

PROJECT SPECIAL PROVISIONS



SCOPE OF WORK

Location and Description of Bridge

Beaufort County Bridge No. 28 is located on NC 92 over Bath Creek in Bath, NC. The bridge was built in 1959 and is approximately 1,052' long and consists of 48 spans of reinforced concrete deck slabs approximately 21' in length and a 40' main channel span with 5 lines of W30 I-Beams. The clear roadway width is 28' and the average daily traffic is approximately 3600 vehicles per day.

Description of Work

This work shall consist of furnishing all labor, materials and equipment to make repairs to deteriorated concrete caps and piles; including shotcrete or poured in place repairs to caps, encapsulating existing 12" square and 22" octagonal concrete piles with fiberglass reinforced polymer pile jackets with a pumped epoxy grout fill material; and installing reinforced concrete jackets on existing 12" square concrete piles as shown in the contract documents and plans. Contractor shall provide all necessary access; boats, barges, scaffolding, ladders, etc.; provide all traffic control (both vehicular and navigational); coordinate all navigation channel work with the US Coast Guard; provide all staging area, material storage, boat storage and boat access; provide environmental controls to limit loss of materials into water and air; sawing and chipping equipment, and cleaning equipment; and all else necessary to complete the work.

The contractor shall be responsible for fulfilling all requirements of the NCDOT Standard Specifications for Roads and Structures dated July 2006, except as otherwise specified herein.

COORDINATION WITH THE U. S. COAST GUARD

(SPECIAL)

The Contractor will not be allowed to begin any work associated with the channel span until a proposed work plan has been approved by the United States Coast Guard. The Contractor shall submit his proposed work plan to the Resident Engineer for forwarding to the Coast Guard for approval. These plans shall be submitted to the Engineer a minimum of one (1) month prior to the date the Contractor plans on beginning his operations in the channel span. The proposed work plan shall detail the change in vertical clearance for the channel span during the Contractor's time of operation in this area; detail the work to be performed and how said work will be performed; detail schedules (including begin and end dates) for the work on the channel span; and any other information that will help describe the work to the Coast Guard and assist in putting together the required Notice to Mariners that will be broadcast during the channel span work.

At no time during work will the waterway be closed or narrowed to navigation without prior approval from the Coast Guard. The contractor is required to maintain close and regular contact with the Coast Guard, Sector North Carolina to keep them informed to activities in the waterway with Steve Lyons at (252)-247-4525 or email Stephen.w.lyons2@uscg.mil. Also must contact

the 5th Coast Guard District with Bill Brazier at (757) 271-1016 or email at Bill.H.Brazier@uscg.mil.

All waterway narrowing or closures shall be requested in writing and shall be received by the District Commander of the Coast Guard at least 30 days in advance of the closure so that the appropriate marine notifications can be made.

All work shall be conducted so that free navigation of the waterway is not unreasonably interfered with and the present navigable depths are not impaired. Timely notice of any and all events that affect navigation shall be given to the District Commander during the work on the moveable span. The channel shall be promptly cleared of all obstructions placed therein or caused by the contractor.

SECURING OF VESSELS

(10-12-01)

Secure vessels in accordance with Section 107 of the Standard Specifications and the following provision:

When utilizing barges, tugboats or other vessels, take all necessary precautions to ensure that such vessels are securely anchored or moored when not in active operation. Take all necessary measures to ensure that the vessels are operated in a manner that avoids damage to or unnecessary contact with bridges and other highway structures and attachments. If severe weather conditions are anticipated, or should be anticipated through reasonable monitoring of weather forecasts, take additional measures to protect bridges and other highway structures and attachments from extreme conditions. The Contractor is strictly liable for damages to any bridge or other highway structure or attachment caused by a vessel owned or controlled by the Contractor. The Contractor is also liable to third parties for property damages and loss of revenue caused by vessels under the Contractor's control.

WORK IN, OVER OR ADJACENT TO NAVIGABLE WATERS:

(SPECIAL)

All work in, over, or adjacent to navigable waters shall be in accordance with the special provisions and conditions contained in the permits obtained by the Department from the U.S. Coast Guard, U.S. Army Corps of Engineers, or other authority having jurisdiction. The work shall have no adverse effect on navigation of the waterway including traffic flow, navigational depths, and horizontal and vertical clearances without approval from the authorities granting the permits.

The Contractor shall prepare drawings necessary to obtain any permits which may be required for his operations which are not included in the Department's permit including but not limited to excavation and dumping, constructing wharves, piers, ramps, and other structures connecting to bank or shore, and drawings for constructing falsework, cofferdams, sheeting, temporary bridges, and any other construction within the waterway. Submittals shall show locations of such work with respect to the navigational opening. The Contractor shall coordinate the submittal of drawings with the Engineer.

All construction shall progress and be maintained in a safe and timely manner. Temporary construction facilities shall be removed completely and promptly upon discontinuation of their useful purpose

The Contractor shall immediately notify the appropriate authorities and take corrective measures as needed when any situation occurs that imposes a threat to the public. He shall also immediately correct any acts or occurrences that contradict or violate any requirements in the plans, special provisions, or permits when corrective measures can be performed in a safe manner. The Contractor shall notify the appropriate authorities when such corrective measures cannot be performed in a safe manner.

All costs incurred by the Contractor in complying with the above requirements shall be included in the prices bid for the various pay items and no additional payment will be made.

SUBMITTAL OF WORKING DRAWINGS

SPECIAL

General

Submit working drawings in accordance with Article 105-2 of the *Standard Specifications* and this provision. For this provision, "submittals" refers to only those listed in this provision. The list of submittals contained herein does not represent a complete list of required submittals for the project. Submittals are only necessary for those items as required by the contract. **Make submittals that are not specifically noted in this provision directly to the Resident Engineer.**

If a submittal contains variations from plan details or specifications or significantly affects project cost, field construction or operations, discuss the submittal with and submit all copies to the Resident Engineer. State the reason for the proposed variation in the submittal. To minimize review time, make sure all submittals are complete when initially submitted. Provide a contact name and information with each submittal. Direct any questions regarding submittal requirements to the Resident Engineer or State Bridge Management Unit.

In order to facilitate in-plant inspection by NCDOT and approval of working drawings, provide the name, address and telephone number of the facility where fabrication will actually be done if different than shown on the title block of the submitted working drawings. This includes, but is not limited to, precast concrete items, prestressed concrete items and fabricated steel or aluminum items.

Addresses and Contacts

Mail submittals to:

Mr. Rick Nelson, PE
 Asst. State Bridge Management Engineer
 NC Dept. of Transportation
 State Bridge Management Unit
 4809 Beryl Drive
 Raleigh, NC 27606
 Fax: 919.733.2348
 Ph: 919.733.4362
 Email: enelson@ncdot.gov

Furnish one complete copy of each submittal, including all attachments, to the Resident Engineer. At the same time, submit the number of hard copies shown below of the same complete submittal directly to the State Bridge Management Unit and/or the Structure Design Unit.

The table below covers "Structure Submittals". The Resident Engineer will receive review comments and drawing markups for these submittals from the State Bridge Management Unit.

Unless otherwise required, submit one set of supporting calculations to either the State Bridge Management Unit or Structure Design Unit unless both units require submittal copies in which case submit a set of supporting calculations to each unit. Provide additional copies of any submittal as directed by the Engineer.

STRUCTURE SUBMITTALS

Submittal	Copies Required by SBMU	Copies Required by Structure Design Unit	Contract Reference Requiring Submittal ¹
Falsework & Forms ² (substructure)	5	0	Article 420-3 & "Falsework and Formwork"
Falsework & Forms (superstructure)	5	0	Article 420-3 & "Falsework and Formwork"
Placement of Equipment on Structures (cranes, blasting/painting equip., etc.)	5	0	Article 420-20
Painting Platforms and Containment	5	0	SP

FALSEWORK AND FORMWORK

(8-4-09)

Description

Use this Special Provision as a guide to develop temporary works submittals required by the Standard Specifications or other provisions; no additional submittals are required herein. Such temporary works include, but are not limited to, falsework and formwork.

Falsework is any temporary construction used to support the permanent structure until it becomes self-supporting. Formwork is the temporary structure or mold used to retain plastic or fluid concrete in its designated shape until it hardens. Access scaffolding is a temporary structure that functions as a work platform that supports construction personnel, materials, and tools, but is not intended to support the structure. Scaffolding systems that are used to temporarily support permanent structures (as opposed to functioning as work platforms) are considered to be falsework under the definitions given. Shoring is a component of falsework such as horizontal, vertical, or inclined support members. Where the term "temporary works" is used, it includes all of the temporary facilities used in bridge construction that do not become part of the permanent structure.

Design and construct safe and adequate temporary works that will support all loads imposed and provide the necessary rigidity to achieve the lines and grades shown on the plans in the final structure.

Materials

Select materials suitable for temporary works; however, select materials that also ensure the safety and quality required by the design assumptions. The Engineer has authority to reject material on the basis of its condition, inappropriate use, safety, or nonconformance with the plans. Clearly identify allowable loads or stresses for all materials or manufactured devices on the plans. Revise the plan and notify the Engineer if any change to materials or material strengths is required.

Design Requirements

Working Drawings:

Provide working drawings for items as specified in the contract, or as required by the Engineer, with design calculations and supporting data in sufficient detail to permit a structural and safety review of the proposed design of the temporary work.

When concrete placement is involved, include data such as the drawings of proposed sequence, rate of placement, direction of placement, and location of all construction joints. Submit the number of copies as called for by the contract.

When required, have the drawings and calculations prepared under the guidance of, and sealed by, a North Carolina Registered Professional Engineer who is knowledgeable in temporary works design.

Design falsework and formwork requiring submittals in accordance with the 1995 AASHTO Guide Design Specifications for Bridge Temporary Works except as noted herein.

Wind Loads:

Table 2.2 of Article 2.2.5.1 is modified to include wind velocities up to 110 mph (177 km/hr). In addition, Table 2.2A is included to provide the maximum wind speeds by county in North Carolina.

Table 2.2 - Wind Pressure Values

Height Zone feet (m) above ground	Pressure, lb/ft ² (kPa) for Indicated Wind Velocity, mph (km/hr)				
	70 (112.7)	80 (128.7)	90 (144.8)	100 (160.9)	110 (177.0)
0 to 30 (0 to 9.1)	15 (0.72)	20 (0.96)	25 (1.20)	30 (1.44)	35 (1.68)
30 to 50 (9.1 to 15.2)	20 (0.96)	25 (1.20)	30 (1.44)	35 (1.68)	40 (1.92)
50 to 100 (15.2 to 30.5)	25 (1.20)	30 (1.44)	35 (1.68)	40 (1.92)	45 (2.15)
over 100 (30.5)	30 (1.44)	35 (1.68)	40 (1.92)	45 (2.15)	50 (2.39)

Time of Removal:

The following requirements replace those of Article 3.4.8.2.

Do not remove forms until the concrete has attained strengths required in Article 420-16 of the Standard Specifications and these Special Provisions.

Do not remove forms until the concrete has sufficient strength to prevent damage to the surface.

Table 2.2A - Steady State Maximum Wind Speeds by Counties in North Carolina

COUNTY	25 YR (mph) (km/hr)	COUNTY	25 YR (mph) (km/hr)	COUNTY	25 YR (mph) (km/hr)
Alamance	70 (112.7)	Franklin	70 (112.7)	Pamlico	100 (160.9)
Alexander	70 (112.7)	Gaston	70 (112.7)	Pasquotank	100 (160.9)
Alleghany	70 (112.7)	Gates	90 (144.8)	Pender	100 (160.9)
Anson	70 (112.7)	Graham	80 (128.7)	Perquimans	100 (160.9)
Ashe	70 (112.7)	Granville	70 (112.7)	Person	70 (112.7)
Avery	70 (112.7)	Greene	80 (128.7)	Pitt	90 (144.8)
Beaufort	100 (160.9)	Guilford	70 (112.7)	Polk	80 (128.7)
Bertie	90 (144.8)	Halifax	80 (128.7)	Randolph	70 (112.7)
Bladen	90 (144.8)	Harnett	70 (112.7)	Richmond	70 (112.7)
Brunswick	100 (160.9)	Haywood	80 (128.7)	Robeson	80 (128.7)
Buncombe	80 (128.7)	Henderson	80 (128.7)	Rockingham	70 (112.7)
Burke	70 (112.7)	Hertford	90 (144.8)	Rowan	70 (112.7)
Cabarrus	70 (112.7)	Hoke	70 (112.7)	Rutherford	70 (112.7)
Caldwell	70 (112.7)	Hyde	110 (177.0)	Sampson	90 (144.8)
Camden	100 (160.9)	Iredell	70 (112.7)	Scotland	70 (112.7)
Carteret	110 (177.0)	Jackson	80 (128.7)	Stanley	70 (112.7)
Caswell	70 (112.7)	Johnston	80 (128.7)	Stokes	70 (112.7)
Catawba	70 (112.7)	Jones	100 (160.9)	Surry	70 (112.7)
Cherokee	80 (128.7)	Lee	70 (112.7)	Swain	80 (128.7)
Chatham	70 (112.7)	Lenoir	90 (144.8)	Transylvania	80 (128.7)
Chowan	90 (144.8)	Lincoln	70 (112.7)	Tyrell	100 (160.9)
Clay	80 (128.7)	Macon	80 (128.7)	Union	70 (112.7)
Cleveland	70 (112.7)	Madison	80 (128.7)	Vance	70 (112.7)
Columbus	90 (144.8)	Martin	90 (144.8)	Wake	70 (112.7)
Craven	100 (160.9)	McDowell	70 (112.7)	Warren	70 (112.7)
Cumberland	80 (128.7)	Mecklenburg	70 (112.7)	Washington	100 (160.9)
Currituck	100 (160.9)	Mitchell	70 (112.7)	Watauga	70 (112.7)
Dare	110 (177.0)	Montgomery	70 (112.7)	Wayne	80 (128.7)
Davidson	70 (112.7)	Moore	70 (112.7)	Wilkes	70 (112.7)
Davie	70 (112.7)	Nash	80 (128.7)	Wilson	80 (128.7)
Duplin	90 (144.8)	New Hanover	100 (160.9)	Yadkin	70 (112.7)
Durham	70 (112.7)	Northampton	80 (128.7)	Yancey	70 (112.7)
Edgecombe	80 (128.7)	Onslow	100 (160.9)		
Forsyth	70 (112.7)	Orange	70 (112.7)		

Note on the working drawings any anchorages, connectors, inserts, steel sleeves or other such devices used as part of the falsework or formwork that remains in the permanent structure. If the plan notes indicate that the structure contains the necessary corrosion protection required for a Corrosive Site, epoxy coat, galvanize, metallize or otherwise protect these devices as directed by the Engineer. Any coating required by the Engineer will be considered incidental to the various pay items requiring temporary works.

Review and Approval

The Engineer is responsible for the review and approval of temporary works' drawings. Submit the working drawings sufficiently in advance of proposed use to allow for their review, revision (if needed), and approval without delay to the work.

Do not start construction of any temporary work for which working drawings are required until the drawings have been approved. Such approval does not relieve the Contractor of the responsibility for the accuracy and adequacy of the working drawings.

The time period for review of the working drawings does not begin until complete drawings and design calculations, when required, are received by the Engineer.

On the drawings, show all information necessary to allow the design of any component to be checked independently as determined by the Engineer.

If requested by the Engineer, submit with the working drawings manufacturer's catalog data listing the weight of all construction equipment that will be supported on the temporary work. Show anticipated total settlements and/or deflections of falsework and forms on the working drawings. Include falsework footing settlements, joint take-up, and deflection of beams or girders. Falsework hangers that support concentrated loads and are installed at the edge of thin top flange concrete girders (such as bulb tee girders) shall be spaced so as not to exceed 75% of the manufacturer's stated safe working load. Use of dual leg hangers (such as Meadow Burke HF-42 and HF-43) are not allowed. Design the falsework and forms supporting deck slabs and overhangs on girder bridges so that there will be no differential settlement between the girders and the deck forms during placement of deck concrete.

Construction Requirements

All requirements of Section 420 of the Standard Specifications apply.

Construct temporary works in conformance with the approved working drawings. Ensure that the quality of materials and workmanship employed is consistent with that assumed in the design of the temporary works. Do not weld falsework members to any portion of the permanent structure unless approved. Show any welding to the permanent structure on the approved construction drawings.

Provide tell-tales attached to the forms and extending to the ground, or other means, for accurate measurement of falsework settlement. Make sure that the anticipated compressive settlement and/or deflection of falsework does not exceed 1 inch (25 mm). For cast-in-place concrete structures, make sure that the calculated deflection of falsework flexural members does not

exceed 1/240 of their span regardless of whether or not the deflection is compensated by camber strips.

Maintenance and Inspection:

Inspect and maintain the temporary work in an acceptable condition throughout the period of its use. Certify that the manufactured devices have been maintained in a condition to allow them to safely carry their rated loads. Clearly mark each piece so that its capacity can be readily determined at the job site.

Perform an in-depth inspection of an applicable portion(s) of the temporary works, in the presence of the Engineer, not more than 24 hours prior to the beginning of each concrete placement. Inspect other temporary works at least once a month to ensure that they are functioning properly. Have a North Carolina Registered Professional Engineer inspect the cofferdams, shoring, sheathing, support of excavation structures, and support systems for load tests prior to loading.

Foundations:

Determine the safe bearing capacity of the foundation material on which the supports for temporary works rest. If required by the Engineer, conduct load tests to verify proposed bearing capacity values that are marginal or in other high-risk situations.

The use of the foundation support values shown on the contract plans of the permanent structure is permitted if the foundations are on the same level and on the same soil as those of the permanent structure.

Allow for adequate site drainage or soil protection to prevent soil saturation and washout of the soil supporting the temporary works supports.

If piles are used, the estimation of capacities and later confirmation during construction using standard procedures based on the driving characteristics of the pile is permitted. If preferred, use load tests to confirm the estimated capacities; or, if required by the Engineer conduct load tests to verify bearing capacity values that are marginal or in other high risk situations.

The Engineer reviews and approves the proposed pile and soil bearing capacities.

Removal

Unless otherwise permitted, remove and keep all temporary works upon completion of the work. Do not disturb or otherwise damage the finished work.

Remove temporary works in conformance with the contract documents. Remove them in such a manner as to permit the structure to uniformly and gradually take the stresses due to its own weight.

Method of Measurement

Unless otherwise specified, temporary works will not be directly measured.

Basis of Payment

Payment at the contract unit prices for the various pay items requiring temporary works will be full compensation for the above falsework and formwork.

ADHESIVELY ANCHORED ANCHOR BOLTS OR DOWELS

(6-11-07)

General

Installation and Testing of Adhesively anchored anchor bolts and dowels shall be in accordance with Section 420-13, 420-21 and 1081-1 of the Standard Specifications except as modified in this provision.

Installation

Installation of the adhesive anchors shall be in accordance with manufacturer's recommendations and shall occur when the concrete is above 40 degrees Fahrenheit and has reached its 28 day strength.

The anchors shall be installed before the adhesive's initial set ('gel time').

Field Testing

Replace the third paragraph of Section 420-13 (C) with the following:

"In the presence of the Engineer, field test the anchor bolt or dowel in accordance with the test level shown on the plans and the following:

Level One Field testing: Test a minimum of 1 anchor but not less than 10% of all anchors to 50% of the yield load shown on the plans. If less than 60 anchors are to be installed, install and test the required number of anchors prior to installing the remaining anchors. If more than 60 anchors are to be installed, test the first 6 anchors prior to installing the remaining anchors, then test 10% of the number in excess of 60 anchors.

Level Two Field testing: Test a minimum of 2 anchors but not less than 10% of the all anchors to 80% of the yield load shown on the plans. If less than 60 anchors are to be installed, install and test the required number of anchors prior to installing the remaining anchors. If more than 60 anchors are to be installed, test the first 6 anchors prior to installing the remaining anchors, then test 10% of the number in excess of 60 anchors.

Testing should begin only after the Manufacturer's recommended cure time has been reached. For testing, apply and hold the test load for three minutes. If the jack experiences any drop in gage reading, the test must be restarted. For the anchor to be deemed satisfactory, the test load must be held for three minutes with no movement or drop in gage reading."

Removal and Replacement of Failed Test specimens:

Remove all anchors and dowels that fail the field test without damage to the surrounding concrete. Redrill holes to remove adhesive bonding material residue and clean the hole in accordance with specifications. For reinstalling replacement anchors or dowels, follow the same procedures as new installations. Do not reuse failed anchors or dowels unless approved by the Engineer.

Usage

The use of adhesive anchors for overhead installments is not permitted without written permission from the Engineer.

Basis of Payment

No separate measurement or payment will be made for furnishing, installing, and testing anchor bolts/dowels. Payment at the contract unit prices for the various pay items will be full compensation for all materials, equipment, tools, labor, and incidentals necessary to complete the work.

CONCRETE REPAIRS TO CAPS

SPECIAL

Description

Work includes removal of concrete in spalled areas of the existing caps in reasonably close conformity with the lines, depth, and details shown on the plans, described herein and as established by the Engineer. This work also includes straightening, cleaning, and replacement of reinforcing steel, dowelling new reinforcing steel, removing all loose materials, removing and disposing of debris, applying repair material, and protecting adjacent areas of the bridge and environment from material leakage. The repair material shall be one of the below described materials unless otherwise noted in the plans or provisions.

The location and extent of repairs shown on the plans described herein are general in nature. The Engineer determines the exact extent of removal in the field based on an evaluation of the condition of the exposed surfaces.

Repair, to the Engineer's satisfaction, any portion of the structure that is damaged from construction operations. No extra payment is provided for these repairs.

A. Polymer Modified Concrete Repair Material

Materials

Repair material shall be polymer modified cement mortar for vertical or overhead applications and shall be suitable for applications in marine environments. Material shall be approved for use by NCDOT. Submit repair material to the Engineer for review and approval prior to beginning the work. Color of repair material shall be concrete gray.

Surface Preparation

Prior to the application of repair mortar, square up edges in repair areas, thoroughly clean surfaces to be repaired and remove all loose materials. Remove grease, wax, salt, and oil contaminants by scrubbing with an industrial grade detergent or degreasing compound followed by a mechanical cleaning. Remove weak or deteriorated concrete to sound concrete by bush hammering, gritblasting, scarifying, waterblasting, or other approved methods. Remove dirt, dust, laitance and curing compounds by gritblasting, sanding, or etching with 15% hydrochloric acid. Only acid etch if approved and follow it by scrubbing and flushing with copious amounts of clean water. Check the cleaning using moist pH paper. Water cleaning is complete when the paper reads 10 or higher. Follow all mechanical cleaning with vacuum cleaning.

Application

When surface preparation is completed, mix and apply repair mortar in accordance with manufacturer's recommendations. Use aggregate that is washed, kiln-dried, and bagged. Apply

bonding agent to all repair areas immediately prior to placing repair mortar. Repair areas shall be formed unless otherwise approved by the Engineer. Form areas to establish the original neat lines of the member being repaired.

Apply repair mortar to damp surfaces only when approved. In such instances, remove all free water by air-blasting. After applying the repair mortar, remove excessive material and provide a smooth, flush surface.

B. Shotcrete Repair Material

Shotcrete

Qualification of Shotcrete Contractor

Shotcrete Contractors are not acceptable as a Prime Contractor or Subcontractor unless all of following requirements are met:

The Shotcrete Contractor furnishes proof that his or her company has a minimum of 5 years experience in shotcrete repair work on jobs of similar size and character.

The Shotcrete Contractor furnishes five references who were responsible for supervision of similar projects and testifies to the successful completion of these projects. Include name, address, and telephone number. Prior to starting work, the Contractor's nozzle men are required to pass a test demonstrating their competence. This test is conducted at the job site and approximates actual working conditions as near as possible. For test requirements, see ACI 506.3R, Chapters 2.5 and 3. Only workmanship demonstration is tested.

General

When shotcreting, meet all requirements of ACI 506.2, published by the American Concrete Institute, Detroit, Michigan, except as modified by the requirements of this Special Provision.

Prior to beginning any repair work, provide a sufficiently sized temporary work platform at each repair location as required. Design steel members meeting the requirements of the American Institute of Steel Construction Manual. Design timber members in accordance with the "National Design Specification for Stress-Grade Lumber and Its Fastenings" of the National Forest Products Association. Submit the platform structure design for review and approval. Do not install the platform until the design is approved. Do not drill holes into the superstructure. When the platform is removed, remove all anchorages made in the substructure and repair the substructure at no additional cost to the Department.

Material

Use materials conforming to the requirements of the applicable sections of the Standard Specifications and the following provisions:

Use Type II Cement. Replace ten percent by weight of the cement with silica fume. Do not use admixtures without approval.

Produce shotcrete cores with a compressive strength of 5000 psi (34.5 MPa) at 28 days. The provisions of ACI 506.2, Section 1.6.3.3, Paragraph 2, do not apply.

Submit the shotcrete mix design, including the source of the material, to the Engineer for acceptance before using it.

Use size 2S or 2MS fine aggregate unless otherwise approved.

Finish

Slightly build up and trim the shotcrete surface to the final surface by cutting with the leading edge of a sharp trowel. Use a rubber float to float any imperfections. Limit work on the finished surface to correcting imperfections caused by trowel cutting.

Testing

Each day shotcreting takes place, have each nozzleman shoot one 18" x 18" x 3" (460 mm x 460 mm x 75 mm) Test Panel. Shoot the panel in the same position as the repair work that is being done. The panel demonstrates whether the shotcrete is being properly applied and furnishes cores for testing compressive strength. Drill three 3" (76 mm) diameter cores from each test panel and also drill cores from the repair areas as directed by the Engineer. Do not take cores from repaired areas until the shotcrete has cured for 7 days. Drill a core that penetrates into the existing substructure concrete at least 2 inches (50 mm). These cores are inspected for delaminations and sand pockets and tested for bond strength and/or compressive strength. If a core taken from a repaired area indicates unsatisfactory application or performance of the shotcrete, take additional cores from the applicable repair area(s) for additional evaluation and testing as directed by the Engineer. No extra payment is provided for drilling extra cores. Patch all core holes in the repaired substructure units to the satisfaction of the Engineer.

All material, sample, and core testing is done by the Materials and Tests Unit of North Carolina Department of Transportation.

Mixture

Mix the shotcrete in the proportions of one part of portland cement to four parts of sand, and as directed by the Engineer.

Measure this mixture by volume in the dry loose state. Check batching equipment daily or at the discretion of the Engineer.

Repair Method and Operations

Prior to starting the repair operation, delineate all surfaces and areas assumed to be deteriorated by visually examining and by sounding the concrete surface with a hammer or any other alternative approved method. The Engineer is the sole judge in determining the limits of deterioration.

Remove all deteriorated concrete to sound concrete with a 17 lb (7.7 kg) (maximum) pneumatic hammer with points that do not exceed the width of the shank or with hand picks or chisels as directed by the Engineer. Do not cut or remove the existing reinforcing steel. Do not remove more existing concrete than required to expose the surface of the sound concrete. Unless specifically

directed by the Engineer, do not remove concrete deeper than 6 inches (150 mm) or deeper than 1 inch (25 mm) below the reinforcing steel.

If sound concrete is encountered before existing reinforcing steel is exposed, prepare and repair the surface without removing any more concrete. However, if the reinforcing steel is wholly or partially exposed, remove the deteriorated and/or sound concrete to a minimum clearance of 1 inch (25 mm) all around the reinforcing steel.

Sandblast all exposed concrete surfaces and existing reinforcing steel in repair areas to remove all debris, loose concrete, loose mortar, rust, scale, etc. Use a wire brush to clean all exposed reinforcing steel surfaces facing away from the sandblast nozzle to remove all dust and loose particles.

All material removed becomes the Contractor's. Use an approved method to dispose of the material.

Restore all repaired members, including chamfered edges, as close as practicable to their original "As Built" dimensions and configuration. Provide a minimum of 2" (50 mm) shotcrete cover over reinforcing steel exposed during repair. Finish the shotcrete by cutting the surface to final grade with the leading edge of a trowel.

Provide welded wire fabric at each repair area larger than 1 ft² (0.1 m²). Provide a minimum 2" x 2" (50 mm x 50 mm) - 12 gage galvanized welded wire fabric. Rigidly secure the welded wire fabric to existing steel or to 3/16" (4.76 mm) minimum diameter adequately spaced galvanized hook fasteners to prevent sagging. Encase the welded wire fabric in shotcrete to a minimum depth of 1½ inches (38 mm).

If preferred, use steel or synthetic fiber reinforcement as an alternate to welded wire fabric. Work only with experienced personnel. Always work under the direction of an experienced superintendent. The superintendent is required to show a certified experience record indicating at least 5 years experience on work of similar type. No nozzleman is deemed experienced unless they have worked on several other jobs similar to that specified herein and have passed the required pre-qualification test listed in this Special Provision.

Before applying the shotcrete to the surface, thoroughly clean the surface of all dirt, grease, oil or foreign matter, and remove all loose or weakened material.

Wash the roughened existing concrete surface with fresh potable water and an air blast, or with a "stiff" hose stream of fresh water until all loosened materials and salt water spray are removed. Perform this operation 30 minutes to 1 hour prior to applying the shotcrete. Maximum time allowed between removal of deteriorated concrete and shotcrete application is 5 days. If the time allowance is exceeded it will be necessary to prepare the surface again using the methods described above before shotcrete can be applied.

Apply shotcrete in layers. The properties of the applied shotcrete determine the proper thickness of each layer or lift.

If a work stoppage longer than 2 hours takes place on any shotcrete layer prior to the time it has been built up to required thickness, thoroughly wash the surface with a fresh water stream and air hose as outlined previously, prior to continuing with the remaining shotcrete course. Do not apply shotcrete to a dry surface.

Have the nozzleman hold the nozzle 3 – 4 feet (0.9 to 1.2 m) from the surface being covered in a position that ensures the stream of flowing material strikes at approximately right angles to the surface being covered without excessive impact. Have the nozzleman control the water content so it never exceeds 3½ gallons (13.25 liters) per sack of cement. Direct the nozzlemen to maintain the water at a practicable minimum, dependent on weather conditions, so that the mix properly adheres. Control water content so that it does not become high enough to cause the mix to sag or fall from vertical or inclined surfaces, or to separate in horizontal layers.

Use shooting strips or guide wires that do not entrap rebound sand to bring the finished work to approximate shape. Use guide wires to provide a positive means of checking the total thickness of the shotcrete applied. Remove the guide wires prior to the final finish coat.

Blow or rake off sand that rebounds and does not fall clear of the work, or which collects in pockets in the work, to avoid leaving sand pockets in the shotcrete. Do not reuse rebound material in the work.

Apply shotcrete only when the air temperature is at least 40°F (4°C) and rising, but less than 95°F (35°C). Do not apply shotcrete to frosted surfaces. Maintain shotcrete at a minimum temperature of 40°F (4°C) for 3 days.

Testing Shotcrete Surfaces

Immediately after bringing shotcrete surfaces to final thickness, thoroughly check them for sags, bridging, and other deficiencies. Approximately 3 days after completing the final shotcrete placement, thoroughly test it again with a hammer. At this time, the shotcrete should have sufficient strength for all sound sections to ring sharply. Remove and replace any unsound portions of the work found during this 3 day old inspection period, or at any other time prior to the final inspection of the work. No additional compensation is provided for removal and replacement of concrete during or after the 3 day old inspection.

Curing

Begin curing as soon as the finished shotcrete surface withstands the curing operation without damage in accordance with Section 3.7 of ACI 506.2.

Measurement and Payment

Concrete Cap Repair will be measured and paid for at the contract unit price bid per cubic foot and will be full compensation for removal, containment and disposal off-site of unsound concrete including the cost of materials, labor, tools, equipment and incidentals necessary to accomplish removal. Depth will be measured from a place at the original outside concrete face. The Contractor and Engineer will measure repair quantities after removal of unsound concrete and before application of repair material. Such payment will also include the cost of sandblasting, surface cleaning and preparation, cleaning of reinforcing steel, cost of temporary work platform,

testing of the soundness of the exposed concrete surface, furnishing and installation of repair mortar material, curing and sampling of concrete, and protection/cleaning of adjacent areas from splatter or leakage.

Reinforcing Steel that is required for the repairs will be incidental to work of *Concrete Cap Repair*.

Payment will be made under:

Pay Item	Pay Unit
Concrete Cap Repair	Cubic Feet

PILE ENCAPSULATION

SPECIAL

Description

The work specified in this section consists of surface preparation of the pile, placement of a translucent, fiberglass reinforced plastic (FRP) jacket around the pile and injecting a water insensitive epoxy grout into the space between the jacket and the pile. The epoxy grout is batched, mixed and pumped by equipment, expressly designed for that purpose.

Materials

FRP Outer Jacket

The FRP Outer Jacket shall be Translucent FRP Jacket, as described in this section. For a submission to be approved it must meet ALL requirements of this section and approved by the engineer prior to the bid.

The translucent outer jacket shall be a marine grade laminate of fiberglass reinforced plastic (FRP), constructed of layers of woven roving and mat. Construction by the spray-up process, using a chopper gun, is not acceptable. The glass content shall be sufficient to meet the strength requirements found in Section 3.1.6, herein, but shall not be less than 30% of the laminate. An Ultra-Violet (UV) screening ingredient shall be integrally bound within the polyester matrix.

The strength and thickness of the outer jacket shall be as required to provide adequate strength and rigidity to withstand the forces and stresses it may be subjected to during handling, installation and the injection of epoxy grout, but shall not be less than 1/8 inch (3 mm) thick.

The outer jacket shall be translucent to the extent that the progression of epoxy grout inside the jacket during injection can be visually monitored from outside the jacket.

The outer jacket shall be equipped with 1" NPT injection ports, spaced at intervals not to exceed five (5) feet, along its entire length. The injection ports shall be positioned on alternately opposite sides of the jacket to allow for more even distribution of grout. The injection ports shall be of all-polymer construction and be fitted into the jacket wall prior to jacket installation, except in special situations, approved by the engineer, where a port may be added to accommodate an unanticipated jobsite condition.

The outer jacket shall have a sufficient number of polymer stand-offs, adhered to its inside surface, to maintain a minimum space between the pile and the jacket of 3/8 inch (9.5 mm). When loss of pile section exists, it may be necessary to use adjustable stand-offs to keep the outer jacket in proper alignment with the pile. At an adjustable stand-off location, a polymer boss shall be adhered to the inside surface of the jacket to provide adequate thread length to accommodate the adjustable polymer screw.

The outer jacket material, exclusive of polymer stand-offs and injection ports, shall possess the following minimum physical properties.

1. Ultimate Tensile Strength per ASTM D-638: 10,000 PSI
2. IZOD Impact Strength per ASTM D-256: 15 ft-lbf/inch. (Notched Sample)
3. Barcol Hardness per ASTM D-2583: 30
4. Water Absorption per ASTM D-570: 1% Maximum
5. Ultra Violet (UV) Stability as demonstrated by Accelerated Weathering Tests per ASTM G-23: Samples of outer jacket subjected to 500 hour exposure in Twin Carbon Arc Weather-ometer (ASTM G-23, Type D) operated at 145 degrees F., shall not exhibit any chipping, flaking or peeling. Said test to be conducted in twenty (20) minute cycles, consisting of seventeen (17) minutes of arc light and three (3) minutes of water spray, throughout the 500 hour test duration.

The outer jacket shall be fabricated in sections. Each section shall not contain more than two (2) longitudinal joints. Sections of jacket may be placed one above the other and joined together with transverse joints. All joints in the outer jacket shall meet the following minimum requirements:

1. All joints shall have sufficient strength to assure that they will not open or separate when subjected to installation stresses, sea forces and epoxy grout injection pressures.
2. The longitudinal joint design shall be of overlapping configuration and shall allow for minor field adjustment to pile size. The design of all joints shall ensure that a minimum 3/8 inch annulus between jacket and pile is maintained.
3. Transverse joints (if any) shall be of overlapping configuration.

The lower end of each outer jacket shall be provided with a molded upset cavity to properly receive and contain a bottom seal gasket.

Epoxy Grout

The Epoxy Grout must meet ALL requirements of this section and approved by the Engineer.

The epoxy grout shall be a manufactured, prepackaged, solvent-free, underwater curing, three component product, consisting of epoxy resin (component A), epoxy hardener (component B) and graded dry silica aggregate (component C). The ratio of the epoxy components A and B (collectively called the binder) shall be 1:1 by volume. The A and B components shall be of sharply contrasting colors, as supplied to the project, to minimize error in field proportioning and to assist in evaluating thoroughness of mixing. The grout shall be proportioned to meet the

handling and placement requirements of this specification and the ratio of the filler to binder shall not exceed 3.5:1, by weight.

The mixed epoxy grout shall exhibit the following characteristics in the plastic state:

1. Viscosity of filled resin and filled curing agent shall be such that it may be pumped without segregation and be inject able into the space between the jacket and the pile without causing distortion or rupture of the jacket. The viscosity shall also be such that the blended grout completely fills the space between jacket and pile without voids and be reasonably self-leveling, once placed within the jacket.
2. The gel time or "Pot Life" of the blended grout shall be suitable for proper placement without voids, and allow sufficient time for reasonable self leveling within the jacket, yet in no case shall exceed 65 minutes after blending at a control temperature of 77 degrees F. (This requirement minimizes the possibility of the filler settling out of the liquid components.)
3. The blended grout shall be uniform in color and not contain any pockets or streaks of the original component colors.

The catalyzed Epoxy Grout, after curing under water, shall possess the following minimum physical properties in the hardened state.

1. 7 Day Compressive Strength per ASTM C-579: 7,000 PSI
2. 7 day Tensile Strength per ASTM C-307: 2,000 PSI
3. 7 day Bond/Shear Strength per ASTM C-882: 150 PSI
4. Shrinkage after 7 day's cure per ASTM C-531: 0.07% (Maximum)
5. Water Absorption after 7 day's cure per ASTM C-413: 0.45% (Maximum)

Marine Epoxy Pastes

The epoxy paste used to adhere the outer jacket seams and bottom seal gaskets, shall be a two component epoxy compound, capable of being applied underwater. The ratio of resin component to hardener component shall be 1:1 by volume and each component shall be of sharply contrasting color (e.g. black and white) to the other, to assist in evaluating the thoroughness of jobsite mixing.

The epoxy paste used to finish the tops of the encapsulations and to seal any in-situ bond test locations, shall be a non-sag, two component epoxy compound, capable of being applied underwater. The ratio of resin component to hardener component shall be 1:1 by volume and each component shall be of sharply contrasting color (e.g. black and white) to the other, to assist in evaluating the thoroughness of jobsite mixing.

Epoxy Grout Hose Lubricant shall be approved by the manufacturer of the epoxy grout. The lubricant must be an epoxy diluent, compatible with the chemistry of the epoxy grout used.

Equipment

The epoxy grout to be injected into the outer jackets shall be proportioned, mixed and pumped with equipment expressly designed for that purpose. The equipment shall be capable of delivering mixed grout into the jackets at the rate of 2 GPM or greater.

Temperature Control Equipment

When ambient and/or water temperatures are expected to fall below 70 degrees F., a source of heated water, such as a diver's water heater, shall be provided. The heated water shall be directed into water jackets surrounding the epoxy grout hoppers and injection hose(s). This equipment shall be capable of delivering a sufficient amount of heated water to maintain grout viscosity suitable for proper grout placement.

Materials Handling and Storage

Handling and storage of pile encapsulation materials shall strictly conform to the manufacturer's recommendations. A list of minimum handling and storage requirements follows:

Outer Jackets

Outer jackets shall be shipped in closed containers or covered with tarpaulins to prevent contamination by dirt or road films. Outer jackets shall be properly stored at the jobsite to minimize distortion and to prevent contamination by foot traffic and blown debris. If storage at project is to exceed 30 days, shaded storage shall be provided.

Epoxy Grout Components

The silica aggregate component of the epoxy grout shall be properly packaged and labeled to indicate point of origin and manufacturer's lot number. The aggregate shall be stored to assure that it is thoroughly dry when mixed in the epoxy grout.

All liquid epoxy components to be used in the work shall be delivered to the jobsite in tightly sealed unopened containers, clearly labeled to indicate:

Name of manufacturer.

Manufacturer's product name and component designation.

Manufacturer's lot number and "Use before" date.

ANSI (American National Standards institute) hazardous material rating and handling precautions.

Epoxy liquid epoxy components shall be stored in a covered, well ventilated space. The storage temperature of the liquid components shall not exceed 120 degrees F nor be less than 40 degrees F at any time after receipt by the contractor. (See Epoxy Grout Preparation)

Containers containing liquid epoxy components shall always be sealed and air tight from time of receipt by contractor until entering the proportioning and blending process. When containers are opened for sampling or other purposes and containers remain partially filled, their lids will be tightly closed to prevent contamination by moisture or other substances. After the seal has been broken on a container, its contents must be used within seven (7) days or removed from the project.

All project personnel handling the epoxy grout or its liquid components shall be properly alerted to the Epoxy Safety Requirements supplied by the manufacturer. A Material Safety Data Sheet (MSDS) shall be supplied with each shipment of liquid epoxy materials.

Submittals

Submit shop drawings and calculations to the Engineer for approval prior to start of fabrication. Submittal shall include:

1. Top and bottom elevations relative to project datum of each outer jacket to be installed.
2. Details and locations of typical longitudinal and transverse joints in the outer jackets, including a description of the joint sealing method(s).
3. Details of fixed and/or adjustable stand-offs and their location on the outer jackets.
4. Detail of typical outer jacket bottom seal.
5. Location and details of temporary bracing and outer jacket support required during placement and curing of epoxy grout.
6. Details of injection ports or other access points into outer jacket to facilitate placement of epoxy grout.
7. Details of installation sequence to be used to place the epoxy grout in the space between jacket and pile.
8. Detail of final finishing of epoxy grout at the top of the encapsulation.
9. Details of permanent closure of all injection ports and test locations in the outer jacket to be accomplished after epoxy grout placement is complete.

Material Certification

For materials to be used, the Supplier shall furnish a certificate to the Engineer attesting that the materials meet all the requirements contained herein and that they conform in all respects to the materials subjected to the tests required. Copies of current test reports shall be attached to the certificate. No test report for tests made more than one year prior to shipment will be accepted for the form material.

Construction Methods

Pile Cleaning

Prior to application of the encapsulation process, all pile surfaces shall be thoroughly cleaned of marine growth, oil, grease, mud, rust, broken concrete, micro-organisms and any other deleterious material which might prevent proper bonding between the epoxy grout and the pile. Pile cleaning may be accomplished by grit blasting, water blasting, or by powered rotary abraders, and shall meet the satisfaction of the Engineer.

In environments where active marine growth occurs, it may be necessary to perform the pile cleaning in two (2) phases. In such environments, the first phase shall consist of removing marine growth, oil, grease, rust, broken concrete, etc., and shall occur not more than seven (7) days prior to the encapsulation. The second phase shall be a final surface preparation, removing all remaining deleterious substances including micro-organisms and shall occur not more than 48 hours prior to the placement of the epoxy grout in the outer pile jacket.

Outer Jacket Assembly

Only jackets with pre-fitted injection ports (by the contractor) are to be used.

The entire inside surface of the jacket shall be lightly grit blasted by the contractor to remove any bond breaking residue that may be present.

All fixed stand-offs or adjustable stand-off bosses shall be affixed to the jacket by the contractor in accordance with approved shop drawings. Maximum spacing between fixed stand-offs shall be 18" in the longitudinal direction and 12" in the transverse direction.

Jacket assembly and positioning around the pile shall be performed by the contractor in such a manner as to assure that no damage to stand-offs and/or set screws occurs and that there will be no detrimental movement of the joints while joint adhesive is curing.

Both the longitudinal and transverse seams, if any, shall be sealed by the contractor with marine epoxy paste as described above and fastened with 3/16" diameter stainless steel rivets. The spacing between individual fasteners shall not exceed 5".

The jacket shall be supported by temporary bracing or other means supplied by the contractor to assure that it will not move or distort during the epoxy grout placement and curing period and that the minimum annular space of 3/8 inch between pile and jacket is maintained throughout the entire encapsulation.

The contractor shall install a gasket to prevent the epoxy grout from leaving the bottom of the jacket during the injection process. The gasket shall be fitted into the molded cavity at the lower end of the jacket and adhered in place with marine epoxy paste. Any gasket material used in the bottom seal shall be contained within the molded cavity and shall not extend up into the jacket above the cavity.

Epoxy Grout Preparation

Proportioning and mixing of the epoxy grout shall be accomplished with equipment expressly designed for that purpose and shall be performed in a suitable work area within hose distance of the piles to be encapsulated.

Proportioning of the silica aggregate and the liquid epoxy components shall be performed in strict accordance with the manufacturer's recommendations, with particular regard to temperature control. When ambient and/or water temperatures are expected to fall below 70 degrees F., the day's supply of grout filler and liquid components shall be pre-heated to above 80 degrees F., but never greater than 120 degrees F., prior to being introduced into the grout handling equipment. In no case shall open flame be used in direct contact with the equipment or the epoxy components.

Epoxy Grout Placement (Injection)

Before the injection process begins, at least 2 gallons of an approved grout hose lubricant shall be placed in each grout hopper. This lubricant shall be pumped through the entire system to coat all wetted surfaces of the hopper(s), pump(s) and hoses. When the lubricant level has reached the bottom of the hopper(s), it may be immediately followed by the epoxy grout and the remaining

lubricant "chased" out of the hoses. All lubricant, that is not intermixed with the epoxy grout, may be collected at the downstream end of the hoses for re-use.

The premixed, aggregate filled epoxy grout shall be pumped through hoses to the jacket injection ports. If the plural component method of grout handling is used, the separate aggregate filled components shall be pumped through separate hoses to the mixer/blender assembly, where the components are then thoroughly blended and catalyzed, just prior to entering the pile jacket.

Grout injection shall begin at the bottom injection port. As the grout appears at the next higher port, and it has been determined that the space between the pile and the jacket is filled to that port, the lower port shall be capped off and the injection begun at the next higher port where the grout appeared. This process is repeated from port to port until the grout reaches the top of the jacket. NOTE: If project experience indicates that the grout can be injected from a lower port, past the next higher port or ports, without difficulty or undo stress on the jacket, the higher port or ports may be plugged and bypassed. The plugs shall be 1" NPT, Schedule 40, PVC, CPVC or Polypropylene.

At the contractor's option, he may inject a short lift of grout (six inches to 1 foot in height) into the bottom-most port and allow it to cure before proceeding with subsequent lifts. If this practice is used, the jackets shall be fitted with an additional injection port to coincide with the top of the first lift. Subsequent lifts of grout will follow the above procedures.

The injection process shall be continuous, except for brief interruptions when the injector is moved from port to port, and the speed of the injection process shall be controlled to prevent entrapment of water or air in the grout cavity being filled.

The maximum permissible voids in the epoxy grout within the jackets shall not exceed 0.01 square foot per one (1) square foot of encapsulation area. Any voids larger than two (2) inches in diameter shall be repaired by the contractor, using an approved method, at no expense to the owner.

Final Finishing and Inspection of the Completed Encapsulation

After the grouting process is completed and the grout has sufficiently cured, all temporary support for the jacket shall be removed.

The exposed epoxy grout at the top of each encapsulation shall be finished with the marine epoxy paste using the method shown in the approved shop drawings.

Measurement and Payment

Pile Encapsulation will be measured and paid for at the contract unit price bid per linear foot of encased pile and will be full compensation for removal, containment and disposal off-site of unsound concrete including the cost of materials, labor, tools, equipment and incidentals necessary to accomplish removal; shop drawings, cleaning the pile, jacket installation, falsework; furnishing and placement of epoxy grout including pumping equipment, pollution control, turbidity curtains, and all else required to repair deteriorated piles using pile encapsulation.

Payment will be made under:

Pay Item

Pile Encapsulation

Pay Unit

Linear Feet

PILE JACKETS

SPECIAL

Description

This specification establishes the minimum requirements for furnishing installing permanent outer pile forms, standoffs, steel reinforcement bands and steel bracing collars for the installation of pile jackets for pile repair. It is intended to ensure that the supplier's forms and installation/reinforcing details shall allow the forms to be filled with concrete without failure, and provide durable, corrosion resistant pile protection.

Materials

Forms:

The form shall be fabricated from fiberglass and polyester resins, or other inert materials that are compatible with Portland cement and produce a form with equal levels of corrosion resistance and durability. The inside face of the forms shall have a texture equal to that provided by sandblasting, and shall have no bond inhibiting agents in contact with cementitious materials. Forms shall include polymer standoffs of sufficient number and spacing to maintain a minimum space of 2" between the reinforcing steel and the jacket. Provide forms with dimensions in accordance with the sizing chart shown in the plans. For pumped applications provide ports in accordance with the plans. The minimum allowable thickness of the forms is 1/8". Upon opening to place around a pile, the form shall be capable of returning to its original shape without assistance or damage. It shall have an interlocking joint along one side, which will permit the form to be assembled and sealed in place around the pile. Contractor shall submit form details to the Engineer for approval prior to beginning work.

Transverse joints (if any) shall be of overlapping configuration.

The material furnished must meet the following physical property requirements:

- (a) Water Absorption (ASTM-D570) 1% Maximum
- (b) Ultimate Tensile Strength (ASTM-D638)* 9,000 psi Minimum
- (c) Flexural Strength (ASTM-D796)* 16,000 psi Minimum
- (d) Flexural Modulus of Elasticity (ASTM-D790) 700,000 psi Minimum
- (e) Barcol Hardness (ASTM-D2583) 30 – 40
- (f) Color Similar to Federal Color Standard 595 Number 36622 – the color shall be integral in the form material.
- (g) Accelerated Weathering ... The fabricated form material shall be subjected to a 500 hour exposure test in a Twin-Carbon-ARC-Weather-Ometer (ASTM G-23, Type D) at an operating temperature of 145°F. Said test to be made at twenty minute cycles consisting of seventeen minutes of light and three minutes of water spray plus light. At the end of the exposure test the exposed samples shall not show any chipping, flaking or peeling. The test panels shall be prepared from the

materials meeting the physical property requirements above, and they shall be in accordance with the manufacturer's recommendations.

“*”= On original specimen whose flat surfaces are not machined to disturb the fiberglass.

Epoxy Gel Sealant

Use an approved marine epoxy gel to adhere the outer jacket seams. The epoxy paste must be a two component epoxy compound, capable of being applied underwater. The ratio of resin component to hardener component is 1:1 by volume. To assist in evaluating the thoroughness of job site mixing, each component must be of sharply contrasting color.

Concrete

Concrete shall meet the requirements of the *Standard Specs* for Class A Concrete. Use a pea gravel mix suitable for pumped applications. An anti-washout admixture may be used for in water applications. Concrete mix design shall be submitted for approval prior to beginning work.

Reinforcement Bands

Provide reinforcement bands similar to that shown in the plans. The bands shall be reusable, and shall be equipped with quick release fasteners.

Submit details and calculations showing design loads and the placement of the bands on the pile form necessary to reinforce the form against failure from the concrete pressure, or any other loading the form may experience, including its use on battered piles.

Steel Bracing Collar

For forms to be supported on temporary falsework provide a steel bracing collar which will reinforce the bottom of the form and allow connection to the falsework.

Construction Methods

Prior to jacket installation, remove all deteriorated concrete and thoroughly clean pile of marine growth, oil, grease, mud, rust, and any other deleterious material which might prevent proper bonding between the concrete and the pile. Accomplish pile preparation and cleaning by chipping, grit blasting, high pressure water blasting, or by divers using powered rotary abraders. Any method that produces the quality of cleaning necessary to meet the bond requirements of this specification may be considered. When necessary, perform the pile cleaning in 2 phases where active marine growth occurs. In the first phase, a maximum of 7 days before the encapsulation, remove marine growth, oil, grease, rust, and broken concrete, etc. In the second phase, a maximum of 48 hrs before placement of concrete in the outer pile jacket, perform a final surface preparation, removing all remaining deleterious substances including micro-organisms.

Place the jacket assembly and position it around the pile in such a manner as to assure that no damage to stand-offs and rebar cage occur. Ensure there will be no detrimental movement of the joints while joint adhesive is curing. The placement of the jacket is to be determined by the location of the affected pile and in accordance with the contract drawings.

Seal the longitudinal and transverse seams, if any, with marine epoxy paste as described above.

Place concrete using a pump or tremie. Free fall placement of concrete will not be allowed if any part of the jacket is submerged below water. For above water applications free fall placement will only be allowed for jacket lengths up to 10'. Cope the top of the repair to drain water.

No tainted water above pH 9.0 will be allowed to discharge from the work site. Monitoring of pH levels inside and outside of the jacket is required during the pumping operation. Perimeter monitoring site should be no more than 10 ft down flow from the work area. If the pH of the water within the jacket exceeds then pump to a container and hold until the pH level returns to 9.0.

Concrete shall attain a minimum strength of 3000 psi prior to removing form work.

Submittals

Submit shop drawings and calculations to the Engineer for approval prior to start of fabrication. Submittal shall include form dimensions, standoffs, pump ports (where applicable), reinforcing cage installation, reinforcement bands, collars, temporary falsework, methods to seal the form, form installation, and sequence of concrete placement.

Material Certification

For materials to be used, the Supplier shall furnish a certificate to the Engineer attesting that the materials meet all the requirements contained herein and that they conform in all respects to the materials subjected to the tests required. Copies of current test reports shall be attached to the certificate. No test report for tests made more than one year prior to shipment will be accepted for the form material.

Measurement and Payment

Concrete Pile Jackets will be measured and paid for at the contract unit price bid per linear foot of concrete encased pile jacket and will be full compensation for removal, containment and disposal off-site of unsound concrete including the cost of materials, labor, tools, equipment and incidentals necessary to accomplish removal; cleaning the pile, furnishing and installation of reinforcement, jacket installation, falsework; furnishing and placement of concrete including pumping equipment, pH monitoring, pollution control, turbidity curtains, and all else required to repair existing deteriorated concrete piles using pile jackets.

Payment will be made under:

Pay Item

Concrete Pile Jackets

Pay Unit

Linear Feet