

**PROJECT SPECIAL PROVISIONS****ROADWAY****SHOULDER RECONSTRUCTION PER SHOULDER MILE:**

(1-18-00) (Rev 1-19-10)

R1 R07A

**Description**

This work consists of reconstructing each shoulder (including median shoulders as applicable) in accordance with Roadway Standard Nos. 560.01 and 560.02 except that the rate of slope and width will be as shown on typical section, or to the existing shoulder point, whichever is nearer, as long as the desired typical is achieved, and when completed, seeding and mulching. This work shall be performed immediately after the resurfacing operations are complete as directed by the Engineer.

**Materials**

The Contractor shall furnish all earth material necessary for the construction of the shoulders. Provide soil with a P.I. greater than 6 and less than 25 and with a pH ranging from 5.5 to 6.8 and capable of supporting vegetation. Remove stones and other foreign material 2 inches or larger in diameter. All soil is subject to test and acceptance or rejection by the Engineer.

**Construction Methods**

Obtain material from within the project limits or approved borrow source. Prior to adding borrow material, the existing shoulder shall be scarified to provide the proper bond and shall be compacted to the satisfaction of the Engineer.

Any excess material generated by the shoulder reconstruction shall be disposed of by the Contractor in an approved disposal site.

**Measurement and Payment**

*Shoulder Reconstruction* will be measured and paid for as the actual number of miles of shoulders that have been reconstructed. Measurement will be made along the surface of each shoulder to the nearest 0.01 of a mile. Such price will include disposing of any excess material in an approved disposal site, and for all labor, tools, equipment, and incidentals necessary to complete the work.

*Borrow Excavation* will be paid for in accordance with Section 230 of the *Standard Specifications* for earth material furnished by the Contractor. The requirements of Article 104-5 of the *Standard Specifications* pertaining to revised contract prices for overrunning minor items will not apply to the item of *Borrow Excavation*.

*Incidental Stone Base* will be measured and paid for as provided in Article 545-6 of the *Standard Specifications*.

*Seeding and Mulching* will be measured and paid for as shown elsewhere in the contract documents.

Payment will be made under:

| <b>Pay Item</b>         | <b>Pay Unit</b> |
|-------------------------|-----------------|
| Shoulder Reconstruction | Shoulder Mile   |
| Borrow Excavation       | Cubic Yard      |

**FINAL ACCEPTANCE AND FOURTEEN DAY OBSERVATION PERIOD:**

(7-1-95)

R1 R13

Upon completion of construction as shown on each map, a 14 day observation period is required before acceptance. During the 14-day period, warrant the resurfaced area against failure.

No payment will be made for replacing failed pavement, as the cost of it will be considered incidental to the work initially paid for under the various items in the contract.

Completion and final acceptance of the project is contingent upon successful completion of the Observation Period. The observation period will be considered a part of the work required to be completed by the final completion date specified herein.

**CONSTRUCTION SEQUENCE:**

(7-1-95)

R1 R34

Pave each section of roadway begun in a continuous operation. Do not begin work on another section of roadway unless satisfactory progress is being made toward completion of intersections and all other required incidental work by satisfactorily furnishing additional paving equipment and personnel, except for milling and patching operations.

**INCIDENTAL STONE BASE:**

(7-1-95) (Rev.7-18-06)

R5 R28

**Description**

Place incidental stone base on driveways, mailboxes, etc. immediately after paving and do not have the paving operations exceed stone base placement by more than one week without written permission of the Engineer.

**Materials and Construction**

Provide and place incidental stone base in accordance with the requirements of Section 545 of the *2006 Standard Specifications*.

## Measurement and Payment

*Incidental Stone Base* will be measured and paid for in accordance with Article 545-6 of the *2006 Standard Specifications*.

### **ASPHALT PAVEMENTS - SUPERPAVE:**

(7-18-06)(Rev 10-20-09)

R6 R01

Revise the *2006 Standard Specifications* as follows:

**Page 6-2, Article 600-9 Measurement and Payment, delete the second paragraph.**

**Page 6-12, Subarticle 609-5(C)2, Required Sampling and Testing Frequencies, first partial paragraph at the top of the page, delete last sentence and add the following:**

If the Engineer allows the mix to remain in place, payment will be made in accordance with Article 105-3.

**Page 6-12, Subarticle 609-5(C)2, QUALITY CONTROL MINIMUM SAMPLING AND TESTING SCHEDULE**

**First paragraph, delete and replace with the following.**

Sample and test the completed mixture from each mix design per plant per year at the following minimum frequency during mix production:

**Second paragraph, delete the fourth sentence, and replace with the following**

When daily production of each mix design exceeds 100 tons and a regularly scheduled full test series random sample location for that mix design does not occur during that day's production, perform at least one partial test series consisting of Items A and B in the schedule below.

**Page 6-12, Subarticle 609-5(C)2(c) Maximum Specific Gravity, add after (AASHTO T 209):**

*or ASTM D 2041*

**Page 6-13, last line and on page and Page 6-14, Subarticle 609-5(C)(2)(e) Tensile Strength Ratio (TSR), add a heading before the first paragraph as follows:**

- (i) Option 1

**Insert the following immediately after the first paragraph:****(ii) Option 2**

Mix sampled from truck at plant with one set of specimens prepared by the Contractor and then tested jointly by QA and QC at a mutually agreed upon lab site within the first 7 calendar days after beginning production of each new mix design.

**Second paragraph, delete and replace with the following:**

Test all TSR specimens required by either option noted above on either a recording test press or a test press that maintains the peak load reading after the specimen has broken.

**Subarticle 609-5(C)(3) Control Charts, delete the second sentence of the first paragraph and replace with the following:**

For mix incorporated into the project, record full test series data from all regularly scheduled random samples or directed samples that replace regularly scheduled random samples, on control charts the same day the test results are obtained.

**Page 6-15, Subarticle 609-5(C)(3) Control Charts, first paragraph on this page, delete the last sentence and substitute the following:**

Denote the moving average control limits with a dash green line and the individual test limits with a dash red line.

**Subarticle 609-5(C)(3)(a), (b) and (c), replace (a) (b) and (c) with the following:**

- (a) A change in the binder percentage, aggregate blend, or  $G_{mm}$  is made on the JMF, or,
- (b) When the Contractor elects to stop or is required to stop production after one or two moving average values, respectively, fall outside the moving average limits as outlined in subarticle 609-5(C)6 or,
- (c) If failure to stop production after two consecutive moving averages exceed the moving average limits occurs, but production does stop at a subsequent time, re-establish a new moving average beginning at the actual production stop point.

**Subarticle 609-5(C)(4) Control Limits, replace the first paragraph and the CONTROL LIMITS Table on page 6-16 with the following.**

The following are established as control limits for mix production. Apply the individual limits to the individual test results. Control limits for the moving average limits are based on a moving average of the last 4 data points. Apply all control limits to the applicable target source.

| <b>CONTROL LIMITS</b>                      |                  |                      |                  |
|--|------------------|----------------------|------------------|
| Mix Control Criteria                       | Target Source    | Moving Average Limit | Individual Limit |
| 2.36 mm Sieve                              | JMF              | ±4.0 %               | ±8.0 %           |
| 0.075mm Sieve                              | JMF              | ±1.5 %               | ±2.5 %           |
| Binder Content                             | JMF              | ±0.3 %               | ±0.7 %           |
| VTM @ N <sub>des</sub>                     | JMF              | ±1.0 %               | ±2.0 %           |
| VMA @ N <sub>des</sub>                     | Min. Spec. Limit | Min Spec. Limit      | -1.0%            |
| P <sub>0.075</sub> / P <sub>be</sub> Ratio | 1.0              | ±0.4                 | ±0.8             |
| %G <sub>mm</sub> @ N <sub>ini</sub>        | Max. Spec. Limit | N/A                  | +2.0%            |
| TSR  | Min. Spec. Limit | N/A                  | - 15%            |

**Page 6-16, Subarticle 609-5(C)(5) Warning Bands, delete this subarticle in its entirety.**

**Pages 6-16 through 6-19, Subarticle 609-5(C)(6), delete the word "warning" and substitute the words "moving average".**

**Page 6-16, Subarticle 609-5(C)(6) Corrective Actions, first paragraph, first sentence, delete and replace with the following:**

Immediately notify the Engineer when moving averages exceed the moving average limits.

**Page 6-17, third full paragraph, delete and replace with the following:**

Failure to stop production when required due to an individual mix test not meeting the specified requirements will subject all mix from the stop point tonnage to the point when the next individual test is back on or within the moving average limits, or to the tonnage point when production is actually stopped, whichever occurs first, to being considered unacceptable.

**Sixth full paragraph, delete the first, second, and third sentence and replace with the following:**

Immediately notify the Engineer when any moving average value exceeds the moving average limit. If two consecutive moving average values for any one of the mix control criteria fall outside the moving average limits, cease production of that mix, immediately notify the Engineer of the stoppage, and make adjustments. The Contractor may elect to stop production after only one moving average value falls outside the moving average limits.

**Page 6-18, Subarticle 609-5(C)(6) Corrective Actions second full paragraph, delete and replace with the following:**

If the process adjustment improves the property in question such that the moving average after four additional tests is on or within the moving average limits, the Contractor may continue production with no reduction in payment

**Page 6-18, delete the third and fourth full paragraphs, including the Table for Payment for Mix Produced in the Warning Bands and substitute the following:**

If the adjustment does not improve the property in question such that the moving average after four additional individual tests is outside the moving average limits, the mix will be evaluated for acceptance in accordance with Article 105-3. Reduced payment for or removal of the mix in question will be applied starting from the plant sample tonnage at the stop point to the sample tonnage when the moving average is on or within the moving average limits. In addition, any mix that is obviously unacceptable will be rejected for use in the work.

**Page 6-19, First paragraph, delete and replace with the following:**

Failure to stop production and make adjustments when required due to two consecutive moving average values falling outside the moving average limits will subject all mix produced from the stop point tonnage to the tonnage point when the moving average is back on or within the moving average limits or to the tonnage point when production is actually stopped, whichever occurs first, to being considered unacceptable. Remove this material and replaced with materials that comply with the Specifications at no additional costs to the Department, unless otherwise approved. Payment will be made for the actual quantities of materials required to replace the removed quantities, not to exceed the original amounts.

**Page 6-20, Subarticle 609-5(D)(1) General, delete the third full paragraph, and replace with the following:**

Perform the sampling and testing at the minimum test frequencies as specified above. Should the density testing frequency fail to meet the minimum frequency as specified above, all mix without the required density test representation will be considered unsatisfactory. If the Engineer allows the mix to remain in place, payment will be made in accordance with Article 105-3.

**Page 6-22, Subarticle 609-5(D)(4) Nuclear Gauge Density Procedures, third paragraph, insert the following as the second sentence:**

Determine the Daily Standard Count in the presence of the QA Roadway Technician or QA Nuclear Gauge Technician on days when a control strip is being placed.

**Page 6-23, Subarticle 609-5(D)(5) Limited Production Procedure, delete the first paragraph including (a), (b), (c) and substitute the following:**

Proceed on limited production when, for the same mix type and on the same contract, one of the following conditions occur (except as noted in the first paragraph below).

- (a) Two consecutive failing lots, except on resurfacing\*
- (b) Three consecutive failing lots on resurfacing\*
- (c) Two consecutive failing nuclear control strips.

\* Resurfacing is defined as the first new uniform layer placed on an existing pavement.

**Page 6-25, Article 609-6 Quality Assurance, Density Quality Assurance, insert the following items after item (E):**

- (F) By retesting Quality Control core samples from control strips (either core or nuclear) at a frequency of 100% of the frequency required of the Contractor;
- (G) By observing the Contractor perform all standard counts of the Quality Control nuclear gauge prior to usage each nuclear density testing day; or
- (H) By any combination of the above

**Page 6-28, Subarticle 610-3(A) Mix Design-General, delete the fourth and fifth paragraphs and replace with the following:**

Reclaimed Asphalt Pavement (RAP) or Reclaimed Asphalt Shingles (RAS) may be incorporated into asphalt plant mixes in accordance with Article 1012-1 and the following applicable requirements.

Reclaimed asphalt pavement (RAP) may constitute up to 50% of the total material used in recycled mixtures, except for mix Type S 12.5D, Type S 9.5D, and mixtures containing reclaimed asphalt shingle material (RAS). Reclaimed asphalt shingle (RAS) material may constitute up to 6% by weight of total mixture for any mix. When both RAP and RAS are used, do not use a combined percentage of RAS and RAP greater than 20% by weight of total mixture, unless otherwise approved. When the percent of binder contributed from RAS or a combination of RAS and RAP exceeds 20% but not more than 30% of the total binder in the completed mix, the virgin binder PG grade shall be one grade below (both high and low temperature grade) the binder grade specified in Table 610-2 for the mix type, unless otherwise approved. When the percent of binder contributed from RAS or a combination of RAS and RAP exceeds 30% of the total binder in the completed mix, the Engineer will establish and approve the virgin binder PG grade. Use approved methods to determine if any binder grade adjustments are necessary to achieve the performance grade for the specified mix type.

For Type S 12.5D and Type S 9.5D mixes, the maximum percentage of reclaimed asphalt material is limited to 20% and shall be produced using virgin asphalt binder grade PG 76-22. For all other recycled mix types, the virgin binder PG grade shall be as specified in Table 610-2A for the specified mix type.

When the percentage of RAP is greater than 20% but not more than 30% of the total mixture, use RAP meeting the requirements for processed or fractionated RAP in accordance with the requirements of Section 1012-1.

When the percentage of RAP is greater than 30% of the total mixture, use an approved stockpile of RAP in accordance with Section 1012-1(C). Use approved test methods to determine if any binder grade adjustments are necessary to achieve the performance grade for the specified mix type. The Engineer will establish and approve the virgin asphalt binder grade to be used.



## Page 6-34, Subarticle 610-3(C),

Delete Table 610-2 and associated notes. Substitute the following:

**TABLE 610-2**  
**SUPERPAVE MIX DESIGN CRITERIA**

| Mix Type      | Design ESALs Millions (a)   | Binder PG Grade (b) | Compaction Levels No. Gyration @ |                  | Max. Rut Depth (mm) | Volumetric Properties (c)                           |            |                 |                                     |
|---------------|---|---------------------|----------------------------------|------------------|---------------------|---|------------|-----------------|-------------------------------------|
|               |   |                     | N <sub>ini</sub>                 | N <sub>des</sub> |                     | VMA % Min.  | VTM %      | VFA Min. - Max. | %G <sub>mm</sub> @ N <sub>ini</sub> |
| S-4.75A(e)    | < 0.3   | 64 -22              | 6                                | 50               | -----               | 20.0  | 7.0 - 15.0 | -----           | -----                               |
| SF-9.5A       | < 0.3   | 64 -22              | 6                                | 50               | 11.5                | 16.0  | 3.0 - 5.0  | 70 - 80         | ≤ 91.5                              |
| S-9.5B        | 0.3 - 3   | 64 -22              | 7                                | 65               | 9.5                 | 15.5  | 3.0 - 5.0  | 65 - 80         | ≤ 90.5                              |
| S-9.5C        | 3 - 30  | 70 -22              | 7                                | 75               | 6.5                 | 15.5  | 3.0 - 5.0  | 65 - 78         | ≤ 90.5                              |
| S-9.5D        | > 30  | 76 -22              | 8                                | 100              | 4.5                 | 15.5  | 3.0 - 5.0  | 65 - 78         | ≤ 90.0                              |
| S-12.5C       | 3 - 30  | 70 -22              | 7                                | 75               | 6.5                 | 14.5  | 3.0 - 5.0  | 65 - 78         | ≤ 90.5                              |
| S-12.5D       | > 30  | 76 -22              | 8                                | 100              | 4.5                 | 14.5  | 3.0 - 5.0  | 65 - 78         | ≤ 90.0                              |
| I-19.0B       | < 3   | 64 -22              | 7                                | 65               | -----               | 13.5  | 3.0 - 5.0  | 65 - 78         | ≤ 90.5                              |
| I-19.0C       | 3 - 30  | 64 -22              | 7                                | 75               | -----               | 13.5  | 3.0 - 5.0  | 65 - 78         | ≤ 90.0                              |
| I-19.0D       | > 30  | 70 -22              | 8                                | 100              | -----               | 13.5  | 3.0 - 5.0  | 65 - 78         | ≤ 90.0                              |
| B-25.0B       | < 3   | 64 -22              | 7                                | 65               | -----               | 12.5  | 3.0 - 5.0  | 65 - 78         | ≤ 90.5                              |
| B-25.0C       | > 3   | 64 -22              | 7                                | 75               | -----               | 12.5  | 3.0 - 5.0  | 65 - 78         | ≤ 90.0                              |
| All Mix Types | <b>Design Parameter</b><br>1. Dust to Binder Ratio (P <sub>0.075</sub> / P <sub>be</sub> )<br>2. Retained Tensile Strength (TSR) (AASHTO T283 Modified) |                     |                                  |                  |                     | <b>Design Criteria</b><br>0.6 - 1.4<br>85% Min. (d) |            |                 |                                     |

- Notes:
- (a) Based on 20 year design traffic.
  - (b) When Recycled Mixes are used, select the binder grade to be added in accordance with Subarticle 610-3(A).
  - (c) Volumetric Properties based on specimens compacted to N<sub>des</sub> as modified by the Department.
  - (d) AASHTO T 283 Modified (No Freeze-Thaw cycle required). TSR for Type S 4.75A, Type B 25.0B, and Type B 25.0C mixes is 80% minimum.
  - (e) Mix Design Criteria for Type S 4.75A may be modified subject to the approval of the Engineer.

**Page 6-34, Insert the following immediately after Table 610-2:**

**TABLE 610-2A  
SUPERPAVE MIX DESIGN CRITERIA**

| Mix Type                                | Percentage of RAP in Mix |                                    |                            |
|---|--------------------------|------------------------------------|----------------------------|
|   | Category 1<br>% RAP ≤20% | Category 2<br>20.1% ≤ %RAP ≤ 30.0% | Category 3<br>%RAP > 30.0% |
| All A and B Level Mixes, I19.0C, B25.0C | PG 64 -22                | PG 64 -22                          | TBD                        |
| S9.5C, S12.5C, I19.0D                   | PG 70 -22                | PG 64-22                           | TBD                        |
| S 9.5D and S12.5D                       | PG 76-22                 | N/A                                | N/A                        |

- Note: (1) Category 1 RAP has been processed to a maximum size of 2 inches.  
 (2) Category 2 RAP has been processed to a maximum size of 1 inch by either crushing and or screening to reduce variability in the gradations.  
 (3) Category 3 RAP has been processed to a maximum size of 1 inch, fractionating the RAP into 2 or more sized stockpiles

**Page 6-35, Table 610-3 delete and replace with the following:**

**TABLE 610-3  
ASPHALT PLACEMENT- MINIMUM TEMPERATURE REQUIREMENTS**

| Asphalt Concrete Mix Type           | Minimum Air Temperature | Minimum Surface Temperature |
|-------------------------------------|-------------------------|-----------------------------|
| ACBC, Type B 25.0B, C, B 37.5C      | 35°F                    | 35°F                        |
| ACIC, Type I 19.0B, C, D            | 35°F                    | 35°F                        |
| ACSC, Type S 4.75A, SF 9.5A, S 9.5B | 40°F                    | 50°F*                       |
| ACSC, Type S 9.5C, S 12.5C          | 45°F                    | 50°F                        |
| ACSC, Type S 9.5D, S 12.5D          | 50°F                    | 50°F                        |

\* 35°F if surface is soil or aggregate base for secondary road construction.

**Page 6-44, Article 610-8 Spreading and Finishing, third full paragraph, replace the first sentence with the following:**

Use the 30 foot minimum length mobile grade reference system or the non-contacting laser or sonar type ski *with at least four referencing stations mounted on the paver at a minimum length of 24 feet* to control the longitudinal profile when placing the initial lanes and all adjacent lanes of all layers, including resurfacing and asphalt in-lays, unless otherwise specified or approved.

**Page 6-50, Article 610-13 Density Acceptance, delete the second paragraph and replace with the following:**

As an exception, when the first layer of mix is a surface course and is being placed directly on an unprimed aggregate or soil base, the layer will be included in the "Other" construction category.

**Page 6-50, Article 610-13 Density Acceptance, delete the formula and description in the middle of the page and replace with the following:,**

|        |    |  |
|--------|----|--|
| where: | PF | = $100 - 10(D)^{1.465}$  |
|        | PF | = Pay Factor (computed to 0.1%)                                    |
|        | D  | = the deficiency of the lot average density,<br>not to exceed 2.0% |

**Page 6-53, Article 620-4 Measurement and Payment:**

**Sixth paragraph, delete the last sentence.**

**Seventh paragraph, delete the paragraph and replace with the following:**

The adjusted contract unit price will then be applied to the theoretical quantity of asphalt binder authorized for use in the plant mix placed during the partial payment period involved, except that where recycled plant mix is used, the adjusted unit price will be applied only to the theoretical number of tons of additional asphalt binder materials required by the job mix formula.

**Page 6-54, Article 620-4 Measurement and Payment, add the following pay item:**

|  |                 |
|--|-----------------|
| <b>Pay Item</b>                              | <b>Pay Unit</b> |
| Asphalt Binder for Plant Mix, Grade PG 70-28 | Ton             |

**Page 6-69, Table 660-1 Material Application Rates and Temperatures, add the following:**

| Type of Coat | Grade of Asphalt | Asphalt Rate gal/yd <sup>2</sup> | Application Temperature °F | Aggregate Size | Aggregate Rate lb./sq. yd. Total |
|--------------|------------------|----------------------------------|----------------------------|----------------|----------------------------------|
| Sand Seal    | CRS-2 or CRS-2P  | 0.22-0.30                        | 150-175                    | Blotting Sand  | 12-15                            |

**Page 6-75, Subarticle 660-9(B), add the following as sub-item (5)**

(5) Sand Seal

Place the fully required amount of asphalt material in one application and immediately cover with the seal coat aggregate. Uniformly spread the fully required amount of aggregate in one application and correct all non-uniform areas prior to rolling.

Immediately after the aggregate has been uniformly spread, perform rolling.

When directed, broom excess aggregate material from the surface of the seal coat.

When the sand seal is to be constructed for temporary sealing purposes only and will not be used by traffic, other grades of asphalt material meeting the requirements of Articles 1020-6 and 1020-7 may be used in lieu of the grade of asphalt required by Table 660-1 when approved.

**Page 6-76, Article 661-1 Description, add the following as the 2nd paragraph:**

Provide and conduct the quality control and required testing for acceptance of the UBWC in accordance with "Quality Management System for Asphalt Pavements (OGAFC, PADL, and Ultra-Thin HMA Version)", included in the contract.

**Page 6-80, Subarticle 661-3(A) Equipment, add the following as the first paragraph:**

Use asphalt mixing plants in accordance with Article 610-5.

**Page 10-41, Table 1012-1, delete the last row of entries for OGAFC and add the following:**

| Mix Type | Coarse Aggregate Angularity <sup>(b)</sup><br>ASTM D5821 | Fine Aggregate Angularity % Minimum<br>AASHTO T304 Method A | Sand Equivalent % Minimum<br>AASHTO T176 | Flat & Elongated 5:1 Ratio % Maximum<br>ASTM D4791 Section 8.4 |
|----------|--|---|--|--|
| S 9.5 D  | 100/100  | 45  | 50                                       | 10   |
| OGAFC    | 100/100  | N/A   | N/A                                      | 10   |
| UBWC     | 100/85   | 40  | 45                                       | 10   |

**Delete Note (c) under the Table 1012-1 and replace with the following:**

(c) Does not apply to Mix Types SF 9.5A and S 9.5B.

**Page 10-43, Subarticle 1012-1(F): Reclaimed Asphalt Shingle Material (RAS), insert the following immediately following the first paragraph:**

(1) Mix Design RAS

Incorporate RAS from stockpiles that have been tested for uniformity of gradation and binder content prior to use in an asphalt mix design.

(2) Mix Production RAS

New Source RAS is defined as acceptable material which was not included in the stockpile when samples were taken for mix design purposes. Process new source RAS so that all materials will pass a 1/2" sieve prior to introduction into the plant mixer unit.

After a stockpile of processed RAS has been sampled and mix designs made from these samples, do not add new source RAS to the original stockpile without prior field testing to insure gradation and binder uniformity. Sample and test new source RAS before blending with the existing stockpile.

Store new source RAS in a separate stockpile until the material can be sampled and tested for comparison with the original recycled mix design data. New source RAS may also be placed against the existing stockpile in a linear manner provided it is sampled for mix design conformity prior to its use in the recycled mix.

RAS contamination including but not limited to excessive dirt, debris, clean stone, concrete will not be allowed.

Field approval of new source RAS will be based on the table below and volumetric mix properties on the mix with the new source RAS included. Provided these tolerances are met, volumetric properties of the new mix will then be performed. If all volumetric mix properties meet the mix design criteria for that mix type, the new source RAS may continue to be used.

If the gradation, binder content, or any of the volumetric mix properties are not within the allowable tolerances of the table below, do not use the new source RAS unless approved by the Engineer. The Contractor may elect to either not use the stockpile, to request an adjustment to the JMF, or to redesign the mix.

**NEW SOURCE RAS GRADATION and BINDER TOLERANCES**  
(Apply Tolerances to Mix Design Data)

| 0-6% RAS         |           |
|------------------|-----------|
| P <sub>b</sub> % | ±1.6%     |
| Sieve Size (mm)  | Tolerance |
| 9.5              | ±1        |
| 4.75             | ±5        |
| 2.36             | ±4        |
| 1.18             | ±4        |
| 0.300            | ±4        |
| 0.150            | ±4        |
| 0.075            | ±2.0      |

Page 10-43 through 10-45, Subarticle 1012-1(G), delete this in its entirety and replace with the following:

**(G) Reclaimed Asphalt Pavement (RAP)**

**(1) Mix Design RAP**

Incorporate RAP from stockpiles or other sources that have been tested for uniformity of gradation and binder content prior to use in an asphalt mix design. Use reclaimed asphalt pavement that meets all requirements specified for *one of* the following *two* classifications.

**(a) Millings**

Existing reclaimed asphalt pavement (RAP) that is removed from its original location by a milling process as specified in Section 607. Millings should be such that it has a uniform gradation and binder content and all materials will pass a 2" sieve prior to introduction into the plant mixer unit.

**(b) Processed RAP**

RAP that is processed in some manner (possibly by crushing and/or use of a blending method) to produce a uniform gradation and binder content in the RAP prior to use in a recycled mix. Process RAP so that all materials have a uniform gradation and binder content and will pass a 1" sieve prior to introduction into the plant mixer unit.

**(c) Fractionated RAP**

Fractionated RAP is defined as having two or more RAP stockpiles, where the RAP is divided into coarse and fine fractions. Grade RAP so that all materials will pass a 1" sieve. The coarse RAP stockpile shall only contain material retained on a 3/8" screen, unless otherwise approved. The fine RAP stockpile shall only contain material passing the 3/8" screen, unless otherwise approved. The Engineer may allow the Contractor to use an alternate to the 3/8" screen to fractionate the RAP. The maximum percentages of fractionated RAP may be comprised of coarse, fine, or the combination of both. Utilize a separate cold feed bin for each stockpile of fractionated RAP used.

**(d) Approved Stockpiled RAP**

Approved Stockpiled RAP is defined as fractionated RAP which has been isolated and tested for asphalt content, gradation, and asphalt binder characteristics with the intent to be used in mix designs with greater than 30% RAP materials. Fractionate the RAP in accordance with Section 1012-1(G)(c). Utilize a separate cold feed bin for each approved stockpile of RAP used.

Perform extraction tests at a rate of 1 per 1000 tons of RAP, with a minimum of 5 tests per stockpile to determine the asphalt content and gradation. Separate stockpiles of RAP material by fine and coarse fractions. Erect and maintain a sign satisfactory to the Engineer on each stockpile to identify the material. Assure that no deleterious material is allowed in any stockpile. The Engineer may reject by visual inspection any stockpiles that are not kept clean, separated, and free of foreign materials.

Submit requests for RAP stockpile approval to the Engineer with the following information at the time of the request:

- (1) Approximate tons of materials in stockpile
- (2) Name or Identification number for the stockpile
- (3) Asphalt binder content and gradation test results
- (4) Asphalt characteristics of the Stockpile.

For the Stockpiled RAP to be considered for approval, the gradation and asphalt content shall be uniform. Individual test results, when compared to the target, will be accepted if within the tolerances listed below:

**APPROVED STOCKPILED RAP GRADATION and BINDER TOLERANCES**  
(Apply Tolerances to Mix Design Data)

| P <sub>b</sub> % | ±0.3%           |
|------------------|-----------------|
| Sieve Size (mm)  | Percent Passing |
| 25.0             | ±5%             |
| 19.0             | ±5%             |
| 12.5             | ±5%             |
| 9.5              | ±5%             |
| 4.75             | ±5%             |
| 2.36             | ±4%             |
| 1.18             | ±4%             |
| 0.300            | ±4%             |
| 0.150            | ±4%             |
| 0.075            | ±1.5%           |

Note: If more than 20% of the individual sieves are out of the gradation tolerances, or if more than 20% of the asphalt binder content test results fall outside the appropriate tolerances, the RAP shall not be used in HMA unless the RAP representing the failing tests is removed from the stockpile.

Do not add additional material to any approved RAP stockpile, unless otherwise approved by the Engineer.

Maintain at the plant site a record system for all approved RAP stockpiles. Include at a minimum the following: Stockpile identification and a sketch of all stockpile areas at the plant site; all RAP test results (including asphalt content, gradation, and asphalt binder characteristics).

**(2) Mix Production RAP**

During mix production, use RAP that meets the criteria for one of the following categories:

**(a) Mix Design RAP**

RAP contained in the mix design stockpiles as described above may be used in all applicable JMFs. These stockpiles have been pretested; however, they are subject to required QC/QA testing in accordance with Subarticle 609-5(C)(2).

**(b) New Source RAP**

New Source RAP is defined as any acceptable material that was not included in the stockpile or other source when samples were taken for mix design purposes. Process new source RAP so that all materials have a uniform gradation and binder content and will pass a 2" sieve prior to introduction into the plant mixer unit.

After a stockpile of millings, processed RAP, or fractionated RAP has been sampled and mix designs made from these samples, do not add new source RAP to the original stockpile without prior field testing to insure gradation and binder uniformity. Sample and test new source RAP before blending with the existing stockpile.

Store new source RAP in a separate stockpile until the material can be sampled and tested for comparison with the original recycled mix design data. New source RAP may also be placed against the existing stockpile in a linear manner provided it is sampled for mix design conformity prior to its use in the recycled mix.

Unprocessed RAP is asphalt material that was not milled and/or has not been processed to obtain a uniform gradation and binder content and is not representative of the RAP used during the applicable mix design. Unprocessed RAP shall not be incorporated into any JMFs prior to processing. Different sources of unprocessed RAP may be stockpiled together provided it is generally free of contamination and will be processed prior to use in a recycled mix. RAP contamination in the form of excessive dirt, debris, clean stone, concrete, etc. will not



be allowed. Incidental amounts of dirt, concrete, and clean stone may be acceptable. Unprocessed RAP may be processed and then classified as a new source RAP as described above.

Field approval of new source RAP will be based on Table 1012-2 below and volumetric mix properties on the mix with the new source RAP included. Provided the Table 1012-2 tolerances are met, volumetric properties of the new mix will then be performed. If all volumetric mix properties meet the mix design criteria for that mix type, the new source RAP may continue to be used.

If the gradation, binder content, or any of the volumetric mix properties are not within the allowable tolerances of Table 1012-2, do not use the new source RAP unless approved by the Engineer. The Contractor may elect to either not use the stockpile, to request an adjustment to the JMF, or to redesign the mix.

**TABLE 1012-2**  
**NEW SOURCE RAP GRADATION and BINDER TOLERANCES**  
(Apply Tolerances to Mix Design Data)

| Mix Type         | 0-20% RAP |        |       | 20 <sup>+</sup> -30 % RAP |        |       | 30 <sup>+</sup> % RAP |        |       |
|------------------|-----------|--------|-------|---------------------------|--------|-------|-----------------------|--------|-------|
|                  | Base      | Inter. | Surf. | Base                      | Inter. | Surf. | Base                  | Inter. | Surf. |
| P <sub>b</sub> % | ± 0.7%    |        |       | ± 0.4%                    |        |       | ± 0.3%                |        |       |
| 25.0             | ±10       | -      | -     | ±7                        | -      | -     | ±5                    | -      | -     |
| 19.0             | ±10       | ±10    | -     | ±7                        | ±7     | -     | ±5                    | ±5     | -     |
| 12.5             | -         | ±10    | ±10   | -                         | ±7     | ±7    | -                     | ±5     | ±5    |
| 9.5              | -         | -      | ±10   | -                         | -      | ±7    | -                     | -      | ±5    |
| 4.75             | ±10       | -      | ±10   | ±7                        | -      | ±7    | ±5                    | -      | ±5    |
| 2.36             | ±8        | ±8     | ±8    | ±5                        | ±5     | ±5    | ±4                    | ±4     | ±4    |
| 1.18             | ±8        | ±8     | ±8    | ±5                        | ±5     | ±5    | ±4                    | ±4     | ±4    |
| 0.300            | ±8        | ±8     | ±8    | ±5                        | ±5     | ±5    | ±4                    | ±4     | ±4    |
| 0.150            | -         | -      | ±8    | -                         | -      | ±5    | -                     | -      | ±4    |
| 0.075            | ±4        | ±4     | ±4    | ±2                        | ±2     | ±2    | ±1.5                  | ±1.5   | ±1.5  |
|                  |           |        |       |                           |        |       |                       |        |       |

**ASPHALT PAVEMENTS - WARM MIX ASPHALT SUPERPAVE:**

(5-19-09)

R6 R02

Warm Mix Asphalt (WMA) is defined as additives or processes that allow a reduction in the temperature at which asphalt mixtures are produced and placed. WMA is allowed for use at the Contractor's option when shown on the plans or in the project provisions.

Notify the Engineer at least 2 weeks before producing the Warm Mix so the Engineer can arrange a preconstruction meeting. Discuss special testing requirements necessary for warm mix asphalt at the pre-pave meeting. Included at the pre-pave meeting the Contractor's QC manager, Paving Superintendent, and manufacturer's representative for the process or additive used for producing warm mix asphalt, the Department's Roadway Construction Engineer, Resident Engineer, State Pavement Construction, and Quality Assurance Supervisor.

Require a manufacturer's representative for the process or additive used to be present on site at the plant during the initial production and on the roadway during the laydown of the warm mix asphalt.

Revise the *2006 Standard Specifications* as follows:

**Page 6-8, Article 609-1 Description, insert the following as the second paragraph.**

Warm Mix Asphalt (WMA) is defined as additives or processes that allow a reduction in the temperature at which asphalt mixtures are produced and placed. WMA is allowed for use at the Contractor's option when shown in the contract documents.

**Page 6-9, Article 609-4 Field Verification of Mixture and Job Mix Formula Adjustments,**

**Second paragraph, insert the following immediately after the first sentence.**

When producing a WMA, field verification testing will also consist of performing a Tensile Strength Ratio(TSR) testing in accordance with AASHTO T283 as Modified by the Department.

**Third paragraph, delete the third sentence and replace with the following:**

Verification is considered satisfactory for HMA when all volumetric properties except  $\%G_{mm}@N_{ini}$  are within the applicable mix design criteria, and the gradation, binder content, and  $\%G_{mm}@N_{ini}$  are within the individual limits for the mix type being produced. Verification is considered satisfactory for WMA when all volumetric properties except  $\%G_{mm}@N_{ini}$  are within the applicable mix design criteria, the TSR is equal to or above the minimum design criteria, and the gradation, binder content, and  $\%G_{mm}@N_{ini}$  are within the individual limits for the mix type being produced.

**Page 6-12, Subarticle 609-5(C)2(d) Bulk Specific Gravity of Compacted Specimens, add after (AASHTO T 312):**

When producing Warm Mix Asphalt, gyrate specimens to specified  $N_{des}$  compaction effort without reheating mix other than to desired compaction temperature. Record time needed to reheat samples (if any).

**Page 6-14, Subarticle 609-5(C)(2)(e) Tensile Strength Ratio, insert the following immediately after the third paragraph:**

- When producing WMA, perform TSR testing at
  - i. Beginning of production for each JMF
  - ii. Monthly thereafter

**Page 6-27, Article 610-1 Description, insert the following as the third paragraph:**

Warm Mix Asphalt (WMA) is defined as additives or processes that allow a reduction in the temperature at which asphalt mixtures are produced and placed. Use WMA at the Contractor’s option unless otherwise shown on the plans.

**Page 6-27, Article 610-2 Materials, insert the following at the end of this Article:**

Use only WMA additives or processes listed on the Department’s approved list maintained by the Materials and Tests Unit.

**Page 6-31, Subarticle 610-3(B) Mix Design-Criteria, add the following as the fifth paragraph:**

When WMA is used, submit the mix design without including the WMA additive.

**Page 6-32, Subarticle 610-3(C) Job Mix Formula, Add the following as the second paragraph:**

When WMA is used, document the additive or process used and recommended rate on the JMF submittal. Verify the JMF based on plant produced mixture from the trial batch.

**Immediately following PG 76-22 335°F, add the following paragraph:**

When WMA is used, produce an asphalt mixture within the temperature range of 225°F and 275 °F.

**ASPHALT BINDER CONTENT OF ASPHALT PLANT MIXES:**

(1-1-02)

R6 R15

The approximate asphalt binder content of the asphalt concrete plant mixtures used on this project will be as follows:

|                                      |              |       |
|--------------------------------------|--------------|-------|
| Asphalt Concrete Base Course         | Type B 25.0  | 4.3 % |
| Asphalt Concrete Intermediate Course | Type I 19.0  | 4.7 % |
| Asphalt Concrete Surface Course      | Type S 4.75A | 7.0 % |
| Asphalt Concrete Surface Course      | Type SF 9.5A | 6.5 % |
| Asphalt Concrete Surface Course      | Type S 9.5   | 6.0 % |
| Asphalt Concrete Surface Course      | Type S 12.5  | 5.5 % |

The actual asphalt binder content will be established during construction by the Engineer within the limits established in the *2006 Standard Specifications*.

**ASPHALT PLANT MIXTURES:**

(7-1-95)

R6 R20

Place asphalt concrete base course material in trench sections with asphalt pavement spreaders made for the purpose or with other equipment approved by the Engineer.

**PRICE ADJUSTMENT - ASPHALT BINDER FOR PLANT MIX:**

(11-21-00)

R6 R25

Price adjustments for asphalt binder for plant mix will be made in accordance with Section 620 of the *2006 Standard Specifications*.

The base price index for asphalt binder for plant mix is \$ **419.67** per ton.

This base price index represents an average of F.O.B. selling prices of asphalt binder at supplier's terminals on **December 1, 2009**.

**ASPHALT CONCRETE SURFACE COURSE COMPACTION:**

(7-1-95)

R6 R49

Compact the asphalt surface course on this project in accordance with Subarticle 610-9 of the *2006 Standard Specifications* and the following provision:

Perform the first rolling with a steel wheel roller followed by rolling with a self-propelled pneumatic tired roller with the final rolling by a steel wheel roller.

**RESURFACING EXISTING BRIDGES:**

(7-1-95)

R6 R61 (Rev.)

The Contractor's attention is directed to the fact that he will be required to resurface the bridges on this project if directed by the Engineer.

Place the surface so as to follow a grade line set by the Engineer with the minimum thickness as shown on the sketch herein or as directed by the Engineer. State Forces will make all necessary repairs to the bridge floors prior to the time that the Contractor places the proposed surfacing. Give the Engineer at least 15 days notice prior to the expected time to begin operations so that State Forces will have sufficient time to complete their work.

At all bridges that are not to be resurfaced, **mill a taper into existing pavement for a length of 25' per inch of final surface. A temporary asphalt wedge will be required immediately after milling to ensure smooth travel if the final layer of surface course is not placed on the same day as milling.**

**TRENCHING FOR BASE COURSE:**

(7-1-95)

R6 R79 A

Perform all trenching necessary to place the asphalt concrete base course widening in accordance with the typical sections, at locations shown on the sketch maps, and as directed by the Engineer.

Perform the trenching for the base course on the same day that the base course is to be placed. If the base course cannot be placed on the same day the trench section is excavated, backfill the trench with earth material and compact it to the satisfaction of the Engineer. Once the trench is open, perform backfilling and re-opening of the trench at no cost to the Department.

The Contractor will be restricted to widening one side of the project at a time unless otherwise permitted by the Engineer. In widening, operate equipment and conduct operations in the same direction as the flow of traffic.

Density tests may be taken every 2000 feet in the widened areas as directed by the Engineer. Shape and compact the subgrade in the widened areas to the satisfaction of the Engineer. Compact the asphalt concrete base course in the widened areas in accordance with the provisions of Article 610-9 of the *2006 Standard Specifications*.

Place the excavated material from trenching operation on the adjacent shoulder area as directed by the Engineer. Cut adequate weep holes in the excavated material to provide for adequate drainage as directed by the Engineer. Remove all excavated material from all drives to provide ingress and egress to abutting properties and from in front of mailboxes and paper boxes. Saw a neat edge and remove all asphalt and/or concrete driveways, and existing asphalt widening, as directed by the Engineer, to the width of the widening and dispose of any excavated concrete or asphalt materials. Properly reconnect driveways.

Upon completion of the paving operation, backfill the trench to the satisfaction of the Engineer. Properly dispose of any excess material remaining after this operation.

No direct payment will be made for trenching, sawing, and removal of driveways, depositing material on shoulder area, backfilling trench, or removal of spoil material, as the cost of this work shall be included in the contract unit price per ton for *Asphalt Concrete Base Course*, Type \_\_\_\_.

**PATCHING EXISTING PAVEMENT:**

(1-15-02) (Rev.7-18-06)

R6 R88

**Description**

The Contractor's attention is directed to the fact that there are areas of existing pavement on this project that will require repair prior to resurfacing. Patch the areas that, in the opinion of the Engineer, need repairing. The areas to be patched will be delineated by the Engineer prior to the Contractor performing repairs.

**Materials**

The patching consists of Asphalt Concrete Base Course, Asphalt Concrete Intermediate Course, Asphalt Concrete Surface Course, or a combination of base, binder and surface course.

**Construction Methods**

Remove existing pavement at locations directed by the Engineer in accordance with Section 250 of the *Standard Specifications*.

Place Asphalt Concrete Base Course, in lifts not exceeding 5 1/2 inches. Utilize compaction equipment suitable for compacting patches as small as 3.5 feet by 6 feet on each lift. Use an approved compaction pattern to achieve proper compaction. If patched pavement is to be open to traffic for more than 48 hours prior to overlay, use Asphalt Surface Course in the top 1.25 inches of the patch.

Schedule operations so that all areas where pavement has been removed, will be repaired on the same day of the pavement removal and all lanes of traffic restored.

**Measurement and Payment**

*Patching Existing Pavement* will be measured and paid for as the actual number of tons of asphalt plant mix complete in place that has been used to make completed and accepted repairs. The asphalt plant mixed material will be measured by being weighed in trucks on certified platform scales or other certified weighing devices. The above price and payment will be full compensation for all work covered by this provision, including but not limited to removal and disposal of pavement; furnishing and applying tack coat; furnishing, placing, and compacting of asphalt plant mix; furnishing of asphalt binder for the asphalt plant mix; and furnishing scales.

Patching Existing Pavement will be considered a minor item. In the event that the item of Patching Existing Pavement overruns the original bid quantity by more than 100 percent, the provisions of Article 104-5 of the *Standard Specifications* pertaining to revised contract unit price for overrunning minor items will not apply to this item. Any provisions included in the contract that provides for adjustments in compensation due to variations in the price of asphalt binder will not be applicable to payment for the work covered by this provision.

Payment will be made under:

| <b>Pay Item</b>            | <b>Pay Unit</b> |
|----------------------------|-----------------|
| Patching Existing Pavement | Ton             |

**BORROW EXCAVATION AND SHPO DOCUMENTATION FOR BORROW/WASTE****SITES:**

(12-18-07) (4-15-08)

R8 R02

Revise the *2006 Standard Specifications* as follows:

**Division 2 Earthwork**

**Page 2-16, Subarticle 230-1(D)**, add the words: *The Contractor specifically waives* as the first words of the sentence.

**Page 2-17, Article 230-4(B) Contractor Furnished Sources, first paragraph, first sentence** replace with the following:

Prior to the approval of any borrow sources developed for use on any project, obtain certification from the State Historic Preservation Officer of the State Department of Cultural Resources certifying that the removal of the borrow material from the borrow sources(s) will have no effect on any known district, site building, structure, or object, architectural and/or archaeological that is included or eligible for inclusion in the National Register of Historic Places.

**Division 8 Incidentals**

**Page 8-9, Article 802-2 General Requirements, add the following as the 1st paragraph:**

Prior to the removal of any waste from any project, obtain certification from the State Historic Preservation Officer of the State Department of Cultural Resources certifying that the deposition of the waste material to the proposed waste area will have no effect on any known district, site building, structure, or object, architectural and/or archaeological that is included or eligible for inclusion in the National Register of Historic Places. Furnish a copy of this certification to the Engineer prior to performing any work in the proposed waste site.

**Page 8-10, Article 802-2, General Requirements, 4th paragraph, add the following as the 2nd sentence:**

The Department's borrow and waste site reclamation procedures for contracted projects is available on the NCDOT website and shall be used for all borrow and waste sites on this project.

**ADJUSTMENT OF MANHOLES AND WATER VALVES (CITY OF DURHAM)**

The Contractor's attention is directed to Section 858-3 of the *2006 Standard Specifications* and to included Details A-1 and B-2.

Concrete shall be Class A Concrete, 3000 psi and shall meet the requirements of Section 1000 of the *2006 Standard Specifications*.

The use of cast iron or steel fittings in the adjustment of manholes will not be permitted on this project except where it is considered by the Engineer to be in the best interest of the Department to allow rings to be used. When rings are permitted for the adjustment of manholes, the rings shall have satisfactory bearing on the existing manhole frames and 50 percent of the

circumference shall be tack welded at four equally spaced locations as directed by the Engineer. If the existing covers do not fit the rings, furnish and install new covers at no additional expense to the Department.

**AGGREGATE PRODUCTION:**

(11-20-01) (Rev. 11-21-06)

R10 R05

Provide aggregate from a producer who uses the current Aggregate Quality Control/Quality Assurance Program which is in effect at the time of shipment.

No price adjustment is allowed to contractors or producers who use the program. Participation in the program does not relieve the producer of the responsibility of complying with all requirements of the *2006 Standard Specifications*. Copies of this procedure are available upon request from the Materials and Test Unit.

**CONCRETE BRICK AND BLOCK PRODUCTION:**

(11-20-01) (Rev. 11-21-06)

R10 R10

Provide concrete brick and block from a producer who uses the current Solid Concrete Masonry Brick/Unit Quality Control/Quality Assurance Program that is in effect on the date that material is received on the project.

No price adjustment is allowed to contractors or producers who use the program. Participation in the program does not relieve the producer of the responsibility of complying with all requirements of the *2006 Standard Specifications*. Copies of this procedure are available upon request from the Materials and Test Unit.

**GLASS BEADS:**

(7-18-06)

R10 R35

Revise the *2006 Standard Specifications* as follows:

**Page 10-223, 1087-4(C) Gradation & Roundness**

Replace the second sentence of the first paragraph with the following:

*All Drop-On and Intermixed Glass Beads shall be tested in accordance with ASTM D1155.*

Delete the last paragraph.

**CHANGEABLE MESSAGE SIGNS**

(11-21-06)

R11 R11

Revise the *2006 Standard Specifications* as follows:

**Page 11-9, Article 1120-3, Replace the 3rd sentence with the following:**

Sign operator will adjust flash rate so that no more than two messages will be displayed and be legible to a driver when approaching the sign at the posted speed.



**PAVEMENT MARKING LINES:**

(11-21-06) (Rev. 9-18-07)

R12 R01

Revise the 2006 *Standard Specifications* as follows:

**Page 12-2, 1205-3(D) Time Limitations for Replacement**, add the following at the beginning of the chart:

| Facility Type  | Marking Type                   | Replacement Deadline  |
|--|--------------------------------|---|
| Full-control-of-access multi-lane roadway (4 or more total lanes) and ramps, including Interstates | All markings including symbols | By the end of each workday's operation if the lane is opened to traffic |

**Page 12-14, Subarticle 1205-10, Measurement and Payment**, delete the first sentence of the first paragraph and replace with the following:

*Pavement Marking Lines* will be measured and paid for as the actual number of linear feet of pavement marking lines per application that has been satisfactorily placed and accepted by the Engineer.

**GUIDELINES FOR LANE WIDTH  
ON RESURFACING PROJECTS IN DIVISION FIVE**

The Contractor shall place the pavement markings in accordance with this table and detail unless otherwise directed by the Engineer.

**TWO-LANE ROADWAY**

| <b><u>ROADWAY WIDTH</u></b> | <b><u>LANE WIDTH</u></b> | <b><u>SHOULDER WIDTH</u></b> |
|-----------------------------|--------------------------|------------------------------|
| 18'                         | 9'*                      | 0'                           |
| 20'                         | 10'*                     | 0'                           |
| 22'                         | 10'                      | 1'                           |
| 24'                         | 10'                      | 2'                           |
| 26'                         | 11'                      | 2'                           |
| 28'                         | 12'                      | 2'                           |
| 32'                         | 12'                      | 4'                           |

\*May vary due to pavement width.

**SCHEMATIC OF ROADWAY  
(Not to Scale)**

