## STATE OF NORTH CAROLINA

DEPARTMENT OF TRANSPORTATION
DIVISION OF HIGHWAYS
GEOTECHNICAL ENGINEERING UNIT

## **CONTENTS**

<u>SHEET</u>	<b>DESCRIPTION</b>
1	TITLE SHEET
2	LEGEND
3 .	SITE PLAN
4	CROSS SECTIONS
5-7	BORE LOG & CORE REPORTS
8	SCOUR REPORT
9	CORE PHOTOGRAPHS

# STRUCTURE SUBSURFACE INVESTIGATION

PROJ. REFERENCE NO	38621.1.1 B-4851	F.A. PROJ. <i>BRZ-13</i>	98(6)
COUNTY YANCEY			
PROJECT DESCRIPTION	BRIDGE NO. 31 ON	N SR 1308 OVER	
BRUSH CREEK	· .		
SITE DESCRIPTION			
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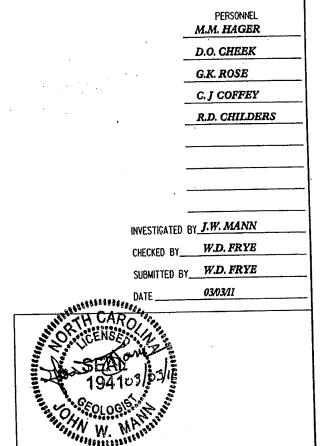
STATE	STATE PROJECT REFERENCE NO.	SHIBBT NO.	SHEETS	l
N.C.	38621.1.1 B-4851	1	9	l
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### CAUTION NOTICE

THE SUBSURFACE DEFORMATION AND THE SUBSURFACE INVESTIGATION ON WHICH IT IS BASED WERE MADE FOR THE PURPOSE OF STUDY, PLANNING, AND DESIGN, AND NOT FOR CONSTRUCTION OR PAY PURPOSES. THE VARIOUS FELD BORNING LOOS, ROCK CORES, AND SOL TEST DATA AVAILABLE MAY BE REVEWED OR INSPECTED IN RALEIGH BY CONTACTING THE N.C. DEPARTMENT OF TRAISFORTATION, ROTTED THE CONTENS OF THE PROPERTY. HOR THE FIELD BORNING LOOS, ROCK CORES, OR SOIL TEST DATA ARE PART OF THE CONTRACT.

CENERAL SOIL AND ROCK STRATA DESCRIPTIONS AND INDICATED BOUNDARIES ARE BASED ON A GEOTECHNICAL INTERPRETATION OF ALL AVAILABLE SUBSURFACE DATA AND MAY NOT NECESSARLY REFLECT THE ACTUAL SUBSURFACE CONDITIONS BETWEEN BORINGS OR BETWEEN SAMPLED STRATA WITHIN THE BOREHOLE, THE LABORATORY SAMPLE DATA AND THE IN STU HIN-PLACE ITEST OATA CAN BE RELED ON ONLY TO THE DEGREE OF RELABILITY BMERENT IN THE STRANDAR ISEST METHOD. THE OBSERVED WATER LEVELS OR SOIL MOSTURE CONDITIONS BURCATED IN THE SUBSPIACE INVESTIGATIONS ARE AS RECORDED AT THE TIME OF THE INVESTIGATION, THESE WATER LEVELS OR SOIL MOSTURE CONDITIONS DURCATED TO CLIMATIC CONDITIONS BURCATURES, PREOPITATION, AND WIND, AS WELL AS OTHER NON-CLNATIC FACTORS.

THE ODDER OR CONTRACTOR IS CAUTIONED THAT DETALS SHOWN ON THE SUBSUPFACE PLANS ARE PRELIMINARY ONLY AND IN MAIN CASES THE FIRML DESIGN DETAILS ARE DIFFERENT. FOR BUDDING AND CONSTRUCTION PURPOSES, REFER TO THE CONSTRUCTION PLANS AND DOCUMENTS FOR FIRML DESIGN REFORMATION ON THIS PROJECT. THE DEPARTMENT DOES NOT WARRANT OR GUARANTEE THE SUFFICIENCY OR ACCURACY OF THE RIVESTIGATION MADE, NOR THE HITEMPRETATIONS MADE, OR OPINION OF THE DEPARTMENT AS TO THE TYPE OF MATERIALS AND CONDITIONS TO BE ENCOUNTERED. THE BIDDER OR CONTRACTOR IS CAUTIONED TO MAKE SUCH MODERATION SUSPECTIONS AS A ED DEMISSION FACE INVESTIGATIONS AS HE DEEMS NECESSARY TO SATISTY HARSELF AS TO CONDITIONS TO BE ENCOUNTERED ON THIS PROJECT. THE CONTRACTOR SHALL HAVE NO CLAIM FOR ADDITIONAL COMPRESATION OF FOR AN EXTENSION OF TIME FOR ANY REASON RESULTION FROM THE ACTUAL CONDITIONS ENCOUNTERED AT THE SITE DIFFERING FROM THOSE MODICATED IN THE SUBSURFACE INFORMATION.



NOTE - THE INFORMATION CONTAINED HEREIN IS NOT IMPLIED OR GUARANTEED BY THE N.C. DEPARTMENT OF TRANSPORTATION AS BEING ACCURATE NOR IT IS CONSIDERED TO BE PART OF THE PLANS, SPECIFICATIONS, OR CONTRACT FOR THE PROJECT.

NOTE - BY HAVING REQUESTED THIS INFORMATION THE CONTRACTOR SPECIFICALLY WAIVES ANY CLAIMS
FOR INCREASED COMPENSATION OR EXTENSION OF TIME BASED ON DIFFERENCES BETWEEN THE
CONDITIONS INDICATED HEREIN AND THE ACTUAL CONDITIONS AT THE PROJECT SITE.

SHEET NO. 2 OF 9

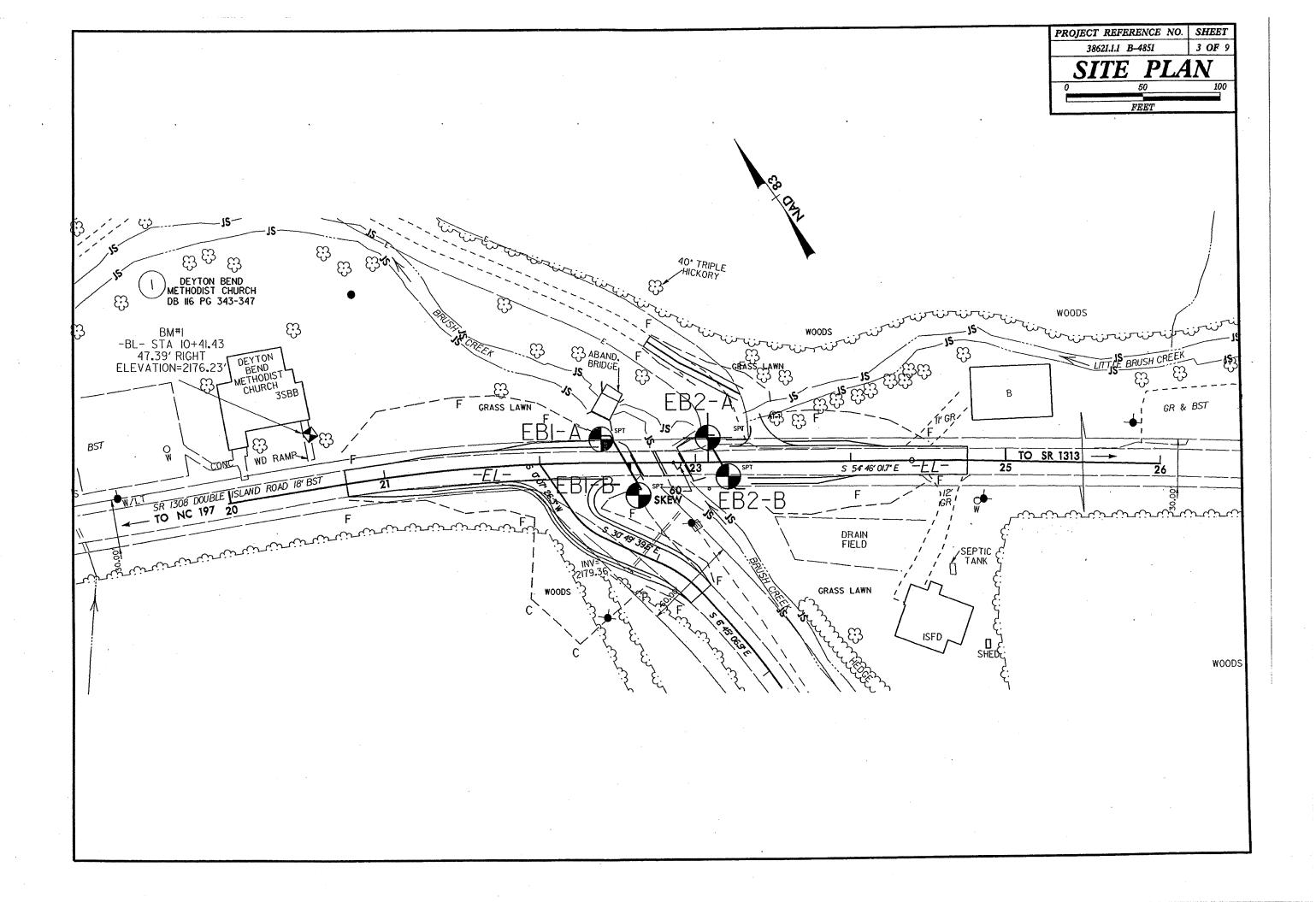
## NORTH CAROLINA DEPARTMENT OF TRANSPORTATION

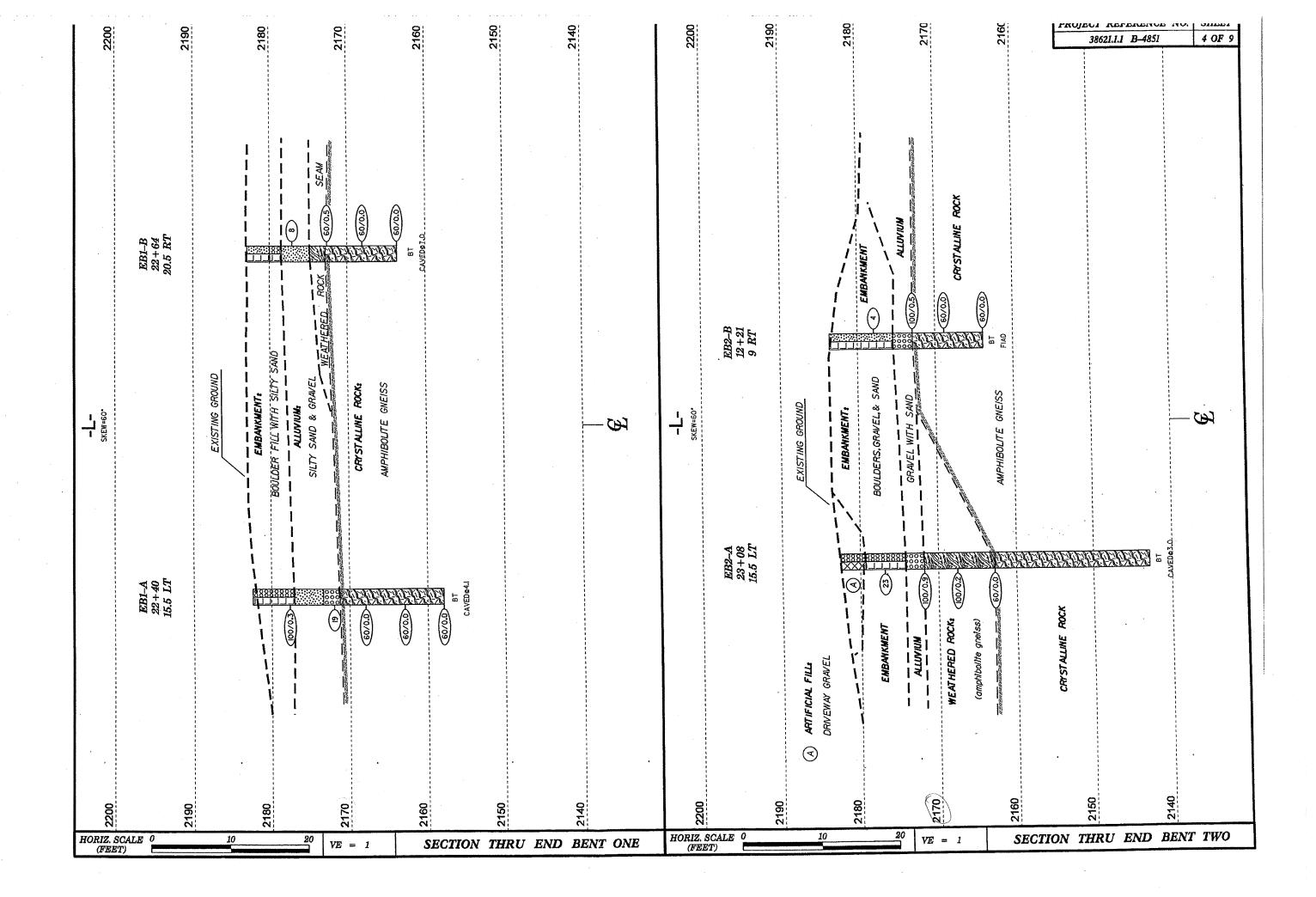
## DIVISION OF HIGHWAYS

GEOTECHNICAL ENGINEERING UNIT

## SUBSURFACE INVESTIGATION

	SOIL AND ROCK LEGEND, TERM	S, SYMBOLS, AND ABBREVIATIONS	
SOIL DESCRIPTION	GRADATION	ROCK DESCRIPTION	TERMS AND DEFINITIONS
SOIL IS CONSIDERED TO BE THE UNCONSOLIDATED, SEMI-CONSOLIDATED, OR WEATHERED EARTH MATERIALS	WELL GRACED - INDICATES A GOOD REPRESENTATION OF PARTICLE SIZES FROM FINE TO COARSE. UNIFORM - INDICATES THAT SOIL PARTICLES ARE ALL APPROXIMATELY THE SAME SIZE. (ALSO	HARD ROCK IS NON-COASTAL PLAIN MATERIAL THAT IF TESTED, WOULD YIELD SPT REFUSAL, AN INFERRED ROCK LINE INDICATES THE LEVEL AT WHICH NON-COASTAL PLAIN MATERIAL MOULD YIELD SPT REFUSAL.	ALLUVIUM (ALLUV.) - SOILS THAT HAVE BEEN TRANSPORTED BY WATER. ADUIFER - A WATER BEARING FORMATION OR STRATA.
THAT CAN BE PENETRATED WITH A CONTINUOUS FLIGHT POWER AUGER, AND YIELD LESS THAN 100 BLOWS PER FOOT ACCORDING TO STANDARD PENETRATION TEST (AASHTO T206, ASTM D-1586), SDIL	POORLY GRADED GAP-GRADED - INDICATES A MIXTURE OF UNIFORM PARTICLES OF TWO OR MORE SIZES.	ST REFUSAL IS PENETRATION BY A SPLIT SPOON SAMPLER EQUAL TO OR LESS THAN &I FOOT PER 60 BLOWS. IN NON-COASTAL PLAIN MATERIAL, THE TRANSITION BETWEEN SOIL AND ROCK IS OFTEN REPRESENTED BY A ZONE	ARENACEOUS - APPLIED TO ROCKS THAT HAVE BEEN DERIVED FROM SAND OR THAT CONTAIN SAND.
CLASSIFICATION IS BASED ON THE AASHTO SYSTEM, BASIC DESCRIPTIONS GENERALLY SHALL INCLUDE: CONSISTENCY, COLOR, TEXTURE, NOISTURE, AASHTO CLASSIFICATION, AND OTHER PERTINENT FACTORS SUCH	ANGULARITY OF GRAINS	OF MEATHERED ROCK. ROCK MATERIALS ARE TYPICALLY DIVIDED AS FOLLOWS:	ARCILLAFERIS - APPLIED TO ALL ROCKS OR SUBSTANCES COMPOSED OF CLAY MINERALS.
AS MINERALOGICAL COMPOSITION, ANGULARITY, STRUCTURE, PLASTICITY, ETC. EXAMPLES	THE ANGULARITY OR ROUNDNESS OF SOIL GRAINS IS DESIGNATED BY THE TERMS: ANGULAR, SUBROUNDED, OR ROUNDED.	WEATHERED NON-COASTAL PLAIN MATERIAL THAT WOULD YIELD SPT N VALUES > 188	OR HAVING A NOTABLE PROPORTION OF CLAY IN THEIR COMPOSITION, AS SHALE, SLATE, ETC.  RETESIAN - GROUND WATER THAT IS UNDER SUFFICIENT PRESSURE TO RISE ABOVE THE LEVEL
VERY STATE COMPOSITY COMPOSITY WITH INTERCEDED FINE SHIP LARGE PLANE PLANE LATE OF	MINERALOGICAL COMPOSITION	ROCK (NR)  BLOWS PER FOOT IF TESTED.  FINE TO COARSE GRAIN IGNEOUS AND METAMORPHIC ROCK THAT	AT WHICH IT IS ENCOUNTERED, BUT WHICH DOES NOT NECESSARILY RISE TO OR ABOVE THE
SOIL LEGEND AND AASHTO CLASSIFICATION  GENERAL GRANUAR MATERIALS SILT-CLAY MATERIALS CONTROL OF THE PROPERTY O	MINERAL NAMES SUCH AS QUARTZ, FELDSPAR, MICA, TALC, KAOLIN, ETC, ARE USED IN DESCRIPTIONS	POCH TOTAL PROBLEM TO THE POCH TO THE POCH TYPE INCLUDES GRANITE,	CROUND SURFACE CALCAREOUS (CALC.) - SOILS THAT CONTAIN APPRECIABLE AMOUNTS OF CALCIUM CARBONATE.
CLASS. (≤ 35% PASSING *200) (> 35% PASSING *200) URGANIC MATERIALS	WHENEVER THEY ARE CONSIDERED OF SIGNIFICANCE.	UNCLESS, GARBERU, SCHIST, ETT.	COLLUVIUM - ROCK FRAGMENTS MIXED WITH SOIL DEPOSITED BY GRAVITY ON SLOPE OR AT BOTTOM
GROUP A-1 A-3 A-2 A-4 A-5 A-6 A-7 A-1, A-2 A-4, A-5 CLASS, A-1-a A-1-b A-2-4 A-2-5 A-2-6 A-7-4 A-3 A-6, A-7	COMPRESSIBILITY  SUBSTITUTE COMPRESSIBLE  CLOUID LIMIT LESS THAN 31	NON-CRYSTALLINE SEDIMENTARY ROCK THAT WOULD YELLD SPT REFUSAL IF TESTED. ROCK TYPE INCLUDES PHYLLITE, SLATE, SANOSTONE, ETC.	OF SLOPE,
SYMBOL POOD POOD SOON SOON SOON SOON SOON SOON SOON S	MODERATELY COMPRESSIBLE LIQUID LIMIT EQUAL TO 31-50	COASTAL PLAIN CDASTAL PLAIN SEDIMENTS CEMENTED INTO ROCK, BUT MAY NOT YIELD SEDIMENTARY ROCK SPT REFUSAL, ROCK TYPE INCLUDES LIMESTONE, SANDSTONE, CEMENTED	CORE RECOVERY (REC.) - TOTAL LENGTH OF ALL MATERIAL RECOVERED IN THE CORE BARREL DIVIDED BY TOTAL LENGTH OF CORE RUN AND EXPRESSED AS A PERCENTAGE.
Z PASSING	HIGHLY COMPRESSIBLE LIQUID LIMIT GREATER THAN 50  PERCENTAGE OF MATERIAL	(CP) SHELL BEDS, ETC.	DIKE - A TABULAR BODY OF IGNEOUS ROCK THAT CUTS ACROSS THE STRUCTURE OF ADJACENT
* 10 58 HX GRANULAR CLAY MUCK,	ORGANIC MATERIAL SOUS SOUS OTHER MATERIAL	WEATHERING TO THE PROPERTY OF	ROCKS OR CUTS MASSIVE ROCK.  DIP - THE ANGLE AT WHICH A STRATUM OR ANY PLANAR FEATURE IS INCLINED FROM THE
# 40 38 HX 58 HX 51 MH S0 HX 35 HX 35 HX 35 HX 35 HX 35 HX 35 HX 36 HX 3	TRACE OF ORGANIC MATTER 2 - 3% 3 - 5% TRACE 1 - 18%	FRESH ROCK FRESH, CRYSTALS BRIGHT, FEW JOINTS MAY SHOW SLIGHT STAINING, ROCK RINGS UNDER HAMMER IF CRYSTALLINE.	HORIZONTAL
LIOJO LIMIT 48 MX 41 MN 48 MX 41 MN 48 MX 41 MN 48 MX 41 MN SDILS WITH	LITTLE DRGANIC MATTER 3 - 5% 5 - 12% LITTLE 10 - 20% MODERATELY ORGANIC 5 - 10% 12 - 20% SOME 20 - 35%	VERY SLIGHT ROCK GENERALLY FRESH, JOINTS STAINED, SOME JOINTS MAY SHOW THIN CLAY COATINGS IF OPEN,	DIP DIRECTION (DIP AZIMUTH) - THE DIRECTION OR BEARING OF THE HORIZONTAL TRACE OF THE LINE OF DIP, HEASURED CLOCKWISE FROM NORTH.
PLASTIC NOEX 6 MX 11º 10 MX 10 MX 10 MX 10 MX 10 MX 10 MX 11 MX 11 MX LITTLE OR HIGHLY	HIGHLY ORGANIC >10% >20% HIGHLY 35% AND ABOVE	(V SLIJ) CRYSTALS ON A BROKEN SPECIMEN FACE SHINE BRIGHTLY. ROCK RINGS UNDER HAMMER BLOWS IF OF A CRYSTALLINE NATURE.	FAULT - A FRACTURE OR FRACTURE ZONE ALONG WHICH THERE HAS BEEN DISPLACEMENT OF THE
GROUP INDEX 0 0 0 1 Hx 8 HX 12 HX 16 HX No HX MODERATE DRGANI USUAL TYPES STONE FRADS.		SLIGHT ROCK GENERALLY FRESH, JOINTS STAINED AND DISCOLORATION EXTENDS INTO ROCK UP TO 1 INCH. OPEN JOINTS MAY CONTAIN CLAY, IN GRANITOID ROCKS SOME OCCASIONAL FELDSPAR	SIDES RELATIVE TO DIVE ANOTHER PARALLEL TO THE FRACTURE.
OF HAJOR GRAVEL MO CHAYEY SILTY CLAYEY ORGANIC	▼ WATER LEVEL IN BORE HOLE INMEDIATELY AFTER DRILLING  STATIC WATER LEVEL AFTER 24 HOURS	CRYSTALS ARE DULL AND DISCOLORED, CRYSTALLINE ROCKS RING UNDER HAMMER BLDWS.	FISSILE - A PROPERTY OF SPLITTING ALONG CLOSELY SPACED PARALLEL PLANES.
MAIEMIRLS SANU		MODERATE HODE: HOD	FLOAT - ROCK FRAGMENTS ON SURFACE NEAR THEIR ORIGINAL POSITION AND DISLOGED FROM PARENT MATERIAL.
AS A EXCELLENT TO GOOD FAIR TO POOR POOR UNSUITABLE SUBGRADE	PERCHED WATER, SATURATED ZONE, OR WATER BEARING STRATA	DULL SOUND UNDER HAMMER BLOWS AND SHOWS SIGNIFICANT LOSS OF STHENGTH AS COMPARED	FLOOD PLAIN (FP) - LAND BORDERING A STREAM, BUILT OF SEDIMENTS DEPOSITED BY
PI OF A-7-5 SUBGROUP IS ≤ LL - 30 ; PI OF A-7-6 SUBGROUP IS > LL - 30	OM SPRING OR SEEP	MITH FRESH ROCK.  MODERATELY ALL ROCK EXCEPT QUARTZ DISCOLORED OR STAINED, IN GRANITOID ROCKS, ALL FELDSPARS DULL	THE STREAM.
CONSISTENCY OR DENSENESS	MISCELLANEOUS SYMBOLS	SEVERE AND DISCOLORED AND A MAJORITY SHOW KAOLINIZATION, ROCK SHOWS SEVERE LOSS OF STRENGTH	FORMATION (FM.) - A MAPPABLE GEOLOGIC UNIT THAT CAN BE RECOGNIZED AND TRACED IN THE FIELD.
PRIMARY SOIL TYPE COMPACTNESS OR RANGE OF STANDARD RANGE OF UNCONFINED PENETRATION RESISTENCE COMPRESSIVE STRENGTH	ROADWAY EMBANKMENT (RE) SAMPLE WITH SOIL DESCRIPTION STATE WITH SOIL DESCRIPTION  OF THE PROPERTY OF THE PROPE	MOD. SEV.)  AND CAN BE EXCAVATED WITH A GEOLOGIST'S PICK. ROCK GIVES "CLURK" SOUND WHEN STROCK.  IF TESTED, MOULD YIELD SPT REFUSAL.	JOINT - FRACTURE IN ROCK ALONG WHICH NO APPRECIABLE HOVEHENT HAS OCCURRED.
CONSISTENCY (N-VALUE) (TONS/FT <sup>2</sup> )	WITH SOIL DESCRIPTION  VIST HIT  DESIGNATIONS  S - BULK SAMPLE	SEVERE ALL ROCK EXCEPT QUARTZ DISCOLORED OR STAINED, ROCK FABRIC CLEAR AND EVIDENT BUT REDUCED	LEDGE - A SHELF-LIKE RIDGE OR PROJECTION OF ROCK WHOSE THICKNESS IS SMALL COMPARED TO
GENERALLY VERY LOOSE (4 GRANULAR LOOSE 4 TO 10	SOIL SYMBOL AUGER BORING SS - SPLIT SPOON	(SEV.) IN STRENGTH TO STRONG SOIL. IN GRANITOID ROCKS ALL FELDSPARS ARE KAOLINIZED TO SOME EXTENT. SOME FRAGMENTS OF STRONG ROCK USUALLY REMAIN.	ITS LATERAL EXTENT.  LENS - A BODY OF SOIL OR ROCK THAT THINS OUT IN ONE OR HORE DIRECTIONS.
MATERIAL MEDIUM DENSE 10 TO 36 N/A	ARTIFICIAL FILL (AF) OTHER CORE BORING SAMPLE	IF TESTED, YIELDS SPT. N. VALUES > 169 BPF  VERY SEVERE ALL ROCK EXCEPT QUARTZ DISCOLORED OR STAINED, ROCK FABRIC ELEMENTS ARE DISCERNIBLE BUT	MOTTLED (MOT.) - IRREGULARLY MARKED WITH SPOTS OF DIFFERENT COLORS. MOTTLING IN
(NON-COHESIVE) VERY DENSE 36 0 56	ST - SHELBY TUBE	I'V GEV! THE MASS IS EFFECTIVELY REDUCED TO SOIL STATUS, WITH ONLY FRAGMENTS OF STRONG ROCK	SOILS USUALLY INDICATES POOR AERATION AND LACK OF GOOD DRAINAGE.  PERCHED WATER - WATER MAINTAINED ABOVE THE NORMAL GROUND WATER LEVEL BY THE PRESENCE OF AN
VERY SOFT C2 (0.25 GENERALLY SOFT 2 TO 4 0.25 TO 0.50	MONITORING WELL BS - BOOK SAMPLE	REMAINING, SAPROLITE IS AN EXAMPLE OF ROCK WEATHERED TO A DEGREE SUCH THAT ONLY MINOR VESTIGES OF THE ORIGINAL ROCK FABRIC REMAIN. IF TESTED, YIELDS SPT N. VALUES < 199 BPF.	Intervening impervious stratum.
SILT-CLAY MEDIUM STIFF 4 TO 8 8.5 TO 1.8	INFERRED ROCK LINE PIEZOMETER DT - DECOMPACTED TRIAVIAL	COMPLETE - ROCK REDUCED TO SOIL, ROCK FABRIC NOT DISCERNIBLE, OR DISCERNIBLE ONLY IN SMALL AND	RESIDUAL (RES.) SOIL - SOIL FORMED IN PLACE BY THE WEATHERING OF ROCK.
MATERIAL STIFF 8 TO 15 1 TO 2 (COHESIVE) VERY STIFF 15 TO 30 2 TO 4	SLOPE INDICATOR	SCATTERED CONCENTRATIONS, QUARTZ MAY BE PRESENT AS DIKES OR STRINGERS. SAPROLITE IS ALSO AN EXAMPLE.	ROCK QUALITY DESIGNATION (ROD) - A MEASURE OF ROCK QUALITY DESCRIBED BY TOTAL LENGTH OF ROCK SEGMENTS EQUAL TO OR GREATER THAN 4 INCHES DIVIDED BY THE TOTAL LENGTH OF CORE RUN AND
HARD >38 >4	25/825 DIP & DIP DIRECTION OF INSTALLATION CBR - CALIFORNIA BEARING RATIO SAMPLE ROCK STRUCTURES	ROCK HARDNESS	EXPRESSED AS A PERCENTAGE:
TEXTURE OR GRAIN SIZE	SPT N-VALUE	VERY HARD CANNOT BE SCRATCHED BY KNIFE OR SHARP PICK, BREAKING OF HAND SPECIMENS REDUIRES	SAPPOLITE (SAP.) - RESIDUAL SOIL THAT RETAINS THE RELIC STRUCTURE OR FABRIC OF THE PARENT ROCK.
U.S. STD. SIEVE SIZE 4 10 46 69 200 270 OPENING (MM) 4.76 2.00 0.42 0.25 0.075 0.053	SOUNDING ROD     GEF SPT REFUSAL	SEVERAL HARD BLOWS OF THE GEOLOGIST'S PICK.  HARD CAN BE SCRATCHED BY KNIFE OR PICK ONLY WITH DIFFICULTY, HARD HAMMER BLOWS REQUIRED	SILL - AN INTRUSIVE BODY OF IGNEOUS ROCK OF APPROXIMATELY UNIFORM THICKNESS AND RELATIVELY ININ COMPARED WITH ITS LATERAL EXTENT, THAT HAS BEEN EMPLACED PARALLEL
COARSE FINE	ABBREVIATIONS	HARD CAN BE SCRATCHED BY KNIFE OR PICK ONLY WITH DIFFICULTI. THIND HARDEN BLOWS RECORDED  TO DETACH HAND SPECIMEN.	TO THE BEDDING OR SCHISTOSITY OF THE INTRUDED ROCKS.
BOULDER COBBLE GRAVEL SAND SAND SILT CLAY (BLDR.) (COB.) (GR.) (CSE, SD.) (F SD.) (SL.) (CL.)	AR - AUGER REFUSAL HI HIGHLY W - MOISTURE CONTENT BT - BORING TERNINATED MED MEDIUM V - VERY	MODERATELY CAN BE SCRATCHED BY KNIFE OR PICK, GOUGES OR GROOVES TO 0.25 INCHES DEEP CAN BE EXCAVATED BY HARD BLOW OF A GEOLOGIST'S PICK, HARD SPECIMENS CAN BE DETACHED	SLICKENSIDE - POLISHED AND STRIATED SURFACE THAT RESULTS FROM FRICTION ALONG A FAULT OR SLIP PLANE.
GRAIN NAM 305 75 2.0 0.25 0.05 0.005	CL CLAY MICA MICACEOUS VST - VANE SHEAR TEST CPT - CONE PENETRATION TEST MOD NODERATELY WEA WEATHERED	BY MODERATE BLOWS.	ATTIONED PRINTING TEST (PENETRATION PESISTANCE) (SPT) - NIMBER OF BLOWS (N OR BPF) DF
SIZE IN. 12 3	CSE COARSE NP - NON PLASTIC 7- UNIT WEIGHT	MEDIUM  CAN BE GROOVED OR GOUGED 6.05 INCHES DEEP BY FIRM PRESSURE OF KNIFE OR PICK POINT.  HARD  CAN BE EXCAVATED IN SMALL CHIPS TO PEICES 1 INCH MAXIMUM SIZE BY HARD BLOWS OF THE	A 140 LB, HAWMER FALING 30 INCHES REQUIRED TO PRODUCE A PENETRATION OF 1 FOOT INTO SOIL WITH A 2 INCH OUTSIDE DIAMETER SPLIT SPOON SAMPLER, SPT REFUSAL IS PENETRATION EQUAL TO OR LESS
SOIL MOISTURE - CORRELATION OF TERMS	DHT - DILATOMETER TEST ORG ORGANIC 7g- DRY UNIT MEIGHT  DPT - DYNAMIC PENETRATION TEST PMT - PRESSUREMETER TEST TABLET IN ACTED	POINT OF A GEOLOGIST'S PICK.	THAN 6.1 FOOT PER 60 BLDVS.
SOIL MOISTURE SCALE FIELD MOISTURE GUIDE FOR FIELD MOISTURE DESCRIPTION GUIDE FOR FIELD MOISTURE DESCRIPTION	SAP SAPROLITIC FIAD- FILLED IN AFTER  SAP SAPROLITIC FIAD- FILLED IN AFTER  S.D SAND, SANDY  DRILLING	SOFT CAN BE GROVED OR GOUGED READILY BY KNIFE OR PICK, CAN BE EXCAVATED IN FRAGMENTS FROM CHIPS TO SEVERAL INCHES IN SIZE BY MODERATE BLOWS OF A PICK POINT, SMALL, THIN	STRATA CORE RECOVERY ISRECJ - TOTAL LENGTH OF STRATA MATERIAL RECOVERED DIVIDED BY TOTAL LENGTH OF STRATUM AND EXPRESSED AS A PERCENTAGE.
- SATURATED - USUALLY LIQUID; VERY WET, USUALLY	FOSS FOSSILIFEROUS SL SILT, SILTY	PIECES CAN BE BROKEN BY FINGER PRESSURE.	CTRATA DOCK CHALITY DESIGNATION (SRIP) - A HEASIRE OF ROCK QUALITY DESCRIBED BY
(SAT.) FROM BELOW THE GROUND WATER TABLE	FRAC FRACTURED, FRACTURES SLI SLIGHTLY FRAGS FRAGMENTS TCR - TRICONE REFUSAL	VERY  CAN BE CARVED WITH KNIFE. CAN BE EXCAVATED READILY WITH POINT OF PICK, PIECES 1 INCH SOFT  OR MORE IN THICKNESS CAN BE BROKEN BY FINGER PRESSURE. CAN BE SCRATCHED READILY BY	TOTAL LENGTH OF ROCK SEGMENTS WITHIN A STRATUM EQUAL TO OR GREATER THAN 4 INCHES DIVIDED BY THE TOTAL LENGTH OF STRATA AND EXPRESSED AS A PERCENTAGE.
PLASTIC SEMISOU ID. DEDUTES OPVING TO	FOUNDMENT HOED ON AUDITOT DDO ITOT	FINGERNAIL.	TOPSOIL (TS.) - SURFACE SOILS USUALLY CONTAINING ORGANIC MATTER.
RANGE - WET - (W) ATTAIN OPTIMUM MOISTURE	EQUIPMENT USED ON SUBJECT PROJECT	TERM SPOCING TERM THICKNESS	BENCH MARK: BL-3: -L- STA. 23+24.95 18.94' LT.
	DRILL UNITS: ADVANCING TOOLS: HANNER TYPE:	VERY MIDE MORE THAN 10 FEET VERY MEDDED > 4 FEET	
OM OPTIMUM MOISTURE - MOIST - (M) SOLID: AT OR NEAR OPTIMUM MOISTURE SL SHRINKAGE LIMIT	CLAY BITS X AUTOMATIC MANUAL	WIDE 3 TO 10 FEET THINLY BEDDED 8.16 - 1.5 FEET	ELEVATION: 2182.38 FT.
REQUIRES ADDITIONAL WATER TO	6° CONTINUOUS FLIGHT AUGER CORE SIZE:	CLOSE 0.16 TO 1 FEET THICKLY LAMINATED 0.008 - 0.03 FEET	NOTES:
- DRY - (D) ATTAIN OPTIMUM MOISTURE	BK-51 8' HOLLOW AUGERS	VERY CLOSE LESS THAN 8.16 FEET THINLY LAMINATED ( 0.608 FEET	<b>-</b>
PLASTICITY	CME-45C HARD FACED FINGER BITS X-N_XWL	INDURATION  FOR SEDIMENTARY ROCKS, INDURATION IS THE HARDENING OF THE MATERIAL BY CEMENTING, HEAT, PRESSURE, ETC.	┪
PLASTICITY INDEX (PI) DRY STRENGTH	TUNG,-CARBIDE INSERTS	RUBBING WITH FINGER FREES NUMEROUS GRAINS;	
NONPLASTIC 9-5 VERY LOW LOW PLASTICITY 6-15 SLIGHT	X CME-550 X CASING X W/ ADVANCER HAND TOOLS:	GENTLE BLOW BY HAMMER DISINTEGRATES SAMPLE.	
MED. PLASTICITY 16-25 MEDIUM HIGH PLASTICITY 26 OR MORE HIGH	PORTABLE HOIST TRICONE STEEL TEETH POST HOLE DIGGER	MODERATELY INDURATED GRAINS CAN BE SEPARATED FROM SAMPLE WITH STEEL PROBE; BREAKS EASILY WHEN HIT WITH HAMMER.	
COLOR	TRICONETUNG,-CARB. HAND AUGER	A STATE OF THE STA	·
DESCRIPTIONS MAY INCLUDE COLOR OR COLOR COMBINATIONS (TAN, RED, YELLOW-BROWN, BLUE-GRAY).	CORE BIT SOUNDING ROD	DIFFICULT TO BREAK WITH HANNER.	
MODIFIERS SUCH AS LIGHT, DARK, STREAKED, ETC. ARE USED TO DESCRIBE APPEARANCE.	☐ VANE SHEAR TEST	EXTREMELY INDURATED SHARP HANNER BLOWS REQUIRED TO BREAK SAMPLE; SAMPLE BREAKS ACROSS GRAINS.	





NCDOT GEOTECHNICAL ENGINEERING UNIT

SHEET

VBS	38621	.1.1			T	P B	-4851			COUNT	Y Y	ANCEY				GEOLOGIST Hager, M.	M
ITE	DESCR	PTION	Brid	ge No	. 31 o	n SR	1308	over Br	ush (	Creek							GROUND WTF
ORII	NG NO.	EB1-	·A		S	TATIO	ON 2	2+40			OFF	SET	6 ft LT			ALIGNMENT -EL-	0 HR.
OLL	AR ELE	V. 2,	182.4	ft	T	OTAL	. DEP	TH 24.	5 ft		NOI	RTHING	835,5	97		EASTING 1,044,049	24 HR. Caved@
RILL	RIG/HAN	MER E	FF/DA	TE AF	FO0070	CME-	550X 8	31% 09/0	3/200	9	L	****	DRILL I	METHO	D N	W Casing w/ SPT	HAMMER TYPE Automa
	ER C							E 02/1			COI	MP. DA				SURFACE WATER DEPT	***************************************
		DEPTH		W CO		П		·		R FOOT			SAMP.	<b>V</b> /	L		
ft)	(ft)	(ft)	0.5ft	0.5ft	0.5ft	0	:	25	50	)	75	100	NO.	МОІ	O G	SOIL AND ROCK ELEV. (ft)	DESCRIPTION DEP
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70		<b>.</b>			l	<u> </u>	• • ':	1===				1				CRYSTALL	INE ROCK
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	2.157.9	245				$   \cdot  $					1.	$\overline{\cdots}$				2,157.9	
ľ	-		60/0.0							***************************************		60/0.0	1			Boring Terminated at Crystalline Rock: A	Elevation 2,157.9 ft In
		-														- Crystaline Nock. 7	Ampribolite Grieiss
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SHEET 50F9

Cupe C	20024			\		P B-4851		COLINITY	YANCEY				CEOLOGIST Honor M M	
	38621		Deid	aa Na					YANCEY				GEOLOGIST Hager, M. M.	GROUND WTR (ft)
				ge No		SR 1308 C		Creek	OFFSET 2	4 & DT	•		TALICABATAIT EL	<b> </b> ` ` '
	NG NO.			······································		ATION 22						· · · · · · · · · · · · · · · · · · ·	ALIGNMENT -EL-	
	AR ELE					TAL DEPT			NORTHING				EASTING 1,044,048	24 HR. Caved@7.0
ļ				TE AF	<del></del>	CME-550X 8						D NV		MMER TYPE Automatic
DRIL	LER R					ART DATE			COMP. DA	· · · · · · · · · · · · · · · · · · ·			SURFACE WATER DEPTH	N/A
ELEV (ft)	DRIVE ELEV (ft)	DEPTH (ft)	0.5ft	0.5ft	UNT 0.5ft	0 2	BLOWS P	ER FOOT	75 100	SAMP. NO.	моі	C G	SOIL AND ROCK D	ESCRIPTION DEPTH (ft)
2185		-										l ⊦	-	
	1	-		ļ			r			<del> </del>	<u> </u>	1 50	2,183.0 GROUND SU ROADWAY EMB	
2180	-	-										F	Tan-yellow-brown	
2100	 _2.178.1	4.9											2 178 5 ROADWAY EMB	ANKMENT 45
	-2,1/0,1	- 4.9	2	4	4	-•8							BOULDE	AL .
2175	-	-				<u> </u>			<u> </u>				Dark brown-red-dark	8.2
	2,173.1	9.9	00/0.5									<b>2</b>	2,172.8 WEATHERED	
	_		60/0.5				: : : :		60/0.5	1		冠	CRYSTALLIN Amphibolite C	ROCK
2170	_	-		1		<del> </del>			+			冠	- Amprilionite C	r till O
	2,168.1	14.9	60/0.0			• • • •	: : : :		60/0.0	1		爱		
2165	-	-												
	2,163.7-	- 19.3	60/0.0	ļ					60/0.0	4		1	2,163.7 Boring Terminated with	19.3
	-	-	00,0.0	1								l F	<ul> <li>Refusal at Elevation 2,16</li> </ul>	3.7 ft in Crystalline
	-	-										F	Rock: Amphibo	ite Gneiss
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# NCDOT GEOTECHNICAL ENGINEERING UNIT

SHEET

<b>VBS</b>	38621	1.1.1			TI	P B-4851	COUNT	Y YANCEY				GEOLOGIST Hager, M. M.	
ITE	DESCR	IPTION	Brid	ge No	. 31 or	SR 1308 over Brush	Creek						GROUND WTR (f
ORII	NG NO.	EB2	·A		ST	TATION 23+08		OFFSET	16 ft LT			ALIGNMENT -EL-	0 HR. N//
OLL	AR ELI	EV. 2,	182.6	ft	TO	OTAL DEPTH 39.7 f	t	NORTHING	835,5	59		EASTING 1,044,105	24 HR. Caved@3.
RILL	RIG/HA	MMER E	FF/DA	TE A	FO0070	CME-550X 81% 09/03/2	009		DRILL N	ETHO	) NW	Casing W/SPT & Core HAMM	ER TYPE Automatic
RILL	ER R	ose, G	. K.		ST	FART DATE 02/17/1	1	COMP. DA	TE 02/	17/11		SURFACE WATER DEPTH N	/A
LEV (ft)	DRIVE ELEV (ft)	DEPTH (ft)	-	0.5ft	<del></del>	1	PER FOOT 50	75 100	SAMP. NO.	MOI	L O G	SOIL AND ROCK DESC ELEV. (ft)	CRIPTION DEPTH
185		<u> </u>							·		  -  -	2,182.6 GROUND SURF.	ACE.
		<del> </del>		<b></b> -		<u> </u>	T	.	<del> </del>		XISF	ARTIFICIAL FI	LL.
80		‡					<b></b>				<b>ॐ</b>	Driveway GRAV	
	2,177.8:	4.8										ROADWAY EMBAN BOULDERS, GRAVEL	KMENT & SAND
75	•	‡	9	17	6	23		:   : : : :				,,	,,,
	-	‡					<del> </del>					2,174.1 ALLUVIAL	· · · · · · · · · · · · · · · · · · ·
ŀ	2,172.8.	9.8	5	8	100/0.4			<u>: ::::</u>	.1		000-	2,171.8 GRAVEL with s	
70	-	‡						- 100/0.9	<u>'</u>			WEATHERED Ro (amphibolite Gne	
	2.167.8	14.8				:::: ::::		: ::::]				•	
		†	100/0.2					. 100/0.2					
65	-	‡					<del> </del>					•	
-	2,162.8	19.8	60/0.0			: : : :   : : : :	1:::	60/0.0				2,162.8 CRYSTALLINE F	
60		±	l				<u> </u>					Amphibolite GNf Run 1: 19.8-24.7' REC=9	EISS 2% RQD=82%
		Ŧ					1::::	:   : : : :	*	1	漫	Run 2: 24.7-29.7' REC=10 Run 3: 29.7-34.7' REC=10	00% RQD=68%
		Ŧ									漫	Run 4: 34.7-39.7' REC=9:	2% RQD=64%
55	-	Ŧ	1.								漫	-	
		Ŧ		•									
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		‡		1			1					<u>.</u>	
		‡									4		
45	-	‡					<u> </u>					-	
-		‡	<u> </u>		<u> </u>		<u> </u>	<u>:                                    </u>	Ц	L	过	2,142.9	tion 2 142 0 ft in
		‡									-	Boring Terminated at Eleva Crystalline Rock: Amphi	
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		‡		}							<u> </u>		
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# NCDOT GEOTECHNICAL ENGINEERING UNIT

SHEET 60F9

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WBS	38621.	1.1			TIP	B-485	1	C	TMUC	YY	ANCEY GEOLOGIST Hager, M. M.	
SITE	DESCRI	PTION	Brid	ge No. 3	on S	R 1308	3 over B	ush C	reek		GROUND	WTR (ft)
BORI	NG NO.	EB2-	A		STAT	ION	23+08			OF	FSET 16 ft LT ALIGNMENT -EL- 0 HR.	N/A
COLL	AR ELE	V. 2,1	182.6 f	t	TOTAL DEPTH 39.7 ft						<b>DRTHING</b> 835,559 <b>EASTING</b> 1,044,105 <b>24 HR.</b> Ca	ved@3.0
DRILL	RIG/HAN	IMER E	FF./DAT	E AFOO	070 CM	E-550X	81% 09/	03/2009			DRILL METHOD NW Casing W/SPT & Core HAMMER TYPE /	Automatic
DRIL	LER R	se, G.	K.		STAF	RT DA	TE 02/1	7/11	•	CO	OMP. DATE 02/17/11 SURFACE WATER DEPTH N/A	
CORI	SIZE	NXWL					N 19.91					
ELEV	RUN	DEPTH	RUN	DRILL RATE	REC.	RQD (ft) %	SAMP.	STR REC.	ATA RQD (ft) %	LΟ	DESCRIPTION AND DEMARKS	
(ft)	ELEV (ft)	(ft)	(ft)	(Min/ft)	(ft) %	(ft) %	NO.	(ft) %	(ft) %	Ğ	DESCRIPTION AND REMARKS ELEV. (ft)	DEPTH (ft
162.79											Begin Coring @ 19.8 ft	
	2,162.8	19.8	4.9	N=60/0.0 1:47/1.0	(4.5) 92%	(4.0) 82%				K	2,162.8 CRYSTALLINE ROCK  Recovered rock is slightly weathered to fresh, hard, white to dark gray	19.8
2160	-	-		1:41/1.0						晁	<ul> <li>Amphibolite Gneiss. Fracture spacing is very to moderately close. Joir angles average ~30° with some high angle fractures.</li> </ul>	nt
	2,157.9	24.7	5.0	1:23/0.9	(5.0)	(3.4)					angios arolago los manosino ingri angio nastarso.	
2155	1	-		1:22/1.0 1:44/1.0 1:36/1.0	100%	68%					<u>}</u>	•
	2,152.9	29.7		1:34/1.0 1:16/1.0							·	
		•	5.0	1:16/1.0 1:29/1.0	(5.0) 100%	(4.4) 88%						
2150	1	-		1:47/1.0	100%	0070						
	2,147.9	34.7	5.0	1:47/1.0	(4.6)	(2.0)						
2145	1		5.0	2:16/1.0 1:51/1.0	(4.6) 92%	(3.2) 64%						
2140	2,142.9	39.7		1:10/1.0					İ		2,142.9	39.7
	2,142.9	. 39.1		1:28/1.0	<del>                                     </del>			<b> </b>			Boring Terminated at Elevation 2,142.9 ft In Crystalline Rock: Amphibol	
	_	-									Gneiss	
	-	-							1		<u>t</u>	
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NCDOT GEOTECHNICAL ENGINEERING UNIT **BORELOG REPORT** COUNTY YANCEY GEOLOGIST Hager, M. M. WBS 38621.1.1 TIP B-4851 SITE DESCRIPTION Bridge No. 31 on SR 1308 over Brush Creek BORING NO. EB2-B STATION 23+21 ALIGNMENT -EL-**NORTHING** 835,531 TOTAL DEPTH 19.8 ft **EASTING** 1,044,101 COLLAR ELEV. 2,183.7 ft DRILL METHOD NW Casing w/ SPT DRILL RIG/HAMMER EFF/DATE AFO0070 CME-550X 81% 09/03/2009 HAMMER TYPE Automatic COMP. DATE 02/22/11 SURFACE WATER DEPTH N/A DRILLER Cheek, D. O. START DATE 02/22/11 ELEV | DRIVE | DEPTH | BLOW COUNT | ELEV | (ft) | (ft) | 0.5ft | 0.5ft | 0.5ft | SAMP. BLOWS PER FOOT NO. MOI G ELEV. (ft) SOIL AND ROCK DESCRIPTION 0.5ft 0.5ft 0.5ft 0 75 100 GROUND SURFACE
ROADWAY EMBANKMENT
Silty SAND 2,183.7 2180 2175 ALLUVIAL - 100+/0.5 GRAVEL with sand CRYSTALLINE ROCK Amphibolite GNEISS 2.168.9 14.8 60/0.0 60/0.0

2165

2.163.9 I 19.8

SHEET

7049

FIAD

N/A

GROUND WTR (ft)

0 HR.

24 HR.

Boring Terminated at Elevation 2,163.9 ft In Crystalline Rock: Amphibolite Gneiss



# FIELD SCOUR REPORT

. WBS:	38621.1.1	_ TIP:	B-4851	COUNTY: Yancey
DESCRIPTION(1): B	Bridge No. 31 o	n SR 130	8 over Brush Cre	eek
			EXISTING	BRIDGE
Information from:	Field II Other	nspection (explain)	X Mic BSR dated 12/0	crofilm (reel pos:) 02/10
				4 Bents in Channel: 2 Bents in Floodplain: 2 te & timber footings
EVIDENCE OF SO Abutments or Er		: Beneath	both end bent's	s wingwalls
Interior Bents: E				
Channel Bed: N	**************************************	er than at		of the two creeks
Channel Bank: E	End Bent Two t	oanks ups	tream & downstr	ream
EXISTING SCOU Type(3): <u>A</u>		)N		
Extent(4): <u>r</u>	n/a			
Effectiveness(5): F	Poor			
Obstructions(6): <u>H</u>				of a wooden bridge downstream blocking the channel

### **INSTRUCTIONS**

- 1 Describe the specific site's location, including route number and body of water crossed.
- 2 Note scour evidence at existing end bents or abutments (e.g. undermining, sloughing, degradations).
- 3 Note existing scour protection (e.g. rip rap).
- 4 Describe extent of existing scour protection.
- 5 Describe whether or not the scour protection appears to be working.
- 6 Note obstructions such as dams, fallen trees, debris at bents, etc.
- 7 Describe the channel bed material based on observation and/or samples. Include any lab results with report.
- 8 Describe the channel bank material based on observation and/or samples. Include any lab results with report.
- 9 Describe the material covering the banks (e.g. grass, trees, rip rap, none).
- 10 Determine the approximate floodplain width from field observation or a topographic map.
- 11 Describe the material covering the floodplain (e.g. grass, trees, crops).
- 12 Use professional judgement to specify if the stream is degrading, aggrading, or static.
- 13 Describe potential and direction of the stream to migrate laterally during the bridge's life (approx. 100 years).
- 14 Give the design scour elevation (DSE) expected over the life of the bridge (approx. 100 years). This elevation can be given as a range across the site, or for each bent. Discuss the relationship between the Hydraulics Unit theoritical scour and the DSE. If the DSE is dependent on scour counter measures, explain (e.g. rip rap armoring on slopes). The DSE is based on the erodability of materials, giving consideration to the influence of joints, foliation, bedding characteristics, % core recovery, % RQD, differential weathering, shear strength, observations at existing structures, other tests deemed appropriate, and overall geologic conditions at the site.

Channel Bed Material(7): Silt, sand, gravel & cobbles  Channel Bank Material(8): Gravel & sand  Channel Bank Cover(9): Trees, bramble  Floodplain Width(10): ~200'  Floodplain Cover(11): Trees, grass  Stream is(12): Aggrading DegradingX Static  sannel Migration Tendency(13): Toward End Bent Two  Observations and Other Comments:	
Channel Bank Cover(9): Trees, bramble  Floodplain Width(10): ~200'  Floodplain Cover(11): Trees, grass  Stream is(12): Aggrading DegradingX Static  nannel Migration Tendency(13): Toward End Bent Two  Observations and Other Comments:	
Floodplain Width(10): ~200'  Floodplain Cover(11): Trees, grass  Stream is(12): Aggrading Degrading _X Static  annel Migration Tendency(13): Toward End Bent Two  Observations and Other Comments:	
Stream is(12): Aggrading Degrading _X Static annel Migration Tendency(13): Toward End Bent Two  Observations and Other Comments:	
Stream is(12): Aggrading Degrading _X Static annel Migration Tendency(13): Toward End Bent Two  Observations and Other Comments:	
annel Migration Tendency(13): Toward End Bent Two Observations and Other Comments:	
Observations and Other Comments:	
DESIGN SCOUR ELEVATIONS(14) Feet X Meters	
<u>BENTS</u>	
N/A	
Comparison of DSE to Hydraulics Unit theoretical scour:  No scour is predicted through calculations. DSE is in agreement with BSR dated 12/02/10.	
SOIL ANALYSIS RESULTS FROM CHANNEL BED AND BANK MATERIAL	<del></del>
Bed or Bank	
Sample No. Retained #4	
Passed #10	
Passed #40	
Passed #200	
Coarse Sand	
Fine Sand	
Silt	
Clay	
LL	
PI	
AASHTO	
Station	
Offset Doort	
Depth	

Reported by: J.W. Mann Date: 2/16/2011

## 38621.1.1 (B-4851) YANCEY COUNTY BRIDGE # 31 ON SR 1308 OVER BRUSH CREEK



