

NOTE: SEE SHEET 2A FOR PLAN SHEET LAYOUT AT TIME OF INVESTIGATION

STATE OF NORTH CAROLINA
DEPARTMENT OF TRANSPORTATION
DIVISION OF HIGHWAYS
GEOTECHNICAL ENGINEERING UNIT

| STATE | STATE PROJECT REFERENCE NO. | SHEET NO. | TOTAL SHEETS |
|-----------------|-----------------------------|----------------|--------------|
| N.C. | U-4703 | 1 | 17 |
| STATE PROJ. NO. | F.A. PROJ. NO. | DESCRIPTION | |
| 35871.1.1 | STP-0508(2) | PE | |
| 35871.2.1 | STP-0508(2) | RW & UTILITIES | |
| 35871.3.1 | STPDA-0508(3) | CONST. | |

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| -L- | 56+50 to 59+50 | 7-8 | 13 | 16-18 |
| -L- | 59+50 to 84+14 | 8-9 | 13-14 | |
| -Y3- | 10+00 to 18+83 | 10 | 15 | |

ROADWAY
SUBSURFACE INVESTIGATION

PROJ. REFERENCE NO. 35871.1.1 (U-4703) F.A. PROJ. STP-0508 (2)
COUNTY WAKE
PROJECT DESCRIPTION TIMBER DRIVE EXTENSION (SR 2812)
FROM NC 50 TO WHITE OAK ROAD (SR 2547) IN GARNER

INVENTORY

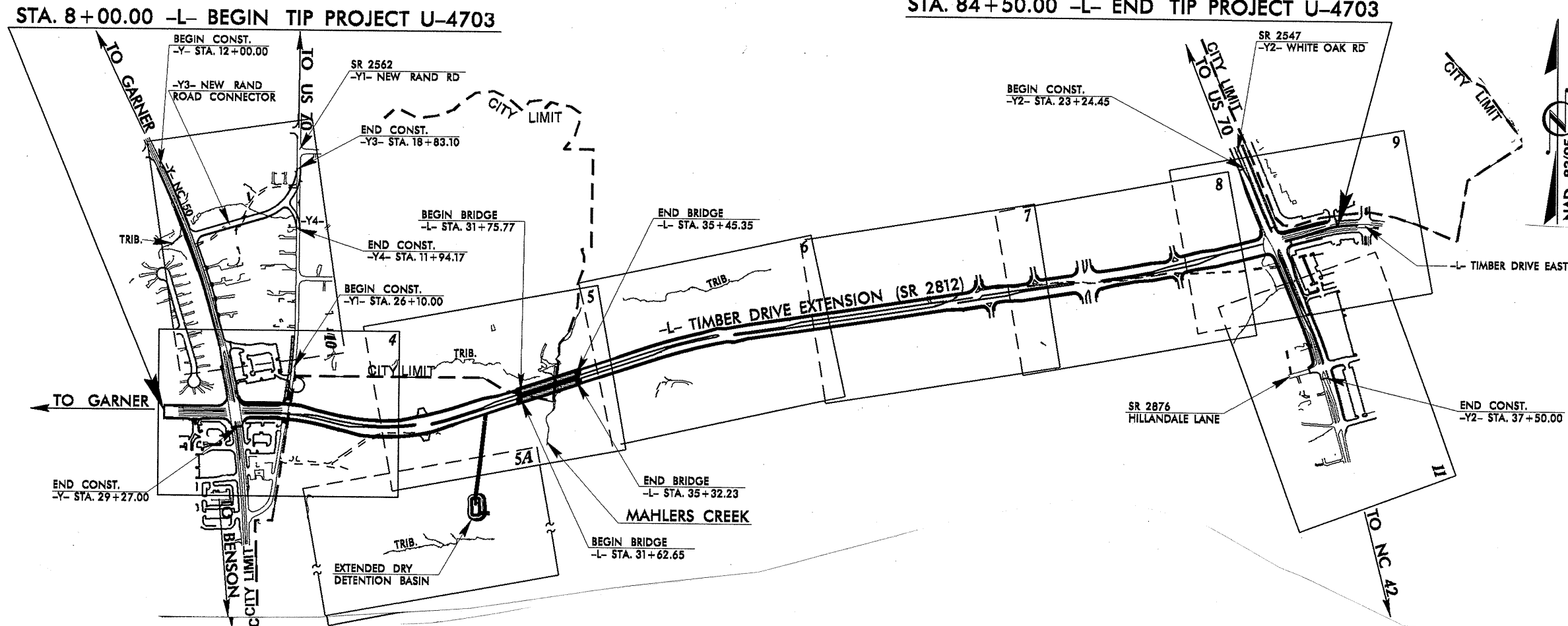
CAUTION NOTICE
THE SUBSURFACE INFORMATION AND THE SUBSURFACE INVESTIGATION ON WHICH IT IS BASED WERE MADE FOR THE PURPOSE OF STUDY, PLANNING, AND DESIGN, AND NOT FOR CONSTRUCTION OR PAY PURPOSES. THE VARIOUS FIELD BORING LOGS, ROCK CORES, AND SOIL TEST DATA AVAILABLE MAY BE REVIEWED OR INSPECTED IN RALEIGH BY CONTACTING THE N.C. DEPARTMENT OF TRANSPORTATION, GEOTECHNICAL ENGINEERING UNIT AT (919) 250-4088. NEITHER THE SUBSURFACE PLANS AND REPORTS, NOR THE FIELD BORING LOGS, ROCK CORES, OR SOIL TEST DATA ARE PART OF THE CONTRACT.

GENERAL SOIL AND ROCK STRATA DESCRIPTIONS AND INDICATED BOUNDARIES ARE BASED ON A GEOLOGICAL INTERPRETATION OF ALL AVAILABLE SUBSURFACE DATA AND MAY NOT NECESSARILY REFLECT THE ACTUAL SUBSURFACE CONDITIONS BETWEEN BORINGS OR BETWEEN SAMPLED STRATA WITHIN THE BOREHOLE. THE LABORATORY SAMPLE DATA AND THE IN SITU (IN-PLACE) TEST DATA CAN BE RELIED ON ONLY TO THE DEGREE OF RELIABILITY INHERENT IN THE STANDARD TEST METHOD. THE OBSERVED WATER LEVELS OR SOIL MOISTURE CONDITIONS INDICATED IN THE SUBSURFACE INVESTIGATIONS ARE AS RECORDED AT THE TIME OF THE INVESTIGATION. THESE WATER LEVELS OR SOIL MOISTURE CONDITIONS MAY VARY CONSIDERABLY WITH TIME ACCORDING TO CLIMATIC CONDITIONS INCLUDING TEMPERATURES, PRECIPITATION, AND WIND, AS WELL AS OTHER NON-CLIMATIC FACTORS.

THE BIDDER OR CONTRACTOR IS CAUTIONED THAT DETAILS SHOWN ON THE SUBSURFACE PLANS ARE PRELIMINARY ONLY AND IN MANY CASES THE FINAL DESIGN DETAILS ARE DIFFERENT. FOR BIDDING AND CONSTRUCTION PURPOSES, REFER TO THE CONSTRUCTION PLANS AND DOCUMENTS FOR FINAL DESIGN INFORMATION ON THIS PROJECT. THE DEPARTMENT DOES NOT WARRANT OR GUARANTEE THE SUFFICIENCY OR ACCURACY OF THE INVESTIGATION MADE, NOR THE INTERPRETATIONS MADE, OR OPINION OF THE DEPARTMENT AS TO THE TYPE OF MATERIALS AND CONDITIONS TO BE ENCOUNTERED. THE BIDDER OR CONTRACTOR IS CAUTIONED TO MAKE SUCH INDEPENDENT SUBSURFACE INVESTIGATIONS AS HE DEEMS NECESSARY TO SATISFY HIMSELF AS TO CONDITIONS TO BE ENCOUNTERED ON THIS PROJECT. THE CONTRACTOR SHALL HAVE NO CLAIM FOR ADDITIONAL COMPENSATION OR FOR AN EXTENSION OF TIME FOR ANY REASON RESULTING FROM THE ACTUAL CONDITIONS ENCOUNTERED AT THE SITE DIFFERING FROM THOSE INDICATED IN THE SUBSURFACE INFORMATION.

ID: U-4703

PROJECT: C202379



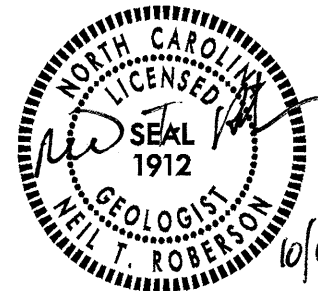
PERSONNEL
Y. KUNTUKOVA
D.S. TIGNOR

INVESTIGATED BY N.T. ROBERSON
CHECKED BY N.T. ROBERSON
SUBMITTED BY N.T. ROBERSON
DATE OCTOBER 2007

DRAWN BY: N.T. ROBERSON, T.T. WALKER

NOTE - THE INFORMATION CONTAINED HEREIN IS NOT IMPLIED OR GUARANTEED BY THE N.C. DEPARTMENT OF TRANSPORTATION AS BEING ACCURATE NOR IS IT CONSIDERED TO BE PART OF THE PLANS, SPECIFICATIONS, OR CONTRACT FOR THE PROJECT.

NOTE - BY HAVING REQUESTED THIS INFORMATION THE CONTRACTOR SPECIFICALLY WAIVES ANY CLAIMS FOR INCREASED COMPENSATION OR EXTENSION OF TIME BASED ON DIFFERENCES BETWEEN THE CONDITIONS INDICATED HEREIN AND THE ACTUAL CONDITIONS AT THE PROJECT SITE.



NORTH CAROLINA DEPARTMENT OF TRANSPORTATION

DIVISION OF HIGHWAYS

GEOTECHNICAL ENGINEERING UNIT

SUBSURFACE INVESTIGATION

SOIL AND ROCK LEGEND, TERMS, SYMBOLS, AND ABBREVIATIONS

| | |
|---------------------------------|----------------|
| PROJECT REFERENCE NO. U-4703 | SHEET NO. 2 |
|---------------------------------|----------------|

| SOIL DESCRIPTION | | GRADATION | | ROCK DESCRIPTION | | TERMS AND DEFINITIONS | | | | | |
|---|--|--|--|--|--|---|--|--|--|--|--|
| SOIL IS CONSIDERED TO BE THE UNCONSOLIDATED, SEMI-CONSOLIDATED, OR WEATHERED EARTH MATERIALS THAT CAN BE PENETRATED WITH A CONTINUOUS FLIGHT POWER AUGER, AND YIELD LESS THAN 100 BLOWS PER FOOT ACCORDING TO STANDARD PENETRATION TEST (ASTM D-1586). SOIL CLASSIFICATION IS BASED ON THE AASHTO SYSTEM. BASIC DESCRIPTIONS GENERALLY SHALL INCLUDE: CONSISTENCY, COLOR, TEXTURE, MOISTURE, AASHTO CLASSIFICATION, AND OTHER PERTINENT FACTORS SUCH AS MINERALOGICAL COMPOSITION, ANGULARITY, STRUCTURE, PLASTICITY, ETC. EXAMPLE: <i>VERY STIFF, GRAY, SILTY CLAY, MOST WITH INTERBEDDED FINE SAND LAYERS, HIGHLY PLASTIC, A-7-6</i> | | WELL GRADED - INDICATES A GOOD REPRESENTATION OF PARTICLE SIZES FROM FINE TO COARSE. UNIFORM - INDICATES THAT SOIL PARTICLES ARE ALL APPROXIMATELY THE SAME SIZE. (ALSO POORLY GRADED) BGP-GRADED - INDICATES A MIXTURE OF UNIFORM PARTICLES OF TWO OR MORE SIZES. THE ANGULARITY OR ROUNDNESS OF SOIL GRAINS IS DESIGNATED BY THE TERMS: ANGULAR, SUBANGULAR, SUBROUNDED, OR ROUNDED. | | HARD ROCK IS NON-COASTAL PLAIN MATERIAL THAT IF TESTED, WOULD YIELD SPT REFUSAL, AN INFERRED ROCK LINE INDICATES THE LEVEL AT WHICH NON-COASTAL PLAIN MATERIAL WOULD YIELD SPT REFUSAL. SPT REFUSAL IS PENETRATION BY A SPLIT SPOON SAMPLER EQUAL TO OR LESS THAN 0.1 FOOT PER 60 BLOWS. IN NON-COASTAL PLAIN MATERIAL, THE TRANSITION BETWEEN SOIL AND ROCK IS OFTEN REPRESENTED BY A ZONE OF WEATHERED ROCK. ROCK MATERIALS ARE TYPICALLY DIVIDED AS FOLLOWS: WEATHERED ROCK (WR)  NON-COASTAL PLAIN MATERIAL THAT WOULD YIELD SPT N VALUES > 100 BLOWS PER FOOT IF TESTED. CRYSTALLINE ROCK (CR)  FINE TO COARSE GRAIN IGNEOUS AND METAMORPHIC ROCK THAT WOULD YIELD SPT REFUSAL IF TESTED. ROCK TYPE INCLUDES GRANITE, GNEISS, GABBRO, SCHIST, ETC. NON-CRYSTALLINE ROCK (NCR)  FINE TO COARSE GRAIN METAMORPHIC AND NON-COASTAL PLAIN SEDIMENTARY ROCK THAT WOULD YIELD SPT REFUSAL IF TESTED. ROCK TYPE INCLUDES PHYLLITE, SLATE, SANDSTONE, ETC. COASTAL PLAIN SEDIMENTARY ROCK (CP)  COASTAL PLAIN SEDIMENTS CEMENTED INTO ROCK, BUT MAY NOT YIELD SPT REFUSAL. ROCK TYPE INCLUDES LIMESTONE, SANDSTONE, CEMENTED SHELL BEDS, ETC. | | ALLUVIUM (ALLUV.) - SOILS THAT HAVE BEEN TRANSPORTED BY WATER. AQUIFER - A WATER BEARING FORMATION OR STRATA. ARENACEOUS - APPLIED TO ROCKS THAT HAVE BEEN DERIVED FROM SAND OR THAT CONTAIN SAND. ARGILLACEOUS - APPLIED TO ALL ROCKS OR SUBSTANCES COMPOSED OF CLAY MINERALS, OR HAVING A NOTABLE PROPORTION OF CLAY IN THEIR COMPOSITION, AS SHALE, SLATE, ETC. ARTESIAN - GROUND WATER THAT IS UNDER SUFFICIENT PRESSURE TO RISE ABOVE THE LEVEL AT WHICH IT IS ENCOUNTERED, BUT WHICH DOES NOT NECESSARILY RISE TO OR ABOVE THE GROUND SURFACE. CALCAREOUS (CALC.) - SOILS THAT CONTAIN APPRECIABLE AMOUNTS OF CALCIUM CARBONATE. COLLUVIUM - ROCK FRAGMENTS MIXED WITH SOIL DEPOSITED BY GRAVITY ON SLOPE OR AT BOTTOM OF SLOPE. CORE RECOVERY (REC.) - TOTAL LENGTH OF ALL MATERIAL RECOVERED IN THE CORE BARREL DIVIDED BY TOTAL LENGTH OF CORE RUN AND EXPRESSED AS A PERCENTAGE. DIKE - A TABULAR BODY OF IGNEOUS ROCK THAT CUTS ACROSS THE STRUCTURE OF ADJACENT ROCKS OR CUTS MASSIVE ROCK. DIP - THE ANGLE AT WHICH A STRATUM OR ANY PLANAR FEATURE IS INCLINED FROM THE HORIZONTAL. DIP DIRECTION (DIP AZIMUTH) - THE DIRECTION OR BEARING OF THE HORIZONTAL TRACE OF THE LINE OF DIP, MEASURED CLOCKWISE FROM NORTH. FAULT - A FRACTURE OR FRACTURE ZONE ALONG WHICH THERE HAS BEEN DISPLACEMENT OF THE SIDES RELATIVE TO ONE ANOTHER PARALLEL TO THE FRACTURE. FISSILE - A PROPERTY OF SPLITTING ALONG CLOSELY SPACED PARALLEL PLANES. FLOAT - ROCK FRAGMENTS ON SURFACE NEAR THEIR ORIGINAL POSITION AND DISLODGED FROM PARENT MATERIAL. FLOOD PLAIN (FP) - LAND BORDERING A STREAM, BUILT OF SEDIMENTS DEPOSITED BY THE STREAM. FORMATION (FM) - A MAPPABLE GEOLOGIC UNIT THAT CAN BE RECOGNIZED AND TRACED IN THE FIELD. JOINT - FRACTURE IN ROCK ALONG WHICH NO APPRECIABLE MOVEMENT HAS OCCURRED. LEDGE - A SHELF-LIKE RIDGE OR PROJECTION OF ROCK WHOSE THICKNESS IS SMALL COMPARED TO ITS LATERAL EXTENT. LENS - A BODY OF SOIL OR ROCK THAT THINS OUT IN ONE OR MORE DIRECTIONS. MOTTLED (MOT.) - IRREGULARLY MARKED WITH SPOTS OF DIFFERENT COLORS. MOTTLING IN SOILS USUALLY INDICATES POOR AERATION AND LACK OF GOOD DRAINAGE. PERCHED WATER - WATER MAINTAINED ABOVE THE NORMAL GROUND WATER LEVEL BY THE PRESENCE OF AN INTERVENING IMPERVIOUS STRATUM. RESIDUAL (RES.) SOIL - SOIL FORMED IN PLACE BY THE WEATHERING OF ROCK. ROCK QUALITY DESIGNATION (ROQ) - A MEASURE OF ROCK QUALITY DESCRIBED BY TOTAL LENGTH OF ROCK SEGMENTS EQUAL TO OR GREATER THAN 4 INCHES DIVIDED BY THE TOTAL LENGTH OF CORE RUN AND EXPRESSED AS A PERCENTAGE. SAPROLITE (SAP) - RESIDUAL SOIL THAT RETAINS THE RELIC STRUCTURE OR FABRIC OF THE PARENT ROCK. SILL - AN INTRUSIVE BODY OF IGNEOUS ROCK OF APPROXIMATELY UNIFORM THICKNESS AND RELATIVELY THIN COMPARED WITH ITS LATERAL EXTENT, THAT HAS BEEN EMPLACED PARALLEL TO THE BEDDING OR SCHISTOSITY OF THE INTRUDED ROCKS. SLICKENSIDE - POLISHED AND STRIATED SURFACE THAT RESULTS FROM FRICTION ALONG A FAULT OR SLIP PLANE. STANDARD PENETRATION TEST (PENETRATION RESISTANCE) (SPT) - NUMBER OF BLOWS (N OR BPF) OF A 140 LB. HAMMER FALLING 30 INCHES REQUIRED TO PRODUCE A PENETRATION OF 1 FOOT INTO SOIL WITH A 2 INCH OUTSIDE DIAMETER SPLIT SPOON SAMPLER. SPT REFUSAL IS PENETRATION EQUAL TO OR LESS THAN 0.1 FOOT PER 60 BLOWS. STRATA CORE RECOVERY (SREC) - TOTAL LENGTH OF STRATA MATERIAL RECOVERED DIVIDED BY TOTAL LENGTH OF STRATUM AND EXPRESSED AS A PERCENTAGE. STRATA ROCK QUALITY DESIGNATION (SRQD) - A MEASURE OF ROCK QUALITY DESCRIBED BY TOTAL LENGTH OF ROCK SEGMENTS WITHIN A STRATUM EQUAL TO OR GREATER THAN 4 INCHES DIVIDED BY THE TOTAL LENGTH OF STRATA AND EXPRESSED AS A PERCENTAGE. TOPSOIL (TS) - SURFACE SOILS USUALLY CONTAINING ORGANIC MATTER. | | | | | |
| SOIL LEGEND AND AASHTO CLASSIFICATION | | MINERALOGICAL COMPOSITION | | WEATHERING | | | | | | | |
| GENERAL CLASS. GRANULAR MATERIALS (<= 35% PASSING #200) SILT-CLAY MATERIALS (> 35% PASSING #200) ORGANIC MATERIALS | | MINERAL NAMES SUCH AS QUARTZ, FELDSPAR, MICA, TALC, KAOLIN, ETC. ARE USED IN DESCRIPTIONS WHENEVER THEY ARE CONSIDERED OF SIGNIFICANCE. | | FRESH ROCK FRESH, CRYSTALS BRIGHT, FEW JOINTS MAY SHOW SLIGHT STAINING. ROCK RINGS UNDER HAMMER IF CRYSTALLINE. VERY SLIGHT (V SLI) ROCK GENERALLY FRESH, JOINTS STAINED, SOME JOINTS MAY SHOW THIN CLAY COATINGS IF OPEN. CRYSTALS ON A BROKEN SPECIMEN FACE SHINE BRIGHTLY. ROCK RINGS UNDER HAMMER BLOWS IF OF A CRYSTALLINE NATURE. SLIGHT (SLI) ROCK GENERALLY FRESH, JOINTS STAINED AND DISCOLORATION EXTENDS INTO ROCK UP TO 1 INCH. OPEN JOINTS MAY CONTAIN CLAY. IN GRANITOID ROCKS SOME OCCASIONAL FELDSPAR CRYSTALS ARE DULL AND DISCOLORED. CRYSTALLINE ROCKS RING UNDER HAMMER BLOWS. MODERATE (MOD.) SIGNIFICANT PORTIONS OF ROCK SHOW DISCOLORATION AND WEATHERING EFFECTS. IN GRANITOID ROCKS, MOST FELDSPARS ARE DULL AND DISCOLORED, SOME SHOW CLAY. ROCK HAS DULL SOUND UNDER HAMMER BLOWS AND SHOWS SIGNIFICANT LOSS OF STRENGTH AS COMPARED WITH FRESH ROCK. MODERATELY SEVERE (MOD. SEV.) ALL ROCK EXCEPT QUARTZ DISCOLORED OR STAINED. IN GRANITOID ROCKS, ALL FELDSPARS DULL AND DISCOLORED AND A MAJORITY SHOW KAOLINIZATION. ROCK SHOWS SEVERE LOSS OF STRENGTH AND CAN BE EXCAVATED WITH A GEOLOGIST'S PICK. ROCK GIVES "CLUNK" SOUND WHEN STRUCK. IF TESTED, WOULD YIELD SPT REFUSAL. SEVERE (SEV.) ALL ROCK EXCEPT QUARTZ DISCOLORED OR STAINED. ROCK FABRIC CLEAR AND EVIDENT BUT REDUCED IN STRENGTH TO STRONG SOIL. IN GRANITOID ROCKS ALL FELDSPARS ARE KAOLINIZED TO SOME EXTENT. SOME FRAGMENTS OF STRONG ROCK USUALLY REMAIN. IF TESTED, YIELDS SPT N VALUES > 100 BPF. VERY SEVERE (V SEV.) ALL ROCK EXCEPT QUARTZ DISCOLORED OR STAINED. ROCK FABRIC ELEMENTS ARE DISCERNIBLE BUT THE MASS IS EFFECTIVELY REDUCED TO SOIL STATUS, WITH ONLY FRAGMENTS OF STRONG ROCK REMAINING. SAPROLITE IS AN EXAMPLE OF ROCK WEATHERED TO A DEGREE SUCH THAT ONLY MINOR VESTIGES OF THE ORIGINAL ROCK FABRIC REMAIN. IF TESTED, YIELDS SPT N VALUES < 100 BPF. COMPLETE ROCK REDUCED TO SOIL. ROCK FABRIC NOT DISCERNIBLE, OR DISCERNIBLE ONLY IN SMALL AND SCATTERED CONCENTRATIONS. QUARTZ MAY BE PRESENT AS DIKES OR STRINGERS. SAPROLITE IS ALSO AN EXAMPLE. | | SLIGHTLY COMPRESSIBLE LIQUID LIMIT LESS THAN 31 MODERATELY COMPRESSIBLE LIQUID LIMIT EQUAL TO 31-50 HIGHLY COMPRESSIBLE LIQUID LIMIT GREATER THAN 50 | | ROCK HARDNESS | | | |
| PERCENTAGE OF MATERIAL | | GROUND WATER | | MISCELLANEOUS SYMBOLS | | | | | | | |
| ORGANIC MATERIAL GRANULAR SOILS SILT-CLAY SOILS OTHER MATERIAL | | WATER LEVEL IN BORE HOLE IMMEDIATELY AFTER DRILLING STATIC WATER LEVEL AFTER 24 HOURS PERCHED WATER, SATURATED ZONE, OR WATER BEARING STRATA SPRING OR SEEP | | ROADWAY EMBANKMENT (RE) WITH SOIL DESCRIPTION SOIL SYMBOL ARTIFICIAL FILL (AF) OTHER THAN ROADWAY EMBANKMENT INFERRED SOIL BOUNDARY INFERRED ROCK LINE ALLUVIAL SOIL BOUNDARY DIP & DIP DIRECTION OF ROCK STRUCTURES SOUNDING ROD | | SPT TEST BORING AUGER BORING CORE BORING MONITORING WELL PIEZOMETER INSTALLATION SLOPE INDICATOR INSTALLATION SPT N-VALUE SPT REFUSAL | | | | | |
| CONSISTENCY OR DENSENESS | | ABBREVIATIONS | | EQUIPMENT USED ON SUBJECT PROJECT | | FRACTURE SPACING | | | | | |
| PRIMARY SOIL TYPE COMPACTNESS OR CONSISTENCY RANGE OF STANDARD PENETRATION RESISTANCE (N-VALUE) RANGE OF UNCONFINED COMPRESSIVE STRENGTH (TONS/FT ²) | | AR - AUGER REFUSAL BT - BORING TERMINATED CL - CLAY CPT - CONE PENETRATION TEST CSE - COARSE DMT - DILATOMETER TEST DPT - DYNAMIC PENETRATION TEST e - VOID RATIO F - FINE FOSS. - FOSSILIFEROUS FRAC. - FRACTURED, FRACTURES FRAGS. - FRAGMENTS HI. - HIGHLY MED. - MEDIUM MICA - MICACEOUS MOD. - MODERATELY NP - NON PLASTIC ORG. - ORGANIC PMT - PRESSUREMETER TEST SAP. - SAPROLITIC SD. - SAND, SANDY SL. - SILT, SILTY SLI. - SLIGHTLY TCR - TRICONE REFUSAL w - MOISTURE CONTENT V - VERY VST - VANE SHEAR TEST WEA. - WEATHERED γ - UNIT WEIGHT γ _d - DRY UNIT WEIGHT | | MOBILE B- BK-51 CME-45C CME-550 PORTABLE HOIST CLAY BITS 6" CONTINUOUS FLIGHT AUGER 6" HOLLOW AUGERS HARD FACED FINGER BITS TUNG-CARBIDE INSERTS CASING w/ ADVANCER TRICONE * STEEL TEETH TRICONE * TUNG-CARB. CORE BIT | | HAMMER TYPE: <input checked="" type="checkbox"/> AUTOMATIC <input type="checkbox"/> MANUAL CORE SIZE: <input type="checkbox"/> B <input type="checkbox"/> N <input type="checkbox"/> H HAND TOOLS: <input type="checkbox"/> POST HOLE DIGGER <input checked="" type="checkbox"/> HAND AUGER <input type="checkbox"/> SOUNDING ROD <input type="checkbox"/> VANE SHEAR TEST | | TERM SPACING VERY WIDE MORE THAN 10 FEET WIDE 3 TO 10 FEET MODERATELY CLOSE 1 TO 3 FEET CLOSE 0.16 TO 1 FEET VERY CLOSE LESS THAN 0.16 FEET | | | |
| TEXTURE OR GRAIN SIZE | | INDURATION | | BEDDING | | | | | | | |
| U.S. STD. SIEVE SIZE OPENINGS (MM) 4 10 40 60 200 270 4.76 2.00 0.42 0.25 0.075 0.053 | | FOR SEDIMENTARY ROCKS, INDURATION IS THE HARDENING OF THE MATERIAL BY CEMENTING, HEAT, PRESSURE, ETC. FRIABLE RUBBING WITH FINGER FREES NUMEROUS GRAINS; GENTLE BLOW BY HAMMER DISINTEGRATES SAMPLE. MODERATELY INDURATED GRAINS CAN BE SEPARATED FROM SAMPLE WITH STEEL PROBE; BREAKS EASILY WHEN HIT WITH HAMMER. INDURATED GRAINS ARE DIFFICULT TO SEPARATE WITH STEEL PROBE; DIFFICULT TO BREAK WITH HAMMER. EXTREMELY INDURATED SHARP HAMMER BLOWS REQUIRED TO BREAK SAMPLE; SAMPLE BREAKS ACROSS GRAINS. | | TERM THICKNESS VERY THICKLY BEDDED > 4 FEET THICKLY BEDDED 1.5 - 4 FEET THINLY BEDDED 0.16 - 1.5 FEET VERY THINLY BEDDED 0.03 - 0.16 FEET THICKLY LAMINATED 0.008 - 0.03 FEET THINLY LAMINATED < 0.008 FEET | | BENCH MARK: ELEVATION: FT. | | | | | |
| SOIL MOISTURE - CORRELATION OF TERMS | | COLOR | | NOTES: | | | | | | | |
| SOIL MOISTURE SCALE (ATTERBERG LIMITS) FIELD MOISTURE DESCRIPTION GUIDE FOR FIELD MOISTURE DESCRIPTION | | NONPLASTIC PLASTICITY INDEX (PI) DRY STRENGTH LOW PLASTICITY 0-5 VERY LOW SLIGHT MED. PLASTICITY 6-15 SLIGHT MEDIUM HIGH PLASTICITY 16-25 MEDIUM HIGH 26 OR MORE HIGH | | DESCRIPTIONS MAY INCLUDE COLOR OR COLOR COMBINATIONS (TAN, RED, YELLOW-BROWN, BLUE-GRAY). MODIFIERS SUCH AS LIGHT, DARK, STREAKED, ETC. ARE USED TO DESCRIBE APPEARANCE. | | | | | | | |

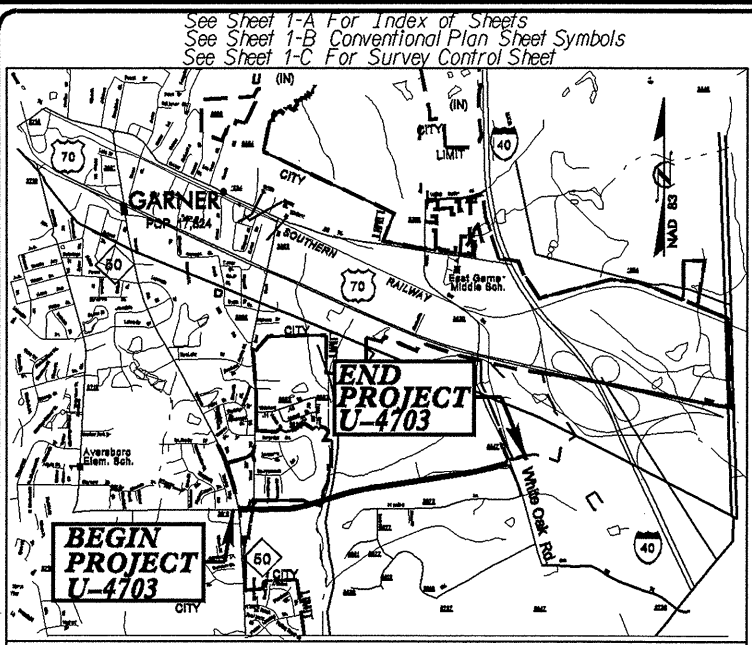
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| 35871.1.1 | STP-0508(2) | PE | |

STATE OF NORTH CAROLINA
DIVISION OF HIGHWAYS

WAKE COUNTY

LOCATION: **TIMBER DRIVE EAST EXTENSION (SR 2812)**
FROM NC 50 TO WHITE OAK ROAD (SR 2547) IN GARNER

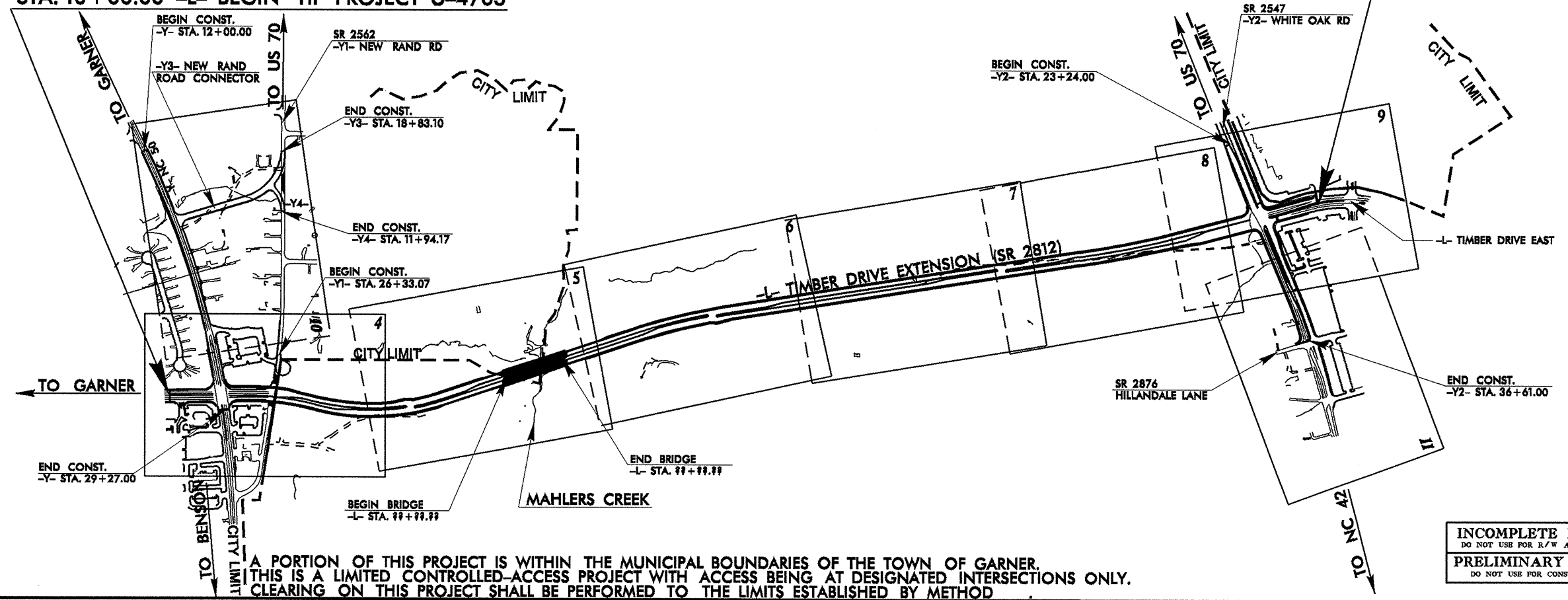
TYPE OF WORK: **GRADING, DRAINAGE, WIDENING, PAVING, CURB & GUTTER, STRUCTURES, AND SIGNALS**



VICINITY MAP

STA. 10+00.00 -L- BEGIN TIP PROJECT U-4703

STA. 84+14.00 -L- END TIP PROJECT U-4703

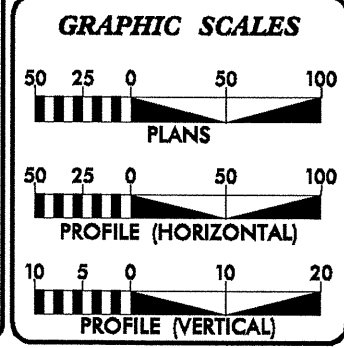


INCOMPLETE PLANS
DO NOT USE FOR R/W ACQUISITION
PRELIMINARY PLANS
DO NOT USE FOR CONSTRUCTION

A PORTION OF THIS PROJECT IS WITHIN THE MUNICIPAL BOUNDARIES OF THE TOWN OF GARNER.
THIS IS A LIMITED CONTROLLED-ACCESS PROJECT WITH ACCESS BEING AT DESIGNATED INTERSECTIONS ONLY.
CLEARING ON THIS PROJECT SHALL BE PERFORMED TO THE LIMITS ESTABLISHED BY METHOD

TIP PROJECT: U-4703

CONTRACT:



DESIGN DATA

| | |
|-------------------|----------------------|
| ADT 2007 = | 14,620 |
| ADT 2030 = | 24,000 |
| DHV = | 10 % |
| D = | 55 % |
| T = | 6 % * |
| V = | 50 MPH |
| * TTST 2% DUAL 4% | |
| FUNC. CLASS = | URBAN MINOR ARTERIAL |

PROJECT LENGTH

| | |
|--|-----------|
| LENGTH ROADWAY TIP PROJECT U-4703 = | 0.000 mi. |
| LENGTH STRUCTURE OF TIP PROJECT U-4703 = | 0.000 mi. |
| TOTAL LENGTH OF TIP PROJECT U-4703 = | 1.404 mi. |

Prepared In the Office of:

DIVISION OF HIGHWAYS
1000 Birch Ridge Dr., Raleigh, NC, 27618

2006 STANDARD SPECIFICATIONS

| | |
|--------------------|--------------------|
| RIGHT OF WAY DATE: | APRIL 18, 2008 |
| LETTING DATE: | SEPTEMBER 15, 2009 |

HYDRAULICS ENGINEER

SIGNATURE: _____ P.E.

ROADWAY DESIGN ENGINEER

SIGNATURE: _____ P.E.

DIVISION OF HIGHWAYS
STATE OF NORTH CAROLINA

STATE HIGHWAY DESIGN ENGINEER

04-OCT-2007 14:53
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T.WALKER AT 05:22:425



STATE OF NORTH CAROLINA
DEPARTMENT OF TRANSPORTATION

Michael F. Easley
GOVERNOR

P.O. BOX 25201, RALEIGH, N.C. 27611-5201

Lyndo Tippett
SECRETARY

October 5, 2007

STATE PROJECT: 35871.1.1 (U-4703)
FEDERAL PROJECT: STP-0508(2)
COUNTY: Wake
DESCRIPTION: Timber Drive Extension (SR 2812) from NC 50 to White Oak Road (SR 2547) in Garner
SUBJECT: Geotechnical Report – Inventory

Project Description

This project consists of a proposed 1.404 mile, two-lane roadway (-L-) on new location to extend Timber Drive (SR 2812) from White Oak Road to NC 50. Also proposed is a connector (-Y3-) between NC 50 and New Rand Road (SR 2562). A bridge is proposed to carry -L- over Mahlers Creek (see Plan Sheet No. 5).

The geotechnical field investigation was conducted during June of 2007. F&R was contracted to drill the project. A CME-550 with automatic hammer was during the investigation. Standard Penetration Tests were performed in selected borings and additional borings were advanced using continuous flight augers. Representative soil samples were collected for visual classification in the field and for laboratory analysis by the Materials and Tests Unit.

The following alignments, totaling 1.404 miles, were investigated. Subsurface soil profiles, or cross-sections, of these alignments are included in this report.

| <u>Line</u> | <u>Station</u> |
|-------------|----------------|
| -L- | 10+00 to 84+14 |
| -Y3- | 10+00 to 18+83 |

Areas of Special Geotechnical Interest

1) Highly Plastic Clay Soils: Highly plastic clays (PI > 25) were encountered on the project at the following locations:

| <u>Alignment</u> | <u>Station</u> | <u>Offset</u> |
|------------------|----------------|---------------|
| -L- | 15+50 | 60 LT |
| -L- | 57+00 | CL |

2) Crystalline Rock: Crystalline rock was encountered in the following borings:

| <u>Alignment</u> | <u>Station</u> | <u>Offset</u> |
|------------------|----------------|---------------|
| -L- | 31+70 | 10 RT |
| -L- | 35+40 | 10 LT |
| -L- | 39+00 | CL |
| -L- | 45+00 | 50 LT |
| -L- | 57+00 | CL |
| -L- | 71+00 | 10 RT |
| -Y3- | 10+50 | 35 LT |

Physiography and Geology

The project is located in the eastern Piedmont of North Carolina and is underlain by bedrock typical of the Raleigh Belt. Soils are derived from the weathering of the underlying granite is characterized by gently rolling hills. Mahlers Creek flows across the project from north to south. Fallow fields, wooded areas and abandoned homes comprise the eastern two-thirds of the project and a mixture of single-family homes and businesses is located near the intersections of Timber drive with NC 50 and White Oak road.

Soil Properties

Soils encountered at the project site include residual soils and alluvial sediments.

The residual soils are derived from the in-situ weathering of the underlying granite. Clayey soils are most common throughout the project and generally consist of medium stiff to very stiff, sandy clay and silty clay (A-6, A-7-5, and A-7-6). Medium stiff to stiff, sandy and clayey silt (A-4, A-5) and medium dense, silty sand (A-2-4) are also present.

Alluvial soils are present in the floodplain of Mahlers Creek and consist of shallow, wet, loose sands.

Rock Properties

Weathered rock and crystalline rock occur in several areas on the project. The weathered rock is derived from the underlying Raleigh Belt bedrock and ranges from 3 to 6 feet in thickness. The crystalline bedrock consists mostly of granite.

Groundwater

Groundwater was encountered in several borings. When present in residual soil, groundwater was determined to be 6 to 16 feet below the ground surface. Based on this investigation, groundwater is not anticipated to cause problems during construction.

Prepared by,

Yekaterina Kuntukova
Engineering Geologist

DIVISION OF HIGHWAYS
 STATE OF NORTH CAROLINA

SUMMARY OF EARTHWORK

IN CUBIC YARDS

| LOCATION | TOTAL UNCLASSIFIED EXCAVATION | ROCK EXCAVATION | UNDERCUT | UNSUITABLE EXCAVATION | SUITABLE EXCAVATION | TOTAL EMBANKMENT | ROCK EMBANKMENT | EARTH EMBANKMENT | EMBANKMENT (+20%) | BORROW | SUITABLE WASTE | UNSUITABLE WASTE | TOTAL WASTE |
|--|-------------------------------|-----------------|----------|-----------------------|---------------------|------------------|-----------------|------------------|-------------------|--------------|----------------|------------------|-------------|
| SUMMARY NO. 1 | | | | | | | | | | | | | |
| -L- 8+00.00 TO 12+92.77 | | | | | | | | | | | | | |
| TOTAL SUMMARY NO. 1 | | | | | | | | | | | | | |
| SUMMARY NO. 2 | | | | | | | | | | | | | |
| -L- 13+61.48 TO 31+69.16 (BB) | 5517 | | | | 5517 | 38298 | | 38298 | 45958 | 40441 | | | |
| -Y- 12+00.00 TO 29+27.00 | 874 | | | | 874 | 3916 | | 3916 | 4699 | 3825 | | | |
| -Y1- 26+33.07 TO 27+55.94 | 71 | | | | 71 | 138 | | 138 | 166 | 95 | | | |
| -Y3- 10+85.09 TO 18+83.10 | 1398 | | | | 1398 | 7281 | | 7281 | 8737 | 7339 | | | |
| -Y4- 10+94.36 TO 11+94.17 | 231 | | | | 231 | 350 | | 350 | 420 | 189 | | | |
| TOTAL SUMMARY NO. 2 | 8091 | | | | 8091 | 49983 | | 49983 | 59980 | 51889 | | | |
| SUMMARY NO. 3 | | | | | | | | | | | | | |
| -L- 35+38.79 (EB) TO 57+00.00 | 29477 | | | | 29477 | 24955 | | 24955 | 29946 | 469 | | | |
| TOTAL SUMMARY NO. 3 | 29477 | | | | 29477 | 24955 | | 24955 | 29946 | 469 | | | |
| SUMMARY NO. 4 | | | | | | | | | | | | | |
| -L- 57+00.00 TO 80+21.10 | 24225 | | | | 24225 | 32669 | | 32669 | 39203 | 14978 | | | |
| -Y2- 23+24.45 TO 27+50.00 | 1085 | | | | 1085 | | | | | | 1085 | | 1085 |
| -Y2- 29+00.00 TO 37+50.00 | 789 | | | | 789 | 668 | | 668 | 802 | 13 | | | |
| TOTAL SUMMARY NO. 4 | 26099 | | | | 26099 | 33337 | | 33337 | 40005 | 14991 | 1085 | | 1085 |
| SUMMARY NO. 5 | | | | | | | | | | | | | |
| -L- 80+65.61 TO 84+50.00 | 360 | | | | 360 | 11 | | 11 | 13 | | 347 | | 347 |
| TOTAL SUMMARY NO. 5 | 360 | | | | 360 | 11 | | 11 | 13 | | 347 | | 347 |
| SUMMARY TOTALS | 64027 | | | | 64027 | 108286 | | 108286 | 129944 | 67349 | 1432 | | 1432 |
| LOSS DUE TO C & G | -5900 | | | | -5900 | | | | | 5900 | | | |
| WASTE IN LIEU OF BORROW | | | | | | | | | | -1432 | -1432 | | -1432 |
| EST. SHOULDER MATERIAL | | | | | | 320 | | 320 | 384 | 384 | | | |
| PROJECT SUBTOTALS | 58127 | | | | 58127 | 108606 | | 108606 | 130328 | 72201 | | | |
| EST. 5% FOR REPLACING TOPSOIL ON BORROW PIT | | | | | | | | | | 3611 | | | |
| PROJECT TOTALS | 58127 | | | | 58127 | 108606 | | 108606 | 130328 | 75812 | | | |
| SAY | 58200 | | | | | | | | | 75900 | | | |
| DIVISION UNDERCUT QUANTITY (PLFI) =2500 CY | | | | | | | | | | | | | |
| FABRIC FOR SOIL STABILIZATION=800 SY | | | | | | | | | | | | | |
| FABRIC FOR SOIL STABILIZATION IN UNDERCUT OF SUBGRADE=850 SY | | | | | | | | | | | | | |
| SELECT GRANULAR MATERIAL=1650 CY | | | | | | | | | | | | | |
| UNDERDRAINS=1000 LF (SEE "SUBSURFACE DRAINAGE-UNDERDRAIN" PROJECT SPECIAL PROVISION) | | | | | | | | | | | | | |
| PAVEMENT STRUCTURE VOLUME = 9500 CY | | | | | | | | | | | | | |

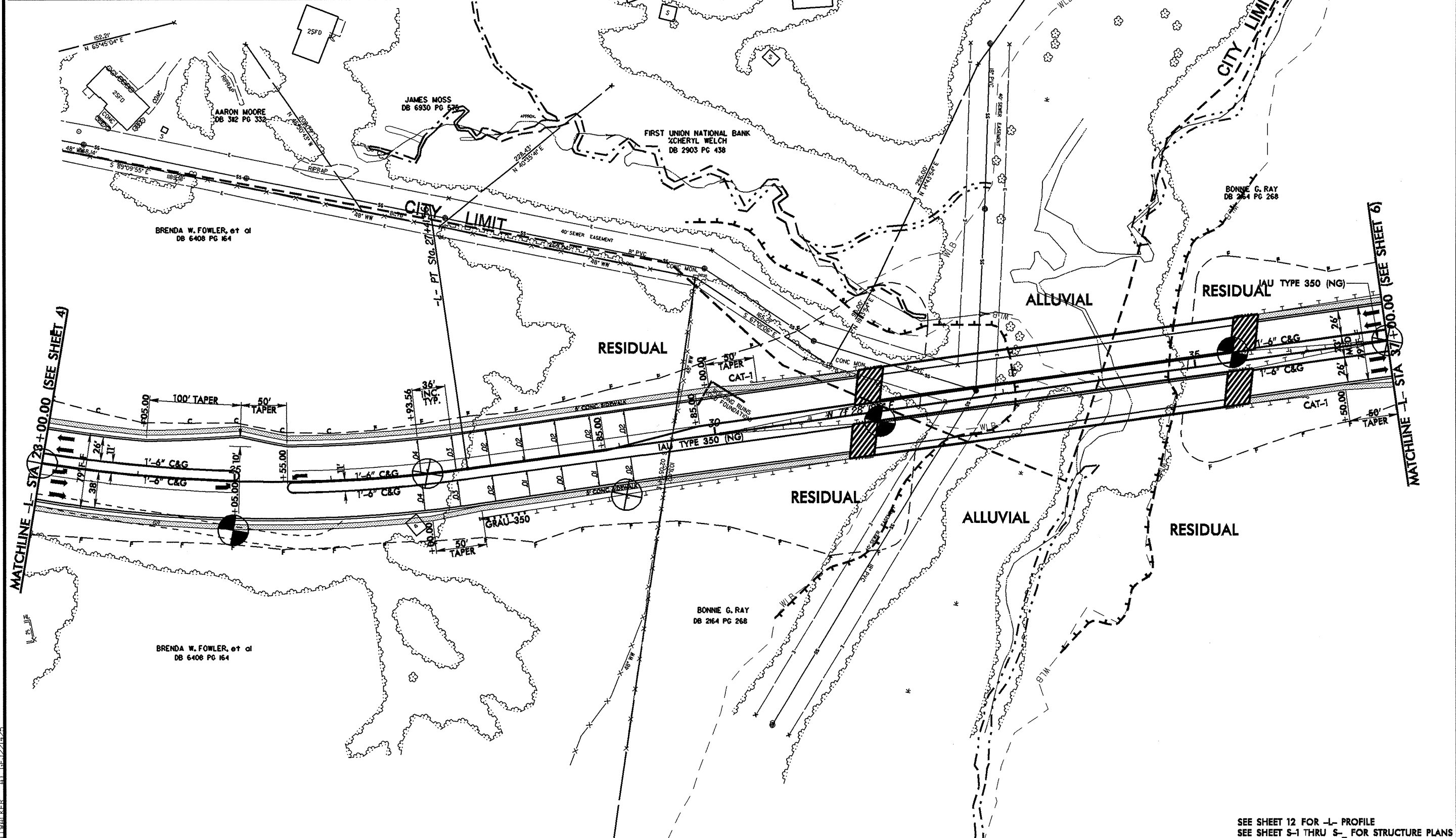
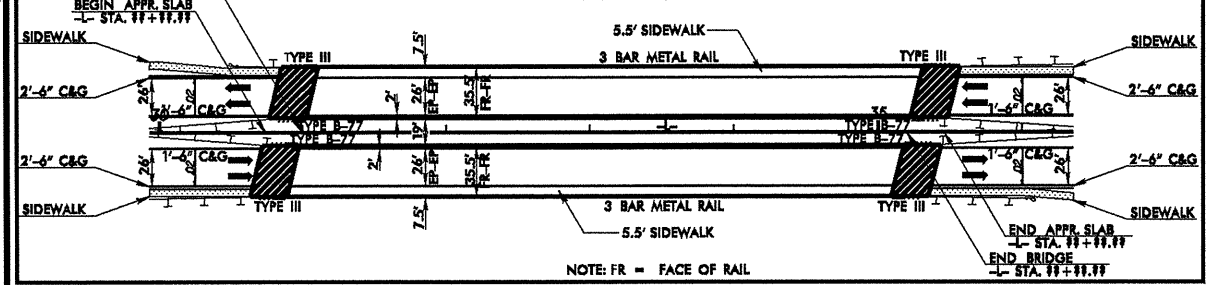
NOTE: Earthwork quantities are calculated by the Roadway Design Unit. These earthwork quantities are based in part on subsurface data provided by the Geotechnical Engineering Unit.

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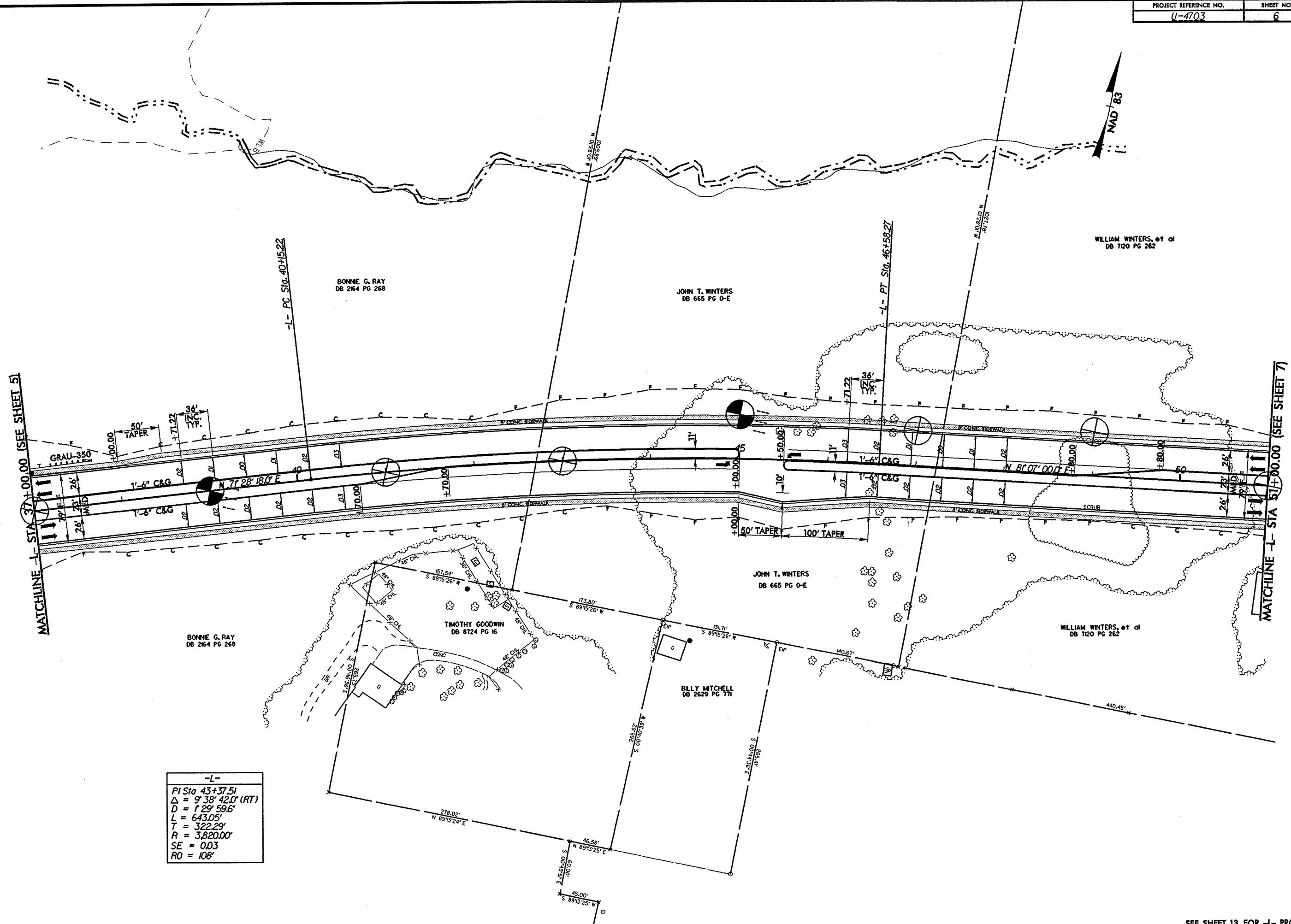
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|-----------------------|-----------|
| PROJECT REFERENCE NO. | SHEET NO. |
| U-4703 | 5 |

SKETCH SHOWING BRIDGE IN RELATION TO PAVEMENT STRUCTURE OVER MAHLERS CREEK



SEE SHEET 12 FOR -L- PROFILE
SEE SHEET S-1 THRU S- FOR STRUCTURE PLANS

8/17/99
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| | |
|----------|-----------------------------|
| -L- | |
| PI Sta | 43+37.51 |
| Δ | $9^{\circ} 38' 42.0''$ (RT) |
| D | 129' 59.6' |
| L | 643.05' |
| T | 322.29' |
| R | 3,820.00' |
| SE | 0.03 |
| RO | 108' |

SEE SHEET 13 FOR -L- PROFILE



WILLIAM WINTERS, et al
DB 7120 PG 262

DELORES W. PRYOR
DB 665 PG 0-E

SHERMAN A. YEARGAN, JR. et al
DB 10762 PG 1216

JESSIE MCCULLERS
DB 665 PG 0-E

MATCHLINE - STA 61+00.00 (SEE SHEET 6)

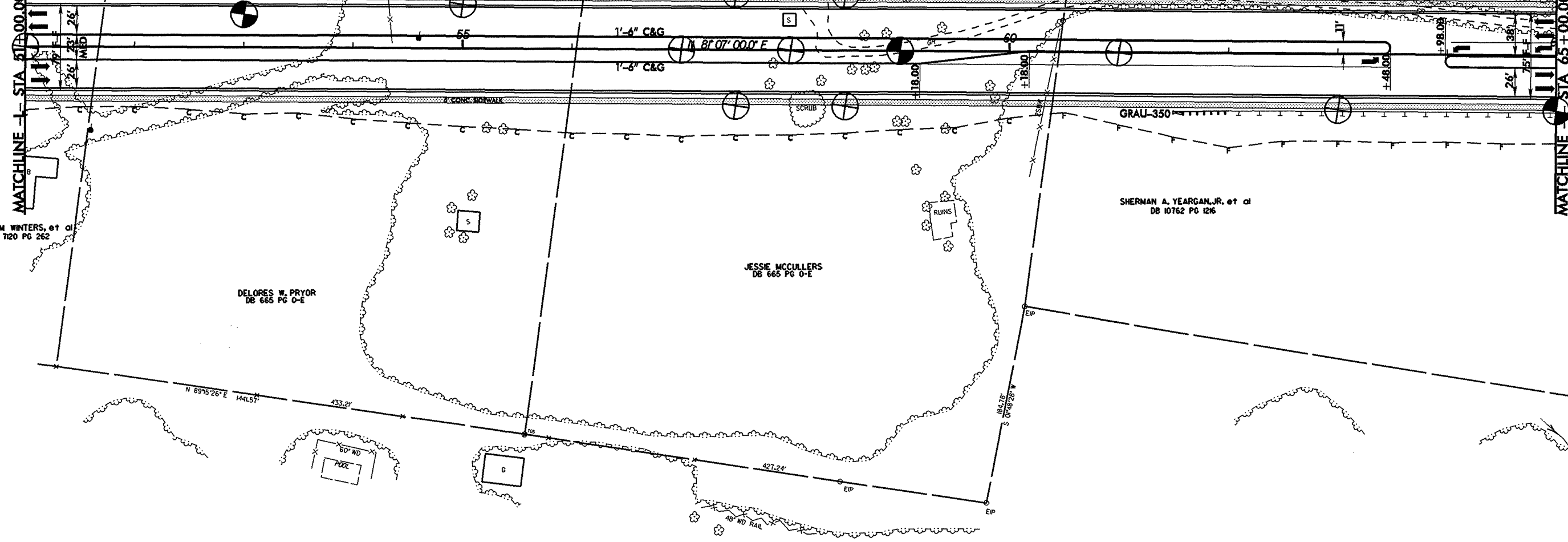
MATCHLINE - STA 65+00.00 (SEE SHEET 8)

WILLIAM WINTERS, et al
DB 7120 PG 262

DELORES W. PRYOR
DB 665 PG 0-E

JESSIE MCCULLERS
DB 665 PG 0-E

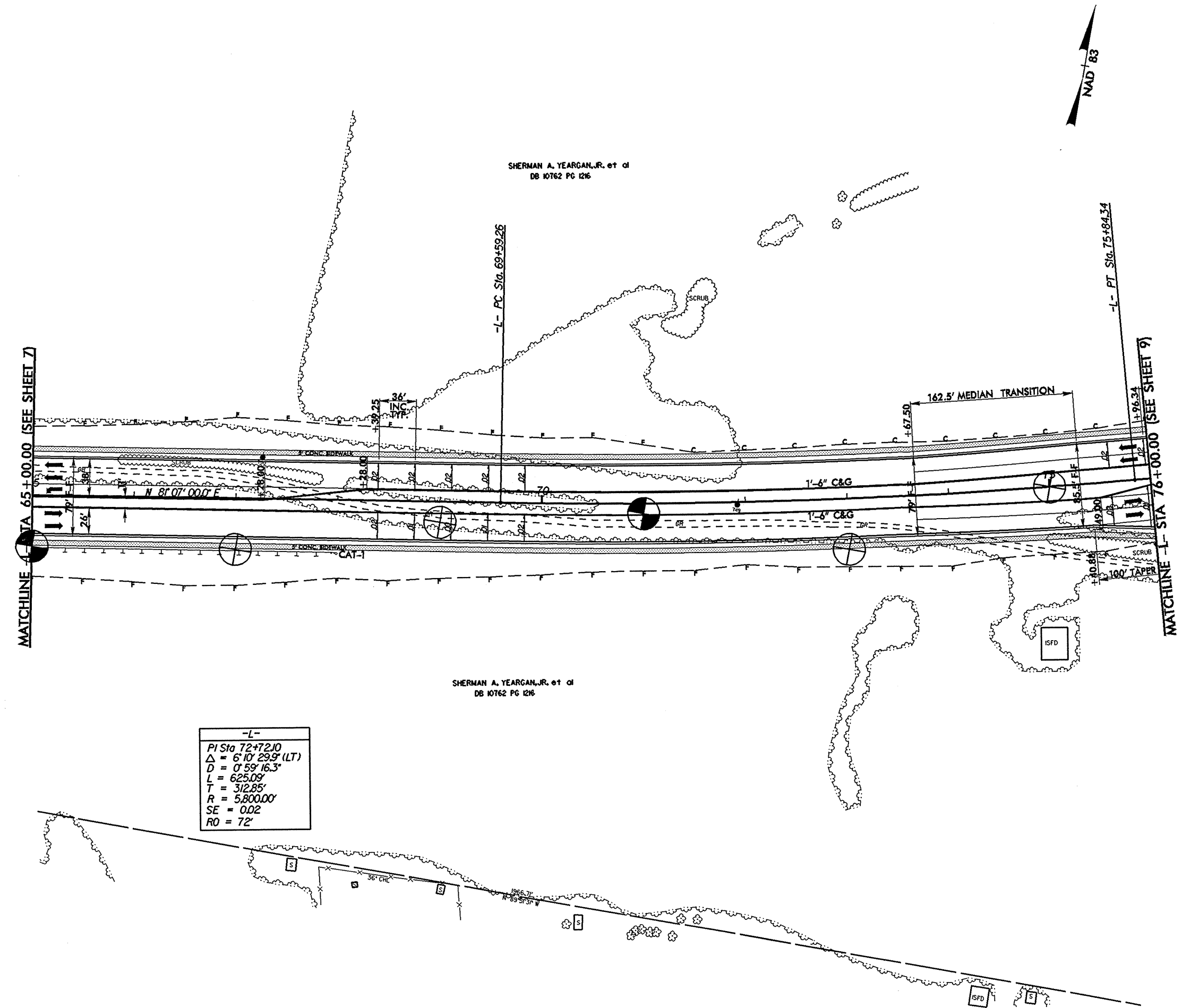
SHERMAN A. YEARGAN, JR. et al
DB 10762 PG 1216



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 user: jh



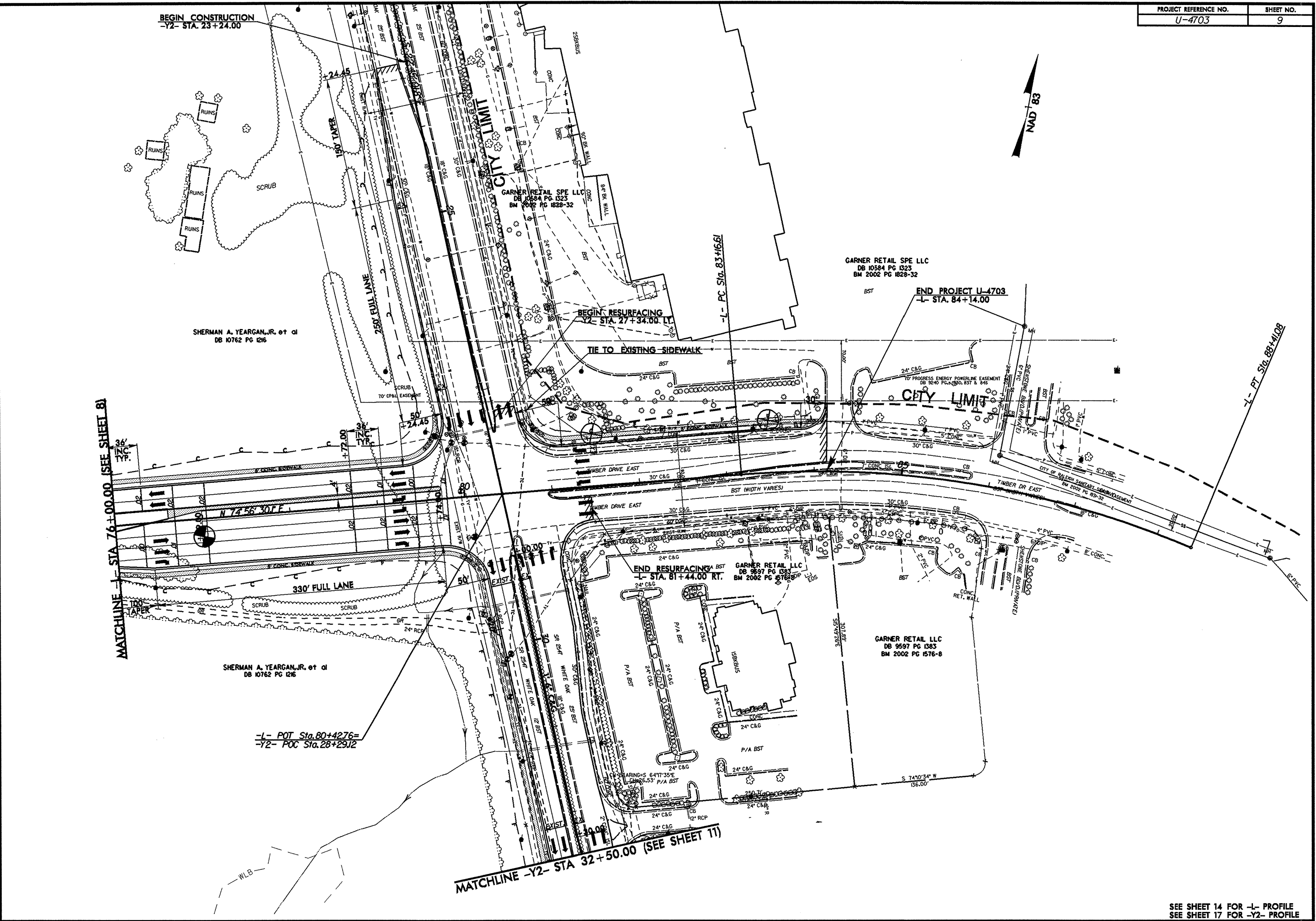
SHERMAN A. YEARGAN, JR. et al
 DB 10762 PG 1216

SHERMAN A. YEARGAN, JR. et al
 DB 10762 PG 1216

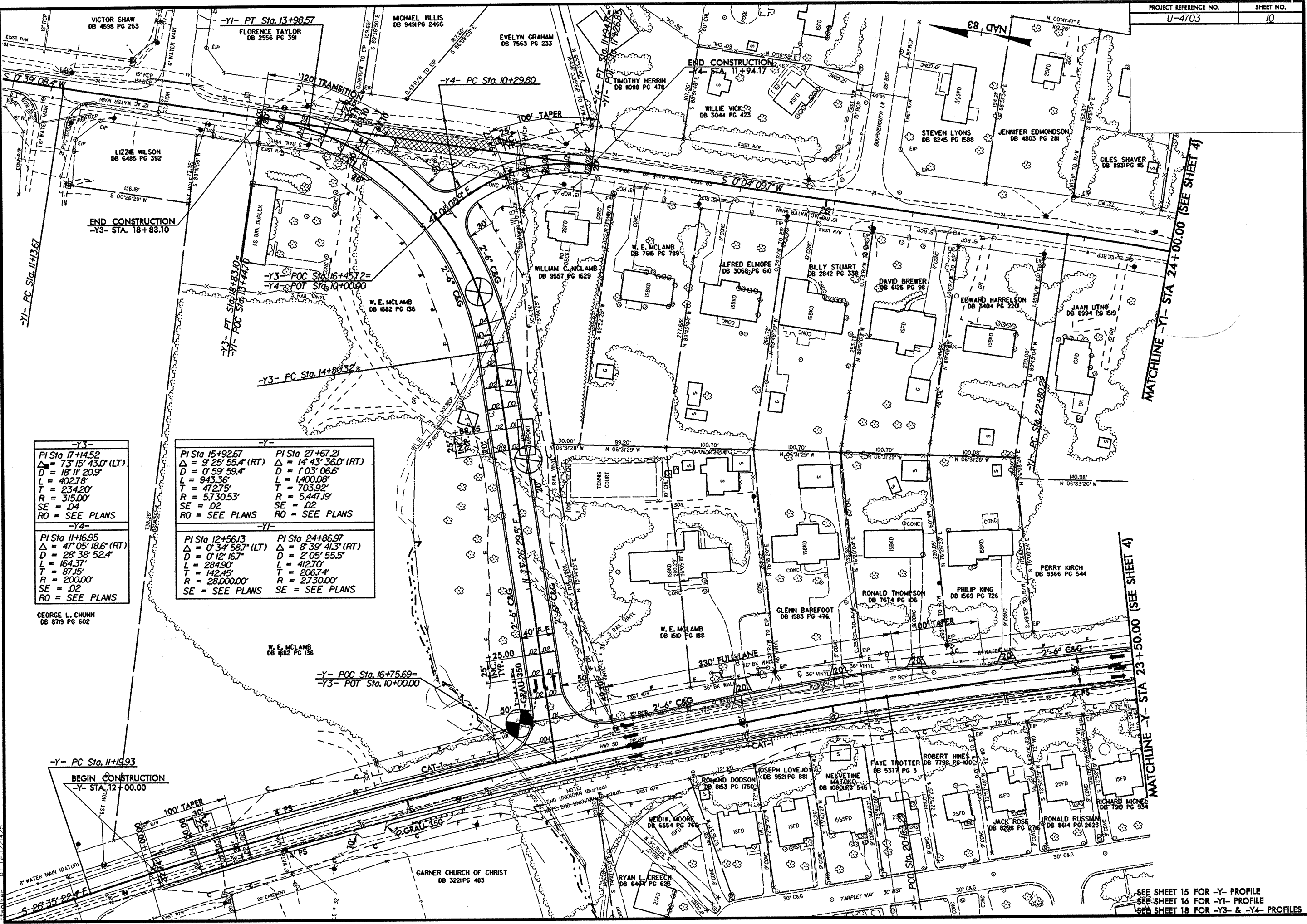
| | |
|----------|-------------------|
| -L- | |
| PI Sta | 72+72.10 |
| Δ | 6' 10" 29.9" (LT) |
| D | 0' 59" 16.3" |
| L | 625.09' |
| T | 312.85' |
| R | 5,800.00' |
| SE | 0.02 |
| RO | 72' |

SEE SHEET 14 FOR -L- PROFILE

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-Y3-

| |
|-------------------------------|
| PI Sta 17+14.52 |
| $\Delta = 73' 15" 43.0'$ (LT) |
| D = 18' 11" 20.9" |
| L = 402.78' |
| T = 234.20' |
| R = 315.00' |
| SE = .04 |
| RO = SEE PLANS |

-Y4-

| |
|-------------------------------|
| PI Sta 11+16.95 |
| $\Delta = 47' 05" 18.6'$ (RT) |
| D = 28' 38" 52.4" |
| L = 164.37' |
| T = 87.15' |
| R = 200.00' |
| SE = .02 |
| RO = SEE PLANS |

GEORGE L. CHUNN
 DB 8719 PG 602

-Y-

| |
|------------------------------|
| PI Sta 15+92.67 |
| $\Delta = 9' 25" 55.4'$ (RT) |
| D = 0' 59" 59.4" |
| L = 943.36' |
| T = 472.75' |
| R = 5,730.53' |
| SE = .02 |
| RO = SEE PLANS |

-Y1-

| |
|------------------------------|
| PI Sta 12+56.13 |
| $\Delta = 0' 34" 58.7'$ (LT) |
| D = 0' 12" 16.7" |
| L = 284.90' |
| T = 142.45' |
| R = 28,000.00' |
| SE = SEE PLANS |

| |
|-------------------------------|
| PI Sta 27+67.21 |
| $\Delta = 14' 43" 36.0'$ (RT) |
| D = 1' 03" 06.6" |
| L = 1,400.08' |
| T = 703.92' |
| R = 5,447.19' |
| SE = .02 |
| RO = SEE PLANS |

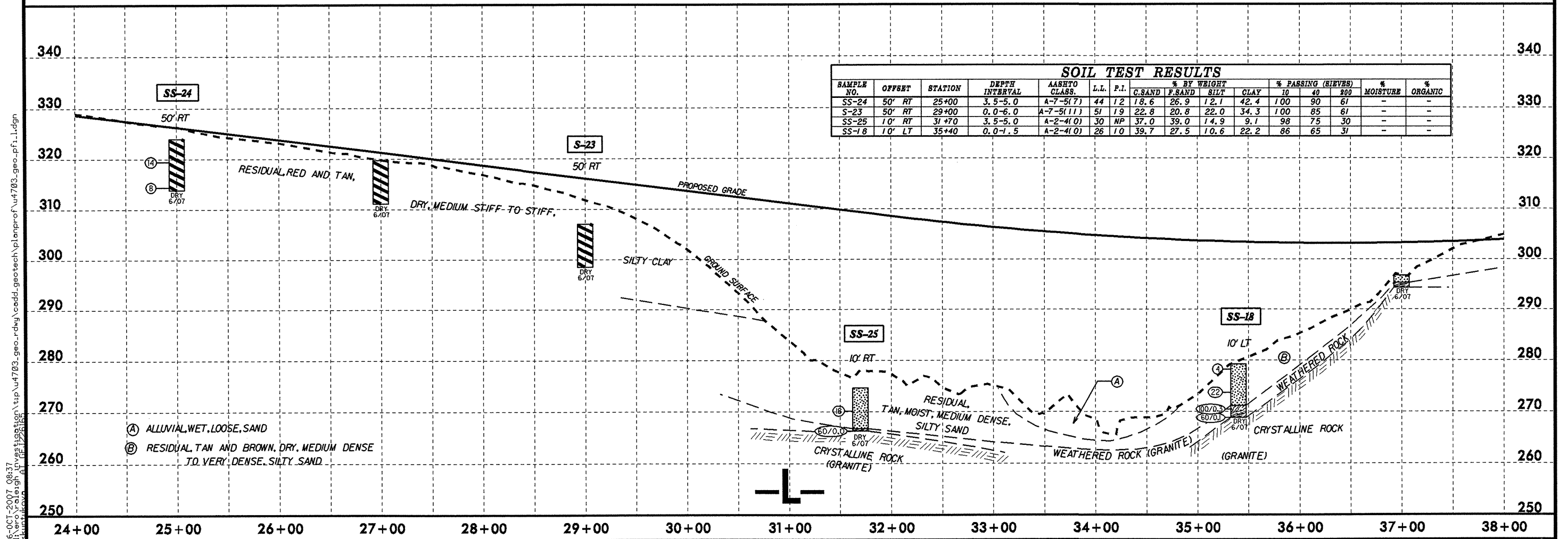
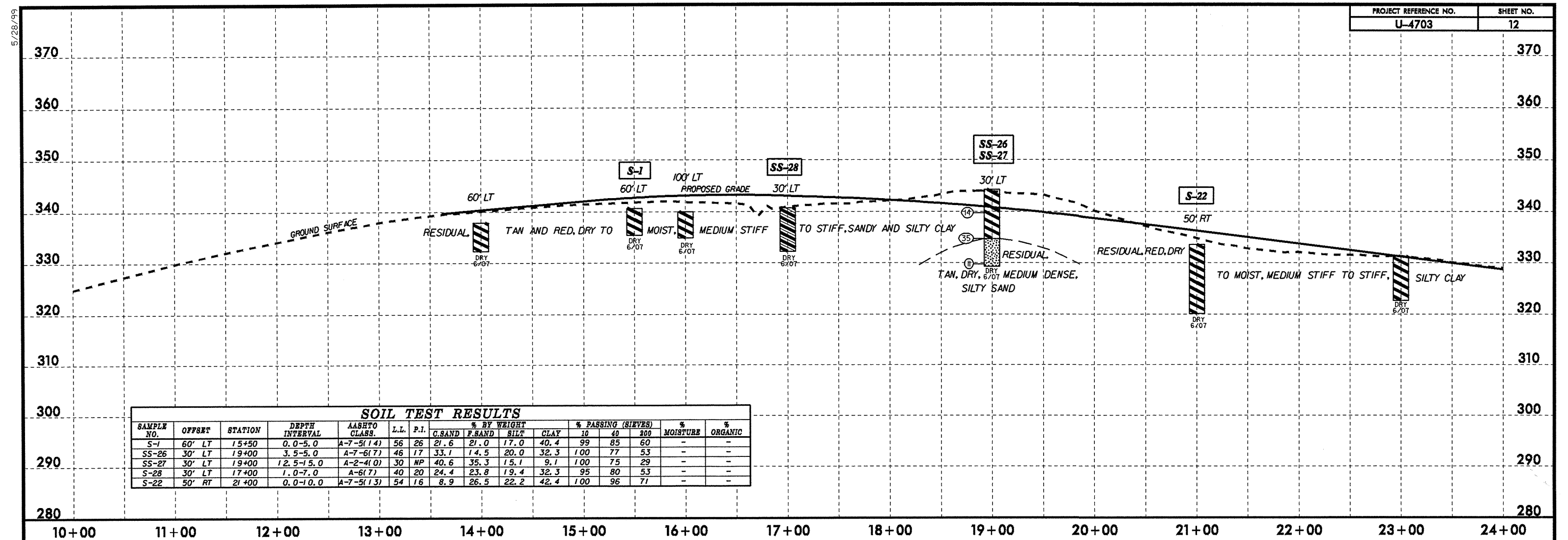
| |
|------------------------------|
| PI Sta 24+86.97 |
| $\Delta = 8' 39" 41.3'$ (RT) |
| D = 2' 05" 55.5" |
| L = 412.0' |
| T = 206.74' |
| R = 2,730.00' |
| SE = SEE PLANS |

-Y- PC Sta. 11+19.93
BEGIN CONSTRUCTION
-Y- STA. 12+00.00

MATCHLINE -Y1- STA 24+00.00 (SEE SHEET 4)

MATCHLINE -Y- STA 23+50.00 (SEE SHEET 4)

SEE SHEET 15 FOR -Y- PROFILE
 SEE SHEET 16 FOR -Y1- PROFILE
 SEE SHEET 18 FOR -Y3- & -Y4- PROFILES



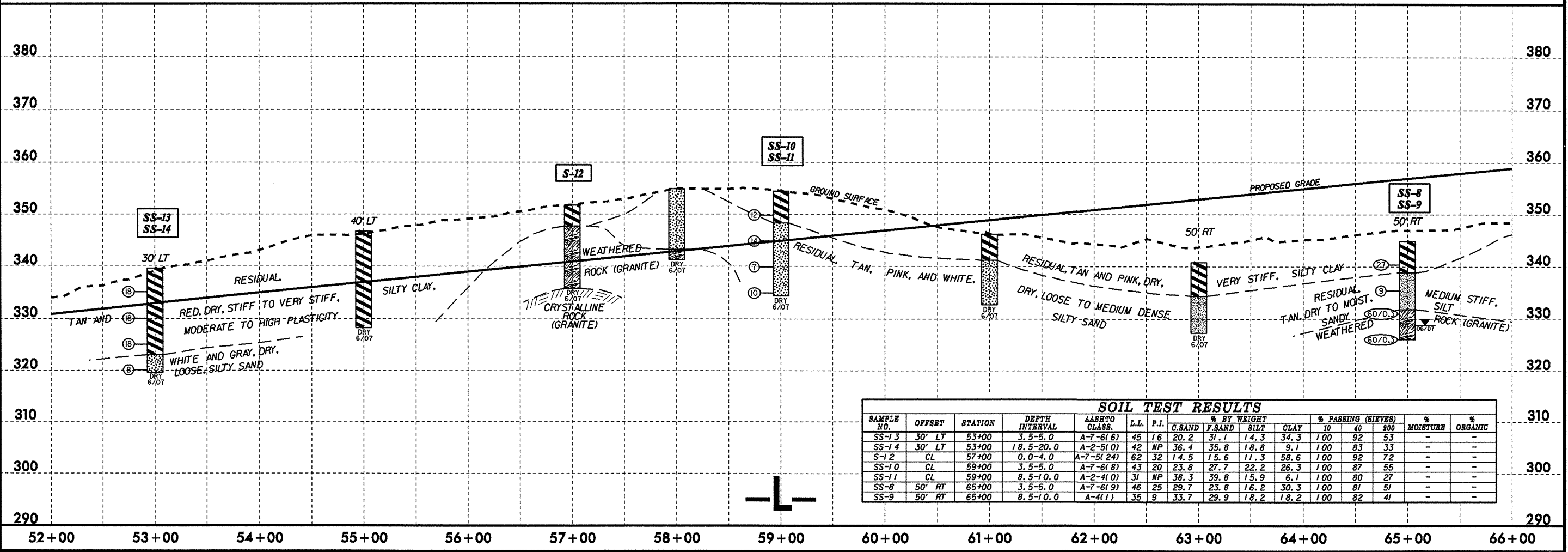
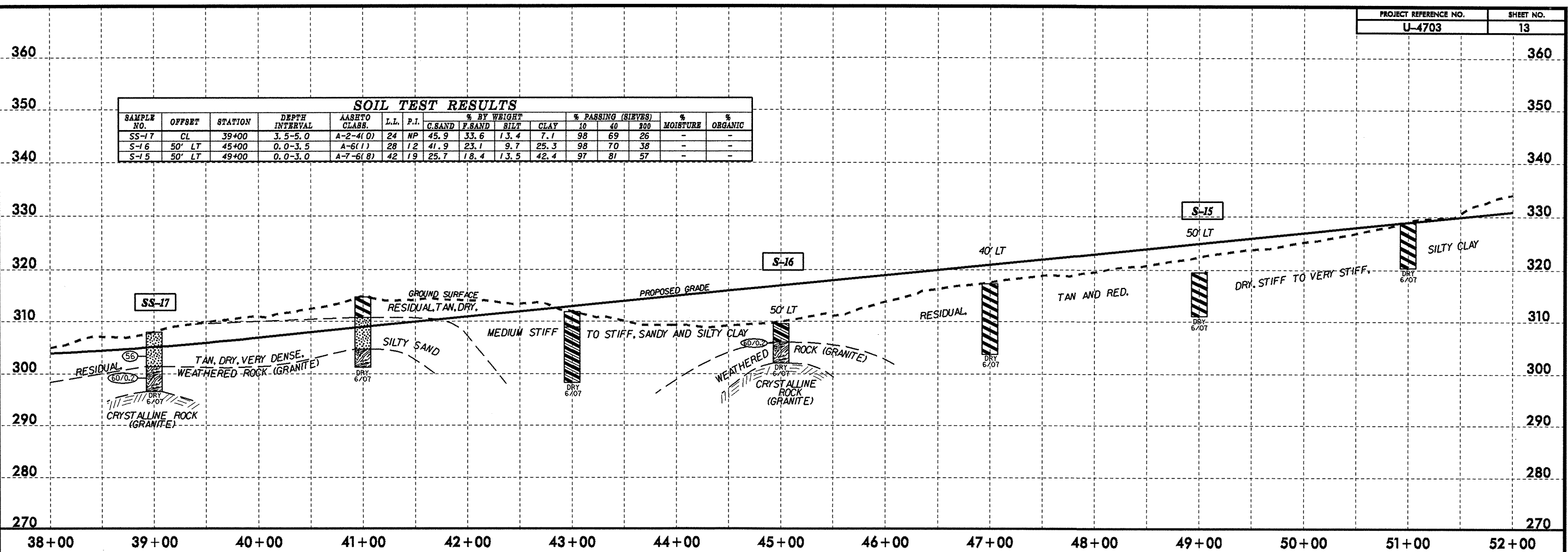
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- (A) ALLUVIAL WET, LOOSE, SAND
- (B) RESIDUAL TAN AND BROWN, DRY, MEDIUM DENSE TO VERY DENSE, SILTY SAND



SOIL TEST RESULTS

| SAMPLE NO. | OFFSET | STATION | DEPTH INTERVAL | AASHTO CLASS. | L.L. | P.I. | % BY WEIGHT | | | | % PASSING (SIEVES) | | | % MOISTURE | % ORGANIC |
|------------|--------|---------|----------------|---------------|------|------|-------------|--------|------|------|--------------------|----|-----|------------|-----------|
| | | | | | | | C.SAND | F.SAND | SILT | CLAY | 10 | 40 | 200 | | |
| SS-17 | CL | 39+00 | 3.5-5.0 | A-2-4(0) | 24 | NP | 45.9 | 33.6 | 13.4 | 7.1 | 98 | 69 | 26 | - | - |
| S-16 | 50' LT | 45+00 | 0.0-3.5 | A-6(1) | 28 | 12 | 41.9 | 23.1 | 9.7 | 25.3 | 98 | 70 | 38 | - | - |
| S-15 | 50' LT | 49+00 | 0.0-3.0 | A-7-6(8) | 42 | 19 | 25.7 | 18.4 | 13.5 | 42.4 | 97 | 81 | 57 | - | - |

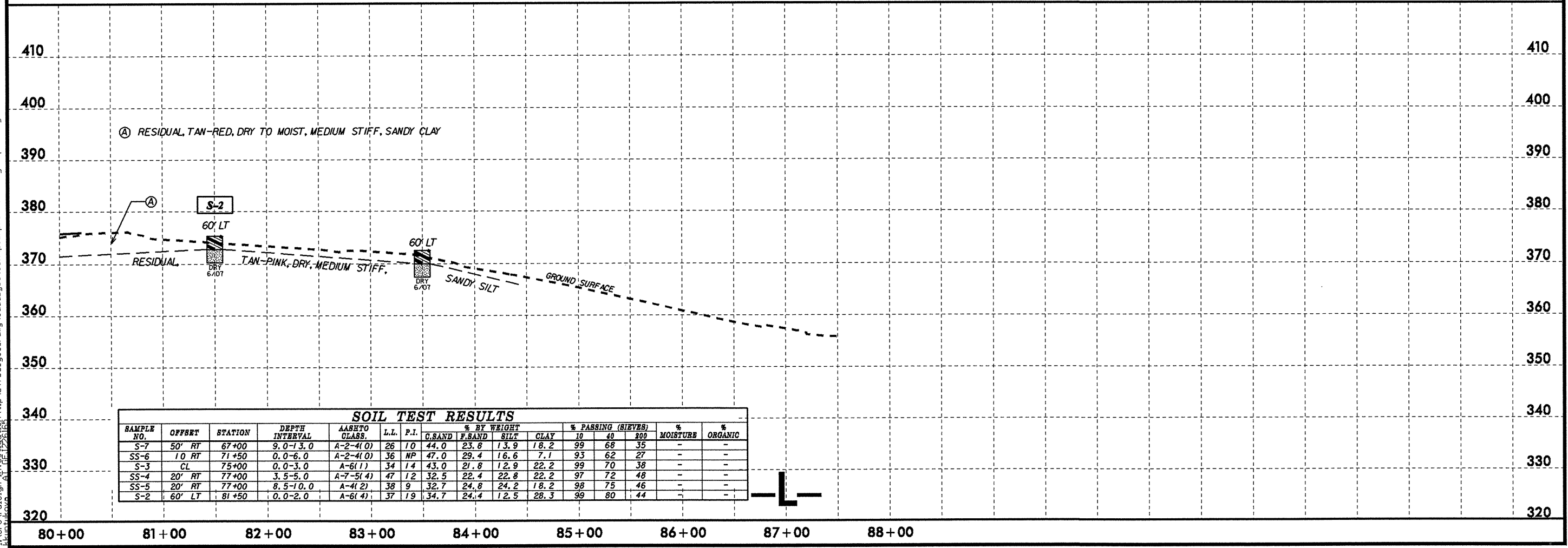
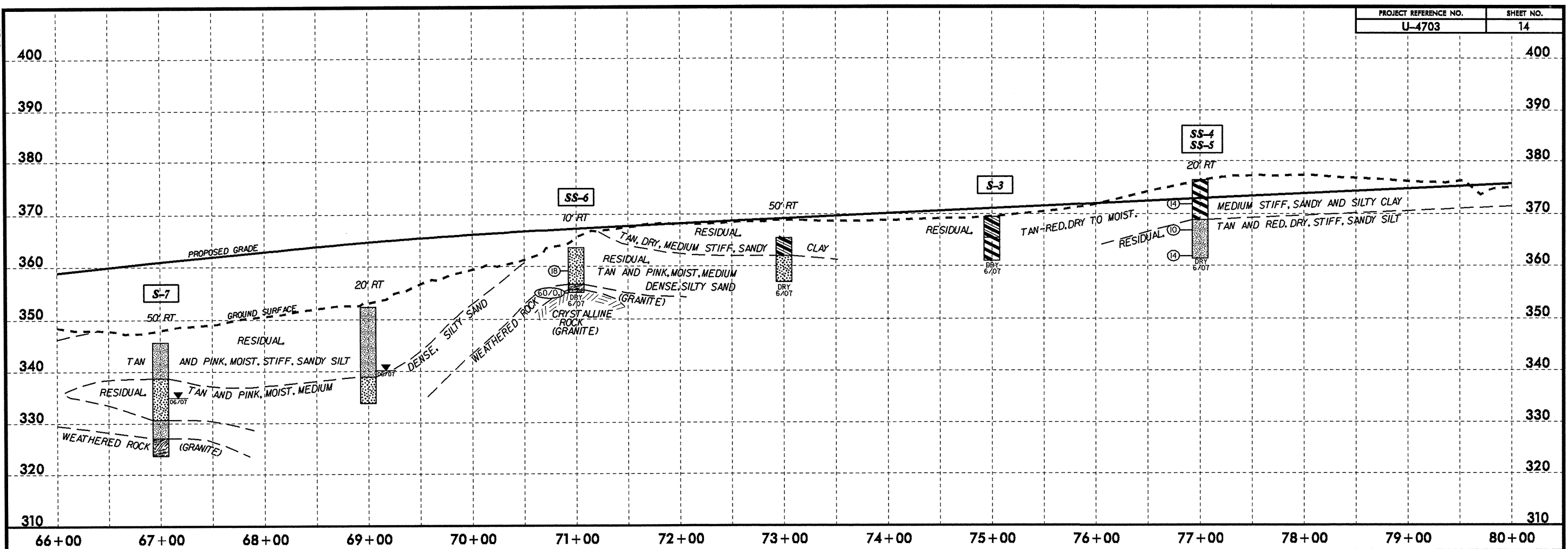


SOIL TEST RESULTS

| SAMPLE NO. | OFFSET | STATION | DEPTH INTERVAL | AASHTO CLASS. | L.L. | P.I. | % BY WEIGHT | | | | % PASSING (SIEVES) | | | % MOISTURE | % ORGANIC |
|------------|--------|---------|----------------|---------------|------|------|-------------|--------|------|------|--------------------|----|-----|------------|-----------|
| | | | | | | | C.SAND | F.SAND | SILT | CLAY | 10 | 40 | 200 | | |
| SS-13 | 30' LT | 53+00 | 3.5-5.0 | A-7-6(6) | 45 | 16 | 20.2 | 31.1 | 14.3 | 34.3 | 100 | 92 | 53 | - | - |
| SS-14 | 30' LT | 53+00 | 18.5-20.0 | A-2-5(0) | 42 | NP | 36.4 | 35.8 | 18.8 | 9.1 | 100 | 83 | 33 | - | - |
| S-12 | CL | 57+00 | 0.0-4.0 | A-7-5(24) | 62 | 32 | 14.5 | 15.6 | 11.3 | 58.6 | 100 | 92 | 72 | - | - |
| SS-10 | CL | 59+00 | 3.5-5.0 | A-7-6(8) | 43 | 20 | 23.8 | 27.7 | 22.2 | 26.3 | 100 | 87 | 55 | - | - |
| SS-11 | CL | 59+00 | 8.5-10.0 | A-2-4(0) | 31 | NP | 38.3 | 39.8 | 15.9 | 6.1 | 100 | 80 | 27 | - | - |
| SS-8 | 50' RT | 65+00 | 3.5-5.0 | A-7-6(9) | 46 | 25 | 29.7 | 23.8 | 16.2 | 30.3 | 100 | 81 | 51 | - | - |
| SS-9 | 50' RT | 65+00 | 8.5-10.0 | A-4(1) | 35 | 9 | 33.7 | 29.9 | 18.2 | 18.2 | 100 | 82 | 41 | - | - |

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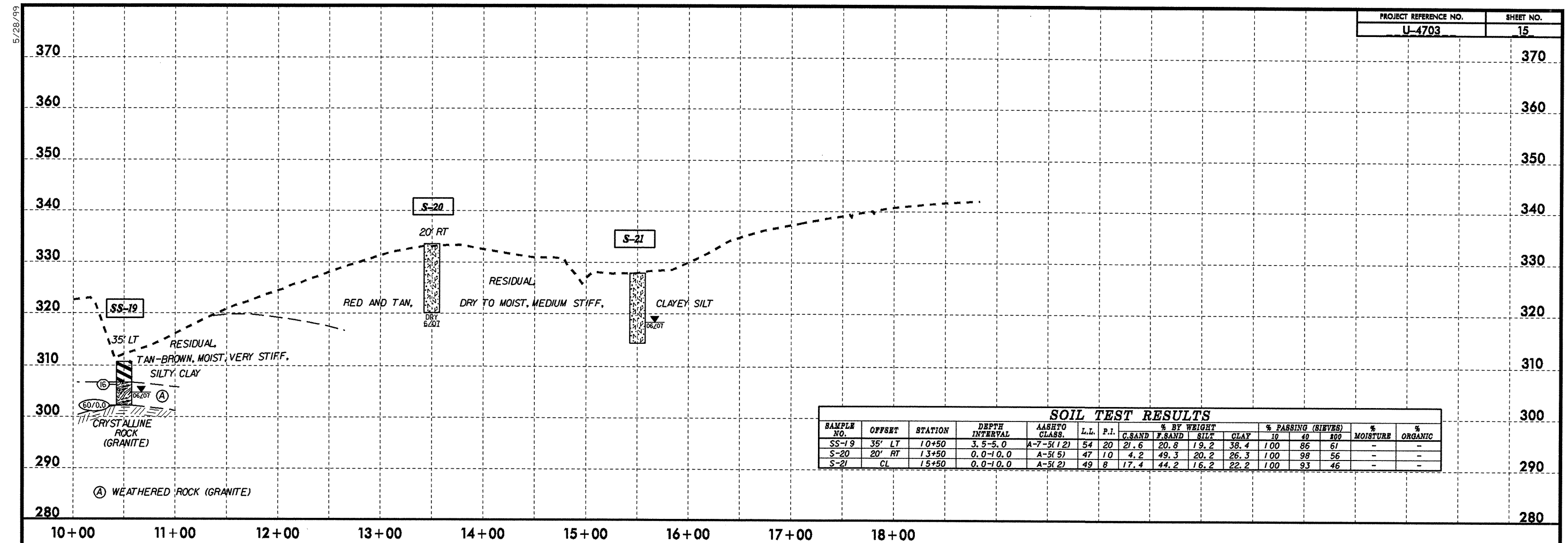
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SOIL TEST RESULTS

| SAMPLE NO. | OFFSET | STATION | DEPTH INTERVAL | LABORATORY CLASS. | L.L. | P.I. | % BY WEIGHT | | | | % PASSING (SIEVES) | | | % MOISTURE | % ORGANIC |
|------------|--------|---------|----------------|-------------------|------|------|-------------|---------|------|------|--------------------|----|-----|------------|-----------|
| | | | | | | | C. SAND | F. SAND | SILT | CLAY | 10 | 40 | 200 | | |
| S-7 | 50' RT | 67+00 | 9.0-13.0 | A-2-4(0) | 26 | 10 | 44.0 | 23.8 | 13.9 | 18.2 | 99 | 68 | 35 | - | - |
| SS-6 | 10' RT | 71+50 | 0.0-6.0 | A-2-4(0) | 36 | NP | 47.0 | 29.4 | 16.6 | 7.1 | 93 | 62 | 27 | - | - |
| S-3 | CL | 75+00 | 0.0-3.0 | A-6(1) | 34 | 14 | 43.0 | 21.8 | 12.9 | 22.2 | 99 | 70 | 38 | - | - |
| SS-4 | 20' RT | 77+00 | 3.5-5.0 | A-7-5(4) | 47 | 12 | 32.5 | 22.4 | 22.8 | 22.2 | 97 | 72 | 46 | - | - |
| SS-5 | 20' RT | 77+00 | 8.5-10.0 | A-4(2) | 38 | 9 | 32.7 | 24.8 | 24.2 | 18.2 | 98 | 75 | 46 | - | - |
| S-2 | 60' LT | 81+50 | 0.0-2.0 | A-6(4) | 37 | 19 | 34.7 | 24.4 | 12.5 | 28.3 | 99 | 80 | 44 | - | - |

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| SAMPLE NO. | OFFSET | STATION | DEPTH INTERVAL | AASHTO CLASS. | L.L. | P.I. | % BY WEIGHT | | | | % PASSING (SIEVES) | | | % MOISTURE | % ORGANIC |
|------------|--------|---------|----------------|---------------|------|------|-------------|---------|------|------|--------------------|----|----|------------|-----------|
| | | | | | | | G. SAND | F. SAND | SILT | CLAY | 10 | 40 | 80 | | |
| SS-19 | 35' LT | 10+50 | 3.5-5.0 | A-7-5(12) | 54 | 20 | 21.6 | 20.8 | 19.2 | 38.4 | 100 | 86 | 61 | - | - |
| S-20 | 20' RT | 13+50 | 0.0-10.0 | A-5(5) | 47 | 10 | 4.2 | 49.3 | 20.2 | 26.3 | 100 | 98 | 56 | - | - |
| S-21 | CL | 15+50 | 0.0-10.0 | A-5(2) | 49 | 8 | 17.4 | 44.2 | 16.2 | 22.2 | 100 | 93 | 46 | - | - |

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350
GROUND SURFACE
RESIDUAL TAN AND RED-BROWN, DRY, STIFF TO VERY STIFF, SILTY CLAY
PROPOSED GRADE
TAN, PINK, AND WHITE, DRY, LOOSE TO MEDIUM DENSE, SILTY SAND

59+50

| SOIL TEST RESULTS | | | | | | | | | | | | | | |
|-------------------|--------|---------|----------------|----------------|------|------|-------------|------|------|---------|--------------------|--------|------------|-----------|
| SAMPLE NO. | OFFSET | STATION | DEPTH INTERVAL | UNIFORM CLASS. | L.L. | P.I. | % BY WEIGHT | | | | % PASSING (NO. 20) | | % MOISTURE | % ORGANIC |
| | | | | | | | CLAY | SILT | SAND | FLY ASH | NO. 40 | NO. 60 | | |
| SS-10 | CL | 59+00 | 3'-6"-5'-0" | A-2-6(II) | 43 | 20 | 21.8 | 27.1 | 26.3 | 100 | 57 | 53 | - | - |
| SS-11 | CL | 59+00 | 6'-5"-10'-0" | A-2-4(0) | 37 | 16 | 15.3 | 15.9 | 6.7 | 100 | 80 | 27 | - | - |

350
GROUND SURFACE
RESIDUAL TAN AND RED-BROWN, DRY, STIFF TO VERY STIFF, SILTY CLAY
PROPOSED GRADE
TAN, PINK, AND WHITE, DRY, LOOSE TO MEDIUM DENSE, SILTY SAND

59+00

350
GROUND SURFACE
RESIDUAL TAN AND RED-BROWN, DRY, STIFF TO VERY STIFF, SILTY CLAY
PROPOSED GRADE
RESIDUAL TAN, PINK AND WHITE, DRY, LOOSE TO MEDIUM DENSE, SILTY SAND
WEATHERED ROCK (GRANITE)

58+50

-L-

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