STATE OF NORTH CAROLINA

DEPARTMENT OF TRANSPORTATION
DIVISION OF HIGHWAYS
GEOTECHNICAL ENGINEERING UNIT

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STRUCTURE SUBSURFACE INVESTIGATION

PROJ. REFERENCE NO. <u>33768.1.1 (B-4556)</u> F.A. PROJ. <u>BRSTP-0050(7)</u>
COUNTY <u>JOHNSTON</u>
SITE DESCRIPTION <u>BRIDGE NO. 74 ON -L- (NC 50) OVER BLACK</u>
CREEK AT STATION 16+44

INVENTORY

 STATE
 STATE PROJECT REFERENCE NO.
 SHEET SHEETS
 TOTAL SHEETS

 N.C.
 33768.1.1 (B-4556)
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CAUTION NOTICE

THE SUBSURFACE INFORMATION AND THE SUBSURFACE INVESTIGATION ON WHICH IT IS BASED WERE MADE FOR THE PURPOSE OF STUDY, PLANNING, AND DESIGN, AND NOT FOR CONSTRUCTION OR PAY PURPOSES. THE VARIOUS FIELD BORING LOGS, ROCK CORES, AND SOLI TEST DATA AVAILABLE MAY BE REVIEWED OR INSPECTED IN RALEIGH BY CONTACTING THE N. C. DEPARTMENT OF TRANSPORTATION, GEOTECHNICAL ENGINEERING UNIT AT (1919) 250-4088. NEITHER THE SUBSURFACE PLANS AND REPORTS, NOR THE FIELD BORING LOGS, ROCK CORRES, OR SOIL TEST DATA ARE PART OF THE CONTRACT.

CENERAL SOIL AND ROCK STRATA DESCRIPTIONS AND INDICATED BOUNDARIES ARE BASED ON A GEOTECHNICAL INTERPRETATION OF ALL AVAILABLE SUBSURFACE DATA AND MAY NOT NECESSARILY REFLECT THE ACTUAL SUBSURFACE CONDITIONS BETWEEN BORINGS OR BETWEEN SAMPLED STRATA WITHIN THE BOREHOLE, THE LABORATORY SAMPLE DATA AND THE IN STUI MEPHACED TEST DATA CAN BE RELIED ON ONLY TO THE DEGREE OF RELIABLITY INHERENT IN THE STANDARD TEST METHOD. THE OBSERVED WATER LEVELS OR SOIL MOSITURE CONDITIONS INDICATED IN THE SUBSURFACE INVESTIGATIONS ARE AS RECORDED AT THE TIME OF THE INVESTIGATION, THESE WATER LEVELS OR SOIL MOISTURE CONDITIONS MAY VARY CONSIDERABLY WITH TIME ACCORDING TO CLIMATIC CONDITIONS INCLUDING TEMPERATURES, PRECIPITATION, AND WIND, AS WELL AS OTHER NON-CLIMATIC FACTORS.

THE BIDDER OR CONTRACTOR IS CAUTIONED THAT DETAILS SHOWN ON THE SUBSURFACE PLANS ARE PRELIMINARY ONLY AND IN MANY CASES THE FIRML DESIGN DETAILS ARE DIFFERENT. FOR BIDDING AND CONSTRUCTION PLANS AND DOCUMENTS FOR FINAL DESIGN INFORMATION ON THIS PROJECT. THE CEPARTMENT DOES NOT WARRANT OR CUARANTEE THE SUFFICIENCY OR ACCURACY OF THE INVESTICATION MADE, NOR THE INTERPRETATIONS MADE, OR OPINION OF THE DEPARTMENT AS TO THE TYPE OF MATERIALS AND CONDITIONS TO BE ENCOUNTERED. THE BIDDER OR CONTRACTOR IS CAUTIONED TO MAKE SUCH INDEPENDENT SUBSURFACE INVESTICATIONS AS HE DEEMS NECESSARY TO SATISFY HIMSELF AS TO CONDITIONS TO BE ENCOUNTERED ON THIS PROJECT. THE CONTRACTOR SHALL HAVE NO CLAIM FOR ADDITIONAL COMPENSATION OR FOR AN EXTENSION OF TIME FOR ANY REASON RESULTING FROM THE ACTUAL CONDITIONS ENCOUNTERED AT THE SITE DIFFERING FROM THE OCCUPATION.

PERSONNEL N.D. MOHS

J.R. MATULA
D.W. DIXON

INVESTIGATED BY N.D. MOHS

SUBMITTED BY N.T. ROBERSON



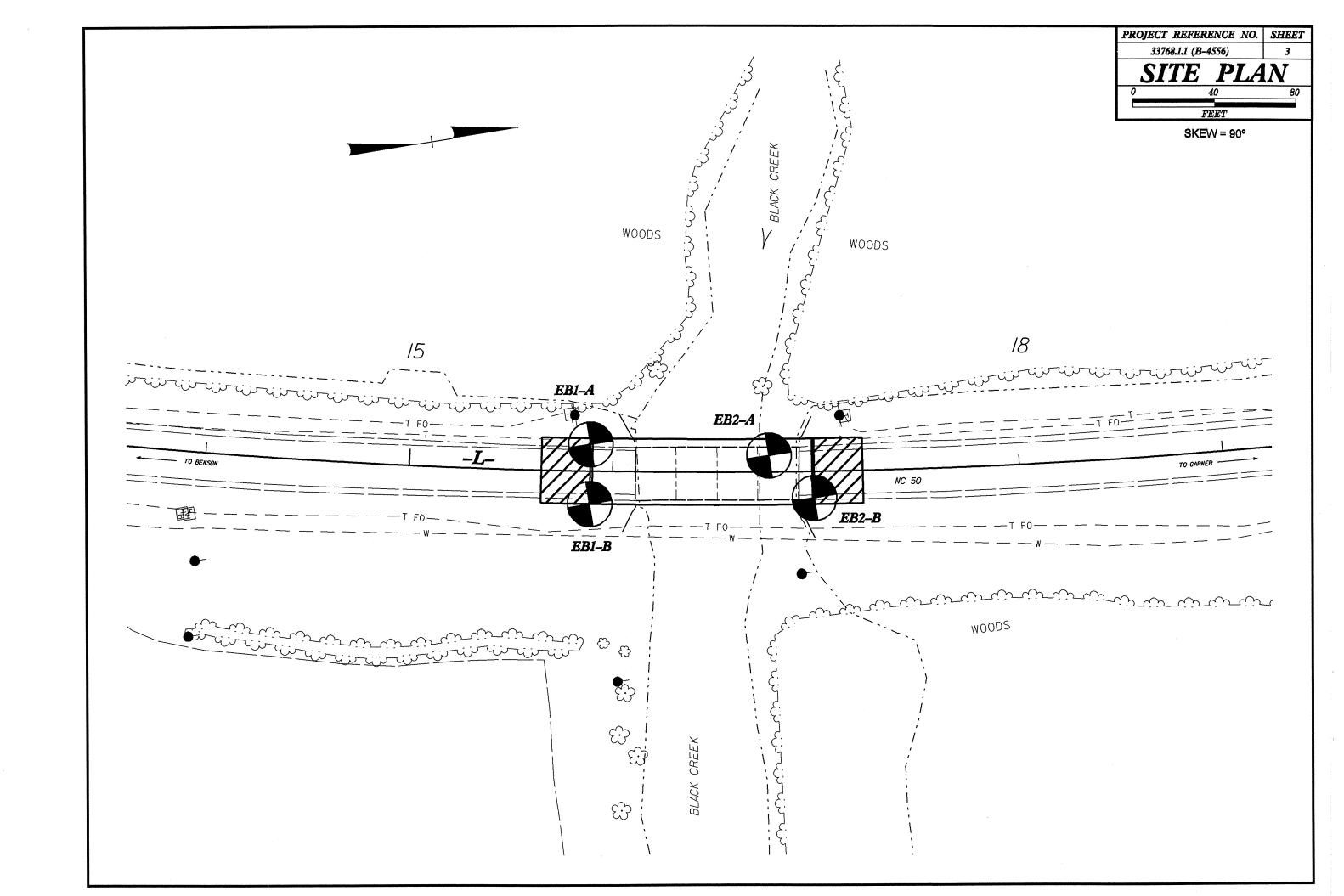
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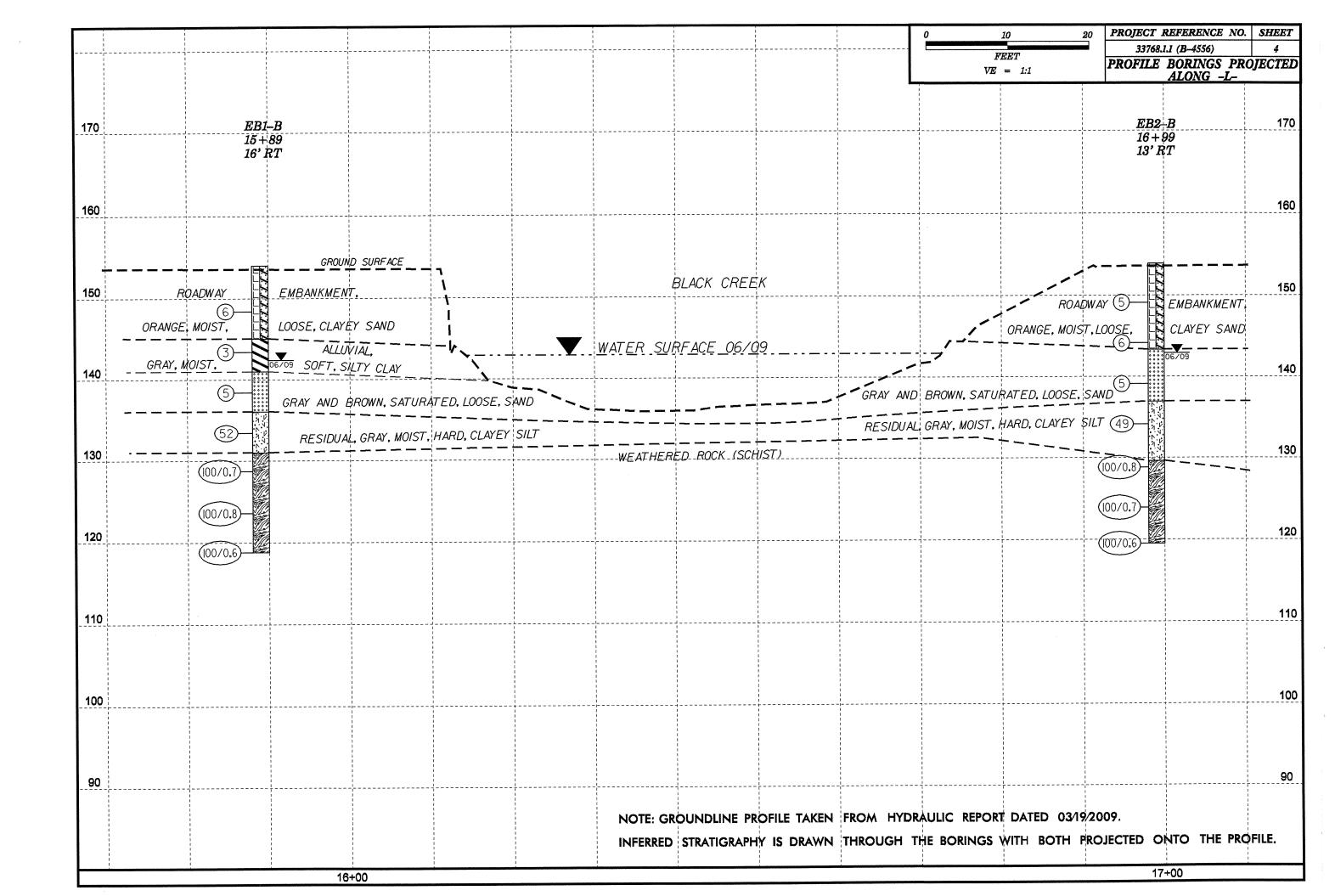
DIVISION OF HIGHWAYS

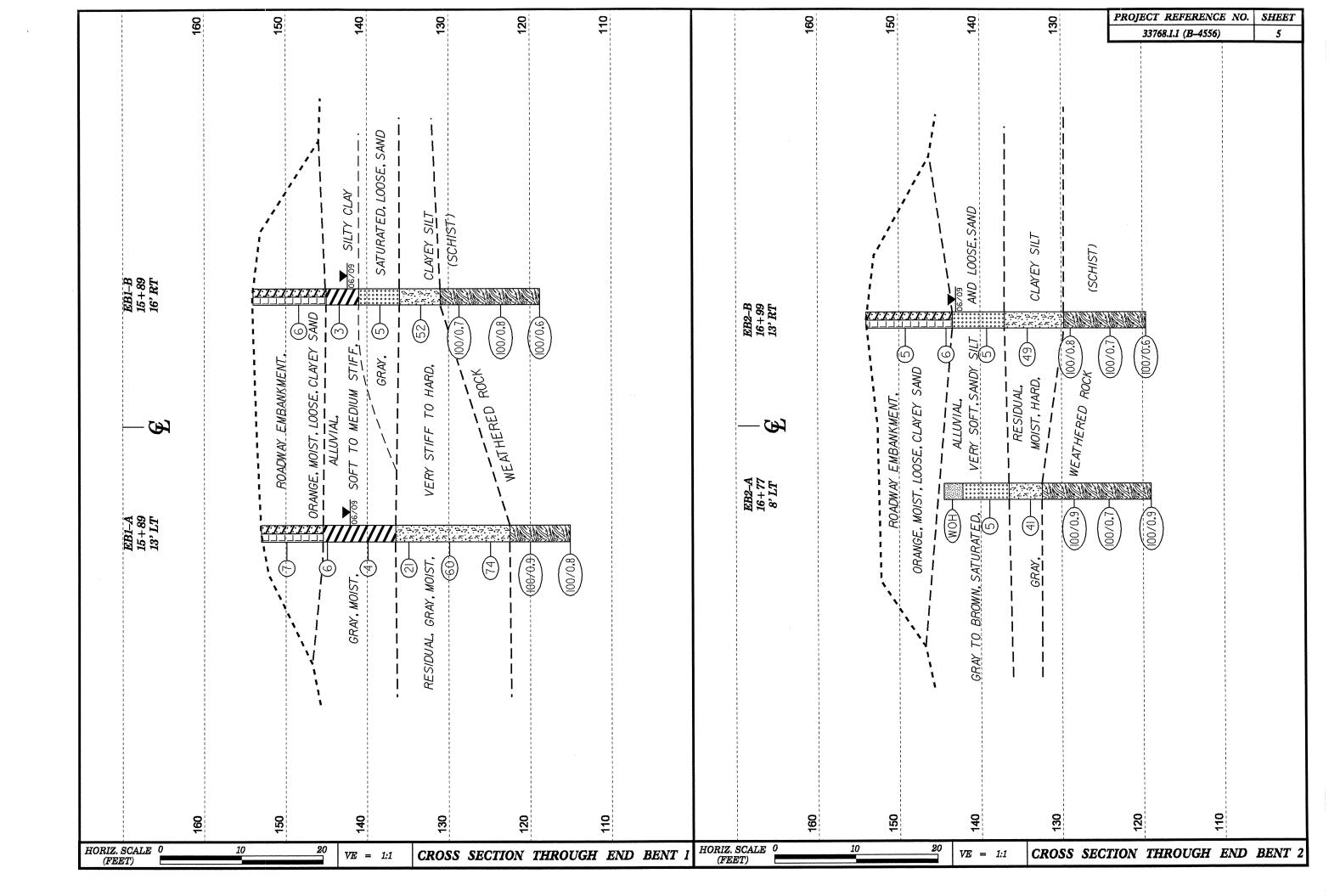
GEOTECHNICAL ENGINEERING UNIT

SUBSURFACE INVESTIGATION

	SOIL AND ROCK LEGEND, TERM	IS, SYMBOLS, AND ABBREVIATIONS	
SOIL DESCRIPTION	GRADATION	ROCK DESCRIPTION	TERMS AND DEFINITIONS
SOIL IS CONSIDERED TO BE THE UNCONSOLIDATED, SEMI-CONSOLIDATED, OR WEATHERED EARTH MATERIALS	WELL GRADED - INDICATES A GOOD REPRESENTATION OF PARTICLE SIZES FROM FINE TO COARSE. UNIFORM - INDICATES THAT SOIL PARTICLES ARE ALL APPROXIMATELY THE SAME SIZE. (ALSO	HARD ROCK IS NON-COASTAL PLAIN MATERIAL THAT IF TESTED, WOULD YIELD SPT REFUSAL. AN INFERRED ROCK LINE INDICATES THE LEVEL AT WHICH NON-COASTAL PLAIN MATERIAL WOULD YIELD SPT REFUSAL.	ALLUVIUM (ALLUV.) - SOILS THAT HAVE BEEN TRANSPORTED BY WATER.
THAT CAN BE PENETRATED WITH A CONTINUOUS FLIGHT POWER AUGER, AND YIELD LESS THAN	POORLY GRADED) GAP-GRADED - INDICATES A MIXTURE OF UNIFORM PARTICLES OF TWO OR MORE SIZES.	SPT REFUSAL IS PENETRATION BY A SPLIT SPOON SAMPLER EQUAL TO OR LESS THAN 0.1 FOOT PER 60 BLOWS. IN NON-COASTAL PLAIN MATERIAL, THE TRANSITION BETWEEN SOIL AND ROCK IS OFTEN REPRESENTED BY A ZONE	AGUIFER - A WATER BEARING FORMATION OR STRATA.
CLASSIFICATION IS BASED ON THE AASHTO SYSTEM, BASIC DESCRIPTIONS GENERALLY SHALL INCLUDE:	ANGULARITY OF GRAINS	OF WEATHERED ROCK. ROCK MATERIALS ARE TYPICALLY DIVIDED AS FOLLOWS:	ARENACEOUS - APPLIED TO ROCKS THAT HAVE BEEN DERIVED FROM SAND OR THAT CONTAIN SAND, ARGILLACEOUS - APPLIED TO ALL ROCKS OR SUBSTANCES COMPOSED OF CLAY MINERALS,
AS MINERALOGICAL COMPOSITION, ANGULARITY, STRUCTURE, PLASTICITY, ETC. EXAMPLE:	THE ANGULARITY OR ROUNDNESS OF SOIL GRAINS IS DESIGNATED BY THE TERMS; ANGULAR,	WEATHERED NON-COASTAL PLAIN MATERIAL THAT WOULD YIELD SPT N VALUES > 100	OR HAVING A NOTABLE PROPORTION OF CLAY IN THEIR COMPOSITION, AS SHALE, SLATE, ETC.
FER STATE GOVERNOOF WITH MELIDEBULY FIRE STATE OF BUSINESS FOR STATE OF	SUBANGULAR, SUBROUNDED, OR ROUNDED. MINERALOGICAL COMPOSITION	ROCK (WR) BLOWS PER FOOT IF TESTED.	ARTESIAN - GROUND WATER THAT IS UNDER SUFFICIENT PRESSURE TO RISE ABOVE THE LEVEL AT WHICH IT IS ENCOUNTERED, BUT WHICH DOES NOT NECESSARILY RISE TO OR ABOVE THE
SOIL LEGEND AND AASHTO CLASSIFICATION GENERAL GRANULAR MATERIALS SILT-CLAY MATERIALS OPERANTE MATERIALS	MINERAL NAMES SUCH AS DUARTZ, FELDSPAR, MICA, TALC, KAOLIN, ETC. ARE USED IN DESCRIPTIONS	CRYSTALLINE ROCK (CR) FINE TO CDARSE GRAIN IGNEOUS AND METAMORPHIC ROCK THAT WOULD YIELD SPT REFUSAL IF TESTED. ROCK TYPE INCLUDES GRANITE,	GROUND SURFACE.
	WHENEVER THEY ARE CONSIDERED OF SIGNIFICANCE.	GREISS, GABBRO, SCHIST, ETC.	CALCAREOUS (CALC.) - SOILS THAT CONTAIN APPRECIABLE AMOUNTS OF CALCIUM CARBONATE.
GROUP A-1 A-3 A-2 A-4 A-5 A-6 A-7 A-1, A-2 A-4, A-5	COMPRESSIBILITY	NON-LATS HILLINE SEDIMENTARY ROCK THAT WOULD YEILD SPT REFUSAL IF TESTED. ROCK TYPE	COLLUYIUM - ROCK FRAGMENTS MIXED WITH SOIL DEPOSITED BY GRAVITY ON SLOPE OR AT BOTTOM OF SLOPE.
CLASS. A-1-a A-1-b A-2-4 A-2-5 A-2-6 A-2-7 A-7-8 A-3 A-6, A-7	SLIGHTLY COMPRESSIBLE LIQUID LIMIT LESS THAN 31 MODERATELY COMPRESSIBLE LIQUID LIMIT EQUAL TO 31-50	COASTAL PLAIN COASTAL PLAIN SEDIMENTS CEMENTED INTO ROCK, BUT MAY NOT YIELD	CORE RECOVERY (REC.) - TOTAL LENGTH OF ALL MATERIAL RECOVERED IN THE CORE BARREL DIVIDED BY TOTAL
SYMBOL 000000000000000000000000000000000000	HIGHLY COMPRESSIBLE LIQUID LIMIT GREATER THAN 50	SEDIMENTARY ROCK SPT REFUSAL, ROCK TYPE INCLUDES LIMESTONE, SANDSTONE, CEMENTED SHELL BEDS, ETC.	LENGTH OF CORE RUN AND EXPRESSED AS A PERCENTAGE.
X PASSING GRANULAR SILT- MUCK, GRANULAR CLAY	PERCENTAGE OF MATERIAL GRANULAR SILT - CLAY	WEATHERING	DIKE - A TABULAR BODY OF IGNEOUS ROCK THAT CUTS ACROSS THE STRUCTURE OF ADJACENT ROCKS OR CUTS MASSIVE ROCK.
* 40 30 MX 50 MX 51 MN SILS SOILS SOILS SOILS	ORGANIC MATERIAL SOILS SOILS OTHER MATERIAL TRACE OF ORGANIC MATTER 2 - 3% 3 - 5% TRACE 1 - 10%	FRESH ROCK FRESH, CRYSTALS BRIGHT, FEW JOINTS MAY SHOW SLIGHT STAINING, ROCK RINGS UNDER	DIP - THE ANGLE AT WHICH A STRATUM OR ANY PLANAR FEATURE IS INCLINED FROM THE
	LITTLE ORGANIC MATTER 3 - 5% 5 - 12% LITTLE 10 - 20%	HAMMER IF CRYSTALLINE.	HORIZONTAL.
PLACTIC INDEX 6 MX NP 10 MV 10 MV 11 MN 10 MV 10 MV 11 MN 11 MN	MODERATELY ORGANIC 5 - 10% 12 - 20% SOME 20 - 35% HIGHLY ORGANIC >10% >20% HIGHLY 35% AND ABOVE	VERY SLIGHT ROCK GENERALLY FRESH, JOINTS STAINED, SOME JOINTS MAY SHOW THIN CLAY COATINGS IF OPEN, (V SLI.) CRYSTALS ON A BROKEN SPECIMEN FACE SHINE BRIGHTLY. ROCK RINGS UNDER HAMMER BLOWS IF	<u>DIP DIRECTION (DIP AZIMUTH) -</u> THE DIRECTION OR BEARING OF THE HORIZONTAL TRACE OF THE LINE OF DIP, MEASURED CLOCKWISE FROM NORTH.
GROUP INDEX & A & A MX & B MX 12 HX 15 MX No MX MODERATE ORGANIC	GROUND WATER	OF A CRYSTALLINE NATURE. SLIGHT ROCK GENERALLY FRESH, JOINTS STAINED AND DISCOLORATION EXTENDS INTO ROCK UP TO	FAULT - A FRACTURE OR FRACTURE ZONE ALONG WHICH THERE HAS BEEN DISPLACEMENT OF THE
INIMITY TYPES STORE FROMS	✓ WATER LEVEL IN BORE HOLE IMMEDIATELY AFTER DRILLING	(SLI.) 1 INCH, OPEN JOINTS MAY CONTAIN CLAY, IN GRANITOID ROCKS SOME OCCASIONAL FELDSPAR	SIDES RELATIVE TO ONE ANOTHER PARALLEL TO THE FRACTURE.
OF MAIOR GRAVEL AND SAND GRAVEL AND SAND SOILS SOILS MATTER	▼ STATIC WATER LEVEL AFTER 24 HOURS	CRYSTALS ARE DULL AND DISCOLORED. CRYSTALLINE ROCKS RING UNDER HAMMER BLOWS. MODERATE SIGNIFICANT PORTIONS OF ROCK SHOW DISCOLORATION AND WEATHERING EFFECTS. IN	FISSILE - A PROPERTY OF SPLITTING ALONG CLOSELY SPACED PARALLEL PLANES. FLOAT - ROCK FRAGMENTS ON SURFACE NEAR THEIR ORIGINAL POSITION AND DISLODGED FROM
GEN, RATING FAIR TO	→ PERCHED WATER, SATURATED ZONE, OR WATER BEARING STRATA	(MDD.) GRANITOID ROCKS, MOST FELDSPARS ARE DULL AND DISCOLORED, SOME SHOW CLAY, ROCK HAS	PARENT MATERIAL.
AS A EXCELLENT TO GOOD FAIR TO POOR POOR UNSUITABLE SUBGRADE		DULL SOUND UNDER HAMMER BLOWS AND SHOWS SIGNIFICANT LOSS OF STRENGTH AS COMPARED WITH FRESH ROCK.	FLOOD PLAIN (FP) - LAND BORDERING A STREAM, BUILT OF SEDIMENTS DEPOSITED BY
PI OF A-7-5 SUBGROUP IS ≤ LL - 30; PI OF A-7-6 SUBGROUP IS > LL - 30	SPRING OR SEEP	MODERATELY ALL ROCK EXCEPT QUARTZ DISCOLORED OR STAINED, IN GRANITOID ROCKS, ALL FELDSPARS DULL	THE STREAM. FORMATION (FM.) - A MAPPABLE GEOLOGIC UNIT THAT CAN BE RECOGNIZED AND TRACED IN
CONSISTENCY OR DENSENESS RANGE OF STANDARD RANGE OF UNCONFINED	MISCELLANEOUS SYMBOLS	SEVERE AND DISCOLORED AND A MAJORITY SHOW KAOLINIZATION. ROCK SHOWS SEVERE LOSS OF STRENGTH (MOD, SEV.) AND CAN BE EXCAVATED WITH A GEOLOGIST'S PICK, ROCK GIVES "CLUNK" SOUND WHEN STRUCK.	THE FIELD.
PRIMARY SOIL TYPE COMPACTNESS OR PENETRATION RESISTENCE COMPRESSIVE STRENGTH	ROADWAY EMBANKMENT (RE) SPT CPT OFT DET DET DET DESTING SAMPLE OFT DEST BORING DESIGNATIONS	IF TESTED, WOULD YIELD SPT REFUSAL	JOINT - FRACTURE IN ROCK ALONG WHICH NO APPRECIABLE MOVEMENT HAS OCCURRED.
W-AHTDE, CLOSELLA	S - BULK SAMPLE	SEVERE ALL ROCK EXCEPT QUARTZ DISCOLORED OR STAINED ROCK FABRIC CLEAR AND EVIDENT BUT REDUCED IN STRENGTH TO STRONG SOIL. IN GRANITOID ROCKS ALL FELDSPARS ARE KAOLINIZED TO SOME	LEDGE - A SHELF-LIKE RIDGE OR PROJECTION OF ROCK WHOSE THICKNESS IS SMALL COMPARED TO
GENERALLY	SS - SPLIT SPOON	EXTENT, SOME FRAGMENTS OF STRONG ROCK USUALLY REMAIN. IF TESTED, YIELDS SPT N VALUES > 100 BPF	ITS LATERAL EXTENT. LENS - A BODY OF SOIL OR ROCK THAT THINS OUT IN ONE OR MORE DIRECTIONS.
MATERIAL DENSE 20 TO 50	ARTIFICIAL FILL (AF) OTHER SAMPLE THAN ROADWAY EMBANKMENT CORE BORING	VERY SEVERE ALL ROCK EXCEPT QUARTZ DISCOLORED OR STAINED, ROCK FABRIC ELEMENTS ARE DISCERNIBLE BUT	MOTTLED (MOT.) - IRREGULARLY MARKED WITH SPOTS OF DIFFERENT COLORS, MOTTLING IN
(NDN-COHESIVE) VERY DENSE >50	INFERRED SOIL BOUNDARY SAMPLE	(V SEV.) THE MASS IS EFFECTIVELY REDUCED TO SOIL STATUS, WITH ONLY FRAGMENTS OF STRONG ROCK	SOILS USUALLY INDICATES POOR AERATION AND LACK OF GOOD DRAINAGE. PERCHED WATER - WATER MAINTAINED ABOVE THE NORMAL GROUND WATER LEVEL BY THE PRESENCE OF AN
VERY SOFT <2 <0.25 GENERALLY SOFT 2 TO 4 0.25 TO 0.50	"" MONITORING WELL RS - BOCK SAMPLE	REMAINING, SAPROLITE IS AN EXAMPLE OF ROCK WEATHERED TO A DEGREE SUCH THAT ONLY MINOR VESTIGES OF THE ORIGINAL ROCK FABRIC REMAIN. <i>IF TESTED, YIELDS SPT N VALUES < 100 BPF</i>	INTERVENING IMPERVIOUS STRATUM.
SILT-CLAY MEDIUM STIFF 4 TO 8 0.5 TO 1.0	INFERRED ROCK LINE A PIEZOMETER INSTALLATION RT - RECOMPACTED TRIAXIAI THE PIEZOMETER INSTALLATION RT - RECOMPACTED TRIAXIAI	COMPLETE ROCK REDUCED TO SOIL, ROCK FABRIC NOT DISCERNIBLE, OR DISCERNIBLE ONLY IN SMALL AND	RESIDUAL (RES.) SOIL - SOIL FORMED IN PLACE BY THE WEATHERING OF ROCK.
(CDHESIVE) VERY STIFF 15 TO 30 2 TO 4	SLOPE INDICATOR	SCATTERED CONCENTRATIONS. DUARTZ MAY BE PRESENT AS DIKES OR STRINGERS. SAPROLITE IS ALSO AN EXAMPLE.	ROCK QUALITY DESIGNATION (ROD) - A MEASURE OF ROCK QUALITY DESCRIBED BY TOTAL LENGTH OF ROCK SEGMENTS EQUAL TO OR GREATER THAN 4 INCHES DIVIDED BY THE TOTAL LENGTH OF CORE RUN AND
HARD >30 >4	25/025 DIP & DIP DIRECTION OF INSTALLATION CBR - CALIFORNIA BEARING ROCK STRUCTURES RATIO SAMPLE	ROCK HARDNESS	EXPRESSED AS A PERCENTAGE.
TEXTURE OR GRAIN SIZE	— SPT N-VALUE	VERY HARD CANNOT BE SCRATCHED BY KNIFE OR SHARP PICK. BREAKING OF HAND SPECIMENS REQUIRES	SAPROLITE (SAP.) - RESIDUAL SOIL THAT RETAINS THE RELIC STRUCTURE OR FABRIC OF THE
U.S. STD. SIEVE SIZE 4 10 40 60 200 270 OPENING (MM) 4.76 2,00 0.42 0.25 0.075 0.053	SOUNDING ROD REF— SPT REFUSAL	SEVERAL HARD BLOWS OF THE GEOLOGIST'S PICK.	PARENT ROCK. SILL - AN INTRUSIVE BODY OF IGNEOUS ROCK OF APPROXIMATELY UNIFORM THICKNESS AND
CDARSE FINE	ABBREVIATIONS	HARD CAN BE ECRATCHED BY KNIFE OR PICK ONLY WITH DIFFICULTY, HARD HAMMER BLOWS REQUIRED TO DETACH HAND SPECIMEN.	RELATIVELY THIN COMPARED WITH ITS LATERAL EXTENT, THAT HAS BEEN EMPLACED PARALLEL TO THE BEDDING OR SCHISTOSITY OF THE INTRUDED ROCKS.
BOULDER COBBLE GRAVEL SAND SAND SILI CLAY	AR - AUGER REFUSAL HI HIGHLY # - MOISTURE CONTENT BT - BORING TERMINATED MED MEDIUM V - VERY	MODERATELY CAN BE SCRATCHED BY KNIFE OR PICK, GOUGES OR GROOVES TO 0.25 INCHES DEEP CAN BE	SLICKENSIDE - POLISHED AND STRIATED SURFACE THAT RESULTS FROM FRICTION ALONG A FAULT OR
(CSE, SDJ) (F SDJ)	CL CLAY MICA MICACEOUS VST - VANE SHEAR TEST	HARD EXCAVATED BY HARD BLOW OF A GEOLOGIST'S PICK, HAND SPECIMENS CAN BE DETACHED BY MODERATE BLOWS.	SLIP PLANE.
GRAIN MM 305 75 2.0 0.25 0.005 0.005 SIZE IN. 12 3	CPT - CONE PENETRATION TEST MOD MODERATELY WEA WEATHERED CSE COARSE NP - NON PLASTIC 7 - UNIT WEIGHT	MEDIUM CAN BE GROOVED OR GOUGED 0.05 INCHES DEEP BY FIRM PRESSURE OF KNIFE OR PICK POINT.	STANDARD PENETRATION TEST (PENETRATION RESISTANCE) (SPT) - NUMBER OF BLOWS (N OR BPF) OF A 140 LB, HAMMER FALLING 30 INCHES REQUIRED TO PRODUCE A PENETRATION OF 1 FOOT INTO SOIL WITH
SOIL MOISTURE - CORRELATION OF TERMS	DMT - DILATOMETER TEST ORG ORGANIC $\dot{\gamma}_{d}^{-}$ DRY UNIT WEIGHT DPT - DYNAMIC PENETRATION TEST PMT - PRESSUREMETER TEST	HARD CAN BE EXCAVATED IN SMALL CHIPS TO PEICES 1 INCH MAXIMUM SIZE BY HARD BLOWS OF THE POINT OF A GEOLOGIST'S PICK.	A 2 INCH OUTSIDE DIAMETER SPLIT SPOON SAMPLER. SPT REFUSAL IS PENETRATION EQUAL TO OR LESS THAN 0.1 FOOT PER 60 BLOWS.
SOIL MOISTURE SCALE FIELD MOISTURE GUIDE FOR FIELD MOISTURE DESCRIPTION	e - VOID RATIO SAP SAPROLITIC	SOFT CAN BE GROVED OR GOUGED READILY BY KNIFE OR PICK. CAN BE EXCAVATED IN FRAGMENTS	STRATA CORE RECOVERY (SREC.) - TOTAL LENGTH OF STRATA MATERIAL RECOVERED DIVIDED BY TOTAL LENGTH
(ATTERBERG LIMITS) DESCRIPTION OFFICE OF A POLICY OF A	F - FINE SD SAND, SANDY FOSS FOSSILIFEROUS SL SILT, SILTY	FROM CHIPS TO SEVERAL INCHES IN SIZE BY MODERATE BLOWS OF A PICK POINT. SMALL, THIN PIECES CAN BE BROKEN BY FINGER PRESSURE.	OF STRATUM AND EXPRESSED AS A PERCENTAGE.
- SATURATED - USUALLY LIQUID; VERY WET, USUALLY (SAT.) FROM BELOW THE GROUND WATER TABLE	FRAC FRACTURED, FRACTURES SLI SLIGHTLY	VERY CAN BE CARVED WITH KNIFE. CAN BE EXCAVATED READILY WITH POINT OF PICK, PIECES 1 INCH	STRATA ROCK QUALITY DESIGNATION (SROD) - A MEASURE OF ROCK QUALITY DESCRIBED BY TOTAL LENGTH OF ROCK SEGMENTS WITHIN A STRATUM EQUAL TO OR GREATER THAN 4 INCHES DIVIDED BY THE
LL LIQUID LIMIT	FRAGS FRAGMENTS TCR - TRICONE REFUSAL	SOFT OR MORE IN THICKNESS CAN BE BROKEN BY FINGER PRESSURE. CAN BE SCRATCHED READILY BY FINGERNAIL.	TOTAL LENGTH OF STRATA AND EXPRESSED AS A PERCENTAGE.
PLASTIC SEMISOLID; REQUIRES DRYING TO RANGE WET - (W) ATTAIN OPTIMUM MOISTURE	EQUIPMENT USED ON SUBJECT PROJECT	FRACTURE SPACING BEDDING	IOPSOIL (TS.) - SURFACE SOILS USUALLY CONTAINING ORGANIC MATTER.
(PI) PL PLASTIC LIMIT	DRILL UNITS: ADVANCING TOOLS: HAMMER TYPE:	TERM SPACING TERM THICKNESS VERY THICKLY BEDDED > 4 FEET	BENCH MARK: BL-103, BL STATION 16+66.1
OM OPTIMUM MOISTURE - MOIST - (M) SOLID; AT OR NEAR OPTIMUM MOISTURE	CLAY DITS AUTOMATIC MANUAL	VERY WIDE MURE HAN 10 FEET THICKLY BEDDED 1.5 - 4 FEET	ELEVATION: 153.59 FT.
SL_ SHRINKAGE LIMIT	MOBILE B- CLAY BITS	MODERATELY CLOSE 1 TO 3 FEET VERY THINLY BEDDED 0.15 - 1.5 FEET VERY THINLY BEDDED 0.03 - 0.16 FFFT	
REQUIRES ADDITIONAL WATER TO - DRY - (D) ATTAIN OPTIMUM MOISTURE	6' CONTINUOUS FLIGHT AUGER CORE SIZE:	CLOSE 0.16 TO 1 FEET VERY CLOSE LESS THAN 0.16 FEET THINKLY LAMINATED 4.008 F.0.03 FEET THINKLY LAMINATED 4.008 FEET	NOTES:
HITHIN OF TIPON POISTORE	5 HOLLOW HODENS	INDURATION	-
PLASTICITY	CME-45C HARD FACED FINGER BITS	FOR SEDIMENTARY ROCKS, INDURATION 1S THE HARDENING OF THE MATERIAL BY CEMENTING, HEAT, PRESSURE, ETC.	1
PLASTICITY INDEX (PI) DRY STRENGTH NONPLASTIC 8-5 VERY LDW	TUNGCARBIDE INSERTS -H	RUBBING WITH FINGER FREES NUMEROUS GRAINS;	
LOW PLASTICITY 6-15 SLIGHT	CASING W/ ADVANCER HAND TODLS:	GENTLE BLOW BY HAMMER DISINTEGRATES SAMPLE.	
MED. PLASTICITY 16-25 MEDIUM HIGH PLASTICITY 26 OR MORE HIGH	PORTABLE HOIST TRICONESTEEL TEETH POST HOLE DIGGER	MODERATELY INDURATED GRAINS CAN BE SEPARATED FROM SAMPLE WITH STEEL PROBE; BREAKS EASILY WHEN HIT WITH HAMMER.	
COLOR	■ CME-550X TRICONE TUNGCARB. HAND AUGER		
DESCRIPTIONS MAY INCLUDE COLOR OR COLOR COMBINATIONS (TAN, RED, YELLOW-BROWN, BLUE-GRAY).	CORE BIT SOUNDING HOD	INDURATED GRAINS ARE DIFFICULT TO SEPARATE WITH STEEL PROBE; DIFFICULT TO BREAK WITH HAMMER.	
MODIFIERS SUCH AS LIGHT, DARK, STREAKED, ETC. ARE USED TO DESCRIBE APPEARANCE.	VANE SHEAR TEST	EXTREMELY INDURATED SHARP HAMMER BLOWS REQUIRED TO BREAK SAMPLE;	,
	,	SAMPLE BREAKS ACROSS GRAINS.	1

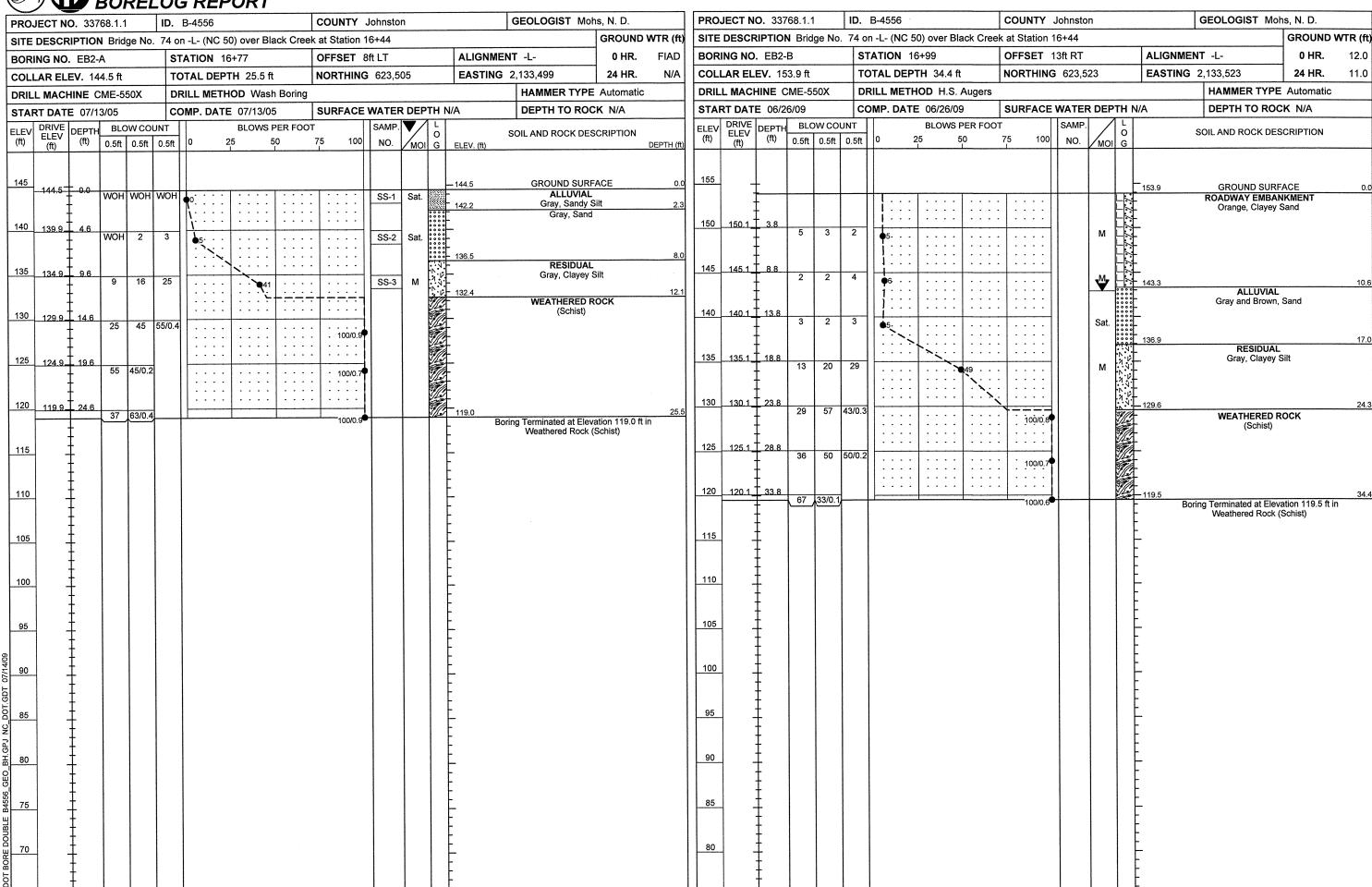






	ID. B-4556	COUNTY Johnston	GEOLOGIST M			ECT NO.				B-4556	 	COUNTY		n		GEOLOGIST	
ITE DESCRIPTION Bridge No.	74 on -L- (NC 50) over Black Cre	eek at Station 16+44	,	GROUND WTR (ft)				ridge No			 ack Creek						GROUND V
ORING NO. EB1-A	STATION 15+89	OFFSET 13ft LT	ALIGNMENT -L-	0 HR. 14.0		NG NO.				ATION 15	 	OFFSET 1				SNMENT -L-	0 HR.
OLLAR ELEV. 152.8 ft	TOTAL DEPTH 37.9 ft	NORTHING 623,419	EASTING 2,133,480	24 HR. 10.9		AR ELE\				TAL DEP	 	NORTHING	623,4	14	EAS	TING 2,133,509	24 HR.
PRILL MACHINE CME-550X	DRILL METHOD H.S. Augers		HAMMER TYP	PE Automatic		MACHII				ILL METH	 						YPE Automatic
TART DATE 06/25/09	COMP. DATE 06/25/09	SURFACE WATER DEPTH N	/A DEPTH TO RO	OCK N/A		T DATE		·····		MP. DATE	 	SURFACE	-,		H N/A	DEPTH TO	ROCK N/A
LEV CRIVE ELEV (ft) DEPTH BLOW COU		OT SAMP. C C C C C C C C C	SOIL AND ROCK D	ESCRIPTION DEPTH (ft)	ELEV (ft)	DRIVE DI (ft)	EPTH BI (ft) 0.5f	LOW CO ft 0.5ft		0 2	PER FOOT	5 100	SAMP.	MOI	O G	SOIL AND ROCK	DESCRIPTION
150.7 + 2.1 3 3	4		- 152.8 GROUND SU ROADWAY EMB Orange, Claye	ANKMENT	155	149.5] : : :] : : :] : : :	 				154.1	GROUND S ROADWAY EN Orange, Cla	BANKMENT
145 7 - 7.1	2	M M	145.2 ALLUVI/ Gray, Silty	7.6 AL Clay	145	144.5	9.6		1	•6				×	145.1	ALLU \ Gray, Sil	/IAL ty Clay
140 12.1 3 2	2	M M	- 136.3	16.5	140	139.5	14.6	3	2	55				Sat.	141.1	Gray, S	
135.7 + 17.1 6 9 130.7 + 22.1 19 24	12 21	M N N N N N N N N N N N N N N N N N N N	- RESIDU, Gray, Claye		130	134.5	9	20	32		52			M	N	RESID Gray, Cla WEATHER	yey Silt
25 125.7 + 27.1 24 34	40	74 M / 74	- -		125	129.5	29.6	62/0.2			 	100/0.7		ARCZARCZARCZAR	118.9	(Schi	st)
20 120.7 = 32.1 35 65/0.4		100/0.5	WEATHERED (Schist)		120	119.5	34.6	36/0.1				100/0.6			118.9	Boring Terminated at Weathered R	Elevation 118.9 ft in ock (Schist)
15 115.7 + 37.1 50 50/0.3		100/0.8	Boring Terminated at El Weathered Roc	37.9 levation 114.9 ft in k (Schist)		† † †									- - - - -		
05			-		110	+											
100					100	T + +									- - - -		
95			- - -		95_	† + + +									- - - - -		
90			· · ·		90	† + + +											
85			· -		85	†									<u>-</u> - - -		
80 7			<u>-</u>		80	‡									.		

NCDOT GEOTECHNICAL ENGINEERING UNIT



SHEET 8 33768.1.1 (B-4556)

EB2-A

	SOIL TEST RESULTS														
SAMPLE			DEPTH	AASHTO				% BY W	/EIGHT		% PAS	SING (S	IEVES)	%	%
NO.	OFFSET	STATION	INTERVAL	CLASS.	L.L.	P.I.	C.SAND	F.SAND	SILT	CLAY	10	40	200	MOISTURE	ORGANIC
SS-1	8' LT	16+77	0.0-1.5	A-4(4)	26	8	2.6	34.7	38.4	24.3	100	99	73	-	-
SS-2	8' LT	16+77	4.6-6.1	A-3(0)	21	NP	49.5	44.5	2.9	3.0	95	65	9	*	-
SS-3	8' LT	16+77	9.6-11.1	A-5(14)	48	9	2.8	3.6	79.3	14.2	100	99	95	-	•

FIELD SCOUR REPORT

WBS:	S: 33768.1.1 TIP: B-4556 COUNTY: Johnston	
DESCRIPTION(1):): Bridge No. 74 on -L- (NC 50) over Black Creek at Station 16+44	
	EXISTING BRIDGE	
Information from:	n: Field Inspection X Microfilm (reel pos: Other (explain))
Bridge No.: Foundation Type:	74 Length: 80.4' Total Bents: 5 Bents in Channel: 3 e: Concrete caps on timber piles with steel crutch bents.	Bents in Floodplain: 2
EVIDENCE OF S Abutments or E	F SCOUR(2) r End Bent Slopes: None	
Interior Bents:	s: None	
Channel Bed:	d: None	
Channel Bank:	k: None	
	OUR PROTECTION 3): Wooden wing walls at end bents.	
Extent(4):	e): 5' to 6' up and down stream.	
Effectiveness(5):	i): Effective	
Obstructions(6):	s): None	

INSTRUCTIONS

- 1 Describe the specific site's location, including route number and body of water crossed.
- 2 Note scour evidence at existing end bents or abutments (e.g. undermining, sloughing, degradations).
- 3 Note existing scour protection (e.g. rip rap).
- 4 Describe extent of existing scour protection.
- 5 Describe whether or not the scour protection appears to be working.
- 6 Note obstructions such as dams, fallen trees, debris at bents, etc.
- 7 Describe the channel bed material based on observation and/or samples. Include any lab results with report.
- 8 Describe the channel bank material based on observation and/or samples. Include any lab results with report.
- **9** Describe the material covering the banks (e.g. grass, trees, rip rap, none).
- 10 Determine the approximate floodplain width from field observation or a topographic map.
- 11 Describe the material covering the floodplain (e.g. grass, trees, crops).
- 12 Use professional judgement to specify if the stream is degrading, aggrading, or static.
- 13 Describe potential and direction of the stream to migrate laterally during the bridge's life (approx. 100 years).
- 14 Give the design scour elevation (DSE) expected over the life of the bridge (approx. 100 years). This elevation can be given as a range across the site, or for each bent. Discuss the relationship between the Hydraulics Unit theoritical scour and the DSE. If the DSE is dependent on scour counter measures, explain (e.g. rip rap armoring on slopes). The DSE is based on the erodability of materials, giving consideration to the influence of joints, foliation, bedding characteristics, % core recovery, % RQD, differential weathering, shear strength, observations at existing structures, other tests deemed appropriate, and overall geologic conditions at the site.

			DES	SIGN IN	IFORM	ATIO	N						
Channel	Bed Material(7):												
Channel B	Bank Material(8):												
Channe	l Bank Cover(9):	Grass											
Flood	Iplain Width(10):												
	plain Cover(11):												
	Stream is(12):								tic X				
hannel Migratio										-			
_	ration Tendency(13): None ons and Other Comments:												
Observations	and Other Comm												
DESIGN SCO	UR ELEVATION	JQ(1A)				For	ot Y	Mete	are				
DEGIGIA GOO	OK LLLVATION	10(14)				100	<u> </u>	Wick		-			
	[<u>400 </u>	T		т	1						<u> </u>		
	130.5												
Comparison o	f DSE to Hydrau	lics Unit t	theoretic	eal scour									
The Design So	cour Elevation a	cross the				to the F	łydraulic	Units The	oretical S	Scour Ele	evation		
of 122.5' for th	ne 100 year even	nt.											
SOIL ANALY	SIS RESULTS F	ROM CH	IANNEL	BED A	ND BAN	K MAT	ERIAL						
Bed or Bank													
Sample No.													
Retained #4 Passed #10	See Sheet 9,			-									
Passed #40	"Soil Test Resu	ults",		<u> </u>									
Passed #200	for samples:												
Coarse Sand	SS-1												
Fine Sand	SS-2	S-2											
Silt													
Clay													
LĽ													
PI													
AASHTO													
Station					<u> </u>								
Offset Depth					<u> </u>								

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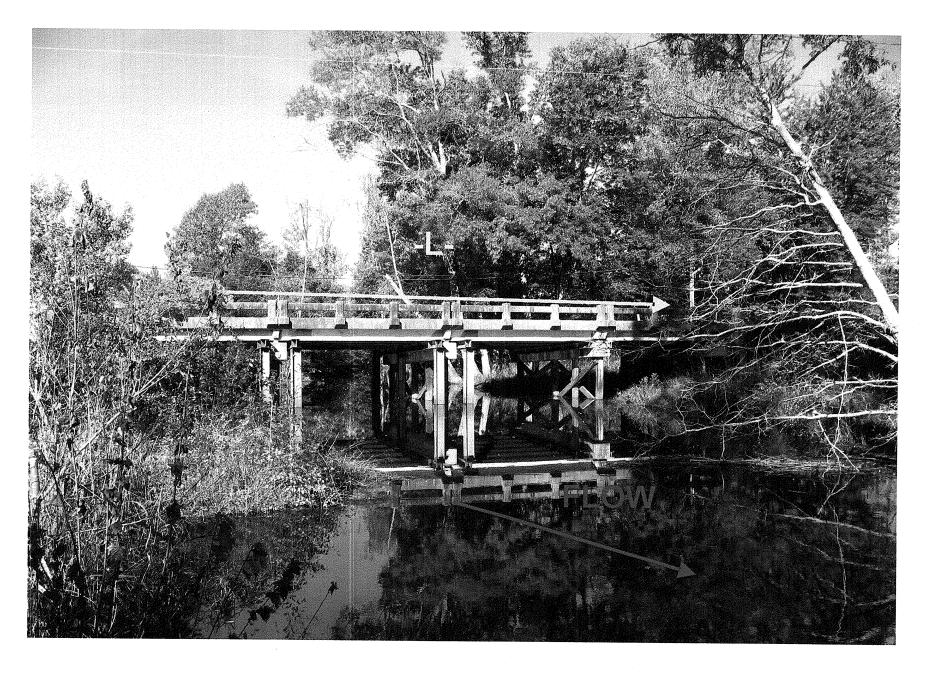
Form GEU-017e Revised 7/26/2007

Date: 6/25/2009

SHEET

Site Photograph

Bridge No. 74 on -L- (NC 50) Over Black Creek at Station 16+44



D: B-4560

20/ECT: 33771.1.1

STATE OF NORTH CAROLINA

DEPARTMENT OF TRANSPORTATION
DIVISION OF HIGHWAYS
GEOTECHNICAL ENGINEERING UNIT

CONTENTS

SHEET	DESCRIPTION
I	TITLE SHEET
2	LEGEND
3	SITE PLAN
4	PROFILE(S)
5-6	CROSS SECTION(S)
7-10	BORE LOG(S)
11	SOIL TEST RESULTS
12	SCOUR REPORT
13	SITE PHOTOGRAPH(S)

STRUCTURE SUBSURFACE INVESTIGATION

PROJ. REFERENCE NO. 33771.1.1(B-4560) F.A. PROJ. BRZ-1331(10)

COUNTY JOHNSTON

PROJECT DESCRIPTION BRIDGE NO. 102 ON -L- SR 1331

(FEDERAL ROAD) OVER BLACK CREEK

INVENTORY

 STATE
 STATE PROJECT REFERENCE NO.
 SHEET STEAM
 No. 1
 N.C.
 33771.1.1(B-4560)
 1
 13

CAUTION NOTICE

THE SUBSURFACE INFORMATION AND THE SUBSURFACE INVESTIGATION ON WHICH IT IS BASED WERE MADE FOR THE PURPOSE OF STUDY, PLANNING, AND DESIGN, AND NOT FOR CONSTRUCTION OR PAY PURPOSES. THE VARIOUS FIELD BORING LOGS, ROCK CORES, AND SOL TEST DATA AVAILABLE MAY BE REVIEWED OR INSPECTED IN RALEIGH BY CONTACTING THE N. C. DEPARTMENT OF TRANSPORTATION, CEOTECHNICAL ENGINEERING LUNT AT 1919 250-40408. RETHER THE SUBSURFACE PLANS AND REPORTS, NOR THE FIELD BORING LOGS, ROCK CORES, OR SOIL TEST DATA ARE PART OF THE CONTRACT.

GENERAL SOIL AND ROCK STRATA DESCRIPTIONS AND INDICATED BOUNDARIES ARE BASED ON A GEOTECHNICAL INTERPRETATION OF ALL AVAILABLE SUBSURFACE DATA AND MAY NOT NECESSARILY REFLECT THE ACTUAL SUBSURFACE CONDITIONS BETWEEN BORINGS OR BETWEEN SAMPLED STRATA WITHIN THE BOREHOLE, THE LABORATORY SAMPLE DATA AND THE IN SITU UN-PLACE) TEST DATA CAN BE RELIED ON ONLY TO THE DEGREE OF RELIABLITY INMERSTRIP IN THE STRANDARD TEST METHOD, THE OBSERVED WATER LEVELS OR SOIL MOISTURE CONDITIONS INDICATED IN THE SUBSURFACE INVESTIGATIONS ARE AS RECORDED AT THE TIME OF THE INVESTIGATION, THESE WATER LEVELS OR SOIL MOISTURE CONDITIONS OF CONDITIONS INDICATED IN CLIMATIC CONDITIONS TO CLIMATIC CONDITIONS INDICATED IN THE SUBSURFACE INVESTIGATIONS ARE AS RECORDED AT THE TIME ACCORDING TO CLIMATIC CONDITIONS INCLUDING TEMPERATURES, PRECIPITATION, AND WIND, AS WELL AS OTHER NON-CLIMATIC FACTORS.

THE BIDDER OR CONTRACTOR IS CALITIONED THAT DETAILS SHOWN ON THE SUBSURFACE PLANS ARE PRELIMINARY ONLY AND IN MANY CASES THE FINAL DESIGN DETAILS ARE DIFFERENT. FOR BIDDING AND CONSTRUCTION PLANS AND DOLUMENTS FOR FINAL DESIGN INFORMATION ON THIS PROJECT. THE DEPARTMENT DOES NOT WARRANT OR CULARANTEE THE SUFFICIENCY OR ACCURACY OF THE INVESTIGATION MADE, NOR THE INTERPRETATIONS MADE, OR OPINION OF THE DEPARTMENT AS TO THE TYPE OF MATERIALS AND CONDITIONS TO BE ENCOUNTERED. THE BIDDER OR CONTRACTOR IS CALITIONED TO MAKE SUCH INDEPENDENT SUBSURFACE INVESTIGATIONS AS HE DEEMS NECESSARY TO SATISTY HANGEL AS TO CONDITIONS TO BE ENCOUNTERED ON THIS PROJECT. THE CONTRACTOR SHALL HAVE NO CLAIM FOR ADDITIONAL COMPENSATION OR FOR AN EXTENSION OF TIME FOR ANY BREADN RESULTING FROM THE ACTUAL CONDITIONS TO BE ENCOUNTERED AT THE SITE DIFFERING FROM THOSE INDICATED IN THE SUBSURFACE INFORMATION.

PERSONNEL O.B. OTI

T.T. WALKER

CONSULTANT PERSONNEL

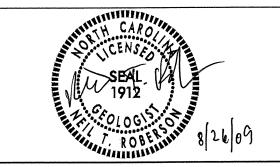
F&R

INVESTIGATED BY O.B. OTT

ECKED BY N.T. ROBERSON

SUBMITTED BY__ N.T. ROBERSON

DATE AUGUST 2009



NORTH CAROLINA DEPARTMENT OF TRANSPORTATION

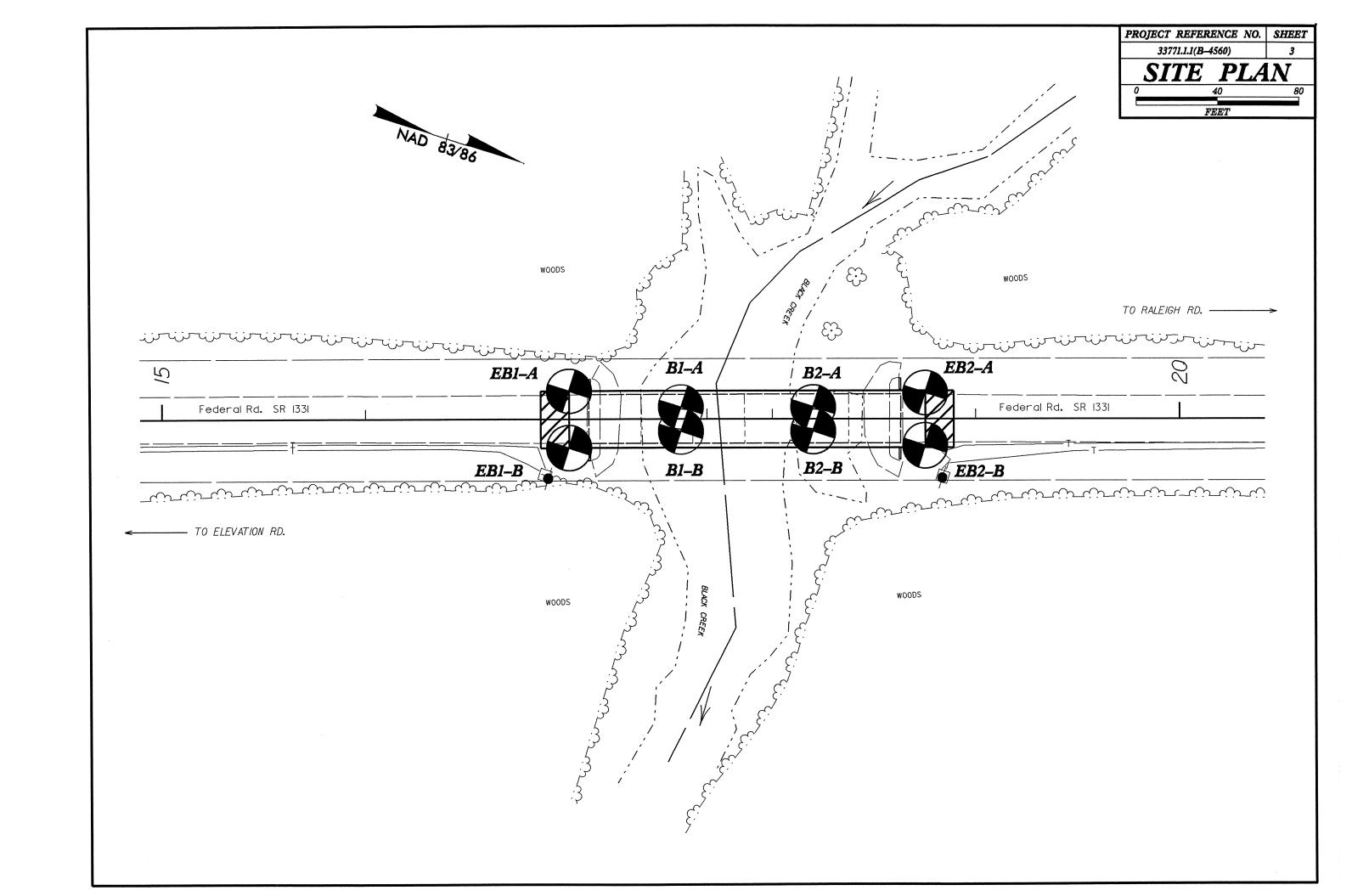
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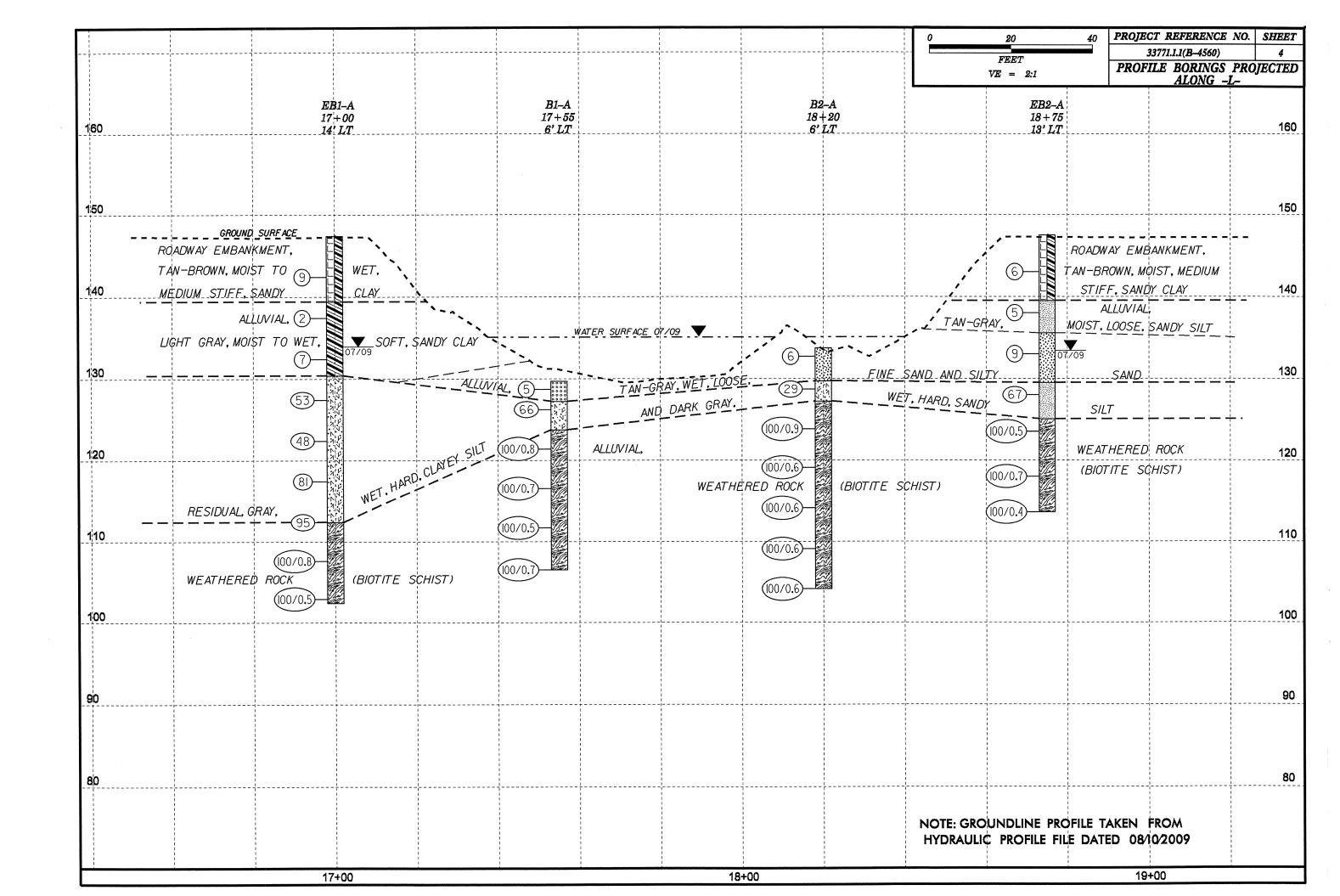
GEOTECHNICAL ENGINEERING UNIT

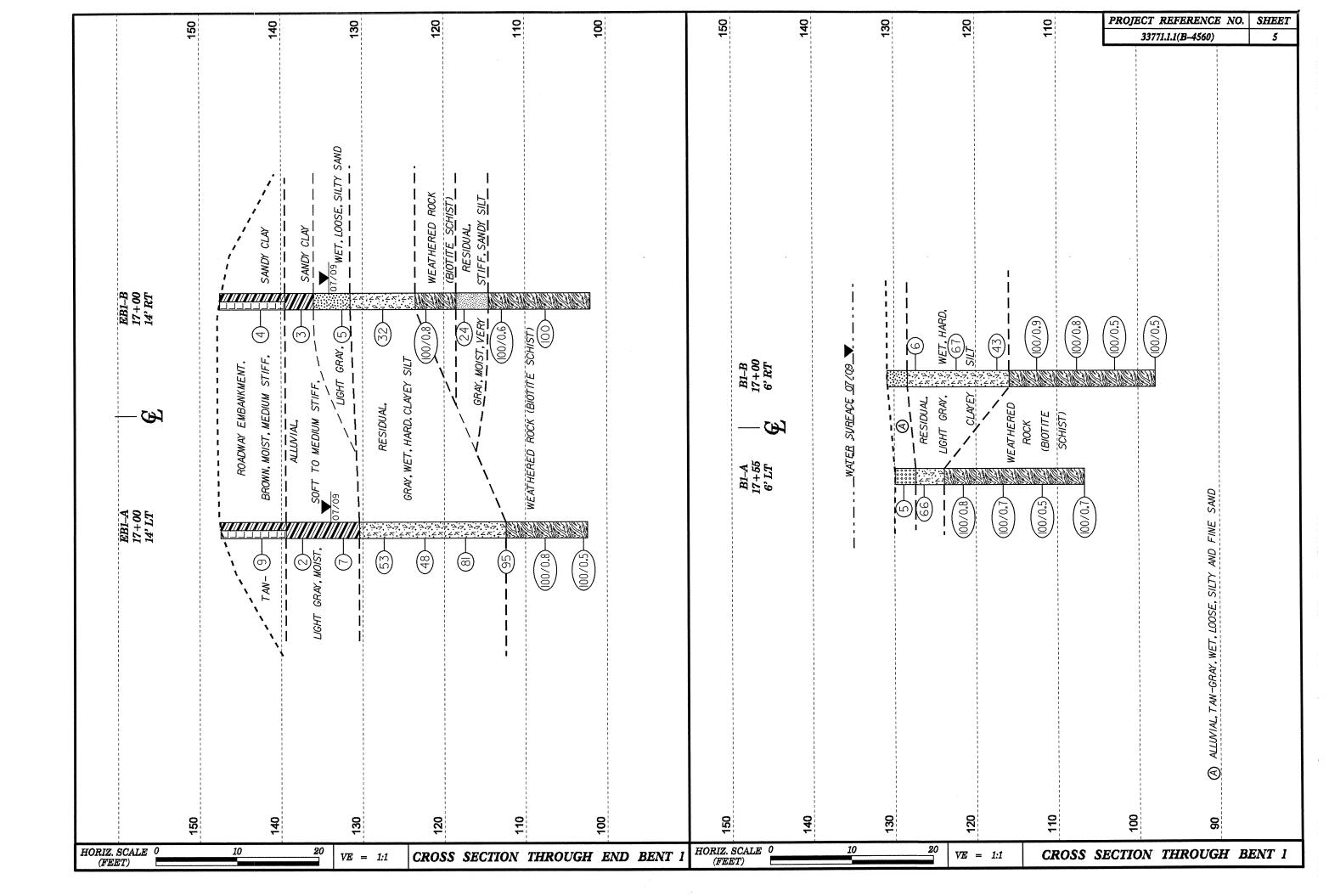
SUBSURFACE INVESTIGATION

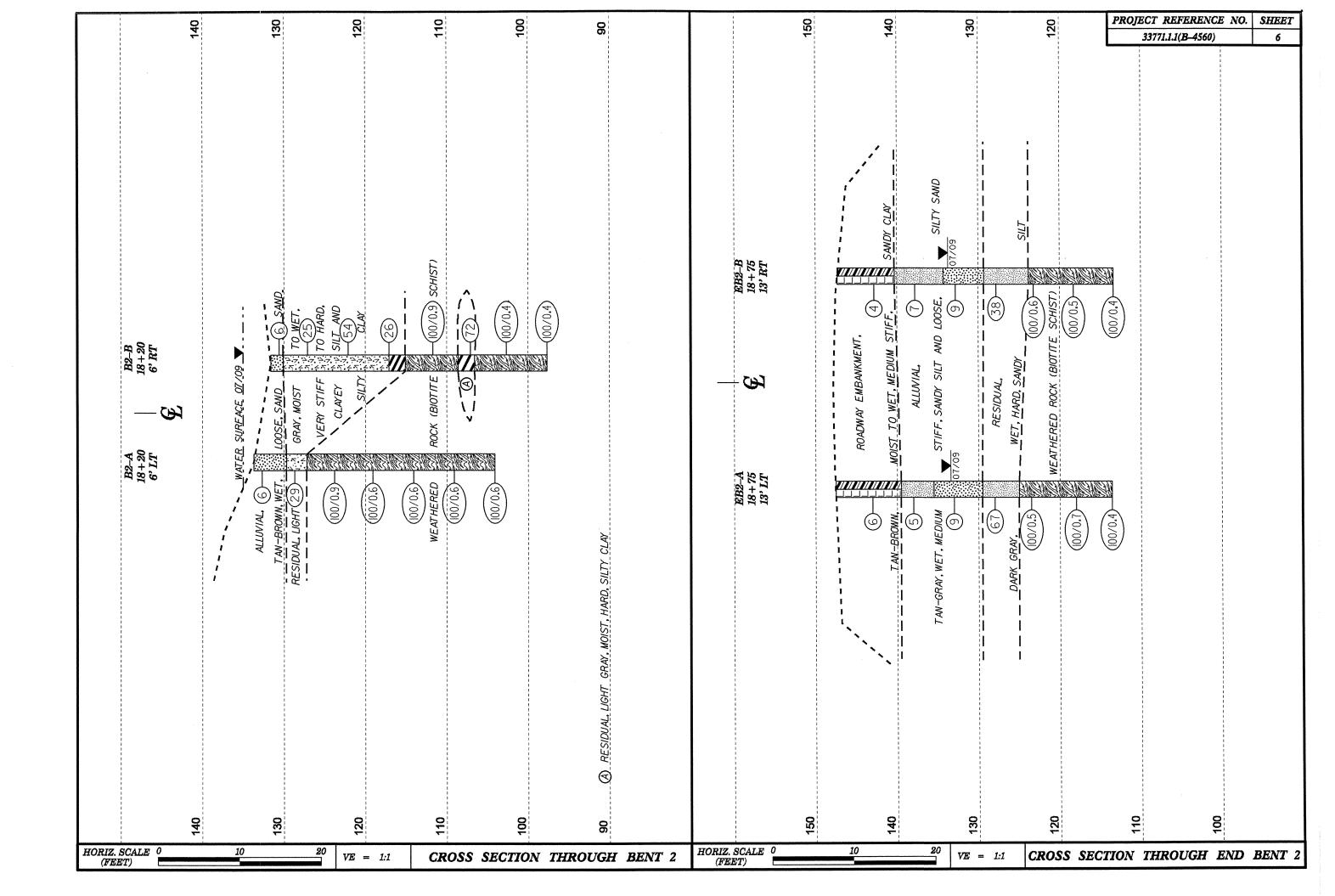
SOIL AND ROCK LEGEND, TERMS, SYMBOLS, AND ABBREVIATIONS

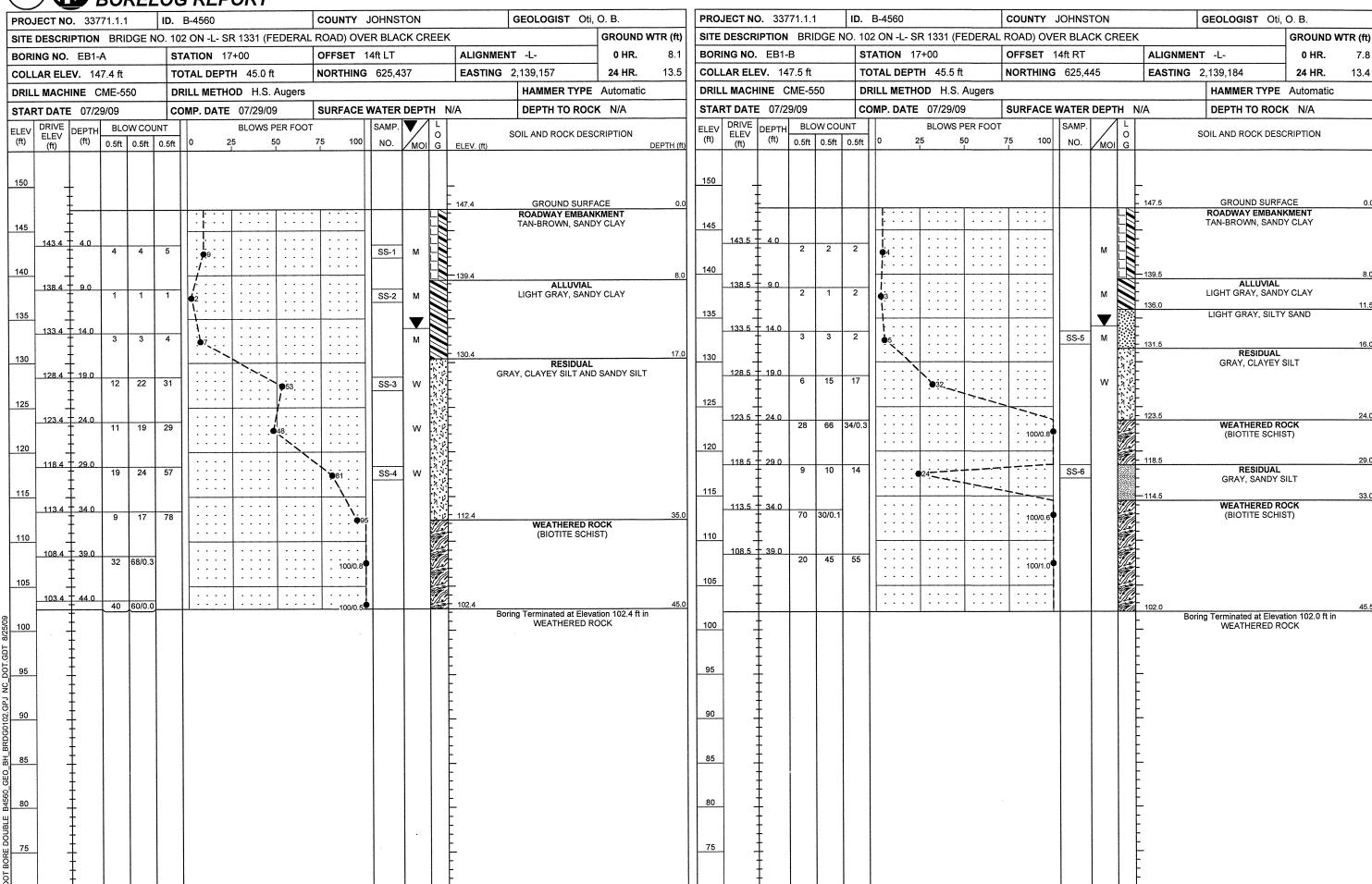
	SOIL AND ROCK LEGEND, TERM	1S, SYMBOLS, AND ABBREVIATIONS	
SOIL DESCRIPTION	GRADATION	ROCK DESCRIPTION	TERMS AND DEFINITIONS
SDIL IS CONSIDERED TO BE THE UNCONSOLIDATED, SEMI-CONSOLIDATED, OR WEATHERED EARTH MATERIALS	WELL GRADED - INDICATES A GOOD REPRESENTATION OF PARTICLE SIZES FROM FINE TO COARSE. UNIFORM - INDICATES THAT SOIL PARTICLES ARE ALL APPROXIMATELY THE SAME SIZE. (ALSO	HARD ROCK IS NON-COASTAL PLAIN MATERIAL THAT IF TESTED, WOULD YIELD SPT REFUSAL, AN INFERRED ROCK LINE INDICATES THE LEVEL AT WHICH NON-COASTAL PLAIN MATERIAL WOULD YIELD SPT REFUSAL,	ALLUVIUM (ALLUV.) - SOILS THAT HAVE BEEN TRANSPORTED BY WATER.
THAT CAN BE PENETRATED WITH A CONTINUOUS FLIGHT POWER AUGER, AND YIELD LESS THAN 100 BLOWS PER FOOT ACCORDING TO STANDARD PENETRATION TEST (AASHTO T206, ASTM D-1586). SOIL	PODRLY GRADED) GAP-GRADED - INDICATES A MIXTURE OF UNIFORM PARTICLES OF TWO OR MORE SIZES.	SPT REFUSAL IS PENETRATION BY A SPLIT SPOON SAMPLER EQUAL TO OR LESS THAN 0.1 FOOT PER 60 BLOWS. IN NON-COASTAL PLAIN MATERIAL, THE TRANSITION BETWEEN SOIL AND ROCK IS OFTEN REPRESENTED BY A ZONE	ADUIFER - A WATER BEARING FORMATION OR STRATA.
CLASSIFICATION IS BASED ON THE AASHTO SYSTEM. BASIC DESCRIPTIONS GENERALLY SHALL INCLUDE:	ANGULARITY OF GRAINS	OF WEATHERED ROCK. ROCK MATERIALS ARE TYPICALLY DIVIDED AS FOLLOWS:	ARENACEOUS - APPLIED TO ROCKS THAT HAVE BEEN DERIVED FROM SAND OR THAT CONTAIN SAND.
CONSISTENCY, COLOR, TEXTURE, MOISTURE, AASHTO CLASSIFICATION, AND OTHER PERTINENT FACTORS SUCH AS MINERALOGICAL COMPOSITION, ANGULARITY, STRUCTURE, PLASTICITY, ETC. EXAMPLE:	THE ANGULARITY OR ROUNDNESS OF SOIL GRAINS IS DESIGNATED BY THE TERMS: ANGULAR,		ARGILLACEOUS - APPLIED TO ALL ROCKS OR SUBSTANCES COMPOSED OF CLAY MINERALS, OR HAVING A NOTABLE PROPORTION OF CLAY IN THEIR COMPOSITION, AS SHALE, SLATE, ETC.
VERY STIFF, GRAY, SILTY CLAY, MOIST WITH INTERBEDDED FINE SAND LAYERS, HIGHLY PLASTIC, A-7-6	SUBANGULAR, SUBROUNDED, OR ROUNDED.	WEATHERED NON-COASTAL PLAIN MATERIAL THAT WOULD YIELD SPT N VALUES > 100 BLOWS PER FOOT IF TESTED.	ARTESIAN - GROUND WATER THAT IS UNDER SUFFICIENT PRESSURE TO RISE ABOVE THE LEVEL
SOIL LEGEND AND AASHTO CLASSIFICATION	MINERALOGICAL COMPOSITION	CRYSTALLINE FINE TO COARSE GRAIN IGNEOUS AND METAMORPHIC ROCK THAT	AT WHICH IT IS ENCOUNTERED, BUT WHICH DOES NOT NECESSARILY RISE TO OR ABOVE THE GROUND SURFACE.
GENERAL GRANULAR MATERIALS SILT-CLAY MATERIALS ORGANIC MATERIALS CLASS. (≤ 35% PASSING *200) (> 35% PASSING *200) ORGANIC MATERIALS	MINERAL NAMES SUCH AS QUARTZ, FELDSPAR, MICA, TALC, KAOLIN, ETC. ARE USED IN DESCRIPTIONS WHENEVER THEY ARE CONSIDERED OF SIGNIFICANCE.	ROCK (CR) WOULD YIELD SPT REFUSAL IF TESTED, ROCK TYPE INCLUDES GRANITE, GNEISS, GABBRO, SCHIST, ETC.	CALCAREOUS (CALC.) - SOILS THAT CONTAIN APPRECIABLE AMOUNTS OF CALCIUM CARBONATE.
GROUP A-1 A-3 A-2 A-4 A-5 A-6 A-7 A-1, A-2 A-4, A-5	COMPRESSIBILITY	NON-CRYSTALLINE FINE TO COARSE GRAIN METAMORPHIC AND NON-COASTAL PLAIN SEDIMENTARY ROCK THAT WOULD YELLD SPT REFUSAL IF TESTED, ROCK TYPE	COLLUVIUM - ROCK FRAGMENTS MIXED WITH SOIL DEPOSITED BY GRAVITY ON SLOPE OR AT BOTTOM
CLASS. A-1-a A-1-b A-2-4 A-2-5 A-2-6 A-2-7 A-7-8 A-3 A-6, A-7	SLIGHTLY COMPRESSIBLE LIQUID LIMIT LESS THAN 31	INCLUDES PHYLLITE, SLATE, SANDSTONE, ETC.	OF SLOPE.
SYMBOL 000000000000000000000000000000000000	MODERATELY COMPRESSIBLE LIQUID LIMIT EQUAL TO 31-50 HIGHLY COMPRESSIBLE LIQUID LIMIT GREATER THAN 50	COASTAL PLAIN SEDIMENTARY ROCK COASTAL PLAIN SEDIMENTS CEMENTED INTO ROCK, BUT MAY NOT YIELD SPT REFUSAL. ROCK TYPE INCLUDES LIMESTONE, SANDSTONE, CEMENTED	CORE RECOVERY (REC.) - TOTAL LENGTH OF ALL MATERIAL RECOVERED IN THE CORE BARREL DIVIDED BY TOTAL LENGTH OF CORE RUN AND EXPRESSED AS A PERCENTAGE.
7. PASSING SILT-	PERCENTAGE OF MATERIAL	(CP) SHELL BEDS, ETC. WEATHERING	DIKE - A TABULAR BODY OF IGNEOUS ROCK THAT CUTS ACROSS THE STRUCTURE OF ADJACENT
# 10 50 MX GRANULAR CLAY MULK,	ORGANIC MATERIAL GRANULAR SILT - CLAY SOILS SOILS OTHER MATERIAL		ROCKS OR CUTS MASSIVE ROCK.
# 40 30 MX 51 MX 15 MX 35 MX 35 MX 35 MX 35 MX 36 MN 36 MN	TRACE OF ORGANIC MATTER 2 - 3% 3 - 5% TRACE 1 - 10%	FRESH ROCK FRESH, CRYSTALS BRIGHT, FEW JOINTS MAY SHOW SLIGHT STAINING ROCK RINGS UNDER HAMMER IF CRYSTALLINE.	DIP - THE ANGLE AT WHICH A STRATUM OR ANY PLANAR FEATURE IS INCLINED FROM THE HORIZONTAL.
LIGUID LIMIT 48 MX 41 MN 48 MX 41 MN 48 MX 41 MN 48 MX 41 MN SOILS WITH	LITTLE ORGANIC MATTER	VERY SLIGHT ROCK GENERALLY FRESH, JOINTS STAINED, SOME JOINTS MAY SHOW THIN CLAY COATINGS IF OPEN,	DIP DIRECTION (DIP AZIMUTH) - THE DIRECTION OR BEARING OF THE HORIZONTAL TRACE OF
PLASTIC INDEX 6 MX NP 18 MX 18 MX 11 MN 11 MN 18 MX 18 MX 11 MN 11 MN LITTLE OR HIGHLY	HIGHLY ORGANIC >10% >20% HIGHLY 35% AND ABOVE	(V SLI.) CRYSTALS DN A BROKEN SPECIMEN FACE SHINE BRIGHTLY. ROCK RINGS UNDER HAMMER BLOWS IF OF A CRYSTALLINE NATURE.	THE LINE OF DIP, MEASURED CLOCKWISE FROM NORTH.
GROUP INDEX 8 8 8 4 MX 8 MX 12 MX 16 MX No MX MODERATE AMOUNTS OF SOILS	GROUND WATER	SLIGHT ROCK GENERALLY FRESH, JOINTS STAINED AND DISCOLORATION EXTENDS INTO ROCK UP TO	FAULT - A FRACTURE OR FRACTURE ZONE ALONG WHICH THERE HAS BEEN DISPLACEMENT OF THE SIDES RELATIVE TO ONE ANOTHER PARALLEL TO THE FRACTURE.
USUAL ITYES SILTY PROBLEM HARDS FINE SILTY OR CLAYEY SILTY CLAYEY ORGANIC	water level in bore hole immediately after drilling	(SLI.) 1 INCH. OPEN JOINTS MAY CONTAIN CLAY. IN GRANITOID ROCKS SOME OCCASIONAL FELDSPAR CRYSTALS ARE DULL AND DISCOLORED, CRYSTALLINE ROCKS RING UNDER HAMMER BLDWS.	FISSILE - A PROPERTY OF SPLITTING ALONG CLOSELY SPACED PARALLEL PLANES.
MATERIALS SAND GRAVEL AND SAND SOILS SOILS	STATIC WATER LEVEL AFTER 24 HOURS	MODERATE SIGNIFICANT PORTIONS OF ROCK SHOW DISCOLORATION AND WEATHERING EFFECTS. IN	FLOAT - ROCK FRAGMENTS ON SURFACE NEAR THEIR ORIGINAL POSITION AND DISLODGED FROM
GEN. RATING AS A EXCELLENT TO GOOD FAIR TO POOR POOR UNSUITABLE	PERCHED WATER, SATURATED ZONE, DR WATER BEARING STRATA	(MOD.) GRANITOID ROCKS, MOST FELDSPARS ARE DULL AND DISCOLORED, SOME SHOW CLAY. ROCK HAS DULL SOUND UNDER HAMMER BLOWS AND SHOWS SIGNIFICANT LOSS OF STRENGTH AS COMPARED	PARENT MATERIAL.
SUBGRADE	SPRING OR SEEP	WITH FRESH ROCK.	FLOOD PLAIN (FP) - LAND BORDERING A STREAM, BUILT OF SEDIMENTS DEPOSITED BY THE STREAM.
PI OF A-7-5 SUBGROUP IS ≤ LL - 30 ;PI OF A-7-6 SUBGROUP IS > LL - 30	MISCELLANEOUS SYMBOLS	MODERATELY ALL ROCK EXCEPT QUARTZ DISCOLORED OR STAINED. IN GRANITOID ROCKS, ALL FELDSPARS DULL SEVERE AND DISCOLORED AND A MAJORITY SHOW KAOLINIZATION. ROCK SHOWS SEVERE LOSS OF STRENGTH	FORMATION (FM.) - A MAPPABLE GEOLOGIC UNIT THAT CAN BE RECOGNIZED AND TRACED IN
CONSISTENCY OR DENSENESS CONSISTENCY OR DENSENESS RANGE OF STANDARD RANGE OF UNCONFINED	COV. COV.	(MOD, SEV.) AND CAN BE EXCAVATED WITH A GEOLOGIST'S PICK. ROCK GIVES 'CLUNK' SOUND WHEN STRUCK.	THE FIELD.
PRIMARY SOIL TYPE COMPACTNESS OR CONSISTENCY PENETRATION RESISTENCE (N-VALUE) COMPRESSIVE STRENGTH	ROADWAY EMBANKMENT (RE) WITH SOIL DESCRIPTION ROADWAY EMBANKMENT (RE) WITH SOIL DESCRIPTION SAMPLE DESIGNATIONS	IF TESTED, WOULD YIELD SPT REFUSAL SEVERE ALL ROCK EXCEPT QUARTZ DISCOLORED OR STAINED, ROCK FABRIC CLEAR AND EVIDENT BUT REDUCED.	JOINT - FRACTURE IN ROCK ALONG WHICH NO APPRECIABLE MOVEMENT HAS OCCURRED.
VERY LODGE	S - BULK SAMPLE AUGER BORING S - BULK SAMPLE	(SEV.) IN STRENGTH TO STRONG SOIL. IN GRANITOID ROCKS ALL FELDSPARS ARE KAOLINIZED TO SOME	LEDGE - A SHELF-LIKE RIDGE OR PROJECTION OF ROCK WHOSE THICKNESS IS SMALL COMPARED TO ITS LATERAL EXTENT.
GENERALLY	SS - SPLIT SPOON	EXTENT. SOME FRAGMENTS OF STRONG ROCK USUALLY REMAIN. IF TESTED, YIELDS SPT N VALUES > 100 BPF	LENS - A BODY OF SDIL OR ROCK THAT THINS OUT IN ONE OR MORE DIRECTIONS.
MATERIAL DENSE 30 TO 50	ARTIFICIAL FILL (AF) OTHER THAN ROADWAY EMBANKMENT CORE BORING ST - 9HELBY TUBE	VERY SEVERE ALL ROCK EXCEPT QUARTZ DISCOLORED OR STAINED. ROCK FABRIC ELEMENTS ARE DISCERNIBLE BUT	MOTTLED (MOT.) - IRREGULARLY MARKED WITH SPOTS OF DIFFERENT COLORS, MOTTLING IN
AERI DENZE >200	INFERRED SOIL BOHNDARY SAMPLE	(V SEV.) THE MASS IS EFFECTIVELY REDUCED TO SOIL STATUS, WITH DNLY FRAGMENTS OF STRONG ROCK	SDILS USUALLY INDICATES POOR AERATION AND LACK OF GOOD DRAINAGE. PERCHED WATER - WATER MAINTAINED ABOVE THE NORMAL GROUND WATER LEVEL BY THE PRESENCE OF AN
VERY SOFT <2 <0.25 GENERALLY SDFT 2 TO 4 0.25 TD 0.50	MONITORING WELL RS - ROCK SAMPLE	REMAINING. SAPROLITE IS AN EXAMPLE OF ROCK WEATHERED TO A DEGREE SUCH THAT ONLY MINOR VESTIGES OF THE ORIGINAL ROCK FABRIC REMAIN. IF TESTED YIELDS SPT N VALUES < 100 BPF	INTERVENING IMPERVIOUS STRATUM.
SILT-CLAY MEDIUM STIFF 4 TO 8 0.5 TO 1.0	A PIEZUMETER BT - RECOMPACTED TRIAXIA	COMPLETE ROCK REDUCED TO SOIL. ROCK FABRIC NOT DISCERNIBLE, OR DISCERNIBLE ONLY IN SMALL AND	RESIDUAL (RES.) SOIL - SOIL FORMED IN PLACE BY THE WEATHERING OF ROCK.
MATERIAL STIFF 8 TO 15 1 TO 2 (COHESIVE) VERY STIFF 15 TO 30 2 TO 4	ALLUVIAL SOIL BOUNDARY INSTALLATION SAMPLE SLOPE INDICATOR	SCATTERED CONCENTRATIONS. QUARTZ MAY BE PRESENT AS DIKES OR STRINGERS, SAPROLITE IS ALSO AN EXAMPLE.	ROCK QUALITY DESIGNATION (RQD) - A MEASURE OF ROCK QUALITY DESCRIBED BY TOTAL LENGTH OF
HARD >30 >4	25/825 DIP & DIP DIRECTION OF INSTALLATION CBR - CALIFORNIA BEARING ROCK STRUCTURES RATIO SAMPLE	ROCK HARDNESS	ROCK SEGMENTS EQUAL TO DR GREATER THAN 4 INCHES DIVIDED BY THE TOTAL LENGTH OF CORE RUN AN EXPRESSED AS A PERCENTAGE.
TEXTURE OR GRAIN SIZE	SPT N-VALUE	VERY HARD CANNOT BE SCRATCHED BY KNIFE OR SHARP PICK, BREAKING OF HAND SPECIMENS REQUIRES	SAPROLITE (SAP.) - RESIDUAL SOIL THAT RETAINS THE RELIC STRUCTURE OR FABRIC OF THE
u.s. STD. SIEVE SIZE 4 10 40 60 200 270	SOUNDING ROD REF SPT REFUSAL	SEVERAL HARD BLOWS OF THE GEOLOGIST'S PICK.	PARENT ROCK.
OPENING (MM) 4.76 2.00 0.42 0.25 0.075 0.053	ABBREVIATIONS	HARD CAN BE SCRATCHED BY KNIFE OR PICK ONLY WITH DIFFICULTY. HARD HAMMER BLOWS REQUIRED	SILL - AN INTRUSIVE BODY OF IGNEOUS ROCK OF APPROXIMATELY UNIFORM THICKNESS AND RELATIVELY THIN COMPARED WITH ITS LATERAL EXTENT, THAT HAS BEEN EMPLACED PARALLEL
BOULDER COBBLE GRAVEL COARSE FINE SILT CLAY	AR - AUGER REFUSAL HI HIGHLY W - MOISTURE CONTENT	TO DETACH HAND SPECIMEN. MODERATELY CAN BE SCRATCHED BY KNIFE OR PICK, GOUGES OR GROOVES TO 0.25 INCHES DEEP CAN BE	TO THE BEDDING OR SCHISTOSITY OF THE INTRUDED ROCKS.
(BLDR.) (COB.) (GR.) (CSE. SD.) (F SD.) (SL.) (CL.)	BT - BORING TERMINATED MED MEDIUM V - VERY - CL CLAY MICA MICACEDUS VST - VANE SHEAR TEST	HARD EXCAVATED BY HARD BLOW OF A GEOLOGIST'S PICK, HAND SPECIMENS CAN BE DETACHED	SLICKENSIDE - POLISHED AND STRIATED SURFACE THAT RESULTS FROM FRICTION ALONG A FAULT OR
GRAIN MM 305 75 2.0 0.25 0.05 0.005	CPT - CONE PENETRATION TEST MOD MODERATELY WEA WEATHERED	BY MODERATE BLOWS. MEDIUM CAN BE GROOVED OR GOUGED 0.05 INCHES DEEP BY FIRM PRESSURE OF KNIFE OR PICK POINT.	STANDARD PENETRATION TEST (PENETRATION RESISTANCE) (SPT) - NUMBER OF BLOWS (N OR BPF) OF
SIZE IN. 12 3	CSE COARSE NP - NON PLASTIC 7 - UNIT WEIGHT DMT - DILATOMETER TEST ORG ORGANIC 7 - DRY UNIT WEIGHT	MEDIUM CAN BE GROOVED OR GOUGED 0.05 INCHES DEEP BY FIRM PRESSURE OF KNIFE OR PICK POINT. HARD CAN BE EXCAVATED IN SMALL CHIPS TO PEICES 1 INCH MAXIMUM SIZE BY HARD BLOWS OF THE	A 140 LB. HAMMER FALLING 30 INCHES REQUIRED TO PRODUCE A PENETRATION OF 1 FOOT INTO SOIL WITH A 2 INCH OUTSIDE DIAMETER SPLIT SPOON SAMPLER, SPT REFUSAL IS PENETRATION EQUAL TO OR LESS
SOIL MOISTURE - CORRELATION OF TERMS SOIL MOISTURE SCALE FIELD MOISTURE CHIDE FOR EIGHD MOISTURE DESCRIPTION	DPT - DYNAMIC PENETRATION TEST PMT - PRESSUREMETER TEST	POINT OF A GEOLOGIST'S PICK.	THAN 0.1 FOOT PER 60 BLOWS.
(ATTERBERG LIMITS) OBSCRIPTION GUIDE FOR FIELD MOISTURE DESCRIPTION	e - VOID RATID SAP SAPROLITIC F - FINE SD SAND, SANDY	SOFT CAN BE GROVED OR GOUGED READILY BY KNIFE OR PICK. CAN BE EXCAVATED IN FRAGMENTS FROM CHIPS TO SEVERAL INCHES IN SIZE BY MODERATE BLOWS OF A PICK POINT. SMALL, THIN	STRATA CORE RECOVERY (SREC.) - TOTAL LENGTH OF STRATA MATERIAL RECOVERED DIVIDED BY TOTAL LENGTH OF STRATUM AND EXPRESSED AS A PERCENTAGE.
- SATURATED - USUALLY LIQUID; VERY WET, USUALLY	FOSS FOSSILIFEROUS SL SILT, SILTY	PIECES CAN BE BROKEN BY FINGER PRESSURE.	STRATA ROCK QUALITY DESIGNATION (SROD) - A MEASURE OF ROCK QUALITY DESCRIBED BY
(SAT.) FROM BELOW THE GROUND WATER TABLE	FRAC FRACTURED, FRACTURES SLI SLIGHTLY FRAGS FRAGMENTS TCR - TRICONE REFUSAL	VERY CAN BE CARVED WITH KNIFE, CAN BE EXCAVATED READILY WITH POINT OF PICK. PIECES 1 INCH SOFT OR MORE IN THICKNESS CAN BE BROKEN BY FINGER PRESSURE. CAN BE SCRATCHED READILY BY	TOTAL LENGTH OF ROCK SEGMENTS WITHIN A STRATUM EDUAL TO OR GREATER THAN 4 INCHES DIVIDED BY THE TOTAL LENGTH OF STRATA AND EXPRESSED AS A PERCENTAGE.
PLASTIC SEMISOLIDA REQUIRES DRYING TO		FINGERNAIL.	TOPSOIL (TS.) - SURFACE SOILS USUALLY CONTAINING ORGANIC MATTER.
RANGE - WET - (W) ATTAIN OPTIMUM MOISTURE	EQUIPMENT USED ON SUBJECT PROJECT	FRACTURE SPACING BEDDING TERM SPACING TERM THICKNESS	
PLL + PLASTIC LIMIT -	DRILL UNITS: ADVANCING TOOLS: HAMMER TYPE:	VERY THICKLY BEDDED > 4 FEET	BENCH MARK: BM #2 -L- STA. 18+54, 47.66' LT
OM OPTIMUM MOISTURE - MOIST - (M) SOLID; AT OR NEAR OPTIMUM MOISTURE	CLAY BITS AUTOMATIC MANUAL	WIDE 3 TO 10 FEET THINK Y DEDDED 0.15 - 4 FEET THINK Y DEDDED 0.15 - 15 FFFT	ELEVATION: 139.86 FT.
SL SHRINKAGE LIMIT	MOBILE 8- CLAY BITS CLAY BITS 6 CONTINUOUS FLIGHT AUGER CORE SIZE:	MUDERATELY CLUSE 1 TO 3 FEET VERY THINLY BEDDED 0.03 - 0.16 FEET	NOTEC
- DRY - (D) REQUIRES ADDITIONAL WATER TO ATTAIN OPTIMUM MOISTURE		VERY CLOSE LESS THAN 0.16 FEET THICKLY LAMINATED 0.008 - 0.03 FEET THINLY LAMINATED 4.008 FEET	NOTES:
PLASTICITY		INDURATION]
PLHSTICITY INDEX (PI) DRY STRENGTH	- CINE-190	FOR SEDIMENTARY ROCKS, INDURATION IS THE HARDENING OF THE MATERIAL BY CEMENTING, HEAT, PRESSURE, ETC.	
NONPLASTIC 0-5 VERY LOW	CME-550 TUNG,-CARBIDE INSERTS	FRIABLE RUBBING WITH FINGER FREES NUMEROUS GRAINS:	
LOW PLASTICITY 6-15 SLIGHT MED. PLASTICITY 16-25 MEDIUM	CASING W/ ADVANCER HAND TOOLS:	GENTLE BLUW BY HAMMER DISINIEGRATES SAMPLE.	
HIGH PLASTICITY 26 OR MORE HIGH	PORTABLE HOIST TRICONE STEEL TEETH POST HOLE DIGGER	MODERATELY INDURATED GRAINS CAN BE SEPARATED FROM SAMPLE WITH STEEL PROBE; BREAKS EASILY WHEN HIT WITH HAMMER.	
COLOR	TRICONE TUNG,-CARB, HAND AUGER	INDURATED GRAINS ARE DIFFICULT TO SEPARATE WITH STEEL PROBE;	
DESCRIPTIONS MAY INCLUDE COLOR OR COLOR COMBINATIONS (TAN, RED, YELLOW-BROWN, BLUE-GRAY).	CORE BIT SOUNDING ROD VANE SHEAR TEST	DIFFICULT TO BREAK WITH HAMMER.	
MODIFIERS SUCH AS LIGHT, DARK, STREAKED, ETC. ARE USED TO DESCRIBE APPEARANCE.	6' HOLLOW AUGERS VANE SHEAR TEST	EXTREMELY INDURATED SHARP HAMMER BLOWS REQUIRED TO BREAK SAMPLE; SAMPLE BREAKS ACROSS GRAINS.	
		SHUIFLE DIEHAS HUIUSS UNHINS.	







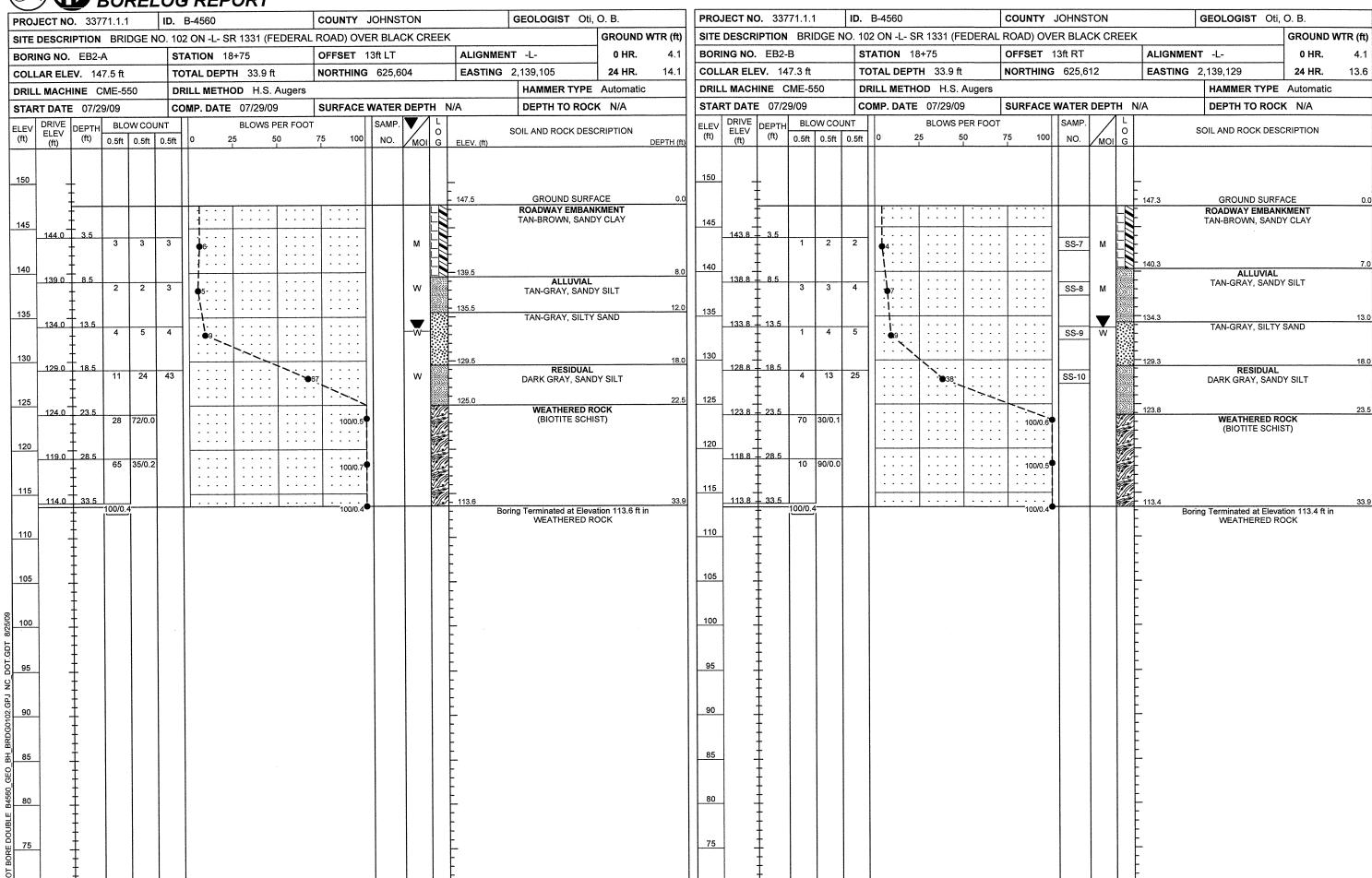




NCDOT GEOTECHNICAL ENGINEERING UNIT

PROJECT NO. 33771.1.1	ID. B-4560	COUNTY JOHNSTON	GEOLOGIST Oti, O. B.		ID. B-4560	COUNTY JOHNSTON	GEOLOGIST Oti, O. B.
SITE DESCRIPTION BRIDGE N		AL ROAD) OVER BLACK CREEK	GROUND WTR (ft)	SITE DESCRIPTION BRIDGE N		· · · · · · · · · · · · · · · · · · ·	GROUND WTR (ft)
BORING NO. B1-A	STATION 17+55	OFFSET 6ft LT ALIGNM		BORING NO. B1-B	STATION 17+55		GNMENT -L- 0 HR. N/A
COLLAR ELEV. 129.7 ft	TOTAL DEPTH 23.2 ft		2,139,148 24 HR. N/A	COLLAR ELEV. 130.7 ft	TOTAL DEPTH 33.0 ft		STING 2,139,159 24 HR. N/A
DRILL MACHINE CME-550	DRILL METHOD H.S. Augers		HAMMER TYPE Automatic	DRILL MACHINE CME-550	DRILL METHOD H.S. Augers		HAMMER TYPE Automatic
START DATE 07/30/09	COMP. DATE 07/30/09 INT BLOWS PER FO	SURFACE WATER DEPTH 3.3ft	DEPTH TO ROCK N/A	START DATE 07/31/09 ELEV DRIVE DEPTH BLOW COU	INT BLOWS PER FOO	SURFACE WATER DEPTH 4.5ft DT SAMP. L	DEPTH TO ROCK N/A
ELEV Cft) DRIVE ELEV (ft) DEPTH BLOW COU		75 100 NO. MOI G ELEV. (ft)	SOIL AND ROCK DESCRIPTION DEPTH (ft)			75 100 NO. MOI G	SOIL AND ROCK DESCRIPTION
(iy			WATER SURFACE (07/30/09)				
130		129.7	GROUND SURFACE 0.0	135		 	WATER SURFACE (07/31/09)
129.7 0.0 4 2	3 •5	W	ALLUVIAL TAN-GRAY, FINE SAND 2.5				
127.2 7 2.5 23 26		♣66 SS-13 W	RESIDUAL LIGHT GRAY, CLAYEY SILT	1 130 +		130.7	
125		123.7	6.0	1 - 1 1 1		128.2	
122.2 7.5 50 50/0.3]	WEATHERED ROCK (BIOTITE SCHIST)		4	w [N]	RESIDUAL LIGHT GRAY, CLAYEY SILT
120							
117.2 12.5				123.2 7.5 23 27	40	W N N N N N N N N N N N N N N N N N N	
115 54 46/0.2		100/0.7		120 +	/ .		
112.2 + 17.5				118.2	23		
110 100/0.5		100/0.5 •		115		1 1 1 1 1 1 1 1 1 1 1 1 1 1 1 1 1 1 1 1	WEATHERED ROCK
T I I				113.2 17.5			(BIOTITE SCHIST)
107.2 22.5 70 30/0.2		100/0.7	23.2 Boring Terminated at Elevation 106.5 ft in	28 72/0.4		. 100/0.9	
105			WEATHERED ROCK	108.2 7 22.5			
				57 43/0.3		100/0.8	
100				105 +			
‡				103.2 7 27.5 1 100/0.5		100/0.5	
95				<u> </u>			
				98.2 + 32.5		100/0.5	33.0
				95		100/0.5	Boring Terminated at Elevation 97.7 ft in WEATHERED ROCK
90							
85 +				90			
		E				[
80 80				85 +			
78							
5 10 75							
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ORE DOUBILE B4550 GEO BH BRDG0102 GPJ NC DOT.GDT GPJ 62 9 9 9 9 9 9 9 9 9 9 9 9 9 9 9 9 9 9							
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				55		1 1 1 +	

	ID. B-4560	COUNTY JOHNSTON	GEOLOGIST Oti	<u> </u>		ECT NO.			ID. B-4560	COUNTY				GEOLOGIST Oti,	
ITE DESCRIPTION BRIDGE NO	D. 102 ON -L- SR 1331 (FEDERA	AL ROAD) OVER BLACK CREEK		GROUND WTR (ft)				DGE NO). 102 ON -L- SR 1331 (FEDER			K CREE	:K		GROUND WT
ORING NO. B2-A	STATION 18+20		GNMENT -L-	0 HR. N/A		IG NO. B2	····		STATION 18+20	OFFSET			ALIGNMEN		0 HR.
COLLAR ELEV. 133.7 ft	TOTAL DEPTH 29.6 ft		STING 2,139,128	24 HR. N/A		AR ELEV.			TOTAL DEPTH 33.9 ft	NORTHIN	G 625,55	57	EASTING	- 	24 HR.
PRILL MACHINE CME-550	DRILL METHOD H.S. Augers		HAMMER TYPE			MACHINE			DRILL METHOD H.S. Augers					HAMMER TYPE	
TART DATE 07/30/09	COMP. DATE 07/30/09	SURFACE WATER DEPTH 1.5ft	DEPTH TO ROO	CK N/A		DBIVE I			COMP. DATE 07/31/09	SURFACE		DEPTH	4.0ft	DEPTH TO ROC	K N/A
DRIVE DEPTH BLOW COUNT		75 100 NO. MOI G ELEV	SOIL AND ROCK DES	SCRIPTION DEPTH (ft)	ELEV (ft)	DRIVE ELEV (ft) (ft		0.5ft 0.		OT 75 100	SAMP. NO.	MOI G		SOIL AND ROCK DES	CRIPTION
35 133.7 + 0.0				FACE 0.0	135	+						▼ .	- - -	WATER SURFACE (C	07/31/09)
+ WOH 1	5	1 1 1 1 1 1 1 1 1 1 1 1 1 1 1 1 1 1 1 1	ALLUVIAL TAN-GRAY, SILTY		1 1	131.7 + 0	0 2	2	4			w	131.7	GROUND SURF	ACE
30 129.7 4.0 5 12		129.7	RESIDUAL	4.0	130	\pm			4			٧٧	130.2	TAN-GRAY, SILTY	SAND
	17	-+ 127.2	LIGHT GRAY, SILT	TY CLAY 6.5	-	128.2 <u> </u>	5 8	11 1	14			W N	<u> </u>	RESIDUAL LIGHT GRAY, CLAY	EY SILT
25 124.7 9.0		1 1 1 1 1 1 1 1 1 1 1 1 1 1 1 1 1 1 1 1	WEATHERED R (BIOTITE SCH		125	<u> </u>						<i>x</i> .			
32 68/0.4		100/0.9				123.2 8						N.			
						Ŧ	13	22 3	32 54			[x:	1		
0 119.7 14.0 75 25/0.1					120	Ŧ					+	, i	118.7		
75 25/0.1		100/0.6			-	118.2 T 13		13 1	13		SS-14	w	÷ 110.7	LIGHT GRAY, SILT	Y CLAY
5 114 7 10 0					115	Ŧ							115.2		
5 114.7 + 19.0 + 34 66/0.1		100/0.6				113.2 18	.5						-	WEATHERED RO (BIOTITE SCHI	
						‡	21	60 40			,		.	,	,
109.7 24.0					110	‡					41 1		<u></u>		
90 10/0.1		100/0.6			-	108.2 23	35	42 3	30		11	w	108.7	RESIDUAL	
.					105	‡					11		106.7	LIGHT GRAY, SILT	
05 104.7 29.0 70 30/0.1		100/0.6	Boring Terminated at Elev	29.6		103.2 + 28					11 1		97.8	(BIOTITE SCHI	ST)
			WEATHERED F			103.2 + 28	100/0.4			100/0.4	!				
00 +	·				100	‡					41 1				
						98.2 33	100/0.4			100/0.4		97	97.8		
_					0.5	‡	100/0.5			100/0.4	•		Boi	ing Terminated at Elevi WEATHERED R	
5 1					95	‡							 		
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	SHEET 11
771.1.1	(B-4560)

EB1-A

	SOIL TEST RESULTS														
SAMPLE			DEPTH	AASHTO				% BY W	VEIGHT		% PAS	SING (S	IEVES)	%	%
NO.	OFFSET	STATION	INTERVAL	CLASS.	L.L.	P.I.	C.SAND	F.SAND	SILT	CLAY	10	40	200	MOISTURE	ORGANIC
SS-1	14' LT	17+00	4.0-5.5	A-6(1)	29	15	32.2	34.3	8.3	25.2	97	77	37	-	-
SS-2	14' LT	17+00	9.0-10.5	A-6(7)	28	11	1.6	27.6	34.5	36.3	99	98	80	-	-
SS-3	14' LT	17+00	19.0-20.5	A-5(10)	46	9	9.5	4.6	65.8	20.1	93	85	81	-	-
SS-4	14' LT	17+00	29.0-30.5	A-4(4)	35	6	21.8	10.1	56.1	12.1	100	86	70		-

EB1-B

	SOIL TEST RESULTS														
SAMPLE			DEPTH	AASHTO				% BY W	/EIGHT		% PAS	SING (S	IEVES)	%	%
NO.	OFFSET	STATION	INTERVAL	CLASS.	L.L.	P.I.	C.SAND	F.SAND	SILT	CLAY	10	40	200	MOISTURE	ORGANIC
SS-5	14' RT	17+00	14.0-15.5	A-2-4(0)	20	NP	7.7	72.7	11.6	8.1	100	.99	28	-	•
SS-6	14' RT	17+00	0.0-0.0	A-4(8)	38	7	5.2	7.3	71.4	16.1	100	96	91	-	

B1-A

DI-A															
	SOIL TEST RESULTS														
SAMPLE			DEPTH	AASHTO				% BY V	VEIGHT		% PAS	SING (S	SIEVES)	%	%
NO.	OFFSET	STATION	INTERVAL	CLASS.	L.L.	P.I.	C.SAND	F.SAND	SILT	CLAY	10	40	200	MOISTURE	ORGANIC
SS-13	6' LT	17+55	2.5-4.0	A-5(13)	47	8	2.4	3.4	74.0	20.1	100	99	95	-	-

B2-A

B 2-A	SOIL TEST RESULTS														
SAMPLE			DEPTH	AASHTO				% BY V	VEIGHT		% PAS	SING (S	IEVES)	%	%
NO.	OFFSET	STATION	INTERVAL	CLASS.	L.L.	P.I.	C.SAND	F.SAND	SILT	CLAY	10	40	200	MOISTURE	ORGANIC
SS-11	6' LT	18+20	0.0-1.5	A-2-4(0)	20	NP	25.2	59.3	9.5	6.0	96	88	21	-	-

B2-B

SOIL TEST RESULTS															
SAMPLE			DEPTH	AASHTO				% BY W	/EIGHT		% PAS	SING (S	IEVES)	%	%
NO.	OFFSET	STATION	INTERVAL	CLASS.	L.L.	P.I.	C.SAND	F.SAND	SILT	CLAY	10	40	200	MOISTURE	ORGANIC
SS-12	6' RT	18+20	4.0-5.5	A-5(12)	45	9	2.2	5.4	74.2	18.1	100	99	94		-
SS-14	6' RT	18+20	13.5-15.0	A-7-5(14)	44	11	3.4	5.2	73.2	18.1	100	98	93	-	-

EB2-B

	SOIL TEST RESULTS														
SAMPLE			DEPTH	AASHTO				% BY W	/EIGHT		% PAS	SING (S	IEVES)	%	%
NO.	OFFSET	STATION	INTERVAL	CLASS.	L.L.	P.I.	C.SAND	F.SAND	SILT	CLAY	10	40	200	MOISTURE	ORGANIC
SS-7	13' RT	18+75	3.5-5.0	A-6(3)	36	18	31.0	32.4	8.4	28.2	95	78	39	-	
SS-8	13' RT	18+75	8.5-10.0	A-4(0)	18	2	1.4	53.9	28.6	16.1	100	100	61	-	-
SS-9	13' RT	18+75	0.0-0.0	A-2-4(0)	20	NP	36.2	53.2	5.6	5.0	100	93	16	-	-
SS-10	13' RT	18+75	18.5-20.0	A-4(8)	37	8	10.7	10.1	71.2	8.1	100	92	83	-	-



FIELD SCOUR REPORT

WBS:	33771.1.1	_ TIP:	B-4560	COUNTY: Johnston				
DESCRIPTION(1):	Bridge No. 102	on -L- (S	R 1331) over the	Black Creek				
EXISTING BRIDGE								
Information from:	Field In Other	nspection (explain)	x Mic	rofilm (reel pos:)			
Bridge No.: <u>1</u> Foundation Type: _	02 Length Timber piles wi	: 151 th addition	Total Bents: nal crutch H-piles	7 Bents in Channel: 2 Bents	s in Floodplain: 5			
EVIDENCE OF S Abutments or E		: None						
Interior Bents:	Local scour at a	all piers in	channel					
Channel Bed:	Minor Local and	d contract	ion scour					
Channel Bank: _	Local scour arc	und Bent	5					
EXISTING SCOL Type(3):	JR PROTECTION Concrete cover		ent slopes.					
Extent(4):	Concrete = 75'	L x 25' W	1					
Effectiveness(5):	Both are effecti	ve						
Obstructions(6):	None							

INSTRUCTIONS

- 1 Describe the specific site's location, including route number and body of water crossed.
- 2 Note scour evidence at existing end bents or abutments (e.g. undermining, sloughing, degradations).
- 3 Note existing scour protection (e.g. rip rap).
- 4 Describe extent of existing scour protection.
- 5 Describe whether or not the scour protection appears to be working.
- 6 Note obstructions such as dams, fallen trees, debris at bents, etc.
- 7 Describe the channel bed material based on observation and/or samples. Include any lab results with report.
- 8 Describe the channel bank material based on observation and/or samples. Include any lab results with report.
- **9** Describe the material covering the banks (e.g. grass, trees, rip rap, none).
- 10 Determine the approximate floodplain width from field observation or a topographic map.
- 11 Describe the material covering the floodplain (e.g. grass, trees, crops).
- 12 Use professional judgement to specify if the stream is degrading, aggrading, or static.
- 13 Describe potential and direction of the stream to migrate laterally during the bridge's life (approx. 100 years).
- 14 Give the design scour elevation (DSE) expected over the life of the bridge (approx. 100 years). This elevation can be given as a range across the site, or for each bent. Discuss the relationship between the Hydraulics Unit theoritical scour and the DSE. If the DSE is dependent on scour counter measures, explain (e.g. rip rap armoring on slopes). The DSE is based on the erodability of materials, giving consideration to the influence of joints, foliation, bedding characteristics, % core recovery, % RQD, differential weathering, shear strength, observations at existing structures, other tests deemed appropriate, and overall geologic conditions at the site.

<u>DESIGN I</u>	NFORMATION									
Channel Bed Material(7): Alluvial, tan-brown, silty	sand (SS-11)									
	/laterial(8): Alluvial, tan - gray, sandy silt (SS-8)									
Channel Bank Cover(9): Grass vines, shrubs small and large trees										
Floodplain Width(10): +/- 600 feet										
Floodplain Cover(11): Grass, trees, shrubs and woods										
Stream is(12): Aggrading	Degrading X Static									
Channel Migration Tend.(13): South East towards End	Bent 1									
Observations and Other Comments:										
Observations and other dominients.										
DESIGN SCOUR ELEVATIONS(14)	Feet X Meters									
B1-A B1-B B2-A B2-B 124 122 127 122 Comparison of DSE to Hydraulics Unit theoretical scour Design Scour Elevations are 3.0 to 5.0 feet higher at be Hydraulics Unit's estimates for the 100year storm event	ent 1 and 3.0 to 8.0 feet higher at bent 2 compared to the									
Trydraulics Offics estimates for the Tooyear storm event										
SOIL ANALYSIS RESULTS FROM CHANNEL BED A	ND BANK MATERIAL									
Bed or Bank Sample No.										
Retained #4										
Passed #10										
Passed #40 See Sheet 11,										
Passed #200 See Sheet 11, Coarse Sand "Soil Test Results",										
Fine Sand for samples:										
Silt SS-11										
Clay SS-8										
LĹ										
PI										
AASHTO L										
Station Offset										
Depth										

Template Revised 02/07/

Reported by:

SITE PHOTO

BRIDGE NO. 102 ON SR 1331 (FEDERAL ROAD) OVER BLACK CREEK



LOOKING NORTH TOWARD END BENT 1