

NOTE: SEE SHEET 1A FOR PLAN SHEET LAYOUT AT TIME OF INVESTIGATION

STATE OF NORTH CAROLINA  
DEPARTMENT OF TRANSPORTATION  
DIVISION OF HIGHWAYS  
GEOTECHNICAL ENGINEERING UNIT

STATE	STATE PROJECT REFERENCE NO.	SHEET NO.	TOTAL SHEETS
N.C.	U-2551	1	73
STATE PROJ. NO.	F.A. PROJ. NO.	DESCRIPTION	
34832.1.1	STP-1922 (1)	PE	
34832.2.2	STP-1922 (1)	ROW, UTIL	
34832.3.1	STP-1922 (1)	CONSTR.	

CONTENTS

LINE	STATION	PLAN	PROFILE	XSECT
-L-	10+00 - 127+08	4-13		20-54
-RPA-	10+00 - 23+82	9	16	55-57
-RPB-	10+00 - 24+71	9	17	58-60
-RPC-	10+00 - 23+95	9	18	61-63
-RPD-	10+00 - 25+55	9	19	64-66
-Y8-	10+00 - 62+29	9,14,15		67-69
-Y11-	10+00 - 12+60	11		70
-Y12-	10-00 - 15+60	11		71,72

ROADWAY  
SUBSURFACE INVESTIGATION

PROJ. REFERENCE NO. 34832.1.1 F.A. PROJ. STP-1922(1)  
COUNTY BURKE  
PROJECT DESCRIPTION MORGANTON-SR 1922 (ENOLA ROAD) /  
SR 1924 (OLD NC 18) FROM SOUTH of PETE BRITAIN ROAD  
(SR 1940) to NC 18

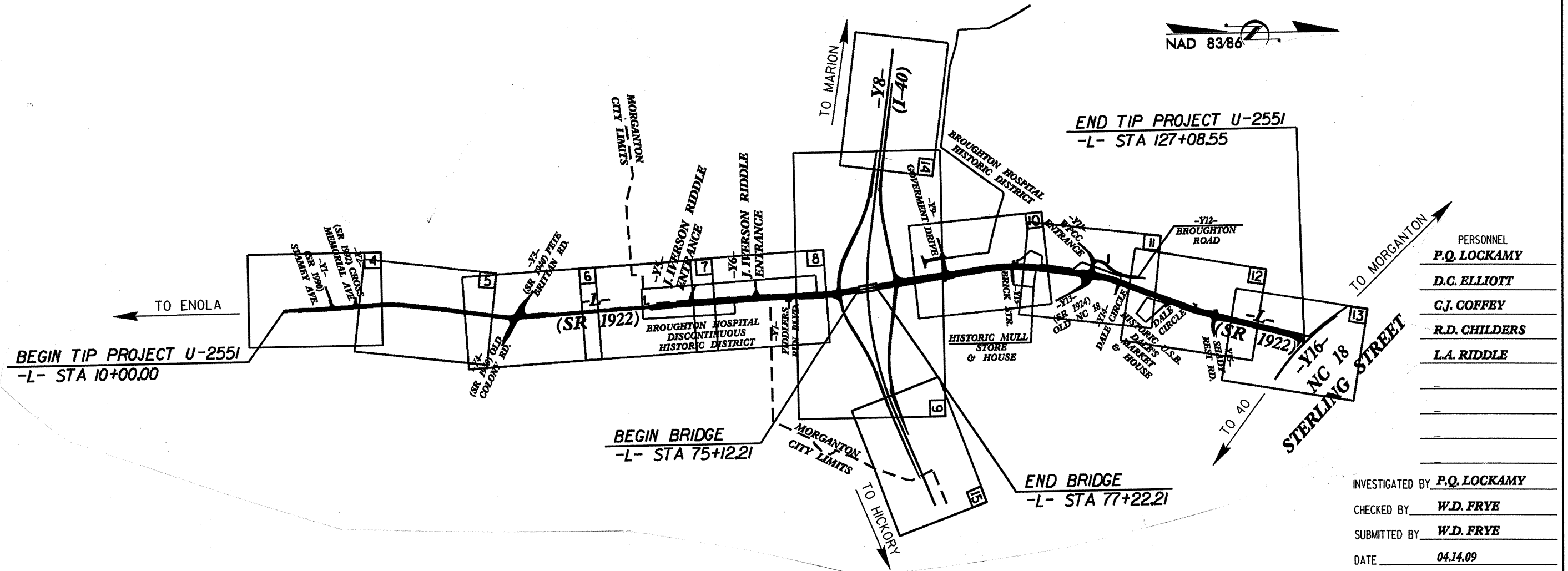
INVENTORY

**CAUTION NOTICE**  
THE SUBSURFACE INFORMATION AND THE SUBSURFACE INVESTIGATION ON WHICH IT IS BASED WERE MADE FOR THE PURPOSE OF STUDY, PLANNING, AND DESIGN, AND NOT FOR CONSTRUCTION OR PAY PURPOSES. THE VARIOUS FIELD BORING LOGS, ROCK CORES, AND SOIL TEST DATA AVAILABLE MAY BE REVIEWED OR INSPECTED IN RALEIGH BY CONTACTING THE N. C. DEPARTMENT OF TRANSPORTATION, GEOTECHNICAL ENGINEERING UNIT AT (919) 250-4088. NEITHER THE SUBSURFACE PLANS AND REPORTS, NOR THE FIELD BORING LOGS, ROCK CORES, OR SOIL TEST DATA ARE PART OF THE CONTRACT.

GENERAL SOIL AND ROCK STRATA DESCRIPTIONS AND INDICATED BOUNDARIES ARE BASED ON A GEOTECHNICAL INTERPRETATION OF ALL AVAILABLE SUBSURFACE DATA AND MAY NOT NECESSARILY REFLECT THE ACTUAL SUBSURFACE CONDITIONS BETWEEN BORINGS OR BETWEEN SAMPLED STRATA WITHIN THE BOREHOLE. THE LABORATORY SAMPLE DATA AND THE IN SITU UN-PLACED TEST DATA CAN BE RELIED ON ONLY TO THE DEGREE OF RELIABILITY INHERENT IN THE STANDARD TEST METHOD. THE OBSERVED WATER LEVELS OR SOIL MOISTURE CONDITIONS INDICATED IN THE SUBSURFACE INVESTIGATIONS ARE AS RECORDED AT THE TIME OF THE INVESTIGATION. THESE WATER LEVELS OR SOIL MOISTURE CONDITIONS MAY VARY CONSIDERABLY WITH TIME ACCORDING TO CLIMATIC CONDITIONS INCLUDING TEMPERATURES, PRECIPITATION, AND WIND, AS WELL AS OTHER NON-CLIMATIC FACTORS.

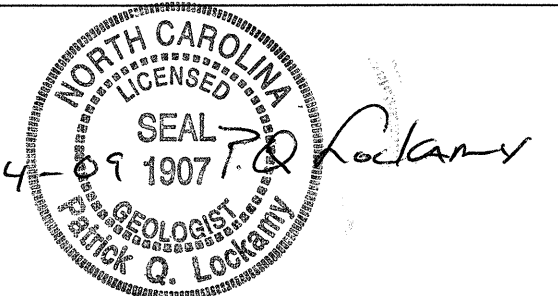
THE BIDDER OR CONTRACTOR IS CAUTIONED THAT DETAILS SHOWN ON THE SUBSURFACE PLANS ARE PRELIMINARY ONLY AND IN MANY CASES THE FINAL DESIGN DETAILS ARE DIFFERENT. FOR BIDDING AND CONSTRUCTION PURPOSES, REFER TO THE CONSTRUCTION PLANS AND DOCUMENTS FOR FINAL DESIGN INFORMATION ON THIS PROJECT. THE DEPARTMENT DOES NOT WARRANT OR GUARANTEE THE SUFFICIENCY OR ACCURACY OF THE INVESTIGATION MADE, NOR THE INTERPRETATIONS MADE, OR OPINION OF THE DEPARTMENT AS TO THE TYPE OF MATERIALS AND CONDITIONS TO BE ENCOUNTERED. THE BIDDER OR CONTRACTOR IS CAUTIONED TO MAKE SUCH INDEPENDENT SUBSURFACE INVESTIGATIONS AS HE DEEMS NECESSARY TO SATISFY HIMSELF AS TO CONDITIONS TO BE ENCOUNTERED ON THIS PROJECT. THE CONTRACTOR SHALL HAVE NO CLAIM FOR ADDITIONAL COMPENSATION OR FOR AN EXTENSION OF TIME FOR ANY REASON RESULTING FROM THE ACTUAL CONDITIONS ENCOUNTERED AT THE SITE OFFERING FROM THOSE INDICATED IN THE SUBSURFACE INFORMATION.

CONTRACT: C202815 ID: U-2551



- PERSONNEL
- P.Q. LOCKAMY
  - D.C. ELLIOTT
  - C.J. COFFEY
  - R.D. CHILDERS
  - L.A. RIDDLE

INVESTIGATED BY P.Q. LOCKAMY  
CHECKED BY W.D. FRYE  
SUBMITTED BY W.D. FRYE  
DATE 04.14.09



DRAWN BY: J.T. WILLIAMS

NOTE - THE INFORMATION CONTAINED HEREIN IS NOT IMPLIED OR GUARANTEED BY THE N. C. DEPARTMENT OF TRANSPORTATION AS BEING ACCURATE NOR IT IS CONSIDERED TO BE PART OF THE PLANS, SPECIFICATIONS, OR CONTRACT FOR THE PROJECT.

NOTE - BY HAVING REQUESTED THIS INFORMATION THE CONTRACTOR SPECIFICALLY WAIVES ANY CLAIMS FOR INCREASED COMPENSATION OR EXTENSION OF TIME BASED ON DIFFERENCES BETWEEN THE CONDITIONS INDICATED HEREIN AND THE ACTUAL CONDITIONS AT THE PROJECT SITE.

NORTH CAROLINA DEPARTMENT OF TRANSPORTATION  
DIVISION OF HIGHWAYS  
GEOTECHNICAL ENGINEERING UNIT

PROJECT REFERENCE NO. U-2551	SHEET NO. 2
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## SUBSURFACE INVESTIGATION

### SOIL AND ROCK LEGEND, TERMS, SYMBOLS, AND ABBREVIATIONS

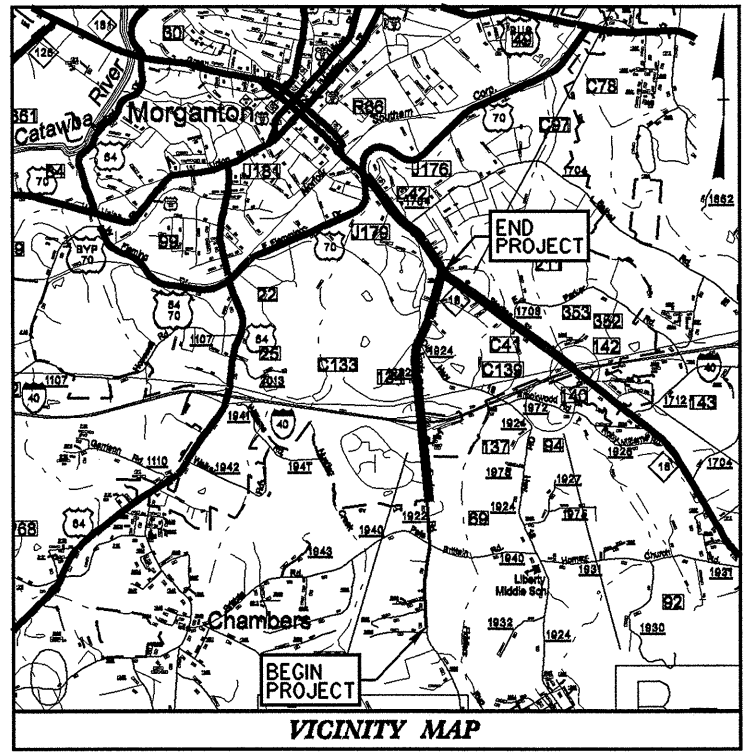
SOIL DESCRIPTION		GRADATION		ROCK DESCRIPTION		TERMS AND DEFINITIONS																																																												
<p>SOIL IS CONSIDERED TO BE THE UNCONSOLIDATED, SEMI-CONSOLIDATED, OR WEATHERED EARTH MATERIALS THAT CAN BE PENETRATED WITH A CONTINUOUS FLIGHT POWER AUGER, AND YIELD LESS THAN 100 BLOWS PER FOOT ACCORDING TO STANDARD PENETRATION TEST (ASTM D-1586). SOIL CLASSIFICATION IS BASED ON THE AASHTO SYSTEM. BASIC DESCRIPTIONS GENERALLY SHALL INCLUDE: CONSISTENCY, COLOR, TEXTURE, MOISTURE, AASHTO CLASSIFICATION AND OTHER PERTINENT FACTORS SUCH AS MINERALOGICAL COMPOSITION, ANGULARITY, STRUCTURE, PLASTICITY, ETC. EXAMPLE:</p> <p style="text-align: center;"><i>VERY STIFF, GRANULITY CLAY, MOST WITH INTERBEDDED FINE SAND LAYERS, HIGH PLASTIC, A-7-6</i></p>		<p>WELL GRADED - INDICATES A GOOD REPRESENTATION OF PARTICLE SIZES FROM FINE TO COARSE. UNIFORM - INDICATES THAT SOIL PARTICLES ARE ALL APPROXIMATELY THE SAME SIZE. (ALSO POORLY GRADED) GAP-GRADED - INDICATES A MIXTURE OF UNIFORM PARTICLES OF TWO OR MORE SIZES.</p> <p style="text-align: center;">ANGULARITY OF GRAINS</p> <p>THE ANGULARITY OR ROUNDNESS OF SOIL GRAINS IS DESIGNATED BY THE TERMS: ANGULAR, SUBANGULAR, SUBROUNDED, OR ROUNDED.</p>		<p>HARD ROCK IS NON-COASTAL PLAIN MATERIAL THAT IF TESTED, WOULD YIELD SPT REFUSAL, AN INFERRED ROCK LINE INDICATES THE LEVEL AT WHICH NON-COASTAL PLAIN MATERIAL WOULD YIELD SPT REFUSAL. SPT REFUSAL IS PENETRATION BY A SPLIT SPON SAMPLER EQUAL TO OR LESS THAN 0.1 FOOT PER 60 BLOWS. IN NON-COASTAL PLAIN MATERIAL, THE TRANSITION BETWEEN SOIL AND ROCK IS OFTEN REPRESENTED BY A ZONE OF WEATHERED ROCK. ROCK MATERIALS ARE TYPICALLY DIVIDED AS FOLLOWS:</p>		<p>ALLUVIUM (ALLUV.) - SOILS THAT HAVE BEEN TRANSPORTED BY WATER.</p> <p>AQUIFER - A WATER BEARING FORMATION OR STRATA.</p> <p>ARENACEOUS - APPLIED TO ROCKS THAT HAVE BEEN DERIVED FROM SAND OR THAT CONTAIN SAND.</p> <p>ARGILLACEOUS - APPLIED TO ALL ROCKS OR SUBSTANCES COMPOSED OF CLAY MINERALS, OR HAVING A NOTABLE PROPORTION OF CLAY IN THEIR COMPOSITION, AS SHALE, SLATE, ETC.</p> <p>ARTESIAN - GROUND WATER THAT IS UNDER SUFFICIENT PRESSURE TO RISE ABOVE THE LEVEL AT WHICH IT IS ENCOUNTERED, BUT WHICH DOES NOT NECESSARILY RISE TO OR ABOVE THE GROUND SURFACE.</p> <p>CALCAREOUS (CALC.) - SOILS THAT CONTAIN APPRECIABLE AMOUNTS OF CALCIUM CARBONATE.</p> <p>COLLUVIUM - ROCK FRAGMENTS MIXED WITH SOIL DEPOSITED BY GRAVITY ON SLOPE OR AT BOTTOM OF SLOPE.</p> <p>CORE RECOVERY (REC.) - TOTAL LENGTH OF ALL MATERIAL RECOVERED IN THE CORE BARREL DIVIDED BY TOTAL LENGTH OF CORE RUN AND EXPRESSED AS A PERCENTAGE.</p> <p>DIKE - A TABULAR BODY OF IGNEOUS ROCK THAT CUTS ACROSS THE STRUCTURE OF ADJACENT ROCKS OR CUTS MASSIVE ROCK.</p> <p>DIP - THE ANGLE AT WHICH A STRATUM OR ANY PLANAR FEATURE IS INCLINED FROM THE HORIZONTAL.</p> <p>DIP DIRECTION (DIP AZIMUTH) - THE DIRECTION OR BEARING OF THE HORIZONTAL TRACE OF THE LINE OF DIP, MEASURED CLOCKWISE FROM NORTH.</p> <p>FAULT - A FRACTURE OR FRACTURE ZONE ALONG WHICH THERE HAS BEEN DISPLACEMENT OF THE SIDES RELATIVE TO ONE ANOTHER PARALLEL TO THE FRACTURE.</p> <p>FISSILE - A PROPERTY OF SPLITTING ALONG CLOSELY SPACED PARALLEL PLANES.</p> <p>FLOAT - ROCK FRAGMENTS ON SURFACE NEAR THEIR ORIGINAL POSITION AND DISLOADED FROM PARENT MATERIAL.</p> <p>FLOOD PLAIN (FP) - LAND BORDERING A STREAM, BUILT OF SEDIMENTS DEPOSITED BY THE STREAM.</p> <p>FORMATION (FM.) - A MAPPABLE GEOLOGIC UNIT THAT CAN BE RECOGNIZED AND TRACED IN THE FIELD.</p> <p>JOINT - FRACTURE IN ROCK ALONG WHICH NO APPRECIABLE MOVEMENT HAS OCCURRED.</p> <p>LEDGE - A SHELF-LIKE RIDGE OR PROJECTION OF ROCK WHOSE THICKNESS IS SMALL COMPARED TO ITS LATERAL EXTENT.</p> <p>LENS - A BODY OF SOIL OR ROCK THAT THINS OUT IN ONE OR MORE DIRECTIONS.</p> <p>MOTTLED (MOT.) - IRREGULARLY MARKED WITH SPOTS OF DIFFERENT COLORS. MOTTLING IN SOILS USUALLY INDICATES POOR AERATION AND LACK OF GOOD DRAINAGE.</p> <p>PERCHED WATER - WATER MAINTAINED ABOVE THE NORMAL GROUND WATER LEVEL BY THE PRESENCE OF AN INTERVENING IMPERVIOUS STRATUM.</p> <p>RESIDUAL (RES.) SOIL - SOIL FORMED IN PLACE BY THE WEATHERING OF ROCK.</p> <p>ROCK QUALITY DESIGNATION (RQD) - A MEASURE OF ROCK QUALITY DESCRIBED BY TOTAL LENGTH OF ROCK SEGMENTS EQUAL TO OR GREATER THAN 4 INCHES DIVIDED BY THE TOTAL LENGTH OF CORE RUN AND EXPRESSED AS A PERCENTAGE.</p> <p>SAPROLITE (SAP.) - RESIDUAL SOIL THAT RETAINS THE RELIC STRUCTURE OR FABRIC OF THE PARENT ROCK.</p> <p>SILL - AN INTRUSIVE BODY OF IGNEOUS ROCK OF APPROXIMATELY UNIFORM THICKNESS AND RELATIVELY THIN COMPARED WITH ITS LATERAL EXTENT, THAT HAS BEEN EMPLACED PARALLEL TO THE BEDDING OR SCHISTOSITY OF THE INTRUDED ROCKS.</p> <p>SLICKENSIDE - POLISHED AND STRIATED SURFACE THAT RESULTS FROM FRICTION ALONG A FAULT OR SLIP PLANE.</p> <p>STANDARD PENETRATION TEST (PENETRATION RESISTANCE) (SPT) - NUMBER OF BLOWS (N OR BPF) OF A 140 LB. HAMMER FALLING 30 INCHES REQUIRED TO PRODUCE A PENETRATION OF 1 FOOT INTO SOIL WITH A 2 INCH OUTSIDE DIAMETER SPLIT SPOON SAMPLER. SPT REFUSAL IS PENETRATION EQUAL TO OR LESS THAN 0.1 FOOT PER 60 BLOWS.</p> <p>STRATA CORE RECOVERY (SREC) - TOTAL LENGTH OF STRATA MATERIAL RECOVERED DIVIDED BY TOTAL LENGTH OF STRATUM AND EXPRESSED AS A PERCENTAGE.</p> <p>STRATA ROCK QUALITY DESIGNATION (SRQD) - A MEASURE OF ROCK QUALITY DESCRIBED BY TOTAL LENGTH OF ROCK SEGMENTS WITHIN A STRATUM EQUAL TO OR GREATER THAN 4 INCHES DIVIDED BY THE TOTAL LENGTH OF STRATA AND EXPRESSED AS A PERCENTAGE.</p> <p>TOPSOIL (TS.) - SURFACE SOILS USUALLY CONTAINING ORGANIC MATTER.</p>																																																												
SOIL LEGEND AND AASHTO CLASSIFICATION		MINERALOGICAL COMPOSITION		WEATHERING		ROCK HARDNESS																																																												
<table border="1" style="width: 100%; border-collapse: collapse;"> <tr> <th>GENERAL CLASS.</th> <th>GRANULAR MATERIALS (≤ 35% PASSING #200)</th> <th>SILT-CLAY MATERIALS (&gt; 35% PASSING #200)</th> <th>ORGANIC MATERIALS</th> </tr> <tr> <td>GROUP CLASS.</td> <td>A-1, A-1-b, A-2, A-2-5, A-2-6, A-2-7</td> <td>A-4, A-5, A-6, A-7</td> <td>A-1, A-2, A-3, A-4, A-5, A-6, A-7</td> </tr> <tr> <td>SYMBOL</td> <td></td> <td></td> <td></td> </tr> <tr> <td>% PASSING</td> <td>#10, #40, #200</td> <td>#10, #40, #200</td> <td>GRANULAR SOILS, SILT-CLAY SOILS, MUCK, PEAT</td> </tr> <tr> <td>LIQUID LIMIT PLASTIC INDEX</td> <td>6 MX, NP, 10 MX, 11 MX, 12 MX, 13 MX, 14 MX, 15 MX, 16 MX, 17 MX, 18 MX, 19 MX, 20 MX, 21 MX, 22 MX, 23 MX, 24 MX, 25 MX, 26 MX, 27 MX, 28 MX, 29 MX, 30 MX, 31 MX, 32 MX, 33 MX, 34 MX, 35 MX, 36 MX, 37 MX, 38 MX, 39 MX, 40 MX, 41 MX, 42 MX, 43 MX, 44 MX, 45 MX, 46 MX, 47 MX, 48 MX, 49 MX, 50 MX</td> <td>SOILS WITH LITTLE OR MODERATE AMOUNTS OF ORGANIC MATTER</td> <td>HIGHLY ORGANIC SOILS</td> </tr> <tr> <td>GROUP INDEX</td> <td>0, 1, 2, 3, 4, 5, 6, 7, 8, 9, 10, 11, 12, 13, 14, 15, 16, 17, 18, 19, 20, 21, 22, 23, 24, 25, 26, 27, 28, 29, 30, 31, 32, 33, 34, 35, 36, 37, 38, 39, 40, 41, 42, 43, 44, 45, 46, 47, 48, 49, 50</td> <td></td> <td></td> </tr> <tr> <td>USUAL TYPES OF MAJOR MATERIALS</td> <td>STONE FRAGS, GRAVEL, SAND, FINE SAND, SILTY OR CLAYEY GRAVEL AND SAND, SILTY SOILS, CLAYEY SOILS</td> <td></td> <td></td> </tr> <tr> <td>GEN. RATING AS A SUBGRADE</td> <td>EXCELLENT TO GOOD</td> <td>FAIR TO POOR</td> <td>POOR, UNSUITABLE</td> </tr> </table>		GENERAL CLASS.	GRANULAR MATERIALS (≤ 35% PASSING #200)	SILT-CLAY MATERIALS (> 35% PASSING #200)	ORGANIC MATERIALS	GROUP CLASS.	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RATING AS A SUBGRADE	EXCELLENT TO GOOD	FAIR TO POOR	POOR, UNSUITABLE	<p>MINERAL NAMES SUCH AS QUARTZ, FELDSPAR, MICA, TALC, KAOLIN, ETC. ARE USED IN DESCRIPTIONS WHENEVER THEY ARE CONSIDERED OF SIGNIFICANCE.</p> <p style="text-align: center;">COMPRESSIBILITY</p> <p>SLIGHTLY COMPRESSIBLE MODERATELY COMPRESSIBLE HIGHLY COMPRESSIBLE</p> <p>LIQUID LIMIT LESS THAN 31 LIQUID LIMIT EQUAL TO 31-50 LIQUID LIMIT GREATER THAN 50</p> <p style="text-align: center;">PERCENTAGE OF MATERIAL</p> <table border="1" style="width: 100%; border-collapse: collapse;"> <tr> <th>ORGANIC MATERIAL</th> <th>SILT - CLAY SOILS</th> <th>OTHER MATERIAL</th> </tr> <tr> <td>TRACE OF ORGANIC MATTER</td> <td>2 - 3%</td> <td>TRACE</td> </tr> <tr> <td>LITTLE ORGANIC MATTER</td> <td>3 - 5%</td> <td>LITTLE</td> </tr> <tr> <td>MODERATELY ORGANIC</td> <td>5 - 10%</td> <td>SOME</td> </tr> <tr> <td>HIGHLY ORGANIC</td> <td>&gt;10%</td> <td>HIGHLY</td> </tr> </table>		ORGANIC MATERIAL	SILT - CLAY SOILS	OTHER MATERIAL	TRACE OF ORGANIC MATTER	2 - 3%	TRACE	LITTLE ORGANIC MATTER	3 - 5%	LITTLE	MODERATELY ORGANIC	5 - 10%	SOME	HIGHLY ORGANIC	>10%	HIGHLY	<p>WEATHERED ROCK (WR) </p> <p>CRYSTALLINE ROCK (CR) </p> <p>NON-CRYSTALLINE ROCK (NCR) </p> <p>COASTAL PLAIN SEDIMENTARY ROCK (CP) </p> <p>FRESH ROCK FRESH, CRYSTALS BRIGHT, FEW JOINTS MAY SHOW SLIGHT STAINING, ROCK RINGS UNDER HAMMER IF CRYSTALLINE.</p> <p>VERY SLIGHT (V SL.) ROCK GENERALLY FRESH, JOINTS STAINED, SOME JOINTS MAY SHOW THIN CLAY COATINGS IF OPEN, CRYSTALS ON A BROKEN SPECIMEN FACE SHINE BRIGHTLY, ROCK RINGS UNDER HAMMER BLOWS IF OF A CRYSTALLINE NATURE.</p> <p>SLIGHT (SL.) ROCK GENERALLY FRESH, JOINTS STAINED AND DISCOLORATION EXTENDS INTO ROCK UP TO 1 INCH. OPEN JOINTS MAY CONTAIN CLAY. IN GRANITOID ROCKS SOME OCCASIONAL FELDSPAR CRYSTALS ARE DULL AND DISCOLORED, CRYSTALLINE ROCKS RING UNDER HAMMER BLOWS.</p> <p>MODERATE (MOD.) SIGNIFICANT PORTIONS OF ROCK SHOW DISCOLORATION AND WEATHERING EFFECTS. IN GRANITOID ROCKS, MOST FELDSPARS ARE DULL AND DISCOLORED, SOME SHOW CLAY. ROCK HAS DULL SOUND UNDER HAMMER BLOWS AND SHOWS SIGNIFICANT LOSS OF STRENGTH AS COMPARED WITH FRESH ROCK.</p> <p>MODERATELY SEVERE (MOD. SEV.) ALL ROCK EXCEPT QUARTZ DISCOLORED OR STAINED. IN GRANITOID ROCKS, ALL FELDSPARS DULL AND DISCOLORED AND A MAJORITY SHOW KAOLINIZATION. ROCK SHOWS SEVERE LOSS OF STRENGTH AND CAN BE EXCAVATED WITH A GEOLOGIST'S PICK. ROCK GIVES "CLUNK" SOUND WHEN STRUCK. <i>IF TESTED, WOULD YIELD SPT REFUSAL</i></p> <p>SEVERE (SEV.) ALL ROCK EXCEPT QUARTZ DISCOLORED OR STAINED, ROCK FABRIC CLEAR AND EVIDENT BUT REDUCED IN STRENGTH TO STRONG SOIL. IN GRANITOID ROCKS ALL FELDSPARS ARE KAOLINIZED TO SOME EXTENT. SOME FRAGMENTS OF STRONG ROCK USUALLY REMAIN. <i>IF TESTED, YIELDS SPT N VALUES &gt; 100 BPF</i></p> <p>VERY SEVERE (V SEV.) ALL ROCK EXCEPT QUARTZ DISCOLORED OR STAINED, ROCK FABRIC ELEMENTS ARE DISCERNIBLE BUT THE MASS IS EFFECTIVELY REDUCED TO SOIL STATUS, WITH ONLY FRAGMENTS OF STRONG ROCK REMAINING. SAPROLITE IS AN EXAMPLE OF ROCK WEATHERED TO A DEGREE SUCH THAT ONLY MINOR VESTIGES OF THE ORIGINAL ROCK FABRIC REMAIN. <i>IF TESTED, YIELDS SPT N VALUES &lt; 100 BPF</i></p> <p>COMPLETE ROCK REDUCED TO SOIL. ROCK FABRIC NOT DISCERNIBLE OR DISCERNIBLE ONLY IN SMALL AND SCATTERED CONCENTRATIONS. QUARTZ MAY BE PRESENT AS DIKES OR STRINGERS. SAPROLITE IS ALSO AN EXAMPLE.</p>		<p>PI OF A-7-5 SUBGROUP IS ≤ LL - 30 ; PI OF A-7-6 SUBGROUP IS &gt; LL - 30</p> <p style="text-align: center;">CONSISTENCY OR DENSENESS</p> <table border="1" style="width: 100%; border-collapse: collapse;"> <tr> <th>PRIMARY SOIL TYPE</th> <th>COMPACTNESS OR CONSISTENCY</th> <th>RANGE OF STANDARD PENETRATION RESISTANCE (N-VALUE)</th> <th>RANGE OF UNCONFINED COMPRESSIVE STRENGTH (TONS/FT<sup>2</sup>)</th> </tr> <tr> <td>GENERALLY GRANULAR MATERIAL (NON-COHESIVE)</td> <td>VERY LOOSE, LOOSE, MEDIUM DENSE, DENSE, VERY DENSE</td> <td>&lt;4, 4 TO 10, 10 TO 30, 30 TO 50, &gt;50</td> <td>N/A</td> </tr> <tr> <td>GENERALLY SILT-CLAY MATERIAL (COHESIVE)</td> <td>VERY SOFT, SOFT, MEDIUM STIFF, STIFF, VERY STIFF, HARD</td> <td>&lt;2, 2 TO 4, 4 TO 8, 8 TO 15, 15 TO 30, &gt;30</td> <td>&lt;0.25, 0.25 TO 0.50, 0.5 TO 1.0, 1 TO 2, 2 TO 4, &gt;4</td> </tr> </table>		PRIMARY SOIL TYPE	COMPACTNESS OR CONSISTENCY	RANGE OF STANDARD PENETRATION RESISTANCE (N-VALUE)	RANGE OF UNCONFINED COMPRESSIVE STRENGTH (TONS/FT <sup>2</sup> )	GENERALLY GRANULAR MATERIAL (NON-COHESIVE)	VERY LOOSE, LOOSE, MEDIUM DENSE, DENSE, VERY DENSE	<4, 4 TO 10, 10 TO 30, 30 TO 50, >50	N/A	GENERALLY SILT-CLAY MATERIAL (COHESIVE)	VERY SOFT, SOFT, MEDIUM STIFF, STIFF, VERY STIFF, HARD	<2, 2 TO 4, 4 TO 8, 8 TO 15, 15 TO 30, >30	<0.25, 0.25 TO 0.50, 0.5 TO 1.0, 1 TO 2, 2 TO 4, >4
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HIGHLY ORGANIC	>10%	HIGHLY																																																																
PRIMARY SOIL TYPE	COMPACTNESS OR CONSISTENCY	RANGE OF STANDARD PENETRATION RESISTANCE (N-VALUE)	RANGE OF UNCONFINED COMPRESSIVE STRENGTH (TONS/FT <sup>2</sup> )																																																															
GENERALLY GRANULAR MATERIAL (NON-COHESIVE)	VERY LOOSE, LOOSE, MEDIUM DENSE, DENSE, VERY DENSE	<4, 4 TO 10, 10 TO 30, 30 TO 50, >50	N/A																																																															
GENERALLY SILT-CLAY MATERIAL (COHESIVE)	VERY SOFT, SOFT, MEDIUM STIFF, STIFF, VERY STIFF, HARD	<2, 2 TO 4, 4 TO 8, 8 TO 15, 15 TO 30, >30	<0.25, 0.25 TO 0.50, 0.5 TO 1.0, 1 TO 2, 2 TO 4, >4																																																															
TEXTURE OR GRAIN SIZE		MISCELLANEOUS SYMBOLS		ROCK HARDNESS		ROCK HARDNESS																																																												
<table border="1" style="width: 100%; border-collapse: collapse;"> <tr> <th>U.S. STD. SIEVE SIZE OPENING (MM)</th> <th>4</th> <th>10</th> <th>40</th> <th>60</th> <th>200</th> <th>270</th> </tr> <tr> <td></td> <td>4.75</td> <td>2.00</td> <td>0.42</td> <td>0.25</td> <td>0.075</td> <td>0.053</td> </tr> </table>		U.S. STD. SIEVE SIZE OPENING (MM)	4	10	40	60	200	270		4.75	2.00	0.42	0.25	0.075	0.053	<p>ROADWAY EMBANKMENT (RE) WITH SOIL DESCRIPTION </p> <p>SOIL SYMBOL </p> <p>ARTIFICIAL FILL (AF) OTHER THAN ROADWAY EMBANKMENT </p> <p>INFERRED SOIL BOUNDARY </p> <p>INFERRED ROCK LINE </p> <p>ALLUVIAL SOIL BOUNDARY </p> <p>DIP &amp; DIP DIRECTION OF ROCK STRUCTURES </p> <p>SOUNDING ROD </p> <p>SPT DMT VST DMT PHT TEST BORING</p> <p>AUGER BORING</p> <p>CORE BORING</p> <p>MONITORING WELL</p> <p>PIEZOMETER INSTALLATION</p> <p>SLOPE INDICATOR INSTALLATION</p> <p>SPT N-VALUE</p> <p>SPT REFUSAL</p> <p>HAND AUGER BORING</p>		<p>VERY HARD CANNOT BE SCRATCHED BY KNIFE OR SHARP PICK. BREAKING OF HAND SPECIMENS REQUIRES SEVERAL HARD BLOWS OF THE GEOLOGIST'S PICK.</p> <p>HARD CAN BE SCRATCHED BY KNIFE OR PICK ONLY WITH DIFFICULTY. HARD HAMMER BLOWS REQUIRED TO DETACH HAND SPECIMEN.</p> <p>MODERATELY HARD CAN BE SCRATCHED BY KNIFE OR PICK. GOUGES OR GROOVES TO 0.25 INCHES DEEP CAN BE EXCAVATED BY HARD BLOW OF A GEOLOGIST'S PICK. HAND SPECIMENS CAN BE DETACHED BY MODERATE BLOWS.</p> <p>MEDIUM HARD CAN BE GROUDED OR GOUGED 0.05 INCHES DEEP BY FIRM PRESSURE OF KNIFE OR PICK POINT. CAN BE EXCAVATED IN SMALL CHIPS TO PEICES 1 INCH MAXIMUM SIZE BY HARD BLOWS OF THE POINT OF A GEOLOGIST'S PICK.</p> <p>SOFT CAN BE GROUDED OR GOUGED READILY BY KNIFE OR PICK. CAN BE EXCAVATED IN FRAGMENTS FROM CHIPS TO SEVERAL INCHES IN SIZE BY MODERATE BLOWS OF A PICK POINT. SMALL, THIN PIECES CAN BE BROKEN BY FINGER PRESSURE.</p> <p>VERY SOFT CAN BE CARVED WITH KNIFE. CAN BE EXCAVATED READILY WITH POINT OF PICK. PEECES 1 INCH OR MORE IN THICKNESS CAN BE BROKEN BY FINGER PRESSURE. CAN BE SCRATCHED READILY BY FINGER NAIL.</p>		<p>MOISTURE CONTENT</p> <p>V - VERY</p> <p>VST - VANE SHEAR TEST</p> <p>WEA. - WEATHERED</p> <p>W - UNIT WEIGHT</p> <p>W<sub>d</sub> - DRY UNIT WEIGHT</p>																																														
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SOIL MOISTURE - CORRELATION OF TERMS		ABBREVIATIONS		EQUIPMENT USED ON SUBJECT PROJECT		FRACTURE SPACING																																																												
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<p>DESCRIPTIONS MAY INCLUDE COLOR OR COLOR COMBINATIONS (TAN, RED, YELLOW-BROWN, BLUE-GRAY). MODIFIERS SUCH AS LIGHT, DARK, STREAKED, ETC. ARE USED TO DESCRIBE APPEARANCE.</p>		<p></p>		<p></p>		<p></p>																																																												

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**TIP PROJECT: U-2551**

**PROJECT: 34832.1.1**

See Sheet 1-A For Index of Sheets  
See Sheet 1-B For Conventional Symbols



**VICINITY MAP**  
A PORTION OF THIS PROJECT IS WITHIN THE MUNICIPAL BOUNDARIES OF MORGANTON

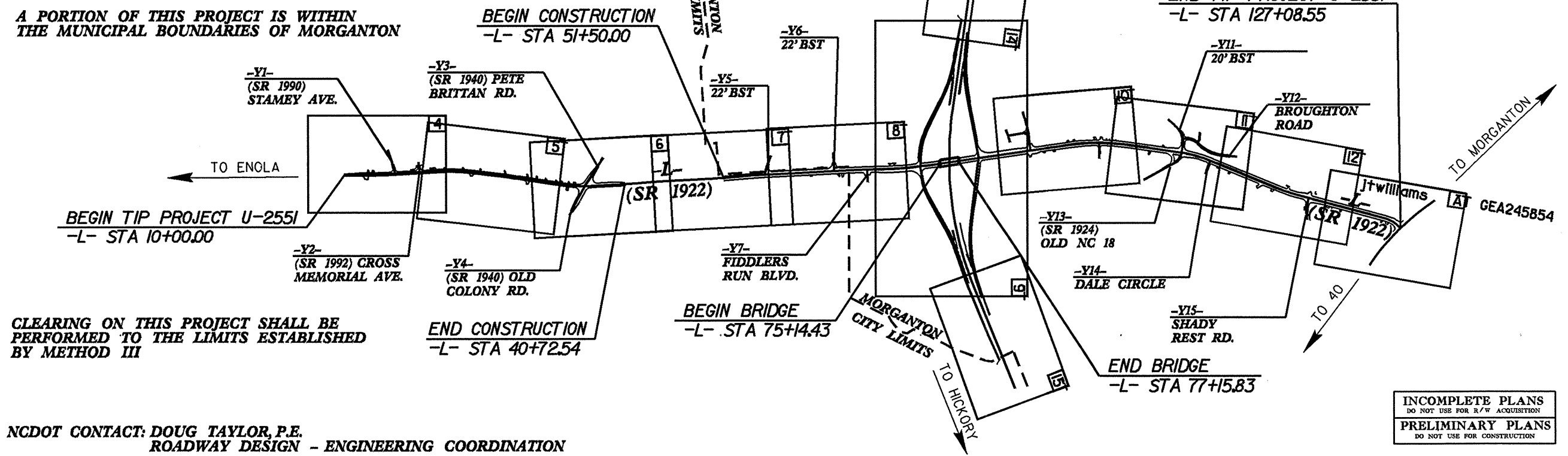
STATE OF NORTH CAROLINA  
DIVISION OF HIGHWAYS

**BURKE COUNTY**

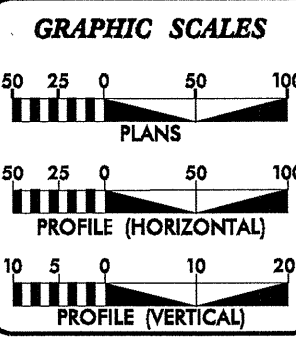
**LOCATION:** Morganton - SR 1922 (Enola Rd) / SR 1924 (Old NC 18) from South of Pete Brittan (SR 1940) Road to NC 18

**TYPE OF WORK:** GRADING, PAVING, DRAINAGE, STRUCTURE, AND SIGNALS

STATE	STATE PROJECT REFERENCE NO.	SUBST. NO.	TOTAL SHEETS
N.C.	U-2551	1A	73
STATE PROJ. NO.	F.A. PROJ. NO.	DESCRIPTION	
34832.1.1	STP-1922 (1)	PE	



INCOMPLETE PLANS  
DO NOT USE FOR R/W ACQUISITION  
PRELIMINARY PLANS  
DO NOT USE FOR CONSTRUCTION



**DESIGN DATA**

ADT 2010 =	14650
ADT 2030 =	21790
DHV =	9 %
D =	60 %
T =	7 % *
V =	40 MPH
* TTST 3	DUAL 4

**PROJECT LENGTH**

LENGTH OF ROADWAY TIP PROJECT U-2551 =	1.975 miles
LENGTH OF STRUCTURE TIP PROJECT U-2551 =	0.038 miles
TOTAL LENGTH TIP PROJECT U-2551 =	2.013 miles

PLANS PREPARED BY: **ARCADIS**

FOR THE DIVISION OF HIGHWAYS

2006 STANDARD SPECIFICATIONS

RIGHT OF WAY DATE: **STEVE SMALLWOOD, P.E.**  
AUGUST 21, 2009  
PROJECT ENGINEER

LETTING DATE: **FEBRUARY 15, 2011**

**HYDRAULICS ENGINEER**

SIGNATURE: \_\_\_\_\_ P.E.

**ROADWAY DESIGN ENGINEER**

SIGNATURE: \_\_\_\_\_ P.E.

**DIVISION OF HIGHWAYS**  
STATE OF NORTH CAROLINA

STATE HIGHWAY DESIGN ENGINEER

**NORTH CAROLINA DEPARTMENT OF TRANSPORTATION**  
**DIVISION OF HIGHWAYS**  
**GEOTECHNICAL ENGINEERING UNIT**

**SUBSURFACE INVESTIGATION**

**SOIL AND ROCK LEGEND, TERMS, SYMBOLS, AND ABBREVIATIONS**

SOIL DESCRIPTION										GRADATION										ROCK DESCRIPTION										TERMS AND DEFINITIONS																													
SOIL IS CONSIDERED TO BE THE UNCONSOLIDATED, SEMI-CONSOLIDATED, OR WEATHERED EARTH MATERIALS THAT CAN BE PENETRATED WITH A CONTINUOUS FLIGHT POWER AUGER, AND YIELD LESS THAN 100 BLOWS PER FOOT ACCORDING TO STANDARD PENETRATION TEST (AASHTO T206, ASTM D-1586). SOIL CLASSIFICATION IS BASED ON THE AASHTO SYSTEM. BASIC DESCRIPTIONS GENERALLY SHALL INCLUDE: CONSISTENCY, COLOR, TEXTURE, MOISTURE, AASHTO CLASSIFICATION, AND OTHER PERTINENT FACTORS SUCH AS MINERALOGICAL COMPOSITION, ANGULARITY, STRUCTURE, PLASTICITY, ETC. EXAMPLE: <i>VERY STIFF, GRAY, SILTY CLAY, MOST WITH INTERBEDDED FINE SAND LAYERS, HEAVY PLASTIC, A-7-6</i>										WELL GRADED - INDICATES A GOOD REPRESENTATION OF PARTICLE SIZES FROM FINE TO COARSE. UNIFORM - INDICATES THAT SOIL PARTICLES ARE ALL APPROXIMATELY THE SAME SIZE. ALSO POORLY GRADED. GAP-GRADED - INDICATES A MIXTURE OF UNIFORM PARTICLES OF TWO OR MORE SIZES. ANGULARITY OF GRAINS THE ANGULARITY OR ROUNDNESS OF SOIL GRAINS IS DESIGNATED BY THE TERMS: ANGULAR, SUBANGULAR, SUBROUNDED, OR ROUNDED.										HARD ROCK IS NON-COASTAL PLAIN MATERIAL THAT IF TESTED, WOULD YIELD SPT REFUSAL. AN INFERRED ROCK LINE INDICATES THE LEVEL AT WHICH NON-COASTAL PLAIN MATERIAL WOULD YIELD SPT REFUSAL. SPT REFUSAL IS PENETRATION BY A SPLIT SPOON SAMPLER EQUAL TO OR LESS THAN 0.1 FOOT PER 60 BLOWS. IN NON-COASTAL PLAIN MATERIAL, THE TRANSITION BETWEEN SOIL AND ROCK IS OFTEN REPRESENTED BY A ZONE OF WEATHERED ROCK. ROCK MATERIALS ARE TYPICALLY DIVIDED AS FOLLOWS: WEATHERED ROCK (WR) NON-COASTAL PLAIN MATERIAL THAT WOULD YIELD SPT N VALUES > 100 BLOWS PER FOOT IF TESTED. CRYSTALLINE ROCK (CR) FINE TO COARSE GRAIN IGNEOUS AND METAMORPHIC ROCK THAT WOULD YIELD SPT REFUSAL IF TESTED. ROCK TYPE INCLUDES GRANITE, GNEISS, GABBRO, SCHIST, ETC. NON-CRYSTALLINE ROCK (NCR) FINE TO COARSE GRAIN METAMORPHIC AND NON-COASTAL PLAIN SEDIMENTARY ROCK THAT WOULD YIELD SPT REFUSAL IF TESTED. ROCK TYPE INCLUDES PHYLLITE, SLATE, SANDSTONE, ETC. COASTAL PLAIN SEDIMENTARY ROCK (CP) COASTAL PLAIN SEDIMENTS CEMENTED INTO ROCK, BUT MAY NOT YIELD SPT REFUSAL. ROCK TYPE INCLUDES LIMESTONE, SANDSTONE, CEMENTED SHELL BEDS, ETC.										ALLOVIUM (ALLUV.) - SOILS THAT HAVE BEEN TRANSPORTED BY WATER. AQUIFER - A WATER BEARING FORMATION OR STRATA. ARENACEOUS - APPLIED TO ROCKS THAT HAVE BEEN DERIVED FROM SAND OR THAT CONTAIN SAND. ARGILLACEOUS - APPLIED TO ALL ROCKS OR SUBSTANCES COMPOSED OF CLAY MINERALS, OR HAVING A NOTABLE PROPORTION OF CLAY IN THEIR COMPOSITION, AS SHALE, SLATE, ETC. ARTESIAN - GROUND WATER THAT IS UNDER SUFFICIENT PRESSURE TO RISE ABOVE THE LEVEL AT WHICH IT IS ENCOUNTERED, BUT WHICH DOES NOT NECESSARILY RISE TO OR ABOVE THE GROUND SURFACE. CALCAREOUS (CALC.) - SOILS THAT CONTAIN APPRECIABLE AMOUNTS OF CALCIUM CARBONATE. COLLUVIUM - ROCK FRAGMENTS MIXED WITH SOIL DEPOSITED BY GRAVITY ON SLOPE OR AT BOTTOM OF SLOPE. CORE RECOVERY (REC.) - TOTAL LENGTH OF ALL MATERIAL RECOVERED IN THE CORE BARREL DIVIDED BY TOTAL LENGTH OF CORE RUN AND EXPRESSED AS A PERCENTAGE. DIKE - A TABULAR BODY OF IGNEOUS ROCK THAT CUTS ACROSS THE STRUCTURE OF ADJACENT ROCKS OR CUTS MASSIVE ROCK. DIP - THE ANGLE AT WHICH A STRATUM OR ANY PLANAR FEATURE IS INCLINED FROM THE HORIZONTAL. DIP DIRECTION (DIP AZIMUTH) - THE DIRECTION OR BEARING OF THE HORIZONTAL TRACE OF THE LINE OF DIP, MEASURED CLOCKWISE FROM NORTH. FAULT - A FRACTURE OR FRACTURE ZONE ALONG WHICH THERE HAS BEEN DISPLACEMENT OF THE SIDES RELATIVE TO ONE ANOTHER PARALLEL TO THE FRACTURE. FISSILE - A PROPERTY OF SPLITTING ALONG CLOSELY SPACED PARALLEL PLANES. FLOAT - ROCK FRAGMENTS ON SURFACE NEAR THEIR ORIGINAL POSITION AND DISLOGGED FROM PARENT MATERIAL. FLOOD PLAIN (FP) - LAND BORDERING A STREAM, BUILT OF SEDIMENTS DEPOSITED BY THE STREAM. FORMATION (FM) - A MAPPABLE GEOLOGIC UNIT THAT CAN BE RECOGNIZED AND TRACED IN THE FIELD. JOINT - FRACTURE IN ROCK ALONG WHICH NO APPRECIABLE MOVEMENT HAS OCCURRED. LEDGE - A SHELF-LIKE RIDGE OR PROJECTION OF ROCK WHOSE THICKNESS IS SMALL COMPARED TO ITS LATERAL EXTENT. LENS - A BODY OF SOIL OR ROCK THAT THINS OUT IN ONE OR MORE DIRECTIONS. MOTTLED (MOT.) - IRREGULARLY MARKED WITH SPOTS OF DIFFERENT COLORS. MOTTLING IN SOILS USUALLY INDICATES POOR AERATION AND LACK OF GOOD DRAINAGE. PERCHED WATER - WATER MAINTAINED ABOVE THE NORMAL GROUND WATER LEVEL BY THE PRESENCE OF AN INTERVENING IMPERVIOUS STRATUM. RESIDUAL (RES.) SOIL - SOIL FORMED IN PLACE BY THE WEATHERING OF ROCK. ROCK QUALITY DESIGNATION (RQD) - A MEASURE OF ROCK QUALITY DESCRIBED BY TOTAL LENGTH OF ROCK SEGMENTS EQUAL TO OR GREATER THAN 4 INCHES DIVIDED BY THE TOTAL LENGTH OF CORE RUN AND EXPRESSED AS A PERCENTAGE. SAPROLITE (SAP.) - RESIDUAL SOIL THAT RETAINS THE RELIC STRUCTURE OR FABRIC OF THE PARENT ROCK. SILL - AN INTRUSIVE BODY OF IGNEOUS ROCK OF APPROXIMATELY UNIFORM THICKNESS AND RELATIVELY THIN COMPARED WITH ITS LATERAL EXTENT, THAT HAS BEEN EMPLACED PARALLEL TO THE BEDDING OR SCHISTOSITY OF THE INTRODUCED ROCKS. SLICKENSIDE - POLISHED AND STRIATED SURFACE THAT RESULTS FROM FRICTION ALONG A FAULT OR SLIP PLANE. STANDARD PENETRATION TEST (PENETRATION RESISTANCE) (SPT) - NUMBER OF BLOWS (IN OR BPF) OF A 140 LB. HAMMER FALLING 30 INCHES REQUIRED TO PRODUCE A PENETRATION OF 1 FOOT INTO SOIL WITH A 2 INCH OUTSIDE DIAMETER SPLIT SPOON SAMPLER. SPT REFUSAL IS PENETRATION EQUAL TO OR LESS THAN 0.1 FOOT PER 60 BLOWS. STRATA CORE RECOVERY (SCREC) - TOTAL LENGTH OF STRATA MATERIAL RECOVERED DIVIDED BY TOTAL LENGTH OF STRATUM AND EXPRESSED AS A PERCENTAGE. STRATA ROCK QUALITY DESIGNATION (SRQD) - A MEASURE OF ROCK QUALITY DESCRIBED BY TOTAL LENGTH OF ROCK SEGMENTS WITHIN A STRATUM EQUAL TO OR GREATER THAN 4 INCHES DIVIDED BY THE TOTAL LENGTH OF STRATA AND EXPRESSED AS A PERCENTAGE. TOPSOIL (TS) - SURFACE SOILS USUALLY CONTAINING ORGANIC MATTER.																													
SOIL LEGEND AND AASHTO CLASSIFICATION										MINERALOGICAL COMPOSITION										WEATHERING										ROCK HARDNESS																													
GENERAL CLASS. GRANULAR MATERIALS (<= 35% PASSING #200) SILT-CLAY MATERIALS (> 35% PASSING #200) ORGANIC MATERIALS										MINERAL NAMES SUCH AS QUARTZ, FELDSPAR, MICA, TALC, KAOLIN, ETC. ARE USED IN DESCRIPTIONS WHENEVER THEY ARE CONSIDERED OF SIGNIFICANCE.										FRESH ROCK FRESH, CRYSTALS BRIGHT, FEW JOINTS MAY SHOW SLIGHT STAINING. ROCK RINGS UNDER HAMMER IF CRYSTALLINE. VERY SLIGHT (V SLI.) ROCK GENERALLY FRESH, JOINTS STAINED, SOME JOINTS MAY SHOW THIN CLAY COATINGS IF OPEN. CRYSTALS ON A BROKEN SPECIMEN FACE SHINE BRIGHTLY. ROCK RINGS UNDER HAMMER BLOWS IF OF A CRYSTALLINE NATURE. SLIGHT (SLI.) ROCK GENERALLY FRESH, JOINTS STAINED AND DISCOLORATION EXTENDS INTO ROCK UP TO 1 INCH. OPEN JOINTS MAY CONTAIN CLAY. IN GRANITOID ROCKS SOME OCCASIONAL FELDSPAR CRYSTALS ARE DULL AND DISCOLORED. CRYSTALLINE ROCKS RING UNDER HAMMER BLOWS. MODERATE (MOD.) SIGNIFICANT PORTIONS OF ROCK SHOW DISCOLORATION AND WEATHERING EFFECTS. IN GRANITOID ROCKS, MOST FELDSPARS ARE DULL AND DISCOLORED, SOME SHOW CLAY. ROCK HAS DULL SOUND UNDER HAMMER BLOWS AND SHOWS SIGNIFICANT LOSS OF STRENGTH AS COMPARED WITH FRESH ROCK. MODERATELY SEVERE (MOD. SEV.) ALL ROCK EXCEPT QUARTZ DISCOLORED OR STAINED. IN GRANITOID ROCKS, ALL FELDSPARS DULL AND DISCOLORED AND A MAJORITY SHOW KAOLINIZATION. ROCK SHOWS SEVERE LOSS OF STRENGTH AND CAN BE EXCAVATED WITH A GEOLOGIST'S PICK. ROCK GIVES "CLUNK" SOUND WHEN STRUCK. <i>IF TESTED, WOULD YIELD SPT REFUSAL</i> SEVERE (SEV.) ALL ROCK EXCEPT QUARTZ DISCOLORED OR STAINED. ROCK FABRIC CLEAR AND EVIDENT BUT REDUCED IN STRENGTH TO STRONG SOIL. IN GRANITOID ROCKS ALL FELDSPARS ARE KAOLINIZED TO SOME EXTENT. SOME FRAGMENTS OF STRONG ROCK USUALLY REMAIN. <i>IF TESTED, YIELDS SPT N VALUES &gt; 100 BPF</i> VERY SEVERE (V SEV.) ALL ROCK EXCEPT QUARTZ DISCOLORED OR STAINED. ROCK FABRIC ELEMENTS ARE DISCERNIBLE BUT THE MASS IS EFFECTIVELY REDUCED TO SOIL STATUS, WITH ONLY FRAGMENTS OF STRONG ROCK REMAINING. SAPROLITE IS AN EXAMPLE OF ROCK WEATHERED TO A DEGREE SUCH THAT ONLY MINOR VESTIGES OF THE ORIGINAL ROCK FABRIC REMAIN. <i>IF TESTED, YIELDS SPT N VALUES &lt; 100 BPF</i> COMPLETE ROCK REDUCED TO SOIL. ROCK FABRIC NOT DISCERNIBLE, OR DISCERNIBLE ONLY IN SMALL AND SCATTERED CONCENTRATIONS. QUARTZ MAY BE PRESENT AS DIXES OR STRINGERS. SAPROLITE IS ALSO AN EXAMPLE.										COMPRESSIONIBILITY SLIGHTLY COMPRESSIBLE LIQUID LIMIT LESS THAN 31 MODERATELY COMPRESSIBLE LIQUID LIMIT EQUAL TO 31-50 HIGHLY COMPRESSIBLE LIQUID LIMIT GREATER THAN 50										WEATHERING FRESH ROCK FRESH, CRYSTALS BRIGHT, FEW JOINTS MAY SHOW SLIGHT STAINING. ROCK RINGS UNDER HAMMER IF CRYSTALLINE. VERY SLIGHT (V SLI.) ROCK GENERALLY FRESH, JOINTS STAINED, SOME JOINTS MAY SHOW THIN CLAY COATINGS IF OPEN. CRYSTALS ON A BROKEN SPECIMEN FACE SHINE BRIGHTLY. 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PERCENTAGE OF MATERIAL ORGANIC MATERIAL GRANULAR SOILS SILT-CLAY SOILS OTHER MATERIAL TRACE OF ORGANIC MATTER 2-3% 3-5% TRACE 1-10% LITTLE ORGANIC MATTER 3-5% 5-12% LITTLE 10-20% MODERATELY ORGANIC 5-10% 12-20% SOME 20-35% HIGHLY ORGANIC >10% >20% HIGHLY 35% AND ABOVE										GROUND WATER WATER LEVEL IN BORE HOLE IMMEDIATELY AFTER DRILLING STATIC WATER LEVEL AFTER 24 HOURS PERCHED WATER, SATURATED ZONE, OR WATER BEARING STRATA SPRING OR SEEP										ROCK HARDNESS VERY HARD CANNOT BE SCRATCHED BY KNIFE OR SHARP PICK. BREAKING OF HAND SPECIMENS REQUIRES SEVERAL HARD BLOWS OF THE GEOLOGIST'S PICK. HARD CAN BE SCRATCHED BY KNIFE OR PICK ONLY WITH DIFFICULTY. HARD HAMMER BLOWS REQUIRED TO DETACH HAND SPECIMEN. MODERATELY HARD CAN BE SCRATCHED BY KNIFE OR PICK. GOUGES OR GROOVES TO 0.25 INCHES DEEP CAN BE EXCAVATED BY HARD BLOW OF A GEOLOGIST'S PICK. HAND SPECIMENS CAN BE DETACHED BY MODERATE BLOWS. MEDIUM HARD CAN BE GROOVED OR GOUGED 0.05 INCHES DEEP BY FIRM PRESSURE OF KNIFE OR PICK POINT. CAN BE EXCAVATED IN SMALL CHIPS TO PIECES 1 INCH MAXIMUM SIZE BY HARD BLOWS OF THE POINT OF A GEOLOGIST'S PICK. SOFT CAN BE GROOVED OR GOUGED READILY BY KNIFE OR PICK. CAN BE EXCAVATED IN FRAGMENTS FROM CHIPS TO SEVERAL INCHES IN SIZE BY MODERATE BLOWS OF A PICK POINT. SMALL, THIN PIECES CAN BE BROKEN BY FINGER PRESSURE. VERY SOFT CAN BE CARVED WITH KNIFE. CAN BE EXCAVATED READILY WITH POINT OF PICK. PIECES 1 INCH OR MORE IN THICKNESS CAN BE BROKEN BY FINGER PRESSURE. CAN BE SCRATCHED READILY BY FINGERNAIL.																																							
USUAL TYPES OF MAJOR MATERIALS GRAVEL AND SAND FINE SAND SILTY OR CLAYEY GRAVEL AND SAND SILTY SOILS CLAYEY SOILS GRANULAR SOILS SILT-CLAY SOILS MUCK, PEAT HIGHLY ORGANIC SOILS										MISCELLANEOUS SYMBOLS ROADWAY EMBANKMENT (RE) WITH SOIL DESCRIPTION SOIL SYMBOL ARTIFICIAL FILL (AF) OTHER THAN ROADWAY EMBANKMENT INFERRED SOIL BOUNDARY INFERRED ROCK LINE ALLUVIAL SOIL BOUNDARY DIP & DIP DIRECTION OF ROCK STRUCTURES SOUNDING ROD SPT TEST BORING AUGER BORING CORE BORING MONITORING WELL PIEZOMETER INSTALLATION SLOPE INDICATOR INSTALLATION SPT N-VALUE SPT REFUSAL S - BULK SAMPLE SS - SPLIT SPOON SAMPLE ST - SHELBY TUBE SAMPLE RS - ROCK SAMPLE RT - RECOMPACTED TRIAXIAL SAMPLE CBR - CALIFORNIA BEARING RATIO SAMPLE HAND AUGER BORING										ROCK HARDNESS VERY HARD CANNOT BE SCRATCHED BY KNIFE OR SHARP PICK. BREAKING OF HAND SPECIMENS REQUIRES SEVERAL HARD BLOWS OF THE GEOLOGIST'S PICK. HARD CAN BE SCRATCHED BY KNIFE OR PICK ONLY WITH DIFFICULTY. HARD HAMMER BLOWS REQUIRED TO DETACH HAND SPECIMEN. MODERATELY HARD CAN BE SCRATCHED BY KNIFE OR PICK. GOUGES OR GROOVES TO 0.25 INCHES DEEP CAN BE EXCAVATED BY HARD BLOW OF A GEOLOGIST'S PICK. HAND SPECIMENS CAN BE DETACHED BY MODERATE BLOWS. MEDIUM HARD CAN BE GROOVED OR GOUGED 0.05 INCHES DEEP BY FIRM PRESSURE OF KNIFE OR PICK POINT. CAN BE EXCAVATED IN SMALL CHIPS TO PIECES 1 INCH MAXIMUM SIZE BY HARD BLOWS OF THE POINT OF A GEOLOGIST'S PICK. SOFT CAN BE GROOVED OR GOUGED READILY BY KNIFE OR PICK. CAN BE EXCAVATED IN FRAGMENTS FROM CHIPS TO SEVERAL INCHES IN SIZE BY MODERATE BLOWS OF A PICK POINT. SMALL, THIN PIECES CAN BE BROKEN BY FINGER PRESSURE. VERY SOFT CAN BE CARVED WITH KNIFE. CAN BE EXCAVATED READILY WITH POINT OF PICK. PIECES 1 INCH OR MORE IN THICKNESS CAN BE BROKEN BY FINGER PRESSURE. CAN BE SCRATCHED READILY BY FINGERNAIL.																																							
TEXTURE OR GRAIN SIZE U.S. STD. SIEVE SIZE OPENING (MM) 4 10 40 60 200 270 4.76 2.00 0.42 0.25 0.075 0.053										ABBREVIATIONS AR - AUGER REFUSAL BT - BORING TERMINATED CL - CLAY CPT - CONE PENETRATION TEST CSE - COARSE DMT - DILATOMETER TEST DPT - DYNAMIC PENETRATION TEST e - VOID RATIO F - FINE FOSS - FOSSILIFEROUS FRAC - FRACTURED, FRACTURES FRAGS. - FRAGMENTS HI. - HIGHLY MED. - MEDIUM MICA - MICACEOUS MOD. - MODERATELY NP - NON PLASTIC ORG. - ORGANIC PMT - PRESSUREMETER TEST SAP. - SAPROLITIC SD. - SAND, SANDY SL. - SILT, SILTY SLI. - SLIGHTLY TCR - TRICONE REFUSAL w - MOISTURE CONTENT v - VERY VST - VANE SHEAR TEST WEA. - WEATHERED % - UNIT WEIGHT % - DRY UNIT WEIGHT										ROCK HARDNESS VERY HARD CANNOT BE SCRATCHED BY KNIFE OR SHARP PICK. BREAKING OF HAND SPECIMENS REQUIRES SEVERAL HARD BLOWS OF THE GEOLOGIST'S PICK. HARD CAN BE SCRATCHED BY KNIFE OR PICK ONLY WITH DIFFICULTY. HARD HAMMER BLOWS REQUIRED TO DETACH HAND SPECIMEN. MODERATELY HARD CAN BE SCRATCHED BY KNIFE OR PICK. GOUGES OR GROOVES TO 0.25 INCHES DEEP CAN BE EXCAVATED BY HARD BLOW OF A GEOLOGIST'S PICK. HAND SPECIMENS CAN BE DETACHED BY MODERATE BLOWS. MEDIUM HARD CAN BE GROOVED OR GOUGED 0.05 INCHES DEEP BY FIRM PRESSURE OF KNIFE OR PICK POINT. CAN BE EXCAVATED IN SMALL CHIPS TO PIECES 1 INCH MAXIMUM SIZE BY HARD BLOWS OF THE POINT OF A GEOLOGIST'S PICK. SOFT CAN BE GROOVED OR GOUGED READILY BY KNIFE OR PICK. CAN BE EXCAVATED IN FRAGMENTS FROM CHIPS TO SEVERAL INCHES IN SIZE BY MODERATE BLOWS OF A PICK POINT. SMALL, THIN PIECES CAN BE BROKEN BY FINGER PRESSURE. VERY SOFT CAN BE CARVED WITH KNIFE. CAN BE EXCAVATED READILY WITH POINT OF PICK. PIECES 1 INCH OR MORE IN THICKNESS CAN BE BROKEN BY FINGER PRESSURE. CAN BE SCRATCHED READILY BY FINGERNAIL.																																							
SOIL MOISTURE - CORRELATION OF TERMS SOIL MOISTURE SCALE (ATTERBERG LIMITS) FIELD MOISTURE DESCRIPTION GUIDE FOR FIELD MOISTURE DESCRIPTION LL - LIQUID LIMIT PL - PLASTIC LIMIT OM - OPTIMUM MOISTURE SL - SHRINKAGE LIMIT - SATURATED - (SAT.) USUALLY LIQUID; VERY WET, USUALLY FROM BELOW THE GROUND WATER TABLE - WET - (w) SEMISOLID; REQUIRES DRYING TO ATTAIN OPTIMUM MOISTURE - MOIST - (m) SOLID; AT OR NEAR OPTIMUM MOISTURE - DRY - (d) REQUIRES ADDITIONAL WATER TO ATTAIN OPTIMUM MOISTURE										EQUIPMENT USED ON SUBJECT PROJECT DRILL UNITS: MOBILE B-51, CME-45C, CME-550, PORTABLE HOIST ADVANCING TOOLS: CLAY BITS, 6" CONTINUOUS FLIGHT AUGER, 6" HOLLOW AUGERS, HARD FACED FINGER BITS, TUNG-CARBIDE INSERTS, CASING w/ ADVANCER, TRICONE STEEL TEETH, TRICONE TUNG-CARB. CORE BIT HAMMER TYPE: AUTOMATIC, MANUAL CORE SIZE: B, N, H HAND TOOLS: POST HOLE DIGGER, HAND AUGER, SOUNDING ROD, VANE SHEAR TEST										ROCK HARDNESS VERY HARD CANNOT BE SCRATCHED BY KNIFE OR SHARP PICK. BREAKING OF HAND SPECIMENS REQUIRES SEVERAL HARD BLOWS OF THE GEOLOGIST'S PICK. HARD CAN BE SCRATCHED BY KNIFE OR PICK ONLY WITH DIFFICULTY. HARD HAMMER BLOWS REQUIRED TO DETACH HAND SPECIMEN. MODERATELY HARD CAN BE SCRATCHED BY KNIFE OR PICK. GOUGES OR GROOVES TO 0.25 INCHES DEEP CAN BE EXCAVATED BY HARD BLOW OF A GEOLOGIST'S PICK. HAND SPECIMENS CAN BE DETACHED BY MODERATE BLOWS. MEDIUM HARD CAN BE GROOVED OR GOUGED 0.05 INCHES DEEP BY FIRM PRESSURE OF KNIFE OR PICK POINT. CAN BE EXCAVATED IN SMALL CHIPS TO PIECES 1 INCH MAXIMUM SIZE BY HARD BLOWS OF THE POINT OF A GEOLOGIST'S PICK. SOFT CAN BE GROOVED OR GOUGED READILY BY KNIFE OR PICK. CAN BE EXCAVATED IN FRAGMENTS FROM CHIPS TO SEVERAL INCHES IN SIZE BY MODERATE BLOWS OF A PICK POINT. SMALL, THIN PIECES CAN BE BROKEN BY FINGER PRESSURE. VERY SOFT CAN BE CARVED WITH KNIFE. CAN BE EXCAVATED READILY WITH POINT OF PICK. PIECES 1 INCH OR MORE IN THICKNESS CAN BE BROKEN BY FINGER PRESSURE. CAN BE SCRATCHED READILY BY FINGERNAIL.																																							
PLASTICITY NONPLASTIC LOW PLASTICITY MED. PLASTICITY HIGH PLASTICITY PLASTICITY INDEX (PI) DRY STRENGTH VERY LOW SLIGHT MEDIUM HIGH										FRACTURE SPACING TERM SPACING VERY WIDE MORE THAN 10 FEET WIDE 3 TO 10 FEET MODERATELY CLOSE 1 TO 3 FEET CLOSE 0.16 TO 1 FEET VERY CLOSE LESS THAN 0.16 FEET TERM THICKNESS VERY THICKLY BEDDED > 4 FEET THICKLY BEDDED 1.5 - 4 FEET THINLY BEDDED 0.16 - 1.5 FEET VERY THINLY BEDDED 0.03 - 0.16 FEET THICKLY LAMINATED 0.008 - 0.03 FEET THINLY LAMINATED < 0.008 FEET										ROCK HARDNESS VERY HARD CANNOT BE SCRATCHED BY KNIFE OR SHARP PICK. BREAKING OF HAND SPECIMENS REQUIRES SEVERAL HARD BLOWS OF THE GEOLOGIST'S PICK. HARD CAN BE SCRATCHED BY KNIFE OR PICK ONLY WITH DIFFICULTY. HARD HAMMER BLOWS REQUIRED TO DETACH HAND SPECIMEN. MODERATELY HARD CAN BE SCRATCHED BY KNIFE OR PICK. GOUGES OR GROOVES TO 0.25 INCHES DEEP CAN BE EXCAVATED BY HARD BLOW OF A GEOLOGIST'S PICK. HAND SPECIMENS CAN BE DETACHED BY MODERATE BLOWS. MEDIUM HARD CAN BE GROOVED OR GOUGED 0.05 INCHES DEEP BY FIRM PRESSURE OF KNIFE OR PICK POINT. CAN BE EXCAVATED IN SMALL CHIPS TO PIECES 1 INCH MAXIMUM SIZE BY HARD BLOWS OF THE POINT OF A GEOLOGIST'S PICK. SOFT CAN BE GROOVED OR GOUGED READILY BY KNIFE OR PICK. CAN BE EXCAVATED IN FRAGMENTS FROM CHIPS TO SEVERAL INCHES IN SIZE BY MODERATE BLOWS OF A PICK POINT. SMALL, THIN PIECES CAN BE BROKEN BY FINGER PRESSURE. VERY SOFT CAN BE CARVED WITH KNIFE. CAN BE EXCAVATED READILY WITH POINT OF PICK. PIECES 1 INCH OR MORE IN THICKNESS CAN BE BROKEN BY FINGER PRESSURE. CAN BE SCRATCHED READILY BY FINGERNAIL.																																							
COLOR DESCRIPTIONS MAY INCLUDE COLOR OR COLOR COMBINATIONS (TAN, RED, YELLOW-BROWN, BLUE-GRAY). MODIFIERS SUCH AS LIGHT, DARK, STREAKED, ETC. ARE USED TO DESCRIBE APPEARANCE.										INDURATION FOR SEDIMENTARY ROCKS, INDURATION IS THE HARDENING OF THE MATERIAL BY CEMENTING, HEAT, PRESSURE, ETC. FRIABLE RUBBING WITH FINGER FREES NUMEROUS GRAINS; GENTLE BLOW BY HAMMER DISINTEGRATES SAMPLE. MODERATELY INDURATED GRAINS CAN BE SEPARATED FROM SAMPLE WITH STEEL PROBE; BREAKS EASILY WHEN HIT WITH HAMMER. INDURATED GRAINS ARE DIFFICULT TO SEPARATE WITH STEEL PROBE; DIFFICULT TO BREAK WITH HAMMER. EXTREMELY INDURATED SHARP HAMMER BLOWS REQUIRED TO BREAK SAMPLE; SAMPLE BREAKS ACROSS GRAINS.										ROCK HARDNESS VERY HARD CANNOT BE SCRATCHED BY KNIFE OR SHARP PICK. BREAKING OF HAND SPECIMENS REQUIRES SEVERAL HARD BLOWS OF THE GEOLOGIST'S PICK. HARD CAN BE SCRATCHED BY KNIFE OR PICK ONLY WITH DIFFICULTY. HARD HAMMER BLOWS REQUIRED TO DETACH HAND SPECIMEN. MODERATELY HARD CAN BE SCRATCHED BY KNIFE OR PICK. GOUGES OR GROOVES TO 0.25 INCHES DEEP CAN BE EXCAVATED BY HARD BLOW OF A GEOLOGIST'S PICK. HAND SPECIMENS CAN BE DETACHED BY MODERATE BLOWS. MEDIUM HARD CAN BE GROOVED OR GOUGED 0.05 INCHES DEEP BY FIRM PRESSURE OF KNIFE OR PICK POINT. CAN BE EXCAVATED IN SMALL CHIPS TO PIECES 1 INCH MAXIMUM SIZE BY HARD BLOWS OF THE POINT OF A GEOLOGIST'S PICK. SOFT CAN BE GROOVED OR GOUGED READILY BY KNIFE OR PICK. CAN BE EXCAVATED IN FRAGMENTS FROM CHIPS TO SEVERAL INCHES IN SIZE BY MODERATE BLOWS OF A PICK POINT. SMALL, THIN PIECES CAN BE BROKEN BY FINGER PRESSURE. VERY SOFT CAN BE CARVED WITH KNIFE. CAN BE EXCAVATED READILY WITH POINT OF PICK. PIECES 1 INCH OR MORE IN THICKNESS CAN BE BROKEN BY FINGER PRESSURE. CAN BE SCRATCHED READILY BY FINGERNAIL.																																							
BENCH MARK: BM-3 MARKER IN 24" PINE -BL- STA. 37+93 106 RT										BENCH MARK: BM-3 MARKER IN 24" PINE -BL- STA. 37+93 106 RT										BENCH MARK: BM-3 MARKER IN 24" PINE -BL- STA. 37+93 106 RT										BENCH MARK: BM-3 MARKER IN 24" PINE -BL- STA. 37+93 106 RT																													
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STATE OF NORTH CAROLINA  
DEPARTMENT OF TRANSPORTATION

BEVERLY EAVES PERDUE  
GOVERNOR

EUGENE A. CONTI, JR.  
SECRETARY

April 14, 2009

STATE PROJECT: 34832.1.1 (U-2551)  
COUNTY: Burke  
DESCRIPTION: Morganton – SR 1922 (Enola Road)/ SR 1924 (Old NC 18)  
From South of Pete Brittian (SR 1940) Road to NC 18  
SUBJECT: Geotechnical Report – Inventory

**Project Description**

The project consists of widening approximately two miles of existing 2-lane to improved 2-lane or 4-lane. A stretch of Enola Road from Station 40+72 to 51+50 is not included in the project. Much of the project borders the J. Iverson Riddle Developmental Center and Broughton Hospital. A section of I-40 and the interchange at exit 104 is included along with new alignment for all four ramps. Many sections of -Y- Line are also improved with limited relocated alignment.

**Areas of Special Geotechnical Interest**

Structures

The bridge from Stations 75+18 to 77+23 -L- is founded on deep residuum/saprolite. The bridge investigation and recommendations are not part of this inventory and will be addressed at a later date.

Potentially Unsuitable Subsurface Conditions

Fill: An elaborate subsurface drainage system on the grounds of Broughton Hospital left of Stations 119+00 to 126+50 -L- carries headwater stream flow through rather extensive areas of deep fill. The fill is predominantly earth material and has rubble from demolished structures and some asphalt. Much of the fill appears usable for roadway

foundation with the exception of an area which may have formerly been impounded (Lt. of -L- Sta. 119+00 to approximately 121+00).

Fill: An area along -Y12- from Stations 12+70 to 15+00 contains fill, less than four feet deep, that has some rubble in it.

Fill: Red clay runoff sedimentation up to four feet deep has been deposited over brown floodplain soils of Fiddlers Run to the right of -Y8-. This material apparently originated from the gulling of a ditch right of -Y8- from Stations ± 51+00 to ± 55+00.

Portions of Ramp B have embankment placed there as apparent waste material from the original construction of the interstate. This material is clayey and has standard penetration test resistance values comparable to that of nearby residual clays.

**Physiography and Geology**

From the -L- Station 10+00 to 111+00 the project runs along a drainage divide and then descends and ends along a filled in gully at Sterling Street. The land is easily gullied and most near by streams are choked with sediments from historic times. One creek, Fiddlers Run, is very unusual because it has recently lowered its stream bed and cut through basal alluvium as observed just upstream of -Y8-. Two other small creeks located to the left and right of -Y8- near Station 25+00 are recent gullies that have intercepted the water table. They have no alluvium.

Rock type is obscured by thick soils but was observed in samples of saprolite to be dominantly gneisses and schists with some intrusives (granite to granodiorite). If large enough in extent, granitic areas tend to have shallow rock, otherwise granite is found in thin veins in the country rock.

Soil Properties

Thick piedmont type ridge top residuum is present along most of the project. Red residual clays there are typically three to six feet thick and often have several more feet of an underlying transition soil of red/orange clayey silt. Consistency of residuum is typically medium stiff. Underlying saprolitic soils tend to be sandy silts with varying amounts of sericitic mica. Saprolitic soils can be more than 100 feet deep in areas of metamorphic parent rock or less than ten feet deep in granitic areas.

Rock Properties

Gneissic and schistose rocks here are weathered to considerable depths and covered by thick residuum. A few areas along the project have intrusive rocks that are more resistant to weathering and nearly outcrop. This is noticeable near -L- Stations 13+00, 31+00 to 34+00, and 108+50. One area to the right of -L- Stations 121+50 to 124+50 has shallow crystalline rock.

### Groundwater

Groundwater was measured in only six borings. It was observed to be deep in borings near the bridge on -L- over -Y8- and conversely was shallow where a clay embankment was placed over residual clay soil on Ramp B and at -L- Station 20+00. The heads of drainages near the ridge top also exhibited a tendency for clayey residual soils to be wet as was observed in the boring at -L- Station 39+00.

### Wells

A monitoring well is located within construction limits at -L- Station 102+55, 60 feet right and is shown on plan view.

### **Geotechnical Descriptive Analysis**

-L- Station 10+00 to 37+00: The alignment will consist of short cuts and low fills in residual clays.

-L- Station 37+00 to 40+72.54: This section of -L- is poorly drained and possesses deep clays.

-L- Stations 51+50 to 111+00: The alignment is on ridge top with residual clays and has short cuts and low fills. The bridge over I-40 and all four ramps with the bulk of the earthwork on the project is included in this section.

-L- Stations 111+00 to 119+00: -L- runs along side slopes with deep residuum.

-L- Stations 119+00 to 127+08: The right of -L- is in an existing cut area with shallow crystalline rock. Left of -L- was a deep gully and creek valley that has been completely obscured by filling. Fill is predominantly earth material and contains some rubble of brick, stone, asphalt and occasional metal. The area is now a level woodlawn with large trees. An apparent pond was observed in the filled area left of Stations  $\pm$  119+10 to  $\pm$  120+00. Small areas to the left of -L- were apparently excavated but so extensively altered that they are included in the fill area noted on sheet 13.

Respectfully Submitted,

*P. Q. Lockamy*

P. Q. Lockamy P.G.

**EARTHWORK BALANCE SHEET**

PROJECT NAME Enola Road  
 STATE PROJECT NUMBER U-2551

COMPUTED BY STS  
 CHECKED BY \_\_\_\_\_

DATE SHEET February 23, 2012  
1 OF 4

STATION	STATION	TOTAL EXCAV. (UNCL.)	ROCK EXCAV.	UNDERCUT EXCAV.	UNSUIT. EXCAV.	SUITABLE EXCAV.	TOTAL EMB.	ROCK EMB.	EARTH EMB.	EMB. + 15%	BORROW	ROCK	SUITABLE WASTE	UNSUIT. WASTE	TOTAL WASTE
<b>SUMMARY #1</b>															
<b>-L- Left</b>															
10+50.00	40+72.54	2587				2587	170		170	196	0		2391		2391
<b>-Y1-</b>															
11+63.00	12+43.24	13				13	1		1	2	0		11		11
<b>-Y2-</b>															
10+38.00	10+93.42	13				13	0		0	0	0		13		13
<b>-Y3-</b>															
11+04.98	13+46.26	603				603	55		55	63	0		540		540
<b>TOTAL SUMMARY #1</b>		<b>3216</b>				<b>3216</b>	<b>227</b>		<b>227</b>	<b>261</b>	<b>0</b>		<b>2955</b>		<b>2955</b>
<b>SUMMARY #2</b>															
<b>-L Right-</b>															
10+50.00	40+72.54	2055				2055	3093		3093	3557	1502		0		0
<b>-Y4-</b>															
10+25.75	13+15.43	2101				2101	177		177	204	0		1897		1897
<b>TOTAL SUMMARY #2</b>		<b>4156</b>				<b>4156</b>	<b>3270</b>		<b>3270</b>	<b>3761</b>	<b>1502</b>		<b>1897</b>		<b>1897</b>
<b>SUMMARY #3</b>															
<b>-L- Left</b>															
51+50.00	75+12.21	1472				1472	1134		1134	1304	0		168		168
<b>-Y5-</b>															
10+94.00	11+86.63	271				271	26		26	29	0		241		241
<b>-RPB-</b>															
16+00.00	23+77.27	9217				9217	724		724	833	0		8384		8384
<b>-Y8RT-(RAMPB)</b>															
21+50.00	30+50.00	2086				2086	2119		2119	2437	351		0		0
<b>TOTAL SUMMARY #3</b>		<b>13046</b>				<b>13046</b>	<b>4003</b>		<b>4003</b>	<b>4604</b>	<b>351</b>		<b>8793</b>		<b>8793</b>

Earthwork quantities are calculated by the Roadway Design Unit. These earthwork quantities are based in part on subsurface data provided by the Geotechnical Engineering Unit.

## EARTHWORK BALANCE SHEET

SHEET

2

OF

4

STATION	STATION	TOTAL EXCAV. (UNCL.)	ROCK EXCAV.	UNDERCUT EXCAV.	UNSUIT. EXCAV.	SUITABLE EXCAV.	TOTAL EMB.	ROCK EMB.	EARTH EMB.	EMB. + 15%	BORROW	ROCK	SUITABLE WASTE	UNSUIT. WASTE	TOTAL WASTE
SUMMARY #4															
<b>-L- Right</b>															
51+50.00	75+12.21	1842				1842	2492		2492	2866	1024		0		0
<b>-RPC-</b>															
17+04.78	22+70.00	1978				1978	4190		4190	4819	2841		0		0
<b>-Y8RT-(RAMPC)</b>															
43+50.00	59+48.00	3345				3345	7495		7495	8619	5274		0		0
<b>TOTAL SUMMARY #4</b>		<b>7165</b>				<b>7165</b>	<b>14177</b>		<b>14177</b>	<b>16304</b>	<b>9139</b>		<b>0</b>		<b>0</b>
SUMMARY #5															
<b>-L- Left</b>															
77+22.21	101+65.00	880				880	1824		1824	2098	1218		0		0
<b>-RPA-</b>															
15+70.00	22+95.75	12400				12400	221		221	254	0		12146		12146
<b>-Y9-</b>															
11+84.82	13+00.00	86				86	46		46	53	0		33		33
<b>-SR1-</b>															
10+10.00	12+08.53	0				0	1667		1667	1917	1917		0		0
<b>-Y8LT-(RAMPA)</b>															
14+00.00	29+00.00	2895				2895	2196		2196	2525	0		370		370
<b>TOTAL SUMMARY #5</b>		<b>16261</b>				<b>16261</b>	<b>5954</b>		<b>5954</b>	<b>6847</b>	<b>3135</b>		<b>12549</b>		<b>12549</b>
SUMMARY #6															
<b>-L- Right</b>															
77+22.21	101+65.00	3834				3834	4130		4130	4750	916		0		0
<b>-RPD-</b>															
16+36.00	24+60.00	20766				20766	0		0	0	0		20766		20766
<b>-Y8LT-(RAMPA)</b>															
44+50.00	54+50.00	7876				7876	446		446	513	0		7363		7363
<b>-SR4-</b>															
10+29.73	11+15.54	182				182	14		14	16	0		166		166

Earthwork quantities are calculated by the Roadway Design Unit. These earthwork quantities are based in part on subsurface data provided by the Geotechnical Engineering Unit.



EARTHWORK BALANCE SHEET

SHEET

3

OF

4

STATION	STATION	TOTAL EXCAV. (UNCL.)	ROCK EXCAV.	UNDERCUT EXCAV.	UNSUIT. EXCAV.	SUITABLE EXCAV.	TOTAL EMB.	ROCK EMB.	EARTH EMB.	EMB. + 15%	BORROW	ROCK	SUITABLE WASTE	UNSUIT. WASTE	TOTAL WASTE
<b>-SR5- (-L- Station)</b>															
81+37.39	84+71.05	173				173	212		212	244	71		0		0
<b>-Y10-</b>															
10+29.52	11+20.00	18				18	26		26	30	12		0		0
<b>TOTAL SUMMARY #6</b>		<b>32849</b>				<b>32849</b>	<b>4828</b>		<b>4828</b>	<b>5553</b>	<b>999</b>		<b>28295</b>		<b>28295</b>
<b>SUMMARY #7</b>															
<b>-L- Left</b>															
101+65.00	126+74.09	3533		480		3533	5774		5774	6640	3107		0	480	480
<b>-Y11-</b>															
10+33.51	12+60.00	1865				1865	65		65	75	0		1790		1790
<b>-Y12-</b>															
10+25.24	15+60.00	182				182	6098		6098	7013	6831		0		0
<b>-DWY1-</b>															
10+00.00	11+06.23	3				3	425		425	489	486		0		0
<b>TOTAL SUMMARY #7</b>		<b>5583</b>		<b>480</b>		<b>5583</b>	<b>12362</b>		<b>12362</b>	<b>14217</b>	<b>10424</b>		<b>1790</b>	<b>480</b>	<b>2270</b>
<b>SUMMARY #8</b>															
<b>-L- RIGHT</b>															
101+65.00	126+74.09	3262	115			3147	2584	115	2469	2954	0		308		308
<b>-Y13-</b>															
12+30.73	14+93.88	710				710	288		288	331	0		379		379
<b>-Y14-</b>															
10+40.00	11+29.03	62				62	22		22	25	0		37		37
<b>-Y15-</b>															
10+45.00	12+13.87	1129				1129	1		1	1	0		1128		1128
<b>TOTAL SUMMARY #8</b>		<b>5163</b>	<b>115</b>			<b>5048</b>	<b>2895</b>	<b>115</b>	<b>2780</b>	<b>3311</b>	<b>0</b>		<b>1852</b>		<b>1852</b>

Earthwork quantities are calculated by the Roadway Design Unit. These earthwork quantities are based in part on subsurface data provided by the Geotechnical Engineering Unit.

EARTHWORK BALANCE SHEET

SHEET

4

OF

4

STATION	STATION	TOTAL EXCAV. (UNCL.)	ROCK EXCAV.	UNDERCUT EXCAV.	UNSUIT. EXCAV.	SUITABLE EXCAV.	TOTAL EMB.	ROCK EMB.	EARTH EMB.	EMB. + 15%	BORROW	ROCK	SUITABLE WASTE	UNSUIT. WASTE	TOTAL WASTE
SUBTOTAL SUMMARY #1, #2,#3,#4, #5,#6, #7 & 8		87439	115	480		87324	47716	115	47601	54858	25550		58131	480	58611
TOTALS		87439	115	480		87324	47716	115	47601	54858	25550		58131	480	58611
LOSS DUE TO CLEARING & GRUBBING		-1250				-1250							-1250		-1250
ADDITIONAL UNDERCUT				920			920		920	1058	1058			920	920
WASTE TO REPLACE BORROW											-26608		-26608		-26608
GRAND TOTALS		86189	115	1400		86074	48636	115	48521	55916	0		30273	1400	31673
SAY		87000													

Estimated Undercut (Shallow) = 4000 CY

Estimated Shoulder Borrow = 4200 CY

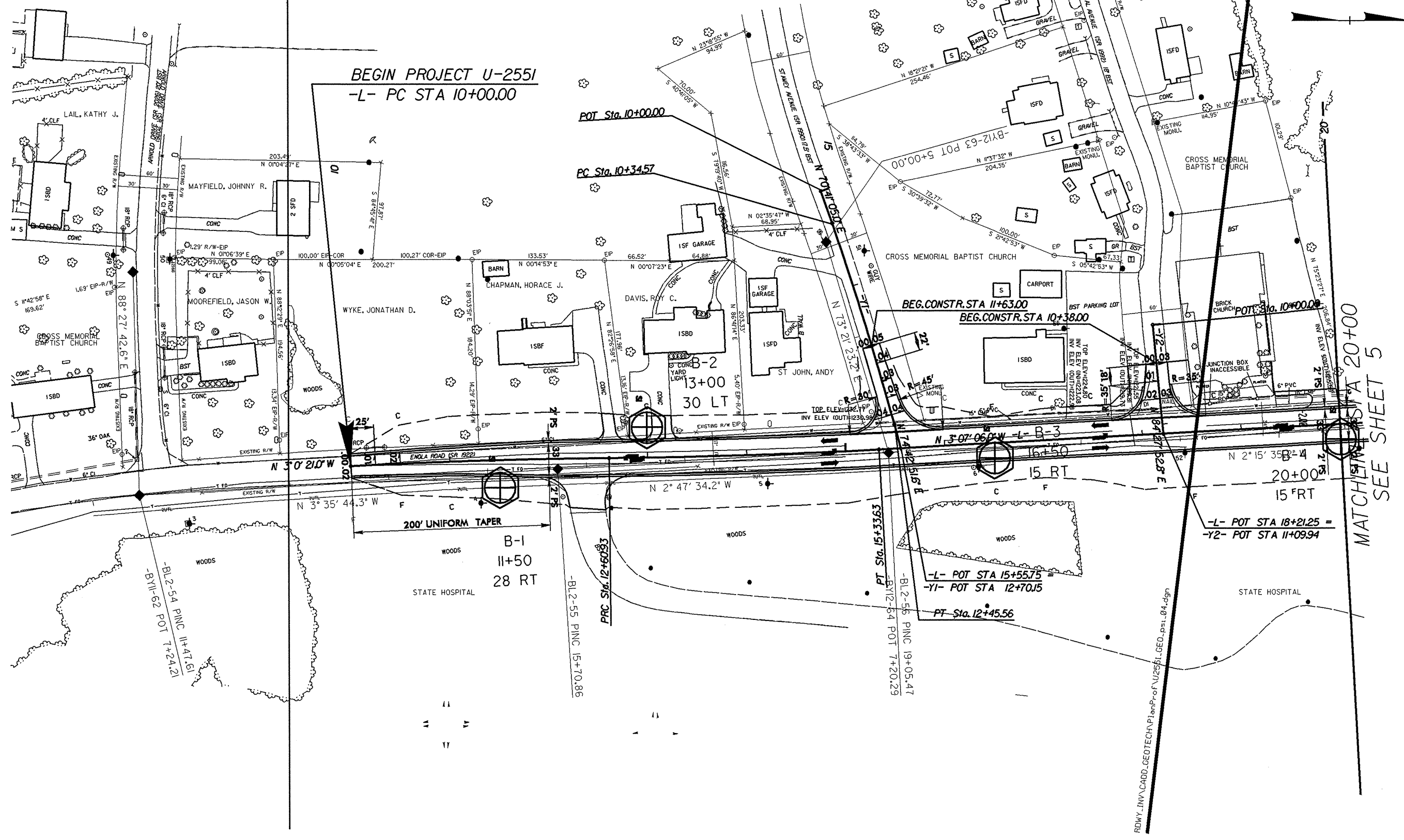
Estimated DDE = 1537 CY

Earthwork quantities are calculated by the Roadway Design Unit. These earthwork quantities are based in part on the subsurface data provided by the Geotechnical Engineering Unit.

5/14/99

PROJECT REFERENCE NO. <b>U-2551</b>		SHEET NO. <b>4/23</b>	
RW SHEET NO.			
ROADWAY DESIGN ENGINEER		HYDRAULICS ENGINEER	
<b>INCOMPLETE PLANS</b> DO NOT USE FOR P/W ACQUISITION <b>PRELIMINARY PLANS</b> DO NOT USE FOR CONSTRUCTION			

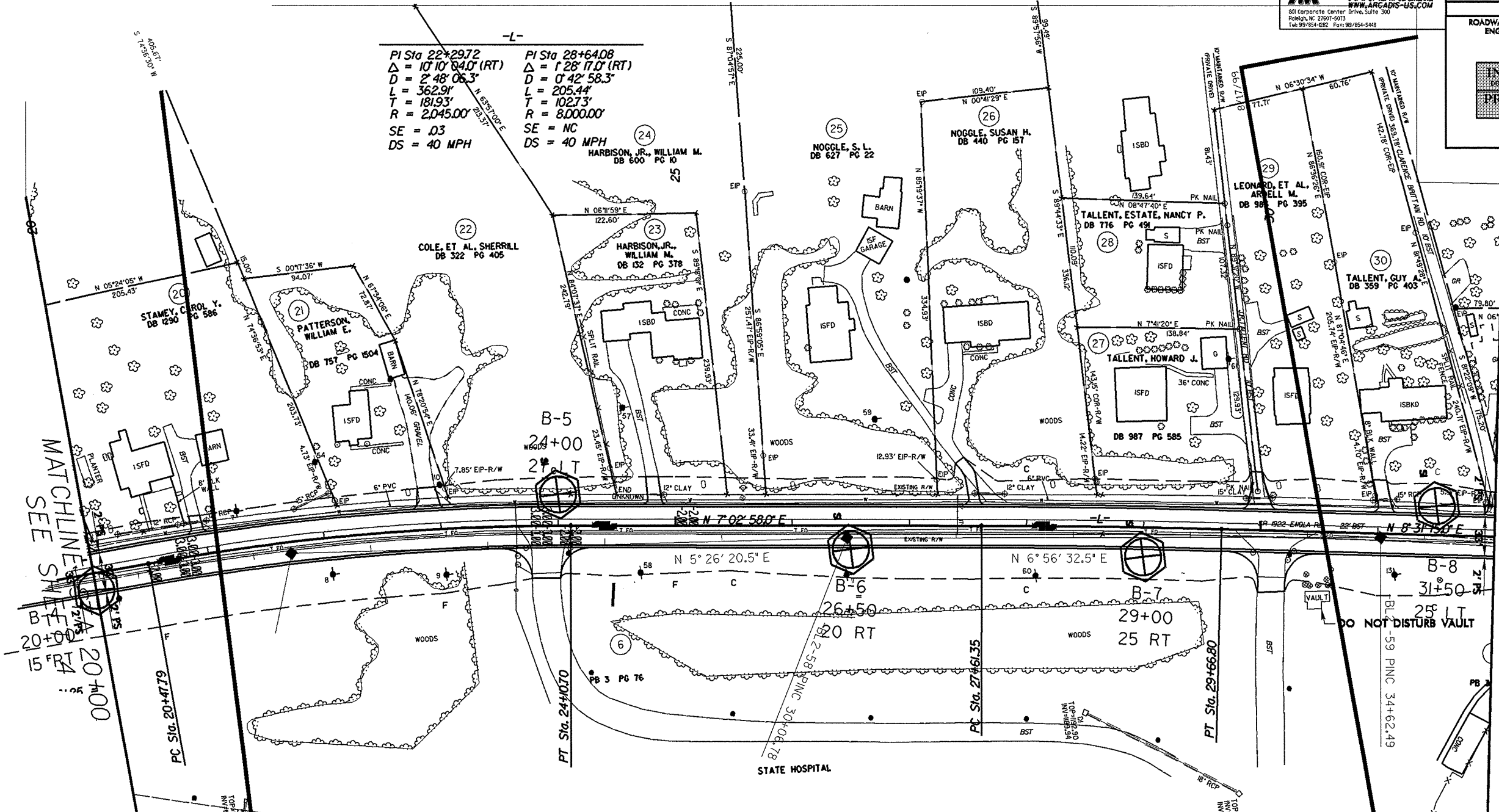
-L-		-YI-	
PI Sta 11+30.48	PI Sta 13+97.30	PI Sta 11+40.11	
$\Delta = 2' 29' 30.0''$ (RT)	$\Delta = 2' 36' 15.0''$ (LT)	$\Delta = 4' 01' 46.6''$ (RT)	
$D = 0' 57' 17.7''$	$D = 0' 57' 17.7''$	$D = 1' 54' 35.5''$	
$L = 260.93'$	$L = 272.71'$	$L = 210.99'$	
$T = 130.48'$	$T = 136.38'$	$T = 105.54'$	
$R = 6,000.00'$	$R = 6,000.00'$	$R = 3,000.00'$	
SE = NC	SE = NC	SE = SEE PLANS	
DS = 40 MPH	DS = 40 MPH	DS = 40 MPH	



MATCHLINE STA 20+00  
SEE SHEET 5

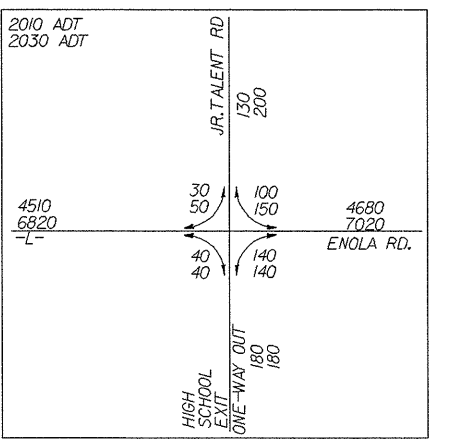
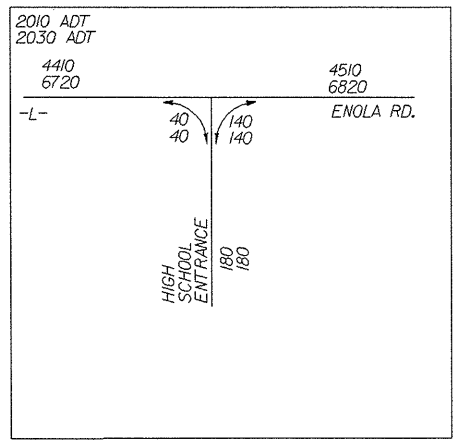
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PROJECT REFERENCE NO. U-2551	SHEET NO. 5/23
RW SHEET NO.	
ROADWAY DESIGN ENGINEER	HYDRAULICS ENGINEER
<b>INCOMPLETE PLANS</b> DO NOT USE FOR CONSTRUCTION <b>PRELIMINARY PLANS</b> DO NOT USE FOR CONSTRUCTION	

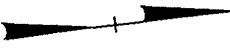


$PI\ Sta\ 22+29.72$   
 $\Delta = 10^\circ 10' 04.0'' (RT)$   
 $D = 2' 48' 06.3''$   
 $L = 362.91'$   
 $T = 181.93'$   
 $R = 2,045.00'$   
 $SE = .03$   
 $DS = 40\ MPH$

$PI\ Sta\ 28+64.08$   
 $\Delta = 1^\circ 28' 17.0'' (RT)$   
 $D = 0' 42' 58.3''$   
 $L = 205.44'$   
 $T = 102.73'$   
 $R = 8,000.00'$   
 $SE = NC$   
 $DS = 40\ MPH$



MATCHLINE STA 32+00  
SEE SHEET 6



RDWY\_INV\CADD\_GEO\TECH\Plan\Prof\U2551\_GEO.p01\_05.dgn

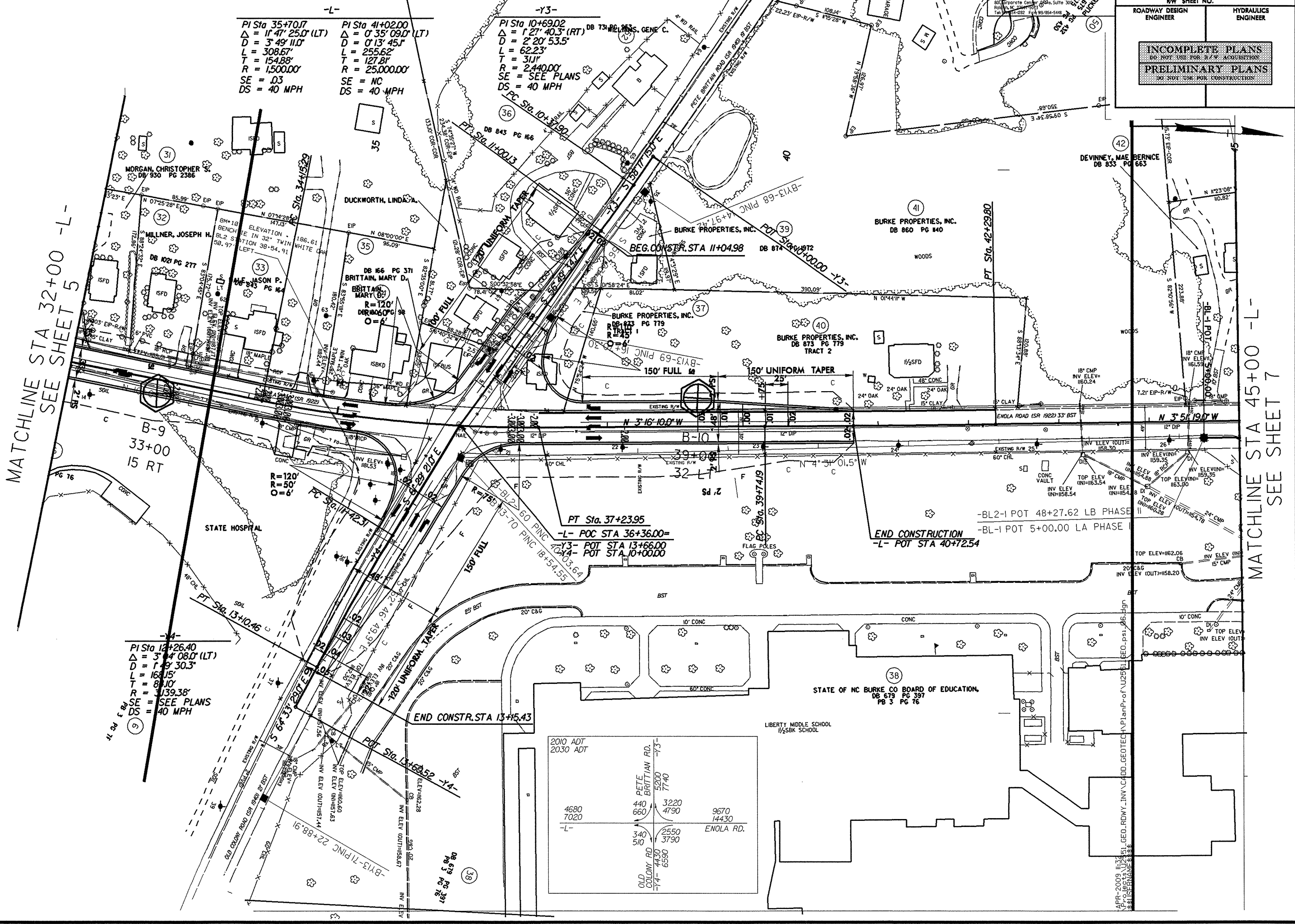


PROJECT REFERENCE NO. U-2551	SHEET NO. 6/23
R/W SHEET NO.	HYDRAULICS ENGINEER
ROADWAY DESIGN ENGINEER	

**INCOMPLETE PLANS**  
DO NOT USE FOR ACQUISITION  
**PRELIMINARY PLANS**  
NO SITE USE FOR CONSTRUCTION

MATCHLINE STA 32+00 -L-  
SEE SHEET 5

MATCHLINE STA 45+00 -L-  
SEE SHEET 7



-L-

PI Sta 35+70.7 Δ = 1° 47' 25.0" (LT) D = 3' 49' 11.0" L = 308.67' T = 154.88' R = 1500.00' SE = .03 DS = 40 MPH	PI Sta 41+02.00 Δ = 0° 35' 09.0" (LT) D = 0' 13' 45.1" L = 255.62' T = 127.81' R = 2500.00' SE = NC DS = 40 MPH
--	--

-Y3-

PI Sta 10+69.02 Δ = 1° 27' 40.3" (RT) D = 2' 20' 53.5" L = 62.23' T = 31.1' R = 2440.00' SE = SEE PLANS DS = 40 MPH	DB 73 PG 165 GENE C.
--	----------------------

-L-

PI Sta 12+26.40 Δ = 3° 04' 08.0" (LT) D = 1' 49' 30.3" L = 163.15' T = 81.10' R = 1339.38' SE = SEE PLANS DS = 40 MPH	DB 679 PG 307
--	---------------

2010 ADT 2030 ADT	PETE BRITIAN RD. 440 660	3220 4790	9670 14430
4680 7020	340 510	2550 3790	ENOLA RD.

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5/14/99

13 APR 2009 10:30 AM C:\GEO\TECH\PlanP\of\U2551\_GEO.p01\_08.dgn

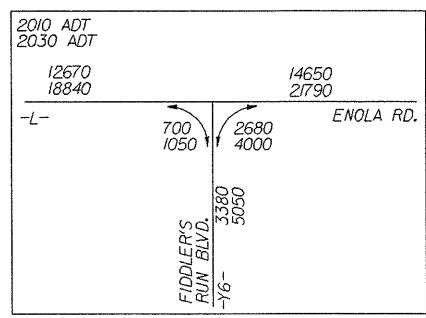
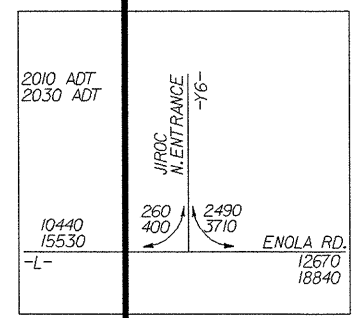
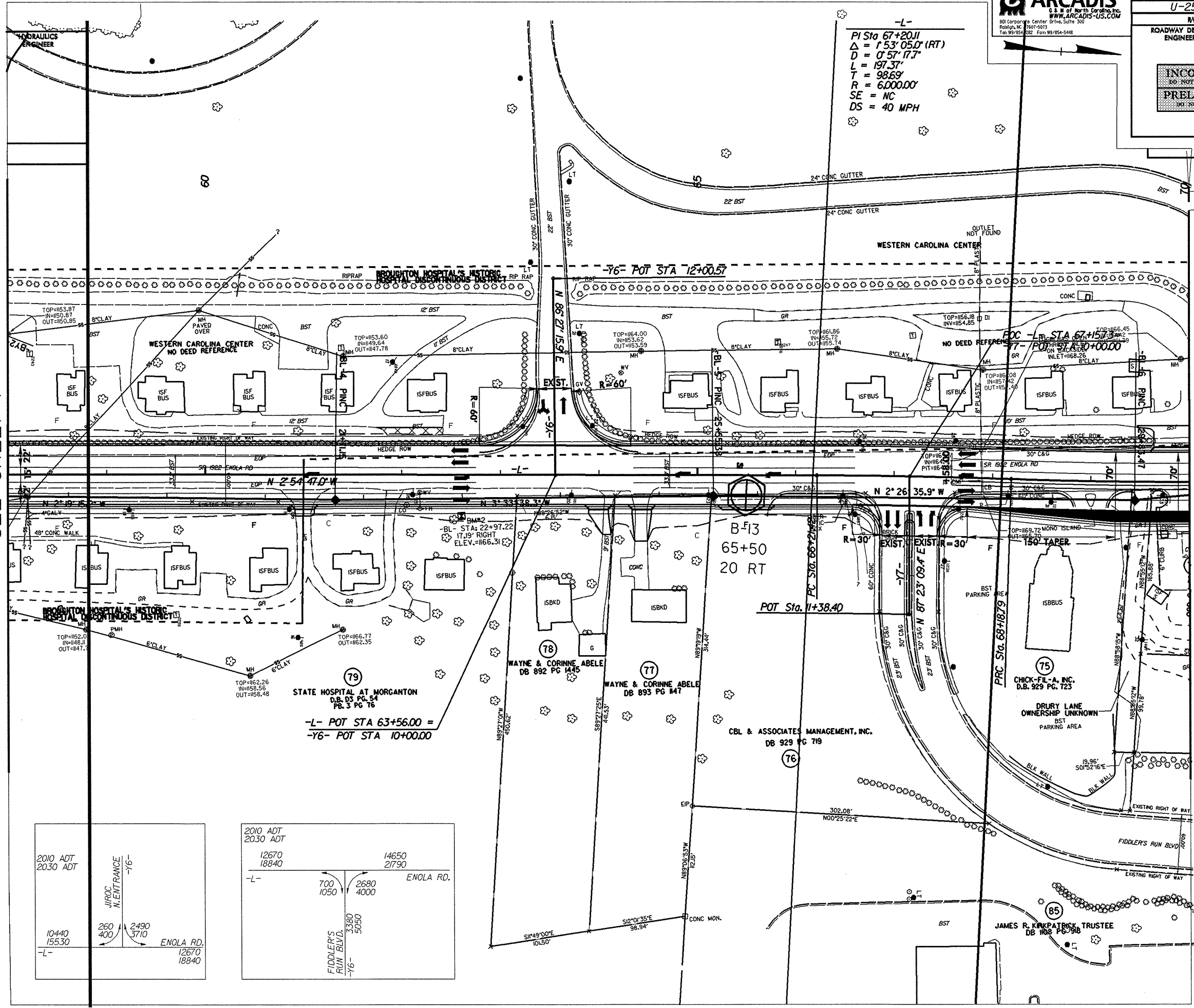


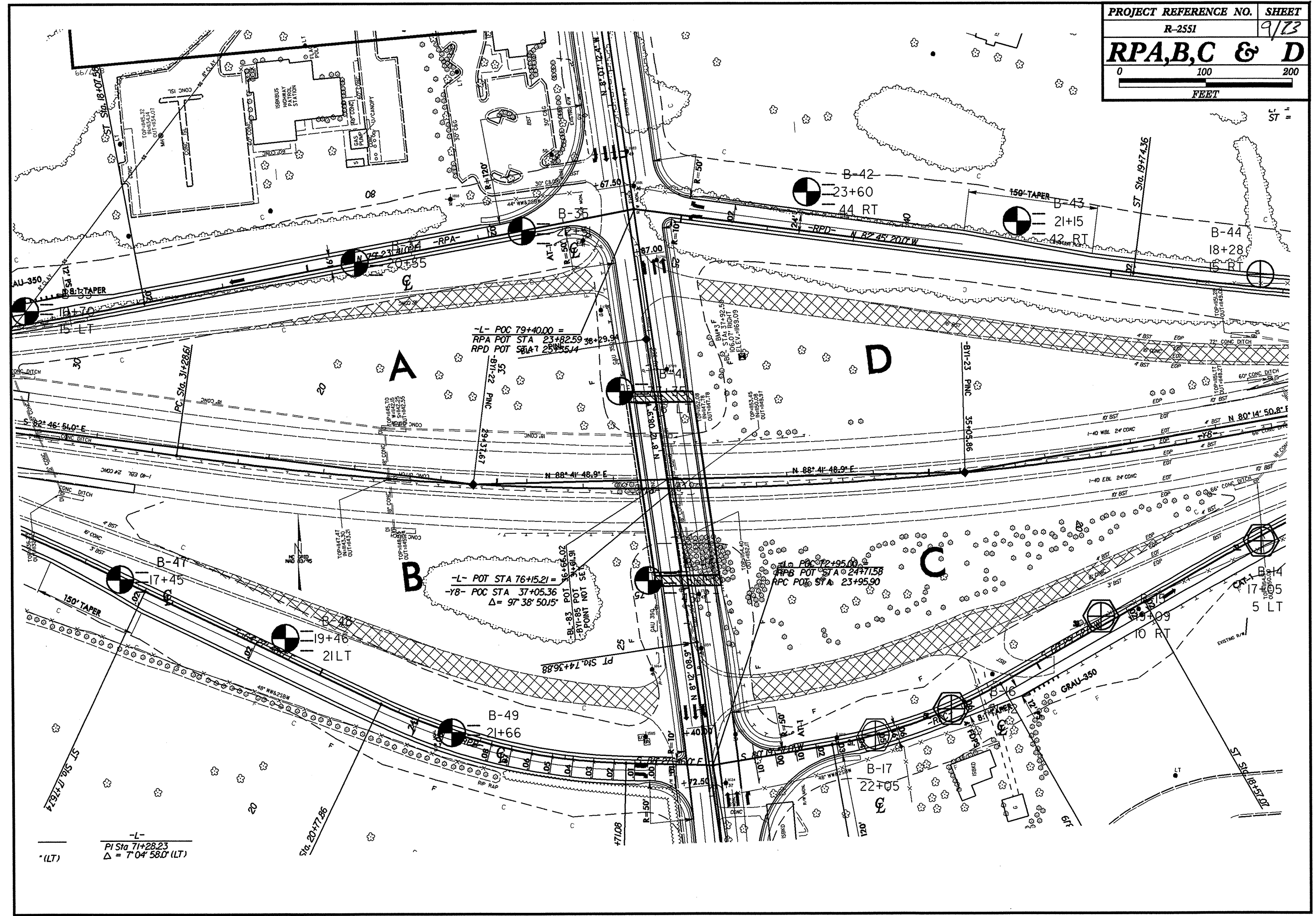
PROJECT REFERENCE NO. U-2551		SHEET NO. 8/73	
RW SHEET NO.			
ROADWAY DESIGN ENGINEER		HYDRAULICS ENGINEER	
<b>INCOMPLETE PLANS</b> DO NOT USE FOR 3/4" ACQUISITION <b>PRELIMINARY PLANS</b> DO NOT USE FOR CONSTRUCTION			

-L-  
 PI Sta 67+20.11  
 $\Delta = 1'53''05.0''$  (RT)  
 $D = 0'57''17.7''$   
 $L = 197.37'$   
 $T = 98.69'$   
 $R = 6000.00'$   
 $SE = NC$   
 $DS = 40$  MPH

MATCHLINE STA 58+00 -L-  
SEE SHEET 7

MATCHLINE STA 70+00 -L-  
SEE SHEET 9





-L- POC 79+40.00 =  
 RPA POT STA 23+82.59 38+29.94  
 RPD POT STA 23+55.14

-L- POT STA 76+15.21 =  
 -YB- POC STA 37+05.36  
 $\Delta = 97^\circ 38' 50.15''$

-L- POC STA 22+95.00 =  
 RPB POT STA 24+71.58  
 RPC POT STA 23+95.90

-L-  
 PI Sta 71+28.23  
 $\Delta = 7^\circ 04' 58.0''$  (LT)







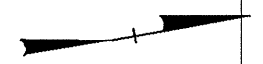
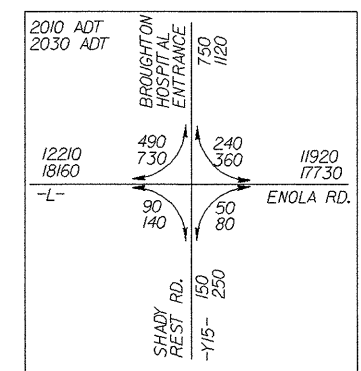
5/14/99



PROJECT REFERENCE NO.		SHEET NO.	
U-2551		12/73	
RW SHEET NO.			
ROADWAY DESIGN ENGINEER		HYDRAULICS ENGINEER	
<b>INCOMPLETE PLANS</b> DO NOT USE FOR A/C ACQUISITION <b>PRELIMINARY PLANS</b> DO NOT USE FOR CONSTRUCTION			

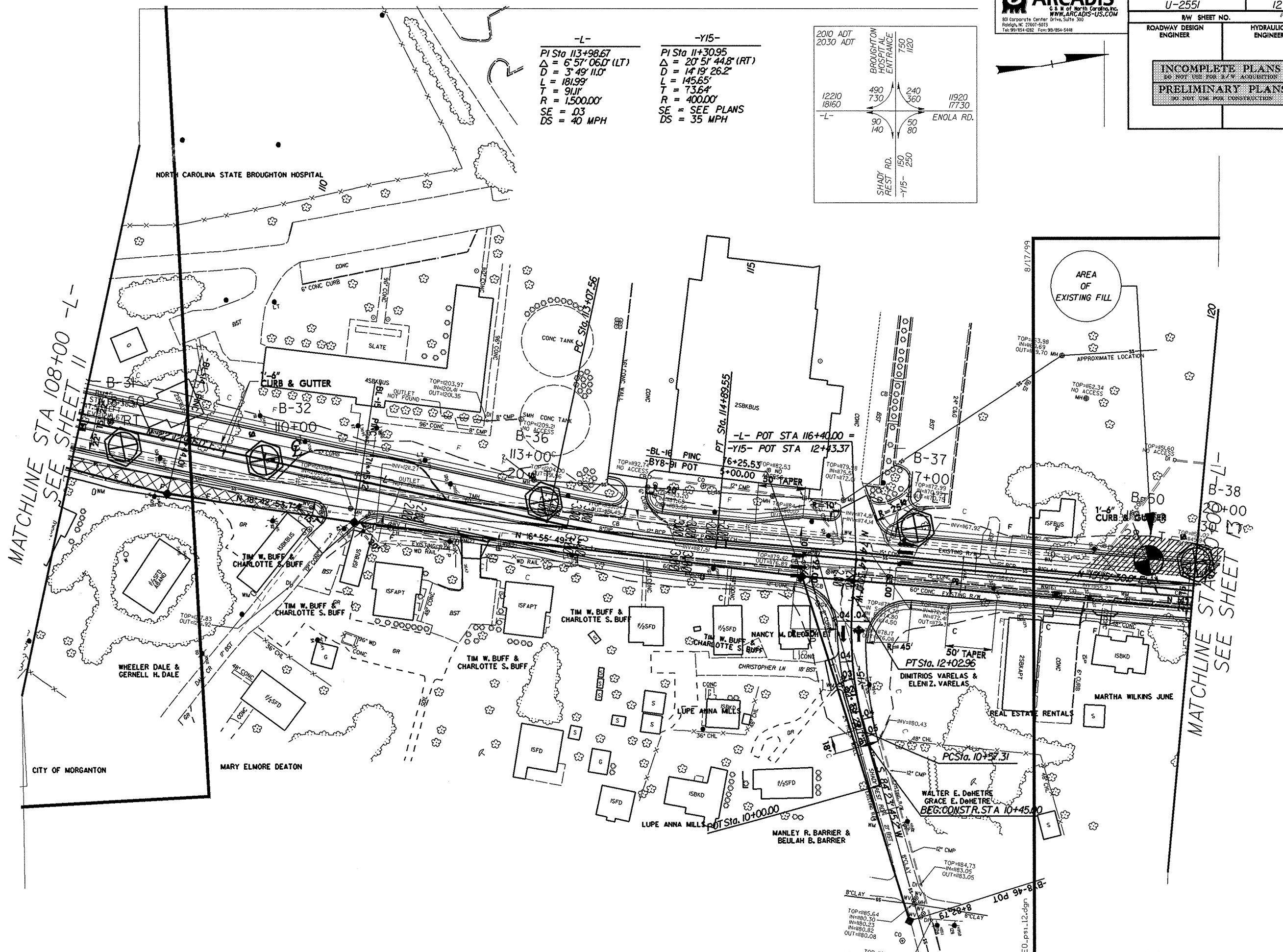
**-L-**  
 PI Sta 113+98.67  
 $\Delta = 6' 57'' 06.0''$  (LT)  
 $D = 3' 49'' 11.0''$   
 $L = 181.99'$   
 $T = 91.11'$   
 $R = 1,500.00'$   
 $SE = .03$   
 $DS = 40$  MPH

**-Y15-**  
 PI Sta 11+30.95  
 $\Delta = 20' 51'' 44.8''$  (RT)  
 $D = 14' 19'' 26.2''$   
 $L = 145.65'$   
 $T = 73.64'$   
 $R = 400.00'$   
 $SE = \text{SEE PLANS}$   
 $DS = 35$  MPH



MATCHLINE STA 108+00 -L-  
SEE SHEET 11

MATCHLINE STA 130+00 -L-  
SEE SHEET 13

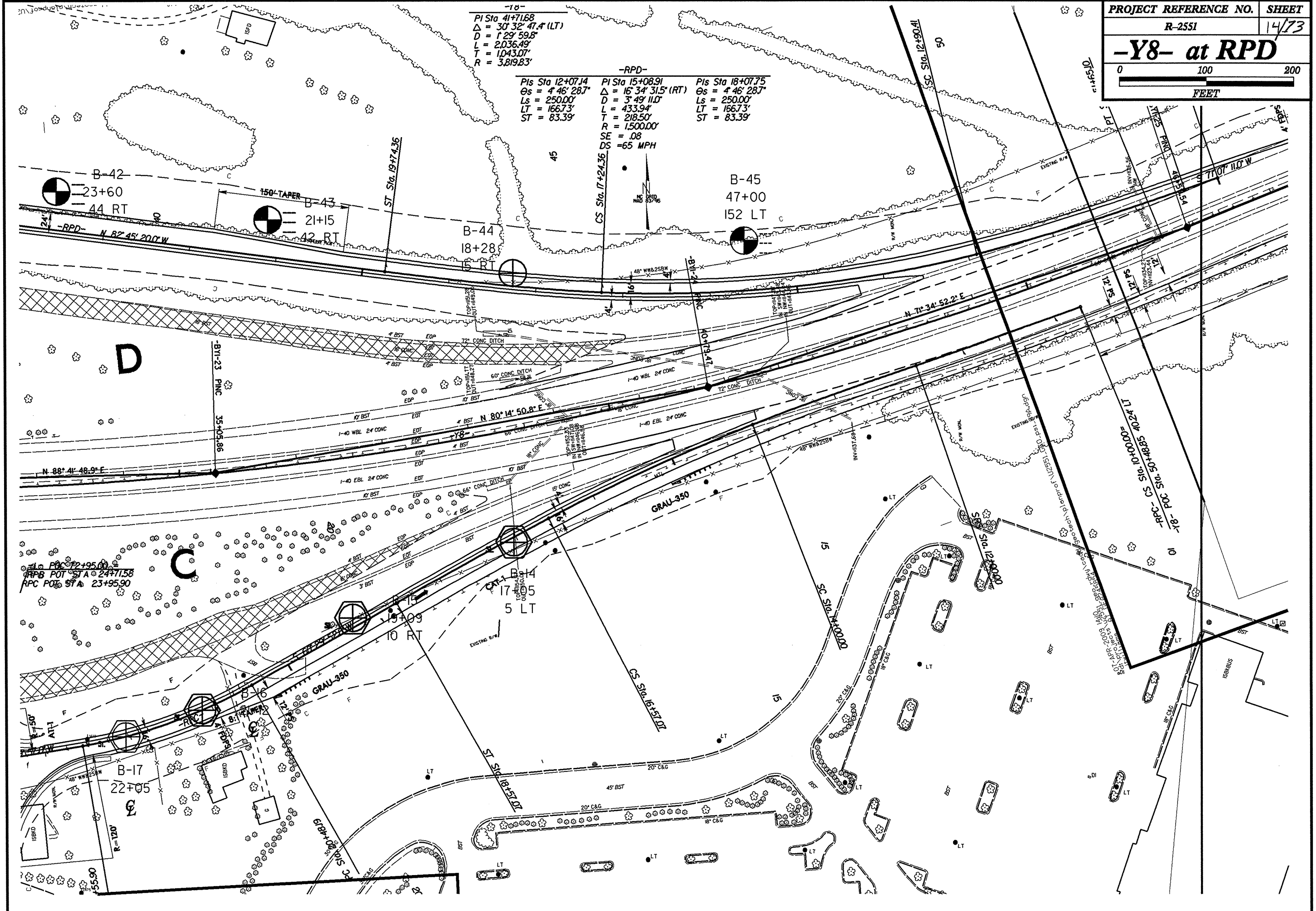


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-18-

<p>PI Sta 41+71.68  <math>\Delta = 30^\circ 32' 41.4" (LT)</math>  <math>D = 129' 59.5"</math>  <math>L = 2,036.49'</math>  <math>T = 1,043.07'</math>  <math>R = 3,819.83'</math></p>	<p style="text-align: center;">-RPD-</p> <p>PIs Sta 12+07.14    PI Sta 15+08.91    PIs Sta 18+07.75  <math>\Theta_s = 4^\circ 46' 28.7"</math>    <math>\Delta = 16^\circ 34' 31.5" (RT)</math>    <math>\Theta_s = 4^\circ 46' 28.7"</math>  <math>L_s = 250.00'</math>    <math>D = 3' 49' 11.0"</math>    <math>L_s = 250.00'</math>  <math>LT = 166.73'</math>    <math>T = 433.94'</math>    <math>LT = 166.73'</math>  <math>ST = 83.39'</math>    <math>R = 218.50'</math>    <math>ST = 83.39'</math>  <math>R = 1,500.00'</math>  <math>SE = .08</math>  <math>DS = 65 \text{ MPH}</math></p>
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POC STA 22+95.00  
 RPB POT STA 24+71.58  
 RPC POT STA 23+95.90

5/14/99

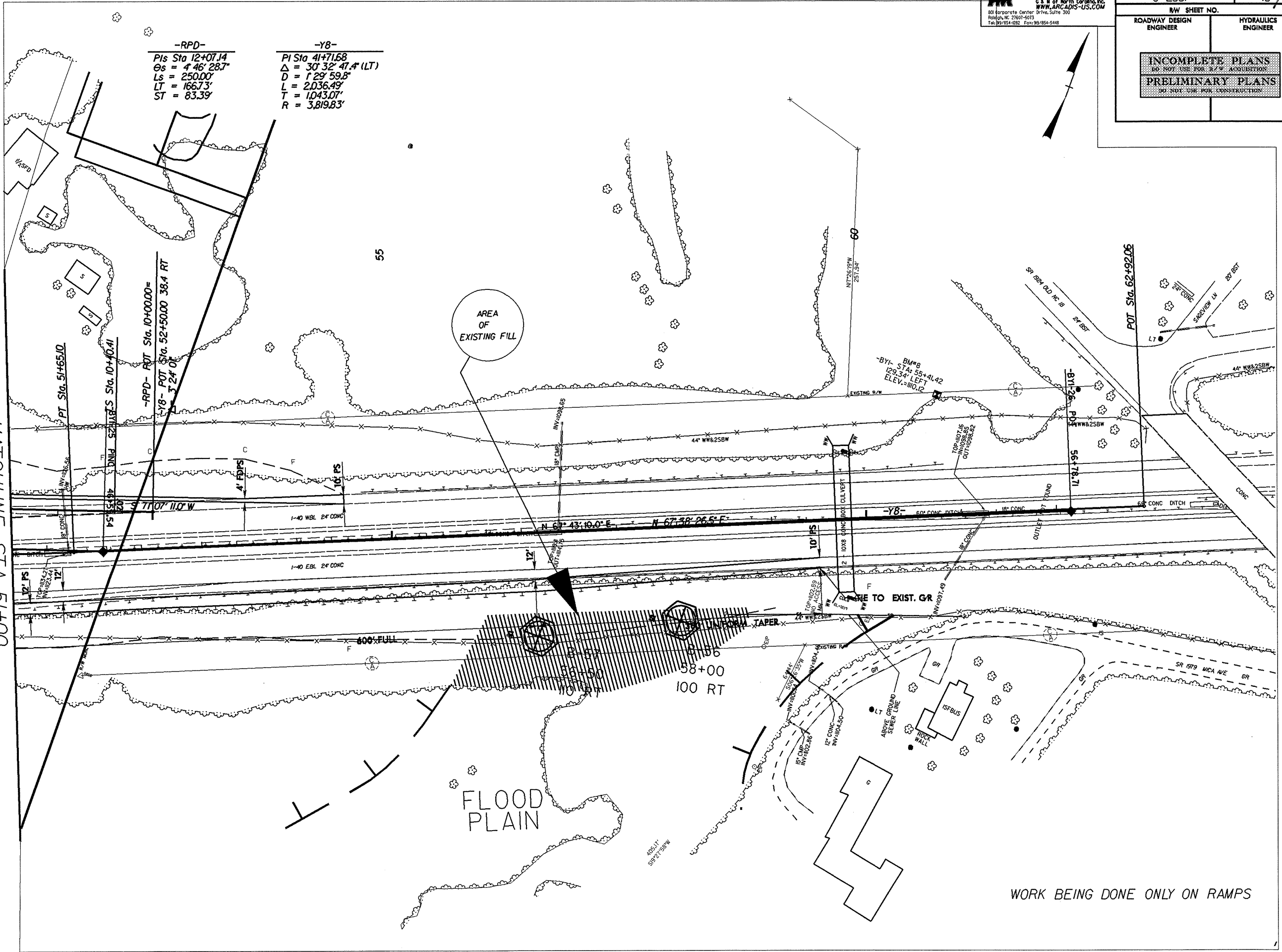


PROJECT REFERENCE NO. U-2551	SHEET NO. 15 / 73
RAW SHEET NO.	
ROADWAY DESIGN ENGINEER	HYDRAULICS ENGINEER
<b>INCOMPLETE PLANS</b> DO NOT USE FOR ACQUISITION <b>PRELIMINARY PLANS</b> DO NOT USE FOR CONSTRUCTION	

**-RPD-**  
 PIs Sta 12+07.14  
 Os = 4' 46" 28.7"  
 Ls = 250.00'  
 Lt = 166.73'  
 ST = 83.39'

**-Y8-**  
 PI Sta 41+71.68  
 $\Delta = 30' 32' 41.4" (LT)$   
 D = 1' 29' 59.8"  
 L = 2,036.49'  
 T = 1,043.07'  
 R = 3,819.83'

MATCHLINE STA 51+00  
SEE SHEET 9

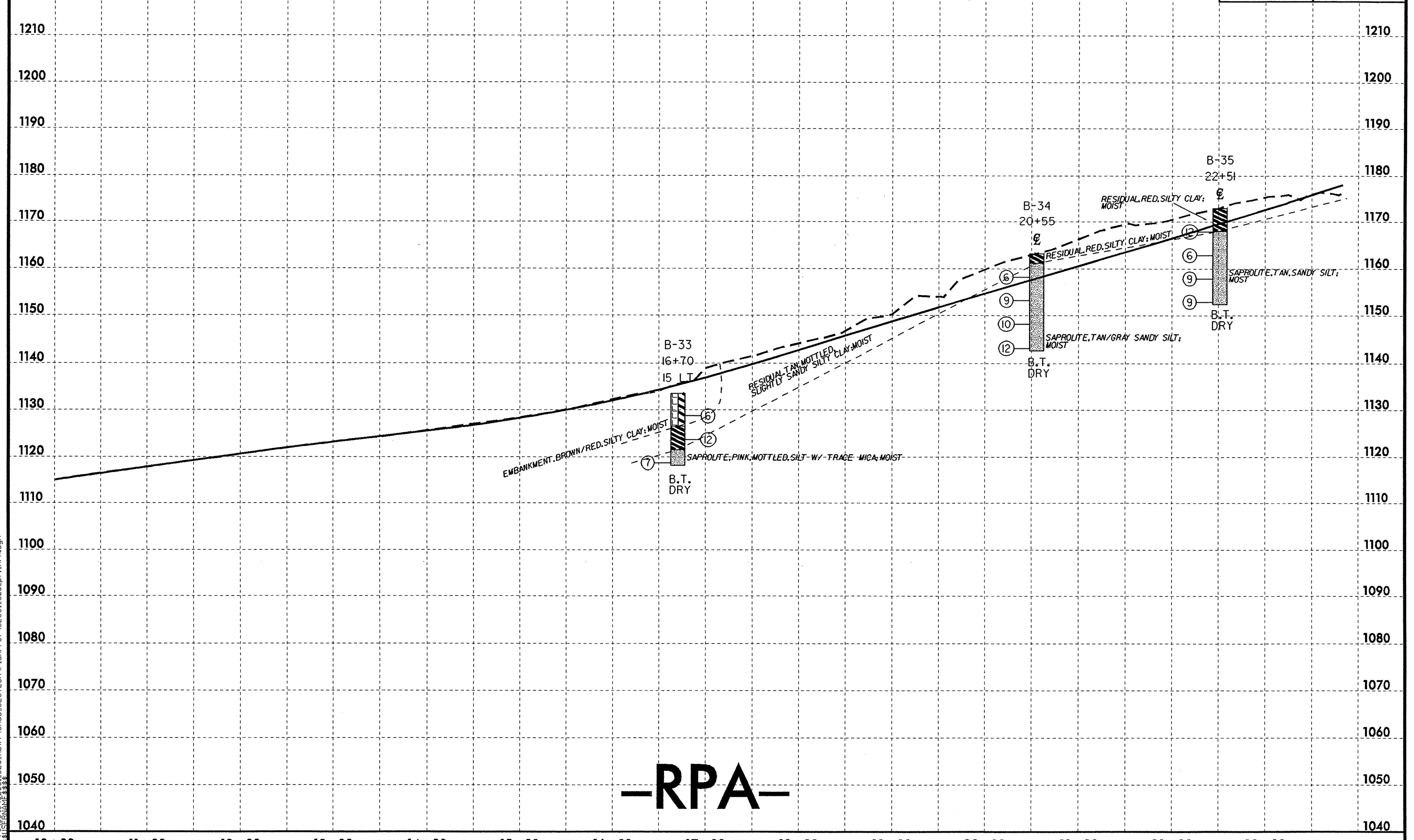


WORK BEING DONE ONLY ON RAMPS

20 APR 2009 08:56 GEOTECH\Plan\pof\U2551\_GEO.psi.15.dgn

5/14/99

PROJECT REFERENCE NO. <b>U-2551</b>	SHEET NO. <b>16</b>
ROADWAY DESIGN ENGINEER	HYDRAULICS ENGINEER
<b>INCOMPLETE PLANS</b> DO NOT USE FOR ACQUISITION <b>PRELIMINARY PLANS</b> DO NOT USE FOR CONSTRUCTION	
	1230 1220

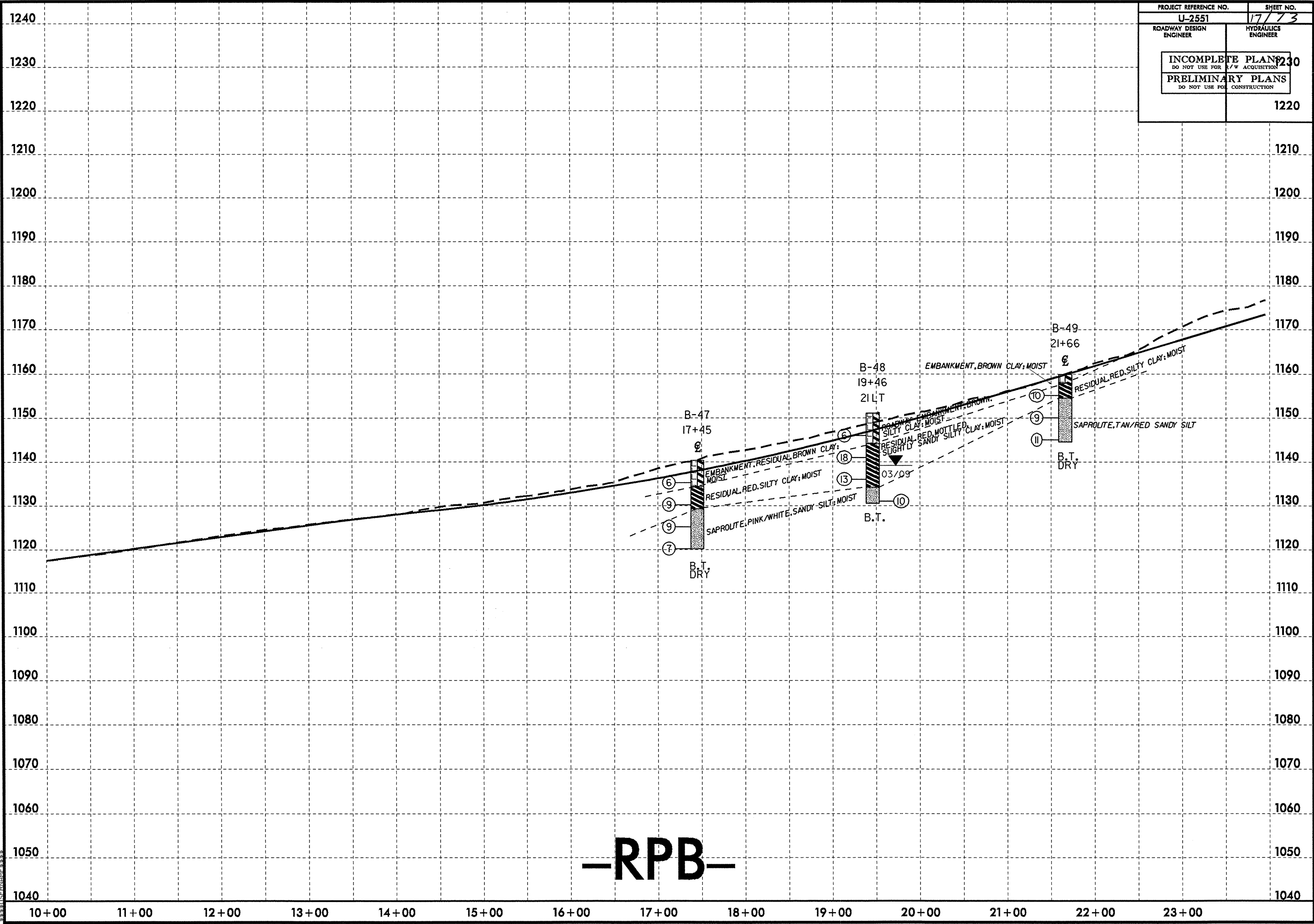


**-RPA-**

16-JUN-2009 16:14  
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5/14/99

PROJECT REFERENCE NO. U-2551	SHEET NO. 1773
ROADWAY DESIGN ENGINEER	HYDRAULICS ENGINEER
<b>INCOMPLETE PLANS</b> DO NOT USE FOR ACQUISITION	
<b>PRELIMINARY PLANS</b> DO NOT USE FOR CONSTRUCTION	



-RPB-

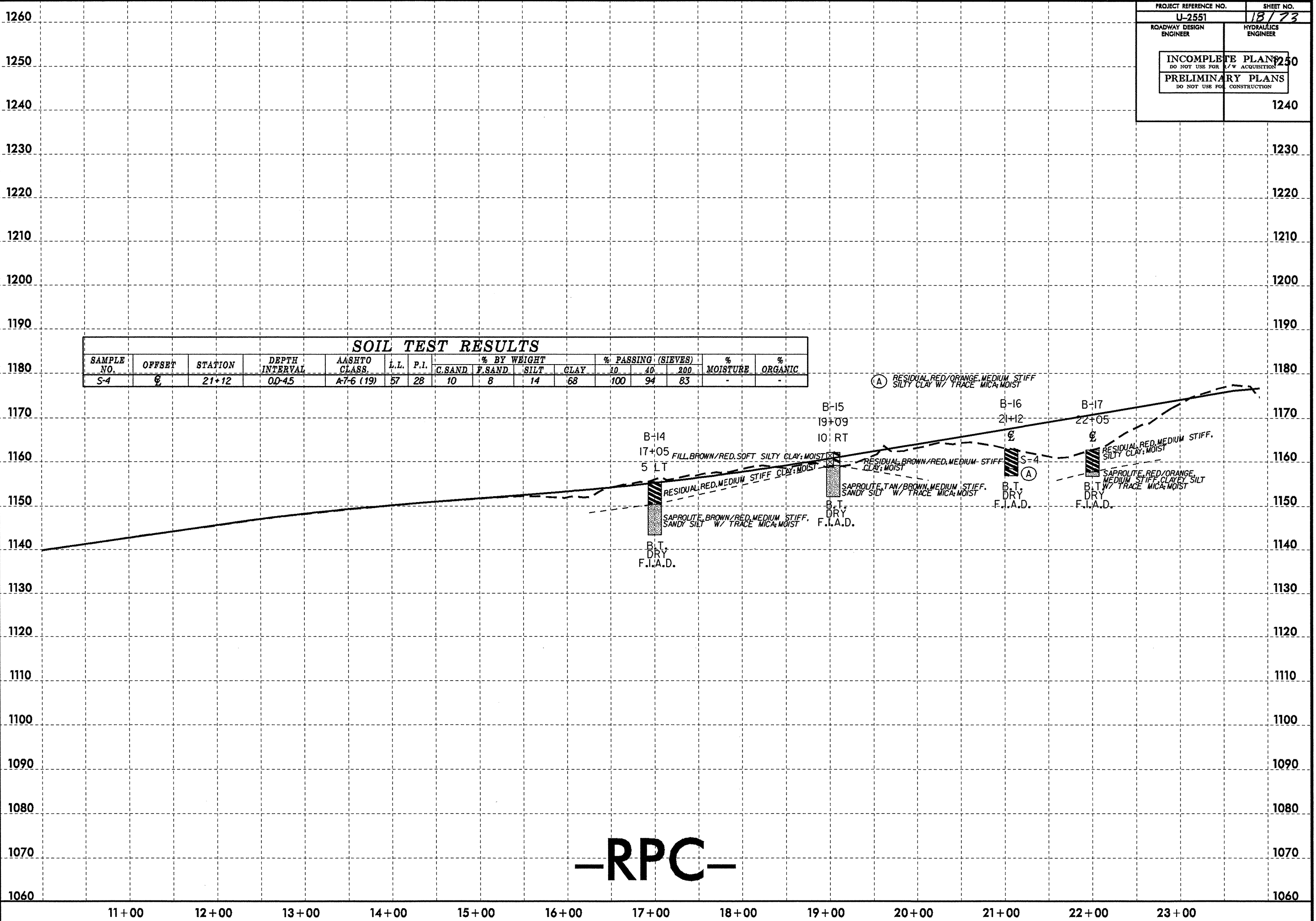
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5/14/99

09-APR-2009 07:51 GEO.RDWAY\CADD.GEOTECH\Plan\Pro\U2551.GEO\_pf1\_RPC.dgn

PROJECT REFERENCE NO. U-2551		SHEET NO. 18/73	
ROADWAY DESIGN ENGINEER		HYDRAULICS ENGINEER	
INCOMPLETE PLAN DO NOT USE FOR ACQUISITION		1250	
PRELIMINARY PLANS DO NOT USE FOR CONSTRUCTION		1240	



SOIL TEST RESULTS															
SAMPLE NO.	OFFSET	STATION	DEPTH INTERVAL	AASHTO CLASS.	L.L.	P.I.	% BY WEIGHT				% PASSING (SIEVES)			% MOISTURE	% ORGANIC
							C.SAND	F.SAND	SILT	CLAY	10	40	200		
S-4	0	21+12	0.0-4.5	A-7-6 (19)	57	28	10	8	14	68	100	94	83	-	-

B-14  
17+05  
5 LT  
FILL BROWN/RED SOFT SILTY CLAY; MOIST  
RESIDUAL RED. MEDIUM STIFF CLAY; MOIST  
SAPROLITE BROWN/RED MEDIUM STIFF SANDY SILT W/ TRACE MICA; MOIST  
B.T. DRY F.I.A.D.

B-15  
19+09  
10 RT  
RESIDUAL BROWN/RED MEDIUM STIFF CLAY; MOIST  
SAPROLITE TAN/BROWN MEDIUM STIFF SANDY SILT W/ TRACE MICA; MOIST  
B.T. DRY F.I.A.D.

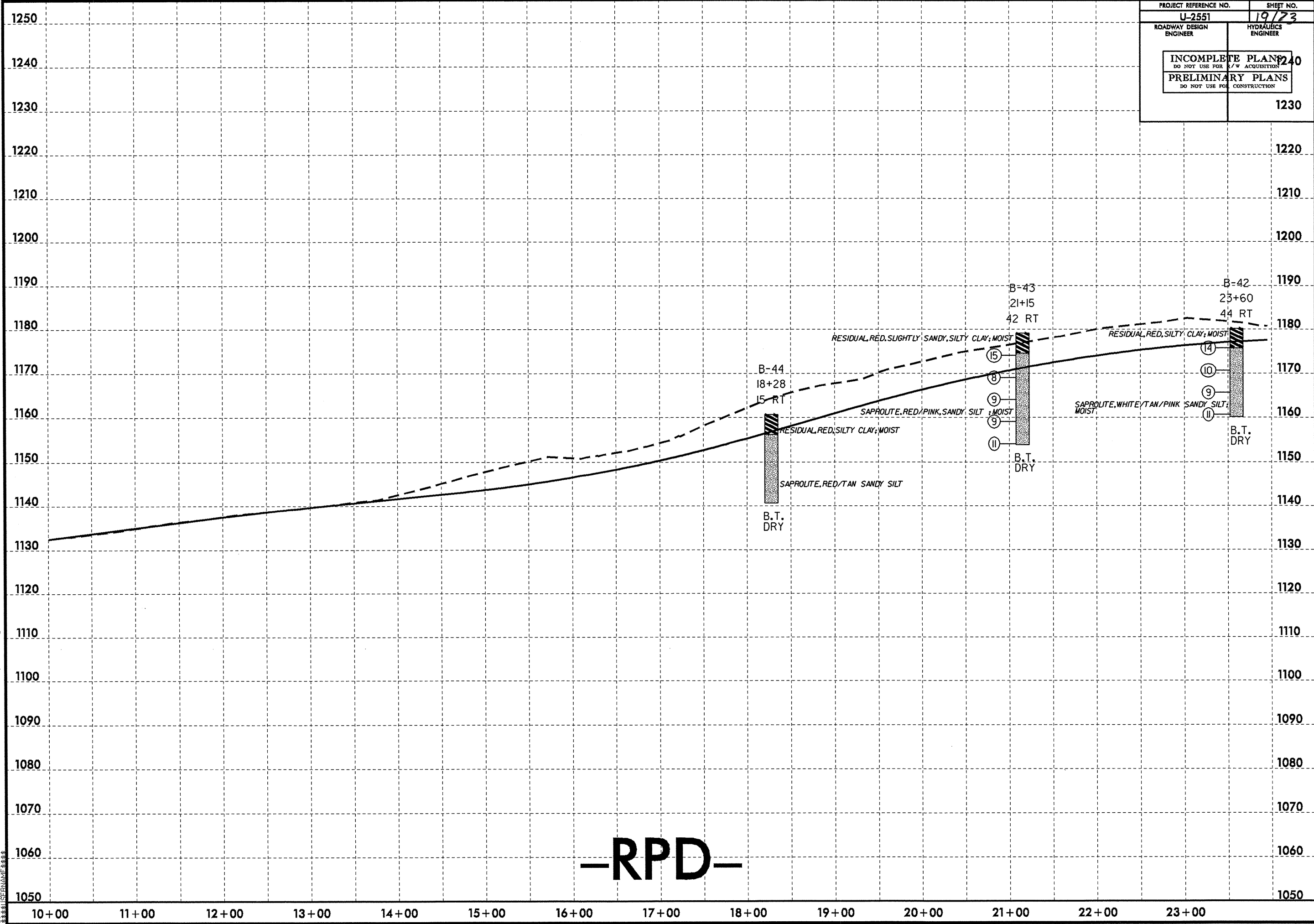
B-16  
21+12  
S-4  
RESIDUAL RED/ORANGE MEDIUM STIFF SILTY CLAY W/ TRACE MICA; MOIST  
B.T. DRY F.I.A.D.

B-17  
22+05  
RESIDUAL RED MEDIUM STIFF SILTY CLAY; MOIST  
SAPROLITE RED/ORANGE MEDIUM STIFF CLAY; SILT  
B.T. DRY W/ TRACE MICA; MOIST  
B.T. DRY F.I.A.D.

-RPC-

5/14/99

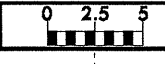
PROJECT REFERENCE NO. U-2551	SHEET NO. 19/23
ROADWAY DESIGN ENGINEER	HYDRAULICS ENGINEER
<b>INCOMPLETE PLANS</b> DO NOT USE FOR ACQUISITION	
<b>PRELIMINARY PLANS</b> DO NOT USE FOR CONSTRUCTION	
	1230



-RPD-

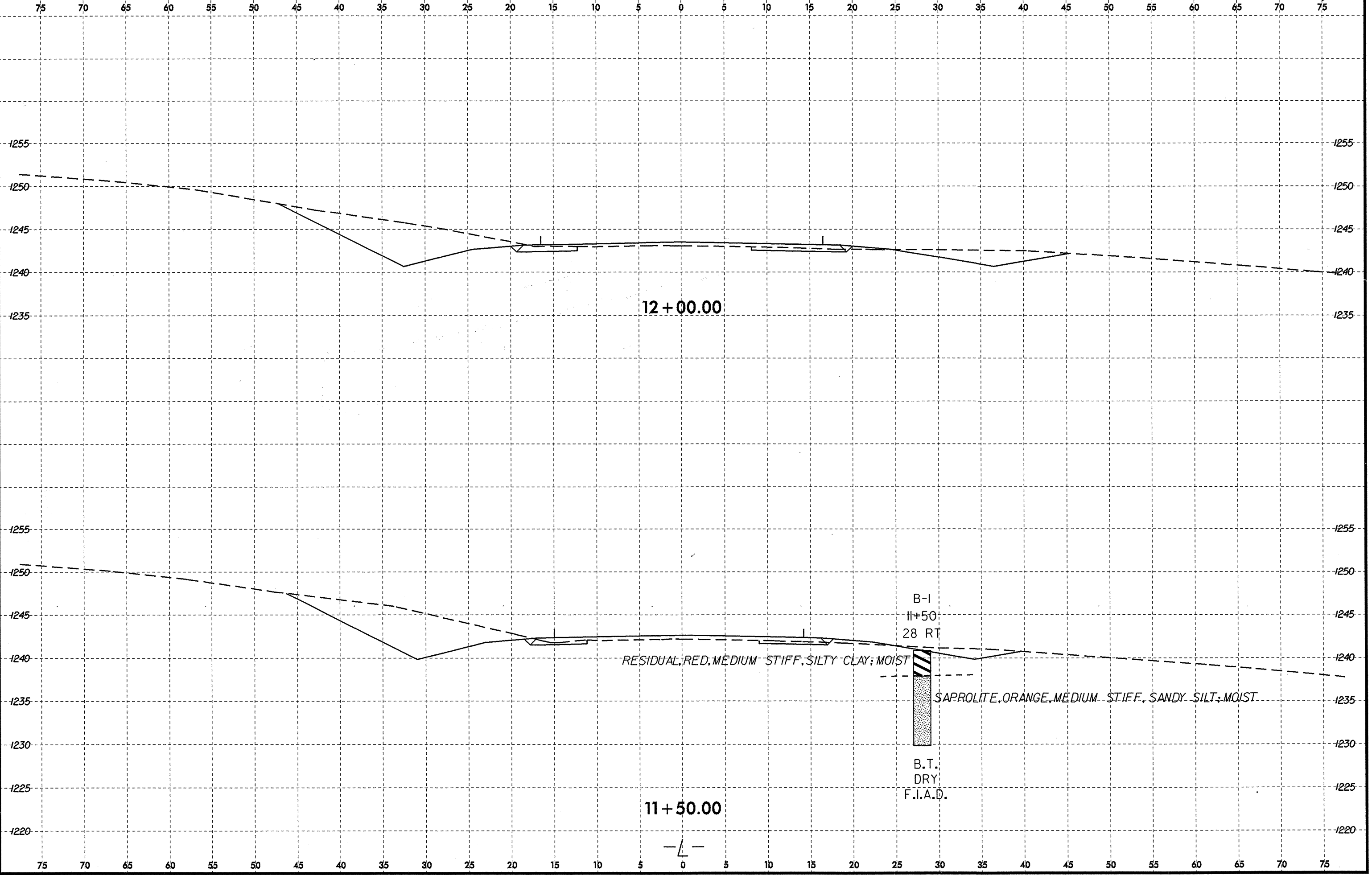
08-APR-2003 09:04 GEO\_RDWY\_CADD\_GEO TECH\171.mpr of U2551.GEO.pf...RPD.dgn

8/23/99



PROJ. REFERENCE NO.  
U-2551

SHEET NO.  
20/73



25-MAR-2009 09:19  
D:\PROJECTS\U2551\_GEO\RDWY\CADD\_GEDTECH\PlanProf\U2551\_GEO\_xst\_1.dgn  
\*\*\*USER NAME\*\*\*

12 + 00.00

11 + 50.00

B-1  
11+50  
28 FT

RESIDUAL RED, MEDIUM STIFF, SILTY CLAY; MOIST

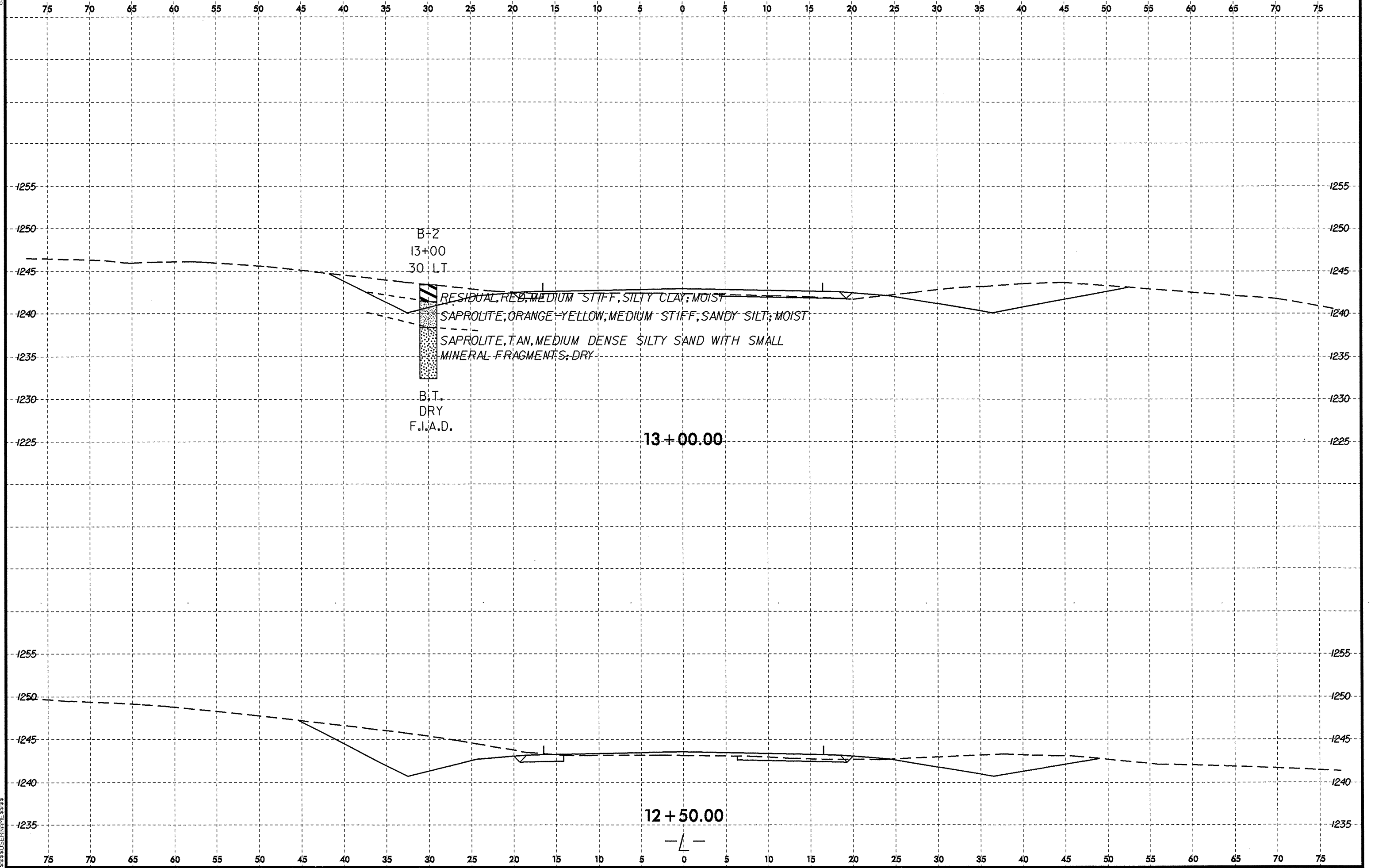
SAPROLITE, ORANGE, MEDIUM STIFF, SANDY SILT; MOIST

B.T.  
DRY  
F.I.A.D.

8/23/99



PROJ. REFERENCE NO. U-2551 SHEET NO. 21/73



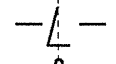
B+2  
13+00  
30 LT

RESIDUAL, RED, MEDIUM STIFF, SILTY CLAY; MOIST  
SAPROLITE, ORANGE-YELLOW; MEDIUM STIFF, SANDY SILT; MOIST  
SAPROLITE, TAN, MEDIUM DENSE SILTY SAND WITH SMALL  
MINERAL FRAGMENTS; DRY

B, T.  
DRY  
F.I.A.D.

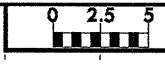
13 + 00.00

12 + 50.00

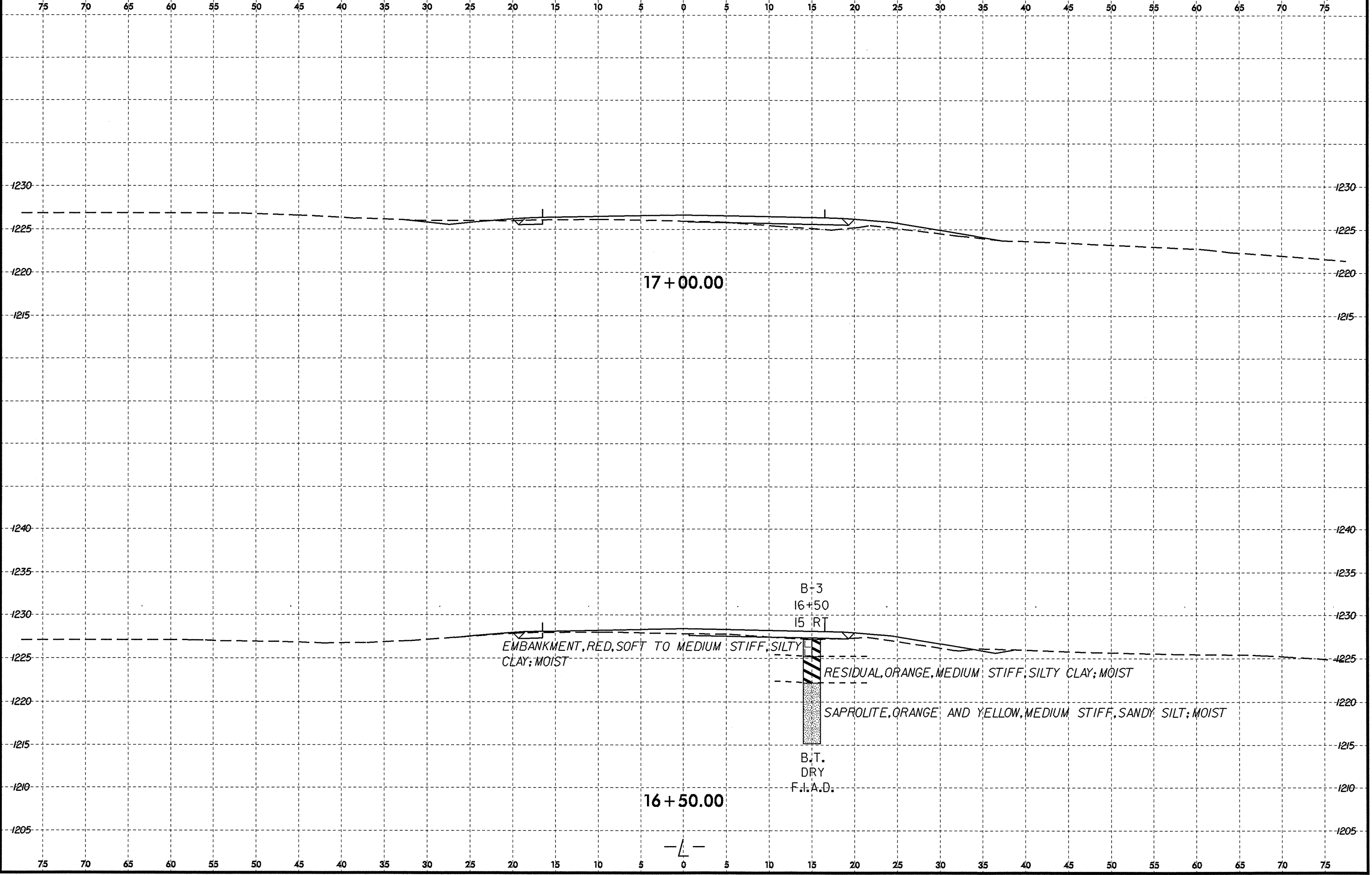


25-MAR-2009 09:19 D:\PROJECTS\2551\_GEO\RDWY\CADD\_GEO\TECH\PlanProf\U2551\_GEO\_xst\_1.dgn

8/23/99



PROJ. REFERENCE NO. U-2551 SHEET NO. 22/23



17 + 00.00

16 + 50.00

EMBANKMENT, RED, SOFT TO MEDIUM STIFF, SILTY CLAY; MOIST

B-3  
16+50  
15 RT

RESIDUAL, ORANGE, MEDIUM STIFF, SILTY CLAY; MOIST

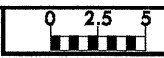
SAPROLITE, ORANGE AND YELLOW, MEDIUM STIFF, SANDY SILT; MOIST

B.T.  
DRY  
F.I.A.D.

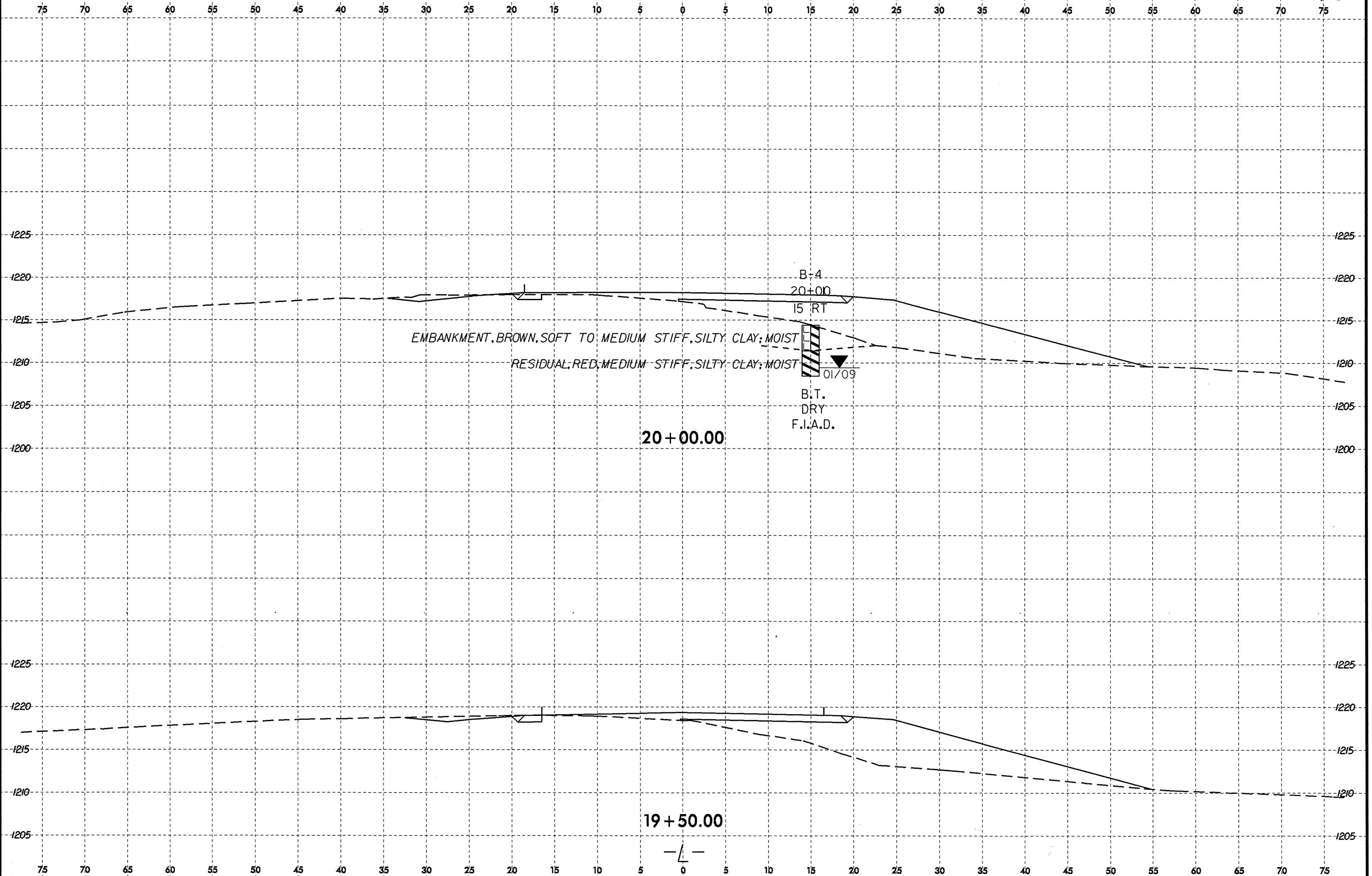


01-APR-2009 12:16  
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\*\*\*\$USER\$\*\*\*

8/23/99



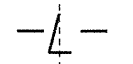
PROJ. REFERENCE NO.	SHEET NO.
U-2551	23/73



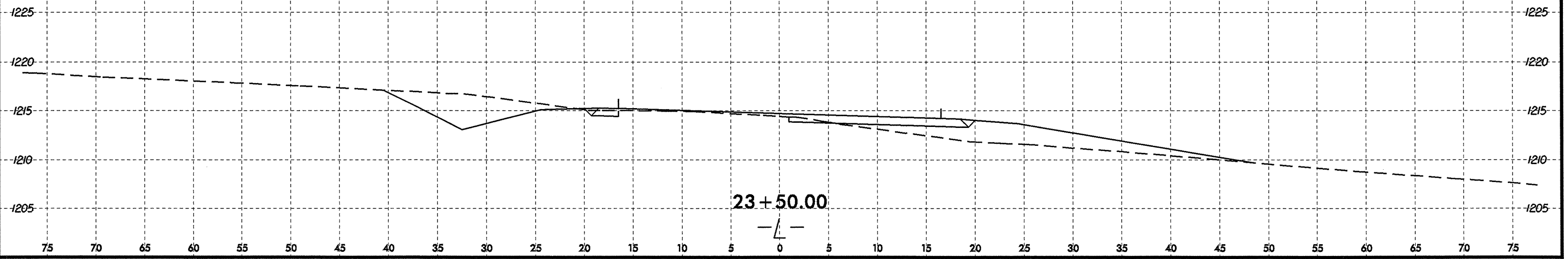
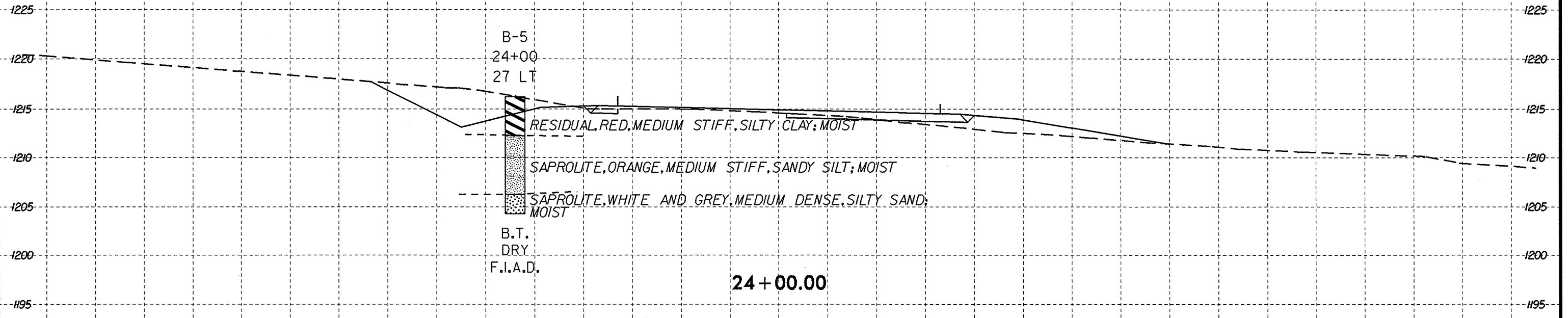
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\$\$\$\$USERNAME\$\$\$\$

20 + 00.00

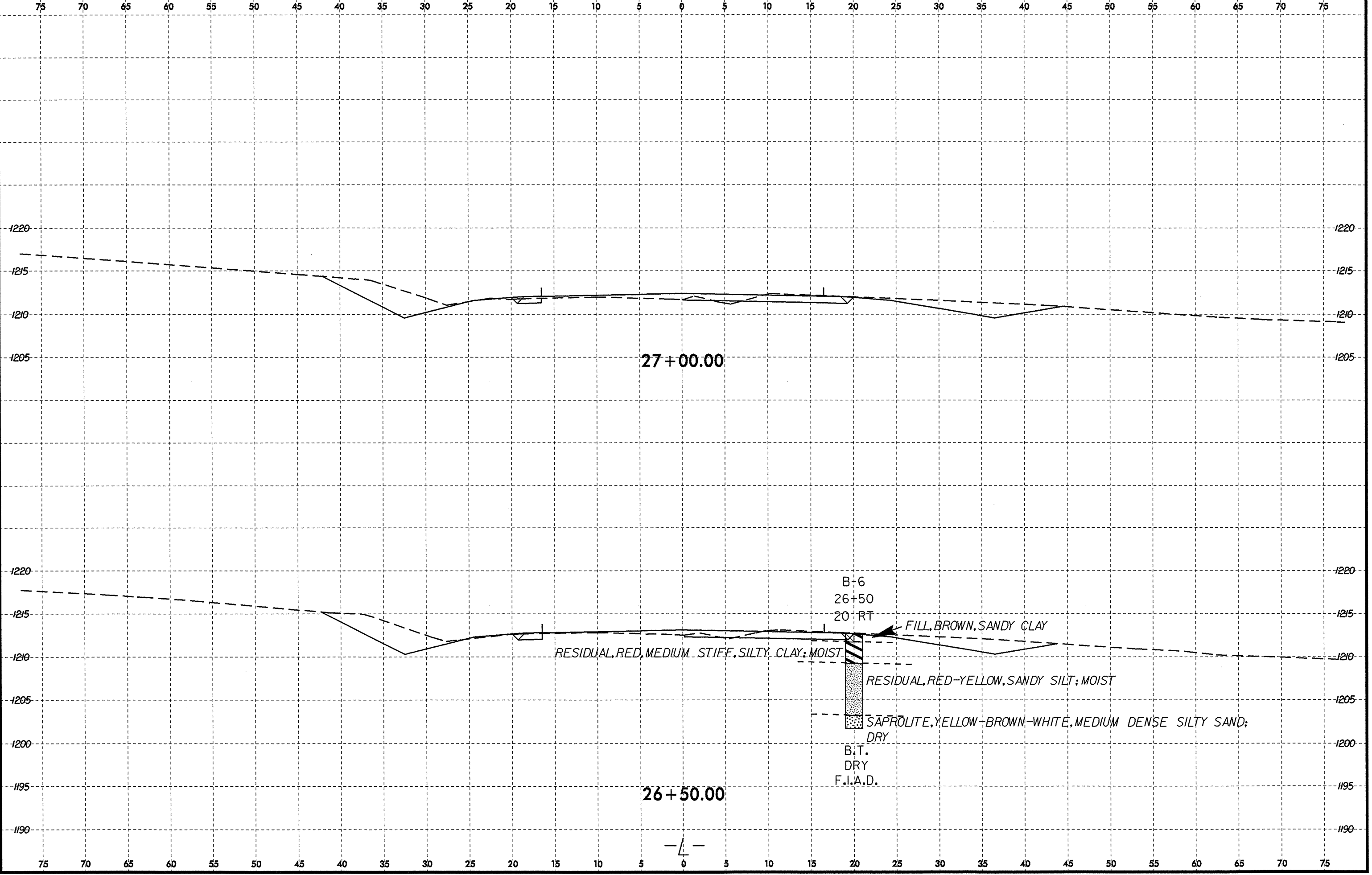
19 + 50.00



75 70 65 60 55 50 45 40 35 30 25 20 15 10 5 0 5 10 15 20 25 30 35 40 45 50 55 60 65 70 75

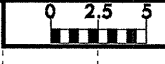


8/23/99  
09-APR-2009 08:39  
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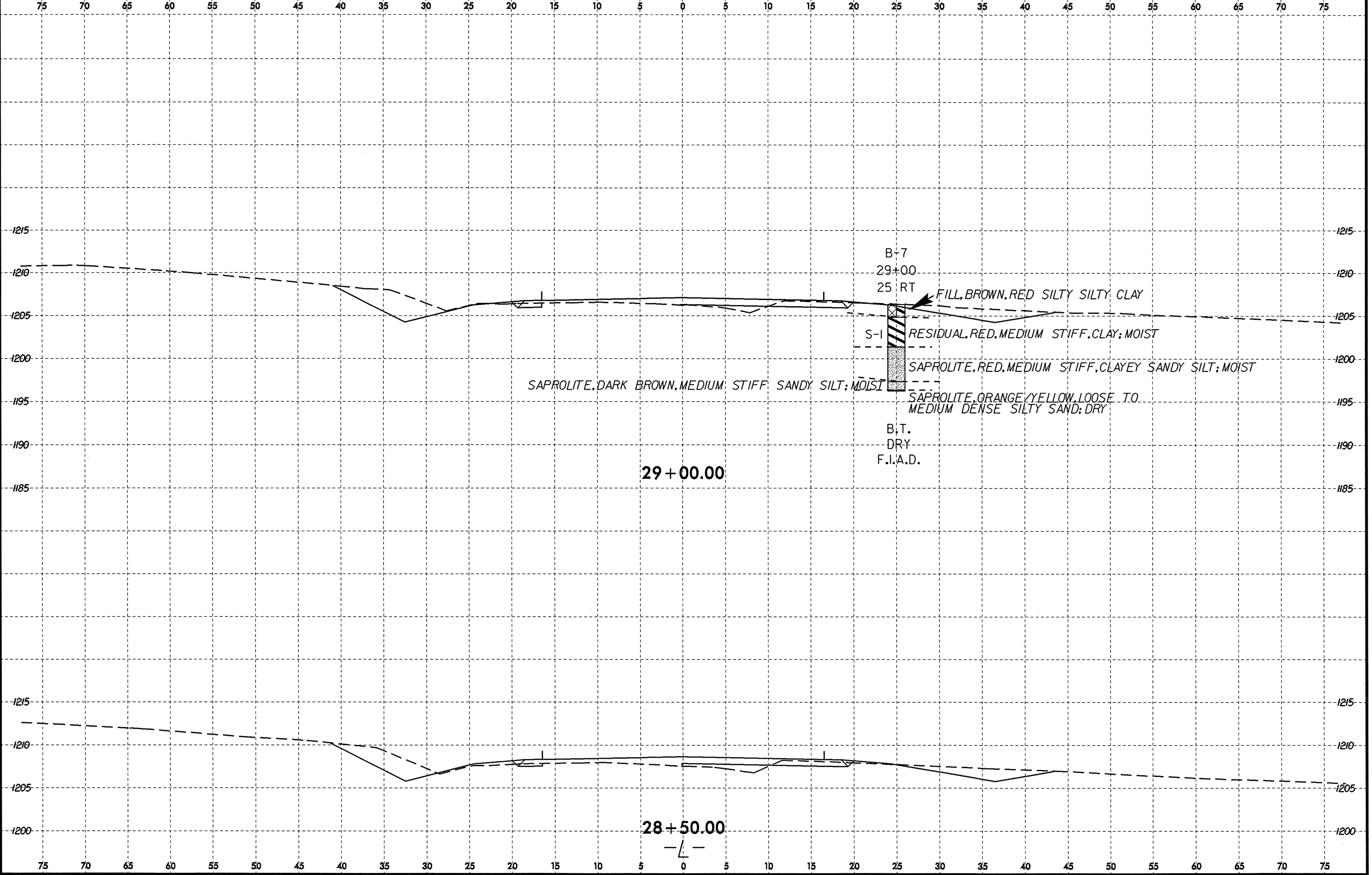




8/23/99



PROJ. REFERENCE NO.	SHEET NO.
U-2551	26/23



B-7  
29+00  
25 RT

FILL, BROWN, RED SILTY SILTY CLAY

S-1

RESIDUAL, RED, MEDIUM STIFF, CLAY; MOIST

SAPROLITE, RED, MEDIUM STIFF, CLAYEY SANDY SILT; MOIST

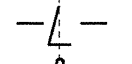
SAPROLITE, DARK BROWN, MEDIUM STIFF, SANDY SILT; MOIST

SAPROLITE, ORANGE/YELLOW, LOOSE TO MEDIUM DENSE SILTY SAND; DRY

B.T.  
DRY  
F.I.A.D.

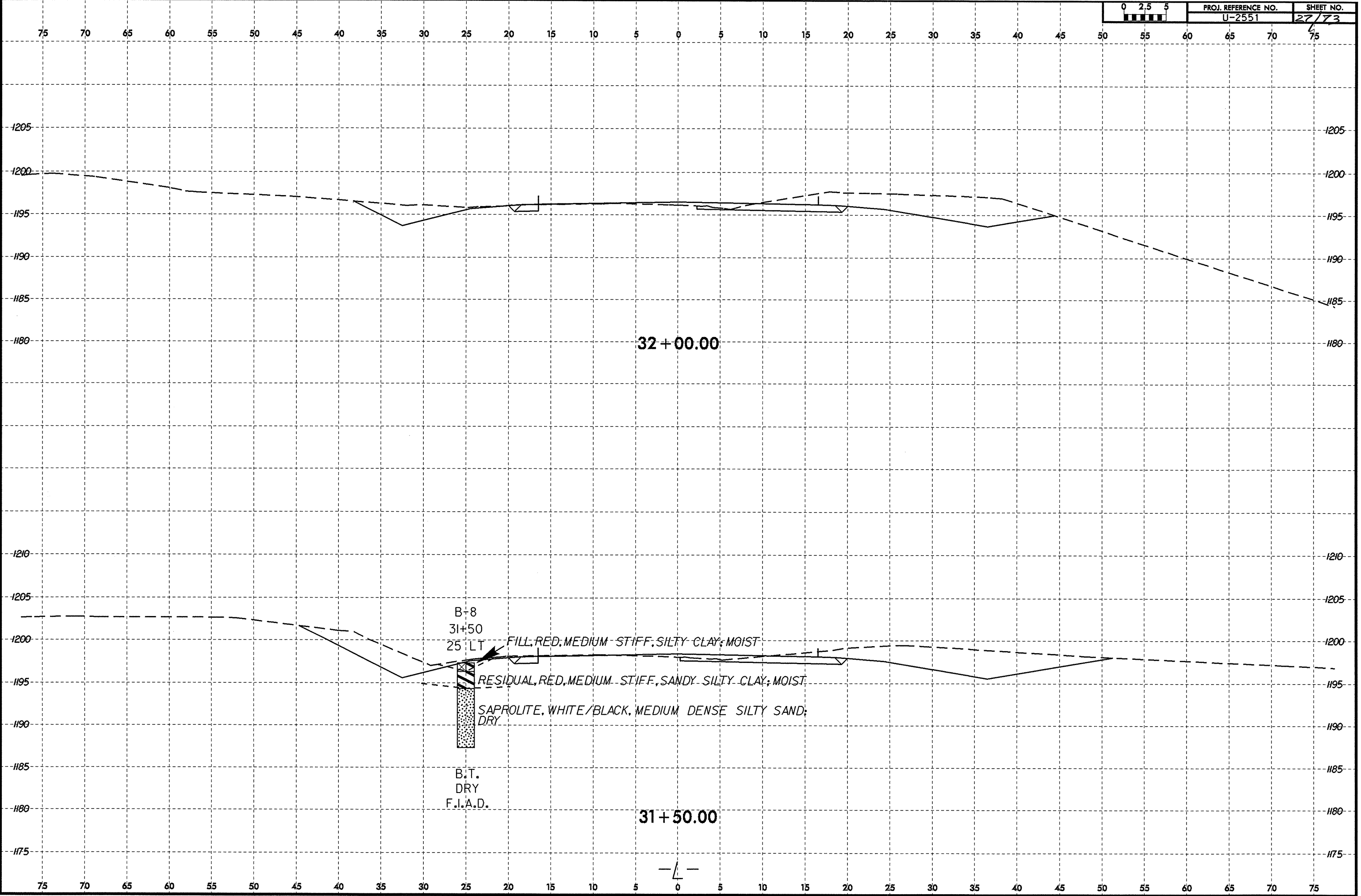
29+00.00

28+50.00

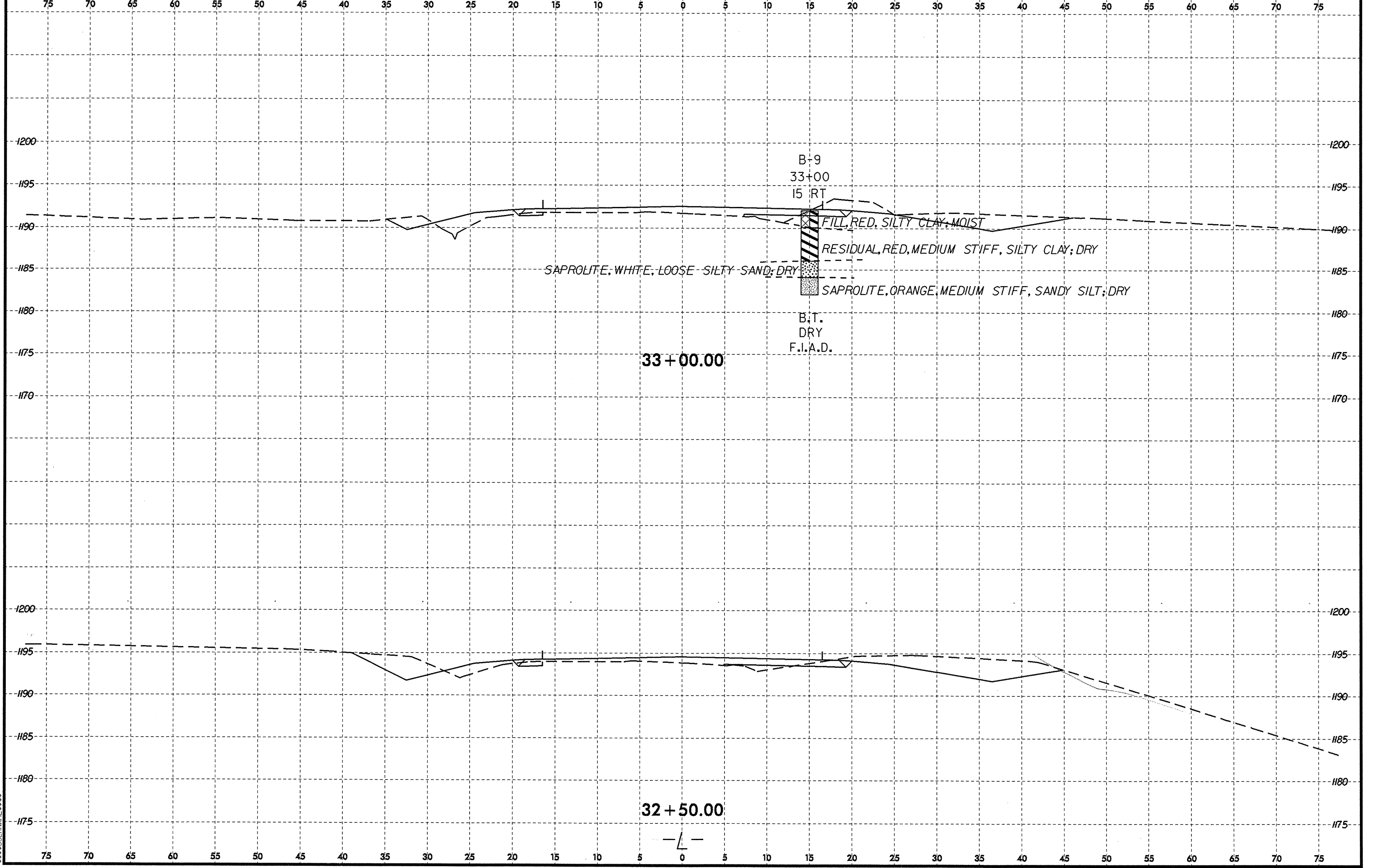


06-APR-2009 09:29  
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 \*\*\*USER NAME\*\*\*

8/23/99  
06-APR-2009 09:29  
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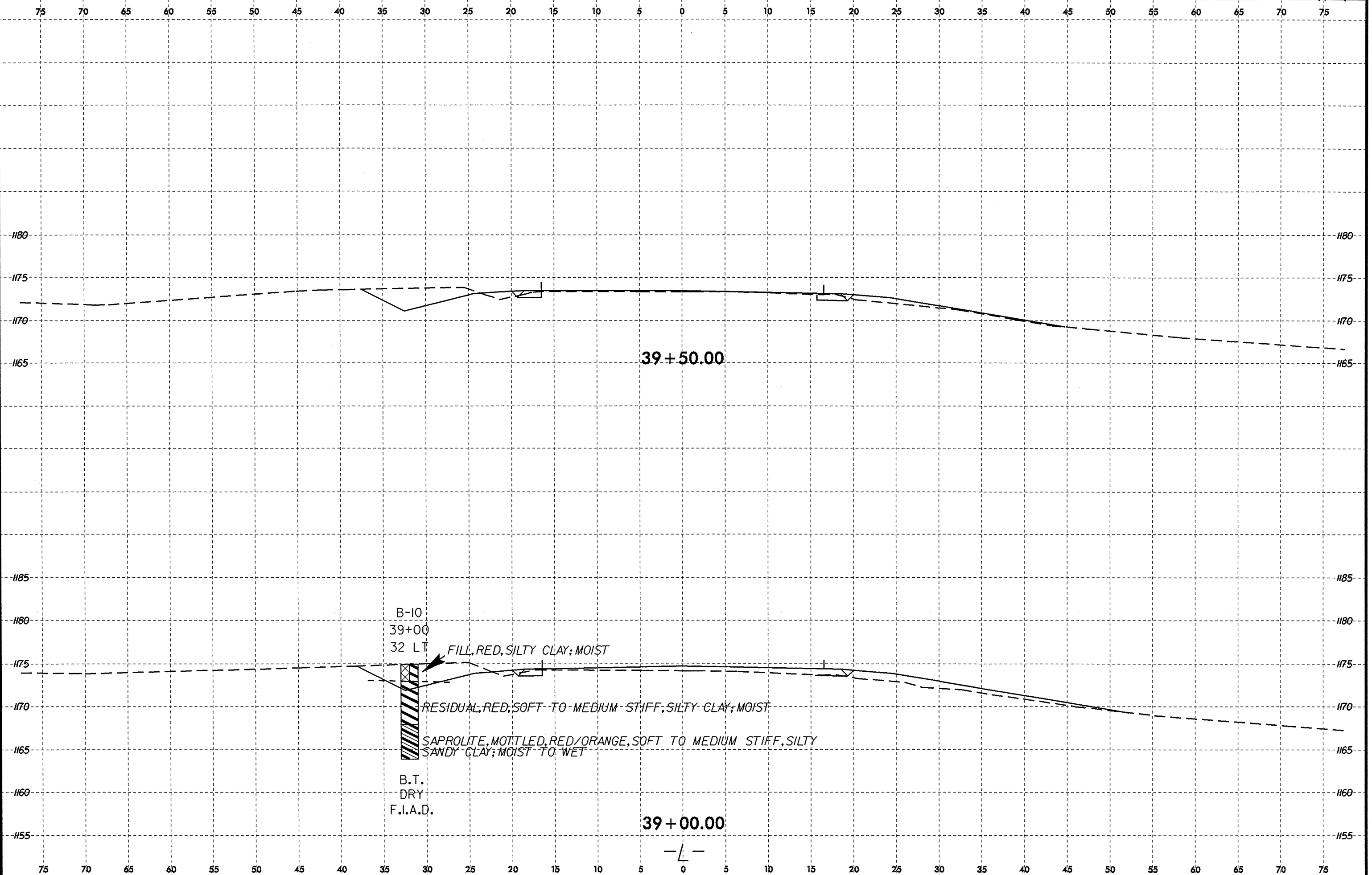


8/23/99



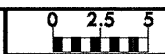
05-MAR-2009 09:24 0264 GEO\_RDWY\CADD\_GEO\TECH\PI\PI\U2551\_GEO\_xst\_1.dgn

8/23/99

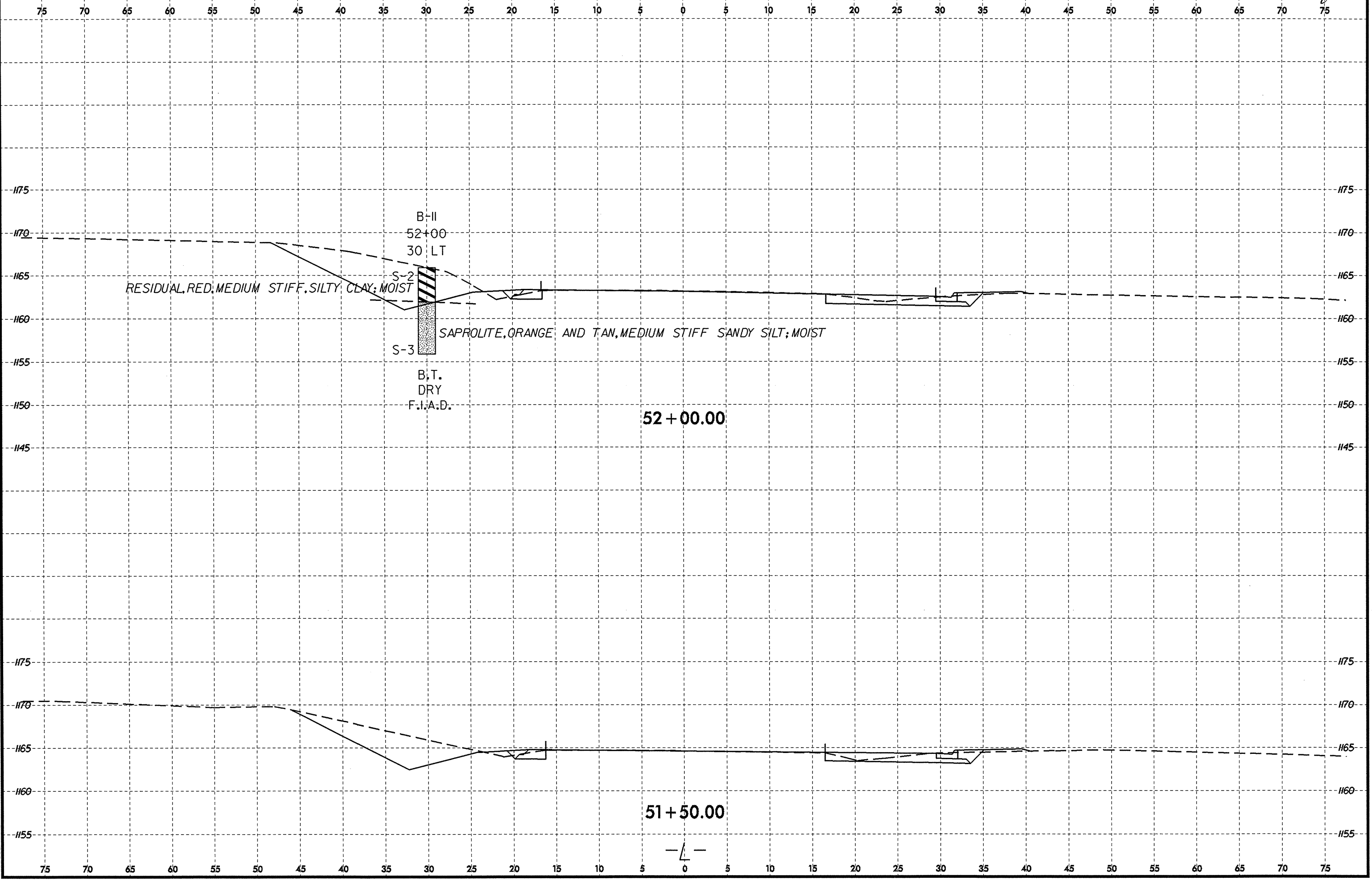


07-APR-2009 08:54  
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 \$\$\$USERNAME\$\$\$

8/23/99



PROJ. REFERENCE NO. U-2551 SHEET NO. 30/73



RESIDUAL, RED, MEDIUM STIFF, SILTY CLAY; MOIST

B-II  
52+00  
30 LT

S-2

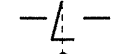
S-3

SAPROLITE, ORANGE AND TAN, MEDIUM STIFF SANDY SILT; MOIST

B, T.  
DRY  
F.I.A.D.

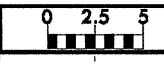
52 + 00.00

51 + 50.00

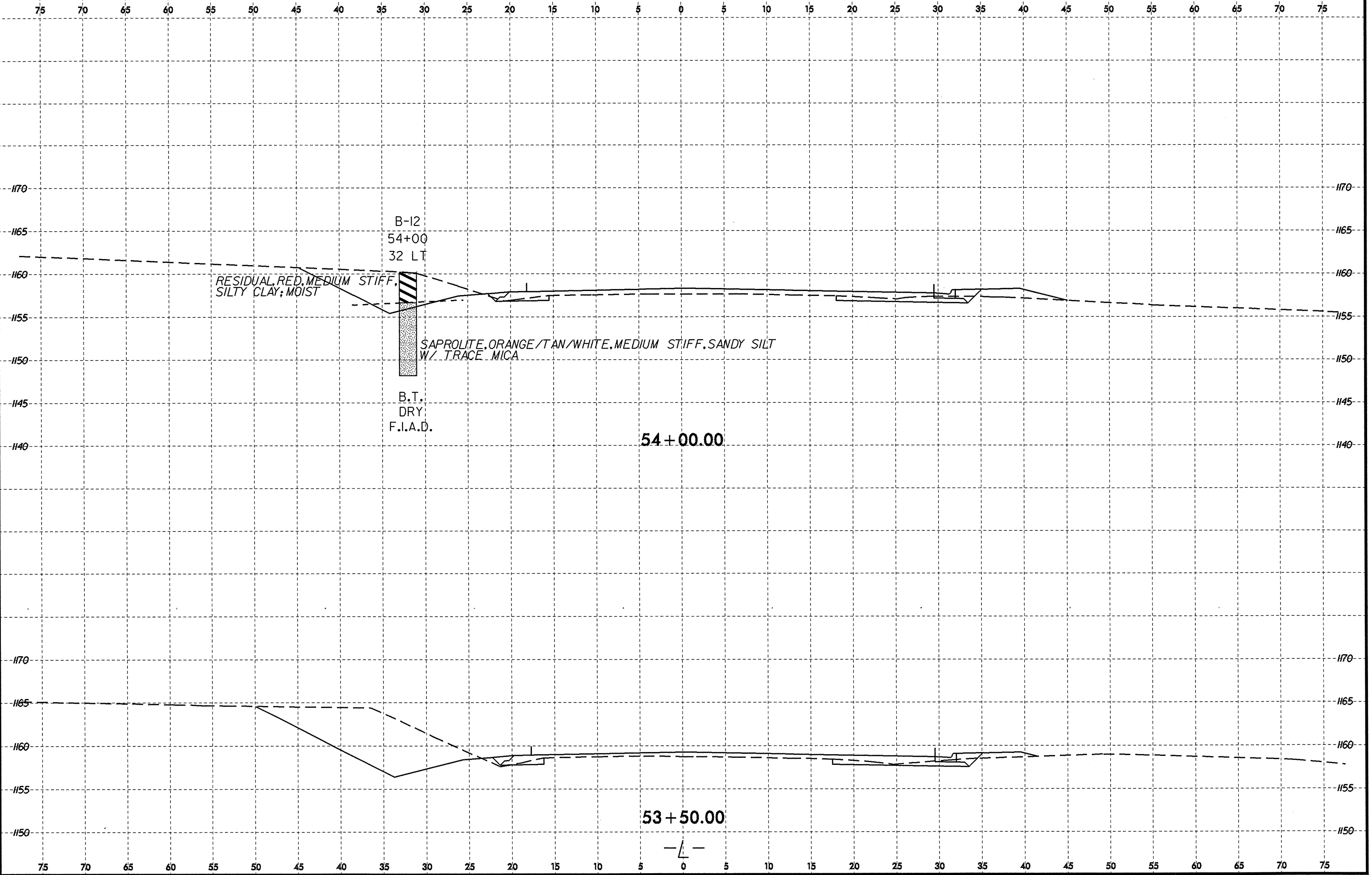


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\$\$\$USERNAME\$\$\$

8/23/99



PROJ. REFERENCE NO. U-2551 SHEET NO. 31/23



RESIDUAL RED, MEDIUM STIFF,  
SILTY CLAY; MOIST

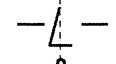
B-12  
54+00  
32 LT

SAPROLITE, ORANGE/TAN/WHITE, MEDIUM STIFF, SANDY SILT  
W/ TRACE MICA

B.T.  
DRY  
F.I.A.D.

54 + 00.00

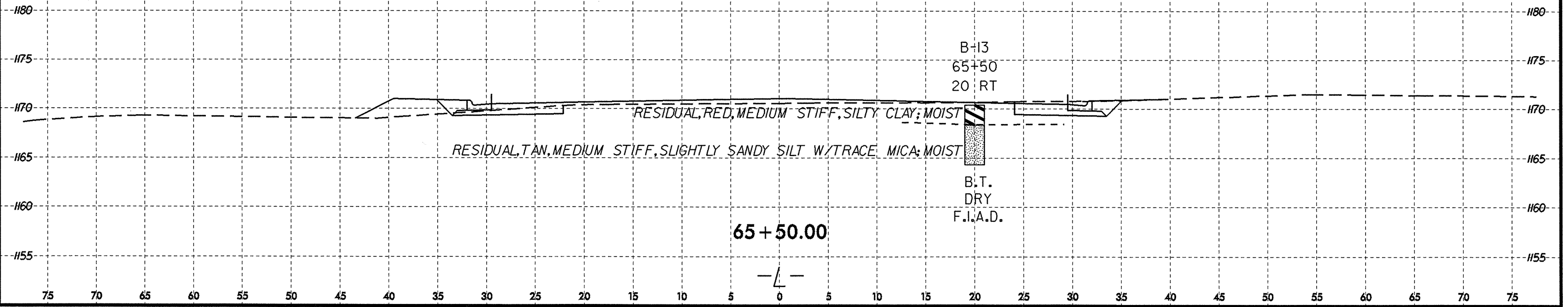
53 + 50.00



01-APR-2009 12:22  
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\$\$\$\$SERVINGME\$\$\$\$

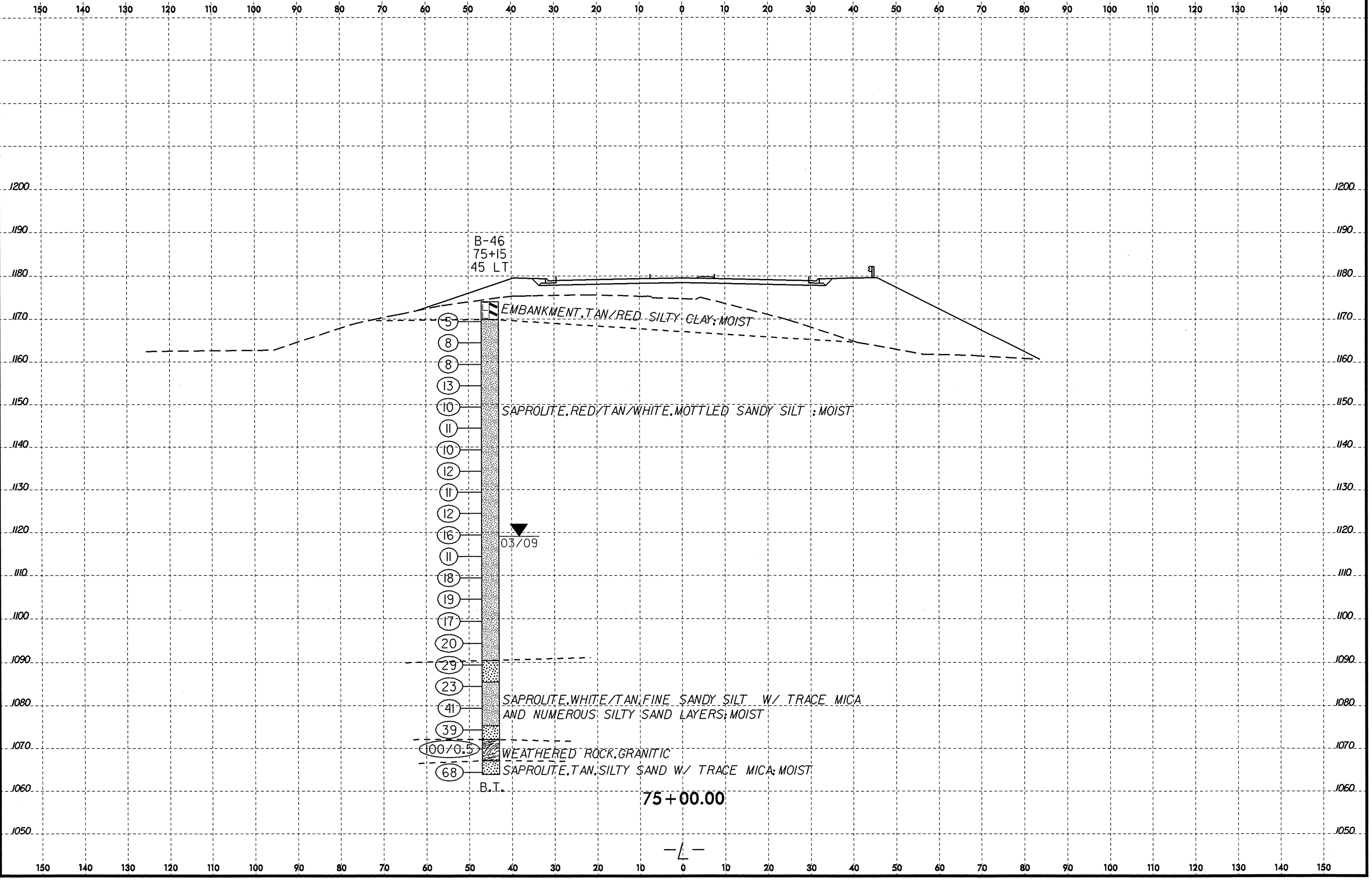
8/22/99

75 70 65 60 55 50 45 40 35 30 25 20 15 10 5 0 5 10 15 20 25 30 35 40 45 50 55 60 65 70 75



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8/23/99



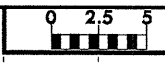
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USER:RDMY

-L-

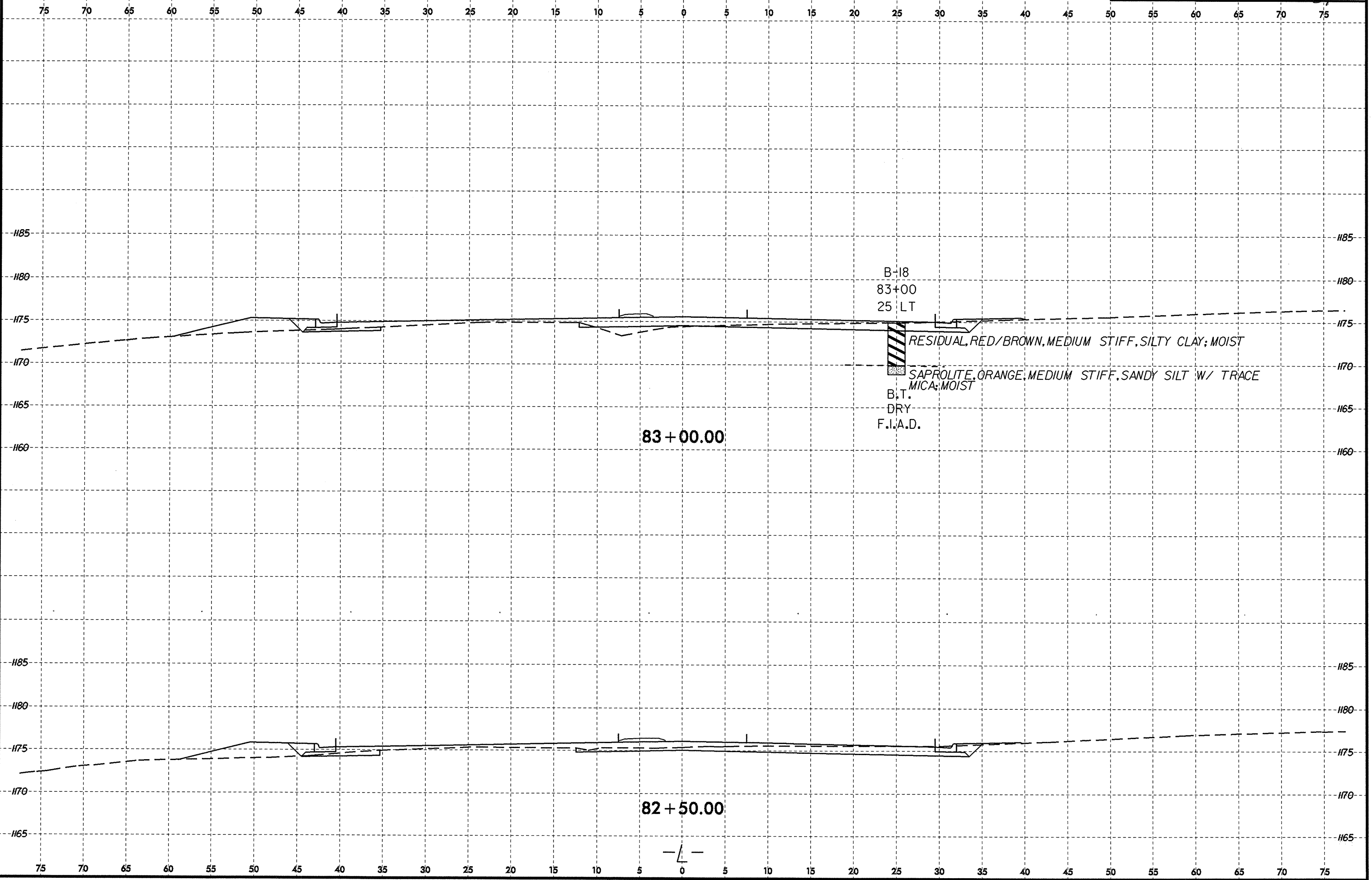




8/23/99



PROJ. REFERENCE NO. U-2551  
SHEET NO. 35/73



83 + 00.00

82 + 50.00

B-18  
83+00  
25 LT

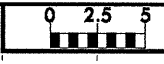
RESIDUAL, RED/BROWN, MEDIUM STIFF, SILTY CLAY; MOIST

SAPROLITE, ORANGE, MEDIUM STIFF, SANDY SILT W/ TRACE MICA; MOIST

B.T.  
DRY  
F.I.A.D.

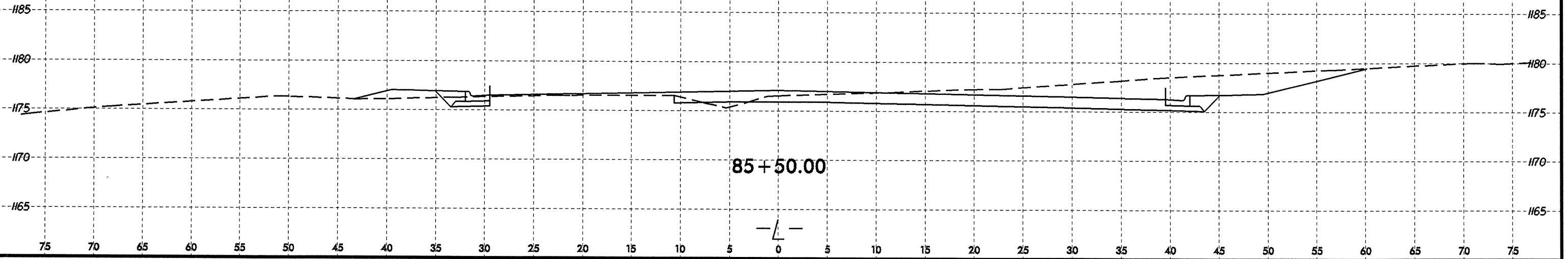
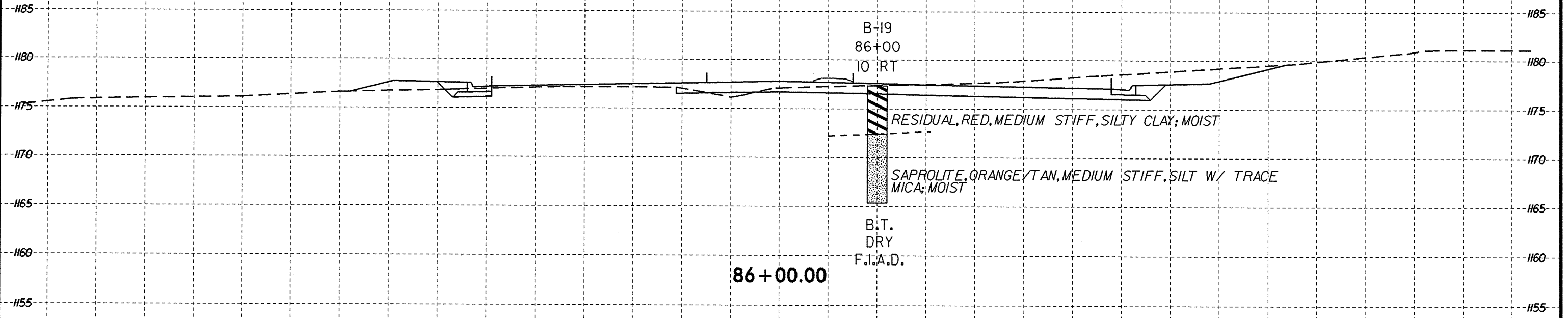
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8/23/99

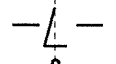


PROJ. REFERENCE NO. U-2551 SHEET NO. 36/73

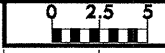
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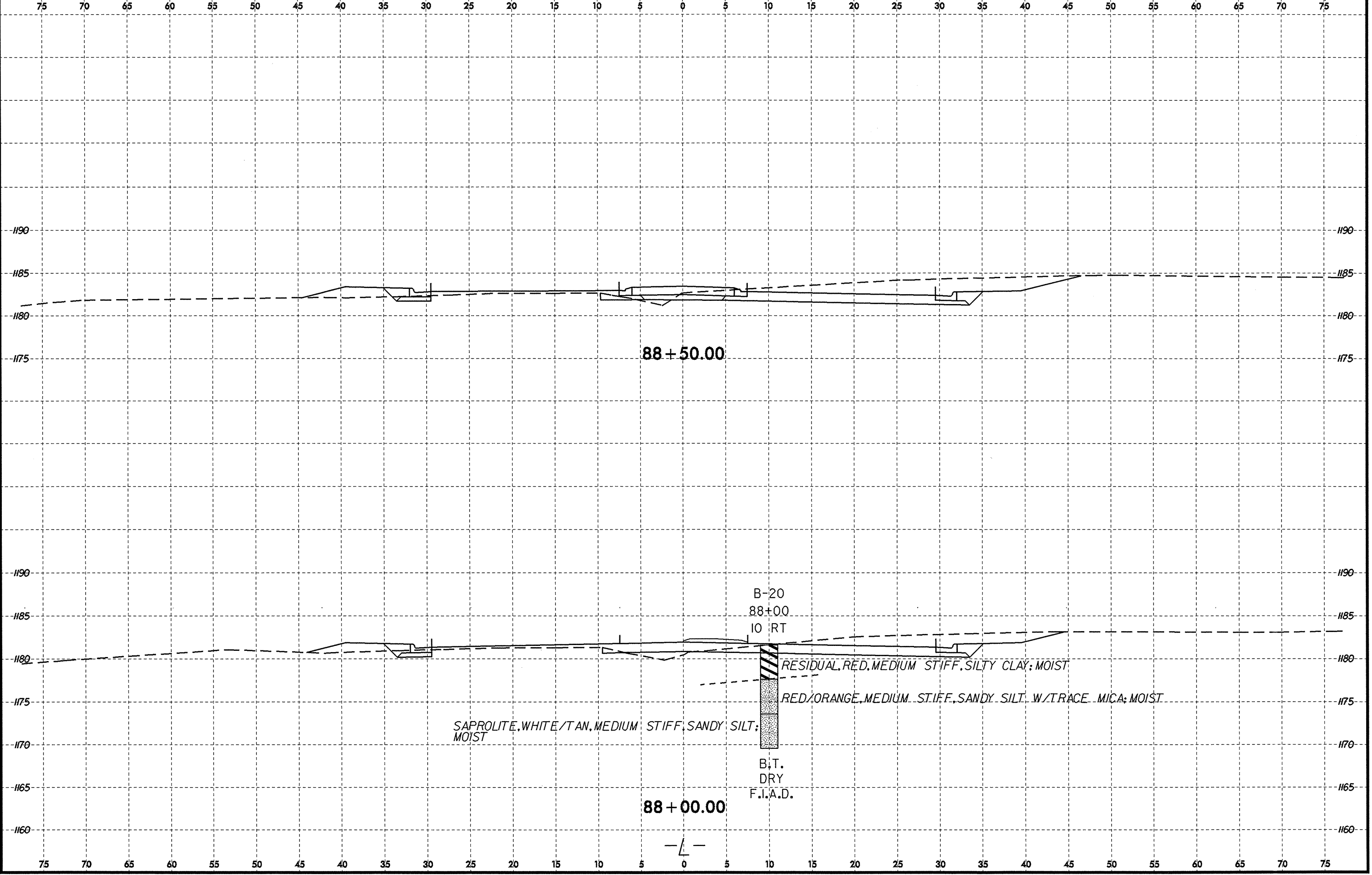


8/23/99



PROJ. REFERENCE NO.  
U-2551

SHEET NO.  
27/73



88 + 50.00

B-20  
88+00  
10 RT

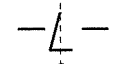
RESIDUAL, RED, MEDIUM STIFF, SILTY CLAY; MOIST

RED/ORANGE, MEDIUM STIFF, SANDY SILT. W/TRACE MICA; MOIST

SAPROLITE, WHITE/TAN, MEDIUM STIFF, SANDY SILT; MOIST

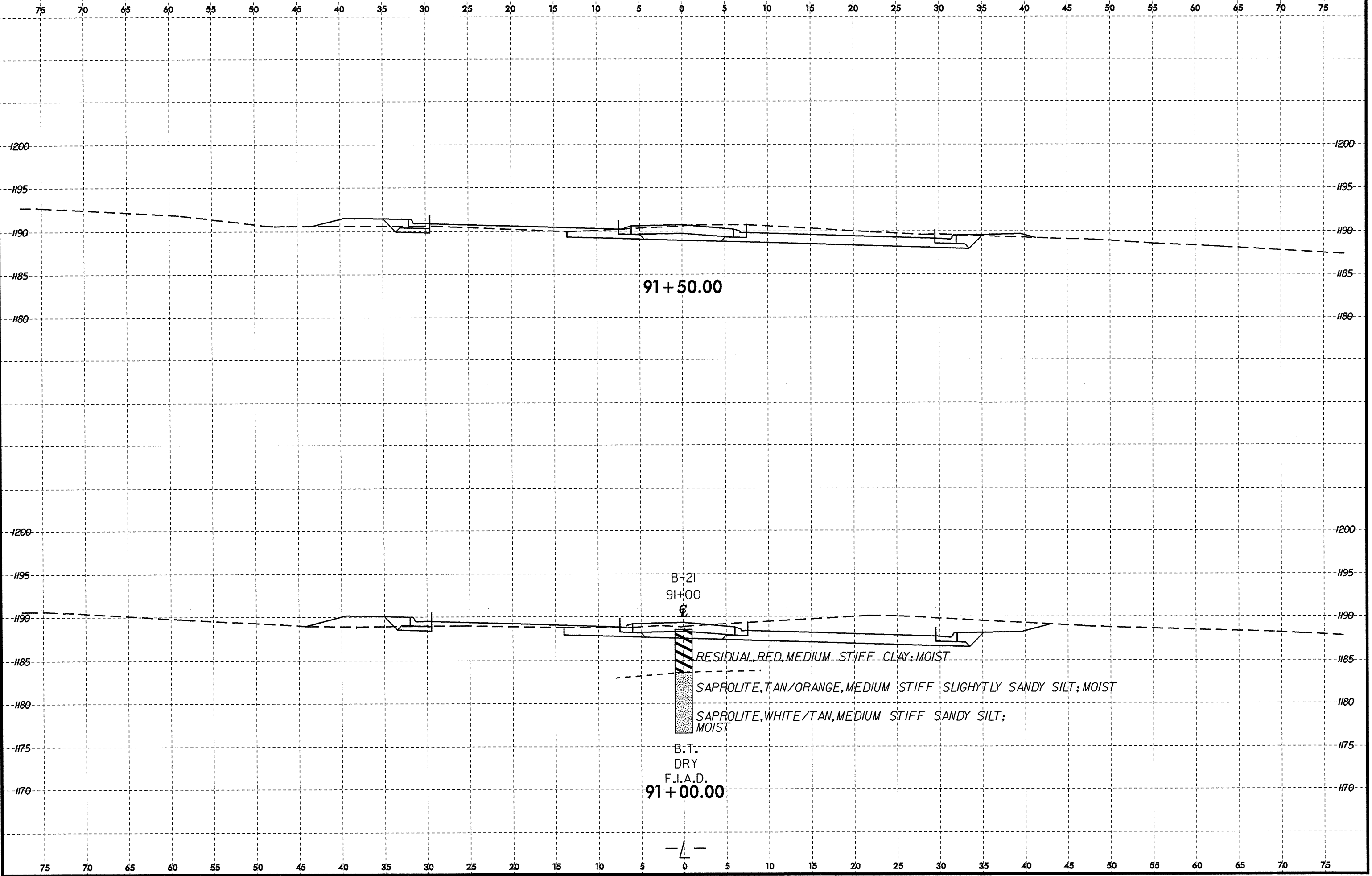
B.T.  
DRY  
F.I.A.D.

88 + 00.00



01-APR-2009 12:27  
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USER:RDWY

8/23/99



91 + 50.00

B-21  
91+00

RESIDUAL, RED, MEDIUM STIFF CLAY; MOIST

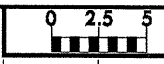
SAPROLITE, TAN/ORANGE, MEDIUM STIFF SLIGHTLY SANDY SILT; MOIST

SAPROLITE, WHITE/TAN, MEDIUM STIFF SANDY SILT; MOIST

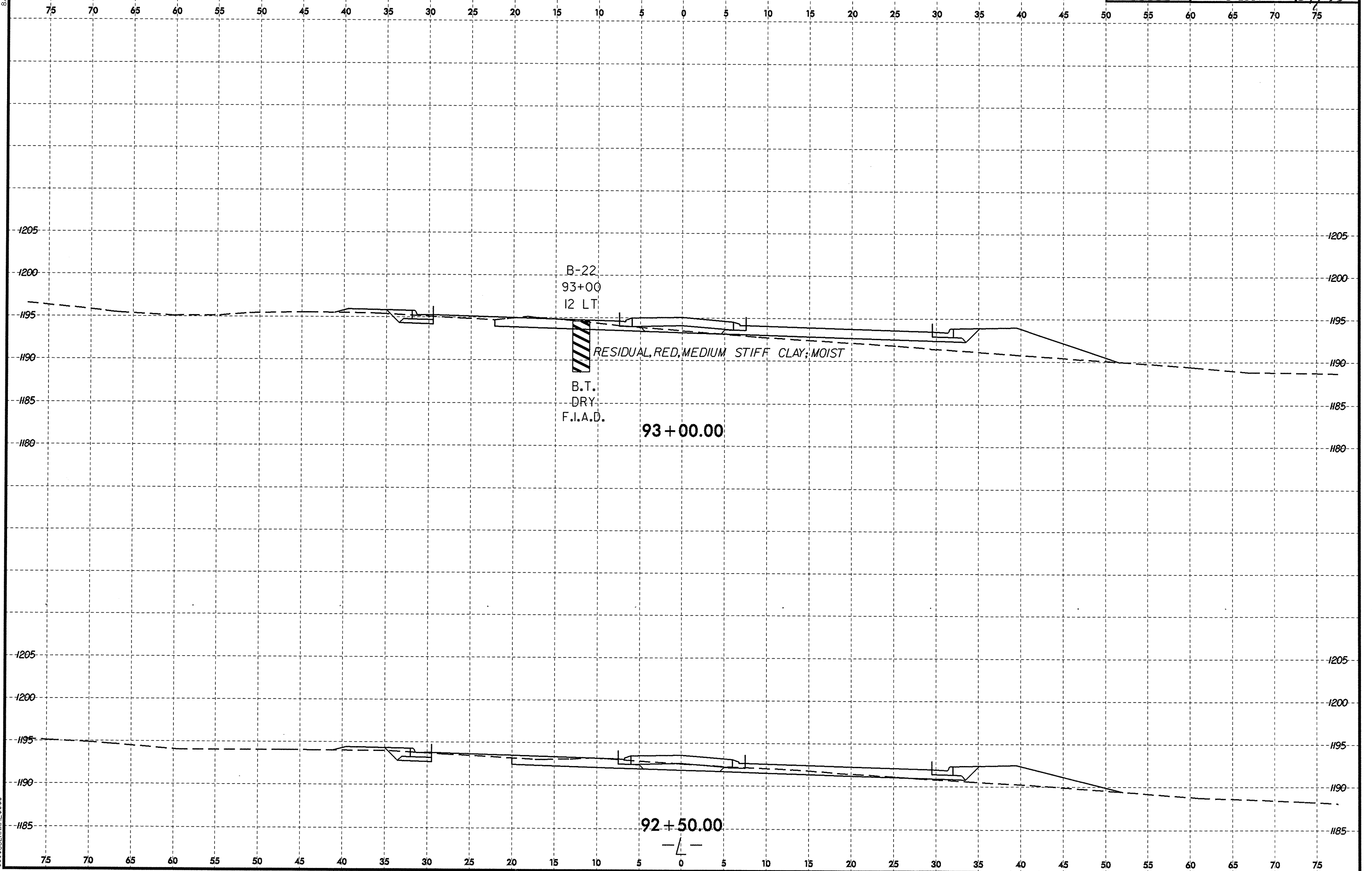
B.T.  
DRY  
F.I.A.D.  
91 + 00.00

06-APR-2009 12:09  
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8/23/99



PROJ. REFERENCE NO.	SHEET NO.
U-2551	39/73



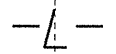
B-22  
93+00  
12 LT

RESIDUAL, RED, MEDIUM STIFF CLAY; MOIST

B.T.  
DRY  
F.I.A.D.

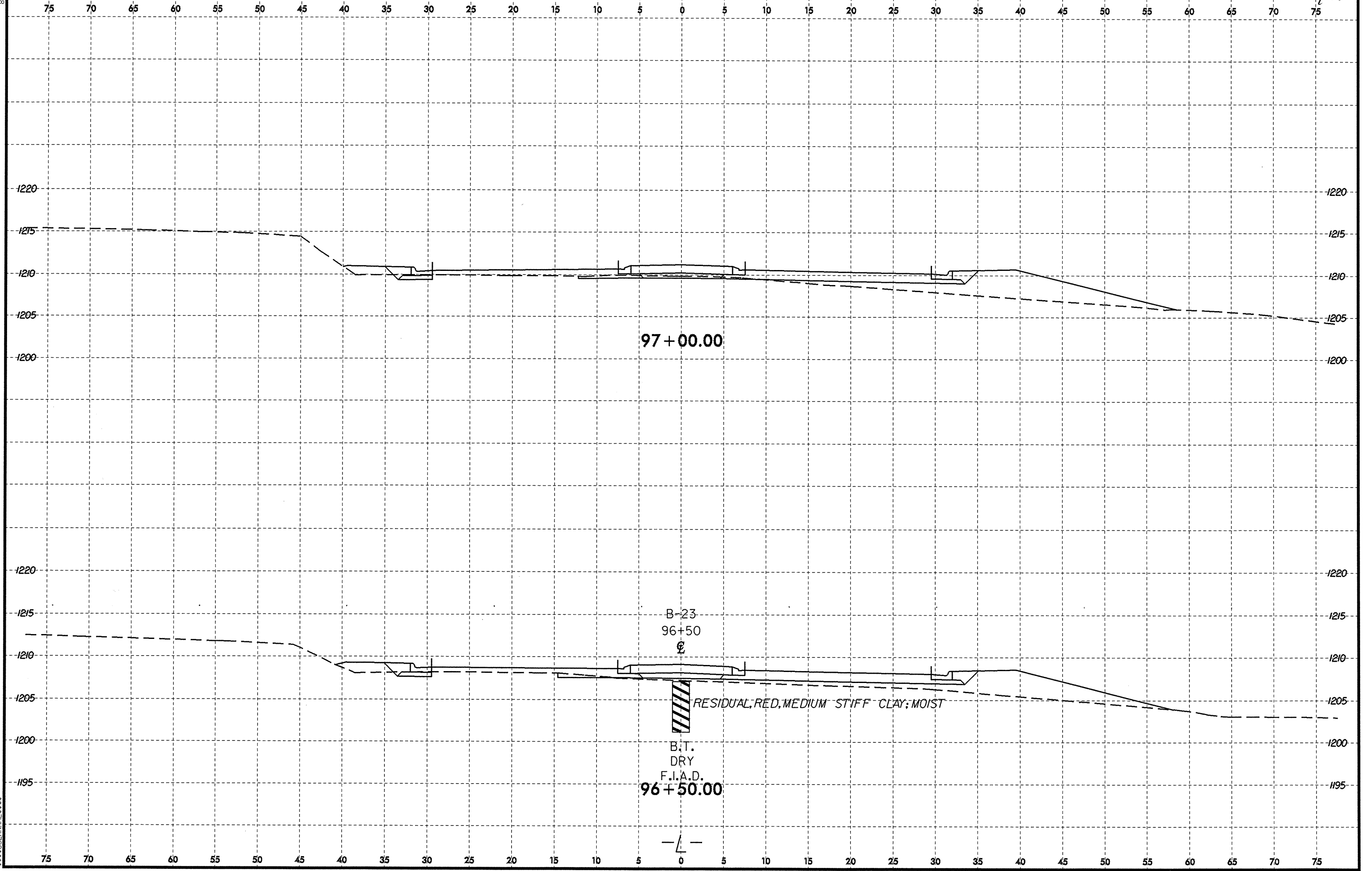
93 + 00.00

92 + 50.00



05-MAR-2009 09:45  
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8/23/99



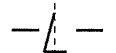
97 + 00.00

B-23  
96+50



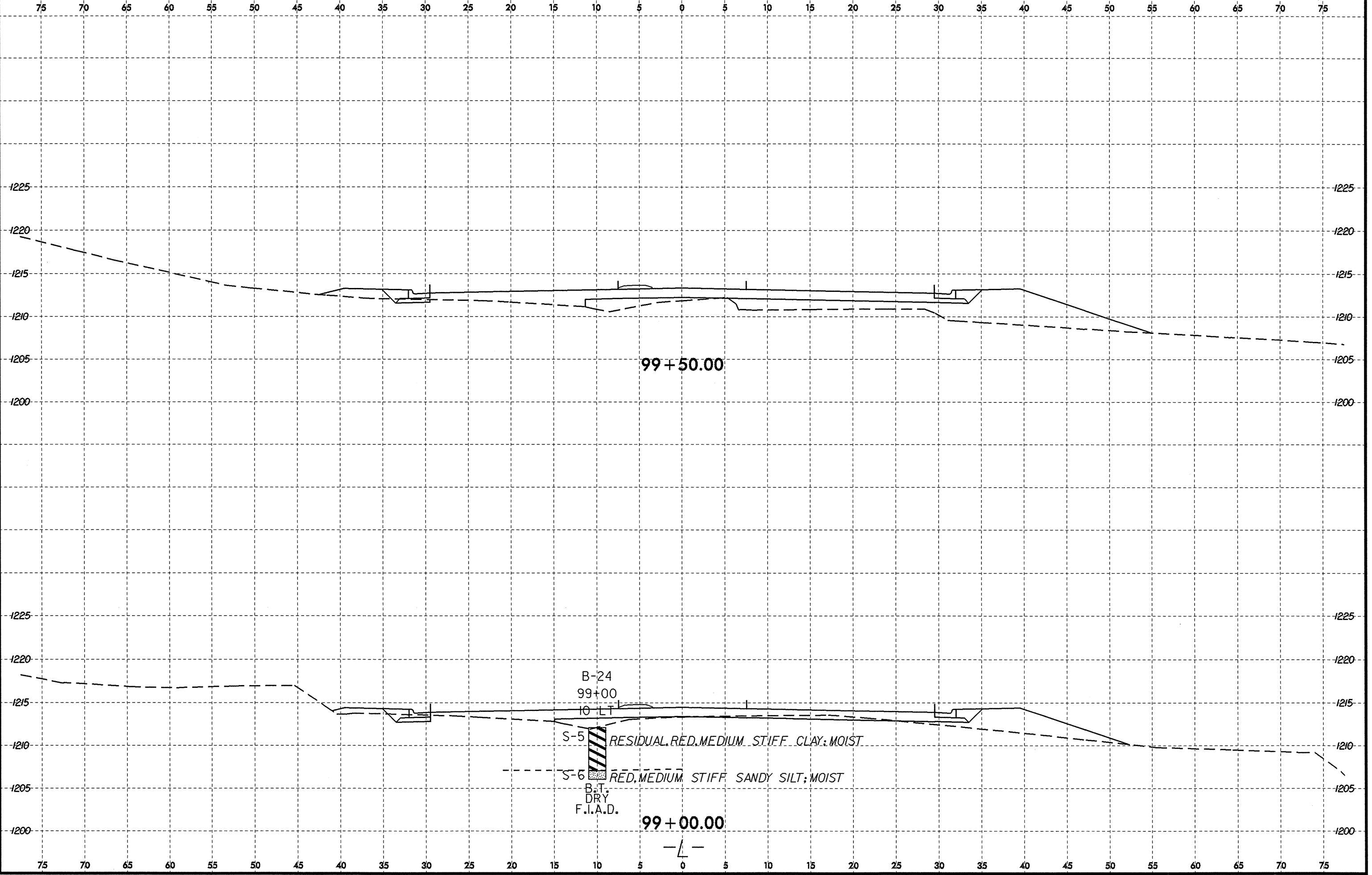
RESIDUAL, RED, MEDIUM STIFF CLAY; MOIST

B.T.  
DRY  
F.I.A.D.  
96 + 50.00



25-MAR-2009 09:38 C:\P\1\GEO\RDWY\CADD\GEO\TECH\P1\enr\Pro\U2551\_GEO\_xst\_1.dgn

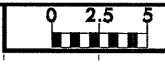
8/23/99



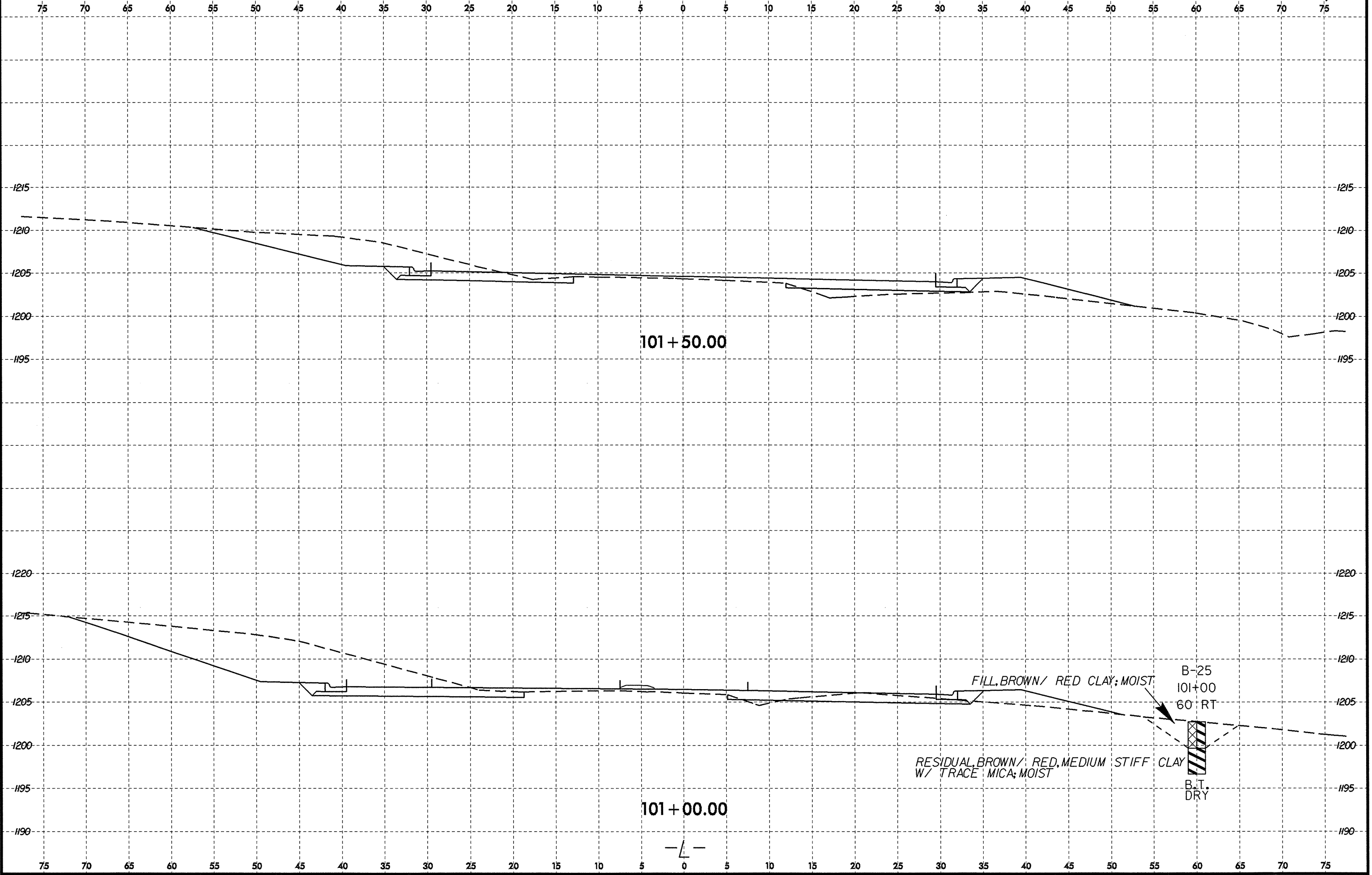
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8/23/99



PROJ. REFERENCE NO. U-2551 SHEET NO. 42/23



101 + 50.00

101 + 00.00

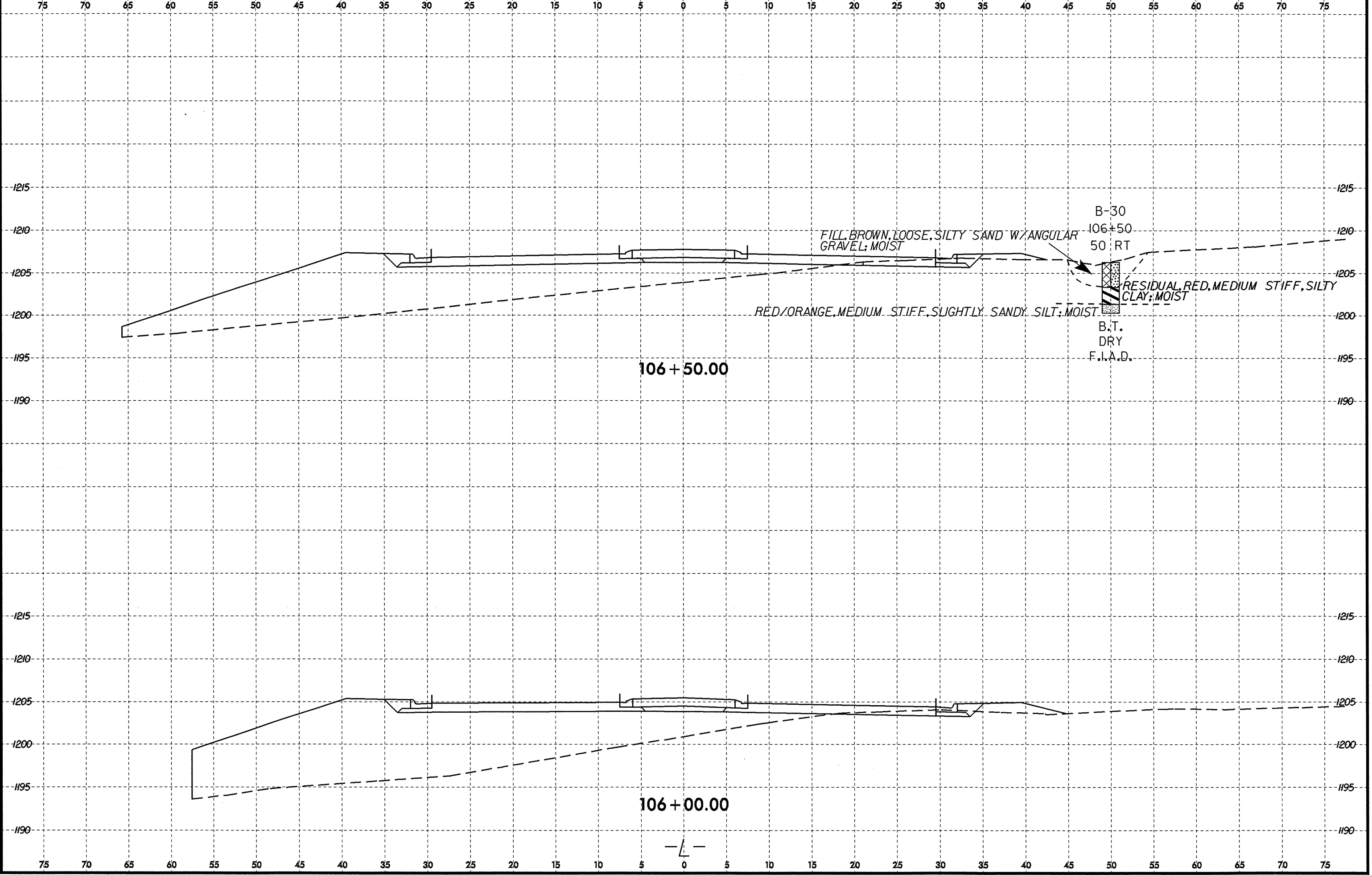
FILL, BROWN / RED CLAY; MOIST

RESIDUAL, BROWN / RED, MEDIUM STIFF CLAY W/ TRACE MICA; MOIST

B-25  
101+00  
60 RT  
B.T.  
DRY

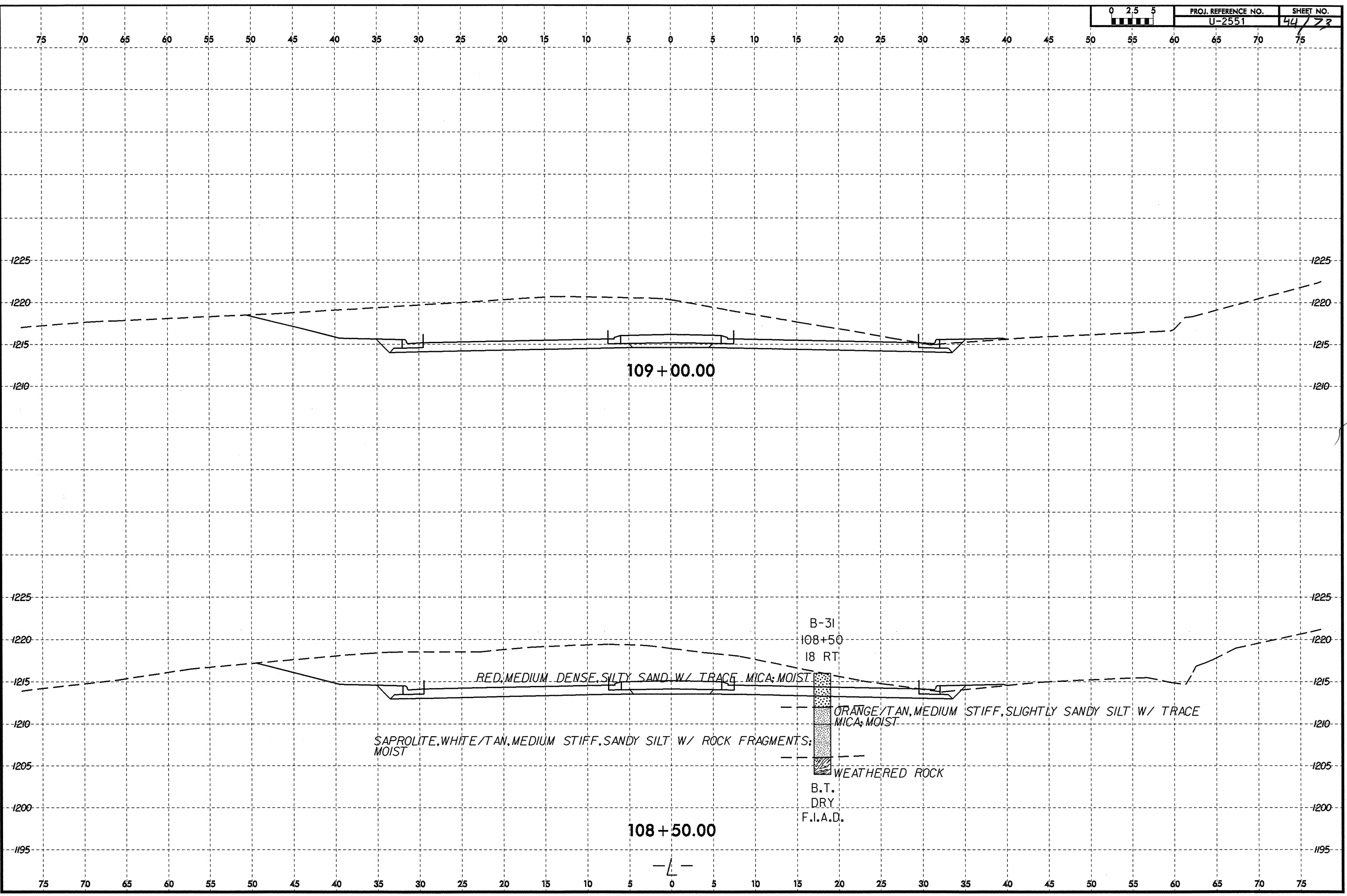
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8/23/99



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\$\$\$\$\$USER\$NAME\$\$\$\$\$



109 + 00.00

108 + 50.00

B-31  
108+50  
18 RT

RED, MEDIUM DENSE, SILTY SAND W/ TRACE MICA; MOIST

ORANGE/TAN, MEDIUM STIFF, SLIGHTLY SANDY SILT W/ TRACE MICA; MOIST

SAPROLITE, WHITE/TAN, MEDIUM STIFF, SANDY SILT W/ ROCK FRAGMENTS; MOIST

WEATHERED ROCK

B.T.  
DRY  
F.I.A.D.



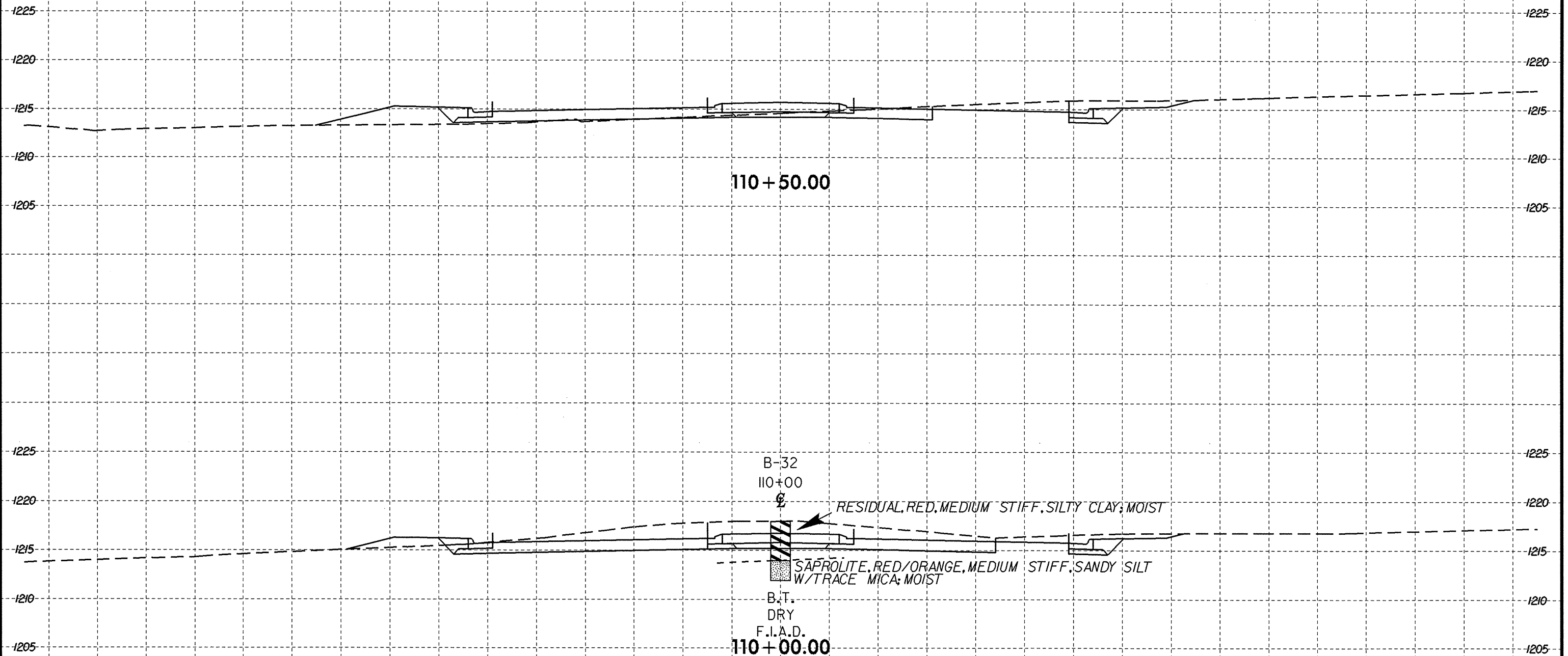
8/23/99



PROJ. REFERENCE NO.  
U-2551

SHEET NO.  
45/73

75 70 65 60 55 50 45 40 35 30 25 20 15 10 5 0 5 10 15 20 25 30 35 40 45 50 55 60 65 70 75



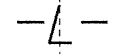
110 + 50.00

B-32  
110+00

RESIDUAL, RED, MEDIUM STIFF, SILTY CLAY; MOIST

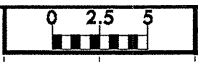
SAPROLITE, RED/ORANGE, MEDIUM STIFF, SANDY SILT  
W/TRACE MICA; MOIST

B.T.  
DRY  
F.I.A.D.  
110 + 00.00

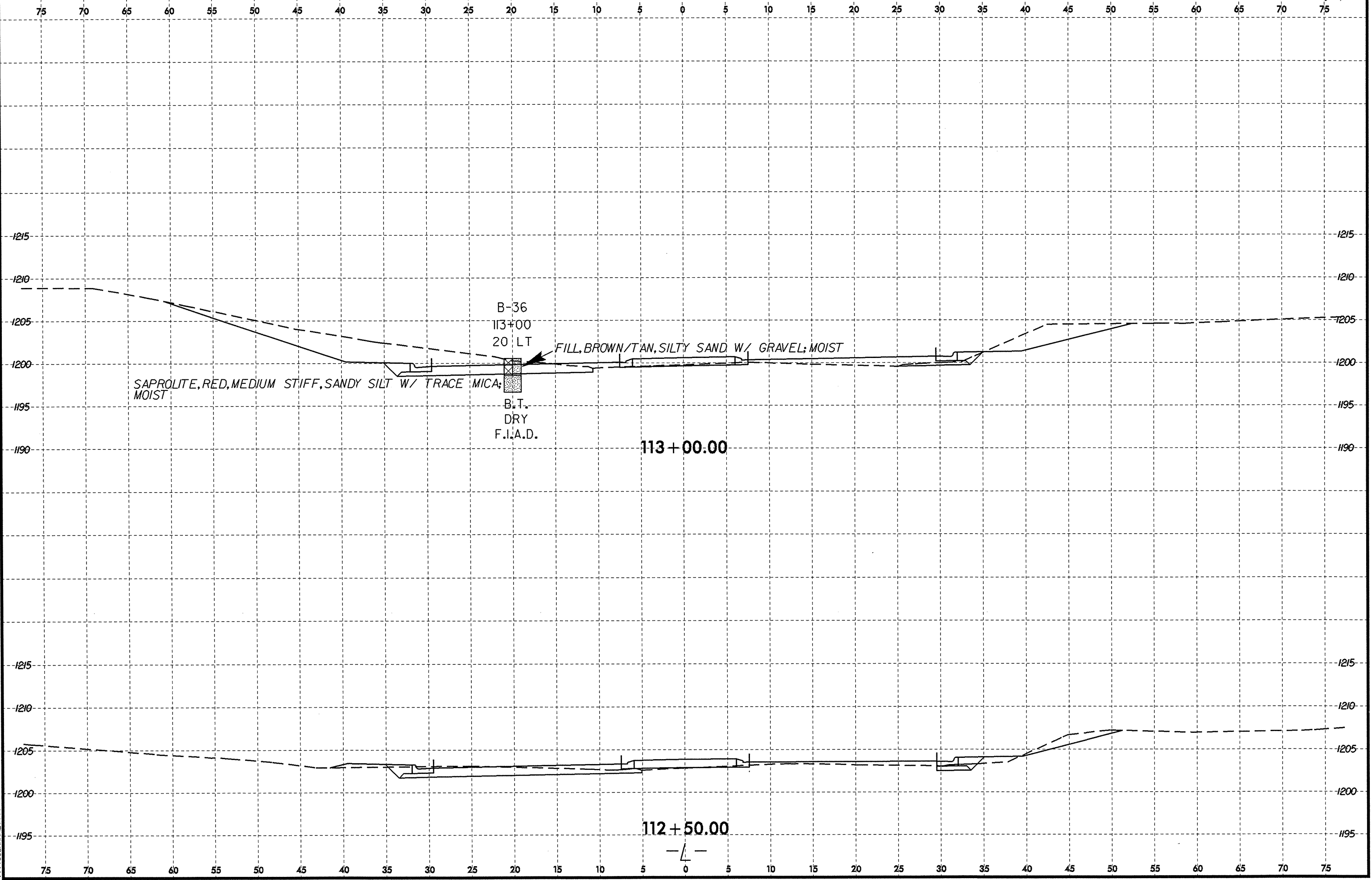


01-APR-2009 12:35  
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8/23/99



PROJ. REFERENCE NO. U-2551	SHEET NO. 46/73
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07-APR-2009 09:02  
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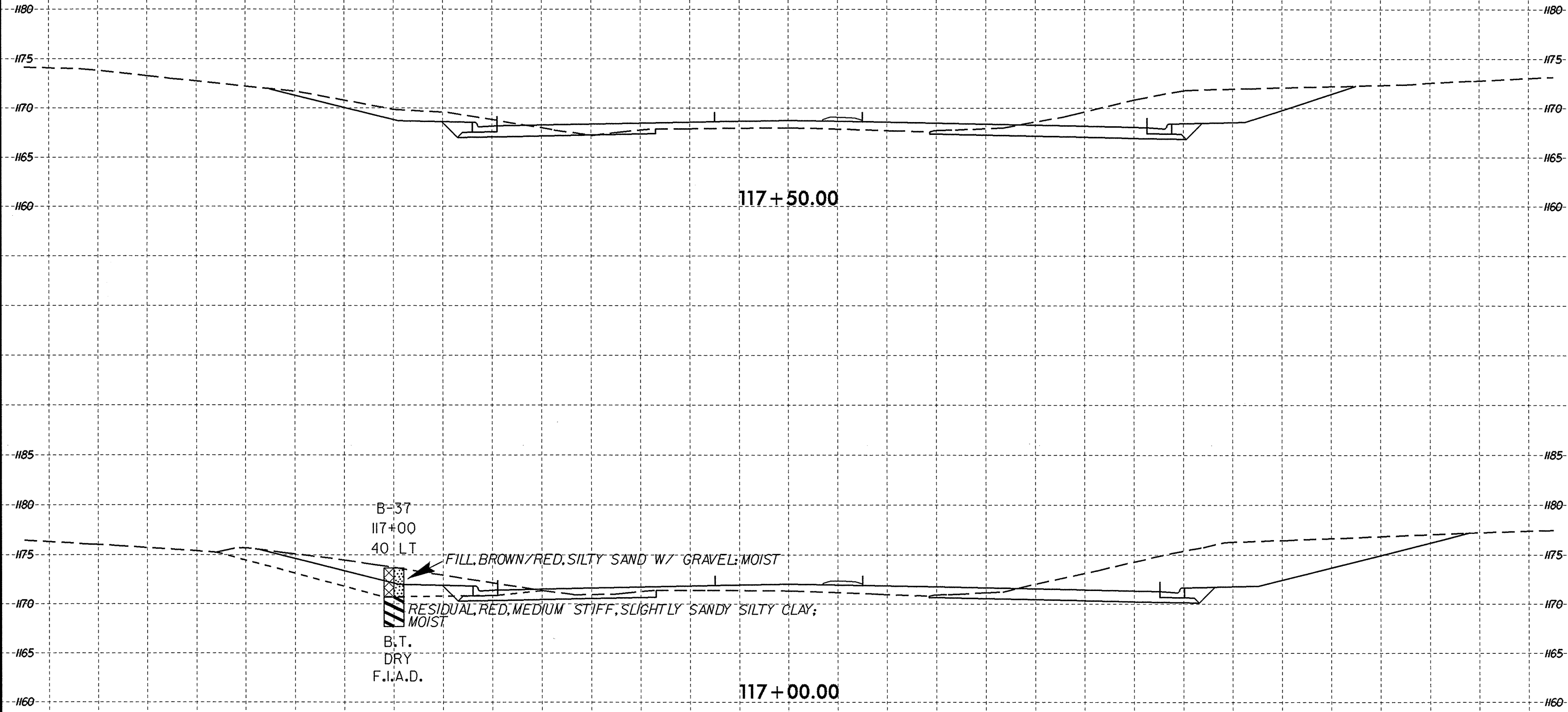
8/23/99



PROJ. REFERENCE NO.  
U-2551

SHEET NO.  
47/23

75 70 65 60 55 50 45 40 35 30 25 20 15 10 5 0 5 10 15 20 25 30 35 40 45 50 55 60 65 70 75



117 + 50.00

117 + 00.00

B-37  
117+00  
40 LT  
B.T.  
DRY  
F.I.A.D.

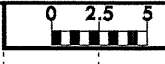
FILL, BROWN/RED, SILTY SAND W/ GRAVEL; MOIST

RESIDUAL, RED, MEDIUM STIFF, SLIGHTLY SANDY SILTY CLAY;  
MOIST

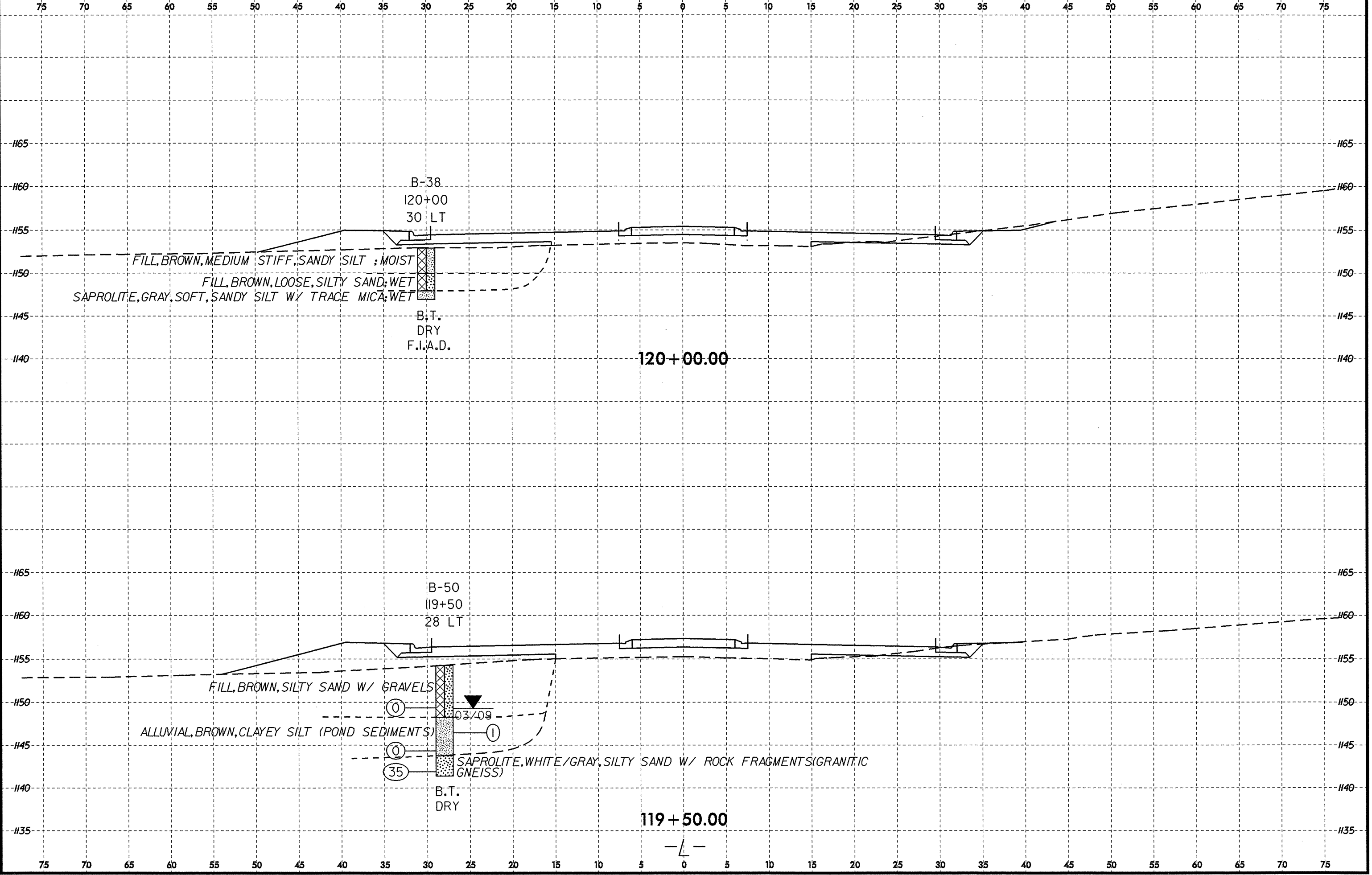
-L-

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\*\*\*USERNAME\*\*\*

8/23/99



PROJ. REFERENCE NO. U-2551 SHEET NO. 48/73



B-38  
120+00  
30 LT

FILL, BROWN, MEDIUM STIFF, SANDY SILT ; MOIST  
FILL, BROWN, LOOSE, SILTY SAND; WET  
SAPROLITE, GRAY, SOFT, SANDY SILT WY TRACE MICA; WET

B.T.  
DRY  
F.I.A.D.

120 + 00.00

B-50  
119+50  
28 LT

FILL, BROWN, SILTY SAND W/ GRAVELS  
ALLUVIAL, BROWN, CLAYEY SILT (POND SEDIMENTS)  
SAPROLITE, WHITE/GRAY, SILTY SAND W/ ROCK FRAGMENTS (GRANITIC GNEISS)

B.T.  
DRY

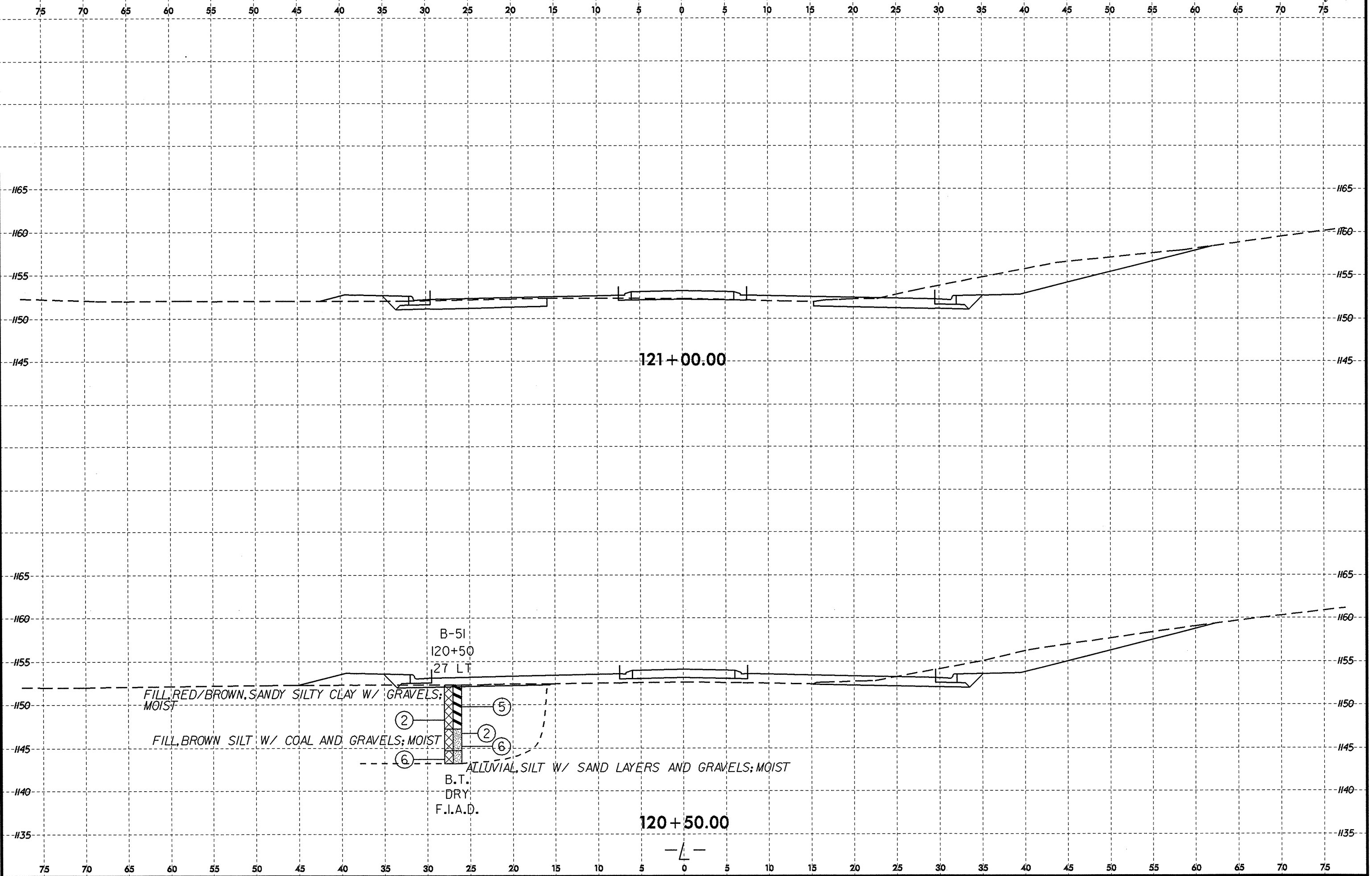
119 + 50.00

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8/23/99



PROJ. REFERENCE NO. U-2551 SHEET NO. 49/73



FILL, RED/BROWN, SANDY SILTY CLAY W/ GRAVELS; MOIST

FILL, BROWN SILT W/ COAL AND GRAVELS; MOIST

B-51  
120+50  
27 LT

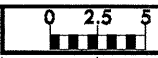
B.T.  
DRY  
F.I.A.D.

ALLUVIAL SILT W/ SAND LAYERS AND GRAVELS; MOIST

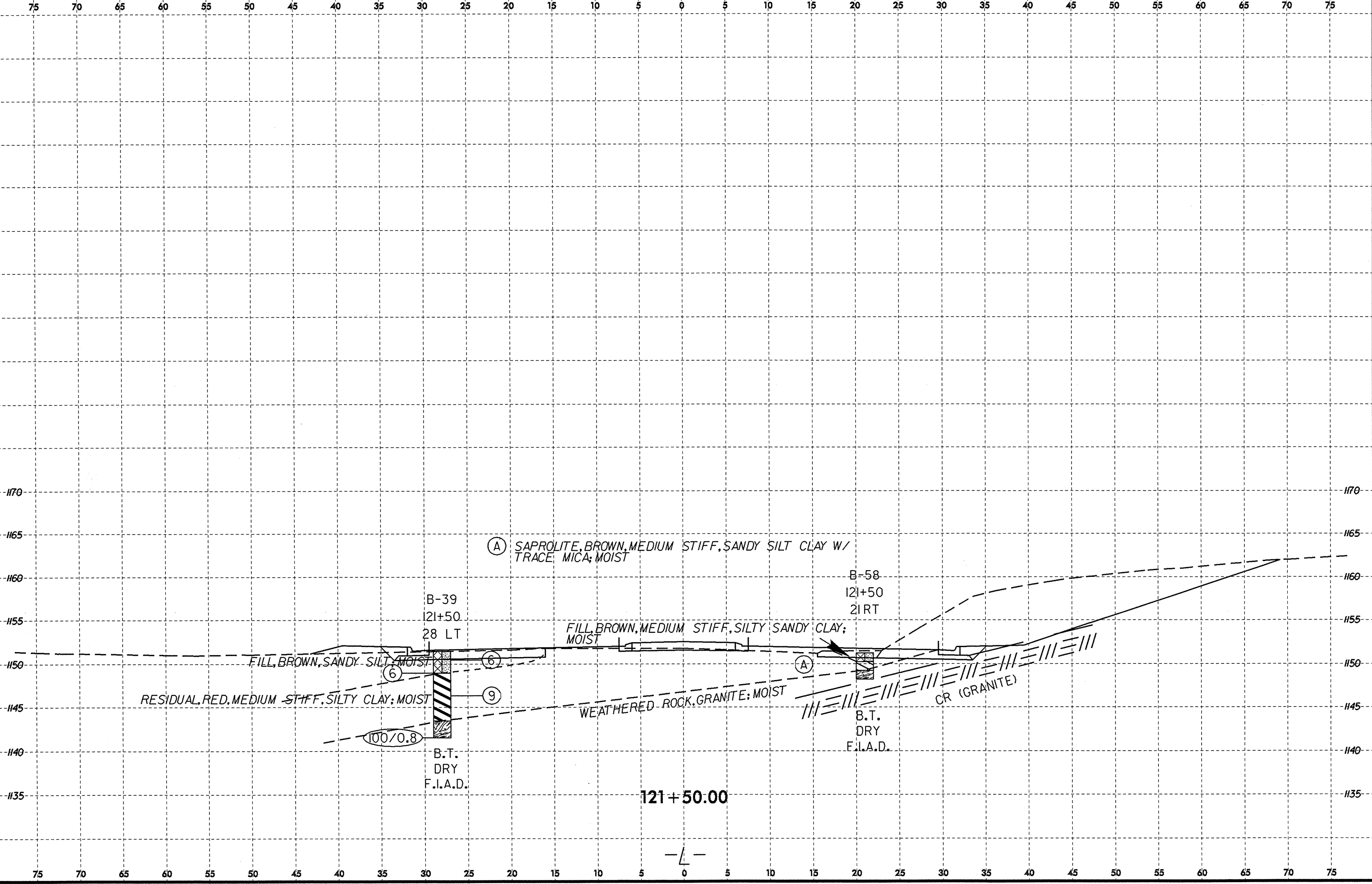
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\$\$\$\$\$USERNAME\$\$\$\$\$



8/23/99



PROJ. REFERENCE NO. U-2551 SHEET NO. 50/73



(A) SAPROLITE, BROWN, MEDIUM STIFF, SANDY SILT CLAY W/  
TRACE MICA; MOIST

B-39  
121+50  
28 LT

FILL, BROWN, MEDIUM STIFF, SILTY SANDY CLAY;  
MOIST

B-58  
121+50  
21 RT

FILL, BROWN, SANDY SILT, MOIST

RESIDUAL, RED, MEDIUM STIFF, SILTY CLAY; MOIST

WEATHERED ROCK, GRANITE; MOIST

CR (GRANITE)

100/0.8

B.T.  
DRY  
F.I.A.D.

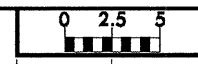
B.T.  
DRY  
F.I.A.D.

121 + 50.00

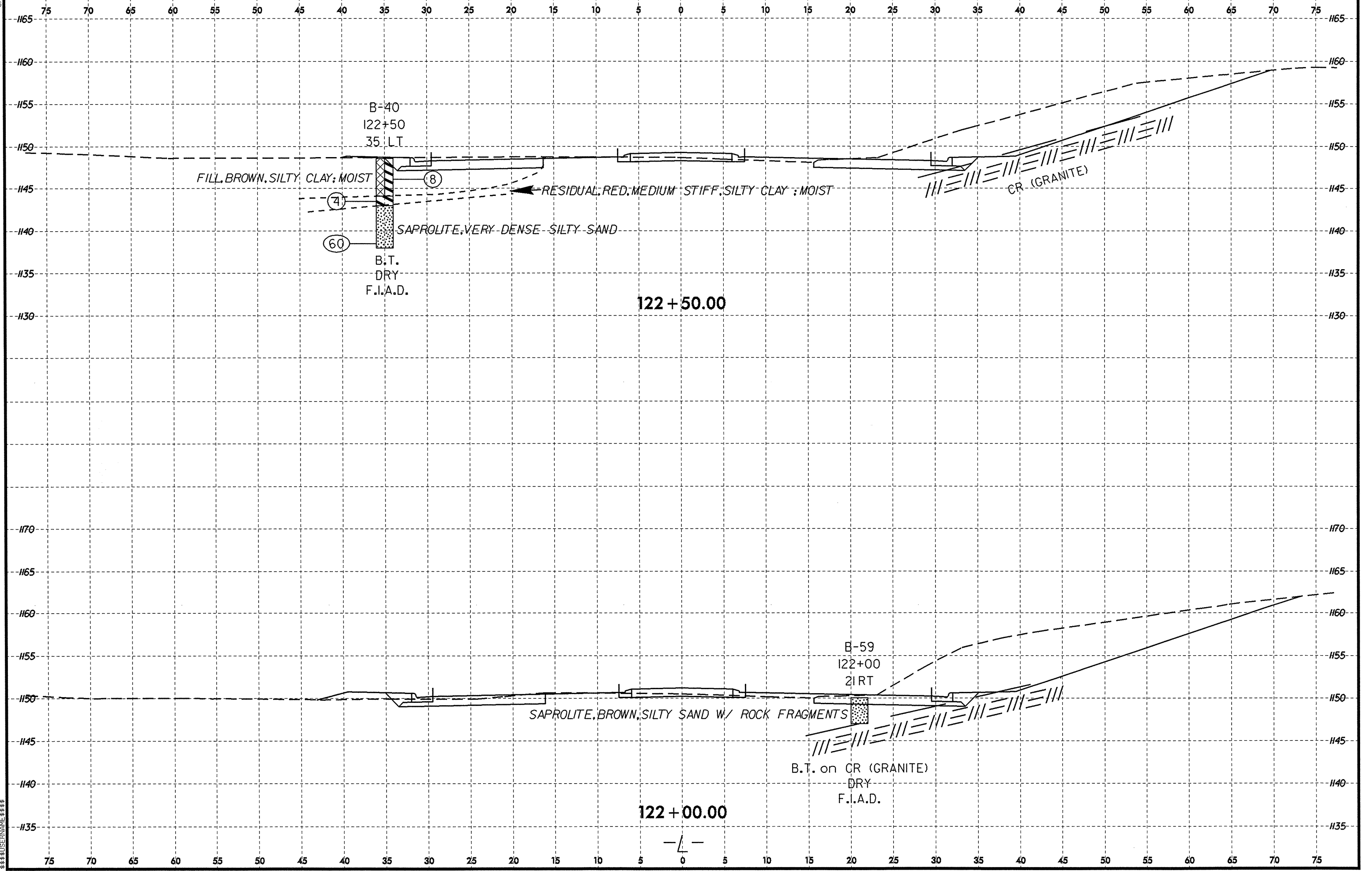
-L-

09-APR-2009 09:33  
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8/23/99

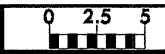


PROJ. REFERENCE NO.	SHEET NO.
U-2551	51/73

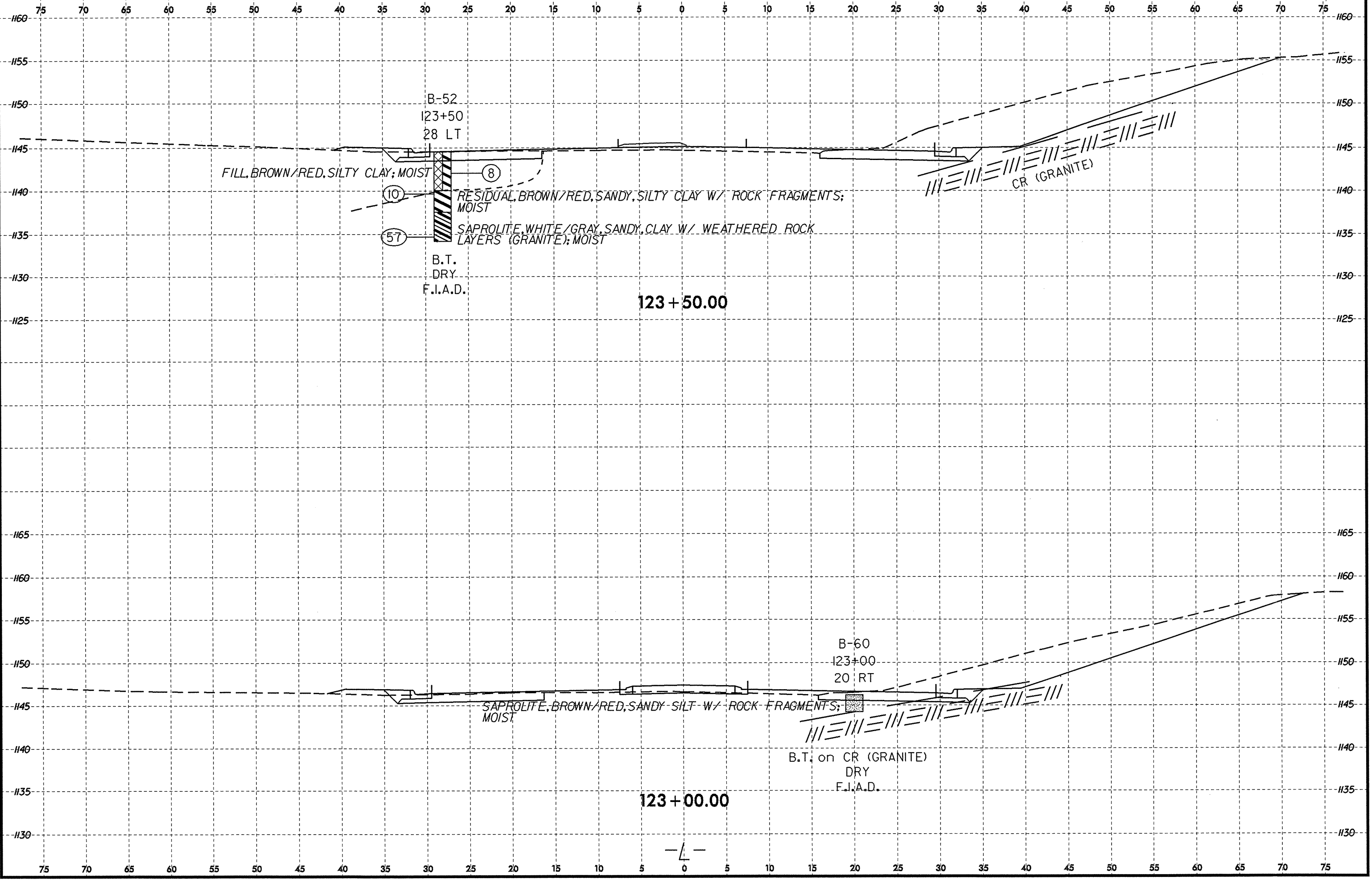


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\$\$\$\$USERNAME\$\$\$\$

8/23/99



PROJ. REFERENCE NO. U-2551 SHEET NO. 52/73



B-52  
123+50  
28 LT

FILL, BROWN/RED, SILTY CLAY; MOIST

(8)

(10)

RESIDUAL, BROWN/RED, SANDY, SILTY CLAY W/ ROCK FRAGMENTS;  
MOIST

(57)

SAPROLITE, WHITE/GRAY, SANDY, CLAY W/ WEATHERED ROCK  
LAYERS (GRANITE); MOIST

B.T.  
DRY  
F.I.A.D.

123 + 50.00

B-60  
123+00  
20 RT

SAPROLITE, BROWN/RED, SANDY SILT W/ ROCK FRAGMENTS;  
MOIST

B.T. on CR (GRANITE)  
DRY  
F.I.A.D.

123 + 00.00

14-APR-2009 11:10 D:\P\0\03\U2551\_GEO\_ROWY\_INW\CADD\_GEDTECH\PlanPr\of\U2551\_GEO\_xst\_1.dgn \$\$\$USERNAME\$\$\$

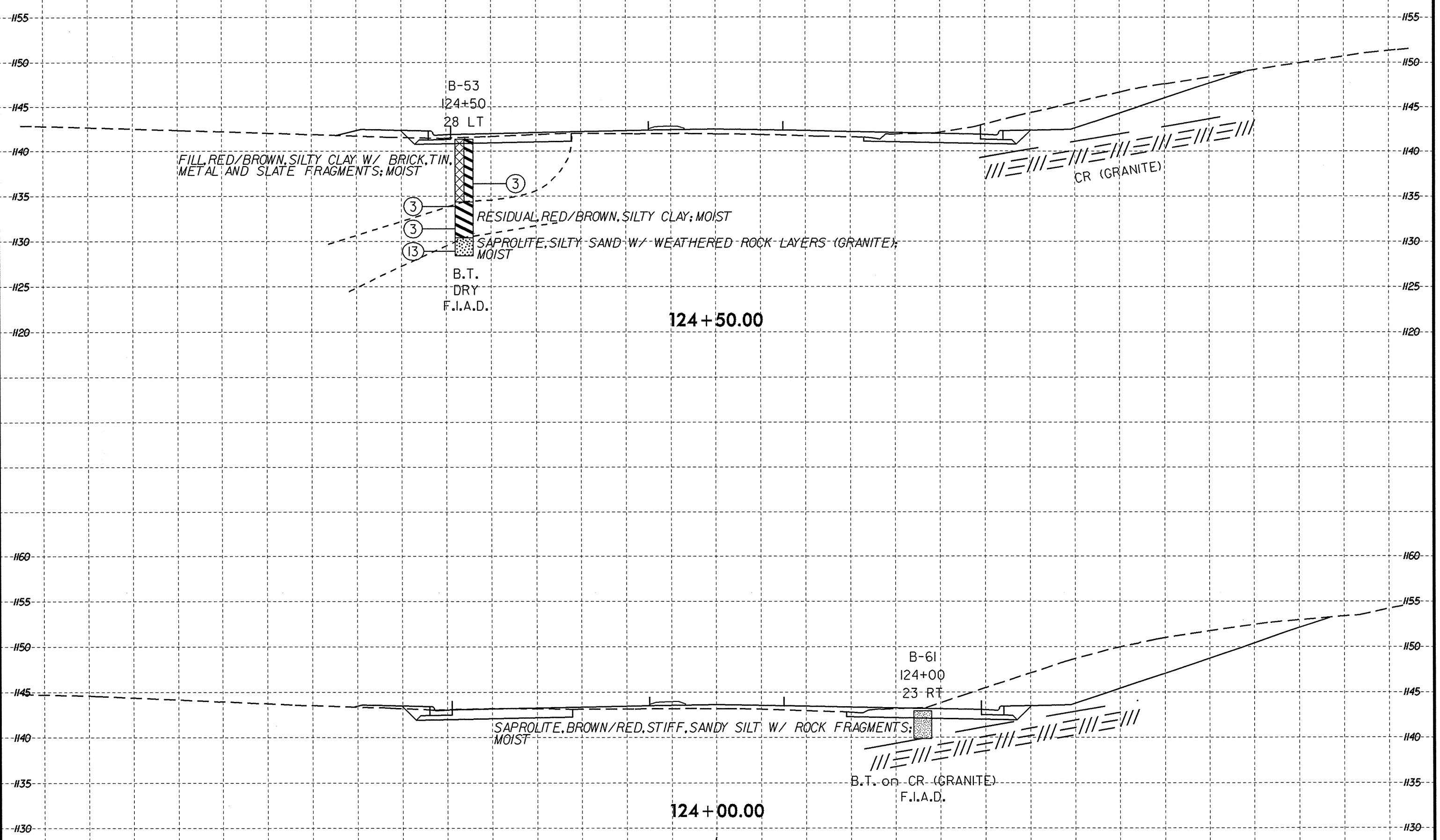
8/23/99



PROJ. REFERENCE NO.  
U-2551

SHEET NO.  
33/73

75 70 65 60 55 50 45 40 35 30 25 20 15 10 5 0 5 10 15 20 25 30 35 40 45 50 55 60 65 70 75



FILL, RED/BROWN, SILTY CLAY W/ BRICK, TIN, METAL AND SLATE FRAGMENTS; MOIST

B-53  
124+50  
28 LT

③

RESIDUAL, RED/BROWN, SILTY CLAY; MOIST

③

③

⑬

SAPROLITE, SILTY SAND W/ WEATHERED ROCK LAYERS (GRANITE); MOIST

B.T.  
DRY  
F.I.A.D.

124 + 50.00

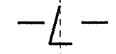
CR (GRANITE)

B-61  
124+00  
23 RT

SAPROLITE, BROWN/RED, STIFF, SANDY SILT W/ ROCK FRAGMENTS; MOIST

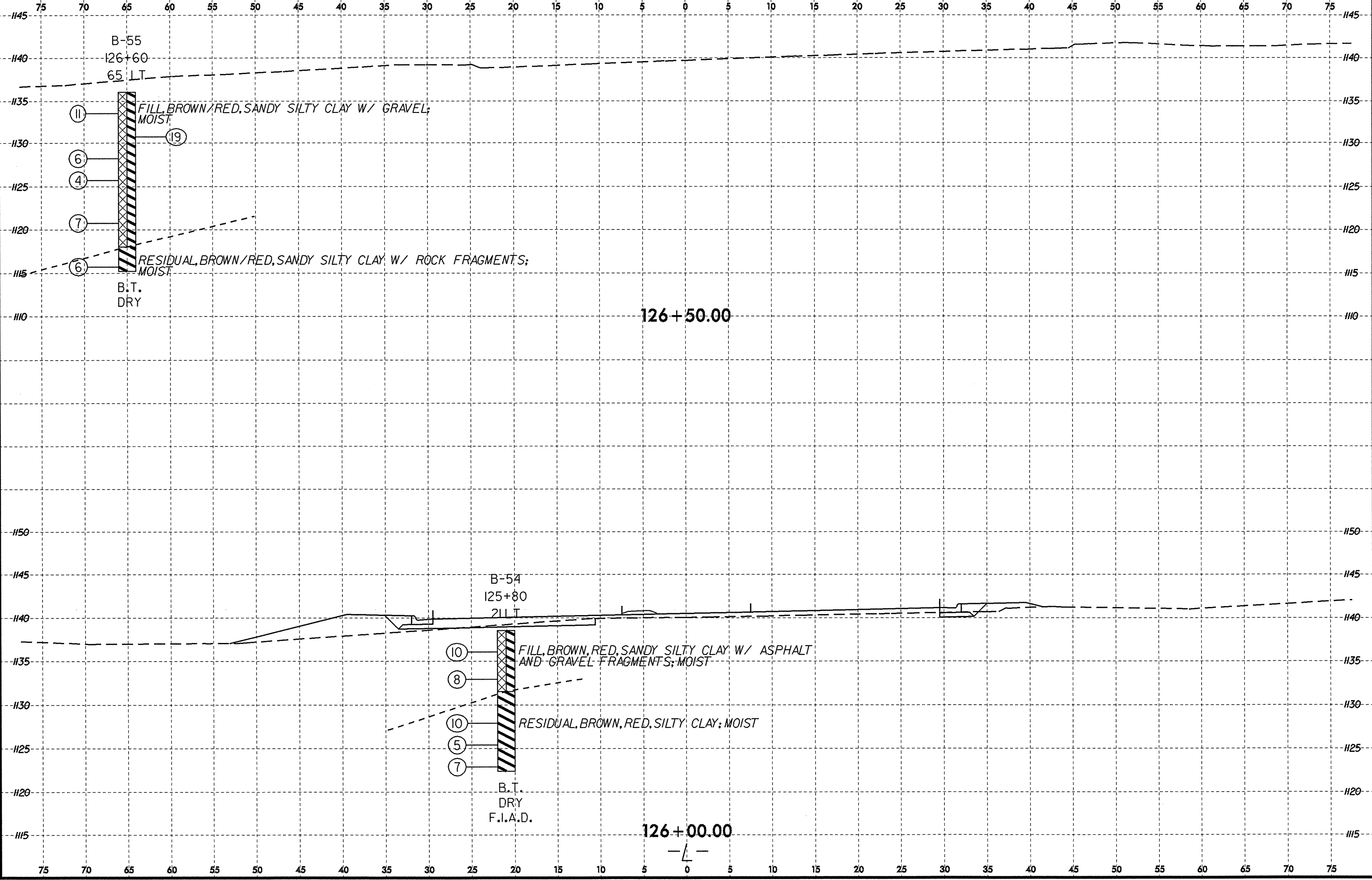
B.T. on CR (GRANITE)  
F.I.A.D.

124 + 00.00



09-APR-2003 09:40  
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\$\$\$\$\$USERNAME\$\$\$\$\$

8/23/99



09-APR-2009 09:40 D:\P\2551\GEO\_RDWY\_CADD\_GEDTECHN\Plan\Prof\U2551\_GEO\_xst.dgn \$\$\$USERNAME\$\$\$

126+50.00

126+00.00

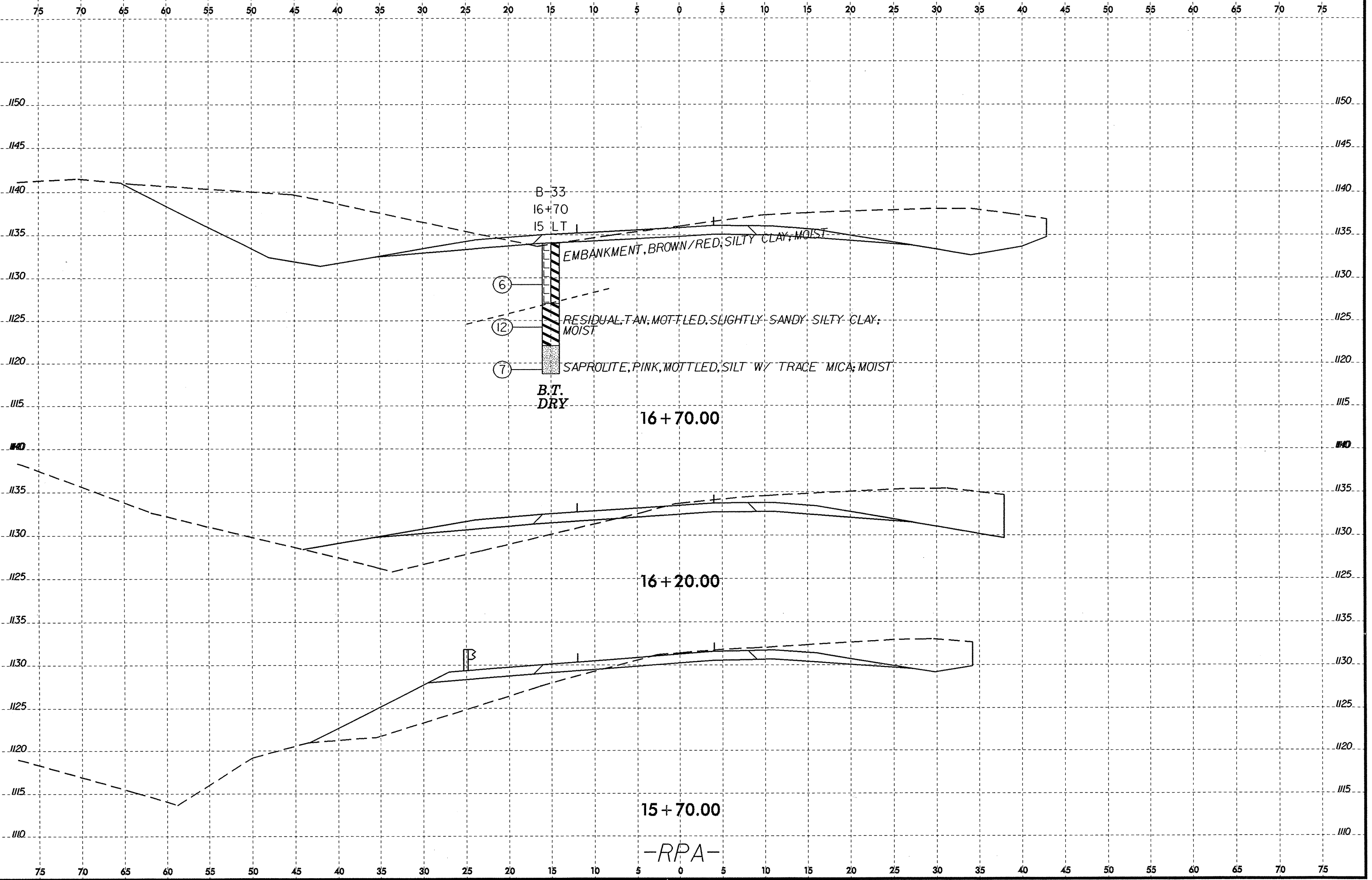
-L-

8/23/99

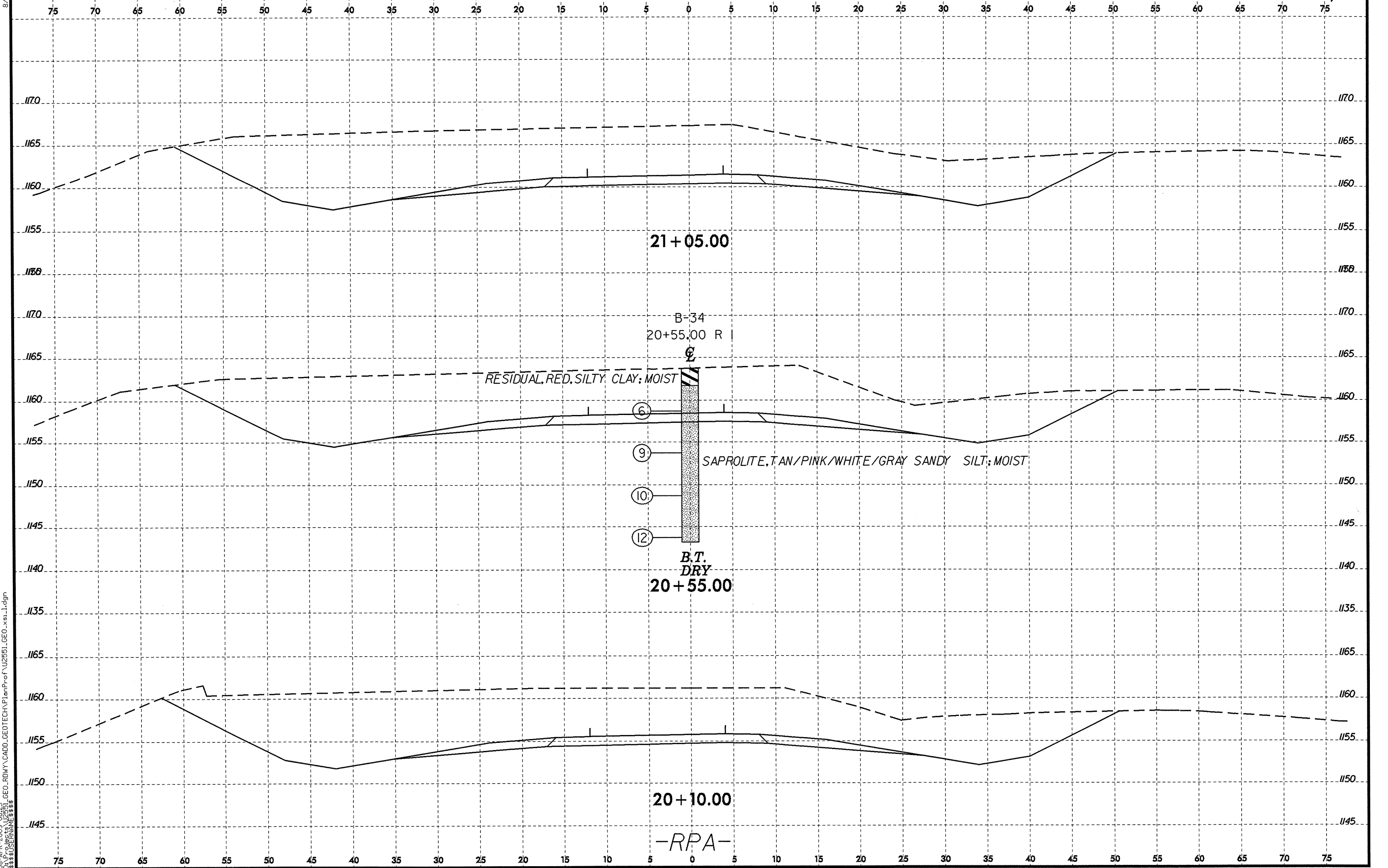


PROJ. REFERENCE NO.  
U-2551

SHEET NO.  
55

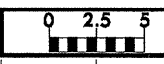


15 JUN 2009 14:55  
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3:33 PM GERRARD

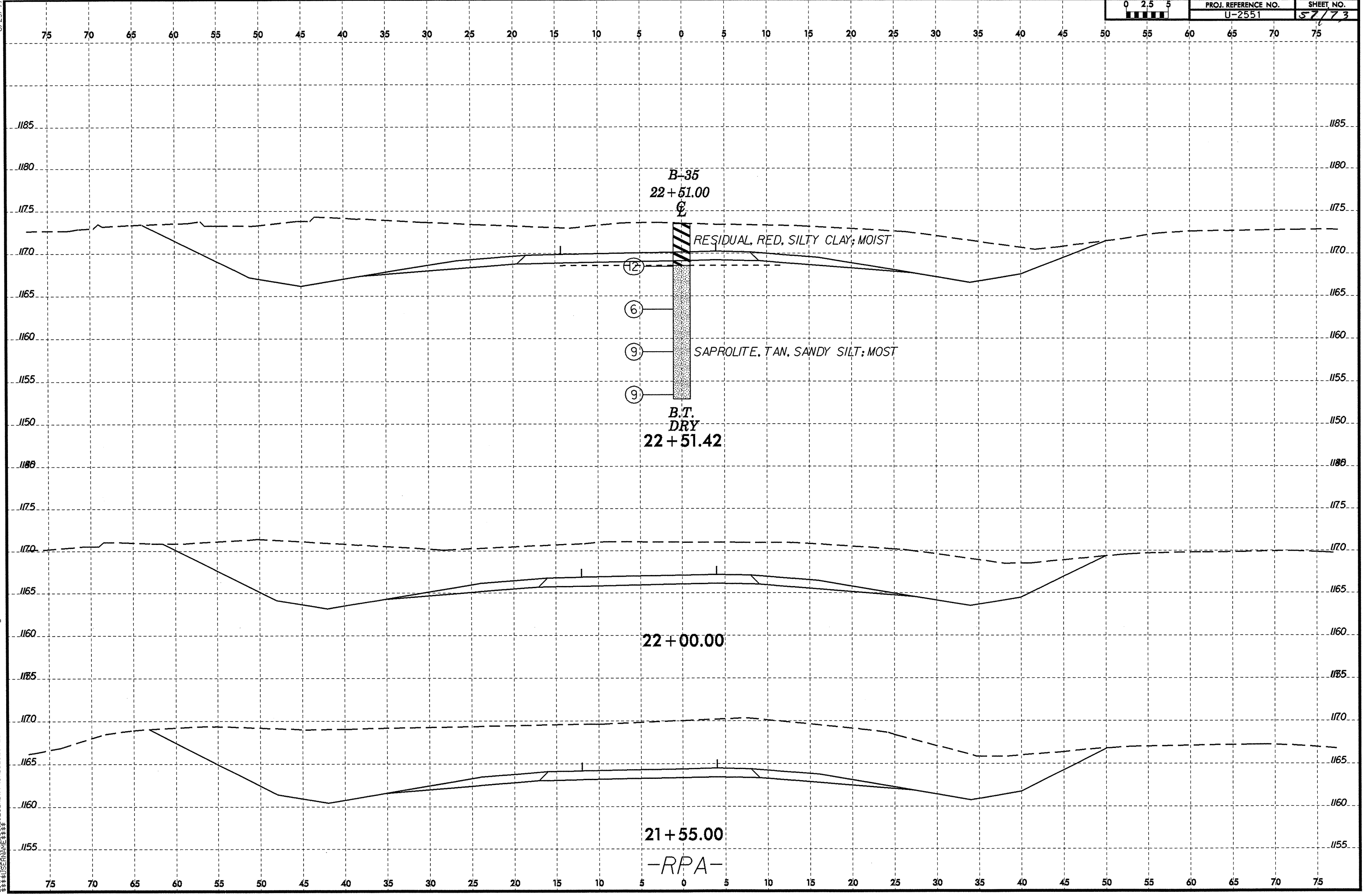


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8/23/99



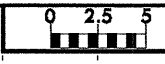
PROJ. REFERENCE NO.	SHEET NO.
U-2551	57/73



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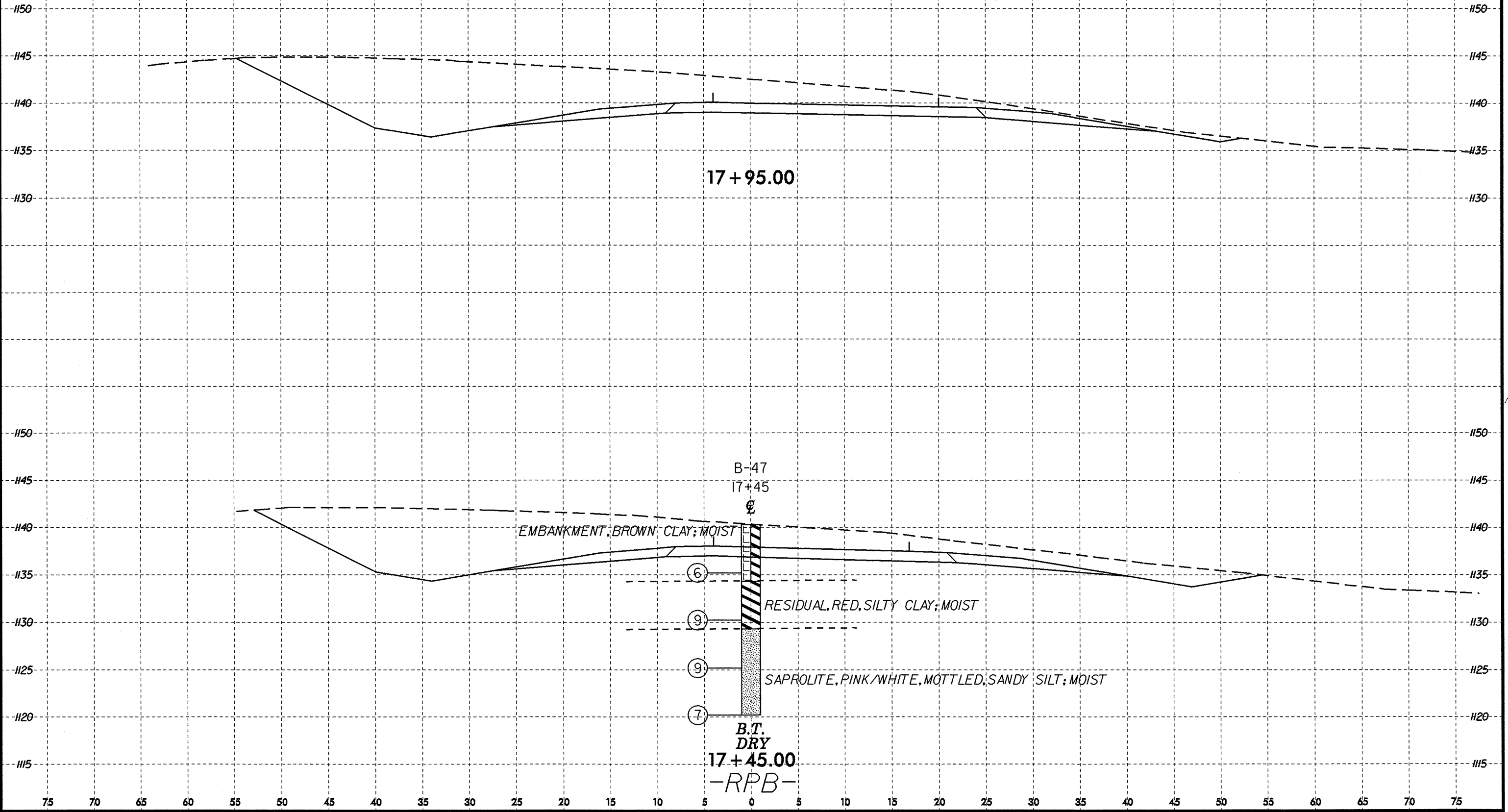


8/23/99



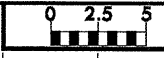
PROJ. REFERENCE NO. U-2551  
SHEET NO. 58/73

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01-APR-2009 13:17  
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USER:RWB

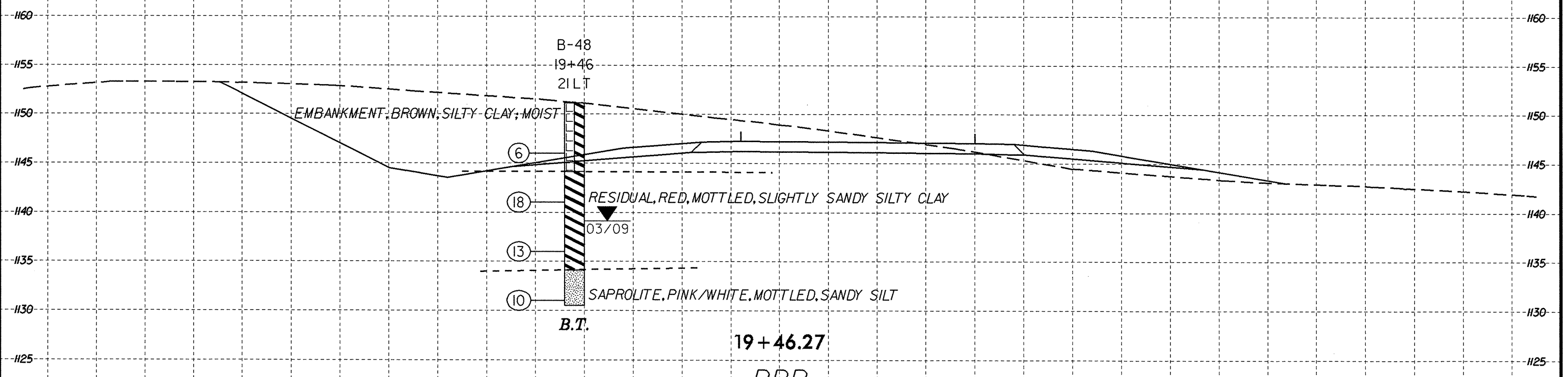
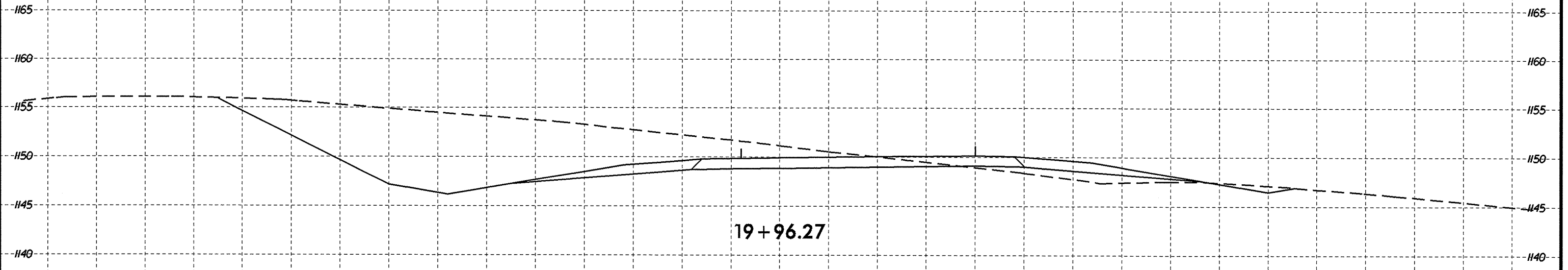
8/23/99



PROJ. REFERENCE NO.  
U-2551

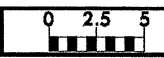
SHEET NO.  
59/73

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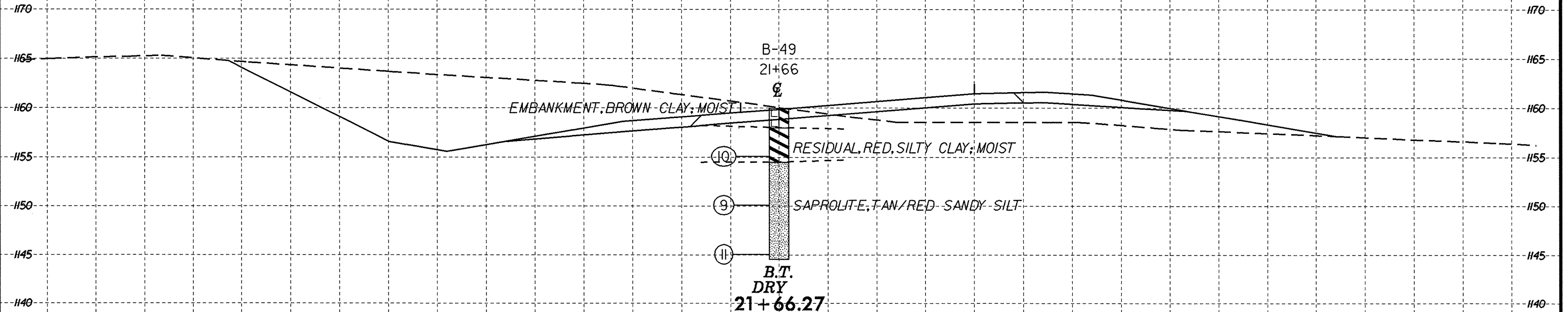
8/23/99



PROJ. REFERENCE NO.  
U-2551

SHEET NO.  
60/73

75 70 65 60 55 50 45 40 35 30 25 20 15 10 5 0 5 10 15 20 25 30 35 40 45 50 55 60 65 70 75



B-49

21+66

⊕

EMBANKMENT, BROWN CLAY, MOIST

RESIDUAL, RED, SILTY CLAY, MOIST

SAPROLITE, TAN/RED SANDY SILT

10

9

11

B.T.  
DRY

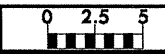
21+66.27

-RPB-

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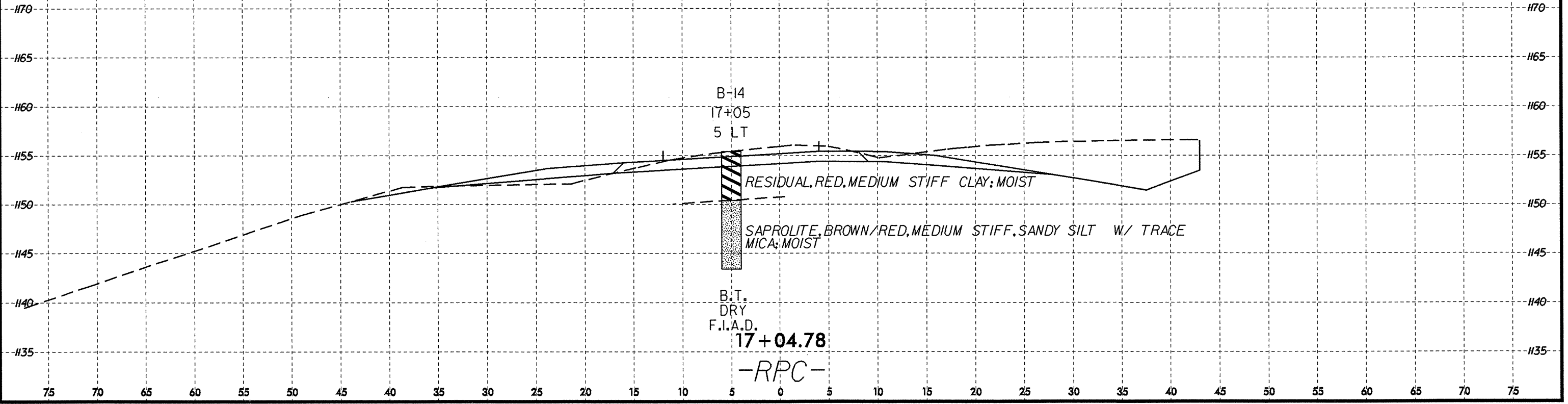
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8/23/99



PROJ. REFERENCE NO. U-2551 SHEET NO. 61/73

75 70 65 60 55 50 45 40 35 30 25 20 15 10 5 0 5 10 15 20 25 30 35 40 45 50 55 60 65 70 75



B-14  
17+05  
5 LT

RESIDUAL, RED, MEDIUM STIFF CLAY; MOIST

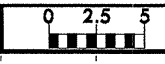
SAPROLITE, BROWN/RED, MEDIUM STIFF, SANDY SILT W/ TRACE MICA; MOIST

B.T.  
DRY  
F.I.A.D.  
17+04.78

-RPC-

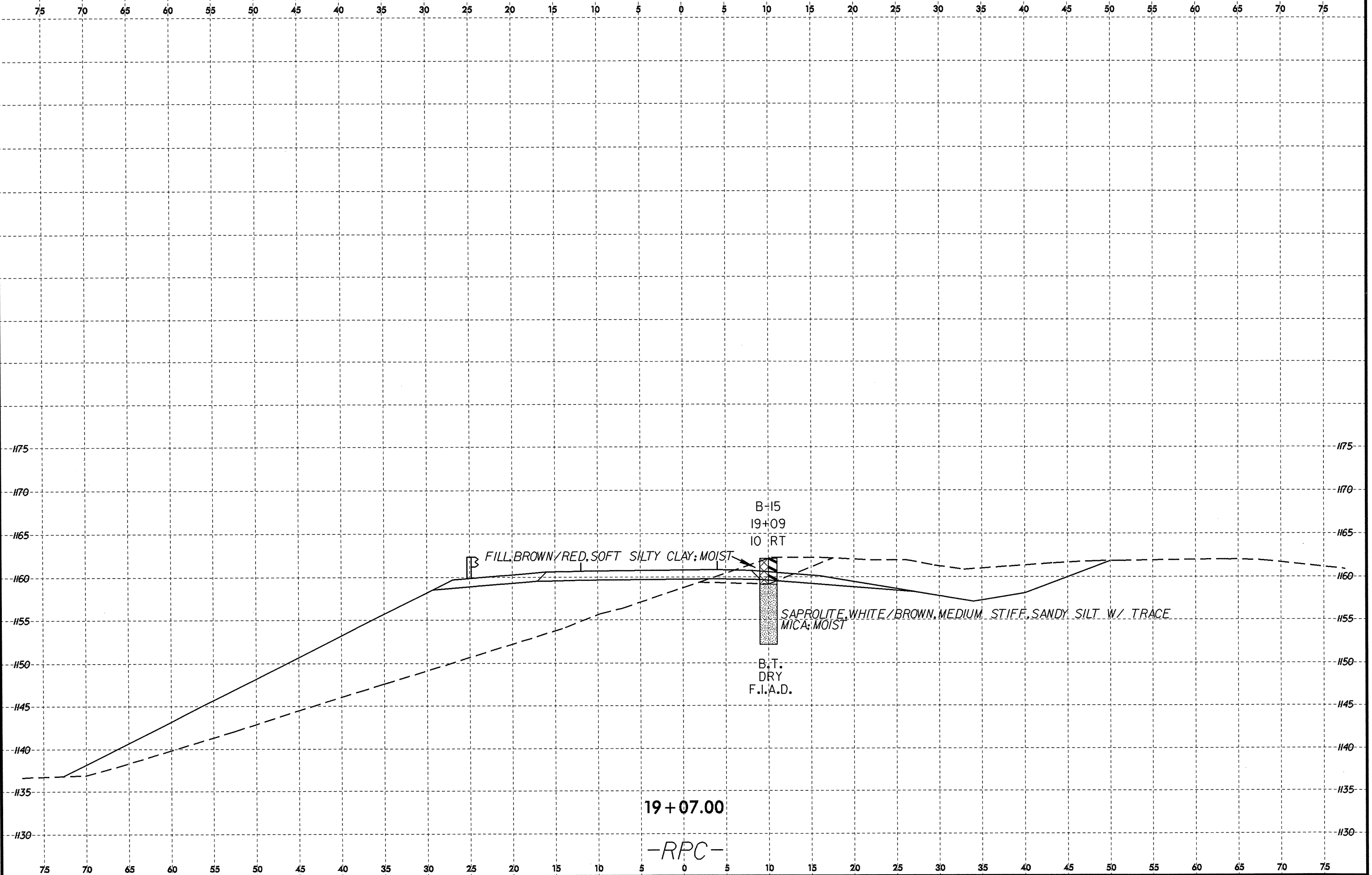
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8/23/99



PROJ. REFERENCE NO.  
U-2551

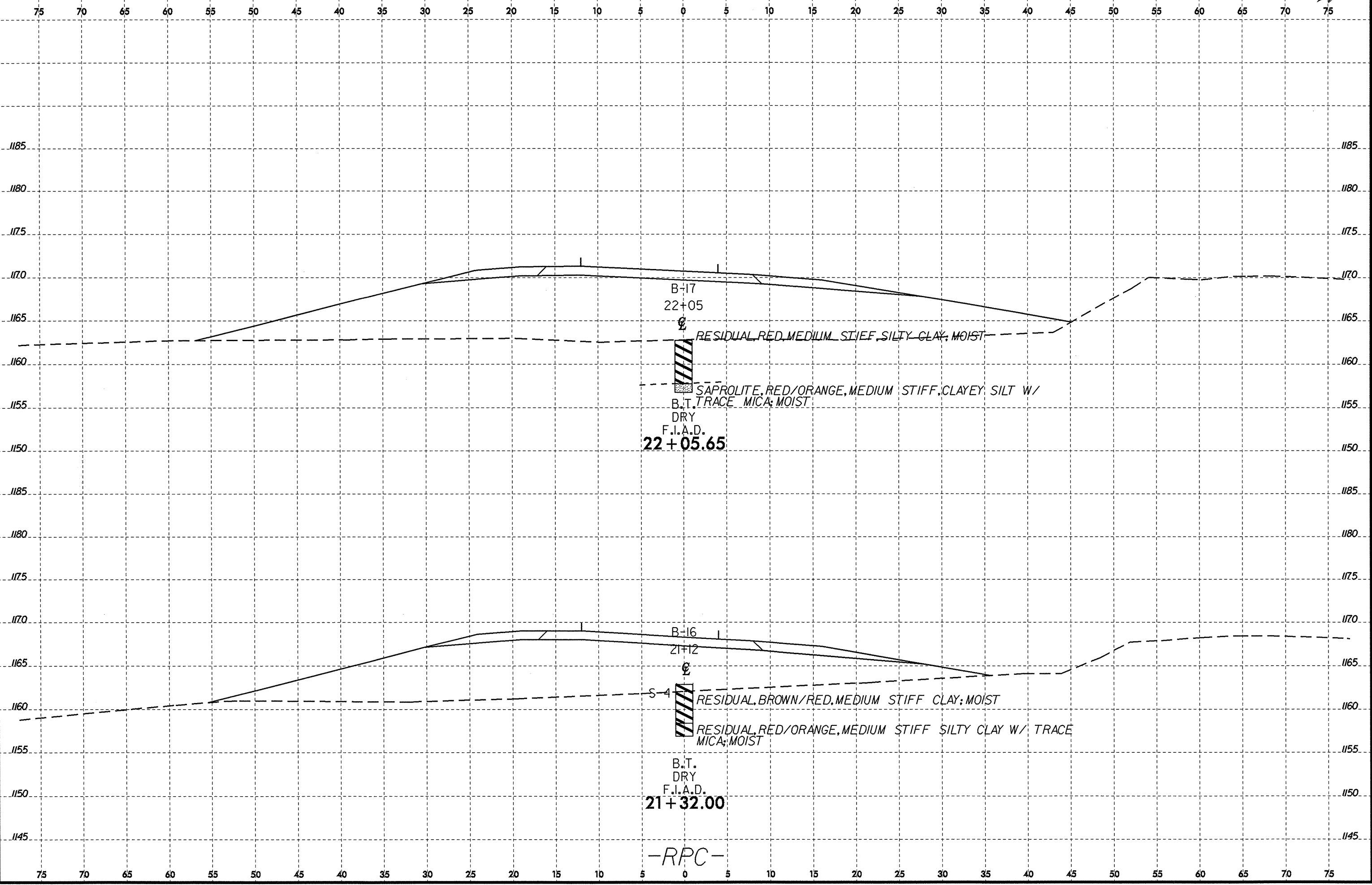
SHEET NO.  
62/72



19 + 07.00

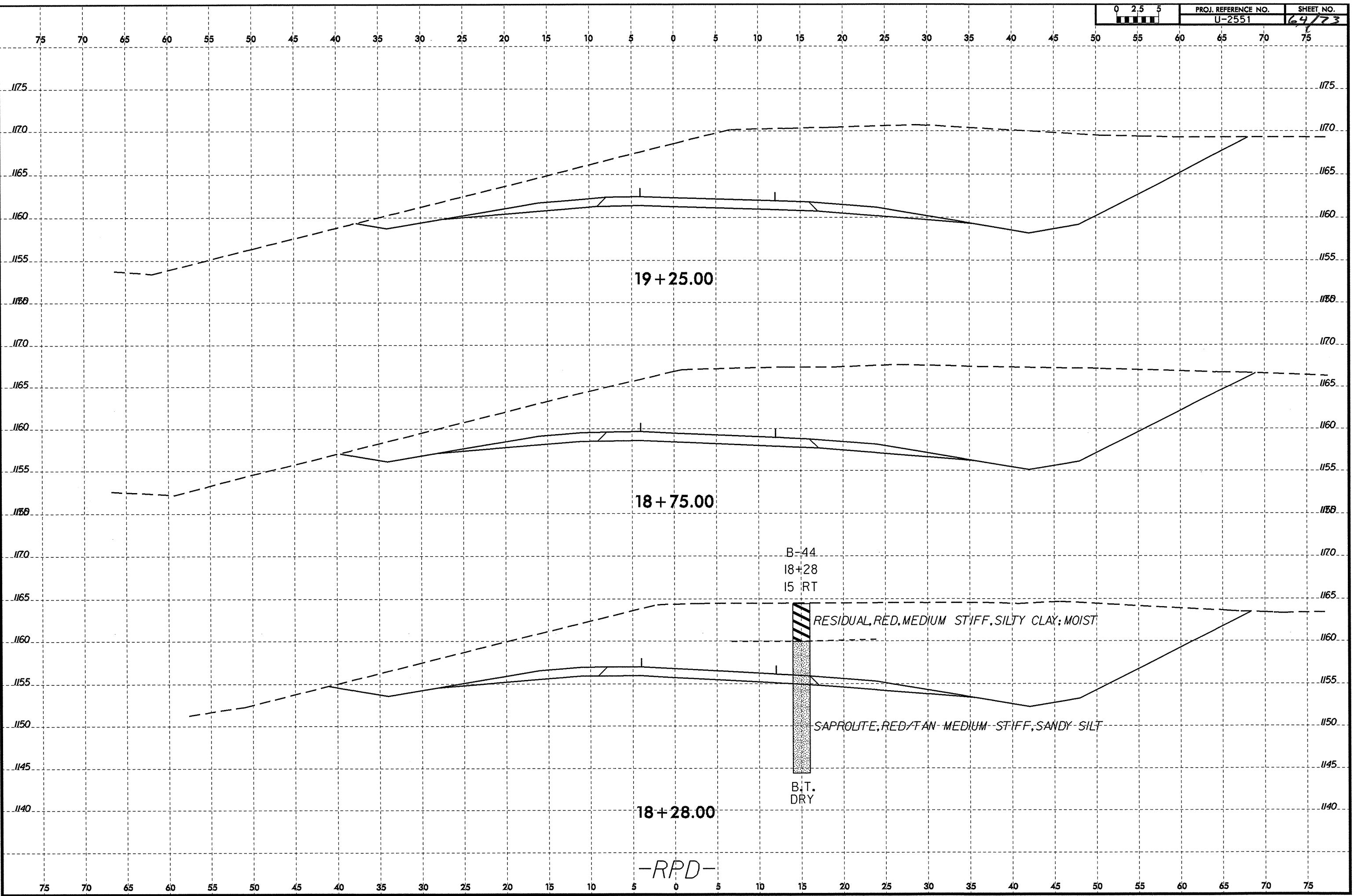
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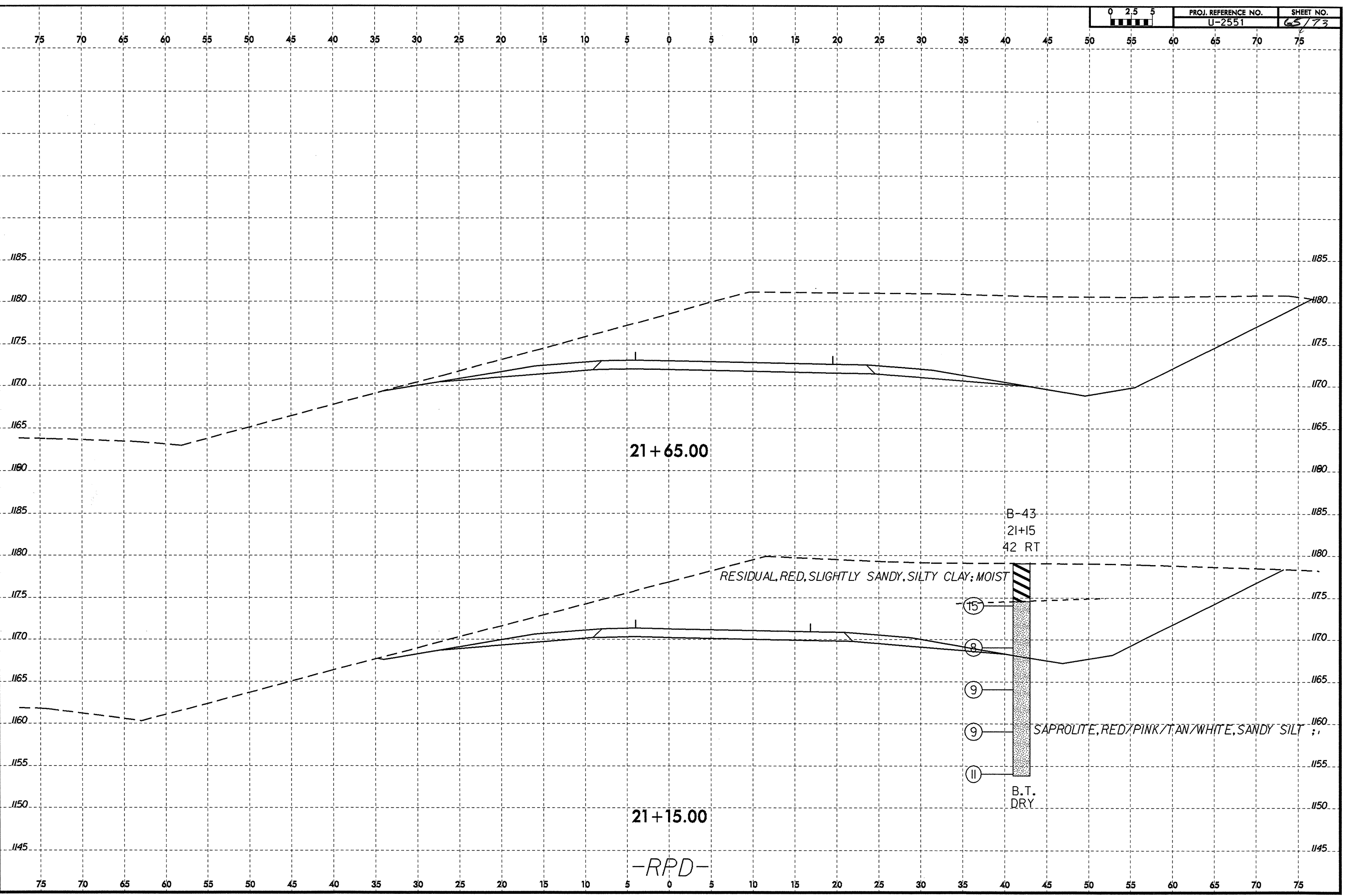


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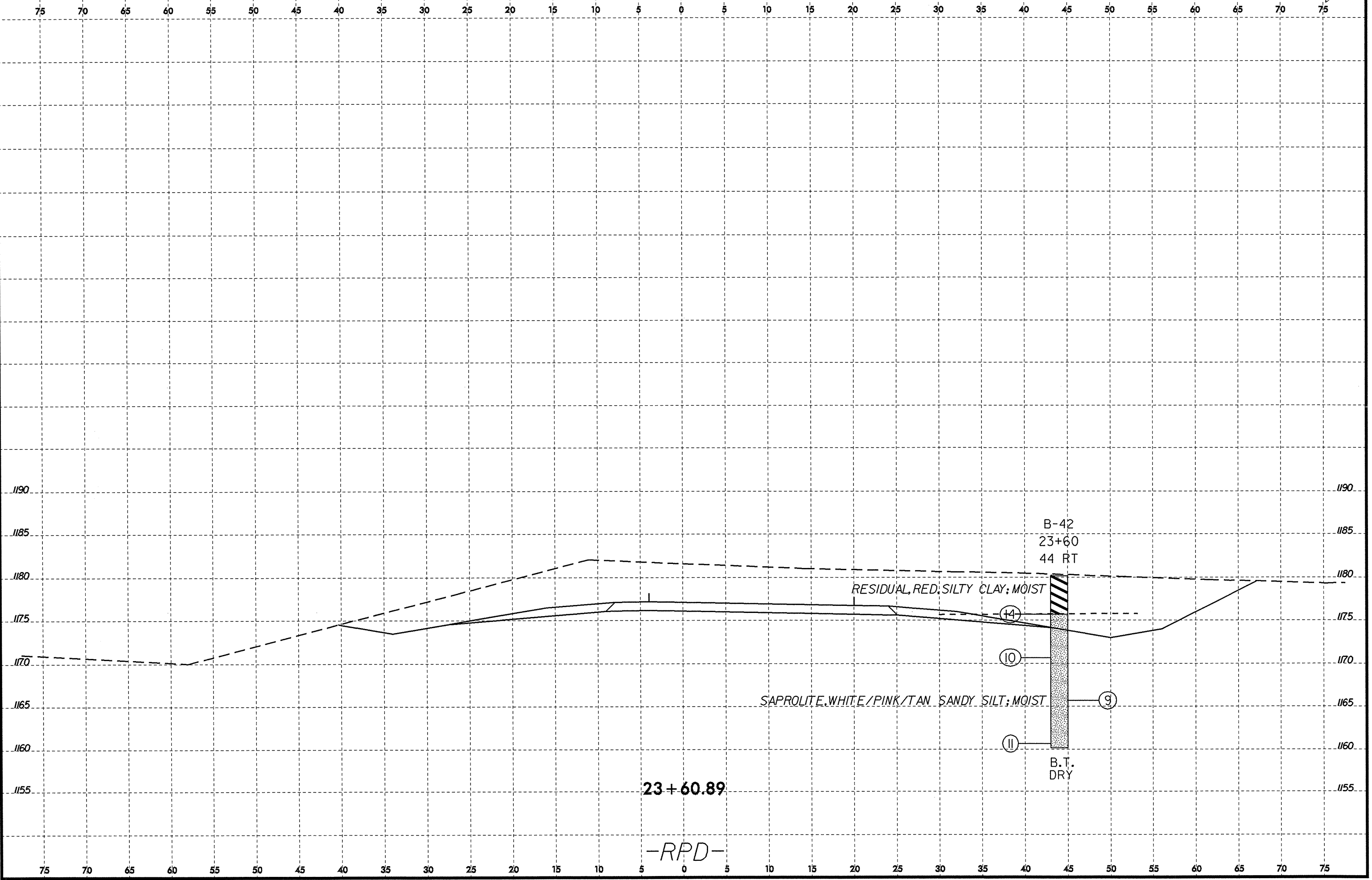


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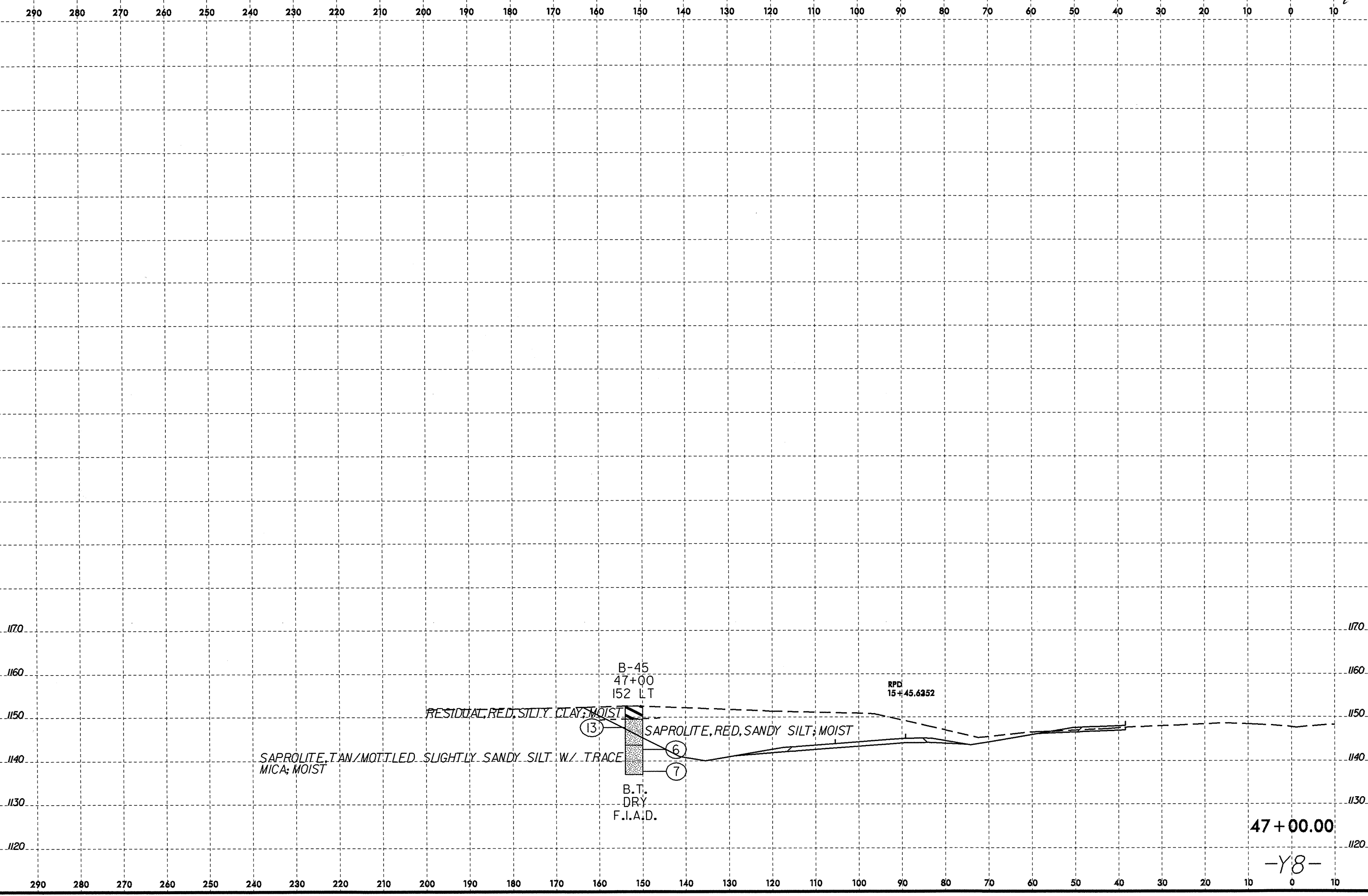




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8/23/99

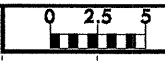


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47 + 00.00

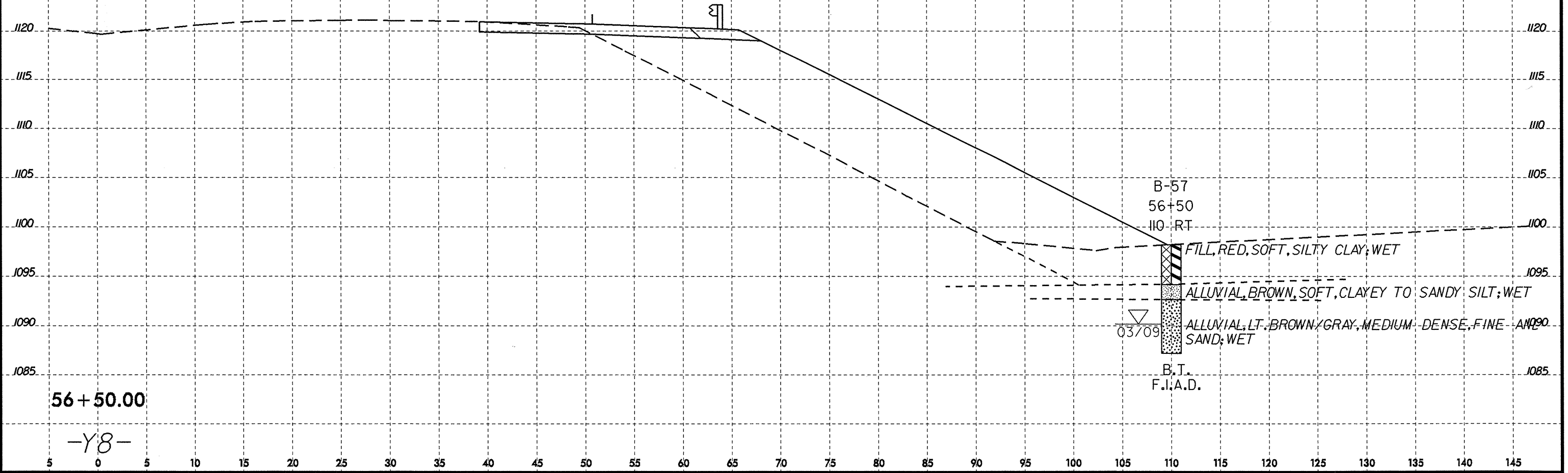
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8/23/99



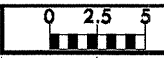
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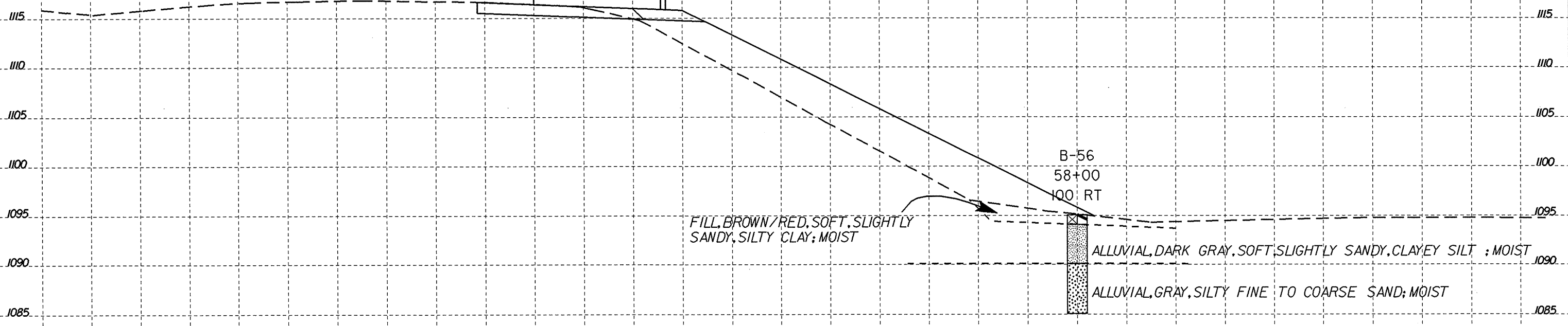
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8/23/99



PROJ. REFERENCE NO. U-2551 SHEET NO. 69/73

5 0 5 10 15 20 25 30 35 40 45 50 55 60 65 70 75 80 85 90 95 100 105 110 115 120 125 130 135 140 145



FILL, BROWN/RED, SOFT, SLIGHTLY SANDY, SILTY CLAY; MOIST

B-56  
58+00  
100 RT

ALLUVIAL, DARK GRAY, SOFT, SLIGHTLY SANDY, CLAYEY SILT; MOIST

ALLUVIAL, GRAY, SILTY FINE TO COARSE SAND; MOIST

B.T.

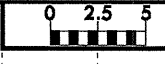
58+00.00

-Y8-

5 0 5 10 15 20 25 30 35 40 45 50 55 60 65 70 75 80 85 90 95 100 105 110 115 120 125 130 135 140 145

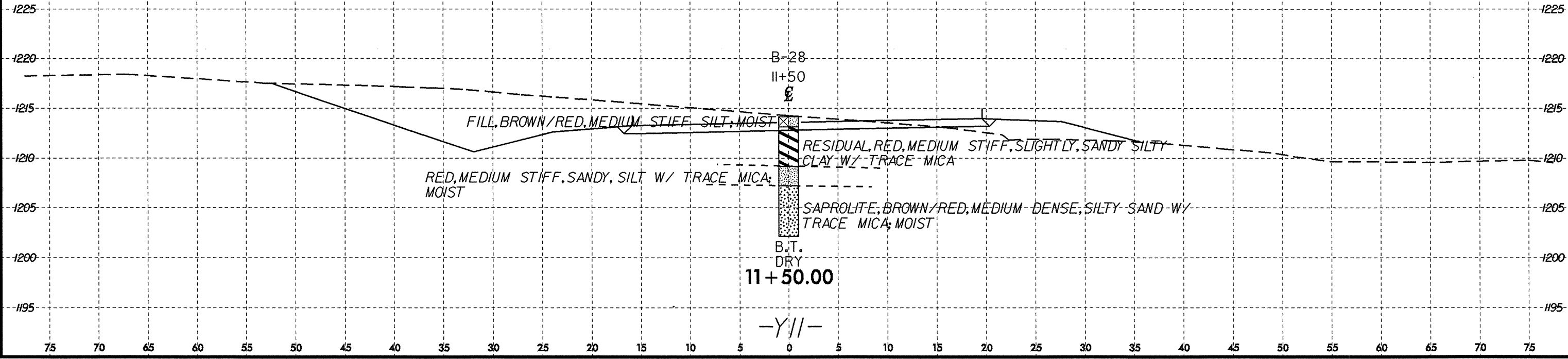
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8/23/99



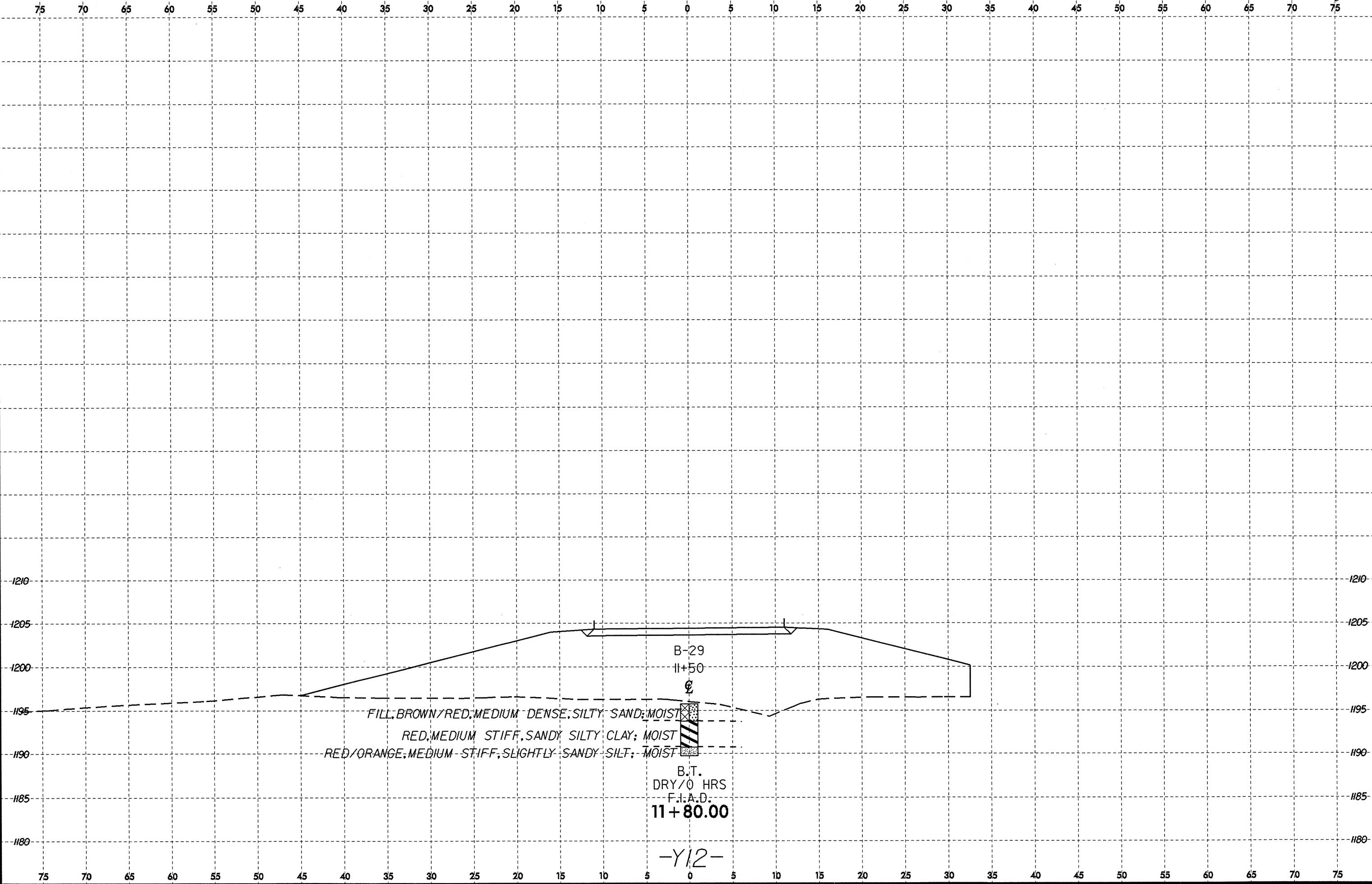
PROJ. REFERENCE NO. U-2551 SHEET NO. 70/73

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8/23/99



FILL, BROWN/RED, MEDIUM DENSE, SILTY SAND; MOIST  
 RED, MEDIUM STIFF, SANDY SILTY CLAY; MOIST  
 RED/ORANGE, MEDIUM-STIFF, SLIGHTLY SANDY SILT; MOIST

B-29  
 11+50  
 Ⓞ  
 B.T.  
 DRY/0 HRS  
 F.I.A.D.  
 11+80.00

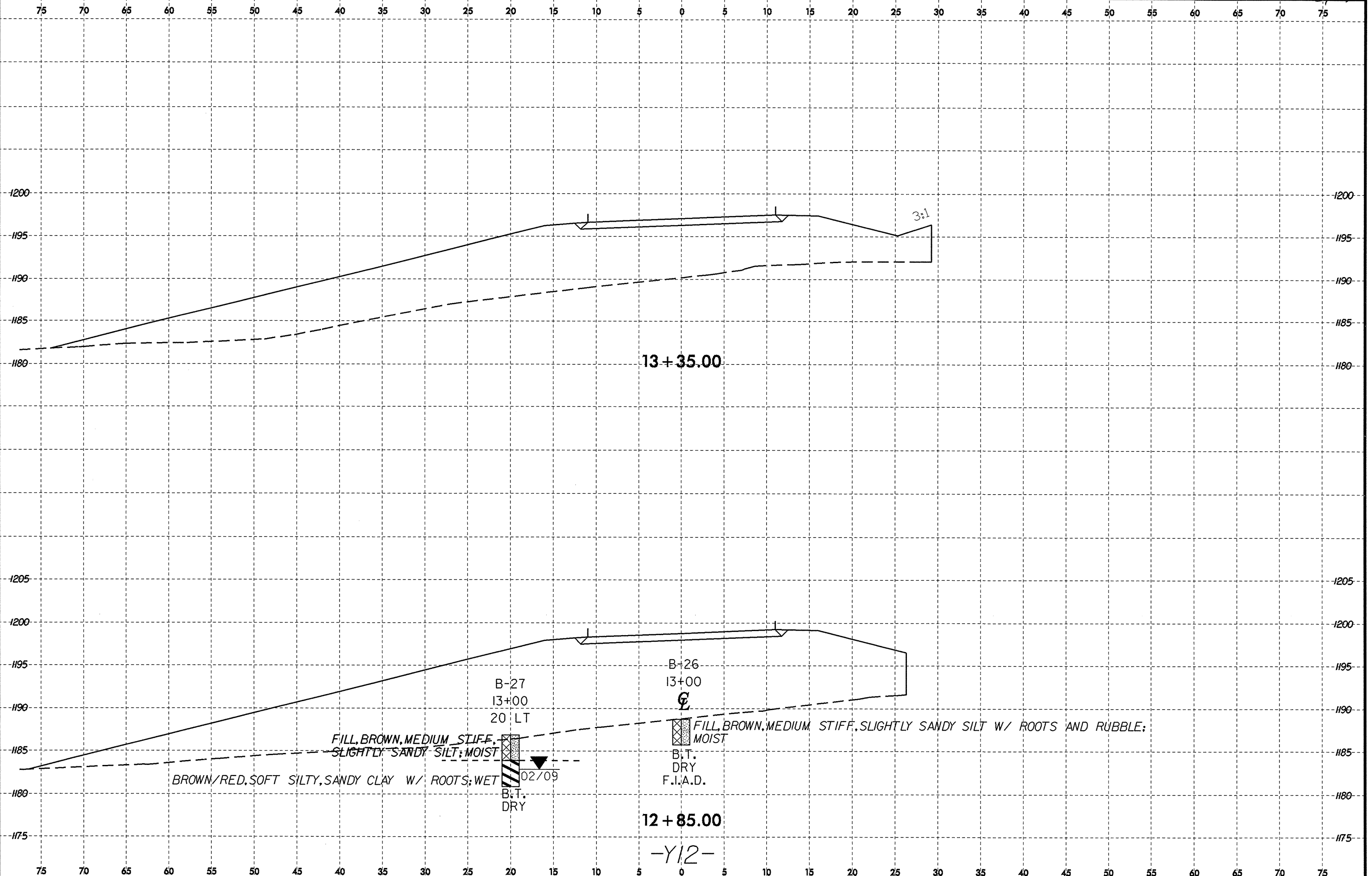
-Y12-

06-APR-2009 10:09  
 D:\PROJECTS\U2551\_GEO\_FDWY\CADD\_GEOTECH\PlanProf\U2551\_GEO\_xsl.dgn  
 \$\$\$USERNAME\$\$\$

8/23/99



PROJ. REFERENCE NO. U-2551 SHEET NO. 22/73



13 + 35.00

3:1

12 + 85.00

-Y12-

FILL, BROWN, MEDIUM STIFF, SLIGHTLY SANDY SILT; MOIST

BROWN/RED, SOFT SILTY, SANDY CLAY W/ ROOTS; WET

B.T. DRY

B-27  
13+00  
20 LT

02/09

B-26  
13+00

FILL, BROWN, MEDIUM STIFF, SLIGHTLY SANDY SILT W/ ROOTS AND RUBBLE; MOIST

B.T. DRY  
F.I.A.D.

01-APR-2009 13:45 D:\PROJECTS\2009\U2551\GEO\RDWY\CADD\GEO\TECH\PlanProf\U2551\_GEO\_x.s...1.dgn

JCS  
 NORTH CAROLINA DEPARTMENT OF TRANSPORTATION  
 DIVISION OF HIGHWAYS-MATERIALS AND TESTS UNIT  
 SOILS TEST REPORT-SOILS LABORATORY

T.I.P. ID #: N/A

REPORT ON SAMPLES OF: Soils for Quality

PROJECT:	U-2551	COUNTY:	Burke	Owner:	NCDOT
DATE SAMPLED:	2.09	DATE RECEIVED:	2.27.09	DATE REPORTED:	3.5.09
SAMPLED FROM:	Roadway	SAMPLED BY:	P. Q. Lockamy		
SUBMITTED BY:	W. D. Frye	2002	STANDARD SPECIFICATION		
LABORATORY:	Asheville				

**TEST RESULTS**

Project Sample No.	S-1	SS-1	S-2	SS-2	S-3	SS-3	S-4	SS-4
Lab Sample No. A	159940	159941	159942	159943	159944	159945	159946	159947
HiCAMS Sample #	--	--	--	--	--	--	--	--
Retained #4 Sieve %	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0
Passing #10 Sieve %	99	100	100	100	100	100	100	99
Passing #40 Sieve %	89	98	92	86	87	91	94	81
Passing #200 Sieve %	73	85	73	45	37	52	83	45

**MINUS #10 FRACTION**

Soil Mortar - 100%								
Coarse Sand -Ret. #60	16	5	15	28	28	18	10	30
Fine Sand - Ret. #270	11	15	14	35	41	40	8	32
Silt 0.05-0.005 mm %	13	32	21	29	27	34	14	30
Clay < 0.005 mm %	60	48	50	8	4	8	68	8
Passing # 40 Sieve %	--	--	--	--	--	--	--	--
Passing # 200 Sieve %	--	--	--	--	--	--	--	--

Liquid Limit	57	60	58	34	38	34	57	38
Plastic Index	25	24	25	NP	NP	NP	28*	NP
AASHTO Classification	A-7-5 (17)	A-7-5 (18)	A-7-5 (17)	A-4 (2)	A-4 (0)	A-4 (3)	A-7-6 (19)	A-4 (2)
Quantity								
Texture								
Station	29+00	77+34	52+00	77+35	52+00	77+35	21+12	77+35
Hole No.								
Depth (ft) From:	1.8	4.3	1.0	8.7	9.5	18.7	0.0	38.7
To:	3.5	5.2	4.0	10.2	10.5	20.2	4.5	40.2
	OK	OK	OK	OK	OK	OK	See	OK
							Remarks	

**Remarks:**

A-159940 – 159947; \* Acceptable, but not to be used in top 2 feet of embankment or backfill.

**CC:**

P. Q. Lockamy	
File	

SOILS ENGINEER:

JCS  
 NORTH CAROLINA DEPARTMENT OF TRANSPORTATION  
 DIVISION OF HIGHWAYS-MATERIALS AND TESTS UNIT  
 SOILS TEST REPORT-SOILS LABORATORY

T.I.P. ID #: N/A

REPORT ON SAMPLES OF: Soils for Quality

PROJECT:	U-2551	COUNTY:	Burke	Owner:	NCDOT
DATE SAMPLED:	2.09	DATE RECEIVED:	2.27.09	DATE REPORTED:	3.5.09
SAMPLED FROM:	Roadway	SAMPLED BY:	P. Q. Lockamy		
SUBMITTED BY:	W. D. Frye	2002	STANDARD SPECIFICATION		
LABORATORY:	Asheville				

**TEST RESULTS**

Project Sample No.	S-5	S-6						
Lab Sample No. A	159948	159949						
HiCAMS Sample #	--	--						
Retained #4 Sieve %	0.0	0.0						
Passing #10 Sieve %	100	100						
Passing #40 Sieve %	91	84						
Passing #200 Sieve %	63	46						

**MINUS #10 FRACTION**

Soil Mortar - 100%								
Coarse Sand -Ret. #60	19	29						
Fine Sand - Ret. #270	21	30						
Silt 0.05-0.005 mm %	18	19						
Clay < 0.005 mm %	42	22						
Passing # 40 Sieve %	--	--						
Passing # 200 Sieve %	--	--						

Liquid Limit	52	40						
Plastic Index	19	NP						
AASHTO Classification	A-7-5 (11)	A-4 (2)						
Quantity								
Texture								
Station	99+00	99+00						
Hole No.								
Depth (ft) From:	1.0	4.0						
To:	2.0	5.0						
	OK	OK						

**Remarks:**

A-159948 & 159949

**CC:**

P. Q. Lockamy	
File	

SOILS ENGINEER: