

**PROJECT SPECIAL PROVISIONS****ROADWAY****CLEARING AND GRUBBING METHOD:**

(9-17-02) (Rev 3-18-08)

M2 R01

Perform clearing on this project to the limits established by Method "II" shown on Standard No. 200.02 of the *2006 Metric Roadway Standard Drawings* at the following areas:

-Tank Trails,            -Perimeter Roads,            -            -Y2- (Smith Lake Road)

Perform clearing on this project to the limits established by Method "III" shown on Standard No. 200.03 of the *2006 Metric Roadway Standard Drawings* along the remainder of the project:

Revise the *2006 Metric Standard Specifications* as follows:

**Page 2-2, Article 200-3, Clearing, add the following as the 6th paragraph:**

At bridge sites, clear the entire width of the right of way beginning at a station 1 m back of the beginning extremity of the structure and ending at a station 1 m beyond the ending extremity of the structure.

**BURNING RESTRICTIONS:**

(7-1-95)

M2 R05

Open burning is not permitted on any portion of the right-of-way limits established for this project. Do not burn the clearing, grubbing or demolition debris designated for disposal and generated from the project at locations within the project limits, off the project limits or at any waste or borrow sites in this county. Dispose of the clearing, grubbing and demolition debris by means other than burning, according to state or local rules and regulations.

**BUILDING AND UNDERGROUND STORAGE TANK REMOVAL:**

(1-1-02) (Rev.6-21-05)

M2 R15

**Building Removal**

Remove the buildings and appurtenances listed below in accordance with Section 215 of the *2006 Metric Standard Specifications* and the following:

Prior to removal of any building, comply with the notification requirements of *Title 40 Code of Federal Regulations*, Part 61, Subpart M, which are applicable to asbestos. Give notification to the North Carolina Department of Health and Human Services, Division of Public Health Epidemiology Branch and/or the appropriate county agency when the county performs enforcement of the Federal Regulation. Submit a copy of the notification to the Engineer prior to the building removal.

Perform removal and disposal of asbestos in accordance with the requirements of *Title 40 Code of Federal Regulations*; comply with all Federal, State and local regulations when performing building removal and/or asbestos removal and disposal. Any fines resulting from violations of any regulation are the sole responsibility of the Contractor and the Contractor agrees to indemnify and hold harmless the Department against any assessment of such fines.

The Department has performed asbestos assessments for building items identified below. Copies of this report may be obtained through the Division Right-of-Way Agent. When asbestos is discovered after the opening of bids for the project, the Engineer may have the work performed by others or the cost of asbestos removal and disposal will be paid for in accordance with Article 104-7 of the *2006 Metric Standard Specifications*. When a building has had or will have asbestos removed and the Contractor elects to remove the building such that it becomes a public area, the Contractor is responsible for any additional costs incurred including final air monitoring.

### **Underground Storage Tank Removal**

Prior to removal of any Underground Storage Tank (UST), comply with the notification requirements of the *Title 40 Code of Federal Regulations*, Part 280.71(a). Give notification to the appropriate regional office of the North Carolina Department of Environment and Natural Resources, Division of Waste Management, UST Section. Submit a copy of the notification to the Engineer prior to the removal of the underground storage tank.

Permanently close UST systems by removal and disposal in compliance with the regulations set forth in *Title 40, Code of Federal Regulations*, Part 280.71 and *North Carolina Administrative Code (NCAC)* Title 15A, Chapter 2, Subchapter 2N and any applicable local regulations. Assess Underground Storage Tank sites at closure for the presence of contamination as required in *NCAC* Title 15A, Chapter 2, Subchapter 2N, Section .0803 and as directed by the appropriate Regional Office of the Division of Waste Management. Remove and dispose of UST systems and contents in a safe manner in conformance with requirements of *American Petroleum Institute Bulletin 1604*, Removal and Disposal of Used Underground Petroleum Storage Tanks, Chapters 3 through 6. (Note: As an exception to these requirements, the filling of the tank with water as a means of expelling vapors from the tank as described in Section 4.2.6.1 of *American Petroleum Institute Bulletin 1604*, will not be allowed. Comply with all Federal, State and local regulations when performing UST removal and contaminated material disposal. Any fines resulting from violations of any regulation are the sole responsibility of the Contractor and the Contractor agrees to indemnify and hold harmless the Department against any assessment of such fines.

Where underground storage tanks are indicated below, there will be no direct payment for the assessment or closure. When the contract does not indicate the presence of storage tanks and storage tanks are discovered after the opening of bids for the project, the Engineer may have the work performed by others or the cost of assessment, closure, and/or removal will be paid for in accordance with Article 104-7 of the *2006 Metric Standard Specifications*.

Disposition of any contaminated material associated with underground storage tanks will be made as provided in Article 107-26 of the *2006 Metric Standard Specifications*.

Building Removal
Parcel 055 - Left. of Survey Station 13+15, Line -Y2-
1 S Framed Dwelling

Building Removal
Parcel 055 - Left of Survey Station 13+40, Line -Y2-
Shed

Building Removal
Parcel 056 – Left of Survey Station 108+35, Line -L-
1 S Framed Dwelling

Building Removal
Parcel 056 – Left of Survey Station 108+50, Line -L-
Shed

Building Removal
Parcel 057 – Left of Survey Station 108+50, Line -L-
1 S Framed Dwelling

Building Removal
Parcel 057 – Left of Survey Station 108+70, Line -L-
Shed

Building Removal
Parcel 062- Right of Survey Station 108+60, Line -L-
H Tr

Building Removal
Parcel 065 – Right of Survey Station 13+60, Line -Y2-
Shed

Building Removal
Parcel 065 – on Survey Station 14+00, Line -Y2-
1 ½ S Framed Dwelling

Building Removal
Parcel 067 – Left of Survey Station 109+70, Line -L-
1 S Framed Dwelling

Building Removal
Parcel 067 – Left of Survey Station 109+80, Line -L-
1 S Framed Dwelling

Building Removal
Parcel 068 – Left of Survey Station 110+15, Line -L-
1 S Framed Dwelling

Building Removal
Parcel 080 – Right of Survey Station 112+60, Line -L-
1 S Framed Dwelling

Building Removal
Parcel 080 – Right of Survey Station 112+80, Line -L-
Detached Garage

Building Removal
Parcel 082 – Right of Survey Station 113+10, Line -L-
Detached Garage

Building Removal
Parcel 084 – Right of Survey Station 113+30, Line -L-
1 S Framed Dwelling

Building Removal
Parcel 084 – Right of Survey Station 113+55, Line -L-
Shed

Building Removal
Parcel 102 – Right of Survey Station 13+00, Line -Y9-
1 S Framed Dwelling

Building Removal
Parcel 102 – Left of Survey Station 13+00, Line -Y9-
Shed

Building Removal
Parcel 104 – Right of Survey Station 13+50, Line -Y9-
1 S Framed Dwelling

Building Removal
Parcel 114 – Right of Survey Station 6+50, Line -Y12- RPC
Framed Barn

Building Removal
Parcel 110 – on Survey Station 144+00, Line -L-
1 S Brick Dwelling

Building Removal
Parcel 097 – Left of Survey Station 139+00, Line -L-
1 S Framed Dwelling

Building Removal
Parcel 125A – Right of Survey Station 11+85, Line -Y Rev-
1 S Brick Apt. – Partially outside the right of way and/or construction limits

When the description of the work for an item requires a portion of the building to be cut off, that portion of the buildings and appurtenances located within the right of way and/or construction area shall be cut off by the Contractor and disposed of by him/her. The Engineer will denote on the building the line where the building is to be cut off. The Contractor will be required to cut the building off on a neat line along the construction line of right of boundary designated by the Engineer. The Contractor will not be required to do any repairing to that portion of the building located outside the right of way or construction area or to shore it up in any respect. All of the Contractor's work shall be confined to the right of way and construction area designated by the Engineer. **This paragraph pertains to the Building Removal Item on Parcel 125A.**

Building Removal
Parcel 097 – Left of Survey Station 139+60, Line -L-
Shed

Building Removal
Parcel 097 – Left of Survey Station 139+65, Line -L-
Garage

Building Removal
Parcel 097 – Left of Survey Station 139+80, Line -L-
1 S Brick Dwelling

### **LATERAL DITCHES:**

Excavate lateral ditches to full depth and designated outlets. Allow drainage to function for 30 days or an adequate time designated by the Engineer before undercutting or any embankment construction. Payment will be made under Section 240 of the NCDOT *Standard Specifications*.

### **EMBANKMENTS:**

(5-16-06) (Rev 10-19-10)

M2 R18

Revise the *2006 Metric Standard Specifications* as follows:

**Page 2-17, Article 235-3 MATERIALS**, amend as follows:

Add the following as the second sentence of the first paragraph:

Do not use material meeting the requirements of AASHTO M145 for soil classification A-2-5 and A-5 with a plasticity index (PI) of less than 8 within 300 mm of the subgrade.

Add the following as the second sentence of the second paragraph:

Aerate and dry material containing moisture content in excess of what is required to achieve embankment stability and specified density.

**Page 2-18, Subarticle 235-4(B) Embankment Formation**, add the following:

- (16) Do not place rock or broken pavement in embankment areas where piles or drilled shaft foundations are to be constructed. This shall include but not be limited to piles and foundations for structures, metal signal poles, overhead sign structures, and high mount lighting.

**AGGREGATE SUBGRADE:**

(9-18-07) (Rev 3-16-10)

M2 R35

**Description**

Construct aggregate subgrades in accordance with the contract or as directed by the Engineer. Undercut as needed in cut areas. Install fabric for soil stabilization and place Class IV Subgrade Stabilization at locations shown on the plans.

**Materials**

Refer to Division 10 of the *Metric Standard Specifications*.

<b>Item</b>	<b>Section</b>
Select Material, Class IV	1016
Fabric for Soil Stabilization, Type 4	1056

Use Class IV Select Material for Class IV Subgrade Stabilization. If Class IV Subgrade Stabilization does not meet the requirements of Article 1010-2 of the *Metric Standard Specifications*, the Engineer may consider the material reasonably acceptable in accordance with Article 105-3 of the *Metric Standard Specifications*.

**Construction Methods**

When shallow undercut is required to construct aggregate subgrades, undercut 150 mm to 600 mm as shown on the plans or as directed by the Engineer. Perform undercut excavation in accordance with Section 225 of the *Metric Standard Specifications*. Install fabric for soil stabilization in accordance with Article 270-3 of the *Metric Standard Specifications*. Place Class IV Subgrade Stabilization (standard size no. ABC) by end dumping ABC on the fabric. Do not operate heavy equipment on the fabric until it is covered with Class IV Subgrade Stabilization. Compact ABC to 92% of AASHTO T180 as modified by the Department or to the highest density that can be reasonably obtained.

Maintain Class IV Subgrade Stabilization in an acceptable condition and minimize the use of heavy equipment on ABC in order to avoid damaging aggregate subgrades. Provide and maintain drainage ditches and drains as required to prevent entrapping water in aggregate subgrades.

### Measurement and Payment

*Shallow Undercut* will be measured and paid for in cubic meters. Shallow undercut will be measured in accordance with Article 225-7 of the *Metric Standard Specifications*. The contract unit price for *Shallow Undercut* will be full compensation for excavating, hauling and disposing of materials to construct aggregate subgrades.

*Class IV Subgrade Stabilization* will be measured and paid for in metric tons. Class IV Subgrade Stabilization will be measured by weighing material in trucks in accordance with Article 106-7 of the *Metric Standard Specifications*. The contract unit price for *Class IV Subgrade Stabilization* will be full compensation for furnishing, hauling, handling, placing, compacting and maintaining ABC.

*Fabric for Soil Stabilization* will be measured and paid for in accordance with Article 270-4 of the *Metric Standard Specifications*.

Payment will be made under:

<b>Pay Item</b>	<b>Pay Unit</b>
Shallow Undercut	Cubic Meter
Class IV Subgrade Stabilization	Metric Ton

### **FALSE SUMPS:**

(7-1-95)

M2 R40

Construct false sumps in accordance with the details in the plans and at locations shown in the plans or at other locations as directed by the Engineer.

Payment for the work of construction of the false sumps will be made at the contract unit price per cubic meter for *Unclassified Excavation* or *Borrow Excavation* depending on the source of material, or included in *Grading-Lump Sum*.

### **SHOULDER AND FILL SLOPE MATERIAL:**

(5-21-02)

M2 R45 C

### **Description**

Perform the required shoulder and slope construction for this project in accordance with the applicable requirements of Section 560 and Section 235 of the *2006 Metric Standard Specifications* except as follows:

Construct the top 150 mm of shoulder and fill slopes with soils capable of supporting vegetation.

Provide soil with a P.I. greater than 6 and less than 25 and with a pH ranging from 5.5 to 6.8. Remove stones and other foreign material 50 mm or larger in diameter. All soil is subject to test and acceptance or rejection by the Engineer.

Obtain material from within the project limits or approved borrow source.

### **Compensation**

When the Contractor elects to obtain material from an area located beneath a proposed fill sections which does not require excavation for any reason other than to generate acceptable shoulder and fill slope material, the work of performing the excavation will be considered incidental to the item of *Borrow Excavation* or *Shoulder Borrow*. If there is no pay item for *Borrow* or *Shoulder Excavation* in the contract, this work will be considered incidental to *Unclassified Excavation*. Stockpile the excavated material in a manner to facilitate measurement by the Engineer. Fill the void created by the excavation of the shoulder and fill slope material with suitable material. Payment for material used from the stockpile will be made at the contract unit price for *Borrow Excavation* or *Shoulder Borrow*. If there is no pay item for *Borrow Excavation* or *Shoulder Borrow*, then the material will be paid for at the contract unit price for *Unclassified Excavation*. The material used to fill the void created by the excavation of the shoulder and fill slope material will be made at the contract unit price for *Unclassified Excavation*, *Borrow Excavation*, or *Shoulder Borrow*, depending on the source of the material.

Material generated from undercut excavation, unclassified excavation or clearing and grubbing operations that is placed directly on shoulders or slope areas, will not be measured separately for payment, as payment for the work requiring the excavation will be considered adequate compensation for depositing and grading the material on the shoulders or slopes.

When undercut excavation is performed at the direction of the Engineer and the material excavated is found to be suitable for use as shoulder and fill slope material, and there is no area on the project currently prepared to receive the material generated by the undercut operation, the Contractor may construct a stockpile for use as borrow at a later date. Payment for the material used from the stockpile will be made at the contract unit price for *Borrow Excavation* or *Shoulder Borrow*.

When shoulder material is obtained from borrow sources or from stockpiled material, payment for the work of shoulder construction will be made at the contract unit price per cubic yard for *Borrow Excavation* or *Shoulder Borrow* in accordance with the applicable provisions of Section 230 or Section 560 of the *2006 Metric Standard Specifications*.

## **GEOGRID REINFORCED SLOPES**

### **1. DESCRIPTION**

This work consists of furnishing and installing geogrid reinforcement for stabilizing the embankment slopes in accordance with these provisions and the plans and as directed by the Engineer. Primary geogrid reinforcement shall be used at the dual bridges on Fayetteville Outer



is required at End Bents 1 and 2 to stabilize the proposed 1.5:1 front slopes. Secondary geogrid reinforcement shall be used between the layers of primary reinforcement to limit surface sloughing. A preconstruction conference shall be scheduled with representatives of the Contractor, Resident Engineer, and Geotechnical Engineering Unit to discuss construction details and quality control measures.

## 2. MATERIALS

### 2.1 Geogrid

The geogrid (primary and secondary) shall be composed of polypropylene, high density polyethylene or polyester. The geogrid shall be a regular network of integrally connected elements with aperture geometry sufficient to permit significant mechanical interlock with the surrounding soil. The geogrid shall have high flexural rigidity and high tensile modulus in relation to the soil being reinforced and shall also have a high continuity of tensile strength through all of its elements. The geogrid shall be dimensionally stable and able to retain its geometry under construction stresses. The material shall have high resistance to ultraviolet degradation and to all forms of chemical and biological degradation encountered in the soil being reinforced.

The Contractor shall furnish a Type 2 Typical Certified Mill Test Report for the primary and secondary geogrid in accordance with Section 106-3 of the NCDOT Standard Specifications; however, the material shall be subject to inspection, test, or rejection by the Engineer at any time.

#### Primary Geogrid

Primary geogrid shall provide a minimum long-term design tensile strength ( $T_a$ ) of 45 kN/m.  $T_a$  is computed based on the following formula:

$$T_a = \frac{T_{ULT} \text{ (per ASTM D6637)}}{RF_{CR} \times RF_{ID} \times RF_D}$$

Only geogrids that have a National Transportation Product Evaluation Program (NTPEP) report will be allowed. Use the recommended values for  $RF_{CR}$ ,  $RF_{ID}$ , and  $RF_D$  from the NTPEP reports for default values.

### Secondary Geogrid

Secondary geogrid shall provide a minimum tensile strength of 10 kN/m and a minimum ultimate tensile strength of 19 kN/m determined in accordance with ASTM D 6637. These strength values are in the cross-machine direction (i.e., cross-roll direction).

### 2.2 Borrow Material

Borrow material incorporated into the bridge end slopes reinforced with primary geogrid shall meet the criteria for Coastal Plan Borrow outlined in Section 1018 of the NCDOT Standard Specifications with the additional criteria of a maximum P.I. of 10 and a maximum of 35% passing the No. 200 Sieve.

### 3. CONSTRUCTION

During all periods of shipment and storage, the geogrid shall be protected from temperatures greater than 140° F, direct sunlight, mud, wet cement, epoxy, or other materials which may alter its physical properties. At the time of installation, the geogrid shall be rejected if it has defects, tears, punctures, flaws, deterioration or damage incurred during manufacturing, transportation or storage. Any geogrid damaged during storage or installation shall be replaced by the Contractor at no additional cost to the Department.

The proper geogrid (primary or secondary) shall be placed and pulled tight at the proper location and orientation as shown on the plans and as directed by the Engineer. Correct orientation (machine direction) of the geogrid shall be verified by the Contractor. The geogrid shall be secured in-place to prevent movement during fill operations. The geogrid shall be secured with staples, pins, sandbags, or fill, or as directed by the Engineer. Tolerance in spacing of geogrid layers shall be within 50 mm at any place unless otherwise noted in the plans.

The first layer of primary geogrid shall be placed on the existing ground surface with a length as specified in the plans and with the machine direction (roll direction) perpendicular to the toe of slope. Subsequent layers of primary geogrid shall be placed horizontally as shown on the plans and as directed by the Engineer. Spacing between primary geogrid layers is two meters as shown on the plan typical section. To avoid potential deformation of the face and/or upper layer of geogrid, holes shall be cut in the top layer of primary geogrid at the anticipated pile locations prior to driving piles.

The primary geogrid shall be placed in continuous strips in the direction specified in the plans (perpendicular to the end slopes). No overlaps or connections shall be permitted in the machine direction of the primary geogrid layers. Adjacent rolls of primary geogrid, in the direction perpendicular to the toe of the end slopes, shall be rolled out and butted up against each other (side to side).

The secondary geogrid shall be placed between the layers of primary geogrid with the machine direction (roll direction) parallel to the toe of slope. Spacing between secondary geogrid layers is two meters as shown the plan typical section. Rolls of secondary geogrid shall be butted up next to each other (end to end) as shown on the plans and as directed by the Engineer.

Soil meeting the requirements for Coastal Plain Borrow (with the additional criteria of a maximum P.I. of 10 and a maximum of 35% passing the No. 200 Sieve) is required on top of each geogrid layer to the limits shown on the plans. Placement and compaction of borrow material fill shall conform to all applicable requirements of the NCDOT Standard Specifications. The fill shall be placed, spread, and compacted in a manner that prevents the development of wrinkles or movement of the geogrid. No equipment shall be allowed to operate directly on the geogrid. A minimum fill thickness of 150 mm is required prior to operation of any equipment or vehicle over the geogrid. Turning of vehicles shall be kept to a minimum, and sudden braking and sharp turning shall be avoided. Damaged geogrids shall be replaced at no cost to the Department.

The entire embankment should be constructed simultaneously with the geogrid reinforced slopes.

#### 4. FACE OF SLOPE

Each layer of geogrid must be exposed on the face of the slope before placing riprap. Overbuilding of the slope face is acceptable, but excavation to expose the geogrid is required without damaging the geogrid prior to placement of the riprap.

5. METHOD OF MEASUREMENT

The quantity of geogrid to be paid for will be the total number of square meters of each type (primary and secondary) of geogrid correctly placed in the completed embankment as shown on the plans or as directed by the Engineer.

The quantity of borrow material for geogrid reinforcement to be paid for will be the actual number of cubic meters of approved material measured in-place which has been incorporated into the completed embankment.

6. BASIS OF PAYMENT

The quantity of primary and secondary geogrid, measured as provided above, will be paid for at the contract unit price per square meter for "Primary Geogrid Reinforcement" and "Secondary Geogrid Reinforcement", respectively.

The quantity of borrow material, measured as provided above, will be paid for at the contract unit price per cubic meter for "Borrow Material for Geogrid Reinforcement".

Such prices and payments will be full compensation for all the work required by this provision including but not limited to: furnishing all materials, labor, equipment, and tools; placing and installing geogrids; hauling, placing and compacting select material fill; and all incidentals necessary to complete the work.

Pay Items:

Primary Geogrid Reinforcement .....	Square Meter
Secondary Geogrid Reinforcement.....	Square Meter
Borrow Material for Geogrid Reinforcement.....	Cubic Meter

**EMBANKMENT MONITORING:**

**(SPECIAL)**

The instrumentation will consist of 17 settlement gauges. Place the settlement gauges at the locations as shown in the plans or as directed by the Engineer.

**Settlement Gauge:**

Furnish and install Settlement Gauges as shown in the plans. Place the base on a level surface near the natural ground as shown in the plans. Provide threaded 50 mm (minimum) diameter metal pipe meeting ASTM A53 Type F and having a black finish. Add pipe sections at threaded couplings as the embankment progresses. Maintain the top of the pipe be no less than 0.3 meter above the embankment surface and no higher than 1.8 meter. Make the exposed length of pipe conspicuous to avoid damage.

Compact fill around the gauge pipes and plates to the same density as the surrounding material. Restore or replace any settlement gauge pipe damaged or destroyed due to fault or negligence on the part of the Contractor at no additional cost to the Department. No additional payment will be made for compaction of fill around and over the settlement gauges or for interference with the Contractor's operations resulting from settlement gauge installations. Care shall be taken that the pipe remains plumb.

Method of Measurement:

The quantity of settlement gauges to be paid for will be the actual number of each of these items which have been incorporated into the completed and accepted work.

Basis of Payment:

The quantity of settlement gauges, measured as provided above, will be paid for at the contract unit price each for "Embankment Settlement Gauge". Such price and payment will be full compensation for all materials, labor, equipment and other necessary to complete the work satisfactorily.

Pay Item :     Embankment Settlement Gauge.....Each

**INSTALLATION OF VERTICAL WICK DRAINS AND DRAINAGE LAYER:**

**(SPECIAL)**

Description:

Furnish, place and install vertical wick drains, including augering, and Select Material, Class III in accordance with the details in the plans and as specified in the provisions, or as directed by the Engineer. Select Material, Class III is the same as Select Granular Material, Class III within this provision and plans.

Materials:

A. Wick Drain

The wick drains must be a prefabricated type composed of a drainage plastic core. The core must be fabricated with suitable drainage channels. The assembled drain shall be band-shaped with an aspect ratio (width divided by thickness) not exceeding 50, and it shall have a minimum equivalent diameter of 50 millimeter using the following definition of equivalent diameter:

$$d_w = (w+t)/2 \quad \text{where, } d_w = \text{diameter of a circular drain equivalent to the band shape}$$

w = width of a band shaped drain  
t = thickness of a band shaped drain

The plastic core must be wrapped in a filter of a non-woven polyester material. The filter fabric material used must meet the following minimum requirements:

<u>Item</u>	<u>ASTM Standard</u>	<u>Min. Roll Value</u>
Grab Tensile Strength	D4632	400 N
Trapezoidal Tear	D4533	180 N
Puncture Strength	D3787	155 N
Mullen Burst	D3786	900 kPa
Permeability	D4491	0.25 mm/sec

Furnish to the Engineer a Type 2 Typical Certified Mill Test Report for the wick drain in accordance with Section 106-3 of the NCDOT Standard Specification. All wick drain materials shall, however, be subject to inspection, test or approval by the Engineer. At least four (4) weeks before construction of wick drains, provide a sample of 1.5 meter long wick drain to the Engineer for review and approval.

#### B. Select Material

Select Material, Class III, must meet the requirements of Section 1016 of the Standard Specifications.

#### Equipment:

Select the proper size and amount of equipment to provide the desired results, but provide the following basic items. The type of carrier to be used will depend on the desired installation force, but it must be equipped with a mandrel or sleeve of minimum cross sectional area not to exceed 65 square centimeters.

Submit to the Engineer for review and approval full details on all equipment proposed for drain installation at least two weeks before beginning work. Replace or supplement any equipment found unsatisfactory. All equipment approved for use will be on a trial basis. If after a short test section the equipment proves unsatisfactory, it must be removed, replaced or supplemented as deemed necessary to accomplish the desired results.

#### Installation of Wick Drains:

At least four weeks prior to the installation of wick drains, submit to the Engineer for his review and approval, details regarding the sequence of construction and method of installation. Approval by the Engineer of the sequence and method of installation will not necessarily constitute acceptance for the duration of the project. If, at any time, the Engineer considers that the method of installation is not satisfactory, the Contractor must alter his method and/or equipment as necessary to comply with the requirements.

If installation of wick drains through overlying layers and/or obstructions cannot be accomplished with the proper equipment, the Contractor will be permitted to use augering or other approved methods. Any holes augered must have a minimum diameter required to permit the mandrel or sleeve carrying the wick and wick anchorage to penetrate into the underlying soft soils. Penetration of more than 0.6 meter into the soft layer will not be allowed.

Install the wick drains after placement of the nonwoven fabric for soil separation and drainage layer. For nonwoven fabric for soil separation, see Nonwoven Fabric for Soil Separation special provision. The drainage layer must consist of Select Granular Material, Class III. Install wick drains at the designated locations using a mandrel or sleeve which completely encloses the wick drain, thereby protecting it from tears, cuts, and abrasions during installation. Provide the mandrel or sleeve with an anchor plate or similar arrangement at the bottom to prevent the soil from entering the bottom of the mandrel during installation of the drain, and to anchor the drain tip at the required depth at the time of mandrel withdrawal. Push the mandrel into the ground to the depth indicated on the plans unless otherwise directed by the Engineer. Retract the mandrel leaving the wick in place to function as a vertical drain. Cut the wick neatly at its upper end with 150 millimeter of wick material protruding above the drainage layer.

Splices or connections of the wick drain material must be done in a workmanlike manner to ensure the hydraulic continuity of the drain. One (1) splice per wick drain location is permitted. Overlap the jacket and core a minimum of 150 millimeter per splice. Form the splice by inserting the bottom side of the wick drain into the upper end to ensure continuous full flow. Use a minimum of ten (10) staples (4 on each side and 2 in the middle) to hold the splice.

Installed wicks must not deviate more than 25 millimeters per 300 millimeter from the vertical. Wicks that are out of their proper location by more than 150 millimeter, damaged in construction or improperly completed will be rejected by the Engineer.

Provide a suitable means of making a linear determination of the depth of the wick drain at any time during installation. Each wick drain length that is complete and in place will be recorded and used to determine total quantity of vertical wick drains for payment purposes.

Provide the necessary steps to protect the instrumentation devices. Any devices that are damaged or become unreliable must be replaced at no additional cost to the Department.

**Measurement and Payment:**

Wick drain will be measured and paid for at the contract unit price per linear meter for "Wick Drains" complete and in place. Payment will be full compensation for work required to install the wick drains, including any augering required and furnishing all labor, equipment, tools, and incidentals necessary to complete the work.

Augering for wick drains installation will be considered to be incidental to the cost of wick drains, and no separate measurement for payment will be made.

Select Material, Class III will be paid for as Select Granular Material, Class III unless the material is obtained from the same source as the borrow material and the contract includes a pay item for Borrow Excavation. When this occurs, Select Granular Material will be paid for as Borrow Excavation in accordance with Article 230-5 of the Standard Specifications and no payment for Select Granular Material, Class III will be made.

Select Granular Material, Class III will be measured and paid for in cubic meters. Select Granular Material will be measured by in place measurement in accordance with Article 230-5 of the Standard Specifications or by weighing material in trucks in accordance with Article 106-7 of the Standard Specifications as determined by the Engineer. When Select Granular Material is weighed in trucks, a unit weight of 21.2 kN/m<sup>3</sup> will be used to convert the weight of Select Granular Material to cubic meters. At the Engineer’s discretion, truck measurement in accordance with Article 230-5 of the Standard Specifications may be used in lieu of weighing material in trucks. The contract unit price for Select Granular Material, Class III or as described above will be full compensation for furnishing, hauling, handling, placing, compacting and maintaining Select Granular Material.

Payment will be made under:

Pay Items: Wick Drains ..... Meter  
Select Granular Material, Class III ..... Cubic Meter

**SURCHARGE PLACEMENT, MAINTENANCE AND REMOVAL (SPECIAL)**

Construct the embankment between Stations indicated on the plans, to a point 0.6 meter above finished graded roadway section in order to surcharge the embankment. Place the surcharge earth material 0.6 meter above finished graded roadway section or as directed by the Engineer to the limits as shown on the plans. Compact the surcharge material as normal roadway embankment.

Notify the Engineer when the waiting period is ready to begin.

Maintain the embankments at an elevation of 0.6 meter above finished graded roadway section during the waiting period.

Completed sections of roadway shall be restored to full surcharge after it has settled a maximum of 150 mm. Surcharge earth material required to maintain this elevation will be paid for at the contract unit price per cubic meter for “Borrow Excavation”, unless Unclassified Excavation was utilized for surcharge placement.

The surcharge earth material shall remain in place for eleven (11) months or as directed by the Engineer. Remove the surcharge earth material after the material has remained in place for the required waiting period.



The placement of the surcharge material above finished graded roadway section will be measured and paid for at the contract unit price per cubic meter for "Borrow Excavation", unless Unclassified Excavation was utilized for surcharge placement.

Removal of the surcharge earth material above graded roadway section will be measured and paid for at the contract unit price per cubic meter for "Unclassified Excavation".

**SELECT GRANULAR MATERIAL:**

(3-16-10)

M2 R80(Rev.)

Revise the *2006 Metric Standard Specifications* as follows:

Page 2-23, Delete Section 265 **SELECT GRANULAR MATERIAL** and replace it with the following:

**SECTION 265  
SELECT GRANULAR MATERIAL**

**265-1 Description**

Furnish and place select granular material in accordance with the contract or as directed by the Engineer.

**Materials**

Refer to Division 10 of the *Metric Standard Specifications*.

<b>Item</b>	<b>Section</b>
Select Material, Class II	1016
Select Material, Class III	1016

**265-2 Construction Methods**

Use Class II or III Select Material over fabric for soil stabilization and only Class III Select Material for backfill in water.

Place select granular material to 1 m above fabric and water level.

**265-3 Measurement and Payment**

*Select Granular Material* will be measured and paid for in cubic meters. When *Undercut Excavation* is in accordance with Section 226 (Comprehensive Grading) of the *Metric Standard Specifications* and the Engineer requires undercut to be backfilled with select granular material, the second sentence of the sixth paragraph of Article 226-3 will not apply, as payment for the backfill will be made as specified in this provision.

Select granular material will be measured by in place measurement in accordance with Article 230-5 of the *Metric Standard Specifications* or by weighing material in trucks in accordance with Article 106-7 of the *Metric Standard Specifications* as determined by the Engineer. When select granular material is weighed in trucks, a unit weight of 21.2 kN/m<sup>3</sup> will be used to convert the weight of select granular material to cubic meters. At the Engineer's discretion, truck measurement in accordance with Article 230-5 of the *Metric Standard Specifications* may be used in lieu of weighing material in trucks.

The contract unit prices for *Select Granular Material* as described above will be full compensation for furnishing, hauling, handling, placing, compacting and maintaining select granular material.

Payment will be made under:

<b>Pay Item</b>	<b>Pay Unit</b>
Select Granular Material	Cubic Meter

### **NONWOVEN FABRIC FOR SOIL SEPARATION**

**(SPECIAL)**

#### **Description:**

This work consists of furnishing and installing nonwoven fabric for soil separation in accordance with this provision or as directed by the Engineer. The work shall include maintaining the fabric in the required configuration until completion and acceptance of overlying work items. Place fabric at the locations shown in the plans or as directed by the Engineer.

#### **Material:**

The nonwoven fabric for soil separation shall be made of polyester or polypropylene. The fabric shall meet the following physical requirements:

<b><u>Fabric Properties</u></b>	<b><u>Test Method</u></b>	<b><u>Requirements</u></b>
Grab Tensile Strength	ASTM D-4632	530 N
Elongation	ASTM D-4632	50% Max.
Puncture Strength	ASTM D-4833	310 N
Trapezoidal Tear	ASTM D-4533	220 N
Max. Apparent Opening Size	ASTM D-4751	# 70 US Sieve
Permittivity	ASTM D-4491	1.80 sec <sup>-1</sup>
UV Resistance, % Retained	ASTM D-4355	70 %

Any sampled roll must meet or exceed the minimum values in this table. Furnish a Type 2 Typical Certified Mill Test Report for the nonwoven fabric for soil separation in accordance with Section 106-3 of the NCDOT Standard Specifications; however, the material shall be subject to inspection, test, or approval by the Engineer. Four weeks prior to construction of nonwoven fabric for soil separation, provide a sample of one (1) meter by one (1) meter nonwoven fabric for soil separation to the Engineer for review and approval.

**Construction Methods:**

Install nonwoven fabric for soil separation in accordance with Article 270-3 of the Standard Specifications.

**Method of Measurement:**

The quantity of fabric to be paid for will be the number of square meter of surface area of the ground on which the fabric has been acceptably placed. No separate measurement will be made of overlapping fabric for payment.

**Basis of Payment:**

The quantity of fabric, measured as provided above, will be paid for at the contract unit price per square meter for "Nonwoven Fabric for Soil Separation". Such price and payment will be full compensation for furnishing, placing, and all incidentals necessary to complete the work.

Pay Item: Nonwoven Fabric for Soil Separation..... Square Meter

**EMBANKMENT AND MSE WALL INSTRUMENTATION:**

**DESCRIPTION**

The instrumentation will consist of eight (8) vibrating wire (vw) piezometers, six (6) high pressure cells, three (3) low pressure cells, two (2) vertical inclinometer casings, one (1) complete horizontal inclinometer probe system, and five (5) horizontal inclinometer casings with pull cables. Provide a total of ten (10) horizontal inclinometer casing access boxes, two per casing. Provide instrumentation calibration data sheets for all piezometers, high and low pressure cells, and horizontal inclinometer probe.

Purchase and deliver to the Engineer instruments and materials for piezometer gauges, pressure cells, and vertical and horizontal inclinometer casings, and complete horizontal inclinometer probe system at least 20 working days prior to the start of wick drain installation. Before starting wick drain installation, conduct a preconstruction meeting to discuss the instrumentation installation. Schedule this meeting after instrumentation items have been delivered to the Engineer. The Resident Engineer, Geotechnical Operations Engineer, and Contractor will attend this preconstruction meeting.

The Engineer will install and monitor all instrumentation. The Engineer will retain ownership of all instruments and materials after completion of the project. Provide access and assistance to the Engineer in installing the instrumentation and casings as shown on the plans. Provide access to the Engineer to monitor instrumentation during the entire duration of the project. Access and assistance is incidental to the cost of the *Embankment and MSE Wall Instrumentation* pay items.

Supply vw piezometers, high and low pressure cells, vertical and horizontal inclinometer casings, and complete horizontal inclinometer probe system from one of the companies listed below or equal as approved by the Engineer. Contact Information for Durham Geo Slope Indicator and Geokon is listed below

- 1) Durham Geo Slope Indicator  
2175 West Park Court  
Stone Mountain, GA 30087  
Tel (770) 465-7557  
Fax (770) 465-7447  
[www.durhamgeo.com](http://www.durhamgeo.com)  
[www.slopeindicator.com](http://www.slopeindicator.com)
- 2) Geokon  
48 Spencer Street  
Lebanon, NH 03766  
Tel (603) 448-1562  
Fax (603) 448-3216  
[www.geokon.com](http://www.geokon.com)

## **MATERIALS AND CONSTRUCTION**

### **1. VW Piezometers:**

Furnish 8 vibrating wire piezometers with signal cables from the list of specified manufactures and models as listed below. All vw piezometer signal cables shall be a minimum of 80 meters long.

- 1) Durham Geo Slope Indicator Model 52611020 with Durham Geo Slope Indicator Polyurethane Signal Cable Model 50613524.
- 2) Geokon Model 4500S with a pressure range of 350 kPa and Geokon signal cable.
- 3) Equivalent type and quality of vw piezometer and signal cable as approved by the Engineer.

VW Piezometers will be installed by the Engineer prior to construction of the embankment and surcharge. Provide installation assistance to the Engineer in the form of excavation and backfilling of a 300 mm by 300 mm trench extending to outside the slope stake limits as shown on the Embankment and MSE Wall Instrumentation plan sheet for each of the four vw

piezometer locations. Two vw piezometers will be installed at each of the 4 piezometer locations shown on the Embankment and MSE Wall Instrumentation plan sheet. Piezometer instrumentation cables can be installed in the Horizontal Inclinator Casing trench. The Engineer will read vw piezometers over the duration of the project.

## 2. High Pressure Cells

Furnish 6 high pressure cells with signal cables from the list of specified manufactures and models as listed below. All high pressure cell signal cables shall be a minimum of 30 meters long.

- 1) Durham Geo Slope Indicator Model 52608240 with Durham Geo Slope Indicator Polyurethane Signal Cable Model 50613524.
- 2) Geokon Model 4800 with a pressure range of 2 MPa with Geokon signal cable or approved equal high pressure cell and cable.
- 3) Equivalent type and quality of high pressure cell and signal cable as approved by the engineer.

High pressure cells will be installed by the Engineer after the first 200 mm lift of Select Material, Class IV has been placed for the Load Transfer Platform as shown on the Embankment and MSE Wall Instrumentation plan sheet. Provide installation assistance to the Engineer in the form of excavation and backfilling of a 300 mm by 300 mm instrumentation cable trench extending to outside the limits of the MSE walls for each of the three pressure cell instrumentation clusters. The Engineer will read pressure cells over the duration of the project.

## 3. Low Pressure Cells

Furnish 3 low pressure cells with signal cables from the list of specified manufactures and models as listed below. All low pressure cell signal cables shall be a minimum of 30 meters long.

- 1) Durham Geo Slope Indicator Model 52608220 with Durham Geo Slope Indicator Polyurethane Signal Cable Model 50613524.
- 2) Geokon Model 4800 with a pressure range of 350 kPa with Geokon signal cable.
- 3) Equivalent type and quality of low pressure cell and signal cable as approved by the engineer.

Low pressure cells will be installed by the Engineer after the first 200 mm lift of Select Material, Class IV has been placed for the Load Transfer Platform as shown on the Embankment and MSE Wall Instrumentation plan sheet. Provide installation assistance to the Engineer in the form of excavation and backfilling of a 300 mm by 300 mm instrumentation cable trench extending to

outside the limits of the MSE walls for each of the three pressure cell instrumentation clusters. The Engineer will read pressure cells over the duration of the project.

#### 4. Horizontal Inclinometer Casings with Pull Cables.

Furnish sufficient horizontal inclinometer casings and pull cable lengths to extend from one meter outside the left temporary surcharge slope stake line or MSE wall face to one meter outside the right temporary surcharge slope stake line or MSE wall face as shown on the Embankment and MSE Wall Instrumentation plan sheet. Pull cables shall be 4 meters longer than the inclinometer casing in which the pull cable will be installed. Pull cables shall be 3.2 mm stranded stainless steel cable with stainless steel saddle clamps at both ends. Horizontal inclinometer casings shall be from the list of specified manufactures and models as shown below.

- 1) Durham Geo Slope Indicator QC Casing with an external diameter of 85 mm.
- 2) Geokon Model 6400 Glue-Snap ABS Inclinometer Casing with an external diameter of 85 mm.
- 3) Equivalent type and quality of Horizontal Inclinometer Casings as approved by the Engineer.

Horizontal inclinometer casings and pull cables will be installed by the Engineer prior to construction of the embankment and surcharge for Casings No. 1 through 4. Casing No. 5 will be installed by the Engineer after the first 200 mm lift of Select Material, Class IV has been placed for the Load Transfer Platform as shown on the Embankment and MSE Wall Instrumentation plan sheet. Provide installation assistance to the Engineer in the form of excavation and backfilling of a 300 mm by 300 mm trench extending to outside the temporary surcharge slope stake limits as shown on the plans or limits of the MSE wall face for each of the five casing locations. The Engineer will read horizontal inclinometer casings over the duration of the project.

#### 5. Complete Horizontal Inclinometer Probe System

Furnish a complete English Unit Horizontal Inclinometer Probe system from one of the vendors listed below. A complete Horizontal Inclinometer Probe system includes the probe, probe carrying case, data readout box, 300 foot long control cable, 300 foot minimum capacity control cable reel.

- 1) Complete English Unit Horizontal Inclinometer Probe system from Durham Geo Slope Indicator shall consist of:
  - a) Horizontal Inclinometer Probe, English Units, Model 50303510
  - b) Probe carrying case
  - c) Roadout Box, Digitilt DataMate II, Model 50310900
  - d) Control Cable Complete, Model 50601004
  - e) 360' Capacity Storage Reel, Model 50502110

- 2) Complete English Unit Horizontal Inclinometer Probe system from Geokon shall consist of:
  - a) Horizontal Inclinometer Probe, English Units, Model 6015
  - b) Probe carrying case, Model 6000-3
  - c) Readout Box, Model GK-603
  - d) Control Cable, Model 6000-4 with a minimum length of 300 foot
  - e) 300 foot capacity Cable Reel, Model 6000-5
- 3) Equivalent type and quality of Complete English Unit Horizontal Inclinometer Probe System as approved by the Engineer.

Reading Horizontal Inclinometer Probe System will be performed by the Engineer. No assistance other than site access is required.

#### 6. Horizontal Casing Access Boxes

Furnish and install an access box at each end of the horizontal casing as shown on the Embankment and MSE Wall Instrumentation plan sheet. The box must be sturdy and durable to the Engineer's satisfaction and meet all OSHA requirements. Install the boxes prior to beginning construction of the embankment and MSE walls. The contractor shall maintain the horizontal inclinometer access boxes for the duration of the project. Dewater the casing access boxes as necessary to allow the Engineer to use the Horizontal Inclinometer Probe System within the Horizontal Casing Access Boxes.

#### 7. Vertical Inclinometer Casings

Furnish 20 meters of vertical inclinometer casing for each of the two vertical inclinometer casing locations for a total length of 40 meters. Provide one bottom cap per vertical inclinometer casing location for a total of two bottom caps. Vertical inclinometer casings and bottom caps shall be from the list of specified manufactures and models as shown below.

- 1) Durham Geo Slope Indicator QC Casing with an external diameter of 85 mm.
- 2) Geokon Model 6400 Glue-Snap ABS Inclinometer Casing with an external diameter of 85 mm.
- 3) Equivalent type and quality of Vertical Inclinometer Casings as approved by the engineer.

The Engineer will install the vertical inclinometer casings prior to construction of the MSE walls and will read the vertical inclinometer casings over the duration of the project.

## MEASUREMENT AND PAYMENT

The quantity of "*VW Piezometer*" to be measured for payment will be the actual number of each of these items which have been delivered to the Engineer for use. The quantity of piezometers, measured as provided above, will be paid for at the contract unit price per each for "*VW Piezometer*". Such price and payment will be full compensation for all materials, instrumentation cables, labor, equipment and other necessary to complete the work satisfactorily.

The quantity of "*High Pressure Cell*" to be measured for payment will be the actual number of each of these items which have been delivered to the Engineer for use. The quantity of high pressure cells, measured as provided above, will be paid for at the contract unit price per each for "*High Pressure Cell*". Such price and payment will be full compensation for all materials, instrumentation cables, labor, equipment and other necessary to complete the work satisfactorily.

The quantity of "*Low Pressure Cell*" to be measured for payment will be the actual number of each of these items which have been delivered to the Engineer for use. The quantity of low pressure cells, measured as provided above, will be paid for at the contract unit price per each for "*Low Pressure Cell*". Such price and payment will be full compensation for all materials, instrumentation cables, labor, equipment and other necessary to complete the work satisfactorily.

The quantity of "*Horizontal Inclinator Casing with Pull Cable*" to be measured for payment will be the linear meters of this item which have been delivered to the Engineer for use. The quantity of horizontal inclinometer casing with pull cable, measured as provided above, will be paid for at the contract unit price per meter for "*Horizontal Inclinator Casing with Pull Cable*". Such price and payment will be full compensation for all materials, labor, equipment and other necessary to complete the work satisfactorily. The cost of the pull cables and saddle clamps is incidental to the cost of the pay item for "*Horizontal Inclinator Casing with Pull Cable*".

The quantity of "*Complete Horizontal Inclinator Probe System*" to be measured for payment will be the actual number of each of these items which have been delivered to the Engineer for use. The quantity of horizontal inclinometer probe systems, measured as provided above, will be paid for at the contract unit price per each for "*Complete Horizontal Inclinator Probe System*". Such price and payment will be full compensation for all materials including the probe, probe carrying case, data readout box, 300 foot long control cable, 300 foot minimum capacity control cable reel, labor, equipment and other necessary to complete the work satisfactorily.

The quantity of "*Horizontal Casing Access Box*" to be measured for payment will be the actual number of each of these items which have been incorporated into the completed and accepted work. The quantity of casing access boxes, measured as provided above, will be paid for at the contract unit price per each for "*Horizontal Casing Access Box*". Such price and payment will be full compensation for all materials, labor, equipment and other necessary to complete the work satisfactorily.



The quantity of "Vertical Inclinometer Casing" to be measured for payment will be the linear meters of this item which have been delivered to the Engineer for use. The quantity of vertical inclinometer casing, measured as provided above, will be paid for at the contract unit price per meter for "Vertical Inclinometer Casing". Such price and payment will be full compensation for all materials, bottom caps, labor, equipment and other necessary to complete the work satisfactorily.

Payment will be made under:

VW Piezometer.....	Each
High Pressure Cell.....	Each
Low Pressure Cell.....	Each
Horizontal Inclinometer Casing with Pull Cable.....	Linear Meter
Complete Horizontal Inclinometer Probe System.....	Each
Horizontal Casing Access Box.....	Each
Vertical Inclinometer Casing.....	Linear Meter

**WELDED STEEL PIPE:**

10-20-09

M3 R25

Revise the 2006 Metric Standard Specifications as follows:

Page 3-9, Article 330-4 Measurement and Payment, replace the phrase mm Welded Steel Pipe in Soil with mm Welded Steel Pipe, mm Thick, Grade in Soil in each place shown. Replace the phrase mm Welded Steel Pipe Not in Soil with the phrase mm Welded Steel Pipe, mm Thick Grade Not in Soil in each place shown.

Payment will be made under:

<b>Pay Item</b>	<b>Pay Unit</b>
<u>mm Welded Steel Pipe, mm Thick Grade in Soil</u>	Linear Meter
<u>mm Welded Steel Pipe, mm Thick Grade Not in Soil</u>	Linear Meter

**FLOWABLE FILL:**

(9-17-02) (Rev 8-21-07)

M3 R30

**Description**

This work consists of all work necessary to place flowable fill in accordance with these provisions, the plans, and as directed.

**Materials**

Provide flowable fill material in accordance with Article 340-2 of the 2006 Metric Standard Specifications.

### Construction Methods

Discharge flowable fill material directly from the truck into the space to be filled, or by other approved methods. The mix may be placed full depth or in lifts as site conditions dictate. The Contractor shall provide a method to plug the ends of the existing pipe in order to contain the flowable fill.

### Measurement and Payment

At locations where flowable fill is called for on the plans and a pay item for flowable fill is included in the contract, *flowable fill* will be measured in cubic meters and paid for as the actual number of cubic meters that have been satisfactorily placed and accepted. Such price and payment will be full compensation for all work covered by this provision including but not limited to the mix design, furnishing, hauling, placing and containing the flowable fill.

Payment will be made under:

Pay Item	Pay Unit
Flowable Fill	Cubic Meter

### PIPE ALTERNATES:

(7-18-06) (Rev 4-17-07)

M3 R36

### Description

The Contractor may substitute Aluminized Corrugated Steel Pipe, Type IR or HDPE Pipe, Type S or Type D up to 1219 mm in diameter in lieu of concrete pipe in accordance with the following requirements.

### Material

Item	Section
HDPE Pipe, Type S or D	1032-10
Aluminized Corrugated Steel Pipe, Type IR	1032-3(A)(7)

Aluminized Corrugated Steel Pipe will not be permitted in counties listed in Article 310-2 of the *2006 Metric Standard Specifications*.

### Construction Methods

Aluminized Corrugated Steel Pipe Culverts and HDPE Pipe Culverts shall be installed in accordance with the requirements of Section 300 of the *2006 Metric Standard Specifications* for Method A, except that the minimum cover shall be at least 300 mm. Aluminized Corrugated Steel Pipe Culvert and HDPE Pipe Culvert will not be permitted for use under travelways, including curb and gutter.

**Measurement and Payment**

\_\_\_\_\_ mm *Aluminized Corrugated Steel Pipe Culvert* to be paid for will be the actual number of linear meters installed and accepted. Measurement will be in accordance with Section 310-6 of the *2006 Metric Standard Specifications*.

\_\_\_\_\_ mm *HDPE Pipe Culvert* to be paid for will be the actual number of linear meters installed and accepted. Measurement will be in accordance with Section 310-6 of the *2006 Metric Standard Specifications*.

Payment will be made under:

**Pay Item**

\_\_\_\_\_ mm Aluminized Corrugated Steel Pipe Culverts, \_\_\_\_\_ mm Thick  
 \_\_\_\_\_ mm HDPE Pipe Culverts

**Pay Unit**

Linear Meter  
 Linear Meter

**PIPE INSTALLATION:**

(10-20-09)(Rev 01-18-11)

M3 R40A

Revise the *Metric Standard Specifications for Roads and Structures* as follows:

Replace Section 300 with the following:

**SECTION 300  
 PIPE INSTALLATION**

**300-1 DESCRIPTION**

Excavate, undercut, provide material, condition foundation, lay pipe, joint and couple pipe sections, and furnish and place all backfill material as necessary to install the various types of pipe culverts and fittings required to complete the project.

Install pipe in accordance with the detail in the plans.

Do not waste excavation unless permitted. Use suitable excavated material as backfill; or in the formation of embankments, subgrades, and shoulders; or as otherwise directed. Furnish disposal areas for the unsuitable material. The Engineer will identify excavated materials that are unsuitable.

Where traffic is to be maintained, install pipe in sections so that half the width of the roadway is available to traffic.

**300-2 MATERIALS**

Refer to Division 10:

<b>Item</b>	<b>Section</b>
Flowable Fill	1000
Select Materials	1016
Joint Materials	1032-9(G)
Engineering Fabrics	1056

Provide foundation conditioning material meeting the requirements of Article 1016-3 for Class V or VI Select Material as shown in the contract documents.

Provide bedding material meeting the requirements of Article 1016-3 for Class II (Type 1 only) or Class III Select Material as shown in contract documents.

Provide backfill material meeting the requirements of Article 1016-3 for Class II (Type 1 for Flexible Pipe) or Class III Select Material as shown in the contract documents.

Provide filter fabric meeting the requirements of Article 1056-2 for any type of engineering fabric.

Provide foundation conditioning fabric meeting the requirements of Article 1056-2 for Type 2 Engineering Fabric.

Do not use corrugated steel pipe in the following counties:

Beaufort, Bertie, Bladen, Brunswick, Camden, Carteret, Chowan, Columbus, Craven, Currituck, Dare, Gates, Hertford, Hyde, Jones, Martin, New Hanover, Onslow, Pamlico, Pasquotank, Pender, Perquimans, Tyrrell, and Washington.

**300-3 UNLOADING AND HANDLING**

Unload and handle pipe with reasonable care. Do not roll or drag metal pipe or plates over gravel or rock during handling. Take necessary precautions to ensure the method used in lifting or placing the pipe does not induce stress fatigue in the pipe. Use a lifting device that uniformly distributes the weight of the pipe along its axis or circumference. Repair minor damage to pipe when permitted. Remove pipe from the project that is severely damaged or is rejected as being unfit for use. Undamaged portions of a joint or section may be used where partial lengths are required.

**300-4 PREPARATION OF PIPE FOUNDATION**

Prepare the pipe foundation in accordance with the applicable method as shown in the contract documents, true to line and grade, and uniformly firm.

Camber invert grade an amount sufficient to prevent the development of sag or back slope in the flow line. The Contractor shall determine the amount of camber required and submit to the Engineer for approval.

Where material is found to be of poor supporting value or of rock and when the Engineer cannot make adjustment in the location of the pipe, undercut existing foundation material within the limits established on the plans. Backfill the undercut with foundation conditioning material. Encapsulate the foundation conditioning material with foundation conditioning fabric prior to placing bedding material. Overlap all transverse and longitudinal joints in the fabric at least 450 mm.

Maintain the pipe foundation in a dry condition.

### 300-5 INVERT ELEVATIONS

The proposed pipe culvert invert elevations shown on the Drainage Summary Sheets are based upon information available when the plans were prepared. If proposed invert elevations are adjusted during construction based upon actual conditions encountered, no claim for an extension of time for any reason resulting from this information will be allowed.

When a pipe culvert is to be installed in a trench and the average actual elevation of the pipe between drainage structures deviates from the average proposed elevation shown on the Drainage Summary Sheets by more than 0.3 m a pay adjustment will be made as follows:

$$\text{Pay Adjustment (per linear meter)} = [(APE - AAE) \pm 0.3 \text{ meter}] (0.15 \times \text{CUP})$$

Where: CUP = Contract Unit Price of Pipe Culvert

$$AAE = \text{Average Actual Elevation} \quad \frac{(\text{Actual Inlet elev.} + \text{Actual Outlet elev.})}{2}$$

$$APE = \text{Average Plan Elevation} \quad \frac{(\text{Plan Inlet elev.} + \text{Plan Outlet elev.})}{2}$$

When the actual location of a pipe culvert is changed from the location shown on the plans, the Engineer will make a pay adjustment deemed warranted based upon the relation of the pipe culvert as shown on the plans to the finished roadway and the relation of the pipe culvert as constructed to the finished roadway.

The top elevation column on the drainage summary sheet indicates the flow elevation at the top of structures intended to collect surface water.

The top elevation column on drainage structures not intended to collect surface water indicates the elevation at the top of the cover.

### 300 -6 LAYING PIPE

The Department reserves the right to perform forensic testing on any installed pipe.

**(A) Rigid Pipe**

Concrete and welded steel pipe will be considered rigid pipe. Lay pipe on prepared foundation, bell or groove end upgrade with the spigot or tongue fully inserted. Check each joint for alignment and grade as the work proceeds.

Use flexible plastic joint material except when material of another type is specified in the contract documents. Joint material of another type may be used when permitted.

Repair lift holes in concrete pipe, if present. Thoroughly clean and soak the lift hole and completely fill the void with an approved non-shrink grout. Submit alternate details for repairing lift holes to the engineer for review and approval.

For all pipes 1050 mm in diameter and larger, wrap filter fabric around all pipe joints. Extend fabric at least 300 mm beyond each side of the joint. Secure fabric against the outside of the pipe by methods approved by the Engineer.

**(B) Flexible Pipe (Except Structural Plate Pipe)**

Corrugated steel, corrugated aluminum, corrugated polyethylene (HDPE), and polyvinylchloride (PVC) pipe will be considered flexible pipe. Place flexible pipe carefully on the prepared foundation starting at the downstream end with the inside circumferential laps pointing downstream and with the longitudinal laps at the side or quarter points.

Handle coated corrugated steel pipe with special care to avoid damage to coatings.

Join pipe sections with coupling band, fully bolted and properly sealed. Provide coupling bands for annular and helical corrugated metal pipe with circumferential and longitudinal strength sufficient to preserve the alignment, prevent separation of the sections, and prevent backfill infiltration. Match-mark all pipe 1500 mm or larger in diameter at the plant for proper installation on the project.

At locations indicated in the plans, corrugated steel pipe sections shall be jointed together with rod and lug coupling bands, fully bolted. Sleeve gaskets shall be used in conjunction with rod and lug couplings and the joints properly sealed. Coupling bands shall provide circumferential and longitudinal strength sufficient to preserve the alignment, prevent separation of the sections and prevent infiltration of backfill material.

**300-7 BEDDING AND BACKFILLING**

Loosely place bedding material, in a uniform layer, a depth equal to the inside diameter of the pipe divided by 6 or 150 mm, whichever is greater. Leave bedding material directly beneath the pipe uncompacted and allow pipe seating and backfill to accomplish compaction. Excavate recesses to receive the bells where bells and spigot type pipe is used.

Place fill around the pipe in accordance with the applicable method shown on the plans in layers not to exceed 150 mm loose unless otherwise permitted. Compact to the density required by Subarticle 235-4(C). Approval of the backfill material is required prior to its use. Use select material as shown in the contract documents.

Take care during backfill and compaction operations to maintain alignment and prevent damage to the joints. Keep backfill free from stones, frozen lumps, chunks of highly plastic clay, or other objectionable material.

Grade and maintain all pipe backfill areas in such a condition that erosion or saturation will not damage the pipe foundation or backfill.

Excavatable flowable fill may be used for backfill when approved by the Engineer. When using excavatable flowable fill, ensure that the pipe is not displaced and does not float during backfill. Submit methods for supporting the pipe and material placement to the Engineer for review and approval.

Do not operate heavy equipment over any pipe until it has been properly backfilled with a minimum 1 m of cover. Place, maintain, and finally remove the required cover that is above the proposed finished grade at no cost to the Department. Remove and replace, at no cost to the Department, pipe that becomes misaligned, shows excessive settlement, or has been otherwise damaged by the Contractor's operations.

### **300-8 INSPECTION AND MAINTENANCE**

Prior to final acceptance, the Engineer will perform random video camera and or mandrel inspections to ensure proper jointing and that deformations do not exceed allowable limits. Replace pipes having cracks greater than 2.5 mm or deflections greater than 7.5 percent. Repair or replace pipes with cracks greater than 0.25 mm, exhibiting displacement across a crack, exhibiting bulges, creases, tears, spalls, or delamination. Maintain all pipe installations in a condition such that they will function continuously from the time the pipe is installed until the project is accepted.

### **300-9 MEASUREMENT AND PAYMENT**

#### **General**

No measurement will be made of any work covered by this section except as listed below. Removal and disposal of existing pavement is a part of the excavation for the new pipe culvert installation. Repair of the pavement will be made in accordance with Section 654.

## **Foundation Conditioning**

### **Using Local Material**

Undercut excavation is all excavation removed by undercutting below the bottom of the trench as staked. *Undercut Excavation* will be measured as the actual number of cubic meters of undercut excavation, measured in its original position and computed by the average end area method, that has been removed as called for in the contract and will be paid for at double the contract unit price for *Unclassified Excavation* as provided in Article 225-7.

Local material used for conditioning the foundation will be measured and paid for in accordance with Article 225-7 for *Unclassified Excavation* or in accordance with Article 230-5 for *Borrow Excavation* depending on the source of the material.

Local material used to replace pipe undercut excavation will be measured and paid for in accordance with Article 225-7 or Article 230-5.

### **Using Other Than Local Material**

No measurement and payment will be made for *Undercut Excavation*. The material used to replace pipe undercut excavation will be classified as foundation conditioning material.

*Foundation Conditioning Material, Minor Structures* will be measured and paid as the actual number of metric tons of this material weighed in trucks on certified platform scales or other certified weighing devices.

No direct payment will be paid for undercut excavation. Payment at the contract unit price for *Foundation Conditioning Material, Minor Structures* will be full compensation for all work of pipe undercut excavation.

### **Foundation Conditioning Fabric**

*Foundation Conditioning Fabric* will be measured and paid in square meters. The measurement will be based on the theoretical calculation using length of pipe installed and two times the standard trench width. No separate measurement will be made for overlapping fabric or the vertical fabric dimensions required to encapsulate the foundation conditioning material.

### **Bedding and Backfill - Select Material**

No measurement will be made for select bedding and backfill material required in the contract documents. The select bedding and backfill material will be included in the cost of the installed pipe.

Where unclassified excavation or borrow material meets the requirements for select bedding and backfill and is approved for use by the Engineer, no deductions will be made to these pay items to account for use in the pipe installation.



Payment will be made under:

<b>Pay Item</b>	<b>Pay Unit</b>
Foundation Conditioning Material, Minor Structures	Metric Ton
Foundation Conditioning Fabric	Square Meter

**REINFORCED CONCRETE PIPE DESIGN (CONTRACTOR DESIGN):**

(10-20-09)

SPI 3-06 (Rev.)

**DESCRIPTION**

This work consists of the design, manufacture and installation of reinforced concrete pipes in locations that require fill heights greater than 12 meters and less than or equal to 24 meters.

**Materials**

**(A) Design**

When the design of a reinforced concrete pipe is required in the contract plans, design the reinforced concrete pipe in accordance with the current edition of the AASHTO LRFD Bridge Design Specifications. Provide the diameter of pipe as indicated on the plans and manufacture the pipe in accordance with ASTM C 1417. Provide a reinforced concrete pipe that meets the requirements of Section 1032-9, Section 1077 and any other applicable parts of the Standard Specifications.

The design of the reinforced concrete pipe is the responsibility of the Contractor and is subject to review, comments and approval. Submit two sets of detailed plans for review. Include all details in the plans, including the size and spacing of the required reinforcement necessary to fabricate the reinforced concrete pipe. Include checked design calculations for the reinforced concrete pipe. Have a North Carolina Registered Professional Engineer seal the plans and design calculations. After the plans are reviewed and, if necessary, the corrections made, submit one set of reproducible tracings on 22" x 34" sheets to become part of the contract plans.

**(B) REINFORCED CONCRETE PIPE SECTIONS**

**(1) Class**

Reinforced concrete pipe sections manufactured in accordance with this Special Provision are designated by inside pipe diameter and design earth cover.

**(2) Design Criteria**

The design of the reinforced concrete pipe shall be in accordance with Article 12.10.4.2 "Direct Design Method" of the current edition of the AASHTO LRFD Bridge Design Specifications. The following assumptions shall be used in the design calculations:

NCDOT Criteria for Direct Design Method
Process and Material Factors, Radial Tension, $F_{rp}=1.0$ Shear Strength, $F_{vp}=1.0$
Design Concrete Strength - $f'_c$ 5,000 psi < $f'_c$ < 7,000 psi
Heger Pressure Distribution - Type 2 Installation Vertical Arching Factor = 1.40 Horizontal Arching Factor = 0.40
Soil Unit Weight = 120 lb/ft <sup>3</sup>
Depth of Fluid = Inside Pipe Diameter
Minimum Concrete Cover = 1.00"
Crack Control = 0.90 (maximum)

**(C) Joints**

Produce the reinforced concrete pipe sections with spigot and bell ends. Design and form the ends of the pipe section so, when the sections are laid together, they make a continuous line of pipe with a smooth interior free of appreciable irregularities in the flow line, and compatible with the permissible variations given in Standard Specifications and ASTM C 1417.

**(D) Manufacture**

In addition to the requirements of the *Standard Specifications* and ASTM C 1417, devices or holes are permitted in each pipe section for the purpose of handling and placement. Submit details of handling devices or holes for approval and do not cast any concrete until approval is granted. Remove all handling devices flush with concrete surfaces as directed. Fill holes in a neat and workmanlike manner with an approved non-metallic non-shrink grout, concrete or plug.

**MEASUREMENT AND PAYMENT**

\_\_\_ " R.C. Pipe Culvert (*Contractor Design*) will be measured and paid for in linear meters. Such price and payment will be full compensation for all work and will include, but not be limited to, furnishing all labor, materials, equipment and other incidentals necessary to complete this work.

Payment will be made under:

**Pay Item**

\_\_\_ " R.C. Pipe Culvert (*Contractor Design*)

**Pay Unit**

Linear Meter

**BRIDGE APPROACH FILLS:**

(10-19-10)

M4 R01

**Description**

Construct bridge approach fills in accordance with the contract. Bridge approach fills include bridge approach fills for sub regional tier bridges and reinforced bridge approach fills. Geotextiles include engineering fabrics and geomembranes.

**Materials**

Refer to Division 10 of the *2006 Metric Standard Specifications*:

<b>Item</b>	<b>Section</b>
Portland Cement Concrete, Class B	1000
Select Material	1016
Subsurface Drainage Materials	1044
Engineering Fabrics	1056

Use Class III or V Select Material for reinforced approach fills and only Class V Select Material (standard size no. 78M stone) for bridge approach fills for sub regional tier bridges. Provide polyvinyl chloride (PVC) plastic drainage pipes, fittings and outlet pipes for subsurface drainage materials for all bridge approach fills. For bridge approach fills for sub regional tier bridges, use Type 1 Engineering Fabric for filter fabric to encase no. 78M stone. For reinforced bridge approach fills, use Type 5 Engineering Fabric for woven fabrics and Type 2 Engineering Fabric and no. 78M stone for drains.

Load, transport, unload and store geomembranes such that they are kept clean and free of damage. Geomembranes with defects, flaws, deterioration or damage will be rejected. Do not unwrap geomembranes until just before installation and do not leave geomembranes exposed for more than 7 days before covering geomembranes with woven fabrics.

Use either polyvinyl chloride (PVC), high density polyethylene (HDPE) or linear low density polyethylene (LLDPE) geomembranes. For PVC geomembranes, provide grade PVC30 geomembranes meeting the requirements of ASTM D7176. For HDPE and LLDPE geomembranes, use geomembranes with a nominal thickness of 30 mils meeting the requirements of *Geosynthetic Research Institute Standard Specifications* GM13 or GM17, respectively.

**Construction Methods**

Excavate as necessary for bridge approach fills in accordance with the contract. Notify the Engineer when foundation excavation is complete. Do not place geomembranes or filter fabrics until obtaining approval of the excavation depth and foundation material.

Attach geomembranes or filter fabrics to back of end bent caps and wing walls with adhesives, tapes or other approved methods. Use wire staples as needed to hold filter fabrics in place until covered. Overlap adjacent fabrics a minimum of 450 mm such that overlaps are parallel to the roadway centerline. Glue or weld geomembrane seams to prevent leakage. Contact the Engineer when existing or future structures such as foundations, pavements, pipes, inlets or utilities will interfere with geotextiles.

For reinforced bridge approach fills, place woven fabrics within 50 mm of locations shown on the plans and in slight tension free of kinks, folds, wrinkles or creases. Place first layer of woven fabric directly on geomembranes with no void or material in between. Install woven fabrics with the machine direction (MD) parallel to the roadway centerline. The MD is the direction of the length or long dimension of the roll. Do not splice or overlap woven fabrics in the MD such that splices or overlaps are perpendicular to the roadway centerline. Install woven fabrics with the orientation, dimensions and number of layers shown on the plans. Wrap woven fabrics as shown on the plans or as directed by the Engineer.

For reinforced bridge approach fills, construct 0.3 m by 0.3 m drains consisting of 100 mm diameter perforated PVC pipes surrounded by no. 78M stone wrapped in type 2 fabric. For bridge approach fills for sub regional tier bridges, install 100 mm diameter perforated PVC drainage pipes as shown on the plans.

Firmly connect PVC pipes together as needed. Connect perforated pipes to outlet pipes near the back faces of wing walls. Provide drains with positive drainage towards outlets. Place pipe sleeves in or under wing walls for outlet pipes such that positive drainage is maintained. Use sleeves of sufficient strength to withstand wing wall loads.

Place select material in 200 to 250 mm thick lifts. Compact Class III Select Material in accordance with Subarticle 235-4(C) of the *2006 Metric Standard Specifications*. Do not displace or damage fabrics or drains when placing and compacting select material. End dumping directly on fabrics and drains is not permitted. Do not operate heavy equipment on woven fabrics or drains until they are covered with at least 200 mm of select material. Replace any damaged fabrics and drains to the satisfaction of the Engineer.

Use only hand operated compaction equipment for bridge approach fills for sub regional tier bridges and within 1 m of end bent cap back or wing walls for reinforced bridge approach fills. At a distance greater than 1 m for reinforced bridge approach fills, compact select material with at least 4 passes of an 7.3 – 9.1 metric ton vibratory roller. Smooth wheeled or rubber tired rollers are also acceptable for compacting select material. Do not use sheepsfoot, grid rollers or other types of compaction equipment with feet.

Use solvent cement for connecting outlet pipes and fittings such as wyes, tees and elbows. Provide connectors for outlet pipes and fittings that are watertight and suitable for gravity flow conditions. Cover open ends of outlet pipes with rodent screens as shown on the plans.

Connect drains to concrete pads or existing drainage structures at ends of outlet pipes as directed by the Engineer. Construct concrete pads and provide an Ordinary Surface Finish in accordance with Subarticle 825-6(B) of the 2006 Metric Standard Specifications.

**Measurement and Payment**

*Reinforced Bridge Approach Fill, Station \_\_\_\_\_* will be paid at the contract lump sum price. Such price and payment will be full compensation for all reinforced bridge approach fills at each bridge for excavating and furnishing, transporting and placing geotextiles, select material, drains, pipe sleeves and concrete pads, compacting select material, connecting pipes to existing drainage structures and providing any labor, tools, equipment and materials to complete the work.

*Bridge Approach Fill – Sub Regional Tier, Station \_\_\_\_\_* will be paid at the contract lump sum price. Such price and payment will be full compensation for all bridge approach fills at each sub regional tier bridge for excavating and furnishing, transporting and placing filter fabrics, no. 78M stone, drainage pipes, pipe sleeves and concrete pads, compacting no. 78M stone, connecting pipes to existing drainage structures and providing any labor, tools, equipment and materials to complete the work.

Payment will be made under:

<b>Pay Item</b>	<b>Pay Unit</b>
Reinforced Bridge Approach Fill, Station _____	Lump Sum
Bridge Approach Fill – Sub Regional Tier, Station _____	Lump Sum

**FINE GRADING SUBGRADE, SHOULDERS AND DITCHES:**

(7-21-09)

SP5 R01

Revise the *Standard Specifications* as follows:

**Page 5-1, Article 500-1 DESCRIPTION**, replace the first sentence with the following:

Perform the work covered by this section including but not limited to preparing, grading, shaping, manipulating moisture content, and compacting either an unstabilized or stabilized roadbed to a condition suitable for placement of base course, pavement, and shoulders.

**AGGREGATE BASE COURSE:**

12-19-06

M5 R03

Revise the 2006 *Metric Standard Specifications* as follows:

**Page 5-9, Article 520-5 Hauling and Placing Aggregate Base Material**, 6th paragraph, replace the first sentence with the following:

Base course that is in place on November 15 shall have been covered with a subsequent layer of pavement structure or with a sand seal. Base course that has been placed between November 16 and March 15 inclusive shall be covered within 7 calendar days with a subsequent layer of pavement structure or with a sand seal.

**#57 STONE:**

7-18-06

SPI0 -1(Rev.)

**Description**

The Contractor shall place #57 stone in the in accordance with the details in the plans and the following provision.

**Materials**

<b>Item</b>	<b>Section</b>
# 57 Stone	1005

**Construction Methods**

The stone shall be placed and compacted as directed by the Engineer.

**Measurement and Payment**

#57 stone will be measured and paid for in metric tons that are completed and accepted. The stone will be measured by being weighed in trucks on certified platform scales or other certified weighing devices. The price and payment will be full compensation for furnishing, hauling, placing, and all incidentals necessary to complete the work.

Payment will be made under:

<b>Pay Item</b>	<b>Pay Unit</b>
#57 Stone	Metric Ton

**RIPRAP ENERGY DISSIPATOR BASIN:**

(10-14-09)

SPI-10-06 (Rev.)

**Description**

This work consists of the construction and maintenance of an armored outlet structure located at culvert outlets or ditch termini.

**Materials**

Refer to Division 10 of the *Standard Specifications*.

<b>Item</b>	<b>Section</b>
Class I Riprap	Section 1042
Filter Fabric for Drainage, Type 2	Section 1056

### Construction Methods

Riprap energy dissipators shall be constructed in accordance with the detail shown in the plans or as directed. From the outlet, invert of a culvert or bottom of a ditch excavation will drop to a specified depth. Excavation will continue to widen through the dissipater. Riprap shall be placed along the banks and bottom of the dissipater and along the apron.

Excavate ditch in accordance with Section 240 of the *Standard Specifications*.

The quantity of energy dissipater material may be affected by site conditions during construction of the project. The quantity of materials may be increased, decreased, or eliminated at the direction of the Engineer. Such variations in quantity will not be considered as alterations in the details of construction or a change in the character of the work.

### Measurement and Payment

*Energy Dissipator Basin* will be paid for in units of each. Such price and payment will be full compensation for all work covered by this section, including, but not limited to furnishing and placing stone, filter fabric, materials, labor, tools, equipment, and incidentals necessary to complete the work.

Payment will be made under:

<b>Pay Item</b>	<b>Pay Unit</b>
Energy Dissipater Basin	Each

### ASPHALT PAVEMENTS - SUPERPAVE:

(7-18-06)(Rev 11-16-10)

M6 R01

Revise the *2006 Metric Standard Specifications* as follows:

**Page 6-2, Article 600-9 Measurement and Payment, delete the second paragraph.**

**Page 6-10, Subarticle 609-5(C)(2), Required Sampling and Testing Frequencies, first partial paragraph at the top of the page, delete last sentence and replace with the following:**

If the Engineer allows the mix to remain in place, payment will be made in accordance with Article 105-3.

**Page 6-10, Subarticle 609-5(C)(2), Quality Control Minimum Sampling and Testing Schedule, delete first paragraph and replace with the following:**

Sample and test the completed mixture from each mix design per plant per year at the following minimum frequency during mix production:

**Second paragraph, delete the fourth sentence and replace with the following:**

When daily production of each mix design exceeds 100 metric tons and a regularly scheduled full test series random sample location for that mix design does not occur during that day's production, perform at least one partial test series consisting of Items A and B in the schedule below.

**Page 6-10, Subarticle 609-5(C)(2)(c) Maximum Specific Gravity, add after (AASHTO T 209):**

or ASTM D 2041

**Page 6-11, Subarticle 609-5(C)(2)(e) Tensile Strength Ratio (TSR), add a heading before the first paragraph as follows:**

- (i) Option 1

**Insert the following immediately after the first paragraph:**

- (ii) Option 2

Mix sampled from truck at plant with one set of specimens prepared by the Contractor and then tested jointly by QA and QC at a mutually agreed upon lab site within the first 7 calendar days after beginning production of each new mix design.

**Second paragraph, delete and replace with the following:**

Test all TSR specimens required by either option noted above on either a recording test press or a test press that maintains the peak load reading after the specimen has broken.

**Subarticle 609-5(C)(3) Control Charts, delete the second sentence of the first paragraph and replace with the following:**

For mix incorporated into the project, record full test series data from all regularly scheduled random samples or directed samples that replace regularly scheduled random samples, on control charts the same day the test results are obtained.

**Page 6-12, Subarticle 609-5(C)(3) Control Charts, fourth paragraph on this page, delete the last sentence and replace with the following:**

Denote the moving average control limits with a dash green line and the individual test limits with a dash red line.

**Subarticle 609-5(C)(3)(a), (b) and (c), replace (a) (b) and (c) with the following:**

- (a) A change in the binder percentage, aggregate blend, or  $G_{mm}$  is made on the JMF, or,



- (b) When the Contractor elects to stop or is required to stop production after one or two moving average values, respectively, fall outside the moving average limits as outlined in Subarticle 609-5(C)(6) or,
- (c) If failure to stop production after two consecutive moving averages exceed the moving average limits occurs, but production does stop at a subsequent time, re-establish a new moving average beginning at the actual production stop point.

**Subarticle 609-5(C)(4) Control Limits, replace the first paragraph and the CONTROL LIMITS Table on page 6-13 with the following:**

The following are established as control limits for mix production. Apply the individual limits to the individual test results. Control limits for the moving average limits are based on a moving average of the last 4 data points. Apply all control limits to the applicable target source.

**CONTROL LIMITS**

Mix Control Criteria	Target Source	Moving Average Limit	Individual Limit
2.36 mm Sieve	JMF	±4.0 %	±8.0 %
0.075 mm Sieve	JMF	±1.5 %	±2.5 %
Binder Content	JMF	±0.3 %	±0.7 %
VTM @ $N_{des}$	JMF	±1.0 %	±2.0 %
VMA @ $N_{des}$	Min. Spec. Limit	Min Spec. Limit	-1.0%
$P_{0.075} / P_{be}$ Ratio	1.0	±0.4	±0.8
% $G_{mm}$ @ $N_{ini}$	Max. Spec. Limit	N/A	+2.0%
TSR	Min. Spec. Limit	N/A	- 15%

**Page 6-13, Subarticle 609-5(C)(5) Warning Bands, delete this subarticle in its entirety.**

**Pages 6-13 through 6-15, Subarticle 609-5(C)(6) Corrective Actions, delete the word "warning" and replace with the words "moving average".**

**Page 6-13, Subarticle 609-5(C)(6) Corrective Actions, first paragraph, first sentence, delete and replace with the following:**

Immediately notify the Engineer when moving averages exceed the moving average limits.

**Page 6-14, Subarticle 609-5(C)(6) Corrective Actions, second paragraph, delete and replace with the following:**

Failure to stop production when required due to an individual mix test not meeting the specified requirements will subject all mix from the stop point tonnage to the point when the next individual test is back on or within the moving average limits, or to the tonnage point when production is actually stopped, whichever occurs first, to being considered unacceptable.

**Fifth full paragraph, delete the first, second, and third sentence and replace with the following:**

Immediately notify the Engineer when any moving average value exceeds the moving average limit. If two consecutive moving average values for any one of the mix control criteria fall outside the moving average limits, cease production of that mix, immediately notify the Engineer of the stoppage, and make adjustments. The Contractor may elect to stop production after only one moving average value falls outside the moving average limits.

**Page 6-14, Subarticle 609-5(C)(6) Corrective Actions, eighth paragraph, delete and replace with the following:**

If the process adjustment improves the property in question such that the moving average after four additional tests is on or within the moving average limits, the Contractor may continue production with no reduction in payment.

**Page 6-14, delete the last paragraph and the first paragraphs on Page 6-15, including the Table for Payment for Mix Produced in the Warning Bands and substitute the following:**

If the adjustment does not improve the property in question such that the moving average after four additional individual tests is outside the moving average limits, the mix will be evaluated for acceptance in accordance with Article 105-3. Reduced payment for or removal of the mix in question will be applied starting from the plant sample tonnage at the stop point to the sample tonnage when the moving average is on or within the moving average limits. In addition, any mix that is obviously unacceptable will be rejected for use in the work.

**Page 6-15, Second full paragraph, delete and replace with the following:**

Failure to stop production and make adjustments when required due to two consecutive moving average values falling outside the moving average limits will subject all mix produced from the stop point tonnage to the tonnage point when the moving average is back on or within the moving average limits or to the tonnage point when production is actually stopped, whichever occurs first, to being considered unacceptable. Remove this material and replaced with materials that comply with the Specifications at no additional costs to the Department, unless otherwise approved. Payment will be made for the actual quantities of materials required to replace the removed quantities, not to exceed the original amounts.

**Page 6-16, Subarticle 609-5(D)(1) General, delete the last paragraph, and replace with the following:**

Perform the sampling and testing at the minimum test frequencies as specified above. Should the density testing frequency fail to meet the minimum frequency as specified above, all mix without the required density test representation will be considered unsatisfactory. If the Engineer allows the mix to remain in place, payment will be made in accordance with Article 105-3.

**Page 6-18, Subarticle 609-5(D)(4) Nuclear Gauge Density Procedures, third paragraph, insert the following as the second sentence:**

Determine the Daily Standard Count in the presence of the QA Roadway Technician or QA Nuclear Gauge Technician on days when a control strip is being placed.

**Page 6-18, Subarticle 609-5(D)(5) Limited Production Procedure, delete the last paragraph including (a), (b), (c) and substitute the following:**

Proceed on limited production when, for the same mix type and on the same contract, one of the following conditions occur (except as noted in the first paragraph below).

- (a) Two consecutive failing lots, except on resurfacing\*
- (b) Three consecutive failing lots on resurfacing\*
- (c) Two consecutive failing nuclear control strips.

\* Resurfacing is defined as the first new uniform layer placed on an existing pavement.

**Page 6-20, Article 609-6 QUALITY ASSURANCE, DENSITY QUALITY ASSURANCE, insert the following items after item (E):**

- (F) By retesting Quality Control core samples from control strips (either core or nuclear) at a frequency of 100% of the frequency required of the Contractor;
- (G) By observing the Contractor perform all standard counts of the Quality Control nuclear gauge prior to usage each nuclear density testing day; or
- (H) By any combination of the above

**Page 6-23 through Page 6-24, Subarticle 610-3(A) Mix Design-General, delete the fourth and fifth paragraphs and replace with the following:**

Reclaimed Asphalt Pavement (RAP) or Reclaimed Asphalt Shingles (RAS) may be incorporated into asphalt plant mixes in accordance with Article 1012-1 and the following applicable requirements.

Reclaimed asphalt pavement (RAP) may constitute up to 50% of the total material used in recycled mixtures, except for mix Type S 12.5D, Type S 9.5D, and mixtures containing reclaimed asphalt shingle material (RAS). Reclaimed asphalt shingle (RAS) material may constitute up to 6% by weight of total mixture for any mix. When both RAP and RAS are used, do not use a combined percentage of RAS and RAP greater than 20% by weight of total mixture, unless otherwise approved. When the percent of binder contributed from RAS or a combination of RAS and RAP exceeds 20% but not more than 30% of the total binder in the completed mix, the virgin binder PG grade shall be one grade below (both high and low temperature grade) the binder grade specified in Table 610-2 for the mix type, unless otherwise approved. When the percent of binder contributed from RAS or a combination of RAS and RAP exceeds 30% of the total binder in the completed mix, the Engineer will establish and approve the virgin binder PG

grade. Use approved methods to determine if any binder grade adjustments are necessary to achieve the performance grade for the specified mix type.

For Type S 12.5D and Type S 9.5D mixes, the maximum percentage of reclaimed asphalt material is limited to 20% and shall be produced using virgin asphalt binder grade PG 76-22. For all other recycled mix types, the virgin binder PG grade shall be as specified in Table 610-2A for the specified mix type.

When the percentage of RAP is greater than 20% but not more than 30% of the total mixture, use RAP meeting the requirements for processed or fractionated RAP in accordance with the requirements of Article 1012-1.

When the percentage of RAP is greater than 30% of the total mixture, use an approved stockpile of RAP in accordance with Subarticle 1012-1(C). Use approved test methods to determine if any binder grade adjustments are necessary to achieve the performance grade for the specified mix type. The Engineer will establish and approve the virgin asphalt binder grade to be used.



**Page 6-28, Insert the following immediately after Table 610-2:**

**TABLE 610-2A  
SUPERPAVE MIX DESIGN CRITERIA**

	Percentage of RAP in Mix		
	Category 1	Category 2	Category 3
Mix Type	% RAP ≤20%	20.1% ≤ %RAP ≤ 30.0%	%RAP > 30.0%
All A and B Level Mixes, I19.0C, B25.0C	PG 64 -22	PG 64 -22	TBD
S9.5C, S12.5C, I19.0D	PG 70 -22	PG 64-22	TBD
S 9.5D and S12.5D	PG 76-22	N/A	N/A

- Note: (1) Category 1 RAP has been processed to a maximum size of 50mm.  
 (2) Category 2 RAP has been processed to a maximum size of 25 mm by either crushing and or screening to reduce variability in the gradations.  
 (3) Category 3 RAP has been processed to a maximum size of 25 mm, fractionating the RAP into 2 or more sized stockpiles.

**Page 6-29, Table 610-3 delete and replace with the following:**

**TABLE 610-3  
ASPHALT PLACEMENT- MINIMUM TEMPERATURE REQUIREMENTS**

Asphalt Concrete Mix Type	Minimum Air Temperature	Minimum Surface Temperature
ACBC, Type B 25.0B, C, B 37.5C	2°C	2°C
ACIC, Type I 19.0B, C, D	2°C	2°C
ACSC, Type S 4.75A, SF 9.5A, S 9.5B	4°C	10°C*
ACSC, Type S 9.5C, S 12.5C	7°C	10°C
ACSC, Type S 9.5D, S 12.5D	10°C	10°C

\* 2°C if surface is soil or aggregate base for secondary road construction.

**Page 6-36, Article 610-8 SPREADING AND FINISHING, third full paragraph, replace the first sentence with the following:**

Use the 9 m minimum length mobile grade reference system or the non-contacting laser or sonar type ski *with at least four referencing stations mounted on the paver at a minimum length of 7.3 m* to control the longitudinal profile when placing the initial lanes and all adjacent lanes of all layers, including resurfacing and asphalt in-lays, unless otherwise specified or approved.

**Page 6-37, Article 610-8 SPREADING AND FINISHING, delete the fourth paragraph on page 6-37 and replace with the following:**

Use a Material Transfer Vehicle (MTV) when placing all asphalt concrete plant mix pavements which require the use of asphalt binder grade PG 76-22 and for all types of OGAFc, unless otherwise approved. Use a MTV for all surface mix regardless of binder grade placed on

Interstate facilities. Where required above, utilize the MTV when placing all full width travel lanes, collector lanes, ramps, and loops.

**Page 6-41, Article 610-13 DENSITY ACCEPTANCE, delete the second full paragraph and replace with the following:**

As an exception, when the first layer of mix is a surface course and is being placed directly on an unprimed aggregate or soil base, the layer will be included in the "Other" construction category.

**Page 6-41, Article 610-13 DENSITY ACCEPTANCE, delete the formula and description in the middle of the page and replace with the following:**

	PF	=	$100 - 10(D)^{1.465}$
Where:	PF	=	Pay Factor (computed to 0.1%)
	D	=	the deficiency of the lot average density, not to exceed 2.0%

**Page 6-44, Article 620-4 MEASUREMENT AND PAYMENT, sixth paragraph, delete the last sentence and seventh paragraph, delete the paragraph and replace with the following:**

The adjusted contract unit price will then be applied to the theoretical quantity of asphalt binder authorized for use in the plant mix placed during the partial payment period involved, except that where recycled plant mix is used, the adjusted unit price will be applied only to the theoretical number of tons of additional asphalt binder materials required by the job mix formula.

**Page 6-44, Article 620-4 MEASUREMENT AND PAYMENT, add the following pay item:**

<b>Pay Item</b>	<b>Pay Unit</b>
Asphalt Binder for Plant Mix, Grade PG 70-28	Metric Ton

**Page 6-49, Article 650-5 CONSTRUCTION REQUIREMENTS delete the seventh paragraph on page 6-49 beginning "Use a Material Transfer Vehicle (MTV)..." and replace with the following:**

Use a Material Transfer Vehicle (MTV) when placing all asphalt concrete plant mix pavements which require the use of asphalt binder grade PG 76-22 and for all types of OGAFC, unless otherwise approved. Use a MTV for all surface mix regardless of binder grade placed on Interstate facilities. Where required above, utilize the MTV when placing all full width travel lanes, collector lanes, ramps, and loops.

**Page 6-57, TABLE 660-1 MATERIAL APPLICATION RATES AND TEMPERATURES, add the following:**

Type of Coat	Grade of Asphalt	Asphalt Rate L/M <sup>2</sup>	Application Temperature °C	Aggregate Size	Aggregate Rate KG./ M <sup>2</sup> Total
Sand Seal	CRS-2 or CRS-2P	1.00-1.36	66-79	Blotting Sand	6-8

**Page 6-62, Subarticle 660-9(B) Asphalt Seal Coat, add the following as sub-item (5):**

(5) Sand Seal

Place the fully required amount of asphalt material in one application and immediately cover with the seal coat aggregate. Uniformly spread the fully required amount of aggregate in one application and correct all non-uniform areas prior to rolling.

Immediately after the aggregate has been uniformly spread, perform rolling.

When directed, broom excess aggregate material from the surface of the seal coat.

When the sand seal is to be constructed for temporary sealing purposes only and will not be used by traffic, other grades of asphalt material meeting the requirements of Articles 1020-6 and 1020-7 may be used in lieu of the grade of asphalt required by Table 660-1 when approved.

**Page 6-63, Article 661-1 DESCRIPTION, add the following as the 2nd paragraph:**

Provide and conduct the quality control and required testing for acceptance of the UBWC in accordance with *Quality Management System for Asphalt Pavements (OGAFC, PADL, and Ultra-Thin HMA Version)*, included in the contract.

**Page 6-63, Article 661-2 MATERIALS, add the following after Asphalt Binder, Grade 70-28:**

Item	Section
Asphalt Binder, Grade 76-22	1020
Reclaimed Asphalt Shingles	1012

**Page 6-65, Subarticle 661-2(E), Asphalt Binder For Plant Mix, Grade PG 70-28, rename as ASPHALT BINDER FOR PLANT MIX and add the following as the first paragraph:**

Use either PG 70-28 or PG 76-22 binder in the mix design. Where PG 76-22 is being used in the production of Ultra-thin, the grade of asphalt binder to be paid for will be PG 70-28, unless otherwise approved.



**Page 6-65, Subarticle 661-2(G) Composition of Mix, add the following as the third sentence of the first paragraph:**

The percent of asphalt binder contributed from the RAS shall not exceed 20% of the total binder in the completed mix.

**Page 6-66, Article 661-2(G) Composition of Mix, replace Table 661-4 and associated notes with the following:**

**TABLE 661-4 – MIXTURE DESIGN CRITERIA**

Standard Sieves		12.7 mm Type A	9.5 mm Type B	6.4 mm Type C
ASTM	mm	(% Passing by Weight)		
¾ inch	19.0	100		
½ inch	12.5	85 - 100	100	
3/8 inch	9.5	60 - 80	85 - 100	100
#4	4.75	28 - 38	28 - 44	40 - 55
#8	2.36	19 - 32	17 - 34	22 - 32
#16	1.18	15 - 23	13 - 23	15 - 25
#30	0.600	10 - 18	8 - 18	10 - 18
#50	0.300	8 - 13	6 - 13	8 - 13
#100	0.150	6 - 10	4 - 10	6 - 10
#200	0.075	4.0 - 7.0	3.0 - 7.0	4.0 - 7.0

Mix Design Criteria			
	12.7 mm Type A	9.5 mm Type B	6.4 mm Type C
Asphalt Content, %	4.6 - 5.6	4.6 - 5.8	5.0 - 5.8
Draindown Test, AASHTO T 305	0.1% max.		
Moisture Sensitivity, AASHTO T 283*	80% min.		
Application Rate, Kg/M <sup>2</sup>	49	38	27
Approximate Application Depth, mm	19.0	15.9	12.5
Asphalt PG Grade, AASHTO M 320	PG 70-28 or PG 76-22	PG 70-28 or PG 76-22	PG 70-28 or PG 76-22

NOTE: \*Specimens for T-283 testing are to be compacted using the SUPERPAVE gyratory compactor. The mixtures shall be compacted using 100 gyrations to achieve specimens approximately 95 mm in height. Use mixture and compaction temperatures recommended by the binder supplier.

**Page 6-66, Subarticle 661-3(A) Equipment, add the following as the first paragraph:**

Use asphalt mixing plants in accordance with Article 610-5 of the *Standard Specifications*.

**Page 6-68, Subarticle 661-3(C), Application of Ultra-thin Bonded Wearing Course, delete the first paragraph and add the following as the first and second paragraphs:**

Use only one asphalt binder PG grade for the entire project, unless the Engineer gives written approval.

Do not place Ultra-thin Bonded Wearing Course between October 31 and April 1, when the pavement surface temperature is less than 10°F or on a wet pavement. In addition, when PG 76-22 binder is used in the JMF, place the wearing course only when the road pavement surface temperature is 16°F or higher and the air temperature in the shade away from artificial heat is 16°F or higher.

**Page 10-33, Subarticle 1012-1(A) General, add the following at the end of the last paragraph, last sentence:**

or ultra-thin bonded wearing course.

**Page 10-34, Table 1012-1, delete the entries for OGAFC and add new entries for OGAFC and a row for UBWC with entries:**

Mix Type	Coarse Aggregate Angularity <sup>(b)</sup> ASTM D5821	Fine Aggregate Angularity % Minimum AASHTO T304 Method A	Sand Equivalent % Minimum AASHTO T176	Flat & Elongated 5:1 Ratio % Maximum ASTM D4791 Section 8.4
S 9.5 D	100/100	45	50	10
OGAFC	100/100	N/A	N/A	10
UBWC	100/85	40	45	10

**Delete Note (c) under the Table 1012-1 and replace with the following:**

(c) Does not apply to Mix Types SF 9.5A and S 9.5B.

**Page 10-34, Subarticle 1012-1(B)(6) Toughness (Resistance to Abrasion), add as the last sentence:**

The percentage loss for aggregate used in UBWC shall be no more than 35%.

**Page 10-35, Subarticle 1012-1(F) Reclaimed Asphalt Shingle Material (RAS), insert the following immediately following the first paragraph:**

(1) Mix Design RAS

Incorporate RAS from stockpiles that have been tested for uniformity of gradation and binder content prior to use in an asphalt mix design.

**(2) Mix Production RAS**

New Source RAS is defined as acceptable material which was not included in the stockpile when samples were taken for mix design purposes. Process new source RAS so that all materials will pass a 12.5 mm sieve prior to introduction into the plant mixer unit.

After a stockpile of processed RAS has been sampled and mix designs made from these samples, do not add new source RAS to the original stockpile without prior field testing to insure gradation and binder uniformity. Sample and test new source RAS before blending with the existing stockpile.

Store new source RAS in a separate stockpile until the material can be sampled and tested for comparison with the original recycled mix design data. New source RAS may also be placed against the existing stockpile in a linear manner provided it is sampled for mix design conformity prior to its use in the recycled mix.

RAS contamination including but not limited to excessive dirt, debris, clean stone, concrete will not be allowed.

Field approval of new source RAS will be based on the table below and volumetric mix properties on the mix with the new source RAS included. Provided these tolerances are met, volumetric properties of the new mix will then be performed. If all volumetric mix properties meet the mix design criteria for that mix type, the new source RAS may continue to be used.

If the gradation, binder content, or any of the volumetric mix properties are not within the allowable tolerances of the table below, do not use the new source RAS unless approved by the Engineer. The Contractor may elect to either not use the stockpile, to request an adjustment to the JMF, or to redesign the mix.

**NEW SOURCE RAS GRADATION and BINDER TOLERANCES  
(Apply Tolerances to Mix Design Data)**

<b>0-6% RAS</b>	
<b>P<sub>b</sub> %</b>	<b>±1.6%</b>
<b>Sieve Size (mm)</b>	<b>Tolerance</b>
9.5	±1
4.75	±5
2.36	±4
1.18	±4
0.300	±4
0.150	±4
0.075	±2.0

**Page 10-35 through 10-37, Subarticle 1012-1(G), delete this in its entirety and replace with the following:**

**(G) Reclaimed Asphalt Pavement (RAP)**

**(1) Mix Design RAP**

Incorporate RAP from stockpiles or other sources that have been tested for uniformity of gradation and binder content prior to use in an asphalt mix design. Use reclaimed asphalt pavement that meets all requirements specified for *one of* the following *two* classifications.

**(a) Millings**

Existing reclaimed asphalt pavement (RAP) that is removed from its original location by a milling process as specified in Section 607. Millings should be such that it has a uniform gradation and binder content and all materials will pass a 50 mm sieve prior to introduction into the plant mixer unit.

**(b) Processed RAP**

RAP that is processed in some manner (possibly by crushing and/or use of a blending method) to produce a uniform gradation and binder content in the RAP prior to use in a recycled mix. Process RAP so that all materials have a uniform gradation and binder content and will pass a 25 mm sieve prior to introduction into the plant mixer unit.

**(c) Fractionated RAP**

Fractionated RAP is defined as having two or more RAP stockpiles, where the RAP is divided into coarse and fine fractions. Grade RAP so that all materials will pass a 25 mm sieve. The coarse RAP stockpile shall only contain material retained on a 9.5 mm screen, unless otherwise approved. The fine RAP stockpile shall only contain material passing the 9.5 mm screen, unless otherwise approved. The Engineer may allow the Contractor to use an alternate to the 9.5 mm screen to fractionate the RAP. The maximum percentages of fractionated RAP may be comprised of coarse, fine, or the combination of both. Utilize a separate cold feed bin for each stockpile of fractionated RAP used.

**(d) Approved Stockpiled RAP**

Approved Stockpiled RAP is defined as fractionated RAP which has been isolated and tested for asphalt content, gradation, and asphalt binder characteristics with the intent to be used in mix designs with greater than 30% RAP materials. Fractionate the RAP in accordance with Subarticle 1012-1(G)(1)(c). Utilize a separate cold feed bin for each approved stockpile of RAP used.

Perform extraction tests at a rate of 1 per 1000 metric tons of RAP, with a minimum of 5 tests per stockpile to determine the asphalt content and gradation. Separate stockpiles of RAP material by fine and coarse fractions. Erect and maintain a sign satisfactory to the Engineer on each stockpile to identify the material. Assure that no deleterious material is allowed in any stockpile. The Engineer may reject by visual inspection any stockpiles that are not kept clean, separated, and free of foreign materials.

Submit requests for RAP stockpile approval to the Engineer with the following information at the time of the request:

- (1) Approximate tons of materials in stockpile
- (2) Name or Identification number for the stockpile
- (3) Asphalt binder content and gradation test results
- (4) Asphalt characteristics of the Stockpile.

For the Stockpiled RAP to be considered for approval, the gradation and asphalt content shall be uniform. Individual test results, when compared to the target, will be accepted if within the tolerances listed below:

**APPROVED STOCKPILED RAP GRADATION and BINDER TOLERANCES**  
(Apply Tolerances to Mix Design Data)

<b>P<sub>b</sub> %</b>	<b>±0.3%</b>
<b>Sieve Size (mm)</b>	<b>Percent Passing</b>
25.0	±5%
19.0	±5%
12.5	±5%
9.5	±5%
4.75	±5%
2.36	±4%
1.18	±4%
0.300	±4%
0.150	±4%
0.075	±1.5%

Note: If more than 20% of the individual sieves are out of the gradation tolerances, or if more than 20% of the asphalt binder content test results fall outside the appropriate tolerances, the RAP shall not be used in HMA unless the RAP representing the failing tests is removed from the stockpile.

Do not add additional material to any approved RAP stockpile, unless otherwise approved by the Engineer.

Maintain at the plant site a record system for all approved RAP stockpiles. Include at a minimum the following: Stockpile identification and a sketch of all stockpile areas at the plant site; all RAP test results (including asphalt content, gradation, and asphalt binder characteristics).

**(2) Mix Production RAP**

During mix production, use RAP that meets the criteria for one of the following categories:

**(a) Mix Design RAP**

RAP contained in the mix design stockpiles as described above may be used in all applicable JMFs. These stockpiles have been pretested; however, they are subject to required QC/QA testing in accordance with Subarticle 609-5(C)(2).

**(b) New Source RAP**

New Source RAP is defined as any acceptable material that was not included in the stockpile or other source when samples were taken for mix design purposes. Process new source RAP so that all materials have a uniform gradation and binder content and will pass a 50 mm sieve prior to introduction into the plant mixer unit.

After a stockpile of millings, processed RAP, or fractionated RAP has been sampled and mix designs made from these samples, do not add new source RAP to the original stockpile without prior field testing to insure gradation and binder uniformity. Sample and test new source RAP before blending with the existing stockpile.

Store new source RAP in a separate stockpile until the material can be sampled and tested for comparison with the original recycled mix design data. New source RAP may also be placed against the existing stockpile in a linear manner provided it is sampled for mix design conformity prior to its use in the recycled mix.

Unprocessed RAP is asphalt material that was not milled and/or has not been processed to obtain a uniform gradation and binder content and is not representative of the RAP used during the applicable mix design. Unprocessed RAP shall not be incorporated into any JMFs prior to processing. Different sources of unprocessed RAP may be stockpiled together provided it is generally free of contamination and will be processed prior to use in a recycled mix. RAP contamination in the form of excessive dirt, debris, clean stone, concrete, etc. will not be allowed. Incidental amounts of dirt, concrete, and clean stone may be acceptable. Unprocessed RAP may be processed and then classified as a new source RAP as described above.

Field approval of new source RAP will be based on Table 1012-2 below and volumetric mix properties on the mix with the new source RAP included. Provided the Table 1012-2 tolerances are met, volumetric properties of the new mix will then be performed. If all volumetric mix properties meet the mix design criteria for that mix type, the new source RAP may continue to be used.

If the gradation, binder content, or any of the volumetric mix properties are not within the allowable tolerances of Table 1012-2, do not use the new source RAP unless approved by the Engineer. The Contractor may elect to either not use the stockpile, to request an adjustment to the JMF, or to redesign the mix.

**TABLE 1012-2  
NEW SOURCE RAP GRADATION and BINDER TOLERANCES  
(Apply Tolerances to Mix Design Data)**

Mix Type	0-20% RAP			20 <sup>+</sup> -30 % RAP			30 <sup>+</sup> % RAP		
	Base	Inter.	Surf.	Base	Inter.	Surf.	Base	Inter.	Surf.
Sieve (mm)									
P <sub>b</sub> %	± 0.7%			± 0.4%			± 0.3%		
25.0	±10	-	-	±7	-	-	±5	-	-
19.0	±10	±10	-	±7	±7	-	±5	±5	-
12.5	-	±10	±10	-	±7	±7	-	±5	±5
9.5	-	-	±10	-	-	±7	-	-	±5
4.75	±10	-	±10	±7	-	±7	±5	-	±5
2.36	±8	±8	±8	±5	±5	±5	±4	±4	±4
1.18	±8	±8	±8	±5	±5	±5	±4	±4	±4
0.300	±8	±8	±8	±5	±5	±5	±4	±4	±4
0.150	-	-	±8	-	-	±5	-	-	±4
0.075	±4	±4	±4	±2	±2	±2	±1.5	±1.5	±1.5

**ASPHALT PAVEMENTS - WARM MIX ASPHALT SUPERPAVE:**

(1-18-11)

M6 R02

Warm Mix Asphalt (WMA) is defined as additives or processes that allow a reduction in the temperature at which asphalt mixtures are produced and placed.

Notify the Engineer at least 2 weeks before producing the WMA so the Engineer can arrange a pre-pave meeting. Discuss special testing requirements necessary for WMA at the pre-pave meeting. Include at the pre-pave meeting the Contractor's QC manager, Paving Superintendent, and manufacturer's representative for the WMA technology, the Department's Roadway Construction Engineer, Resident Engineer, State Pavement Construction Engineer, and Quality Assurance Supervisor.

Require a manufacturer's representative for the WMA technology used to be present on site at the plant during the initial production and on the roadway during the laydown of the warm mix asphalt.

The requirement for the manufacturer's representative to be present at the pre-pave meeting and on-site at the plant may be waived by the Engineer based on previous work experience with the specific WMA technology used.

If the use of WMA is suspended during production, and the Contractor begins using Hot Mix Asphalt (HMA), then the Contractor shall be required to use HMA for the remainder of the specific route or map unless otherwise approved by the Engineer.

Revise the *2006 Metric Standard Specifications* as follows:

**Page 6-7, Article 609-1 Description, insert the following as the second paragraph:**

Warm Mix Asphalt (WMA) is defined as additives or processes that allow a reduction in the temperature at which asphalt mixtures are produced and placed. Use WMA at the Contractor's option when shown in the contract.

**Page 6-7, Article 609-4 Field Verification of Mixture and Job Mix Formula Adjustments, second paragraph, insert the following immediately after the first sentence:**

When producing a WMA, perform field verification testing including Tensile Strength Ratio (TSR) testing in accordance with AASHTO T 283 as modified by the Department.

**Third paragraph, delete the third sentence and replace with the following:**

Verification is satisfactory for HMA when all volumetric properties except  $\%G_{mm}@N_{ini}$  are within the applicable mix design criteria and the gradation, binder content, and  $\%G_{mm}@N_{ini}$  are within the individual limits for the mix type being produced. Verification is satisfactory for WMA when all volumetric properties except  $\%G_{mm}@N_{ini}$  are within the applicable mix design criteria, the TSR meets the design criteria, and the gradation, binder content, and  $\%G_{mm}@N_{ini}$  are within the individual limits for the mix type being produced.

**Page 6-10, Subarticle 609-5(C)(2)(d) Bulk Specific Gravity of Compacted Specimens, add after (AASHTO T 312):**

When producing WMA, gyrate specimens to specified  $N_{des}$  compaction effort without reheating mix other than to desired compaction temperature. Record time needed to reheat samples (if any).



**Page 6-11, Subarticle 609-5(C)(2)(e) Tensile Strength Ratio, insert the following immediately after the third paragraph:**

When producing WMA, perform TSR testing:

- (i.) Prior to initial production for each JMF and
- (ii.) Every 13,600 metric tons.

After three (3) consecutive passing TSR tests for a specific JMF, a request may be submitted to the State Asphalt Design Engineer to revert the *Hot-Mix Asphalt QMS Manual* procedures for TSR testing on that JMF. This request shall be submitted in writing and include all Material and Tests Unit Form 612s performed on the specific JMF.

**Page 6-22, Article 610-1 Description, insert the following as the third paragraph:**

Warm Mix Asphalt (WMA) is defined as additives or processes that allow a reduction in the temperature at which asphalt mixtures are produced and placed. Use WMA at the Contractor's option when shown in the contract.

**Page 6-23, Article 610-2 Materials, insert the following at the end of this Article:**

Use only WMA technologies on the allowable routes listed on the Department's approved list maintained by the Materials and Tests Unit. The Department's approved list can be found at the following website: <http://www.ncdot.org/doh/operations/materials/pdf/wma.pdf>.

**Page 6-26, Subarticle 610-3(B) Mix Design-Criteria, add the following as the fifth paragraph:**

When WMA is used, submit the mix design without including the WMA additive.

**Page 6-26, Subarticle 610-3(C) Job Mix Formula, add the following as the second paragraph:**

When WMA is used, document the technology used, the recommended dosage rate, and the requested plant mix temperature on the JMF submittal. Verify the JMF based on plant produced mixture from the field verification test.

**Immediately following PG 76-22 335°F, add the following paragraph:**

When WMA is used, produce an asphalt mixture within the temperature range of 107°C to 135°C.

**ASPHALT PAVER - FIXED STRING LINE:**

(10-21-03)

M6 R06

The Contractor's attention is directed to Article 610-8 of the *2006 Metric Standard Specifications* dealing with automatically controlled screeds on the asphalt pavement spreaders. A fixed string line is required on this project.

**ASPHALT BINDER CONTENT OF ASPHALT PLANT MIXES:**

(11-21-00)

M6 R15

The approximate asphalt binder content of the asphalt concrete plant mixtures used on this project will be as follows:

Asphalt Concrete Base Course	Type B 25.0	4.3%
Asphalt Concrete Intermediate Course	Type I 19.0	4.7%
Asphalt Concrete Surface Course	Type S 4.75A	7.0%
Asphalt Concrete Surface Course	Type SF 9.5A	6.5%
Asphalt Concrete Surface Course	Type S 9.5	6.0%
Asphalt Concrete Surface Course	Type S 12.5	5.5%

The actual asphalt binder content will be established during construction by the Engineer within the limits established in the *2006 Metric Standard Specifications*.

**ASPHALT PLANT MIXTURES:**

(7-1-95)

M6 R20

Place asphalt concrete base course material in trench sections with asphalt pavement spreaders made for the purpose or with other equipment approved by the Engineer.

**PRICE ADJUSTMENT - ASPHALT BINDER FOR PLANT MIX:**

(11-21-00)

M6 R25

Price adjustments for asphalt binder for plant mix will be made in accordance with Section 620 of the *2006 Metric Standard Specifications*.

The base price index for asphalt binder for plant mix is \$ **506.32** per metric ton.

This base price index represents an average of F.O.B. selling prices of asphalt binder at supplier's terminals on December 1, 2010.

**MASONRY DRAINAGE STRUCTURES:**

(10-16-07)

M8 R01

Revise the *2006 Standard Specifications* as follows:

**Page 8-25, Article 840-4 Measurement and Payment**, add the following at the end of the second paragraph:

For that portion of *Masonry Drainage Structure* measured above a height of 3 meters, payment will be made at 1.3 times the contract unit price per linear meter for *Masonry Drainage Structure*.

**BORROW EXCAVATION AND SHPO DOCUMENTATION FOR BORROW/WASTE****SITES:**

(12-18-07) (4-15-08)

M8 R02

Revise the *2006 Metric Standard Specifications* as follows:

**Division 2 Earthwork**

**Page 2-12, Subarticle 230-1(D)**, add the words: *The Contractor specifically waives* as the first words of the sentence.

**Page 2-13, Article 230-4(B) Contractor Furnished Sources, first paragraph, first sentence** replace with the following:

Prior to the approval of any borrow sources developed for use on any project, obtain certification from the State Historic Preservation Officer of the State Department of Cultural Resources certifying that the removal of the borrow material from the borrow sources(s) will have no effect on any known district, site building, structure, or object, architectural and/or archaeological that is included or eligible for inclusion in the National Register of Historic Places.

**Division 8 Incidentals**

**Page 8-8, Article 802-2 General Requirements, add the following as the 1st paragraph:**

Prior to the removal of any waste from any project, obtain certification from the State Historic Preservation Officer of the State Department of Cultural Resources certifying that the deposition of the waste material to the proposed waste area will have no effect on any known district, site building, structure, or object, architectural and/or archaeological that is included or eligible for inclusion in the National Register of Historic Places. Furnish a copy of this certification to the Engineer prior to performing any work in the proposed waste site.

**Page 8-8, Article 802-2, General Requirements, 7th paragraph, add the following as the 2nd sentence:**

The Department's borrow and waste site reclamation procedures for contracted projects is available on the NCDOT website and shall be used for all borrow and waste sites on this project.

**CONCRETE TRANSITIONAL SECTIONS FOR CATCH BASINS AND DROP INLETS:**

(1-20-09)

M8R03

Revise the *Metric Standard Specifications* as follows:

**Page 8-26, Article 840-4 Measurement and Payment, delete the eighth full paragraph and replace with the following:**

No separate payment will be made for Concrete Aprons as shown in Standard Drawings 840.17, 840.18, 840.19, 840.26, 840.27 and 840.28 and will be incidental to the other work in this section.

**Page 8-31, Article 852-4 Measurement and Payment, add the following as the fourth paragraph.**

*Concrete Transitional Section for Catch Basin* will be measured and paid for in units of each.

*Concrete Transitional Section for Drop Inlet* will be measured and paid for in units of each.

Payment will be made under:

<b>Pay Item</b>	<b>Pay Unit</b>
Concrete Transitional Section for Catch Basin	Each
Concrete Transitional Section for Drop Inlet	Each

Revise the *Metric Roadway Standard Drawings* as follows:

On page 852.04, change Pay Limits for Concrete Apron for Drop Inlets in two places on the drawing to *Pay Limits for Concrete Transitional Section for Drop Inlet*.

On page 852.05, change Concrete Apron for Catch Basin on the drawing to *Concrete Transitional Section for Catch Basin*.

On page 852.06, change Pay Limits for Concrete Apron for Drop Inlets in two places on the drawing to *Pay Limits for Concrete Transitional Section for Drop Inlet*.

**SUBSURFACE DRAINAGE:**

(7-20-10)

M8 R05

Revise the *2006 Metric Standard Specifications* as follows:

**Page 8-11, Delete Section 815 SUBSURFACE DRAINAGE and replace it with the following:**

**SECTION 815  
SUBSURFACE DRAINAGE**

**815-1 Description**

Construct subsurface drains, underdrains, blind drains and other types of drains in accordance with the contract or as directed by the Engineer. Install markers to locate concrete pads for drains as shown on the plans. This provision does not apply to shoulder drains.

**815-2 Materials**

Refer to Division 10 of the *Standard Specifications*.

<b>Item</b>	<b>Section</b>
Portland Cement Concrete, Class B	1000
Select Material, Class V	1016
Subsurface Drainage Materials	1044
Filter Fabric for Subsurface Drains, Type 1	1056
Steel Markers	1072-4
Steel Marker Paint	1080-14
Pavement Marker Paint	1087

Use Class B Concrete for concrete pads and Class V Select Material for subdrain coarse aggregate. Provide subdrain coarse aggregate for subsurface drains and subdrain fine aggregate for underdrains and blind drains.

**815-3 Construction Methods**

Do not leave filter fabrics uncovered for more than 7 days. Excavate trenches as necessary in accordance with the contract or as directed by the Engineer. For subsurface drains, line trench with filter fabric and overlap fabric ends a minimum of 150 mm on top of subdrain coarse aggregate.

Install blind drains at a depth of 1.2 to 1.8 meters below subgrade elevation. Install subdrain pipes for subsurface drains and underdrains at a depth of 1.2 to 1.8 meters below subgrade elevation unless the subgrade will be proof rolled. For subsurface drains and underdrains in subgrades that will be proof rolled, install subdrain pipes at a depth of 1.8 meters below subgrade elevation. Firmly connect subdrain pipes together as needed. Place perforated subdrain pipes with perforations down except for pipes in dry materials, in which case turn perforations up or use non-perforated pipes. For concrete pipes in dry materials, construct mortar joints in accordance with Subarticle 300-6(A) of the *Standard Specifications*.

Place subdrain aggregate beneath, around and over subdrain pipes such that pipes are covered by at least 150 mm of aggregate unless shown otherwise on the plans. Do not displace or damage subdrain pipes while placing and compacting subdrain aggregate. Lightly compact backfill material such that settlement is minimized.

Use solvent cement for connecting polyvinyl chloride (PVC) outlet pipes and fittings such as wyes, tees and elbows. Provide connectors for outlet pipes and fittings that are watertight and suitable for gravity flow conditions. Cover open ends of outlet pipes with rodent screens as shown on the plans.

Connect drains to concrete pads or existing drainage structures at ends of outlet pipes. Construct concrete pads and provide an Ordinary Surface Finish in accordance with Subarticle 825-6(B) of the *Standard Specifications*. Furnish and install steel and pavement markers at concrete pads as shown on the plans.

Allow drains to function for up to 30 days or a sufficient time as determined by the Engineer before undercutting, proof rolling or constructing embankments over drains.

#### **815-4 Measurement and Payment**

*Subdrain Excavation* will be measured and paid for in cubic meters. Excavation will be measured based on the trench width shown on the plans or as directed by the Engineer and the actual trench depth as determined by the Engineer. The contract unit price for *Subdrain Excavation* will be full compensation for excavating trenches and backfilling above subdrain aggregate.

*Filter Fabric for Subsurface Drains* will be measured and paid for in square meters. Filter fabric in a trench will be measured in place based on the subdrain aggregate width shown on the plans or as directed by the Engineer and the actual aggregate depth as determined by the Engineer. No additional payment will be made for overlapping fabric. The contract unit price for *Filter Fabric for Subsurface Drains* will be full compensation for supplying, transporting and installing filter fabric.

*Subdrain Fine Aggregate* and *Subdrain Coarse Aggregate* will be measured and paid for in cubic meters. Subdrain aggregate in a trench will be measured in place based on the aggregate width shown on the plans or as directed by the Engineer and the actual aggregate depth as determined by the Engineer. When subdrain aggregate is not placed in a trench, aggregate will be measured in place based on the aggregate dimensions shown on the plans or as determined by the Engineer. The contract unit prices for *Subdrain Fine Aggregate* and *Subdrain Coarse Aggregate* will be full compensation for furnishing, hauling, handling, placing, compacting and maintaining subdrain aggregate.

*\_\_mm Perforated Subdrain Pipe* and *\_\_mm Outlet Pipe* will be measured and paid for in linear feet. Pipes will be measured in place as the pipe length, including fittings, to the nearest 0.1 meters with no deduction for fittings. The contract unit prices for *\_\_mm Perforated Subdrain Pipe* and *\_\_mm Outlet Pipe* will be full compensation for supplying, transporting and installing pipes, fittings and rodent screens and making joint connections.

*Subdrain Pipe Outlets* will be measured and paid for in units of each. Outlets will be measured as the number of concrete pads or connections to existing drainage structures. The contract unit price for *Subdrain Pipe Outlets* will be full compensation for concrete pads including furnishing concrete, constructing pads and providing and placing markers and connecting pipes to existing drainage structures including cutting into structures, removing existing paved ditches and grouting around connections.

Payment will be made under:

<b>Pay Item</b>	<b>Pay Unit</b>
Subdrain Excavation	Cubic Meter
Filter Fabric for Subsurface Drains	Square Meter
Subdrain Fine Aggregate	Cubic Meter
Subdrain Coarse Aggregate	Cubic Meter
__mm Perforated Subdrain Pipe	Linear Meter
__mm Outlet Pipe	Linear Meter
Subdrain Pipe Outlets	Each

**ENDWALLS:**

(5-20-08)

M8 R25

Revise the *Standard Specifications* as follows:

**Page 8-23, Article 838-4 Replace the 1st and 2nd paragraph with the following:**

*Endwalls* will be measured and paid for in cubic meters of concrete or brick that have been completed and accepted. This quantity will be computed from the dimensions shown on the plans or from revised authorized dimensions. Where precast concrete units have been approved and are used in lieu of cast-in-place units the quantity to be paid for will be computed the same as if cast-in-place units were used, as no reduction in pay quantity will be made due to the use of precast in lieu of cast in place endwalls.

*Reinforced Endwalls* will be measured and paid for in cubic meters of concrete or brick that have been completed and accepted. This quantity will be computed from the dimensions shown on the plans or from revised authorized dimensions. Where precast concrete units have been approved and are used in lieu of cast-in-place units the quantity to be paid for will be computed the same as if cast-in-place units were used, as no reduction in pay quantity will be made due to the use of precast in lieu of reinforced cast in place endwalls.

**GUARDRAIL ANCHOR UNITS, TYPE 350:**

(4-20-04)

M8 R65

**Description**

Furnish and install guardrail anchor units in accordance with the details in the plans, the applicable requirements of Section 862 of the *2006 Metric Standard Specifications*, and at locations shown in the plans.

**Materials**

The Contractor may at his option, furnish any one of the guardrail anchor units.

Guardrail anchor unit (ET-2000) as manufactured by:

Trinity Industries, Inc.  
2525 N. Stemmons Freeway  
Dallas, Texas 75207  
Telephone: 800-644-7976

The guardrail anchor unit (SKT 350) as manufactured by:

Road Systems, Inc.  
3616 Old Howard County Airport  
Big Spring, Texas 79720  
Telephone: 915-263-2435

Prior to installation the Contractor shall submit to the Engineer:

(A) FHWA acceptance letter for each guardrail anchor unit certifying it meets the requirements of NCHRP Report 350, Test Level 3, in accordance with Section 106-2 of the *2006 Standard Specifications*.

(B) Certified working drawings and assembling instructions from the manufacturer for each guardrail anchor unit in accordance with Section 105-2 of the *2006 Metric Standard Specifications*.

No modifications shall be made to the guardrail anchor unit without the express written permission from the manufacturer. Perform installation in accordance with the details in the plans, and details and assembling instructions furnished by the manufacturer.

### **Construction Methods**

Guardrail end delineation is required on all approach and trailing end sections for both temporary and permanent installations. Guardrail end delineation consists of yellow reflective sheeting applied to the entire end section of the guardrail in accordance with Section 1088-3 of the *2006 Metric Standard Specifications* and is incidental to the cost of the guardrail anchor unit.

### **Measurement and Payment**

Measurement and payment will be made in accordance with Articles 862-6 of the *2006 Metric Standard Specifications*.

Payment will be made under:

<b>Pay Item</b>	<b>Pay Unit</b>
Guardrail Anchor Units, Type 350	Each



**FENCE:**

(3-6-06)

M8 R86

Revise the *2006 Metric Standard Specifications* as follows:

**Page 8-44, Subarticle 866-3(A),** second sentence,

Add *existing fencing* after stumps

**PREFORMED SCOUR HOLE WITH LEVEL SPREADER APRON:**

(10-15-02) (Rev 6-17-08)

M8 R105

**Description**

Construct and maintain preformed scour holes with spreader aprons at the locations shown on the plans and in accordance with the details in the plans. Work includes excavation, shaping and maintaining the hole and apron, furnishing and placing filter fabric, rip rap (class as specified in the plans) and permanent soil reinforcement matting.

**Materials**

<b>Item</b>	<b>Section</b>
Plain Rip Rap	1042
Filter Fabric	1056

The permanent soil reinforcement matting shall be permanent erosion control reinforcement mat and shall be constructed of 100% coconut fiber stitch bonded between a heavy duty UV stabilized cuspated (crimped) netting overlaid with a heavy duty UV stabilized top net. The three nettings shall be stitched together on 38 mm centers UV stabilized polyester thread to form a permanent three dimensional structure. The mat shall have the following physical properties:

<i>Property</i>	<i>Test Method</i>	<i>Value Unit</i>
Light Penetration	ASTM D6567	15 %
Thickness	ASTM D6525	13 mm
Mass Per Unit Area	ASTM D6566	0.339 kg/m <sup>2</sup>
Tensile Strength	ASTM D6818	572 kg/m
Elongation (Maximum)	ASTM D6818	49 %
Resiliency	ASTM D6524	> 70 %
UV Stability*	ASTM D4355	≥80 %
Porosity (Permanent Net)	Calculated	≥85 %
Minimum Filament	Measured	0.76 mm
Maximum Permissible Shear Stress (Vegetated)	Performance Test	≥ 39.1 kg/m <sup>2</sup>
Maximum Allowable Velocity	Performance Test	≥ 4.9 m/s

\*ASTM D1682 Tensile Strength and % strength retention of material after 1000 hours of exposure.

Submit a certification from the manufacturer showing:

- (A) the chemical and physical properties of the mat used, and
- (B) conformance of the mat with this specification

### **Soil Preparation**

All areas to be protected with the mat shall be brought to final grade and seeded in accordance with Section 1660. The surface of the soil shall be smooth, firm, stable and free of rocks, clods, roots or other obstructions that would prevent the mat from lying in direct contact with the soil surface. Areas where the mat is to be placed will not need to be mulched.

### **Measurement and Payment**

*Preformed Scour Holes with Level Spreader Aprons* will be measured and paid as the actual number that has been incorporated into the completed and accepted work. Such price and payment will be full compensation for all work covered by this provision.

Payment will be made under:

<b>Pay Item</b>	<b>Pay Unit</b>
Preformed Scour Hole with Level Spreader Aprons	Each

### **STREET SIGNS AND MARKERS AND ROUTE MARKERS:**

(7-1-95)

M9 R01

Move any existing street signs, markers, and route markers out of the construction limits of the project and install the street signs and markers and route markers so that they will be visible to the traveling public if there is sufficient right of way for these signs and markers outside of the construction limits.

Near the completion of the project and when so directed by the Engineer, move the signs and markers and install them in their proper location in regard to the finished pavement of the project.

Stockpile any signs or markers that cannot be relocated due to lack of right of way, or any signs and markers that will no longer be applicable after the construction of the project, at locations directed by the Engineer for removal by others.

The Contractor shall be responsible to the owners for any damage to any street signs and markers or route markers during the above described operations.

No direct payment will be made for relocating, reinstalling, and/or stockpiling the street signs and markers and route markers as such work shall be considered incidental to other work being paid for by the various items in the contract.

**STEEL U-CHANNEL POSTS AND STEEL SQUARE TUBE SUPPORTS:**

(7-18-06) (Rev 1-18-11)

M9 R02

Revise the *2006 Standard Specifications* as follows:

**Page 9-15 Subarticle 903-3(D) delete the last sentence in the first paragraph and add the following:**

Use posts of sufficient length to permit the appropriate sign mounting height. Spliced posts are not permitted on new construction.

**Page 9-16 Subarticle 903-3(G) delete the last sentence in the first paragraph and add the following:**

Use posts of sufficient length to permit the appropriate sign mounting height. Spliced posts are not permitted on new construction.

**Page 9-16 Subarticle 903-3(G), delete the fourth paragraph and add the following:**

Do not weld or cut supports in the field except for the saw cutting of steel square tube material for the frames and cross-braces that may be required for Types D, E, and F signs with two or more supports.

**HIGH STRENGTH CONCRETE FOR DRIVEWAYS:**

(11-21-00) (7-18-06)

M10 R01

Use high early strength concrete for all driveways shown in the plans and as directed by the Engineer. Provide high early strength concrete that meets the requirements of Article 1000-6 of the *2006 Metric Standard Specifications*.

Measurement and payment will be in accordance with Section 848 of the *2006 Metric Standard Specifications*.

**GALVANIZED HIGH STRENGTH BOLTS, NUTS AND WASHERS:**

(2-17-09)

M10R02

Revise the *Metric Standard Specifications* as follows:

**Page 10-101, Subarticle 1072-7(F)(3) Change the AASHTO reference to B 695 Class 55**

**Page 10-201, Table 1092-2, Steel Sign Materials, Change High Strength Bolts, Nuts & Washers ASTM Specifications for Galvanizing to B695 Class 55.**

**Page 10-211, Subarticle 1094-1(A) Breakaway or Simple Steel Beam Sign Supports**, replace the first full paragraph with the following:

Fabricate high strength bolts, nuts, and washers required for breakaway supports from steel in accordance with ASTM A325 and galvanize in accordance with AASHTO B 695 Class 55.

**Page 10-212, Article 1096-2 Steel Overhead Sign Structures**, replace the last sentence with the following:

The galvanizing shall meet the requirements of AASHTO B 695 Class 55 for fasteners and of ASTM A123 for other structural steel.

**GALVANIZING:**

(8-17-10)

M10 R03

Revise the *Standard Specifications* as follows:

**Page 10-121, Subarticle 1076-1, Galvanizing**, add a second paragraph as the follows:

Allow the Engineer to obtain samples of molten zinc directly from the galvanizing vat upon request.

**AGGREGATE PRODUCTION:**

(11-20-01)

M10 R05

Provide aggregate from a producer who uses the current Aggregate Quality Control/Quality Assurance Program that is in effect at the time of shipment.

No price adjustment is allowed to contractors or producers who use the program. Participation in the program does not relieve the producer of the responsibility of complying with all requirements of the *2006 Metric Standard Specifications*. Copies of this procedure are available upon request from the Materials and Test Unit.

**CONCRETE BRICK AND BLOCK PRODUCTION:**

(11-20-01)

M10 R10

Provide concrete brick and block from a producer who uses the current Solid Concrete Masonry Brick/Unit Quality Control/Quality Assurance Program that is in effect on the date that material is received on the project.

No price adjustment is allowed to contractors or producers who use the program. Participation in the program does not relieve the producer of the responsibility of complying with all requirements of the *2006 Metric Standard Specifications*. Copies of this procedure are available upon request from the Materials and Test Unit.

**VOLUMETRIC CONCRETE BATCHING:**

(5-18-10)

M10 R13

Revise the *2006 Standard Specifications* as follows:

**Page 10-19, after Article 1000-12, add the following as a new article:**

**1000-13      VOLUMETRIC MIXED CONCRETE**

Upon written request by the contractor, the Department may approve the use of concrete proportioned by volume. The volumetric producer must submit and have approved a process control plan and product quality control plan by the Materials and Tests Unit. If concrete is proportioned by volume, the other requirements of these specifications with the following modifications will apply. Unless otherwise approved by the Department, use of concrete proportioned by volume shall be limited to Class B concrete and a maximum of 22.94 cubic meters per unit per day.

**(A)      Materials**

Use materials that meet the requirements for the respective items in the *Standard Specifications* except that they will be measured by a calibrated volume-weight relationship.

Storage facilities for all material shall be designed to permit the Department to make necessary inspections prior to the batching operations. The facilities shall also permit identification of approved material at all times, and shall be designed to avoid mixing with or contaminating by unapproved material. Coarse and fine aggregate shall be furnished and handled so variations in the moisture content affecting the uniform consistency of the concrete will be avoided.

Moisture content of the coarse and fine aggregate will be made available onsite for the Engineer's review for each load. The frequency of moisture testing will be dependent on certain variables such as weather, season and source; however, moisture tests should be performed at least once at the beginning of the work day for each source material. Additional daily moisture tests for the coarse and fine aggregate shall be performed if requested by the Engineer.

Unused materials should be emptied from hopper daily. Concrete should not be mixed with materials that have been left in the hopper overnight.

**(B)      Equipment**

Provide volumetric mixers with rating plates indicating that the performance of the mixer is in accordance with the Volumetric Mixer Manufacturer Bureau or equivalent. Mixers must comply with ASTM C685. Unless otherwise specified, all mixing operations must be in strict accordance with the manufacturer's recommended procedures. Such procedures shall be provided to the Department for review upon request.

The volumetric mixer shall be capable of carrying sufficient unmixed dry bulk cement, pozzolan (if required), fine aggregate, coarse aggregate, admixtures and water, in separate compartments and accurately proportioning the specified mix. Each batching or mixing unit (or both) shall carry in a prominent place a metal plate or plates on which are plainly marked the gross volume of the unit in terms of mixed concrete, discharge speed and the weight-calibrated constant of the machine in terms of a revolution counter or other output indicator.

The concrete mixing device shall be an auger-type continuous mixer used in conjunction with volumetric proportioning. The mixer shall produce concrete, uniform in color and appearance, with homogeneous distribution of the material throughout the mixture. Mixing time necessary to produce uniform concrete shall be established by the contractor and shall comply with other requirements of these specifications. Only equipment found acceptable in every respect and capable of producing uniform results will be permitted.

Each volumetric mixer shall be equipped with an onboard ticketing system that will electronically produce a record of all material used and their respective weights and the total volume of concrete placed. Alternate methods of recordation may be used if approved by the Engineer. Tickets should also identify the following information, at minimum:

- Contractor Name
- Contractor Phone Number
- NCDOT Project No. and TIP No.
- Date
- Truck No.
- Ticket No.
- Time Start/End of Pour
- Mix ID & Description (Strength)
- Aggregate Moisture Before Mixing

**(C) Proportioning Devices**

Volume proportioning devices, such as counters, calibrated gate openings or flow meters, shall be easily accessible for controlling and determining the quantities of the ingredients discharged. All indicating devices that affect the accuracy of proportioning and mixing of concrete shall be in full view of and near enough to be read by the operator and Engineer while concrete is being produced. In operation, the entire measuring and dispensing mechanism shall produce the specified proportions of each ingredient.

The volumetric mixer shall provide positive control of the flow of water and admixtures into the mixing chamber. Water flow shall be indicated by a flow meter and be readily adjustable to provide for slump control and/or minor variations in aggregate moisture. The mixer shall be capable of continuously circulating or mechanically agitating the admixtures.

Liquid admixtures shall be dispensed through a controlled, calibrated flow meter. A positive means to observe the continuous flow of material shall be provided. If an admixture requires diluting, the admixture shall be diluted and thoroughly mixed prior to introducing the admixture into the dispenser. When admixtures are diluted, the ratio of dilution and the mixing shall be approved by and performed in the presence of the Department.

The volumetric mixer shall be capable of measurement of cement, pozzolan (if required), liquids and aggregate being introduced into the mix.

**(D) Calibration**

Volume-weight relationships will be based on calibration. The proportioning devices shall be calibrated by the contractor prior to the start of each NCDOT job, and subsequently at intervals recommended by the equipment manufacturer. Calibrations will be performed in the presence of the Department and subject to approval from the Department. Calibration of the cement and aggregate proportioning devices shall be accomplished by weighing (determining the mass of) each component. Calibration of the admixture and water proportioning devices shall be accomplished by weight (mass) or volume. Tolerances in proportioning the individual components will be as follows:

**TABLE 1000-4  
VOLUMETRIC MIXED CONCRETE CALIBRATION  
PROPORTION TOLERANCES**

<b>Item</b>	<b>Tolerance</b>
Cement, Weight (Mass) percent	0 to +4
Fine Aggregate, Weight (Mass) percent	± 2
Coarse Aggregate, Weight (Mass) percent	± 2
Admixtures, Weight (Mass) or Volume percent	± 3
Water, Weight (Mass) or Volume percent	± 1

Each volumetric mixer must be accompanied at all times by completed calibration worksheets and they shall be made available to the Department upon request.

**(E) Verification of Yield**

Verification of the proportioning devices may be required at any time by the Department. Verification shall be accomplished by proportioning the rock and sand based on the cement meter count for each concrete mobile mixer. Once the count (revolutions) for 42.64 kilograms of cement has been determined then delivery of the correct amount of rock and sand can be verified.

**(F) Uniformity**

When concrete is produced, have present during all batching operations a Certified Concrete Batch Technician. During batching and placement, the sole duty of this employee is to supervise the production and control of the concrete, perform moisture tests, adjust mix proportions of aggregates for free moisture, complete and sign approved delivery tickets, and assure quality control of the batching.

Two samples of sufficient size to make the required tests will be taken after discharge of approximately 15 and 85 percent of the load. Each of the 2 samples of concrete will be separately tested for the properties listed in Table 1000-3. Tests will be conducted in accordance with the test procedures specified in Table 1000-3 or procedures established by the Materials and Tests Unit. The Engineer may recheck mixer performance at any time when in his opinion satisfactory mixing is not being accomplished.

**PORTLAND CEMENT CONCRETE (Alkali-Silica Reaction):**

(2-20-07)

M10 R16

Revise the *2006 Metric Standard Specifications* as follows:

**Article 1024-1(A)**, replace the 2nd paragraph with the following:

Certain combinations of cement and aggregate exhibit an adverse alkali-silica reaction. The alkalinity of any cement, expressed as sodium-oxide equivalent, shall not exceed 1.0 percent. For mix designs that contain non-reactive aggregates and cement with an alkali content less than 0.6%, straight cement or a combination of cement and fly ash, cement and ground granulated blast furnace slag or cement and microsilica may be used. The pozzolan quantity shall not exceed the amount shown in Table 1024-1. For mixes that contain cement with an alkali content between 0.6% and 1.0%, and for mixes that contain a reactive aggregate documented by the Department, regardless of the alkali content of the cement, use a pozzolan in the amount shown in Table 1024-1.

Obtain the list of reactive aggregates documented by the Department at:

<http://www.ncdot.org/doh/operations/materials/pdf/quarrysrprob.pdf>

<b>Table 1024-1</b>	
<b>Pozzolans for Use in Portland Cement Concrete</b>	
<i>Pozzolan</i>	<i>Rate</i>
Class F Fly Ash	20% by weight of required cement content, with 1.2 kg Class F fly ash per kg of cement replaced
Ground Granulated Blast Furnace Slag	35%-50% by weight of required cement content with 1 kg slag per kg of cement replaced
Microsilica	4%-8% by weight of required cement content, with 1 kg microsilica per kg of cement replaced



**WATER FOR CONCRETE:**

(10-19-10)

M10 R17

Revise the 2006 *Metric Standard Specifications for Roads and Structures* as follows:

**Page 10-51, Article 1024-4**, replace article with the following:

**1024-4 WATER**

Ensure that water used to condition, wash, or as an integral part of materials is clear and free from injurious amounts of oil, acid, alkali, organic matter, or other deleterious substance. It shall not be salty or brackish. Water used in the production of concrete or grout shall be from wells or public water systems which are suitable for drinking and must meet the criteria listed in Table 1024-1.

Test all water from wells and public water supplies from all out of state locations and in the following counties: Beaufort, Bertie, Brunswick, Camden, Carteret, Chowan, Craven, Currituck, Dare, Gates, Hyde, New Hanover, Onslow, Pamlico, Pasquotank, Pender, Perquimans, Tyrell, and Washington unless the Engineer waives the testing requirements. Water from a municipal water supply in all other NC counties may be accepted by the Engineer without testing.

**TABLE 1024-1  
ACCEPTANCE CRITERIA FOR WATER  
USED IN THE PRODUCTION OF CONCRETE**

<b>Requirement</b>	<b>Limit</b>	<b>Test Method</b>
Compressive Strength, minimum percent of control at 3 and 7 days	90 percent	NCDOT Modified / AASHTO T106
Time of set, deviation from control	From 1:00 hr. earlier to 1:30 hr. later	NCDOT Modified / AASHTO T131
pH	4.5 to 8.5	NCDOT Modified / AASHTO T26
Chloride Ion Content, Max.	250 ppm	ASTM D512
Total Solids Content (Residue), Max.	1000 ppm	NCDOT Modified / Standard Methods for Examination of Water and Wastewater
Resistivity, Min.	0.500 kohm-cm	NCDOT Modified / ASTM D1125
Sulfate as SO <sub>4</sub> , Max.	1500 ppm	NCDOT Modified / ASTM D516
Presence of Sugar	None	NCDOT Procedure
Dissolved Organic Matter	None	NCDOT Modified / AASHTO T26

**Page 10-53, Article 1026-4**, replace article with the following:

**1026-4 WATER**

All water used for curing concrete shall meet the requirements of Article 1024-4 and Table 1024-1. Water from wells, streams, ponds, or public water systems may be used.

**CULVERT PIPE:**

(1-19-10)

M10R32

Revise the *Metric Standard Specifications for Roads and Structures* as follows:

**Page 10-67, Article 1032-1**, replace (A), (B), (C), (D), (E) and (F) with the following:

- (A) Coated corrugated metal culvert pipe and pipe arches.
- (B) Coated corrugated metal end sections, coupling band, and other accessories
- (C) Corrugated aluminum alloy structural plate pipe and pipe arches
- (D) Corrugated aluminum alloy end sections, coupling band, and other accessories
- (E) Welded steel pipe

**Page 10-69, Subarticle 1032-3(A)(5) Coating Repair**, replace with the following:

Repair shall be in accordance with Section 1076-6 of the *Standard Specifications*.

**Subarticle 1032-3(A)(7) Aluminized Pipe**, replace with the following:

Aluminized pipe shall meet all requirements herein, except that the pipe and coupling bands shall be fabricated from aluminum coated steel sheet meeting the requirements of AASHTO M274.

**Page 10-71, Article 1032-4 Coated Culvert Pipe**, replace (A), (1), (2), (3), (4), (B), (C), (D), (E), (F) and (G) with the following:

- (A) Coatings for Steel Culvert Pipe or Pipe Arch

The below coating requirements apply for steel culvert pipe, pipe arch, end sections, tees, elbows, and eccentric reducers.

- (1) Steel Culvert pipe shall have an aluminized coating, meeting the requirement of AASHTO M274
- (2) When shown on the plans or as approved by the Engineer, a polymeric coating meeting the requirements of AASHTO M246 for Type B coating may be substituted for aluminized coating.

**(B) Acceptance**

Acceptance of coated steel culvert pipe, and its accessories will be based on, but not limited to, visual inspections, classification requirements, check samples taken from material delivered to the project, and conformance to the annual Brand Registration.

**Page 10-73, Article 1032-5, sixth paragraph, third sentence, remove the word "spelter"**

**Page 10-74, 1032-7 Vitrified Clay Culvert Pipe, delete section in its entirety.**

**Page 10-75, Article 1032-8 Welded Steel Pipe, change title to WELDED STEEL PIPE FOR DRAINAGE**

**Subarticle 1032-9(B) Plain Concrete Culvert Pipe, delete section in its entirety.**

**Page 10-77, Article 1032-10 Corrugated Polyethylene Culvert Pipe, change title to CORRUGATED POLYETHYLENE (HDPE) CULVERT PIPE**

**Add the following: Article 1032-11 Polyvinyl Chloride (PVC) Pipe**

Polyvinyl Chloride pipe shall conform to AASHTO M 304 or ASTM 949. When rubber gaskets are to be installed in the pipe joint, the gasket shall be the sole element relied on to maintain a tight joint. Test pipe joints at the plant hydrostatically using test methods in ASTM D 3212. Soil tight joints shall be watertight to 13.8 kPa. Watertight joints shall be watertight to 34.5 kPa unless a higher pressure rating is specified in the plans.

**GLASS BEADS:**

(7-18-06)(Rev 10-19-10)

M10 R35

Revise the *2006 Metric Standard Specifications* as follows:

**Page 10-181, 1087-4(A) Composition, add the following as the fourth paragraph:**

Glass beads shall have no more than 75 parts per million of arsenic as determined by the United States Environmental Protection Agency Method 6010B in conjunction with the United States Environmental Protection Agency Method 3052 modified.

**Page 10-182, 1087-4(C) Gradation & Roundness, delete the last paragraph and replace the second sentence of the first paragraph with the following:**

All Drop-On and Intermixed Glass Beads shall be tested in accordance with ASTM D1155.

**Page 10-184, 1087-8 Material Certification, add the following below the first sentence:**

Glass Beads (for paint, thermoplastic and polyurea) – Type 3 Material Certification for no more than 75 parts per million of arsenic

**ENGINEERING FABRICS:**

(7-18-06) (Rev 10-19-10)

M10 R40

Revise the *2006 Metric Standard Specifications* as follows:

**Page 10-78, Delete Section 1056 ENGINEERING FABRICS** and replace it with the following:

**SECTION 1056  
ENGINEERING FABRICS**

**1056-1 General**

Use engineering fabrics that meet the requirements of Article 4.1 of AASHTO M288 and have been evaluated by National Transportation Product Evaluation Program (NTPEP). When required, sew fabrics together in accordance with Article X1.1.4 of AASHTO M288. Provide sewn seams with seam strengths meeting the required strengths for the engineering fabric type and class specified.

Load, transport, unload and store fabrics such that they are kept clean and free of damage. Label, ship and store fabrics in accordance with Section 7 of AASHTO M288. Fabrics with defects, flaws, deterioration or damage will be rejected. Do not unwrap fabrics until just before installation. With the exception of fabrics for temporary silt fences and mechanically stabilized earth (MSE) wall faces, do not leave fabrics exposed for more than 7 days before covering fabrics with material.

When required, use pins a minimum of 5 mm in diameter and 450 mm long with a point at one end and a head at the other end that will retain a steel washer with a minimum outside diameter of 38 mm. When wire staples are required, provide staples in accordance with Subarticle 1060-8(D) of the *2006 Metric Standard Specifications*.

**1056-2 Fabric Properties**

Provide Type 1 Certified Mill Test Report, Type 2 Typical Certified Mill Test Report or Type 4 Certified Test Report in accordance with Article 106-3 of the *2006 Metric Standard Specifications*. Furnish certifications with minimum average roll values (MARV) as defined by ASTM D4439 for all fabric properties with the exception of elongation and apparent opening size (AOS). For testing fabrics, a lot is defined as a single day's production.

Provide engineering fabric types and classes in accordance with the contract. Machine direction (MD) and cross-machine direction (CD) are as defined by ASTM D4439. Use woven or nonwoven fabrics with properties meeting the requirements of Table 1056-1.

**TABLE 1056-1  
FABRIC PROPERTY REQUIREMENTS**

Property	ASTM Test Method	Requirements (MARV <sup>1</sup> )				
		Type 1	Type 2	Type 3 <sup>2</sup>	Type 4	Type 5 <sup>3</sup>
<i>Typical Application</i>		<i>Shoulder Drains</i>	<i>Under Riprap</i>	<i>Temporary Silt Fence</i>	<i>Soil Stabilization</i>	<i>Temporary MSE Walls</i>
Elongation (MD & CD)	D4632	≥ 50 %	≥ 50 %	≤ 25 %	< 50 %	< 50 %
Grab Strength (MD & CD)	D4632	400 N	900 N	445 N	800 N	---
Tear Strength (MD & CD)	D4533	180 N	350 N	---	300 N	---
Puncture Strength	D6241	900 N	1925 N	---	1650 N	---
Wide Width Tensile Strength @ Ultimate (MD & CD)	D4595	---	---	---	---	35 kN/m (unless required otherwise in the contract)
Permittivity	D4491	0.20 sec <sup>-1</sup>	0.20 sec <sup>-1</sup>	0.05 sec <sup>-1</sup>	0.05 sec <sup>-1</sup>	0.20 sec <sup>-1</sup>
Apparent Opening Size (AOS) <sup>4</sup>	D4751	0.25 mm	0.25 mm	0.60 mm	0.43 mm	0.60 mm
Ultraviolet Stability (retained strength) <sup>5</sup>	D4355	50 %	50 %	70 %	50 %	50%

<sup>1</sup>MARV does not apply to elongation and AOS

<sup>2</sup>Minimum roll width of 900 mm required

<sup>3</sup>Minimum roll width of 4 m required unless otherwise approved

<sup>4</sup>Maximum average roll value

<sup>5</sup>After 500 hours of exposure

**PRECAST DRAINAGE STRUCTURES - MACRO-SYNTHETIC FIBERS**

(7-15-08)(Rev 11-18-08)

SP 10 R42

**Description**

Substitute as an option, macro-synthetic fibers in lieu of 100 mm x 100 mm W1.4 x W1.4 welded wire fabric reinforcement for selected precast concrete products in accordance with the following requirements.

**Materials**

<b>Item</b>	<b>Section</b>
Portland Cement Concrete	1077-5

- (A) Substitute macro-synthetic fibers only for steel reinforcement with an area of steel of 254 mm<sup>2</sup>/m or less in the following items:
- (1) **Precast Drainage Structure** units in accordance with the requirements of *Standard Drawing 840.45*.
  - (2) **Precast Manhole 1.2 Meter' Riser Sections** in accordance with the requirements of *Standard Drawing 840.52*.

All other requirements, including reinforcement for these precast concrete items will remain the same.

- (B) **Submittal** Submit to the Department for approval by the precast producer and fiber manufacturer, independently performed test results certifying the macro-synthetic fibers and the precast concrete products meet the requirements listed herein:

(C) **Macro-Synthetic Fibers**

- (1) Manufacture from virgin polyolefins (polypropylene and polyethylene) and comply with ASTM C 1116.4.1.3.  
*Fibers manufactured from materials other than polyolefins* Submit test results certifying resistance to long-term deterioration when in contact with the moisture and alkalis present in cement paste and/or the substances present in air-entraining and chemical admixtures.
- (2) Fiber length - no less than 38 mm.
- (3) Macro-synthetic fibers - aspect ratio (length divided by the equivalent diameter of the fiber) between 45 and 150.
- (4) Macro-synthetic fibers - Minimum tensile strength of 2812 kg/cm<sup>2</sup> when tested in accordance with ASTM D 3822.

- (5) Macro-synthetic fibers - minimum modulus of elasticity of 28,123 kg/cm<sup>2</sup> when tested in accordance with ASTM D 3822.

**(D) Fiber Reinforced Concrete**

- (1) Approved structural fibers may be used as a replacement of steel reinforcement in allowable structures of NCDOT Standards 840.45 and 840.52. The dosage rate, in pounds of fibers per cubic yard, shall be as per recommended by the fiber manufacturer to provide a minimum average residual strength (in accordance with ASTM C 1399) of concrete of no less than that of the concrete with the steel reinforcement that is being replaced, but no less than 2.97 kg/m<sup>3</sup>. Submit the recommendations of the manufacturer that correlate the toughness of steel-reinforced concrete with that of the recommended dosage rate for the fiber-reinforced concrete.
- (2) Fiber reinforced concrete - 4.5% air content, ± 1.5% tolerance.
- (3) Fiber reinforced concrete - develop a minimum compressive strength 2.97 kg/m<sup>3</sup> in 28 days.
- (4) Workability of the concrete mix - determine in accordance with ASTM C995. The flow time - not be less than 7 seconds or greater than 25 seconds.
- (5) Assure the fibers are well dispersed and prevent fiber balling during production. After introduction of all other ingredients, add the plastic concrete and mix the plastic concrete for at least 4 minutes or for 50 revolutions at standard mixing speed.

**Measurement and Payment**

No separate payment will be made for substitution of macro-fiber synthetic reinforcement for the steel reinforcing. The price bid for the precast units will be full compensation for furnishing and incorporating the macro-fiber synthetic reinforcement.

**QUALIFICATION OF WELDS AND PROCEDURES:**

(7-21-09)

M10 R43

**Page 10-114, Subarticle 1072-20(D) Qualification of Welds and Procedures**, replace the third sentence of the first paragraph with the following:

For all prequalified field welds, submit Welding Procedure Specifications (WPS) for each joint configuration for approval at least 30 days prior to performing any welding. In lieu of this, use the WPS provided and preapproved by the Department. These preapproved WPS are available from the Materials and Tests Unit or at:

[http://www.ncdot.org/doh/operations/materials/structural/appr\\_proc.html](http://www.ncdot.org/doh/operations/materials/structural/appr_proc.html). Use non-prequalified welds only if approved by the Engineer. Submit WPS for all non-prequalified welds to the

Engineer for approval. At no cost to the Department, demonstrate their adequacy in accordance with the requirements of the Bridge Welding Code.

**PORTABLE CONCRETE BARRIER**

(2-20-07)

M10 R50

The *2006 Metric Standard Specifications* is revised as follows:

**Page 10-200, Article 1090-1(A) General**, add the following after the first sentence:

The requirement for approved galvanized connectors will be waived if the barrier remains the property of the Contractor.

**CHANNELIZING DEVICES (Drums):**

7-20-10

M10 R60

Revise the *2006 Metric Standard Specifications* as follows:

**Page 10-192, Subarticle 1089-5(A) Drums (1) General**, replace the paragraph with the following:

(1) General

Provide drums composed of a body, alternating orange and white 4 band pattern of Type III-High Intensity Microprismatic Sheeting and ballasts that have been evaluated by NTPEP.

The following guidelines will be used during the transition from drums with the standard 5 band engineer's grade sheeting to the new 4 band configuration.

- (a) All **new** drums purchased **after July 20, 2010** shall have the new sheeting and 4 band configuration.
- (b) Existing 5 band drums with engineer's grade sheeting (both new and used devices in existing inventories) will be allowed for use on all on-going construction projects until project completion and will also be allowed for use on other projects until a sunset date has been established.
- (c) Intermixing of "old drums" and "new drums" on the same project is acceptable during the transition.
- (d) 4 band drums with engineer's grade sheeting will not be allowed at anytime.



**Page 10-192, Subarticle 1089-5(A) Drums (3) Retroreflective Stripes**, replace the paragraph with the following:

(3) Retroreflective Bands

Provide a minimum of 4 retroreflective bands- 2 orange and 2 white alternating horizontal circumferential bands. The top band shall always be orange. Use a 150mm to 200 mm wide band Type III–High Intensity Microprismatic Retroreflective Sheeting or better that meets the requirement of Section 1093 for each band. Do not exceed 50 mm for any non-reflective spaces between orange and white stripes. Do not splice the retroreflective sheeting to create the 150 mm band. Apply the retroreflective sheeting directly to the drum surface. Do not apply the retroreflective sheeting over a pre-existing layer of retroreflective sheeting. Do not place bands over any protruding corrugations areas. No damage to the reflective sheeting should result from stacking and unstacking the drums, or vehicle impact.

**Page 10-193, Subarticle 1089-5(B) Skinny-Drums (1) General**, replace the paragraph with the following:

(1) General

All existing skinny-drums that do not have Type III-High Intensity Microprismatic Sheeting as a minimum will have the same transition requirements as drums as stated above. All new skinny-drums purchased after July 20, 2010 shall have Type III–High Intensity Microprismatic Sheeting as the minimum. Type IV and higher grade sheeting is acceptable for use on both new and used devices.

Provide skinny-drums composed of a body, reflective bands, and ballasts that have been evaluated by NTPEP.

**Page 10-193, Subarticle 1089-5(B) Skinny Drums (3) Retroreflective Stripes**, replace the paragraph with the following:

(3) Retroreflective Bands

Provide a minimum of 4 retroreflective bands- 2 orange and 2 white alternating horizontal circumferential bands for each skinny-drum. The top band shall always be orange. Use a 150mm to 200 mm wide band Type III–High Intensity Microprismatic Retroreflective Sheeting or better that meets the requirement of Section 1093 for each band. Do not exceed 50 mm for any non-reflective spaces between orange and white stripes. Do not splice the retroreflective sheeting to create the 150 mm band. Apply the retroreflective sheeting directly to the skinny-drum surface. Do not apply the retroreflective sheeting over a pre-existing layer of retroreflective sheeting. Do not place bands over any protruding corrugations areas. No damage to the reflective sheeting should result from stacking and unstacking the skinny-drums, or vehicle impact.

**CHANGEABLE MESSAGE SIGNS**

(11-21-06)

M11 R11

Revise the *2006 Metric Standard Specifications* as follows:

**Page 11-7, Article 1120-3,** Replace the 3rd sentence with the following:

Sign operator will adjust flash rate so that no more than two messages will be displayed and be legible to a driver when approaching the sign at the posted speed.

**PAVEMENT MARKING LINES:**

(11-21-06) (Rev. 08-17-10)

M12 R01

Revise the *2006 Metric Standard Specifications* as follows:

**Page 12-2, 1205-3(D) Time Limitations for Replacement,** add the following at the beginning of the chart:

<b>Facility Type</b>	<b>Marking Type</b>	<b>Replacement Deadline</b>
Full-control-of-access multi-lane roadway (4 or more total lanes) and ramps, including Interstates	All markings including symbols	By the end of each workday's operation if the lane is opened to traffic

**Page 12-4, 1205-3 (H) Observation Period,** delete 1205-3 (H) and replace with the following:

Maintain responsibility for debonding and color of the pavement markings during a 12 month observation period beginning upon final acceptance of the project as defined under Article 105-17. Guarantee the markings under the payment and performance bond in accordance with Article 105-17.

During the 12 month observation period, provide pavement marking material that shows no signs of failure due to blistering, chipping, bleeding, discoloration, smearing or spreading under heat or poor adhesion to the pavement materials. Pavement markings that debond due to snowplowing will not be considered a failed marking. Replace, at no additional expense to the Department, any pavement markings that do not perform satisfactorily under traffic during the 12 month observation period.

**Page 12-6, 1205-4 (C) Application,** delete the last two sentences of the second paragraph and replace with the following:

Produce in place markings with minimum retroreflective values shown below, as obtained with a LTL 2000 Retroreflectometer or Department approved mobile retroreflectometer. Retroreflective measurements will be taken within 30 days after final placement of the pavement marking.

**Page 12-7, 1205-4 (D) Observation Period**, delete the entire section and replace with the following:

In addition to the requirements of Subarticle 1205-3(H), maintain responsibility for minimum retroreflective values for a 30-day period beginning upon the Engineer's acceptance of all markings on the project. Guarantee retroreflective values of the markings during the 30-day period under the payment and performance bond in accordance with Article 105-17.

**Page 12-8, 1205-5 (B) Application**, delete the second sentence of the fourth paragraph and replace with the following:

Produce in place markings with minimum retroreflective values shown below, as obtained with a LTL 2000 Retroreflectometer or Department approved mobile retroreflectometer. Retroreflective measurements will be taken within 30 days after final placement of the pavement marking.

**Page 12-8, 1205-5 (C) Observation Period**, delete this entire section and replace with the following:

Maintain responsibility for minimum retroreflective values for a 30-day period beginning upon satisfactory final placement of all markings on the project. Guarantee retroreflective values of the markings during the 30-day period under the payment and performance bond in accordance with Article 105-17.

**Page 12-11, Article 1205-9, Maintenance**, delete Article 1205-9 and replace with the following:

Replace pavement markings that prematurely deteriorate, fail to adhere to the pavement, lack reflectorization, or are otherwise unsatisfactory during the life of the project or during the 12 month observation period as determined by the Engineer at no cost to the Department.

Upon notification from the Engineer, winterize the project by placing an initial or additional application of paint pavement marking lines in accordance with Article 1205-8. Payment for *Paint Pavement Marking Lines* required to winterize the project will be made in accordance with Article 1205-10 except that no payment will be made on resurfacing projects where paving is completed more than 30 days prior to the written notification by the Department that winterization is required.

**Page 12-11, Article 1205-10, Measurement and Payment**, add the following after the first sentence of the first paragraph:

In addition, *Paint Pavement Marking Lines* will be paid per linear foot for each 15 mil application placed in accordance with Subarticle 1205-8(C).

**EXCAVATION, TRENCHING, PIPE LAYING AND BACKFILLING FOR UTILITIES:**

(2-17-09)

M15 R01

Revise the *2006 Metric Standard Specifications* as follows:

**Page 15-4, Article 1505-4 Repair of Pavements, Sidewalks and Driveways, first paragraph, add at the end of the first sentence**

in accordance with Section 848

**Page 15-5, Article 1505-6**

**Second paragraph,**

**Delete (E) Repair of Sidewalks and Driveways in its entirety**

**Add as the eighth paragraph:**

     mm Concrete Sidewalk and      mm Concrete Driveways will be measured and paid for in accordance with Article 848-4.

**PERMANENT SEEDING AND MULCHING:**

(7-1-95)

M16 R01

The Department desires that permanent seeding and mulching be established on this project as soon as practical after slopes or portions of slopes have been graded. As an incentive to obtain an early stand of vegetation on this project, the Contractor's attention is called to the following:

For all permanent seeding and mulching that is satisfactorily completed in accordance with the requirements of Section 1660, Seeding and Mulching, and within the following percentages of elapsed contract times, an additional payment will be made to the Contractor as an incentive additive. The incentive additive will be determined by multiplying the number of acres of seeding and mulching satisfactorily completed times the contract unit bid price per acre for Seeding and Mulching times the appropriate percentage additive.

<b>Percentage of Elapsed Contract Time</b>	<b>Percentage Additive</b>
0% - 30%	30%
30.01% - 50%	15%

Percentage of elapsed contract time is defined as the number of calendar days from the date of availability of the contract to the date the permanent seeding and mulching is acceptably completed divided by the total original contract time.

**FLAGGERS:**  
(2-15-11)

M11 R20

**Revise the 2006 Metric Standard Specifications as follows:**

**Page 11-10, Article 1150-3 Construction Methods, replace the article with the following:**

Provide the service of properly equipped and qualified flaggers (see *Roadway Standard Drawing* 1150.01) at locations and times for such period as necessary for the control and protection of vehicular and pedestrian traffic. Anyone who controls traffic is required to be qualified. Qualification consists of each flagger receiving proper training in the set-up and techniques of safely and competently performing a flagging operation. Qualification of flaggers is to be done at an NCDOT approved training agency. For a complete listing of these, see the Work Zone Traffic Control's webpage, <http://www.ncdot.gov/doh/preconstruct/wztc/>.

Prior to beginning work on the project, a Qualification Statement that all flaggers used on the project have been properly trained through an NCDOT approved training resource shall be provided to the Engineer.

Flagging operations are not allowed for the convenience of the Contractor's operations. However, if safety issues exist (i.e. sight/stopping site distance), the Engineer may approve the use of flagging operations. Use flagging methods that comply with the guidelines in the MUTCD.

**AUTOMATED MACHINE GUIDANCE**

01-02-11

**GENERAL**

This Special Provision contains requirements to be followed if the Contractor elects to use Global Positioning System (GPS) machine control grading and shall be used in conjunction with Section 801 of the Standard Specifications for Roads and Structures. The use of this technology is referenced as Automated Machine Guidance (AMG)

All equipment using AMG shall be able to generate end results that meet the Standard Specifications. Perform test sections for each type of work to be completed with AMG to demonstrate that the system has the capability to achieve acceptable results. If acceptable results can not be achieved, conform to the requirements for conventional stakeout.

The Contractor shall be responsible for all errors resulting from the use of AMG and shall correct deficiencies to the satisfaction of the Engineer at no cost to the Department.

**SUBMITTALS**

If the Contractor elects to use AMG, a Digital Terrain Model (DTM) of the design surface and all intermediate surfaces shall be developed and submitted to the Engineer for review.

At least 90 days prior to beginning grading operations, the Contractor shall submit to the Engineer an AMG work plan to include, but not limited to, proposed equipment, control

software manufacturer and version, types of work to be completed using AMG, project site calibration report, repetitive calibration methods for construction equipment and rover units to be used for the duration of the project, and local GPS base station to be used for broadcasting differential correction data to rover units (this may include the NC Network RTK). All surveys must be tied to existing project control as established by NCDOT.

## **INSPECTION**

The Engineer will perform quality assurance checks of all work associated with AMG. If it is determined that work is not being performed in a manner that will assure accurate results, the Engineer may require corrective action at no cost to the Department.

The Contractor shall provide the Engineer with one GPS rover unit for use during the duration of the contract. The rover will be loaded with the same model that is used with the AMG and have the same capability as rover units used by the Contractor. The rover will be kept in the possession of the Engineer and will be returned to the Contractor upon completion of the contract. Any maintenance or repairs required for the rover will be the responsibility of the Contractor. Formal training of at least 8 hours shall be provided to the Engineer by the Contractor on the use of the proposed AMG system.

## **SUBGRADE AND BASE CONTROLS**

If the Contractor elects to use AMG for fine grading and placement of base or other roadway materials, the GPS shall be supplemented with a laser or robotic total station. Include details of the proposed system in the AMG work plan. In addition, the following requirements apply for the use of AMG for subgrade and base construction.

1. Provide control points at intervals along the project not to exceed 1000 feet. The horizontal position of these points shall be determined by static GPS sessions or by traverse connection from the original base line control points. The elevation of these control points shall be established using differential leveling from project benchmarks, forming closed loops where practical. A copy of all new control point information shall be provided to the Engineer prior to construction activities.
2. Provide control points and conventional survey grade stakes at 500' intervals and at critical points such as, but not limited to, PCs, PTs, superelevation transition points, and other critical points as requested by the Engineer.
3. Provide hubs at the top of the finished subgrade at all hinge points on the cross section at 500 foot intervals. These hubs shall be established using conventional survey methods for use by the Engineer to check the accuracy of construction.

## **MEASUREMENT AND PAYMENT**

No direct payment will be made for work required to utilize this provision. All work will be considered incidental to various grading operations.