

STATE OF NORTH CAROLINA
DEPARTMENT OF TRANSPORTATION
DIVISION OF HIGHWAYS
GEOTECHNICAL ENGINEERING UNIT

STRUCTURE
SUBSURFACE INVESTIGATION

PROJ. REFERENCE NO. 33665.1.1 (B-4328) F.A. PROJ. BRZ-1634(4)
COUNTY WILSON
PROJECT DESCRIPTION BRIDGE NO. 3 ON SR 1634 OVER GREAT SWAMP CREEK AT -L- STA. 16+67

CONTENTS

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CAUTION NOTICE

THE SUBSURFACE INFORMATION AND THE SUBSURFACE INVESTIGATION ON WHICH IT IS BASED WERE MADE FOR THE PURPOSE OF STUDY, PLANNING, AND DESIGN, AND NOT FOR CONSTRUCTION OR PAY PURPOSES. THE VARIOUS FIELD BORING LOGS, ROCK CORES, AND SOIL TEST DATA AVAILABLE MAY BE REVIEWED OR INSPECTED IN RALEIGH BY CONTACTING THE N.C. DEPARTMENT OF TRANSPORTATION, GEOTECHNICAL ENGINEERING UNIT AT 1919 250-4088. NEITHER THE SUBSURFACE PLANS AND REPORTS, NOR THE FIELD BORING LOGS, ROCK CORES, OR SOIL TEST DATA ARE PART OF THE CONTRACT.

GENERAL SOIL AND ROCK STRATA DESCRIPTIONS AND INDICATED BOUNDARIES ARE BASED ON A GEOTECHNICAL INTERPRETATION OF ALL AVAILABLE SUBSURFACE DATA AND MAY NOT NECESSARILY REFLECT THE ACTUAL SUBSURFACE CONDITIONS BETWEEN BORINGS OR BETWEEN SAMPLED STRATA WITHIN THE BOREHOLE. THE LABORATORY SAMPLE DATA AND THE IN SITU (IN-PLACE) TEST DATA CAN BE RELIED ON ONLY TO THE DEGREE OF RELIABILITY INHERENT IN THE STANDARD TEST METHOD. THE OBSERVED WATER LEVELS OR SOIL MOISTURE CONDITIONS INDICATED IN THE SUBSURFACE INVESTIGATIONS ARE AS RECORDED AT THE TIME OF THE INVESTIGATION. THESE WATER LEVELS OR SOIL MOISTURE CONDITIONS MAY VARY CONSIDERABLY WITH TIME ACCORDING TO CLIMATIC CONDITIONS INCLUDING TEMPERATURES, PRECIPITATION, AND WIND, AS WELL AS OTHER NON-CLIMATIC FACTORS.

THE BIDDER OR CONTRACTOR IS CAUTIONED THAT DETAILS SHOWN ON THE SUBSURFACE PLANS ARE PRELIMINARY ONLY AND IN MANY CASES THE FINAL DESIGN DETAILS ARE DIFFERENT. FOR BIDDING AND CONSTRUCTION PURPOSES, REFER TO THE CONSTRUCTION PLANS AND DOCUMENTS FOR FINAL DESIGN INFORMATION ON THIS PROJECT. THE DEPARTMENT DOES NOT WARRANT OR GUARANTEE THE SUFFICIENCY OR ACCURACY OF THE INVESTIGATION MADE, OR THE INTERPRETATIONS MADE, OR OPINION OF THE DEPARTMENT AS TO THE TYPE OF MATERIALS AND CONDITIONS TO BE ENCOUNTERED. THE BIDDER OR CONTRACTOR IS CAUTIONED TO MAKE SUCH INDEPENDENT SUBSURFACE INVESTIGATIONS AS HE DEEMS NECESSARY TO SATISFY HIMSELF AS TO CONDITIONS TO BE ENCOUNTERED ON THIS PROJECT. THE CONTRACTOR SHALL HAVE NO CLAIM FOR ADDITIONAL COMPENSATION OR FOR AN EXTENSION OF TIME FOR ANY REASON RESULTING FROM THE ACTUAL CONDITIONS ENCOUNTERED AT THE SITE DIFFERING FROM THOSE INDICATED IN THE SUBSURFACE INFORMATION.

PROJECT: 33665.1.1 ID: B-4328

PERSONNEL

CMW

MMH

RES

LWD

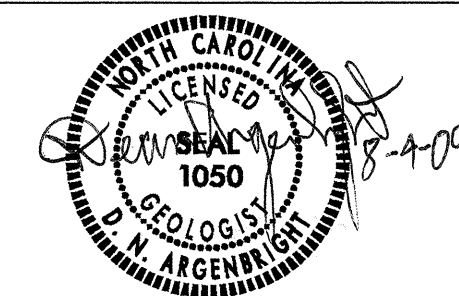
KLEINFELDER

INVESTIGATED BY C.M. WRIKE

CHECKED BY D.N. ARGENBRIGHT

SUBMITTED BY D.N. ARGENBRIGHT

DATE AUGUST 2009



DRAWN BY: C.P. TURNER

NOTE - THE INFORMATION CONTAINED HEREIN IS NOT IMPLIED OR GUARANTEED BY THE N. C. DEPARTMENT OF TRANSPORTATION AS BEING ACCURATE NOR IS IT CONSIDERED TO BE PART OF THE PLANS, SPECIFICATIONS, OR CONTRACT FOR THE PROJECT.

NOTE - BY HAVING REQUESTED THIS INFORMATION THE CONTRACTOR SPECIFICALLY WAIVES ANY CLAIMS FOR INCREASED COMPENSATION OR EXTENSION OF TIME BASED ON DIFFERENCES BETWEEN THE CONDITIONS INDICATED HEREIN AND THE ACTUAL CONDITIONS AT THE PROJECT SITE.

NORTH CAROLINA DEPARTMENT OF TRANSPORTATION
DIVISION OF HIGHWAYS
GEOTECHNICAL ENGINEERING UNIT

PROJECT REFERENCE NO. B-4328	SHEET NO. 2 OF 9
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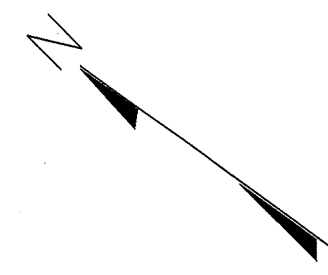
SUBSURFACE INVESTIGATION

SOIL AND ROCK LEGEND, TERMS, SYMBOLS, AND ABBREVIATIONS

SOIL DESCRIPTION						GRADATION						ROCK DESCRIPTION						TERMS AND DEFINITIONS					
SOIL IS CONSIDERED TO BE THE UNCONSOLIDATED, SEMI-CONSOLIDATED, OR WEATHERED EARTH MATERIALS THAT CAN BE PENETRATED WITH A CONTINUOUS FLIGHT POWER AUGER, AND YIELD LESS THAN 100 BLOWS PER FOOT ACCORDING TO STANDARD PENETRATION TEST (ASTM D-1586). SOIL CLASSIFICATION IS BASED ON THE AASHTO SYSTEM. BASIC DESCRIPTIONS GENERALLY SHALL INCLUDE: CONSISTENCY, COLOR, TEXTURE, MOISTURE, AASHTO CLASSIFICATION, AND OTHER PERTINENT FACTORS SUCH AS MINERALOGICAL COMPOSITION, ANGULARITY, STRUCTURE, PLASTICITY, ETC. EXAMPLE: <i>VERY STIFF, GRAY, SILTY CLAY, MOST WITH INTERBEDDED FINE SAND LAYERS, HIGHLY PLASTIC, A-7-6</i>						WELL GRADED - INDICATES A GOOD REPRESENTATION OF PARTICLE SIZES FROM FINE TO COARSE. UNIFORM - INDICATES THAT SOIL PARTICLES ARE ALL APPROXIMATELY THE SAME SIZE. (ALSO POORLY GRADED) GAP-GRADED - INDICATES A MIXTURE OF UNIFORM PARTICLES OF TWO OR MORE SIZES.						HARD ROCK IS NON-COASTAL PLAIN MATERIAL THAT IF TESTED, WOULD YIELD SPT REFUSAL, AN INFERRED ROCK LINE INDICATES THE LEVEL AT WHICH NON-COASTAL PLAIN MATERIAL WOULD YIELD SPT REFUSAL. SPT REFUSAL IS PENETRATION BY A SPLIT SPOON SAMPLER EQUAL TO OR LESS THAN 0.1 FOOT PER 60 BLOWS, IN NON-COASTAL PLAIN MATERIAL. THE TRANSITION BETWEEN SOIL AND ROCK IS OFTEN REPRESENTED BY A ZONE OF WEATHERED ROCK. ROCK MATERIALS ARE TYPICALLY DIVIDED AS FOLLOWS: WEATHERED ROCK (WR) - NON-COASTAL PLAIN MATERIAL THAT WOULD YIELD SPT N VALUES > 100 BLOWS PER FOOT IF TESTED. CRYSTALLINE ROCK (CR) - FINE TO COARSE GRAIN IGNEOUS AND METAMORPHIC ROCK THAT WOULD YIELD SPT REFUSAL IF TESTED. ROCK TYPE INCLUDES GRANITE, ONEISS, GABBRO, SCHIST, ETC. NON-CRYSTALLINE ROCK (NCR) - FINE TO COARSE GRAIN METAMORPHIC AND NON-COASTAL PLAIN SEDIMENTARY ROCK THAT WOULD YIELD SPT REFUSAL IF TESTED. ROCK TYPE INCLUDES PHYLLITE, SLATE, SANDSTONE, ETC. COASTAL PLAIN SEDIMENTARY ROCK (CP) - COASTAL PLAIN SEDIMENTS CEMENTED INTO ROCK, BUT MAY NOT YIELD SPT REFUSAL. ROCK TYPE INCLUDES LIMESTONE, SANDSTONE, CEMENTED SHELL BEDS, ETC.						ALLUVIUM (ALLUV.) - SOILS THAT HAVE BEEN TRANSPORTED BY WATER. AQUIFER - A WATER BEARING FORMATION OR STRATA. ARENACEOUS - APPLIED TO ROCKS THAT HAVE BEEN DERIVED FROM SAND OR THAT CONTAIN SAND. ARGILLACEOUS - APPLIED TO ALL ROCKS OR SUBSTANCES COMPOSED OF CLAY MINERALS, OR HAVING A NOTABLE PROPORTION OF CLAY IN THEIR COMPOSITION, AS SHALE, SLATE, ETC. ARTESIAN - GROUND WATER THAT IS UNDER SUFFICIENT PRESSURE TO RISE ABOVE THE LEVEL AT WHICH IT IS ENCOUNTERED, BUT WHICH DOES NOT NECESSARILY RISE TO OR ABOVE THE GROUND SURFACE. CALCAREOUS (CALC.) - SOILS THAT CONTAIN APPRECIABLE AMOUNTS OF CALCIUM CARBONATE. COLLUVIUM - ROCK FRAGMENTS MIXED WITH SOIL DEPOSITED BY GRAVITY ON SLOPE OR AT BOTTOM OF SLOPE. CORE RECOVERY (REC.) - TOTAL LENGTH OF ALL MATERIAL RECOVERED IN THE CORE BARREL DIVIDED BY TOTAL LENGTH OF CORE RUN AND EXPRESSED AS A PERCENTAGE. DIKE - A TABULAR BODY OF IGNEOUS ROCK THAT CUTS ACROSS THE STRUCTURE OF ADJACENT ROCKS OR CUTS MASSIVE ROCK. DIP - THE ANGLE AT WHICH A STRATUM OR ANY PLANAR FEATURE IS INCLINED FROM THE HORIZONTAL. DIP DIRECTION (DIP AZIMUTH) - THE DIRECTION OR BEARING OF THE HORIZONTAL TRACE OF THE LINE OF DIP, MEASURED CLOCKWISE FROM NORTH. FALLT - A FRACTURE OR FRACTURE ZONE ALONG WHICH THERE HAS BEEN DISPLACEMENT OF THE SIDES RELATIVE TO ONE ANOTHER PARALLEL TO THE FRACTURE. FISSILE - A PROPERTY OF SPLITTING ALONG CLOSELY SPACED PARALLEL PLANES. FLOAT - ROCK FRAGMENTS ON SURFACE NEAR THEIR ORIGINAL POSITION AND DISLODGED FROM PARENT MATERIAL. FLOOD PLAIN (FP) - LAND BORDERING A STREAM, BUILT OF SEDIMENTS DEPOSITED BY THE STREAM. FORMATION (FM) - A MAPPABLE GEOLOGIC UNIT THAT CAN BE RECOGNIZED AND TRACED IN THE FIELD. JOINT - FRACTURE IN ROCK ALONG WHICH NO APPRECIABLE MOVEMENT HAS OCCURRED. LEDGE - A SHELF-LIKE RIDGE OR PROJECTION OF ROCK WHOSE THICKNESS IS SMALL COMPARED TO ITS LATERAL EXTENT. LENS - A BODY OF SOIL OR ROCK THAT THINS OUT IN ONE OR MORE DIRECTIONS. MOTTLED (MOTL.) - IRREGULARLY MARKED WITH SPOTS OF DIFFERENT COLORS, MOTTLING IN SOILS USUALLY INDICATES POOR AERATION AND LACK OF GOOD DRAINAGE. PERCHED WATER - WATER MAINTAINED ABOVE THE NORMAL GROUND WATER LEVEL BY THE PRESENCE OF AN INTERVENING IMPERVIOUS STRATUM. RESIDUAL (RES.) SOIL - SOIL FORMED IN PLACE BY THE WEATHERING OF ROCK. ROCK QUALITY DESIGNATION (RQD) - A MEASURE OF ROCK QUALITY DESCRIBED BY TOTAL LENGTH OF ROCK SEGMENTS EQUAL TO OR GREATER THAN 4 INCHES DIVIDED BY THE TOTAL LENGTH OF CORE RUN AND EXPRESSED AS A PERCENTAGE. SAPROLITE (SAP.) - RESIDUAL SOIL THAT RETAINS THE RELIC STRUCTURE OR FABRIC OF THE PARENT ROCK. SILL - AN INTRUSIVE BODY OF IGNEOUS ROCK OF APPROXIMATELY UNIFORM THICKNESS AND RELATIVELY THIN COMPARED WITH ITS LATERAL EXTENT, THAT HAS BEEN EMPLACED PARALLEL TO THE BEDDING OR SCHISTOSITY OF THE INTRUDED ROCKS. SLICKENSIDE - POLISHED AND STRIATED SURFACE THAT RESULTS FROM FRICTION ALONG A FAULT OR SLIP PLANE. STANDARD PENETRATION TEST (PENETRATION RESISTANCE) (SPT) - NUMBER OF BLOWS IN OR BPF OF A 140 LB. HAMMER FALLING 30 INCHES REQUIRED TO PRODUCE A PENETRATION OF 1 FOOT INTO SOIL WITH A 2 INCH OUTSIDE DIAMETER SPLIT SPOON SAMPLER. SPT REFUSAL IS PENETRATION EQUAL TO OR LESS THAN 0.1 FOOT PER 60 BLOWS. STRATA CORE RECOVERY (SREC) - TOTAL LENGTH OF STRATA MATERIAL RECOVERED DIVIDED BY TOTAL LENGTH OF STRATUM AND EXPRESSED AS A PERCENTAGE. STRATA ROCK QUALITY DESIGNATION (SROD) - A MEASURE OF ROCK QUALITY DESCRIBED BY TOTAL LENGTH OF ROCK SEGMENTS WITHIN A STRATUM EQUAL TO OR GREATER THAN 4 INCHES DIVIDED BY THE TOTAL LENGTH OF STRATA AND EXPRESSED AS A PERCENTAGE. TOPSOIL (TS) - SURFACE SOILS USUALLY CONTAINING ORGANIC MATTER.					
SOIL LEGEND AND AASHTO CLASSIFICATION						MINERALOGICAL COMPOSITION						WEATHERING											
GENERAL CLASS. GRANULAR MATERIALS (≤ 35% PASSING #200) SILT-CLAY MATERIALS (> 35% PASSING #200) ORGANIC MATERIALS						MINERAL NAMES SUCH AS QUARTZ, FELDSPAR, MICA, TALC, KAOLIN, ETC. ARE USED IN DESCRIPTIONS WHENEVER THEY ARE CONSIDERED OF SIGNIFICANCE.						FRESH - ROCK FRESH, CRYSTALS BRIGHT, FEW JOINTS MAY SHOW SLIGHT STAINING, ROCK RINGS UNDER HAMMER IF CRYSTALLINE. VERY SLIGHT (V SLI) - ROCK GENERALLY FRESH, JOINTS STAINED, SOME JOINTS MAY SHOW THIN CLAY COATINGS IF OPEN, CRYSTALS ON A BROKEN SPECIMEN FACE SHINE BRIGHTLY, ROCK RINGS UNDER HAMMER BLOWS IF OF A CRYSTALLINE NATURE. SLIGHT (SLI) - ROCK GENERALLY FRESH, JOINTS STAINED AND DISCOLORATION EXTENDS INTO ROCK UP TO 1 INCH, OPEN JOINTS MAY CONTAIN CLAY, IN GRANITOID ROCKS SOME OCCASIONAL FELDSPAR CRYSTALS ARE DULL AND DISCOLORED, CRYSTALLINE ROCKS RING UNDER HAMMER BLOWS. MODERATE (MOD) - SIGNIFICANT PORTIONS OF ROCK SHOW DISCOLORATION AND WEATHERING EFFECTS, IN GRANITOID ROCKS, MOST FELDSPARS ARE DULL AND DISCOLORED, SOME SHOW SLAY, ROCK HAS DULL SOUND UNDER HAMMER BLOWS AND SHOWS SIGNIFICANT LOSS OF STRENGTH AS COMPARED WITH FRESH ROCK. MODERATELY SEVERE (MOD. SEV.) - ALL ROCK EXCEPT QUARTZ DISCOLORED OR STAINED. IN GRANITOID ROCKS, ALL FELDSPARS DULL AND DISCOLORED AND A MAJORITY SHOW KAOLINIZATION. ROCK SHOWS SEVERE LOSS OF STRENGTH AND CAN BE EXCAVATED WITH A GEOLOGIST'S PICK. ROCK GIVES "CLUNK" SOUND WHEN STRUCK. <i>IF TESTED, WOULD YIELD SPT REFUSAL</i> . SEVERE (SEV.) - ALL ROCK EXCEPT QUARTZ DISCOLORED OR STAINED, ROCK FABRIC CLEAR AND EVIDENT BUT REDUCED IN STRENGTH TO STRONG SOIL. IN GRANITOID ROCKS ALL FELDSPARS ARE KAOLINIZED TO SOME EXTENT, SOME FRAGMENTS OF STRONG ROCK USUALLY REMAIN. <i>IF TESTED, YIELDS SPT N VALUES > 100 BPF</i> . VERY SEVERE (V SEV.) - ALL ROCK EXCEPT QUARTZ DISCOLORED OR STAINED, ROCK FABRIC ELEMENTS ARE DISCERNIBLE BUT THE MASS IS EFFECTIVELY REDUCED TO SOIL STATUS, WITH ONLY FRAGMENTS OF STRONG ROCK REMAINING. SAPROLITE IS AN EXAMPLE OF ROCK WEATHERED TO A DEGREE SUCH THAT ONLY MINOR VESTIGES OF THE ORIGINAL ROCK REMAIN. <i>IF TESTED, YIELDS SPT N VALUES < 100 BPF</i> . COMPLETE - ROCK REDUCED TO SOIL, ROCK FABRIC NOT DISCERNIBLE, OR DISCERNIBLE ONLY IN SMALL AND SCATTERED CONCENTRATIONS, QUARTZ MAY BE PRESENT AS DIKES OR STRINGERS, SAPROLITE IS ALSO AN EXAMPLE.											
COMPRESSIBILITY						PERCENTAGE OF MATERIAL						GROUND WATER											
SLIGHTLY COMPRESSIBLE MODERATELY COMPRESSIBLE HIGHLY COMPRESSIBLE						LIQUID LIMIT LESS THAN 31 LIQUID LIMIT EQUAL TO 31-50 LIQUID LIMIT GREATER THAN 50						▽ WATER LEVEL IN BORE HOLE IMMEDIATELY AFTER DRILLING ▽ STATIC WATER LEVEL AFTER 24 HOURS ▽ PW PERCHED WATER, SATURATED ZONE, OR WATER BEARING STRATA ○ SPRING OR SEEP											
GROUND WATER						MISCELLANEOUS SYMBOLS						ROCK HARDNESS											
PI OF A-7-5 SUBGROUP IS ≤ LL - 30 ; PI OF A-7-6 SUBGROUP IS > LL - 30						ROADWAY EMBANKMENT (RE) WITH SOIL DESCRIPTION SOIL SYMBOL ARTIFICIAL FILL (AF) OTHER THAN ROADWAY EMBANKMENT INFERRED SOIL BOUNDARY INFERRED ROCK LINE ALLUVIAL SOIL BOUNDARY DIP & DIP DIRECTION OF ROCK STRUCTURES SOUNDING ROD						SPT TEST BORING AUGER BORING CORE BORING MONITORING WELL PIEZOMETER INSTALLATION SLOPE INDICATOR INSTALLATION SPT N-VALUE SPT REFUSAL											
CONSISTENCY OR DENSENESS						ABBREVIATIONS						FRACURE SPACING											
PRIMARY SOIL TYPE COMPACTNESS OR CONSISTENCY RANGE OF STANDARD PENETRATION RESISTANCE (N-VALUE) RANGE OF UNCONFINED COMPRESSIVE STRENGTH (TONS/FT ²)						AR - AUGER REFUSAL BT - BORING TERMINATED CL - CLAY CPT - CONE PENETRATION TEST CSE - COARSE DMT - DILATOMETER TEST DPT - DYNAMIC PENETRATION TEST ø - VOID RATIO F - FINE FOSS - FOSSILIFEROUS FRAC. - FRACTURED, FRACTURES FRAGS. - FRAGMENTS HI - HIGHLY MED. - MEDIUM MICA - MICACEOUS MOD. - MODERATELY NP - NON PLASTIC ORG. - ORGANIC PMT - PRESSUREMETER TEST SAP. - SAPROLITIC SD. - SAND, SANDY SL. - SILT, SILTY SLI. - SLIGHTLY TCR - TRICONE REFUSAL W - MOISTURE CONTENT V - VERY VST - VANE SHEAR TEST WEA. - WEATHERED γ - UNIT WEIGHT γ _d - DRY UNIT WEIGHT						VERY HARD - CANNOT BE SCRATCHED BY KNIFE OR SHARP PICK. BREAKING OF HAND SPECIMENS REQUIRES SEVERAL HARD BLOWS OF THE GEOLOGIST'S PICK. HARD - CAN BE SCRATCHED BY KNIFE OR PICK ONLY WITH DIFFICULTY. HARD HAMMER BLOWS REQUIRED TO DETACH HAND SPECIMEN. MODERATELY HARD - CAN BE SCRATCHED BY KNIFE OR PICK, GOUGES OR GROOVES TO 0.25 INCHES DEEP CAN BE EXCAVATED BY HARD BLOW OF A GEOLOGIST'S PICK, HAND SPECIMENS CAN BE DETACHED BY MODERATE BLOWS. MEDIUM HARD - CAN BE GROOVED OR GOUGED 0.05 INCHES DEEP BY FIRM PRESSURE OF KNIFE OR PICK POINT. CAN BE EXCAVATED IN SMALL CHIPS TO PIECES 1 INCH MAXIMUM SIZE BY HARD BLOWS OF THE POINT OF A GEOLOGIST'S PICK. SOFT - CAN BE GROVED OR GOUGED READILY BY KNIFE OR PICK. CAN BE EXCAVATED IN FRAGMENTS FROM CHIPS TO SEVERAL INCHES IN SIZE BY MODERATE BLOWS OF A PICK POINT. SMALL, THIN PIECES CAN BE BROKEN BY FINGER PRESSURE. VERY SOFT - CAN BE CARVED WITH KNIFE. CAN BE EXCAVATED READILY WITH POINT OF PICK. PIECES 1 INCH OR MORE IN THICKNESS CAN BE BROKEN BY FINGER PRESSURE. CAN BE SCRATCHED READILY BY FINGER NAIL.											
TEXTURE OR GRAIN SIZE						EQUIPMENT USED ON SUBJECT PROJECT						BEDDING											
U.S. STD. SIEVE SIZE OPENING (mm) 4 10 40 60 200 270 4.76 2.00 0.42 0.25 0.075 0.053						DRILL UNITS: [] MOBILE B- [] BK-51 [X] CME-45B [] CME-550 [] PORTABLE HOIST [X] CME-55						ADVANCING TOOLS: [] CLAY BITS [] 6" CONTINUOUS FLIGHT AUGER [] 8" HOLLOW AUGERS [] HARD FACED FINGER BITS [] TUNG-CARBIDE INSERTS [X] CASING [X] W/ ADVANCER [X] TRICONE 2 1/2" * STEEL TEETH [] TRICONE * TUNG-CARB. [] CORE BIT											
SOIL MOISTURE - CORRELATION OF TERMS						INTEGRATION						TERMS											
SOIL MOISTURE SCALE (ATTERBERG LIMITS) FIELD MOISTURE DESCRIPTION GUIDE FOR FIELD MOISTURE DESCRIPTION LL LIQUID LIMIT - SATURATED - (SAT) USUALLY LIQUID; VERY WET, USUALLY FROM BELOW THE GROUND WATER TABLE PLASTIC RANGE (PI) PL PLASTIC LIMIT - WET - (W) SEMISOLID; REQUIRES DRYING TO ATTAIN OPTIMUM MOISTURE OM OPTIMUM MOISTURE - MOIST - (M) SOLID; AT OR NEAR OPTIMUM MOISTURE SL SHRINKAGE LIMIT - DRY - (D) REQUIRES ADDITIONAL WATER TO ATTAIN OPTIMUM MOISTURE						FOR SEDIMENTARY ROCKS, INTEGRATION IS THE HARDENING OF THE MATERIAL BY CEMENTING, HEAT, PRESSURE, ETC. FRIABLE - RUBBING WITH FINGER FREES NUMEROUS GRAINS; GENTLE BLOW BY HAMMER DISINTEGRATES SAMPLE. MODERATELY INDURATED - GRAINS CAN BE SEPARATED FROM SAMPLE WITH STEEL PROBE; BREAKS EASILY WHEN HIT WITH HAMMER. INDURATED - GRAINS ARE DIFFICULT TO SEPARATE WITH STEEL PROBE; DIFFICULT TO BREAK WITH HAMMER. EXTREMELY INDURATED - SHARP HAMMER BLOWS REQUIRED TO BREAK SAMPLE; SAMPLE BREAKS ACROSS GRAINS.						TERM SPACING THICKNESS VERY WIDE MORE THAN 10 FEET VERY THICKLY BEDDED > 4 FEET WIDE 3 TO 10 FEET THICKLY BEDDED 1.5 - 4 FEET MODERATELY CLOSE 1 TO 3 FEET THINLY BEDDED 0.16 - 1.5 FEET CLOSE 0.16 TO 1 FEET VERY THINLY BEDDED 0.03 - 0.16 FEET VERY CLOSE LESS THAN 0.16 FEET THICKLY LAMINATED 0.008 - 0.03 FEET THINLY LAMINATED < 0.008 FEET											
PLASTICITY						NOTES:						FRACURE SPACING											
NONPLASTIC LOW PLASTICITY MED. PLASTICITY HIGH PLASTICITY PLASTICITY INDEX (PI) DRY STRENGTH 0-5 VERY LOW 6-15 SLIGHT 16-25 MEDIUM 26 OR MORE HIGH						BENCH MARK: TBM NO. 2 - RR SPIKE IN BASE OF 20"-24" OAK @ I5+24.4, 33.17 RT OF CL. ELEVATION: 81.91 FT.						[X] MODERATELY CLOSE [] CLOSE [] VERY CLOSE											
COLOR						FRACURE SPACING						TERMS											
DESCRIPTIONS MAY INCLUDE COLOR OR COLOR COMBINATIONS (TAN, RED, YELLOW-BROWN, BLUE-GRAY). MODIFIERS SUCH AS LIGHT, DARK, STREAKED, ETC. ARE USED TO DESCRIBE APPEARANCE.						[] MODERATELY CLOSE [] CLOSE [] VERY CLOSE						FRIABLE MODERATELY INDURATED INDURATED EXTREMELY INDURATED											

PROJECT REFERENCE NO.	SHEET
B-4328	3 OF 9
SITE PLAN	

SKEW = 75°



15

18

WOODS

WOODS

THE GREAT SWAMP

BRIDGE #3

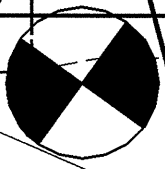
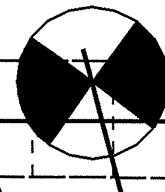
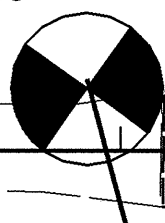
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S 35° 45' 00.9" E

SR 1634

TO OLD US 117

TO SR 1635



EBI-A

BI-B

B2-A

EB2-B

WOODS

WOODS

EBI BENT LINE

BI BENT LINE

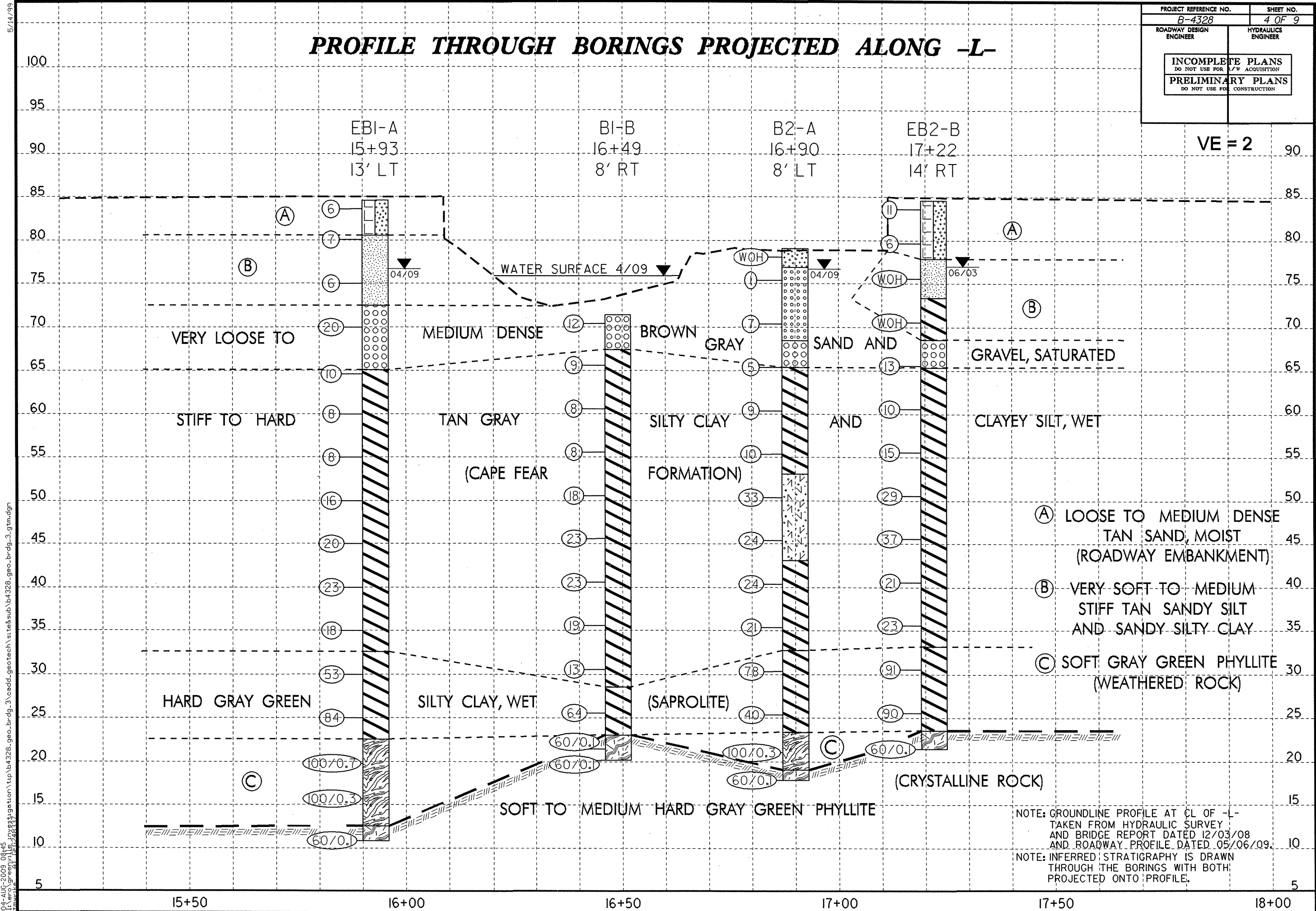
B2 BENT LINE

EB2 BENT LINE

THE GREAT SWAMP

PROFILE THROUGH BORINGS PROJECTED ALONG -L-

VE = 2



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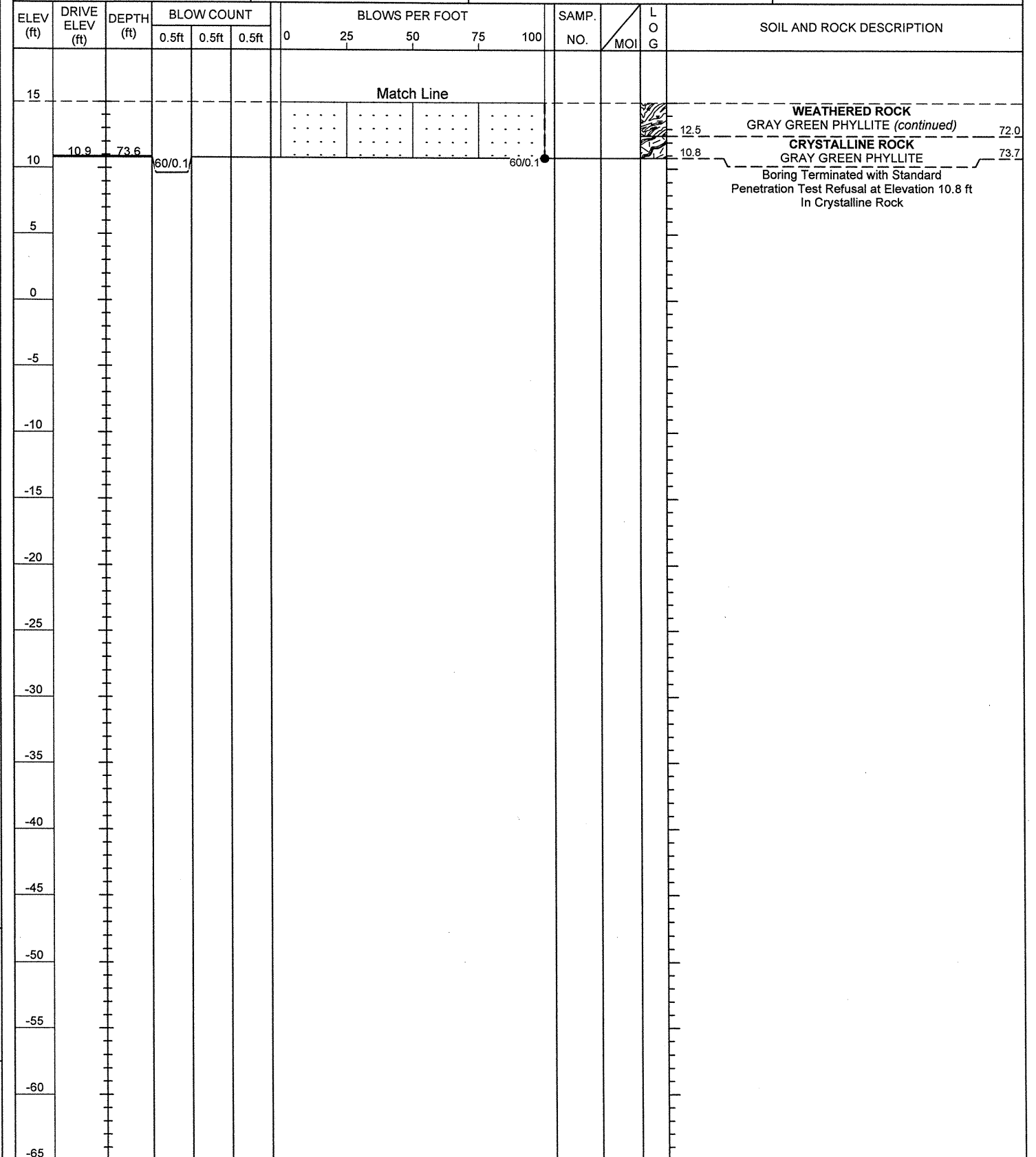
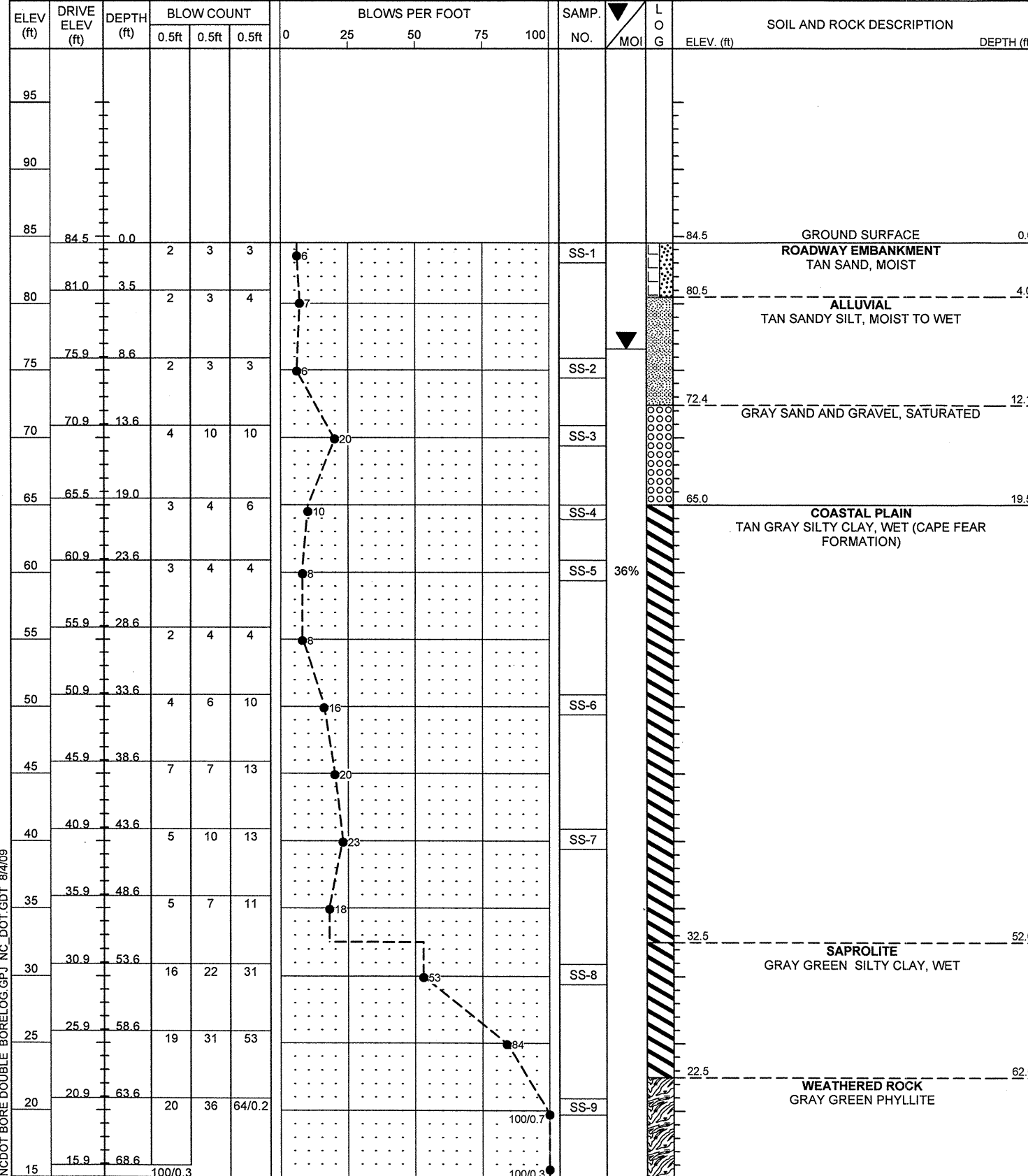


NCDOT GEOTECHNICAL ENGINEERING UNIT

BORELOG REPORT

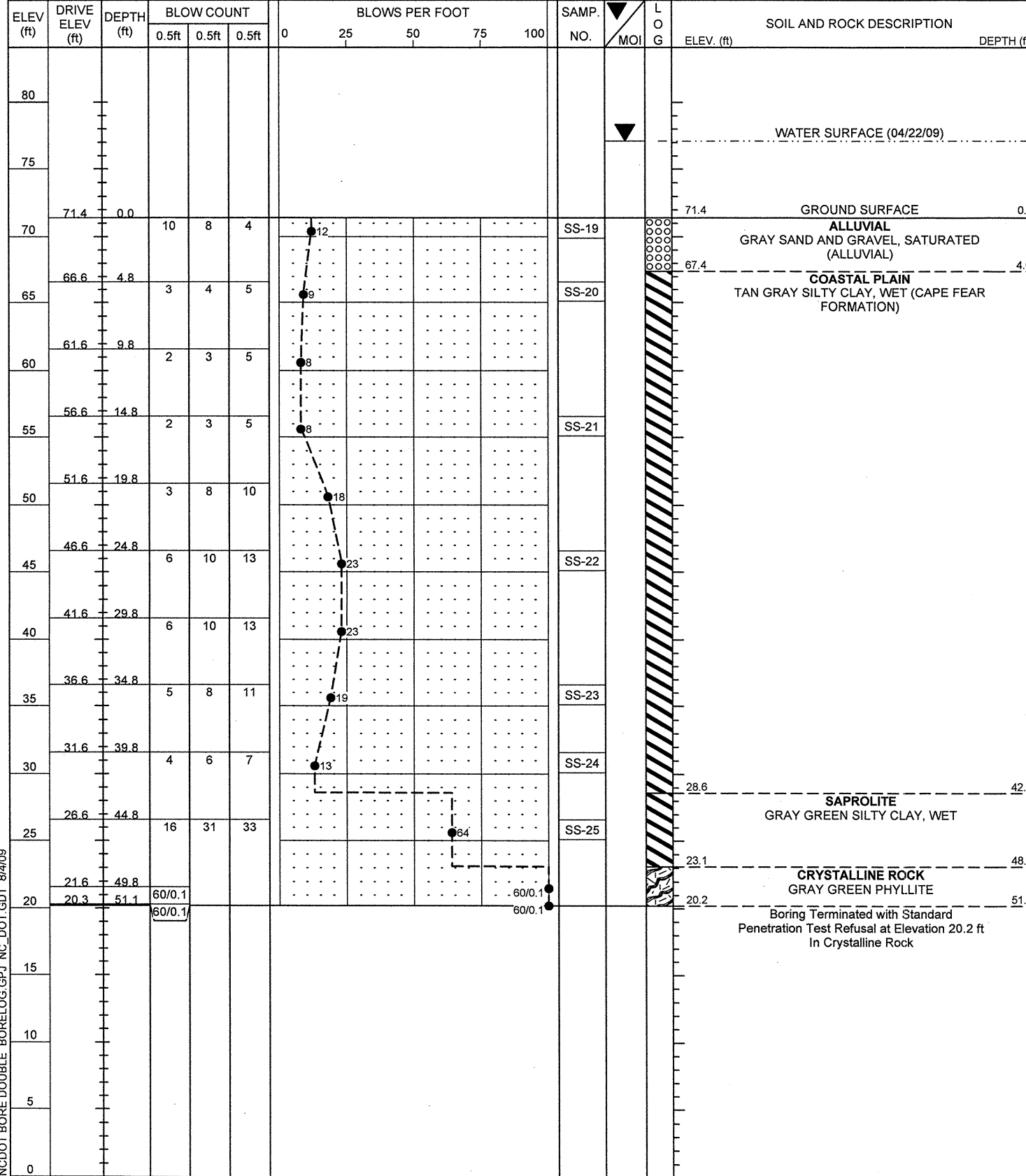
PROJECT NO. 33665.1.1	ID. B-4328	COUNTY Wilson	GEOLOGIST Wrike, C. M.
SITE DESCRIPTION BRIDGE NO. 3 ON -L- (SR 1634) OVER GREAT SWAMP CREEK			GROUND WTR (ft)
BORING NO. EB1-A	STATION 15+93	OFFSET 13ft LT	ALIGNMENT -L-
COLLAR ELEV. 84.5 ft	TOTAL DEPTH 73.7 ft	NORTHING 678,217	EASTING 2,311,485
DRILL MACHINE CME-55	DRILL METHOD Mud Rotary	HAMMER TYPE Automatic	
START DATE 04/17/09	COMP. DATE 04/21/09	SURFACE WATER DEPTH N/A	DEPTH TO ROCK 72.0 ft

PROJECT NO. 33665.1.1	ID. B-4328	COUNTY Wilson	GEOLOGIST Wrike, C. M.
SITE DESCRIPTION BRIDGE NO. 3 ON -L- (SR 1634) OVER GREAT SWAMP CREEK			GROUND WTR (ft)
BORING NO. EB1-A	STATION 15+93	OFFSET 13ft LT	ALIGNMENT -L-
COLLAR ELEV. 84.5 ft	TOTAL DEPTH 73.7 ft	NORTHING 678,217	EASTING 2,311,485
DRILL MACHINE CME-55	DRILL METHOD Mud Rotary	HAMMER TYPE Automatic	
START DATE 04/17/09	COMP. DATE 04/21/09	SURFACE WATER DEPTH N/A	DEPTH TO ROCK 72.0 ft

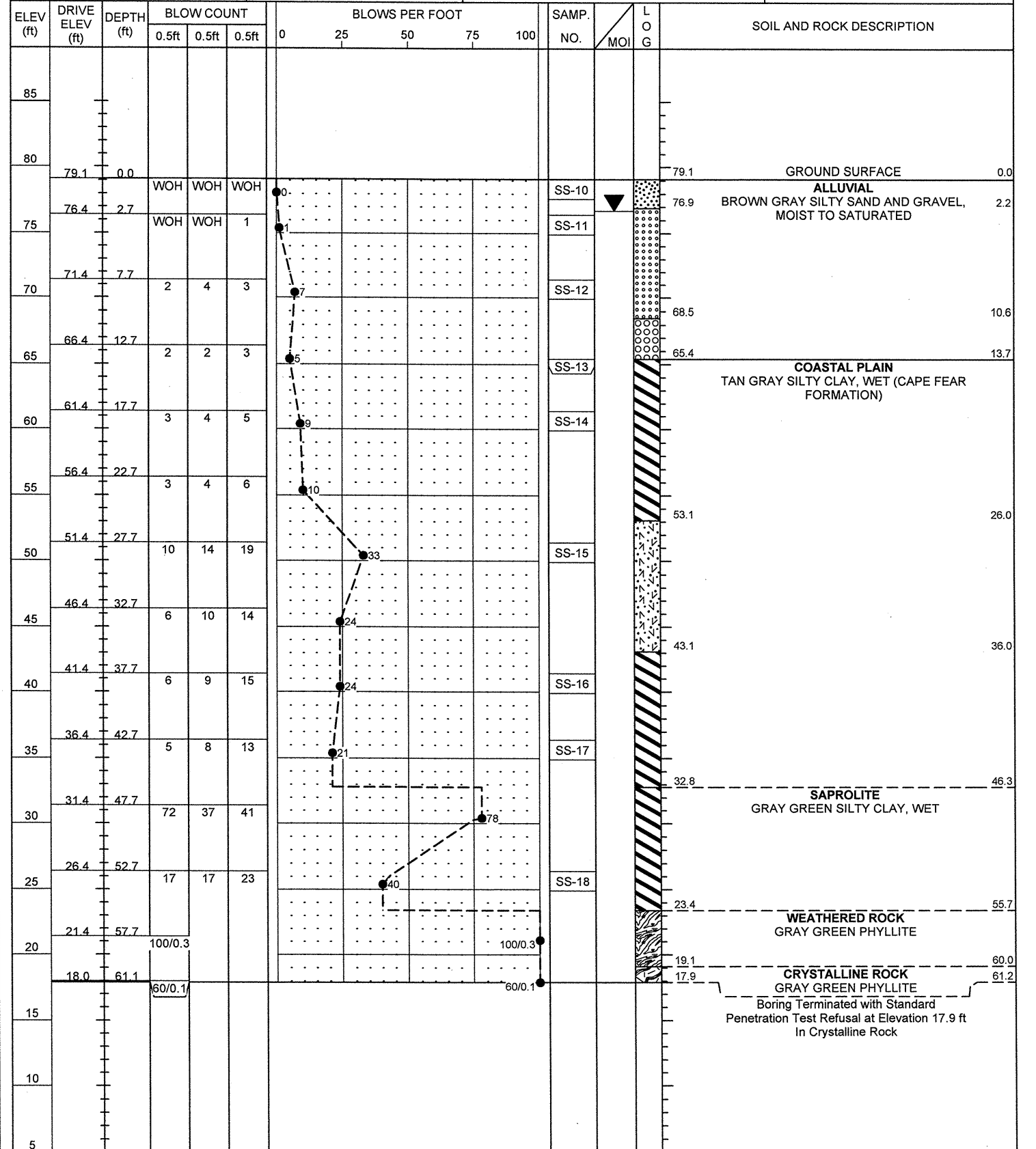


NCDOT BORE DOUBLE BORELOG.GPJ NC_DOT_GDT 8/4/09

PROJECT NO. 33665.1.1	ID. B-4328	COUNTY Wilson	GEOLOGIST Wrike, C. M.
SITE DESCRIPTION BRIDGE NO. 3 ON -L- (SR 1634) OVER GREAT SWAMP CREEK			GROUND WTR (ft)
BORING NO. B1-B	STATION 16+49	OFFSET 8ft RT	ALIGNMENT -L-
COLLAR ELEV. 71.4 ft	TOTAL DEPTH 51.2 ft	NORTHING 678,159	EASTING 2,311,501
DRILL MACHINE CME-55		DRILL METHOD Mud Rotary	HAMMER TYPE Automatic
START DATE 04/22/09	COMP. DATE 04/22/09	SURFACE WATER DEPTH 5.7ft	DEPTH TO ROCK 48.3 ft



PROJECT NO. 33665.1.1	ID. B-4328	COUNTY Wilson	GEOLOGIST Wrike, C. M.
SITE DESCRIPTION BRIDGE NO. 3 ON -L- (SR 1634) OVER GREAT SWAMP CREEK			GROUND WTR (ft)
BORING NO. B2-A	STATION 16+90	OFFSET 8ft LT	ALIGNMENT -L-
COLLAR ELEV. 79.1 ft	TOTAL DEPTH 61.2 ft	NORTHING 678,132	EASTING 2,311,540
DRILL MACHINE CME-55		DRILL METHOD Mud Rotary	HAMMER TYPE Automatic
START DATE 04/21/09	COMP. DATE 04/22/09	SURFACE WATER DEPTH N/A	DEPTH TO ROCK 60.0 ft



NCDOT BORE DOUBLE BORELOG.GPJ NC_DOT_GDT 8/4/09



NCDOT GEOTECHNICAL ENGINEERING UNIT

BORELOG REPORT

PROJECT NO. 33665.1.1	ID. B-4328	COUNTY Wilson	GEOLOGIST Hager, M. M.
SITE DESCRIPTION BRIDGE NO. 3 ON -L- (SR 1634) OVER GREAT SWAMP CREEK			GROUND WTR (ft)
BORING NO. EB2-B	STATION 17+22	OFFSET 14ft RT	ALIGNMENT -L-
COLLAR ELEV. 84.6 ft	TOTAL DEPTH 63.1 ft	NORTHING 678,096	EASTING 2,311,539
DRILL MACHINE CME-45B	DRILL METHOD Mud Rotary	HAMMER TYPE Automatic	
START DATE 06/12/03	COMP. DATE 06/12/03	SURFACE WATER DEPTH N/A	DEPTH TO ROCK 61.0 ft

ELEV (ft)	DRIVE ELEV (ft)	DEPTH (ft)	BLOW COUNT			BLOWS PER FOOT					SAMP. NO.	LOG MOI	SOIL AND ROCK DESCRIPTION	DEPTH (ft)
			0.5ft	0.5ft	0.5ft	0	25	50	75	100				
90														
85	84.6	0.0											GROUND SURFACE	0.0
80	80.7	3.9	2	5	6								ROADWAY EMBANKMENT TAN CLAYEY SAND, MOIST	
75	76.6	8.0	3	3	3								ALLUVIAL TAN SANDY SILT, MOIST TO WET	6.7
70	71.6	13.0	WOR	WOR	WOH								DARK BROWN SANDY SILTY CLAY, WET	11.2
65	66.6	18.0	WOR	WOH	WOH								GRAY SAND AND GRAVEL, SATURATED	16.0
60	61.6	23.0	4	7	6								COASTAL PLAIN TAN GRAY SILTY CLAY, WET (CAPE FEAR FORMATION)	19.2
55	56.6	28.0	4	4	6									
50	51.6	33.0	7	7	8									
45	46.6	38.0	7	12	17									
40	41.6	43.0	8	14	23									
35	36.6	48.0	6	9	12									
30	31.6	53.0	6	10	13									
25	26.6	58.0	27	43	48								SAPROLITE GRAY GREEN SILTY CLAY, WET	51.4
20	21.6	63.0	12	37	53								CRYSTALLINE ROCK GRAY GREEN PHYLLITE	61.0
15													Boring Terminated with Standard Penetration Test Refusal at Elevation 21.5 ft In Crystalline Rock	63.1
10														

NCDOT BORE DOUBLE BORELOG.GPJ NC_DOT.GDT 8/4/09

B-4328

33665.1.1

BRIDGE NO. 3 ON SR 1634 OVER GREAT SWAMP CREEK AT -L- STA. 16+67

EB1-A

SOIL TEST RESULTS															
SAMPLE NO.	OFFSET	STATION	DEPTH INTERVAL	AASHTO CLASS.	LL.	P.I.	% BY WEIGHT				% PASSING (SIEVES)			% MOISTURE	% ORGANIC
							C. SAND	F. SAND	SILT	CLAY	10	40	200		
SS- 1	13 LT	15+93	1.0- 1.5	A- 2- 4(0)	15	2	45.5	31.1	9.3	14.1	98	73	26	-	-
SS- 2	13 LT	15+93	8.6- 10.1	A- 4(0)	18	1	7.3	58.4	20.2	14.1	100	99	44	-	-
SS- 3	13 LT	15+93	13.6- 15.1	A- 1- a(0)	22	NP	68.5	21.4	4.0	6.1	31	15	4	-	-
SS- 4	13 LT	15+93	19.5- 20.5	A- 7- 5(16)	53	21	6.5	15.2	31.9	46.5	86	82	73	-	-
SS- 5	13 LT	15+93	23.6- 25.1	A- 7- 5(26)	60	22	2.0	11.5	52.1	34.3	100	99	92	36.4	-
SS- 6	13 LT	15+93	33.6- 35.1	A- 7- 5(19)	57	16	6.7	15.8	45.3	32.3	100	95	86	-	-
SS- 7	13 LT	15+93	43.6- 45.1	A- 7- 5(17)	50	15	3.4	17.4	44.8	34.3	99	97	87	-	-
SS- 8	13 LT	15+93	53.6- 55.1	A- 7- 5(14)	45	14	3.6	21.8	48.3	26.3	100	98	84	-	-
SS- 9	13 LT	15+93	63.6- 64.8	A- 7- 6(13)	44	16	9.1	22.2	46.5	22.2	100	95	77	-	-

B1-B

SOIL TEST RESULTS															
SAMPLE NO.	OFFSET	STATION	DEPTH INTERVAL	AASHTO CLASS.	LL.	P.I.	% BY WEIGHT				% PASSING (SIEVES)			% MOISTURE	% ORGANIC
							C. SAND	F. SAND	SILT	CLAY	10	40	200		
SS- 19	8 RT	16+49	1.0- 1.5	A- 1- b(0)	23	5	60.0	21.0	10.9	8.1	54	33	12	-	-
SS- 20	8 RT	16+49	4.8- 6.3	A- 7- 5(15)	50	16	7.1	16.0	44.6	32.3	97	92	81	-	-
SS- 21	8 RT	16+49	14.8- 16.3	A- 7- 5(24)	57	21	2.6	14.7	52.3	30.3	100	99	90	-	-
SS- 22	8 RT	16+49	24.8- 26.3	A- 7- 5(15)	51	12	4.4	16.2	45.1	34.3	100	97	87	-	-
SS- 23	8 RT	16+49	34.8- 36.3	A- 7- 5(17)	53	14	3.2	14.1	48.3	34.3	100	98	89	-	-
SS- 24	8 RT	16+49	39.8- 41.3	A- 7- 5(22)	52	18	0.4	11.1	52.1	36.4	100	100	95	-	-
SS- 25	8 RT	16+49	44.8- 46.3	A- 7- 6(18)	46	18	1.8	16.8	61.2	20.2	100	99	89	-	-

B2-A

SOIL TEST RESULTS															
SAMPLE NO.	OFFSET	STATION	DEPTH INTERVAL	AASHTO CLASS.	LL.	P.I.	% BY WEIGHT				% PASSING (SIEVES)			% MOISTURE	% ORGANIC
							C. SAND	F. SAND	SILT	CLAY	10	40	200		
SS- 10	8 LT	16+90	1.0- 1.5	A- 2- 4(0)	19	NP	32.5	44.6	12.7	10.1	99	80	27	-	-
SS- 11	8 LT	16+90	2.7- 4.2	A- 3(0)	19	NP	64.3	32.9	2.7	0.0	100	83	4	-	-
SS- 12	8 LT	16+90	7.7- 9.2	A- 3(0)	25	NP	68.3	29.3	2.4	0.0	99	80	3	-	-
SS- 13	8 LT	16+90	13.7- 14.2	A- 7- 5(7)	56	19	8.3	8.7	46.7	36.4	57	53	50	-	-
SS- 14	8 LT	16+90	17.7- 19.2	A- 7- 5(20)	55	16	2.4	12.7	68.7	16.2	100	99	91	-	-
SS- 15	8 LT	16+90	27.7- 29.2	A- 5(11)	48	9	5.3	19.0	51.5	24.2	100	98	83	-	-
SS- 16	8 LT	16+90	37.7- 39.2	A- 7- 5(24)	57	20	2.2	11.5	60.0	26.3	100	99	92	-	-
SS- 17	8 LT	16+90	42.7- 44.2	A- 7- 5(36)	62	31	0.8	5.7	61.2	32.3	100	100	97	-	-
SS- 18	8 LT	16+90	52.7- 54.2	A- 7- 6(28)	54	26	1.8	11.7	50.1	36.4	99	98	92	-	-

EB2-B

SOIL TEST RESULTS															
SAMPLE NO.	OFFSET	STATION	DEPTH INTERVAL	AASHTO CLASS.	LL.	P.I.	% BY WEIGHT				% PASSING (SIEVES)			% MOISTURE	% ORGANIC
							C. SAND	F. SAND	SILT	CLAY	10	40	200		
SS- 26	14 RT	17+22	3.9- 5.4	A- 2- 4(0)	24	6	18.5	52.7	8.4	20.4	95	87	30	-	-
SS- 27	14 RT	17+22	8.0- 9.5	A- 4(0)	27	10	14.9	51.1	9.6	24.4	100	95	36	-	-
SS- 28	14 RT	17+22	13.0- 14.5	A- 7- 5(8)	46	14	4.1	43.6	32.0	20.4	100	99	60	-	-
SS- 29	14 RT	17+22	23.0- 24.5	A- 7- 5(26)	58	23	2.4	15.1	35.6	46.8	100	99	90	-	-
SS- 30	14 RT	17+22	33.0- 34.5	A- 7- 5(18)	55	15	4.5	13.8	45.0	36.7	98	95	87	-	-
SS- 31	14 RT	17+22	43.0- 44.5	A- 7- 5(30)	63	26	1.8	10.8	34.4	53.0	97	96	91	-	-
SS- 32	14 RT	17+22	48.0- 49.5	A- 7- 6(30)	58	34	2.6	18.9	43.8	34.6	95	93	83	-	-
SS- 33	14 RT	17+22	58.0- 59.5	A- 7- 6(13)	46	20	16.9	22.6	38.1	22.4	99	88	68	-	-



FIELD SCOUR REPORT

WBS: 33665.1.1 TIP: B-4328 COUNTY: WILSON

DESCRIPTION(1): Bridge No. 3 on -L- (SR 1634) over Great Swamp Creek

EXISTING BRIDGE

Information from: Field Inspection Microfilm _____ (reel _____ pos: _____)
 Other (explain) _____

Bridge No.: 3 Length: 103 FT Total Bents: 7 Bents in Channel: 3 Bents in Floodplain: 4
 Foundation Type: Timber piles

EVIDENCE OF SCOUR(2)

Abutments or End Bent Slopes: Scour pocket 1'-2' at EB2

Interior Bents: Scour pockets: 0.5' to 1' deep around piles B1-B3, erosion: 1-2 feet B1 through B3

Channel Bed: None noted

Channel Bank: Erosion 2' on south bank under bridge

EXISTING SCOUR PROTECTION

Type(3): 1) Abutment walls and wing walls, 2) Rip-rap on EB1

Extent(4): 1) 2'-6' past edge of bridge on EB1 & EB2 2) Under bridge & 10' outside east edge of bridge.

Effectiveness(5): Effective

Obstructions(6): Debris against B3 and in cross-bracing on B4 and B5

INSTRUCTIONS

- 1 Describe the specific site's location, including route number and body of water crossed.
- 2 Note scour evidence at existing end bents or abutments (e.g. undermining, sloughing, degradations).
- 3 Note existing scour protection (e.g. rip rap).
- 4 Describe extent of existing scour protection.
- 5 Describe whether or not the scour protection appears to be working.
- 6 Note obstructions such as dams, fallen trees, debris at bents, etc.
- 7 Describe the channel bed material based on observation and/or samples. Include any lab results with report.
- 8 Describe the channel bank material based on observation and/or samples. Include any lab results with report.
- 9 Describe the material covering the banks (e.g. grass, trees, rip rap, none).
- 10 Determine the approximate floodplain width from field observation or a topographic map.
- 11 Describe the material covering the floodplain (e.g. grass, trees, crops).
- 12 Use professional judgement to specify if the stream is degrading, aggrading, or static.
- 13 Describe potential and direction of the stream to migrate laterally during the bridge's life (approx. 100 years).
- 14 Give the design scour elevation (DSE) expected over the life of the bridge (approx. 100 years). This elevation can be given as a range across the site, or for each bent. Discuss the relationship between the Hydraulics Unit theoretical scour and the DSE. If the DSE is dependent on scour counter measures, explain (e.g. rip rap armoring on slopes). The DSE is based on the erodability of materials, giving consideration to the influence of joints, foliation, bedding characteristics, % core recovery, % RQD, differential weathering, shear strength, observations at existing structures, other tests deemed appropriate, and overall geologic conditions at the site.

DESIGN INFORMATION

Channel Bed Material(7): Sand and gravel

Channel Bank Material(8): Sand and silt

Channel Bank Cover(9): Grass and shrubs

Floodplain Width(10): 2100 feet

Floodplain Cover(11): Trees, grass, and shrubs

Stream is(12): Aggrading _____ Degrading Static _____

Channel Migration Tendency(13): Slight tendency to migrate southeast

Observations and Other Comments: 2 channels converge to one at bridge, second channel to west of EB1

DESIGN SCOUR ELEVATIONS(14)

Feet Meters _____

BENTS

B1

65										

Comparison of DSE to Hydraulics Unit theoretical scour:

Geotechnical analysis indicates the DSE is 4.1 feet higher than the Hydraulics Unit's theoretical scour.

SOIL ANALYSIS RESULTS FROM CHANNEL BED AND BANK MATERIAL

Bed or Bank										
Sample No.										
Retained #4										
Passed #10										
Passed #40										
Passed #200										
Coarse Sand										
Fine Sand										
Silt										
Clay										
LL										
PI										
AASHTO										
Station										
Offset										
Depth										

See Sheet 8,
"Soil Test Results",
for samples:
Channel Bed: SS-19
Channel Bank: SS-2, SS-10, SS-11, SS-27

Reported by: *Lynette M. Wrike*
 C.M. Wrike

Date: 6/17/2009