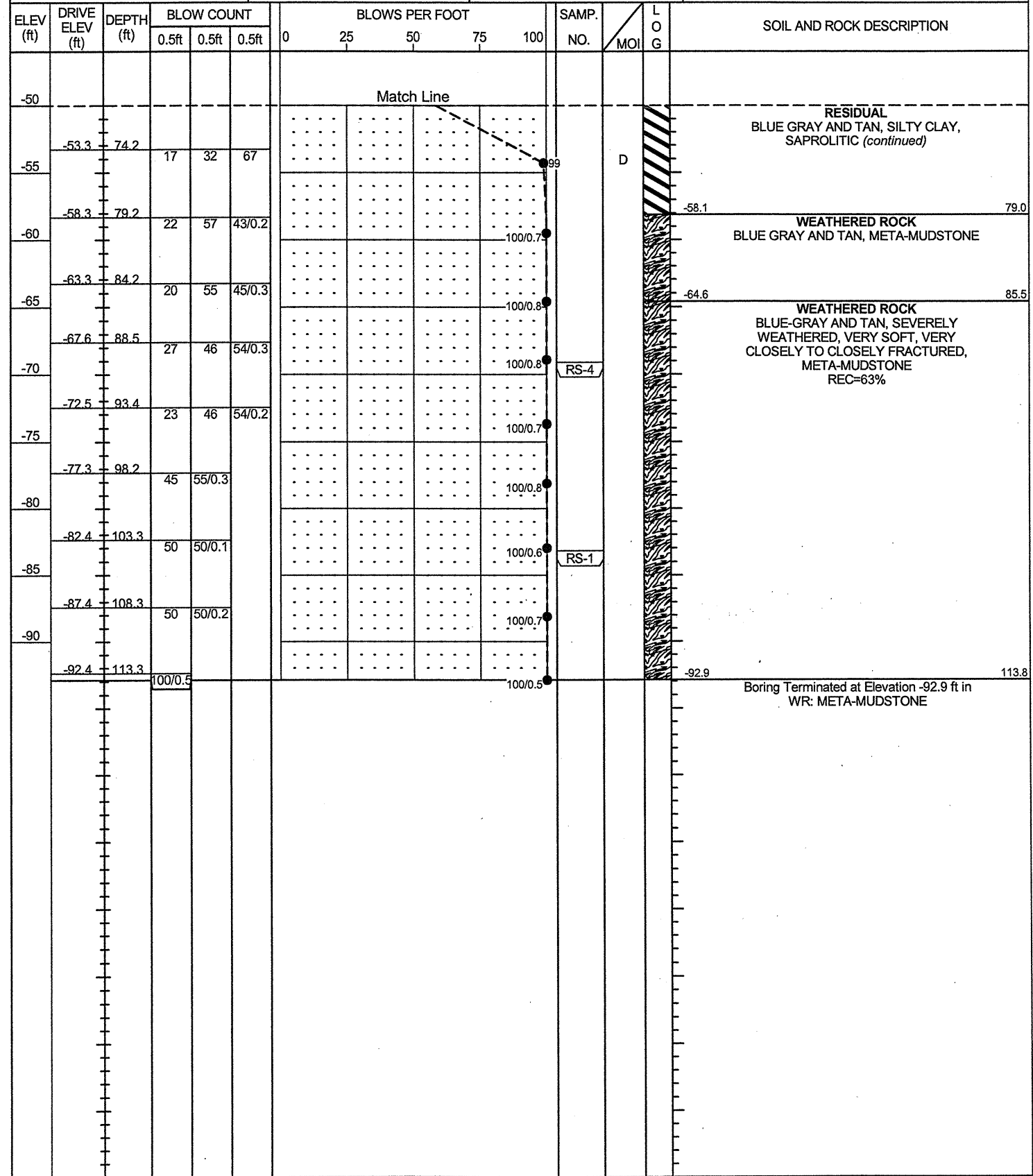
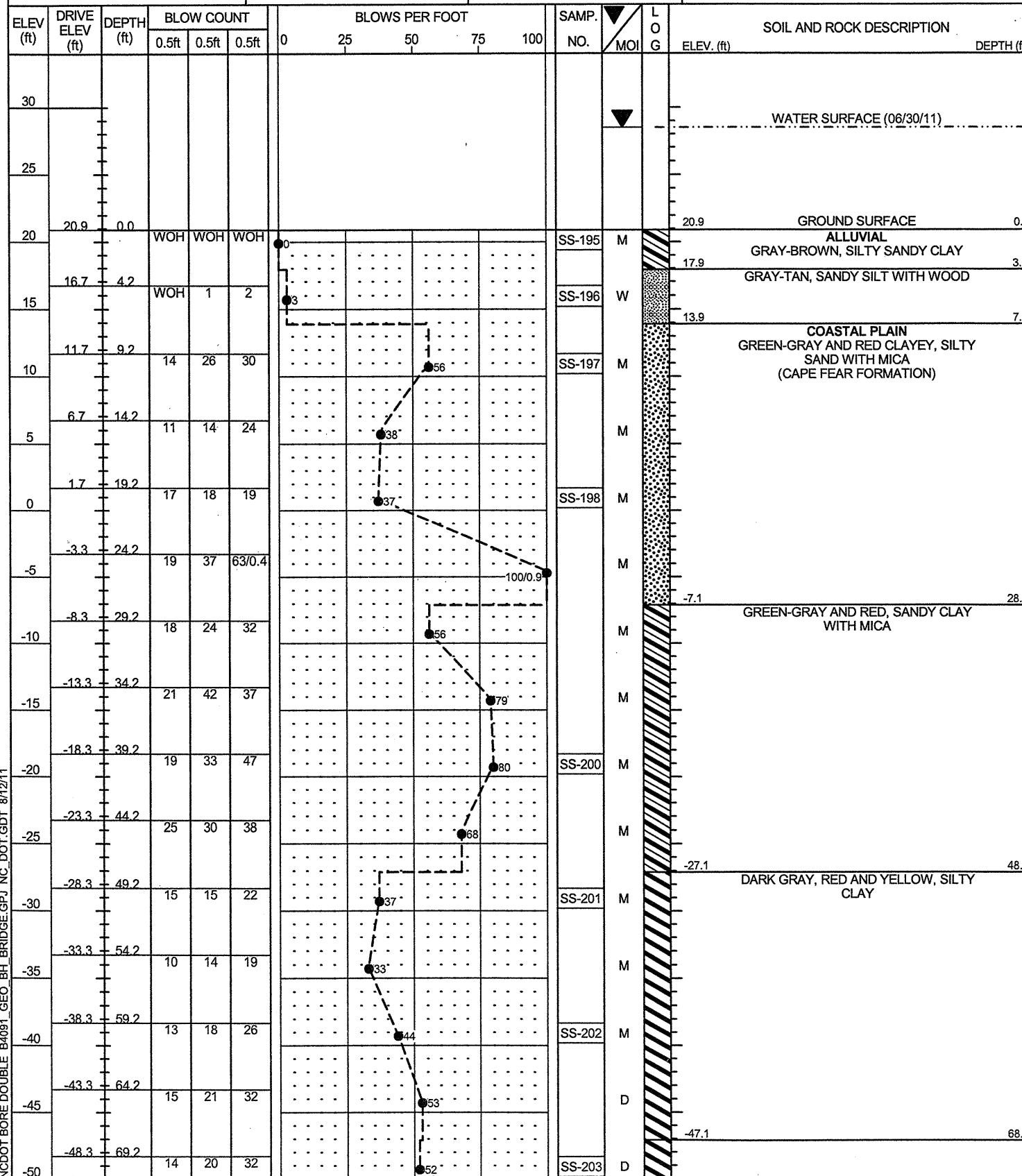


NCDOT GEOTECHNICAL ENGINEERING UNIT

BORELOG REPORT

WBS 33449.1.1	TIP B-4091	COUNTY CUMBERLAND	GEOLOGIST Howard, J.
SITE DESCRIPTION Bridge No. 85 on -L- (US 301/I-95 Bus. Loop) over -Y1- (SR 1738), -Y2- (SR 1741) and Cape Fear River			GROUND WTR (ft)
BORING NO. B3-B	STATION 51+41	OFFSET 50 ft RT	ALIGNMENT -L-
COLLAR ELEV. 20.9 ft	TOTAL DEPTH 113.8 ft	NORTHING 476,199	EASTING 2,043,266
DRILL RIG/HAMMER EFF./DATE MAC2425 CME-55 85% 09/02/2009		DRILL METHOD Mud Rotary	HAMMER TYPE Automatic
DRILLER Contract Driller	START DATE 06/30/11	COMP. DATE 07/01/11	SURFACE WATER DEPTH 7.6ft

WBS 33449.1.1	TIP B-4091	COUNTY CUMBERLAND	GEOLOGIST Howard, J.
SITE DESCRIPTION Bridge No. 85 on -L- (US 301/I-95 Bus. Loop) over -Y1- (SR 1738), -Y2- (SR 1741) and Cape Fear River			GROUND WTR (ft)
BORING NO. B3-B	STATION 51+41	OFFSET 50 ft RT	ALIGNMENT -L-
COLLAR ELEV. 20.9 ft	TOTAL DEPTH 113.8 ft	NORTHING 476,199	EASTING 2,043,266
DRILL RIG/HAMMER EFF./DATE MAC2425 CME-55 85% 09/02/2009		DRILL METHOD Mud Rotary	HAMMER TYPE Automatic
DRILLER Contract Driller	START DATE 06/30/11	COMP. DATE 07/01/11	SURFACE WATER DEPTH 7.6ft



NCDOT BORE DOUBLE B4091 GEO BH BRIDGE GPJ NC_DOT_GDT 8/12/11



**NCDOT GEOTECHNICAL ENGINEERING UNIT
CORE BORING REPORT**

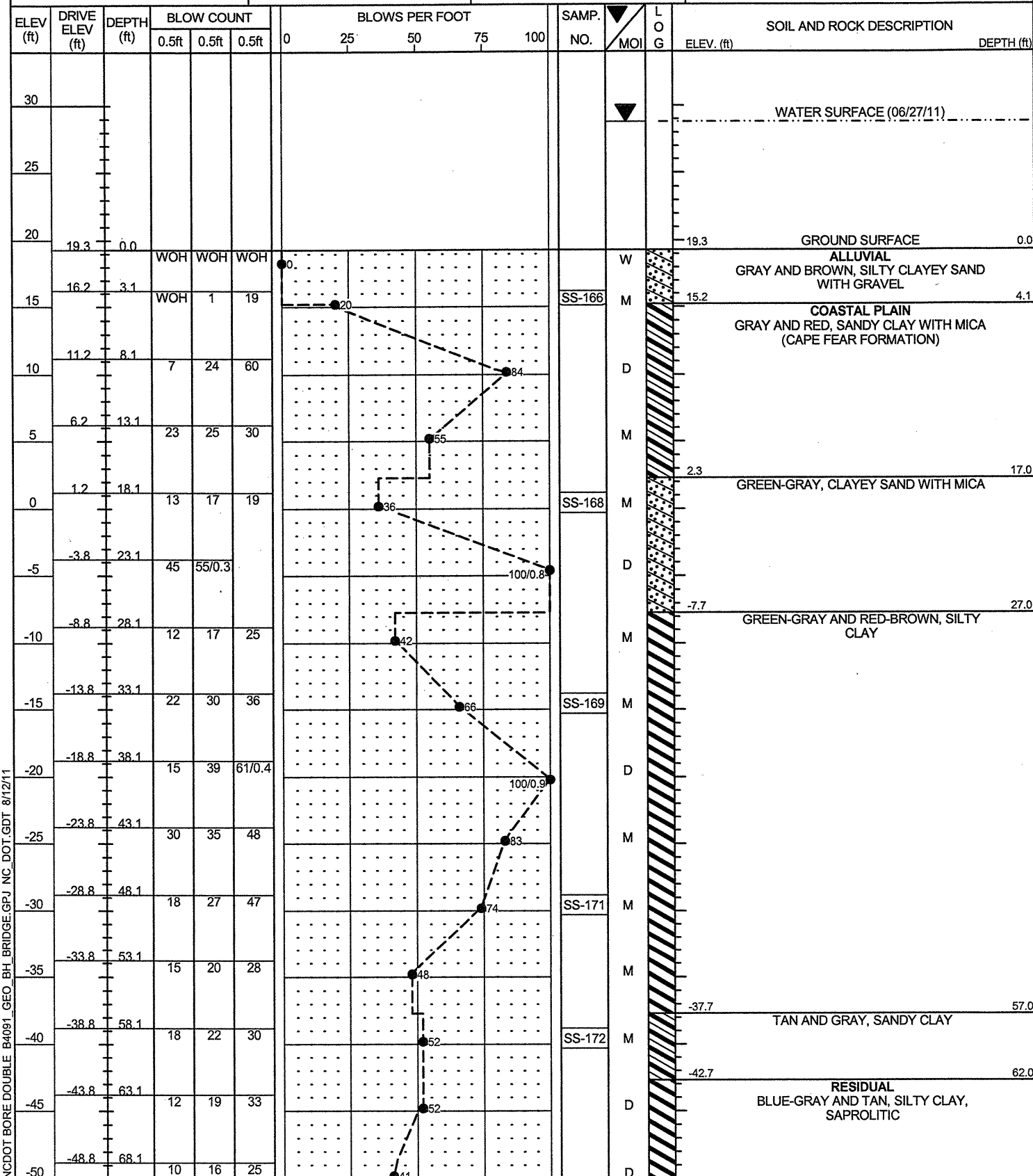
WBS 33449.1.1	TIP B-4091	COUNTY CUMBERLAND	GEOLOGIST Howard, J.
SITE DESCRIPTION Bridge No. 85 on -L- (US 301/I-95 Bus. Loop) over -Y1- (SR 1738), -Y2- (SR 1741) and Cape Fear River			GROUND WTR (ft)
BORING NO. B3-B	STATION 51+41	OFFSET 50 ft RT	ALIGNMENT -L-
COLLAR ELEV. 20.9 ft	TOTAL DEPTH 113.8 ft	NORTHING 476,199	EASTING 2,043,266
DRILL RIG/HAMMER EFF/DATE MAC2425 CME-55 85% 09/02/2009		DRILL METHOD Mud Rotary	HAMMER TYPE Automatic
DRILLER Contract Driller	START DATE 06/30/11	COMP. DATE 07/01/11	SURFACE WATER DEPTH 7.6ft

CORE SIZE N/A				TOTAL RUN 23.2 ft								
ELEV (ft)	RUN ELEV (ft)	DEPTH (ft)	RUN (ft)	DRILL RATE (Min/ft)	RUN		SAMP. NO.	STRATA		LOG	DESCRIPTION AND REMARKS	DEPTH (ft)
					REC. (ft) %	RQD (ft) %		REC. (ft) %	RQD (ft) %			
-64.6	-64.6	85.5	3.0	1:30	(2.6)	N/A		(17.8)	N/A		Begin Coring @ 85.5 ft	85.5
	-67.6	88.5		1:15	87%			63%			WEATHERED ROCK BLUE-GRAY AND TAN, SEVERLEY WEATHERED, VERY SOFT, VERY CLOSELY TO CLOSELY FRACTURED, META-MUDSTONE	
	-68.9	89.8		1:45								
-70			3.6	2:15	(3.2)	N/A	RS-4					
	-72.5	93.4		2:00	89%							
	-73.7	94.6		2:15								
-75			3.6	1:45/0.6								
	-77.3	98.2		3:00	(2.0)	N/A						
	-78.1	99.0		3:00	56%							
				2:45								
-80			4.3	0:45/0.6								
	-82.4	103.3		2:15	(3.4)	N/A						
	-83.0	103.8		2:00	79%							
				2:15								
-85			4.4	2:30			RS-1					
	-87.4	108.3		0:15/0.3	(3.8)	N/A						
	-88.1	109.0		3:00	86%							
				3:00								
-90			4.3	1:00/0.4	(2.8)	N/A						
				2:00	65%							
	-92.4	113.3		2:30								
				2:30								
				3:30								
				0:15/0.3								
				N=100/0.5								
											Boring Terminated at Elevation -92.9 ft in WR: META-MUDSTONE	113.8

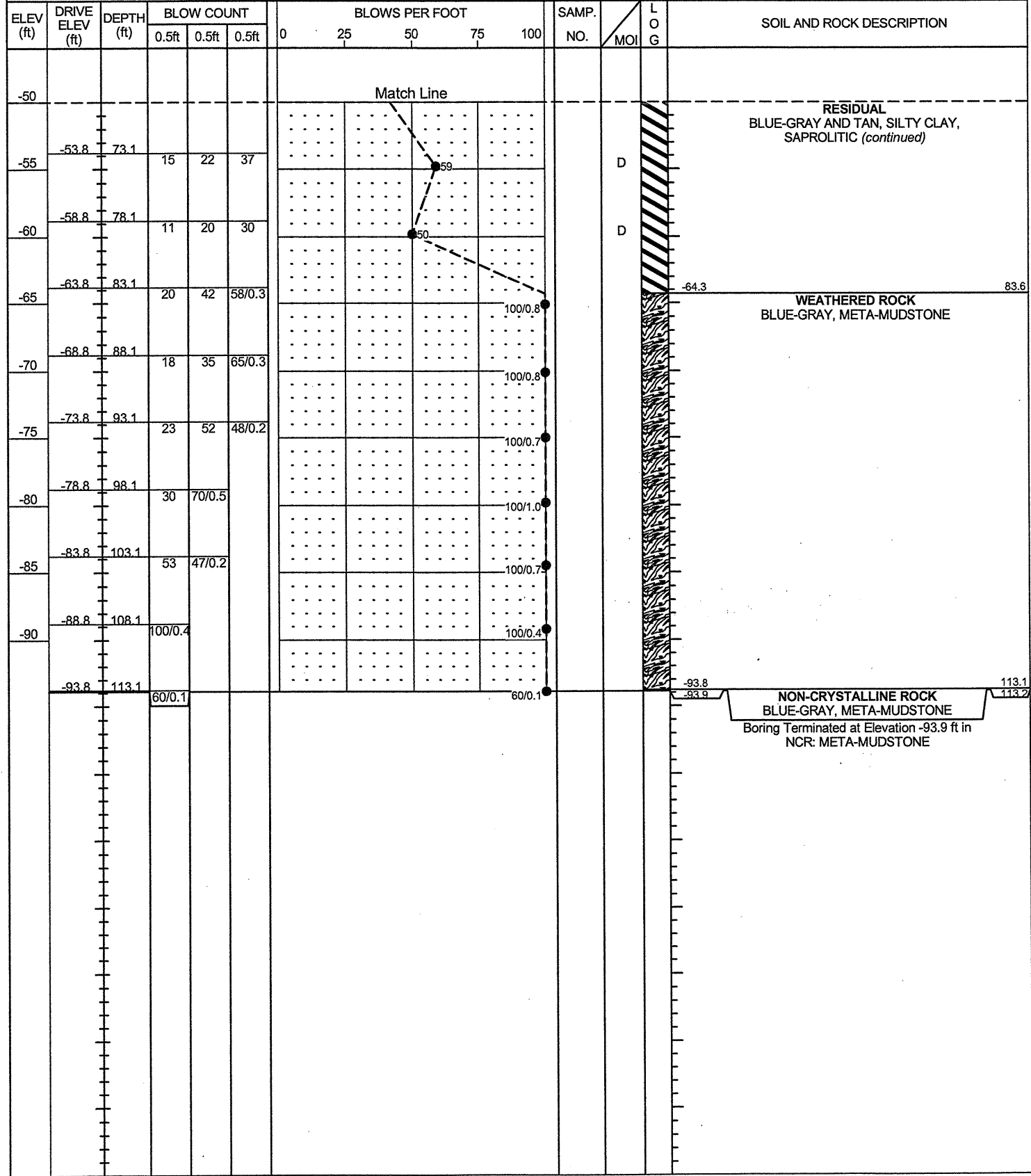
NCDOT CORE SINGLE B4091 GEO_BH_BRIDGE.GPJ NC_DOT_GDT 8/12/11

NCDOT GEOTECHNICAL ENGINEERING UNIT
BORELOG REPORT

WBS 33449.1.1	TIP B-4091	COUNTY CUMBERLAND	GEOLOGIST Howard, J.
SITE DESCRIPTION Bridge No. 85 on -L- (US 301/I-95 Bus. Loop) over -Y1- (SR 1738), -Y2- (SR 1741) and Cape Fear River			GROUND WTR (ft)
BORING NO. B4-A	STATION 52+27	OFFSET 56 ft LT	ALIGNMENT -L-
COLLAR ELEV. 19.3 ft	TOTAL DEPTH 113.2 ft	NORTHING 476,323	EASTING 2,043,322
DRILL RIG/HAMMER EFF./DATE MAC2425 CME-55 85% 09/02/2009		DRILL METHOD Mud Rotary	HAMMER TYPE Automatic
DRILLER Contract Driller	START DATE 06/27/11	COMP. DATE 06/28/11	SURFACE WATER DEPTH 9.5ft



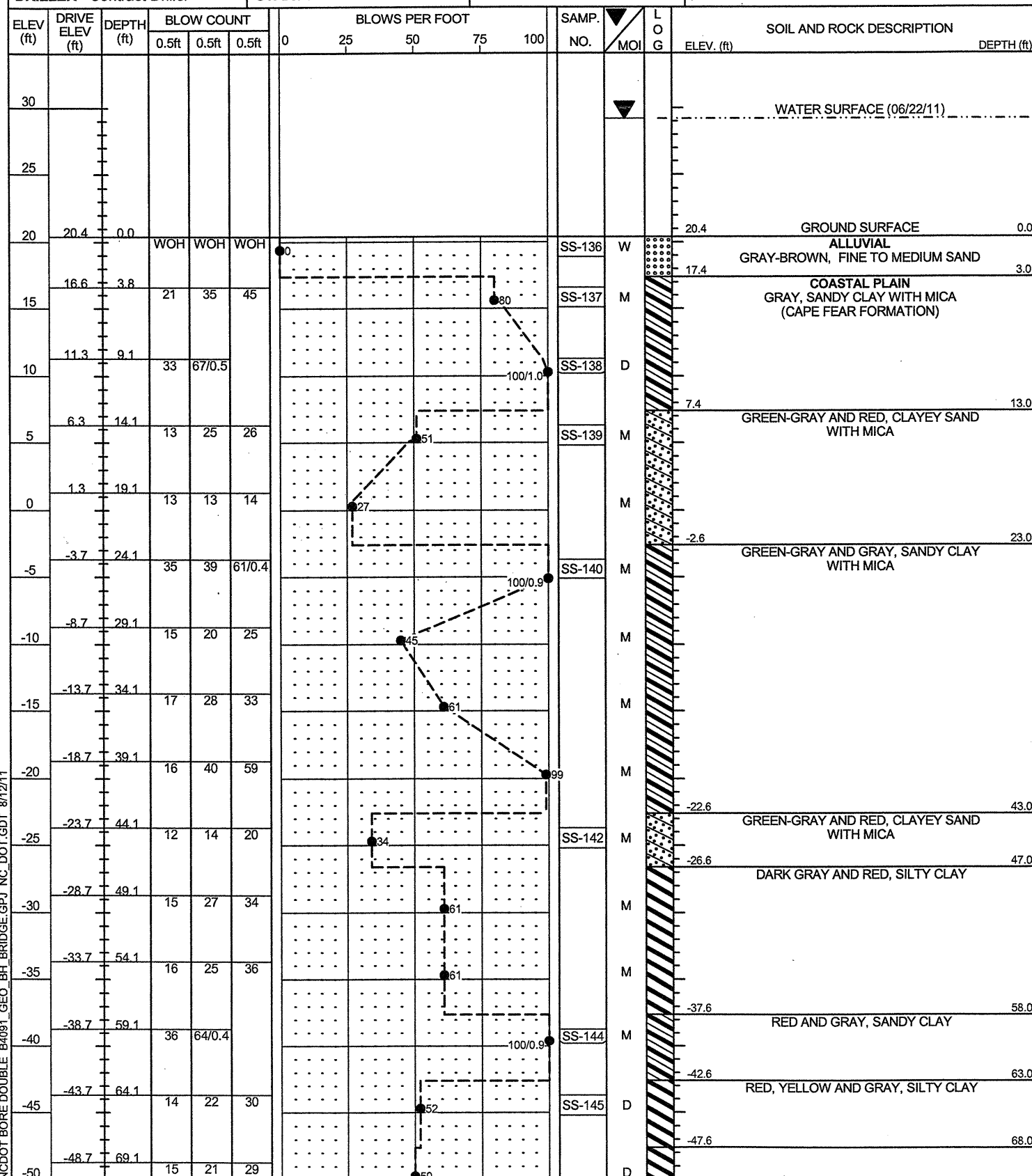
WBS 33449.1.1	TIP B-4091	COUNTY CUMBERLAND	GEOLOGIST Howard, J.
SITE DESCRIPTION Bridge No. 85 on -L- (US 301/I-95 Bus. Loop) over -Y1- (SR 1738), -Y2- (SR 1741) and Cape Fear River			GROUND WTR (ft)
BORING NO. B4-A	STATION 52+27	OFFSET 56 ft LT	ALIGNMENT -L-
COLLAR ELEV. 19.3 ft	TOTAL DEPTH 113.2 ft	NORTHING 476,323	EASTING 2,043,322
DRILL RIG/HAMMER EFF./DATE MAC2425 CME-55 85% 09/02/2009		DRILL METHOD Mud Rotary	HAMMER TYPE Automatic
DRILLER Contract Driller	START DATE 06/27/11	COMP. DATE 06/28/11	SURFACE WATER DEPTH 9.5ft



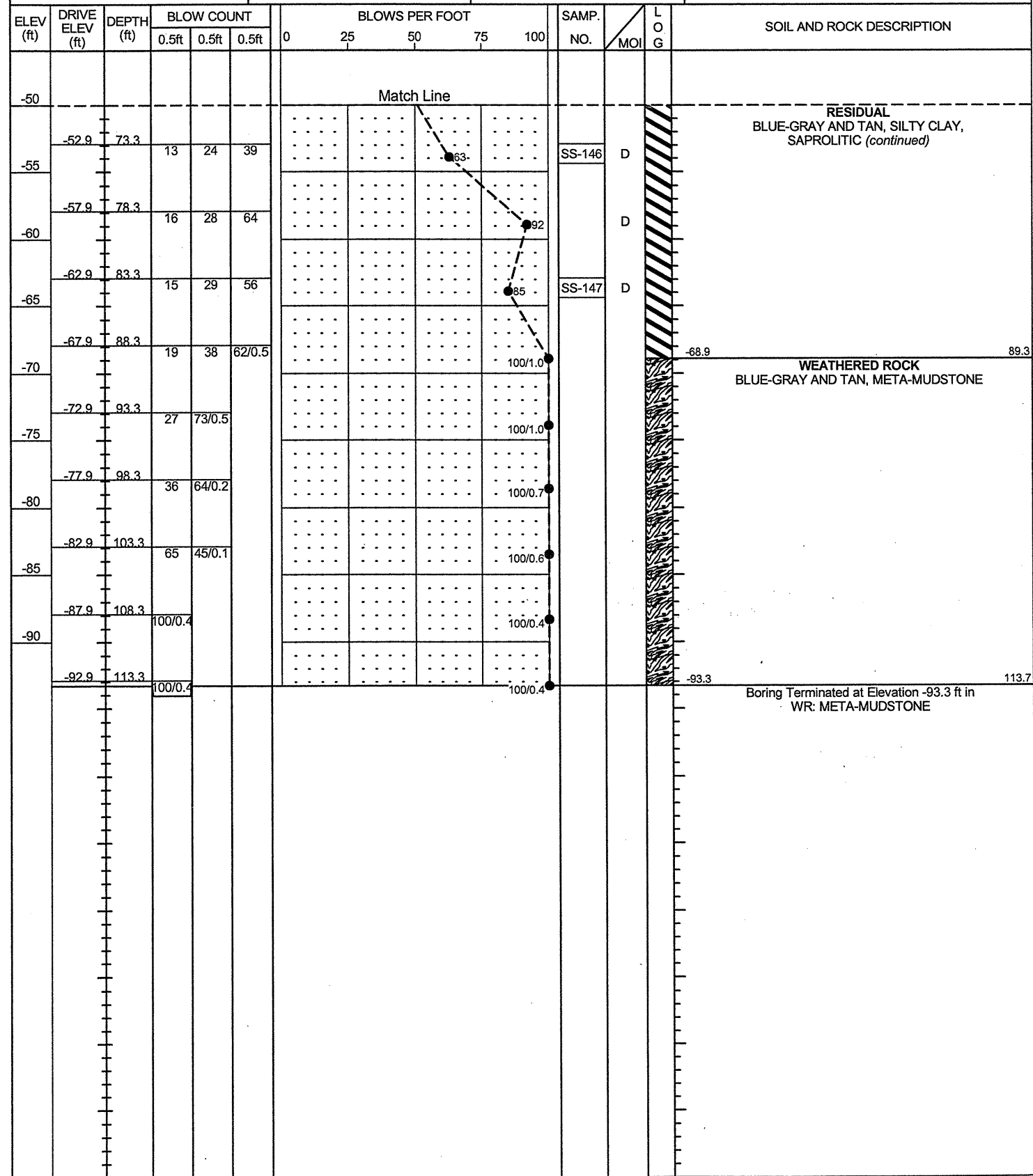
NCDOT BORE DOUBLE B4091_GEO_BH_BRIDGE.GPJ NC_DOT_GDT_8/12/11

NCDOT GEOTECHNICAL ENGINEERING UNIT
BORELOG REPORT

WBS 33449.1.1	TIP B-4091	COUNTY CUMBERLAND	GEOLOGIST Howard, J.
SITE DESCRIPTION Bridge No. 85 on -L- (US 301/I-95 Bus. Loop) over -Y1- (SR 1738), -Y2- (SR 1741) and Cape Fear River			GROUND WTR (ft)
BORING NO. B4-B	STATION 52+84	OFFSET 35 ft RT	ALIGNMENT -L-
COLLAR ELEV. 20.4 ft	TOTAL DEPTH 113.7 ft	NORTHING 476,250	EASTING 2,043,400
DRILL RIG/HAMMER EFF./DATE MAC2425 CME-55 85% 09/02/2009		DRILL METHOD Mud Rotary	HAMMER TYPE Automatic
DRILLER Contract Driller	START DATE 06/22/11	COMP. DATE 06/23/11	SURFACE WATER DEPTH 8.8ft



WBS 33449.1.1	TIP B-4091	COUNTY CUMBERLAND	GEOLOGIST Howard, J.
SITE DESCRIPTION Bridge No. 85 on -L- (US 301/I-95 Bus. Loop) over -Y1- (SR 1738), -Y2- (SR 1741) and Cape Fear River			GROUND WTR (ft)
BORING NO. B4-B	STATION 52+84	OFFSET 35 ft RT	ALIGNMENT -L-
COLLAR ELEV. 20.4 ft	TOTAL DEPTH 113.7 ft	NORTHING 476,250	EASTING 2,043,400
DRILL RIG/HAMMER EFF./DATE MAC2425 CME-55 85% 09/02/2009		DRILL METHOD Mud Rotary	HAMMER TYPE Automatic
DRILLER Contract Driller	START DATE 06/22/11	COMP. DATE 06/23/11	SURFACE WATER DEPTH 8.8ft



NCDOT BORE DOUBLE B4091_GEO_BH_BRIDGE.GPJ NC_DOT.GDT 8/12/11



**NCDOT GEOTECHNICAL ENGINEERING UNIT
CORE BORING REPORT**

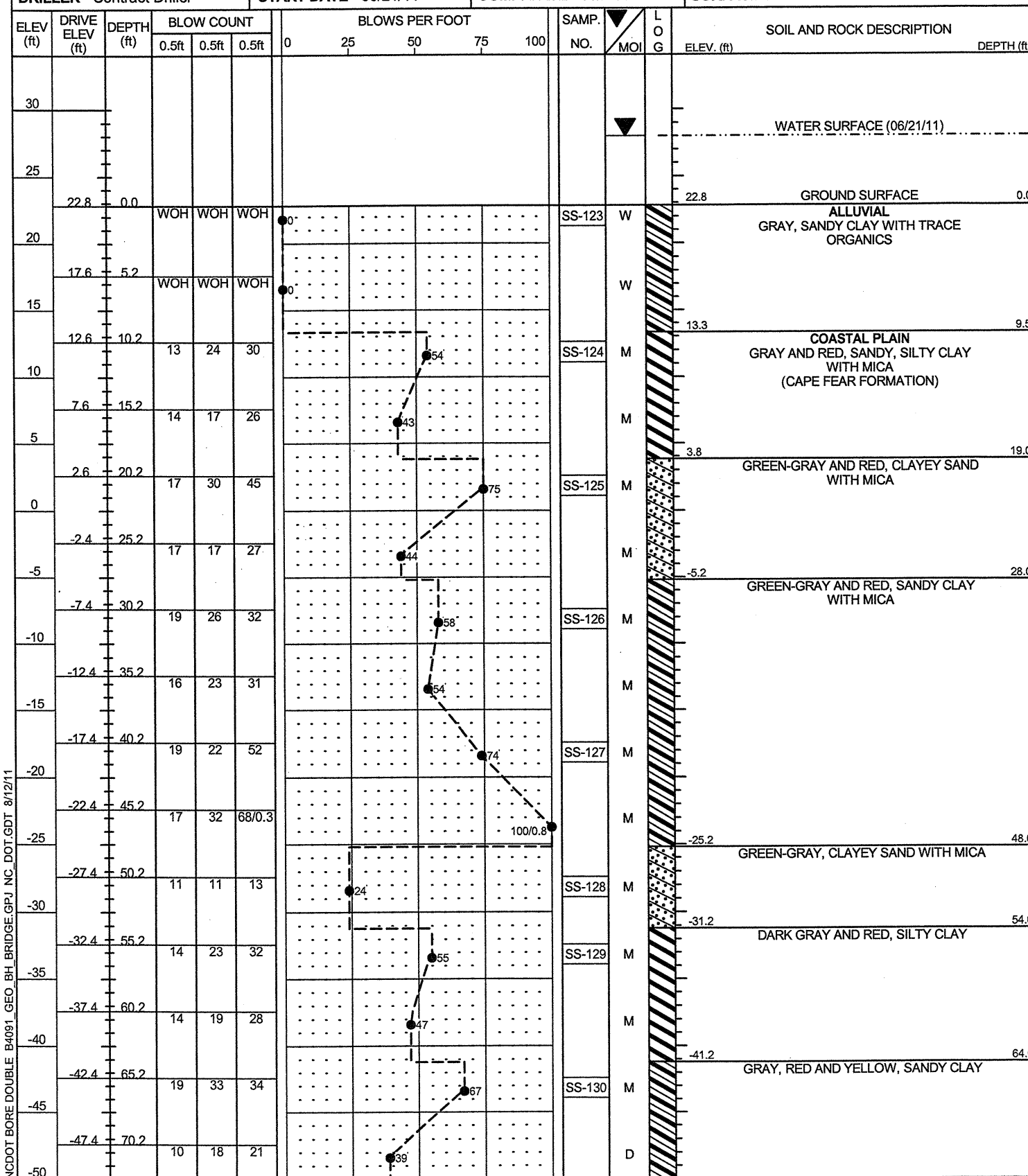
WBS 33449.1.1	TIP B-4091	COUNTY CUMBERLAND	GEOLOGIST Howard, J.
SITE DESCRIPTION Bridge No. 85 on -L- (US 301/I-95 Bus. Loop) over -Y1- (SR 1738), -Y2- (SR 1741) and Cape Fear River			GROUND WTR (ft)
BORING NO. B5-A	STATION 54+10	OFFSET 38 ft LT	ALIGNMENT -L-
COLLAR ELEV. 26.3 ft	TOTAL DEPTH 114.2 ft	NORTHING 476,353	EASTING 2,043,503
DRILL RIG/HAMMER EFF./DATE MAC2425 CME-55 85% 09/02/2009		DRILL METHOD Mud Rotary	HAMMER TYPE Automatic
DRILLER Contract Driller	START DATE 06/28/11	COMP. DATE 06/30/11	SURFACE WATER DEPTH 4.4ft
CORE SIZE N/A	TOTAL RUN 19.4 ft		

ELEV (ft)	RUN ELEV (ft)	DEPTH (ft)	RUN (ft)	DRILL RATE (Min/ft)	RUN		SAMP. NO.	STRATA		LOG	DESCRIPTION AND REMARKS	DEPTH (ft)
					REC. (ft) %	RQD (ft) %		REC. (ft) %	RQD (ft) %			
-63.2											Begin Coring @ 89.5 ft	
-65	-63.2	89.5	3.0	2:30 3:00	(3.0) 100%	N/A		(6.0) 85%	N/A		WEATHERED ROCK BLUE-GRAY AND TAN, SEVERELY WEATHERED, VERY SOFT, VERY CLOSELY TO CLOSELY FRACTURED, META-MUDSTONE	89.5
	-66.2 -67.1	92.5 93.4		2:45 N=100/0.9								
-70			4.1	2:00 2:15 2:30 2:25	(3.0) 73%	N/A						
	-71.2 -72.7	97.5 99.0		0:05/0.1 N=96			SS-91	(0.0) 0%	N/A		RESIDUAL BLUE-GRAY AND TAN, SILTY CLAY, SAPROLITIC	96.6
-75			4.0	3:15 3:30 3:30 3:15	(2.1) 53%	N/A		(7.4) 56%	N/A		WEATHERED ROCK BLUE-GRAY AND TAN, SEVERELY WEATHERED, VERY SOFT, VERY CLOSELY TO CLOSELY FRACTURED, META-MUDSTONE	100.9
	-76.7 -77.7	103.0 104.0		N=100/1.0								
-80			4.0	3:45 3:15 3:15 5:45	(1.8) 45%	N/A						
	-81.7 -82.4	108.0 108.7		N=100/0.7			RS-2					
-85			4.3	4:45 3:45 2:30 2:45	(3.5) 81%	N/A						
	-86.7	113.0		3:00/0.3 N=100/0.7			RS-3					
											Boring Terminated at Elevation -87.9 ft in WR: META-MUDSTONE	114.2

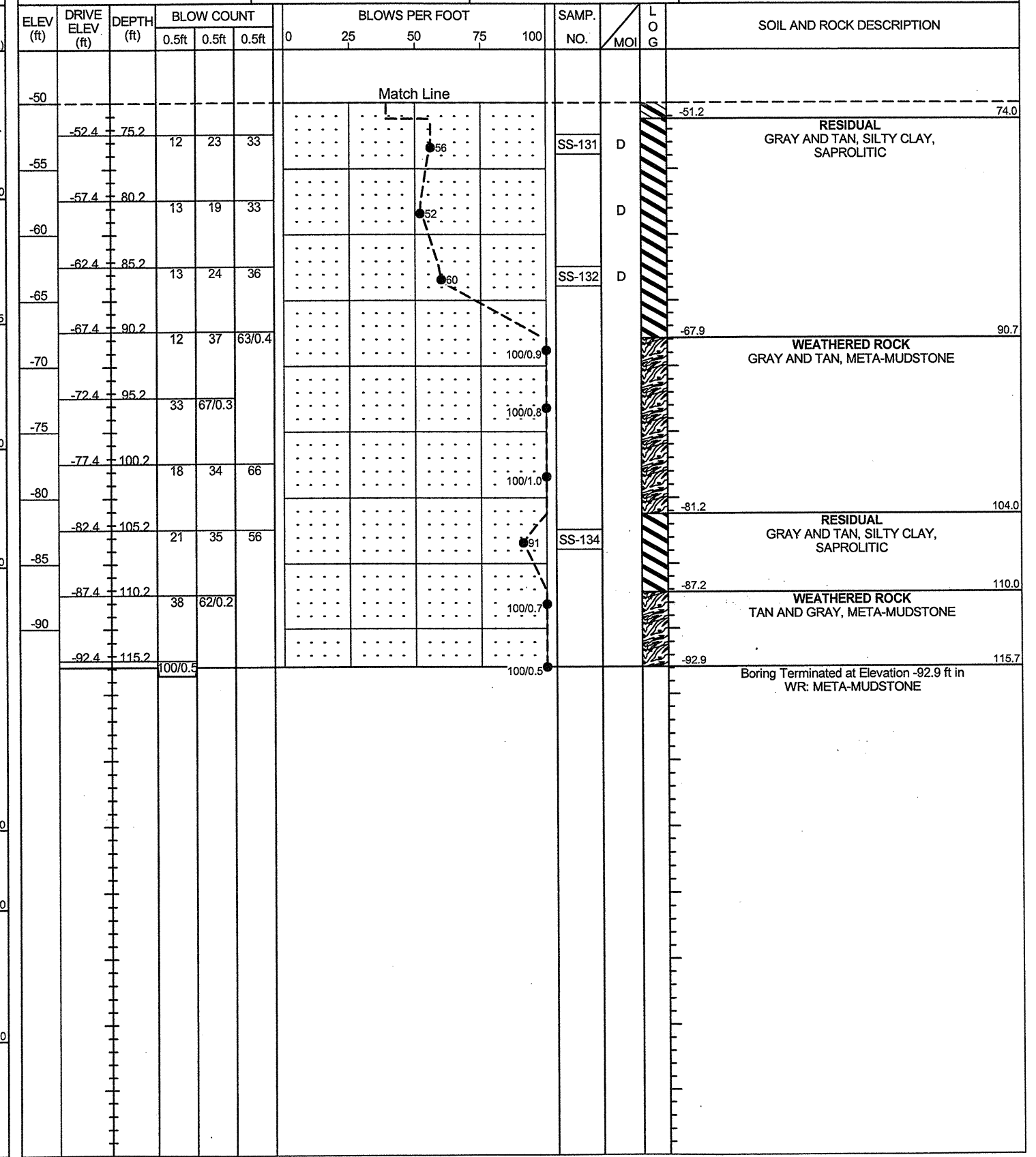
NCDOT CORE SINGLE B4091 GEO_BH_BRIDGE.GPJ NC_DOT.GDT 8/12/11

NCDOT GEOTECHNICAL ENGINEERING UNIT
BORELOG REPORT

WBS 33449.1.1	TIP B-4091	COUNTY CUMBERLAND	GEOLOGIST Howard, J.
SITE DESCRIPTION Bridge No. 85 on -L- (US 301/I-95 Bus. Loop) over -Y1- (SR 1738), -Y2- (SR 1741) and Cape Fear River			GROUND WTR (ft)
BORING NO. B5-B	STATION 54+02	OFFSET 73 ft RT	ALIGNMENT -L-
COLLAR ELEV. 22.8 ft	TOTAL DEPTH 115.7 ft	NORTHING 476,243	EASTING 2,043,524
DRILL RIG/HAMMER EFF./DATE MAC2425 CME-55 85% 09/02/2009		DRILL METHOD Mud Rotary	HAMMER TYPE Automatic
DRILLER Contract Driller	START DATE 06/21/11	COMP. DATE 06/22/11	SURFACE WATER DEPTH 5.2ft



WBS 33449.1.1	TIP B-4091	COUNTY CUMBERLAND	GEOLOGIST Howard, J.
SITE DESCRIPTION Bridge No. 85 on -L- (US 301/I-95 Bus. Loop) over -Y1- (SR 1738), -Y2- (SR 1741) and Cape Fear River			GROUND WTR (ft)
BORING NO. B5-B	STATION 54+02	OFFSET 73 ft RT	ALIGNMENT -L-
COLLAR ELEV. 22.8 ft	TOTAL DEPTH 115.7 ft	NORTHING 476,243	EASTING 2,043,524
DRILL RIG/HAMMER EFF./DATE MAC2425 CME-55 85% 09/02/2009		DRILL METHOD Mud Rotary	HAMMER TYPE Automatic
DRILLER Contract Driller	START DATE 06/21/11	COMP. DATE 06/22/11	SURFACE WATER DEPTH 5.2ft



NCDOT BORE DOUBLE B4091_GEO_BH_BRIDGE.GPJ_NC_DOT_GDT_8/12/11

NCDOT GEOTECHNICAL ENGINEERING UNIT
BORELOG REPORT

WBS 33449.1.1	TIP B-4091	COUNTY CUMBERLAND	GEOLOGIST Howard, J.
SITE DESCRIPTION Bridge No. 85 on -L- (US 301/I-95 Bus. Loop) over -Y1- (SR 1738), -Y2- (SR 1741) and Cape Fear River			GROUND WTR (ft) 0 HR. N/A
BORING NO. B6-A	STATION 55+42	OFFSET 39 ft LT	ALIGNMENT -L-
COLLAR ELEV. 57.0 ft	TOTAL DEPTH 100.2 ft	NORTHING 476,388	EASTING 2,043,630
DRILL RIG/HAMMER EFF./DATE MAC6893 CME-55 88% 02/10/2010		DRILL METHOD Mud Rotary	HAMMER TYPE Automatic
DRILLER Contract Driller	START DATE 06/15/11	COMP. DATE 06/15/11	SURFACE WATER DEPTH N/A

ELEV (ft)	DRIVE ELEV (ft)	DEPTH (ft)	BLOW COUNT			BLOWS PER FOOT					SAMP. NO.	LOG	SOIL AND ROCK DESCRIPTION	DEPTH (ft)
			0.5ft	0.5ft	0.5ft	0	25	50	75	100				
60														
	57.0	0.0											GROUND SURFACE	0.0
55	53.3	3.7	7	6	5						SS-97	M	ALLUVIAL GRAY, SILTY, FINE SAND	3.0
	48.3	8.7	3	4	4						SS-98	W	GRAY, SILTY CLAY WITH TRACE ORGANICS	
50	43.3	13.7	WOH	1	3						SS-99	W		
45	38.3	18.7	WOH	WOH	WOH						SS-100	W	GRAY, SILTY CLAY	13.7
40	33.3	23.7									SS-101	W	COASTAL PLAIN GRAY AND RED, SILTY CLAY (CAPE FEAR FORMATION)	17.5
35	28.3	28.7									SS-102	M	GREEN-GRAY AND RED, SANDY CLAY	22.0
30	23.3	33.7									SS-103	M		
25	18.3	38.7									SS-104	M		
20	13.3	43.7									SS-106	W	GREEN-GRAY AND RED, CLAYEY SILTY SAND	52.0
15	8.3	48.7									SS-107	M	GRAY AND RED, FINE TO COARSE, SANDY CLAY	59.7
10	3.3	53.7									SS-109	M		
5	-1.7	58.7									SS-110	M	GRAY TO GREEN-GRAY, CLAYEY SAND	72.0
0	-6.7	63.7												
-5	-11.7	68.7												
-10	-16.7	73.7												
-15														
-20														

WBS 33449.1.1	TIP B-4091	COUNTY CUMBERLAND	GEOLOGIST Howard, J.
SITE DESCRIPTION Bridge No. 85 on -L- (US 301/I-95 Bus. Loop) over -Y1- (SR 1738), -Y2- (SR 1741) and Cape Fear River			GROUND WTR (ft) 0 HR. N/A
BORING NO. B6-A	STATION 55+42	OFFSET 39 ft LT	ALIGNMENT -L-
COLLAR ELEV. 57.0 ft	TOTAL DEPTH 100.2 ft	NORTHING 476,388	EASTING 2,043,630
DRILL RIG/HAMMER EFF./DATE MAC6893 CME-55 88% 02/10/2010		DRILL METHOD Mud Rotary	HAMMER TYPE Automatic
DRILLER Contract Driller	START DATE 06/15/11	COMP. DATE 06/15/11	SURFACE WATER DEPTH N/A

ELEV (ft)	DRIVE ELEV (ft)	DEPTH (ft)	BLOW COUNT			BLOWS PER FOOT					SAMP. NO.	LOG	SOIL AND ROCK DESCRIPTION	DEPTH (ft)
			0.5ft	0.5ft	0.5ft	0	25	50	75	100				
-20														
	-21.7	78.7											GRAY TO GREEN-GRAY, CLAYEY SAND (continued)	
-25	-26.7	83.7												
-30	-31.7	88.7											GRAY AND RED, FINE TO COARSE, SANDY CLAY	87.0
-35	-36.7	93.7									SS-112	W	GRAY, CLAYEY SILT	92.0
-40	-41.7	98.7												
													Boring Terminated at Elevation -43.2 ft in Coastal Plain, CLAYEY SILT (Cape Fear Formation)	100.2

NCDOT BORE DOUBLE B4091_GEO_BH_BRIDGE.GPJ NC_DOT_GDT_8/12/11



NCDOT GEOTECHNICAL ENGINEERING UNIT

BORELOG REPORT

WBS 33449.1.1	TIP B-4091	COUNTY CUMBERLAND	GEOLOGIST Howard, J.
SITE DESCRIPTION Bridge No. 85 on -L- (US 301/I-95 Bus. Loop) over -Y1- (SR 1738), -Y2- (SR 1741) and Cape Fear River			GROUND WTR (ft)
BORING NO. EB2-A	STATION 57+08	OFFSET 35 ft LT	ALIGNMENT -L-
COLLAR ELEV. 94.0 ft	TOTAL DEPTH 60.3 ft	NORTHING 476,426	EASTING 2,043,792
DRILL RIG/HAMMER EFF./DATE MAC6893 CME-55 88% 02/10/2010		DRILL METHOD Mud Rotary	HAMMER TYPE Automatic
DRILLER Contract Driller	START DATE 06/15/11	COMP. DATE 06/16/11	SURFACE WATER DEPTH N/A

ELEV (ft)	DRIVE ELEV (ft)	DEPTH (ft)	BLOW COUNT			BLOWS PER FOOT					SAMP. NO.	MOI	LOG	SOIL AND ROCK DESCRIPTION	ELEV. (ft)	DEPTH (ft)
			0.5ft	0.5ft	0.5ft	0	25	50	75	100						
95	94.0	0.0	12	9	9									94.0	0.0	GROUND SURFACE
90	90.2	3.8	6	6	8						SS-89	D		ROADWAY EMBANKMENT TAN, ORANGE AND BROWN, SANDY SILT		
85	85.2	8.8	5	8	13						SS-90	M		COASTAL PLAIN TAN-ORANGE, CLAYEY SAND (TERTIARY UPLAND SEDIMENTS)	87.0	7.0
80	80.2	13.8	9	13	14						SS-91	W		TAN-ORANGE, SLIGHTLY SILTY, FINE TO COARSE SAND WITH GRAVEL	82.0	12.0
75	75.2	18.8	2	2	5						SS-92	M		COASTAL PLAIN GRAY AND GRAY-GREEN, SANDY CLAY (CAPE FEAR FORMATION)	77.0	17.0
70	70.2	23.8	5	8	11							M			67.0	27.0
65	65.2	28.8	11	17	26						SS-93	M		GRAY AND RED, GRAY-GREEN, AND BROWN, SANDY CLAY		
60	60.2	33.8	18	23	32							M				
55	55.2	38.8	25	49	51/0.4						SS-94	M				
50	50.2	43.8	18	23	30							M				
45	45.2	48.8	25	32	57						SS-95	M		DARK GREEN-GRAY AND RED, SANDY SILTY CLAY WITH MICA	47.0	47.0
40	40.2	53.8	18	16	20							M				
35	35.2	58.8	16	22	29						SS-96	M		GREEN-GRAY, SANDY SILT	37.0	57.0
														Boring Terminated at Elevation 33.7 ft in Coastal Plain, SANDY SILT (Cape Fear Formation)	33.7	60.3

WBS 33449.1.1	TIP B-4091	COUNTY CUMBERLAND	GEOLOGIST Howard, J.
SITE DESCRIPTION Bridge No. 85 on -L- (US 301/I-95 Bus. Loop) over -Y1- (SR 1738), -Y2- (SR 1741) and Cape Fear River			GROUND WTR (ft)
BORING NO. EB2-B	STATION 57+03	OFFSET 41 ft RT	ALIGNMENT -L-
COLLAR ELEV. 93.7 ft	TOTAL DEPTH 64.5 ft	NORTHING 476,351	EASTING 2,043,807
DRILL RIG/HAMMER EFF./DATE MAC6893 CME-55 88% 02/10/2010		DRILL METHOD Mud Rotary	HAMMER TYPE Automatic
DRILLER Contract Driller	START DATE 06/15/11	COMP. DATE 06/15/11	SURFACE WATER DEPTH N/A

ELEV (ft)	DRIVE ELEV (ft)	DEPTH (ft)	BLOW COUNT			BLOWS PER FOOT					SAMP. NO.	MOI	LOG	SOIL AND ROCK DESCRIPTION	ELEV. (ft)	DEPTH (ft)
			0.5ft	0.5ft	0.5ft	0	25	50	75	100						
95	93.7	0.0	3	4	4									93.7	0.0	GROUND SURFACE
90	89.9	3.8	WOH	6	4						SS-78	D		ROADWAY EMBANKMENT TAN AND BROWN, SANDY SILT		
85	84.9	8.8	7	7	8						SS-79	M		COASTAL PLAIN ORANGE-TAN, SLIGHTLY SILTY, FINE TO COARSE SAND (TERTIARY UPLAND SEDIMENTS)	86.7	7.0
80	79.9	13.8	8	10	11							W				
75	74.9	18.8	10	8	4						SS-80	W		TAN, SILTY SAND	76.7	17.0
70	69.9	23.8	6	6	9						SS-81	M		COASTAL PLAIN GREEN-GRAY, GRAY, RED AND BROWN, CLAYEY SILTY SAND (CAPE FEAR FORMATION)	71.7	22.0
65	64.9	28.8	7	16	19							M				
60	59.9	33.8	9	9	11						SS-82	M		GREEN-GRAY, GRAY, RED AND BROWN, CLAYEY SAND	61.7	32.0
55	54.9	38.8	16	26	36							M				
50	49.9	43.8	11	14	14						SS-83	M				
45	44.9	48.8	9	10	20							M				
40	39.9	53.8	11	15	19						SS-84	M		GREEN-GRAY, GRAY, RED AND BROWN, SANDY CLAY	41.7	52.0
35	34.9	58.8	14	17	21							M				
30	29.9	63.8	55	45/0.2							SS-85	M		GREEN-GRAY, SANDY SILT WITH MICA	31.7	62.0
														Boring Terminated at Elevation 29.2 ft in Coastal Plain, SANDY SILT (Cape Fear Formation)	29.2	64.5

NCDOT BORE DOUBLE B4091_GEO_BH_BRIDGE.GPJ NC_DOT.GDT 8/12/11

EB1-A

SOIL TEST RESULTS															
SAMPLE NO.	OFFSET	STATION	DEPTH INTERVAL	AASHTO CLASS.	L.L.	P.I.	% BY WEIGHT				% PASSING (SIEVES)			% MOISTURE	% ORGANIC
							C.SAND	F.SAND	SILT	CLAY	10	40	200		
SS-51	22 LT	47+13	0.8-1.7	A-1-b(0)	17	NP	74.8	14.4	4.8	6.1	90	40	11	-	-
SS-52	22 LT	47+13	3.7-5.2	A-4(3)	28	9	8.7	42.1	29.0	20.2	100	99	58	-	-
SS-53	22 LT	47+13	13.7-15.2	A-6(7)	34	15	6.5	37.0	28.2	28.3	100	100	64	-	-
SS-54	22 LT	47+13	18.7-20.2	A-4(1)	27	8	7.3	50.6	21.9	20.2	100	100	51	-	-
SS-55	22 LT	47+13	23.7-25.2	A-6(11)	36	17	4.7	30.7	34.3	30.3	100	99	72	-	-
SS-57	22 LT	47+13	48.7-50.2	A-2-6(0)	35	17	57.3	23.1	14.6	5.1	95	61	22	-	-
SS-59	22 LT	47+13	63.7-65.2	A-4(2)	24	10	28.9	25.5	37.5	8.1	100	82	52	-	-

EB1-B

SOIL TEST RESULTS															
SAMPLE NO.	OFFSET	STATION	DEPTH INTERVAL	AASHTO CLASS.	L.L.	P.I.	% BY WEIGHT				% PASSING (SIEVES)			% MOISTURE	% ORGANIC
							C.SAND	F.SAND	SILT	CLAY	10	40	200		
SS-40	78 RT	47+22	0.0-1.5	A-4(0)	26	6	23.7	35.8	22.3	18.2	100	90	47	-	-
SS-41	78 RT	47+22	8.9-10.4	A-6(2)	27	11	16.4	39.2	24.2	20.2	100	98	50	-	-
SS-42	78 RT	47+22	13.9-15.4	A-2-6(0)	34	15	41.1	40.4	10.4	8.1	98	78	21	-	-
SS-43	78 RT	47+22	23.9-25.4	A-2-6(0)	39	16	63.4	19.5	12.0	5.1	96	54	19	-	-
SS-46	78 RT	47+22	53.9-55.4	A-2-6(0)	32	13	62.4	24.5	10.1	3.0	93	55	15	-	-

EB1-A

SOIL TEST RESULTS															
SAMPLE NO.	OFFSET	STATION	DEPTH INTERVAL	AASHTO CLASS.	L.L.	P.I.	% BY WEIGHT				% PASSING (SIEVES)			% MOISTURE	% ORGANIC
							C.SAND	F.SAND	SILT	CLAY	10	40	200		
SS-13	57 LT	48+57	0.0-1.5	A-2-4(0)	17	NP	52.7	33.7	8.5	5.1	98	64	16	-	-
SS-14	57 LT	48+57	3.8-5.3	A-6(14)	38	21	2.0	31.5	32.1	34.3	100	99	75	-	-
SS-15	57 LT	48+57	13.8-15.3	A-7-6(12)	51	28	20.8	32.1	32.9	14.1	100	90	55	-	-
SS-16	57 LT	48+57	18.8-20.3	A-6(2)	29	14	35.7	27.1	16.1	21.2	100	84	40	-	-
SS-17	57 LT	48+57	28.8-30.3	A-7-6(24)	47	28	4.6	16.2	42.8	36.4	100	99	83	-	-
SS-18	57 LT	48+57	33.8-35.3	A-2-6(0)	39	17	60.7	20.4	14.8	4.0	100	58	22	-	-
SS-19	57 LT	48+57	43.8-45.3	A-7-6(26)	54	29	3.6	20.0	48.1	28.3	100	98	84	-	-
SS-20	57 LT	48+57	53.8-55.3	A-6(16)	38	24	4.0	29.3	42.4	24.2	100	98	74	-	-
SS-21	57 LT	48+57	63.8-65.3	A-2-6(1)	31	15	52.6	23.3	17.0	7.1	95	62	26	-	-
SS-22	57 LT	48+57	68.8-70.3	A-2-7(1)	46	27	62.6	18.2	10.1	9.1	99	57	21	-	-

EB1-B

SOIL TEST RESULTS															
SAMPLE NO.	OFFSET	STATION	DEPTH INTERVAL	AASHTO CLASS.	L.L.	P.I.	% BY WEIGHT				% PASSING (SIEVES)			% MOISTURE	% ORGANIC
							C.SAND	F.SAND	SILT	CLAY	10	40	200		
SS-1	40 RT	48+57	0.0-1.5	A-4(2)	27	7	19.2	25.7	37.0	18.2	99	90	60	-	-
SS-2	40 RT	48+57	3.9-4.4	A-6(10)	32	15	2.4	30.7	40.6	26.3	100	99	77	-	-
SS-3	40 RT	48+57	8.9-10.4	A-6(7)	30	14	10.3	33.1	32.3	24.2	100	96	65	-	-
SS-4	40 RT	48+57	18.9-20.4	A-2-6(0)	30	12	40.7	37.7	14.5	7.1	99	76	25	-	-
SS-5	40 RT	48+57	28.9-30.4	A-2-6(0)	34	17	67.0	16.2	11.8	5.1	100	57	19	-	-
SS-6	40 RT	48+57	38.9-40.4	A-6(10)	37	24	26.1	19.6	36.2	18.2	100	85	58	-	-
SS-7	40 RT	48+57	43.9-45.4	A-2-7(2)	43	27	57.3	14.8	12.7	15.2	98	65	29	-	-
SS-8	40 RT	48+57	53.9-55.4	A-6(13)	35	20	9.3	19.8	42.6	28.3	100	95	75	-	-
SS-9	40 RT	48+57	63.9-65.4	A-2-6(1)	37	17	55.7	19.8	15.5	9.1	97	60	27	-	-
SS-10	40 RT	48+57	73.9-75.4	A-7-6(10)	46	24	1.8	50.5	29.5	18.2	100	100	54	-	-
SS-11	40 RT	48+57	83.9-85.4	A-6(12)	37	19	3.0	35.6	43.2	18.2	100	98	73	-	-
SS-12	40 RT	48+57	93.9-94.4	A-7-6(32)	47	33	0.6	11.7	45.3	42.4	100	100	94	-	-

EB2-A

SOIL TEST RESULTS															
SAMPLE NO.	OFFSET	STATION	DEPTH INTERVAL	AASHTO CLASS.	L.L.	P.I.	% BY WEIGHT				% PASSING (SIEVES)			% MOISTURE	% ORGANIC
							C.SAND	F.SAND	SILT	CLAY	10	40	200		
SS-23	44 LT	49+90	0.0-1.5	A-4(1)	27	7	19.6	33.5	32.7	14.1	98	87	54	-	-
SS-24	44 LT	49+90	9.0-10.5	A-4(0)	22	4	7.5	47.9	28.5	16.2	100	100	52	-	-
SS-25	44 LT	49+90	14.0-15.1	A-2-6(1)	31	15	37.2	29.9	12.7	20.2	84	75	30	-	-
SS-27	44 LT	49+90	34.0-35.5	A-2-6(0)	33	12	41.6	32.5	21.8	4.0	95	72	29	-	-
SS-28	44 LT	49+90	49.0-50.5	A-2-7(5)	55	40	57.9	12.7	10.2	19.2	97	61	30	-	-
SS-29	44 LT	49+90	54.0-55.5	A-7-6(40)	60	38	1.4	8.7	43.4	46.5	100	99	94	-	-
SS-30	44 LT	49+90	64.0-65.5	A-2-6(1)	40	22	64.8	13.2	13.8	8.1	99	52	24	-	-

EB2-B

SOIL TEST RESULTS															
SAMPLE NO.	OFFSET	STATION	DEPTH INTERVAL	AASHTO CLASS.	L.L.	P.I.	% BY WEIGHT				% PASSING (SIEVES)			% MOISTURE	% ORGANIC
							C.SAND	F.SAND	SILT	CLAY	10	40	200		
SS-31	40 RT	49+98	0.0-1.5	A-2-4(0)	20	1	31.7	36.2	24.0	8.1	90	74	35	-	-
SS-32	40 RT	49+98	9.0-10.5	A-4(0)	19	NP	21.4	49.3	21.2	8.1	100	97	36	-	-
SS-33	40 RT	49+98	14.4-15.5	A-2-6(3)	40	25	21.0	27.5	17.2	34.3	61	54	34	-	-
SS-34	40 RT	49+98	24.0-25.0	A-2-4(0)	32	8	61.6	24.4	9.9	4.0	95	55	17	-	-
SS-35	40 RT	49+98	29.0-30.5	A-2-4(0)	29	8	57.5	23.8	13.6	5.1	100	64	22	-	-
SS-37	40 RT	49+98	49.0-50.5	A-2-6(0)	35	16	60.6	20.9	11.4	7.1	97	63	21	-	-
SS-39	40 RT	49+98	69.0-70.5	A-2-7(0)	46	23	71.3	12.9	10.7	5.1	98	50	18	-	-

B3-A

SOIL TEST RESULTS															
SAMPLE NO.	OFFSET	STATION	DEPTH INTERVAL	AASHTO CLASS.	L.L.	P.I.	% BY WEIGHT				% PASSING (SIEVES)			% MOISTURE	% ORGANIC
							C.SAND	F.SAND	SILT	CLAY	10	40	200		
SS-152	48 LT	51+64	4.8-6.3	A-6(6)	30	11	7.8	25.4	32.6	34.2	100	99	72	-	-
SS-153	48 LT	51+64	9.8-11.3	A-6(12)	38	21	10.1	31.2	38.6	20.1	100	96	67	-	-
SS-154	48 LT	51+64	14.8-16.3	A-6(4)	34	19	24.5	35.2	26.2	14.1	99	87	44	-	-
SS-155	48 LT	51+64	24.8-26.3	A-2-4(0)	39	NP	59.0	29.6	9.5	2.0	95	56	15	-	-
SS-156	48 LT	51+64	34.8-36.3	A-6(14)	40	22	8.9	31.6	43.5	16.1	100	96	72	-	-
SS-157	48 LT	51+64	49.8-61.3	A-6(3)	38	16	10.5	57.1	24.3	8.0	100	97	42	-	-
SS-158	48 LT	51+64	54.8-56.3	A-7-6(30)	48	30	1.6	5.8	42.3	50.3	100	99	95	-	-
SS-159	48 LT	51+64	64.8-66.3	A-6(22)	40	24	2.8	8.7	46.3	42.3	100	98	92	-	-
SS-161	48 LT	51+64	79.8-81.3	A-7-5(31)	63	23	1.4	1.8	50.5	46.3	100	99	98	-	-

B3-B

SOIL TEST RESULTS															
SAMPLE NO.	OFFSET	STATION	DEPTH INTERVAL	AASHTO CLASS.	L.L.	P.I.	% BY WEIGHT				% PASSING (SIEVES)			% MOISTURE	% ORGANIC
							C.SAND	F.SAND	SILT	CLAY	10	40	200		
SS-195	50 RT	51+41	0.0-1.5	A-6(13)	37	16	2.6	16.9	38.3	42.2	100	100	85	-	-
SS-196	50 RT	51+41	4.2-5.7	A-4(0)	26	3	18.3	50.1	17.6	14.1	95	92	36	-	-
SS-197	50 RT	51+41	9.2-10.7	A-2-4(0)	31	9	36.0	38.8	19.2	6.0	100	88	30	-	-
SS-198	50 RT	51+41	19.2-20.7	A-2-5(0)	42	NP	57.5	32.2	8.3	2.0	98	63	14	-	-
SS-200	50 RT	51+41	39.2-40.7	A-6(2)	36	13	10.3	53.5	20.2	16.1	100	99	42	-	-
SS-201	50 RT	51+41	49.2-50.7	A-7-6(34)	54	31	0.8	3.8	45.1	50.3	100	100	97	-	-
SS-202	50 RT	51+41	59.2-60.7	A-7-6(25)	44	26	3.2	6.4	38.1	52.3	100	98	93	-	-
SS-203	50 RT	51+41	69.2-70.7	A-7-5(34)	66	29	2.6	5.0	52.2	40.2	100	99	94	-	-

B4-A

SOIL TEST RESULTS															
SAMPLE NO.	OFFSET	STATION	DEPTH INTERVAL	AASHTO CLASS.	L.L.	P.I.	% BY WEIGHT				% PASSING (SIEVES)			% MOISTURE	% ORGANIC
							C.SAND	F.SAND	SILT	CLAY	10	40	200		
SS-166	56 LT	52+27	3.1-4.1	A-2-6(1)	29	15	43.7	25.4	16.9	14.1	85	59	30	-	-
SS-168	56 LT	52+27	18.1-19.6	A-2-6(0)	35	13	56.5	33.3	6.1	4.1	98	64	14	-	-
SS-169	56 LT	52+27	28.1-29.6	A-7-6(14)	42	20	10.5	23.5	43.9	22.1	100	95	74	-	-
SS-171	56 LT	52+27	48.1-49.6	A-7-6(34)	52	33	1.6	5.8	32.2	60.4	100	99	95	-	-
SS-172	56 LT	52+27	58.1-59.6	A-6(18)	35	19	1.8	6.0	41.9	50.3	100	99	95	-	-

B4-B

SOIL TEST RESULTS															
SAMPLE NO.	OFFSET	STATION	DEPTH INTERVAL	AASHTO CLASS.	L.L.	P.I.	% BY WEIGHT				% PASSING (SIEVES)			% MOISTURE	% ORGANIC
							C.SAND	F.SAND	SILT	CLAY	10	40	200		
SS-136	35 RT	52+84	0.0-1.5	A-3(0)	17	NP	82.3	15.3	2.4	0.0	100	65	3	-	-
SS-137	35 RT	52+84	3.8-5.3	A-6(1)	29	11	2.6	61.2	18.1	18.1	100	99	42	-	-
SS-138	35 RT	52+84	9.1-10.6	A-6(2)	32	14	6.0	56.3	21.5	16.1	100	98	42	-	-
SS-139	35 RT	52+84	14.1-15.6	A-2-6(0)	35	15	48.7	35.4	11.9	4.0	100	74	22	-	-
SS-140	35 RT	52+84	24.1-25.6	A-6(11)	32	15	2.4	31.6	47.9	18.1	100	99	81	-	-

B4-B (CONTINUED)

SOIL TEST RESULTS															
SAMPLE NO.	OFFSET	STATION	DEPTH INTERVAL	AASHTO CLASS.	L.L.	P.I.	% BY WEIGHT				% PASSING (SIEVES)			% MOISTURE	% ORGANIC
							C.SAND	F.SAND	SILT	CLAY	10	40	200		
SS-142	35 RT	52+84	44.1-45.6	A-2-7(2)	41	21	34.6	44.9	16.5	4.0	98	82	31	-	-
SS-144	35 RT	52+84	59.1-60.6	A-6(14)	36	19	10.9	9.9	41.0	38.2	97	90	80	-	-
SS-145	35 RT	52+84	64.1-65.6	A-7-5(35)	62	31	2.8	3.8	39.0	54.3	100	98	95	-	-
SS-146	35 RT	52+84	73.3-74.8	A-7-5(32)	63	25	1.4	2.6	57.7	38.2	100	99	97	-	-
SS-147	35 RT	52+84	83.3-84.8	A-7-5(28)	60	22	1.8	3.2	58.8	36.2	100	99	96	-	-

B5-A

SOIL TEST RESULTS															
SAMPLE NO.	OFFSET	STATION	DEPTH INTERVAL	AASHTO CLASS.	L.L.	P.I.	% BY WEIGHT				% PASSING (SIEVES)			% MOISTURE	% ORGANIC
							C.SAND	F.SAND	SILT	CLAY	10	40	200		
SS-179	38 LT	54+10	0.0-1.5	A-2-4(0)	27	NP	5.0	70.6	16.3	8.0	100	99	30	-	-
SS-180	38 LT	54+10	8.7-10.2	A-6(5)	40	19	7.8	52.7	25.4	14.1	93	89	45	-	-
SS-181	38 LT	54+10	18.7-20.2	A-2-4(0)	33	5	37.2	43.7	13.1	6.0	100	87	25	-	-
SS-182	38 LT	54+10	28.7-30.2	A-7-6(24)	42	27	3.2	12.5	42.1	42.3	100	99	89	-	-
SS-183	38 LT	54+10	38.7-40.2	A-7-6(15)	46	19	8.7	21.9	35.2	34.2	100	96	76	-	-
SS-186	38 LT	54+10	63.2-65.7	A-6(22)	38	23	1.8	6.2	37.6	54.3	100	99	95	-	-
SS-188	38 LT	54+10	73.2-75.7	A-7-5(23)	63	19	7.2	9.2	55.4	28.1	100	96	87	-	-
SS-191	38 LT	54+10	97.5-99.0	A-7-5(28)	62	23	3.2	5.8	60.8	30.2	100	98	93	-	-

B5-B

SOIL TEST RESULTS															
SAMPLE NO.	OFFSET	STATION	DEPTH INTERVAL	AASHTO CLASS.	L.L.	P.I.	% BY WEIGHT				% PASSING (SIEVES)			% MOISTURE	% ORGANIC
							C.SAND	F.SAND	SILT	CLAY	10	40	200		
SS-123	73 RT	54+02	0.0-1.5	A-6(9)	34	13	1.8	24.9	31.1	42.2	100	100	79	-	-
SS-124	73 RT	54+02	10.2-11.7	A-7-6(9)	42	22	3.6	49.6	28.7	18.1	100	98	55	-	-
SS-125	73 RT	54+02	20.2-21.7	A-2-7(2)	43	21	42.6	30.9	14.5	12.0	100	77	31	-	-
SS-126	73 RT	54+02	30.2-31.7	A-6(12)	38	18	9.4	28.1	44.4	18.1	100	94	72	-	-
SS-127	73 RT	54+02	40.2-41.7	A-6(9)	37	17	11.8	32.1	40.0	16.1	100	95	65	-	-
SS-128	73 RT	54+02	50.2-51.7	A-2-6(0)	35	15	48.2	32.9	10.8	8.0	98	76	22	-	-
SS-129	73 RT	54+02	55.2-56.7	A-7-6(36)	54	34	1.2	3.6	32.9	62.2	100	99	96	-	-
SS-130	73 RT	54+02	65.2-66.7	A-6(13)	35	18	13.1	8.4	38.4	40.2	97	88	78	-	-
SS-131	73 RT	54+02	75.2-76.7	A-7-5(31)	64	25	3.4	5.0	55.4	36.1	100	98	94	-	-
SS-132	73 RT	54+02	85.2-86.7	A-7-5(31)	59	25	1.2	2.0	50.6	46.2	100	99	98	-	-
SS-134	73 RT	54+02	105.1-106.7	A-7-5(27)	59	21	3.0	4.4	56.4	36.1	100	98	95	-	-

B6-A

SOIL TEST RESULTS															
SAMPLE NO.	OFFSET	STATION	DEPTH INTERVAL	AASHTO CLASS.	L.L.	P.I.	% BY WEIGHT				% PASSING (SIEVES)			% MOISTURE	% ORGANIC
							C.SAND	F.SAND	SILT	CLAY	10	40	200		
SS-97	39 LT	55+42	0.0-1.5	A-2-4(0)	24	6	54.9	17.4	16.7	11.1	95	67	28	-	-
SS-98	39 LT	55+42	3.7-5.2	A-6(12)	37	16	7.8	19.5	44.5	28.1	100	95	78	-	-
SS-99	39 LT	55+42	8.7-10.2	A-6(11)	39	15	13.7	10.9	59.4	16.1	99	91	77	-	-
SS-100	39 LT	55+42	13.7-15.2	A-7-6(19)	42	19	0.6	14.3	51.0	34.2	100	100	91	-	-
SS-101	39 LT	55+42	18.7-20.2	A-7-6(25)	54	36	4.8	30.2	34.9	30.2	100	98	73	-	-
SS-102	39 LT	55+42	23.7-25.2	A-6(5)	38	18	21.3	32.8	31.9	14.1	99	88	49	-	-
SS-103	39 LT	55+42	33.7-35.2	A-6(15)	36	19	4.2	17.5	56.2	22.1	100	98	84	-	-
SS-104	39 LT	55+42	43.7-45.2	A-6(8)	35	16	22.5	24.5	30.9	22.1	100	87	62	-	-
SS-106	39 LT	55+42	53.7-55.2	A-2-4(0)	34	10	63.2	22.7	10.1	4.0	99	64	17	-	-
SS-107	39 LT	55+42	58.7-60.2	A-6(17)	40	21	7.0	15.5	47.3	30.2	100	96	83	-	-
SS-109	39 LT	55+42	68.7-70.2	A-6(11)	37	18	10.1	25.5	50.4	14.1	100	96	72	-	-
SS-110	39 LT	55+42	73.7-75.2	A-2-6(1)	39	17	40.4	31.3	21.3	7.0	99	78	33	-	-
SS-112	39 LT	55+42	93.7-95.2	A-4(3)	22	6	2.2	6.0	67.6	24.1	100	99	94	-	-

B6-B

SOIL TEST RESULTS															
SAMPLE NO.	OFFSET	STATION	DEPTH INTERVAL	AASHTO CLASS.	L.L.	P.I.	% BY WEIGHT				% PASSING (SIEVES)			% MOISTURE	% ORGANIC
							C.SAND	F.SAND	SILT	CLAY	10	40	200		
SS-114	42 RT	55+49	0.0-1.5	A-2-6(0)	28	11	53.5	25.5	13.0	8.0	90	57	21	-	-
SS-115	42 RT	55+49	3.9-5.4	A-6(6)	40	23	33.0	23.3	19.6	24.1	95	77	45	-	-
SS-116	42 RT	55+49	8.9-10.4	A-7-6(6)	48	33	45.1	20.4	8.3	26.1	100	73	37	-	-
SS-117	42 RT	55+49	18.9-20.4	A-2-6(1)	32	14	37.4	35.6	22.0	5.0	100	83	33	-	-
SS-118	42 RT	55+49	28.9-30.4	A-4(0)	21	8	32.7	31.9	28.4	7.0	100	82	42	-	-
SS-119	42 RT	55+49	33.9-35.4	A-2-6(1)	35	18	57.4	18.4	17.2	7.0	96	61	26	-	-
SS-120	42 RT	55+49	43.9-45.4	A-6(14)	38	19	6.0	21.5	54.4	18.1	100	97	79	-	-
SS-122	42 RT	55+49	63.9-65.4	A-2-7(1)	45	22	56.7	19.5	15.8	8.0	97	61	25	-	-

EB2-A

SOIL TEST RESULTS															
SAMPLE NO.	OFFSET	STATION	DEPTH INTERVAL	AASHTO CLASS.	L.L.	P.I.	% BY WEIGHT				% PASSING (SIEVES)			% MOISTURE	% ORGANIC
							C.SAND	F.SAND	SILT	CLAY	10	40	200		
SS-89	35 LT	57+08	0.0-1.5	A-4(0)	20	5	31.7	28.7	21.3	18.2	96	79	42	-	-
SS-90	35 LT	57+08	8.8-10.3	A-2-6(1)	34	17	52.2	13.0	14.6	20.2	97	62	35	-	-
SS-91	35 LT	57+08	13.8-15.3	A-1-b(0)	20	NP	83.2	12.1	2.6	2.0	65	26	4	-	-
SS-92	35 LT	57+08	18.8-20.3	A-7-6(9)	41	21	4.7	47.9	29.2	18.2	100	99	55	-	-
SS-93	35 LT	57+08	28.8-30.3	A-6(5)	37	19	34.8	25.1	30.0	10.1	100	79	46	-	-
SS-94	35 LT	57+08	38.8-40.3	A-6(7)	39	21	30.9	23.7	31.2	14.2	100	83	51	-	-
SS-95	35 LT	57+08	48.8-50.3	A-7-6(16)	45	24	12.3	23.1	44.4	20.2	100	94	72	-	-
SS-96	35 LT	57+08	58.8-60.3	A-4(1)	25	10	30.1	31.1	30.6	8.1	100	83	45	-	-

EB2-B

SOIL TEST RESULTS															
SAMPLE NO.	OFFSET	STATION	DEPTH INTERVAL	AASHTO CLASS.	L.L.	P.I.	% BY WEIGHT				% PASSING (SIEVES)			% MOISTURE	% ORGANIC
							C.SAND	F.SAND	SILT	CLAY	10	40	200		
SS-78	41 RT	57+03	0.0-1.5	A-4(0)	22	8	32.2	27.1	22.5	18.2	96	78	44	-	-
SS-79	41 RT	57+03	8.8-10.3	A-2-4(0)	25	4	57.7	26.9	3.2	12.1	86	63	14	-	-
SS-80	41 RT	57+03	18.8-20.3	A-1-b(0)	18	NP	83.5	11.5	1.9	3.0	77	27	5	-	-
SS-81	41 RT	57+03	23.8-25.3	A-2-4(0)	30	3	59.6	30.1	7.3	3.0	100	72	13	-	-
SS-82	41 RT	57+03	33.8-35.3	A-2-6(1)	33	15	51.2	23.1	16.7	9.1	100	71	30	-	-
SS-83	41 RT	57+03	43.8-45.3	A-2-6(0)	36	12	60.4	21.5	13.0	5.1	92	51	19	-	-
SS-84	41 RT	57+03	53.8-55.3	A-6(2)	34	14	14.8	50.4	28.8	6.1	100	94	41	-	-
SS-85	41 RT	57+03	63.8-64.3	A-4(0)	20	5	33.6	30.5	27.8	8.1	100	81	41	-	-

B3-B

ROCK TEST RESULTS								
SAMPLE NO.	OFFSET	STATION	DEPTH INTERVAL	H/D RATIO	UNIT WT lbs/ft3	UNCONFINED STRENGTH (PSI)	UNDRAINED SHEAR STRENGTH (PSI)	SHEAR PLANE ANGLE (DEGREES)
RS-1	50 RT	51+41	104.-104.7	2.26	136.10	31.0	15.518	30
RS-4	50 RT	51+41	90.0-90.4	1.84	117.52	4.0	2.006	35

B5-A

ROCK TEST RESULTS								
SAMPLE NO.	OFFSET	STATION	DEPTH INTERVAL	H/D RATIO	UNIT WT lbs/ft3	UNCONFINED STRENGTH (PSI)	UNDRAINED SHEAR STRENGTH (PSI)	SHEAR PLANE ANGLE (DEGREES)
RS-2	38 LT	54+02	108.3-108.7	1.81	143.37	312.3	156.150	30
RS-3	38 LT	54+02	112.3-113.0	2.32	139.10	128.5	64.246	40



**FIELD
 SCOUR REPORT**

WBS: 33449.1.1 TIP: B-4091 COUNTY: CUMBERLAND

DESCRIPTION(1): Br. No 85 on -L- (US 301/I-95 Bus. Loop) over -Y1- (SR 1738), -Y2- (SR 1741) and Cape Fear River

EXISTING BRIDGE

Information from: Field Inspection Microfilm _____ (reel _____ pos: _____)
 Other (explain) _____

Bridge No.: 85 Length: 965 Total Bents: 19 Bents in Channel: 4 Bents in Floodplain: 15
 Foundation Type: CONCRETE FOOTINGS, CONCRETE PILES

EVIDENCE OF SCOUR(2)

Abutments or End Bent Slopes: NONE EVIDENT

Interior Bents: SCOUR AT EXISTING BENT 7 AND BENT 8 (SOUTH BANK)
GULLY EXTENDING FROM EXISTING BENT 7 TO BENT 8 FROM DRAINAGE

Channel Bed: NO SCOUR EVIDENT

Channel Bank: SCOUR EVIDENT ON SOUTH BANK OF RIVER, DUE TO DRAINAGE CHANNELS
AND GULLYING CAUSED BY SECONDARY DRAINAGE/SEEPS

EXISTING SCOUR PROTECTION

Type(3): NO SIDE SLOPE PROTECTION OR RIP RAP USED ONSITE

Extent(4): N/A

Effectiveness(5): N/A

Obstructions(6): LARGE AMOUNT OF DEBRIS AGAINST BENT 9, 10 AND 11

INSTRUCTIONS

- 1 Describe the specific site's location, including route number and body of water crossed.
- 2 Note scour evidence at existing end bents or abutments (e.g. undermining, sloughing, degradations).
- 3 Note existing scour protection (e.g. rip rap).
- 4 Describe extent of existing scour protection.
- 5 Describe whether or not the scour protection appears to be working.
- 6 Note obstructions such as dams, fallen trees, debris at bents, etc.
- 7 Describe the channel bed material based on observation and/or samples. Include any lab results with report.
- 8 Describe the channel bank material based on observation and/or samples. Include any lab results with report.
- 9 Describe the material covering the banks (e.g. grass, trees, rip rap, none).
- 10 Determine the approximate floodplain width from field observation or a topographic map.
- 11 Describe the material covering the floodplain (e.g. grass, trees, crops).
- 12 Use professional judgement to specify if the stream is degrading, aggrading, or static.
- 13 Describe potential and direction of the stream to migrate laterally during the bridge's life (approx. 100 years).
- 14 Give the design scour elevation (DSE) expected over the life of the bridge (approx. 100 years). This

elevation can be given as a range across the site, or for each bent. Discuss the relationship between the Hydraulics Unit theoretical scour and the DSE. If the DSE is dependent on scour counter measures, explain (e.g. rip rap armoring on slopes). The DSE is based on the erodability of materials, giving consideration to the influence of joints, foliation, bedding characteristics, % core recovery, % RQD, differential weathering, shear strength, observations at existing structures, other tests deemed appropriate, and overall geologic conditions at the site.

DESIGN INFORMATION

Channel Bed Material(7): ALLUVIAL SAND AND CLAY

Channel Bank Material(8): ALLUVIAL CLAYS
COASTAL PLAIN CLAYEY, SILTY SANDS (CAPE FEAR FORMATION)

Channel Bank Cover(9): GRASS, SHRUBS, SMALL TO LARGE TREES

Floodplain Width(10): 700 FT

Floodplain Cover(11): GRASS, SHRUB, YOUNG TO MODERATELY AGED TREES.

Stream is(12): Aggrading _____ Degrading Static _____

Channel Migration Tendency(13): SOUTH

Observations and Other Comments: _____

DESIGN SCOUR ELEVATIONS(14)

Feet Meters _____

BENTS

B1	B2	B3	B4	B5	B6					
66.4	65.7	22.1	12.2	11.9	58.2					

Comparison of DSE to Hydraulics Unit theoretical scour:

The Geotechnical Engineering Unit agrees with the Hydraulic Unit's theoretical scour elevations at Bent 1, Bent 2, Bent 3. The DSE has been raised at Bent 4 and Bent 5 by 8-10 feet. Bent 6 has been adjusted based on revised centerline elevation, which is lower than the elevation given by Hydraulics. The contraction scour is still 3.4 feet at Bent 6.

SOIL ANALYSIS RESULTS FROM CHANNEL BED AND BANK MATERIAL

Bed or Bank Sample No.	Retained #4	Passed #10	Passed #40	Passed #200	Coarse Sand	Fine Sand	Silt	Clay	LL	PI	AASHTO	Station	Offset	Depth
	See Sheet 12, "Soil Test Results", for samples:													
	SS-195 (CHANNEL)													
	SS-196 (CHANNEL)													
	SS-166 (CHANNEL)													
	SS-136 (CHANNEL)													
	SS-179 (CHANNEL)													
	SS-97 (BANK)													
	SS-98 (BANK)													
	SS-100 (BANK)													

Template Revised 02/07/06

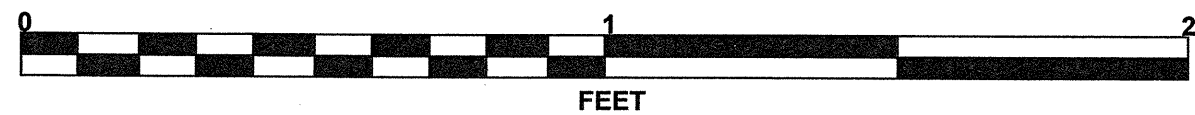
Reported by: Christina M. Bruinsma, L.G.

Date: 6/28/2011

CORE PHOTOGRAPHS

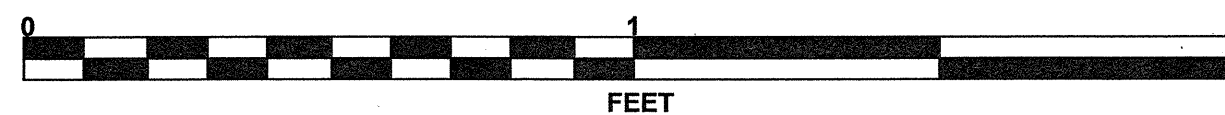
B3-B

BOX 1: 85.5 - 101.1 FEET



B3-B

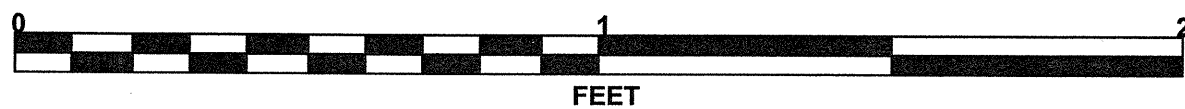
BOX 2: 101.1 - 113.3 FEET



CORE PHOTOGRAPHS

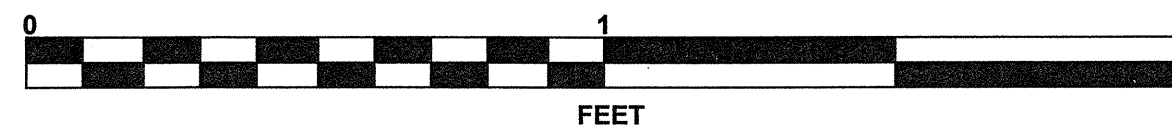
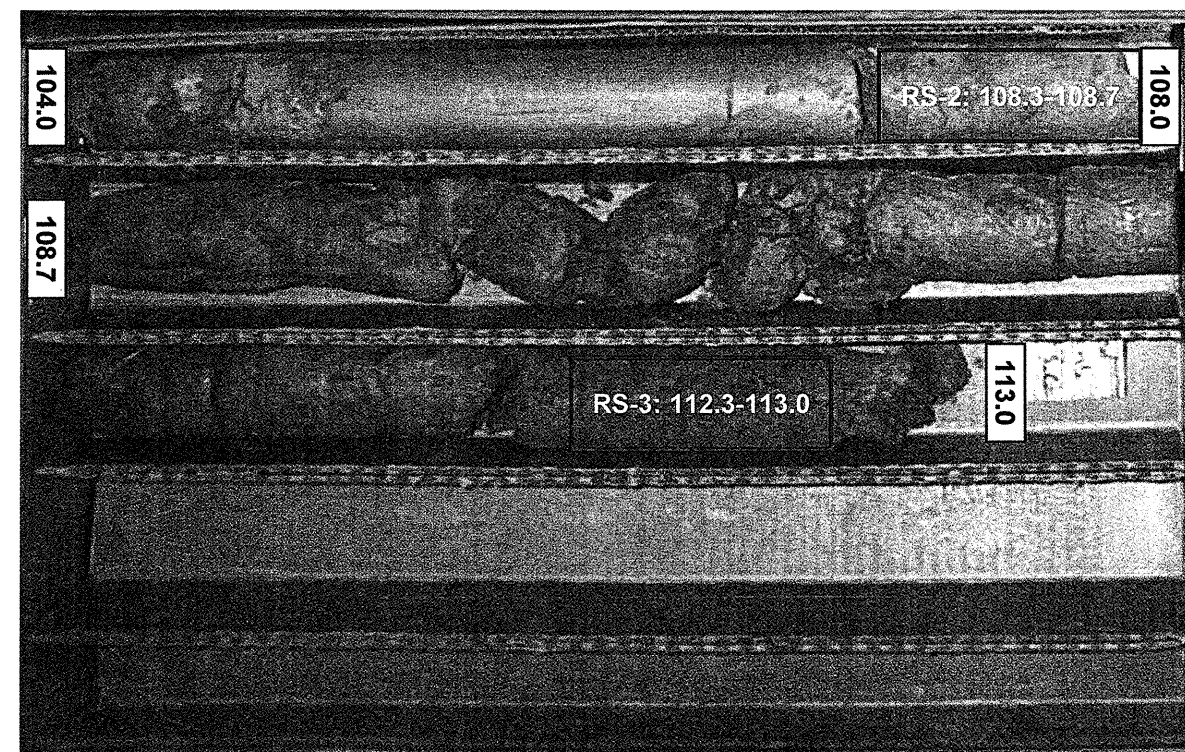
B5-A

BOX 1: 89.5 - 103.0 FEET



B5-A

BOX 2: 104.0 - 113.0 FEET



SITE PHOTOGRAPH



Bridge No. 85 on -L- (US 301/I-95 Bus. Loop) over -Y1- (SR 1738), -Y2- (SR 1741) and Cape Fear River

PROJECT: 33449.1.1 ID: B-4091

STATE	STATE PROJECT REFERENCE NO.	SHEET NO.	TOTAL SHEETS
N.C.	33449.1.1 (B-4091)	1	5

STATE OF NORTH CAROLINA
DEPARTMENT OF TRANSPORTATION
DIVISION OF HIGHWAYS
GEOTECHNICAL ENGINEERING UNIT

CONTENTS

SHEET	DESCRIPTION
1	TITLE SHEET
2	LEGEND
3-4	SITE PLAN & PROFILES
5	SOIL TEST RESULTS

STRUCTURE
SUBSURFACE INVESTIGATION

PROJ. REFERENCE NO. 33449.1.1 (B-4091) F.A. PROJ. BRSTP-0301(12)
COUNTY CUMBERLAND
PROJECT DESCRIPTION BRIDGE NO. 85 ON -L- (US 301A-95 BUS. LOOP)
OVER -Y1- (SR 1738), -Y2- (SR 1741) AND CAPE FEAR RIVER
SITE DESCRIPTION WALL 1 LEFT OF -L- STATION 47+00
WALL 2 RIGHT OF -L- STATION 46+00
WALL 3 LEFT OF -L- STATION 57+50
WALL 4 RIGHT OF -L- STATION 57+50

RETAINING WALLS

CAUTION NOTICE

THE SUBSURFACE INFORMATION AND THE SUBSURFACE INVESTIGATION ON WHICH IT IS BASED WERE MADE FOR THE PURPOSE OF STUDY, PLANNING, AND DESIGN, AND NOT FOR CONSTRUCTION OR PAY PURPOSES. THE VARIOUS FIELD BORING LOGS, ROCK CORES, AND SOIL TEST DATA AVAILABLE MAY BE REVIEWED OR INSPECTED IN RALEIGH BY CONTACTING THE N. C. DEPARTMENT OF TRANSPORTATION, GEOTECHNICAL ENGINEERING UNIT AT (919) 250-4088. NEITHER THE SUBSURFACE PLANS AND REPORTS, NOR THE FIELD BORING LOGS, ROCK CORES, OR SOIL TEST DATA ARE PART OF THE CONTRACT.

GENERAL SOIL AND ROCK STRATA DESCRIPTIONS AND INDICATED BOUNDARIES ARE BASED ON A GEOTECHNICAL INTERPRETATION OF ALL AVAILABLE SUBSURFACE DATA AND MAY NOT NECESSARILY REFLECT THE ACTUAL SUBSURFACE CONDITIONS BETWEEN BORINGS OR BETWEEN SAMPLED STRATA WITHIN THE BOREHOLE. THE LABORATORY SAMPLE DATA AND THE IN SITU (IN-PLACE) TEST DATA CAN BE RELIED ON ONLY TO THE DEGREE OF RELIABILITY INHERENT IN THE STANDARD TEST METHOD. THE OBSERVED WATER LEVELS OR SOIL MOISTURE CONDITIONS INDICATED IN THE SUBSURFACE INVESTIGATIONS ARE AS RECORDED AT THE TIME OF THE INVESTIGATION. THESE WATER LEVELS OR SOIL MOISTURE CONDITIONS MAY VARY CONSIDERABLY WITH TIME ACCORDING TO CLIMATIC CONDITIONS INCLUDING TEMPERATURES, PRECIPITATION, AND WIND, AS WELL AS OTHER NON-CLIMATIC FACTORS.

THE BIDDER OR CONTRACTOR IS CAUTIONED THAT DETAILS SHOWN ON THE SUBSURFACE PLANS ARE PRELIMINARY ONLY AND IN MANY CASES THE FINAL DESIGN DETAILS ARE DIFFERENT. FOR BIDDING AND CONSTRUCTION PURPOSES, REFER TO THE CONSTRUCTION PLANS AND DOCUMENTS FOR FINAL DESIGN INFORMATION ON THIS PROJECT. THE DEPARTMENT DOES NOT WARRANT OR GUARANTEE THE SUFFICIENCY OR ACCURACY OF THE INVESTIGATION MADE, NOR THE INTERPRETATIONS MADE, OR OPINION OF THE DEPARTMENT AS TO THE TYPE OF MATERIALS AND CONDITIONS TO BE ENCOUNTERED. THE BIDDER OR CONTRACTOR IS CAUTIONED TO MAKE SUCH INDEPENDENT SUBSURFACE INVESTIGATIONS AS HE DEEMS NECESSARY TO SATISFY HIMSELF AS TO CONDITIONS TO BE ENCOUNTERED ON THIS PROJECT. THE CONTRACTOR SHALL HAVE NO CLAIM FOR ADDITIONAL COMPENSATION OR FOR AN EXTENSION OF TIME FOR ANY REASON RESULTING FROM THE ACTUAL CONDITIONS ENCOUNTERED AT THE SITE DIFFERING FROM THOSE INDICATED IN THE SUBSURFACE INFORMATION.

PERSONNEL

C.M. BRUINSMA

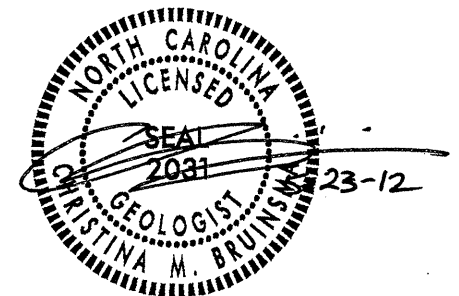
MACTEC

INVESTIGATED BY C.M. BRUINSMA

CHECKED BY C.M. BRUINSMA

SUBMITTED BY N.T. ROBERSON

DATE MAY 2012

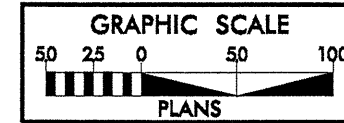


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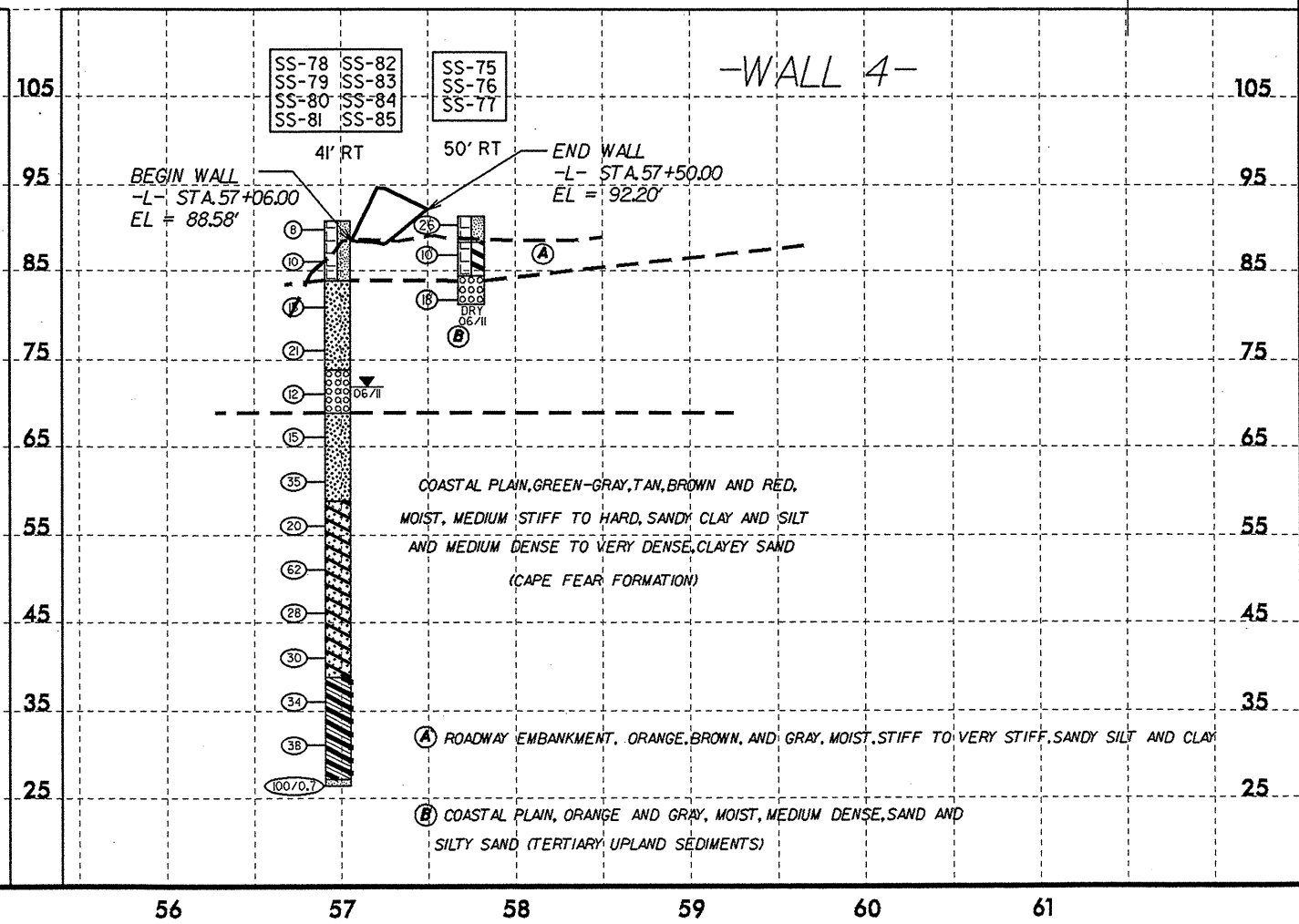
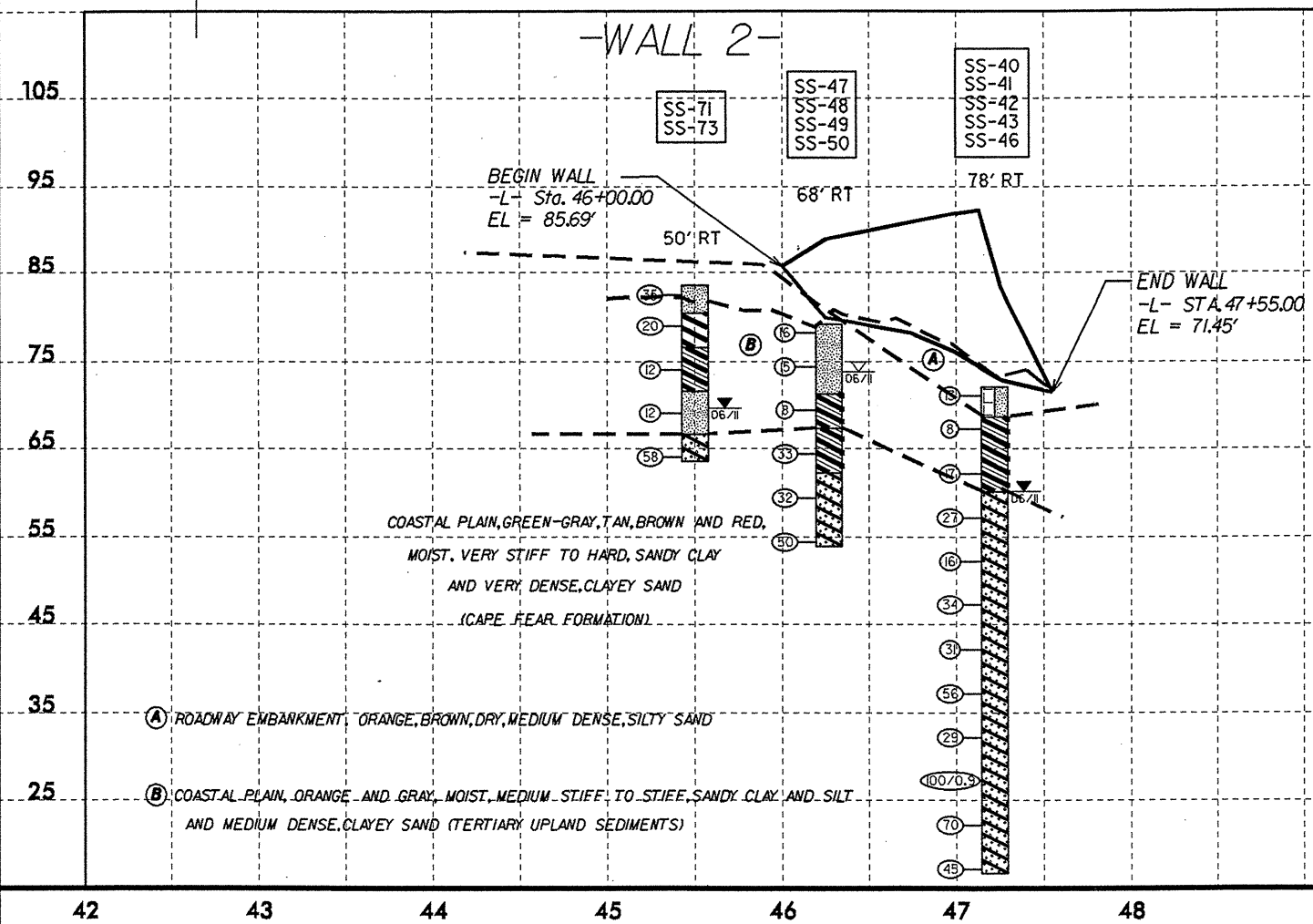
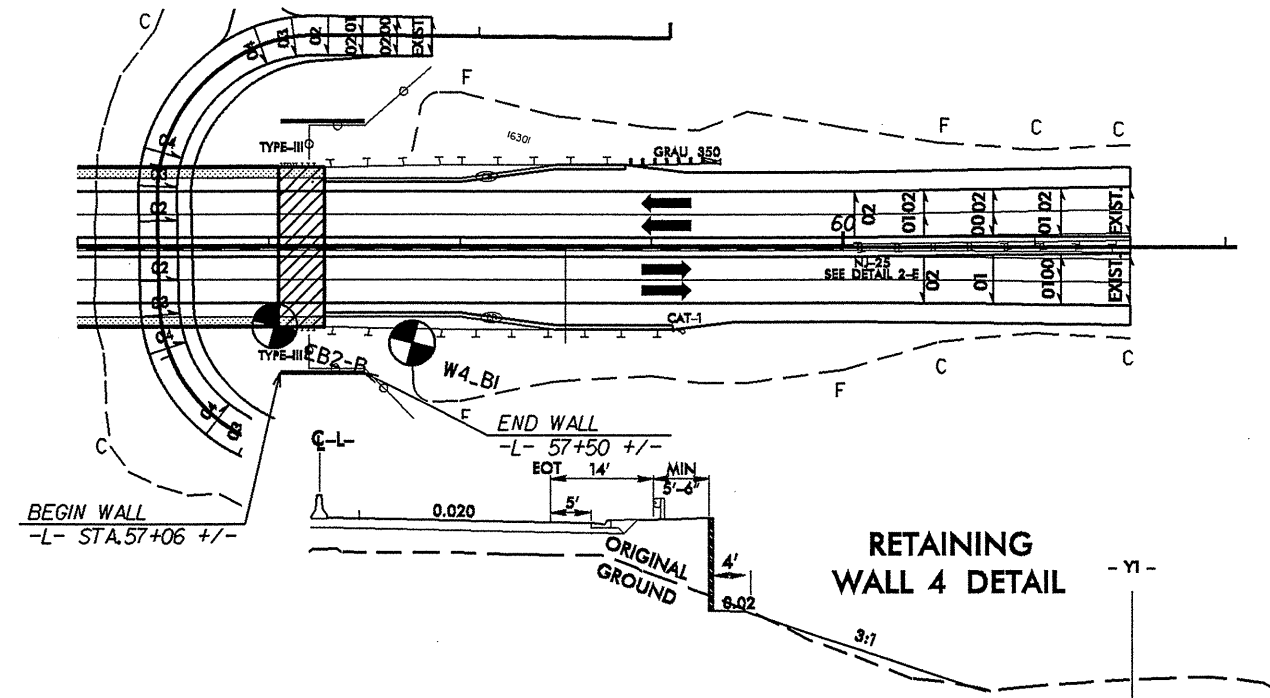
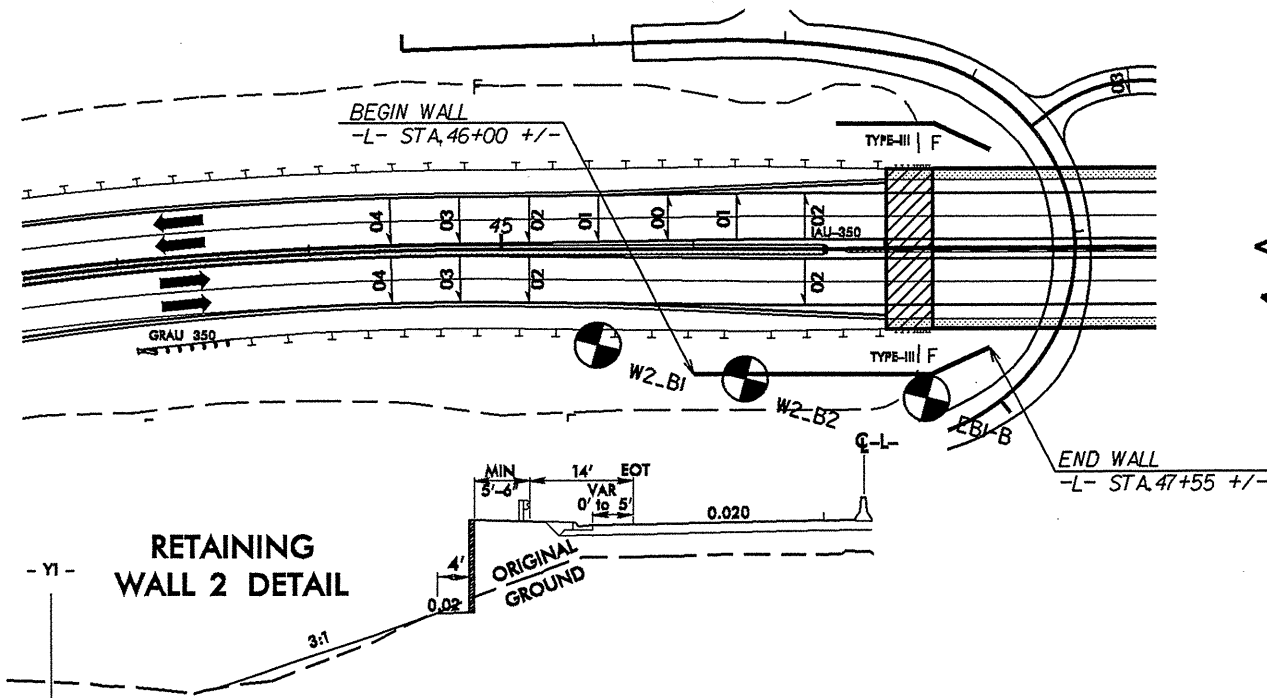
NOTE - THE INFORMATION CONTAINED HEREIN IS NOT IMPLIED OR GUARANTEED BY THE N. C. DEPARTMENT OF TRANSPORTATION AS BEING ACCURATE NOR IS IT CONSIDERED TO BE PART OF THE PLANS, SPECIFICATIONS, OR CONTRACT FOR THE PROJECT.

NOTE - BY HAVING REQUESTED THIS INFORMATION THE CONTRACTOR SPECIFICALLY WAIVES ANY CLAIMS FOR INCREASED COMPENSATION OR EXTENSION OF TIME BASED ON DIFFERENCES BETWEEN THE CONDITIONS INDICATED HEREIN AND THE ACTUAL CONDITIONS AT THE PROJECT SITE.

PLAN VIEW OF RETAINING WALL AND WALL ENVELOPE



PROJECT REFERENCE NO. B-4091	SHEET NO. 4
RW SHEET NO.	
ROADWAY DESIGN ENGINEER	HYDRAULICS ENGINEER
INCOMPLETE PLANS DO NOT USE FOR ACQUISITION	
PRELIMINARY PLANS DO NOT USE FOR CONSTRUCTION	



REVISIONS

8/17/99

22-MAY-2012 10:51:11 AM \\satsat\station\TIP\B4091_GEO_R\WALL\CADD_GEO\TECH\Plan\B4091_geo_psh_wal1_b.dgn

SOIL TEST RESULTS

SAMPLE NO.	OFFSET	STATION	DEPTH INTERVAL	AASHTO CLASS.	L.L.	P.I.	% BY WEIGHT				% PASSING (SIEVES)			% MOISTURE	% ORGANIC
							C.SAND	F.SAND	SILT	CLAY	10	40	200		
SS-60	50 LT	45+00	0.0-1.5	A-4(0)	22	4	30.5	36.7	18.7	14.1	100	88	39	-	-
SS-61	50 LT	45+00	3.5-5.0	A-6(13)	38	18	2.8	28.9	36.1	32.1	100	98	78	-	-
SS-62	50 LT	45+00	8.5-10.0	A-6(8)	29	13	2.0	31.5	34.3	32.1	100	100	78	-	-
SS-63	50 LT	45+00	13.5-15.0	A-4(0)	21	4	26.5	42.2	15.3	16.1	100	80	40	-	-
SS-64	50 LT	45+00	18.5-20.0	A-2-6(1)	36	16	39.6	31.5	20.9	8.0	90	69	33	-	-
SS-67	59 LT	46+05	13.5-15.0	A-4(0)	22	3	3.0	54.0	22.9	20.1	100	100	54	-	-
SS-51	22 LT	47+13	0.8-1.7	A-1-b(0)	17	NP	74.8	14.4	4.8	6.1	90	40	11	-	-
SS-52	22 LT	47+13	3.7-5.2	A-4(3)	28	9	8.7	42.1	29.0	20.2	100	99	58	-	-
SS-53	22 LT	47+13	13.7-15.2	A-6(7)	34	15	6.5	37.0	28.2	28.3	100	100	64	-	-
SS-54	22 LT	47+13	18.7-20.2	A-4(1)	27	8	7.3	50.6	21.9	20.2	100	100	51	-	-
SS-55	22 LT	47+13	23.7-25.2	A-6(11)	36	17	4.7	30.7	34.3	30.3	100	99	72	-	-
SS-57	22 LT	47+13	48.7-50.2	A-2-6(0)	35	17	57.3	23.1	14.6	5.1	95	61	22	-	-
SS-89	35 LT	57+08	0.0-1.5	A-4(0)	20	5	31.7	28.7	21.3	18.2	96	79	42	-	-
SS-90	35 LT	57+08	8.8-10.3	A-2-6(1)	34	17	52.2	13.0	14.6	20.2	97	62	35	-	-
SS-91	35 LT	57+08	13.8-15.3	A-1-b(0)	20	NP	83.2	12.1	2.6	2.0	65	26	4	-	-
SS-92	35 LT	57+08	18.8-20.3	A-7-6(9)	41	21	4.7	47.9	29.2	18.2	100	99	55	-	-
SS-93	35 LT	57+08	28.8-30.3	A-6(5)	37	19	34.8	25.1	30.0	10.1	100	79	46	-	-
SS-94	35 LT	57+08	38.8-40.3	A-6(7)	39	21	30.9	23.7	31.2	14.2	100	83	51	-	-
SS-95	35 LT	57+08	48.8-50.3	A-7-6(16)	45	24	12.3	23.1	44.4	20.2	100	94	72	-	-
SS-96	35 LT	57+08	58.8-60.3	A-4(1)	25	10	30.1	31.1	30.6	8.1	100	83	45	-	-
SS-87	50 LT	57+75	3.5-5.0	A-2-6(2)	35	18	54.4	12.7	10.8	22.1	97	58	34	-	-
SS-88	50 LT	57+75	8.5-10.0	A-1-b(0)	16	NP	83.1	13.3	1.6	2.0	76	32	4	-	-
SS-71	50 RT	45+50	3.5-5.0	A-7-6(16)	42	20	0.4	35.1	30.3	34.1	100	100	79	-	-
SS-73	50 RT	45+50	13.5-15.0	A-4(0)	22	NP	10.8	61.8	13.3	14.1	99	94	39	-	-
SS-47	68 RT	46+27	0.0-1.5	A-4(0)	20	3	28.3	39.0	14.7	18.1	100	89	39	-	-
SS-48	68 RT	46+27	3.9-5.4	A-4(1)	26	8	8.2	56.2	15.5	20.1	100	99	46	-	-
SS-49	68 RT	46+27	8.9-10.4	A-6(7)	30	14	2.0	39.0	30.9	28.1	100	100	70	-	-
SS-50	68 RT	46+27	18.9-20.9	A-2-6(0)	30	12	40.0	33.1	16.9	10.0	99	78	31	-	-
SS-40	78 RT	47+22	0.0-1.5	A-4(0)	26	6	23.7	35.8	22.3	18.2	100	90	47	-	-
SS-41	78 RT	47+22	8.9-10.4	A-6(2)	27	11	16.4	39.2	24.2	20.2	100	98	50	-	-
SS-42	78 RT	47+22	13.9-15.4	A-2-6(0)	34	15	41.1	40.4	10.4	8.1	98	78	21	-	-
SS-43	78 RT	47+22	23.9-25.4	A-2-6(0)	39	16	63.4	19.5	12.0	5.1	96	54	19	-	-
SS-46	78 RT	47+22	53.9-55.4	A-2-6(0)	32	13	62.4	24.5	10.1	3.0	93	55	15	-	-
SS-78	41 LT	57+03	0.0-1.5	A-4(0)	22	8	32.2	27.1	22.5	18.2	96	78	44	-	-
SS-79	41 LT	57+03	8.8-10.3	A-2-4(0)	25	4	57.7	26.9	3.2	12.1	86	63	14	-	-
SS-80	41 LT	57+03	18.8-20.3	A-1-b(0)	18	NP	83.5	11.5	1.9	3.0	77	27	5	-	-
SS-81	41 LT	57+03	23.8-25.3	A-2-4(0)	30	3	59.6	30.1	7.3	3.0	100	72	13	-	-
SS-82	41 LT	57+03	33.8-35.3	A-2-6(1)	33	15	51.2	23.1	16.7	9.1	100	71	30	-	-
SS-83	41 LT	57+03	43.8-45.3	A-2-6(0)	36	12	60.4	21.5	13.0	5.1	92	51	19	-	-
SS-84	41 LT	57+03	53.8-55.3	A-6(2)	34	14	14.8	50.4	28.8	6.1	100	94	41	-	-
SS-85	41 LT	57+03	63.8-65.3	A-4(0)	20	5	33.6	30.5	27.8	8.1	100	81	41	-	-
SS-75	50 RT	57+75	0.0-1.5	A-4(0)	20	4	14.7	43.4	23.9	18.1	100	94	53	-	-
SS-76	50 RT	57+75	3.5-5.0	A-7-6(21)	45	26	3.0	20.3	26.5	50.2	100	99	81	-	-
SS-77	50 RT	57+75	8.5-10.0	A-1-b(0)	14	NP	77.3	17.7	1.0	4.0	89	40	6	-	-