

NOTE: SEE SHEET 2A FOR PLAN SHEET LAYOUT AT TIME OF INVESTIGATION

STATE OF NORTH CAROLINA  
DEPARTMENT OF TRANSPORTATION  
DIVISION OF HIGHWAYS  
GEOTECHNICAL ENGINEERING UNIT

STATE	STATE PROJECT REFERENCE NO.	SHEET NO.	TOTAL SHEETS
N.C.	33791.1.1 (B-4599)	1	18
STATE PROJ. NO.	F.A. PROJ. NO.	DESCRIPTION	
33791.1.1	BRSTP-17(41)	P.E.	
		RW & UTIL.	

CONTENTS

LINE	STATION	PLAN	PROFILE
-L-	12+40 TO 40+00	4-5	6-8
-Y1-	10+00 TO 11+30	4	-
-Y2-	10+00 TO 12+05	5	9
-Y3-	10+00 TO 11+30	5	9
-Y4-	10+00 TO 11+25	5	-

CROSS SECTIONS	STATION	SHEET
-L-	12+50 TO 13+50	10
-L-	16+50 TO 20+50	11-13
-L-	28+50 TO 40+00	14-18

ROADWAY  
SUBSURFACE INVESTIGATION

PROJ. REFERENCE NO. 33791.1.1 (B-4599) F.A. PROJ. BRSTP-17(41)  
COUNTY PASQUOTANK COUNTY  
PROJECT DESCRIPTION BRIDGES NOS. 1 & 2 OVER KNOBBS CREEK  
ON US 17 / US 158

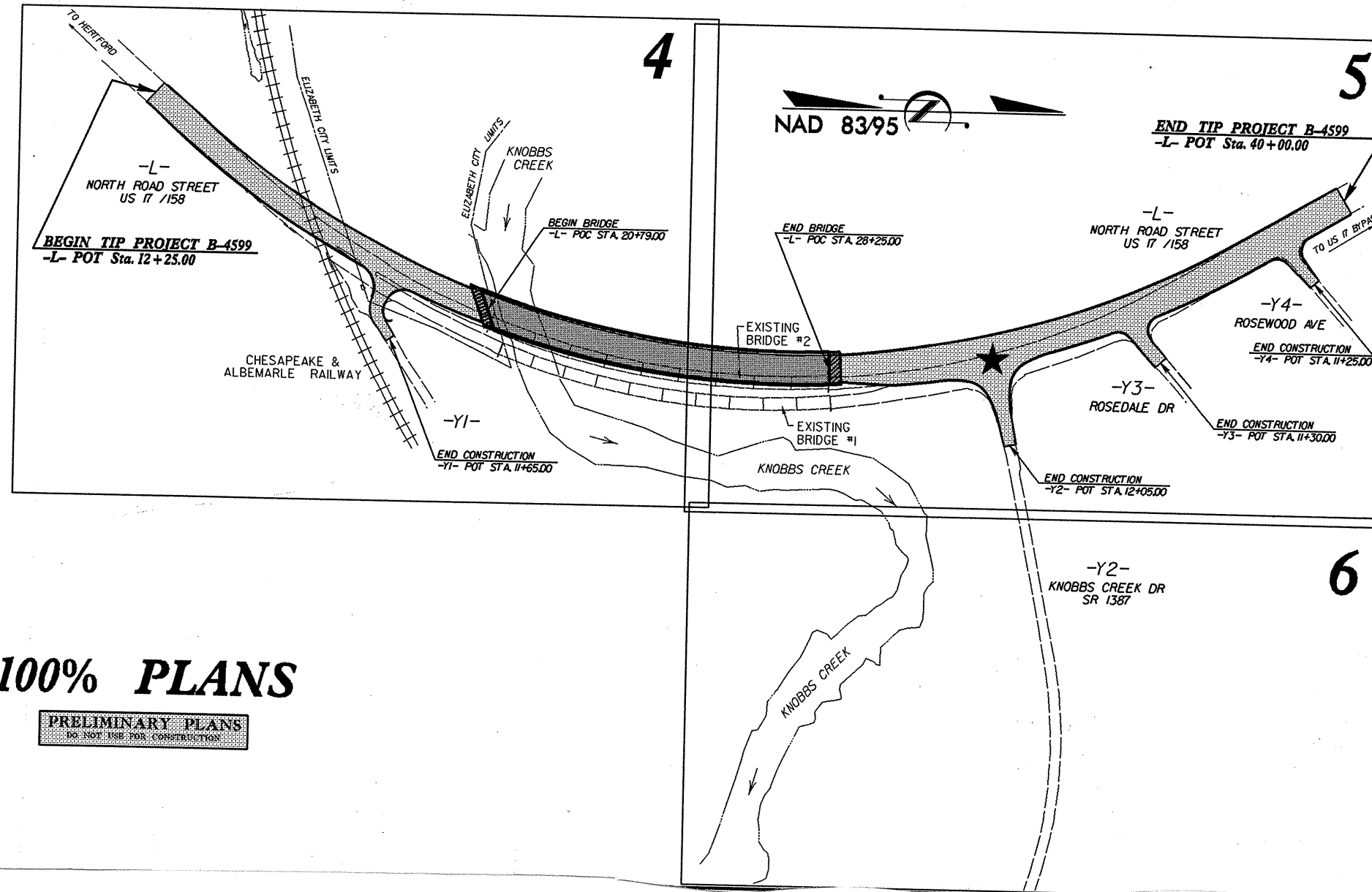
RECOMMENDATIONS

**CAUTION NOTICE**

THE SUBSURFACE INFORMATION AND THE SUBSURFACE INVESTIGATION ON WHICH IT IS BASED WERE MADE FOR THE PURPOSE OF STUDY, PLANNING, AND DESIGN AND NOT FOR CONSTRUCTION OR PAY PURPOSES. THE VARIOUS FIELD BORING LOGS, ROCK CORES, AND SOIL TEST DATA AVAILABLE MAY BE REVIEWED OR INSPECTED IN RALEIGH BY CONTACTING THE N.C. DEPARTMENT OF TRANSPORTATION, GEOTECHNICAL ENGINEERING UNIT AT (919) 250-4088. NEITHER THE SUBSURFACE PLANS AND REPORTS, NOR THE FIELD BORING LOGS, ROCK CORES, OR SOIL TEST DATA ARE PART OF THE CONTRACT.

GENERAL SOIL AND ROCK STRATA DESCRIPTIONS AND INDICATED BOUNDARIES ARE BASED ON A GEOTECHNICAL INTERPRETATION OF ALL AVAILABLE SUBSURFACE DATA AND MAY NOT NECESSARILY REFLECT THE ACTUAL SUBSURFACE CONDITIONS BETWEEN BORINGS OR BETWEEN SAMPLED STRATA WITHIN THE BOREHOLE, THE LABORATORY SAMPLE DATA AND THE IN SITU (IN-PLACE) TEST DATA CAN BE RELIED ON ONLY TO THE DEGREE OF RELIABILITY INHERENT IN THE STANDARD TEST METHOD. THE OBSERVED WATER LEVELS OR SOIL MOISTURE CONDITIONS INDICATED IN THE SUBSURFACE INVESTIGATIONS ARE AS RECORDED AT THE TIME OF THE INVESTIGATION. THESE WATER LEVELS OR SOIL MOISTURE CONDITIONS MAY VARY CONSIDERABLY WITH TIME ACCORDING TO CLIMATIC CONDITIONS INCLUDING TEMPERATURES, PRECIPITATION, AND WIND, AS WELL AS OTHER NON-CLIMATIC FACTORS.

THE BIDDER OR CONTRACTOR IS CAUTIONED THAT DETAILS SHOWN ON THE SUBSURFACE PLANS ARE PRELIMINARY ONLY AND IN MANY CASES THE FINAL DESIGN DETAILS ARE DIFFERENT. FOR BIDDING AND CONSTRUCTION PURPOSES, REFER TO THE CONSTRUCTION PLANS AND DOCUMENTS FOR FINAL DESIGN INFORMATION ON THIS PROJECT. THE DEPARTMENT DOES NOT WARRANT OR GUARANTEE THE SUFFICIENCY OR ACCURACY OF THE INVESTIGATION MADE, NOR THE INTERPRETATIONS MADE, OR OPINION OF THE DEPARTMENT AS TO THE TYPE OF MATERIALS AND CONDITIONS TO BE ENCOUNTERED. THE BIDDER OR CONTRACTOR IS CAUTIONED TO MAKE SUCH INDEPENDENT SUBSURFACE INVESTIGATIONS AS HE DEEMS NECESSARY TO SATISFY HIMSELF AS TO CONDITIONS TO BE ENCOUNTERED ON THIS PROJECT. THE CONTRACTOR SHALL HAVE NO CLAIM FOR ADDITIONAL COMPENSATION OR FOR AN EXTENSION OF TIME FOR ANY REASON RESULTING FROM THE ACTUAL CONDITIONS ENCOUNTERED AT THE SITE DIFFERING FROM THOSE INDICATED IN THE SUBSURFACE INFORMATION.



100% PLANS

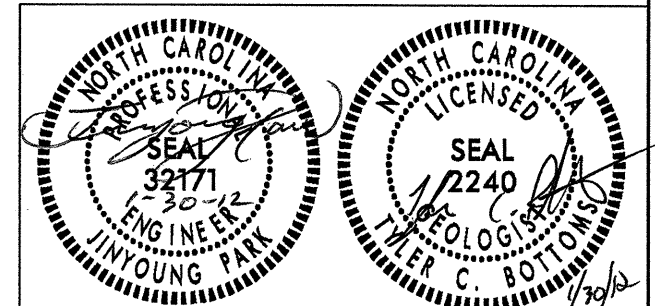
PRELIMINARY PLANS  
DO NOT USE FOR CONSTRUCTION

CONTRACT: C202823 ID: B-4599

PERSONNEL

CMW
JRS
RES
JME
S&ME PERSONNEL

INVESTIGATED BY T.C. BOTTOMS  
CHECKED BY D.N. ARGENBRIGHT  
SUBMITTED BY D.N. ARGENBRIGHT  
DATE JANUARY 2012



DRAWN BY: C.R. SUMNER, C.M. WRIKE

NOTE - THE INFORMATION CONTAINED HEREIN IS NOT IMPLIED OR GUARANTEED BY THE N.C. DEPARTMENT OF TRANSPORTATION AS BEING ACCURATE NOR IS IT CONSIDERED TO BE PART OF THE PLANS, SPECIFICATIONS, OR CONTRACT FOR THE PROJECT.
















NOTE - BY HAVING REQUESTED THIS INFORMATION THE CONTRACTOR SPECIFICALLY WAIVES ANY CLAIMS FOR INCREASED COMPENSATION OR EXTENSION OF TIME BASED ON DIFFERENCES BETWEEN THE CONDITIONS INDICATED HEREIN AND THE ACTUAL CONDITIONS AT THE PROJECT SITE.

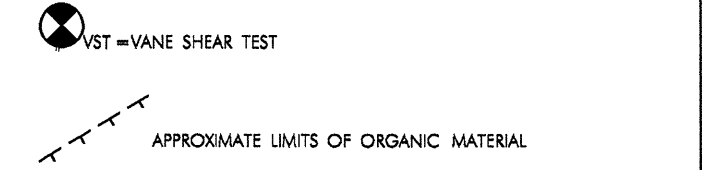
NORTH CAROLINA DEPARTMENT OF TRANSPORTATION  
DIVISION OF HIGHWAYS  
GEOTECHNICAL ENGINEERING UNIT

PROJECT REFERENCE NO. B-4599	SHEET NO. 2 OF 18
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## SUBSURFACE INVESTIGATION

### SOIL AND ROCK LEGEND, TERMS, SYMBOLS, AND ABBREVIATIONS

SOIL DESCRIPTION	GRADATION	ROCK DESCRIPTION	TERMS AND DEFINITIONS																								
SOIL IS CONSIDERED TO BE THE UNCONSOLIDATED, SEMI-CONSOLIDATED, OR WEATHERED EARTH MATERIALS THAT CAN BE PENETRATED WITH A CONTINUOUS FLIGHT POWER AUGER, AND YIELD LESS THAN 100 BLOWS PER FOOT ACCORDING TO STANDARD PENETRATION TEST (ASTM D-1586). SOIL CLASSIFICATION IS BASED ON THE AASHTO SYSTEM. BASIC DESCRIPTIONS GENERALLY SHALL INCLUDE: CONSISTENCY, COLOR, TEXTURE, MOISTURE, AASHTO CLASSIFICATION, AND OTHER PERTINENT FACTORS SUCH AS MINERALOGICAL COMPOSITION, ANGULARITY, STRUCTURE, PLASTICITY, ETC. EXAMPLE: <i>VERY STIFF, GRAY-SILTY CLAY, MOST WITH INTERBEDDED FINE SAND LAYERS, HARD PLASTIC, A-7-6</i>	<b>WELL GRADED</b> - INDICATES A GOOD REPRESENTATION OF PARTICLE SIZES FROM FINE TO COARSE. <b>UNIFORM</b> - INDICATES THAT SOIL PARTICLES ARE ALL APPROXIMATELY THE SAME SIZE. (ALSO POORLY GRADED) <b>GAP-GRADED</b> - INDICATES A MIXTURE OF UNIFORM PARTICLES OF TWO OR MORE SIZES.  <b>ANGULARITY OF GRAINS</b> THE ANGULARITY OR ROUNDNESS OF SOIL GRAINS IS DESIGNATED BY THE TERMS: <b>ANGULAR</b> , <b>SUBANGULAR</b> , <b>SUBROUNDED</b> , OR <b>ROUNDED</b> .	<b>HARD ROCK</b> IS NON-COASTAL PLAIN MATERIAL THAT IF TESTED, WOULD YIELD SPT REFUSAL. AN INFERRED ROCK LINE INDICATES THE LEVEL AT WHICH NON-COASTAL PLAIN MATERIAL WOULD YIELD SPT REFUSAL. SPT REFUSAL IS PENETRATION BY A SPLIT SPOON SAMPLER EQUAL TO OR LESS THAN 60 BLOWS PER FOOT IN NON-COASTAL PLAIN MATERIAL. THE TRANSITION BETWEEN SOIL AND ROCK IS OFTEN REPRESENTED BY A ZONE OF WEATHERED ROCK. ROCK MATERIALS ARE TYPICALLY DIVIDED AS FOLLOWS:  <b>WEATHERED ROCK (WR)</b>  NON-COASTAL PLAIN MATERIAL THAT WOULD YIELD SPT N VALUES > 100 BLOWS PER FOOT IF TESTED.  <b>CRYSTALLINE ROCK (CR)</b>  FINE TO COARSE GRAIN IGNEOUS AND METAMORPHIC ROCK THAT WOULD YIELD SPT REFUSAL. IF TESTED, ROCK TYPE INCLUDES GRANITE, GNEISS, GABBRO, SCHIST, ETC.  <b>NON-CRYSTALLINE ROCK (NCR)</b>  FINE TO COARSE GRAIN METAMORPHIC AND NON-COASTAL PLAIN SEDIMENTARY ROCK THAT WOULD YIELD SPT REFUSAL. IF TESTED, ROCK TYPE INCLUDES PHYLLITE, SLATE, SANDSTONE, ETC.  <b>COASTAL PLAIN SEDIMENTARY ROCK (CP)</b>  COASTAL PLAIN SEDIMENTS CEMENTED INTO ROCK, BUT MAY NOT YIELD SPT REFUSAL. ROCK TYPE INCLUDES LIMESTONE, SANDSTONE, CEMENTED SHELL BEDS, ETC.	<b>ALLUVIUM (ALLUV.)</b> - SOILS THAT HAVE BEEN TRANSPORTED BY WATER. <b>AQUIFER</b> - A WATER BEARING FORMATION OR STRATA. <b>ARENACEOUS</b> - APPLIED TO ROCKS THAT HAVE BEEN DERIVED FROM SAND OR THAT CONTAIN SAND. <b>ARGILLACEOUS</b> - APPLIED TO ALL ROCKS OR SUBSTANCES COMPOSED OF CLAY MINERALS, OR HAVING A NOTABLE PROPORTION OF CLAY IN THEIR COMPOSITION, AS SHALE, SLATE, ETC. <b>ARTESIAN</b> - GROUND WATER THAT IS UNDER SUFFICIENT PRESSURE TO RISE ABOVE THE LEVEL AT WHICH IT IS ENCOUNTERED, BUT WHICH DOES NOT NECESSARILY RISE TO OR ABOVE THE GROUND SURFACE. <b>CALCAREOUS (CALC.)</b> - SOILS THAT CONTAIN APPRECIABLE AMOUNTS OF CALCIUM CARBONATE. <b>COLLUVIUM</b> - ROCK FRAGMENTS MIXED WITH SOIL DEPOSITED BY GRAVITY ON SLOPE OR AT BOTTOM OF SLOPE. <b>CORE RECOVERY (REC.)</b> - TOTAL LENGTH OF ALL MATERIAL RECOVERED IN THE CORE BARREL DIVIDED BY TOTAL LENGTH OF CORE RUN AND EXPRESSED AS A PERCENTAGE. <b>DIKE</b> - A TABULAR BODY OF IGNEOUS ROCK THAT CUTS ACROSS THE STRUCTURE OF ADJACENT ROCKS OR CUTS MASSIVE ROCK. <b>DIP</b> - THE ANGLE AT WHICH A STRATUM OR ANY PLANAR FEATURE IS INCLINED FROM THE HORIZONTAL. <b>DIP DIRECTION (DIP AZIMUTH)</b> - THE DIRECTION OR BEARING OF THE HORIZONTAL TRACE OF THE LINE OF DIP, MEASURED CLOCKWISE FROM NORTH. <b>FAULT</b> - A FRACTURE OR FRACTURE ZONE ALONG WHICH THERE HAS BEEN DISPLACEMENT OF THE SIDES RELATIVE TO ONE ANOTHER PARALLEL TO THE FRACTURE. <b>FISSILE</b> - A PROPERTY OF SPLITTING ALONG CLOSELY SPACED PARALLEL PLANES. <b>FLOAT</b> - ROCK FRAGMENTS ON SURFACE NEAR THEIR ORIGINAL POSITION AND DISLODGED FROM PARENT MATERIAL. <b>FLOOD PLAIN (FP)</b> - LAND BORDERING A STREAM, BUILT OF SEDIMENTS DEPOSITED BY THE STREAM. <b>FORMATION (FM)</b> - A MAPPABLE GEOLOGIC UNIT THAT CAN BE RECOGNIZED AND TRACED IN THE FIELD. <b>JOINT</b> - FRACTURE IN ROCK ALONG WHICH NO APPRECIABLE MOVEMENT HAS OCCURRED. <b>LEDGE</b> - A SHELF-LIKE RIDGE OR PROJECTION OF ROCK WHOSE THICKNESS IS SMALL COMPARED TO ITS LATERAL EXTENT. <b>LENS</b> - A BODY OF SOIL OR ROCK THAT THINS OUT IN ONE OR MORE DIRECTIONS. <b>MOTTLED (MOT.)</b> - IRREGULARLY MARKED WITH SPOTS OF DIFFERENT COLORS, MOTTLED IN SOILS USUALLY INDICATES POOR AERATION AND LACK OF GOOD DRAINAGE. <b>PERCHED WATER</b> - WATER MAINTAINED ABOVE THE NORMAL GROUND WATER LEVEL BY THE PRESENCE OF AN INTERVENING IMPERVIOUS STRATUM. <b>RESIDUAL (RES.) SOIL</b> - SOIL FORMED IN PLACE BY THE WEATHERING OF ROCK. <b>ROCK QUALITY DESIGNATION (RQD)</b> - A MEASURE OF ROCK QUALITY DESCRIBED BY TOTAL LENGTH OF ROCK SEGMENTS EQUAL TO OR GREATER THAN 4 INCHES DIVIDED BY THE TOTAL LENGTH OF CORE RUN AND EXPRESSED AS A PERCENTAGE. <b>SAPROLITE (SAP.)</b> - RESIDUAL SOIL THAT RETAINS THE RELIC STRUCTURE OR FABRIC OF THE PARENT ROCK. <b>SILL</b> - AN INTRUSIVE BODY OF IGNEOUS ROCK OF APPROXIMATELY UNIFORM THICKNESS AND RELATIVELY THIN COMPARED WITH ITS LATERAL EXTENT, THAT HAS BEEN EMPLACED PARALLEL TO THE BEDDING OR SCHISTOSITY OF THE INTRUDED ROCKS. <b>SLICKENSIDE</b> - POLISHED AND STRIATED SURFACE THAT RESULTS FROM FRICTION ALONG A FAULT OR SLIP PLANE. <b>STANDARD PENETRATION TEST (PENETRATION RESISTANCE) (SPT)</b> - NUMBER OF BLOWS IN OR BPF) OF A 140 LB. HAMMER FALLING 30 INCHES REQUIRED TO PRODUCE A PENETRATION OF 1 FOOT INTO SOIL WITH A 2 INCH OUTSIDE DIAMETER SPLIT SPOON SAMPLER. SPT REFUSAL IS PENETRATION EQUAL TO OR LESS THAN 60 BLOWS PER FOOT. <b>STRATA CORE RECOVERY (SREC.)</b> - TOTAL LENGTH OF STRATA MATERIAL RECOVERED DIVIDED BY TOTAL LENGTH OF STRATUM AND EXPRESSED AS A PERCENTAGE. <b>STRATA ROCK QUALITY DESIGNATION (SROD)</b> - A MEASURE OF ROCK QUALITY DESCRIBED BY TOTAL LENGTH OF ROCK SEGMENTS WITHIN A STRATUM EQUAL TO OR GREATER THAN 4 INCHES DIVIDED BY THE TOTAL LENGTH OF STRATA AND EXPRESSED AS A PERCENTAGE. <b>TOPSOIL (TS)</b> - SURFACE SOILS USUALLY CONTAINING ORGANIC MATTER.																								
<b>SOIL LEGEND AND AASHTO CLASSIFICATION</b>	<b>MINERALOGICAL COMPOSITION</b>	<b>WEATHERING</b>																									
GENERAL CLASS. GRANULAR MATERIALS (<= 35% PASSING #200) SILT-CLAY MATERIALS (> 35% PASSING #200) ORGANIC MATERIALS	MINERAL NAMES SUCH AS QUARTZ, FELDSPAR, MICA, TALC, KAOLIN, ETC. ARE USED IN DESCRIPTIONS WHENEVER THEY ARE CONSIDERED OF SIGNIFICANCE.	<b>FRESH</b> - ROCK FRESH, CRYSTALS BRIGHT, FEW JOINTS MAY SHOW SLIGHT STAINING, ROCK RINGS UNDER HAMMER IF CRYSTALLINE. <b>VERY SLIGHT (V SLI.)</b> - ROCK GENERALLY FRESH, JOINTS STAINED, SOME JOINTS MAY SHOW THIN CLAY COATINGS IF OPEN, CRYSTALS ON A BROKEN SPECIMEN FACE SHINE BRIGHTLY, ROCK RINGS UNDER HAMMER BLOWS IF OF A CRYSTALLINE NATURE. <b>SLIGHT (SLI.)</b> - ROCK GENERALLY FRESH, JOINTS STAINED AND DISCOLORATION EXTENDS INTO ROCK UP TO 1 INCH. OPEN JOINTS MAY CONTAIN CLAY. IN GRANITOID ROCKS SOME OCCASIONAL FELDSPAR CRYSTALS ARE DULL AND DISCOLORED, CRYSTALLINE ROCKS RING UNDER HAMMER BLOWS. <b>MODERATE (MOD.)</b> - SIGNIFICANT PORTIONS OF ROCK SHOW DISCOLORATION AND WEATHERING EFFECTS. IN GRANITOID ROCKS, MOST FELDSPARS ARE DULL AND DISCOLORED, SOME SHOW CLAY. ROCK HAS DULL SOUND UNDER HAMMER BLOWS AND SHOWS SIGNIFICANT LOSS OF STRENGTH AS COMPARED WITH FRESH ROCK. <b>MODERATELY SEVERE (MOD. SEV.)</b> - ALL ROCK EXCEPT QUARTZ DISCOLORED OR STAINED. IN GRANITOID ROCKS, ALL FELDSPARS DULL AND DISCOLORED AND A MAJORITY SHOW KAOLINIZATION. ROCK SHOWS SEVERE LOSS OF STRENGTH AND CAN BE EXCAVATED WITH A GEOLOGIST'S PICK. ROCK GIVES "CLUNK" SOUND WHEN STRUCK. <i>IF TESTED, WOULD YIELD SPT REFUSAL</i> <b>SEVERE (SEV.)</b> - ALL ROCK EXCEPT QUARTZ DISCOLORED OR STAINED. ROCK FABRIC CLEAR AND EVIDENT BUT REDUCED IN STRENGTH TO STRONG SOIL. IN GRANITOID ROCKS ALL FELDSPARS ARE KAOLINIZED TO SOME EXTENT. SOME FRAGMENTS OF STRONG ROCK USUALLY REMAIN. <i>IF TESTED, YIELDS SPT N VALUES &gt; 100 BPF</i> <b>VERY SEVERE (V SEV.)</b> - ALL ROCK EXCEPT QUARTZ DISCOLORED OR STAINED. ROCK FABRIC ELEMENTS ARE DISCERNIBLE BUT THE MASS IS EFFECTIVELY REDUCED TO SOIL STATUS, WITH ONLY FRAGMENTS OF STRONG ROCK REMAINING. SAPROLITE IS AN EXAMPLE OF ROCK WEATHERED TO A DEGREE SUCH THAT ONLY MINOR VESTIGES OF THE ORIGINAL ROCK FABRIC REMAIN. <i>IF TESTED, YIELDS SPT N VALUES &lt; 100 BPF</i> <b>COMPLETE</b> - ROCK REDUCED TO SOIL. ROCK FABRIC NOT DISCERNIBLE, OR DISCERNIBLE ONLY IN SMALL AND SCATTERED CONCENTRATIONS. QUARTZ MAY BE PRESENT AS DIKES OR STRINGERS. SAPROLITE IS ALSO AN EXAMPLE.	<b>COMPRESSIONIBILITY</b> SLIGHTLY COMPRESSIBLE LIQUID LIMIT LESS THAN 31 MODERATELY COMPRESSIBLE LIQUID LIMIT EQUAL TO 31-50 HIGHLY COMPRESSIBLE LIQUID LIMIT GREATER THAN 50  <b>PERCENTAGE OF MATERIAL</b> <table border="1" style="width: 100%; border-collapse: collapse;"> <tr> <th>ORGANIC MATERIAL</th> <th>GRANULAR SOILS</th> <th>SILT-CLAY SOILS</th> <th>OTHER MATERIAL</th> </tr> <tr> <td>TRACE OF ORGANIC MATTER</td> <td>2 - 3%</td> <td>3 - 5%</td> <td>TRACE</td> </tr> <tr> <td>LITTLE ORGANIC MATTER</td> <td>3 - 5%</td> <td>5 - 12%</td> <td>LITTLE</td> </tr> <tr> <td>MODERATELY ORGANIC</td> <td>5 - 10%</td> <td>12 - 20%</td> <td>SOME</td> </tr> <tr> <td>HIGHLY ORGANIC</td> <td>&gt;10%</td> <td>&gt;20%</td> <td>HIGHLY</td> </tr> <tr> <td></td> <td></td> <td></td> <td>35% AND ABOVE</td> </tr> </table> <b>GROUND WATER</b>  WATER LEVEL IN BORE HOLE IMMEDIATELY AFTER DRILLING  STATIC WATER LEVEL AFTER 24 HOURS  PERCHED WATER, SATURATED ZONE, OR WATER BEARING STRATA  SPRING OR SEEP	ORGANIC MATERIAL	GRANULAR SOILS	SILT-CLAY SOILS	OTHER MATERIAL	TRACE OF ORGANIC MATTER	2 - 3%	3 - 5%	TRACE	LITTLE ORGANIC MATTER	3 - 5%	5 - 12%	LITTLE	MODERATELY ORGANIC	5 - 10%	12 - 20%	SOME	HIGHLY ORGANIC	>10%	>20%	HIGHLY				35% AND ABOVE
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<b>CONSISTENCY OR DENSENESS</b>	<b>MISCELLANEOUS SYMBOLS</b>	<b>ROCK HARDNESS</b>																									
PRIMARY SOIL TYPE COMPACTNESS OR CONSISTENCY RANGE OF STANDARD PENETRATION RESISTANCE (N-VALUE) RANGE OF UNCONFINED COMPRESSIVE STRENGTH (TONS/FT <sup>2</sup> )	 ROADWAY EMBANKMENT (RE) WITH SOIL DESCRIPTION  SOIL SYMBOL  ARTIFICIAL FILL (AF) OTHER THAN ROADWAY EMBANKMENT  INFERRED SOIL BOUNDARY  INFERRED ROCK LINE  ALLUVIAL SOIL BOUNDARY  DIP & DIP DIRECTION OF ROCK STRUCTURES	<b>VERY HARD</b> - CANNOT BE SCRATCHED BY KNIFE OR SHARP PICK. BREAKING OF HAND SPECIMENS REQUIRES SEVERAL HARD BLOWS OF THE GEOLOGIST'S PICK. <b>HARD</b> - CAN BE SCRATCHED BY KNIFE OR PICK ONLY WITH DIFFICULTY. HARD HAMMER BLOWS REQUIRED TO DETACH HAND SPECIMEN. <b>MODERATELY HARD</b> - CAN BE SCRATCHED BY KNIFE OR PICK, GOUGES OR GROOVES TO 0.25 INCHES DEEP CAN BE EXCAVATED BY HARD BLOW OF A GEOLOGIST'S PICK. HAND SPECIMENS CAN BE DETACHED BY MODERATE BLOWS. <b>MEDIUM HARD</b> - CAN BE GROOVED OR GOUGED 0.05 INCHES DEEP BY FIRM PRESSURE OF KNIFE OR PICK POINT. CAN BE EXCAVATED IN SMALL CHIPS TO PIECES 1 INCH MAXIMUM SIZE BY HARD BLOWS OF THE POINT OF A GEOLOGIST'S PICK. <b>SOFT</b> - CAN BE GROOVED OR GOUGED READILY BY KNIFE OR PICK. CAN BE EXCAVATED IN FRAGMENTS FROM CHIPS TO SEVERAL INCHES IN SIZE BY MODERATE BLOWS OF A PICK POINT. SMALL, THIN PIECES CAN BE BROKEN BY FINGER PRESSURE. <b>VERY SOFT</b> - CAN BE CARVED WITH KNIFE. CAN BE EXCAVATED READILY WITH POINT OF PICK. PIECES 1 INCH OR MORE IN THICKNESS CAN BE BROKEN BY FINGER PRESSURE. CAN BE SCRATCHED READILY BY FINGER NAIL.																									
<b>TEXTURE OR GRAIN SIZE</b>	<b>ABBREVIATIONS</b>	<b>FRACTURE SPACING</b>	<b>BEDDING</b>																								
U.S. STD. SIEVE SIZE OPENING (MM) 4 10 40 60 200 270 4.76 2.00 0.42 0.25 0.075 0.053	AR - AUGER REFUSAL MED. - MEDIUM VST - VANE SHEAR TEST BT - BORING TERMINATED MICA - MICACEOUS WEA. - WEATHERED CL - CLAY MOD. - MODERATELY W - UNIT WEIGHT CPT - CONE PENETRATION TEST NP - NON PLASTIC W <sub>d</sub> - DRY UNIT WEIGHT CSE - COARSE ORG. - ORGANIC DMT - DILATOMETER TEST PMT - PRESSUREMETER TEST DPT - DYNAMIC PENETRATION TEST SAP. - SAPROLITIC SD. - SAND, SANDY F - FINE SL. - SILT, SILTY FOSS. - FOSSILIFEROUS SLL - SLIGHTLY FRAC. - FRACTURED, FRACTURES TCR - TRICONE REFUSAL FRAGS. - FRAGMENTS W - MOISTURE CONTENT HI. - HIGHLY V - VERY	<b>VERY WIDE</b> - MORE THAN 10 FEET <b>WIDE</b> - 3 TO 10 FEET <b>MODERATELY CLOSE</b> - 1 TO 3 FEET <b>CLOSE</b> - 0.16 TO 1 FEET <b>VERY CLOSE</b> - LESS THAN 0.16 FEET	<b>VERY THICKLY BEDDED</b> - > 4 FEET <b>THICKLY BEDDED</b> - 1.5 - 4 FEET <b>THINLY BEDDED</b> - 0.16 - 1.5 FEET <b>VERY THINLY BEDDED</b> - 0.03 - 0.16 FEET <b>THICKLY LAMINATED</b> - 0.008 - 0.03 FEET <b>THINLY LAMINATED</b> - < 0.008 FEET																								
<b>SOIL MOISTURE - CORRELATION OF TERMS</b>	<b>EQUIPMENT USED ON SUBJECT PROJECT</b>	<b>INDURATION</b>																									
SOIL MOISTURE SCALE (ATTERBERG LIMITS) FIELD MOISTURE DESCRIPTION GUIDE FOR FIELD MOISTURE DESCRIPTION	DRILL UNITS: <input type="checkbox"/> MOBILE B- <input type="checkbox"/> BK-51 <input checked="" type="checkbox"/> CME-45B <input type="checkbox"/> CME-550 <input type="checkbox"/> PORTABLE HOIST	<b>FRIABLE</b> - RUBBING WITH FINGER FREES NUMEROUS GRAINS; GENTLE BLOW BY HAMMER DISINTEGRATES SAMPLE. <b>MODERATELY INDURATED</b> - GRAINS CAN BE SEPARATED FROM SAMPLE WITH STEEL PROBE; BREAKS EASILY WHEN HIT WITH HAMMER. <b>INDURATED</b> - GRAINS ARE DIFFICULT TO SEPARATE WITH STEEL PROBE; DIFFICULT TO BREAK WITH HAMMER. <b>EXTREMELY INDURATED</b> - SHARP HAMMER BLOWS REQUIRED TO BREAK SAMPLE; SAMPLE BREAKS ACROSS GRAINS.																									
<b>PLASTICITY</b>	<b>ADVANCING TOOLS:</b> <input type="checkbox"/> CLAY BITS <input type="checkbox"/> 6" CONTINUOUS FLIGHT AUGER <input type="checkbox"/> 8" HOLLOW AUGERS <input type="checkbox"/> HARD FACED FINGER BITS <input type="checkbox"/> TUNG-CARBIDE INSERTS <input checked="" type="checkbox"/> CASING <input type="checkbox"/> W/ ADVANCER <input checked="" type="checkbox"/> TRICONE 2 15/16" STEEL TEETH <input type="checkbox"/> TRICONE " TUNG-CARB. <input type="checkbox"/> CORE BIT	<b>FOR SEDIMENTARY ROCKS, INDURATION IS THE HARDENING OF THE MATERIAL BY CEMENTING, HEAT, PRESSURE, ETC.</b>																									
NONPLASTIC PLASTICITY INDEX (PI) DRY STRENGTH VERY LOW SLIGHT MEDIUM HIGH	<b>HAMMER TYPE:</b> <input checked="" type="checkbox"/> AUTOMATIC <input type="checkbox"/> MANUAL <b>CORE SIZE:</b> <input type="checkbox"/> -B <input type="checkbox"/> -N <input type="checkbox"/> -H <b>HAND TOOLS:</b> <input type="checkbox"/> POST HOLE DIGGER <input checked="" type="checkbox"/> HAND AUGER <input type="checkbox"/> SOUNDING ROD <input checked="" type="checkbox"/> VANE SHEAR TEST																										
DESCRIPTIONS MAY INCLUDE COLOR OR COLOR COMBINATIONS (TAN, RED, YELLOW-BROWN, BLUE-GRAY). MODIFIERS SUCH AS LIGHT, DARK, STREAKED, ETC. ARE USED TO DESCRIBE APPEARANCE.																											



BENCH MARK: BM NO. 5 RR SPIKE IN BASE OF 20' MAPLE AT -L- STA. 19+63  
44' RT  
ELEVATION: 35.70 FT.

NOTES:

See Sheet 1-A For Index of Sheets  
See Sheet 1-B For Symbology

STATE OF NORTH CAROLINA  
DIVISION OF HIGHWAYS

**PASQUOTANK COUNTY**

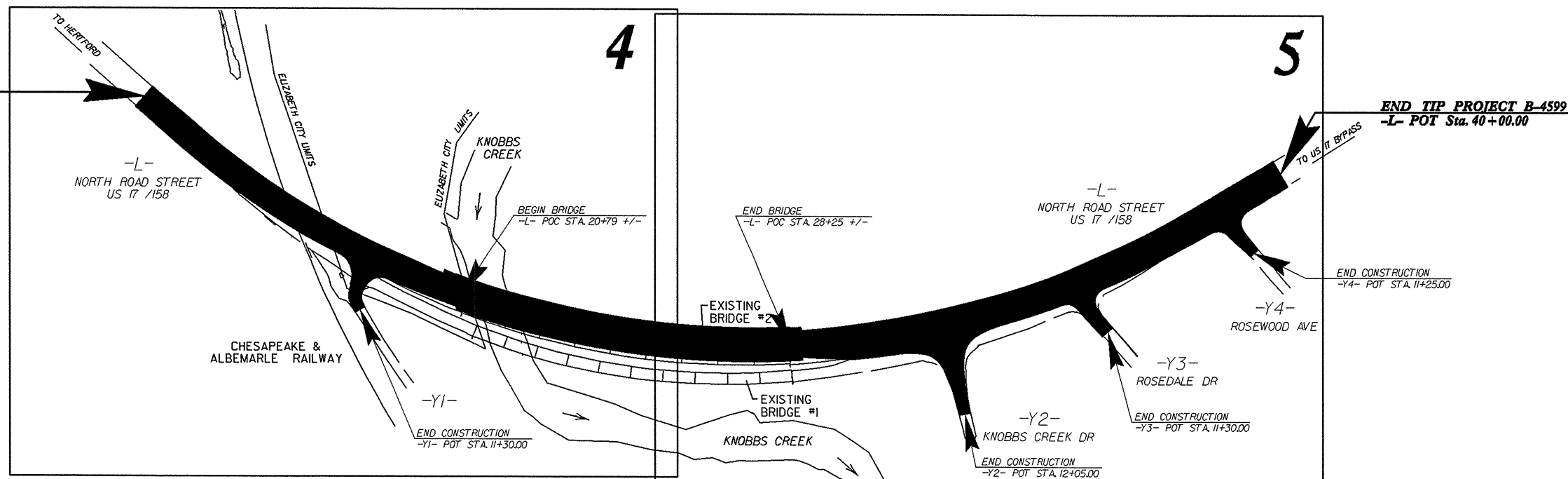
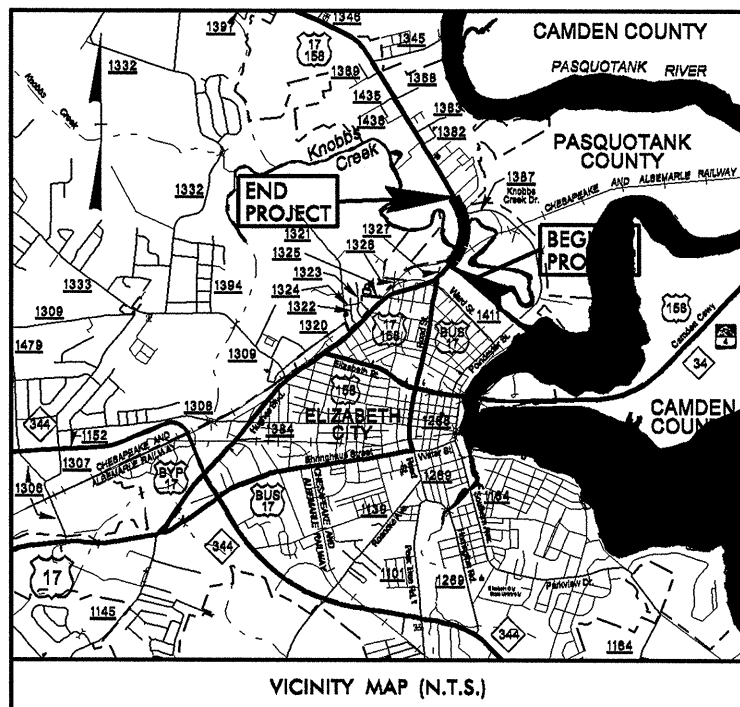
**LOCATION: BRIDGES NOS. 1 & 2 OVER KNOBBS CREEK  
ON US 17 / US 158.**

**TYPE OF WORK: GRADING, DRAINAGE, PAVING,  
CURB & GUTTER & STRUCTURE.**

STATE	STATE PROJECT REFERENCE NO.	SHEET NO.	TOTAL SHEETS
N.C.	B-4599	3	18
STATE PROJ. NO.	P.A. PROJ. NO.	DESCRIPTION	
33791.J.J.	BRSTP-17(41)	PE	

**25% REVISED**

INCOMPLETE PLANS  
DO NOT USE FOR R/W ACQUISITION  
PRELIMINARY PLANS  
DO NOT USE FOR CONSTRUCTION

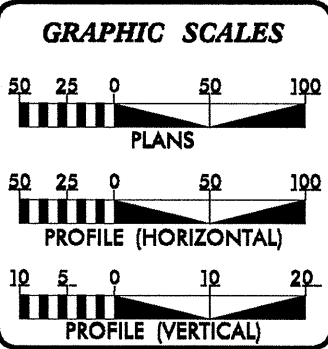


CLEARING ON THIS PROJECT SHALL BE PERFORMED TO THE LIMITS ESTABLISHED BY METHOD II.  
A PORTION OF THIS PROJECT IS WITHIN THE MUNICIPAL BOUNDARIES OF ELIZABETH CITY.

NGDOT CONTACT:  
MR. DOUG TAYLOR, PE - ENGINEERING COORDINATION - PROJECT ENGINEER - ROADWAY DESIGN UNIT

**TIP PROJECT: B-4599**

**CONTRACT:**



**DESIGN DATA**

ADT 2011	=	40,700
ADT 2031	=	58,500
DHV	=	10 %
D	=	60 %
T	=	Z. % *
V	=	50 MPH
* (TTST 4% + DUAL 3%)		
FUNC. CLASS = URBAN PRINCIPAL ARTERIAL		

**PROJECT LENGTH**

LENGTH ROADWAY TIP PROJECT B-4459	=	0.382 mile
LENGTH STRUCTURE TIP PROJECT B-4459	=	0.141 mile
TOTAL LENGTH TIP PROJECT B-4459	=	0.523 mile

Prepared For:  
**DIVISION OF HIGHWAYS**  
1000 Birch Ridge Dr., Raleigh NC, 27610  
By:  
**MA ENGINEERING CONSULTANTS, INC.**  
598 EAST CHATHAM STREET, SUITE 137  
CARY, NORTH CAROLINA 27511  
(919) 297-0220

2006 STANDARD SPECIFICATIONS

RIGHT OF WAY DATE:  
FEBRUARY 19, 2010

LETTING DATE:  
FEBRUARY 15, 2011

ROBERT W. PORTER, JR. PE  
PROJECT ENGINEER

KEVIN S. HUTCHENS  
PROJECT DESIGN ENGINEER

**HYDRAULICS ENGINEER**

SIGNATURE: \_\_\_\_\_ P.E.

**ROADWAY DESIGN ENGINEER**

SIGNATURE: \_\_\_\_\_ P.E.

**DIVISION OF HIGHWAYS  
STATE OF NORTH CAROLINA**

STATE HIGHWAY DESIGN ENGINEER

14-FEB-2011 10:05 L:\ERO\Green\ville\_investigation\TIP\B4599.GEO\_RDWY\CADD\_GEO\TECH\Site&Sub\B4599.GEO\_rdwyttitle&legend\_REC.dgn  
cbturner AT GEG25346

Approximate quantities only. Clearing and grubbing, unclassified excavation, fine grading, removal of existing pavement, and breaking of existing asphalt pavement will be paid for at the contract lump sum price for "GRADING".

# EARTHWORK BALANCE SHEET

Volumes in Cubic Yards

3A

Earthwork quantities are calculated by the Roadway Design Unit. These earthwork quantities are based in part on subsurface data provided by the Geotechnical Engineering Unit.

PROJECT: B-4599 33971.3.1

COUNTY: Pasquotank

DATE: 02/21/2012

COMPUTED BY: RWP

SHEET: 1 OF 1

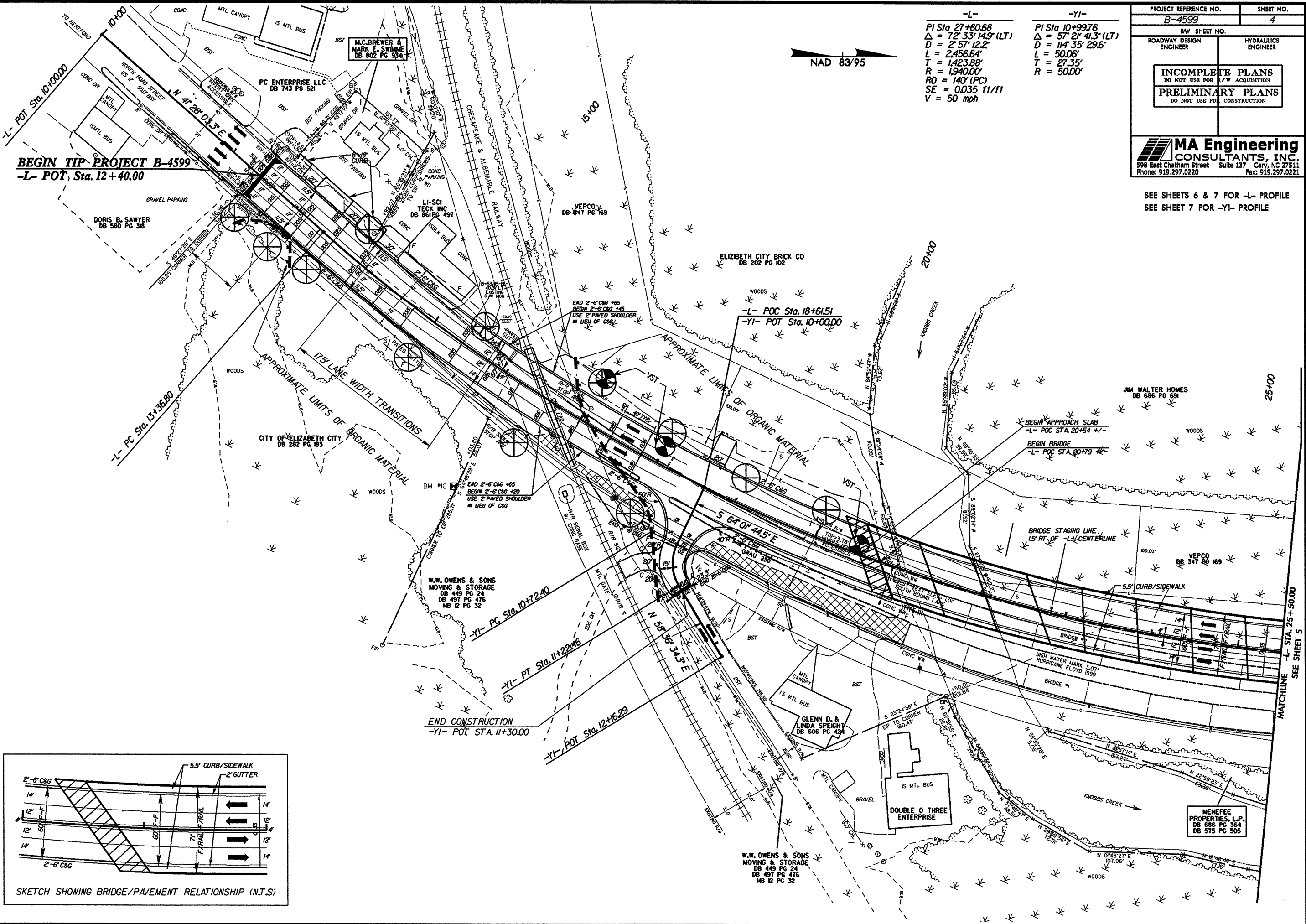
STATION to STATION	EXCAVATION					EMBANKMENT				BORROW	WASTE			
	TOTAL UNCLASS.	ROCK	TOTAL UNDERCUT	UNSUITABLE EARTH	SUITABLE EARTH	TOTAL	ROCK	EARTH	EARTH +30%		ROCK	SUITABLE	UNSUITABLE	TOTAL
<b>PHASE 1</b>														
-L- LT. 12+25.00 TO 20+79.00 (BEGIN BRIDGE)	146				146	1515		1515	1970	1824				
-L- LT. 16+60+/- TO 19+25+/-			2239			2239		2239	2911	2911		2239	2239	
-L- LT. 28+25.00 (END BRIDGE) TO 40+00.00	853				853	1763		1763	2292	1439				
-L- LT. 35+25 TO 40+00			411			411		411	534	534		411	411	
<b>TOTAL (PHASE 1)</b>	<b>999</b>		<b>2650</b>		<b>999</b>	<b>5928</b>		<b>5928</b>	<b>7707</b>	<b>6708</b>		<b>2650</b>	<b>2650</b>	
<b>PHASE 2</b>														
-L- CL-RT. 19+00.00 TO 20+79.00 (BEGIN BRIDGE)						504		504	655	655				
-L- CL-RT. 28+25.00 (END BRIDGE) TO 33+50.00						872		872	1134	1134				
<b>TOTAL (PHASE 2)</b>						<b>1376</b>		<b>1376</b>	<b>1789</b>	<b>1789</b>				
<b>PHASE 3</b>														
-L- RT. 12+25.00 TO 20+79.00 (BEGIN BRIDGE)	18				18	917		917	1192	1174				
-L- RT. 12+60+/- TO 13+30+/-			241			241		241	313	313		241	241	
-Y1- 10+50.00 TO 11+65.00	38				38	25		25	33		5		5	
-L- RT. 28+25.00 (END BRIDGE) TO 40+00.00	11				11	923		923	1200	1189				
-Y2- 10+75.00 TO 12+05.00	6				6	146		146	190	184				
-Y3- 10+50.00 TO 11+30.00	52				52	21		21	27		25		25	
-Y4- 10+50.00 TO 11+25.00	18				18	19		19	25	7				
<b>SUBTOTAL (PHASE 3)</b>	<b>143</b>		<b>241</b>		<b>143</b>	<b>2292</b>		<b>2292</b>	<b>2980</b>	<b>2867</b>	<b>30</b>	<b>241</b>	<b>271</b>	
WASTE TO REPLACE BORROW										-30	-30		-30	
<b>TOTAL (PHASE 3)</b>	<b>143</b>		<b>241</b>		<b>143</b>	<b>2292</b>		<b>2292</b>	<b>2980</b>	<b>2837</b>		<b>241</b>	<b>241</b>	
<b>TOTAL (ALL PHASES)</b>	<b>1142</b>		<b>2891</b>		<b>1142</b>	<b>9596</b>		<b>9596</b>	<b>12476</b>	<b>11334</b>		<b>2891</b>	<b>2891</b>	
EST. LOSS DUE TO CLEARING & GRUBBING*	N/A				N/A					N/A				
ADDITIONAL UNDERCUT (CONTINGENCY)			1000			1000		1000	1300	1300		1000	1000	
USE SELECT GRANULAR MATERIAL, CLASS III IN LIEU OF BACKFILL FOR UNDERCUT						-3891		-3891	-5058	-5058				
<b>PROJECT TOTAL</b>	<b>1142</b>		<b>3891</b>		<b>1142</b>	<b>6705</b>		<b>6705</b>	<b>8718</b>	<b>7576</b>		<b>3891</b>	<b>3891</b>	
ESTIMATED 5% TO REPLACE TOPSOIL ON BORROW PIT										379				
<b>GRAND TOTALS (CUBIC YARDS)</b>	<b>1,142</b>		<b>3,891</b>		<b>1,142</b>	<b>6,705</b>		<b>6,705</b>	<b>8,718</b>	<b>7,955</b>		<b>3,891</b>	<b>3,891</b>	
<b>SAY (CUBIC YARDS)</b>	<b>1,200</b>		<b>3,900</b>							<b>8,000</b>				

SELECT GRANULAR MATERIAL (CLASS III) = 5,400 CY (3,700 CY BACKFILL FOR UNDERCUT + 1,700 CY CONTINGENCY PER 'GEOTECHNICAL REPORT - DESIGN AND CONSTRUCTION RECOMMENDATIONS' LETTER DATED JANUARY 31, 2012)

ESTIMATED GEOTEXTILE FOR SOIL STABILIZATION = 1,600 SY (900 SY FOR UNDERCUT AREAS + 700 SY CONTINGENCY PER 'GEOTECHNICAL REPORT - DESIGN AND CONSTRUCTION RECOMMENDATIONS' LETTER DATED JANUARY 31, 2012)

\* NO SIGNIFICANT LOSS OF UNCLASSIFIED EXCAVATION IS ANTICIPATED DUE TO CLEARING AND GRUBBING (PER 'GEOTECHNICAL REPORT - DESIGN AND CONSTRUCTION RECOMMENDATIONS' LETTER DATED JANUARY 31, 2012)

8/17/99  
 REVISIONS  
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 Author: ALI



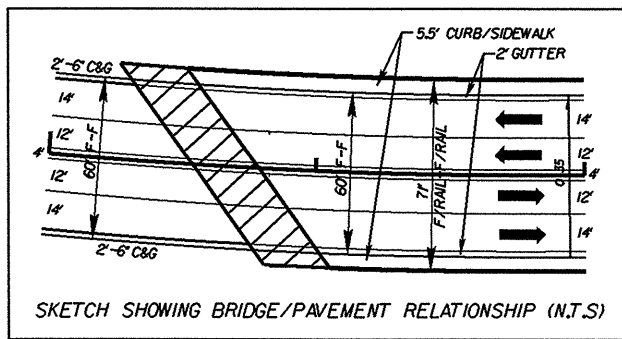
-L-	-YI-
PI Sta 27+60.68	PI Sta 10+99.76
$\Delta = 72^{\circ} 33' 14.9''$ (LT)	$\Delta = 57^{\circ} 21' 41.3''$ (LT)
D = 2' 57' 12.2"	D = 114' 35' 29.6"
L = 2,456.64'	L = 50.06'
T = 1,423.88'	T = 27.35'
R = 1,940.00'	R = 50.00'
RO = 140' (PC)	
SE = 0.035 f1/f1	
V = 50 mph	

NAD 83/95

PROJECT REFERENCE NO. B-4599	SHEET NO. 4
RWY SHEET NO.	
ROADWAY DESIGN ENGINEER	HYDRAULICS ENGINEER
<b>INCOMPLETE PLANS</b> DO NOT USE FOR R/W ACQUISITION <b>PRELIMINARY PLANS</b> DO NOT USE FOR CONSTRUCTION	
<b>MA Engineering</b> CONSULTANTS, INC. 598 East Chatham Street Suite 137 Cary, NC 27511 Phone: 919.297.0220 Fax: 919.297.0221	

SEE SHEETS 6 & 7 FOR -L- PROFILE  
 SEE SHEET 7 FOR -YI- PROFILE

**BEGIN TIP PROJECT B-4599**  
 -L- POT Sta. 12+40.00



MATCHLINE -L- STA. 25+50.00  
SEE SHEET 5



5/14/99

15-FEB-2011 09:43 L:\EROV\green\proj\B4599\GEO\RDWY\_CADD\_GEO\TECH\Plan\Prof\AB-4599\_GEO\_PFI.LI\_REC.dgn

**VANE SHEAR TESTS**

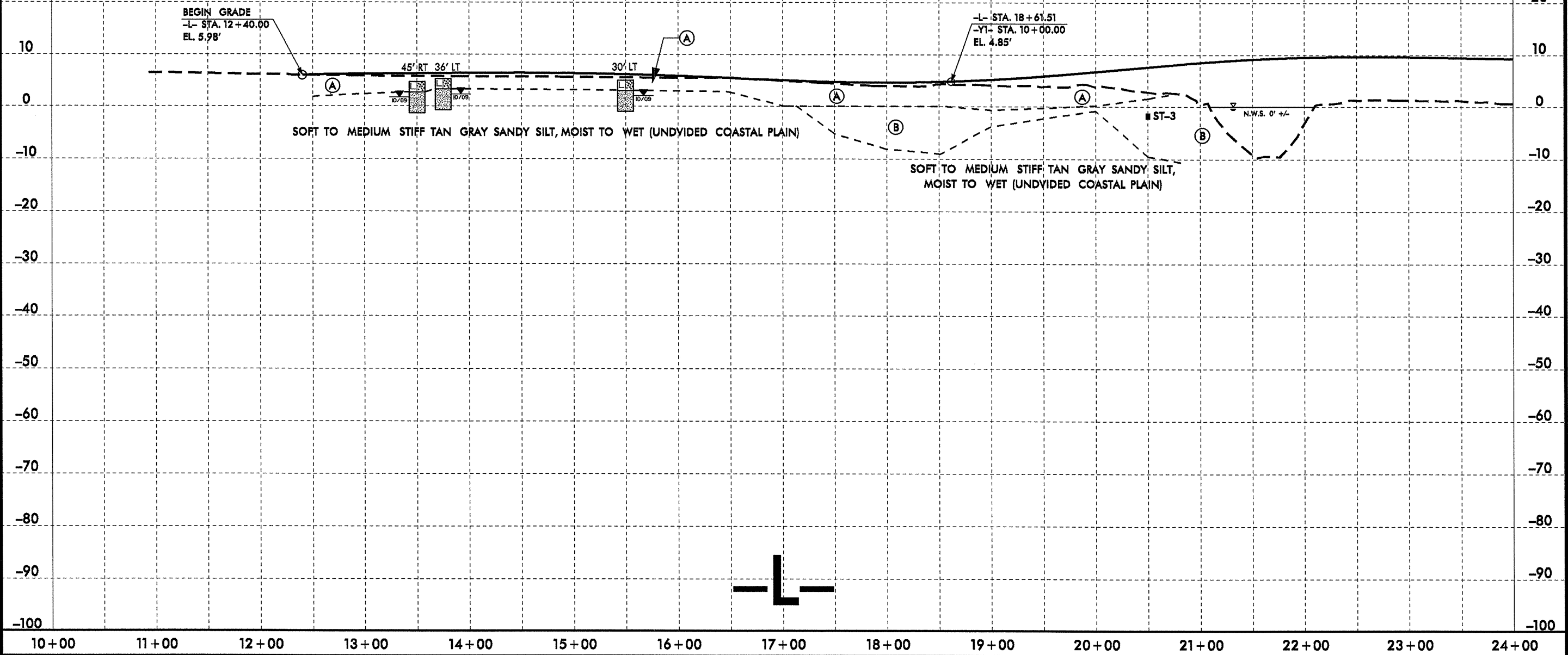
STATION	OFFSET	DEPTH	S (psf)
17+00	55' LT	0.0	271.44
17+00	55' LT	0.5	292.32
17+00	55' LT	1.0	208.80
17+00	55' LT	1.5	167.04
17+00	55' LT	2.0	334.88
17+00	55' LT	2.5	563.76
17+00	55' LT	3.0	167.04
17+00	55' LT	3.5	208.80
17+00	55' LT	4.0	187.92
17+00	55' LT	4.5	375.84
17+00	55' LT	5.0	313.20
17+00	55' LT	5.5	375.84
17+00	55' LT	6.0	709.92
17+00	55' LT	6.5	501.12
17+00	55' LT	7.0	1002.24
17+00	55' LT	7.5	876.96
17+00	55' LT	8.0	1106.64
17+00	55' LT	8.5	1148.40
17+00	55' LT	9.0	793.44
17+00	55' LT	9.5	1106.64
17+00	55' LT	10.0	793.44
17+00	55' LT	10.5	835.20
17+00	55' LT	11.0	772.56
18+00	21' LT	1.0	271.44
18+00	21' LT	2.0	292.32
18+00	21' LT	3.0	208.80
18+00	21' LT	4.0	167.04
18+00	21' LT	5.0	334.88
18+00	21' LT	6.0	563.76
20+50	10' LT	1.0	1739
20+50	10' LT	2.0	2312
20+50	10' LT	3.0	2405
20+50	10' LT	4.0	1739
20+50	10' LT	5.0	2016.5
20+50	10' LT	6.0	MAX

**-L-**  
**US 17 / 158**  
**(NORTH ROAD ST.)**

PROJECT REFERENCE NO. <b>B-4599</b>	SHEET NO. <b>6</b>
ROADWAY DESIGN ENGINEER	HYDRAULICS ENGINEER
<b>INCOMPLETE PLANS</b> DO NOT USE FOR ACQUISITION	
<b>PRELIMINARY PLANS</b> DO NOT USE FOR CONSTRUCTION	

SAMPLE NO.	OFFSET	STATION	DEPTH INTERVAL	AASHTO CLASS.	L.L.	P.I.	% BY WEIGHT				% PASSING (SIEVES)			% MOISTURE	% ORGANIC
							C. SAND	F. SAND	SILT	CLAY	10	40	60		
ST-1	49' LT	17+00	0.00-2.00	A-2-5(0)	68	10	38.8	43.3	9.7	8.2	96	82	19	125.66	22.5
ST-2	20' LT	18+00	1.00-3.00	A-2-6(1)	31	15	49.8	22.3	5.7	22.2	96	69	28	75.93	2.2
ST-3	10' LT	20+50	4.00-5.00	A-4(3)	33	3	8.5	31.2	46.2	14.1	99	96	8	143.87	9.3

- (A) LOOSE TAN SAND, MOIST TO SATURATED (ROADWAY EMBANKMENT)
- (B) SOFT BROWN MUCK AND SANDY SILT WITH LITTLE ORGANIC MATERIAL, WET (ALLUVIAL)



5/14/99

15-FEB-2011 08:28  
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**VANE SHEAR TESTS**

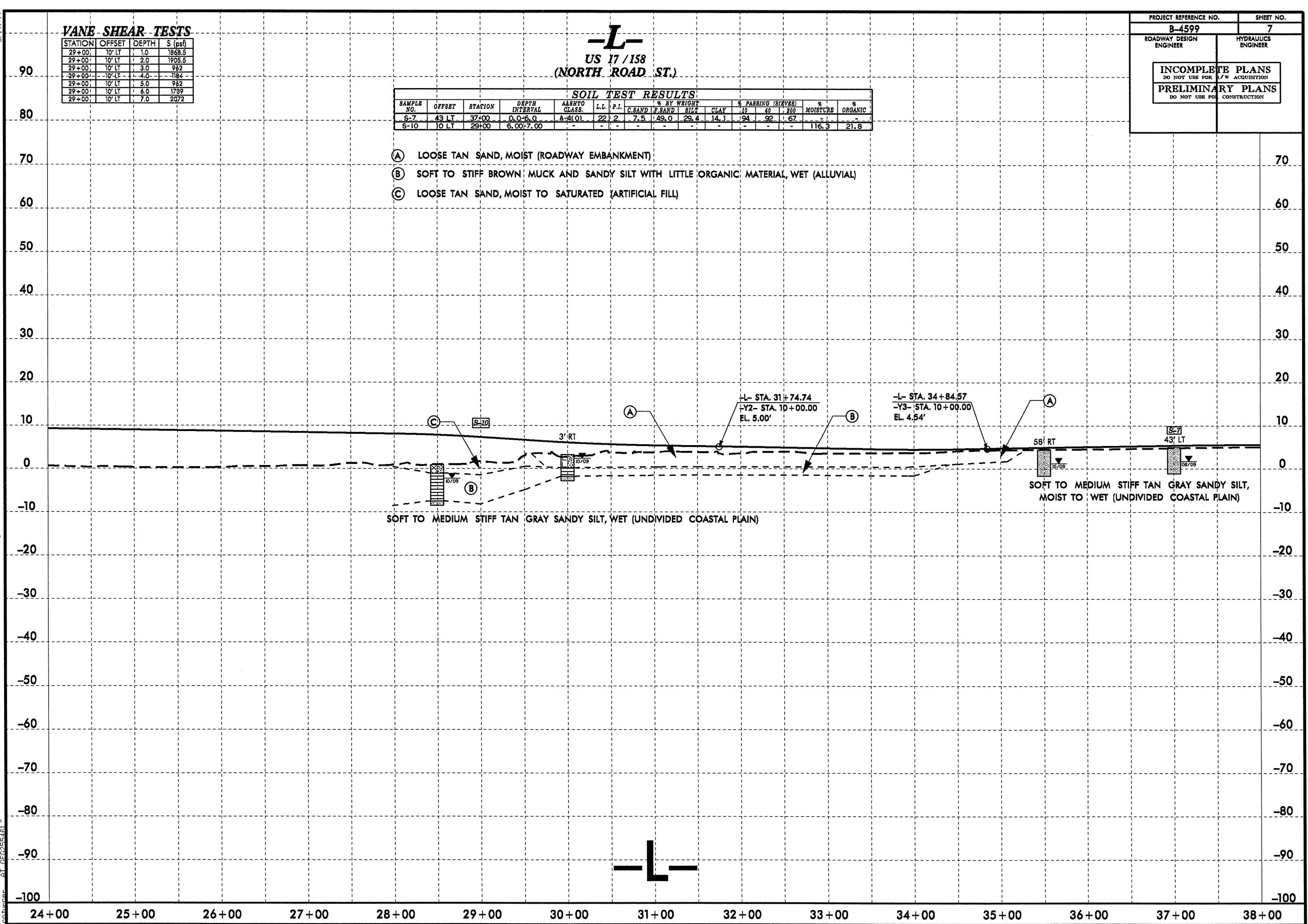
STATION	OFFSET	DEPTH	S (psf)
29+00	10' LT	1.0	1868.5
29+00	10' LT	2.0	1905.5
29+00	10' LT	3.0	962
29+00	10' LT	4.0	1184
29+00	10' LT	5.0	962
29+00	10' LT	6.0	1789
29+00	10' LT	7.0	2072

**-L-**  
**US 17 / 158**  
**(NORTH ROAD ST.)**

PROJECT REFERENCE NO. <b>B-4599</b>	SHEET NO. <b>7</b>
ROADWAY DESIGN ENGINEER	HYDRAULICS ENGINEER
<b>INCOMPLETE PLANS</b> DO NOT USE FOR ACQUISITION	
<b>PRELIMINARY PLANS</b> DO NOT USE FOR CONSTRUCTION	

SOIL TEST RESULTS															
SAMPLE NO.	OFFSET	STATION	DEPTH INTERVAL	ASHTO CLASS.	L.L.	P.I.	% BY WEIGHT				% PASSING (SIEVES)			% MOISTURE	% ORGANIC
							C.SAND	F.SAND	SILT	CLAY	#10	#40	#200		
S-7	43' LT	37+00	0.0-6.0	A-4(O)	22	2	7.5	49.0	23.4	14.1	94	92	67	-	-
S-10	10' LT	29+00	6.00-7.00	-	-	-	-	-	-	-	-	-	116.3	21.8	

- (A) LOOSE TAN SAND, MOIST (ROADWAY EMBANKMENT)
- (B) SOFT TO STIFF BROWN MUCK AND SANDY SILT WITH LITTLE ORGANIC MATERIAL, WET (ALLUVIAL)
- (C) LOOSE TAN SAND, MOIST TO SATURATED (ARTIFICIAL FILL)



70  
60  
50  
40  
30  
20  
10  
0  
-10  
-20  
-30  
-40  
-50  
-60  
-70  
-80  
-90  
-100

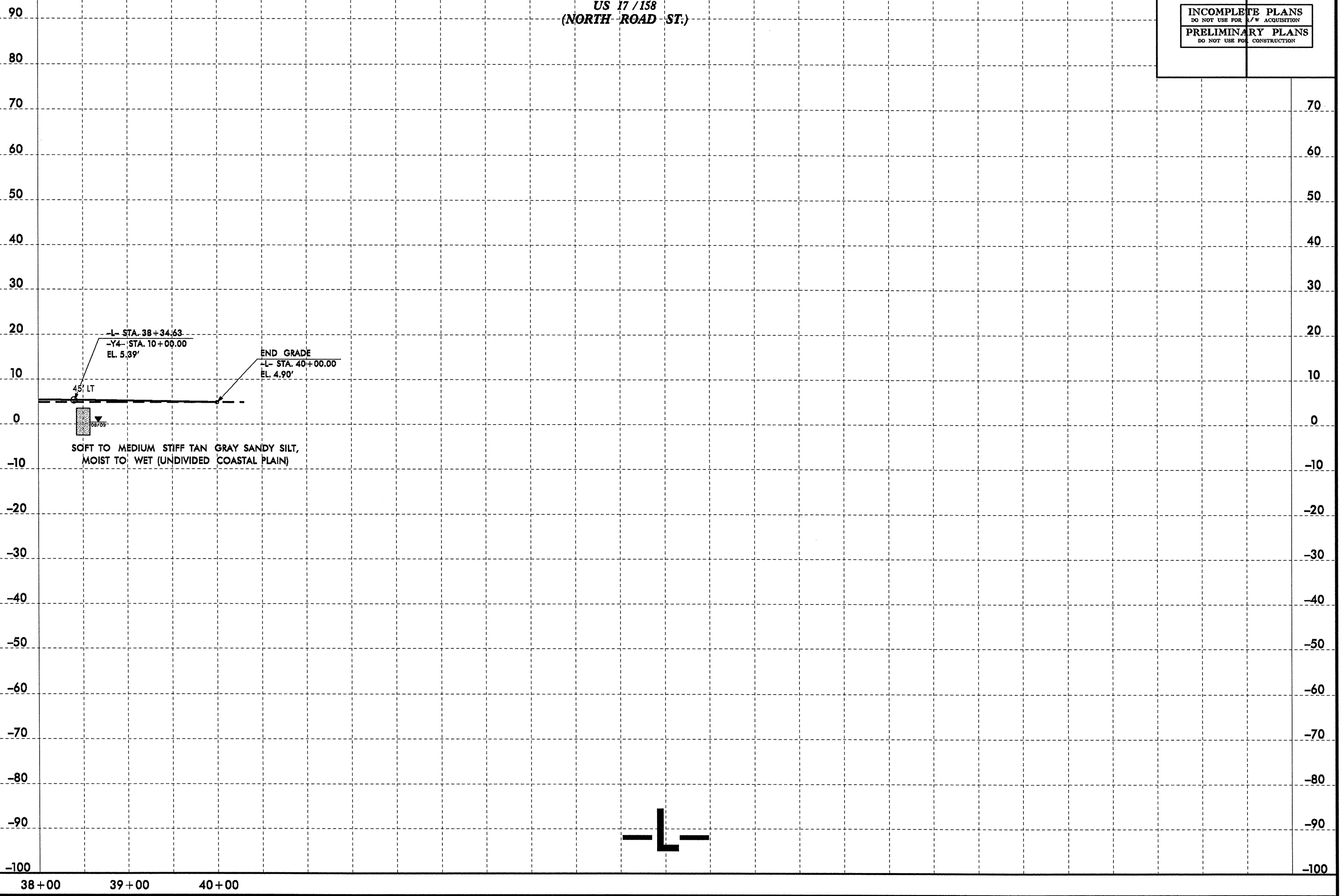
24+00 25+00 26+00 27+00 28+00 29+00 30+00 31+00 32+00 33+00 34+00 35+00 36+00 37+00 38+00



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Author: AT 06/25/06

**-L-**  
**US 17 / 158**  
**(NORTH ROAD ST.)**

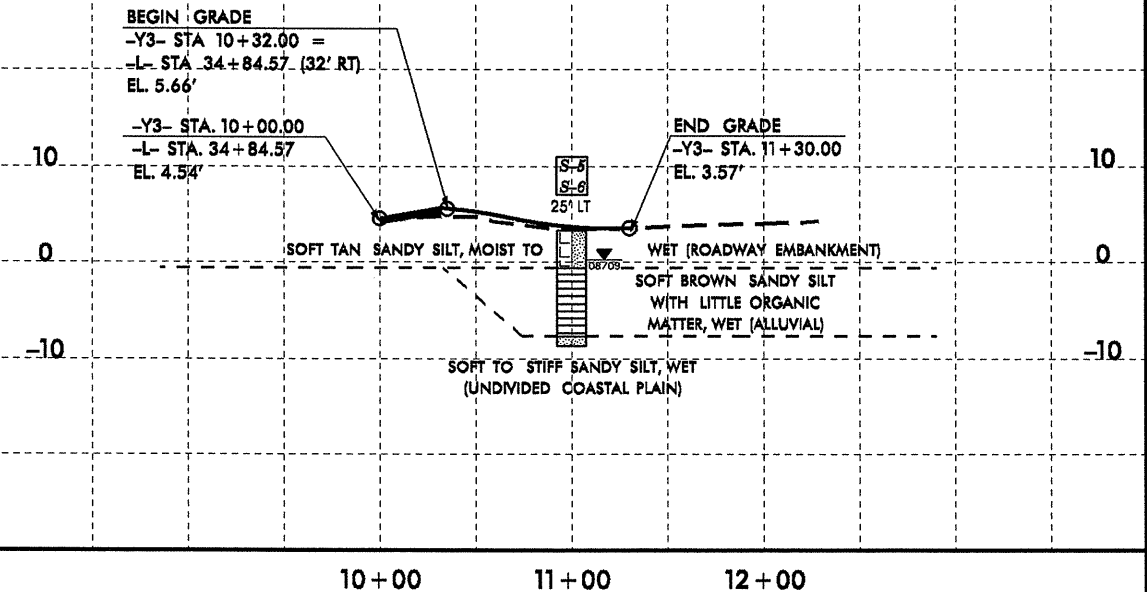
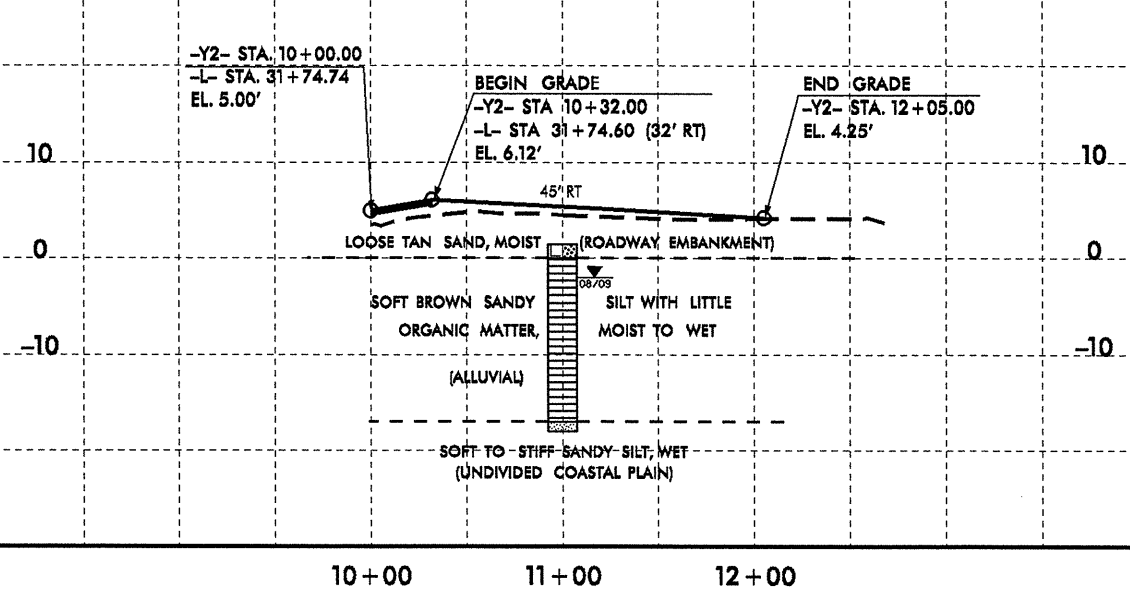
PROJECT REFERENCE NO.	SHEET NO.
B-4599	8
ROADWAY DESIGN ENGINEER	HYDRAULICS ENGINEER
<b>INCOMPLETE PLANS</b> DO NOT USE FOR ACQUISITION	
<b>PRELIMINARY PLANS</b> DO NOT USE FOR CONSTRUCTION	



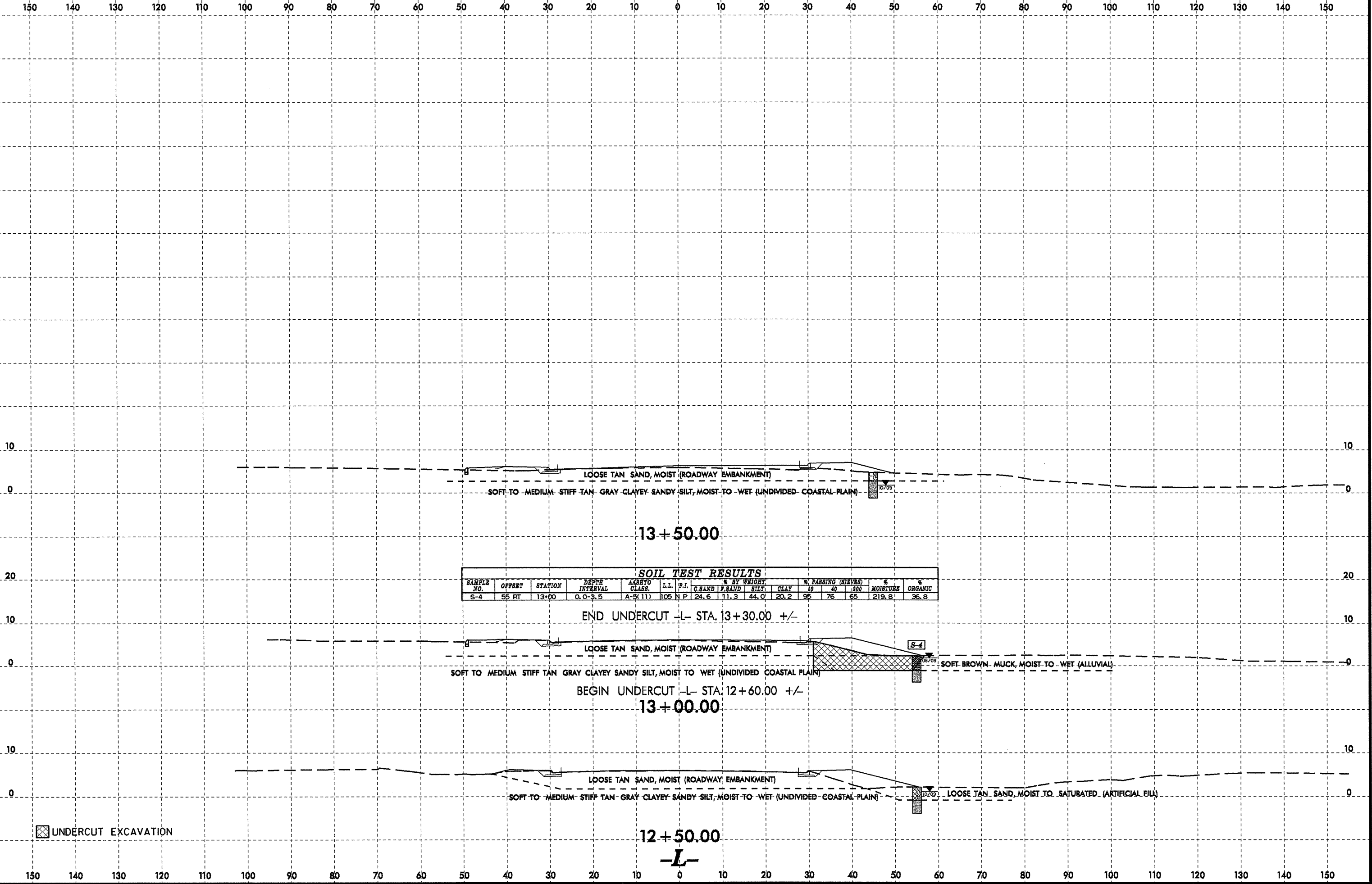
**-Y2-**  
**KNOBBS CREEK DR.**

**-Y3-**  
**ROSEDALE DR.**

SAMPLE NO.	OFFSET	STATION	DEPTH INTERVAL	AASHTO CLASS.	L.L.	P.I.	% BY WEIGHT				% PASSING (SIEVES)			% MOISTURE	% ORGANIC
							C. SAND	F. SAND	SILT	CLAY	10	40	200		
S-5	25 LT	11+00	0.0-4.0	A-4(0)	19	2	6.3	41.1	36.5	16.1	100	99	71	-	-
S-6	25 LT	11+00	4.0-11.0	A-4(6)	30	8	2.4	55.2	2.0	40.3	100	99	88	-	5.6



8/23/99  
3:\JAN-2012 0854  
S:\Projects\B4599\GEO\RDWY\CADD\GEO\RDWY\CADD\GEO\RDWY\CADD\GEO\XSR.L.dgn



**SOIL TEST RESULTS**

SAMPLE NO.	OFFSET	STATION	DEPTH INTERVAL	ASTM CLASS.	L.L.	P.I.	BY WEIGHT				% PASSING (SIEVES)		% MOISTURE	% ORGANIC	
							C. SAND	F. SAND	SILT	CLAY	#10	#40			
S-4	55 RT	13+00	0.0-3.5	A-3(11)	105	N	24.6	11.3	44.0	20.2	95	76	65	219.8	36.8

END UNDERCUT -L- STA. 13+30.00 +/-

BEGIN UNDERCUT -L- STA. 12+60.00 +/-

12+50.00

-L-

⊠ UNDERCUT EXCAVATION

150 140 130 120 110 100 90 80 70 60 50 40 30 20 10 0 10 20 30 40 50 60 70 80 90 100 110 120 130 140 150

**VANE SHEAR TESTS**

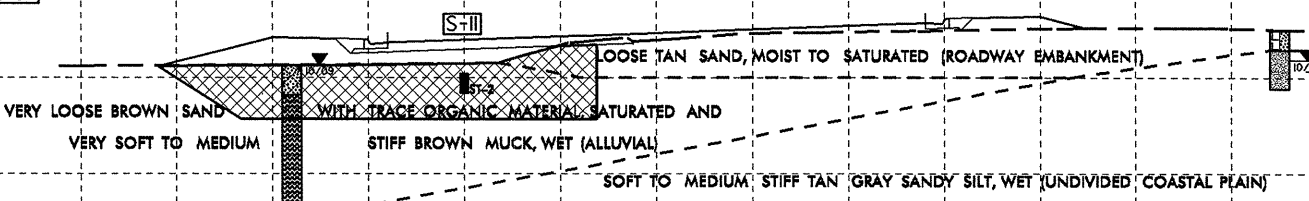
STATION	OFFSET	DEPTH	S (psf)
18+00	21' LT	1.0	271.44
18+00	21' LT	2.0	292.32
18+00	21' LT	3.0	208.80
18+00	21' LT	4.0	167.04
18+00	21' LT	5.0	334.08
18+00	21' LT	6.0	563.76

**SHELBY TUBE RESULTS**

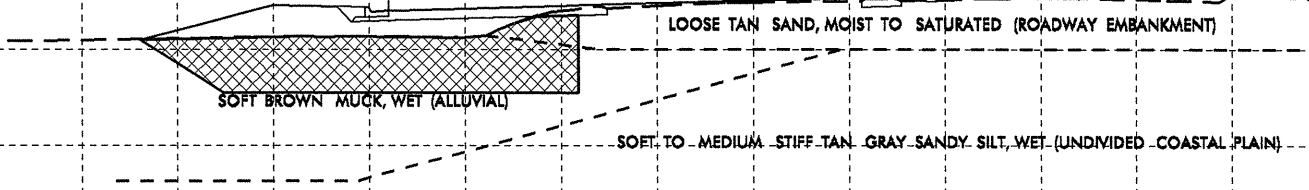
SAMPLE NO.	OFFSET	STATION	DEPTH INTERVAL	AASHTO CLASS.	L.L.	P.I.	% BY WEIGHT				% PASSING (SIEVES)			% ORGANIC	
							C.SAND	F.SAND	SILT	CLAY	10	40	200		MOISTURE
ST-2	20 LT	18+00	1.0-3.0	A-2-6(1)	31	15	49.8	22.9	15.7	22.2	96	69	28	75.93	2.2

**SOIL TEST RESULTS**

SAMPLE NO.	OFFSET	STATION	DEPTH INTERVAL	AASHTO CLASS.	L.L.	P.I.	% BY WEIGHT				% PASSING (SIEVES)			% ORGANIC
							C.SAND	F.SAND	SILT	CLAY	10	40	200	
S-11	20 LT	18+00	3.0-3.5											12.9



18+00.00



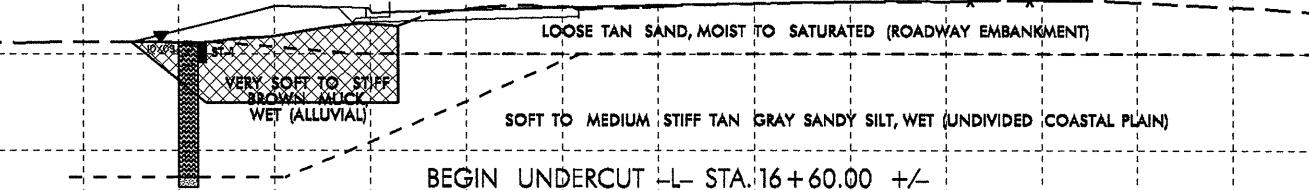
17+50.00

**VANE SHEAR TESTS**

STATION	OFFSET	DEPTH	S (psf)
17+00	55' LT	0.0	271.44
17+00	55' LT	0.5	292.32
17+00	55' LT	1.0	208.80
17+00	55' LT	1.5	167.04
17+00	55' LT	2.0	334.08
17+00	55' LT	2.5	563.76
17+00	55' LT	3.0	167.04
17+00	55' LT	3.5	208.80
17+00	55' LT	4.0	187.92
17+00	55' LT	4.5	375.84
17+00	55' LT	5.0	313.20
17+00	55' LT	5.5	375.84
17+00	55' LT	6.0	709.92
17+00	55' LT	6.5	501.12
17+00	55' LT	7.0	1002.24
17+00	55' LT	7.5	876.96
17+00	55' LT	8.0	1106.64
17+00	55' LT	8.5	1148.40
17+00	55' LT	9.0	793.44
17+00	55' LT	9.5	1106.64
17+00	55' LT	10.0	793.44
17+00	55' LT	10.5	835.20
17+00	55' LT	11.0	772.56

**SHELBY TUBE RESULTS**

SAMPLE NO.	OFFSET	STATION	DEPTH INTERVAL	AASHTO CLASS.	L.L.	P.I.	% BY WEIGHT				% PASSING (SIEVES)			% ORGANIC	
							C.SAND	F.SAND	SILT	CLAY	10	40	200		MOISTURE
ST-1	49 LT	17+00	0.0-2.0	A-2-5(D)	68	NP	38.8	43.9	9.7	8.2	95	82	19	125.66	22.5



17+00.00

BEGIN UNDERCUT -L- STA. 16+60.00 +/-

16+50.00

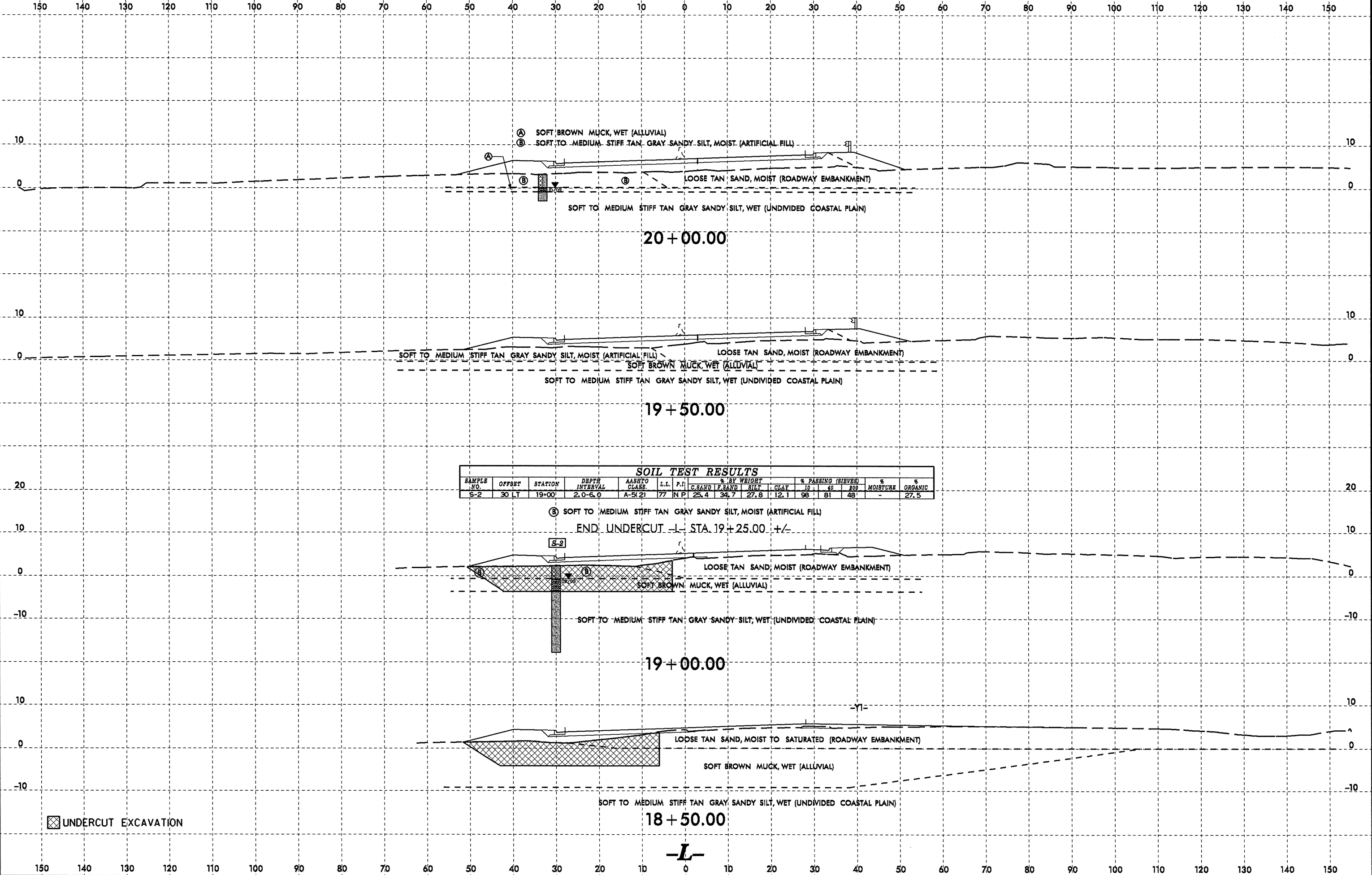
-L-

- Ⓐ LOOSE TAN SAND WITH CONCRETE DEBRIS, MOIST (ARTIFICIAL FILL)
- Ⓑ LOOSE TAN SAND, MOIST (ROADWAY EMBANKMENT)

⊠ UNDERCUT EXCAVATION

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 geotecn AT DECS





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**VANE SHEAR TESTS**

STATION	OFFSET	DEPTH	S (psf)
20+50	10' LT	1.0	1739
20+50	10' LT	2.0	2312
20+50	10' LT	3.0	2405
20+50	10' LT	4.0	1739
20+50	10' LT	5.0	2016.5
20+50	10' LT	6.0	MAX

SHELBY TUBE RESULTS															
SAMPLE NO.	OFFSET	STATION	DEPTH INTERVAL	AASHTO CLASS.	L.L.	P.L.	% BY WEIGHT				% PASSING (SIEVES)			% MOISTURE	% ORGANIC
							C. SAND	F. SAND	SILT	CLAY	10	40	200		
ST-3	10' LT	20+50	4.0-5.0	A-4(8)	33	31	8.5	31.2	46.2	14.1	99	95	81	143.87	9.3

- Ⓐ LOOSE TAN SAND WITH CONCRETE DEBRIS, MOIST (ROADWAY EMBANKMENT)
- Ⓑ SOFT TO MEDIUM STIFF TAN GRAY SANDY SILT, MOIST (ARTIFICIAL FILL)

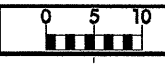
SOFT TO STIFF BROWN MUCK AND SANDY SILT WITH LITTLE ORGANIC MATERIAL, WET (ALLUVIAL)

SOFT TO MEDIUM STIFF TAN GRAY SANDY SILT, WET, (UNDIVIDED COASTAL PLAIN)

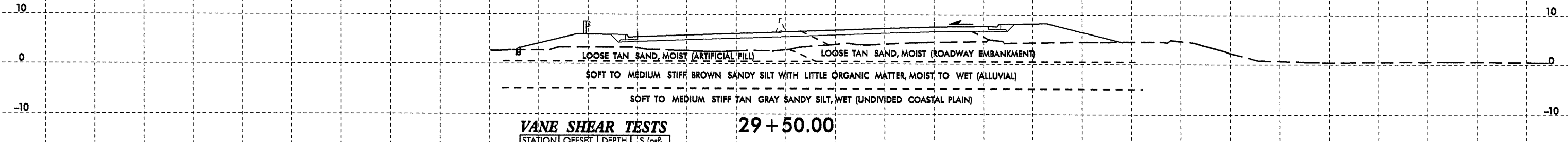
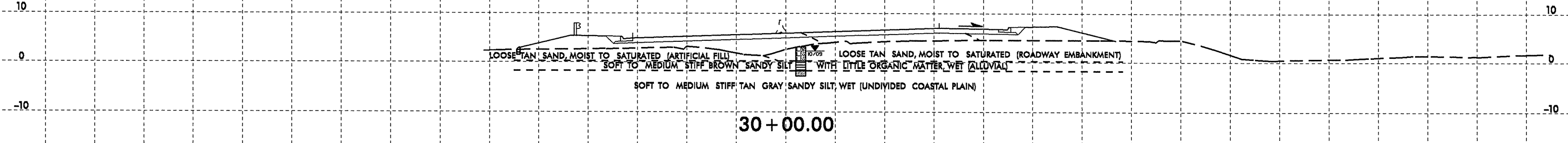
20 + 50.00

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8/23/99



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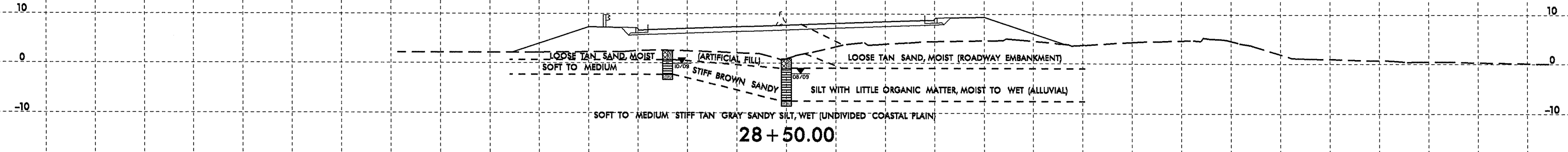
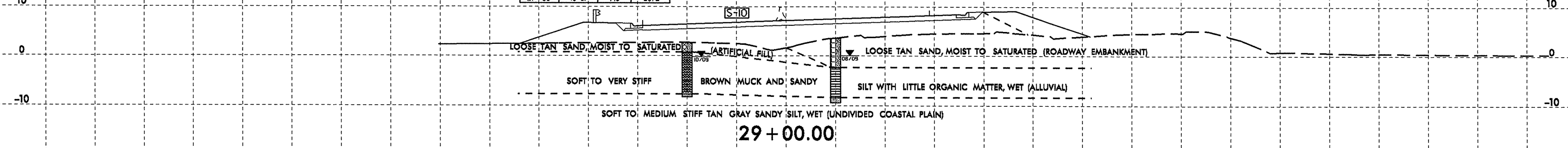


**VANE SHEAR TESTS**

STATION	OFFSET	DEPTH	S (psf)
29+00	10' LT	1.0	1868.5
29+00	10' LT	2.0	1905.5
29+00	10' LT	3.0	962
29+00	10' LT	4.0	1184
29+00	10' LT	5.0	962
29+00	10' LT	6.0	1739
29+00	10' LT	7.0	2072

**SOIL TEST RESULTS**

SAMPLE NO.	OFFSET	STATION	DEPTH INTERVAL	AASHTO CLASS.	L.L.	P.I.	% BY WEIGHT				% PASSING (SIEVES)			% MOISTURE	% ORGANIC
							C.SAND	F.SAND	SILT	CLAY	10	40	200		
S-10	10 LT	29+00	6.0-7.0	-	-	-	-	-	-	-	-	-	-	116.3	21.8

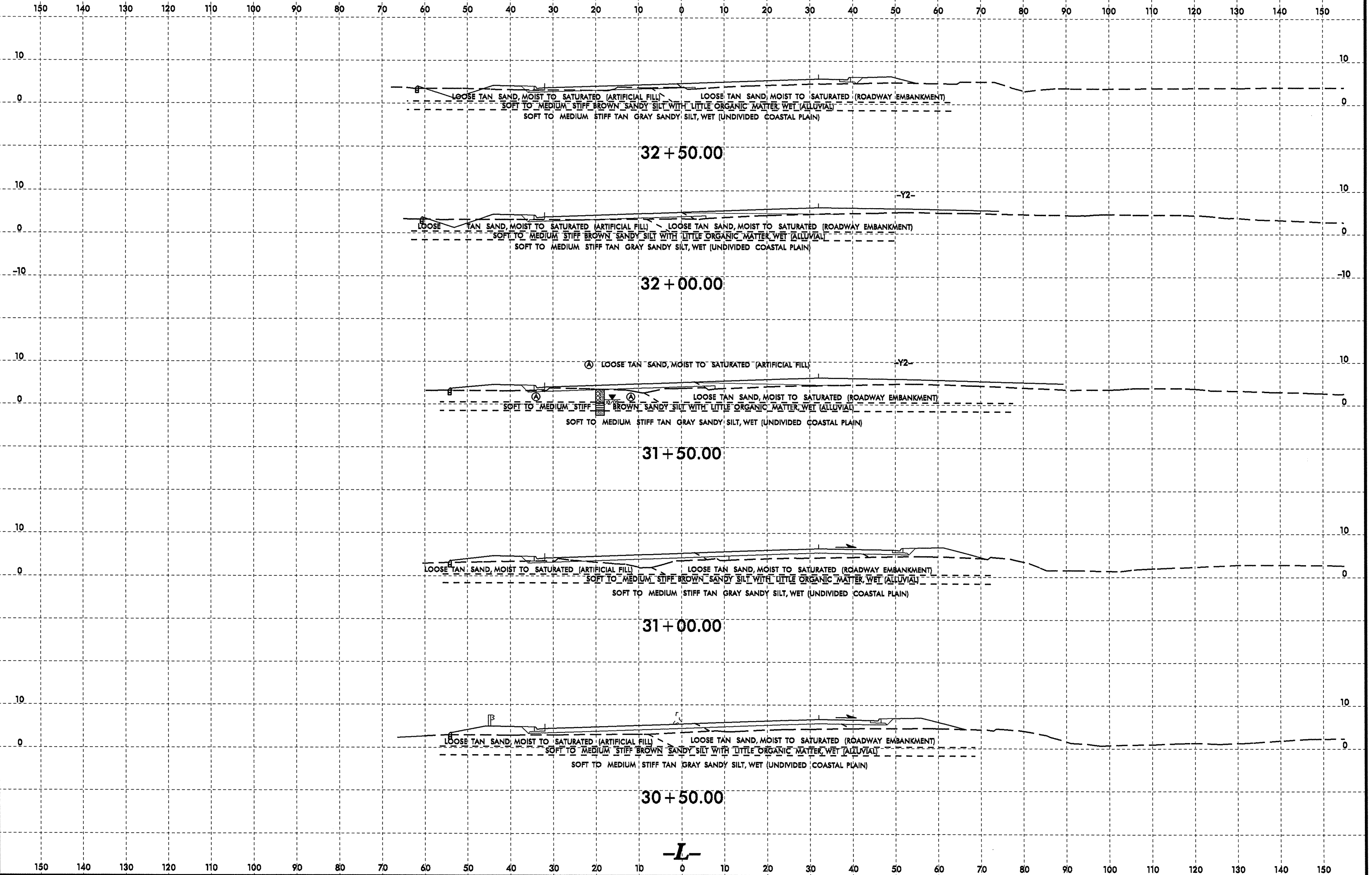


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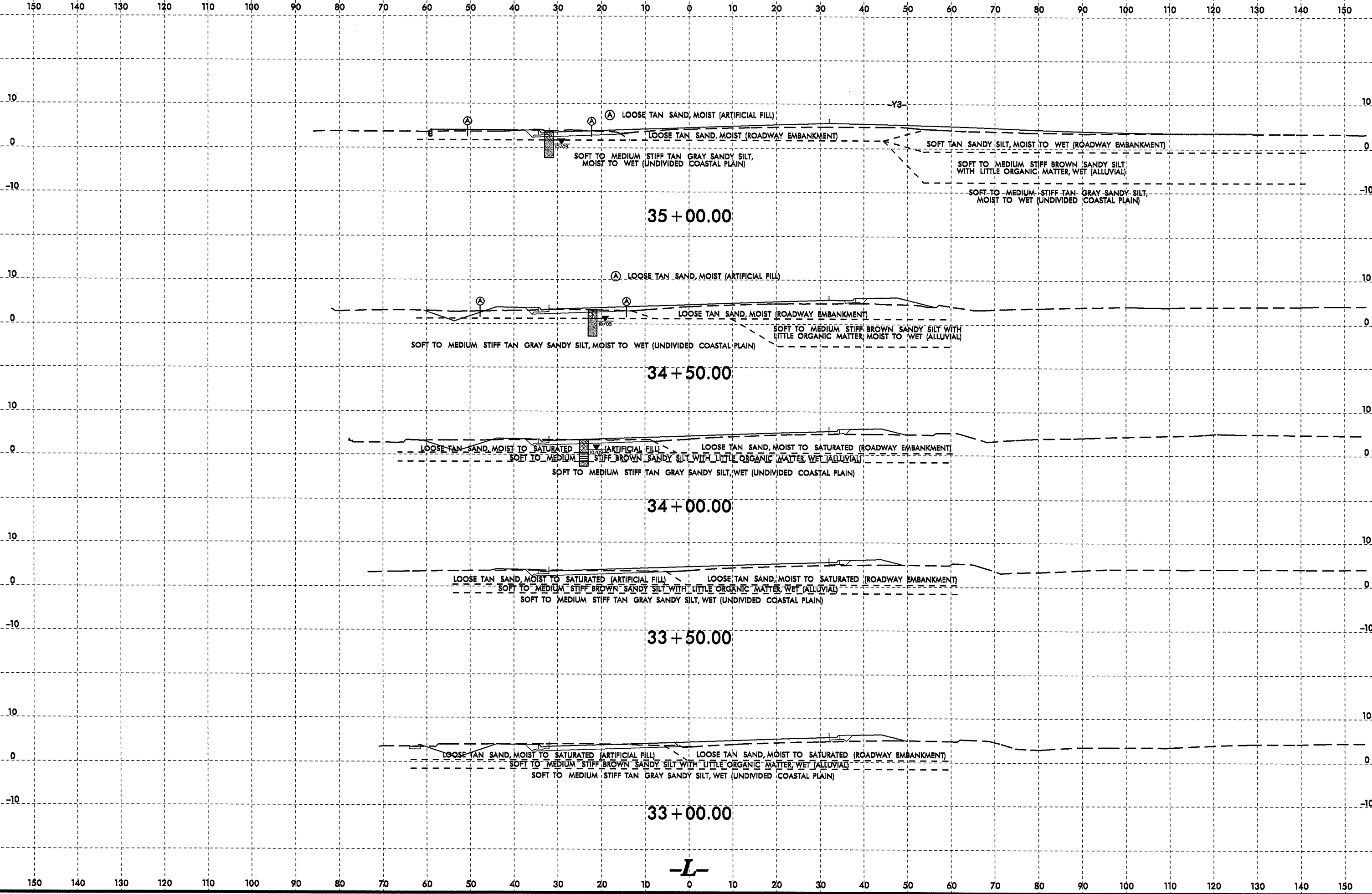


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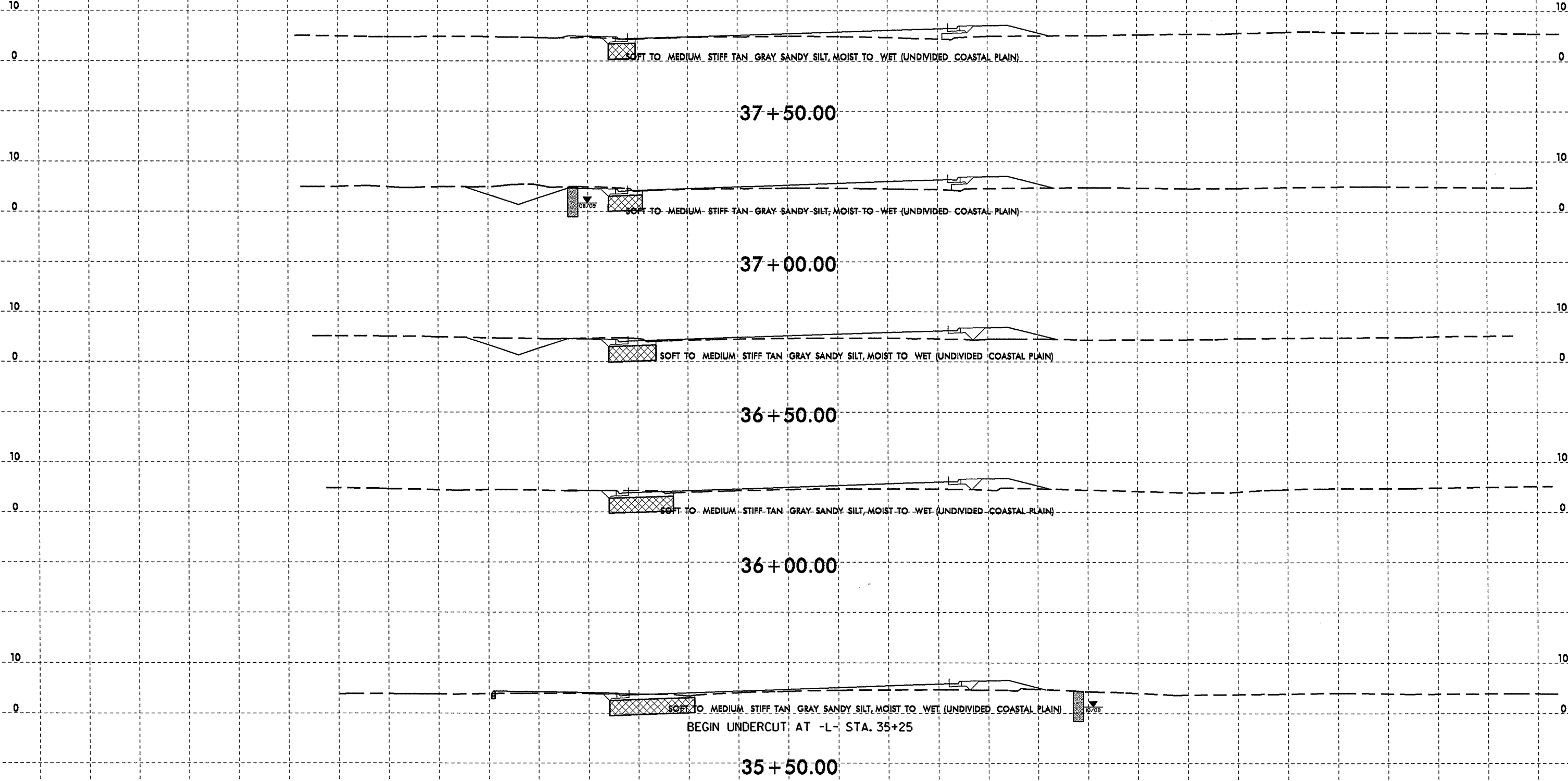


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SOFT TO MEDIUM STIFF TAN GRAY SANDY SILT, MOIST TO WET (UNDIVIDED COASTAL PLAIN)

37+50.00

SOFT TO MEDIUM STIFF TAN GRAY SANDY SILT, MOIST TO WET (UNDIVIDED COASTAL PLAIN)

37+00.00

SOFT TO MEDIUM STIFF TAN GRAY SANDY SILT, MOIST TO WET (UNDIVIDED COASTAL PLAIN)

36+50.00

SOFT TO MEDIUM STIFF TAN GRAY SANDY SILT, MOIST TO WET (UNDIVIDED COASTAL PLAIN)

36+00.00

SOFT TO MEDIUM STIFF TAN GRAY SANDY SILT, MOIST TO WET (UNDIVIDED COASTAL PLAIN)

BEGIN UNDERCUT AT -L- STA. 35+25

35+50.00

-L-

UNDERCUT EXCAVATION

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END UNDERCUT AT -L- STA. 40+00

SOFT TO MEDIUM STIFF TAN GRAY SANDY SILT, MOIST TO WET (UNDIVIDED COASTAL PLAIN)

40+00.00

SOFT TO MEDIUM STIFF TAN GRAY SANDY SILT, MOIST TO WET (UNDIVIDED COASTAL PLAIN)

39+50.00

SOFT TO MEDIUM STIFF TAN GRAY SANDY SILT, MOIST TO WET (UNDIVIDED COASTAL PLAIN)

39+00.00

06705

SOFT TO MEDIUM STIFF TAN GRAY SANDY SILT, MOIST TO WET (UNDIVIDED COASTAL PLAIN)

38+50.00

SOFT TO MEDIUM STIFF TAN GRAY SANDY SILT, MOIST TO WET (UNDIVIDED COASTAL PLAIN)

38+00.00

-L-

UNDERCUT EXCAVATION

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150 140 130 120 110 100 90 80 70 60 50 40 30 20 10 0 10 20 30 40 50 60 70 80 90 100 110 120 130 140 150