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DRAWN BY: STEVEN HUDSON

STATE OF NORTH CAROLINA

DEPARTMENT OF TRANSPORTATION DIVISION OF HIGHWAYS GEOTECHNICAL ENGINEERING UNIT

STRUCTURE SUBSURFACE INVESTIGATION

PROJ. REFERENCE NO.: 33397.1.1	F.A. PROJ.: BRSTP-0130(3)
COUNTY: BRUNSWICK	
PROJECT DESCRIPTION: Bridge No. 9 over Bear	Branch on NC 130
SITE DESCRIPTION:	

STATE PROJECT REFERENCE NO. STATE N.C. B-4030 01 20

CAUTION NOTICE

THE SUBSURFACEINFORMATIONAND THE SUBSURFACE INVESTIGATIONON WHICH IT IS BASED WERE MADE FOR THE PURPOSE OF STUDY, PLANNING AND DESIGN, AND NOT FOR CONSTRUCTION OR PAY PURPOSES. THE VARIOUS FIELD BORING LOGS, ROCK CORES, AND SOIL TEST DATA AVAILABLEMAY BE REVIEWED OR INSPECTED IN RALEIGH BY CONTACTING THE N. C. DEPARTMENT OF TRANSPORTATION, GEOTECHNICAL ENGINEERING UNIT AT (919) 250-4088. NEITHER THE SUBSURFACE PLANS AND REPORTS, NOR THE FIELD BORINGLOGS, ROCK CORES, OR SOIL TEST DATA ARE PART OF THE CONTRACT.

GENERAL SOIL AND ROCK STRATA DESCRIPTIONS AND INDICATED BOLINDARIES ARE GENERAL SOIL AND ROCK STRATA DESCRIPTIONS AND INDICATED BOUNDARIES ARE BASED ON A GEOTECHNICAL INTERPRETATION OF ALL AVAILABLE SUBSURFACE DATA AND MAY NOT NECESSARILY REFLECT THE ACTUAL SUBSURFACE CONDITIONS BETWEEN BORINGS OR BETWEEN SAMPLED STRATA WITHIN THE BOREHOLE. THE LABORATORY SAMPLE DATA AND THE IN SITU (IN-PLACE) TEST DATA CAN BE RELIED ON ONLY TO THE DEGREE OF RELIABILITY INHERENT IN THE STANDARD TEST METHOD. THE OBSERVED WATER LEVIELS OR SOIL MOISTURE CONDITIONS INDICATED IN THE SUBSURFACE INVESTIGATIONS ARE AS RECORDED AT THE TIME OF THE INVESTIGATION. THESE WATER LEVIELS OR SOIL MOISTURE CONDITIONS MAY VARY CONSIDERABLY WITH TIME ACCORDING TO CLIMATIC CONDITIONS INCLUDING TEMPERATURES, PRECIPITATION AND WIND, AS WELL AS OTHER NON-CLIMATIC FACTORS.

THE BIDDER OR CONTRACTOR IS CAUTIONED THAT DETAILS SHOWN ON THE SUBSURFACE PLANS ARE PRELIMINARYONLY AND IN MANY CASES THE FINAL DESIGN DETAILS ARE DIFFERENT. FOR BIDDING AND CONSTRUCTION PURPOSES, REFER TO THE CONSTRUCTION PLANS AND DOCUMENTS FOR FINAL DESIGN INFORMATION ON THIS PROJECT. THE DEPARTMENT DOES NOT WARRANT OR GUARANTEE THE SUFFICIENCY OR ACCURACY OF THE INVESTIGATION MADE, NOR THE INTERPRETATIONS MADE, OR OPINION OF THE DEPARTMENT AS TO THE TYPE OF MATERIALS AND CONDITIONS TO BE ENCOUNTERED. THE BIDDER OR CONTRACTOR IS CAUTIONED TO MAKE SUCH INDEPENDENT SUBSURFACE INVESTIGATIONSAS HE DEEMS NECESSARY TO SATISFY MINSELF AS TO CONDITIONS TO BE ENCOUNTERED ON THIS PROJECT. THE CONTRACTOR SHALL HAVE NO CLAIM FOR ADDITIONAL COMPENSATION OR FOR AN EXTENSION OF TIME FOR ANY REASON RESULTING FROM THE ACTUAL CONDITIONS ENCOUNTERED AT THE SITE DIFFERING FROM THOSE INDICATED IN THE SUBSURFACE INFORMATION.

FOR LETTING

PERSONNEL:

TOM STETLER

MICHAEL D. MASON

BOBBIE FOWLER

JON WOOD

INVESTIGATED BY: CATLIN ENGINEERS AND SCIENTISTS

CHECKED BY:

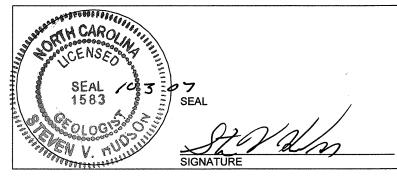
STEVEN V. HUDSON, P.G.

SUBMITTED BY:

STEVEN V. HUDSON, P.G.

DATE:

OCTOBER 3, 2007



NORTH CAROLINA DEPARTMENT OF TRANSPORTATION DIVISION OF HIGHWAYS

GEOTECHNICAL ENGINEERING UNIT

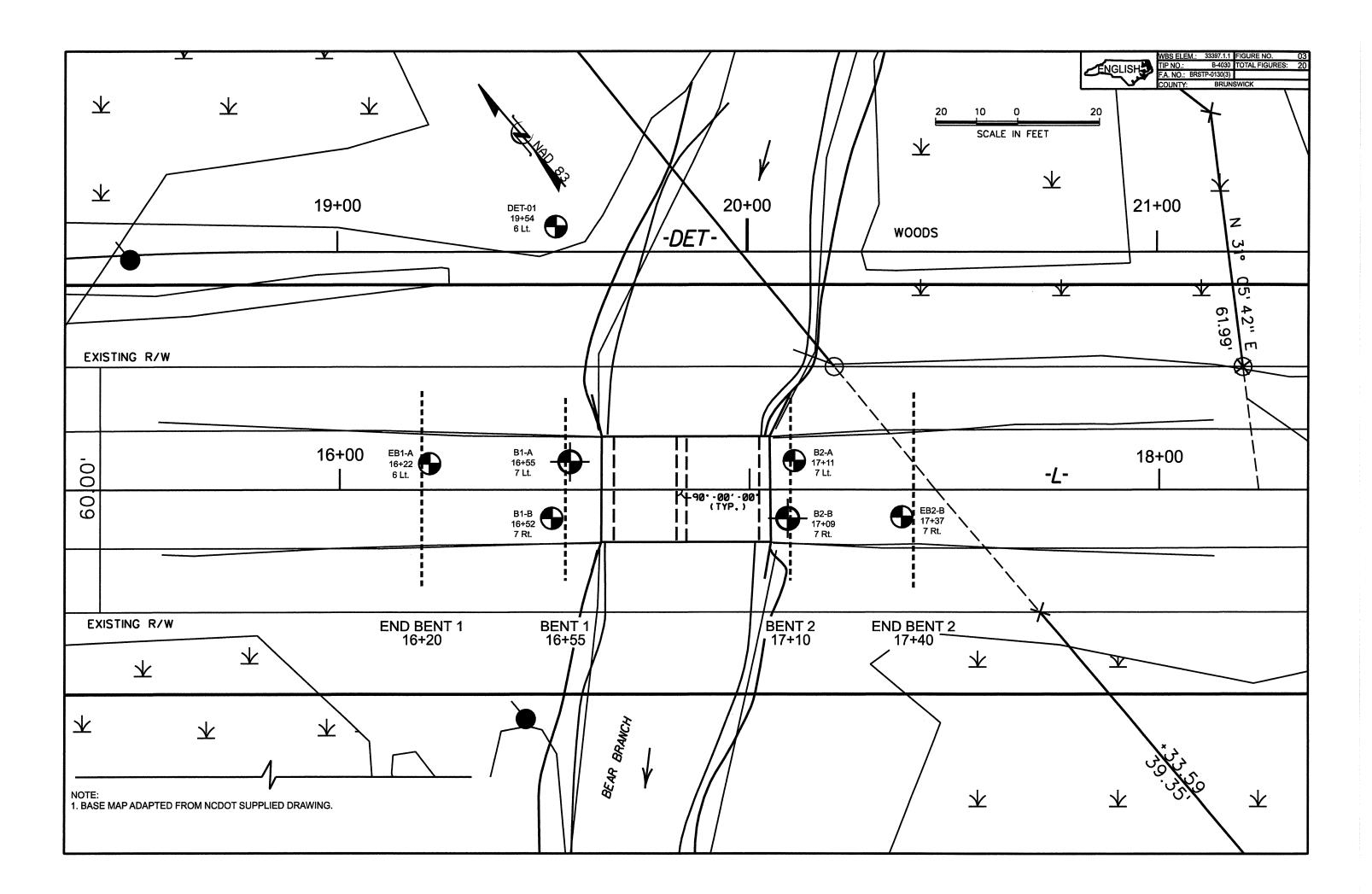


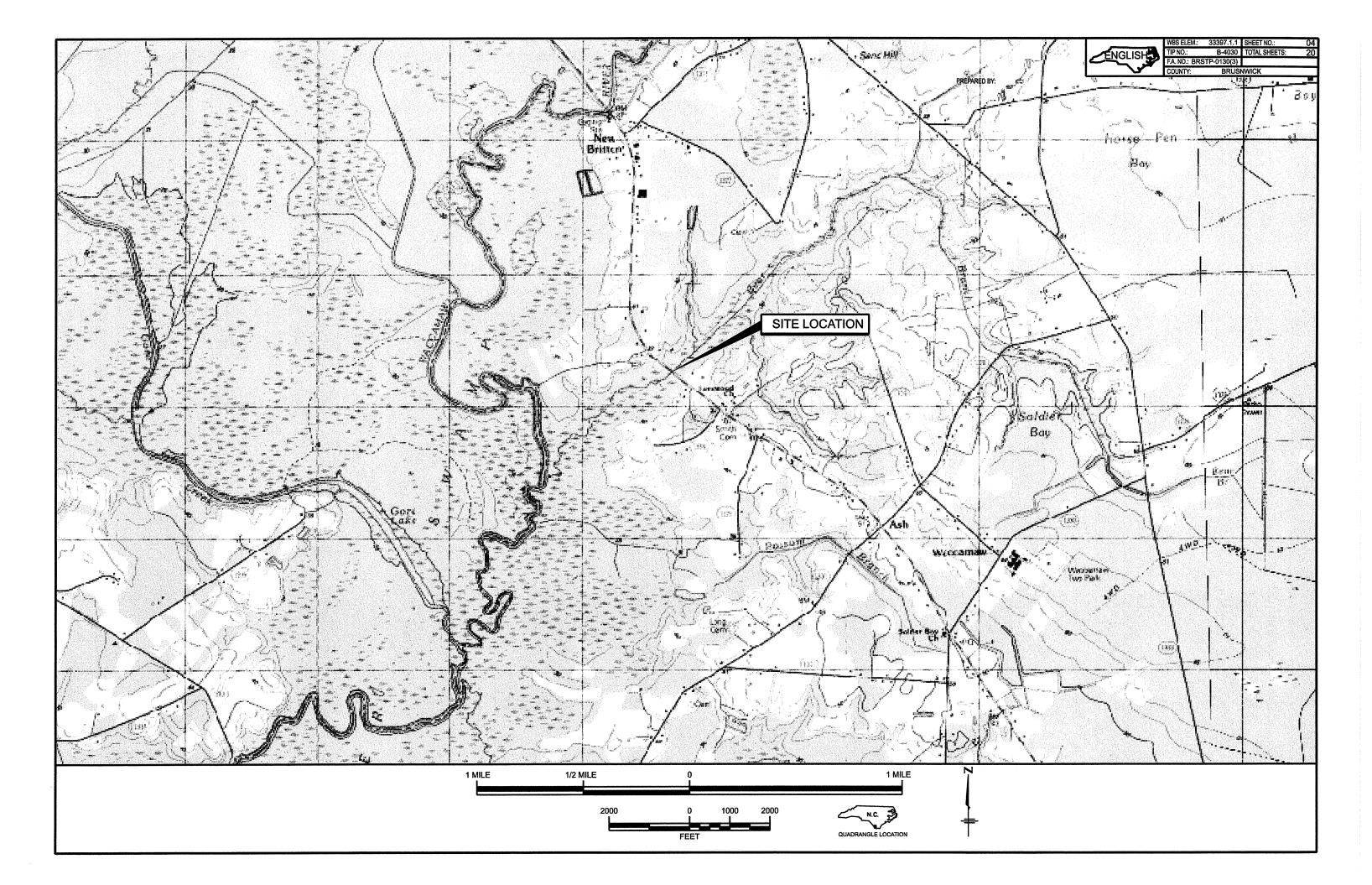
207-057 **ENGLISH** PROJ. NO.: 33397.1.1 SHEET NO.: T.I.P. NO.: B-4030 TOTAL SHEET SO.: COUNTY: BRUNSWICK

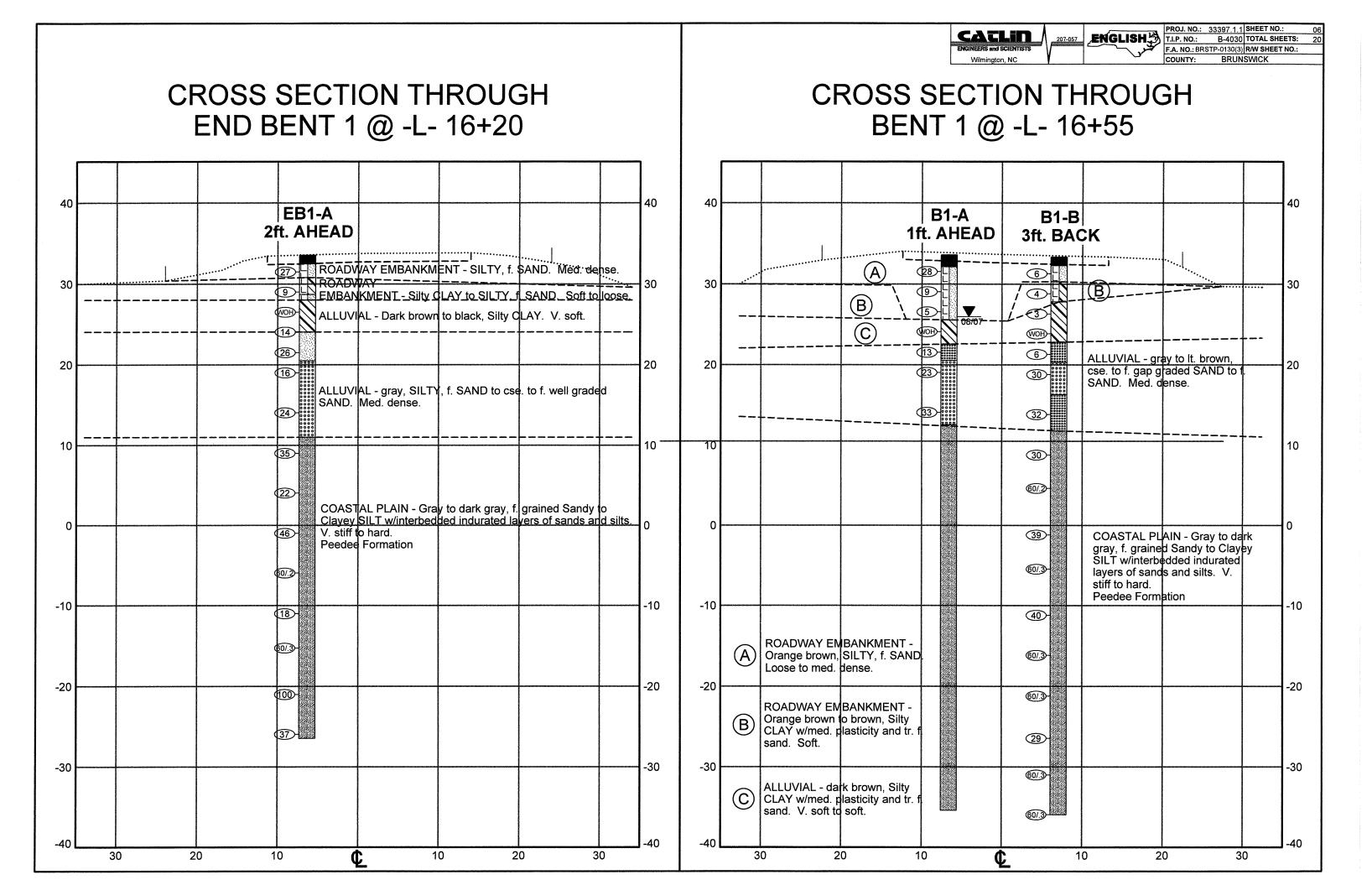
SUBSURFACE INVESTIGATION

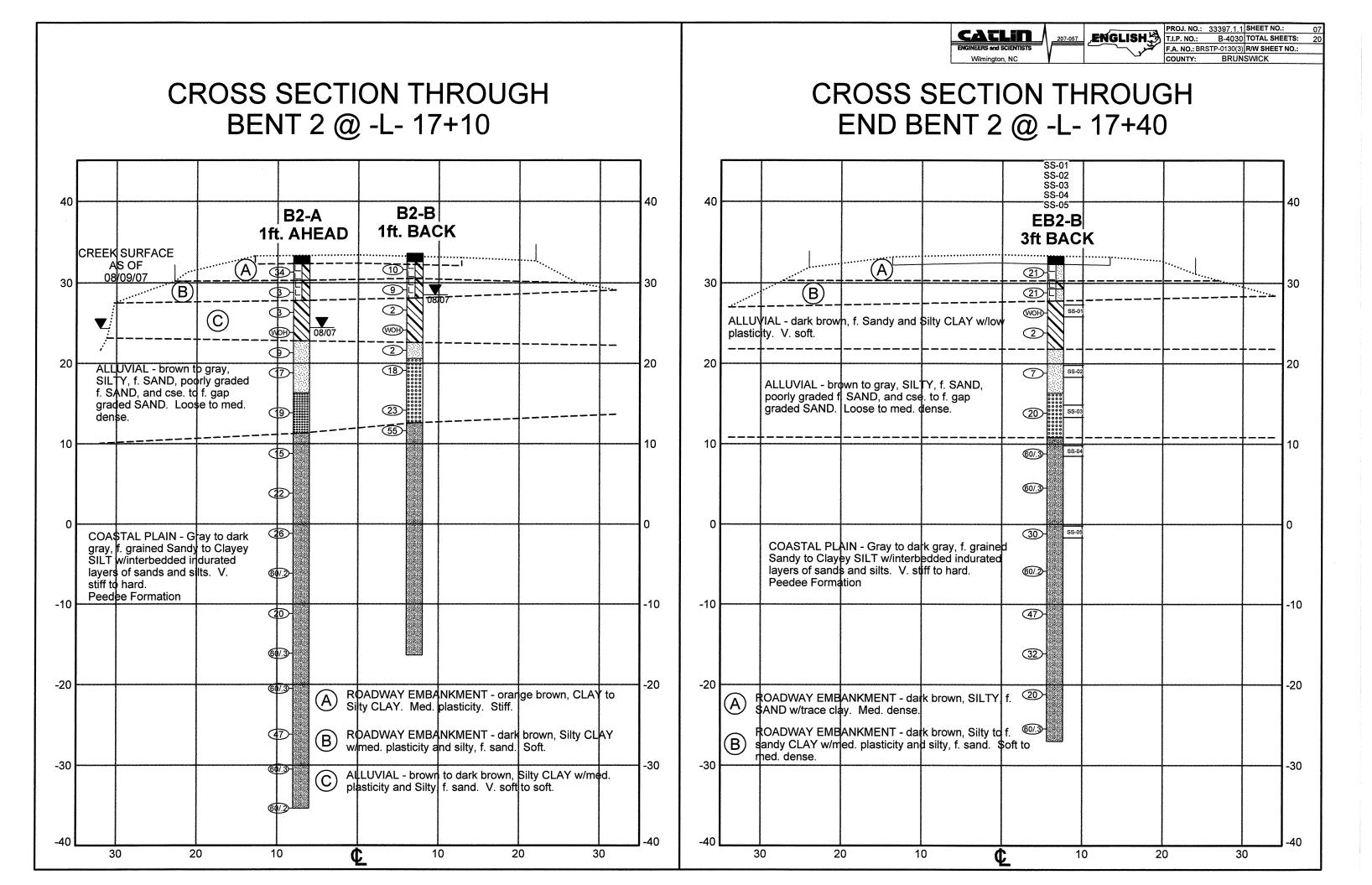
SOIL AND ROCK LEGEND, TERMS, SYMBOLS, AND ABBREVIATIONS

SOIL DESCRIPTION					
SOIL DESCRIPTION	GRADATION	ROCK DESCRIPTION	TERMS AND DEFINITIONS		
SOIL IS CONSIDERED TO BE THE UNCONSOLIDATED, SEMI-CONSOLIDATED OR WEATHERED EARTH MATERIALS WHICH CAN BE PENETRATED WITH A CONTINUOUS FLIGHT POWER AUGE! AND WHICH YIELDS LESS THAN 100 BLOWS PER FOOT ACCORDING TO STANDARD PENETRATION TEST (AASHTO T206, ASTM D-1586). SOIL CLASSIFICATION IS BASED ON THE AASHTO SYSTEM AND BASIC DESCRIPTIONS GENERALLY SHALL INCLUDE: CONSISTENCY, COLOR, TEXTURE, MOISTURE, AASHTO CLASSIFICATION, AND OTHER PERTINENT FACTORS SUCH AS MINERAL OGICAL COMPOSITION, ANGULARITY, STRUCTURE, PLASTICITY, ETC.	WELL GRADED - INDICATES A GOOD REPRESENTATION OF PARTICLE SIZES FROM FINE TO COARSE UNIFORM - INDICATES THAT SOIL PARTICLES ARE ALL APPROXIMATELY THE SAME SIZE. (ALSO POORLY GRADED) GAP-GRADED - INDICATES A MIXTURE OF UNIFORM PARTICLES OF TWO OR MORE SIZES.	HARD ROCK IS NON-COASTAL PLAIN MATERIAL THAT WHEN TESTED, WOULD YIELD SPT REFUSAL. AN INFERRED ROCK LINE INDICATES THE LEVEL AT WHICH NON-COASTAL PLAIN MATERIAL WOULD YIELD SPT REFUSAL. SPT REFUSAL IS PENETRATION BY A SPLIT SPOON SAMPLER EQUAL TO OR LESS THAN 0.1 FOOT PER 60 BLOWS. IN NON-COASTAL PLAIN MATERIAL, THE TRANSITION BETWEEN SOIL AND ROCK IS OFTEN REPRESENTED BY A ZONE OF	ALLUVIUM (ALLUV.) - SOILS WHICH HAVE BEEN TRANSPORTED BY WATER. AQUIFER - A WATER BEARING FORMATION OR STRATA. ARENACEOUS - APPLIED TO ROCKS THAT HAVE BEEN DERIVED FROM SAND OR THAT CONTAIN SAND		
SUCH AS MINERAL OGICAL COMPOSITION, ANGULARITY, STRUCTURE, PLASTICITY, ETC. EXAMPLE: VERY STIFF, GRAY SILTY CLAY, MOIST WITH INTERBEDDED FINE SAND LAYERS, HIGHLY PLASTIC, A-7-6	ANGULARITY OF GRAINS	WEATHERED ROCK, ROCK MATERIALS ARE TYPICALLY AS FOLLOWS:	ARGILLACEOUS - APPLIED TO ALL ROCKS OR SUBSTANCES COMPOSED OF CLAY MINERALS, OR HAVING A NOTABLE PROPORTION OF CLAY IN THEIR COMPOSITION, AS SHALE, SLATE, ETC.		
SOIL LEGEND AND AASHTO CLASSIFICATION	THE ANGULARITY OR ROUNDNESS OF SOIL GRAINS ARE DESIGNATED BY THE TERMS; ANGULAR SUBANGULAR SUBROUNDED OR ROUNDED.	WEATHERED NON-COASTAL PLAIN MATERIAL THAT YIELDS SPT N VALUES >100 BLOWS PER FOOT FINE TO COARSE GRAIN IGNEOUS AND METAMORPHIC ROCK THAT	ARTESIAN - GROUND WATER THAT IS UNDER SUFFICIENT PRESSURE TO RISE ABOVE THE LEVEL AT WHICH IS ENCOUNTERED, BUT WHICH DOES NOT NECESSARILY RISE TO OR ABOVE THE GROUND SURFACE.		
GENERAL CLASS. GRANULAR MATERIALS (≤ 35% PASSING #200) SILT-CLAY MATERIALS (> 635% PASSING #200) ORGANIC MATERIALS (> 635% PASSING #200) GROUP A-1 A-3 A-2 A-4 A-5 A-6 A-7 A-1, A-2 A-4, A-5	MINERALOGICAL COMPOSITION MINERAL NAMES SUCH AS QUARTZ, FELDSPAR, MICA, TALC, KAOLIN, ETC. ARE USED IN	ROCK (CR) WOULD YIELD SPT REFUSAL IF TESTED. ROCK TYPE INCLUDES GRANITE GREISS, GABBRO, SCHIST, ETC.	CALCAREOUS (CALC.) - SOILS WHICH CONTAIN APPRECIABLE AMOUNTS OF CALCIUM CARBONATE. COLLUVIUM - ROCK FRAGMENTS MIXED WITH SOIL DEPOSITED BY GRAVITY ON SLOPE OR AT BOTTO		
CLASS. A-1-a A-1-b A-2-4 A-2-5 A-2-6 A-2-7 A-7-5 A-3 A-6, A-7	DESCRIPTIONS WHENEVER THEY ARE CONSIDERED OF SIGNIFICANCE. COMPRESSIBILITY	NON-CRYSTALLINE SEDIMENTARY ROCK THAT WOULD YIELD SPT REFUSAL IF TESTED. ROCK (NCR) TYPE INCLUDES PHYLLITE, SLATE, SANDSTONE, ETC. COASTAL PLAIN COASTAL PLAIN SEDIMENTS CREMENTED INTO ROCK, BUT MAY NOT YIELD			
% PASSING SILT-	SLIGHTLY COMPRESSIBLE LIQUID LIMIT LESS THAN 30 MODERATELY COMPRESSIBLE LIQUID LIMIT 31 - 50 HIGHLY COMPRESSIBLE LIQUID LIMIT GRATER THAN 50	COASTAL PLANTA SEDIMENTARY ROCK (CP) SPT REFUSAL. ROCK TYPE INCLUDES LIMESTONE, SANDSTONE, CEMENTED SHELL BEDS, ETC. WEATHERING	DIVIDED BY TOTAL LENGTH OF CORE RUN AND EXPRESSED AS A PERCENTAGE. DIKE - A TABULAR BODY OF IGNEOUS ROCK THAT CUTS ACROSS THE STRUCTURE OF ADJACENT ROCKS OR CUTS MASSIVE ROCK.		
= 10 50 MX = 10	PERCENTAGE OF MATERIAL	FRESH ROCK FRESH, CRYSTALS BRIGHT, FEW JOINTS MAY SHOW SLIGHT STAINING. ROCK RINGS UNDER HAMMER IF CRYSTALLINE.	DIP - THE ANGLE AT WHICH A STRATUM OR ANY PLANAR FEATURE IS INCLINED FROM THE HORIZONTAL.		
LIQUID LIMIT	a l	VERY SLIGHT ROCK GENERALLY FRESH, JOINTS STAINED, SOME JOINTS MAY SHOW THIN CLAY (V. SLI.) COATINGS IF OPEN, CRYSTALS ON A BROKEN SPECIMEN FACE SHINE BRIGHTLY. ROCK	DIP DIRECTION (DIP AZIMUTH)- THE DIRECTION OR BEARING OF THE HORIZONTAL TRACE OF THE LIN OF DIP, MEASURED CLOCKWISE FROM NORTH.		
INDEX 0 0 0 4 MX 8 MX 12 MX 16 MX NO MX AMOUNTS OF ORGANIC OF MAJOR GRAVEL AND INDEX SULTY OF CLAYEY SULTY CLAYEY OF MAJOR GRAVEL AND INDEX SULTY OF MAJOR GRA		RINGS UNDER HAMMER BLOWS IF OF A CRYSTALLINE NATURE. SLIGHT ROCK GENERALLY FRESH, JOINTS STAINED AND DISCOLORATION EXTENDS INTO ROCK (SLI.) UP TO 1 INCH. OPEN JOINTS MAY CONTAIN CLAY. IN GRANITOID ROCKS SOME	<u>DIRECT PUSH</u> - ADVANCEMENT OF SAMPLE TOOLING UTILIZING DIRECT PUSH METHODOLOGY (ex. GEOPROBE)		
GEN. RATING AS A SUBGRADE EXCELLENT TO GOOD FAIR TO POOR POOR POOR UNSUITAB	HIGHLY ORGANIC >10% >20% HIGHLY 35% AND ABOVE	OCCASIONAL FELDSPAR CRYSTALS ARE DULL AND DISCOLORED. CRYSTALLINE ROCKS RING UNDER HAMMER BLOWS. MODERATE SIGNIFICANT PORTIONS OF ROCK SHOW DISCOLORATION AND WEATHERING EFFECTS.	FAULT - A FRACTURE OR FRACTURE ZONE ALONG WHICH THERE HAS BEEN DISPLACEMENT OF THE SIDES RELATIVE TO ONE ANOTHER PARALLEL TO THE FRACTURE.		
P.I. OF A-7-5≤_LL 30 : P.I. OF A-7-6≥LL 30 CONSISTENCY OR DENSENESS	WATER LEVEL IN BORE HOLE IMMEDIATELY AFTER DRILLING	(MOD.) IN GRANITOID ROCKS, MOST FELDSPARS ARE DULL AND DISCOLORED, SOME SHOW CLAY. ROCK HAS DULL SOUND UNDER HAMMER BLOWS AND SHOWS SIGNIFICANT LOSS OF STRENGTH AS COMPARED WITH FRESH ROCK.	FISSILE - A PROPERTY OF SPLITTING ALONG CLOSELY SPACED PARALLEL PLANES. FLOAT - ROCK FRAGMENTS ON SURFACE NEAR THEIR ORIGINAL POSITION AND DISLODGED FROM PARENT MATERIAL.		
PRIMARY SOIL TYPE COMPACTNESS OR CONSISTENCY PENETRATION RESISTANCE (N-VALUE) (TONS/FT) VERY LOOSE <4		MODERATELY ALL ROCK EXCEPT QUARTZ DISCOLORED OR STAINED. IN GRANITOID ROCKS, ALL SEVERE FELDSPARS DULL AND DISCOLORED AND A MAJORITY SHOW KAOLINIZATION. ROCK (MOD. SEV.) SHOWS SEVERE LOSS OF STRENGTH AND CAN BE EXCAVATED WITH A GEOLOGIST'S PICK. ROCK GIVES "CLUNK" SOUND WHEN STRUCK	FLOOD PLAIN (F.P.) - LAND BORDERING A STREAM, BUILT OF SEDIMENTS DEPOSITED BY THE STREAM FORMATION (FM.) - A MAPPABLE GEOLOGIC UNIT THAT CAN BE RECOGNIZED AND TRACED IN THE FIELD.		
GENERALLY	MISCELLANEOUS SYMBOLS	IF TESTED, WOULD YIELD SPT REFUSAL. SEVERE ALL ROCKS EXCEPT QUARTZ DISCOLORED OR STAINED. ROCK FABRIC CLEAR AND (SEV.) EVIDENT BUT REDUCED IN STRENGTH TO STRONG SOIL. IN GRANITOID ROCKS ALL	HYDRAULIC PUSH (HP)- ADVANCEMENT OF SAMPLING TOOLS UTILIZING MECHANICAL/HYDRAULIC DOWN-FORCE OF DRILLING MACHINE.		
(NON-COHESIVE)	ROADWAY EMBANKMENT OF SAMPLE DESIGNATIONS WITH SOIL DESCRIPTION	FELDSPARS ARE KAOLINIZED TO SOME EXTENT. SOME FRAGMENTS OF STRONG ROCK USUALLY REMAIN. IF TESTED, YIELDS SPT N VALUES > 100 BPF	JOINT - FRACTURE IN ROCK ALONG WHICH NO APPRECIABLE MOVEMENT HAS OCCURRED. LEDGE - A SHELF-LIKE RIDGE OR PROJECTION OF ROCK WHOSE THICKNESS IS SMALL COMPARED TITS LATERAL EXTENT.		
GENERALLY SOFT 2 TO 4 0.25 TO 0.50 SILT-CLAY MEDIUM STIFF 4 TO 8 0.5 TO 1 MATERIAL STIFF 8 TO 15 1 TO 2 (COHESIVE) VERY STIFF 15 TO 30 2 TO 4	AUGER BORING S - BULK SAMPLE ARTIFICIAL FILL OTHER THAN CORE BORING SS - SPLIT SPOON SAMPLE CORE BORING SS - SPLIT SPOON SAMPLE	VERY SEVERE ALL ROCK EXCEPT QUARTZ DISCOLORED OR STAINED. ROCK FABRIC ELEMENTS ARE (V. SEV.) DISCERNIBLE BUT THE MASS IS EFFECTIVELY REDUCED TO SOIL STATUS, WITH ONLY FRAGMENTS OF STRONG ROCK REMAINING. SAPROLITE IS AN EXAMPLE OF ROCK WEATHERED TO A DEGREE SUCH THAT ONLY MINOR VESTIGES OF THE ORIGINAL ROCK	ITS LATERAL EXTENT. LENS - A BODY OF SOIL OR ROCK THAT THINS OUT IN ONE OR MORE DIRECTIONS. MOTTLED (MOT.) - IRREGULARLY MARKED WITH SPOTS OF DIFFERENT COLORS. MOTTLING IN SOILS USUALLY INDICATES POOR AERATION AND LACK OF GOOD DRAINAGE.		
HARD >30 >4 TEXTURE OR GRAIN SIZE	MONITORING WELL ST - SHELBY TUBE SAMPLE SAMPLE	FABRIC REMAIN. IF TESTED, YIELDS SPT N VALUES < 100 BPF COMPLETE ROCK REDUCED TO SOIL. ROCK FABRIC NOT DISCERNIBLE, OR DISCERNIBLE ONLY IN	USUALLY INDICATES POOR AERATION AND LACK OF GOOD DRAINAGE. PERCHED WATER - WATER MAINTAINED ABOVE THE NORMAL GROUND WATER LEVEL BY THE PRESENCE OF AN INTERVENING IMPERVIOUS STRATUM.		
U.S STD. SIEVE SIZE 4 10 40 60 200 270 OPENING (mm) 4.76 2.0 0.42 0.25 0.075 0.053	■ INFERRED ROCK LINE	SMALL AND SCATTERED CONCENTRATIONS. QUARTZ MAY BE PRESENT AS DIKES OR STRINGERS. SAPROLITE IS ALSO AN EXAMPLE.	RESIDUAL SOIL - SOIL FORMED IN PLACE BY THE WEATHERING OF ROCK.		
BOULDER (BLDR.) COBBLE (GR.) COARSE SAND SAND (SL.) (CL.)	ALLUVIAL SOIL BOUNDARY DIP/DIP DIRECTION OF INSTALLATION RT - RECOMPACTED TRIAXIAL SAMPLE ROCK STRUCTURES	ROCK HARDNESS VERY HARD CANNOT BE SCRATCHED BY KNIFE OR SHARP PICK. BREAKING OF HAND SPECIMENS	ROCK QUALITY DESIGNATION (R.Q.D.) - A MEASURE OF ROCK QUALITY DESCRIBED BY: TOTAL LENG' OF ROCK SEGMENTS EQUIAL TO OR GREATER THAN 4 INCHES DIVIDED BY THE TOTAL LENGTH OF CORE RUN AND EXPRESSED AS A PERCENTAGE.		
GRAIN MM 305 75 2.0 0.25 0.05 0.005	SOUNDING ROD SPT N-VALUE CBR - CBR SAMPLE SPT REFUSAL	REQUIRES SEVERAL HARD BLOWS OF THE GEOLOGISTS PICK. HARD CAN BE SCRATCHED BY KNIFE OR PICK ONLY WITH DIFFICULTY. HARD HAMMER BLOWS REQUIRED TO DETACH HAND SPECIMEN.	SAPROLITE (SAP.) - RESIDUAL SOIL WHICH RETAINS THE RELIC STRUCTURE OR FABRIC OF THE PARENT ROCK.		
SIZE IN. 12 3 SOIL MOISTURE - CORRELATION OF TERMS	ABBREVIATIONS AB ALICED PERIOD MED. MED. MED. M. V. LINIT MEIOLIT	MODERATELY CAN BE SCRATCHED BY KNIFE OR PICK. GOUGES OR GROOVES TO 0.25 INCHES DEEP HARD CAN BE EXCAVATED BY HARD BLOW OF A GEOLOGISTS PICK. HAND SPECIMENS CAN BE	SILL - AN INTRUSIVE BODY OF IGNEOUS ROCK OF APPROXIMATELY UNIFORM THICKNESS AND RELATIVELY THIN COMPARED WITH ITS LATERAL EXTENT, WHICH HAS BEEN EMPLACED PARALLEL TO THE BEDDING OR SCHISTOSITY OF THE INTRUDED ROCKS.		
SOIL MOISTURE SCALE FIELD MOISTURE GUIDE FOR (ATTERBERG LIMITS) DESCRIPTION FIELD MOISTURE DESCRIPTION	AR - AUGER REFUSAL MED MEDIUM Y - UNIT WEIGHT BLS - BELOW LAND SURFACE NA - NOT APPLICABLE BT - BORING TERMINATED NE - NOT ENCOUNTERED 7 - UNIT WEIGHT NE - NOT ENCOUNTERED	DETACHED BY MODERATE BLOWS. MEDIUM CAN BE GROOVED OR GOUGED 0.05 INCHES DEEP BY FIRM PRESSURE OF KNIFE OR HARD PICK POINT. CAN BE EXCAVATED IN SMALL CHIPS TO PIECES 1 INCH MAXIMUM SIZE BY	SLICKENSIDE - POLISHED AND STRIATED SURFACE THAT RESULTS FROM FRICTION ALONG A FAULT OR SLIP PLANE.		
- SATURATED - USUALLY LIQUID; VERY WET, USUALLY FROM BELOW THE GROUND WATER TABLE - WET - SEMISOLID; REQUIRES DRYING TO	CL CLAY NM - NOT MEASURED CPT - CONE PENETRATION TEST PMT - PRESSUREMETER TEST CSE COARSE R.C.P RECENT COASTAL PLAIN SAA - SAME AS ABOVE DPT - DYNAMIC PENETRATION TEST SD SAND SANDY	HARD PICK POINT. CAN BE EXCAVATED IN SWALL CHIPS TO PIECES TINCH MAXIMUM SIZE BY HARD BLOWS OF THE POINT OF A GEOLOGISTS PICK. SOFT CAN BE GROVED OR GOUGED READILY BY KNIFE OR PICK. CAN BE EXCAVATED IN FRAGMENTS FROM CHIPS TO SEVERAL INCHES IN SIZE MODERATE BLOWS OF A PICK POINT. SMALL, THIN PIECES CAN BE BROKEN BY FINGER PRESSURE.			
RANGE (W) ATTAIN OFTIMUM MOISTURE (W) ATTAIN OFTIMUM MOISTURE OM OPTIMUM MOISTURE (W) ATTAIN OFTIMUM MOISTURE (W) ATTAIN OFTIMUM MOISTURE	e - VOID RATIO SL SILT, SILTY F FINE SU SLICHTLY FIAD - FILLED IMMEDIATELY AFTER TCR - TRICONE REFUSAL DRILLING W- MOISTURE CONTENT	VERY CAN BE CARVED WITH KNIFE. CAN BE EXCAVATED READILY WITH POINT OF PICK. SOFT PIECES 1 INCH OR MORE IN THICKNESS CAN BE BROKEN BY FINGER PRESSURE. CAN BE SCRATCHED READILY BY FINGERNAIL.	STRATA CORE RECOVERY (SREC.) - TOTAL LENGTH OF STRATA MATERIAL RECOVERED DIVIDED BY TOTAL LENGTH OF STRATUM AND EXPRESSED AS A PERCENTAGE. STRATA ROCK QUALITY DESIGNATION (S.R.Q.D.) - A MEASURE OF ROCK QUALITY DESCRIBED BY: TOTAL LENGTH OF ROCK SEGMENTS WITHIN A STRATUM EQUAL TO OR GREATER THAN 4 INCHES		
SL + SHRINKAGE LIMIT - DRY - REQUIRES ADDITIONAL WATER TO	FRAC: - FRACTURED VST - VANE SHEAR TEST FRAGS: - FRAGMENTS	FRACTURE SPACING BEDDING TERM SPACING TERM SPACING	DIVIDED BY THE TOTAL LENGTH OF STRATA AND EXPRESSED AS A PERCENTAGE. TOPSOIL (T.S.) - SURFACE SOILS USUALLY CONTAINING ORGANIC MATTER.		
(D) ATTAIN OPTIMUM MOISTURE PLASTICITY	EQUIPMENT USED ON SUBJECT PROJECT	VERY WIDE > 10 FEET VERY THICKLY BEDDED > 4 FEET WIDE 3 TO 10 FEET THICKLY BEDDED 1.5 - 4 FEET MODERATELY CLOSE 1 TO 3 FEET THINLY BEDDED 0.16 - 1.5 FEET			
PLASTICITY INDEX (PI) DRY STRENGTH	DRILL UNITS: ADVANCING TOOLS: HAMMER TYPE: ☐ DIEDRICH D-50 ☐ CLAY BITS ☐ AUTOMATIC ☐ MANUAL	CLOSE 0.16 TO 3 FEET VERY THINLY BEDDED 0.03 - 0.16 FEET VERY CLOSE < 0.16 FEET THICKLY LAMINATED 0.008 - 0.03 FEET THINLY LAMINATED < 0.008 FEET THINLY LAMINATED COME TO THE TEMPORAL THINLY LAMINATED			
NONPLASTIC 0 - 5 VERY LOW LOW PLASTICITY 6 - 15 SLIGHT MED. PLASTICITY 16 - 25 MEDIUM	DIEDRICH D-25 G** CONTINUOUS FLIGHT AUGER CORE SIZE:	INDURATION	BENCH MARK: N/A. Used survey grade GPS.		
HIGH PLASTICITY 26 OR MORE HIGH	CME-45B ATV	FOR SEDIMENTARY ROCKS, INDURATION IS THE HARDENING OF THE MATERIAL BY CEMENTING, HEAT, PRESSURE, ETC.	ELEVATION: N/.		
COLOR DESCRIPTIONS MAY INCLUDE COLOR OR COLOR COMBINATIONS (TAN, RED, YEL-BRN, BLUE-GRAY) MODIFIERS SUCH AS LIGHT, DARK, STREAKED, ETC. ARE USED TO DESCRIBE APPEARANCE.	CME-550 CASING WADVANCER TRICONE 2 7/8" STEEL TEETH TRICONE _ "TUNGCARBIDE CORE BIT OTHER OTHER OTHER CME-550 CASING WADVANCER POST HOLE DIGGER HAND TOULS. OF NOTHER POST HOLE DIGGER HAND TOULS. OF NOTHER POST HOLE DIGGER HAND TOULS.	FRIABLE RUBBING WITH FINGER FREES NUMEROUS GRAINS; GENTLE BLOW BY HAMMER DISINTEGRATES SAMPLE. MODERATELY INDURATED GRAINS CAN BE SEPARATED FROM SAMPLE WITH STEEL PROBE; BREAKS EASILY WHEN HIT WITH HAMMER. INDURATED GRAINS ARE DIFFICULT TO SEPARATE WITH STEEL PROBE; DIFFICULT TO BREAK WITH HAMMER. EXTREMELY INDURATED SHARP HAMMER BLOWS REQUIRED TO BREAK SAMPLE; SAMPLE BREAKS ACROSS GRAINS.	NOTES: ADAPTED FROM NCDOT FILE "RdwyTitle&LegendE spb y02		











NORTH CAROLINA DEPARTMENT OF TRANSPORTATION **GEOTECHNICAL ENGINEERING UNIT**

BORING REPORT

ENGINEERS and SCIENTISTS
CATLIN PROJECT NO.: 207-057

NORTH CAROLINA DEPARTMENT OF TRANSPORTATION GEOTECHNICAL ENGINEERING UNIT

PROJ. NO.: 33397.1.1 SHEET NO.: T.I.P. NO.: B-4030 TOTAL SHEETS: F.A. NO.:BRSTP-0130(3) R/W SHEET NO.: COUNTY: BRUNSWICK

BORING REPORT

CATLIN PROJECT NO.: 207-057 PAGE 1 OF 1

		MG KLF OKT	CATLIN PROJECT			ONING KEPOKI		CATLIN PROJECT NO.: 207-057 PAGE 1 OF
OJECT NO.					PROJECT NO.: 333	······································	COUNTY: BRUNSWICK	GEOLOGIST: Tom Stetler
E DESCRIP	TION: Bri	idge No. 9 over Bear Branch o		GROUND WATER (ft)	SITE DESCRIPTION	I: Bridge No. 9 over Bear Branch o	n NC 130	GROUND WATER (ft)
RING NO.:	EB1-A	BORING LOCATION:	6+22 OFFSET: 6ft Lt ALIGNMENT	: -L- 0 HR. 2.8	BORING NO.: B1-A	BORING LOCATION:	16+56 OFFSET : 7ft Lt	ALIGNMENT: -L- 0 HR. 1.6
LLAR ELEV	7.: 33.5	NORTHING: 119,566	EASTING : 2,138,273	24 HR. FIAD	COLLAR ELEV.: 33	3.2 NORTHING: 119,545	EASTING : 2,138,300	24 HR. 7.6
TAL DEPTH	: 60.0 ft	DRILL MACHINE: Diedrich D-	DRILL METHOD: Mud Rotary	AMMER TYPE: Manual	TOTAL DEPTH: 69.	0 ft DRILL MACHINE: Diedrich D-		Core Boring HAMMER TYPE: Manual
TE STARTE					DATE STARTED: 8/			
V. DEPTH	BLOW CO		SAMP			DW COUNT BLOWS PER FOO		
j <u></u>).5ft 0.5ft		80 100 NO. MOI G ELEV. (ff)	OCK DESCRIPTION	1 1 1	0.5ft 0.5ft 0 20 40 60	80 100 NO. MOI G ELEV.	SOIL AND ROCK DESCRIPTION
, (1)			I I VIOL G ELEV. (π)	DEPTH (ft)			I I NO. D MOI G ELEV.	_(ft) DEPTH

5 + 1.0				0.0 ASPHALT 1.0	32.2 + 1.0		- 33.2	ACCUALT
F 1	22 14	13	ROADV	AY EMBANKMENT 2.7	E 67	16 12	30.6 DRY	ASPHALT ROADWAY EMBANKMENT
0 7 3.5	2 3	6	· · · · · I Ian, SiLiy	, f. SAND. Med. dense. //AY EMBANKMENT 4.8	29.7 3.5	E 4		Orange brown, SILTY, f. SAND. Drive at one foot influenced by debris no return. Loose.
5 + 6.0			28.0 Brown, med.	plasticity Silty CLAY. Stiff. 5.5,	27.2 + 6.0 5	5 4	I WOI L	loot initideficed by debris no feturit. Loose.
o 8.5 W	OH WOH	WOH WOH		LTY, f. SAND. Loose.	2 2	2 3 . •5	25.2	
1 W	/OH 2	12		ALLUVIAL 9.5 lack, Silty CLAY. Very soft.	24.7 <u>8.5</u> WOH V	WOH WOH	WET	ALLUVIAL Dark brown, silty CLAY. Med. plasticity. V.
5 + 11.0	12 13	13	· · · · · · GAT	ALLUVIAL	22.2 11.0		22.2	soft1
13.5		<u></u>	20.5 Gray, SILTT	, f. SAND. Med. Dense. 13.0	19.7 13.5 5	6 7	SAT	ALLUVIAL Gray, f. SAND. Poorly graded. Med. dense.
<u>†</u>	9 8	8 : : : •16; : : : : : : : : :	Gray, well grade	d, cse to f. SAND w/trace silt		11 12	SAT \$1000 - 12.2	ALLUVIAL
$f \mid$				avel. Med. dense.				Gray, cse. to f. SAND. Gap to poorly graded. Trace gravel sized well rounded quartzite grains
18.5	13 14	10	SAT SAT		14.7 18.5	1 1 1 1 1 1 1 1 1 1 1 1 1 1 1 1 1 1 1 1		in base of strata. Med. dense to dense.
‡	13 14	24				15 18	SAT 6886-	:
‡			11.0	22.5				COASTAL PLAIN
23.5	22 14	21	· · · · · WET	ASTAL PLAIN sandy to clayey SILT. Some			::::: 	Boring advanced with NWD4 core barrel. Gray, clayey to sandy SILT w/layers of
+ 1			calcareous grave	I sized fragments throughout.	+		:::::	indurated materials throughout. PEEDEE FORMATION
T 28.5			PEED - PEED	EE FORMATION	F		· · · · · ·	
	3 6	16	····· WET					
‡							::::: 	
33.5			::::: <u>-</u>				::::: 	
± 12	28 19	27	WET		<u> </u>		:::: -	
Ŧ			:::::				:::::	
T 38.5	18 23	60/.2	60(2) WET WET				·····	
‡	10 23	00/.2	.00/,29				<u> </u>	
43.5			· · · · ·					
	9 9	9 18	wet					
‡							::::: 	
48.5							::::: 	
± 60	0/.3		60/.37				:::::	
<u>†</u>			:::: <u> </u>				:::::	
53.5	10 78	22/.1	100/6 MOI					
Į	'				‡			
58.5								
	10 13	24 37	MOI -26.5	60.0				
<u> </u>			Boring Termin	nated at Elevation -26.5 ft			::::: 	
<u>†</u>			- In riard, it gi	ained sandy silt w/clay. dee Formation	<u>+</u>		:::::	
Ŧ l					Ŧ		····	
‡					‡			
‡								
†							-35.8	Boring Terminated at Elevation -35.8 ft
<u>†</u>								with core in sandy to clayey silt w/interlayered indurated calcite cemented sand.
1 1	1		[_		1 1 1			Peedee Formation



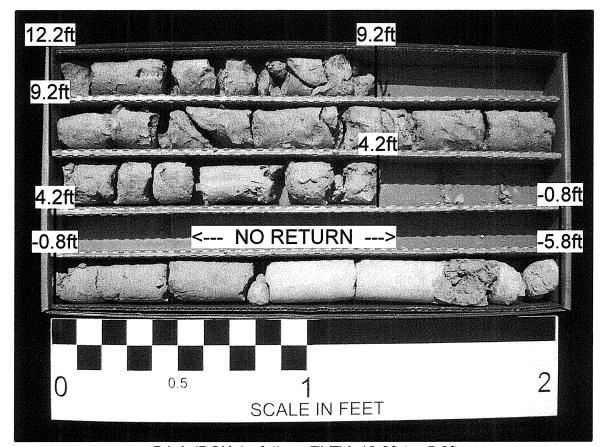
NORTH CAROLINA DEPARTMENT OF TRANSPORTATION GEOTECHNICAL ENGINEERING UNIT

PROJ. NO.: 33397.1.1 SHEET NO.: T.I.P. NO.: B-4030 TOTAL SHEETS:
F.A. NO.:BRSTP-0130(3) R/W SHEET NO.:
COUNTY: BRUNSWICK

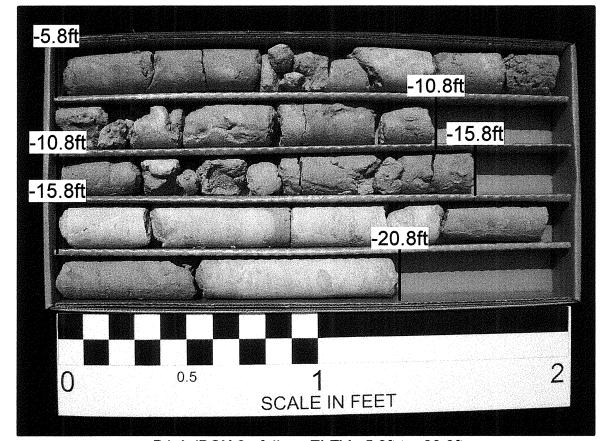
CORE BORING REPORT

CATLIN PROJECT NO.: 207-057 PAGE 1 OF 1 **PROJECT NO.:** 33397.1.1 ID.: B-4030 COUNTY: BRUNSWICK **GEOLOGIST**: Tom Stetler GROUND WATER (ft) 0 HR. 1.6 SITE DESCRIPTION: Bridge No. 9 over Bear Branch on NC 130 BORING NO.: B1-A **BORING LOCATION:** 16+56 OFFSET: 7ft Lt ALIGNMENT: -L-

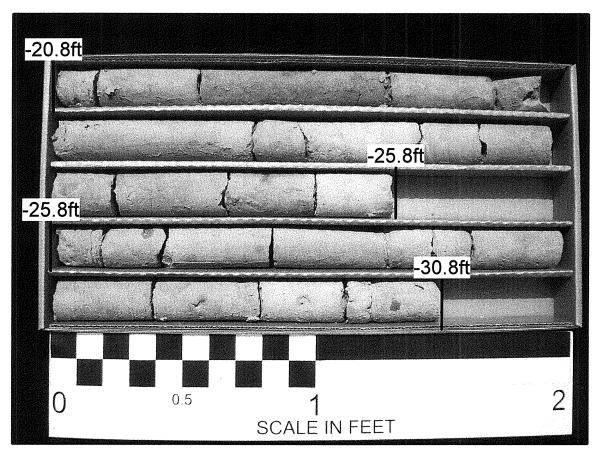
COLL	AR ELI	EV.: 3	33.2	NOR	THIN	G: 119,	545		-	EASTING: 2,138,300 24 HR. 7.6
TOTA	L DEPT	Г Н : 6	9.0 ft	DRIL	L MA	CHINE:	Died	Irich	D-5	50 DRILL METHOD: SPT Core Boring HAMMER TYPE: Manual
DATE	STAR	ΓED:	8/13/07	7		COMPL	LETE	D: 8/	14/(1/07 CORE BARREL TYPE: NWD4 SURFACE WATER DEPTH: N
CORE	SIZE	NWD				L RUN				DRILLER Bobbie D. Fowler
ELEV (ft)	DEPTH (ft)	RUN (ft)	DRILL RATE (Min/ft)	REC. (ft) %	RQD (ft) %	SAMP. NO.	STF REC. (ft) %	RATA RQD (ft) %	L O G	D DESCRIPTION AND REMARKS
										12.2 Begin Coring @ 21.0 ft
9.2	21.0 - 24.0	3.0	3:02 1:28 0:26	(1.2) 40%						COASTAL PLAIN Gray, clayey silt to silty/clayey f. sand. Mod. indurated to friable. 9.2 Peedee Formation
4.2	- - - 29.0	5.0	0:37 0:18 0:25 0:43 1:19	(3.2) 64%						COASTAL PLAIN Same as above w/layering up to 0.2ft. thick. Friable. Dominantly clayey material.
-		5.0	0:52 0:40 0:28 1:13	(0.0) 0%						COASTAL PLAIN No Return
-0.8	34.0	5.0	1:46 0:42 1:26 2:47 0:48	(2.0) 40%						COASTAL PLAIN Gray, clayey silt w/moderately indurated silty, f. sand (limestone) up to 0.7ft thick.
-5.8 T	39.0	5.0	0:37 0:33 2:24 2:28 1:53	(3.4) 68%						COASTAL PLAIN Same as above w/layering up to 0.5ft thick.
-10.8	44.0	5.0	0:53 0:49 1:31 1:05 0:37	(1.5) 30%						COASTAL PLAIN Same as above w/layering up to 0.2ft thick.
-15.8 -	49.0	5.0	0:37 0:24 5:13 4:07 0:31 0:32	(3.3) 66%						-15.8 COASTAL PLAIN Same as above w/layering up to 1.3ft thick.
-20.8	54.0	5.0	4:18 0:24 1:07 1:38 1:32	(5.0) 100%						COASTAL PLAIN Predominantly unindurated friable f. grained sandy silt. Trace moderately indurated layers up to 0.4ft. thick.
-25.8		5.0	0:59 0:25 0:36 1:45 0:54	(3.4) 68%						COASTAL PLAIN Gray, unindurated to friable f. grained sandy to clayey silt.
-30.8		5.0	0:42 0:38 0:57 1:54 1:08	(5.0) 100%						COASTAL PLAIN Same as above.
-35.8	- 69.0		1:36							Boring Terminated at Elevation -35.8 ft in friable to moderately indurated f. grained sandy silt. Peedee Formation



B1-A (BOX 1 of 4) = ELEV. 12.2ft to -5.8ft



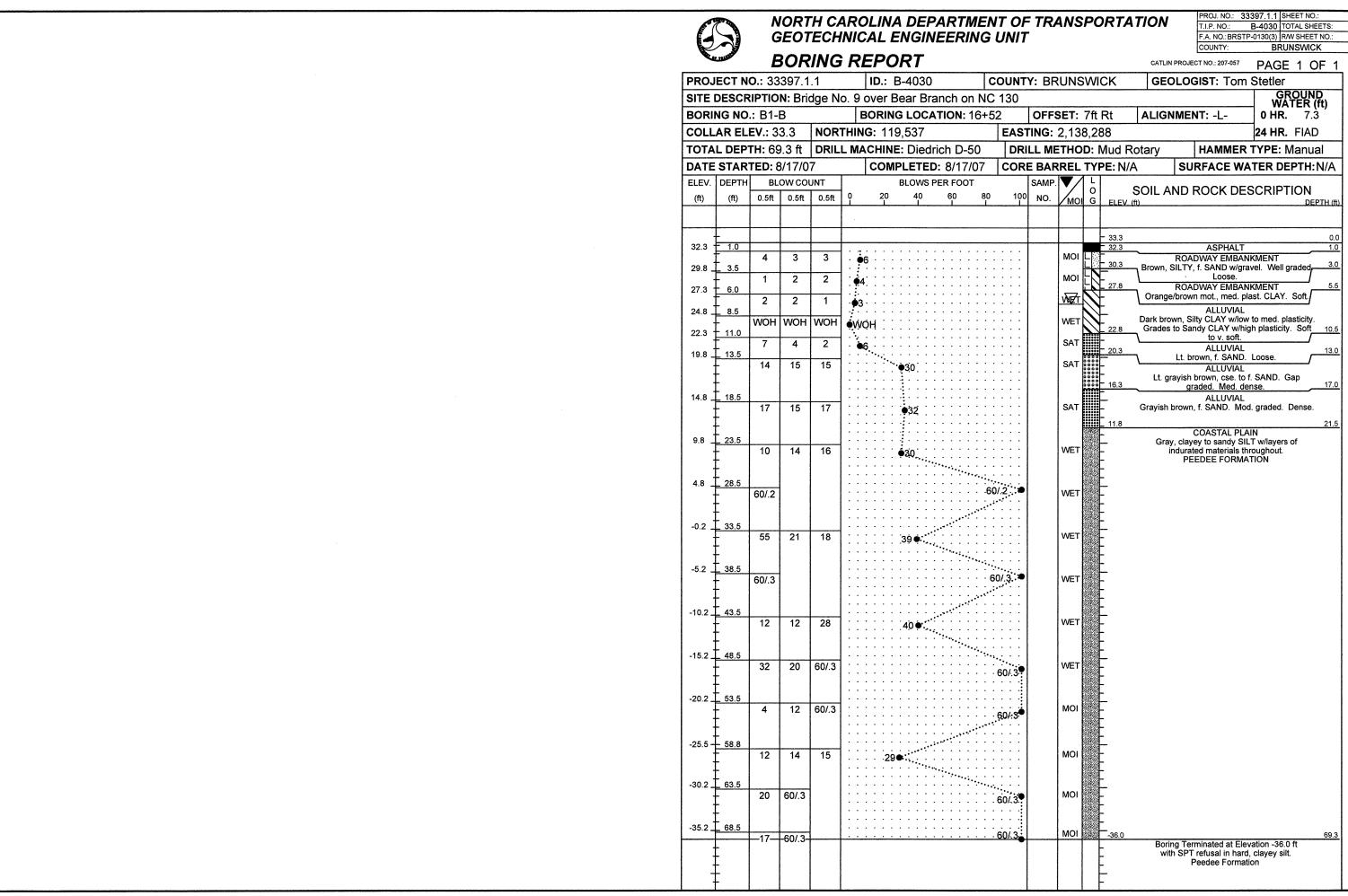
B1-A (BOX 2 of 4) = ELEV. -5.8ft to -20.8ft



B1-A (BOX 3 of 4) = ELEV. -20.8ft to -30.8ft



B1-A (BOX 4 of 4) = ELEV. -30.8ft to -35.8ft



NORTH CAROLINA DEPARTMENT OF TRANSPORTATION GEOTECHNICAL ENGINEERING UNIT

BORING REPORT

CATLIN CATLIN PROJECT NO.: 207-057

PAGE 1 OF 1

PROJECT NO.: 33397.1.1 ID.: B-4030 **COUNTY: BRUNSWICK GEOLOGIST:** Tom Stetler GROUND WATER (ft) SITE DESCRIPTION: Bridge No. 9 over Bear Branch on NC 130 **BORING LOCATION: 17+11** OFFSET: 7ft Lt ALIGNMENT: -L-**BORING NO.: B2-A 0 HR.** 3.1 **COLLAR ELEV.: 33.3 NORTHING: 119,511 EASTING: 2.138.343 24 HR.** 8.9 **DRILL METHOD:** Mud Rotary **HAMMER TYPE: Manual** TOTAL DEPTH: 68.7 ft | DRILL MACHINE: Diedrich D-50 DATE STARTED: 8/15/07 **COMPLETED:** 8/15/07 CORE BARREL TYPE: N/A SURFACE WATER DEPTH: N/A ELEV. DEPTH BLOW COUNT **BLOWS PER FOOT** SOIL AND ROCK DESCRIPTION (ft) 0.5ft 0.5ft 0.5ft NO. - 33.3 32.3 ROADWAY EMBANKMEN 23 Brown to orange brown, Silty CLAY. Low 29.8 plasticity. Blow count affected by asphalt. WET ROADWAY EMBANKMENT 27.3 6.0 Dark brown, Silty CLAY w/med. plasticity and WET interlayered Silty, f. Sand. V. soft to soft. 24.8 SAT ALLUVIAL WOH WOH WOH Dark brown, Silty CLAY w/med. plasticity and 22.3 interlayered Silty, f. Sand. V. soft to soft. SAT 2 5 ALLUVIAL Brown, SILTY f. SAND interlayered w/f. sand. 19.8 SAT Med. dense. ALLUVIAL 14.8 I 18.5 Gray, f. SAND w/trace silt/clay. Med. dense WET COASTAL PLAIN
Gray, f. Sandy to Clayey SILTY w/interbedded
indurated layers. 9.8 <u>I</u> 23.5 WET 8 PEEDEE FORMATION 4.8 ___ 28.5 WET 10 -0.2 I 33.5 WET 10 -5.2 <u>I</u> 38.5 WET 20 60/.2 15 -10.2 **X** 43.5 WET 11 -15.2 <u>I 48.5</u> WET 30 60/.3 25 60/.3 -20.2 ___ 53.5 60/.3 MOI 60/.3 -25.2 ___ 58.5 MOI 13 15 32 -30.2 63.5 MOI 60/.3 $-35.2 \perp 68.5$ MOI 60/.2 Boring Terminated at Elevation -35.4 ft with SPT refusal in hard, f. grained sandy to clayey silt. Peedee Formation



NORTH CAROLINA DEPARTMENT OF TRANSPORTATION **GEOTECHNICAL ENGINEERING UNIT**

T.I.P. NO.: B-4030 TOTAL SHEETS: F.A. NO.: BRSTP-0130(3) R/W SHEET NO.: **BRUNSWICK**

BORING REPORT CATLIN PROJECT NO.: 207-057 PAGE 1 OF 1 PROJECT NO.: 33397.1.1 ID.: B-4030 COUNTY: BRUNSWICK **GEOLOGIST:** Tom Stetler SITE DESCRIPTION: Bridge No. 9 over Bear Branch on NC 130 **BORING NO.:** B2-B **BORING LOCATION: 17+09** OFFSET: 7ft Rt ALIGNMENT: -L-OHR. NM COLLAR ELEV.: 33.6 **NORTHING:** 119,501 **EASTING: 2,138,333 24 HR.** 5.1 TOTAL DEPTH: 50.0 ft | DRILL MACHINE: Diedrich D-50 DRILL METHOD: SPT Core Boring | HAMMER TYPE: Manual CORE BARREL TYPE: NWD4 SURFACE WATER DEPTH: N/A **DATE STARTED: 8/16/07** COMPLETED: 8/17/07 ELEV. DEPTH **BLOW COUNT BLOWS PER FOOT** SOIL AND ROCK DESCRIPTION NO. 0.5ft 0.5ft 0.5ft 32.6 MOI ROADWAY EMBANKMEN **•**10 30.1 Orange brown, CLAY w/low plasticity. Stiff. ROADWAY EMBANKMENT V 12 2 Dark brown, CLAY w/med. plasticity and trace_ 27.6 6.0 silt. Stiff. Blows influenced by debris. No WET 2 return, lithology infered from topography.

ALLUVIAL 25.1 Dark brown, CLAY w/med. plasticity and trace silt. V. soft. WOH WOH WOH WET ♦WOH 22.6 ALLUVIAL Brown, SILTY, f. SAND. V. loose. SAT 20.1 13.5 SAT ALLUVIAL Lt. brown, cse. to f. SAND. Med. dense. Loss of drilling fluids throughout. 15.1 | 18.5 12 21.0 12.6 COASTAL PLAIN 16 39 24 Boring advanced with NWD4 core barrel. Gray, f. Sandy to Clayey SILT w/interbedded indurated layers. Hard. PEEDEE FORMATION Boring Terminated at Elevation -16.4 ft with core in sandy to clayey silt w/interlayered indurated calcite cemented sand. Peedee Formation



CORE SIZE NWD4NWD4

1:40

0:46 1:02 0:27 2:08 0:48

3:02 3:57 1:53 2:57

-16.4 **T** 50.0

(4.8) 96%

(1.8) 36%

NORTH CAROLINA DEPARTMENT OF TRANSPORTATION GEOTECHNICAL ENGINEERING UNIT

CORE BORING REPORT

TOTAL RUN 25.0 ft

CATLIN PROJECT NO.: 207-057 PAGE 1 OF 1

PROJECT NO.: 33397.1.1 ID.: B-4030 COUNTY: BRUNSWICK **GEOLOGIST:** Tom Stetler GROUND WATER (ft) **SITE DESCRIPTION:** Bridge No. 9 over Bear Branch on NC 130 **BORING LOCATION: 17+09** OFFSET: 7ft Rt ALIGNMENT: -L-**BORING NO.:** B2-B **0 HR.** 5.1 COLLAR ELEV.: 33.6 NORTHING: 119,501 **EASTING:** 2,138,333 **24 HR.** 5.1 DRILL METHOD: SPT Core Boring | HAMMER TYPE: Manual TOTAL DEPTH: 50.0 ft | DRILL MACHINE: Diedrich D-50

DATE STARTED: 8/16/07 COMPLETED: 8/17/07 CORE BARREL TYPE: NWD4 SURFACE WATER DEPTH: N/A DRILLER Bobbie D. Fowler

CORE SIZE	14VVD41	44404	ı	IOIA	LIVON	25.0 11			DRILLER Bobble D. I Owler	
ELEV DEPTH	RUN (ft)	DRILL RATE (Min/ft)	REC. (ft) %	UN RQD (ft) %	SAMP. NO.	STF REC. (ft) %	RATA RQD (ft) %	LOG	DESCRIPTION AND REMARKS ELEV.	DEPTH
									Begin Coring @ 25.0 ft	
8.6 <u>25.0</u> 3.6 30.0	5.0	1:21 0:56 1:53 1:50 0:45	(2.0) 40%						- 8.6 COASTAL PLAIN Gray, f. grained sandy to clayey SILT w/intermittent moderately indurated layeri Light colored impressions/casts. Peedee Formation	25.0 ing. 30.0
-1.4 - 35.0	5.0	2:07 0:48 1:34 0:36 1:23	(3.1) 62%						COASTAL PLAIN Same as above w/moderately indurated calcareous sand layers up to 0.4ft thic	ck. 35.0
	5.0	0:23 0:21 1:47	(3.7) 74%						COASTAL PLAIN Same as above.	

COASTAL PLAIN Same as above but dominantly friable w/interlayered moderately indurated layers up to 0.4ft thick.

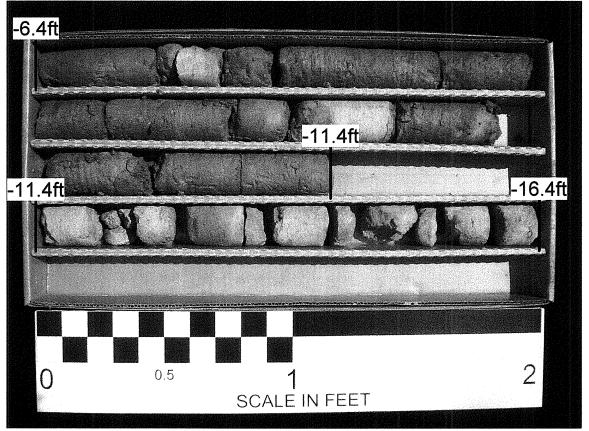
COASTAL PLAIN Same as above w/fines washed from barrel. Very low circulation due to clogged inner barrel.

Boring Terminated at Elevation -16.4 ft in f. grained sandy to clayey silt w/moderately indurated layers.

B-4030 TOTAL SHEETS: F.A. NO.: BRSTP-0130(3) R/W SHEET NO. BRUNSWICK



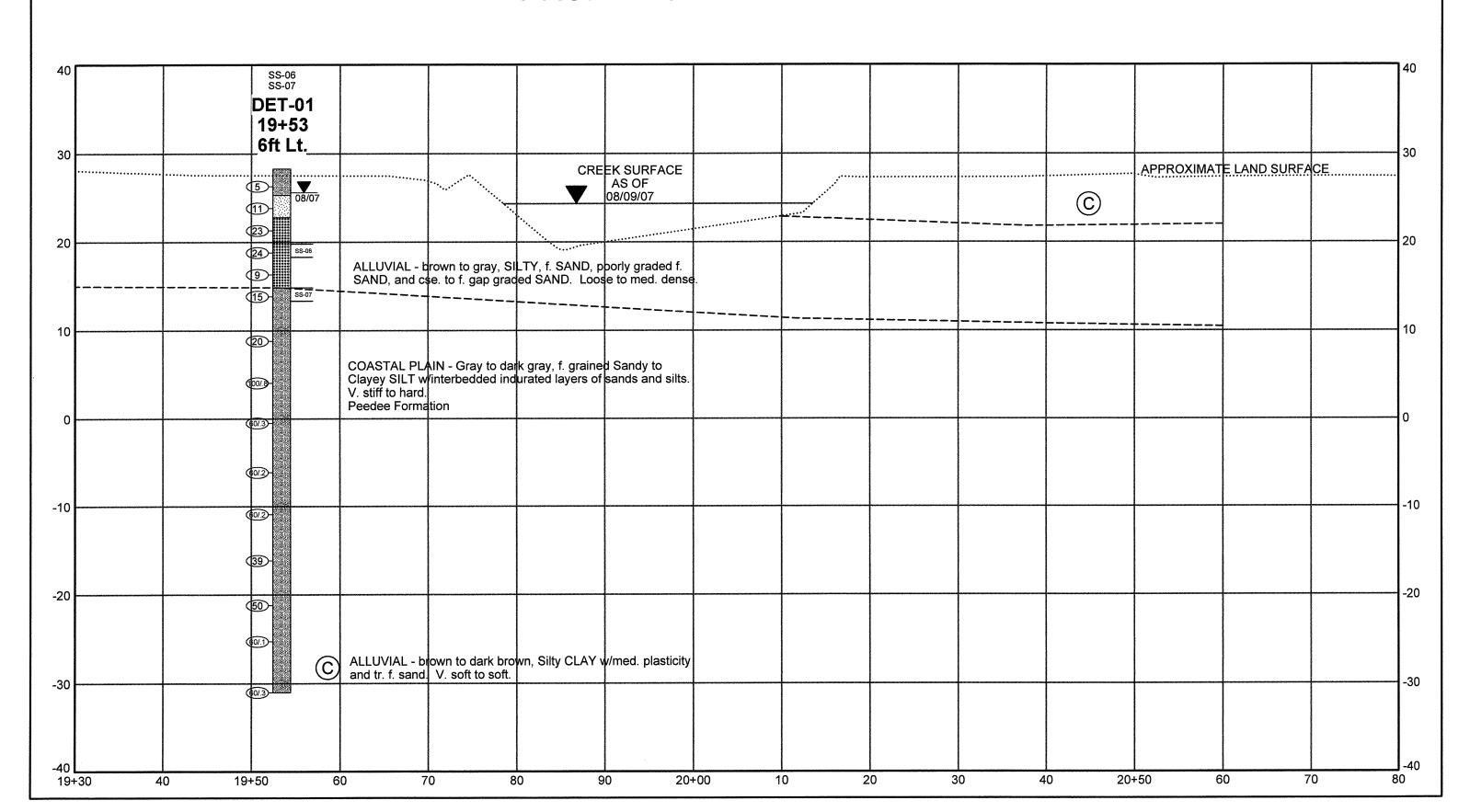
B2-B (BOX 1 of 2) = ELEV. 8.6ft to -6.4ft



B2-B (BOX 2 of 2) = ELEV. -6.4ft to -16.4ft

	JEST NO.
NORTH CAROLINA DEPARTMENT OF TRANSPORTATION PROJ. NO.: 33397.1.1 SHE T.I.P. NO.: B-4030 TOT	TAL SHEETS:
	NSWICK
	1 OF 1
PROJECT NO.: 33397.1.1 ID.: B-4030 COUNTY: BRUNSWICK GEOLOGIST: Tom Stetler	OUND
SITE DESCRIPTION: Bridge No. 9 over Bear Branch on NC 130 BORING NO.: EB2-B BORING LOCATION: 17+37 OFFSET: 7ft Rt ALIGNMENT: -L- 0 HR.	TER (ft)
BORING NO.: EB2-B BORING LOCATION: 17+37 OFFSET: 7ft Rt ALIGNMENT: -L- 0 HR. COLLAR ELEV.: 33.3 NORTHING: 119,484 EASTING: 2,138,355 24 HR.	1
TOTAL DEPTH: 58.8 ft DRILL MACHINE: Diedrich D-50 DRILL METHOD: Mud Rotary HAMMER TYPE: M	
DATE STARTED: 8/7/07 COMPLETED: 8/2/07 CORE BARREL TYPE: N/A SURFACE WATER DEF	PTH:N/A
ELEV. DEPTH BLOW COUNT BLOWS PER FOOT SAMP. U C SOIL AND ROCK DESCRIPTI	ION
(ft) (ft) 0.5ft 0.5ft 0.5ft 0 20 40 60 80 100 NO. MOI G ELEV. (ft)	DEPTH (ft)
- 33.3	0.0
32.3 1.0 ASPHALT	1.0
29.8 3.5 Dark brown grading to black, SILTY, f. S.	SAND $\frac{3.0}{4.0}$
27.3 6.0 Crange brown silty to f sandy CI AY	5.5
24.8 8.5 POADWAY EMBANKMENT	
Light brown, SILTY, f. SAND w/black	k 11,5
19.8 13.5 ALLUVIAL Dark brown, silty CLAY, Low plasticity w/	
4 3 4 · •7 · · · · · · · · · · SS-02 SAT	
+ 16.3 Dark brown, SILTY, f. SAND. Tr. clay. Lo	
5 9 11 · · · · • • • • • · · · · · · · · ·	ap caving
9.8 23.5 during drilling at 22ft. BLS.	22.5
9.8 23.5 COASTAL PLAIN 18 33 60/.3 SS-04 MOI - Gray, clayey to sandy SILT w/friable to indurated lenses throughout. V. stiff to he PEEDEE FORMATION.	to hard
	iaru.
4.8 28.5 MOI	
-0.2 33.5 SS-05 WET	
+	
-5.2	
-10.2	
-15.2 <u>48.5</u>	
† 19 16 16 : : : : : : : : : : : : : : :	
-20.2 53.5	
+ 10 10 10 •20	
-25.2 <u>58.5</u> WET —-25.5	58.8
+ 60/.3 - Boring Terminated at Elevation -25.5 - with SPT refusal in hard, f. grained sand	ft dy to
clayey silt w/indurated layers and through	hout.
Ŧ	
‡	
<u> </u>	
£ E	
‡	

PROFILE CENTER LINE -DET-



7.1.1 ID.: B-4030 COUNTY: BRUNSWICK GEOLOGIST: Tom Stetler Strong State Stron	TH CAROLINA DEPARTMENT OF TRANSPORTATION ECHNICAL ENGINEERING UNIT PING REPORT CATLIN PROJECT NO.: 233397.1.1 SHEET NO.: 16 T.I.P. NO.: B-4030 TOTAL SHEETS: 20 F.A. NO.:BRSTP-0130(3) RW SHEET NO.: COUNTY: BRUNSWICK CATLIN PROJECT NO.: 207-057 PAGE 1 OF 1	GEOTE	
Bridge No. 9 over Bear Branch on NC 130	17,02 1 01 1	JECT NO.: 33397.1.1	PROJ
BORING LOCATION: 19+53 OFFSET: 6ft Lt ALIGNMENT: -DET- 0 HR. 4.4			
Same	BORING LOCATION: 19+53 OFFSET: 6ft Lt ALIGNMENT: -DET- 0 HR. 4.4	NG NO.: DET-01	
The content of the	y		
COMPLETED: 8/9/07 CORE BARREL TYPE: N/A SURFACE WATER DEPTH:			
SAMP NO SAMP SA		STARTED: 8/9/07	
10	JNT BLOWS PER FOOT SAMP. V L SOUL AND POCK DESCRIPTION	DEPTH BLOW COUN	ELEV.
ALLUVIAL 22.8 Brown, clayer SILT wirdsee organic fragments. ALLUVIAL 22.8 Light brown, SILT virtace sit. Poorly graded. Med. dense. ALLUVIAL Light brown, f. SAND wirtace sit. Poorly graded. Med. dense. ALLUVIAL Light brown, f. SAND wirtace sit. Poorly graded. Med. dense. PEEDEE FORMATION Met Me	0.5ft 0 20 40 60 80 100 NO. MOI G SOIL AND ROCK DESCRIPTION	(ft) 0.5ft 0.5ft 0	(ft)
ALLUVIAL 22.8 Brown, clayer SILT wirdsee organic fragments. ALLUVIAL 22.8 Light brown, SILT virtace sit. Poorly graded. Med. dense. ALLUVIAL Light brown, f. SAND wirtace sit. Poorly graded. Med. dense. ALLUVIAL Light brown, f. SAND wirtace sit. Poorly graded. Med. dense. PEEDEE FORMATION Met Me			
ALLUVIAL 22.8 Brown, clayer SILT wirdsee organic fragments. ALLUVIAL 22.8 Light brown, SILT virtace sit. Poorly graded. Med. dense. ALLUVIAL Light brown, f. SAND wirtace sit. Poorly graded. Med. dense. ALLUVIAL Light brown, f. SAND wirtace sit. Poorly graded. Med. dense. PEEDEE FORMATION Met Me		+	
2 3 6 5 111 228		‡	-
Second S	3 As Brown, clayey SILT w/trace organic fragments.	1.0	27.3
13 14 15 15 16 17 17 17 17 17 17 17	25.3 ALLUVIAL 3.0	3.5	24.8
13	22.8 Light blown, SiETT, 1. SAND. Pooling gladed. 5.5	+ 6.0 6 6 6 1	22.3
6 9 11	13 •23 WET ALLUVIAL		19.8
6 9 11	16 SS-06 SAT Lt. brown, T. SAND witrace slit. Poorly graded. Med. dense.	7 8	
6 9 11	4	11.0 4 5	17.3
Gray, Clayer to Sandy SLT windurated lenses throughout. Dense to v. dense. 9 11	13.5	I 13.5	14.8
9 11	SS-07 WE Gray, clayey to sandy SILT w/indurated lenses	 	:
9 11	throughout. Dense to v. dense. PEEDEE FORMATION	F 18 5	00
60/.3 WET		18.5	9.8 _
60/.3 WET		‡	:
60/.3 WET		23.5	4.8
60/.2 60/.2 WET 3 26 435 WET 60/.2 MOI 60/.1 MOI 60/.3 Boring Terminated at Elevation -31.0 ft		69 31/.3	:
60/.2 60/.2 WET 3 26 435 WET 60/.2 MOI 60/.1 MOI 60/.3 Boring Terminated at Elevation -31.0 ft		+ 20.5	
60/.2 60/.2 WET 3 26 435 WET 60/.2 MOI 60/.1 MOI 60/.3 Boring Terminated at Elevation -31.0 ft	· · · · · · · · · · · · · · · · · · ·	28.5 60/.3	-0.2 _
3 26		‡	:
3 26			-5.2
3 26 •39 WET - MOI	60/.2	20 21 6	
3 26 •39 WET - MOI		+ 39.5	:
3 26 •35 WET	WET WET	38.5 24 60/.2	-10.2
60/.1 MOI -		‡	:
60/.1 MOI -			-15.2
60/.1 MOI31.0 Boring Terminated at Elevation -31.0 ft	26	6 13	
60/.1 MOI31.0 Boring Terminated at Elevation -31.0 ft		<u> </u>	::
60/.1 MOI31.0 Boring Terminated at Elevation -31.0 ft	20 MOI		-20.2 _
7/.3 MOI -31.0 Boring Terminated at Elevation -31.0 ft		‡ " "	:
7/.3 MOI -31.0 Boring Terminated at Elevation -31.0 ft		53.5	-25.2
Boring Terminated at Elevation -31.0 ft	· · · · · · · · · · · · · · · · · · ·	60/.1	
Boring Terminated at Elevation -31.0 ft		<u> </u>	:
I I Boring Terminated at Elevation -31.0 ft	MOI -31.0 59.3	58.5	-30.2 _
Peedee Formation	in hard, f. grained sandy to clayey silt w/indurated layers and throughout. Peedee Formation		-

NCDO



FIELD SCOUR REPORT

WBS:	33397.1.1	TIP:	B-4030	COUNTY: BRUNSWICK
DESCRIPTION(1):				
			EXISTING	G BRIDGE
Information from:	Field II Other	nspection (explain)	X Mi	licrofilm (reel pos:)
				3 Bents in Channel: 1 Bents in Floodplain: 3 rpiles and timber endbent abutments.
EVIDENCE OF	SCOUR(2)			
£ 8	End Bent Slopes			dence of scour. Looks like fill may have been placed
	behind abutmer	nts (paveme	nt patches on	n each side of bridge).
Interior Bents:	Stable. No obv	ious evidend	ce of scour.	
Channel Bed:	Stable. No obv	ious evidend	ce of scour.	
Channel Bank:	Stable. No obv	ious evidend	ce of scour.	
EXISTING SCO	JR PROTECTIO	N		
	Wooden wing w			
Extent(4):	Wing walls cove	er endbents	and extend a	pproximately 10ft. Upstream and downstream.
Effectiveness(5):	Appears adequa	ate.		
Obstructions(6):	None.			

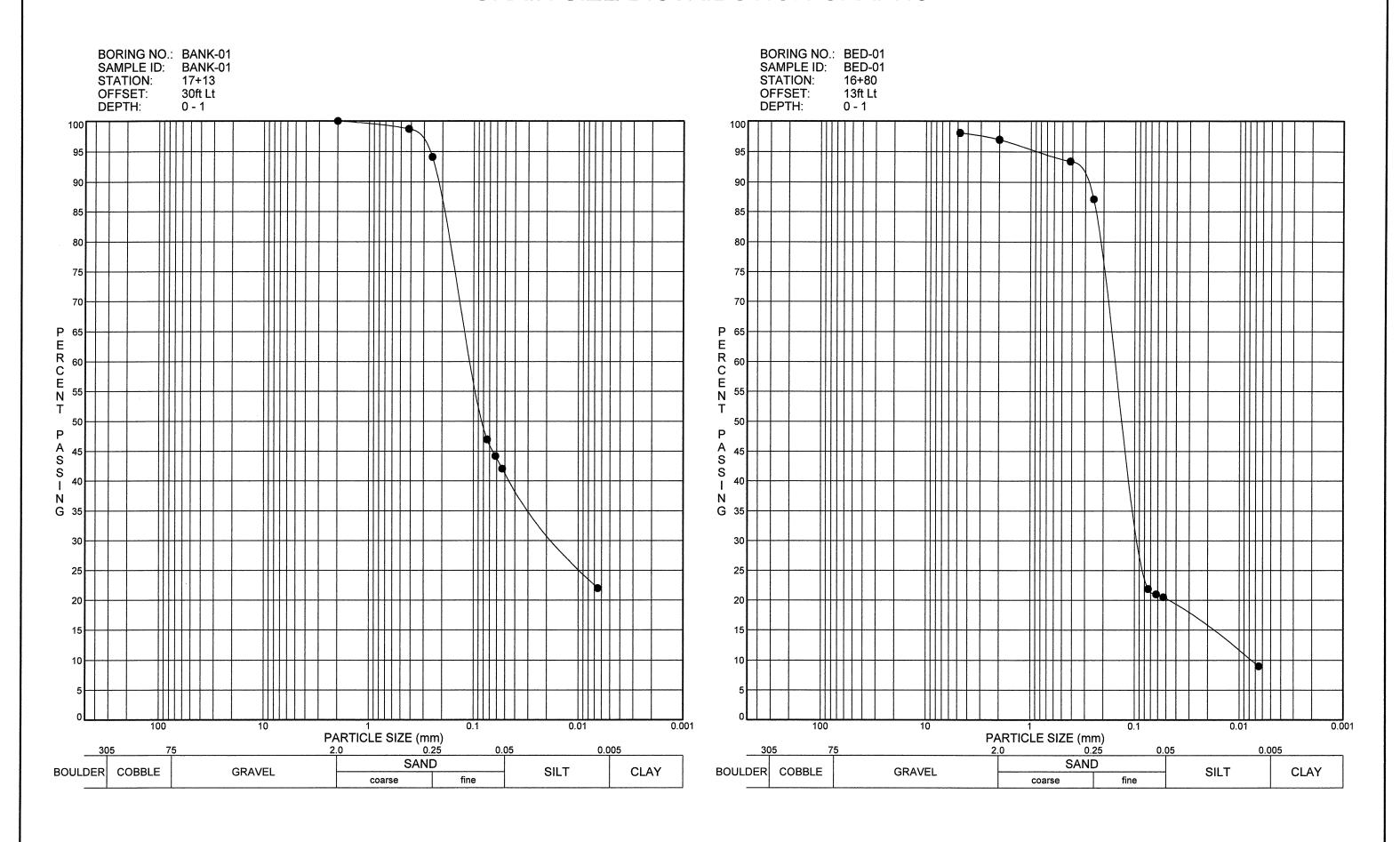
INSTRUCTIONS

- 1 Describe the specific site's location, including route number and body of water crossed.
- 2 Note scour evidence at existing end bents or abutments (e.g. undermining, sloughing, degradations).
- 3 Note existing scour protection (e.g. rip rap).
- 4 Describe extent of existing scour protection.
- 5 Describe whether or not the scour protection appears to be working.
- 6 Note obstructions such as dams, fallen trees, debris at bents, etc.
- 7 Describe the channel bed material based on observation and/or samples. Include any lab results with report.
- 8 Describe the channel bank material based on observation and/or samples. Include any lab results with report.
- 9 Describe the material covering the banks (e.g. grass, trees, rip rap, none).
- 10 Determine the approximate floodplain width from field observation or a topographic map.
- 11 Describe the material covering the floodplain (e.g. grass, trees, crops).
- 12 Use professional judgement to specify if the stream is degrading, aggrading, or static.
- 13 Describe potential and direction of the stream to migrate laterally during the bridge's life (approx. 100 years).
- Give the design scour elevation (DSE) expected over the life of the bridge (approx. 100 years). This elevation can be given as a range across the site, or for each bent. Discuss the relationship between the Hydraulics Unit theoritical scour and the DSE. If the DSE is dependent on scour counter measures, explain (e.g. rip rap armoring on slopes). The DSE is based on the erodability of materials, giving consideration to the influence of joints, foliation, bedding characteristics, % core recovery, % RQD, differential weathering, shear strength, observations at existing structures, other tests deemed appropriate, and overall geologic conditions at the site.

			DESIG	<u>II Nê</u>	NFORM	ATION					
Channel	Bed Material(n, SILTY				ttle cse.	sand and	l organic	debris.	
Channel E	Bank Material(Trace c	se. Sand	l.		
Channe	el Bank Cover(9): Grass and	brush wit	th soı	me trees.						
Flood	dplain Width(1	0): Approxima	itely 900ft	t.							
	dplain Cover(1	-				· · · · · · · · · · · · · · · · · · ·					
.,000		-						01			
		2): Aggı				ading		Sta	atic X		
annel Migratio	n Tendency(1	3): Slight pote	ntial for N	1W m	niration.			***************************************			
Observations	and Other Co	mments:									
		Reported by:		2	0//	7//					
		Reported by:	<u> </u>	<u> </u>	Steven	Hudson			Date:	8/9/20	007
DESIGN SCO	UR ELEVATION	ONS(14)				Feet	X	Met	ers		
	BEN'										
1	00 Year 21.						T	I	<u> </u>		Т
	00 Year 19.										
						<u> </u>	<u></u>	<u> </u>			<u></u>
Comparison of	-						. 4	41	D.:I-I	0	
The Geotechn and Hydraulics		ng Unit agrees	with the	tneoi	retical sc	our eleva	ations sn	own on t	ne Briage	Survey	
and riyuraulic	s neport.			***************************************							
		etermined by:				•			Date:	9/26/2	:007
SOIL ANALYS Bed or Bank	BANK-01	BED-01	NEL BED	O ANI	D BANK	MATERI	AL				
Sample No.	BANK-01	BED-01									
Retained #4	0	2	†								
Passed #10	100	97									
Passed #40	99	93									
Passed #200	47	22									
Coarse Sand	5.9	12.8									
Fine Sand	52.1	66.7									
Silt	20.0	11.5								<u></u>	
Clay	22.0	9.0									
LL	26	23									
PI	11	NP									
AASHTO Station	A-6(2) 17+13	A-2-4(0) 16+80	 		<u> </u>						
Offset	30 Lt	10+60 13 Lt									
Depth		0.0 - 1.0	 		<u> </u>						

GRAIN SIZE DISTRIBUTION GRAPHS





NCDOT LABORATORY SUMMARY SHEET

Bridge No. 9 over Bear Branch on NC 130

ENGLISH

PROJ. NO.: **33397.1.1** SHEET NO.: T.I.P. NO.: **B-4030** TOTAL SHEETS: T.I.P. NO.: F.A. NO.: BRSTP-0130(3) R/W SHEET NO.: CATLIN PROJECT:

207-057

CATLIN

GEOTECHNICAL LABORATORY Wilmington, North Carolina

AASHTO Standard Specifications

(As modified by NCDOT, Material and Tests Unit, 2000.)

							TEST RESU	LTS				
Field Sample Number	BANK-01	BED-01	SS-01	SS-02	SS-03	SS-04	SS-05	SS-06	SS-07			
Lab Sample Number	BANK-01	BED-01	SS-01	SS-02	SS-03	SS-04	SS-05	SS-06	SS-07			
Retained #4 Sieve	6 0	2	0	0	8	4	4	0	0			
Passing #10 Sieve	6 100	97	100	100	70	96	96	100	100	, , , , , , , , , , , , , , , , , , , 		
Passing #40 Sieve	6 99	93	99	98	28	94	92	98	99			
Passing #200 Sieve	6 47	22	59	21	3	42	37	4	44			
						MINUS	NUMBER 10	FRACTION				
SOIL MORTAR - 100%												
Coarse Sand Ret#60	6 5.9	12.8	6.3	8.4	85.4	7.5	10.6	29.7	2.9			
Fine Sand Ret#270	6 52.1	66.7	37.8	75.0	12.5	57.3	59.5	66.7	59.1			
Silt 0.05 - 0.005mm	6 20.0	11.5	27.6	12.8	0.2	15.3	14.0	2.7	18.1			
Clay <0.005mm	6 22.0	9.0	28.3	3.8	1.9	19.9	15.9	0.9	19.9			
Liquid Limit (LL	26	23	37	24	18	23	23	22	23	***************************************		
Plasticity Index (PI	11	NP	15	NP	NP	NP	NP	NP	NP			
AASHTO Classification Group Index	A-6(2)	A-2-4(0)	A-6(7)	A-2-4(0)	A-1-b(0)	A-4(0)	A-4(0)	A-3(0)	A-4(0)			
Station	17+13	16+80	17+37	17+37	17+37	17+37	17+37	19+53	19+53			
Offset	30ft LT	13ft LT	7ft RT	6ft LT	6ft LT							
Alignment	-L-	-DET-	-DET-									
Boring Identification	BANK-01	BED-01	EB2-B	EB2-B	EB2-B	EB2-B	EB2-B	DET-01	DET-01			
Depth ()	0.0	0.0	6.0	13.5	18.5	23.5	33.5	8.5	13.5			
to	1.0	1.0	7.5	15.0	20.0	24.8	35.0	10.0	15.0			
ield Moisture Content	18	50	36	24	13	26	29	23	27			
Tested By	MDMason	MDMason	MDMason									
Submitted By	Steve Hudson	Steve Hudson	Steve Hudson									
Date Submitted	08/14/07	08/14/07	08/08/07	08/08/07	08/08/07	08/08/07	08/08/07	08/14/07	08/14/07			

NP = Non-Plastic

Report Date: 8/21/2007

Laboratory Report Page 1 of 1

PROJ. NO.: 33397.1.1 SHEET NO.:

T.I.P. NO.: B-4030 TOTAL SHEET S:

F.A. NO.: BRSTP-0130(3) RW SHEET NO.:

COUNTY: BRUNSWICK



CENTER LINE -L- FACING SOUTHEAST



BRIDGE 9 FACING DOWNSTREAM (SOUTHWEST)



CENTERLINE -L- FACING NORTHWEST



BRIDGE 9 FACING UPSTREAM (NORTHEAST)