

STATE	STATE PROJECT REFERENCE NO.	SHEET NO.	TOTAL SHEETS
N.C.	R-2719A	1	139
STATE PROJ. NO.	F.A. PROJ. NO.	DESCRIPTION	
34501.1.1	STP-0224(3)	PE	
34501.2.4	STP-0224(8)	ROW, Util.	
34501.3.4	STP-0224(12)	Const.	

STATE OF NORTH CAROLINA
DEPARTMENT OF TRANSPORTATION
DIVISION OF HIGHWAYS
GEOTECHNICAL UNIT

ROADWAY
SUBSURFACE INVESTIGATION

STATE PROJ. 34501.1.1 I.D. R-2719A F.A. PROJ. STP-0224(3)
 COUNTY LENOIR
 PROJECT DESCRIPTION CRESCENT RD. IN KINSTON FROM
US 70 TO US 258

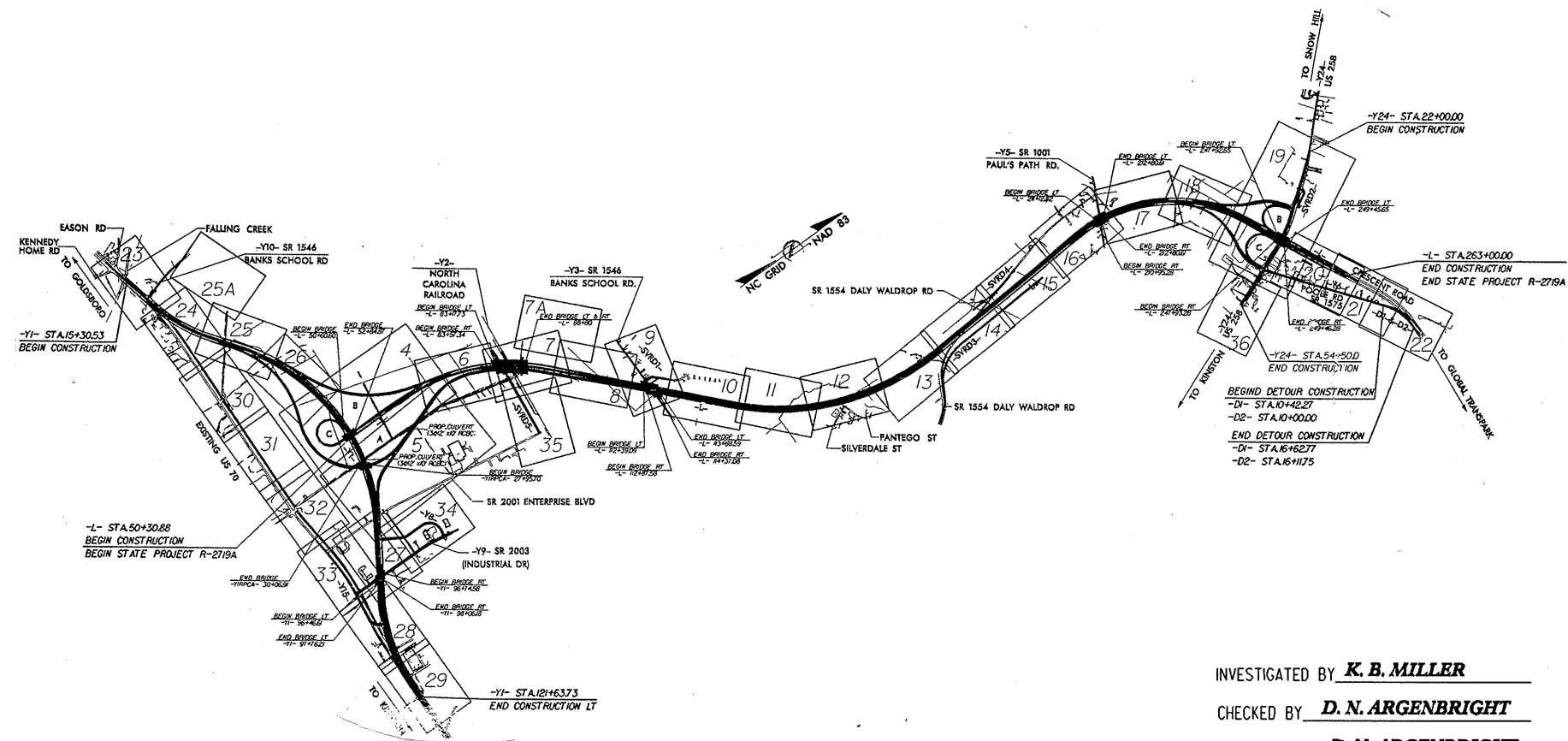
INVENTORY

NOTE: SEE SHEET 2A FOR PLAN SHEET LAYOUT AT TIME OF INVESTIGATION

CONTENTS

LINE	STATION	PLAN	PROFILE
-L-	50+31 TO 263+00	4, 6-21	37-52
-Y1-	15+30 TO 121+63	4,5,23-29	53-57
-YIRPA-	0+00 TO 16+04	5	58, 59
-YIRPB-	0+00 TO 35+55	4, 6, 26	60, 61
-YIRPCA-	0+00 TO 68+73	4-6, 25, 26	62-66
-YILPC-	0+00 TO 14+34	4	67, 68
-Y6-	10+00 TO 34+00	19-21	85
-Y8-	10+00 TO 18+23	27, 34	69
-Y9-	10+00 TO 26+00	27, 33, 34	70
-Y10-	10+00 TO 17+80	23	71
-Y14-	10+00 TO 19+62	5, 34	72
-Y15-	18+00 TO 31+75	28, 33	73
-SVRD1-	10+00 TO 22+00	9	74
-SVRD2-	10+00 TO 14+40	19	75
-SVRD3-	15+00 TO 39+65	13-15	76, 77
-SVRD4-	11+50 TO 18+94	14, 15	78
-SVRD5-	10+00 TO 27+64	7, 35	87
-Y24-	22+00 TO 54+50	19, 36	79, 80
-Y24LPB-	0+00 TO 11+63	19	81
-Y24RPB-	0+00 TO 19+65	18, 19	82
-Y24LPC-	0+00 TO 12+38	19	83
-Y24RPC-	0+00 TO 21+86	18, 19	84
-D1-	10+00 TO 20+09	21, 22	86
-D2-	10+00 TO 19+42	21, 22	86

LINE	STATION	XSECTS
-L-	129+00 TO 133+00	88-90
-L-	143+50 TO 150+50	91-93
-L-	174+00 TO 177+00	94-95
-L-	188+00 TO 194+50	96-99
-Y1-	16+00	100
-Y1-	18+00	101
-Y1-	22+00	102
-Y1-	24+00	103
-Y1-	32+00	104
-Y1-	34+00	105
-Y1-	43+00 TO 68+50	106-115
-Y1-	108+00	116
-YILPC-	9+50 TO 10+50	117
-YIRPCA-	54+50 TO 60+83	118-122
-Y6-	24+00	123
-Y9-	10+50	124
-Y9-	16+00	125
-Y10-	14+00	126
-Y14-	10+50	127
-Y14-	14+00	128
-Y14-	18+00	129
-SVRD5-	14+00 TO 27+50	130-136
-L-	136+00 TO 140+50	137-139



CAUTION NOTICE

THE SUBSURFACE INFORMATION AND THE SUBSURFACE INVESTIGATION ON WHICH IT IS BASED WAS MADE FOR THE PURPOSE OF STUDY, PLANNING AND DESIGN, AND NOT FOR CONSTRUCTION OR PAY PURPOSES. THE VARIOUS FIELD BORING LOGS, ROCK CORES, AND SOIL TEST DATA AVAILABLE MAY BE REVIEWED OR INSPECTED IN RALEIGH BY CONTACTING THE N.C. DEPARTMENT OF TRANSPORTATION, GEOTECHNICAL UNIT & (919) 250-4088. NEITHER THE SUBSURFACE PLANS AND REPORTS, NOR THE FIELD BORING LOGS, ROCK CORES, OR SOIL TEST DATA IS PART OF THE CONTRACT.

GENERAL SOIL AND ROCK STRATA DESCRIPTIONS AND INDICATED BOUNDARIES ARE BASED ON A GEOTECHNICAL INTERPRETATION OF ALL AVAILABLE SUBSURFACE DATA AND MAY NOT NECESSARILY REFLECT THE ACTUAL SUBSURFACE CONDITIONS BETWEEN BORINGS OR BETWEEN SAMPLED STRATA WITHIN THE BOREHOLE. THE LABORATORY SAMPLE DATA AND THE IN SITU (IN-PLACE) TEST DATA CAN BE RELIED ON ONLY TO THE DEGREE OF RELIABILITY INHERENT IN THE STANDARD TEST METHOD. THE OBSERVED WATER LEVELS OR SOIL MOISTURE CONDITIONS INDICATED IN THE SUBSURFACE INVESTIGATIONS ARE AS RECORDED AT THE TIME OF THE INVESTIGATION. THESE WATER LEVELS OR SOIL MOISTURE CONDITIONS MAY VARY CONSIDERABLY WITH TIME ACCORDING TO CLIMATIC CONDITIONS INCLUDING TEMPERATURES, PRECIPITATION AND WIND, AS WELL AS OTHER NON-CLIMATIC FACTORS.

THE BIDDER OR CONTRACTOR IS CAUTIONED THAT DETAILS SHOWN ON THE SUBSURFACE PLANS ARE PRELIMINARY ONLY AND IN MANY CASES THE FINAL DESIGN DETAILS ARE DIFFERENT. FOR BIDDING AND CONSTRUCTION PURPOSES, REFER TO THE CONSTRUCTION PLANS AND DOCUMENTS FOR FINAL DESIGN INFORMATION ON THIS PROJECT. THE DEPARTMENT DOES NOT WARRANT OR GUARANTEE THE SUFFICIENCY OR ACCURACY OF THE INVESTIGATION MADE, NOR THE INTERPRETATIONS MADE OR OPINION OF THE DEPARTMENT AS TO THE TYPE OF MATERIALS AND CONDITIONS TO BE ENCOUNTERED. THE BIDDER OR CONTRACTOR IS CAUTIONED TO MAKE SUCH INDEPENDENT SUBSURFACE INVESTIGATIONS AS HE DEEMS NECESSARY TO SATISFY HIMSELF AS TO CONDITIONS TO BE ENCOUNTERED ON THIS PROJECT. THE CONTRACTOR SHALL HAVE NO CLAIM FOR ADDITIONAL COMPENSATION OR FOR AN EXTENSION OF TIME FOR ANY REASON RESULTING FROM THE ACTUAL CONDITIONS ENCOUNTERED AT THE SITE DEFERRING FROM THOSE INDICATED IN THE SUBSURFACE INFORMATION.

CONTRACT: C201965 ID: R-2719A

DRAWN BY: K. B. MILLER, W. D. FIELDS

NOTE - THE INFORMATION CONTAINED HEREIN IS NOT IMPLIED OR GUARANTEED BY THE N.C. DEPARTMENT OF TRANSPORTATION AS BEING ACCURATE NOR IS IT CONSIDERED TO BE PART OF THE PLANS, SPECIFICATIONS, OR CONTRACT FOR THE PROJECT.

NOTE - BY HAVING REQUESTED THIS INFORMATION THE CONTRACTOR SPECIFICALLY WAIVES ANY CLAIMS FOR INCREASED COMPENSATION OR EXTENSION OF TIME BASED ON DIFFERENCES BETWEEN THE CONDITIONS INDICATED HEREIN AND THE ACTUAL CONDITIONS AT THE PROJECT SITE.

PERSONNEL

C. D. CZAJKA

N. D. MOHS

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W. N. CHERRY

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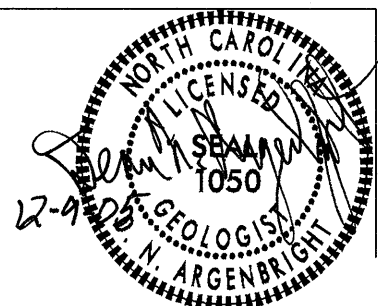
S & ME

INVESTIGATED BY K. B. MILLER

CHECKED BY D. N. ARGENBRIGHT

SUBMITTED BY D. N. ARGENBRIGHT

DATE DECEMBER, 2005








NORTH CAROLINA DEPARTMENT OF TRANSPORTATION
DIVISION OF HIGHWAYS
GEOTECHNICAL UNIT

ID	STATE PROJECT NO.	SHEET NO.	TOTAL SHEETS
R-2719A	34501.1	2	139

SUBSURFACE INVESTIGATION

SOIL AND ROCK LEGEND, TERMS, SYMBOLS, AND ABBREVIATIONS

SOIL DESCRIPTION		GRADATION		ROCK DESCRIPTION		TERMS AND DEFINITIONS					
SOIL IS CONSIDERED TO BE THE UNCONSOLIDATED, SEMI-CONSOLIDATED OR WEATHERED EARTH MATERIALS WHICH CAN BE PENETRATED WITH A CONTINUOUS FLIGHT POWER AUGER, AND WHICH YIELDS LESS THAN 100 BLOWS PER FOOT ACCORDING TO STANDARD PENETRATION TEST (AASHTO T205, ASTM D-1586). SOIL CLASSIFICATION IS BASED ON THE AASHTO SYSTEM AND BASIC DESCRIPTIONS GENERALLY SHALL INCLUDE: CONSISTENCY, COLOR, TEXTURE, MOISTURE, AASHTO CLASSIFICATION, AND OTHER PERTINENT FACTORS SUCH AS MINERALOGICAL COMPOSITION, ANGULARITY, STRUCTURE, PLASTICITY, ETC. EXAMPLE: <i>VERY STIFF, GRAY SILTY CLAY, MOST WITH INTERBEDDED FINE SAND LAYERS, HEAVY PLASTIC, A-7-6</i>		WELL GRADED- INDICATES A GOOD REPRESENTATION OF PARTICLE SIZES FROM FINE TO COARSE UNIFORM- INDICATES THAT SOIL PARTICLES ARE ALL APPROXIMATELY THE SAME SIZE. (ALSO POORLY GRADED) GAP-GRADED- INDICATES A MIXTURE OF UNIFORM PARTICLES OF TWO OR MORE SIZES. ANGULARITY OF GRAINS THE ANGULARITY OR ROUNDNESS OF SOIL GRAINS ARE DESIGNATED BY THE TERMS: ANGULAR, SUBANGULAR, SUBROUNDED, OR ROUNDED.		HARD ROCK IS NON-COASTAL PLAIN MATERIAL THAT WHEN TESTED, WOULD YIELD SPT REFUSAL, AN INFERRED ROCK LINE INDICATES THE LEVEL AT WHICH NON-COASTAL PLAIN MATERIAL WOULD YIELD SPT REFUSAL. SPT REFUSAL IS PENETRATION BY A SPLIT SPOON SAMPLER EQUAL TO OR LESS THAN 0.1 FOOT PER 60 BLOWS, IN NON-COASTAL PLAIN MATERIAL. THE TRANSITION BETWEEN SOIL AND ROCK IS OFTEN REPRESENTED BY A ZONE OF WEATHERED ROCK. ROCK MATERIALS ARE TYPICALLY DIVIDED AS FOLLOWS: WEATHERED ROCK (WR)  NON-COASTAL PLAIN MATERIAL THAT YIELDS SPT N VALUES > 100 BLOWS PER FOOT. CRYSTALLINE ROCK (CR)  FINE TO COARSE GRAIN IGNEOUS AND METAMORPHIC ROCK THAT WOULD YIELD SPT REFUSAL IF TESTED. ROCK TYPE INCLUDES GRANITE, GNEISS, GABBRO, SCHIST, ETC. NON-CRYSTALLINE ROCK (NCR)  FINE TO COARSE GRAIN METAMORPHIC AND NON-COASTAL PLAIN SEDIMENTARY ROCK THAT WOULD YIELD SPT REFUSAL IF TESTED. ROCK TYPE INCLUDES PHYLITE, SLATE, SANDSTONE, ETC. COASTAL PLAIN SEDIMENTARY ROCK (CP)  COASTAL PLAIN SEDIMENTS CEMENTED INTO ROCK, BUT MAY NOT YIELD SPT REFUSAL. ROCK TYPE INCLUDES LIMESTONE, SANDSTONE, CEMENTED SHELL BEDS, ETC.		ALLUVIUM (ALLUV.) - SOILS WHICH HAVE BEEN TRANSPORTED BY WATER. AQUIFER - A WATER BEARING FORMATION OR STRATA. ARENACEOUS - APPLIED TO ROCKS THAT HAVE BEEN DERIVED FROM SAND OR THAT CONTAIN SAND. ARGILLACEOUS - APPLIED TO ALL ROCKS OR SUBSTANCES COMPOSED OF CLAY MINERALS, OR HAVING A NOTABLE PROPORTION OF CLAY IN THEIR COMPOSITION, AS SHALE, SLATE, ETC. ARTESIAN - GROUND WATER THAT IS UNDER SUFFICIENT PRESSURE TO RISE ABOVE THE LEVEL AT WHICH IT IS ENCOUNTERED, BUT WHICH DOES NOT NECESSARILY RISE TO OR ABOVE THE GROUND SURFACE. CALCAREOUS (CALC.) - SOILS WHICH CONTAIN APPRECIABLE AMOUNTS OF CALCIUM CARBONATE. COLLUVIUM - ROCK FRAGMENTS MIXED WITH SOIL DEPOSITED BY GRAVITY ON SLOPE OR AT BOTTOM OF SLOPE. CORE RECOVERY (REC.) - TOTAL LENGTH OF ALL MATERIAL RECOVERED IN THE CORE BARREL DIVIDED BY TOTAL LENGTH OF CORE RUN AND EXPRESSED AS A PERCENTAGE. DIKE - A TABULAR BODY OF IGNEOUS ROCK THAT CUTS ACROSS THE STRUCTURE OF ADJACENT ROCKS OR CUTS MASSIVE ROCK. DIP - THE ANGLE AT WHICH A STRATUM OR ANY PLANAR FEATURE IS INCLINED FROM THE HORIZONTAL. DIP DIRECTION (DIP AZIMUTH) - THE DIRECTION OR BEARING OF THE HORIZONTAL TRACE OF THE LINE OF DIP, MEASURED CLOCKWISE FROM NORTH. FAULT - A FRACTURE OR FRACTURE ZONE ALONG WHICH THERE HAS BEEN DISPLACEMENT OF THE SIDES RELATIVE TO ONE ANOTHER PARALLEL TO THE FRACTURE. FISSILE - A PROPERTY OF SPLITTING ALONG CLOSELY SPACED PARALLEL PLANES. FLOAT - ROCK FRAGMENTS ON SURFACE NEAR THEIR ORIGINAL POSITION AND DISLODGED FROM PARENT MATERIAL. FLOOD PLAIN (F.P.) - LAND BORDERING A STREAM, BUILT OF SEDIMENTS DEPOSITED BY THE STREAM. FORMATION (FM.) - A MAPPABLE GEOLOGIC UNIT THAT CAN BE RECOGNIZED AND TRACED IN THE FIELD. JOINT - FRACTURE IN ROCK ALONG WHICH NO APPRECIABLE MOVEMENT HAS OCCURRED. LEDGE - A SHELF-LIKE RIDGE OR PROJECTION OF ROCK WHOSE THICKNESS IS SMALL COMPARED TO ITS LATERAL EXTENT. LENS - A BODY OF SOIL OR ROCK THAT THINS OUT IN ONE OR MORE DIRECTIONS. MOTTLED (MOT.) - IRREGULARLY MARKED WITH SPOTS OF DIFFERENT COLORS. MOTTLING IN SOILS USUALLY INDICATES POOR AERATION AND LACK OF GOOD DRAINAGE. PERCHED WATER - WATER MAINTAINED ABOVE THE NORMAL GROUND WATER LEVEL BY THE PRESENCE OF AN INTERVENING IMPERVIOUS STRATUM. RESIDUAL SOIL - SOIL FORMED IN PLACE BY THE WEATHERING OF ROCK. ROCK QUALITY DESIGNATION (R.Q.D.) - A MEASURE OF ROCK QUALITY DESCRIBED BY: TOTAL LENGTH OF ROCK SEGMENTS EQUAL TO OR GREATER THAN 4 INCHES DIVIDED BY THE TOTAL LENGTH OF CORE RUN AND EXPRESSED AS A PERCENTAGE. SAPROLITE (SAP.) - RESIDUAL SOIL WHICH RETAINS THE RELIC STRUCTURE OR FABRIC OF THE PARENT ROCK. SILL - AN INTRUSIVE BODY OF IGNEOUS ROCK OF APPROXIMATELY UNIFORM THICKNESS AND RELATIVELY THIN COMPARED WITH ITS LATERAL EXTENT, WHICH HAS BEEN EMPLACED PARALLEL TO THE BEDDING OR SCHISTOSITY OF THE INTRUDED ROCKS. SLICKENSIDE - POLISHED AND STRIATED SURFACE THAT RESULTS FROM FRICTION ALONG A FAULT OR SLIP PLANE. STANDARD PENETRATION TEST (PENETRATION RESISTANCE) (SPT) - NUMBER OF BLOWS IN OR B.P.F.) OF A 140 LB. HAMMER FALLING 30 INCHES REQUIRED TO PRODUCE A PENETRATION OF 1 FOOT INTO SOIL WITH A 2 INCH OUTSIDE DIAMETER SPLIT SPOON SAMPLER. SPT REFUSAL IS LESS THAN 0.1 FOOT PENETRATION WITH 60 BLOWS. STRATA CORE RECOVERY (SREC.) - TOTAL LENGTH OF STRATA MATERIAL RECOVERED DIVIDED BY TOTAL LENGTH OF STRATUM AND EXPRESSED AS A PERCENTAGE. STRATA ROCK QUALITY DESIGNATION (S.R.Q.D.) - A MEASURE OF ROCK QUALITY DESCRIBED BY: TOTAL LENGTH OF ROCK SEGMENTS WITHIN A STRATUM EQUAL TO OR GREATER THAN 10 CENTIMETERS DIVIDED BY THE TOTAL LENGTH OF STRATA AND EXPRESSED AS A PERCENTAGE. TOPSOIL (T.S.) - SURFACE SOILS USUALLY CONTAINING ORGANIC MATTER.					
SOIL LEGEND AND AASHTO CLASSIFICATION		MINERALOGICAL COMPOSITION		WEATHERING							
GENERAL CLASS. GRANULAR MATERIALS (<35% PASSING #200) SILT-CLAY MATERIALS (>35% PASSING #200) ORGANIC MATERIALS		MINERAL NAMES SUCH AS QUARTZ, FELDSPAR, MICA, TALC, KAOLIN, ETC. ARE USED IN DESCRIPTIONS WHENEVER THEY ARE CONSIDERED OF SIGNIFICANCE.		FRESH ROCK FRESH, CRYSTALS BRIGHT, FEW JOINTS MAY SHOW SLIGHT STAINING. ROCK RINGS UNDER HAMMER IF CRYSTALLINE. VERY SLIGHT (V. SLI.) ROCK GENERALLY FRESH, JOINTS STAINED, SOME JOINTS MAY SHOW THIN CLAY COATINGS IF OPEN, CRYSTALS ON A BROKEN SPECIMEN FACE SHINE BRIGHTLY. ROCK RINGS UNDER HAMMER BLOWS IF OF A CRYSTALLINE NATURE. SLIGHT (SLI.) ROCK GENERALLY FRESH, JOINTS STAINED AND DISCOLORATION EXTENDS INTO ROCK UP TO 1 INCH. OPEN JOINTS MAY CONTAIN CLAY. IN GRANITOID ROCKS SOME OCCASIONAL FELDSPAR CRYSTALS ARE DULL AND DISCOLORED. CRYSTALLINE ROCKS RING UNDER HAMMER BLOWS. MODERATE (MOD.) SIGNIFICANT PORTIONS OF ROCK SHOW DISCOLORATION AND WEATHERING EFFECTS. IN GRANITOID ROCKS, MOST FELDSPARS ARE DULL AND DISCOLORED, SOME SHOW CLAY. ROCK HAS DULL SOUND UNDER HAMMER BLOWS AND SHOWS SIGNIFICANT LOSS OF STRENGTH AS COMPARED WITH FRESH ROCK. MODERATELY SEVERE (MOD. SEV.) ALL ROCK EXCEPT QUARTZ DISCOLORED OR STAINED. IN GRANITOID ROCKS, ALL FELDSPARS DULL AND DISCOLORED AND A MAJORITY SHOW KAOLINIZATION. ROCK SHOWS SEVERE LOSS OF STRENGTH AND CAN BE EXCAVATED WITH A GEOLOGIST'S PICK. ROCK GIVES "CLUNK" SOUND WHEN STRUCK. <i>IF TESTED, WOULD YIELD SPT REFUSAL</i> SEVERE (SEV.) ALL ROCKS EXCEPT QUARTZ DISCOLORED OR STAINED. ROCK FABRIC CLEAR AND EVIDENT BUT REDUCED IN STRENGTH TO STRONG SOIL. IN GRANITOID ROCKS ALL FELDSPARS ARE KAOLINIZED TO SOME EXTENT. SOME FRAGMENTS OF STRONG ROCK USUALLY REMAIN. <i>IF TESTED, YIELDS SPT N VALUES > 100 BPF</i> VERY SEVERE (V. SEV.) ALL ROCK EXCEPT QUARTZ DISCOLORED OR STAINED. ROCK FABRIC ELEMENTS ARE DISCERNIBLE BUT THE MASS IS EFFECTIVELY REDUCED TO SOIL STATUS, WITH ONLY FRAGMENTS OF STRONG ROCK REMAINING. SAPROLITE IS AN EXAMPLE OF ROCK WEATHERED TO A DEGREE SUCH THAT ONLY MINOR VESTIGES OF THE ORIGINAL ROCK FABRIC REMAIN. <i>IF TESTED, YIELDS SPT N VALUES < 100 BPF</i> COMPLETE ROCK REDUCED TO SOIL. ROCK FABRIC NOT DISCERNIBLE, OR DISCERNIBLE ONLY IN SMALL AND SCATTERED CONCENTRATIONS. QUARTZ MAY BE PRESENT AS DIKES OR STRINGERS. SAPROLITE IS ALSO AN EXAMPLE.		SLIGHTLY COMPRESSIBLE LIQUID LIMIT LESS THAN 30 MODERATELY COMPRESSIBLE LIQUID LIMIT 31-50 HIGHLY COMPRESSIBLE LIQUID LIMIT GREATER THAN 50		FRESH VERY SLIGHT (V. SLI.) SLIGHT (SLI.) MODERATE (MOD.) MODERATELY SEVERE (MOD. SEV.) SEVERE (SEV.) VERY SEVERE (V. SEV.) COMPLETE			
PERCENTAGE OF MATERIAL		GROUND WATER		ROCK HARDNESS							
ORGANIC MATERIAL GRANULAR SOILS SILT-CLAY SOILS OTHER MATERIAL TRACE OF ORGANIC MATTER 2 - 3% 3 - 5% TRACE 1 - 10% LITTLE ORGANIC MATTER 3 - 5% 5 - 12% LITTLE 10 - 20% MODERATELY ORGANIC 5 - 10% 12 - 20% SOME 20 - 35% HIGHLY ORGANIC >10% >20% HIGHLY 35% AND ABOVE		WATER LEVEL IN BORE HOLE IMMEDIATELY AFTER DRILLING. STATIC WATER LEVEL AFTER 24 HOURS. PERCHED WATER, SATURATED ZONE OR WATER BEARING STRATA SPRING OR SEEPAGE		VERY HARD CANNOT BE SCRATCHED BY KNIFE OR SHARP PICK. BREAKING OF HAND SPECIMENS REQUIRES SEVERAL HARD BLOWS OF THE GEOLOGIST'S PICK. HARD CAN BE SCRATCHED BY KNIFE OR PICK ONLY WITH DIFFICULTY. HARD HAMMER BLOWS REQUIRED TO DETACH HAND SPECIMEN. MODERATELY HARD CAN BE SCRATCHED BY KNIFE OR PICK. GOUGES OR GROOVES TO 0.25 INCHES DEEP CAN BE EXCAVATED BY HARD BLOW OF A GEOLOGIST'S PICK. HAND SPECIMENS CAN BE DETACHED BY MODERATE BLOWS. MEDIUM HARD CAN BE GROOVED OR GOUGED 0.05 INCHES DEEP BY FIRM PRESSURE OF KNIFE OR PICK POINT. CAN BE EXCAVATED IN SMALL CHIPS TO PIECES 1 INCH MAXIMUM SIZE BY HARD BLOWS OF THE POINT OF A GEOLOGIST'S PICK. SOFT CAN BE GROOVED OR GOUGED READILY BY KNIFE OR PICK. CAN BE EXCAVATED IN FRAGMENTS FROM CHIPS TO SEVERAL INCHES IN SIZE BY MODERATE BLOWS OF A PICK POINT. SMALL, THIN PIECES CAN BE BROKEN BY FINGER PRESSURE. VERY SOFT CAN BE CARVED WITH KNIFE. CAN BE EXCAVATED READILY WITH POINT OF PICK. PIECES 1 INCH OR MORE IN THICKNESS CAN BE BROKEN BY FINGER PRESSURE. CAN BE SCRATCHED READILY BY FINGER NAIL.							
CONSISTENCY OR DENSENESS		MISCELLANEOUS SYMBOLS		ABBREVIATIONS							
PRIMARY SOIL TYPE COMPACTNESS OR CONSISTENCY RANGE OF STANDARD PENETRATION RESISTANCE (N-VALUE) RANGE OF UNCONFINED COMPRESSIVE STRENGTH (TONS/FT ²)		ROADWAY EMBANKMENT WITH SOIL DESCRIPTION SOIL SYMBOL ARTIFICIAL FILL OTHER THAN ROADWAY EMBANKMENTS INFERRED SOIL BOUNDARIES INFERRED ROCK LINE ALLUVIAL SOIL BOUNDARY DIP/DIP DIRECTION OF ROCK STRUCTURES SOUNDING ROD		SPT TEST BORING AUGER BORING CORE BORING MONITORING WELL PIEZOMETER INSTALLATION SLOPE INDICATOR INSTALLATION SPT N-VALUE SPT REFUSAL		AR - AUGER REFUSAL BT - BORING TERMINATED CL - CLAY CPT - CONE PENETRATION TEST CSE - COARSE DMT - DILATOMETER TEST DPT - DYNAMIC PENETRATION TEST e - VOID RATIO F - FINE FOSS - FOSSILIFEROUS FRAC - FRACTURED FRAGS - FRAGMENTS MED - MEDIUM PMT - PRESSUREMETER TEST SD - SAND, SANDY SL - SILT, SILTY SLI - SLIGHTLY TCR - TRICONE REFUSAL u - UNIT WEIGHT u _d - DRY UNIT WEIGHT w - MOISTURE CONTENT v - VERY VST - VANE SHEAR TEST					
TEXTURE OR GRAIN SIZE		EQUIPMENT USED ON SUBJECT PROJECT		FRACTURE SPACING		BEDDING					
U.S. STD. SIEVE SIZE OPENING (MM) 4 10 40 60 200 270 4.76 2.0 0.42 0.25 0.075 0.053		DRILL UNITS: <input checked="" type="checkbox"/> CME-750 <input checked="" type="checkbox"/> BK-51 <input checked="" type="checkbox"/> CME-45C <input checked="" type="checkbox"/> CME-45B <input checked="" type="checkbox"/> CME-45 GOTRAK <input type="checkbox"/> OTHER _____ <input type="checkbox"/> OTHER _____		ADVANCING TOOLS: <input checked="" type="checkbox"/> CLAY BITS <input checked="" type="checkbox"/> 6" CONTINUOUS FLIGHT AUGER <input checked="" type="checkbox"/> 8" HOLLOW AUGERS <input type="checkbox"/> HARD FACED FINGER BITS <input type="checkbox"/> TUNG-CARBIDE INSERTS <input checked="" type="checkbox"/> CASING <input type="checkbox"/> w/ ADVANCER <input checked="" type="checkbox"/> TRICONE 2 1/16" STEEL TEETH <input type="checkbox"/> TRICONE _____ TUNG-CARB. <input type="checkbox"/> CORE BIT <input type="checkbox"/> OTHER _____		TERM SPACING MORE THAN 10 FEET 3 TO 10 FEET 1 TO 3 FEET 0.16 TO 1 FEET LESS THAN 0.16 FEET		TERM THICKNESS VERY THICKLY BEDDED > 4 FEET THICKLY BEDDED 1.5 - 4 FEET THINLY BEDDED 0.16 - 1.5 FEET VERY THINLY BEDDED 0.03 - 0.16 FEET THICKLY LAMINATED 0.008 - 0.03 FEET THINLY LAMINATED < 0.008 FEET			
SOIL MOISTURE - CORRELATION OF TERMS		HAMMER TYPE: <input checked="" type="checkbox"/> AUTOMATIC <input type="checkbox"/> MANUAL		CORE SIZE: <input type="checkbox"/> -B <input type="checkbox"/> -N <input type="checkbox"/> -H		HAND TOOLS: <input type="checkbox"/> POST HOLE DIGGER <input checked="" type="checkbox"/> HAND AUGER <input checked="" type="checkbox"/> SOUNDING ROD <input type="checkbox"/> VANE SHEAR TEST <input type="checkbox"/> OTHER _____					
SOIL MOISTURE SCALE (ATTERBERG LIMITS) FIELD MOISTURE DESCRIPTION GUIDE FOR FIELD MOISTURE DESCRIPTION		FOR SEDIMENTARY ROCKS, INDURATION IS THE HARDENING OF THE MATERIAL BY CEMENTING, HEAT, PRESSURE, ETC.		FRIABLE RUBBING WITH FINGER FREES NUMEROUS GRAINS; GENTLE BLOW BY HAMMER DISINTEGRATES SAMPLE. MODERATELY INDURATED GRAINS CAN BE SEPARATED FROM SAMPLE WITH STEEL PROBE; BREAKS EASILY WHEN HIT WITH HAMMER. INDURATED GRAINS ARE DIFFICULT TO SEPARATE WITH STEEL PROBE; DIFFICULT TO BREAK WITH HAMMER. EXTREMELY INDURATED SHARP HAMMER BLOWS REQUIRED TO BREAK SAMPLE; SAMPLE BREAKS ACROSS GRAINS.		BENCH MARK: _____ ELEVATION: _____ NOTES: _____  ORGANIC SOIL BOUNDARY					
PLASTICITY		INDURATION									
NONPLASTIC 0-5 VERY LOW LOW PLASTICITY 6-15 SLIGHT MED. PLASTICITY 16-25 MEDIUM HIGH PLASTICITY 26 OR MORE HIGH											
COLOR											
DESCRIPTIONS MAY INCLUDE COLOR OR COLOR COMBINATIONS (TAN, RED, YEL-BRN, BLUE-GRAY) MODIFIERS SUCH AS LIGHT, DARK, STREAKED, ETC. ARE USED TO DESCRIBE APPEARANCE.											

09/08/05

See Sheet 1-A For Index of Sheets
See Sheet 1-B For Standard Symbology Sheet

STATE OF NORTH CAROLINA
DIVISION OF HIGHWAYS

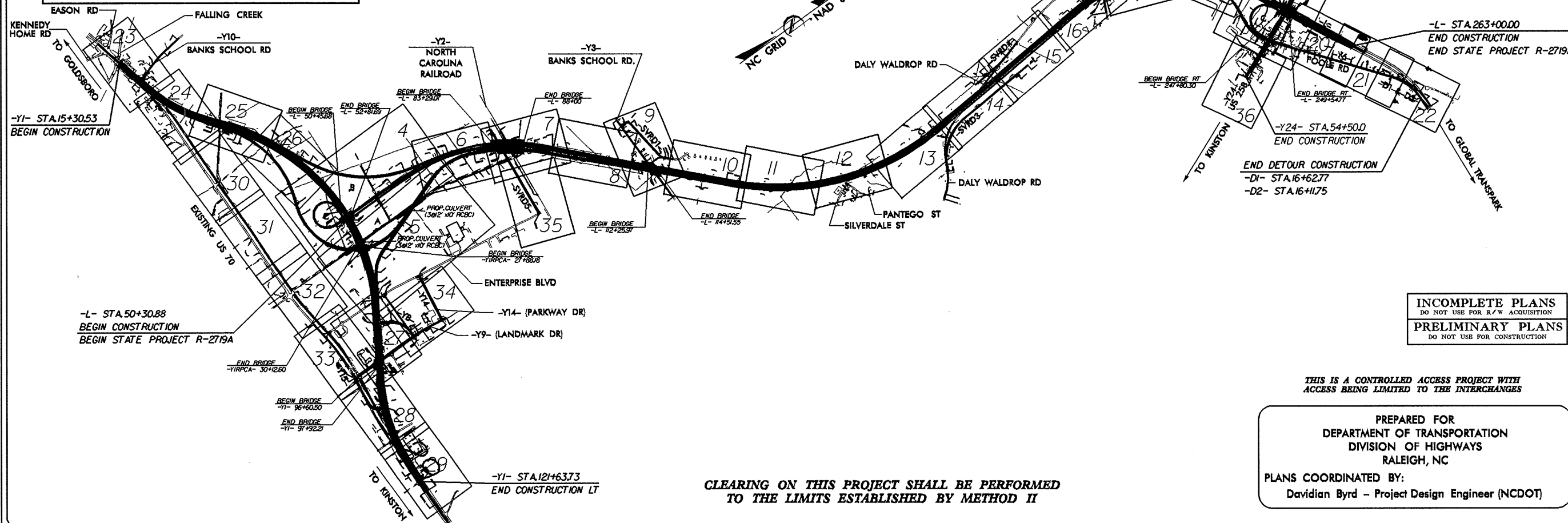
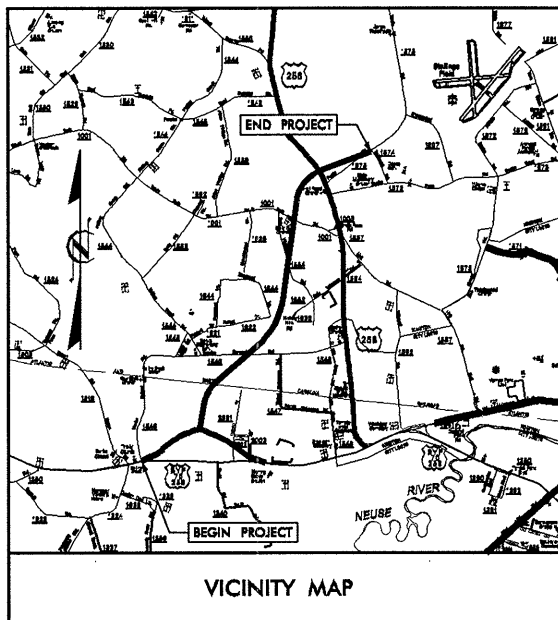
LENOIR COUNTY

LOCATION: CRESCENT ROAD IN KINSTON FROM
US 70 TO US 258

TYPE OF WORK: GRADING, PAVING, STRUCTURES, DRAINAGE

STATE	STATE PROJECT REFERENCE NO.	SHEET NO.	TOTAL SHEETS
N.C.	R-2719A	2A	139
STATE PROJ. NO.	F.A. PROJ. NO.	DESCRIPTION	
34501.1.1	STP-0224(3)	PE, ROW, UTIL.	

TIP PROJECT: R-2719A



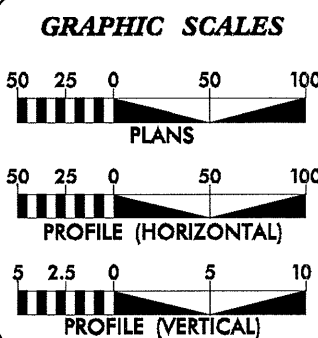
INCOMPLETE PLANS
DO NOT USE FOR R/W ACQUISITION
PRELIMINARY PLANS
DO NOT USE FOR CONSTRUCTION

THIS IS A CONTROLLED ACCESS PROJECT WITH ACCESS BEING LIMITED TO THE INTERCHANGES

PREPARED FOR
DEPARTMENT OF TRANSPORTATION
DIVISION OF HIGHWAYS
RALEIGH, NC
PLANS COORDINATED BY:
Davidian Byrd - Project Design Engineer (NCDOT)

CLEARING ON THIS PROJECT SHALL BE PERFORMED TO THE LIMITS ESTABLISHED BY METHOD II

CONTRACT:



DESIGN DATA

ADT 2007 =	26,510
ADT 2027 =	46,060
DHV =	10 %
D =	55 %
T =	14 % *
V =	70 MPH
* TTST	10% DUAL 4%

PROJECT LENGTH

LENGTH ROADWAY TIP PROJECT R-2719A =	3.772 MI
LENGTH STRUCTURE TIP PROJECT R-2719A =	0.256 MI
TOTAL LENGTH OF TIP PROJECT R-2719A =	4.028 MI

NOTE: SOUTHBOUND LANES WERE USED FOR PROJECT LENGTH CALCULATIONS

Prepared for the Division of Highways in the Office of:
URS CORPORATION - NORTH CAROLINA
1600 Perimeter Park Dr., Morrisville, North Carolina 27560
Telephone (919) 461-1100 Fax (919) 461-1415

2002 STANDARD SPECIFICATIONS

RIGHT OF WAY DATE: **TIMOTHY H. KEENER, P.E.**
AUGUST 19, 2005
PROJECT ENGINEER

LETTING DATE: **MICHAEL LINDGREN, P.E.**
AUGUST 21, 2007
PROJECT DESIGN ENGINEER

HYDRAULICS ENGINEER

SIGNATURE: _____ P.E.

ROADWAY DESIGN ENGINEER

SIGNATURE: _____ P.E.

DIVISION OF HIGHWAYS STATE OF NORTH CAROLINA

STATE DESIGN ENGINEER P.E.

DEPARTMENT OF TRANSPORTATION
FEDERAL HIGHWAY ADMINISTRATION

APPROVED DIVISION ADMINISTRATOR DATE

PFI PLANS
JUNE 14, 2005



STATE OF NORTH CAROLINA
DEPARTMENT OF TRANSPORTATION

MICHAEL F. EASLEY
GOVERNOR

LYNDO TIPPETT
SECRETARY

December 9, 2005

STATE PROJECT: 34501.1.1 R-2719A
F.A. PROJECT: STP-0224 (3)
COUNTY: Lenoir
DESCRIPTION: Crescent Rd in Kinston from US 70 to US 258

SUBJECT: Geotechnical Report - Inventory

Project Description

The project consists of constructing a four lane divided facility along a new location. The project begins just east of the intersection of SR 1519 (Eason Rd) and US 70 and extends approximately 3.8 miles northeast to the intersection of US 258 and ties into the existing Crescent Rd (R-2719BA). The geotechnical investigation of subsurface conditions was confined to the corridor of proposed new construction.

The following base lines were investigated for this project:

<u>Line</u>	<u>Station(±)</u>
-L-	50+30 to 263+00
-Y1-	15+30 to 121+63
-Y1RPA-	0+00 to 16+04
-Y1RPB-	0+00 to 35+55
-Y1RPCA-	0+00 to 68+73
-Y1LPC-	0+00 to 14+34
-Y6-	10+00 to 34+00
-Y8-	10+00 to 18+23
-Y9-	10+00 to 26+00
-Y10-	10+00 to 17+80
-Y14-	10+00 to 19+62

-Y15-	18+00 to 31+75
-SVRD1-	10+00 to 22+00
-SVRD2-	10+00 to 14+40
-SVRD3-	15+00 to 39+65
-SVRD4-	11+50 to 18+94
-SVRD5-	10+00 to 27+64
-Y24-	22+00 to 54+50
-Y24LPB-	0+00 to 11+63
-Y24RPB-	0+00 to 19+65
-Y24LPC-	0+00 to 12+38
-Y24RPC-	0+00 to 21+86
-D1-	10+00 to 20+09
-D2-	10+00 to 19+42

Areas of Special Geotechnical Interest

- 1) The following sections contain cohesive soils which have the potential to cause embankment stability and/or long term settlement problems:

<u>Line</u>	<u>Station (±)</u>
-L-	69+25 to 79+00
-L-	86+50 to 105+50
-L-	143+00 to 153+50
-L-	158+50 to 163+00
-L-	187+25 to 199+00
-L-	205+00 to 212+50
-SVRD1-	14+80 to 22+00
-SVRD5-	12+10 to 27+64
-Y1-	43+20 to 44+75
-Y1-	49+25 to 69+75
-Y1-	83+00 to 87+20
-Y1-	101+60 to 104+25
-Y1LPC-	7+00 to 10+75
-Y1RPB-	23+20 to 26+90
-Y1RPCA-	0+00 to 16+75
-Y1RPCA-	33+00 to 39+00
-Y1RPCA-	43+25 to 63+00
-Y9-	14+80 to 19+25
-Y24RPB-	9+00 to 17+00

- 2) The following sections contain organic soils which have the potential to cause embankment stability and/or long term settlement problems:

<u>Line</u>	<u>Station (±)</u>
-L-	128+70 to 133+25
-L-	135+80 to 140+90

- 3) The following intervals were found to exhibit a high water table, seasonal high ground water or the potential for ground water related construction problems:

<u>Line</u>	<u>Station(±)</u>
-L-	50+30 to 143+50
-L-	151+50 to 158+50
-L-	161+25 to 177+00
-L-	187+50 to 196+75
-L-	202+25 to 257+00
-SVRD2-	10+00 to 14+40
-SVRD5-	10+00 to 27+64
-Y1-	30+25 to 89+00
-Y1-	95+00 to 101+00
-Y1-	115+25 to 121+63
-Y1LPC-	0+00 to 14+34
-Y1RPA-	1+50 to 16+04
-Y1RPB-	0+00 to 35+55
-Y1RPCA-	0+00 to 24+00
-Y1RPCA-	47+00 to 66+73
-Y6-	10+00 to 23+00
-Y14-	10+00 to 19+50
-Y24-	22+00 to 35+00
-Y24-	41+00 to 54+50
-Y24LPB-	0+00 to 11+33
-Y24LPC-	0+00 to 12+38
-Y24RPB-	0+00 to 19+35
-Y24RPC-	0+00 to 21+86
-D1-	10+00 to 13+34

- 3) No water wells were noted within or in close proximity to the proposed right-of-way.
 4) Springs and seeps were encountered from -L- Sta. 104+00 to 110+00.

Physiography, Geology and Surface Water

The project corridor is located in the Middle Coastal Plain Physiographic Province just west of the Surry Scarp, a relict Plio-Pleistocene shoreline. The southern third of the project area is characterized by an extensive broad ancient terrace cut by the present day Neuse River and lies at an elevation of 50± feet. The northern two-thirds of the project is composed of a smaller less prominent marine terrace/ancient shoreline ranging in elevation from 50 to 90 feet and to the north, a gently rolling to flat upland with elevations of 90 to 115 feet.

The geology of the project consists of Pleistocene to Recent age fluvial and coastal plain sediments overlying marine sediments of the Upper Cretaceous age Peedee Formation. The distinction between the Pleistocene to Recent age fluvial and coastal plain sediments is difficult to determine due the amount of re-working, similar composition and high degree of weathering. Therefore, generally no distinction is made between the two on the accompanying soil profile sheets. The Peedee Formation was primarily encountered in deep borings along the project. The project is drained by Falling Creek, Gum Swamp Creek, Briar Run, Poole's Branch and several unnamed streams all within the Neuse River water shed.

Ground Water

Ground water data was collected from December 2004 to February 2005 during average rainfall conditions. Typically, ground water levels were measured at depths of 1.0± to 3.0± feet below the natural ground surface in the southern third of the project area and 2± to 4± feet or more below the natural ground surface areas of the northern two-thirds of the project. Artesian conditions with a flow rate of less than 1 gallon per minute were noted at -L- Station 92+00. The flow originates from an aquifer below elevation 35 feet and will not be penetrated by roadway construction activities. Therefore, this condition should not impact the roadway portion of this project.

Soils

Soils occurring along the project are derived from marine and fluvial sediments ranging in age from Late Cretaceous to Recent. Soils encountered during this investigation are divided into three basic categories: Roadway Embankment, Alluvial and Coastal Plain.

Roadway Embankment soils occur along existing roadways primarily at the beginning and end of the project. Typically these soils consist of loose to medium dense sand and clayey sand (A-2-4, A-2-6) that exhibit good to excellent engineering properties.

Recent alluvial soils were encountered along -L- from station 128+70± to 133+25± and consist of very soft muck with an organic content of 32± percent. Also, moderately organic loose and soft silty sand and sandy silt with an organic content of 6± percent were noted from -L- station 135+80 to 140+90. These soils are highly to moderately organic and exhibit unsuitable engineering characteristics.

Soils categorized as Coastal Plain occur at or near the surface along the entire project and therefore will have the most influence on highway construction. Generally these soils are composed of very loose to dense sand and clayey sand (A-2-4, A-2-6) and very soft to hard sandy silt, sandy clay and silty sandy clay (A-4, A-6, A-7). The granular soils generally possess good to excellent engineering properties. The cohesive soils typically exhibit fair engineering properties with moderate to moderately high moisture contents, low to moderate plasticity indices and generally less than 50 percent passing the No. 200 sieve. These cohesive soils still may have the potential to cause subgrade stability and/or embankment stability as well as settlement problems.

The Peedee Formation underlies the Coastal Plain soils and was encountered across the project in mostly deep borings. Deposits of the Peedee Formation are generally very loose to very dense sand and clayey sand (A-2-4, A-2-6) with some calcium cemented sand layers with interbeds of stiff to hard sandy silt and sandy clay (A-4, A-6, A-7). Due to the depths of occurrence and the fact that the Peedee Formation is primarily composed of sandy sediments it should not impact the roadway portion of this project.

Culvert at -L- Station 139+60

Based on the Culvert Survey and Hydraulic Design Report dated June 27, 2005, a dual 9' x 8' RCBC is proposed for -L- along a tributary to Falling Creek at station 139+60. A SPT boring performed near the proposed culvert site shows 2.5 feet of soft sandy moderately organic silt (A-4) underlain by medium dense sand (A-2-4, A-3). Ground water was measured at an elevation of 69 feet.

Culvert at -L- Station 153+62

Based on the Culvert Survey and Hydraulic Design Report dated June 27, 2005, a dual 8' x 8' RCBC is proposed for -L- along a tributary to Falling Creek at station 153+62. An auger boring performed near the proposed culvert site shows 15 feet or more of loose to medium dense silty sandy (A-2-4). Ground water was measured at an elevation of 76 feet.

Culvert at -Y1- Station 38+22

Based on the Culvert Survey and Hydraulic Design Report dated June 27, 2005, a dual 9' x 6' RCBC is proposed for -Y1- along a tributary to Falling Creek at station 38+22. An auger boring performed near the proposed culvert site shows 13 feet of loose to medium dense sand (A-2-4) underlain by medium dense clayey sand (A-2-6). Ground water was measured at an elevation of 46.5 feet.

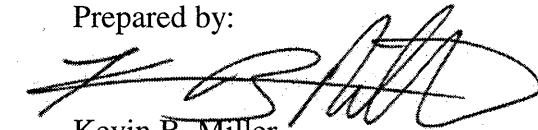
Culvert at -Y1- Station 74+43

Based on the Culvert Survey and Hydraulic Design Report dated June 27, 2005, a triple 12' x 10' RCBC is proposed for -Y1- along a tributary to Falling Creek at station 74+43. SPT and auger borings performed near the proposed culvert shows 25 feet or more of very loose to medium dense sand (A-2-4, A-3) is present at the site along with up to 3 feet of very soft to soft sandy silt in the existing channel. Ground water was measured at an elevation of 46 feet.

Culvert at -Y1RPCA- Station 16+14

Based on the Culvert Survey and Hydraulic Design Report dated June 27, 2005, a triple 12' x 10' RCBC is proposed for -Y1RPCA- along a tributary to Falling Creek at station 16+14. A SPT boring performed near the proposed culvert site shows 2 feet of loose to medium dense sand (A-2-4) underlain by 6 feet of soft sandy clay (A-6) underlain by 45 feet or more of medium dense to very dense sand (A-2-4, A-3, A-1-b). Ground water was measured at an elevation of 47 feet.

Prepared by:



Kevin B. Miller
Engineering Geologist II

EARTHWORK BALANCE SHEET (GRADING ONLY PROJECT)

Volumes in Cubic Yards

PROJECT R-2719A - Crescent Road

COUNTY Lenoir

DATE

6/3/2009

SHEET

1

OF

2

SHEETS

RD10501C

LINE	STATION TO	STATION	TOTAL EXCAV. (UNCL.)	ROCK EXCAV.	UNDERCUT EXCAV.	UNSUIT. EXCAV.	SUITABLE EXCAV.	TOTAL EMB.	ROCK EMB.	UNDERCUT EMB.	EARTH EMB.	EMBANK. +25%	BORROW	SUITABLE WASTE	UNSUIT. WASTE	TOTAL WASTE
SUMMARY POINT 1																
-L-	STA. 50+30.88	STA. 50+60.60 (BEGIN BRIDGE)	0	0	0	0	0	1087	0	0	1087	1359	1359	0	0	0
-L-	STA. 52+84.87 (END BRIDGE)	STA. 83+73.82 (BEGIN BRIDGE)	0	0	0	0	0	306672	0	0	306672	383340	383340	0	0	0
-Y1LPC-	STA. 0+50.00	STA. 11+68.74	51	0	257	0	51	96852	0	257	96595	121065	121014	0	257	257
-Y1RPCA-	STA. 4+49.79	STA. 27+88.18 (BEGIN BRIDGE)	0	0	0	0	0	219678	0	0	219678	274597	274597	0	0	0
-Y1RPCA-	STA. 30+12.60 (END BRIDGE)	STA. 60+83.00	4505	0	1427	1731	2774	76712	0	1427	75285	95890	93116	0	3158	3158
-Y1RPA-	STA. 4+75.00	STA. 11+98.58	0	0	0	0	0	40455	0	0	40455	50569	50569	0	0	0
-Y1RPB-	STA. 6+99.53	STA. 29+08.02	0	0	0	0	0	38844	0	0	38844	48555	48555	0	0	0
SUBTOTAL SUMMARY POINT 1			4556	0	1684	1731	2825	780300	0	1684	778616	975375	972550	0	3415	3415
SUMMARY POINT 2																
-L-	STA. 88+00.00 (END BRIDGE)	STA. 112+25.97 (BEGIN BRIDGE)	0	0	0	0	0	443821	0	0	443821	554777	554777	0	0	0
-SVRD1-	STA. 10+50.00	STA. 14+00.00	2285	0	0	0	2285	30	0	0	30	38	0	2247	0	2247
-SVRD5-	STA. 10+50.00	STA. 27+50.00	590	0	3535	0	590	4841	0	3535	4841	6051	5461	0	3535	3535
SUBTOTAL SUMMARY POINT 2			2875	0	3535	0	2875	448692	0	3535	448692	560866	560238	2247	3535	5782
SUMMARY POINT 3																
-L-	STA. 114+51.55 (END BRIDGE)	STA. 144+00.00	1064	0	13366	0	1064	183743	0	13366	170377	229679	228615	0	13366	13366
SUBTOTAL SUMMARY POINT 3			1064	0	13366	0	1064	183743	0	13366	170377	229679	228615	0	13366	13366
SUMMARY POINT 4																
-L-	STA. 144+00.00	STA. 174+00.00	56879	0	5623	6629	50250	45693	0	5623	40070	57117	6867	0	12252	12252
-SVRD3-	STA. 15+00.00	STA. 45+00.00	3469	0	0	0	3469	4430	0	0	4430	5538	2069	0	0	0
SUBTOTAL SUMMARY POINT 4			60348	0	5623	6629	53719	50123	0	5623	44500	62655	8936	0	12252	12252
SUMMARY POINT 5																
-L-	STA. 174+00.00	STA. 211+04.94 (BEGIN BRIDGE)	21806	0	3666	0	21806	172828	0	3666	169162	216036	194230	0	3666	3666
-SVRD4-	STA. 12+00.00	STA. 19+00.00	690	0	0	0	690	454	0	0	454	568	0	122	0	122
SUBTOTAL SUMMARY POINT 5			22496	0	3666	0	22496	173282	0	3666	169616	216604	194230	122	3666	3788
SUMMARY POINT 6																
-L-	STA. 212+72.83 (END BRIDGE)	STA. 247+80.30 (BEGIN BRIDGE)	4387	0	0	0	4387	302853	0	0	302853	378566	374179	0	0	0
-Y24RPB-	STA. 6+55.00	STA. 19+00.00	215	0	0	0	215	43888	0	0	43888	54860	54645	0	0	0
-Y24LPB-	STA. 3+15.00	STA. 6+70.00	0	0	0	0	0	22537	0	0	22537	28171	28171	0	0	0
-Y24RPC-	STA. 5+75.00	STA. 21+00.00	2671	0	0	0	2671	12409	0	0	12409	15511	12840	0	0	0
-Y24LPC-	STA. 3+00.00	STA. 7+75.00	0	0	0	0	0	15890	0	0	15890	19863	19863	0	0	0
SUBTOTAL SUMMARY POINT 6			7273	0	0	0	7273	397577	0	0	397577	496971	489698	0	0	0
SUMMARY POINT 7																
-L-	STA. 249+52.96 (END BRIDGE)	STA. 262+00.00	477	0	0	0	477	107874	0	0	107874	134843	134366	0	0	0
-Y24-	STA. 22+50.00	STA. 54+00.00	2389	0	0	0	2389	4836	0	0	4836	6045	3656	0	0	0
-Y6-	STA. 10+50.00	STA. 34+00.00	505	0	0	0	505	302	0	0	302	378	0	127	0	127
-D1- & -D2-	STA. 10+00	STA. 16+00.00	418	0	0	0	418	306	0	0	306	383	0	35	0	35
SUBTOTAL SUMMARY POINT 7			3789	0	0	0	3789	113318	0	0	113318	141649	138022	162	0	162
SUMMARY POINT 8																
-Y1-	STA. 30+00.00	STA. 45+00.00	3287	0	0	0	3287	40690	0	1693	38997	50862	47575	0	0	0
SUBTOTAL SUMMARY POINT 8			3287	0	0	0	3287	40690	0	1693	38997	50862	47575	0	0	0
SHEET TOTALS:			105688	0	27874	8360	97328	2187726	0	29567	2161694	2734661	2639864	2531	36234	38765

* EARTHWORK QUANTITIES ARE CALCULATED BY THE ROADWAY DESIGN UNIT. THESE EARTHWORK QUANTITIES ARE BASED IN PART ON SUBSURFACE DATA PROVIDED BY THE GEOTECHNICAL ENGINEERING UNIT.

** EMBANKMENT VOLUMES SHOWN INCLUDE ADDITIONAL VOLUME FOR 5" PLACED OVER SUBGRADE FOR GRADING & STRUCTURES ONLY PLANS.

EARTHWORK BALANCE SHEET (GRADING ONLY PROJECT)

Volumes in Cubic Yards

PROJECT R-2719A - Crescent Road

COUNTY Lenoir

DATE

6/3/2009

SHEET

2 OF 2

SHEETS

RD10S01C

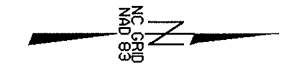
LINE	STATION TO	STATION	TOTAL EXCAV. (UNCL.)	ROCK EXCAV.	UNDERCUT	UNSUIT. EXCAV.	SUITABLE EXCAV.	TOTAL EMB.	ROCK EMB.	UNDERCUT EMB.	EARTH EMB.	EMBANK. +25%	BORROW	SUITABLE WASTE	UNSUIT. WASTE	TOTAL WASTE
SUMMARY POINT 9																
-Y1-	STA. 45+00.00	STA. 75+00.00	18	0	1693	0	18	75575	0	1693	73882	94469	94451	0	1693	1693
SUBTOTAL SUMMARY POINT 9			18	0	1693	0	18	75575	0	1693	73882	94469	94451	0	1693	1693
SUMMARY POINT 10																
-Y1-	STA. 75+00.00	STA. 96+60.50 (BEGIN BRIDGE)	0	0	0	0	0	153173	0	0	153173	191466	191466	0	0	0
SUBTOTAL SUMMARY POINT 10			0	0	0	0	0	153173	0	0	153173	191466	191466	0	0	0
SUMMARY POINT 11																
-Y1-	STA. 97+92.21 (END BRIDGE)	STA. 105+00.00	4	0	0	0	4	79055	0	0	79055	98819	98815	0	0	0
-Y8-	STA. 11+00.00	STA. 14+50.00	1681	0	0	0	1681	6477	0	0	6477	8096	6415	0	0	0
SUBTOTAL SUMMARY POINT 11			1685	0	0	0	1685	85532	0	0	85532	106915	105230	0	0	0
SHEET SUBTOTALS (SUMMARIES 9-11):			1703	0	1693	0	1703	314279	0	1693	312586	392850	391147	0	1693	1693
PREVIOUS SHEET SUBTOTALS (SUMMARIES 1-8):			105688	0	27874	8360	97328	2187726	0	29567	2161694	2734661	2639864	2531	36234	38765
SUMMARY POINT SUBTOTALS:			107391	0	29567	8360	99031	2502005	0	31260	2474280	3127511	3031011	2531	37927	40458
SHOULDER MATERIAL																
WASTE IN LIEU OF BORROW								0			0	0	0			
LOSS DUE TO CLEARING AND GRUBBING			-2200				-2200						-2531	-2531		-2531
ADDITIONAL UNDERCUT PER GEOTECHNICAL RECS					4500			4500		4500		5625	5625		4500	4500
PROJECT SUBTOTAL			105191		34067	8360	96831	2506505		35760	2474280	3133136	3036305	0	42427	42427
ESTIMATE 5% TO REPLACE TOPSOIL ON BORROW PIT													151815			
PROJECT TOTALS:			105191	0	34067								3188120	0	42427	42427
SAY:			105,200	-	34,100	-	-	-	-	-	-	-	3,188,150	-	42,500	42,500

* EARTHWORK QUANTITIES ARE CALCULATED BY THE ROADWAY DESIGN UNIT. THESE EARTHWORK QUANTITIES ARE BASED IN PART ON SUBSURFACE DATA PROVIDED BY THE GEOTECHNICAL ENGINEERING UNIT.

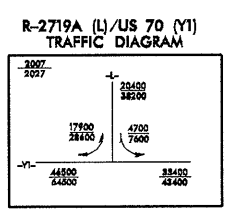
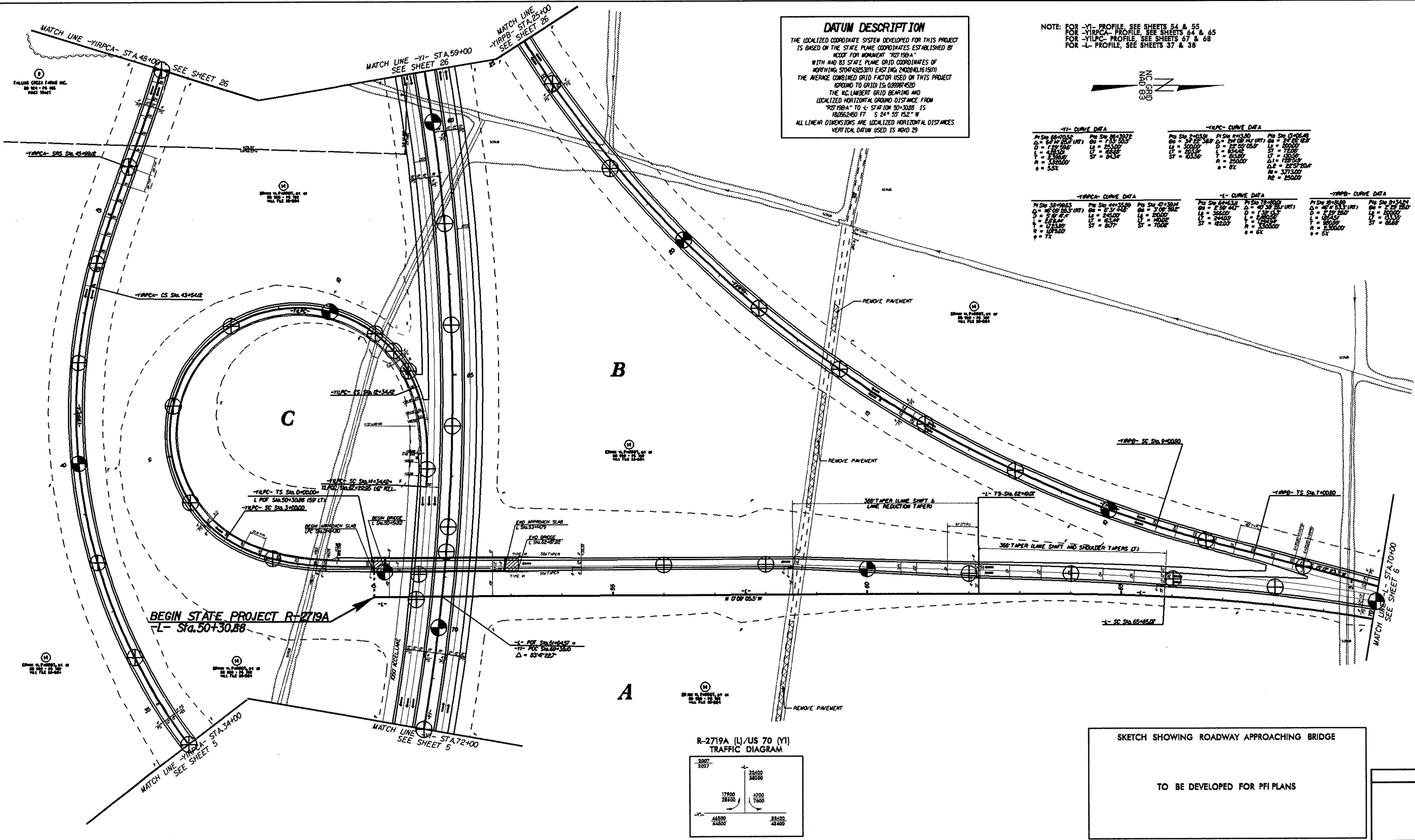
DRAINAGE DITCH EXCAVATION = 16,700 CY
 PAVEMENT STRUCTURE VOLUME = 16,931 CY
 FOR -L-, -Y24RPB-, -Y24RPC-, -Y10-, -SVRD1-, -SVRD3-, -SVRD5-

DATUM DESCRIPTION
 THE LOCALIZED COORDINATE SYSTEM DEVELOPED FOR THIS PROJECT IS BASED ON THE STATE PLANE COORDINATES ESTABLISHED BY "MCD" FOR MONUMENT "19219A"
 WITH 1983 STATE PLANE GRID COORDINATES OF MONUMENT "19219A" (EASTING 280261.15/570) THE AVERAGE COMBINED GRID FACTOR USED ON THIS PROJECT (GROUND TO GRID) IS 0.99994520
 THE N.C. LAMBERT GRID BEARING AND LOCALIZED HORIZONTAL GROUND DISTANCE FROM "19219A" TO "L" STA 10+30.88 IS 180624.00 FT S 84° 50' 15.2" W
 ALL LINEAR DIMENSIONS ARE LOCALIZED HORIZONTAL DISTANCES
 VERTICAL DATUM USED IS MVD 29

NOTE: FOR -YI- PROFILE, SEE SHEETS 54 & 55
 FOR -YIRPCA- PROFILE, SEE SHEETS 64 & 65
 FOR -YLPC- PROFILE, SEE SHEETS 67 & 68
 FOR -L- PROFILE, SEE SHEETS 37 & 38



-YI- CURVE DATA		-YIRPCA- CURVE DATA		-YLPC- CURVE DATA		-L- CURVE DATA	
PI Sta 66+70.52	PE Sta 66+30.72	PI Sta 66+05.00	PE Sta 66+05.00	PI Sta 66+05.00	PE Sta 66+05.00	PI Sta 66+15.00	PE Sta 66+15.00
Δ = 84° 48' 00" (RT)	Δ = 73° 24' 00" (RT)	Δ = 34° 22' 34" (RT)	Δ = 34° 22' 34" (RT)	Δ = 22° 50' 05" (RT)	Δ = 22° 50' 05" (RT)	Δ = 47° 58' 00" (RT)	Δ = 47° 58' 00" (RT)
L = 120.00	L = 65.00	L = 30.00	L = 30.00	L = 30.00	L = 30.00	L = 30.00	L = 30.00
R = 1320.00	R = 650.00	R = 1320.00	R = 1320.00	R = 1320.00	R = 1320.00	R = 1320.00	R = 1320.00
e = 5.5%	e = 5.5%	e = 5.5%	e = 5.5%	e = 5.5%	e = 5.5%	e = 5.5%	e = 5.5%



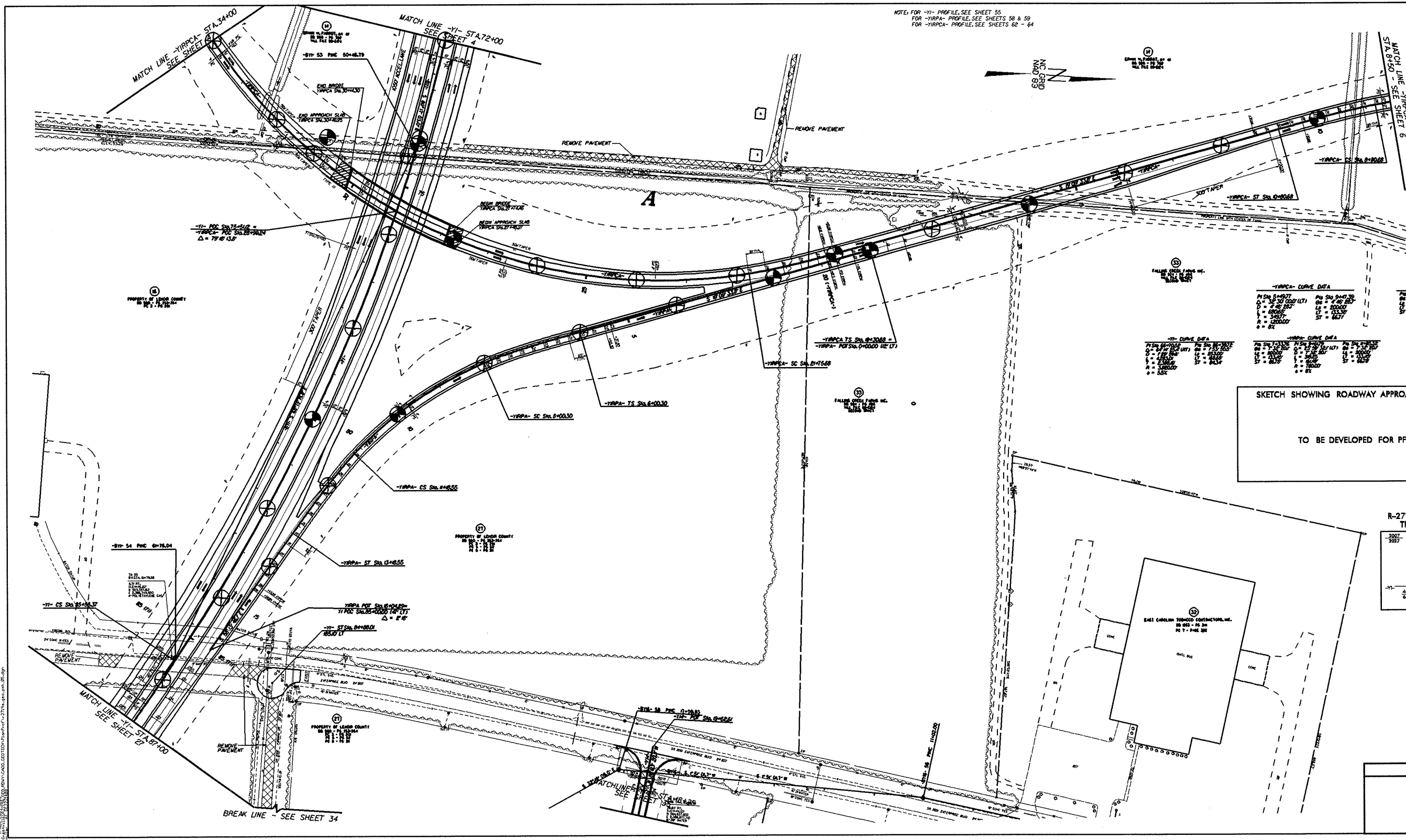
SKETCH SHOWING ROADWAY APPROACHING BRIDGE
 TO BE DEVELOPED FOR PFI PLANS

REVISIONS

THE INFORMATION CONTAINED HEREIN IS THE PROPERTY OF URS CORP. AND IS NOT TO BE REPRODUCED OR TRANSMITTED IN ANY FORM OR BY ANY MEANS, ELECTRONIC OR MECHANICAL, INCLUDING PHOTOCOPYING, RECORDING, OR BY ANY INFORMATION STORAGE AND RETRIEVAL SYSTEM.

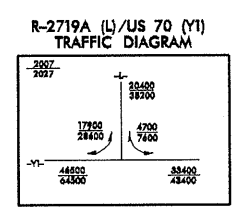
NOTE: FOR -YI- PROFILE, SEE SHEET 55
 FOR -YRPA- PROFILE, SEE SHEETS 58 & 59
 FOR -YIRPCA- PROFILE, SEE SHEETS 62 - 64

PROJECT NUMBER NO.	5770A	SHEET NO.	5
BY	W. J. H. H.	CHECKED BY	W. J. H. H.
DATE	11/11/83	SCALE	AS SHOWN
INCOMPLETE PLANS PRELIMINARY PLANS URS			
URS Corporation - North Carolina 1400 Federal Park Drive Huntersville, North Carolina 27840 TEL: (704) 865-1100 FAX: (704) 865-1110			



-YIRPCA- CURVE DATA		-YI- CURVE DATA		-YRPA- CURVE DATA	
PI Sta 8+89.77	PI Sta 9+47.35	PI Sta 7+31.12	PI Sta 7+31.12	PI Sta 7+31.12	PI Sta 7+31.12
Δ = 7° 02' 00" (LT)	Δ = 4° 42' 00" (RT)	Δ = 12° 00' 00" (LT)	Δ = 12° 00' 00" (LT)	Δ = 12° 00' 00" (LT)	Δ = 12° 00' 00" (LT)
L = 68.00'	L = 20.00'	L = 100.00'	L = 100.00'	L = 100.00'	L = 100.00'
R = 1200.00'	R = 1200.00'	R = 1200.00'	R = 1200.00'	R = 1200.00'	R = 1200.00'
e = 0.2'	e = 0.2'	e = 0.2'	e = 0.2'	e = 0.2'	e = 0.2'

SKETCH SHOWING ROADWAY APPROACHING BRIDGE
 TO BE DEVELOPED FOR PFI PLANS



11/11/83 11:30 AM W. J. H. H. 11/11/83 11:30 AM W. J. H. H. 11/11/83 11:30 AM W. J. H. H.

8/17/99

-L- CURVE DATA

PI Sta 78+80.01
Δ = 40° 36' 26.1" (RT)
D = 1° 38' 13.3"
L = 2,480.56'
T = 1,294.94'
R = 3,500.00'
e = 6%

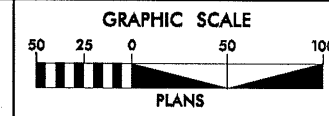
-YIRPB- CURVE DATA

PIs Sta 0+66.67
Θs = 1° 36' 55.2"
Ls = 200.00'
LT = 133.34'
ST = 66.67'

-YIRPCA- CURVE DATA

PIs Sta 1+16.20 PI Sta 5+49.77
Θs = 6° 26' 23.1" (LT) Δ = 32° 30' 00.0" (LT)
Ls = 200.00' D = 4° 46' 28.7"
ST = 84.00' L = 680.88'
LT = 116.20' T = 349.77'
Δ1 = 1° 39' 50.7" R = 1,200.00'
Δ2 = 4° 46' 32.3" e = 8%
R1 = 3,441.00'
R2 = 1,200.00'

NOTE: FOR -L- PROFILE, SEE SHEETS 38 & 39
FOR -YIRPB- PROFILE, SEE SHEET 60
FOR -YIRPCA- PROFILE, SEE SHEET 62

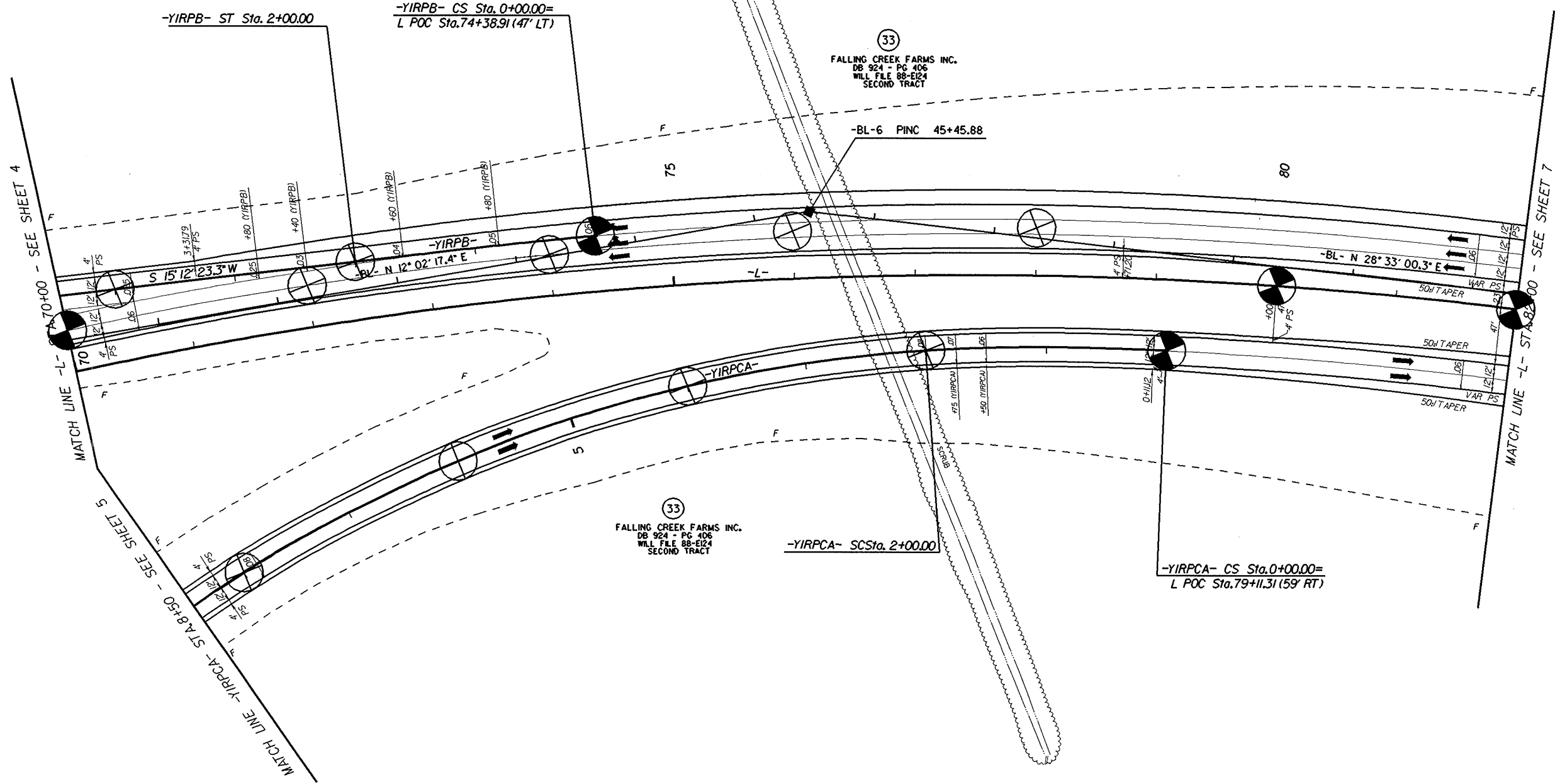


Prepared by
URS
URS Corporation - North Carolina
1600 Perimeter Park Drive
Merrillville, North Carolina 27560
TELEPHONE: (919) 481-1100 FAX: (919) 481-1410

PROJECT REFERENCE NO. R-2719A	SHEET NO. 6
RW SHEET NO. 6	HYDRAULICS ENGINEER
ROADWAY DESIGN ENGINEER	DO NOT USE FOR CONSTRUCTION
INCOMPLETE PLANS DO NOT USE FOR R/W ACQUISITION	
PRELIMINARY PLANS DO NOT USE FOR CONSTRUCTION	



REVISIONS



I:\NOV-2005_08\10
L:\ERO\Projects\Investigation\TIP\Greenville\R2719A_GEO_RDWY\CADD_GEO\TECH\Plan\Pr of R-2719a_geo_psh_06.dgn
8/17/99 AT GE1221488

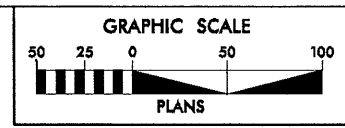
8/17/99

I:\NOV-2005 0914\user\station\TIP\Greenville\194.GEO_ROW\Y\CADD.GEOTECH\PlanProf\2719a_geo_psh_07.dgn
C:\Users\jgallagher\Documents\2719a.dwg

REVISIONS

-L- CURVE DATA

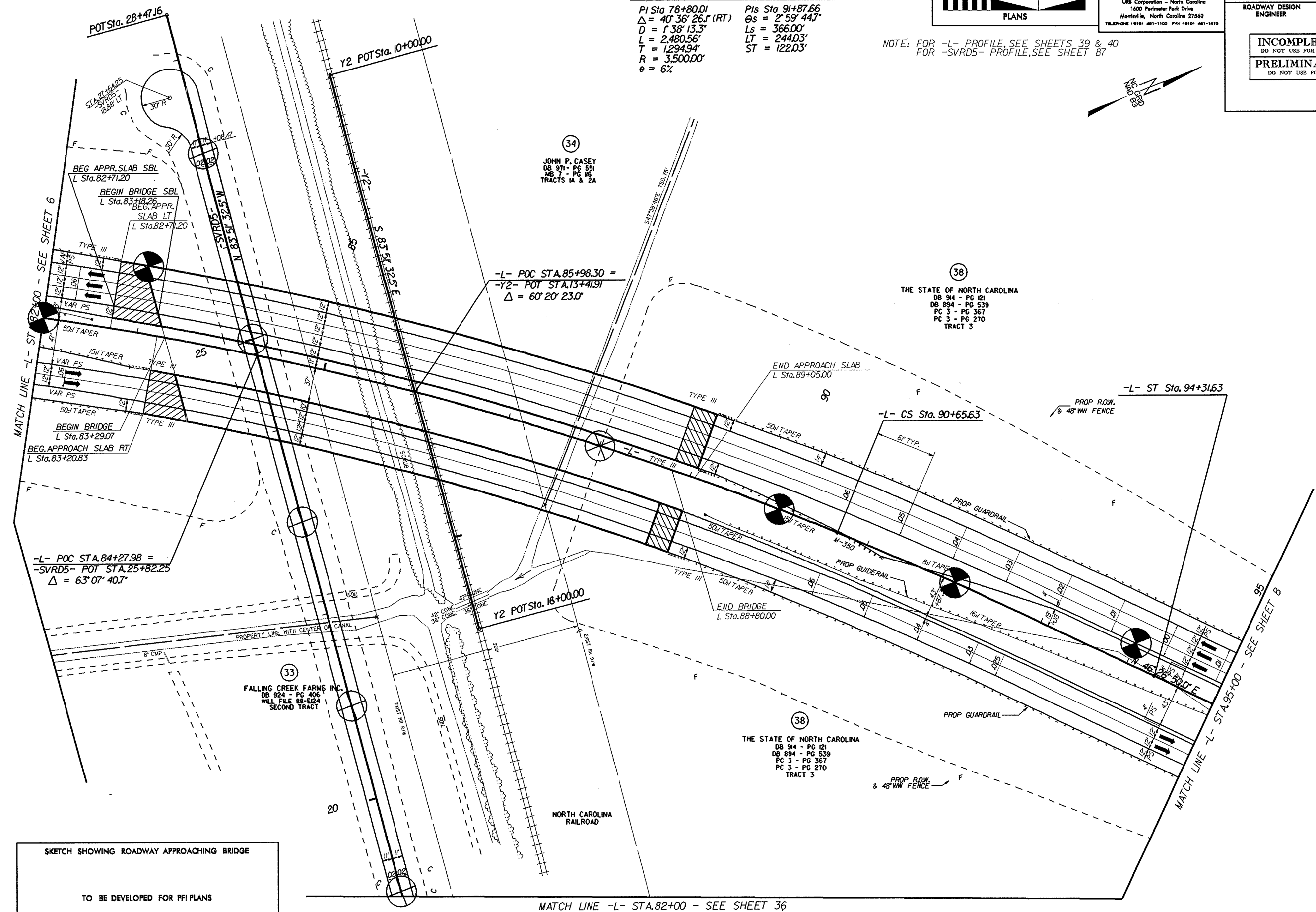
PI Sta 78+80.01	PIs Sta 91+87.66
$\Delta = 40^\circ 36' 26.1" (RT)$	$\Theta_s = 2^\circ 59' 44.7"$
$D = 1^\circ 38' 13.3"$	$L_s = 366.00'$
$L = 2,480.56'$	$LT = 244.03'$
$T = 1,294.94'$	$ST = 122.03'$
$R = 3,500.00'$	
$e = 6\%$	



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Harrisville, North Carolina 27840
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PROJECT REFERENCE NO. R-2719A	SHEET NO. 7
R/W SHEET NO. 7	
ROADWAY DESIGN ENGINEER	HYDRAULICS ENGINEER
INCOMPLETE PLANS DO NOT USE FOR R/W ACQUISITION	
PRELIMINARY PLANS DO NOT USE FOR CONSTRUCTION	

NOTE: FOR -L- PROFILE, SEE SHEETS 39 & 40
FOR -SVRD5- PROFILE, SEE SHEET 87



34
JOHN P. CASEY
DB 971 - PG 551
MB 7 - PG 86
TRACTS 1A & 2A

38
THE STATE OF NORTH CAROLINA
DB 94 - PG 121
DB 894 - PG 539
PC 3 - PG 367
PC 3 - PG 270
TRACT 3

33
FALLING CREEK FARMS INC.
DB 924 - PG 406
WILL FILE 88-E24
SECOND TRACT

38
THE STATE OF NORTH CAROLINA
DB 94 - PG 121
DB 894 - PG 539
PC 3 - PG 367
PC 3 - PG 270
TRACT 3

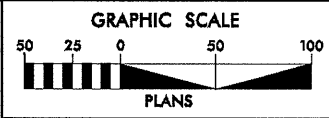
SKETCH SHOWING ROADWAY APPROACHING BRIDGE
TO BE DEVELOPED FOR PFI PLANS

MATCH LINE -L- STA. 82+00 - SEE SHEET 36

MATCH LINE -L- STA. 95+00 - SEE SHEET 8

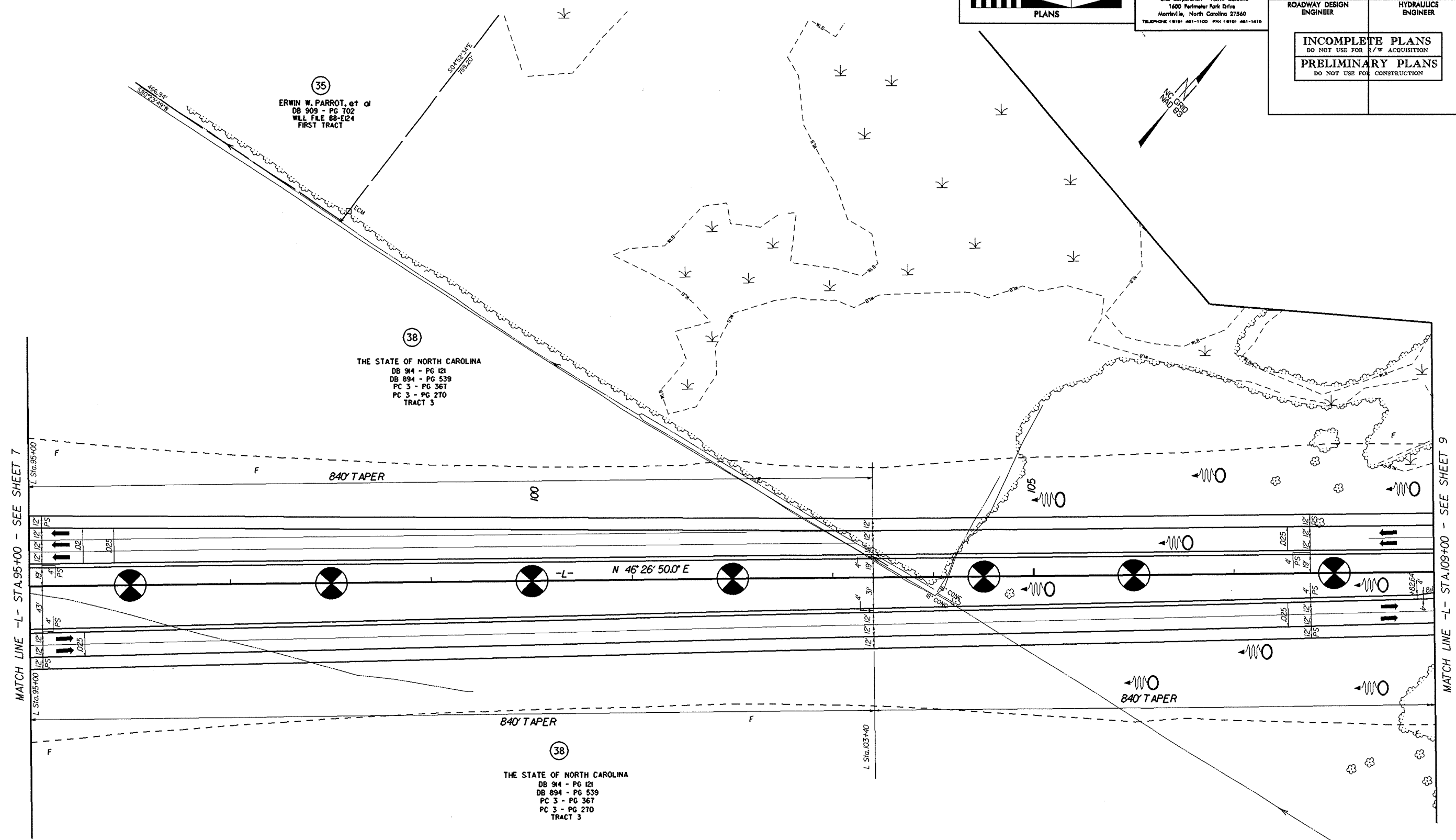
8/17/99

NOTE: FOR -L- PROFILE, SEE SHEETS 40 & 41



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PROJECT REFERENCE NO. R-2719A	SHEET NO. 8
R/W SHEET NO. 8	
ROADWAY DESIGN ENGINEER	HYDRAULICS ENGINEER
INCOMPLETE PLANS DO NOT USE FOR R/W ACQUISITION PRELIMINARY PLANS DO NOT USE FOR CONSTRUCTION	



REVISIONS

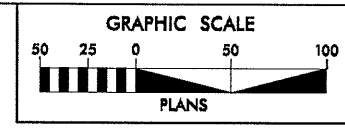
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 8/17/99 10:51:48 AM
 17/09/99 10:51:48 AM
 17/09/99 10:51:48 AM

MATCH LINE -L- STA. 95+00 - SEE SHEET 7

MATCH LINE -L- STA. 109+00 - SEE SHEET 9

8/17/99

NOTE: FOR -L- PROFILE, SEE SHEET 41
FOR -SVRDI- PROFILE, SEE SHEET 74



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Merrifield, North Carolina 27560
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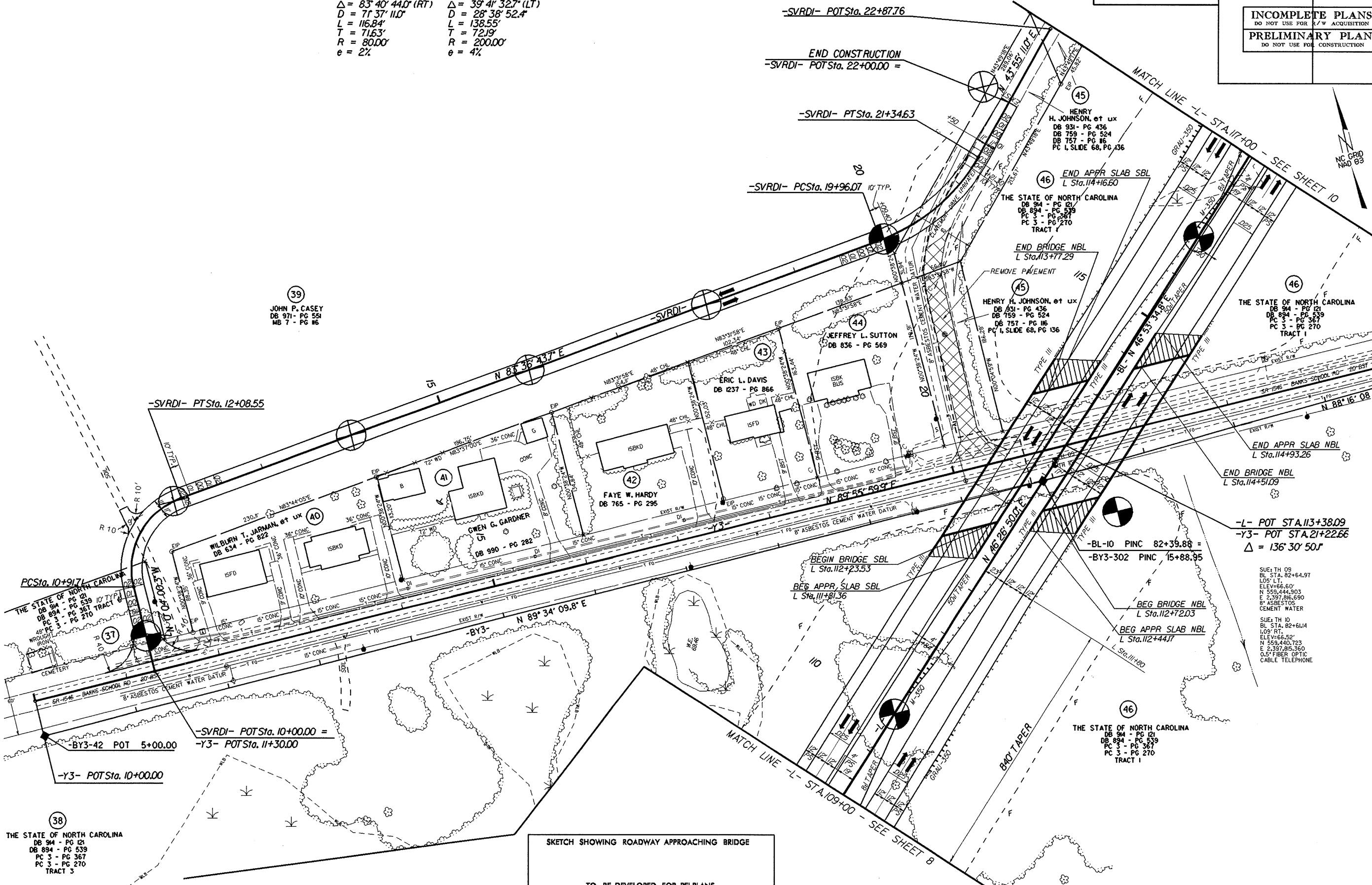
PROJECT REFERENCE NO.	SHEET NO.
R-2719A	9
R/W SHEET NO.	9
ROADWAY DESIGN ENGINEER	HYDRAULICS ENGINEER
INCOMPLETE PLANS DO NOT USE FOR A/W ACQUISITION PRELIMINARY PLANS DO NOT USE FOR CONSTRUCTION	

-SVRDI- DATA CURVE

PI Sta 11+63.34	PI Sta 20+68.26
$\Delta = 83^\circ 40' 44.0''$ (RT)	$\Delta = 39^\circ 41' 32.7''$ (LT)
D = 71' 37" 11.0"	D = 28' 38" 52.4"
L = 116.84'	L = 138.55'
T = 71.63'	T = 72.19'
R = 80.00'	R = 200.00'
e = 2%	e = 4%

REVISIONS

I:\7-NOV-2005 09:22 L:\VRO\Projects\TIP\Greenville\VR719A_GEO_CADD\RDWY\CADD_GEO\TECH\Plan\Prof\vr2719a_geo_psh_09.dgn



38 THE STATE OF NORTH CAROLINA
DB 94 - PG 121
DB 894 - PG 539
PC 3 - PG 367
PC 3 - PG 270
TRACT 3

NOTE: FUTURE BANKS SCHOOL ROAD PROPOSED 3-LANE WITH C&G
R-2719A PROPOSED IS MINIMUM RESURFACING ONLY WHERE NEEDED

SKETCH SHOWING ROADWAY APPROACHING BRIDGE
TO BE DEVELOPED FOR PFI PLANS

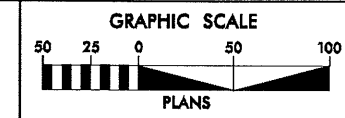
SUE TH 09
BL STA. 82+64.97
LO9' LT
ELEV=66.50'
N 559,444.903
E 2,397,816.590
8" ASBESTOS CEMENT WATER
CABLE TELEPHONE

8/17/99

NOTE: FOR -L- PROFILE, SEE SHEET 42

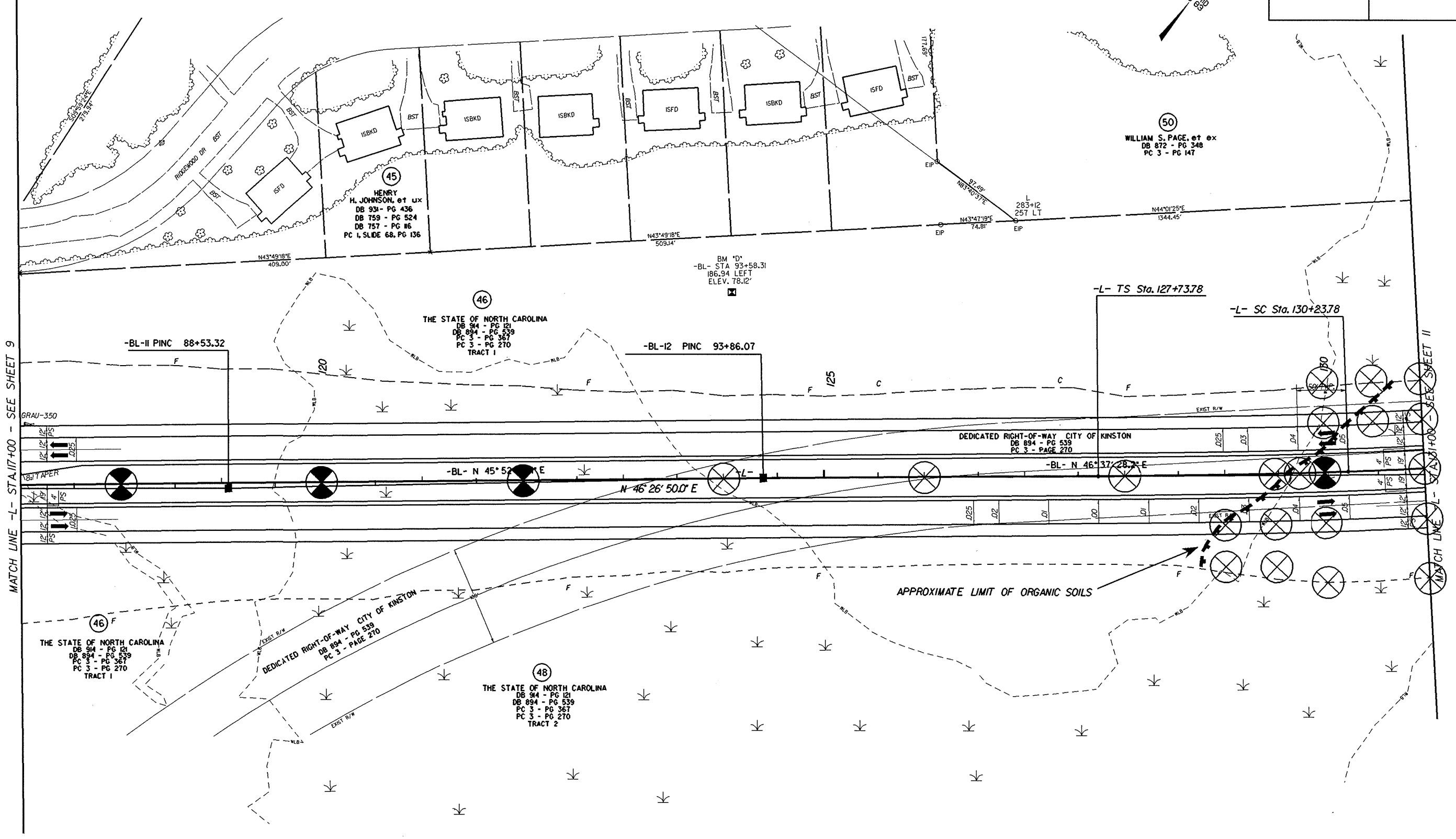
-L- CURVE DATA

PIs Sta 129+40.45	PI Sta 151+30.46
$\Theta_s = 130^\circ 28.0'$	$\Delta = 47^\circ 50' 08.7" (LT)$
$L_s = 250.00'$	$D = 112^\circ 22.4'$
$LT = 166.67'$	$L = 3,965.73'$
$ST = 83.34'$	$T = 2106.68'$
	$R = 4750.00'$
	$e = 5\%$



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 URS Corporation - North Carolina
 1600 Parkcenter Park Drive
 Morrisville, North Carolina 27560
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PROJECT REFERENCE NO. R-2719A	SHEET NO. 10
RW SHEET NO. 10	HYDRAULICS ENGINEER
ROADWAY DESIGN ENGINEER	HYDRAULICS ENGINEER
INCOMPLETE PLANS DO NOT USE FOR R/W ACQUISITION PRELIMINARY PLANS DO NOT USE FOR CONSTRUCTION	



REVISIONS

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 05:15:30 8/17/99
 J. Miller

MATCH LINE -L- STA. 117+00 - SEE SHEET 9

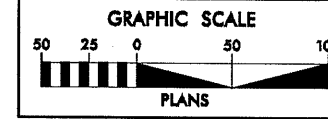
MATCH LINE -L- STA. 131+00 - SEE SHEET 11

8/17/99

-L- CURVE DATA

PI Sta 151+30.46
 $\Delta = 47^{\circ} 50' 08.7"$ (LT)
D = 112' 22.4"
L = 3,965.73'
T = 2106.68'
R = 4750.00'
e = 5%

NOTE: FOR -L- PROFILE, SEE SHEET 43



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1600 Parkmaker Park Drive
Merrillville, North Carolina 27560
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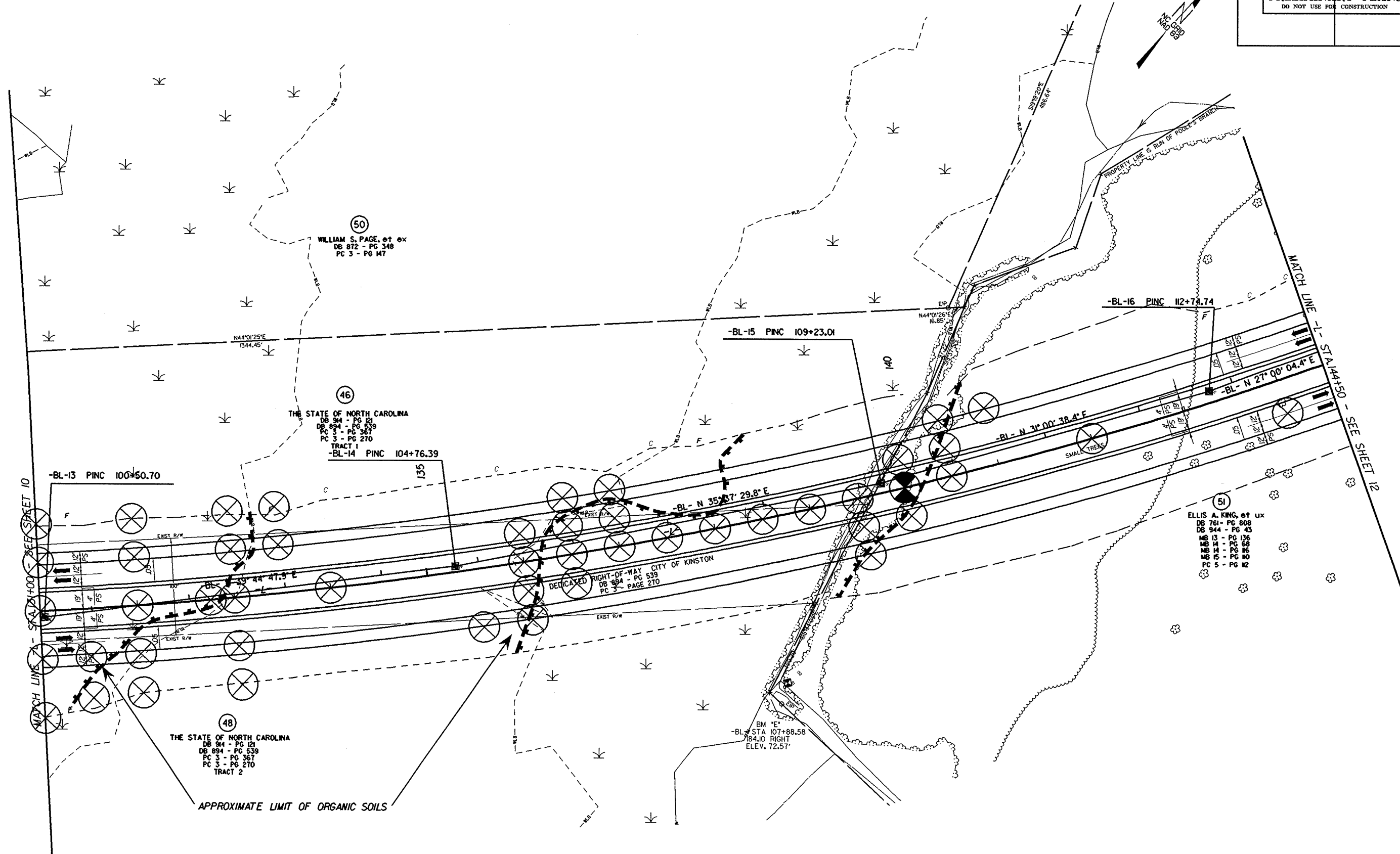
PROJECT REFERENCE NO.	SHEET NO.
R-2719A	11
R/W SHEET NO.	11
ROADWAY DESIGN ENGINEER	HYDRAULICS ENGINEER

INCOMPLETE PLANS
DO NOT USE FOR R/W ACQUISITION

PRELIMINARY PLANS
DO NOT USE FOR CONSTRUCTION

REVISIONS

D:\P2C\2005\1515... \geotech\PlanPror\2719a_geo.psh.ll.dgn
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11/17/99 10:48 AM



(50) WILLIAM S. PAGE, et ux
DB 872 - PG 348
PC 3 - PG 147

(46) THE STATE OF NORTH CAROLINA
DB 894 - PG 121
DB 894 - PG 539
PC 3 - PG 367
PC 3 - PG 270
TRACT 1
-BL-14 PINC 104+76.39

(51) ELLIS A. KING, et ux
DB 761 - PG 808
DB 944 - PG 43
MB 13 - PG 136
MB 14 - PG 66
MB 15 - PG 80
PC 5 - PG 82

(48) THE STATE OF NORTH CAROLINA
DB 94 - PG 121
DB 894 - PG 539
PC 3 - PG 367
PC 3 - PG 270
TRACT 2

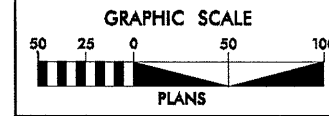
DELICATED RIGHT-OF-WAY CITY OF KINSTON
DB 884 - PG 539
PC 3 - PAGE 270

BM 'E'
-BL STA 107+88.58
184.10 RIGHT
ELEV. 72.51'

APPROXIMATE LIMIT OF ORGANIC SOILS

8/17/99

NOTE: FOR -L- PROFILE, SEE SHEETS 43 & 44

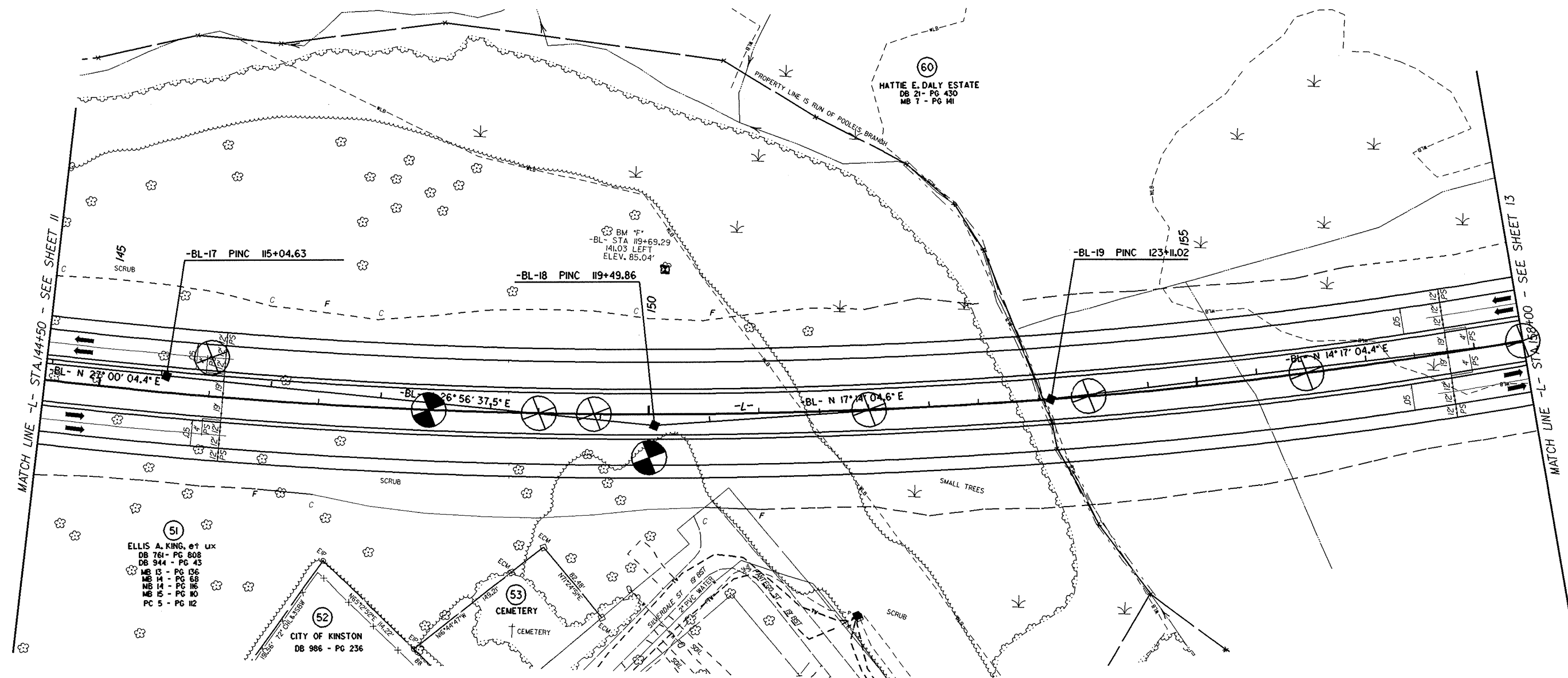


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Merrillville, North Carolina 27560
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PROJECT REFERENCE NO. R-2719A	SHEET NO. 12
RAW SHEET NO. 12	
ROADWAY DESIGN ENGINEER	HYDRAULICS ENGINEER
INCOMPLETE PLANS DO NOT USE FOR R/W ACQUISITION PRELIMINARY PLANS DO NOT USE FOR CONSTRUCTION	



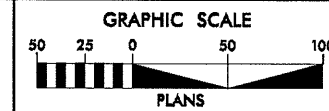
REVISIONS



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 8/17/99

8/17/99

NOTE: FOR -L- PROFILE, SEE SHEETS 44 & 45
FOR -SVRD3- PROFILE, SEE SHEET 76

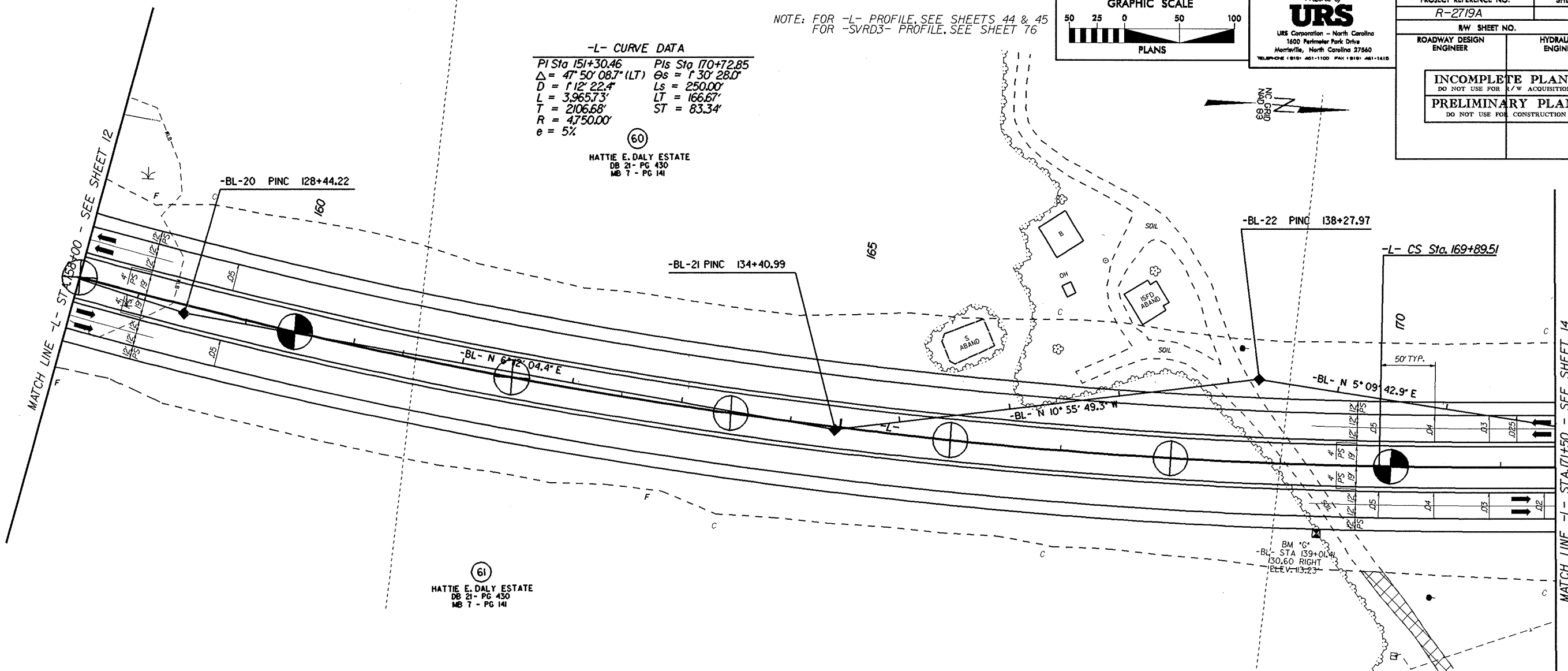


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Harrisville, North Carolina 27540
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PROJECT REFERENCE NO. R-2719A	SHEET NO. 13
RW SHEET NO. 13	HYDRAULICS ENGINEER
ROADWAY DESIGN ENGINEER	CONSTRUCTION
INCOMPLETE PLANS DO NOT USE FOR L/W ACQUISITION PRELIMINARY PLANS DO NOT USE FOR CONSTRUCTION	

-L- CURVE DATA
 PI Sta 151+30.46 Pis Sta 170+72.85
 $\Delta = 47^\circ 50' 08.7''$ (LT) $\Theta_s = 1^\circ 30' 28.0''$
 $D = 1^\circ 12' 22.4''$ $L_s = 250.00'$
 $L = 3,965.73'$ $LT = 166.67'$
 $T = 2,106.68'$ $ST = 83.34'$
 $R = 4,750.00'$
 $e = 5\%$

(60)
 HATTIE E. DALY ESTATE
 DB 21 - PG 430
 MB 7 - PG 141

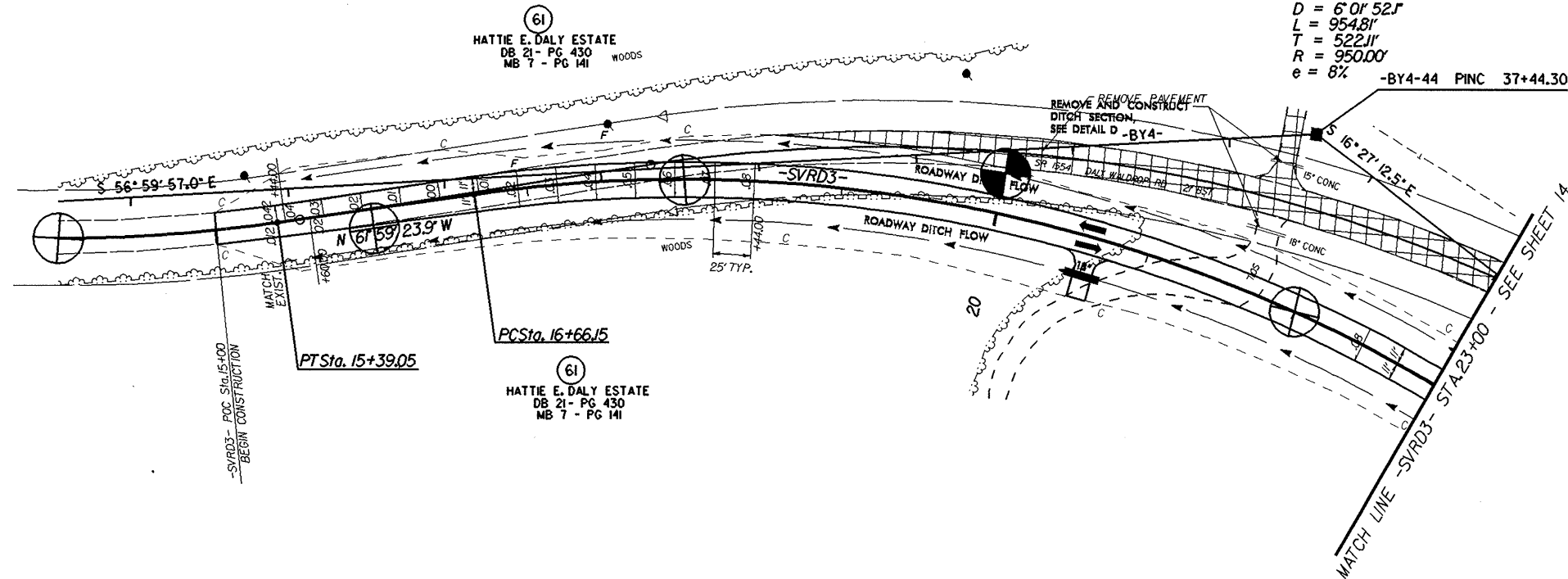


(61)
 HATTIE E. DALY ESTATE
 DB 21 - PG 430
 MB 7 - PG 141

(61)
 HATTIE E. DALY ESTATE
 DB 21 - PG 430
 MB 7 - PG 141

-SVRD3- CURVE DATA
 PI Sta 21+88.27
 $\Delta = 57^\circ 35' 09.2''$ (RT)
 $D = 6^\circ 01' 52.7''$
 $L = 954.81'$
 $T = 522.11'$
 $R = 950.00'$
 $e = 8\%$

-BY4-44 PINC 37+44.30



(61)
 HATTIE E. DALY ESTATE
 DB 21 - PG 430
 MB 7 - PG 141

REVISIONS

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 L:\VRO\Releigh\Miller AT GE\1221408

MATCH LINE -L- STA 158+00 - SEE SHEET 12

MATCH LINE -L- STA 171+50 - SEE SHEET 14

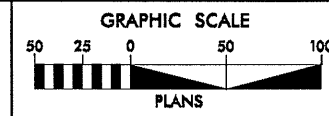
MATCH LINE -SVRD3- STA 23+00 - SEE SHEET 14

8/17/99

-L- CURVE DATA
 PIs Sta 170+72.85
 $\theta_s = 1^\circ 30' 28.0"$
 $L_s = 250.00'$
 $LT = 166.67'$
 $ST = 83.34'$

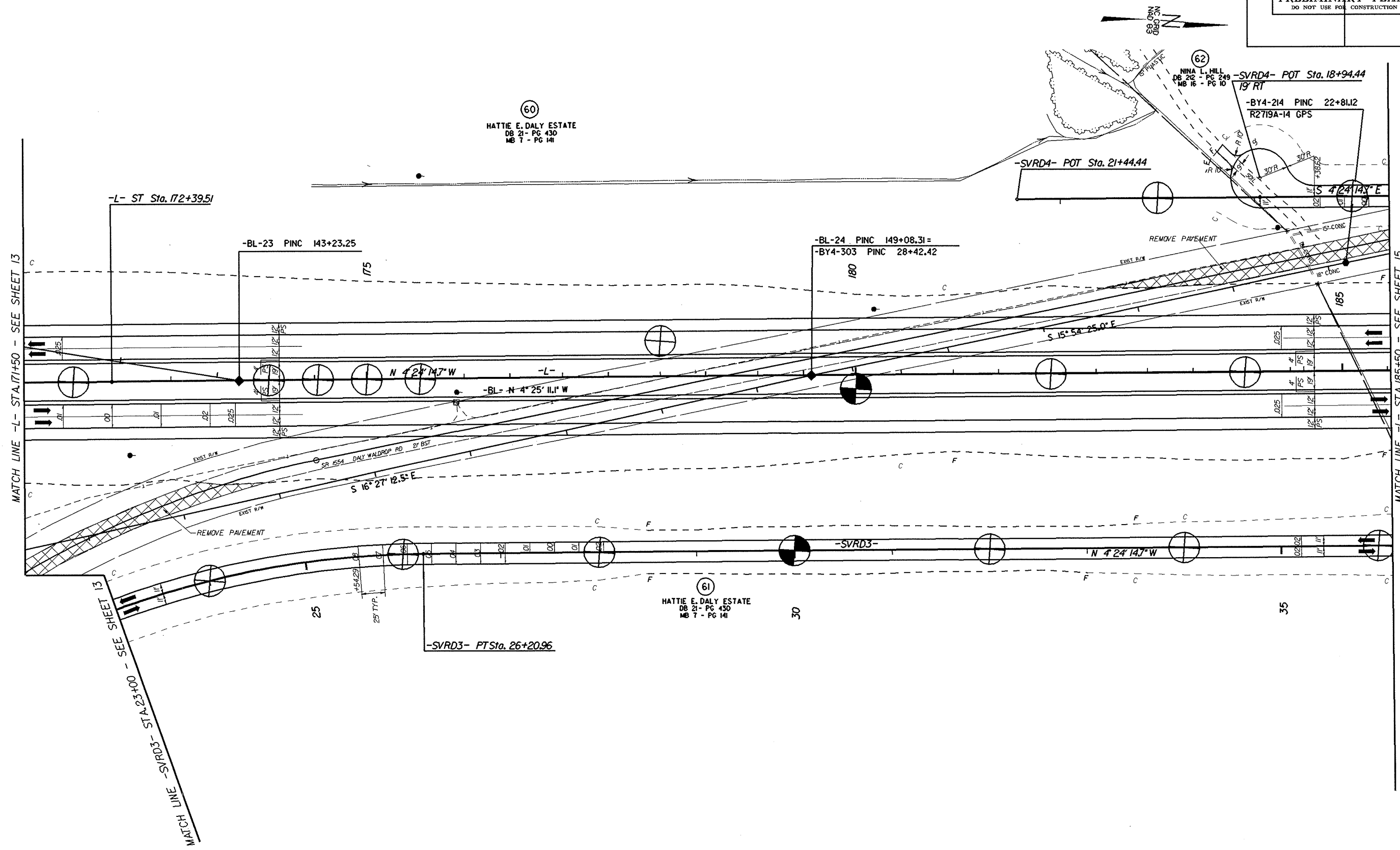
-SVRD3- CURVE DATA
 PI Sta 21+88.27
 $\Delta = 57^\circ 35' 09.2" (RT)$
 $D = 6^\circ 01' 52.7"$
 $L = 954.81'$
 $T = 522.11'$
 $R = 950.00'$
 $e = 8\%$

NOTE: FOR -L- PROFILE, SEE SHEETS 45 & 46
 FOR -SVRD3- PROFILE, SEE SHEET 76
 FOR -SVRD4- PROFILE, SEE SHEET 78



Prepared by
URS
 URS Corporation - North Carolina
 1600 Perimeter Park Drive
 Morrisville, North Carolina 27560
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PROJECT REFERENCE NO. R-2719A	SHEET NO. 14
R/W SHEET NO. 14	
ROADWAY DESIGN ENGINEER	HYDRAULICS ENGINEER
INCOMPLETE PLANS DO NOT USE FOR R/W ACQUISITION PRELIMINARY PLANS DO NOT USE FOR CONSTRUCTION	



REVISIONS

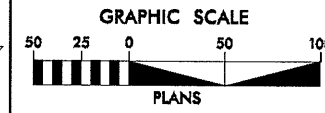
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 8/17/99
 A:\CERO\Projects\2719A_GEO_RDWY\CADD\GEO\TECH\Plan\Prof\2719a-geo_psh_14.dgn

8/17/99

-SVRD4- CURVE DATA

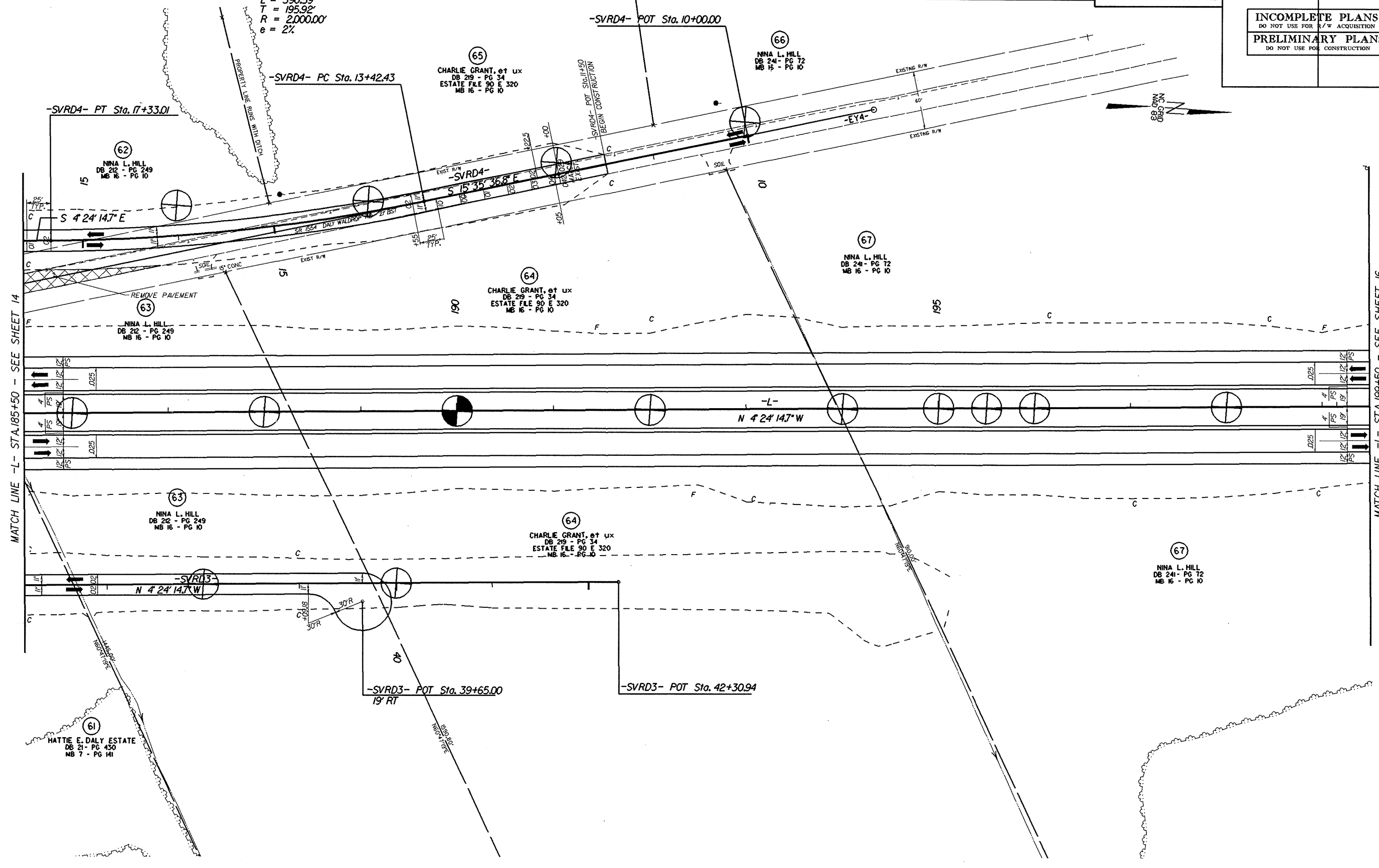
PI Sta 15+38.34
Δ = 11° 22.0' (RT)
D = 2' 5" 53.2"
L = 390.59'
T = 195.92'
R = 2,000.00'
e = 2%

NOTE: FOR -L- PROFILE, SEE SHEETS 46 & 47
FOR -SVRD3- PROFILE, SEE SHEETS 76 & 77
FOR -SVRD4- PROFILE, SEE SHEET 78



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Morrisville, North Carolina 27560
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PROJECT REFERENCE NO.	SHEET NO.
R-2719A	15
R/W SHEET NO.	15
ROADWAY DESIGN ENGINEER	HYDRAULICS ENGINEER
INCOMPLETE PLANS DO NOT USE FOR R/W ACQUISITION PRELIMINARY PLANS DO NOT USE FOR CONSTRUCTION	



REVISIONS

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8/17/99
cmiller AT GEJ221408

MATCH LINE -L- STA. 185+50 - SEE SHEET 14

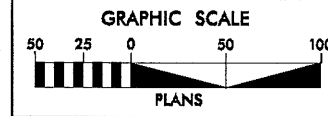
MATCH LINE -L- STA. 199+50 - SEE SHEET 16

8/17/99

-L- CURVE DATA

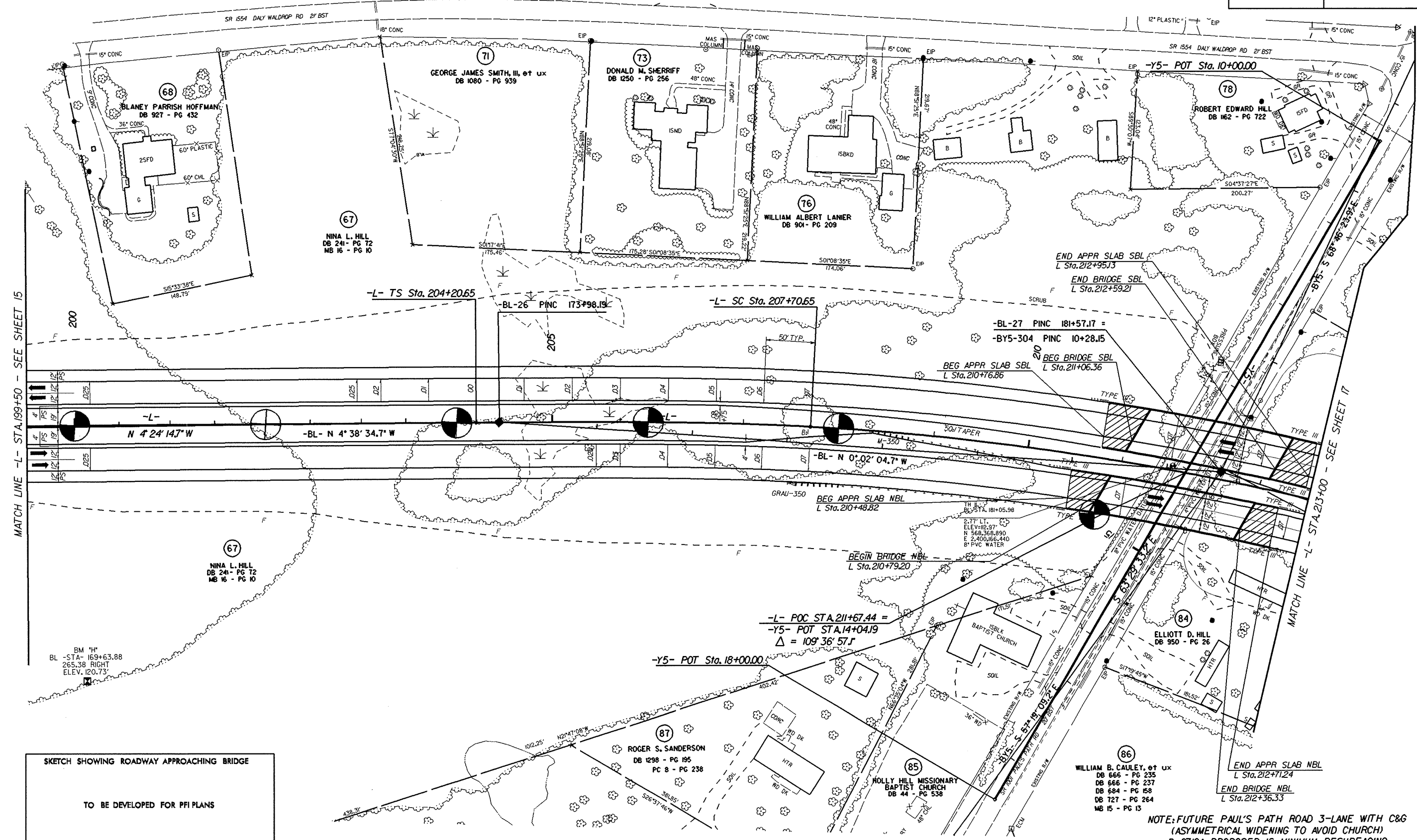
Pls Sta 206+54.03	PI Sta 224+97.17
Es = 3' 27" 27.0"	Δ = 61' 32" 05.9" (RT)
Ls = 350.00'	D = 1' 58" 32.6"
LT = 233.38'	L = 3,114.56'
ST = 116.71'	T = 1726.52'
	R = 2,900.00'
	e = 7%

NOTE: FOR -L- PROFILE, SEE SHEETS 47 & 48



Prepared by
URS
 URS Corporation - North Carolina
 1600 Parkcenter Park Drive
 Morrisville, North Carolina 27560
 TELEPHONE: (919) 461-1100 FAX: (919) 461-1415

PROJECT REFERENCE NO. R-2719A	SHEET NO. 16
R/W SHEET NO. 16	
ROADWAY DESIGN ENGINEER	HYDRAULICS ENGINEER
INCOMPLETE PLANS DO NOT USE FOR R/W ACQUISITION PRELIMINARY PLANS DO NOT USE FOR CONSTRUCTION	



MATCH LINE -L- STA.199+50 - SEE SHEET 15

MATCH LINE -L- STA.213+00 - SEE SHEET 17

SKETCH SHOWING ROADWAY APPROACHING BRIDGE

 TO BE DEVELOPED FOR PFI PLANS

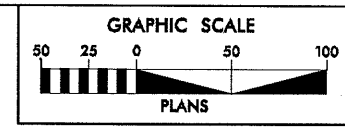
NOTE: FUTURE PAUL'S PATH ROAD 3-LANE WITH C&G
 (ASYMMETRICAL WIDENING TO AVOID CHURCH)
 R-2719A PROPOSED IS MINIMUM RESURFACING
 ONLY WHERE NEEDED

REVISIONS

I:\WORK\2005\1522\LENO\PROJ\investigation\TIP\Greenville\RDWY\CADD_GEO\GEO\TECH\PlanProf\2719a_geo_psh_16.dgn
 8/17/99 1:22:18 PM
 1522

8/17/99

NOTE: FOR -L- PROFILE, SEE SHEETS 48 & 49



Prepared by
URS
URS Corporation - North Carolina
1600 Piedmont Park Drive
Mortville, North Carolina 27540
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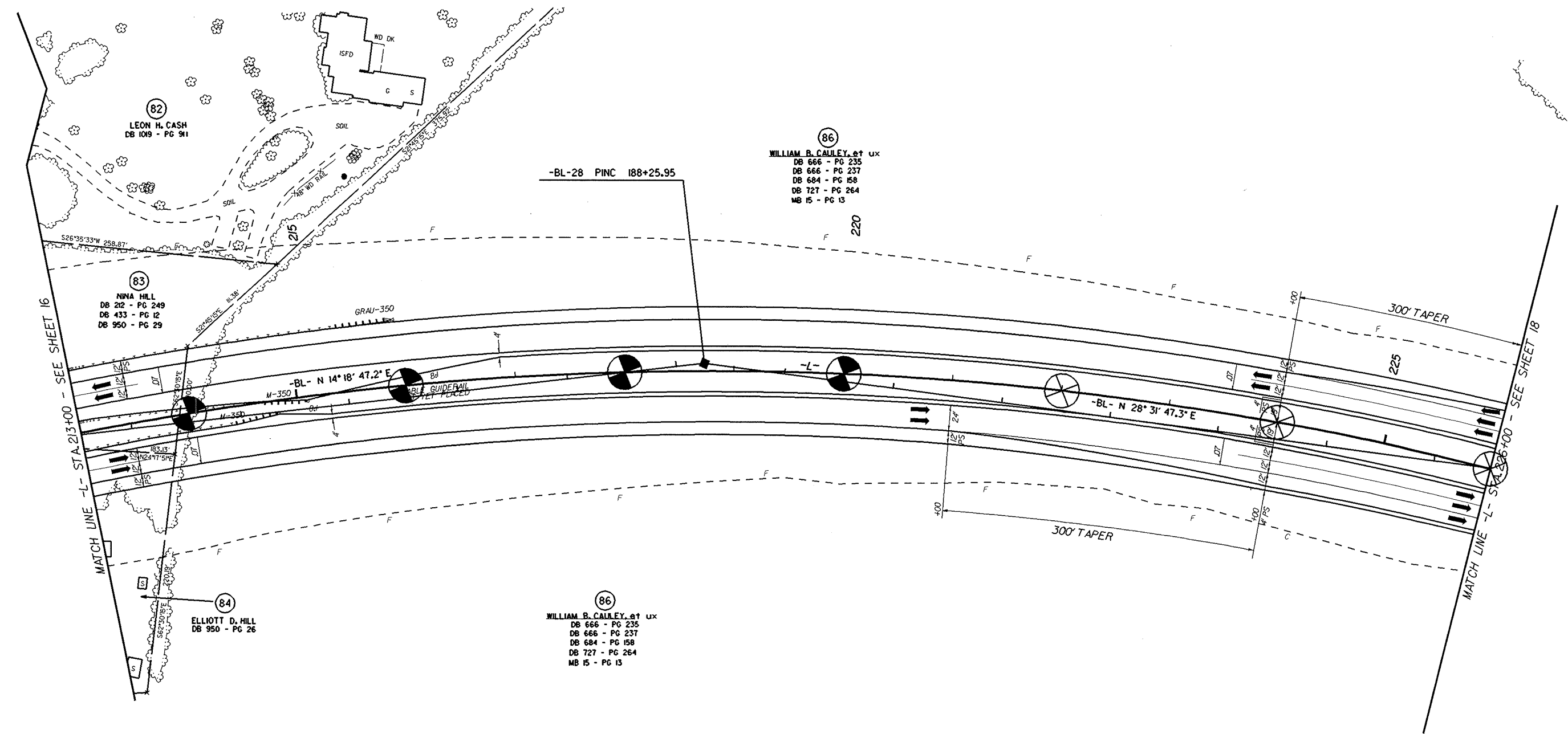
PROJECT REFERENCE NO. R-2719A	SHEET NO. 17
RW SHEET NO. 17	
ROADWAY DESIGN ENGINEER	HYDRAULICS ENGINEER
INCOMPLETE PLANS DO NOT USE FOR R/W ACQUISITION PRELIMINARY PLANS DO NOT USE FOR CONSTRUCTION	

-L- CURVE DATA
 PI Sta 224+97.17
 $\Delta = 61^{\circ} 32' 05.9''$ (RT)
 $D = 1^{\circ} 58' 32.6''$
 $L = 3,114.56'$
 $T = 1,726.52'$
 $R = 2,900.00'$
 $e = 7\%$



REVISIONS

I:\7-NR\2005 13\33
 L:\V\ERO\Hale\proj\Investigation\TIP\Greenville\NR2719A_GEO_RDWY\CADD_GEO\TECH\Plan\Prof\vr2719a_geo_psh_17.dgn
 Miller
 08/17/99



82
 LEON H. CASH
 DB 109 - PG 91

83
 NINA HILL
 DB 212 - PG 249
 DB 433 - PG 12
 DB 950 - PG 29

84
 ELLIOTT D. HILL
 DB 950 - PG 26

86
 WILLIAM B. CALLEY, et ux
 DB 666 - PG 235
 DB 666 - PG 237
 DB 684 - PG 158
 DB 727 - PG 264
 MB 15 - PG 13

86
 WILLIAM B. CALLEY, et ux
 DB 666 - PG 235
 DB 666 - PG 237
 DB 684 - PG 158
 DB 727 - PG 264
 MB 15 - PG 13

MATCH LINE -L- STA 213+00 - SEE SHEET 16

MATCH LINE -L- STA 236+00 - SEE SHEET 18

8/17/99

-L- CURVE DATA

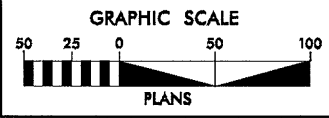
PI Sta 224+97.17 Pls Sta 240+01.92
 $\Delta = 61^{\circ}32'05.9''$ (RT) $\Theta_s = 3^{\circ}27'27.0''$
 $D = 1^{\circ}58'32.6''$ $L_s = 350.00'$
 $L = 3,114.56'$ $LT = 233.38'$
 $T = 1,726.52'$ $ST = 116.71'$
 $R = 2,900.00'$
 $e = 7\%$

-Y24RPB- CURVE DATA

Pls Sta 0+70.01 Pls Sta 5+95.85 Pls Sta 8+30.75
 $\Theta_s = 2^{\circ}02'29.1''$ $\Theta_s = 4^{\circ}35'01.2''$ $\Delta = 11^{\circ}47'45.0''$ (LT)
 $L_s = 210.00'$ $L_s = 240.00'$ $D = 3^{\circ}49'11.0''$
 $L = 140.01'$ $L = 160.05'$ $L = 308.81'$
 $T = 154.95'$ $T = 150.00'$ $T = 154.95'$
 $R = 1,500.00'$ $R = 1,500.00'$ $R = 1,500.00'$
 $e = 8\%$

-Y24RPC- CURVE DATA

Pls Sta 1+13.65 Pls Sta 7+15.43 Pls Sta 12+67.55
 $\Theta_s = 6^{\circ}45'47.2''$ $\Delta = 46^{\circ}19'00.7''$ (RT) $\Theta_s = 6^{\circ}39'24.4''$
 $L_s = 200.00'$ $D = 4^{\circ}45'17.4''$ $L_s = 280.00'$
 $ST = 86.57'$ $L = 974.01'$ $LT = 186.80'$
 $LT = 113.65'$ $T = 515.43'$ $ST = 93.45'$
 $\Delta 1 = 2^{\circ}00'29.8''$ $R = 1,205.00'$
 $\Delta 2 = 4^{\circ}45'17.4''$ $e = 8\%$
 $R1 = 2,853.00'$
 $R2 = 1,205.00'$

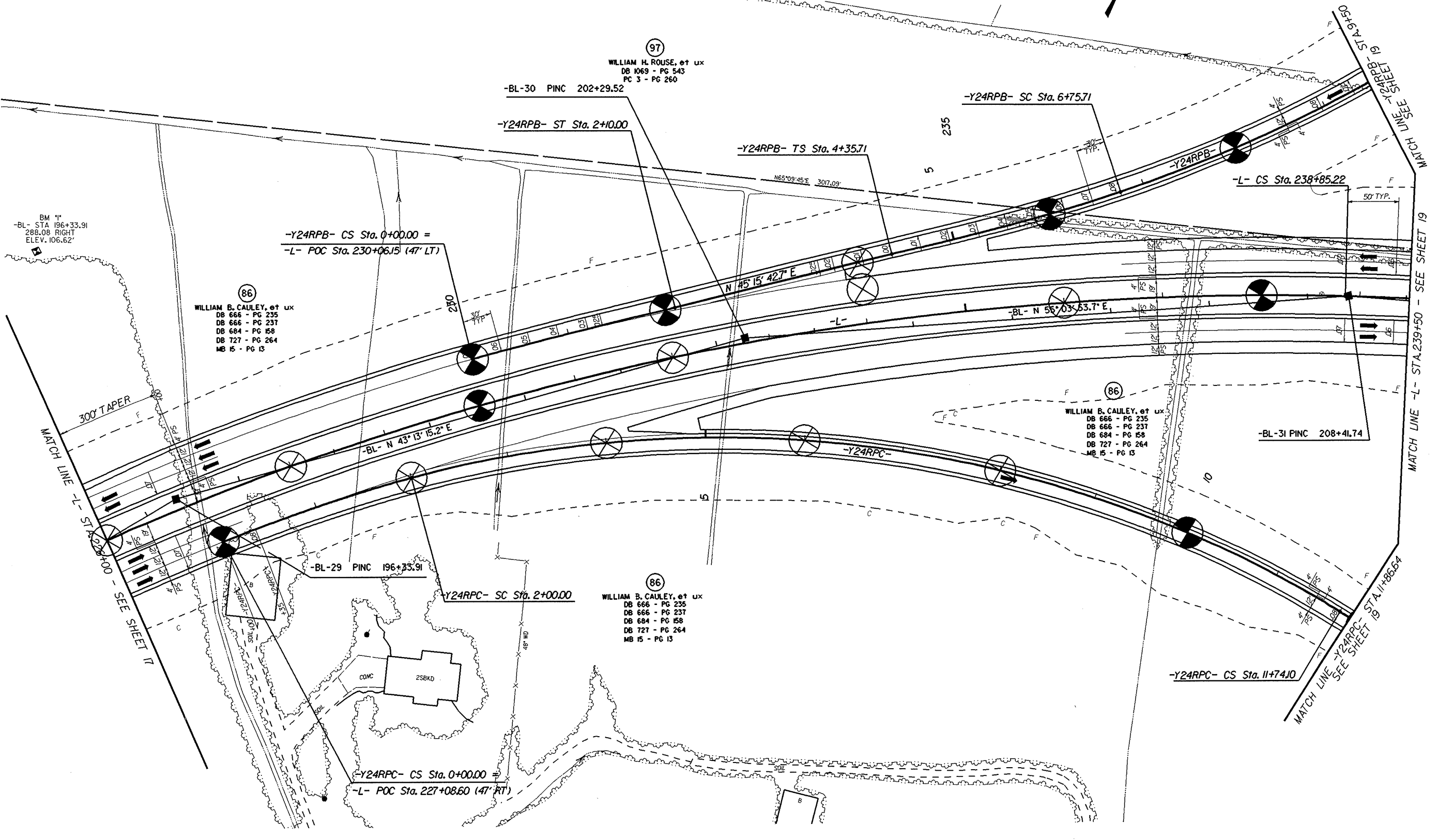


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 Morrisville, North Carolina 27560
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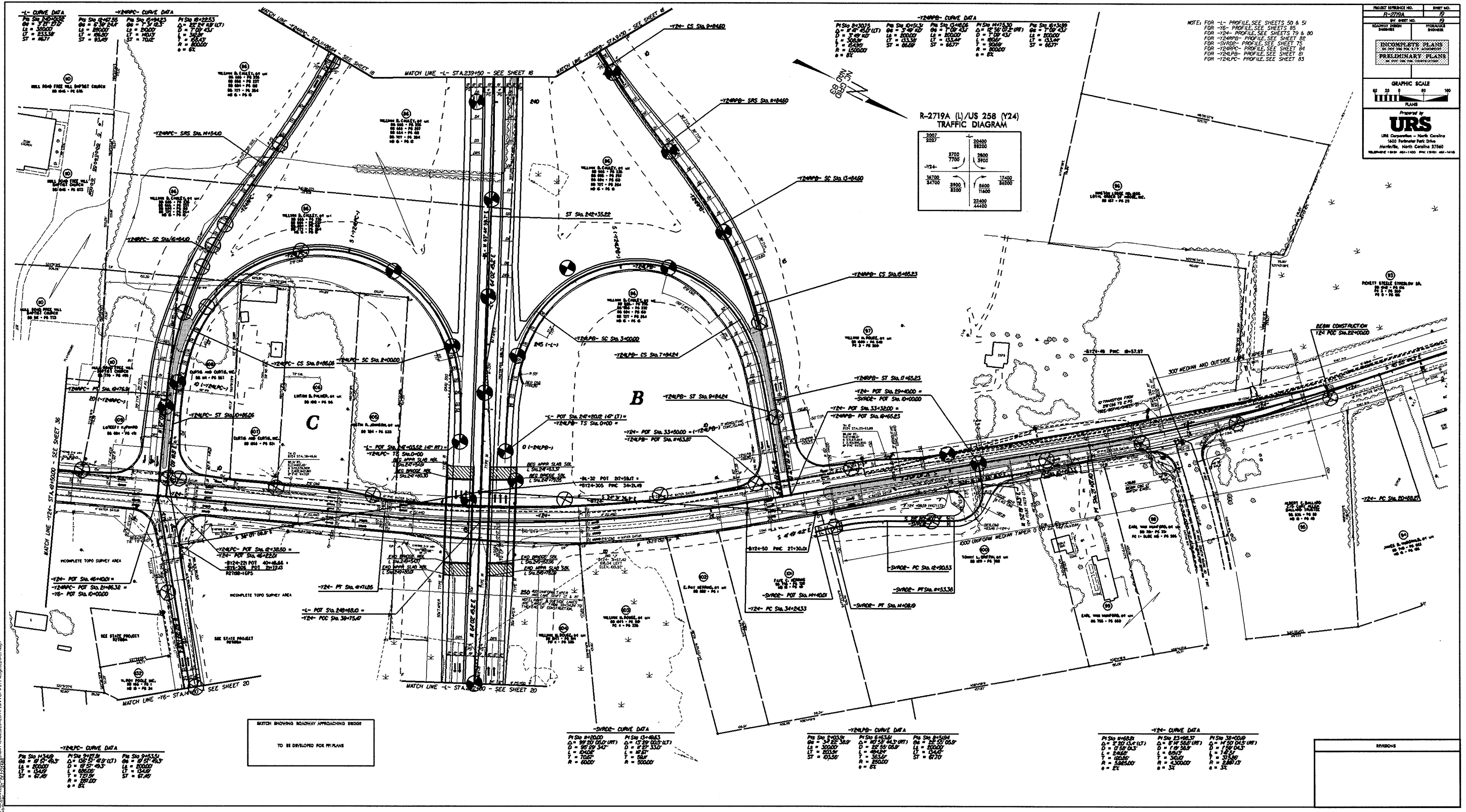
PROJECT REFERENCE NO. R-2719A	SHEET NO. 18
RAW SHEET NO. 18	HYDRAULICS ENGINEER
ROADWAY DESIGN ENGINEER	
INCOMPLETE PLANS DO NOT USE FOR ACQUISITION PRELIMINARY PLANS DO NOT USE FOR CONSTRUCTION	

NOTE: FOR -L- PROFILE, SEE SHEETS 49 & 50
 FOR -Y24RPB- PROFILE, SEE SHEET 82
 FOR -Y24RPC- PROFILE, SEE SHEET 84

REVISIONS



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 L:\NOV-2005 1404...



-L- CURVE DATA

PI Sta. 219+00.00	PI Sta. 219+00.00	PI Sta. 219+00.00	PI Sta. 219+00.00
PC = 218+50.00	PC = 218+50.00	PC = 218+50.00	PC = 218+50.00
PT = 219+10.00	PT = 219+10.00	PT = 219+10.00	PT = 219+10.00
ST = 218+75.00	ST = 218+75.00	ST = 218+75.00	ST = 218+75.00
L = 200.00	L = 200.00	L = 200.00	L = 200.00
T = 125.00	T = 125.00	T = 125.00	T = 125.00
R = 2000.00	R = 2000.00	R = 2000.00	R = 2000.00
Δ = 90°	Δ = 90°	Δ = 90°	Δ = 90°

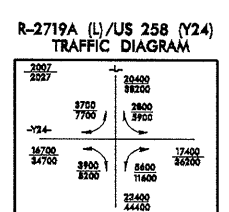
-L- CURVE DATA

PI Sta. 219+00.00	PI Sta. 219+00.00	PI Sta. 219+00.00	PI Sta. 219+00.00
PC = 218+50.00	PC = 218+50.00	PC = 218+50.00	PC = 218+50.00
PT = 219+10.00	PT = 219+10.00	PT = 219+10.00	PT = 219+10.00
ST = 218+75.00	ST = 218+75.00	ST = 218+75.00	ST = 218+75.00
L = 200.00	L = 200.00	L = 200.00	L = 200.00
T = 125.00	T = 125.00	T = 125.00	T = 125.00
R = 2000.00	R = 2000.00	R = 2000.00	R = 2000.00
Δ = 90°	Δ = 90°	Δ = 90°	Δ = 90°

-L- CURVE DATA

PI Sta. 219+00.00	PI Sta. 219+00.00	PI Sta. 219+00.00	PI Sta. 219+00.00
PC = 218+50.00	PC = 218+50.00	PC = 218+50.00	PC = 218+50.00
PT = 219+10.00	PT = 219+10.00	PT = 219+10.00	PT = 219+10.00
ST = 218+75.00	ST = 218+75.00	ST = 218+75.00	ST = 218+75.00
L = 200.00	L = 200.00	L = 200.00	L = 200.00
T = 125.00	T = 125.00	T = 125.00	T = 125.00
R = 2000.00	R = 2000.00	R = 2000.00	R = 2000.00
Δ = 90°	Δ = 90°	Δ = 90°	Δ = 90°

NOTE: FOR -L- PROFILE SEE SHEETS 50 & 51
 FOR -Y6- PROFILE SEE SHEETS 79 & 80
 FOR -Y24- PROFILE SEE SHEETS 79 & 80
 FOR -Y24PC- PROFILE SEE SHEET 81
 FOR -Y24PCB- PROFILE SEE SHEET 81
 FOR -Y24PCB- PROFILE SEE SHEET 81
 FOR -Y24PC- PROFILE SEE SHEET 83



PROJECT SHEET NO. R-2719A
 SHEET NO. 19
 DRAWING SHEET
 INCOMPLETE PLANS
 PRELIMINARY PLANS
 GRAPHIC SCALE
 PREPARED BY
URS
 URS Corporation - North Carolina
 1400 Perimeter Park Drive
 Charlotte, North Carolina 27840
 Telephone: (704) 486-1100 Fax: (704) 486-1100

-L- CURVE DATA

PI Sta. 219+00.00	PI Sta. 219+00.00	PI Sta. 219+00.00	PI Sta. 219+00.00
PC = 218+50.00	PC = 218+50.00	PC = 218+50.00	PC = 218+50.00
PT = 219+10.00	PT = 219+10.00	PT = 219+10.00	PT = 219+10.00
ST = 218+75.00	ST = 218+75.00	ST = 218+75.00	ST = 218+75.00
L = 200.00	L = 200.00	L = 200.00	L = 200.00
T = 125.00	T = 125.00	T = 125.00	T = 125.00
R = 2000.00	R = 2000.00	R = 2000.00	R = 2000.00
Δ = 90°	Δ = 90°	Δ = 90°	Δ = 90°

-L- CURVE DATA

PI Sta. 219+00.00	PI Sta. 219+00.00	PI Sta. 219+00.00	PI Sta. 219+00.00
PC = 218+50.00	PC = 218+50.00	PC = 218+50.00	PC = 218+50.00
PT = 219+10.00	PT = 219+10.00	PT = 219+10.00	PT = 219+10.00
ST = 218+75.00	ST = 218+75.00	ST = 218+75.00	ST = 218+75.00
L = 200.00	L = 200.00	L = 200.00	L = 200.00
T = 125.00	T = 125.00	T = 125.00	T = 125.00
R = 2000.00	R = 2000.00	R = 2000.00	R = 2000.00
Δ = 90°	Δ = 90°	Δ = 90°	Δ = 90°

-L- CURVE DATA

PI Sta. 219+00.00	PI Sta. 219+00.00	PI Sta. 219+00.00	PI Sta. 219+00.00
PC = 218+50.00	PC = 218+50.00	PC = 218+50.00	PC = 218+50.00
PT = 219+10.00	PT = 219+10.00	PT = 219+10.00	PT = 219+10.00
ST = 218+75.00	ST = 218+75.00	ST = 218+75.00	ST = 218+75.00
L = 200.00	L = 200.00	L = 200.00	L = 200.00
T = 125.00	T = 125.00	T = 125.00	T = 125.00
R = 2000.00	R = 2000.00	R = 2000.00	R = 2000.00
Δ = 90°	Δ = 90°	Δ = 90°	Δ = 90°

-L- CURVE DATA

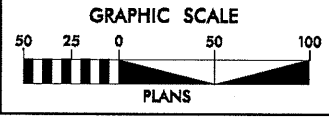
PI Sta. 219+00.00	PI Sta. 219+00.00	PI Sta. 219+00.00	PI Sta. 219+00.00
PC = 218+50.00	PC = 218+50.00	PC = 218+50.00	PC = 218+50.00
PT = 219+10.00	PT = 219+10.00	PT = 219+10.00	PT = 219+10.00
ST = 218+75.00	ST = 218+75.00	ST = 218+75.00	ST = 218+75.00
L = 200.00	L = 200.00	L = 200.00	L = 200.00
T = 125.00	T = 125.00	T = 125.00	T = 125.00
R = 2000.00	R = 2000.00	R = 2000.00	R = 2000.00
Δ = 90°	Δ = 90°	Δ = 90°	Δ = 90°

TO BE DEVELOPED FOR PLANS

ERRORS

8/17/99

NOTE: FOR -L- PROFILE, SEE SHEETS 51 & 52
FOR -Y6- PROFILE, SEE SHEET 85



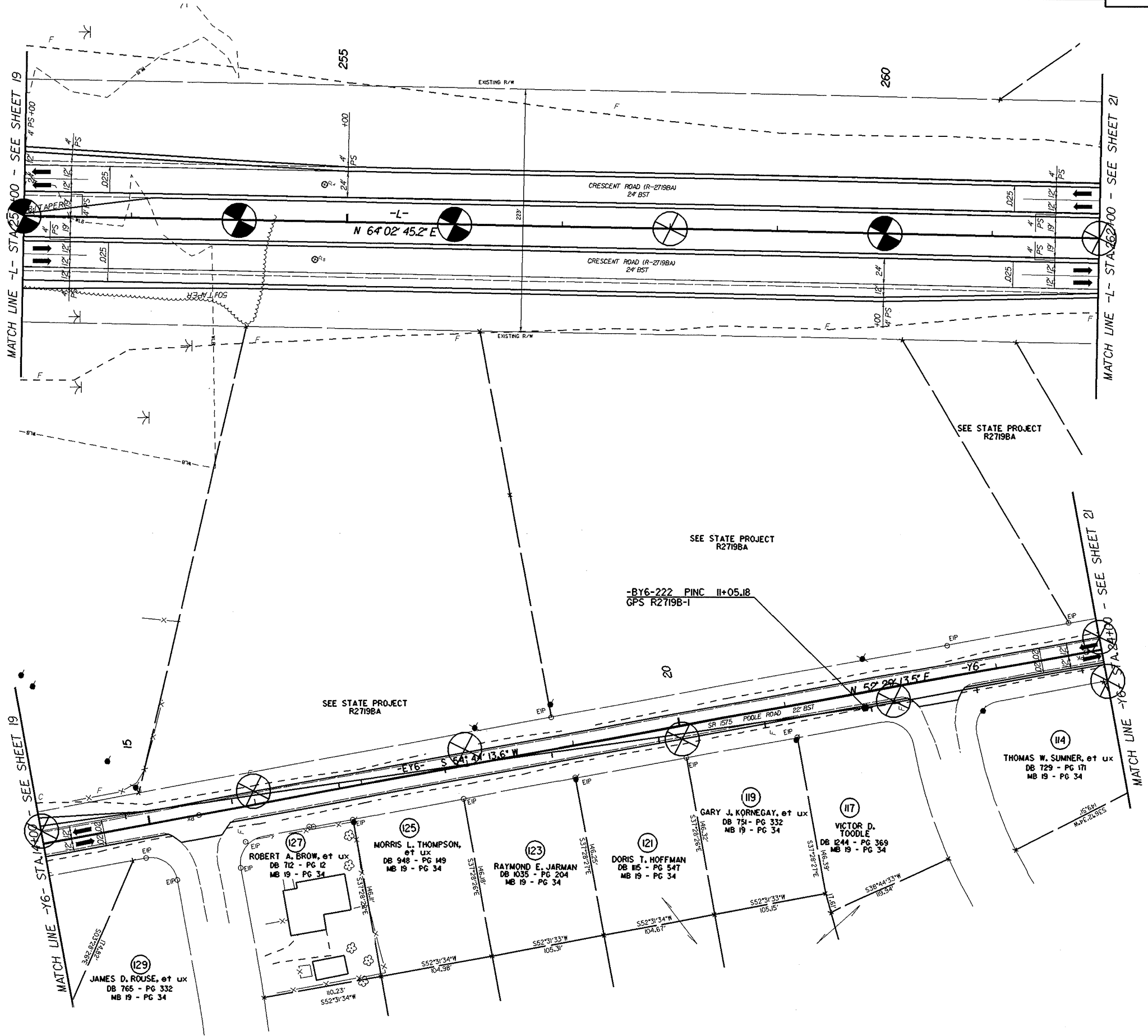
Prepared by
URS
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1500 Penimeter Park Drive
Harrisville, North Carolina 27640
TELEPHONE (813) 481-1100 FAX (813) 481-1410

PROJECT REFERENCE NO.	SHEET NO.
R-2719A	20
R/W SHEET NO.	20
ROADWAY DESIGN ENGINEER	HYDRAULICS ENGINEER
INCOMPLETE PLANS DO NOT USE FOR R/W ACQUISITION PRELIMINARY PLANS DO NOT USE FOR CONSTRUCTION	



REVISIONS

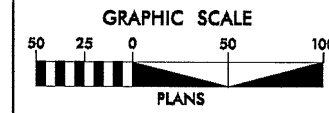
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k.miller AT GEJ221408
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k.miller AT GEJ221408

8/17/99

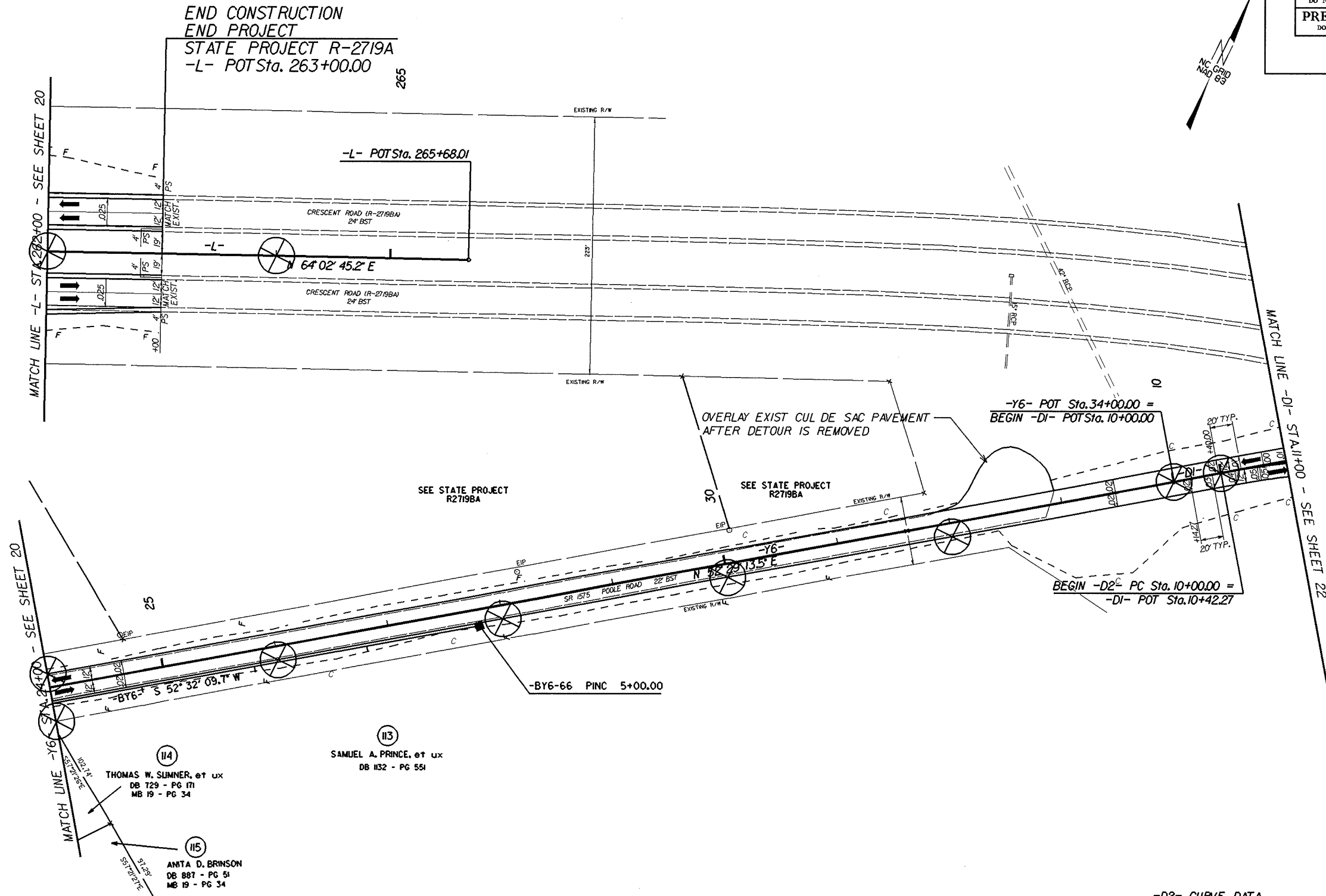
NOTE: FOR -L- PROFILE, SEE SHEET 52
FOR -Y6- PROFILE, SEE SHEET 85
FOR -D1- & -D2- PROFILE, SEE SHEET 86



Prepared by
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 Morrisville, North Carolina 27560
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PROJECT REFERENCE NO. R-2719A	SHEET NO. 21
R/W SHEET NO. 21	
ROADWAY DESIGN ENGINEER	HYDRAULICS ENGINEER
INCOMPLETE PLANS DO NOT USE FOR R/W ACQUISITION PRELIMINARY PLANS DO NOT USE FOR CONSTRUCTION	

END CONSTRUCTION
END PROJECT
STATE PROJECT R-2719A
-L- POT Sta. 263+00.00



-D2- CURVE DATA

PI Sta	13+59.49
Δ	39° 32' 45.6" (RT)
D	5° 43' 46.5"
L	690.21'
T	359.49'
R	1,000.00'
e	5.1%

- (114) THOMAS W. SUMNER, et ux
DB 729 - PG 171
MB 19 - PG 34
- (115) ANITA D. BRINSON
DB 887 - PG 51
MB 19 - PG 34

(113) SAMUEL A. PRINCE, et ux
DB 1132 - PG 551

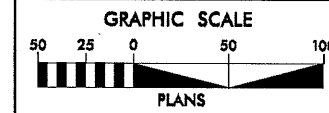
REVISIONS

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bml\er AT 05/22/08

8/17/99

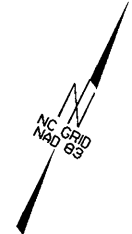
-D1- CURVE DATA	-D2- CURVE DATA
PI Sta 15+13.79	PI Sta 13+59.49
$\Delta = 39^\circ 45' 06.6''$ (RT)	$\Delta = 39^\circ 32' 45.6''$ (RT)
$D = 5^\circ 43' 46.5''$	$D = 5^\circ 43' 46.5''$
$L = 693.80'$	$L = 690.21'$
$T = 361.52'$	$T = 359.49'$
$R = 1,000.00'$	$R = 1,000.00'$
$e =$ SEE PLANS	$e =$ SEE PLANS
$R.O. =$ SEE PLANS	$R.O. =$ SEE PLANS

NOTE: FOR -D1- & -D2- PROFILE, SEE SHEET 86



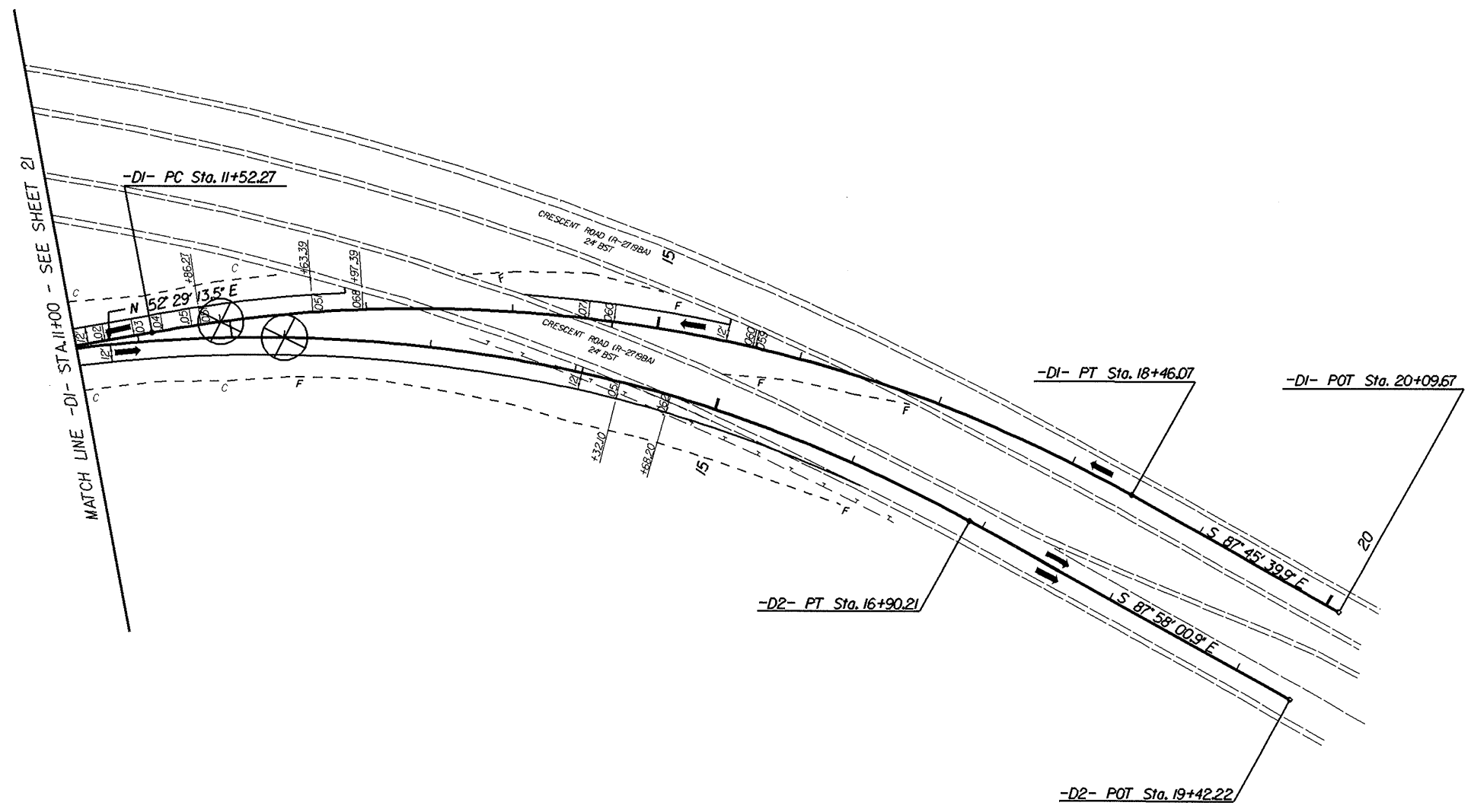
Prepared by
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 1400 Perimeter Park Drive
 Morrisville, North Carolina 27560
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PROJECT REFERENCE NO.	SHEET NO.
R-2719A	22
R/W SHEET NO.	22
ROADWAY DESIGN ENGINEER	HYDRAULICS ENGINEER
INCOMPLETE PLANS DO NOT USE FOR R/W ACQUISITION PRELIMINARY PLANS DO NOT USE FOR CONSTRUCTION	



REVISIONS

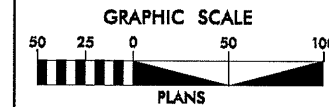
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 Miller, A.T. 8/22/99



NOTE: WB TRAFFIC ON CRESCENT ROAD TO BE SHIFTED TO OUTSIDE LANE PRIOR TO DETOUR AS PART OF TRAFFIC CONTROL PLAN.

8/17/99

NOTE: FOR -YI- PROFILE, SEE SHEET 53
FOR -YIO- PROFILE, SEE SHEET 71



Prepared by
URS
URS Corporation - North Carolina
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Harrisville, North Carolina 27540
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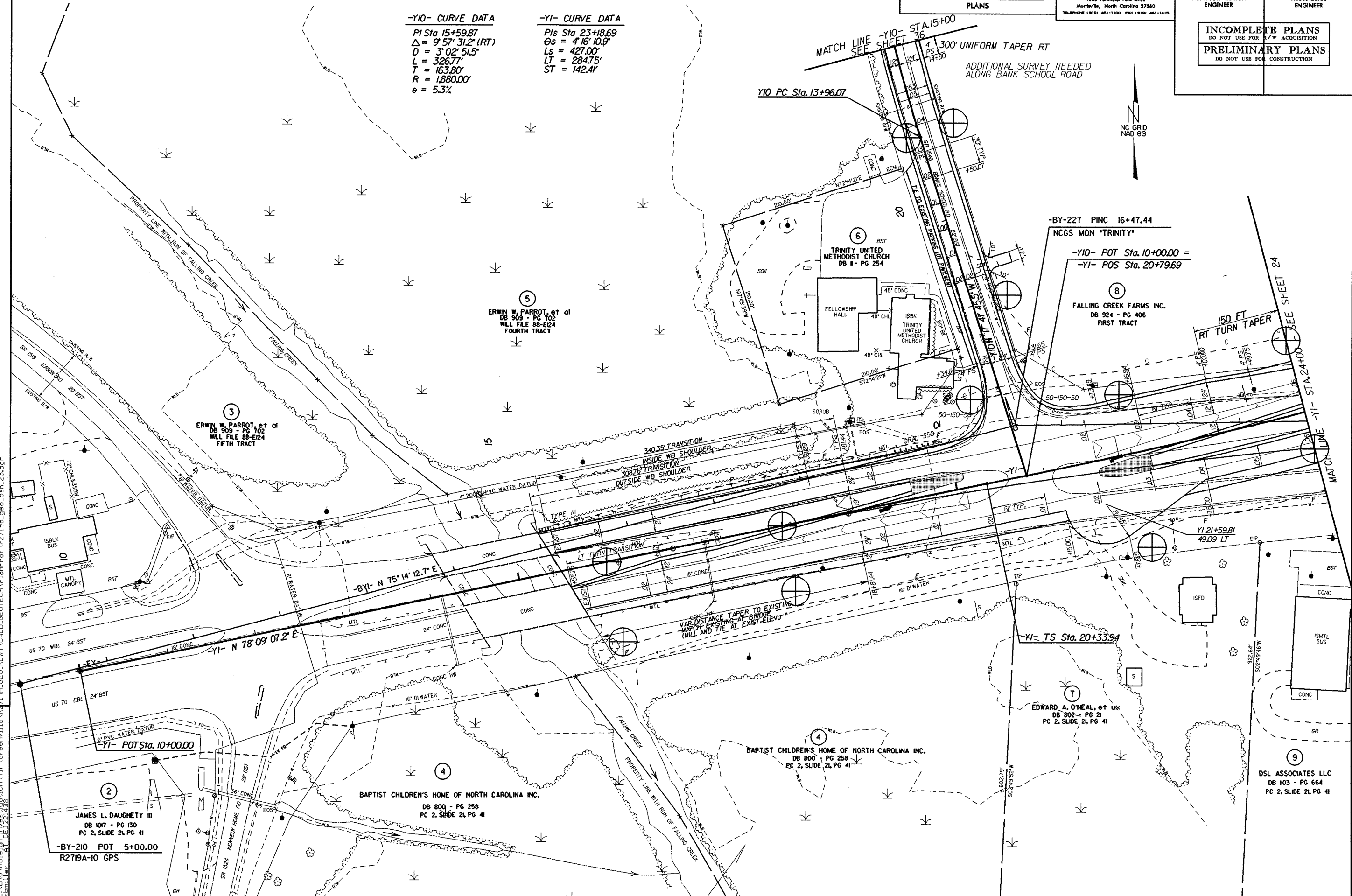
PROJECT REFERENCE NO. R-2719A	SHEET NO. 23
R/W SHEET NO. 23	
ROADWAY DESIGN ENGINEER	HYDRAULICS ENGINEER
INCOMPLETE PLANS DO NOT USE FOR R/W ACQUISITION PRELIMINARY PLANS DO NOT USE FOR CONSTRUCTION	

-YIO- CURVE DATA
 PI Sta 15+59.87
 $\Delta = 9^{\circ} 57' 31.2''$ (RT)
 $D = 3^{\circ} 02' 51.5''$
 $L = 326.77'$
 $T = 163.80'$
 $R = 1,880.00'$
 $e = 5.3\%$

-YI- CURVE DATA
 PIs Sta 23+18.69
 $\Theta s = 4^{\circ} 16' 10.9''$
 $Ls = 427.00'$
 $LT = 284.75'$
 $ST = 142.41'$

REVISIONS

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SEE SHEET 24
MATCH LINE -YI- STA. 24+00

MATCH LINE -YIO- STA. 15+00
SEE SHEET 36

ADDITIONAL SURVEY NEEDED
ALONG BANK SCHOOL ROAD

-BY-227 PINC 16+47.44
NCGS MON "TRINITY"

-YIO- POT Sta. 10+00.00 =
-YI- POS Sta. 20+79.69

150 FT
RT TURN TAPER

-YI- TS Sta. 20+33.94

-BY-210 POT 5+00.00
R2719A-10 GPS

JAMES L. DAUGHETY III
DB 107 - PG 130
PC 2, SLIDE 2L PG 41

BAPTIST CHILDREN'S HOME OF NORTH CAROLINA INC.
DB 800 - PG 258
PC 2, SLIDE 2L PG 41

BAPTIST CHILDREN'S HOME OF NORTH CAROLINA INC.
DB 800 - PG 258
PC 2, SLIDE 2L PG 41

EDWARD A. O'NEAL, et ux
DB 802 - PG 21
PC 2, SLIDE 2L PG 41

DSL ASSOCIATES LLC
DB 103 - PG 664
PC 2, SLIDE 2L PG 41

ERWIN W. PARROT, et al
DB 909 - PG 102
WLL FILE 88-E24
FOURTH TRACT

ERWIN W. PARROT, et al
DB 909 - PG 102
WLL FILE 88-E24
FIFTH TRACT

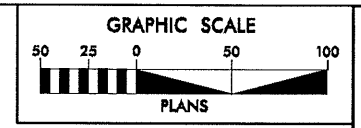
TRINITY UNITED
METHODIST CHURCH
DB I - PG 254

FALLING CREEK FARMS INC.
DB 924 - PG 406
FIRST TRACT

8/17/99
 REVISIONS
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-YI-

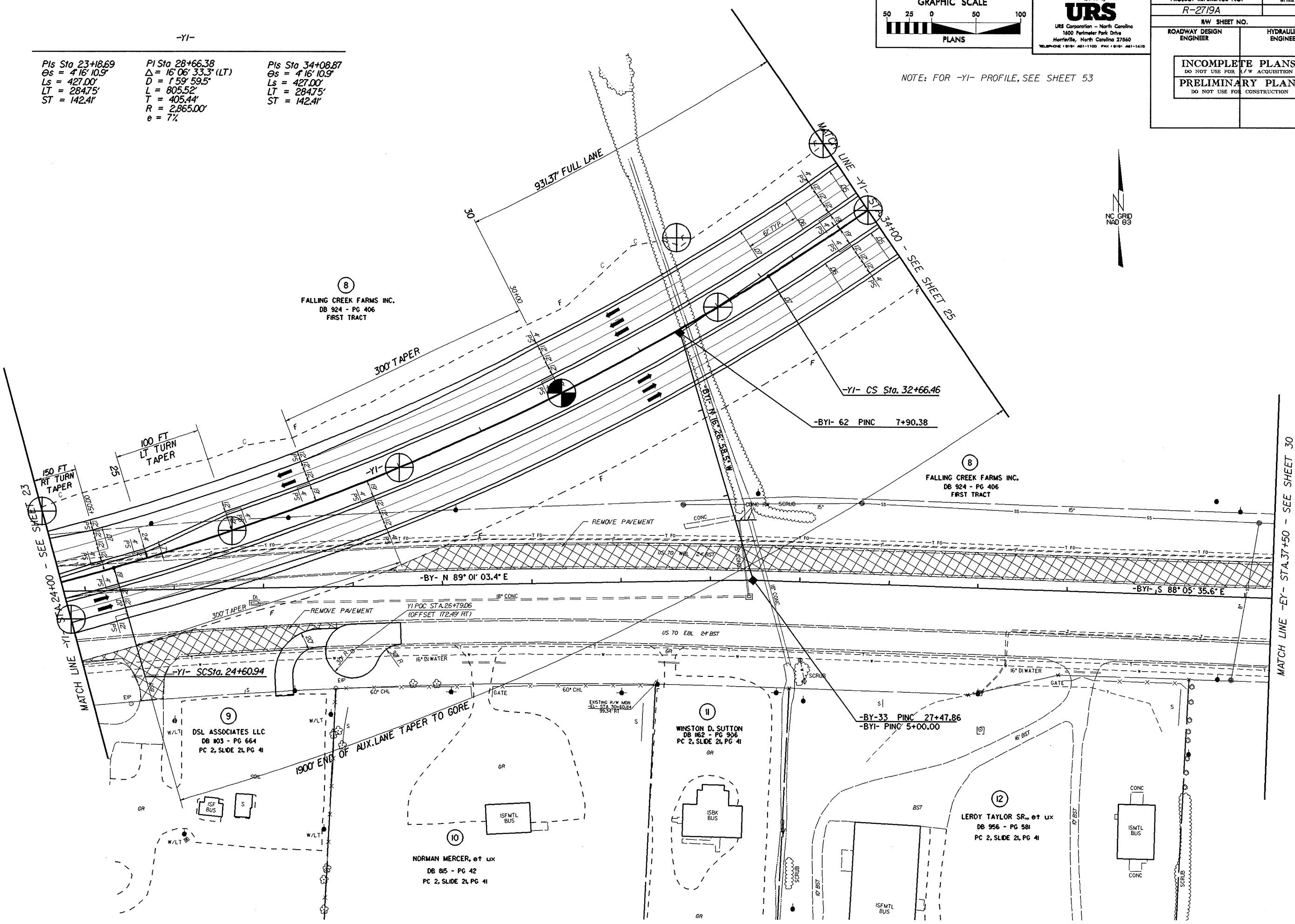
PIs Sta 23+18.69 Os = 4'16" 10.9" Ls = 427.00' LT = 284.75' ST = 142.41'	PI Sta 28+66.38 $\Delta = 16' 06" 33.3' (LT)$ D = 1'59" 59.5" L = 805.52' T = 405.44' R = 2,865.00' e = 7%	PIs Sta 34+08.87 Os = 4'16" 10.9" Ls = 427.00' LT = 284.75' ST = 142.41'
--	--	--



Prepared by
URS
 URS Corporation - North Carolina
 1600 Piedmont Park Drive
 Morrisville, North Carolina 27560
 TELEPHONE (919) 461-1100 FAX (919) 461-1410

PROJECT REFERENCE NO. R-2719A	SHEET NO. 24
R/W SHEET NO. 24	
ROADWAY DESIGN ENGINEER	HYDRAULICS ENGINEER
INCOMPLETE PLANS DO NOT USE FOR A/W ACQUISITION PRELIMINARY PLANS DO NOT USE FOR CONSTRUCTION	

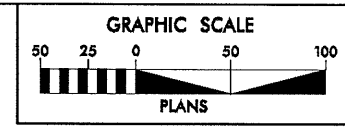
NOTE: FOR -YI- PROFILE, SEE SHEET 53



MATCH LINE -EY- STA. 37+50 - SEE SHEET 30

MATCH LINE -YI- STA. 24+00 - SEE SHEET 23

8/17/99

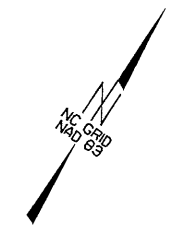


Prepared by
URS
URS Corporation - North Carolina
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Morrisville, North Carolina 27560
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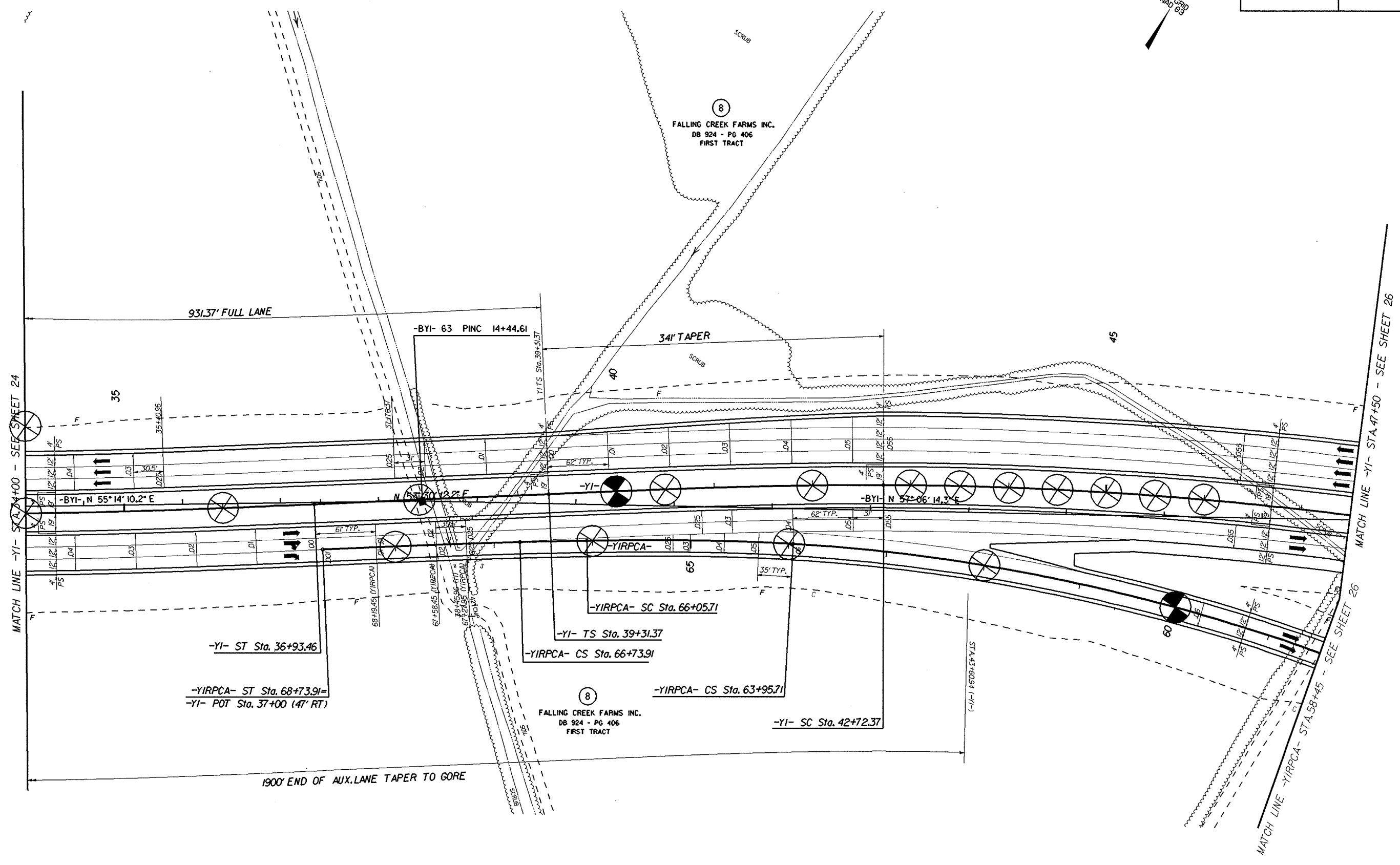
PROJECT REFERENCE NO.	SHEET NO.
R-2719A	25
RAW SHEET NO.	25
ROADWAY DESIGN ENGINEER	HYDRAULICS ENGINEER
INCOMPLETE PLANS DO NOT USE FOR E/W ACQUISITION PRELIMINARY PLANS DO NOT USE FOR CONSTRUCTION	

-YIRPCA- CURVE DATA				-YI- CURVE DATA		
PI Sta 56+51.41	PI Sta 64+82.65	PI Sta 66+39.81	PI Sta 67+40.58	PI Sta 34+08.87	PI Sta 41+58.72	PI Sta 66+70.52
$\Delta = 47^\circ 35' 39.3" (LT)$	$\Delta = 1^\circ 00' 08.6"$	$\Delta = 0^\circ 39' 04.5" (LT)$	$\Delta = 0^\circ 57' 17.7"$	$\Delta = 4^\circ 16' 10.9"$	$\Delta = 2^\circ 33' 26.3"$	$\Delta = 64^\circ 14' 25.4" (RT)$
$D = 2^\circ 59' 59.2"$	$L_s = 210.00'$	$D = 0^\circ 57' 17.7"$	$L_s = 200.00'$	$L_s = 427.00'$	$L_s = 341.00'$	$D = 1^\circ 29' 59.6"$
$L = 1586.59'$	$ST = 86.94'$	$L = 68.20'$	$L = 133.34'$	$LT = 284.75'$	$LT = 227.36'$	$L = 4283.01'$
$T = 842.30'$	$LT = 123.14'$	$T = 34.10'$	$R = 6,000.00'$	$ST = 142.41'$	$ST = 113.69'$	$T = 2,398.16'$
$R = 1,910.00'$	$\Delta 1 = 0^\circ 57' 17.7"$	$R = 6,000.00'$	$e = 2.5\%$			$R = 3,820.00'$
$e = 6\%$	$\Delta 2 = 2^\circ 59' 59.2"$					$e = 5.5\%$
	$R1 = 6,000.00'$					
	$R2 = 1,910.00'$					

NOTE: FOR -YI- PROFILE, SEE SHEETS 53 & 54
FOR -YIRPCA- PROFILE, SEE SHEET 66



REVISIONS



I:\WORK\2005\15126\1\VERO\Road\1.dwg
L:\VERO\Road\1.dwg
8/17/99

8/17/99

-YI- CURVE DATA

PI Sta 66+70.52
Δ = 64°14'25.4" (RT)
D = 129'59.6"
L = 4283.0'
T = 2398.16'
R = 3820.00'
e = 5.5%

-YIRPB- CURVE DATA

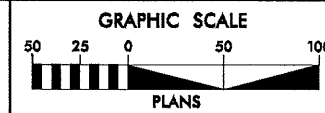
PI Sta 18+81.89
Δ = 46°11'53.3" (RT)
D = 229'28.0"
L = 1854.5'
T = 980.99'
R = 2300.00'
e = 5%

-YIRPCA- CURVE DATA

PIs Sta 47+39.14
Δs = 3°08'59.2"
Ls = 210.00'
Ts = 140.02'
ST = 70.02'

PI Sta 56+51.41
Δ = 47°35'39.3" (LT)
D = 259'59.2"
L = 1586.59'
T = 842.30'
R = 1910.00'
e = 6%

NOTE: FOR -YI- PROFILE, SEE SHEET 54
FOR -YIRPCA- PROFILE, SEE SHEETS 65 & 66
FOR -YIRPB- PROFILE, SEE SHEETS 60 & 61

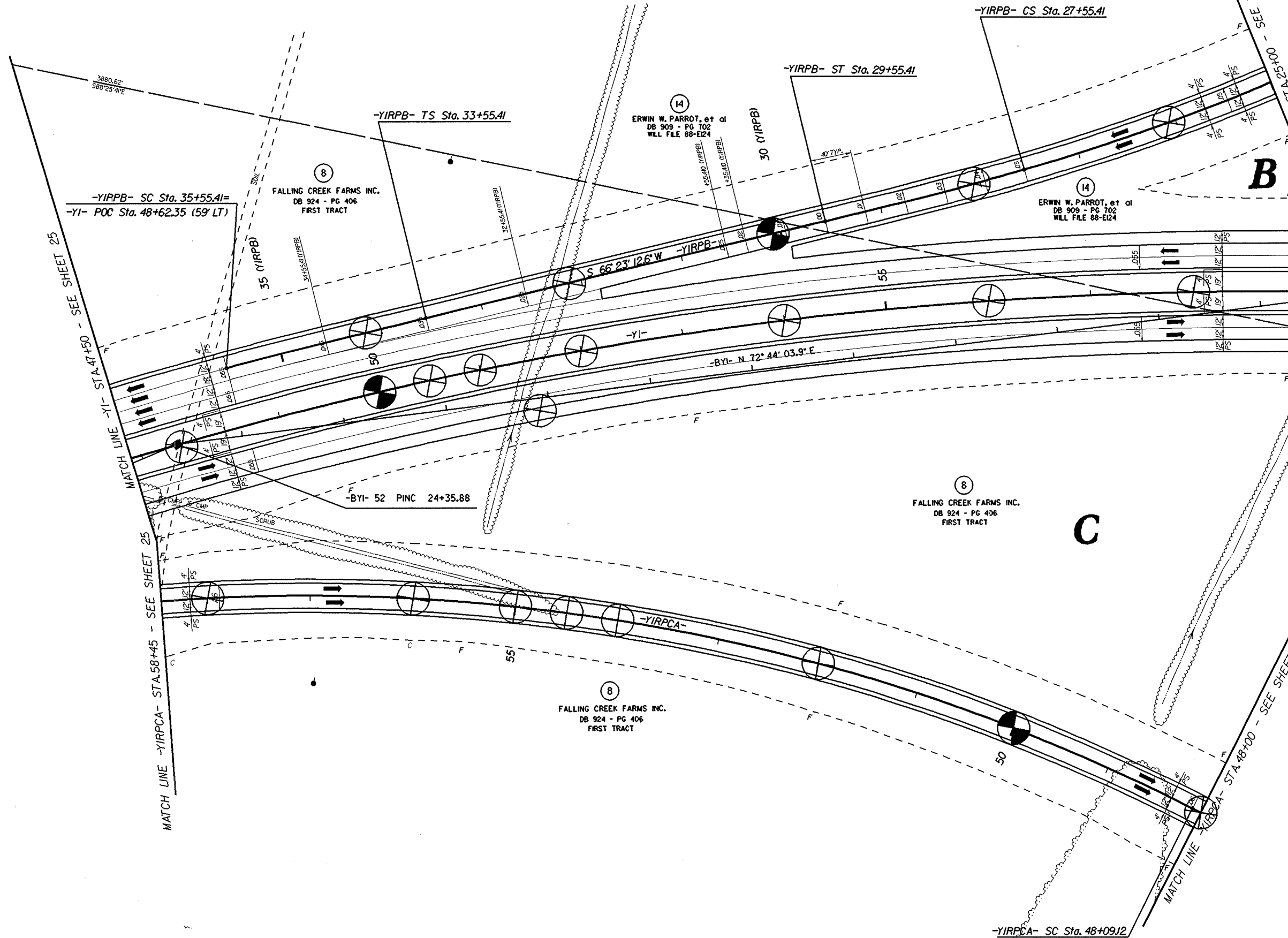


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PROJECT REFERENCE NO.	R-2719A	SHEET NO.	26
RW SHEET NO.	26	ROADWAY DESIGN ENGINEER	
HYDRAULICS ENGINEER			
INCOMPLETE PLANS DO NOT USE FOR R/W ACQUISITION			
PRELIMINARY PLANS DO NOT USE FOR CONSTRUCTION			

REVISIONS

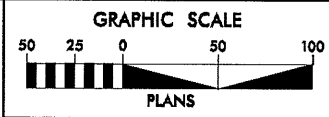
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Kbmler_A1_051221.dwg



8/17/99

-Y8- CURVE DATA
 PI Sta 15+04.43
 $\Delta = 58^{\circ} 42' 00.3" (RT)$
 $D = 14^{\circ} 19' 26.2"$
 $L = 409.80'$
 $T = 224.93'$
 $R = 400.00'$
 $e = 6\%$

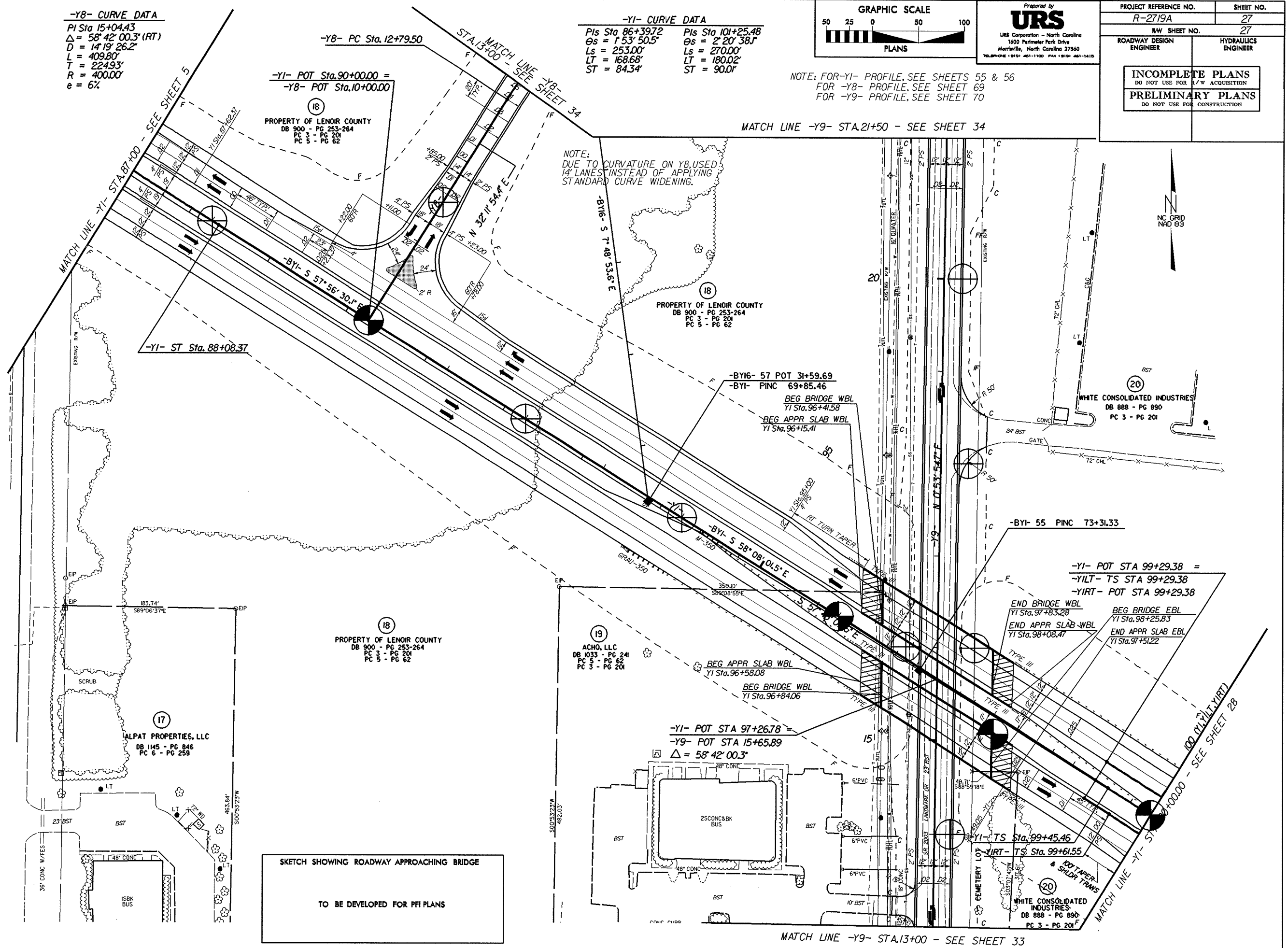
-Y1- CURVE DATA
 PIs Sta 86+397.2 PIs Sta 101+25.48
 $\Theta_s = 1^{\circ} 53' 50.5"$ $\Theta_s = 2^{\circ} 20' 38.1"$
 $L_s = 253.00'$ $L_s = 270.00'$
 $LT = 168.68'$ $LT = 180.02'$
 $ST = 84.34'$ $ST = 90.0'$



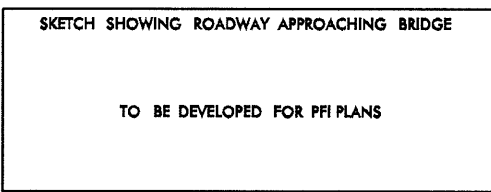
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 Morrisville, North Carolina 27560
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PROJECT REFERENCE NO. R-2719A	SHEET NO. 27
R/W SHEET NO. 27	
ROADWAY DESIGN ENGINEER	HYDRAULICS ENGINEER
INCOMPLETE PLANS DO NOT USE FOR R/W ACQUISITION PRELIMINARY PLANS DO NOT USE FOR CONSTRUCTION	

NOTE: FOR -Y1- PROFILE, SEE SHEETS 55 & 56
 FOR -Y8- PROFILE, SEE SHEET 69
 FOR -Y9- PROFILE, SEE SHEET 70



NOTE:
 DUE TO CURVATURE ON Y8, USED
 14' LANES INSTEAD OF APPLYING
 STANDARD CURVE WIDENING.

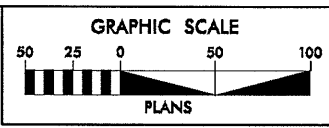


REVISIONS

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 8/17/99

8/17/99

NOTE: FOR -YI- PROFILE, SEE SHEETS 56 & 57
FOR -Y15- PROFILE, SEE SHEET 73



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PROJECT REFERENCE NO.	SHEET NO.
R-2719A	28
RAW SHEET NO.	28
ROADWAY DESIGN ENGINEER	HYDRAULICS ENGINEER
INCOMPLETE PLANS DO NOT USE FOR A/W ACQUISITION	
PRELIMINARY PLANS DO NOT USE FOR CONSTRUCTION	

-YI- CURVE DATA

PIs Sta 101+25.48	PI Sta 109+51.23
Os = 2° 20' 38.1"	Δ = 25° 08' 18.0" (LT)
Ls = 270.00'	D = 1° 44' 10.4"
LT = 180.02'	L = 1,447.86'
ST = 90.01'	T = 735.77'
	R = 3,300.00'
	e = 6%

-YILT- CURVE DATA

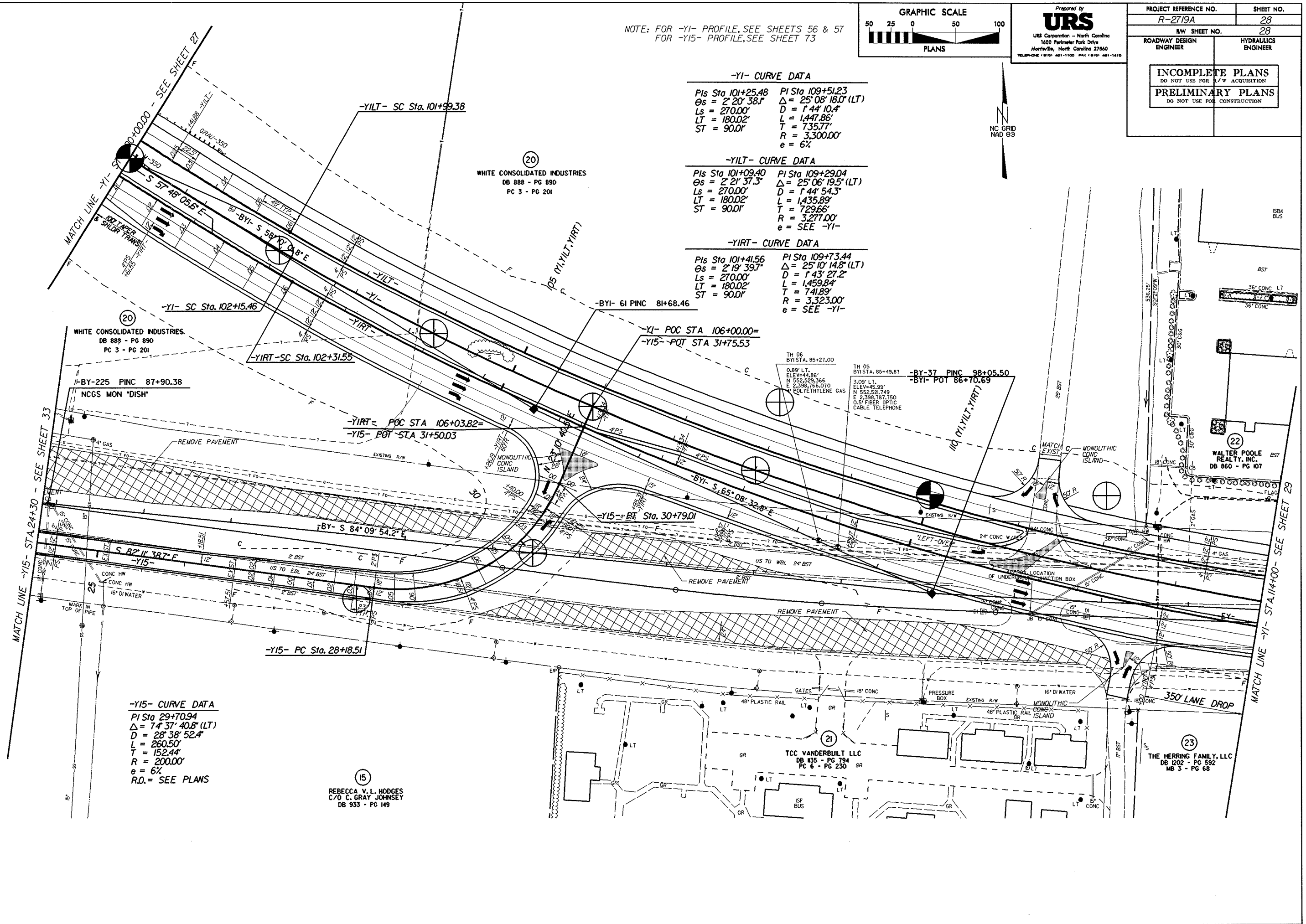
PIs Sta 101+09.40	PI Sta 109+29.04
Os = 2° 21' 37.3"	Δ = 25° 06' 19.5" (LT)
Ls = 270.00'	D = 1° 44' 54.3"
LT = 180.02'	L = 1,435.89'
ST = 90.01'	T = 729.66'
	R = 3,277.00'
	e = SEE -YI-

-YIRT- CURVE DATA

PIs Sta 101+41.56	PI Sta 109+73.44
Os = 2° 19' 39.7"	Δ = 25° 10' 14.8" (LT)
Ls = 270.00'	D = 1° 43' 27.2"
LT = 180.02'	L = 1,459.84'
ST = 90.01'	T = 741.89'
	R = 3,323.00'
	e = SEE -YI-

-Y15- CURVE DATA

PI Sta 29+70.94
Δ = 74° 37' 40.8" (LT)
D = 28° 38' 52.4"
L = 260.50'
T = 152.44'
R = 200.00'
e = 6%
R.O. = SEE PLANS



(20)
WHITE CONSOLIDATED INDUSTRIES
DB 888 - PG 890
PC 3 - PG 201

(20)
WHITE CONSOLIDATED INDUSTRIES
DB 889 - PG 890
PC 3 - PG 201

(20)
BY-225 PINC 87+90.38
NCCS MON 'DISH'

(20)
BY-61 PINC 81+68.46

TH 06
BY1 STA. 85+27.00
0.89' LT.
ELEV=44.86'
N 552,529.366
E 2,398,766.070
POLYETHYLENE GAS

TH 05
BY1 STA. 85+49.81
3.09' LT.
ELEV=45.99'
N 552,529.366
E 2,398,766.070
POLYETHYLENE GAS

(20)
BY-37 PINC 98+05.50
BY- POT 86+70.69

(22)
WALTER POOLE REALTY, INC.
DB 860 - PG 107

(21)
TCC VANDERBILT LLC
DB 135 - PG 794
PC 6 - PG 230

(15)
REBECCA V. L. HODGES
C/O C. GRAY JOHNSEY
DB 933 - PG 149

(23)
THE HERRING FAMILY, LLC
DB 1202 - PG 592
MB 3 - PG 68

REVISIONS

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MATCH LINE -Y15- STA.24+30 - SEE SHEET 33

MATCH LINE -YI- STA.100+00.00 - SEE SHEET 27

MATCH LINE -YI- STA.114+00 - SEE SHEET 29

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-YI- CURVE DATA

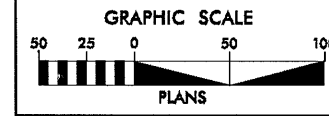
PI Sta 109+51.23 PIs Sta 117+53.34
Delta = 25° 08' 18.0" (LT) Os = 2° 20' 38.8"
D = 1' 44" 10.4" Ls = 270.00'
L = 1,447.86' LT = 180.02'
T = 735.77' ST = 90.01'
R = 3,300.00'
e = 6%

-YILT- CURVE DATA

PI Sta 109+29.04 PIs Sta 117+25.28
Delta = 25° 06' 19.5" (LT) Os = 2° 21' 37.3"
D = 1' 44" 54.3" Ls = 270.00'
L = 1,435.89' LT = 180.02'
T = 729.66' ST = 90.01'
R = 3,277.00'
e = SEE -YI-

-YIRT- CURVE DATA

PI Sta 109+73.44 PIs Sta 117+81.40
Delta = 25° 10' 14.8" (LT) Os = 2° 19' 39.7"
D = 1' 43" 27.2" Ls = 270.00'
L = 1,459.84' LT = 180.02'
T = 741.89' ST = 90.01'
R = 3,323.00'
e = SEE -YI-

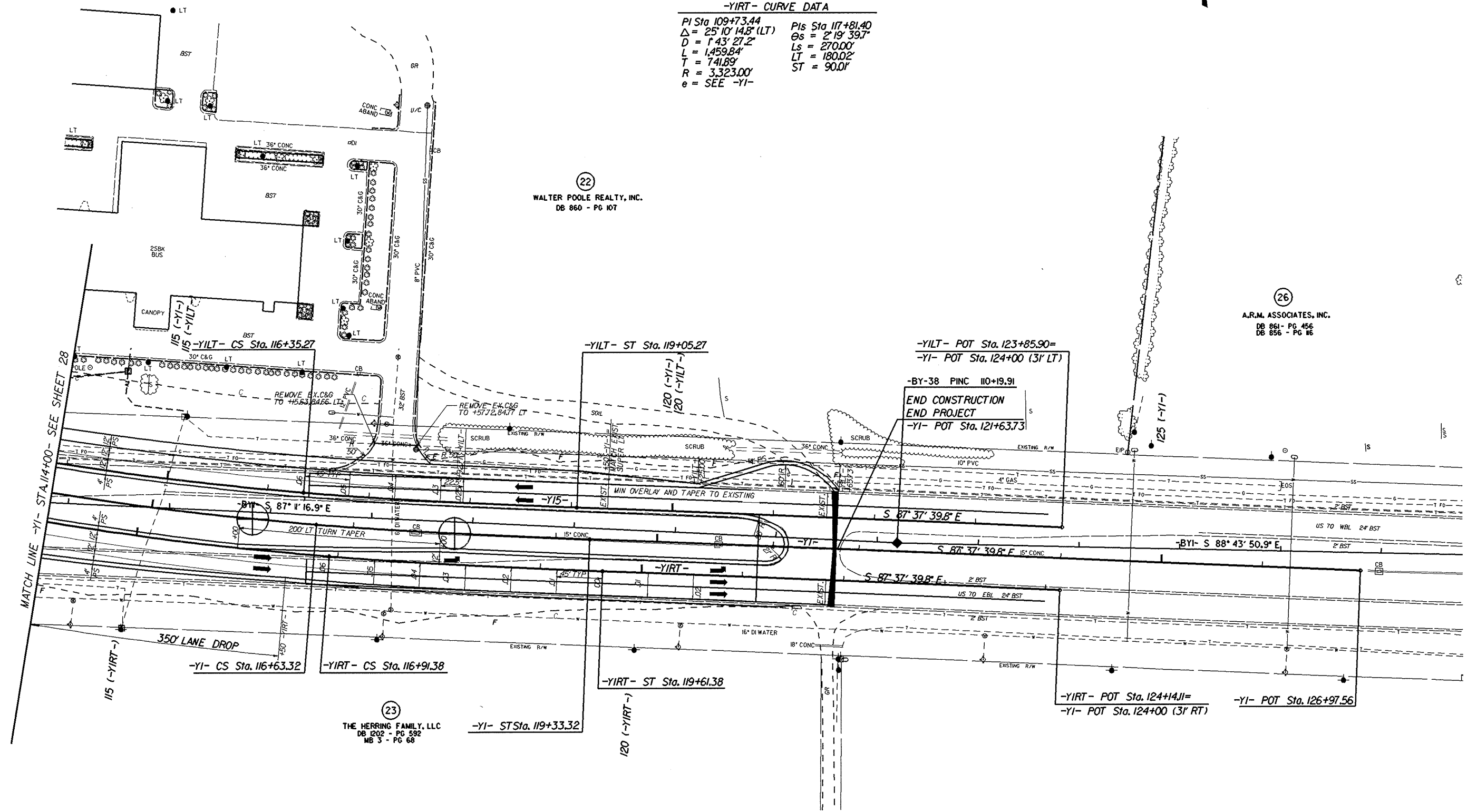


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Morrisville, North Carolina 27560

Table with Project Reference No. R-2719A, Sheet No. 29, and design engineer information.

NOTE: FOR -YI- PROFILE, SEE SHEET 57

NC GRID NAD 83



22 WALTER POOLE REALTY, INC. DB 860 - PG 107

26 A.R.M. ASSOCIATES, INC. DB 861 - PG 456 DB 856 - PG 16

23 THE HERRING FAMILY, LLC DB 1202 - PG 592 MB 3 - PG 68

MATCH LINE -YI- STA. 114+00- SEE SHEET 28

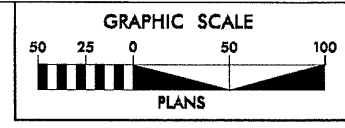
-YILT- POT Sta. 123+85.90=
-YI- POT Sta. 124+00 (31' LT)
-BY-38 PINC 110+19.91
END CONSTRUCTION
END PROJECT
-YI- POT Sta. 121+63.73

-YIRT- POT Sta. 124+14.11=
-YI- POT Sta. 124+00 (31' RT)
-YI- POT Sta. 126+97.56

8/17/99

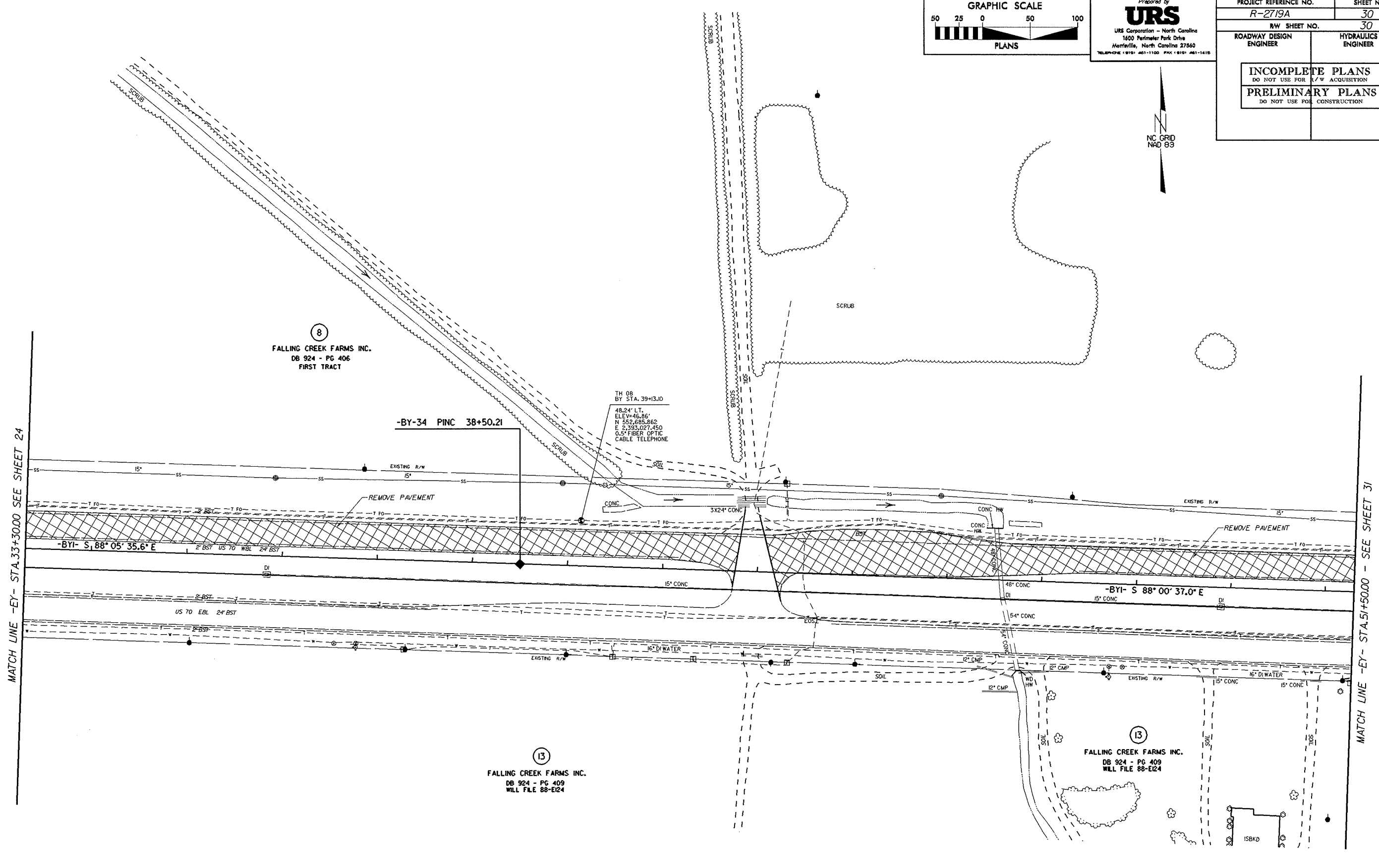
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DATE PLOTTED: 08/17/99 11:22:14 AM

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PROJECT REFERENCE NO. R-2719A	SHEET NO. 30
R/W SHEET NO. 30	
ROADWAY DESIGN ENGINEER	HYDRAULICS ENGINEER
INCOMPLETE PLANS DO NOT USE FOR R/W ACQUISITION PRELIMINARY PLANS DO NOT USE FOR CONSTRUCTION	



8
FALLING CREEK FARMS INC.
DB 924 - PG 406
FIRST TRACT

TH 08
BY STA. 39+13.10
48.24' LT.
ELEV=46.86'
N 52° 58' 56.86"
E 2,353.027,450
0.5" FIBER OPTIC
CABLE TELEPHONE

-BY-34 PINC 38+50.21

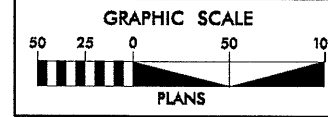
MATCH LINE -EY- STA. 33+30.00 SEE SHEET 24

MATCH LINE -EY- STA. 51+50.00 - SEE SHEET 31

13
FALLING CREEK FARMS INC.
DB 924 - PG 409
WLL FILE 88-E24

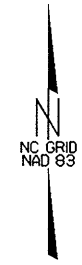
13
FALLING CREEK FARMS INC.
DB 924 - PG 409
WLL FILE 88-E24

8/17/99

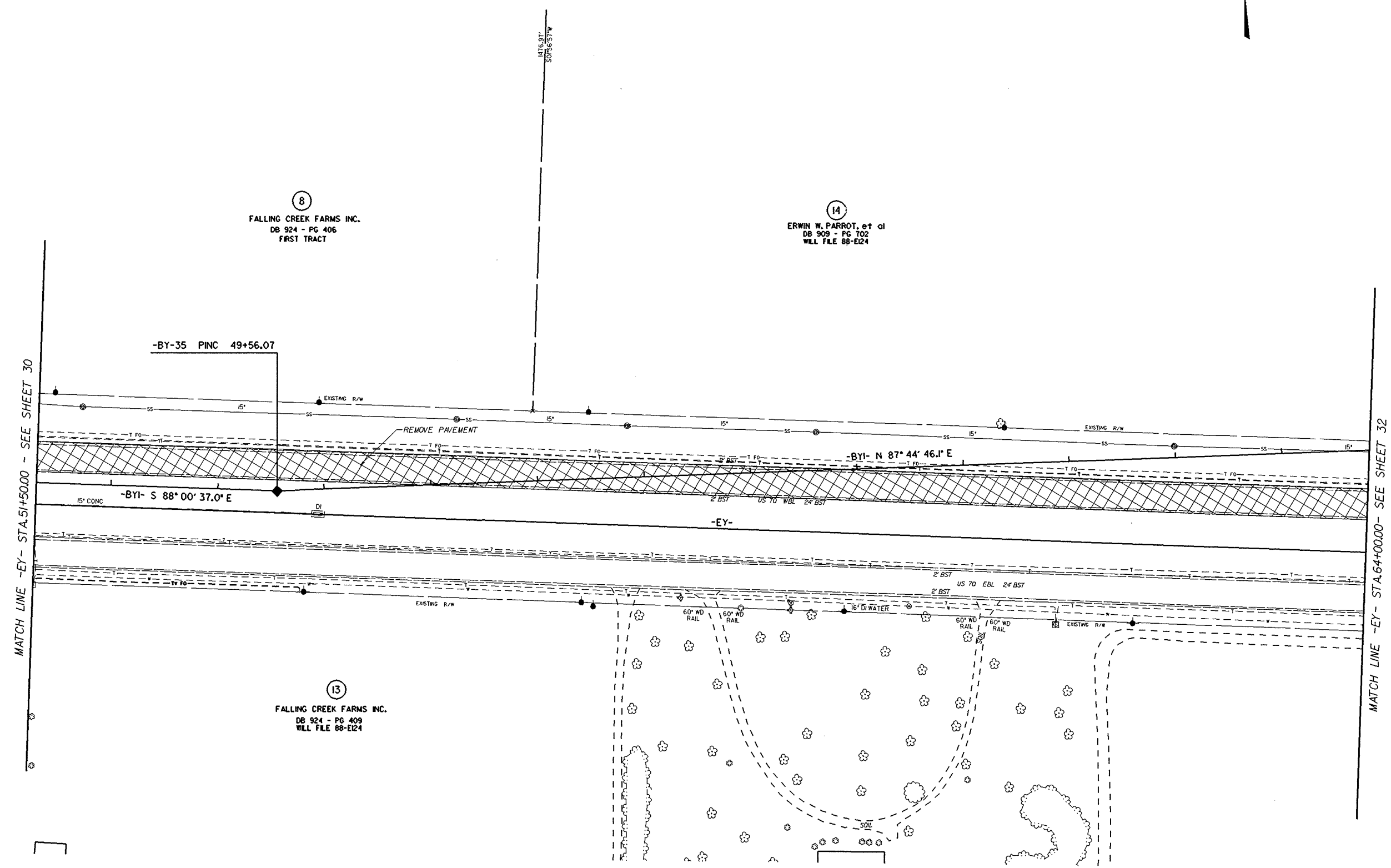


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PROJECT REFERENCE NO.	SHEET NO.
R-2719A	31
R/W SHEET NO.	31
ROADWAY DESIGN ENGINEER	HYDRAULICS ENGINEER
INCOMPLETE PLANS DO NOT USE FOR A/W ACQUISITION PRELIMINARY PLANS DO NOT USE FOR CONSTRUCTION	



REVISIONS
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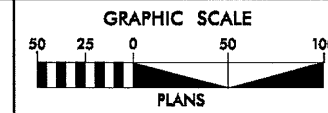


MATCH LINE -EY- STA. 51+50.00 - SEE SHEET 30

MATCH LINE -EY- STA. 64+00.00 - SEE SHEET 32

8/17/99

NOTE: FOR -Y15- PROFILE, SEE SHEET 73

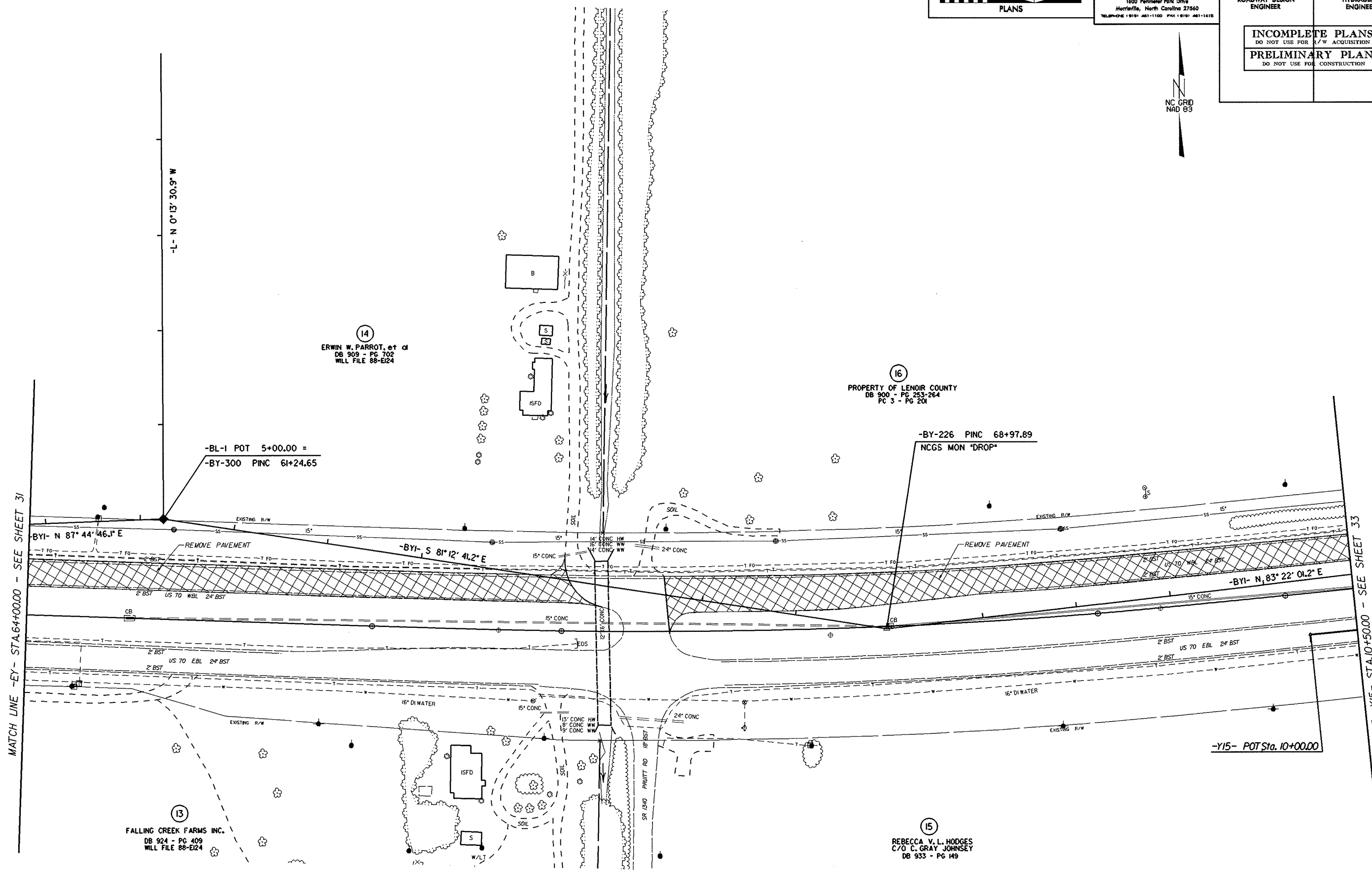


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PROJECT REFERENCE NO. R-2719A	SHEET NO. 32
R/W SHEET NO. 32	
ROADWAY DESIGN ENGINEER	HYDRAULICS ENGINEER
INCOMPLETE PLANS DO NOT USE FOR R/W ACQUISITION PRELIMINARY PLANS DO NOT USE FOR CONSTRUCTION	



REVISIONS



14
ERWIN W. PARROT, et al
DB 909 - PG 702
WILL FILE 88-E124

16
PROPERTY OF LENOIR COUNTY
DB 900 - PG 253-264
PC 3 - PG 204

13
FALLING CREEK FARMS INC.
DB 924 - PG 409
WILL FILE 88-E124

15
REBECCA V. L. HODGES
C/O C. GRAY JOHNSON
DB 933 - PG 149

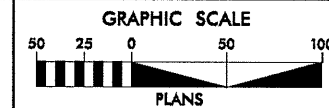
MATCH LINE -EY- STA. 64+00.00 - SEE SHEET 31

MATCH LINE -Y15- STA. 10+50.00 - SEE SHEET 33

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8/17/99

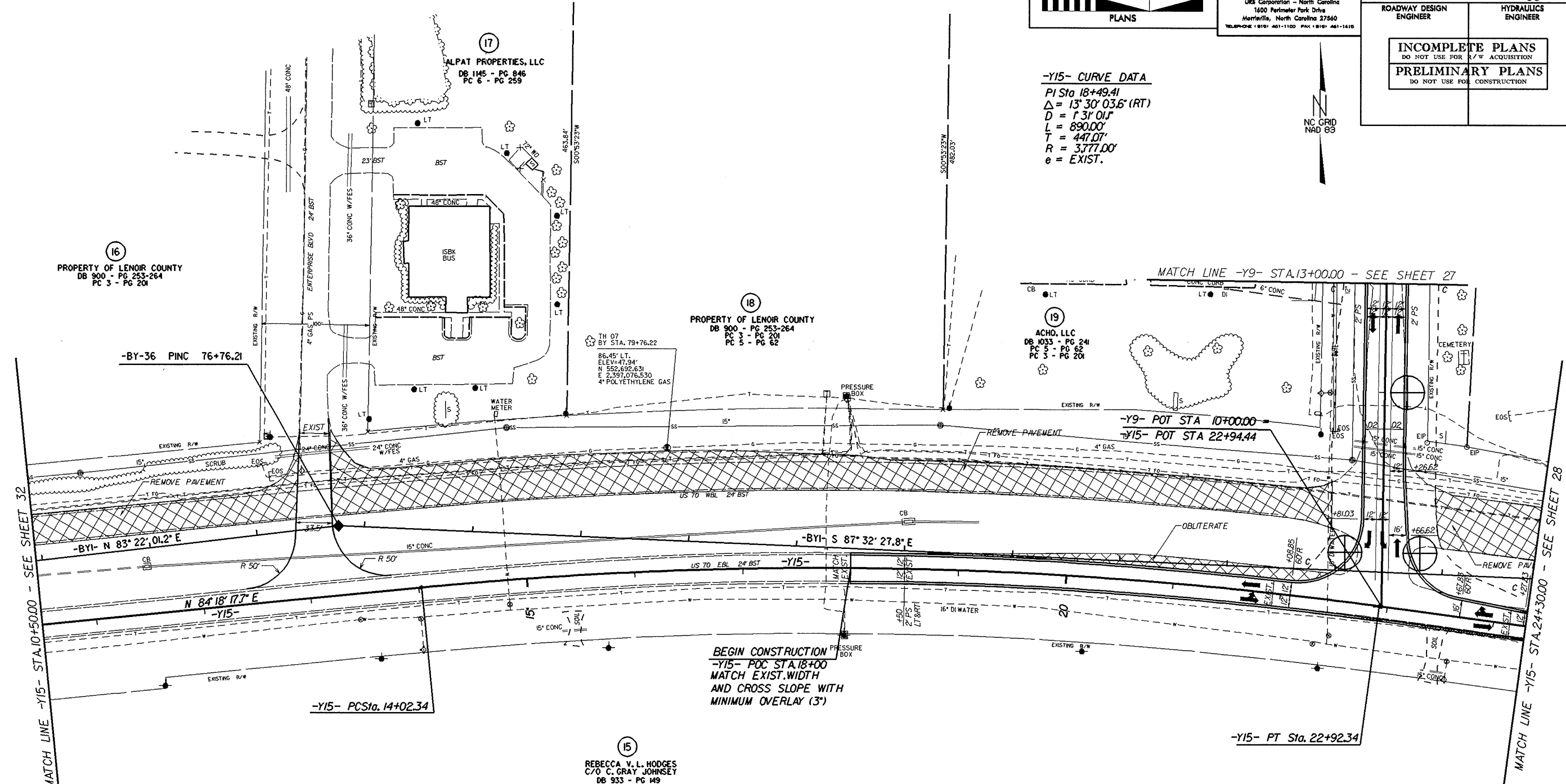
NOTE: FOR -Y15- PROFILE, SEE SHEET 73
FOR -Y9- PROFILE, SEE SHEET 70



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PROJECT REFERENCE NO. R-2719A	SHEET NO. 33
R/W SHEET NO. 33	
ROADWAY DESIGN ENGINEER	HYDRAULICS ENGINEER
INCOMPLETE PLANS DO NOT USE FOR R/W ACQUISITION PRELIMINARY PLANS DO NOT USE FOR CONSTRUCTION	

-Y15- CURVE DATA
 PI Sta 18+49.41
 $\Delta = 13^{\circ} 30' 03.6''$ (RT)
 D = 131.01'
 L = 890.00'
 T = 447.07'
 R = 3,777.00'
 e = EXIST.



MATCH LINE -Y15- STA.10+50.00 - SEE SHEET 32

MATCH LINE -Y15- STA.24+30.00 - SEE SHEET 28

MATCH LINE -Y9- STA.13+00.00 - SEE SHEET 27

BEGIN CONSTRUCTION
 -Y15- POC STA.18+00
 MATCH EXIST. WIDTH
 AND CROSS SLOPE WITH
 MINIMUM OVERLAY (3")

16
 PROPERTY OF LENOIR COUNTY
 DB 900 - PG 253-264
 PC 3 - PG 201

17
 ALPAT PROPERTIES, LLC
 DB 1145 - PG 846
 PC 6 - PG 259

18
 PROPERTY OF LENOIR COUNTY
 DB 900 - PG 253-264
 PC 3 - PG 201
 PC 5 - PG 62

19
 ACHO, LLC
 DB 1033 - PG 241
 PC 5 - PG 62
 PC 3 - PG 201

15
 REBECCA V.L. HODGES
 C/O C. GRAY JOHNSON
 DB 933 - PG 149

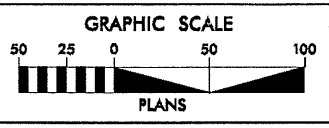
REVISIONS

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 kbell

-Y8- CURVE DATA
 PI Sta 15+04.43
 $\Delta = 58^{\circ} 42' 00.3" (RT)$
 $D = 14' 19" 26.2"$
 $L = 409.80'$
 $T = 224.93'$
 $R = 400.00'$
 $e = 6\%$

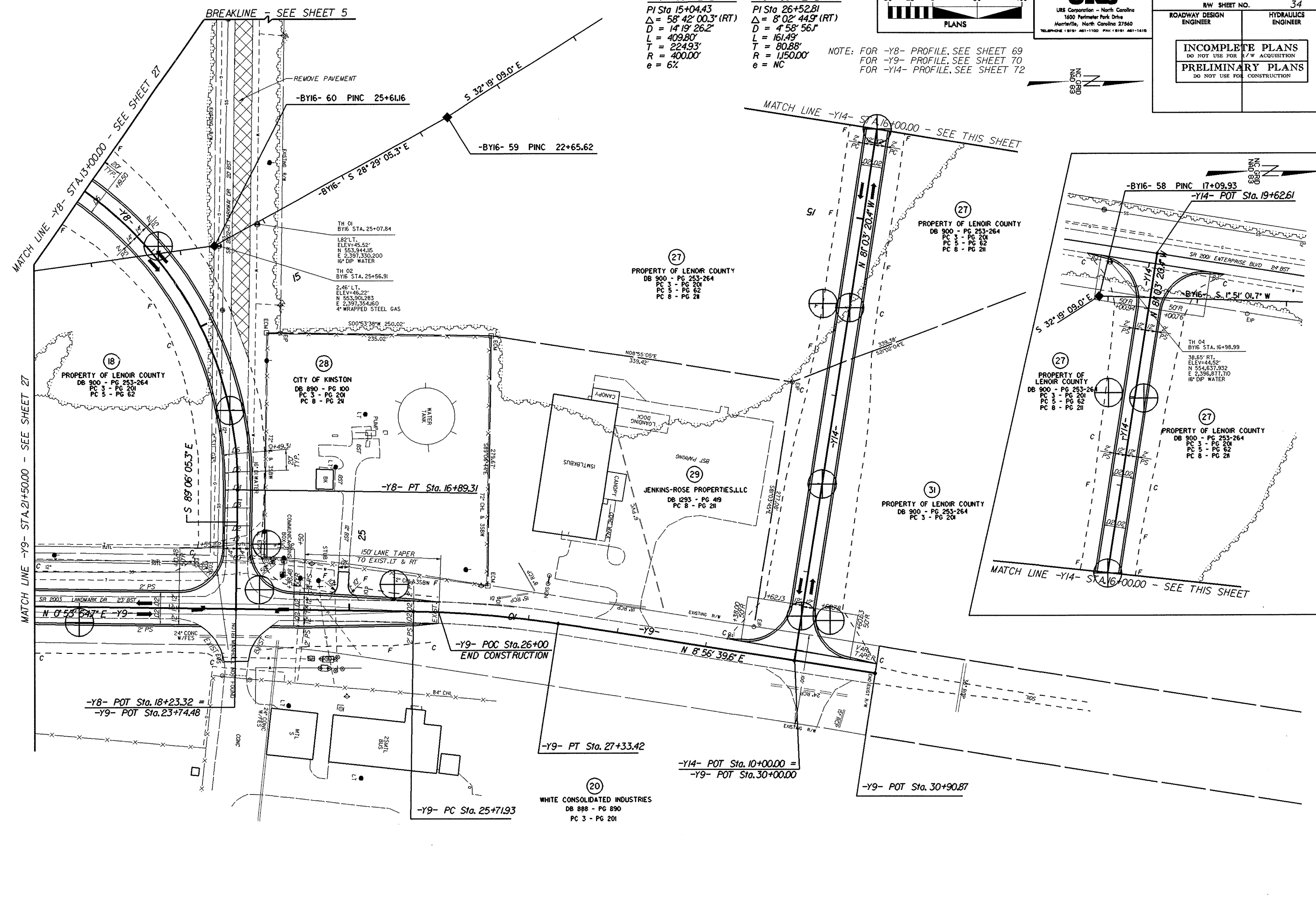
-Y9- CURVE DATA
 PI Sta 26+52.81
 $\Delta = 8^{\circ} 02' 44.9" (RT)$
 $D = 4' 58" 56.1"$
 $L = 161.49'$
 $T = 80.88'$
 $R = 1,150.00'$
 $e = NC$



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PROJECT REFERENCE NO.	SHEET NO.
R-2719A	34
R/W SHEET NO.	34
ROADWAY DESIGN ENGINEER	HYDRAULICS ENGINEER
INCOMPLETE PLANS	
DO NOT USE FOR R/W ACQUISITION	
PRELIMINARY PLANS	
DO NOT USE FOR CONSTRUCTION	

NOTE: FOR -Y8- PROFILE, SEE SHEET 69
 FOR -Y9- PROFILE, SEE SHEET 70
 FOR -Y14- PROFILE, SEE SHEET 72



REMOVE PAVEMENT
 -BY16- 60 PINC 25+61.16
 -BY16- 59 PINC 22+65.62

S 32°19'09.0"E
 S 28°29'05.3"E

PROPERTY OF LENOIR COUNTY
 DB 900 - PG 253-264
 PC 3 - PG 204
 PC 5 - PG 62
 PC 8 - PG 21

PROPERTY OF LENOIR COUNTY
 DB 890 - PG 100
 PC 3 - PG 204
 PC 5 - PG 62
 PC 8 - PG 21

PROPERTY OF LENOIR COUNTY
 DB 900 - PG 253-264
 PC 3 - PG 204
 PC 5 - PG 62
 PC 8 - PG 21

PROPERTY OF LENOIR COUNTY
 DB 900 - PG 253-264
 PC 3 - PG 204
 PC 5 - PG 62
 PC 8 - PG 21

PROPERTY OF LENOIR COUNTY
 DB 898 - PG 890
 PC 3 - PG 201

PROPERTY OF LENOIR COUNTY
 DB 900 - PG 253-264
 PC 3 - PG 204
 PC 5 - PG 62
 PC 8 - PG 21

PROPERTY OF LENOIR COUNTY
 DB 900 - PG 253-264
 PC 3 - PG 204
 PC 5 - PG 62
 PC 8 - PG 21

PROPERTY OF LENOIR COUNTY
 DB 900 - PG 253-264
 PC 3 - PG 204
 PC 5 - PG 62
 PC 8 - PG 21

PROPERTY OF LENOIR COUNTY
 DB 900 - PG 253-264
 PC 3 - PG 204
 PC 5 - PG 62
 PC 8 - PG 21

PROPERTY OF LENOIR COUNTY
 DB 900 - PG 253-264
 PC 3 - PG 204
 PC 5 - PG 62
 PC 8 - PG 21

-Y8- POT Sta. 18+23.32 =
 -Y9- POT Sta. 23+74.48

-Y9- POC Sta. 26+00
 END CONSTRUCTION

-Y14- POT Sta. 10+00.00 =
 -Y9- POT Sta. 30+00.00

-Y9- PC Sta. 25+71.93

WHITE CONSOLIDATED INDUSTRIES
 DB 888 - PG 890
 PC 3 - PG 201

-Y9- POT Sta. 30+90.87

TH 01
 BY16 STA. 25+07.84
 1.82' LT.
 ELEV: 45.52'
 N 553,944.15
 E 2,397,330.200
 16" DIP WATER

TH 02
 BY16 STA. 25+56.91
 2.46' LT.
 ELEV: 46.22'
 N 553,901.283
 E 2,397,354.160
 4" WRAPPED STEEL GAS

TH 04
 BY16 STA. 16+98.99
 38.65' RT.
 ELEV: 44.52'
 N 554,637.932
 E 2,396,871.710
 16" DIP WATER

(18)

(28)

(27)

(27)

(27)

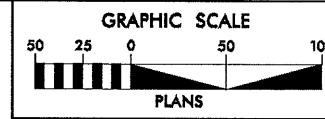
(27)

(31)

(20)

8/17/99

NOTE: FOR -SVRD5- PROFILE, SEE SHEET 87



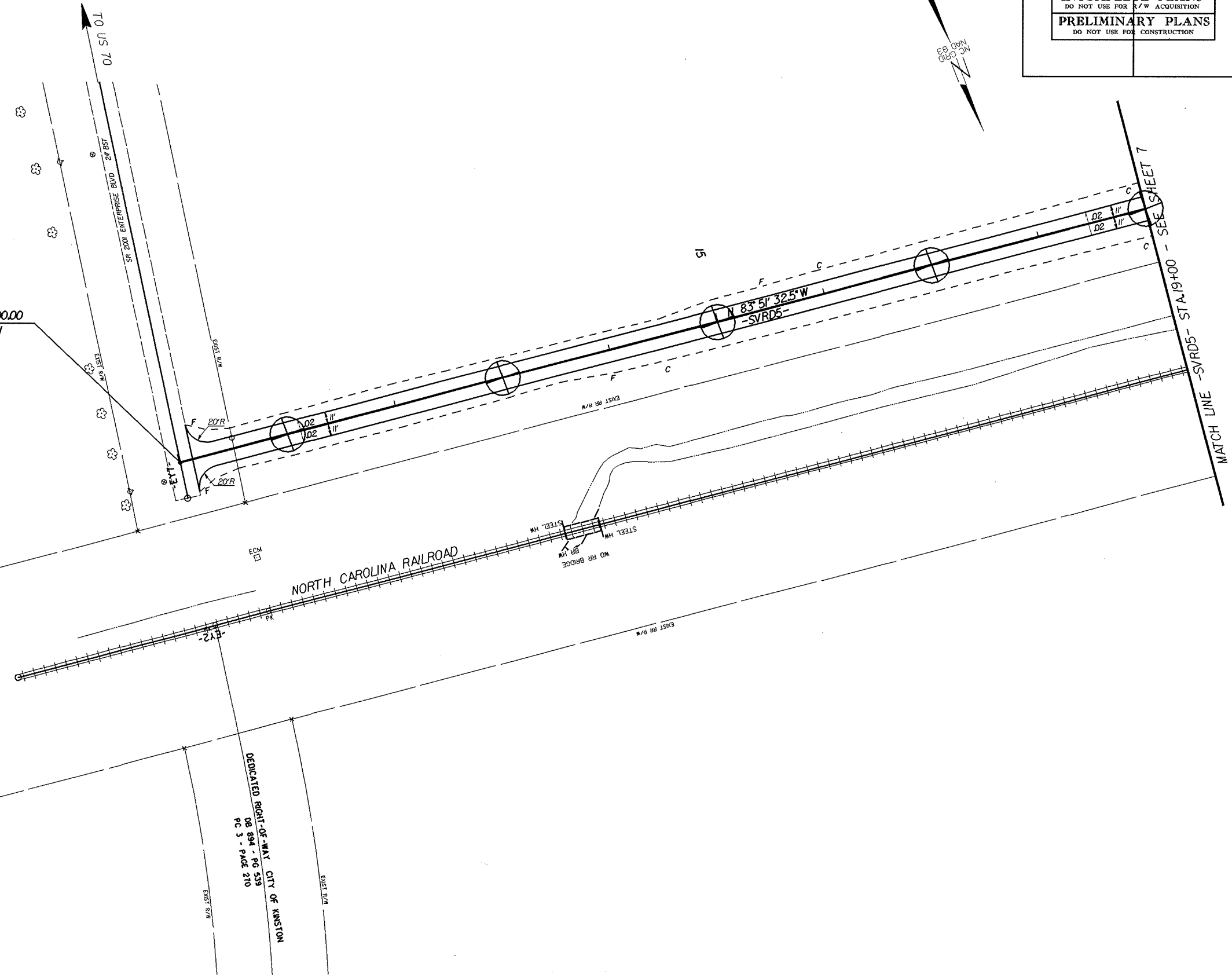
Prepared by
URS
 URS Corporation - North Carolina
 1600 Perimeter Park Drive
 Morrisville, North Carolina 27560
 TELEPHONE (919) 461-1100 FAX (919) 461-1418

PROJECT REFERENCE NO. R-2719A	SHEET NO. 35
R/W SHEET NO. 35	
ROADWAY DESIGN ENGINEER	HYDRAULICS ENGINEER
INCOMPLETE PLANS DO NOT USE FOR R/W ACQUISITION PRELIMINARY PLANS DO NOT USE FOR CONSTRUCTION	

REVISIONS

18-NOV-2005 08:57
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 kbmiller AT GEJ221488

-SVRD5- POTS 10+00.00
 BEGIN CONSTRUCTION



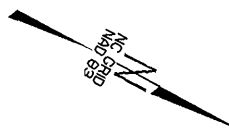
MATCH LINE -SVRD5- STA 19+00 -- SEE SHEET 7

8/17/99

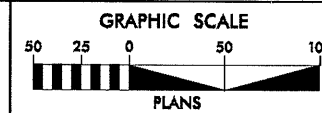
NOTE: FOR -Y10- PROFILE, SEE SHEET 71

-Y10- CURVE DATA

PI Sta 15+59.87
Δ = 9° 57' 31.2" (RT)
D = 3° 02' 51.5"
L = 326.77'
T = 163.80'
R = 1,880.00'
e = 5.3%



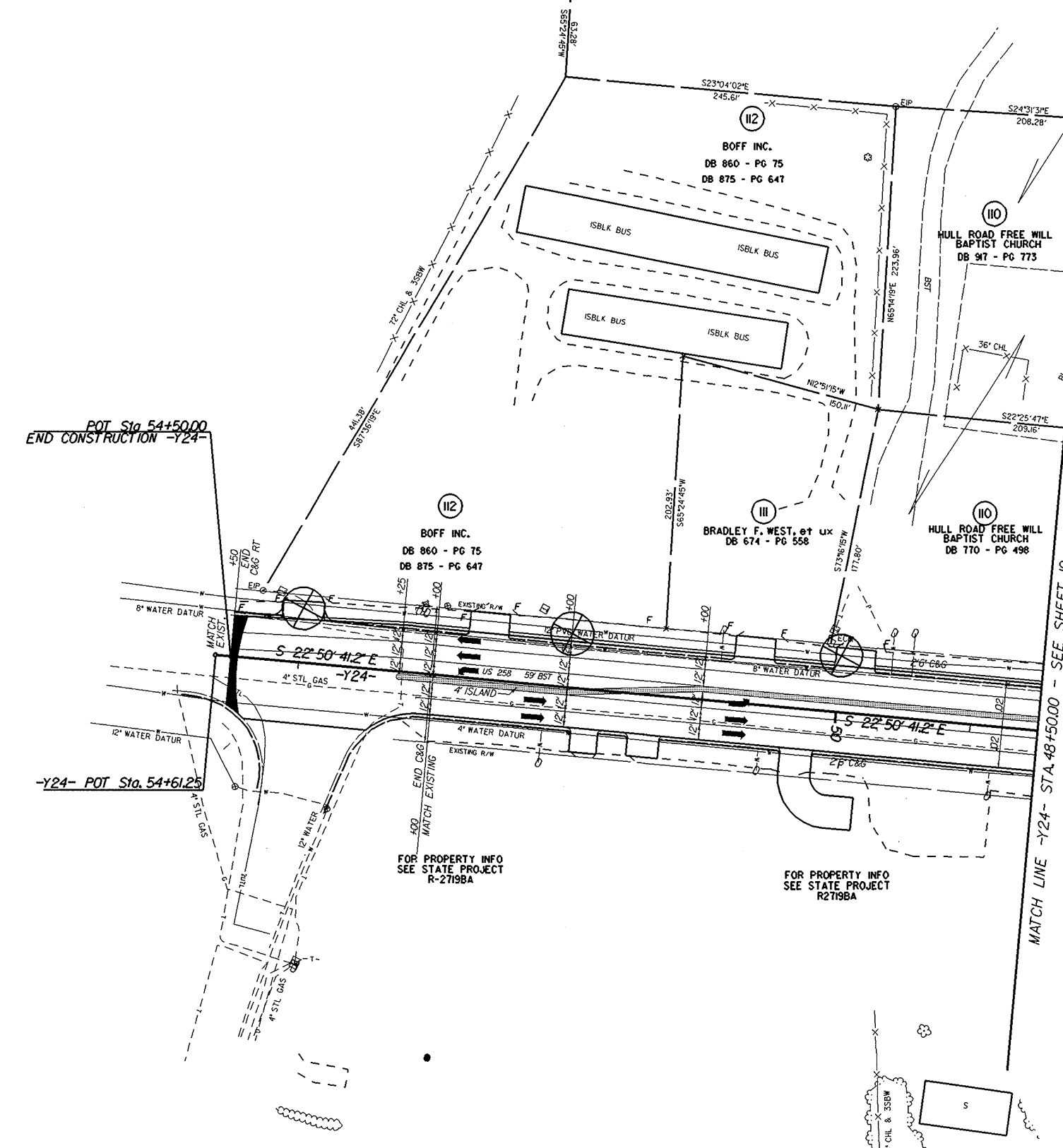
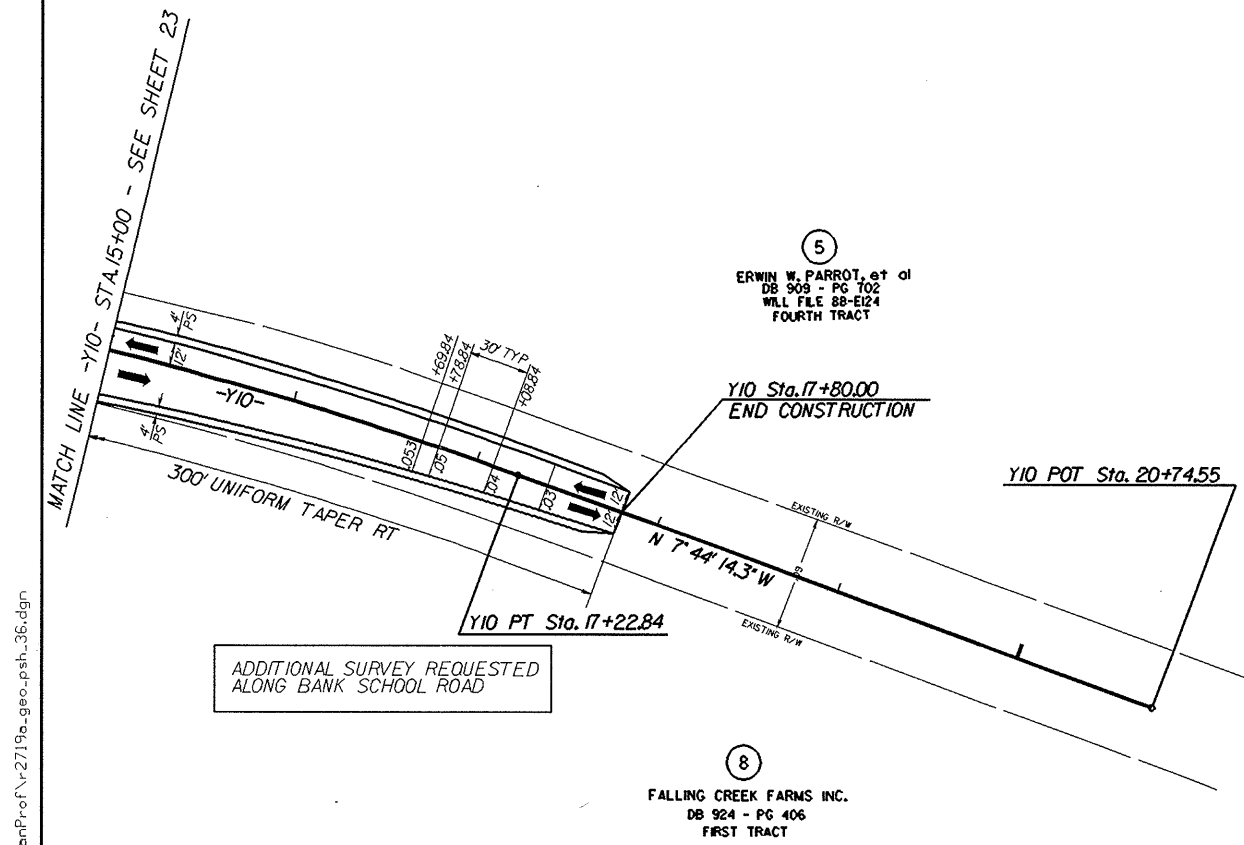
NOTE: FOR -Y24- PROFILE, SEE SHEET 80



Prepared by
URS
URS Corporation - North Carolina
1600 Perimeter Park Drive
Huntville, North Carolina 27540
TELEPHONE (919) 461-1100 FAX (919) 461-1410

PROJECT REFERENCE NO. R-2719A	SHEET NO. 36
R/W SHEET NO. 36	
ROADWAY DESIGN ENGINEER	HYDRAULICS ENGINEER
INCOMPLETE PLANS DO NOT USE FOR A/W ACQUISITION PRELIMINARY PLANS DO NOT USE FOR CONSTRUCTION	

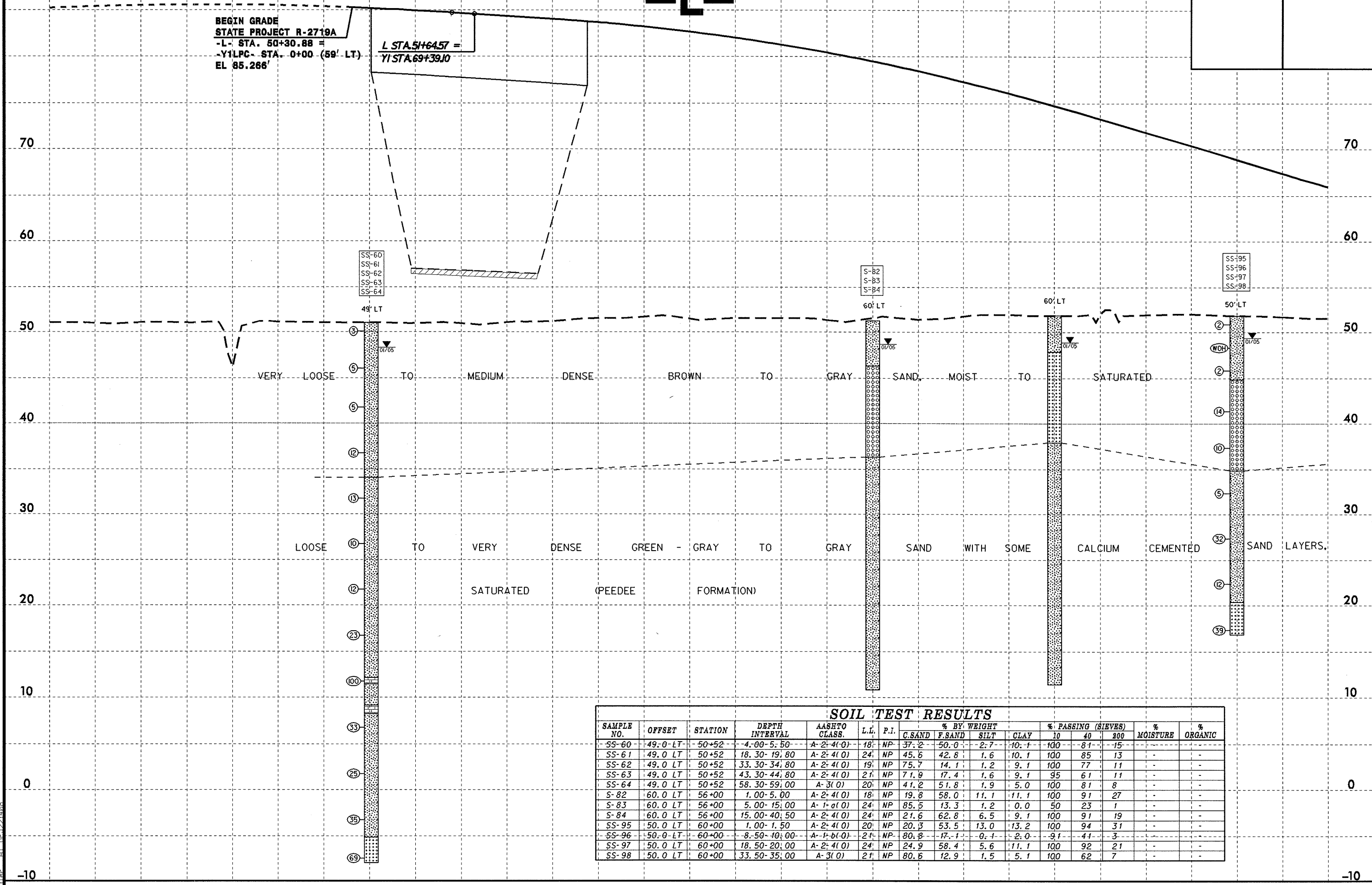
REVISIONS



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Kbmiller AT GEJ221408

5/14/99
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 Hamiller
 12/21/88

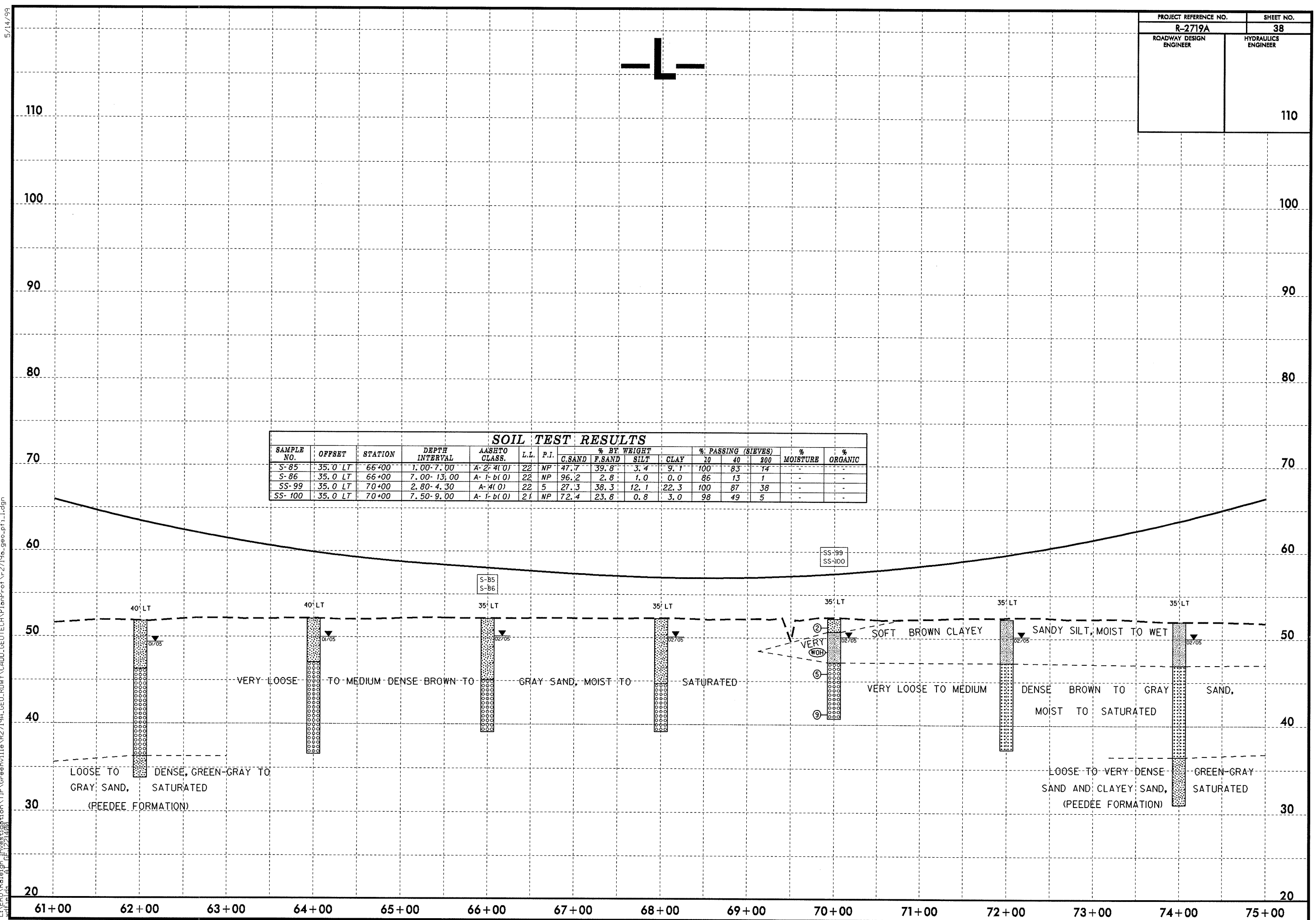
PROJECT REFERENCE NO.	SHEET NO.
R-2719A	37
ROADWAY DESIGN ENGINEER	HYDRAULICS ENGINEER



BEGIN GRADE
STATE PROJECT R-2719A
 -L- STA. 50+30.88 =
 -Y1LPC- STA. 0+00 (59' LT) EL 85.266'
 L STA. 51+64.57 =
 Y1 STA. 69+39.10

SAMPLE NO.	OFFSET	STATION	DEPTH INTERVAL	AASHTO CLASS.	L.L.	P.I.	% BY WEIGHT				% PASSING (SIEVES)			% MOISTURE	% ORGANIC
							C. SAND	F. SAND	SILT	CLAY	10	40	200		
SS-60	49.0 LT	50+52	4.00-5.50	A-2-4(0)	18	NP	37.2	50.0	2.7	10.1	100	81	15	-	-
SS-61	49.0 LT	50+52	18.30-19.80	A-2-4(0)	24	NP	45.6	42.8	1.6	10.1	100	85	13	-	-
SS-62	49.0 LT	50+52	33.30-34.80	A-2-4(0)	19	NP	75.7	14.1	1.2	9.1	100	77	11	-	-
SS-63	49.0 LT	50+52	43.30-44.80	A-2-4(0)	21	NP	71.9	17.4	1.6	9.1	95	61	11	-	-
SS-64	49.0 LT	50+52	58.30-59.00	A-3(0)	20	NP	41.2	51.8	1.9	5.0	100	81	8	-	-
S-82	60.0 LT	56+00	1.00-5.00	A-2-4(0)	18	NP	19.8	58.0	11.1	11.1	100	91	27	-	-
S-83	60.0 LT	56+00	5.00-15.00	A-1-0(0)	24	NP	85.5	13.3	1.2	0.0	50	23	1	-	-
S-84	60.0 LT	56+00	15.00-40.50	A-2-4(0)	24	NP	21.6	62.8	6.5	9.1	100	91	19	-	-
SS-95	50.0 LT	60+00	1.00-1.50	A-2-4(0)	20	NP	20.3	53.5	13.0	13.2	100	94	31	-	-
SS-96	50.0 LT	60+00	8.50-10.00	A-1-0(0)	21	NP	80.8	17.1	0.1	2.0	91	41	3	-	-
SS-97	50.0 LT	60+00	18.50-20.00	A-2-4(0)	24	NP	24.9	58.4	5.6	11.1	100	92	21	-	-
SS-98	50.0 LT	60+00	33.50-35.00	A-3(0)	21	NP	80.6	12.9	1.5	5.1	100	62	7	-	-

SOIL TEST RESULTS															
SAMPLE NO.	OFFSET	STATION	DEPTH INTERVAL	AASHTO CLASS.	L.L.	P.I.	% BY WEIGHT				% PASSING (SIEVES)			% MOISTURE	% ORGANIC
							C.SAND	F.SAND	SILT	CLAY	10	40	200		
S-85	35.0 LT	66+00	1.00-7.00	A-2-4(0)	22	NP	47.7	39.8	3.4	9.1	100	83	14	-	-
S-86	35.0 LT	66+00	7.00-13.00	A-1-b(0)	22	NP	96.2	2.8	1.0	0.0	86	13	1	-	-
SS-99	35.0 LT	70+00	2.80-4.30	A-4(0)	22	5	27.3	38.3	12.1	22.3	100	87	38	-	-
SS-100	35.0 LT	70+00	7.50-9.00	A-1-b(0)	21	NP	72.4	23.8	0.8	3.0	98	49	5	-	-



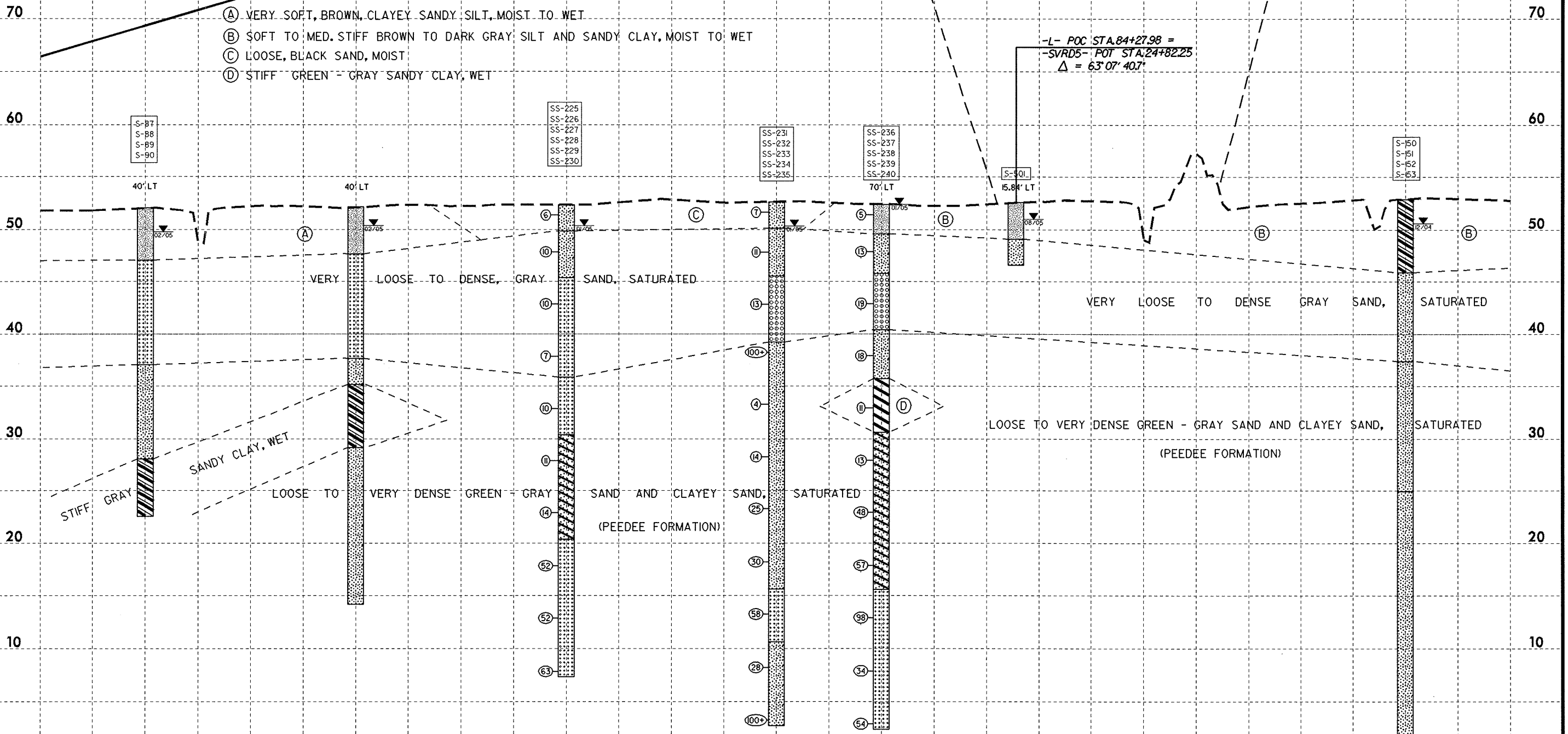
5/14/99
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 12/21/00

5/14/99
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 11/11/98

PROJECT REFERENCE NO.	SHEET NO.
R-2719A	39
ROADWAY DESIGN ENGINEER	HYDRAULICS ENGINEER

SOIL TEST RESULTS															
SAMPLE NO.	OFFSET	STATION	DEPTH INTERVAL	AASHTO CLASS.	L.L.	P.I.	% BY WEIGHT				% PASSING (SIEVES)			% MOISTURE	% ORGANIC
							C.SAND	F.SAND	SILT	CLAY	10	40	200		
S-87	40.0 LT	76+00	1.00-5.00	A-4(0)	32	NP	21.5	41.7	17.7	19.2	100	92	41	-	-
S-88	40.0 LT	76+00	5.00-15.00	A-3(0)	23	NP	56.1	40.2	1.7	2.0	98	71	4	-	-
S-89	40.0 LT	76+00	15.00-24.00	A-2-4(0)	20	NP	50.9	36.5	4.5	8.1	100	81	14	-	-
S-90	40.0 LT	76+00	24.00-29.50	A-6(6)	32	13	12.7	32.3	18.7	36.3	100	95	62	33.9	-
SS-225	CL	80+00	0.00-1.50	A-2-4(0)	32	NP	18.8	50.5	12.5	18.2	100	85	33	-	-
SS-226	CL	80+00	3.50-5.00	A-2-4(0)	25	4	46.1	32.5	3.2	18.2	98	74	22	-	-
SS-227	CL	80+00	8.50-10.00	A-3(0)	19	NP	73.8	19.9	0.3	6.0	95	56	7	-	-
SS-228	CL	80+00	18.50-20.00	A-3(0)	22	NP	42.3	48.5	0.1	9.2	95	74	10	-	-
SS-229	CL	80+00	23.50-25.00	A-2-6(0)	33	12	22.1	52.1	6.8	19.2	100	88	31	-	-
SS-230	CL	80+00	32.50-35.00	A-3(0)	20	NP	52.2	43.8	0.3	4.0	100	85	5	-	-
SS-231	CL	82+00	8.80-10.30	A-1-b(0)	23	NP	82.9	14.2	2.9	0.0	87	36	4	-	-
SS-232	CL	82+00	13.40-14.90	A-2-4(0)	25	NP	36.9	45.2	5.8	12.1	100	86	20	-	-
SS-233	CL	82+00	38.40-39.90	A-3(0)	20	NP	76.5	14.5	4.0	5.0	100	82	10	-	-
SS-234	CL	82+00	43.40-44.90	A-2-4(0)	27	NP	25.2	67.8	6.0	1.0	100	86	17	-	-
SS-235	CL	82+00	48.40-49.90	A-2-4(0)	23	NP	35.3	51.8	4.0	9.1	100	91	14	-	-

- (A) VERY SOFT, BROWN, CLAYEY SANDY SILT, MOIST TO WET
- (B) SOFT TO MED. STIFF BROWN TO DARK GRAY SILT AND SANDY CLAY, MOIST TO WET
- (C) LOOSE, BLACK SAND, MOIST
- (D) STIFF GREEN - GRAY SANDY CLAY, WET



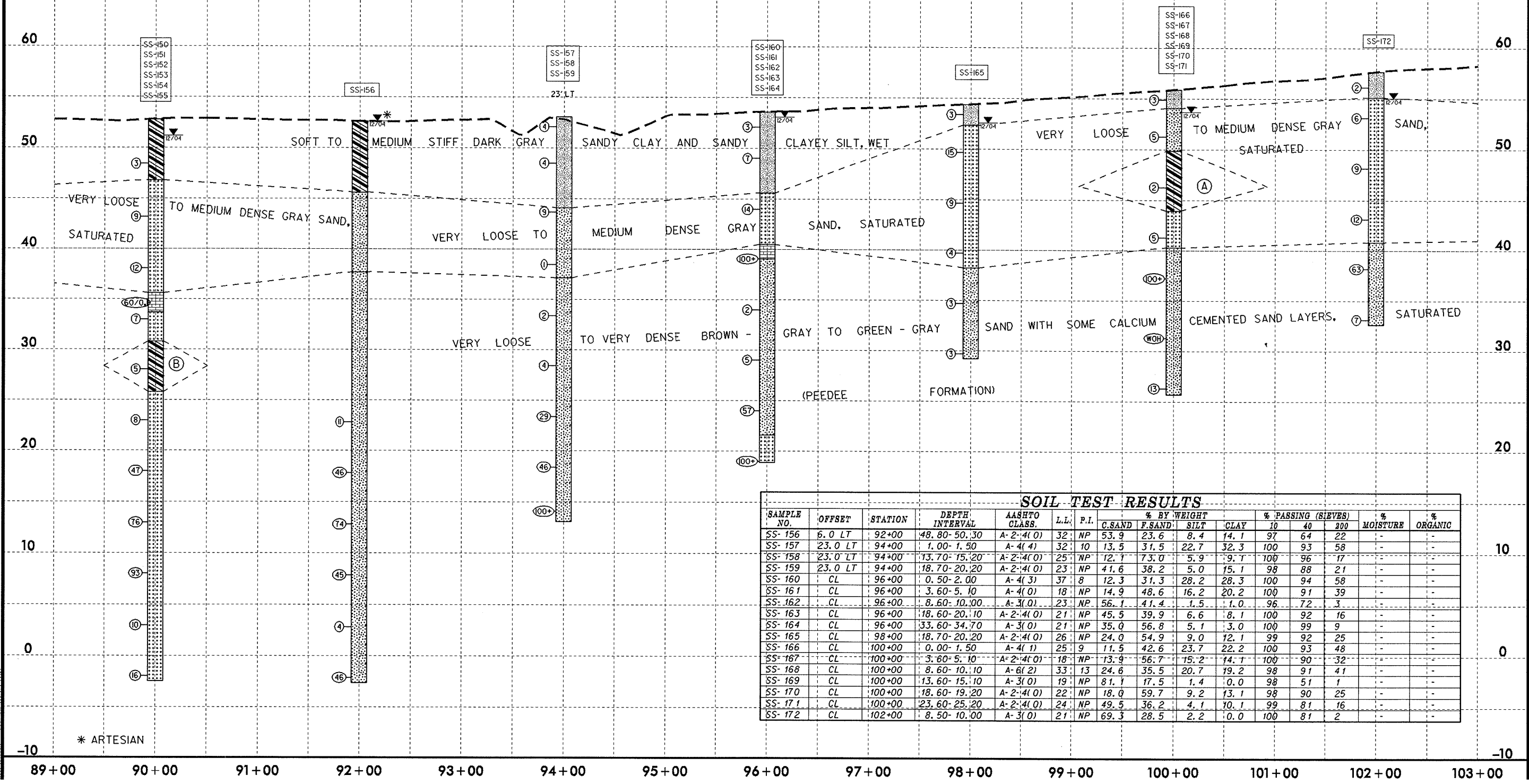
SOIL TEST RESULTS															
SAMPLE NO.	OFFSET	STATION	DEPTH INTERVAL	AASHTO CLASS.	L.L.	P.I.	% BY WEIGHT				% PASSING (SIEVES)			% MOISTURE	% ORGANIC
							C.SAND	F.SAND	SILT	CLAY	10	40	200		
SS-236	70.0 LT	83+00	0.00-1.50	A-4(0)	40	NP	24.1	38.2	27.6	10.1	100	84	40	-	-
SS-237	70.0 LT	83+00	3.50-5.00	A-2-4(0)	21	NP	44.5	35.4	8.0	12.1	99	74	22	-	-
SS-238	70.0 LT	83+00	18.50-20.00	A-7-6(4)	44	26	44.4	21.5	12.0	22.1	100	78	38	26.3	-
SS-239	70.0 LT	83+00	23.50-25.00	A-2-6(1)	37	16	20.9	50.3	10.8	18.1	98	85	33	-	-
SS-240	70.0 LT	83+00	38.50-40.00	A-3(0)	19	NP	78.8	13.5	2.7	5.0	100	73	10	-	-
S-501	CL	25+00	0.00-3.50	A-4(0)	34	8	38.0	28.5	23.4	10.1	100	81	36	45	-

SOIL TEST RESULTS															
SAMPLE NO.	OFFSET	STATION	DEPTH INTERVAL	AASHTO CLASS.	L.L.	P.I.	% BY WEIGHT				% PASSING (SIEVES)			% MOISTURE	% ORGANIC
							C.SAND	F.SAND	SILT	CLAY	10	40	200		
S-150	CL	88+00	1.00-7.00	A-6(4)	37	12	18.6	29.5	19.7	32.3	100	90	54	42.1	-
S-151	CL	88+00	7.00-15.50	A-2-4(0)	15	NP	62.2	23.6	8.2	6.1	94	52	15	-	-
S-152	CL	88+00	15.50-28.00	A-2-4(0)	23	5	25.4	44.6	9.8	20.2	99	87	32	-	-
S-153	CL	88+00	28.00-53.00	A-2-4(0)	19	NP	38.0	48.2	5.7	8.1	100	93	15	-	-

75+00 76+00 77+00 78+00 79+00 80+00 81+00 82+00 83+00 84+00 85+00 86+00 87+00 88+00 89+00

SAMPLE NO.	OFFSET	STATION	DEPTH INTERVAL	AASHTO CLASS.	L.L.	P.I.	% BY WEIGHT				% PASSING (SIEVES)			% MOISTURE	% ORGANIC
							C.SAND	F.SAND	SILT	CLAY	10	40	200		
SS-150	CL	90+00	3.40-4.90	A-6(3)	27	14	25.2	31.5	15.0	28.3	100	87	47	-	-
SS-151	CL	90+00	8.90-10.10	A-3(0)	24	NP	59.1	37.2	0.6	3.0	99	70	4	-	-
SS-152	CL	90+00	23.80-25.30	A-6(1)	37	15	26.5	41.6	11.7	20.2	99	90	37	34.0	-
SS-153	CL	90+00	33.80-35.30	A-3(0)	21	NP	61.8	30.0	3.2	5.0	100	85	9	-	-
SS-154	CL	90+00	43.80-45.30	A-3(0)	23	NP	56.8	36.6	2.5	4.0	100	94	8	-	-
SS-155	CL	90+00	48.80-50.30	A-3(0)	21	NP	81.6	10.5	1.8	6.1	100	63	8	-	-

- (A) SOFT GRAY SILTY SANDY CLAY, WET
- (B) MEDIUM STIFF GRAY SANDY CLAY, WET



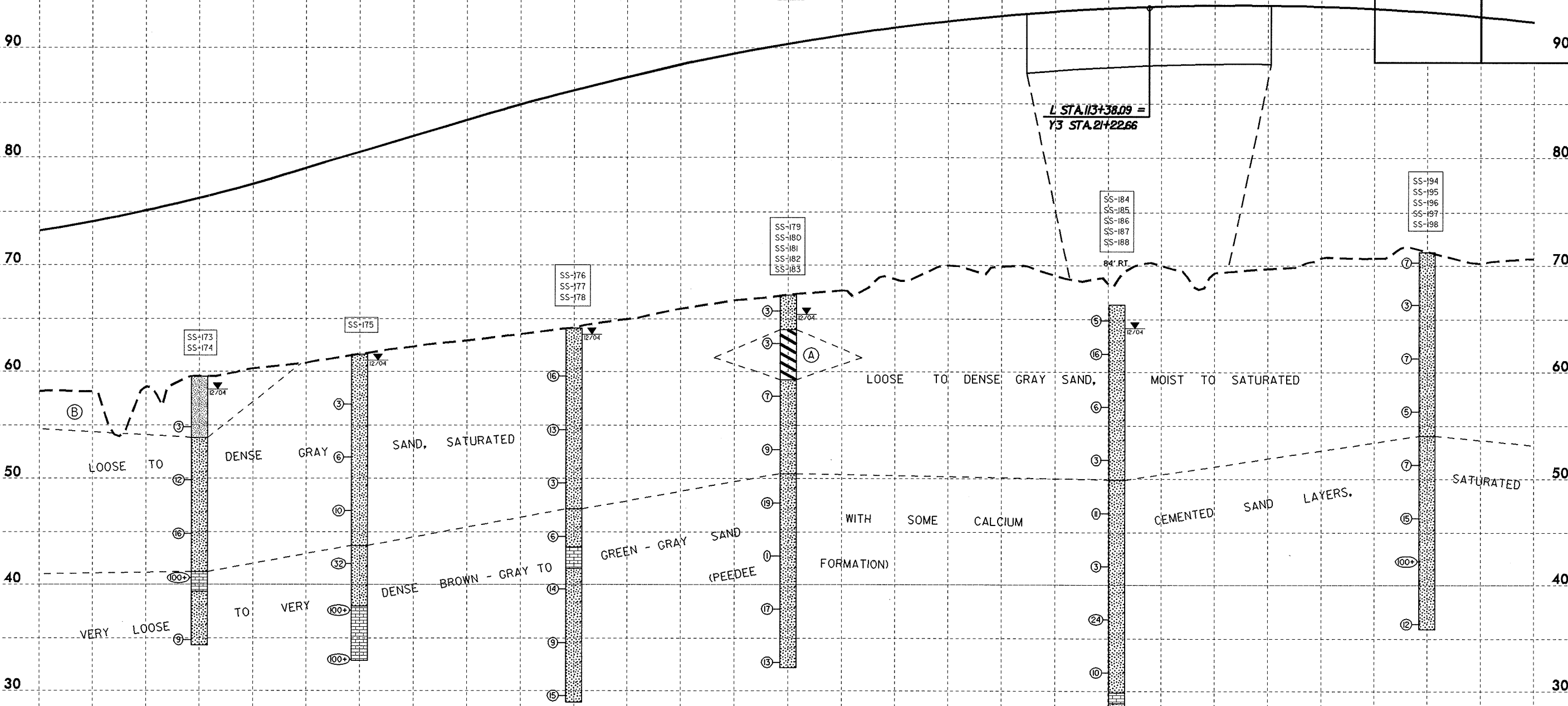
SAMPLE NO.	OFFSET	STATION	DEPTH INTERVAL	AASHTO CLASS.	L.L.	P.I.	% BY WEIGHT				% PASSING (SIEVES)			% MOISTURE	% ORGANIC
							C.SAND	F.SAND	SILT	CLAY	10	40	200		
SS-156	6.0 LT	92+00	48.80-50.30	A-2-4(0)	32	NP	53.9	23.6	8.4	14.1	97	64	22	-	-
SS-157	23.0 LT	94+00	1.00-1.50	A-4(4)	32	10	13.5	31.5	22.7	32.3	100	93	58	-	-
SS-158	23.0 LT	94+00	13.70-15.20	A-2-4(0)	25	NP	12.1	73.0	5.9	9.1	100	96	17	-	-
SS-159	23.0 LT	94+00	18.70-20.20	A-2-4(0)	23	NP	41.6	38.2	5.0	15.1	98	88	21	-	-
SS-160	CL	96+00	0.50-2.00	A-4(3)	37	8	12.3	31.3	28.2	28.3	100	94	58	-	-
SS-161	CL	96+00	3.60-5.10	A-4(0)	18	NP	14.9	48.6	16.2	20.2	100	91	39	-	-
SS-162	CL	96+00	8.60-10.00	A-3(0)	23	NP	56.1	41.4	1.5	1.0	96	72	3	-	-
SS-163	CL	96+00	18.60-20.10	A-2-4(0)	21	NP	45.5	39.9	6.6	8.1	100	92	16	-	-
SS-164	CL	96+00	33.60-34.70	A-3(0)	21	NP	35.0	56.8	5.1	3.0	100	99	9	-	-
SS-165	CL	98+00	18.70-20.20	A-2-4(0)	26	NP	24.0	54.9	9.0	12.1	99	92	25	-	-
SS-166	CL	100+00	0.00-1.50	A-4(1)	25	9	11.5	42.6	23.7	22.2	100	93	48	-	-
SS-167	CL	100+00	3.60-5.10	A-2-4(0)	18	NP	13.9	56.7	15.2	14.1	100	90	32	-	-
SS-168	CL	100+00	8.60-10.10	A-6(2)	33	13	24.6	35.5	20.7	19.2	98	91	41	-	-
SS-169	CL	100+00	13.60-15.10	A-3(0)	19	NP	81.1	17.5	1.4	0.0	98	51	1	-	-
SS-170	CL	100+00	18.60-19.20	A-2-4(0)	22	NP	18.0	59.7	9.2	13.1	98	90	25	-	-
SS-171	CL	100+00	23.60-25.20	A-2-4(0)	24	NP	49.5	36.2	4.1	10.1	99	81	16	-	-
SS-172	CL	102+00	8.50-10.00	A-3(0)	21	NP	69.3	28.5	2.2	0.0	100	81	2	-	-

5/14/99
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5/14/99

PROJECT REFERENCE NO. R-2719A	SHEET NO. 41
ROADWAY DESIGN ENGINEER	HYDRAULICS ENGINEER

- (A) SOFT, ORANGE TO GRAY SANDY CLAY, WET
- (B) SOFT TO MEDIUM STIFF DARK GRAY SANDY CLAY AND SANDY CLAYEY SILT, WET



SAMPLE NO.	OFFSET	STATION	DEPTH INTERVAL	AASHTO CLASS.	L.L.	P.I.	% BY WEIGHT				% PASSING (SIEVES)			% MOISTURE	% ORGANIC
							C.SAND	F.SAND	SILT	CLAY	10	40	200		
SS-173	CL	104+50	8.70-10.20	A-2-4(0)	21	NP	20.0	67.5	7.5	5.0	100	99	14	-	-
SS-174	CL	104+50	23.70-25.20	A-2-4(0)	23	NP	29.5	48.9	7.5	14.1	100	91	25	-	-
SS-175	CL	106+00	3.70-5.20	A-2-4(0)	31	NP	41.4	42.7	12.9	3.0	100	82	18	-	-
SS-176	CL	108+00	18.60-20.20	A-2-4(0)	25	NP	11.1	69.2	7.6	12.1	100	97	23	-	-
SS-177	CL	108+00	28.60-30.20	A-2-4(0)	22	NP	38.0	43.0	5.9	13.1	100	84	21	-	-
SS-178	CL	108+00	33.60-35.20	A-2-4(0)	22	NP	44.1	41.3	4.5	10.1	100	89	16	-	-
SS-179	CL	110+00	0.50-2.00	A-2-4(0)	21	NP	30.1	50.1	13.8	6.1	100	85	23	-	-
SS-180	CL	110+00	-3.50-5.00	A-7-6(8)	53	25	15.3	42.0	10.4	32.3	100	95	47	-	-
SS-181	CL	110+00	8.50-10.00	A-2-4(0)	25	NP	12.9	65.0	8.0	14.1	100	96	26	-	-
SS-182	CL	110+00	23.50-25.00	A-2-4(0)	23	NP	28.6	54.9	5.4	11.1	100	89	18	-	-
SS-183	CL	110+00	33.50-35.00	A-2-4(0)	24	NP	31.5	48.8	5.5	14.1	100	85	22	-	-
SS-184	84.0 RT	113+08	28.60-30.10	A-2-4(0)	23	NP	6.9	76.1	6.0	11.1	100	98	19	-	-
SS-185	84.0 RT	113+08	38.60-40.10	A-2-4(0)	23	NP	46.1	37.7	6.1	10.1	100	87	17	-	-
SS-186	84.0 RT	113+08	63.60-65.10	A-2-4(0)	24	NP	61.4	20.0	6.6	12.1	100	88	20	-	-
SS-187	84.0 RT	113+08	73.60-75.10	A-2-4(0)	25	NP	30.8	62.0	6.3	1.0	100	83	13	-	-
SS-188	84.0 RT	113+08	78.60-80.10	A-4(0)	28	NP	2.1	73.3	12.5	12.1	100	99	44	-	-
SS-194	CL	116+00	0.0-1.5	A-2-4(0)	23	NP	29.3	62.5	4.1	4.0	100	80	11	-	-
SS-195	CL	116+00	9.5-10.4	A-2-4(0)	21	2	15.8	59.5	10.7	14.1	100	90	28	-	-
SS-196	CL	116+00	13.9-15.4	A-2-4(0)	31	NP	12.2	64.7	9.0	14.1	100	95	30	-	-
SS-197	CL	116+00	18.9-20.4	A-2-4(0)	24	NP	39.2	43.3	6.4	11.1	100	92	19	-	-
SS-198	CL	116+00	33.9-35.4	A-2-4(0)	27	NP	10.1	66.2	9.8	14.1	100	94	29	-	-

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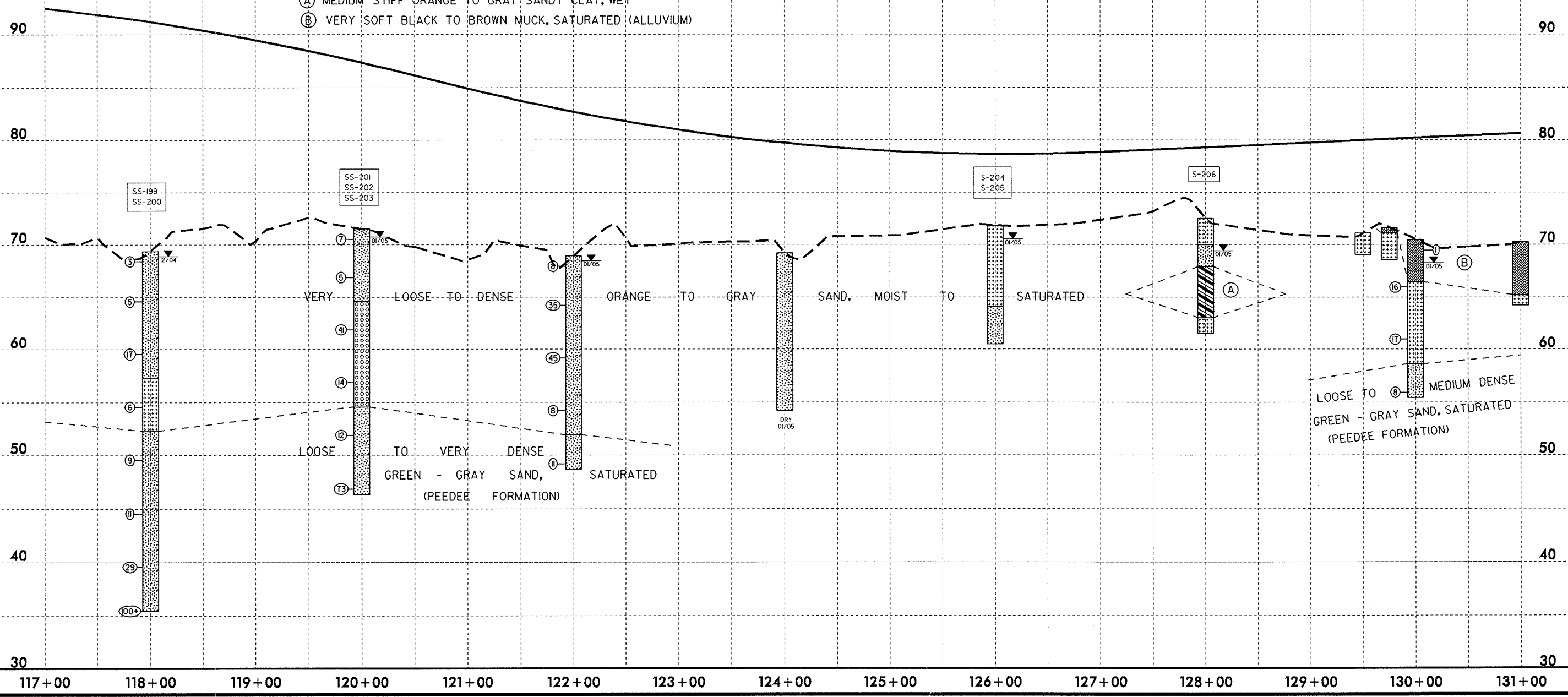
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PROJECT REFERENCE NO.		SHEET NO.
R-2719A		42
ROADWAY DESIGN ENGINEER	HYDRAULICS ENGINEER	
		120

SOIL TEST RESULTS

SAMPLE NO.	OFFSET	STATION	DEPTH INTERVAL	AASHTO CLASS.	L.L.	P.I.	% BY WEIGHT				% PASSING (SIEVES)			% MOISTURE	% ORGANIC
							G.SAND	F.SAND	SILT	CLAY	10	40	200		
SS-199	CL	118+00	13.8-15.3	A-3(0)	24	NP	7.2	87.1	3.6	2.0	100	99	71	-	-
SS-200	CL	118+00	18.80-20.30	A-2-4(0)	27	NP	4.9	75.4	5.6	16.1	99	96	21	-	-
SS-201	CL	120+00	0.00-1.50	A-2-4(0)	25	NP	23.3	64.7	0.0	12.0	100	89	14	-	-
SS-202	CL	120+00	8.60-10.10	A-1-b(0)	19	NP	61.6	31.9	0.4	6.0	74	39	6	-	-
SS-203	CL	120+00	18.60-20.10	A-2-4(0)	29	6	18.9	58.0	5.0	18.1	99	95	25	-	-
S-204	CL	126+00	4.00-5.50	A-3(0)	22	NP	27.6	63.9	0.5	8.0	100	86	9	-	-
S-205	CL	126+00	9.80-11.00	A-2-4(0)	19	NP	12.8	66.6	1.6	19.1	100	95	22	-	-
S-206	CL	128+00	5.00-6.50	A-6(2)	33	14	7.3	56.0	3.5	33.1	100	97	39	-	-

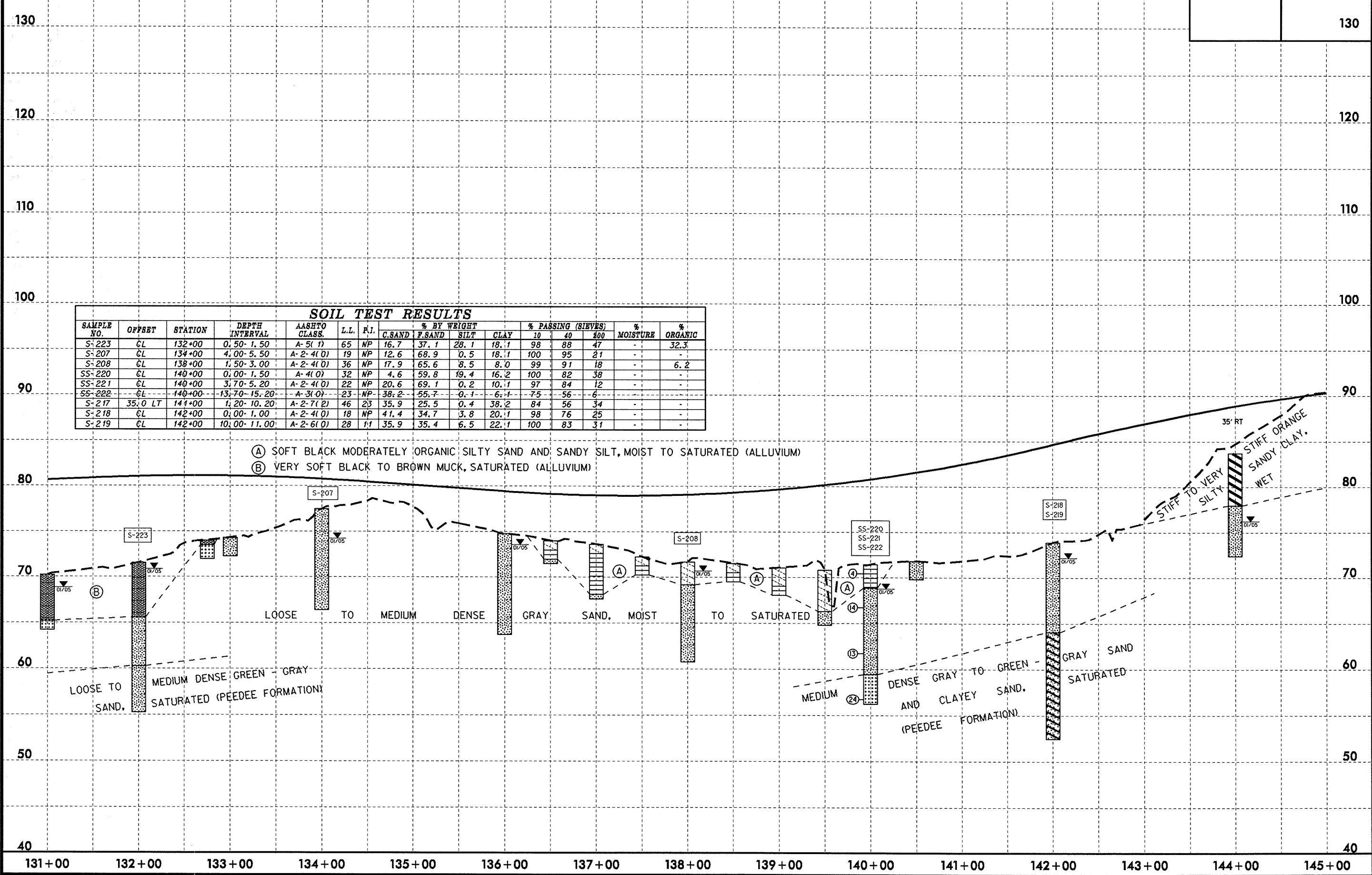
- (A) MEDIUM STIFF ORANGE TO GRAY SANDY CLAY, WET
- (B) VERY SOFT BLACK TO BROWN MUCK, SATURATED (ALLUVIUM)



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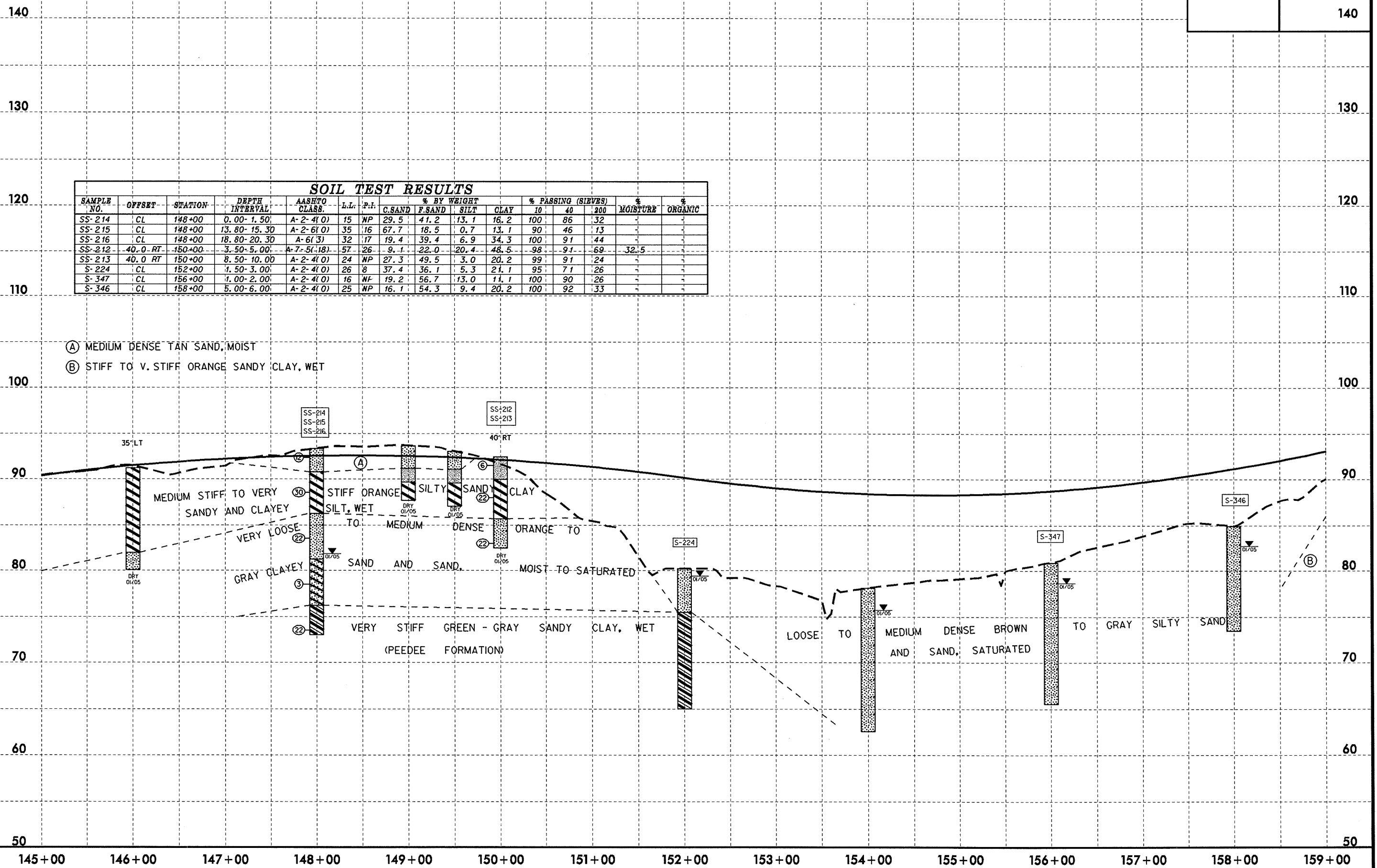
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PROJECT REFERENCE NO. R-2719A	SHEET NO. 43
ROADWAY DESIGN ENGINEER	HYDRAULICS ENGINEER
130	



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PROJECT REFERENCE NO. R-2719A	SHEET NO. 44
ROADWAY DESIGN ENGINEER	HYDRAULICS ENGINEER



SOIL TEST RESULTS

SAMPLE NO.	OFFSET	STATION	DEPTH INTERVAL	AASHTO CLASS.	L.L.	P.I.	% BY WEIGHT				% PASSING (SIEVES)			% MOISTURE	% ORGANIC
							C.SAND	F.SAND	SILT	CLAY	10	40	200		
SS-214	CL	148+00	0.00-1.50	A-2-4(0)	15	NP	29.5	41.2	13.1	16.2	100	86	32	-	-
SS-215	CL	148+00	13.80-15.30	A-2-6(0)	35	16	67.7	18.5	0.7	13.1	90	46	13	-	-
SS-216	CL	148+00	18.80-20.30	A-6(3)	32	17	19.4	39.4	6.9	34.3	100	91	44	-	-
SS-212	40.0-RT	150+00	3.50-5.00	A-7-5(18)	57	26	9.1	22.0	20.4	48.5	98	91	69	32.5	-
SS-213	40.0 RT	150+00	8.50-10.00	A-2-4(0)	24	NP	27.3	49.5	3.0	20.2	99	91	24	-	-
S-224	CL	152+00	1.50-3.00	A-2-4(0)	26	8	37.4	36.1	5.3	21.1	95	71	26	-	-
S-347	CL	156+00	1.00-2.00	A-2-4(0)	16	WF	19.2	56.7	13.0	11.1	100	90	26	-	-
S-346	CL	158+00	5.00-6.00	A-2-4(0)	25	WP	16.1	54.3	9.4	20.2	100	92	33	-	-

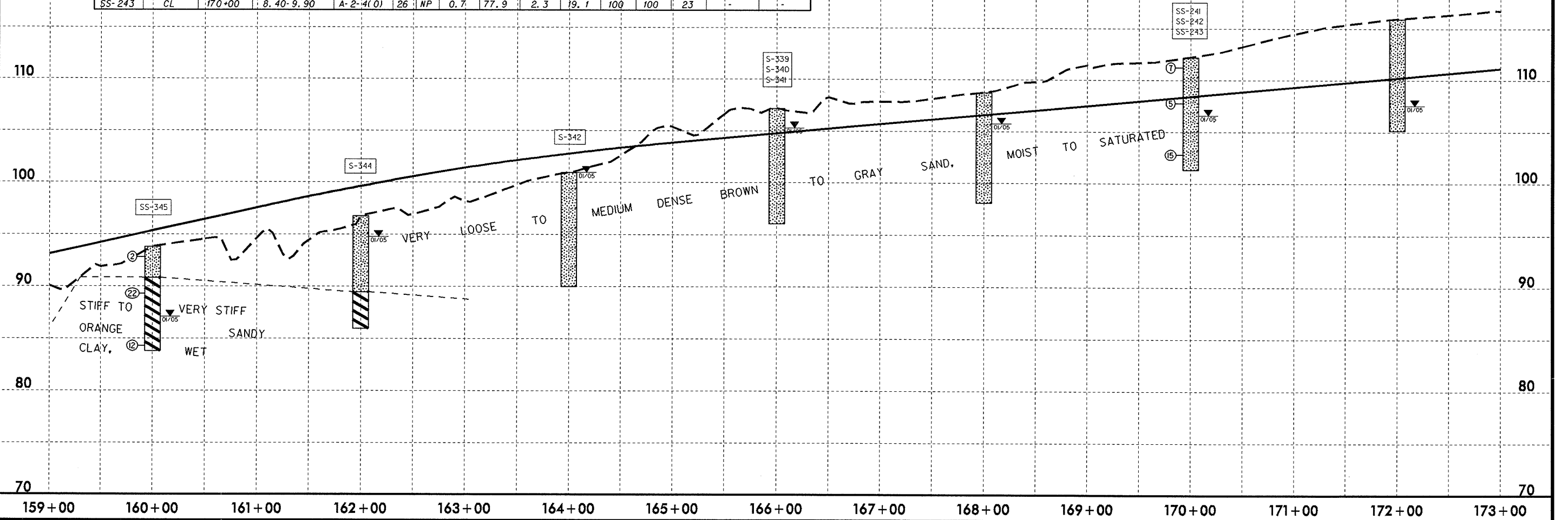
- (A) MEDIUM DENSE TAN SAND, MOIST
- (B) STIFF TO V. STIFF ORANGE SANDY CLAY, WET

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PROJECT REFERENCE NO.	SHEET NO.
R-2719A	45
ROADWAY DESIGN ENGINEER	HYDRAULICS ENGINEER
	160

SOIL TEST RESULTS															
SAMPLE NO.	OFFSET	STATION	DEPTH INTERVAL	AASHTO CLASS.	L.L.	P.I.	% BY WEIGHT				% PASSING (SIEVES)			% MOISTURE	% ORGANIC
							C.SAND	F.SAND	SILT	CLAY	10	40	200		
SS-345	CL	160+00	3.50-5.00	A-7-6(3)	41	17	20.2	41.2	4.2	34.3	100	89	40	46.9	-
S-344	CL	162+00	9.00-10.00	A-7-6(12)	44	21	14.3	22.2	23.0	40.4	100	92	65	-	
S-342	CL	164+00	1.00-2.00	A-2-4(0)	23	NP	3.1	82.7	7.1	7.1	100	99	18	-	
S-339	CL	166+00	1.00-2.00	A-2-4(0)	25	NP	2.2	82.4	8.3	7.1	100	99	20	-	
S-340	CL	166+00	4.00-5.00	A-2-4(0)	19	NP	2.3	84.5	6.1	7.1	100	99	17	-	
S-341	CL	166+00	7.00-8.00	A-2-4(0)	23	NP	0.8	80.1	4.9	14.1	100	100	23	-	
SS-241	CL	170+00	0.00-1.50	A-2-4(0)	21	NP	0.8	82.2	12.0	5.0	100	99	21	-	
SS-242	CL	170+00	3.40-4.90	A-2-4(0)	23	4	2.2	71.8	6.9	19.1	100	99	28	-	
SS-243	CL	170+00	8.40-9.90	A-2-4(0)	26	NP	0.7	77.9	2.3	19.1	100	100	23	-	

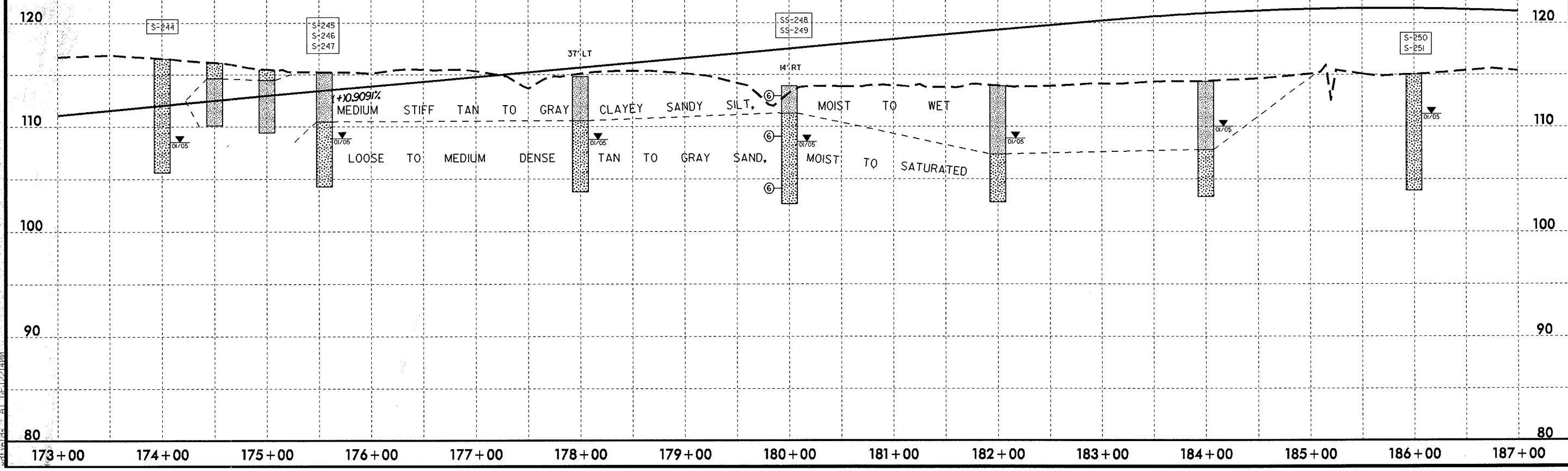


SOIL TEST RESULTS

SAMPLE NO.	OFFSET	STATION	DEPTH INTERVAL	AASHTO CLASS.	L.L.	P.I.	% BY WEIGHT				% PASSING (SIEVES)			% MOISTURE	% ORGANIC
							C.SAND	F.SAND	SILT	CLAY	10	40	200		
S-244	CL	174+00	1.00-2.00	A-2-4(0)	18	NP	3.2	76.1	9.6	11.1	100	99	31		
S-245	CL	175+55	1.00-2.00	A-4(1)	26	10	0.7	60.1	13.1	26.2	100	99	42		
S-246	CL	175+55	5.00-6.00	A-2-4(0)	25	4	1.8	71.9	6.1	20.1	100	100	28		
S-247	CL	175+55	8.00-9.00	A-2-4(0)	22	NP	1.0	83.0	3.9	12.1	100	100	18		
SS-248	14.0 RT	180+00	0.00-1.50	A-4(0)	15	NP	1.9	64.4	16.6	17.1	100	99	39		
SS-249	14.0 RT	180+00	3.80-5.30	A-2-4(0)	26	7	0.8	70.1	7.9	21.1	100	99	31		
S-250	CL	186+00	1.00-2.00	A-2-4(0)	19	NP	3.1	81.8	10.1	5.0	100	99	19		
S-251	CL	186+00	5.00-7.00	A-2-4(0)	19	NP	1.0	81.7	6.2	11.1	100	99	20		

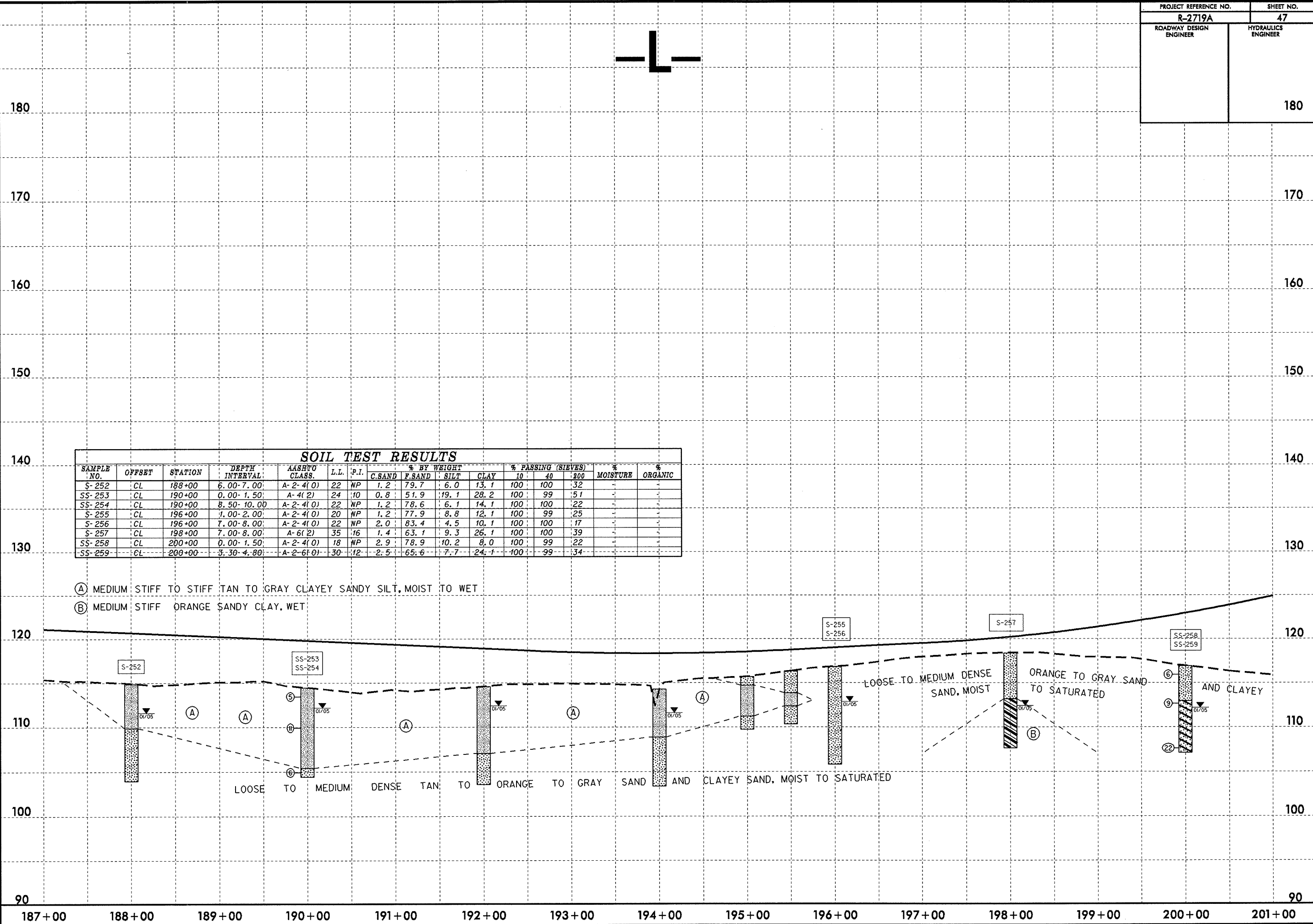
(A) MEDIUM STIFF TO STIFF TAN TO GRAY CLAYEY SANDY SILT, MOIST TO WET

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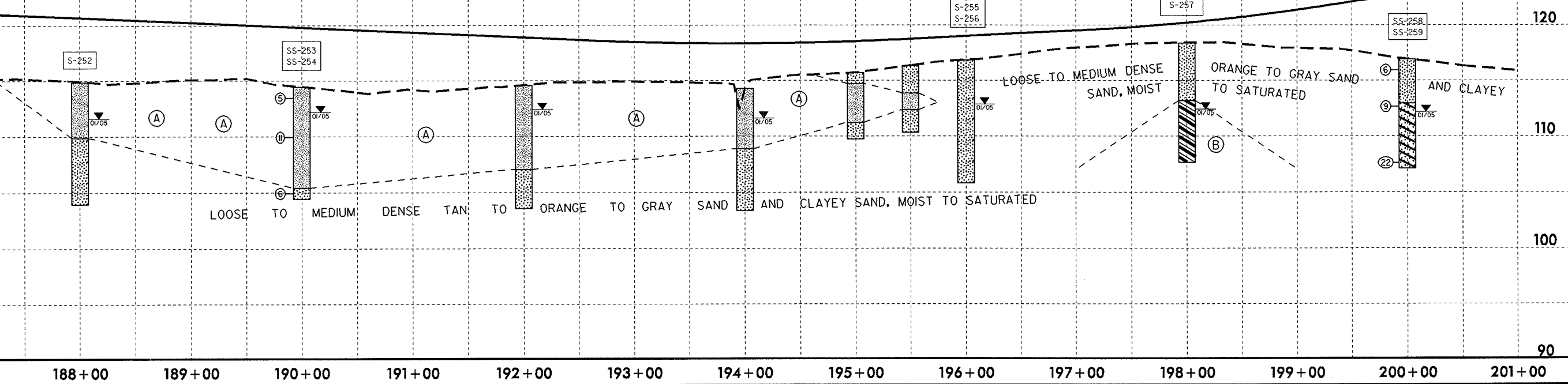
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SOIL TEST RESULTS

SAMPLE NO.	OFFSET	STATION	DEPTH INTERVAL	AASHTO CLASS.	L.L.	P.I.	% BY WEIGHT				% PASSING (SIEVES)			% MOISTURE	% ORGANIC
							C.SAND	F.SAND	SILT	CLAY	10	40	200		
S-252	CL	188+00	6.00-7.00	A-2-4(0)	22	NP	1.2	79.7	6.0	13.1	100	100	32	-	-
SS-253	CL	190+00	0.00-1.50	A-4(2)	24	10	0.8	51.9	19.1	28.2	100	99	51	-	-
SS-254	CL	190+00	8.50-10.00	A-2-4(0)	22	NP	1.2	78.6	6.1	14.1	100	100	22	-	-
S-255	CL	196+00	1.00-2.00	A-2-4(0)	20	NP	1.2	77.9	8.8	12.1	100	99	25	-	-
S-256	CL	196+00	7.00-8.00	A-2-4(0)	22	NP	2.0	83.4	4.5	10.1	100	100	17	-	-
S-257	CL	198+00	7.00-8.00	A-6(2)	35	16	1.4	63.1	9.3	26.1	100	100	39	-	-
SS-258	CL	200+00	0.00-1.50	A-2-4(0)	18	NP	2.9	78.9	10.2	8.0	100	99	22	-	-
SS-259	CL	200+00	3.30-4.80	A-2-6(0)	30	12	2.5	65.6	7.7	24.1	100	99	34	-	-

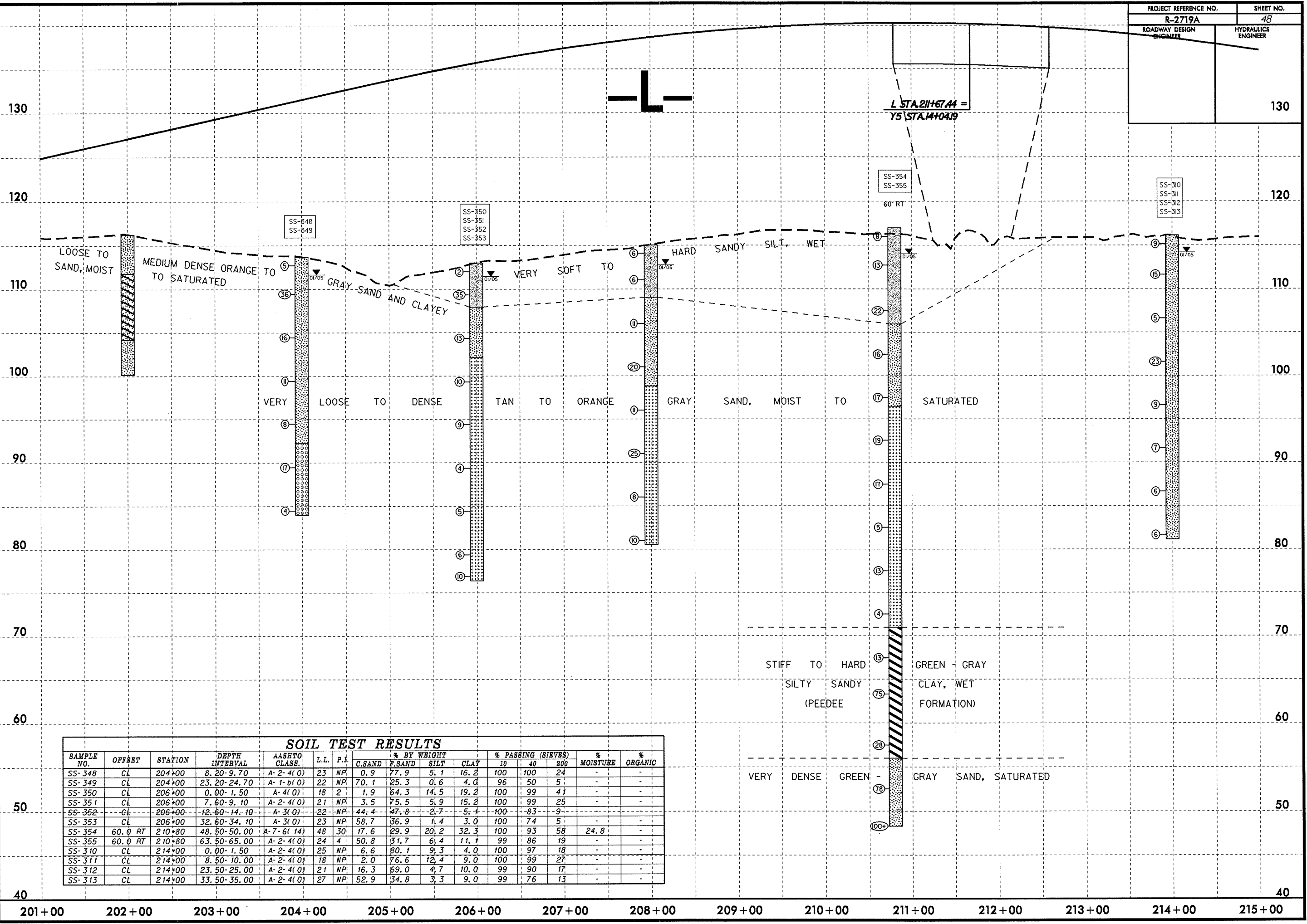
- (A) MEDIUM STIFF TO STIFF TAN TO GRAY CLAYEY SANDY SILT, MOIST TO WET
- (B) MEDIUM STIFF ORANGE SANDY CLAY, WET



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PROJECT REFERENCE NO.	SHEET NO.
R-2719A	48
ROADWAY DESIGN ENGINEER	HYDRAULICS ENGINEER

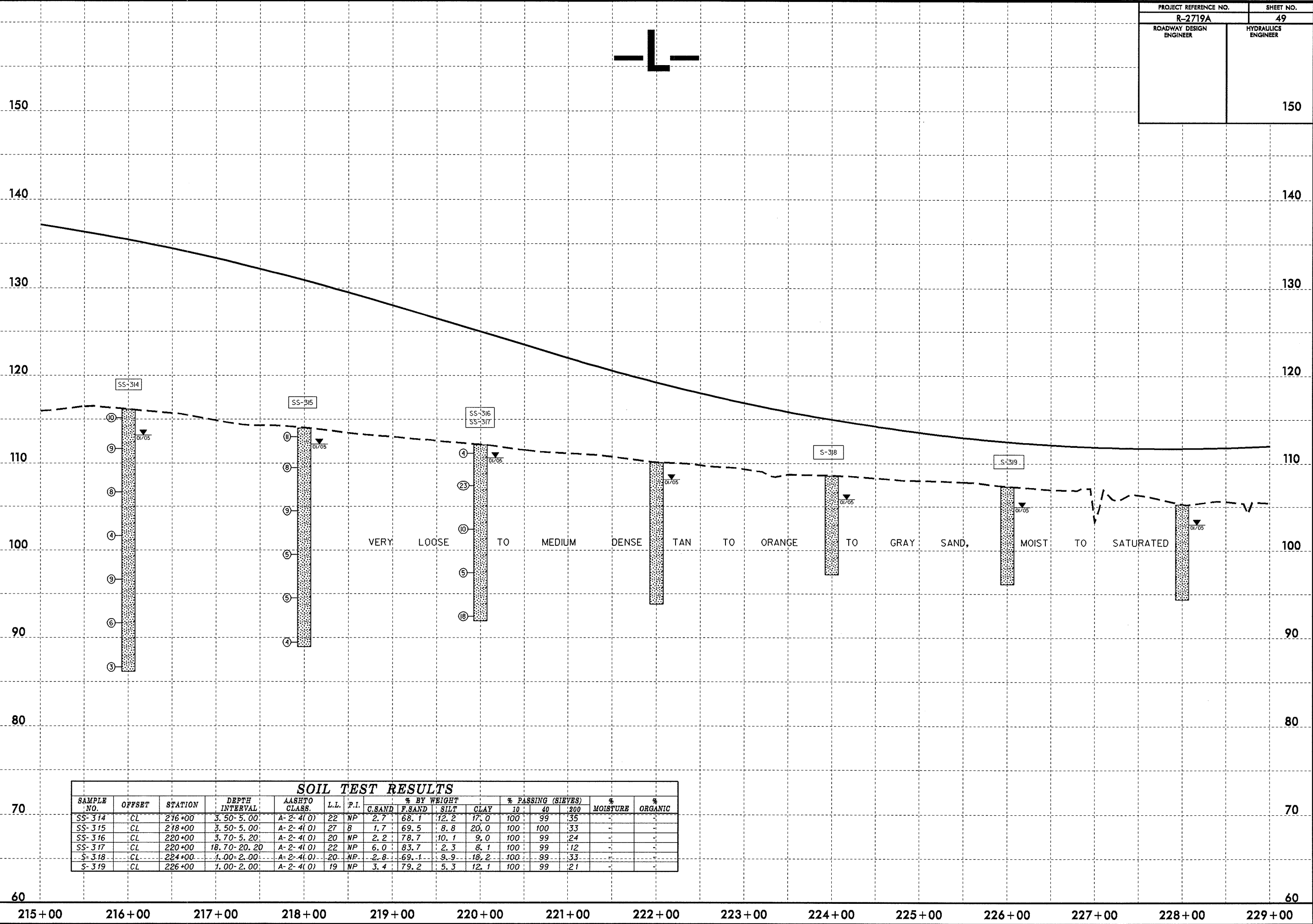


SOIL TEST RESULTS

SAMPLE NO.	OFFSET	STATION	DEPTH INTERVAL	AASHTO CLASS.	L.L.	P.I.	% BY WEIGHT				% PASSING (SIEVES)			% MOISTURE	% ORGANIC
							C.SAND	F.SAND	SILT	CLAY	10	40	200		
SS-348	CL	204+00	8.20-9.70	A-2-4(0)	23	NP	0.9	77.9	5.1	16.2	100	100	24	-	-
SS-349	CL	204+00	23.20-24.70	A-1-b(0)	22	NP	70.1	25.3	0.6	4.0	96	50	5	-	-
SS-350	CL	206+00	0.00-1.50	A-4(0)	18	2	1.9	64.3	14.5	19.2	100	99	41	-	-
SS-351	CL	206+00	7.60-9.10	A-2-4(0)	21	NP	3.5	75.5	5.9	15.2	100	99	25	-	-
SS-352	GL	206+00	12.60-14.10	A-3(0)	22	NP	44.4	47.8	2.7	5.1	100	83	9	-	-
SS-353	CL	206+00	32.60-34.10	A-3(0)	23	NP	58.7	36.9	1.4	3.0	100	74	5	-	-
SS-354	60.0 RT	210+80	48.50-50.00	A-7-6(14)	48	30	17.6	29.9	20.2	32.3	100	93	58	24.8	-
SS-355	60.0 RT	210+80	63.50-65.00	A-2-4(0)	24	4	50.8	31.7	6.4	11.1	99	86	19	-	-
SS-310	CL	214+00	0.00-1.50	A-2-4(0)	25	NP	6.6	80.1	9.3	4.0	100	97	18	-	-
SS-311	CL	214+00	8.50-10.00	A-2-4(0)	18	NP	2.0	76.6	12.4	9.0	100	99	27	-	-
SS-312	CL	214+00	23.50-25.00	A-2-4(0)	21	NP	16.3	69.0	4.7	10.0	99	90	17	-	-
SS-313	CL	214+00	33.50-35.00	A-2-4(0)	27	NP	52.9	34.8	3.3	9.0	99	76	13	-	-

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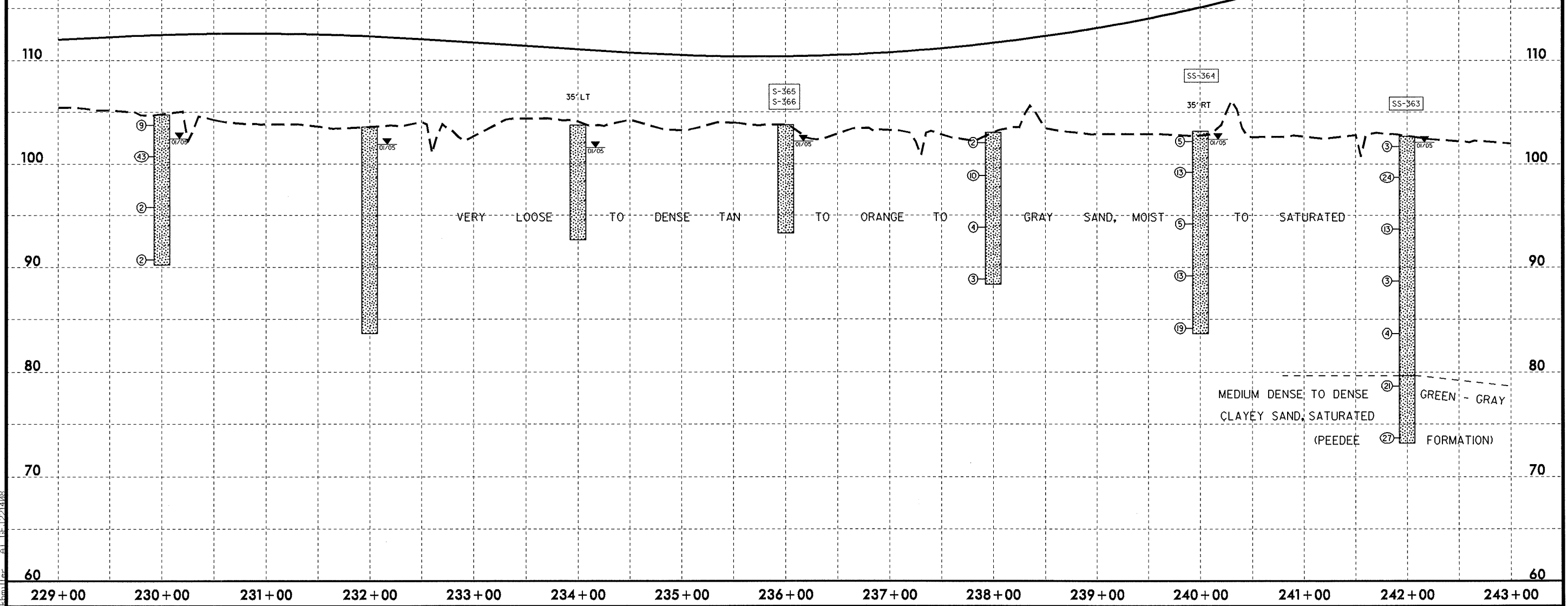


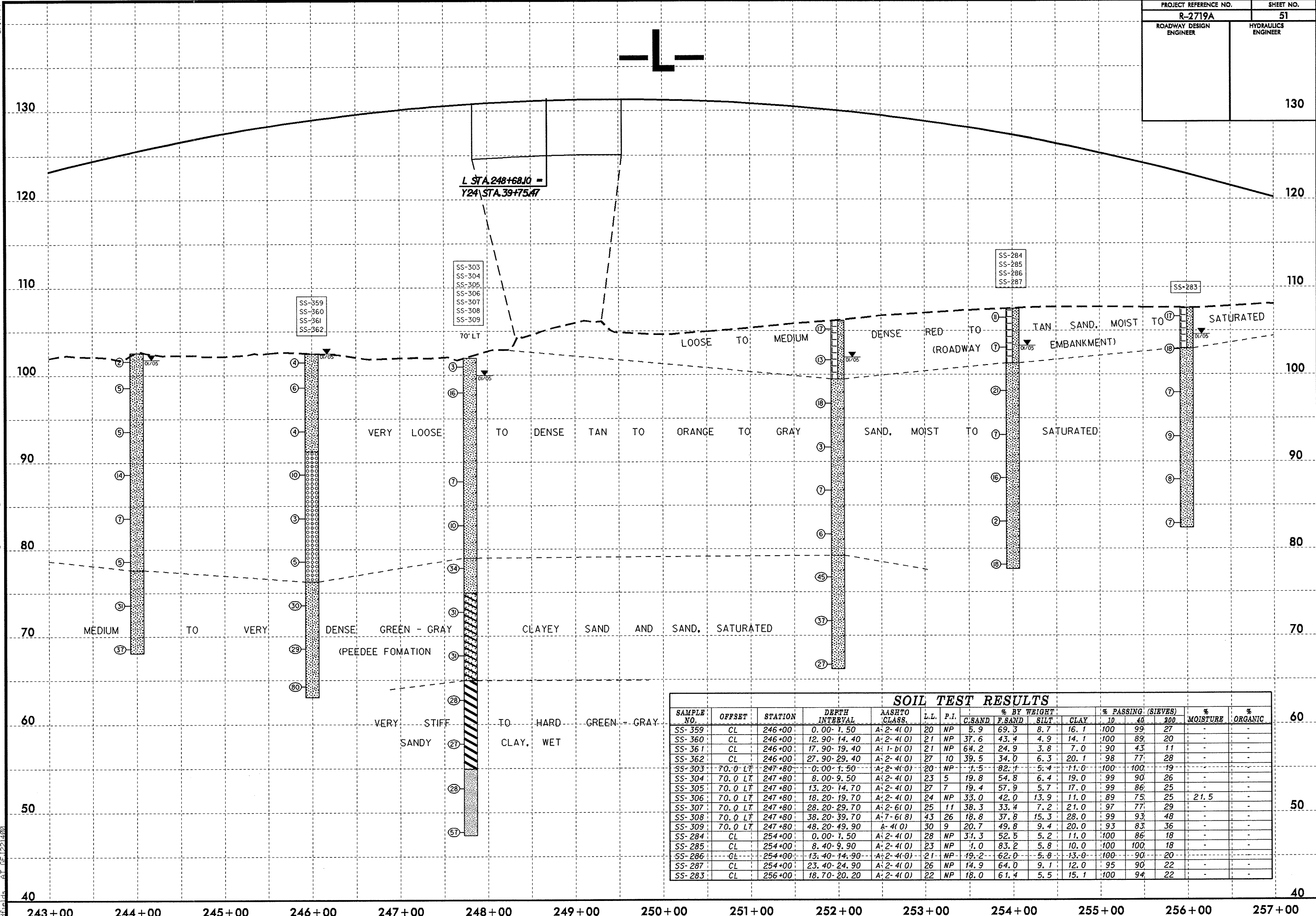
SOIL TEST RESULTS															
SAMPLE NO.	OFFSET	STATION	DEPTH INTERVAL	AASHTO CLASS.	L.L.	P.I.	% BY WEIGHT				% PASSING (SIEVES)			% MOISTURE	% ORGANIC
							C.SAND	F.SAND	SILT	CLAY	10	40	200		
SS-314	CL	216+00	3.50-5.00	A-2-4(0)	22	NP	2.7	68.1	12.2	17.0	100	99	35	-	-
SS-315	CL	218+00	3.50-5.00	A-2-4(0)	27	B	1.7	69.5	8.8	20.0	100	100	33	-	-
SS-316	CL	220+00	3.70-5.20	A-2-4(0)	20	NP	2.2	78.7	10.1	9.0	100	99	24	-	-
SS-317	CL	220+00	18.70-20.20	A-2-4(0)	22	NP	6.0	83.7	2.3	8.1	100	99	12	-	-
S-318	CL	224+00	1.00-2.00	A-2-4(0)	20	NP	2.8	69.1	9.9	18.2	100	99	33	-	-
S-319	CL	226+00	1.00-2.00	A-2-4(0)	19	NP	3.4	79.2	5.3	12.1	100	99	21	-	-

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PROJECT REFERENCE NO.	SHEET NO.
R-2719A	50
ROADWAY DESIGN ENGINEER	HYDRAULICS ENGINEER
	150

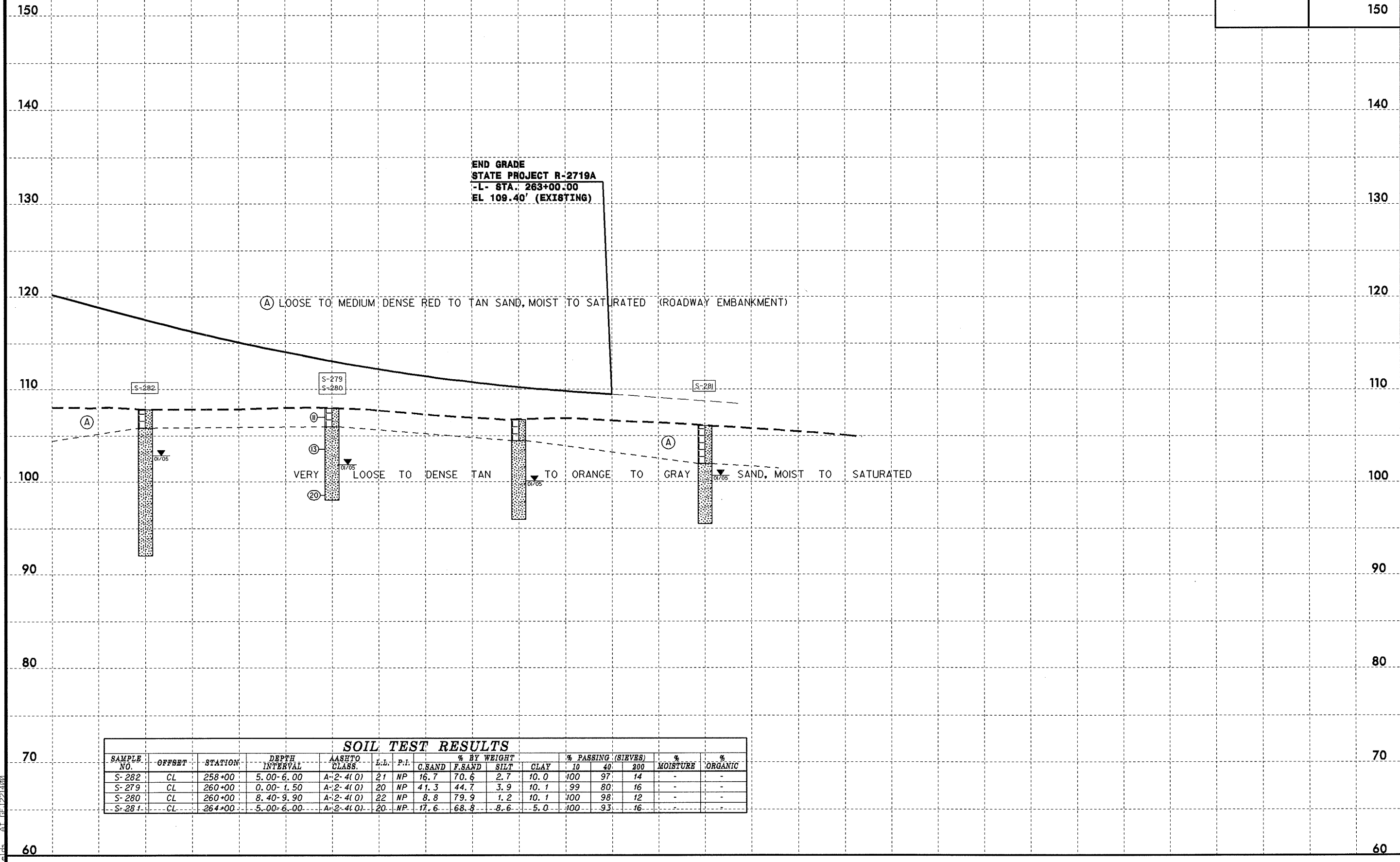
SOIL TEST RESULTS															
SAMPLE NO.	OFFSET	STATION	DEPTH INTERVAL	AASHTO CLASS.	L.L.	P.I.	% BY WEIGHT				% PASSING (SIEVES)			% MOISTURE	% ORGANIC
							C.SAND	F.SAND	SILT	CLAY	10	40	200		
S-365	CL	236+00	1.00-2.00	A-2-4(0)	26	NP	11.8	76.6	5.5	6.0	100	93	14	-	-
S-366	CL	236+00	5.00-2.00	A-2-4(0)	19	NP	20.2	67.1	2.6	10.1	100	90	15	-	-
SS-364	35.0 RT	240+00	18.00-19.50	A-2-4(0)	25	6	23.7	42.9	19.3	14.1	97	89	34	-	-
SS-363	CL	242+00	8.00-9.50	A-2-4(0)	22	NP	2.0	80.7	5.2	12.1	100	99	21	-	-





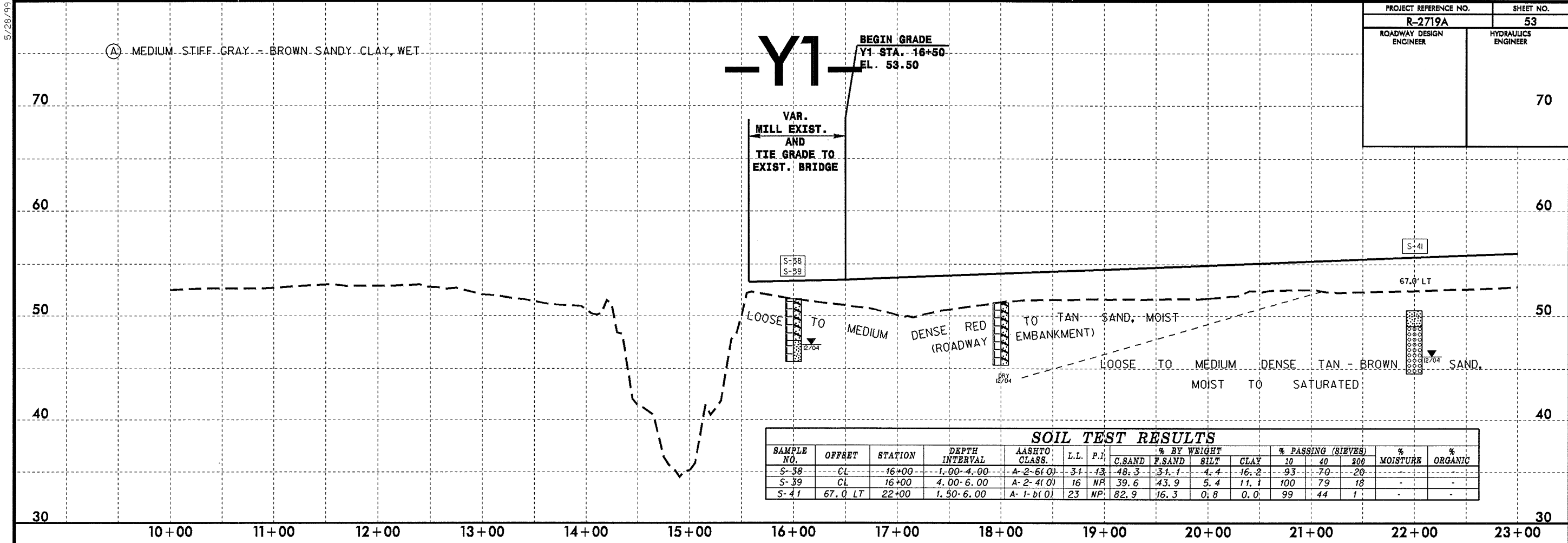
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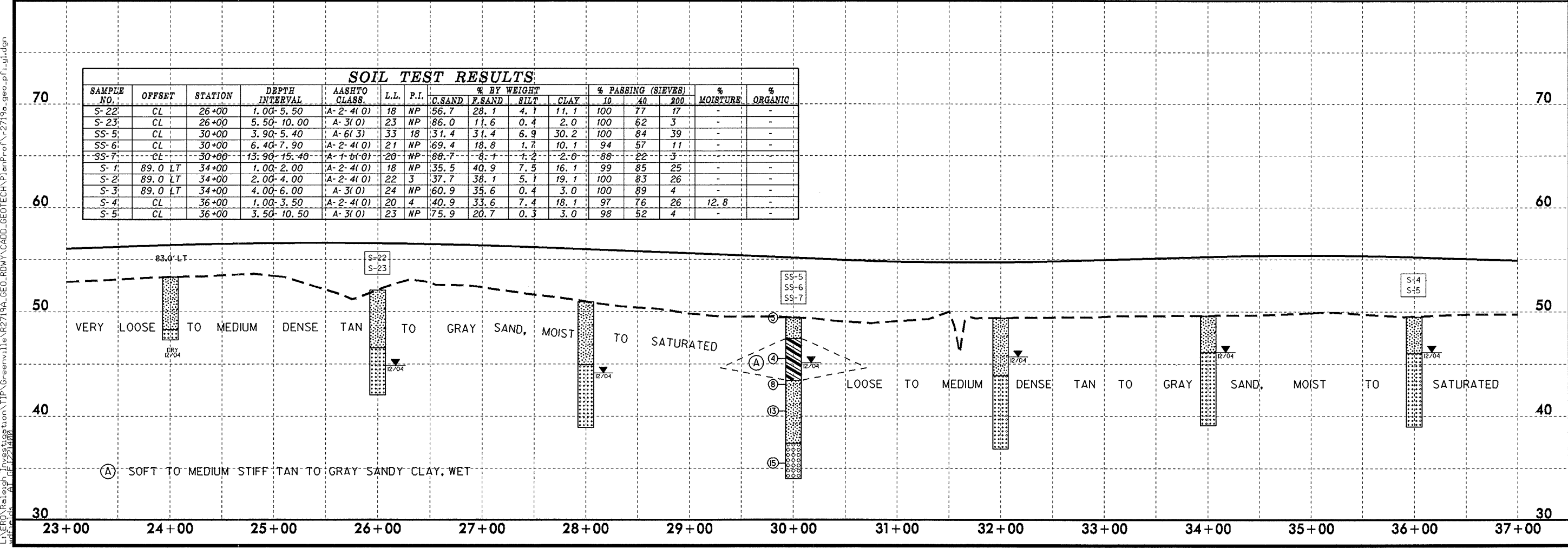


SOIL TEST RESULTS															
SAMPLE NO.	OFFSET	STATION	DEPTH INTERVAL	AASHTO CLASS	L.L.	P.I.	% BY WEIGHT				% PASSING (SIEVES)			% MOISTURE	% ORGANIC
							C.SAND	F.SAND	SILT	CLAY	10	40	200		
S-282	CL	258+00	5.00-6.00	A-2-4(0)	21	NP	16.7	70.6	2.7	10.0	100	97	14	-	-
S-279	CL	260+00	0.00-1.50	A-2-4(0)	20	NP	41.3	44.7	3.9	10.1	99	80	16	-	-
S-280	CL	260+00	8.40-9.90	A-2-4(0)	22	NP	8.8	79.9	1.2	10.1	100	98	12	-	-
S-281	CL	264+00	5.00-6.00	A-2-4(0)	20	NP	17.6	68.8	8.6	5.0	100	93	16	-	-

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SAMPLE NO.	OFFSET	STATION	DEPTH INTERVAL	AASHTO CLASS.	L.L.	P.I.	% BY WEIGHT				% PASSING (SIEVES)			% MOISTURE	% ORGANIC
							C.SAND	F.SAND	SILT	CLAY	10	40	200		
S-38	CL	16+00	1.00-4.00	A-2-6(0)	31	13	48.3	31.1	4.4	16.2	93	70	20	-	-
S-39	CL	16+00	4.00-6.00	A-2-4(0)	16	NP	39.6	43.9	5.4	11.1	100	79	18	-	-
S-41	67.0 LT	22+00	1.50-6.00	A-1-b(0)	23	NP	82.9	16.3	0.8	0.0	99	44	1	-	-



SAMPLE NO.	OFFSET	STATION	DEPTH INTERVAL	AASHTO CLASS.	L.L.	P.I.	% BY WEIGHT				% PASSING (SIEVES)			% MOISTURE	% ORGANIC
							C.SAND	F.SAND	SILT	CLAY	10	40	200		
S-22	CL	26+00	1.00-5.50	A-2-4(0)	18	NP	56.7	28.1	4.1	11.1	100	77	17	-	-
S-23	CL	26+00	5.50-10.00	A-3(0)	23	NP	86.0	11.6	0.4	2.0	100	62	3	-	-
SS-5	CL	30+00	3.90-5.40	A-6(3)	33	18	31.4	31.4	6.9	30.2	100	84	39	-	-
SS-6	CL	30+00	6.40-7.90	A-2-4(0)	21	NP	69.4	18.8	1.7	10.1	94	57	11	-	-
SS-7	CL	30+00	13.90-15.40	A-1-b(0)	20	NP	88.7	8.1	1.2	2.0	88	22	3	-	-
S-1	89.0 LT	34+00	1.00-2.00	A-2-4(0)	18	NP	35.5	40.9	7.5	16.1	99	85	25	-	-
S-2	89.0 LT	34+00	2.00-4.00	A-2-4(0)	22	3	37.7	38.1	5.1	19.1	100	83	26	-	-
S-3	89.0 LT	34+00	4.00-6.00	A-3(0)	24	NP	60.9	35.6	0.4	3.0	100	89	4	-	-
S-4	CL	36+00	1.00-3.50	A-2-4(0)	20	4	40.9	33.6	7.4	18.1	97	76	26	12.8	-
S-5	CL	36+00	3.50-10.50	A-3(0)	23	NP	75.9	20.7	0.3	3.0	98	52	4	-	-

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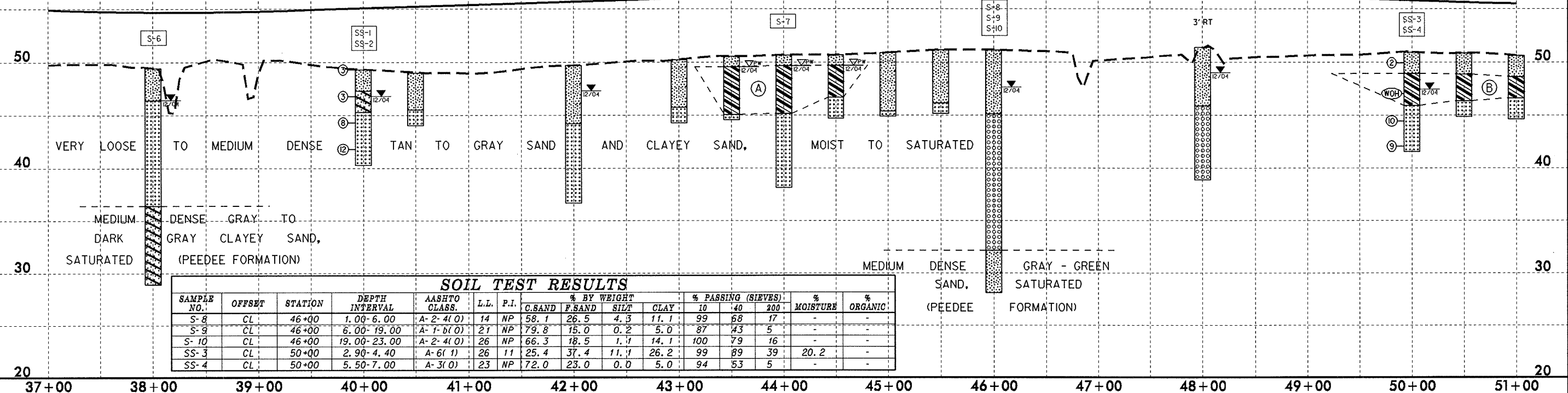
5/28/95

- (A) SOFT TO MEDIUM STIFF TAN TO GRAY SANDY CLAY, WET
- (B) VERY SOFT TO MEDIUM STIFF GRAY TO DARK GRAY SANDY CLAY, WET

PROJECT REFERENCE NO.	SHEET NO.
R-2719A	54
ROADWAY DESIGN ENGINEER	HYDRAULICS ENGINEER

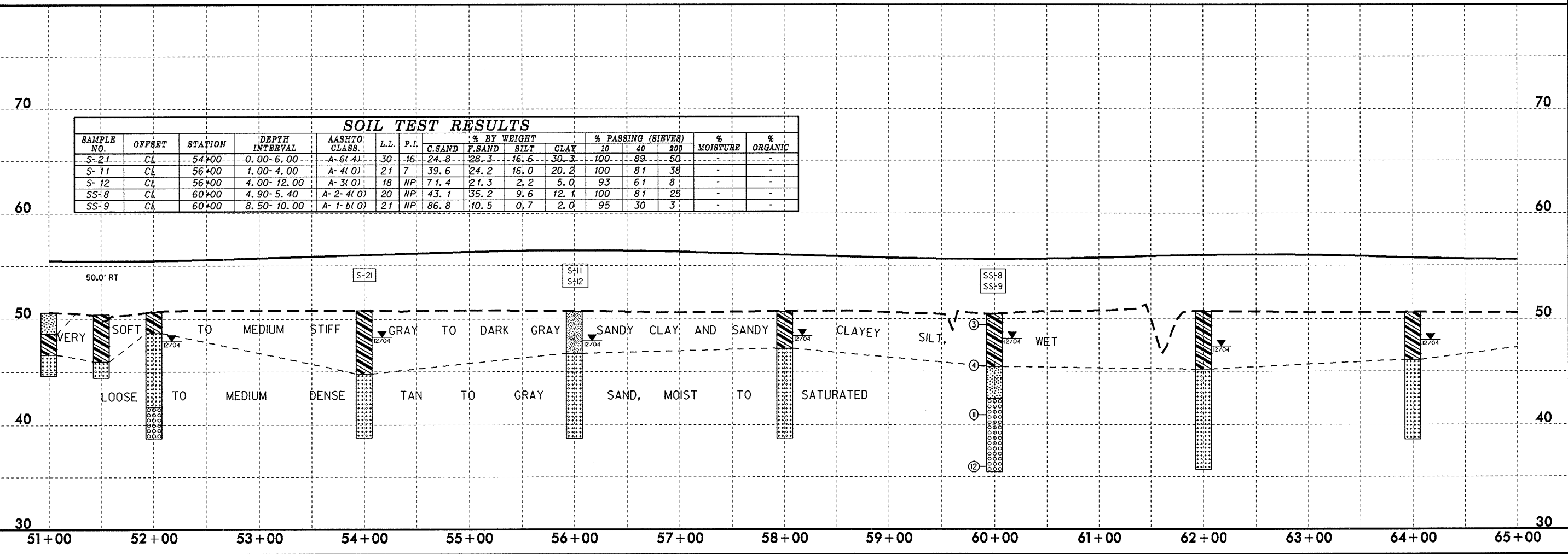
-Y1-

SAMPLE NO.	OFFSET	STATION	DEPTH INTERVAL	AASHTO CLASS.	L.L.	P.I.	% BY WEIGHT				% PASSING (SIEVES)			% MOISTURE	% ORGANIC
							C.SAND	F.SAND	SILT	CLAY	10	40	200		
S-6	CL	38+00	13.00-20.50	A-2-6(1)	39	15	29.9	44.8	6.1	19.1	99	82	29	26.8	-
SS-1	CL	40+00	2.50-4.00	A-2-6(0)	31	12	42.1	29.6	4.1	24.2	100	84	30	-	-
SS-2	CL	40+00	5.00-6.50	A-3(0)	22	NP	71.9	23.3	0.8	4.0	99	59	6	-	-
S-7	CL	44+00	1.00-5.50	A-6(2)	29	14	28.2	33.9	13.7	24.2	100	88	43	-	-



SAMPLE NO.	OFFSET	STATION	DEPTH INTERVAL	AASHTO CLASS.	L.L.	P.I.	% BY WEIGHT				% PASSING (SIEVES)			% MOISTURE	% ORGANIC
							C.SAND	F.SAND	SILT	CLAY	10	40	200		
S-8	CL	46+00	1.00-6.00	A-2-4(0)	14	NP	58.1	26.5	4.3	11.1	99	68	17	-	-
S-9	CL	46+00	6.00-19.00	A-1-b(0)	21	NP	79.8	15.0	0.2	5.0	87	43	5	-	-
S-10	CL	46+00	19.00-23.00	A-2-4(0)	26	NP	66.3	18.5	1.1	14.1	100	79	16	-	-
SS-3	CL	50+00	2.90-4.40	A-6(1)	26	11	25.4	37.4	11.1	26.2	99	89	39	20.2	-
SS-4	CL	50+00	5.50-7.00	A-3(0)	23	NP	72.0	23.0	0.0	5.0	94	53	5	-	-

SAMPLE NO.	OFFSET	STATION	DEPTH INTERVAL	AASHTO CLASS.	L.L.	P.I.	% BY WEIGHT				% PASSING (SIEVES)			% MOISTURE	% ORGANIC
							C.SAND	F.SAND	SILT	CLAY	10	40	200		
S-21	CL	54+00	0.00-6.00	A-6(4)	30	16	24.8	28.3	16.6	30.3	100	89	50	-	-
S-11	CL	56+00	1.00-4.00	A-4(0)	21	7	39.6	24.2	16.0	20.2	100	81	38	-	-
S-12	CL	56+00	4.00-12.00	A-3(0)	18	NP	71.4	21.3	2.2	5.0	93	61	8	-	-
SS-8	CL	60+00	4.90-5.40	A-2-4(0)	20	NP	43.1	35.2	9.6	12.1	100	81	25	-	-
SS-9	CL	60+00	8.50-10.00	A-1-b(0)	21	NP	86.8	10.5	0.7	2.0	95	30	3	-	-



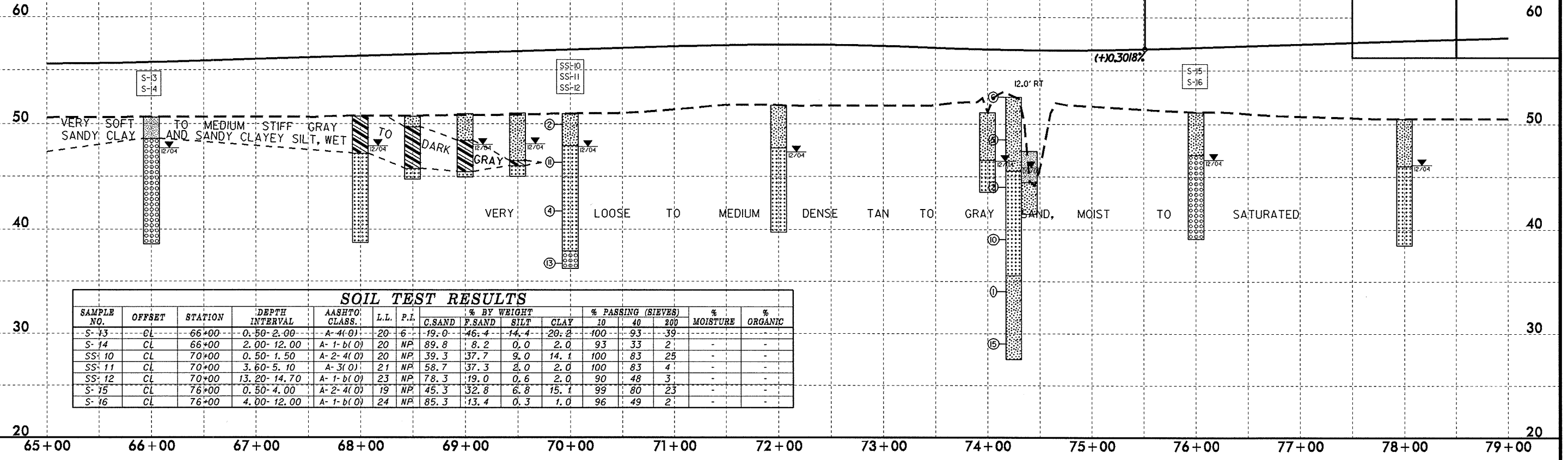
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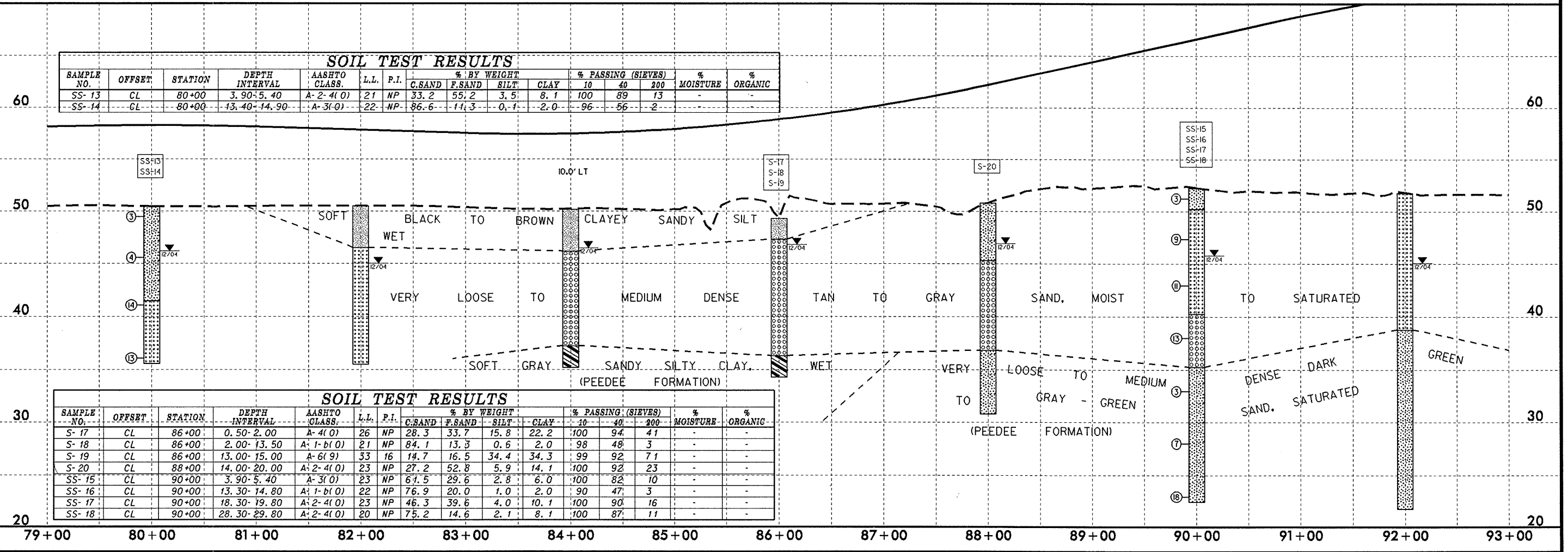
-Y1-

Y1 STA. 75+51.2 =
YIRPCA STA. 28+99.24

PROJECT REFERENCE NO. R-2719A	SHEET NO. 55
ROADWAY DESIGN ENGINEER	HYDRAULICS ENGINEER



SAMPLE NO.	OFFSET	STATION	DEPTH INTERVAL	AASHTO CLASS.	L.L.	P.I.	% BY WEIGHT				% PASSING (SIEVES)			% MOISTURE	% ORGANIC
							C.SAND	F.SAND	SILT	CLAY	10	40	200		
S-13	CL	66+00	0.50-2.00	A-4(0)	20	6	19.0	46.4	14.4	20.2	100	93	39	-	-
S-14	CL	66+00	2.00-12.00	A-1-b(0)	20	NP	89.8	8.2	0.0	2.0	93	33	21	-	-
SS-10	CL	70+00	0.50-1.50	A-2-4(0)	20	NP	39.3	37.7	9.0	14.1	100	83	25	-	-
SS-11	CL	70+00	3.60-5.10	A-3(0)	21	NP	58.7	37.3	2.0	2.0	100	83	41	-	-
SS-12	CL	70+00	13.20-14.70	A-1-b(0)	23	NP	78.3	19.0	0.6	2.0	90	48	31	-	-
S-15	CL	76+00	0.50-4.00	A-2-4(0)	19	NP	45.3	32.8	6.8	15.1	99	80	23	-	-
S-16	CL	76+00	4.00-12.00	A-1-b(0)	24	NP	85.3	13.4	0.3	1.0	96	49	21	-	-



SAMPLE NO.	OFFSET	STATION	DEPTH INTERVAL	AASHTO CLASS.	L.L.	P.I.	% BY WEIGHT				% PASSING (SIEVES)			% MOISTURE	% ORGANIC
							C.SAND	F.SAND	SILT	CLAY	10	40	200		
SS-13	CL	80+00	3.90-5.40	A-2-4(0)	21	NP	33.2	55.2	3.5	8.1	100	89	13	-	-
SS-14	CL	80+00	13.40-14.90	A-3(0)	22	NP	86.6	11.3	0.1	2.0	96	56	2	-	-

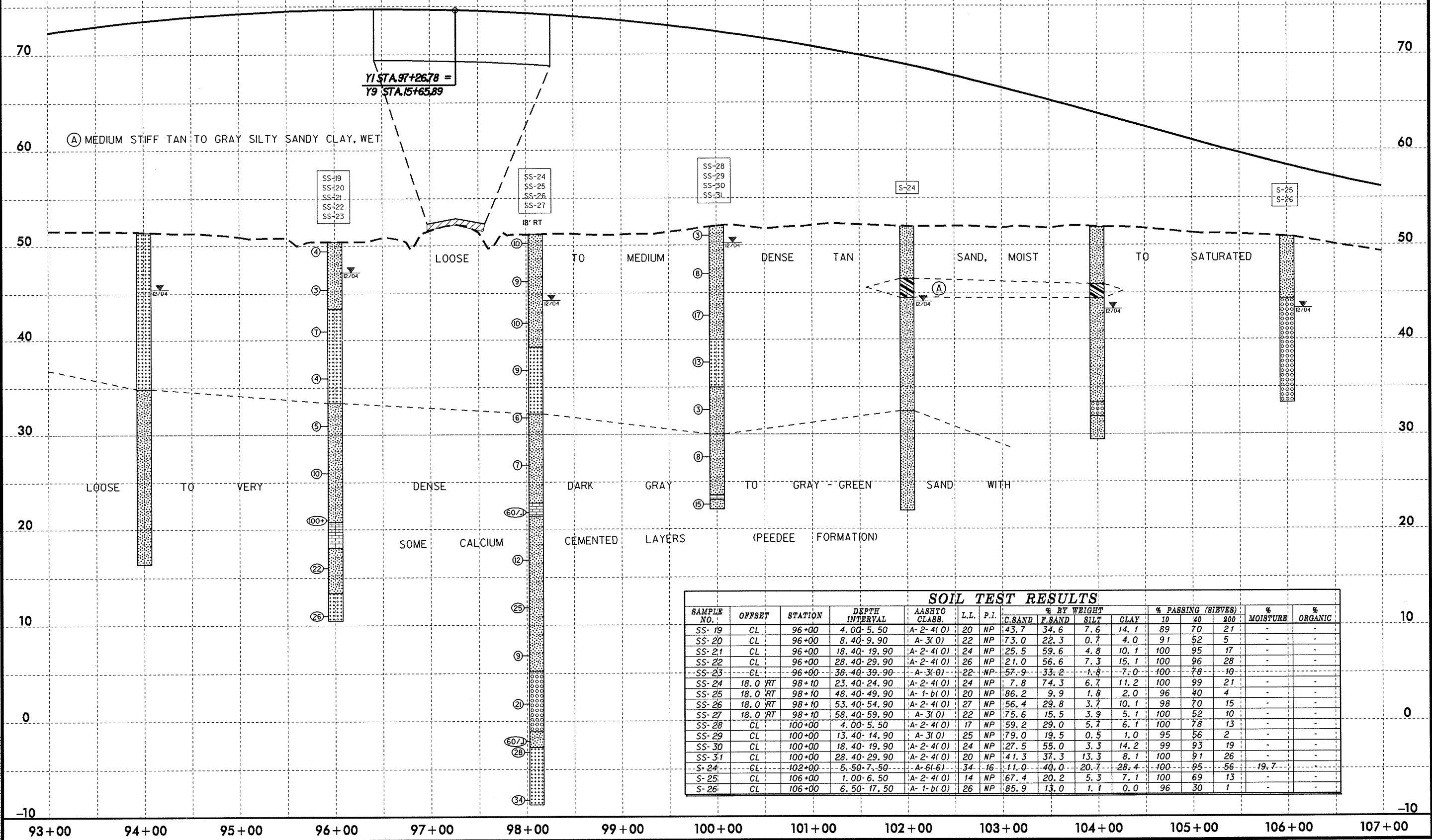
SAMPLE NO.	OFFSET	STATION	DEPTH INTERVAL	AASHTO CLASS.	L.L.	P.I.	% BY WEIGHT				% PASSING (SIEVES)			% MOISTURE	% ORGANIC
							C.SAND	F.SAND	SILT	CLAY	10	40	200		
S-17	CL	86+00	0.50-2.00	A-4(0)	26	NP	28.3	33.7	15.8	22.2	100	94	41	-	-
S-18	CL	86+00	2.00-13.50	A-1-b(0)	21	NP	84.1	13.3	0.6	2.0	98	48	3	-	-
S-19	CL	86+00	13.00-15.00	A-6(9)	33	16	14.7	16.5	34.4	34.3	99	92	71	-	-
S-20	CL	88+00	14.00-20.00	A-2-4(0)	23	NP	27.2	52.8	5.9	14.1	100	92	23	-	-
SS-15	CL	90+00	3.90-5.40	A-3(0)	23	NP	61.5	29.6	2.8	6.0	100	82	10	-	-
SS-16	CL	90+00	13.30-14.80	A-1-b(0)	22	NP	76.9	20.0	1.0	2.0	90	47	3	-	-
SS-17	CL	90+00	18.30-19.80	A-2-4(0)	23	NP	46.3	39.6	4.0	10.1	100	90	16	-	-
SS-18	CL	90+00	28.30-29.80	A-2-4(0)	20	NP	75.2	14.6	2.1	8.1	100	87	11	-	-

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-Y1-

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SOIL TEST RESULTS															
SAMPLE NO.	OFFSET	STATION	DEPTH INTERVAL	AASHTO CLASS.	L.L.	P.I.	% BY WEIGHT				% PASSING (SIEVES)			% MOISTURE	% ORGANIC
							C.SAND	F.SAND	SILT	CLAY	10	40	200		
SS-19	CL	96+00	4.00-5.50	A-2-4(0)	20	NP	43.7	34.6	7.6	14.1	89	70	21	-	-
SS-20	CL	96+00	8.40-9.90	A-3(0)	22	NP	73.0	22.3	0.7	4.0	91	52	5	-	-
SS-21	CL	96+00	18.40-19.90	A-2-4(0)	24	NP	25.5	59.6	4.8	10.1	100	95	17	-	-
SS-22	CL	96+00	28.40-29.90	A-2-4(0)	26	NP	21.0	56.6	7.3	15.1	100	96	28	-	-
SS-23	CL	96+00	38.40-39.90	A-3(0)	22	NP	57.9	33.2	1.8	7.0	100	78	10	-	-
SS-24	18.0 RT	98+10	23.40-24.90	A-2-4(0)	24	NP	7.8	74.3	6.7	11.2	100	99	21	-	-
SS-25	18.0 RT	98+10	48.40-49.90	A-1-b(0)	20	NP	86.2	9.9	1.8	2.0	96	40	4	-	-
SS-26	18.0 RT	98+10	53.40-54.90	A-2-4(0)	27	NP	56.4	29.8	3.7	10.1	98	70	15	-	-
SS-27	18.0 RT	98+10	58.40-59.90	A-3(0)	22	NP	75.6	15.5	3.9	5.1	100	52	10	-	-
SS-28	CL	100+00	4.00-5.50	A-2-4(0)	17	NP	59.2	29.0	5.7	6.1	100	78	13	-	-
SS-29	CL	100+00	13.40-14.90	A-3(0)	25	NP	79.0	19.5	0.5	1.0	95	56	2	-	-
SS-30	CL	100+00	18.40-19.90	A-2-4(0)	24	NP	27.5	55.0	3.3	14.2	99	93	19	-	-
SS-31	CL	100+00	28.40-29.90	A-2-4(0)	20	NP	41.3	37.3	13.3	8.1	100	91	26	-	-
S-24	CL	102+00	5.50-7.50	A-6(6)	34	16	11.0	40.0	20.7	28.4	100	95	56	19.7	-
S-25	CL	106+00	1.00-6.50	A-2-4(0)	14	NP	67.4	20.2	5.3	7.1	100	69	13	-	-
S-26	CL	106+00	6.50-17.50	A-1-b(0)	26	NP	85.9	13.0	1.1	0.0	96	30	1	-	-

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-Y1-

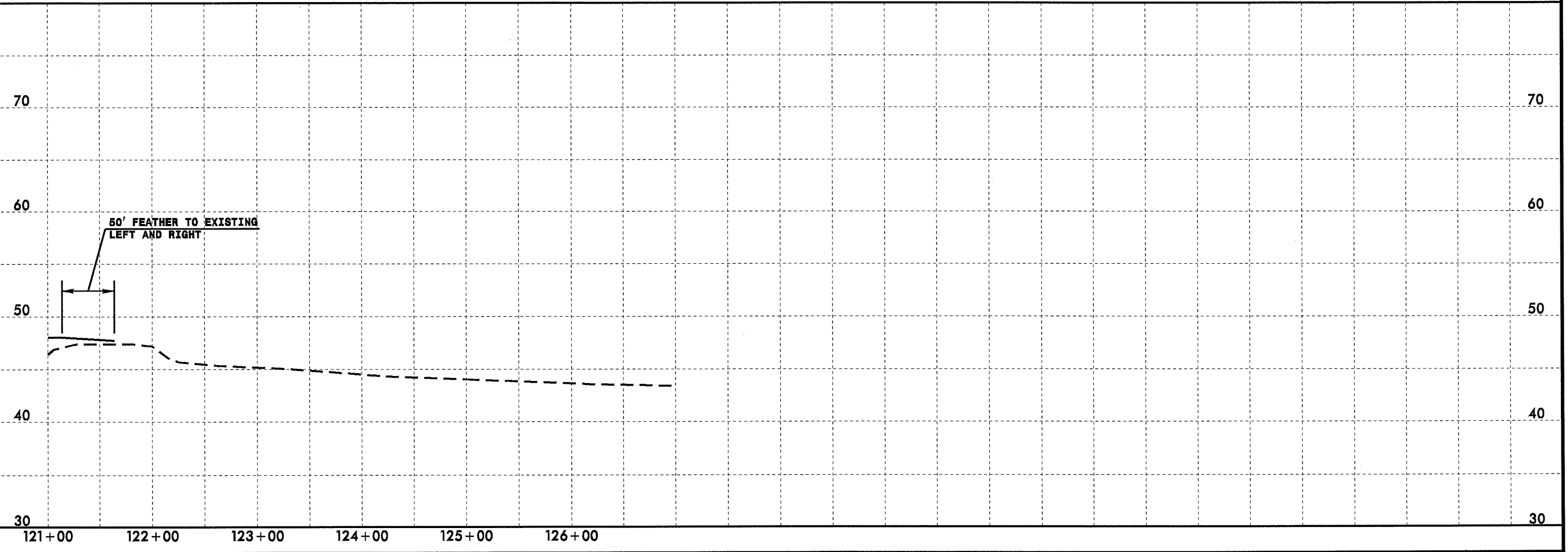
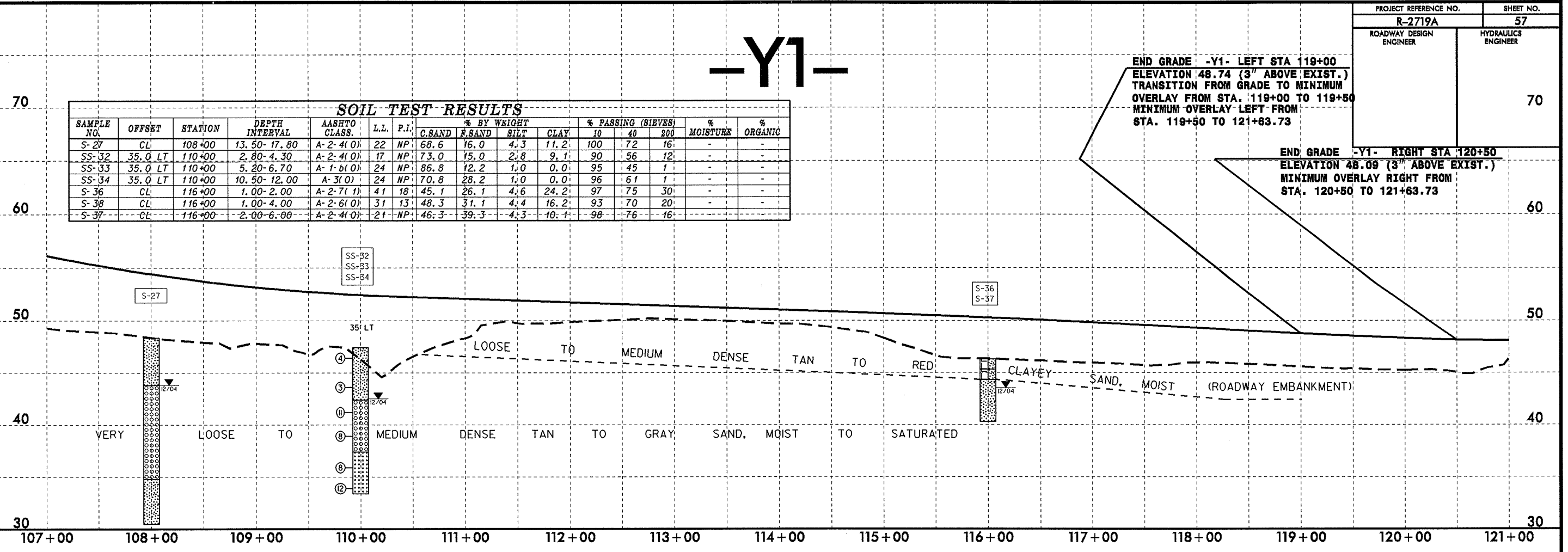
PROJECT REFERENCE NO.	SHEET NO.
R-2719A	57
ROADWAY DESIGN ENGINEER	HYDRAULICS ENGINEER

SOIL TEST RESULTS

SAMPLE NO.	OFFSET	STATION	DEPTH INTERVAL	AASHTO CLASS.	L.L.	P.I.	% BY WEIGHT				% PASSING (SIEVES)			% MOISTURE	% ORGANIC
							C.SAND	F.SAND	SILT	CLAY	10	40	200		
S-27	CL	108+00	13.50-17.80	A-2-4(0)	22	NP	68.6	16.0	4.3	11.2	100	72	16	-	-
SS-32	35.0 LT	110+00	2.80-4.30	A-2-4(0)	17	NP	73.0	15.0	2.8	9.1	90	56	12	-	-
SS-33	35.0 LT	110+00	5.20-6.70	A-1-b(0)	24	NP	86.8	12.2	1.0	0.0	95	45	1	-	-
SS-34	35.0 LT	110+00	10.50-12.00	A-3(0)	24	NP	70.8	28.2	1.0	0.0	96	61	1	-	-
S-36	CL	116+00	1.00-2.00	A-2-7(1)	41	18	45.1	26.1	4.6	24.2	97	75	30	-	-
S-38	CL	116+00	1.00-4.00	A-2-6(0)	31	13	48.3	31.1	4.4	16.2	93	70	20	-	-
S-37	CL	116+00	2.00-6.00	A-2-4(0)	21	NP	46.3	39.3	4.3	10.1	98	76	16	-	-

END GRADE -Y1- LEFT STA 119+00
 ELEVATION 48.74 (3" ABOVE EXIST.)
 TRANSITION FROM GRADE TO MINIMUM
 OVERLAY FROM STA. 119+00 TO 119+50
 MINIMUM OVERLAY LEFT FROM
 STA. 119+50 TO 121+63.73

END GRADE -Y1- RIGHT STA 120+50
 ELEVATION 48.09 (3" ABOVE EXIST.)
 MINIMUM OVERLAY RIGHT FROM
 STA. 120+50 TO 121+63.73

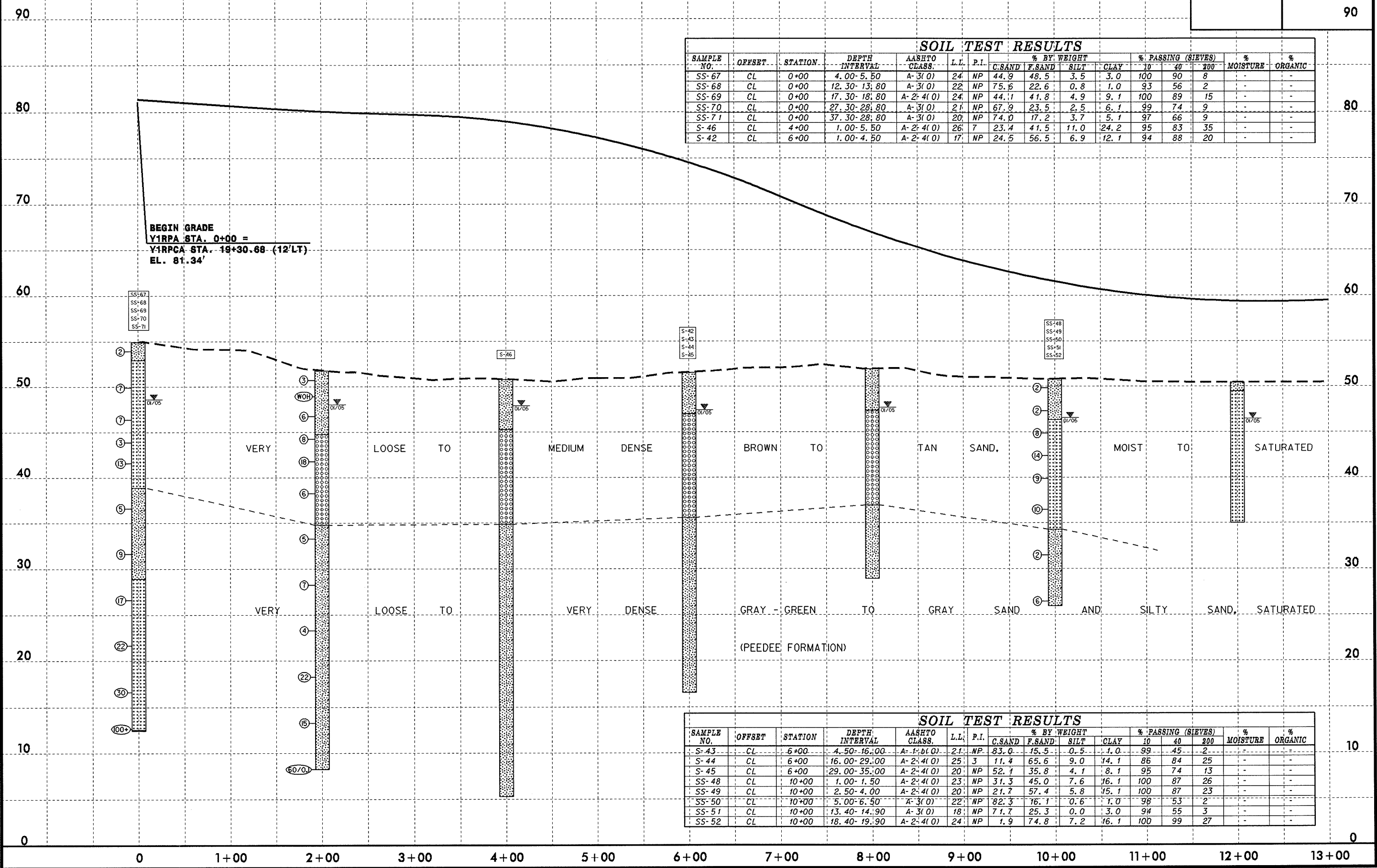


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-Y1RPA-

SOIL TEST RESULTS															
SAMPLE NO.	OFFSET	STATION	DEPTH INTERVAL	AASHTO CLASS.	L.L.	P.I.	% BY WEIGHT				% PASSING (SIEVES)			% MOISTURE	% ORGANIC
							C.SAND	F.SAND	SILT	CLAY	10	40	200		
SS-67	CL	0+00	4.00-5.50	A-3(0)	24	NP	44.9	48.5	3.5	3.0	100	90	8	-	-
SS-68	CL	0+00	12.30-13.80	A-3(0)	22	NP	75.6	22.6	0.8	1.0	93	56	2	-	-
SS-69	CL	0+00	17.30-18.80	A-2-4(0)	24	NP	44.1	41.8	4.9	9.1	100	89	15	-	-
SS-70	CL	0+00	27.30-28.80	A-3(0)	21	NP	67.9	23.5	2.5	6.1	99	74	9	-	-
SS-71	CL	0+00	37.30-28.80	A-3(0)	20	NP	74.0	17.2	3.7	5.1	97	66	9	-	-
S-46	CL	4+00	1.00-5.50	A-2-4(0)	26	7	23.4	41.5	11.0	24.2	95	83	35	-	-
S-42	CL	6+00	1.00-4.50	A-2-4(0)	17	NP	24.5	56.5	6.9	12.1	94	88	20	-	-

BEGIN GRADE
Y1RPA STA. 0+00 =
Y1RPCA STA. 19+30.68 (12' LT)
EL. 81.34'



SOIL TEST RESULTS															
SAMPLE NO.	OFFSET	STATION	DEPTH INTERVAL	AASHTO CLASS.	L.L.	P.I.	% BY WEIGHT				% PASSING (SIEVES)			% MOISTURE	% ORGANIC
							C.SAND	F.SAND	SILT	CLAY	10	40	200		
S-43	CL	6+00	4.50-16.00	A-1-b(0)	21	NP	83.0	15.5	0.5	1.0	99	45	2	-	-
S-44	CL	6+00	16.00-29.00	A-2-4(0)	25	3	11.4	65.6	9.0	14.1	86	84	25	-	-
S-45	CL	6+00	29.00-35.00	A-2-4(0)	20	NP	52.1	35.8	4.1	8.1	95	74	13	-	-
SS-48	CL	10+00	1.00-1.50	A-2-4(0)	23	NP	31.3	45.0	7.6	16.1	100	87	26	-	-
SS-49	CL	10+00	2.50-4.00	A-2-4(0)	20	NP	21.7	57.4	5.8	15.1	100	87	23	-	-
SS-50	CL	10+00	5.00-6.50	A-3(0)	22	NP	82.3	16.1	0.6	1.0	98	53	2	-	-
SS-51	CL	10+00	13.40-14.90	A-3(0)	18	NP	71.7	25.3	0.0	3.0	94	55	3	-	-
SS-52	CL	10+00	18.40-19.90	A-2-4(0)	24	NP	1.9	74.8	7.2	16.1	100	99	27	-	-

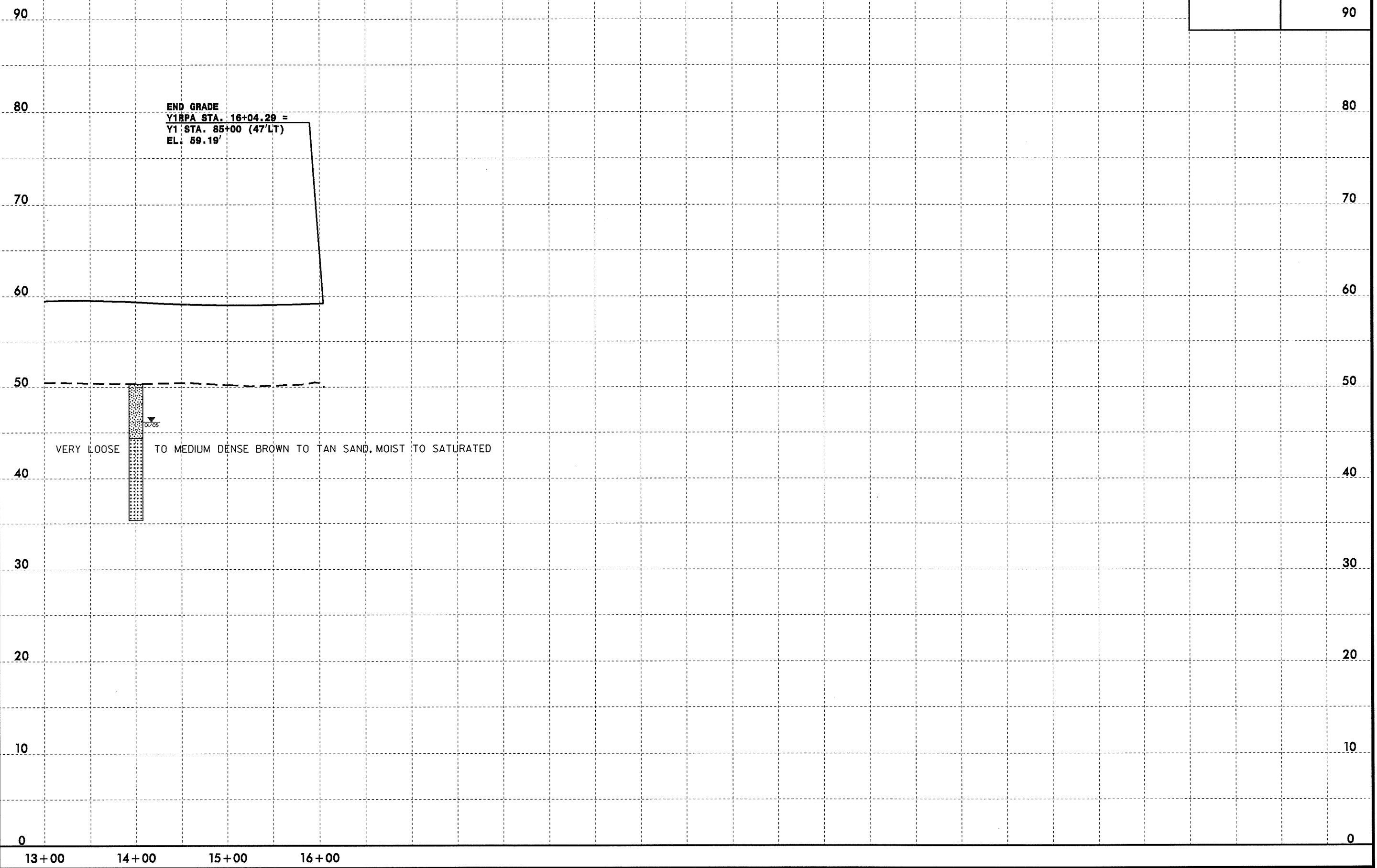
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-Y1RPA-

PROJECT REFERENCE NO.	SHEET NO.
R-2719A	59
ROADWAY DESIGN ENGINEER	HYDRAULICS ENGINEER
	90



13+00 14+00 15+00 16+00

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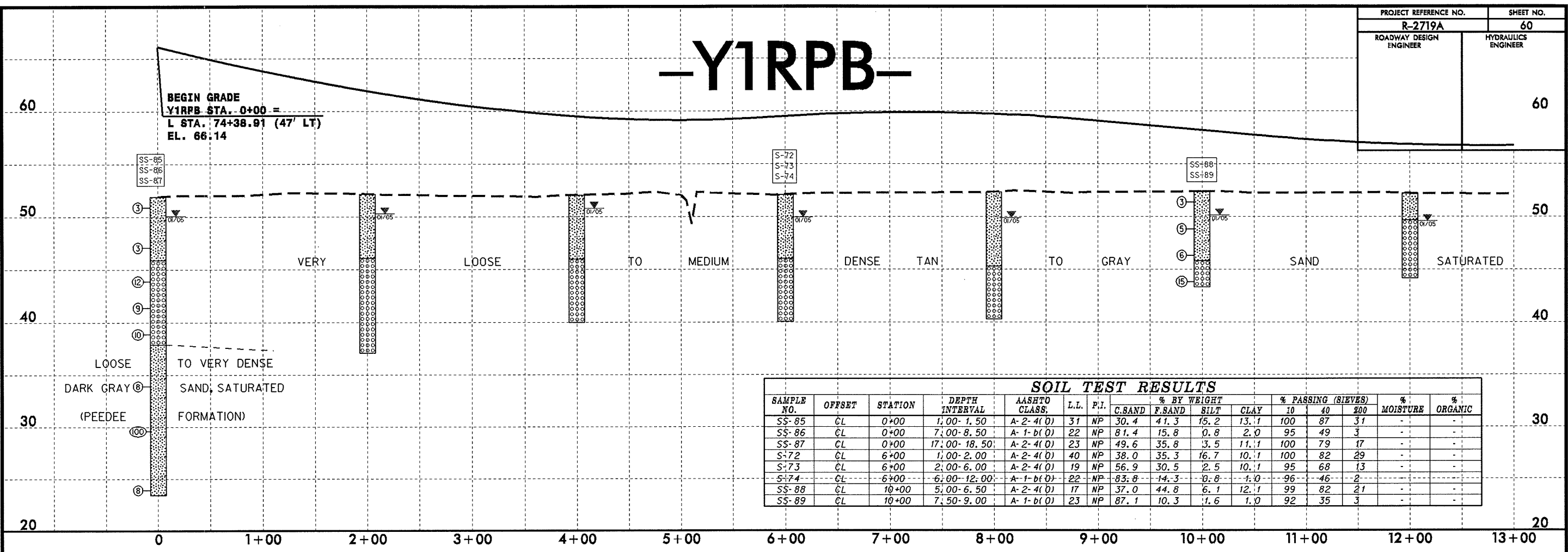
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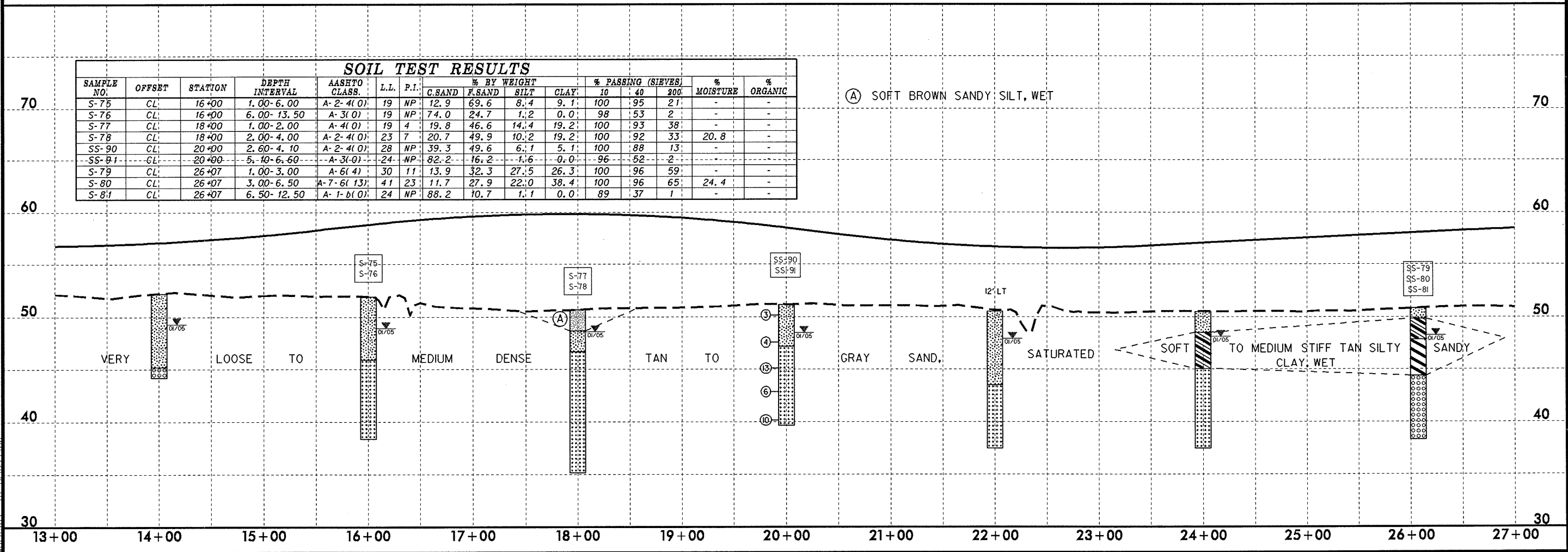
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-Y1RPB-



SAMPLE NO.	OFFSET	STATION	DEPTH INTERVAL	AASHTO CLASS.	L.L.	P.I.	% BY WEIGHT				% PASSING (SIEVES)			% MOISTURE	% ORGANIC
							C.SAND	F.SAND	SILT	CLAY	10	40	200		
SS-85	CL	0+00	1.00-1.50	A-2-4(0)	31	NP	30.4	41.3	15.2	13.1	100	87	31	-	-
SS-86	CL	0+00	7.00-8.50	A-1-b(0)	22	NP	81.4	15.8	0.8	2.0	95	49	3	-	-
SS-87	CL	0+00	17.00-18.50	A-2-4(0)	23	NP	49.6	35.8	3.5	11.1	100	79	17	-	-
S-72	CL	6+00	1.00-2.00	A-2-4(0)	40	NP	38.0	35.3	16.7	10.1	100	82	29	-	-
S-73	CL	6+00	2.00-6.00	A-2-4(0)	19	NP	56.9	30.5	2.5	10.1	95	68	13	-	-
S-74	CL	6+00	6.00-12.00	A-1-b(0)	22	NP	83.8	14.3	0.8	1.0	96	46	2	-	-
SS-88	CL	10+00	5.00-6.50	A-2-4(0)	17	NP	37.0	44.8	6.1	12.1	99	82	21	-	-
SS-89	CL	10+00	7.50-9.00	A-1-b(0)	23	NP	87.1	10.3	1.6	1.0	92	35	3	-	-



SAMPLE NO.	OFFSET	STATION	DEPTH INTERVAL	AASHTO CLASS.	L.L.	P.I.	% BY WEIGHT				% PASSING (SIEVES)			% MOISTURE	% ORGANIC
							C.SAND	F.SAND	SILT	CLAY	10	40	200		
S-75	CL	16+00	1.00-6.00	A-2-4(0)	19	NP	12.9	69.6	8.4	9.1	100	95	21	-	-
S-76	CL	16+00	6.00-13.50	A-3(0)	19	NP	74.0	24.7	1.2	0.0	98	53	2	-	-
S-77	CL	18+00	1.00-2.00	A-4(0)	19	4	19.8	46.6	14.4	19.2	100	93	38	-	-
S-78	CL	18+00	2.00-4.00	A-2-4(0)	23	7	20.7	49.9	10.2	19.2	100	92	33	20.8	-
SS-90	CL	20+00	2.60-4.10	A-2-4(0)	28	NP	39.3	49.6	6.1	5.1	100	88	13	-	-
SS-91	CL	20+00	5.10-6.60	A-3(0)	24	NP	82.2	16.2	1.6	0.0	96	52	2	-	-
S-79	CL	26+07	1.00-3.00	A-6(4)	30	11	13.9	32.3	27.5	26.3	100	96	59	-	-
S-80	CL	26+07	3.00-6.50	A-7-6(13)	41	23	11.7	27.9	22.0	38.4	100	96	65	24.4	-
S-81	CL	26+07	6.50-12.50	A-1-b(0)	24	NP	88.2	10.7	1.1	0.0	89	37	1	-	-

Ⓐ SOFT BROWN SANDY SILT, WET

5/28/99
 12-AUG-2005 09:40
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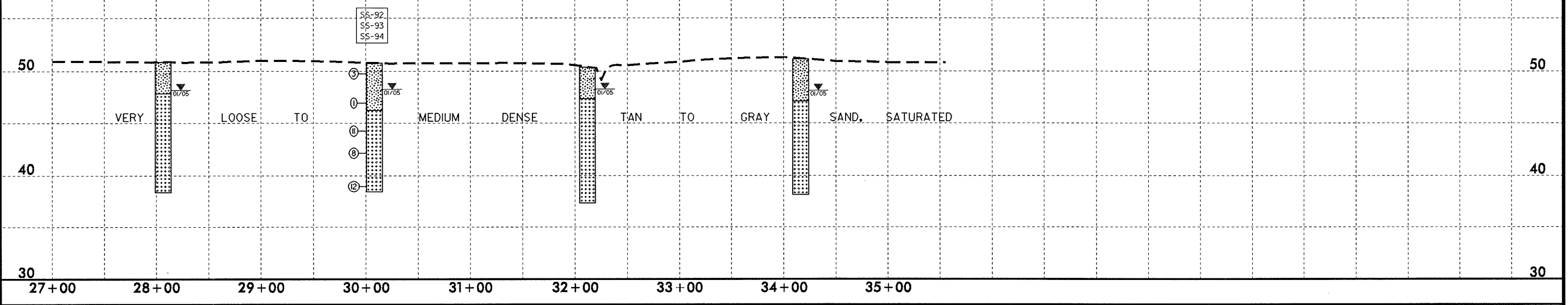
5/28/99

-Y1RPB-

END GRADE
Y1RPB STA. 35+55.41 =
Y1 STA. 48+62.35 (59' LT)
EL. 58.24

PROJECT REFERENCE NO.		SHEET NO.	
R-2719A		61	
ROADWAY DESIGN ENGINEER		HYDRAULICS ENGINEER	
		70	

SOIL TEST RESULTS															
SAMPLE NO.	OFFSET	STATION	DEPTH INTERVAL	AASHTO CLASS.	L.L.	P.I.	% BY WEIGHT				% PASSING (SIEVES)			% MOISTURE	% ORGANIC
							C.SAND	F.SAND	SILT	CLAY	10	40	200		
SS-92	CL	30+08	1.00- 1.50	A-2-4(0)	18	3	36.6	33.5	17.8	12.1	99	78	33	-	-
SS-93	CL	30+08	2.80- 4.30	A-2-4(0)	24	8	35.3	38.3	10.3	16.2	100	80	30	-	-
SS-94	CL	30+08	10.80- 12.30	A-3(0)	21	NP	83.1	14.4	1.4	1.0	94	52	3	-	-

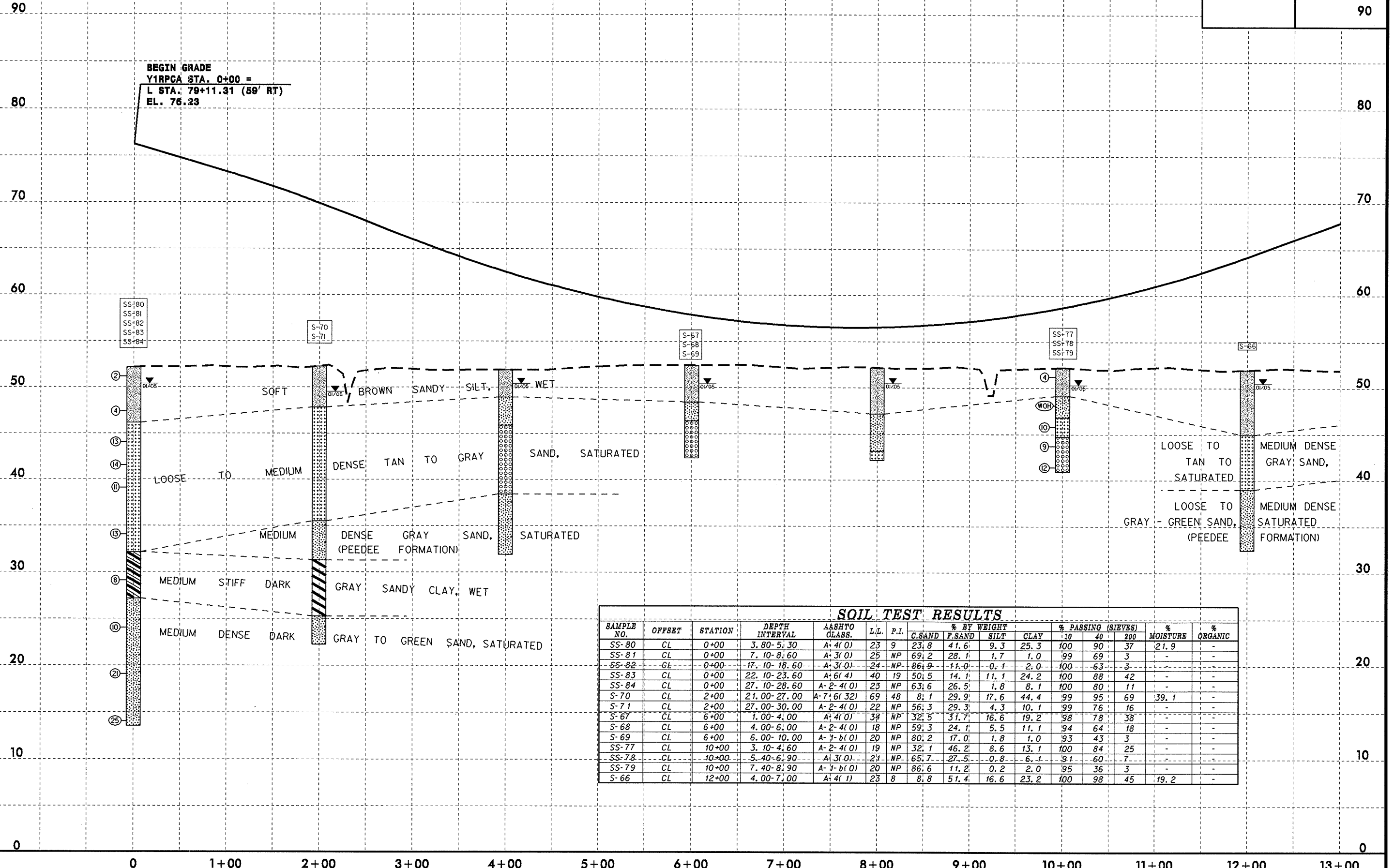


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PROJECT REFERENCE NO.		SHEET NO.	
R-2719A		62	
ROADWAY DESIGN ENGINEER		HYDRAULICS ENGINEER	
		90	

-YIRPCA-

BEGIN GRADE
YIRPCA STA. 0+00 =
L STA. 79+11.31 (59' RT)
EL. 76.23



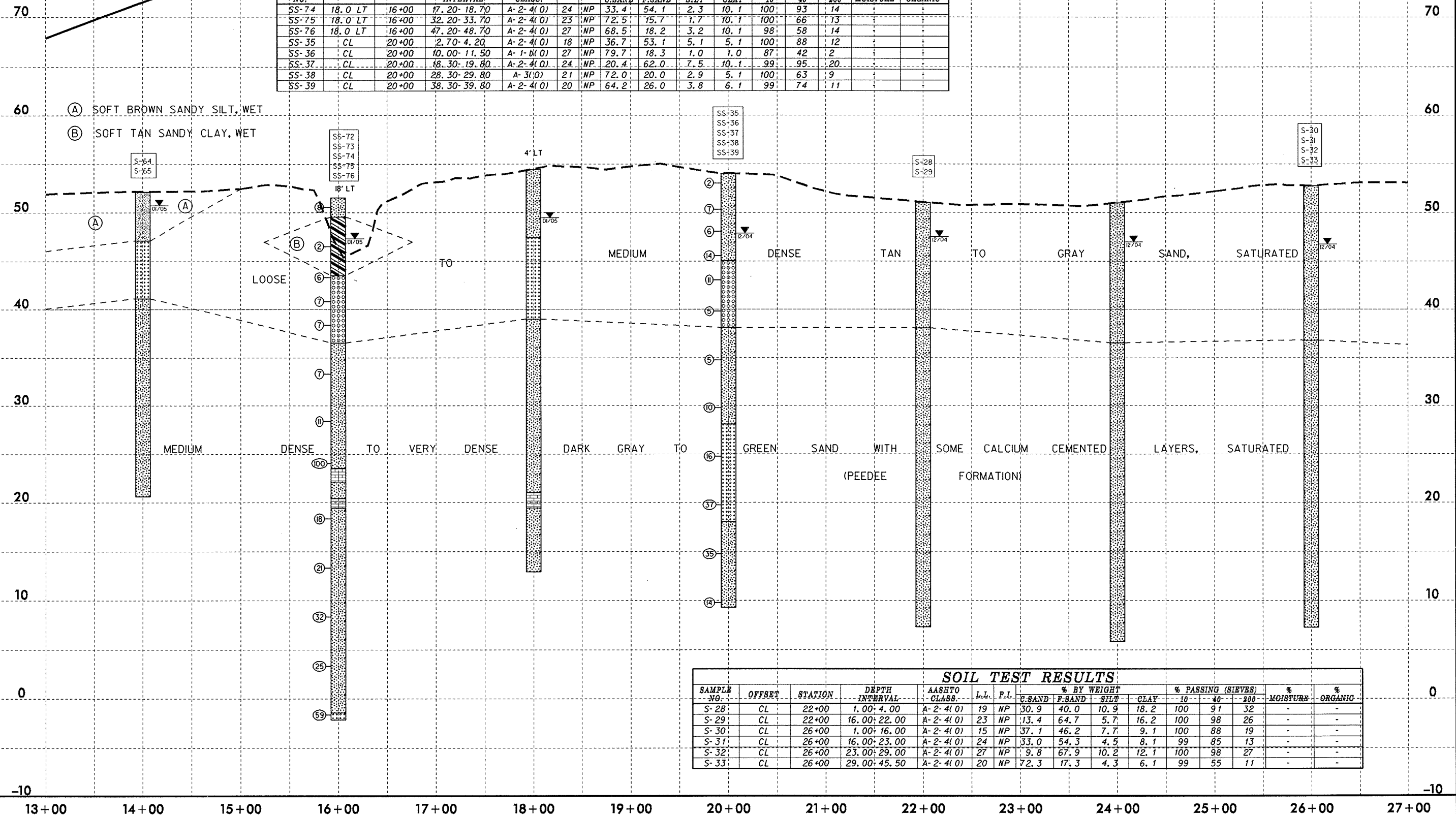
SOIL TEST RESULTS

SAMPLE NO.	OFFSET	STATION	DEPTH INTERVAL	AASHTO CLASS.	L.L.	P.I.	% BY WEIGHT				% PASSING (SIEVES)			% MOISTURE	% ORGANIC
							C.SAND	F.SAND	SILT	CLAY	#10	#40	#200		
SS-80	CL	0+00	3.80-5.30	A-4(0)	23	9	23.8	41.6	9.3	25.3	100	90	37	21.9	-
SS-81	CL	0+00	7.10-8.60	A-3(0)	25	NP	69.2	28.1	1.7	1.0	99	69	3	-	-
SS-82	GL	0+00	17.10-18.60	A-3(0)	24	NP	86.9	11.0	-0.1	-2.0	100	63	3	-	-
SS-83	CL	0+00	22.10-23.60	A-6(4)	40	19	50.5	14.1	11.1	24.2	100	88	42	-	-
SS-84	CL	0+00	27.10-28.60	A-2-4(0)	23	NP	63.6	26.5	1.8	8.1	100	80	11	-	-
S-70	CL	2+00	21.00-27.00	A-7-6(32)	69	48	8.1	29.9	17.6	44.4	99	95	69	39.1	-
S-71	CL	2+00	27.00-30.00	A-2-4(0)	22	NP	56.3	29.3	4.3	10.1	99	76	16	-	-
S-67	CL	6+00	1.00-4.00	A-4(0)	34	NP	32.5	31.7	16.6	19.2	98	78	38	-	-
S-68	CL	6+00	4.00-6.00	A-2-4(0)	18	NP	59.3	24.1	5.5	11.1	94	64	18	-	-
S-69	CL	6+00	6.00-10.00	A-1-b(0)	20	NP	80.2	17.0	1.8	1.0	93	43	3	-	-
SS-77	CL	10+00	3.10-4.60	A-2-4(0)	19	NP	32.1	46.2	8.6	13.1	100	84	25	-	-
SS-78	CL	10+00	5.40-6.90	A-3(0)	21	NP	65.7	27.5	-0.8	-6.1	91	60	7	-	-
SS-79	CL	10+00	7.40-8.90	A-1-b(0)	20	NP	86.6	11.2	0.2	2.0	95	36	3	-	-
S-66	CL	12+00	4.00-7.00	A-4(1)	23	8	8.8	51.4	16.6	23.2	100	98	45	19.2	-

-Y1RPCA-

SOIL TEST RESULTS															
SAMPLE NO.	OFFSET	STATION	DEPTH INTERVAL	AASHTO CLASS.	L.L.	P.I.	% BY WEIGHT				% PASSING (SIEVES)			% MOISTURE	% ORGANIC
							C.SAND	F.SAND	SILT	CLAY	10	40	200		
S-64	CL	14+00	1.00-5.00	A-4(1)	29	9	16.8	42.0	21.0	20.2	100	93	47	-	-
S-65	CL	14+00	24.00-31.50	A-2-4(0)	27	8	34.2	42.4	9.2	14.1	99	83	26	23.2	-
SS-72	18.0 LT	16+00	4.00-5.50	A-6(5)	31	15	22.8	30.7	18.2	28.3	100	86	53	-	-
SS-73	18.0 LT	16+00	9.70-11.20	A-1-b(0)	23	NP	81.1	15.6	0.3	3.0	92	38	4	-	-

SOIL TEST RESULTS															
SAMPLE NO.	OFFSET	STATION	DEPTH INTERVAL	AASHTO CLASS.	L.L.	P.I.	% BY WEIGHT				% PASSING (SIEVES)			% MOISTURE	% ORGANIC
							C.SAND	F.SAND	SILT	CLAY	10	40	200		
SS-74	18.0 LT	16+00	17.20-18.70	A-2-4(0)	24	NP	33.4	54.1	2.3	10.1	100	93	14	-	-
SS-75	18.0 LT	16+00	32.20-33.70	A-2-4(0)	23	NP	72.5	15.7	1.7	10.1	100	66	13	-	-
SS-76	18.0 LT	16+00	47.20-48.70	A-2-4(0)	27	NP	68.5	18.2	3.2	10.1	98	58	14	-	-
SS-35	CL	20+00	2.70-4.20	A-2-4(0)	18	NP	36.7	53.1	5.1	5.1	100	88	12	-	-
SS-36	CL	20+00	10.00-11.50	A-1-b(0)	27	NP	79.7	18.3	1.0	1.0	87	42	2	-	-
SS-37	CL	20+00	18.30-19.80	A-2-4(0)	24	NP	20.4	62.0	7.5	10.1	99	95	20	-	-
SS-38	CL	20+00	28.30-29.80	A-3(0)	21	NP	72.0	20.0	2.9	5.1	100	63	9	-	-
SS-39	CL	20+00	38.30-39.80	A-2-4(0)	20	NP	64.2	26.0	3.8	6.1	99	74	11	-	-

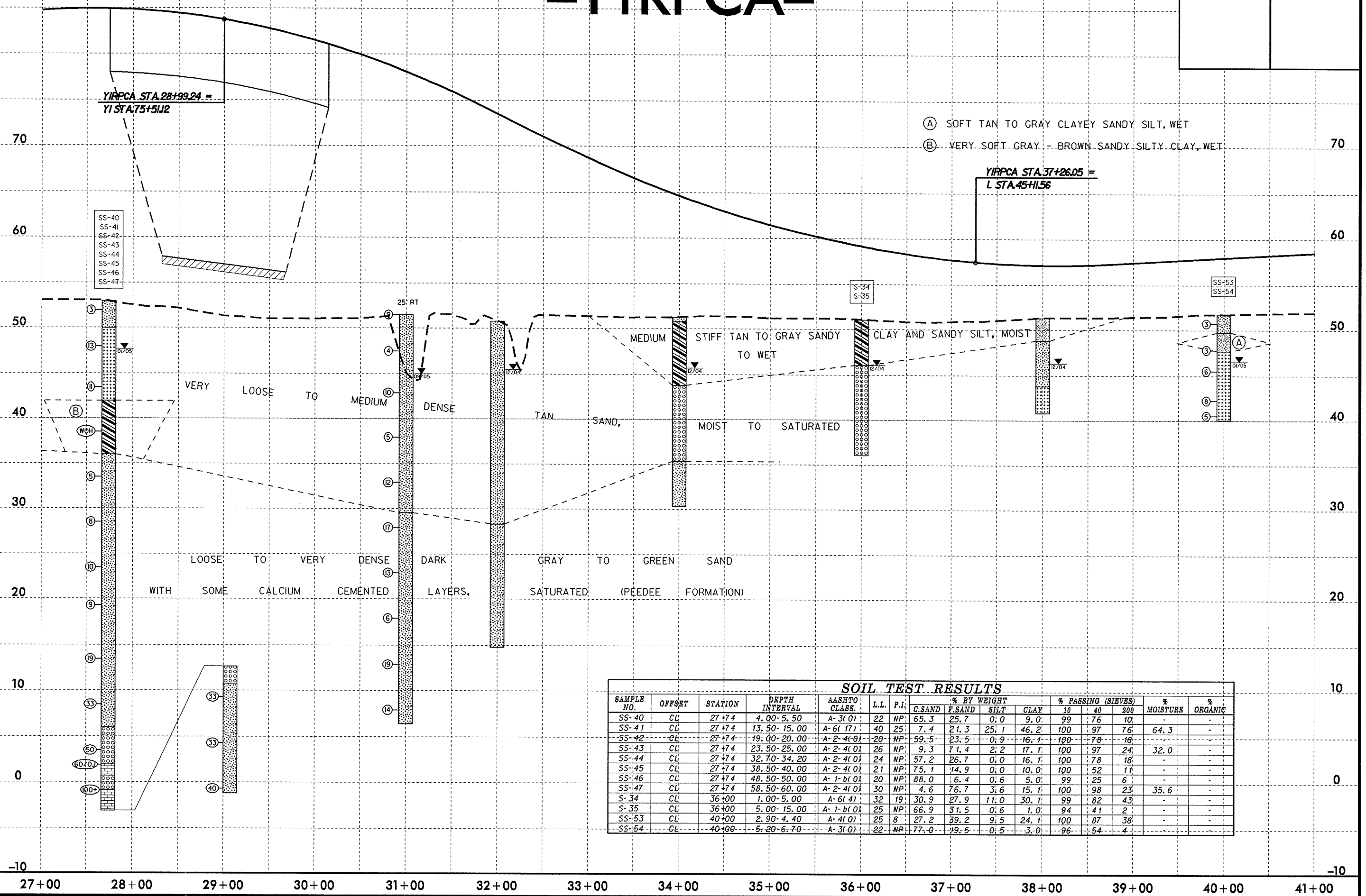


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-YIRPCA-

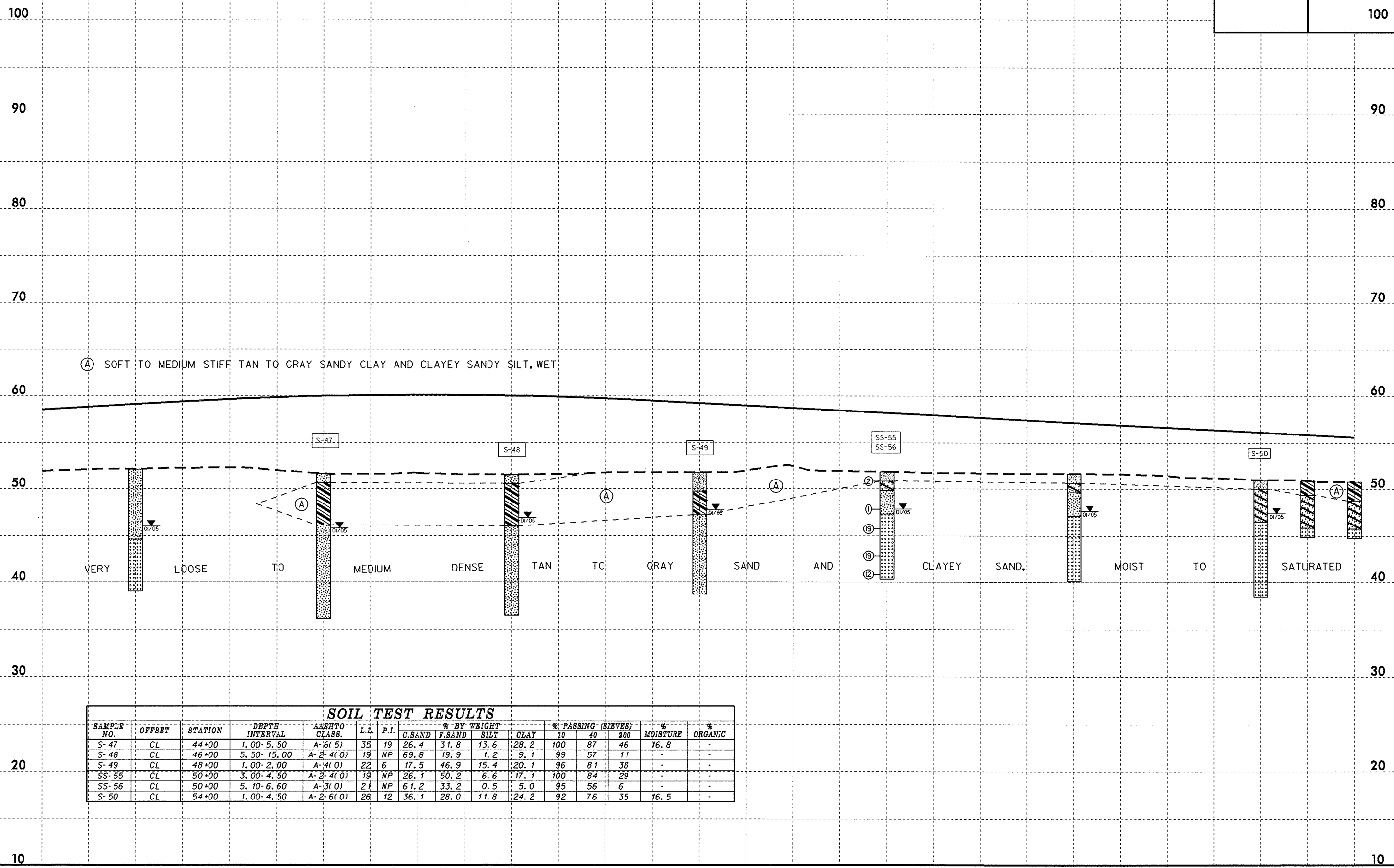
PROJECT REFERENCE NO. R-2719A	SHEET NO. 64
ROADWAY DESIGN ENGINEER	HYDRAULICS ENGINEER



SOIL TEST RESULTS															
SAMPLE NO.	OFFSET	STATION	DEPTH INTERVAL	AASHTO CLASS.	L.L.	P.I.	% BY WEIGHT				% PASSING (SIEVES)			% MOISTURE	% ORGANIC
							C.SAND	F.SAND	SILT	CLAY	10	40	200		
SS-40	CL	27+74	4.00-5.50	A-3(0)	22	NP	65.3	25.7	0.0	9.0	99	76	10	-	-
SS-41	CL	27+74	13.50-15.00	A-6(17)	40	25	7.4	21.3	25.1	46.2	100	97	76	64.3	-
SS-42	CL	27+74	19.00-20.00	A-2-4(0)	20	NP	59.5	23.5	0.9	16.1	100	78	18	-	-
SS-43	CL	27+74	23.50-25.00	A-2-4(0)	26	NP	9.3	71.4	2.2	17.1	100	97	24	32.0	-
SS-44	CL	27+74	32.70-34.20	A-2-4(0)	24	NP	57.2	26.7	0.0	16.1	100	78	18	-	-
SS-45	CL	27+74	38.50-40.00	A-2-4(0)	21	NP	75.1	14.9	0.0	10.0	100	52	11	-	-
SS-46	CL	27+74	48.50-50.00	A-1-b(0)	20	NP	88.0	6.4	0.6	5.0	99	25	6	-	-
SS-47	CL	27+74	58.50-60.00	A-2-4(0)	30	NP	4.6	76.7	3.6	15.1	100	98	23	35.6	-
S-34	CL	36+00	1.00-5.00	A-6(4)	32	19	30.9	27.9	11.0	30.1	99	82	43	-	-
S-35	CL	36+00	5.00-15.00	A-1-b(0)	25	NP	66.9	31.5	0.6	1.0	94	41	2	-	-
SS-53	CL	40+00	2.90-4.40	A-4(0)	25	8	27.2	39.2	9.5	24.1	100	87	38	-	-
SS-54	CL	40+00	5.20-6.70	A-3(0)	22	NP	77.0	19.5	0.5	3.0	96	54	4	-	-

-Y1RPCA-

PROJECT REFERENCE NO. R-2719A	SHEET NO. 65
ROADWAY DESIGN ENGINEER	HYDRAULICS ENGINEER
100	

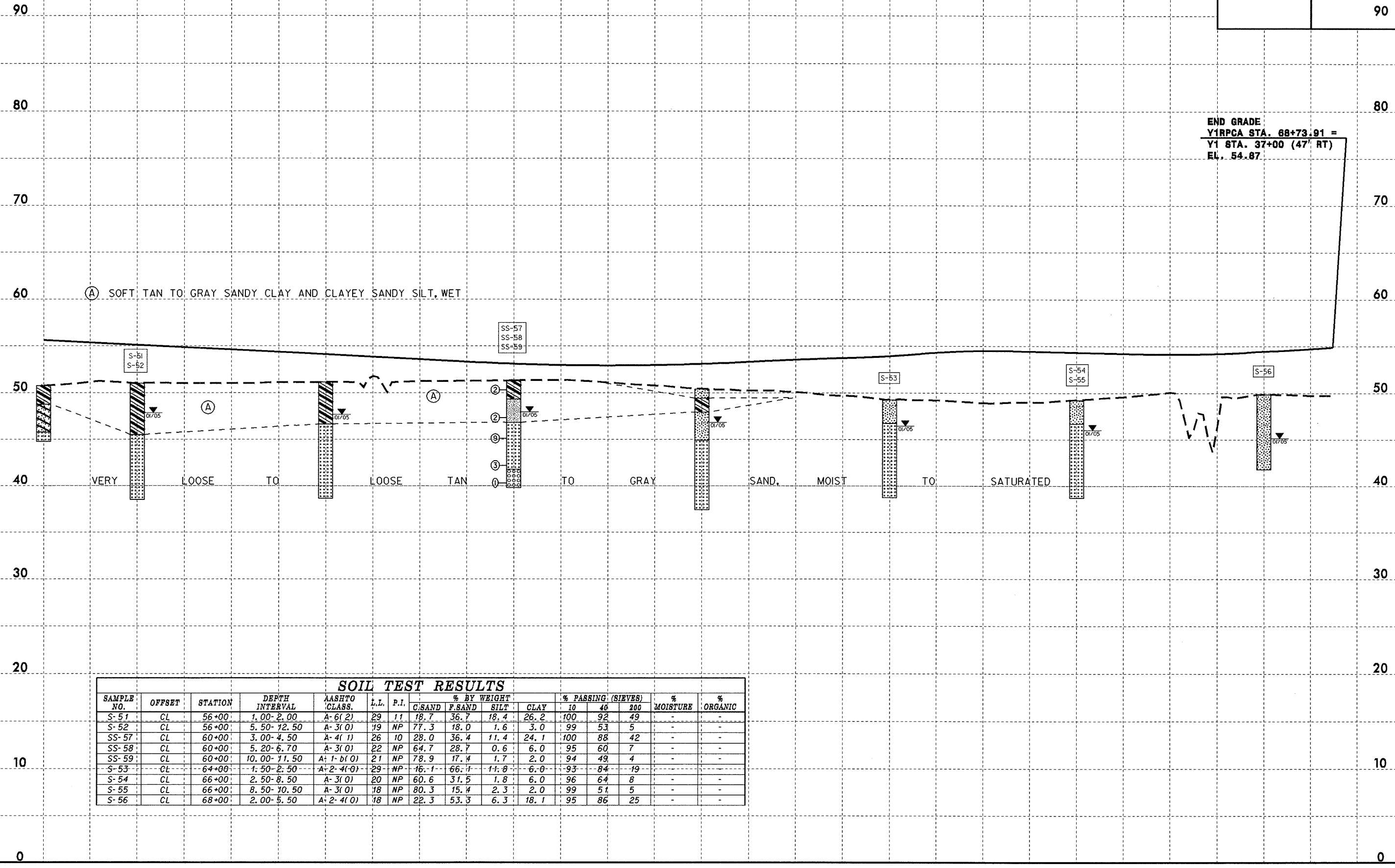


SOIL TEST RESULTS															
SAMPLE NO.	OFFSET	STATION	DEPTH INTERVAL	AASHTO CLASS.	L.L.	P.I.	% BY WEIGHT				% PASSING (SIEVES)			% MOISTURE	% ORGANIC
							C.SAND	F.SAND	SILT	CLAY	20	40	200		
S-47	CL	44+00	1.00-5.50	A-6(5)	35	19	26.4	31.8	13.6	28.2	100	87	46	16.8	-
S-48	CL	46+00	5.50-15.00	A-2-4(0)	19	NP	69.8	19.9	1.2	9.1	99	57	11	-	-
S-49	CL	48+00	1.00-2.00	A-4(0)	22	6	17.5	46.9	15.4	20.1	96	81	38	-	-
SS-55	CL	50+00	3.00-4.50	A-2-4(0)	19	NP	26.1	50.2	6.6	17.1	100	84	29	-	-
SS-56	CL	50+00	5.10-6.60	A-3(0)	21	NP	61.2	33.2	0.5	5.0	95	56	6	-	-
S-50	CL	54+00	1.00-4.50	A-2-6(0)	26	12	36.1	28.0	11.8	24.2	92	76	35	16.5	-

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-Y1RPCA-

END GRADE
Y1RPCA STA. 68+73.91 =
Y1 STA. 37+00 (47' RT)
EL. 54.87

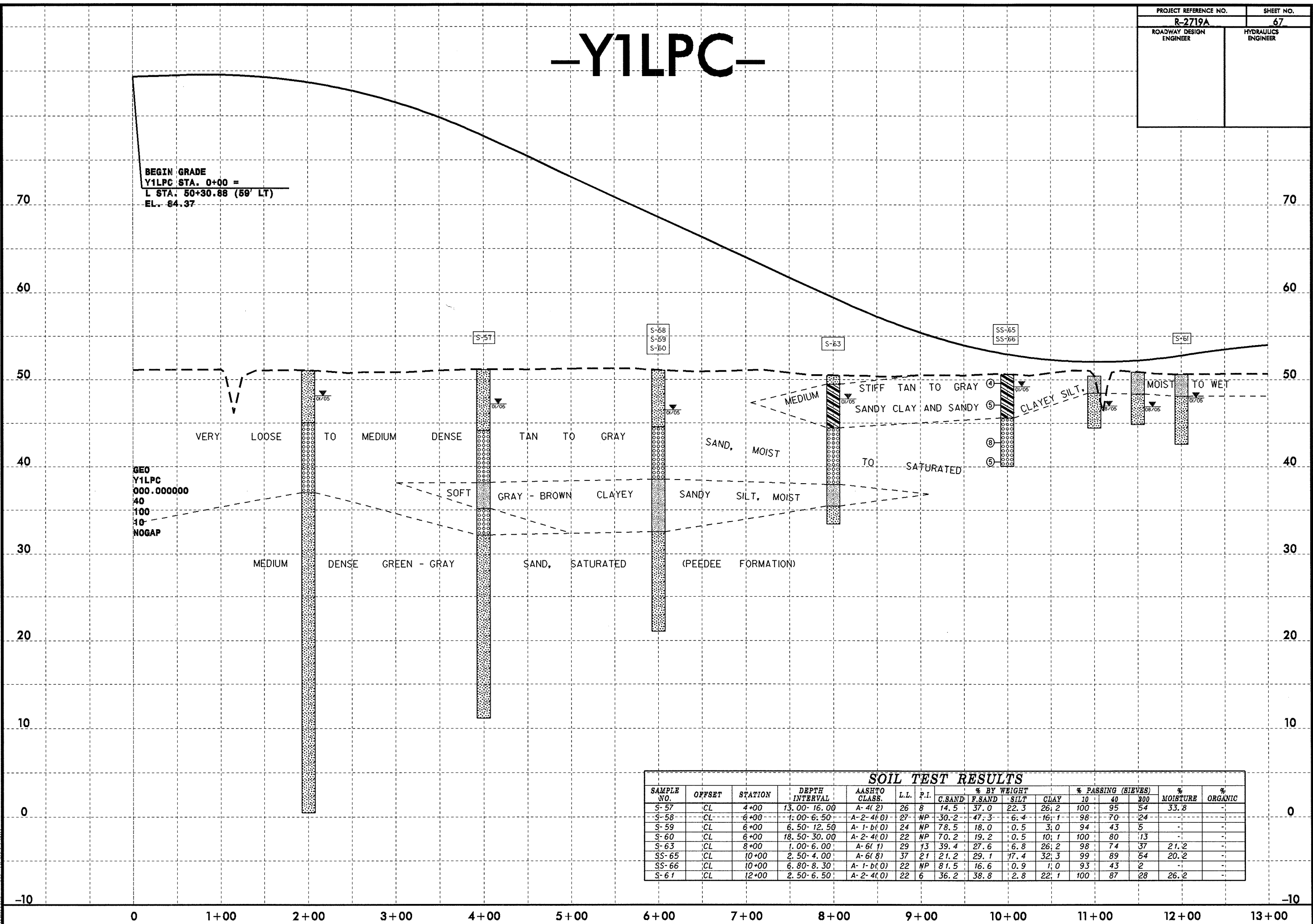


SOIL TEST RESULTS															
SAMPLE NO.	OFFSET	STATION	DEPTH INTERVAL	AASHTO CLASS.	L.L.	P.I.	% BY WEIGHT				% PASSING (SIEVES)			% MOISTURE	% ORGANIC
							C.SAND	F.SAND	SILT	CLAY	10	40	200		
S-51	CL	56+00	1.00-2.00	A-6(2)	29	11	18.7	36.7	18.4	26.2	100	92	49	-	-
S-52	CL	56+00	5.50-12.50	A-3(0)	19	NP	77.3	18.0	1.6	3.0	99	53	5	-	-
SS-57	CL	60+00	3.00-4.50	A-4(1)	26	10	28.0	36.4	11.4	24.1	100	88	42	-	-
SS-58	CL	60+00	5.20-6.70	A-3(0)	22	NP	64.7	28.7	0.6	6.0	95	60	7	-	-
SS-59	CL	60+00	10.00-11.50	A-1-b(0)	21	NP	78.9	17.4	1.7	2.0	94	49	4	-	-
S-53	CL	64+00	1.50-2.50	A-2-4(0)	29	NP	16.1	66.1	11.8	6.0	93	84	19	-	-
S-54	CL	66+00	2.50-8.50	A-3(0)	20	NP	60.6	31.5	1.8	6.0	96	64	8	-	-
S-55	CL	66+00	8.50-10.50	A-3(0)	18	NP	80.3	15.4	2.3	2.0	99	51	5	-	-
S-56	CL	68+00	2.00-5.50	A-2-4(0)	18	NP	22.3	53.3	6.3	18.1	95	86	25	-	-

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-Y1LPC-

BEGIN GRADE
Y1LPC STA. 0+00 =
L STA. 50+30.88 (59' LT)
EL. 84.37



GEO
Y1LPC
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40
100
10
NOGAP

SAMPLE NO.	OFFSET	STATION	DEPTH INTERVAL	AASHTO CLASS.	L.L.	P.I.	% BY WEIGHT				% PASSING (SIEVES)			% MOISTURE	% ORGANIC
							C.SAND	F.SAND	SILT	CLAY	10	40	200		
							27.5	75	200	425					
S-57	CL	4+00	13.00-16.00	A-4(2)	26	8	14.5	37.0	22.3	26.2	100	95	54	33.8	-
S-58	CL	6+00	1.00-6.50	A-2-4(0)	27	NP	30.2	47.3	6.4	16.1	98	70	24	-	-
S-59	CL	6+00	6.50-12.50	A-1-b(0)	24	NP	78.5	18.0	0.5	3.0	94	43	5	-	-
S-60	CL	6+00	18.50-30.00	A-2-4(0)	22	NP	70.2	19.2	0.5	10.1	100	80	13	-	-
S-63	CL	8+00	1.00-6.00	A-6(1)	29	13	39.4	27.6	6.8	26.2	98	74	37	21.2	-
SS-65	CL	10+00	2.50-4.00	A-6(8)	37	21	21.2	29.1	17.4	32.3	99	89	54	20.2	-
SS-66	CL	10+00	6.80-8.30	A-1-b(0)	22	NP	81.5	16.6	0.9	1.0	93	43	2	-	-
S-61	CL	12+00	2.50-6.50	A-2-4(0)	22	6	36.2	38.8	2.8	22.1	100	87	28	26.2	-

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PROJECT REFERENCE NO.		SHEET NO.	
R-2719A		68	
ROADWAY DESIGN ENGINEER		HYDRAULICS ENGINEER	
		90	

-Y1LPC-

90
80
70
60
50
40
30
20
10
0

90
80
70
60
50
40
30
20
10
0

END GRADE
Y1LPC STA. 14+34.42 =
Y1 STA. 67+22.95 (47' RT)
EL. 54.77

S-62

SANDY CLAYEY SILT, MOIST TO WET
LOOSE TAN TO GRAY SAND,
MOIST TO SATURATED

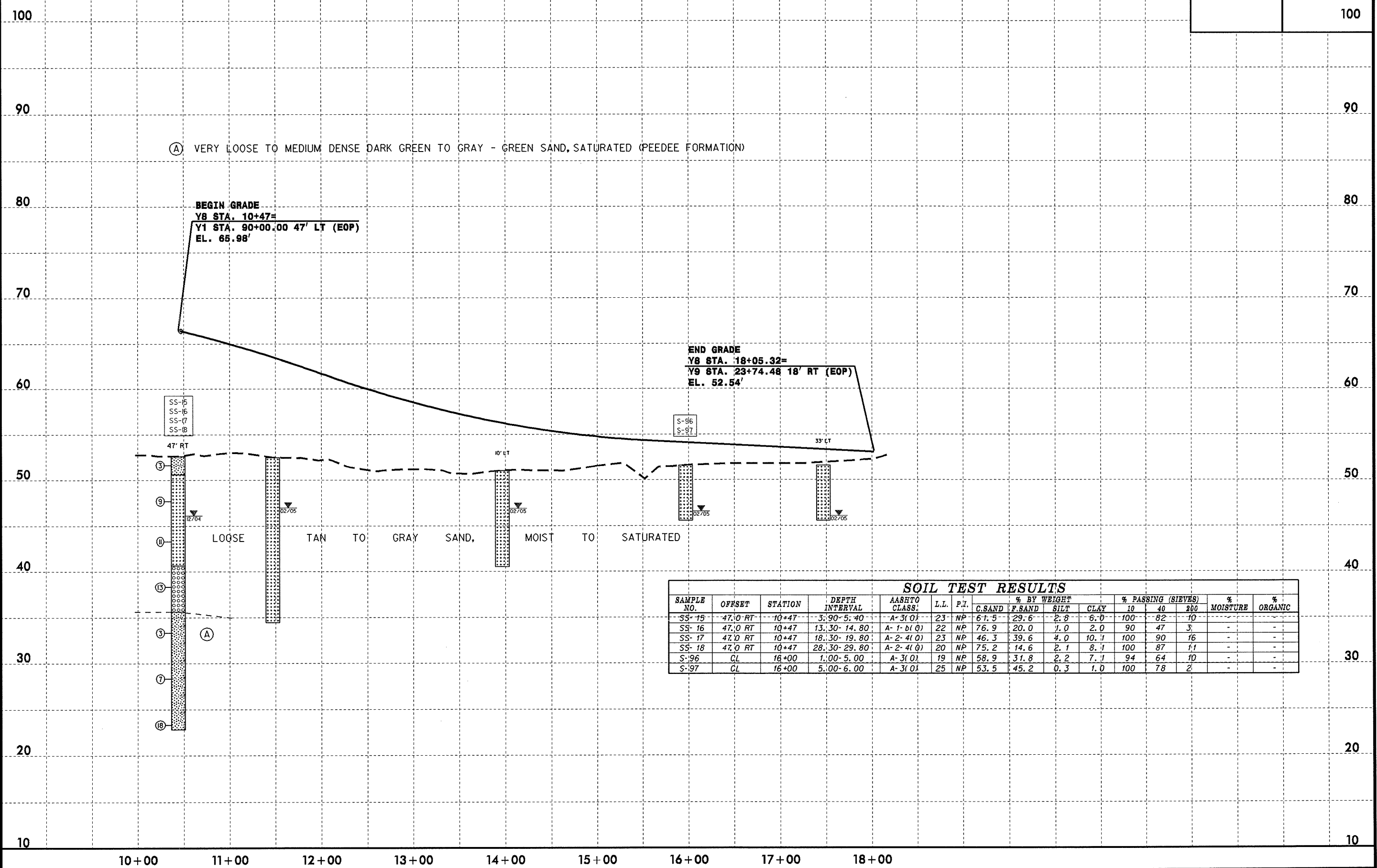
SAMPLE NO.	OFFSET	STATION	DEPTH INTERVAL	AASHTO CLASS	L.L.	P.I.	% BY WEIGHT				% PASSING (SIEVES)			% MOISTURE	% ORGANIC
							C.SAND	F.SAND	SILT	CLAY	10	40	200		
S-62	CL	14+00	1.00-5.00	A-2-4(0)	21	NP	26.8	44.7	12.5	16.1	100	89	33	-	-

13+00

14+00

0

-Y8-



(A) VERY LOOSE TO MEDIUM DENSE DARK GREEN TO GRAY - GREEN SAND, SATURATED (PEEDEE FORMATION)

BEGIN GRADE
 Y8 STA. 10+47=
 Y1 STA. 90+00.00 47' LT (EOP)
 EL. 65.98'

END GRADE
 Y8 STA. 18+05.32=
 Y9 STA. 23+74.48 18' RT (EOP)
 EL. 52.54'

SS-15
 SS-16
 SS-17
 SS-18

S-96
 S-97

47' RT

10' LT

33' CT

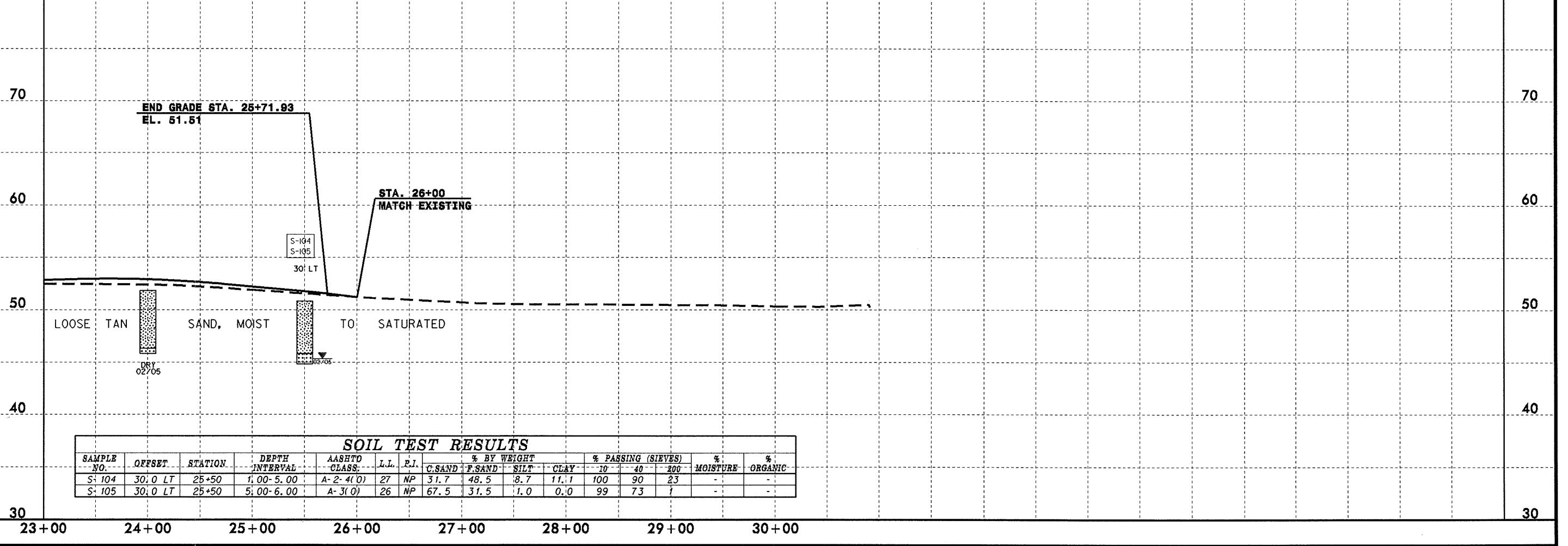
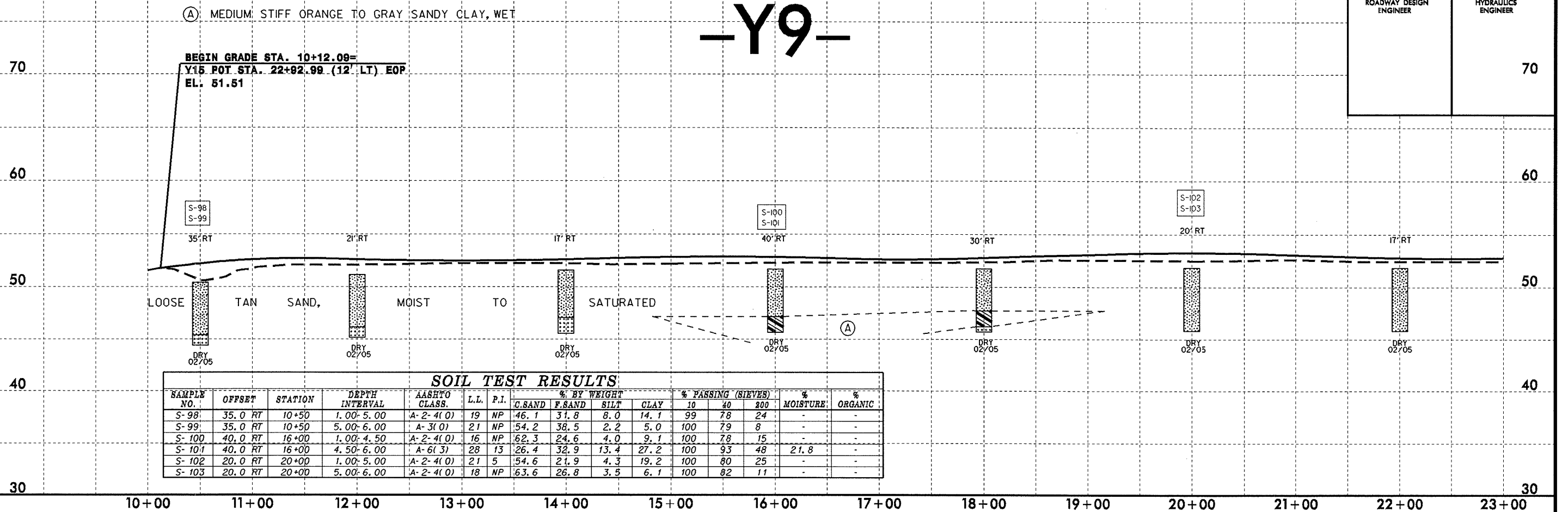
LOOSE TAN TO GRAY SAND, MOIST TO SATURATED

SOIL TEST RESULTS

SAMPLE NO.	OFFSET	STATION	DEPTH INTERVAL	AASHTO CLASS.	L.L.	P.I.	% BY WEIGHT				% PASSING (SIEVES)			% MOISTURE	% ORGANIC
							C.SAND	F.SAND	SILT	CLAY	10	40	200		
SS-15	47.0 RT	10+47	3.90-5.40	A-3(0)	23	NP	61.5	29.6	2.8	6.0	100	82	10	-	-
SS-16	47.0 RT	10+47	13.30-14.80	A-1-b(0)	22	NP	76.9	20.0	1.0	2.0	90	47	3	-	-
SS-17	47.0 RT	10+47	18.30-19.80	A-2-4(0)	23	NP	46.3	39.6	4.0	10.1	100	90	16	-	-
SS-18	47.0 RT	10+47	28.30-29.80	A-2-4(0)	20	NP	75.2	14.6	2.1	8.1	100	87	11	-	-
S-96	CL	16+00	1.00-5.00	A-3(0)	19	NP	58.9	31.8	2.2	7.1	94	64	10	-	-
S-97	CL	16+00	5.00-6.00	A-3(0)	25	NP	53.5	45.2	0.3	1.0	100	78	2	-	-

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-Y9-

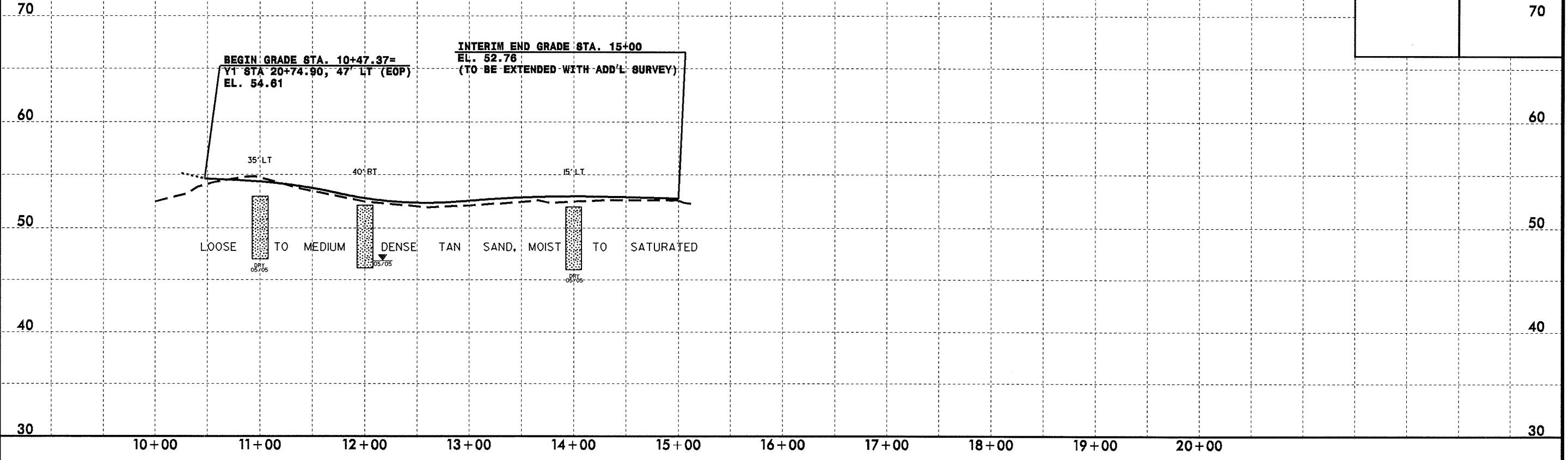


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DATE: 05/28/99

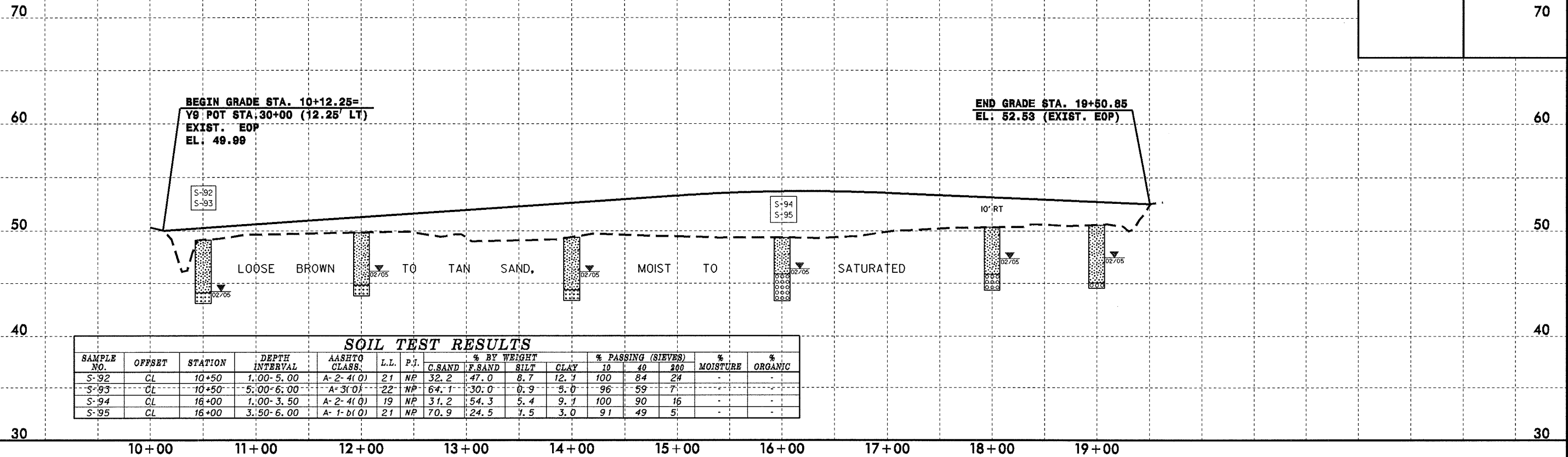
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PROJECT REFERENCE NO. R-2719A	SHEET NO. 71
ROADWAY DESIGN ENGINEER	HYDRAULICS ENGINEER
	70



5/28/99

-Y14-



SOIL TEST RESULTS															
SAMPLE NO.	OFFSET	STATION	DEPTH INTERVAL	AASHTO CLASS.	L.L.	P.I.	% BY WEIGHT				% PASSING (SIEVES)			% MOISTURE	% ORGANIC
							C.SAND	F.SAND	SILT	CLAY	10	40	200		
S-92	CL	10+50	1.00-5.00	A-2-4(0)	21	NP	32.2	47.0	8.7	12.1	100	84	24	-	-
S-93	CL	10+50	5.00-6.00	A-3(0)	22	NP	64.1	30.0	0.9	5.0	96	59	7	-	-
S-94	CL	16+00	1.00-3.50	A-2-4(0)	19	NP	31.2	54.3	5.4	9.1	100	90	16	-	-
S-95	CL	16+00	3.50-6.00	A-1-b(0)	21	NP	70.9	24.5	1.5	3.0	91	49	5	-	-

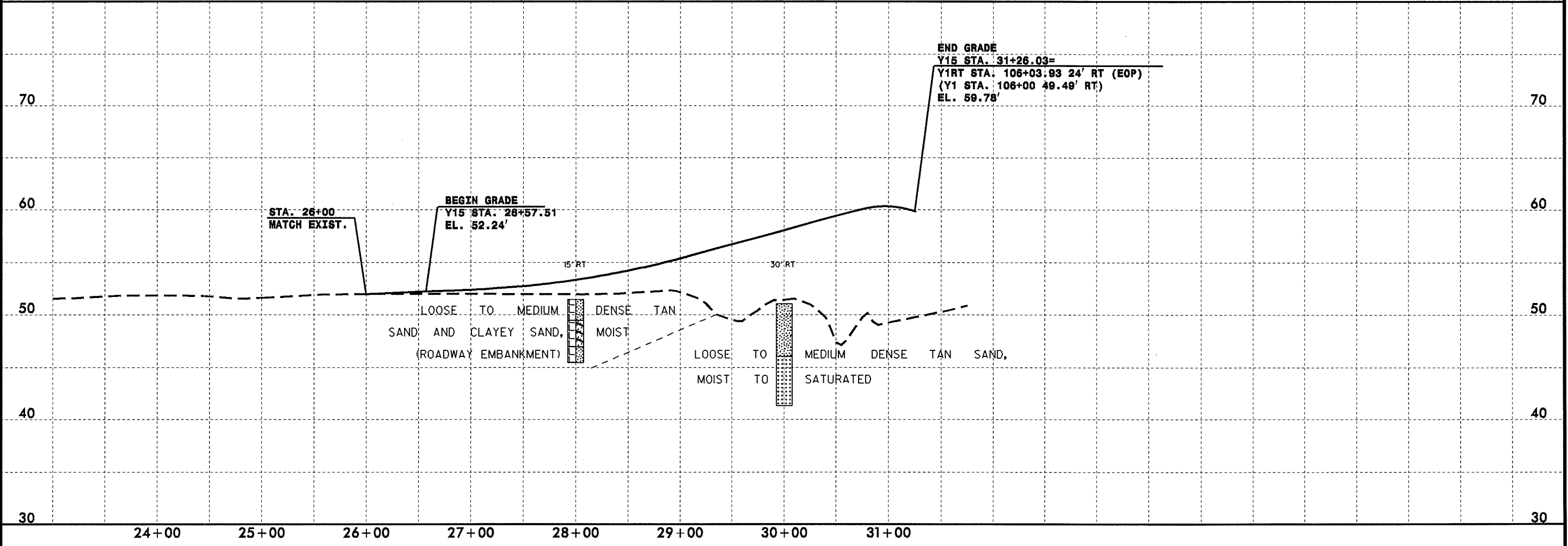
5/28/99

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5/28/99

PROJECT REFERENCE NO.	SHEET NO.
R-2719A	73
ROADWAY DESIGN ENGINEER	HYDRAULICS ENGINEER
	70

-Y15-



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 5/28/99

-SVRD1-

PROJECT REFERENCE NO.	SHEET NO.
R-2719A	74
ROADWAY DESIGN ENGINEER	HYDRAULICS ENGINEER

BEGIN GRADE
SVRD1 STA. 10+10.82
EL. 86.61
(EOP OF EXISTING Y3)

END GRADE
SVRD1 STA. 21+50
EL. 79.08

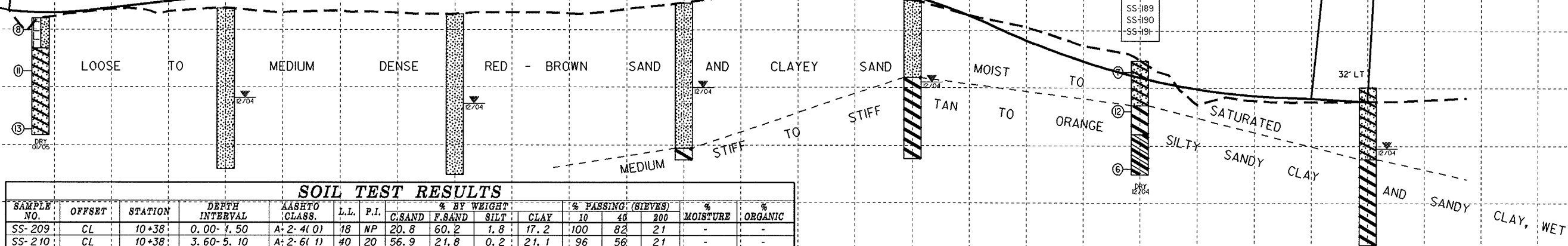
END CONSTRUCTION
STA. 22+00
MATCH EXIST.

100
90
80
70
60

100
90
80
70
60

SOIL TEST RESULTS															
SAMPLE NO.	OFFSET	STATION	DEPTH INTERVAL	AASHTO CLASS.	L.L.	P.I.	% BY WEIGHT				% PASSING (SIEVES)			% MOISTURE	% ORGANIC
							C.SAND	F.SAND	SILT	CLAY	10	40	200		
SS-209	CL	10+38	0.00-1.50	A-2-4(0)	18	NP	20.8	60.2	1.8	17.2	100	82	21	-	-
SS-210	CL	10+38	3.60-5.10	A-2-6(1)	40	20	56.9	21.8	0.2	21.1	96	56	21	-	-
S-192	CL	16+00	2.0-3.0	A-2-4(0)	23	7	40.4	35.6	7.9	16.1	98	69	25	-	-
S-193	CL	16+00	12.6-13.6	A-7-6(22)	62	40	8.0	38.0	25.8	28.1	100	94	62	-	-
SS-189	CL	20+00	0.0-1.5	A-2-6(1)	31	15	29.3	41.8	6.7	22.1	99	79	31	-	-
SS-190	CL	20+00	3.4-4.9	A-7-5(14)	66	29	27.3	17.5	17.0	38.2	98	74	55	-	-
SS-191	CL	20+00	8.4-9.9	A-6(6)	38	20	3.4	47.8	12.6	36.2	100	98	50	-	-

10+00 11+00 12+00 13+00 14+00 15+00 16+00 17+00 18+00 19+00 20+00 21+00 22+00



I:\2005\0918\PROJECTS\12719A\GEO\TECHNICAL\PROF\2719A_GEO_P1-SVRD1.dgn

5/28/99

-SVRD2-

PROJECT REFERENCE NO.	SHEET NO.
R-2719A	75
ROADWAY DESIGN ENGINEER	HYDRAULICS ENGINEER
	120

120

110

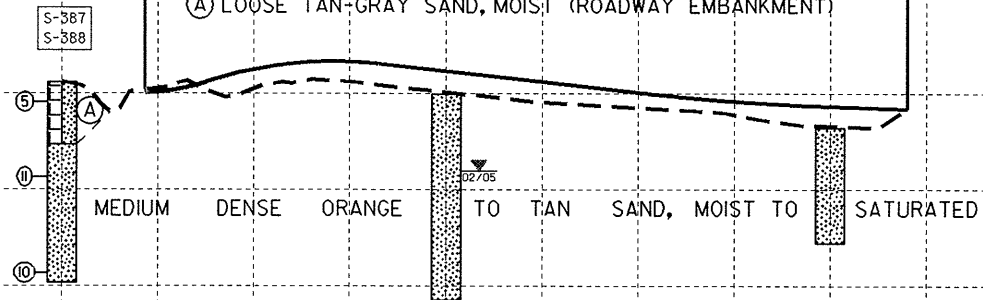
100

90

80

BEGIN GRADE
SVRD2 STA. 10+43.18=
Y24 POT STA. 29+40.00 (43.18' LT) EOP
EL. 105.02

END GRADE
SVRD2 STA. 14+40.01=
EL. 104.21
(EXIST. PARKING LOT)



SOIL TEST RESULTS

SAMPLE NO.	OFFSET	STATION	DEPTH INTERVAL	AASHTO CLASS.	L.L.	P.I.	% BY WEIGHT				% PASSING (SIEVES)			% MOISTURE	% ORGANIC
							C.SAND	F.SAND	SILT	CLAY	10	40	200		
S-387	CL	10+00	0.00- 1.50	A-2-4(0)	18	NP	9.9	71.9	8.2	10.1	100	96	21	-	-
S-388	CL	10+00	3.90- 5.40	A-2-4(0)	21	NP	1.8	79.4	4.7	14.1	99	98	21	-	-

10+00 11+00 12+00 13+00 14+00

120

110

100

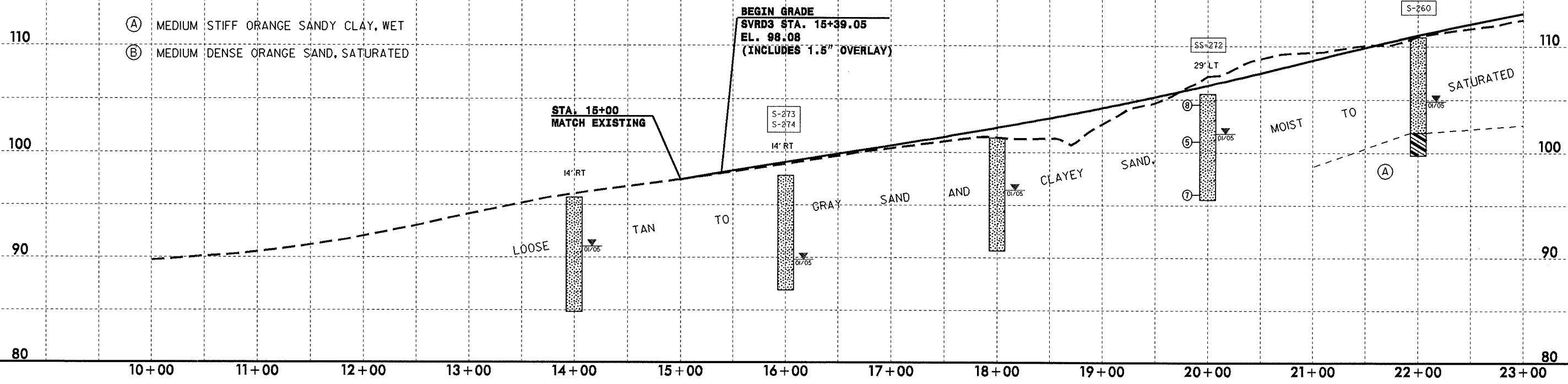
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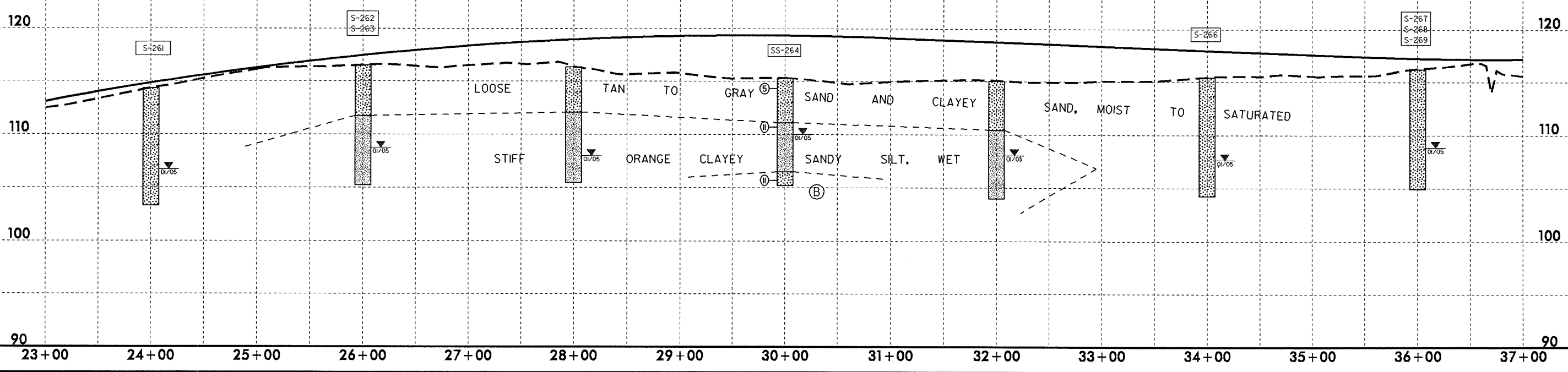
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-SVRD3-

SAMPLE NO.	OFFSET	STATION	DEPTH INTERVAL	AASHTO CLASS.	L.L.	P.I.	% BY WEIGHT				% PASSING (SIEVES)			% MOISTURE	% ORGANIC
							C.SAND	F.SAND	SILT	CLAY	10	40	200		
S-273	14.0' LT	16+00	1.00-2.00	A-2-4(0)	22	3	2.3	67.7	9.8	20.2	100	99	32	-	-
S-274	14.0' RT	16+00	7.00-8.00	A-2-4(0)	23	NP	2.5	80.6	3.7	13.1	100	100	19	-	-
SS-272	29.0' LT	20+00	8.50-10.00	A-2-4(0)	23	NP	17.6	71.2	3.1	8.1	100	95	13	-	-
S-260	CL	22+00	7.00-8.00	A-2-4(0)	26	6	0.9	72.3	7.7	19.1	100	100	30	25.5	-



SAMPLE NO.	OFFSET	STATION	DEPTH INTERVAL	AASHTO CLASS.	L.L.	P.I.	% BY WEIGHT				% PASSING (SIEVES)			% MOISTURE	% ORGANIC
							C.SAND	F.SAND	SILT	CLAY	10	40	200		
S-261	CL	24+00	9.00-10.00	A-6(7)	38	22	3.0	49.4	9.3	38.2	100	99	50	33.3	-
S-262	CL	26+00	1.00-2.00	A-2-4(0)	17	NP	2.2	70.2	11.6	16.1	100	99	31	-	-
S-263	CL	26+00	5.00-6.00	A-4(0)	31	9	1.8	63.2	8.8	26.1	100	99	38	-	-
SS-264	CL	30+00	8.60-10.10	A-2-4(0)	24	NP	11.3	71.1	3.6	14.1	100	99	19	-	-
S-266	CL	34+00	5.00-6.00	A-2-4(0)	24	4	1.1	76.9	4.9	17.1	100	100	24	-	-
S-267	CL	36+00	0.00-1.00	A-2-4(0)	20	2	1.9	69.2	11.8	17.1	100	100	32	-	-
S-268	CL	36+00	3.00-4.00	A-2-4(0)	26	5	2.1	73.7	6.1	18.1	100	100	27	-	-
S-269	CL	36+00	8.00-9.00	A-2-4(0)	25	NP	0.7	78.4	4.7	16.1	100	100	23	-	-



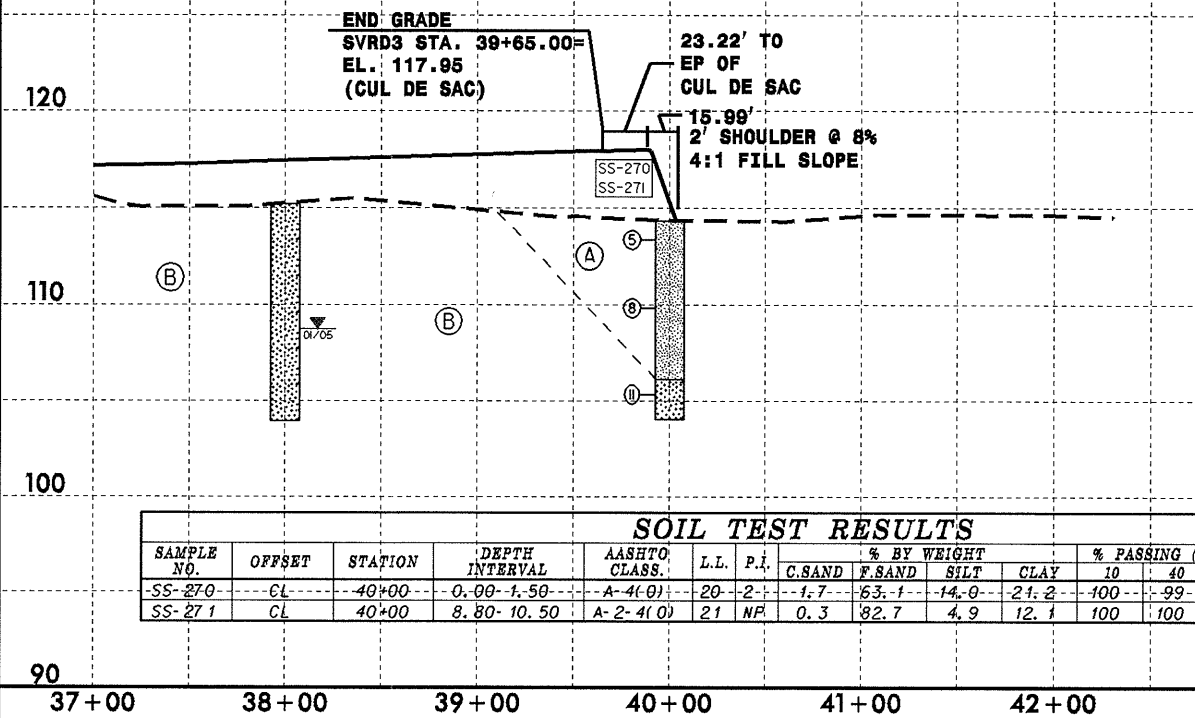
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 12/21/00

5/28/99

-SVRD3-

PROJECT REFERENCE NO. R-2719A	SHEET NO. 77
ROADWAY DESIGN ENGINEER	HYDRAULICS ENGINEER

- (A) MEDIUM STIFF GRAY CLAYEY SANDY SILT, WET
- (B) LOOSE TO MEDIUM DENSE TAN TO GRAY SAND AND CLAYEY SAND, MOIST TO SATURATED

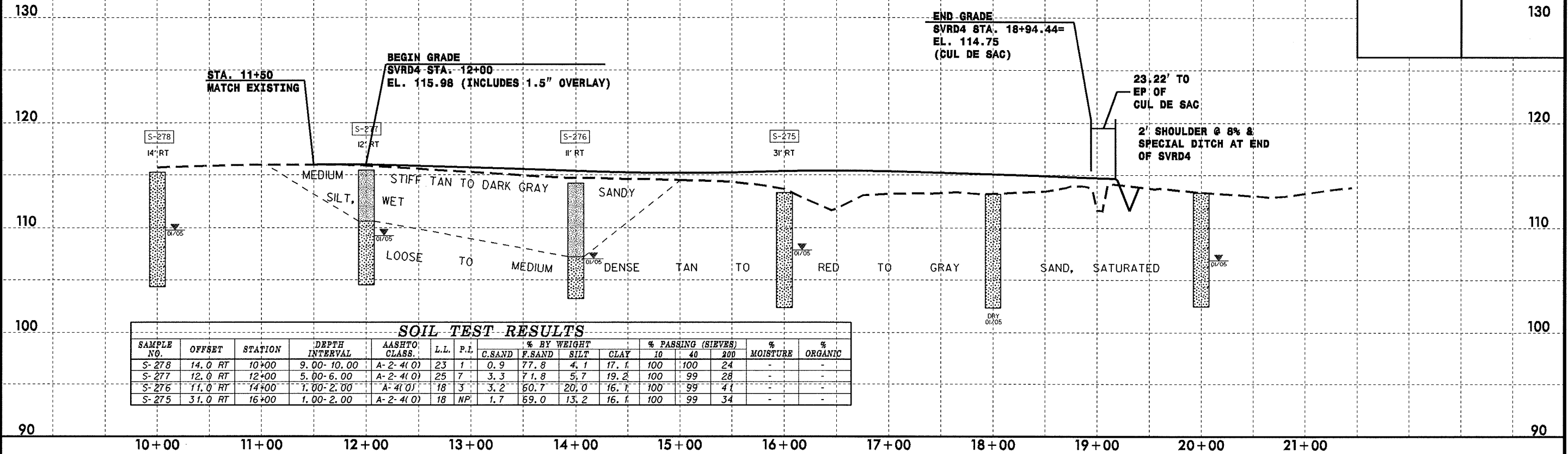


SOIL TEST RESULTS															
SAMPLE NO.	OFFSET	STATION	DEPTH INTERVAL	AASHTO CLASS.	L.L.	P.I.	% BY WEIGHT				% PASSING (SIEVES)			% MOISTURE	% ORGANIC
							C.SAND	F.SAND	SILT	CLAY	10	40	200		
SS-270	CL	40+00	0.00-1.50	A-4(0)	20	2	1.7	63.1	14.0	21.2	100	99	39	-	-
SS-271	CL	40+00	8.80-10.50	A-2-4(0)	21	NP	0.3	82.7	4.9	12.1	100	100	20	-	-

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PROJECT REFERENCE NO.	SHEET NO.
R-2719A	78
ROADWAY DESIGN ENGINEER	HYDRAULICS ENGINEER
	130

-SVRD4-



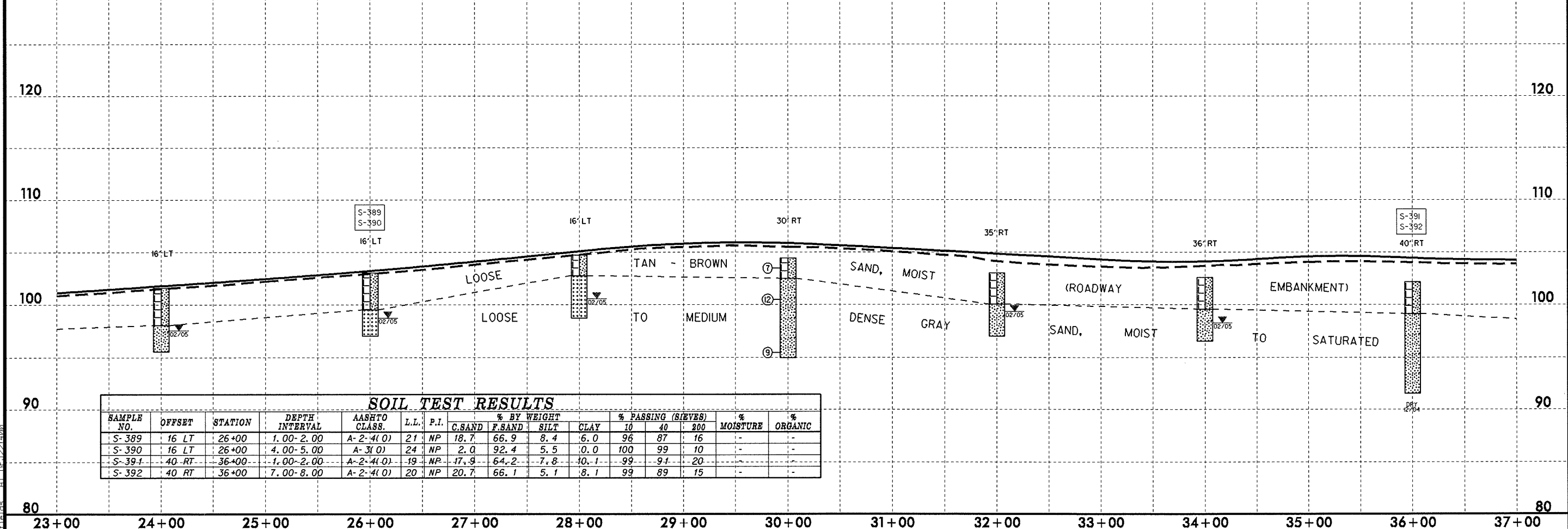
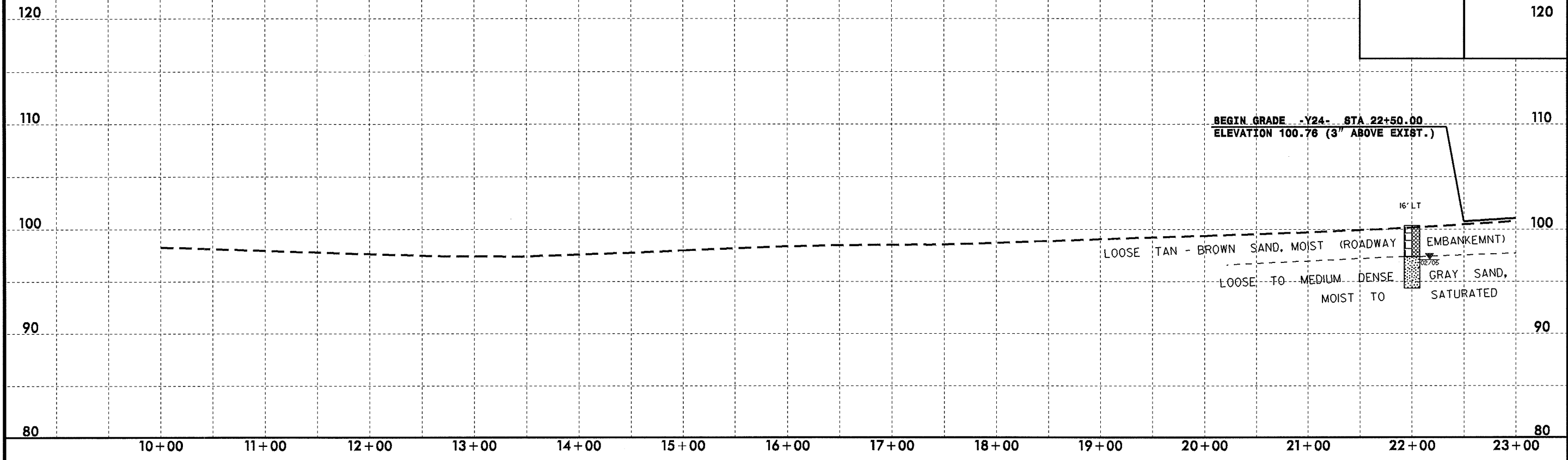
SOIL TEST RESULTS

SAMPLE NO.	OFFSET	STATION	DEPTH INTERVAL	AASHTO CLASS.	L.L.	P.L.	% BY WEIGHT				% PASSING (SIEVES)			% MOISTURE	% ORGANIC
							C.SAND	F.SAND	SILT	CLAY	10	40	200		
S-278	14.0 RT	10+00	9.00-10.00	A-2-4(O)	23	1	0.9	77.8	4.1	17.1	100	100	24	-	-
S-277	12.0 RT	12+00	5.00-6.00	A-2-4(O)	25	7	3.3	71.8	5.7	19.2	100	99	28	-	-
S-276	11.0 RT	14+00	1.00-2.00	A-4(O)	18	3	3.2	60.7	20.0	16.1	100	99	41	-	-
S-275	31.0 RT	16+00	1.00-2.00	A-2-4(O)	18	NP	1.7	59.0	13.2	16.1	100	99	34	-	-

5/28/99

PROJECT REFERENCE NO. R-2719A	SHEET NO. 79
ROADWAY DESIGN ENGINEER	HYDRAULICS ENGINEER

-Y24-



SOIL TEST RESULTS

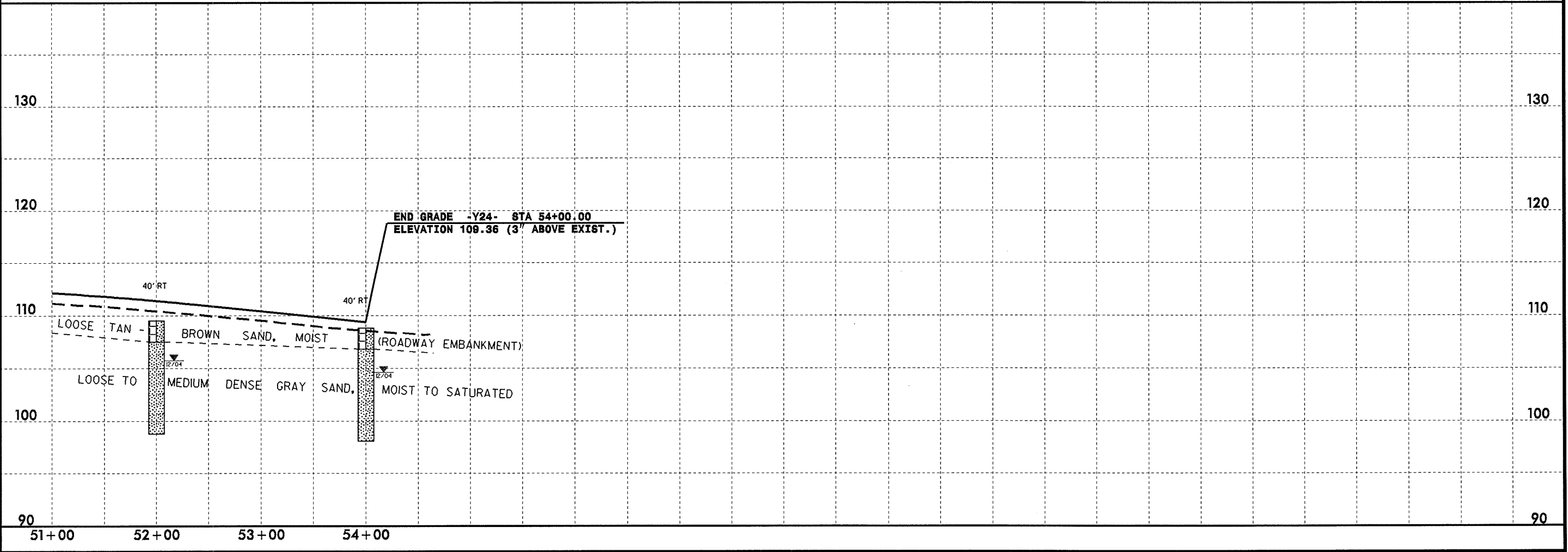
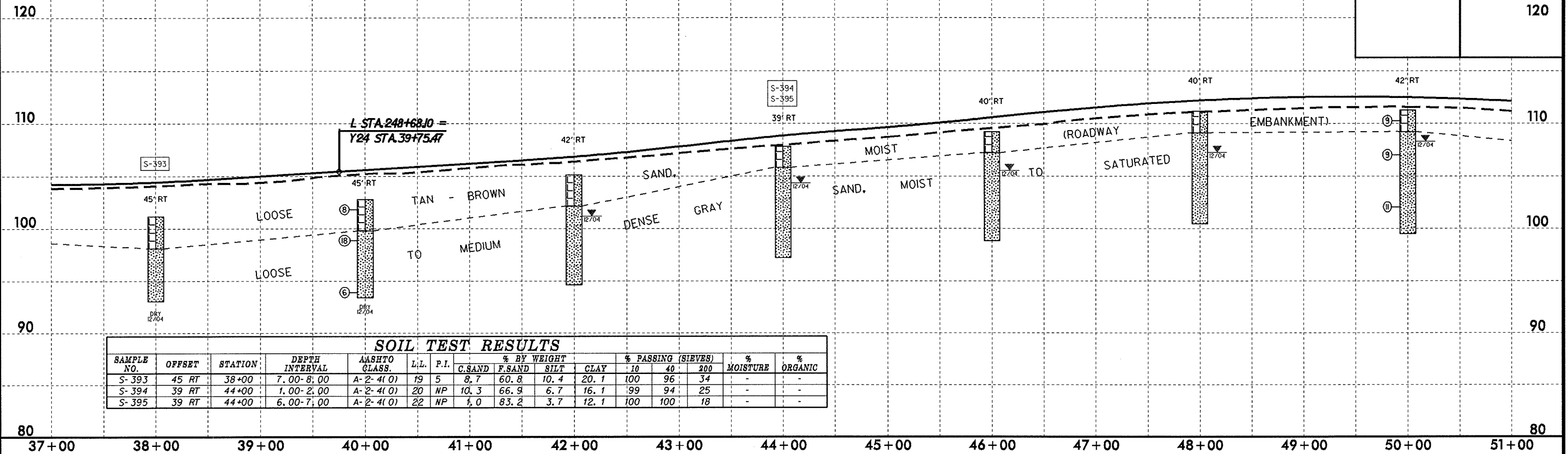
SAMPLE NO.	OFFSET	STATION	DEPTH INTERVAL	AASHTO CLASS.	L.L.	P.I.	% BY WEIGHT				% PASSING (SIEVES)			% MOISTURE	% ORGANIC
							C.SAND	F.SAND	SILT	CLAY	10	40	200		
S-389	16 LT	26+00	1.00-2.00	A-2-4(0)	21	NP	18.7	66.9	8.4	6.0	96	87	16	-	-
S-390	16 LT	26+00	4.00-5.00	A-3(0)	24	NP	2.0	92.4	5.5	0.0	100	99	10	-	-
S-391	40 RT	36+00	1.00-2.00	A-2-4(0)	19	NP	17.9	64.2	7.8	10.1	99	91	20	-	-
S-392	40 RT	36+00	7.00-8.00	A-2-4(0)	20	NP	20.7	66.1	5.1	8.1	99	89	15	-	-

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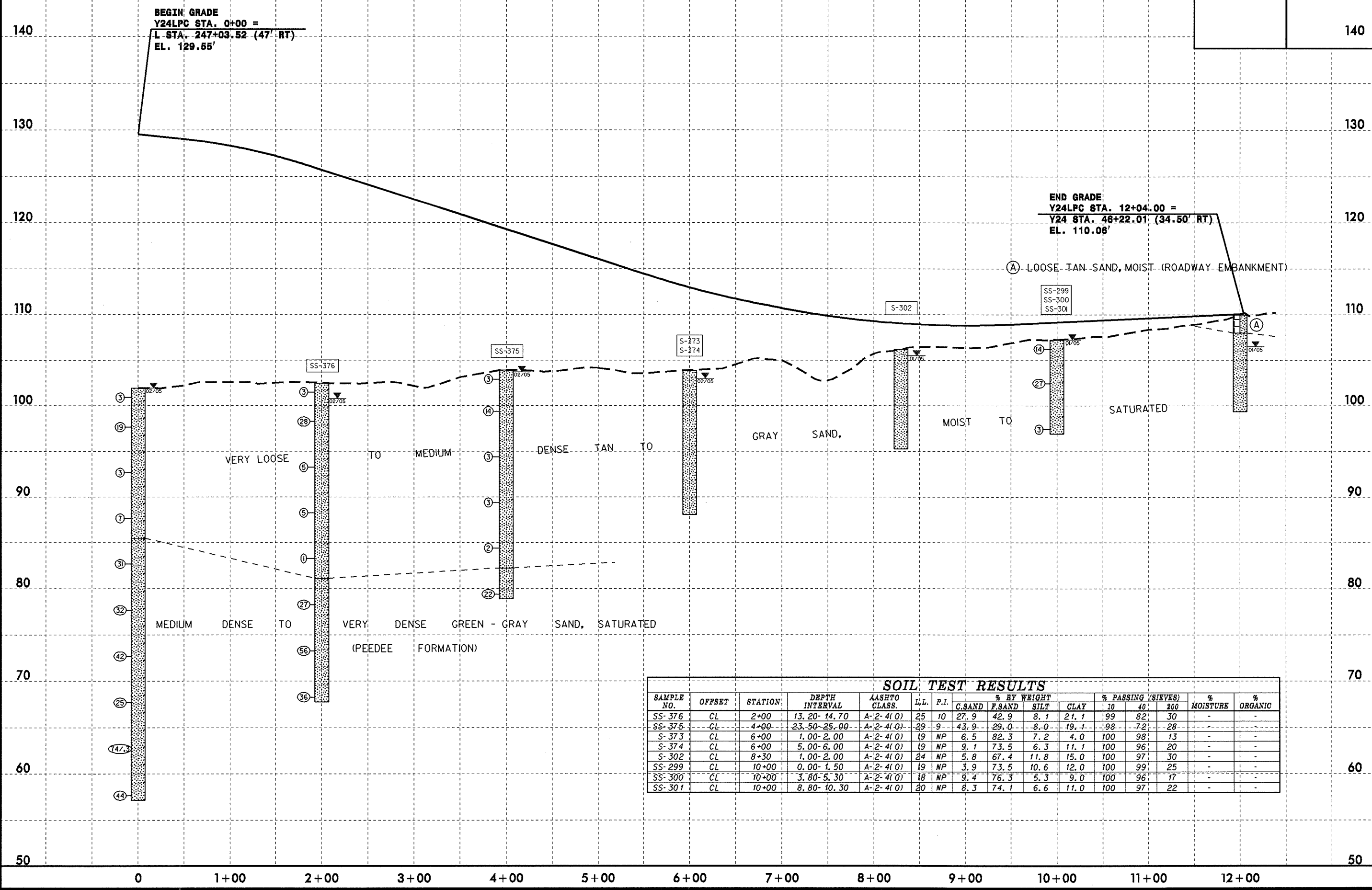
-Y24-

PROJECT REFERENCE NO.	SHEET NO.
R-2719A	80
ROADWAY DESIGN ENGINEER	HYDRAULICS ENGINEER
	120



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-Y24LPC-



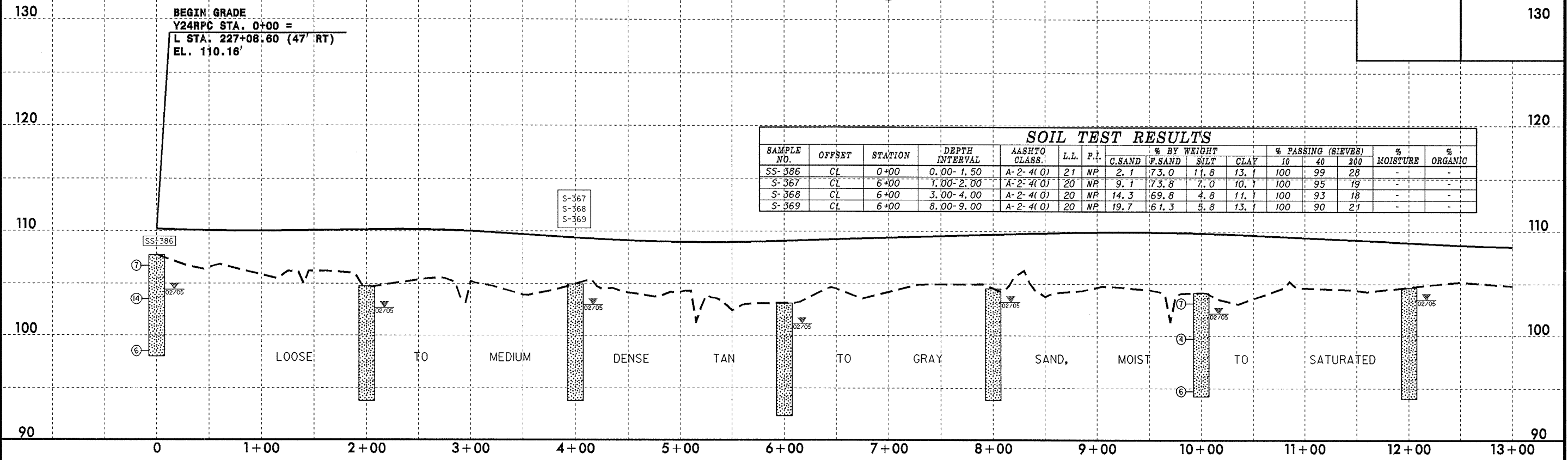
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-Y24RPC-

PROJECT REFERENCE NO. R-2719A	SHEET NO. 84
ROADWAY DESIGN ENGINEER	HYDRAULICS ENGINEER

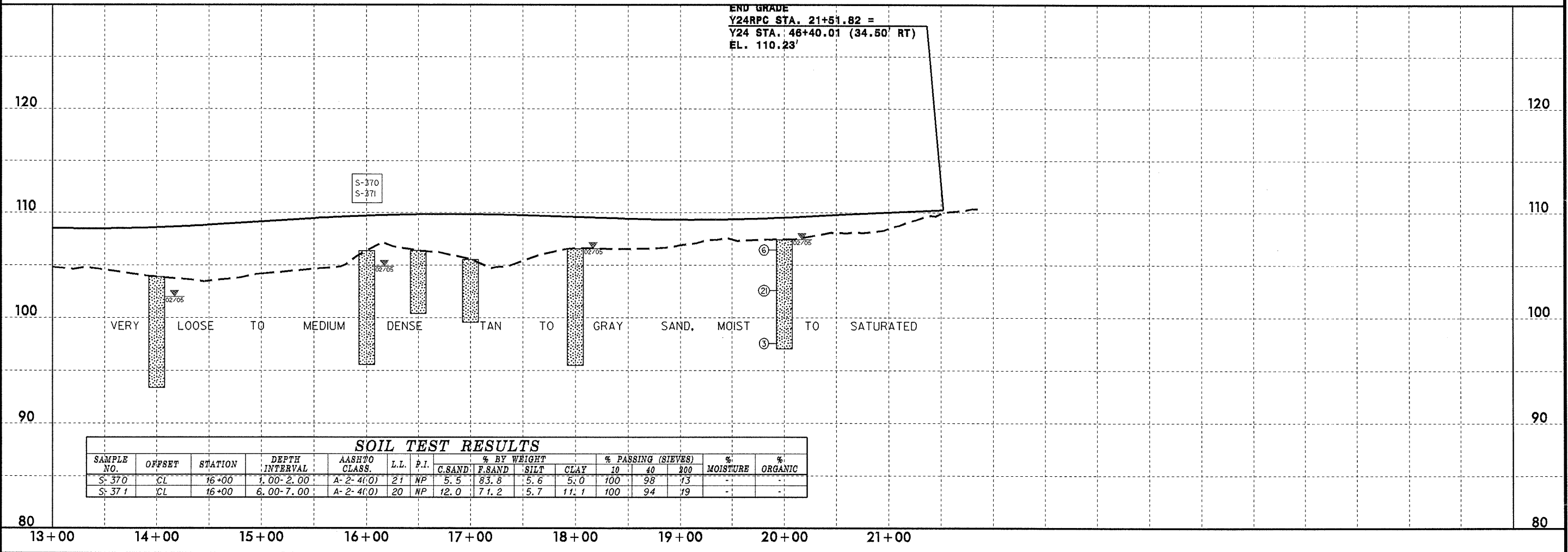
BEGIN GRADE
Y24RPC STA. 0+00 =
L STA. 227+08.60 (47' RT)
EL. 110.16'

SAMPLE NO.	OFFSET	STATION	DEPTH INTERVAL	AASHTO CLASS.	L.L.	P.I.	% BY WEIGHT				% PASSING (SIEVES)			% MOISTURE	% ORGANIC
							C.SAND	F.SAND	SILT	CLAY	10	40	200		
SS-386	CL	0+00	0.00-1.50	A-2-4(0)	21	NP	2.1	73.0	11.8	13.1	100	99	28	-	-
S-367	CL	6+00	1.00-2.00	A-2-4(0)	20	NP	9.1	73.8	7.0	10.1	100	95	19	-	-
S-368	CL	6+00	3.00-4.00	A-2-4(0)	20	NP	14.3	69.8	4.8	11.1	100	93	18	-	-
S-369	CL	6+00	8.00-9.00	A-2-4(0)	20	NP	19.7	61.3	5.8	13.1	100	90	21	-	-



END GRADE
Y24RPC STA. 21+51.82 =
Y24 STA. 46+40.01 (34.50' RT)
EL. 110.23'

SAMPLE NO.	OFFSET	STATION	DEPTH INTERVAL	AASHTO CLASS.	L.L.	P.I.	% BY WEIGHT				% PASSING (SIEVES)			% MOISTURE	% ORGANIC
							C.SAND	F.SAND	SILT	CLAY	10	40	200		
S-370	CL	16+00	1.00-2.00	A-2-4(0)	21	NP	5.5	83.8	5.6	5.0	100	98	13	-	-
S-371	CL	16+00	6.00-7.00	A-2-4(0)	20	NP	12.0	71.2	5.7	11.1	100	94	19	-	-

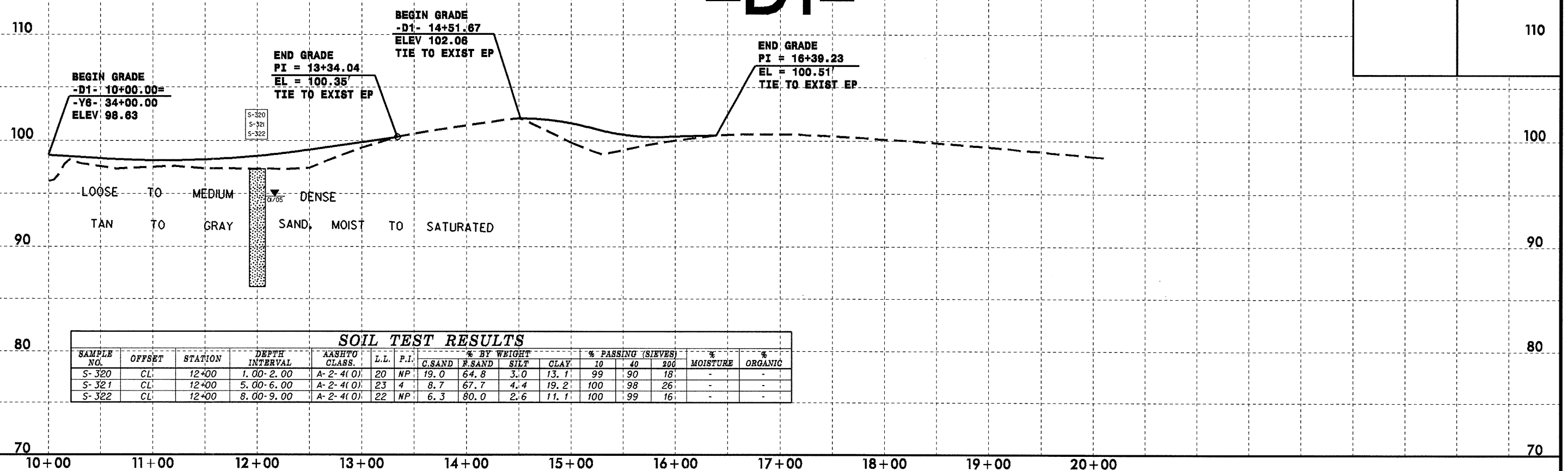


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5/28/99

PROJECT REFERENCE NO.		SHEET NO.	
R-2719A		86	
ROADWAY DESIGN ENGINEER		HYDRAULICS ENGINEER	
			110

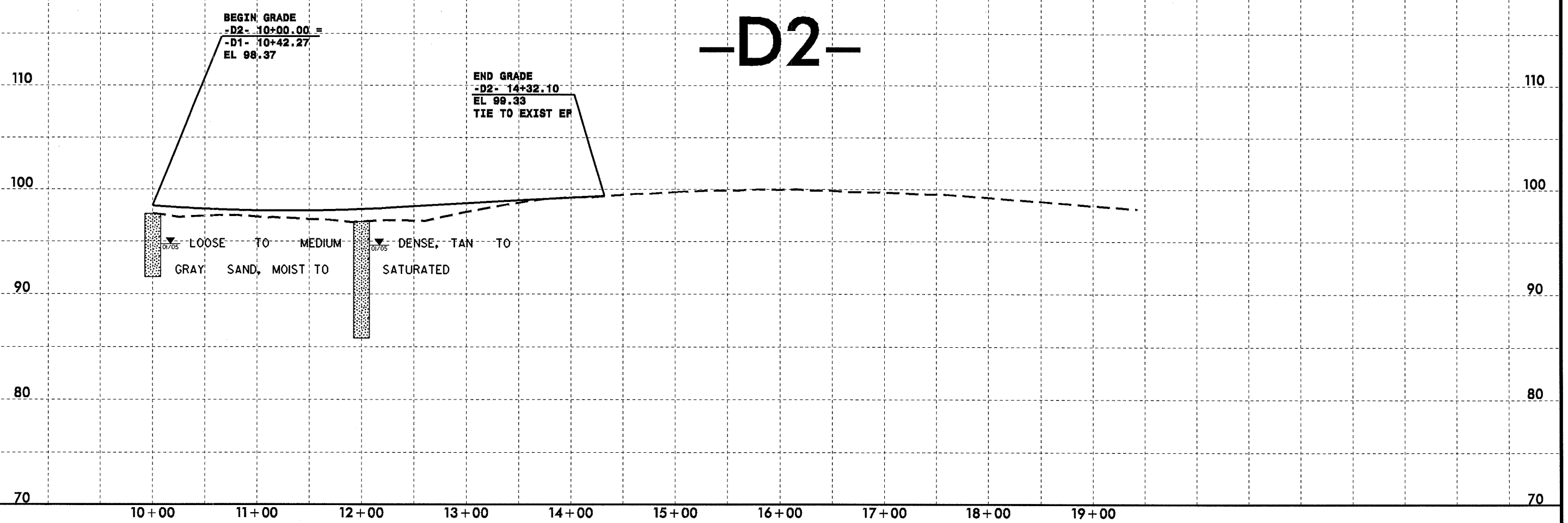
-D1-



SOIL TEST RESULTS

SAMPLE NO.	OFFSET	STATION	DEPTH INTERVAL	AASHTO CLASS.	L.L.	P.I.	% BY WEIGHT				% PASSING (SIEVES)			% MOISTURE	% ORGANIC
							C.SAND	F.SAND	SILT	CLAY	10	40	200		
S-320	CL	12+00	1.00-2.00	A-2-4(0)	20	NP	19.0	64.8	3.0	13.1	99	90	18	-	-
S-321	CL	12+00	5.00-6.00	A-2-4(0)	23	4	8.7	67.7	4.4	19.2	100	98	26	-	-
S-322	CL	12+00	8.00-9.00	A-2-4(0)	22	NP	6.3	80.0	2.6	11.1	100	99	16	-	-

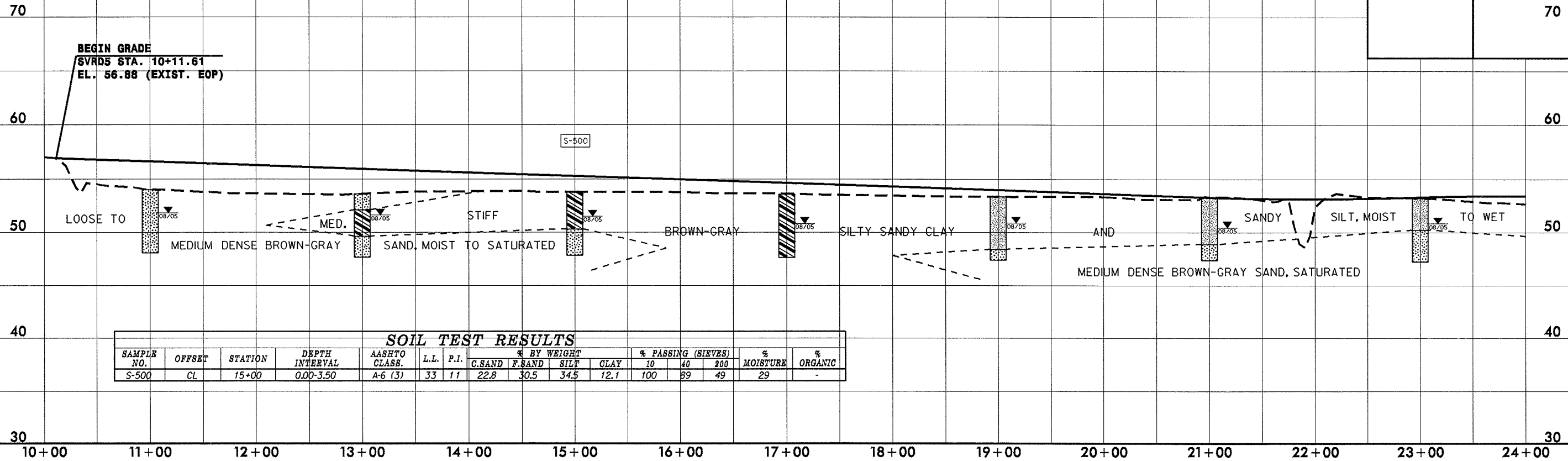
-D2-



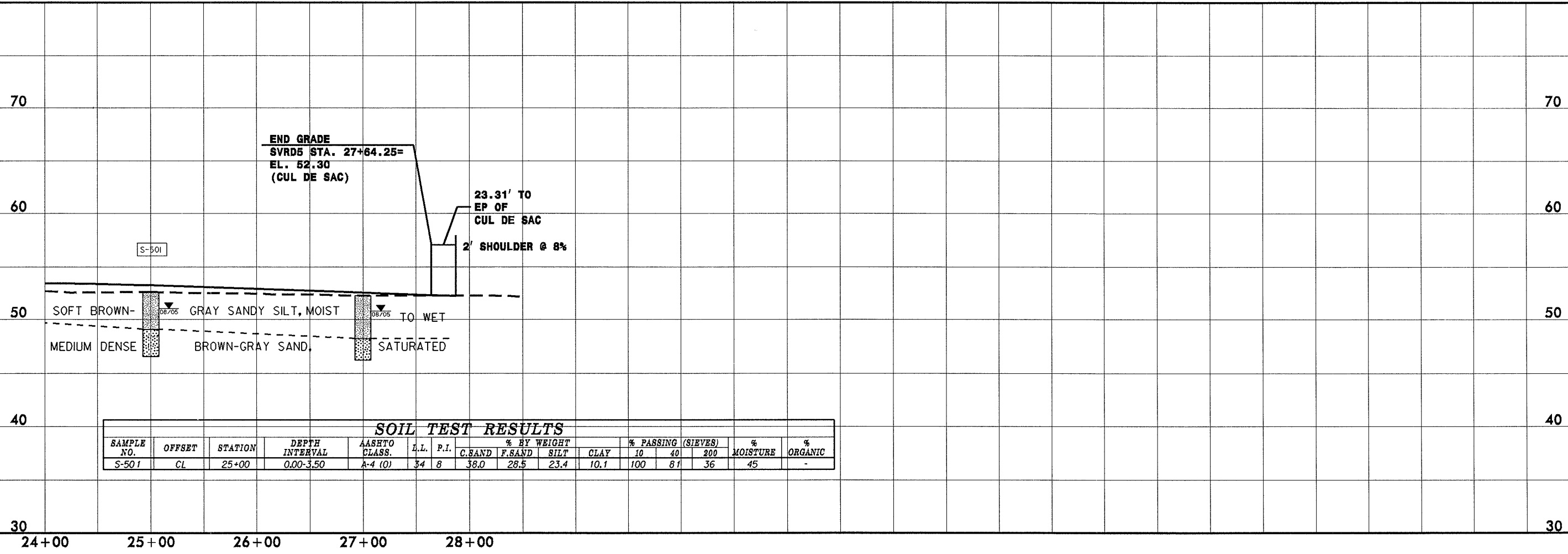
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 Author

-SVRD5-

PROJECT REFERENCE NO. R-2719A	SHEET NO. 87
ROADWAY DESIGN ENGINEER	HYDRAULICS ENGINEER
70	



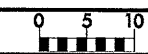
SOIL TEST RESULTS															
SAMPLE NO.	OFFSET	STATION	DEPTH INTERVAL	AASHTO CLASS.	L.L.	P.I.	% BY WEIGHT				% PASSING (SIEVES)			% MOISTURE	% ORGANIC
							C.SAND	F.SAND	SILT	CLAY	10	40	200		
S-500	CL	15+00	0.00-3.50	A-6 (3)	33	11	22.8	30.5	34.5	12.1	100	89	49	29	-



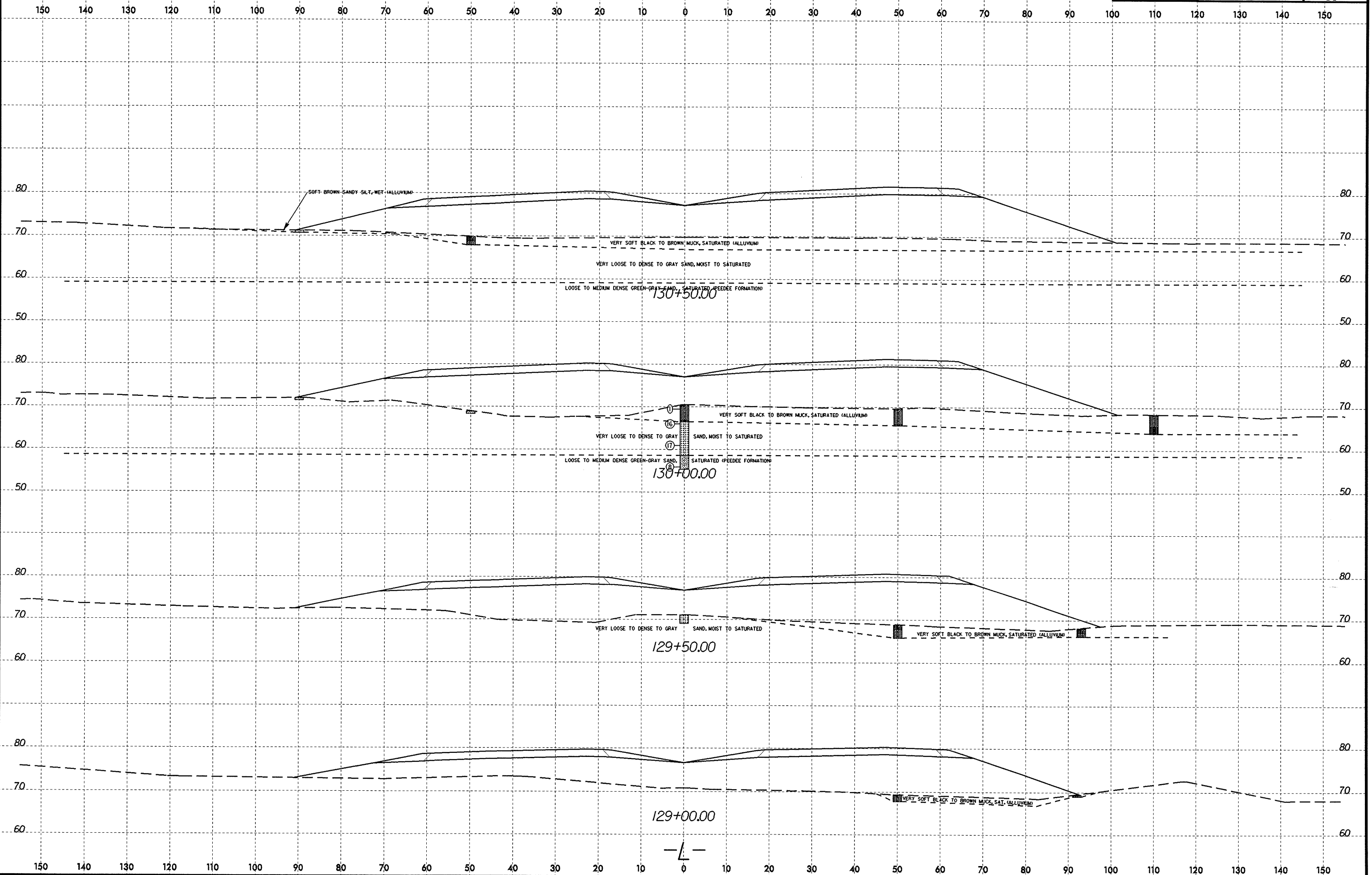
SOIL TEST RESULTS															
SAMPLE NO.	OFFSET	STATION	DEPTH INTERVAL	AASHTO CLASS.	L.L.	P.I.	% BY WEIGHT				% PASSING (SIEVES)			% MOISTURE	% ORGANIC
							C.SAND	F.SAND	SILT	CLAY	10	40	200		
S-501	CL	25+00	0.00-3.50	A-4 (0)	34	8	38.0	28.5	23.4	10.1	100	81	36	45	-

5/28/99
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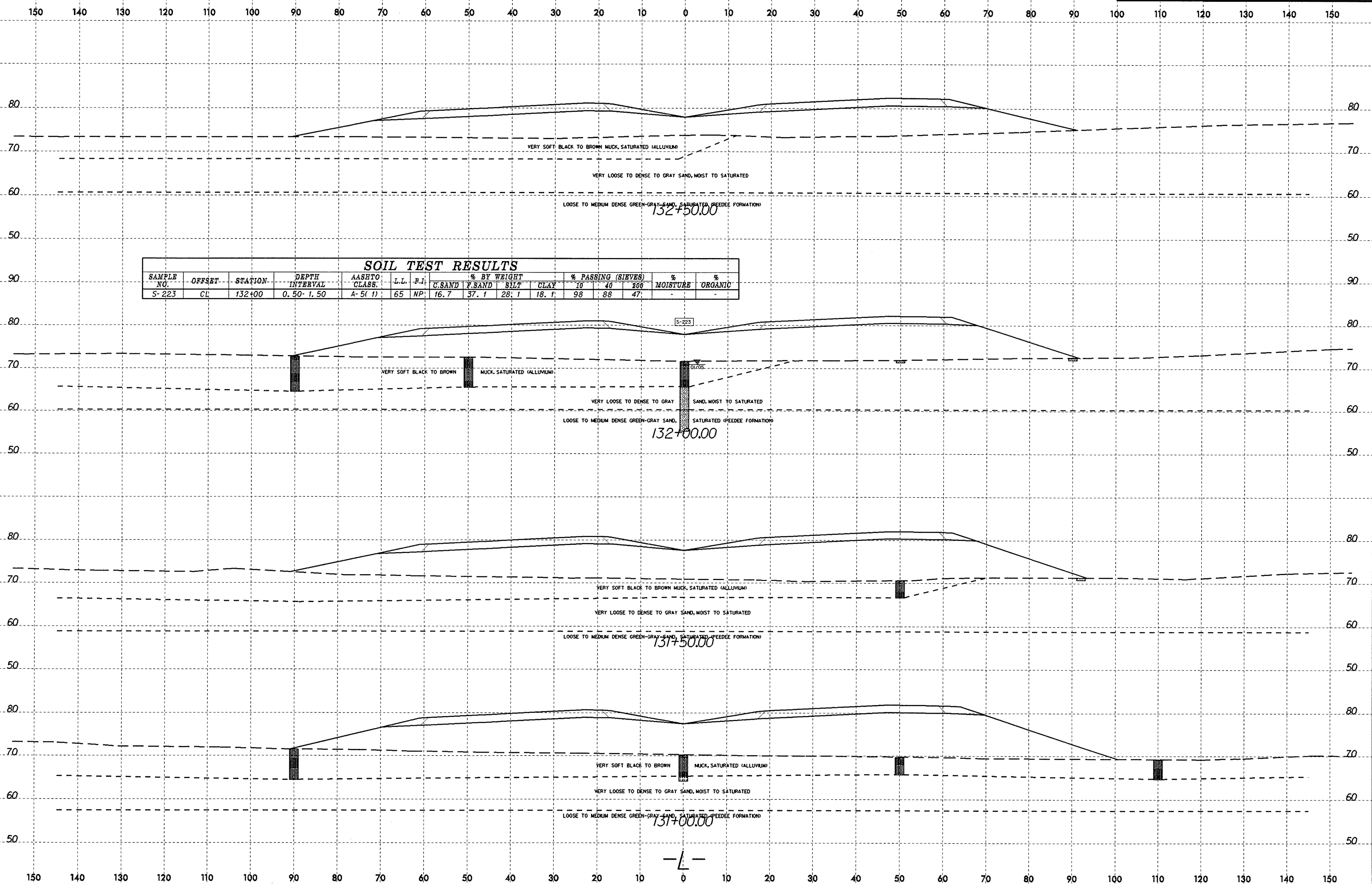
8/23/99



PROJ. REFERENCE NO.	SHEET NO.
R-2719A	88

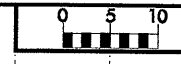


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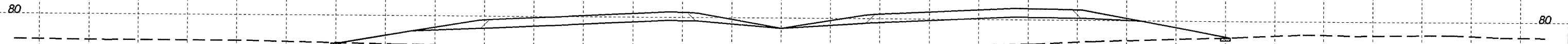
SOIL TEST RESULTS															
SAMPLE NO.	OFFSET	STATION	DEPTH INTERVAL	AASHTO CLASS.	L.L.	P.I.	% BY WEIGHT				% PASSING (SIEVES)			% MOISTURE	% ORGANIC
							G. SAND	F. SAND	SILT	CLAY	10	40	200		
S-223	CL	132+00	0.50-1.50	A-5(1)	65	NP	16.7	37.1	28.1	18.1	98	88	47	-	-

8/23/99



PROJ. REFERENCE NO.	SHEET NO.
R-2719A	90

150 140 130 120 110 100 90 80 70 60 50 40 30 20 10 0 10 20 30 40 50 60 70 80 90 100 110 120 130 140 150



VERY SOFT BLACK TO BROWN MUCK, SATURATED (ALLIUM)

VERY LOOSE TO DENSE TO GRAY SAND, MOIST TO SATURATED

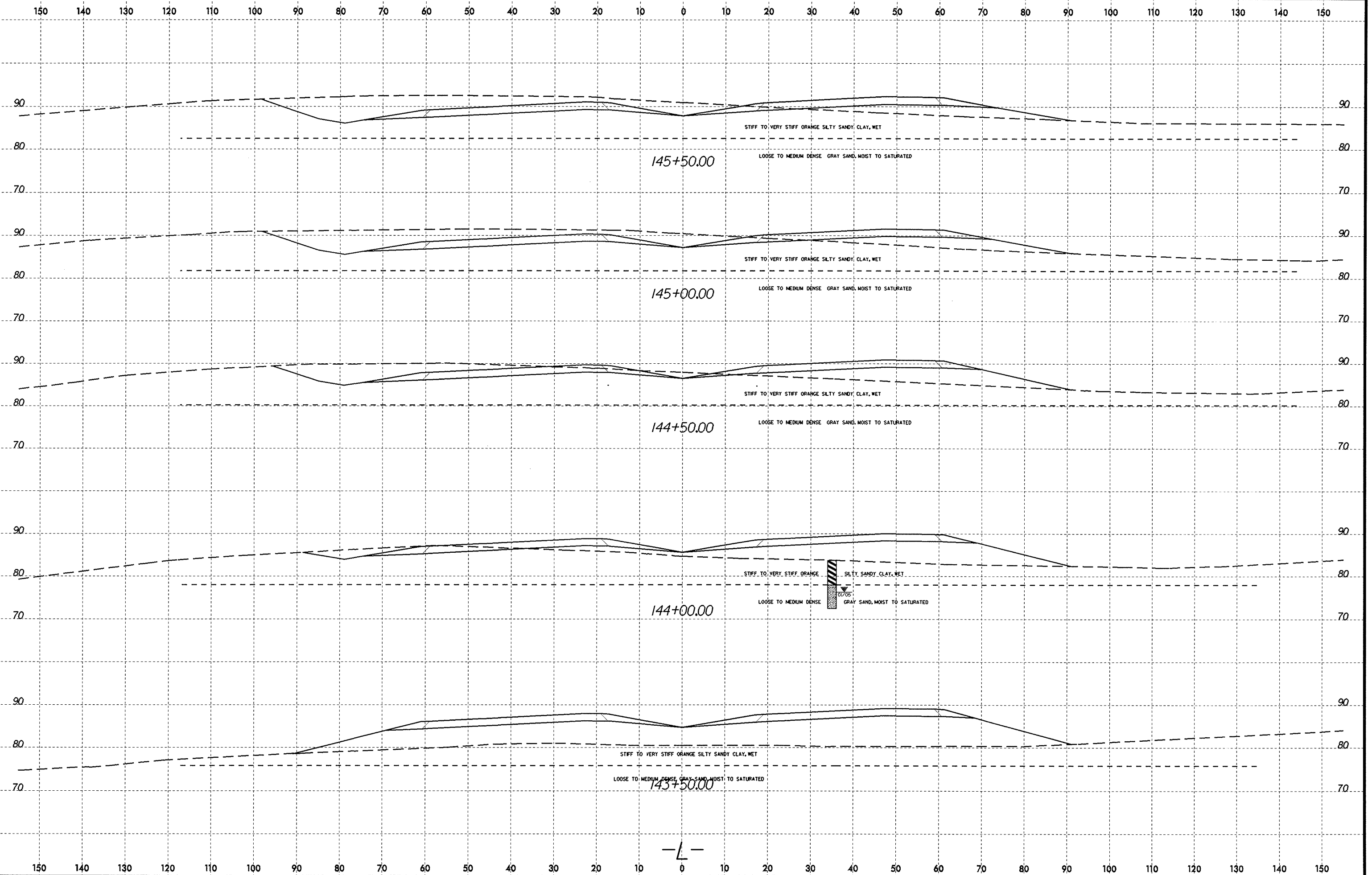
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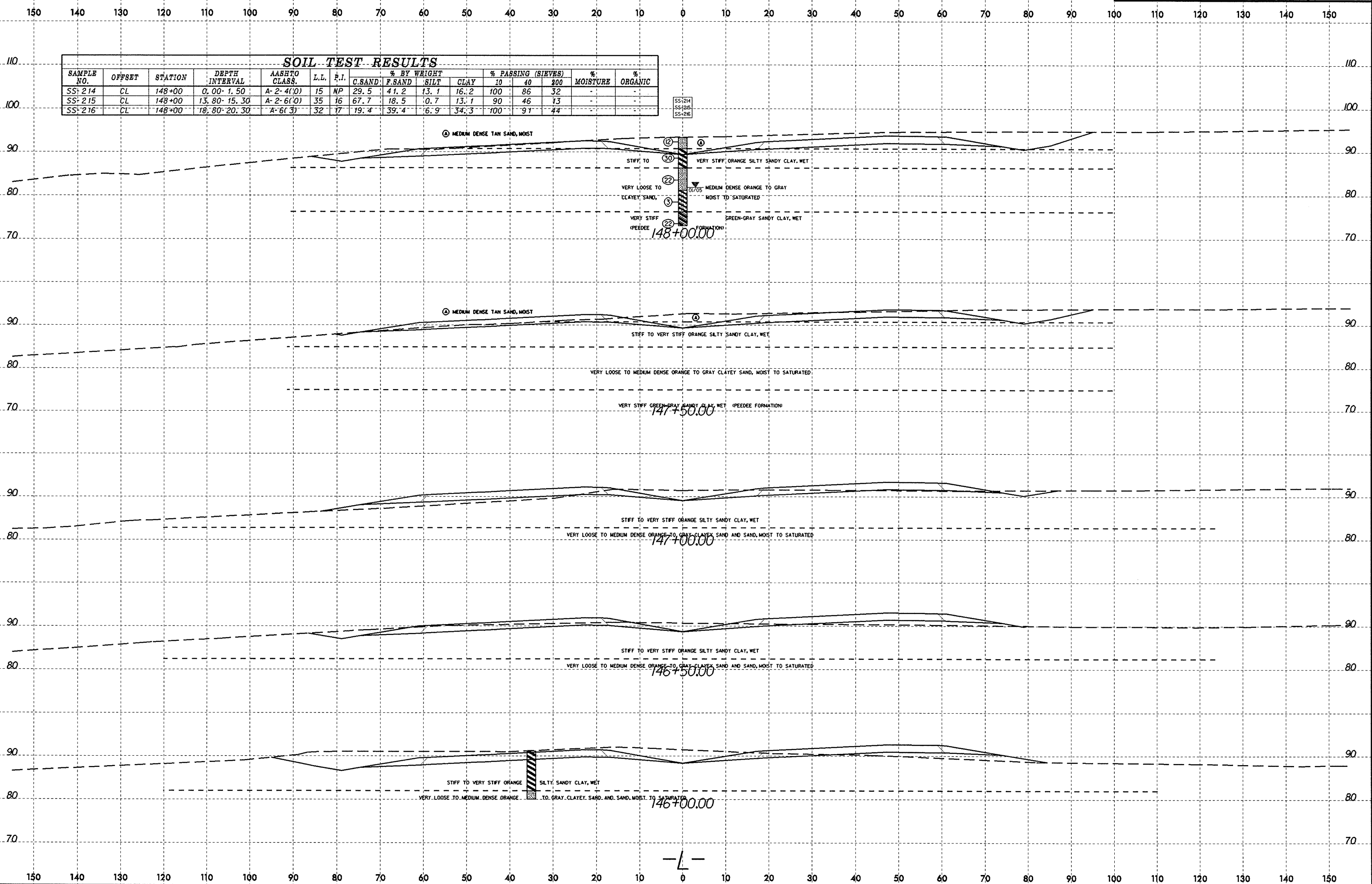
8/23/99



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10/22/08
LbMiller

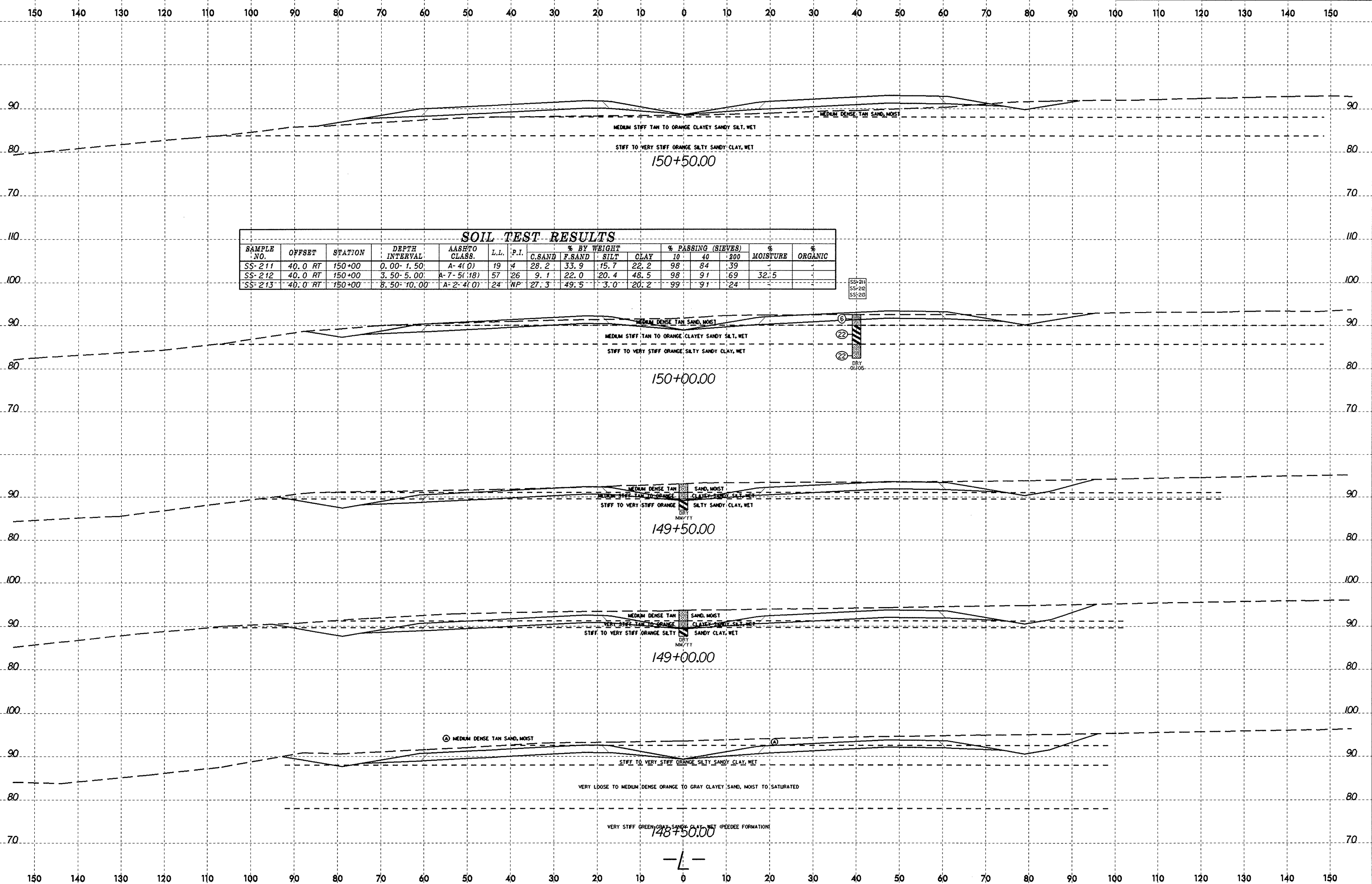
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146+00.00



SOIL TEST RESULTS

SAMPLE NO.	OFFSET	STATION	DEPTH INTERVAL	AASHTO CLASS.	L.L.	P.I.	% BY WEIGHT				% PASSING (SIEVES)			% MOISTURE	% ORGANIC
							C.SAND	F.SAND	SILT	CLAY	10	40	200		
SS-214	CL	148+00	0.00-1.50	A-2-4(0)	15	NP	29.5	41.2	13.1	16.2	100	86	32	-	-
SS-215	CL	148+00	13.80-15.30	A-2-6(0)	35	16	67.7	18.5	0.7	13.1	90	46	13	-	-
SS-216	CL	148+00	18.80-20.30	A-6(3)	32	17	19.4	39.4	6.9	34.3	100	91	44	-	-

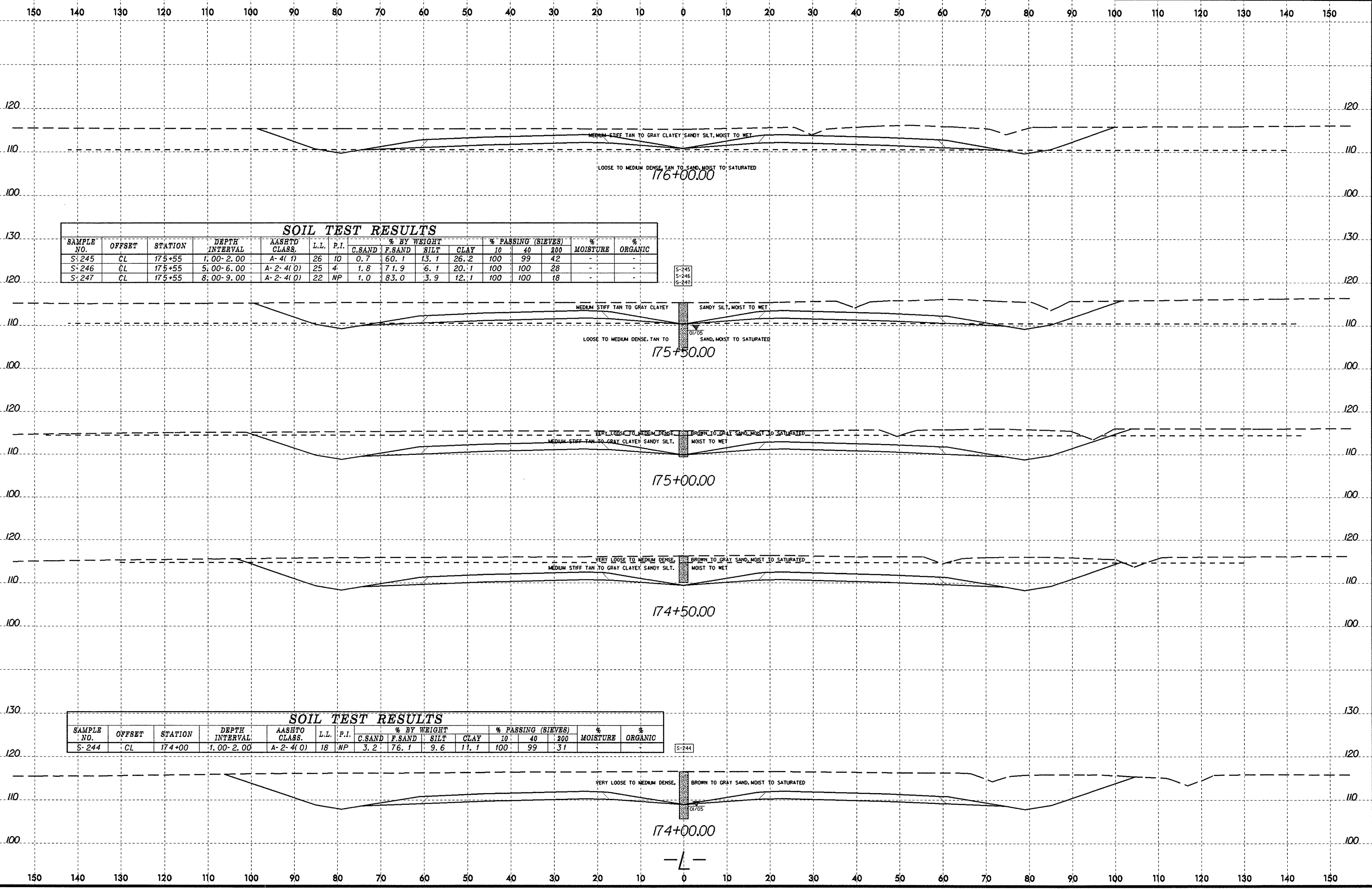


8/23/99



PROJ. REFERENCE NO.
R-2719A

SHEET NO.
94



SOIL TEST RESULTS

SAMPLE NO.	OFFSET	STATION	DEPTH INTERVAL	AASHTO CLASS.	L.L.	P.I.	% BY WEIGHT				% PASSING (SIEVES)			%	%
							C.SAND	F.SAND	SILT	CLAY	10	40	200		
S-245	CL	175+55	1.00-2.00	A-4(1)	26	10	0.7	60.1	13.1	26.2	100	99	42	-	-
S-246	CL	175+55	5.00-6.00	A-2-4(0)	25	4	1.8	71.9	6.1	20.1	100	100	28	-	-
S-247	CL	175+55	8.00-9.00	A-2-4(0)	22	NP	1.0	83.0	3.9	12.1	100	100	18	-	-

S-245
S-246
S-247

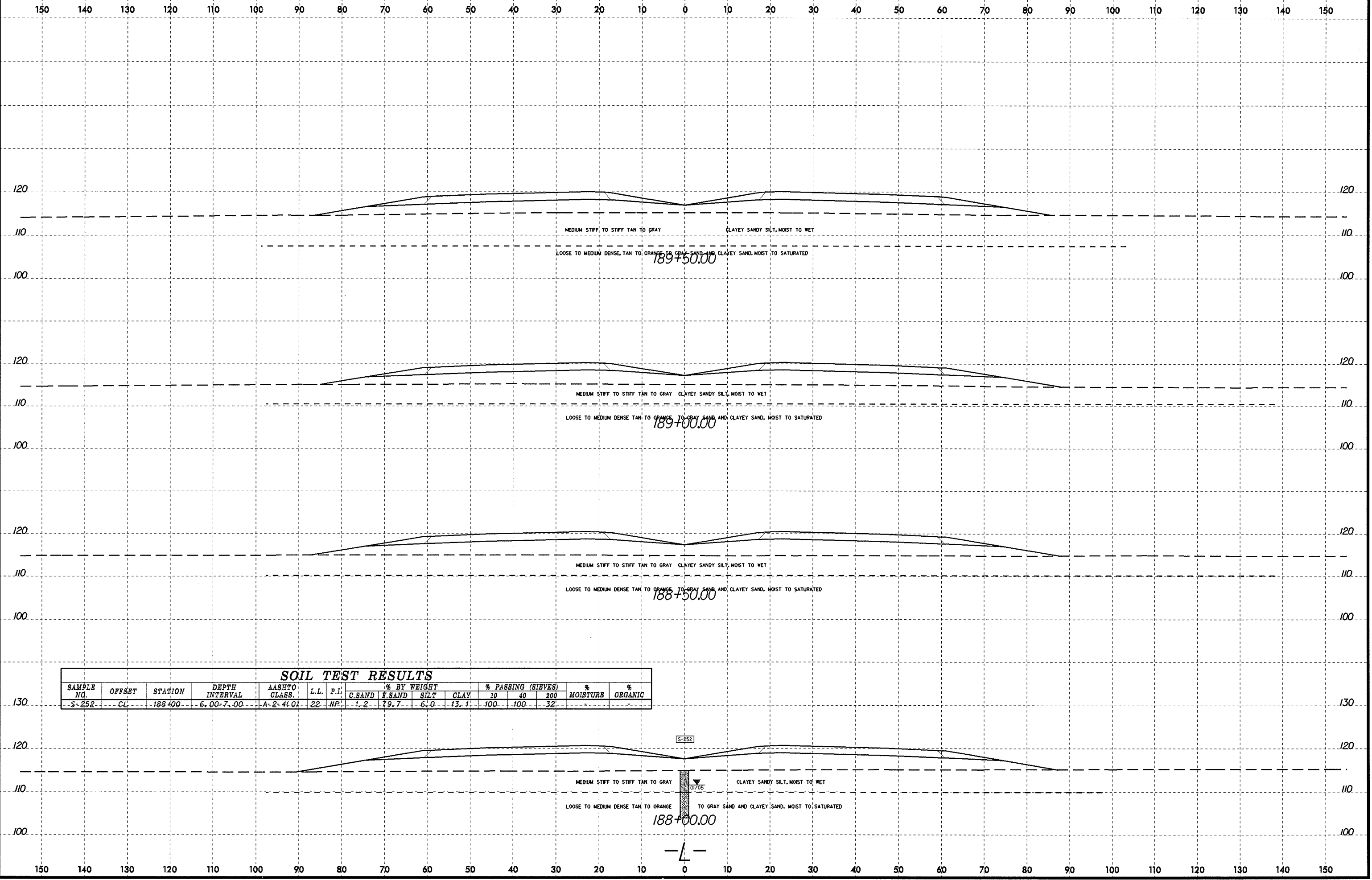
SOIL TEST RESULTS

SAMPLE NO.	OFFSET	STATION	DEPTH INTERVAL	AASHTO CLASS.	L.L.	P.I.	% BY WEIGHT				% PASSING (SIEVES)			%	%
							C.SAND	F.SAND	SILT	CLAY	10	40	200		
S-244	CL	174+00	1.00-2.00	A-2-4(0)	18	NP	3.2	76.1	9.6	11.1	100	99	31	-	-

S-244

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8/23/99



MEDIUM STIFF TO STIFF TAN TO GRAY CLAYEY SANDY SILT, MOIST TO WET
LOOSE TO MEDIUM DENSE TAN TO ORANGE TO GRAY SAND AND CLAYEY SAND, MOIST TO SATURATED

189+50.00

MEDIUM STIFF TO STIFF TAN TO GRAY CLAYEY SANDY SILT, MOIST TO WET
LOOSE TO MEDIUM DENSE TAN TO ORANGE TO GRAY SAND AND CLAYEY SAND, MOIST TO SATURATED

189+00.00

MEDIUM STIFF TO STIFF TAN TO GRAY CLAYEY SANDY SILT, MOIST TO WET
LOOSE TO MEDIUM DENSE TAN TO ORANGE TO GRAY SAND AND CLAYEY SAND, MOIST TO SATURATED

188+50.00

S-252

MEDIUM STIFF TO STIFF TAN TO GRAY CLAYEY SANDY SILT, MOIST TO WET
LOOSE TO MEDIUM DENSE TAN TO ORANGE TO GRAY SAND AND CLAYEY SAND, MOIST TO SATURATED

188+00.00

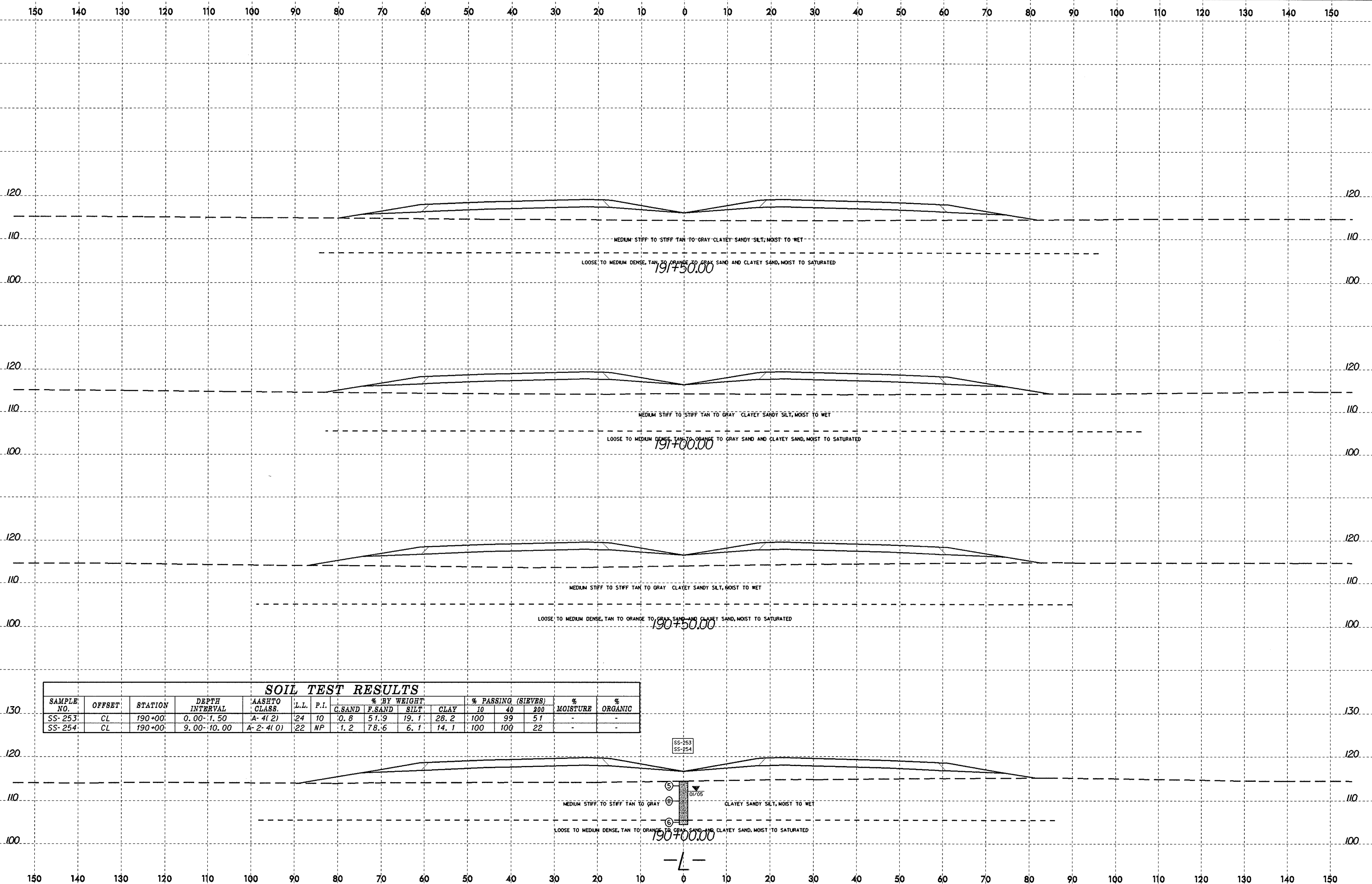
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SOIL TEST RESULTS

SAMPLE NO.	OFFSET	STATION	DEPTH INTERVAL	AASHTO CLASS.	L.L.	P.I.	% BY WEIGHT				% PASSING (SIEVES)			% MOISTURE	% ORGANIC
							C.SAND	F.SAND	SILT	CLAY	10	40	200		
S-252	CL	188+00	6.00-7.00	A-2-4(0)	22	NP	1.2	79.7	6.0	13.1	100	100	32		

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8/23/99



SOIL TEST RESULTS

SAMPLE NO.	OFFSET	STATION	DEPTH INTERVAL	AASHTO CLASS.	L.L.	P.I.	% BY WEIGHT				% PASSING (SIEVES)			% MOISTURE	% ORGANIC
							G. SAND	F. SAND	SILT	CLAY	10	40	200		
SS-253	CL	190+00'	0.00-1.50	A-4(2)	24	10	0.8	51.9	19.1	28.2	100	99	51	-	-
SS-254	CL	190+00'	9.00-10.00	A-2-4(0)	22	NP	1.2	78.6	6.1	14.1	100	100	22	-	-

SS-253
SS-254

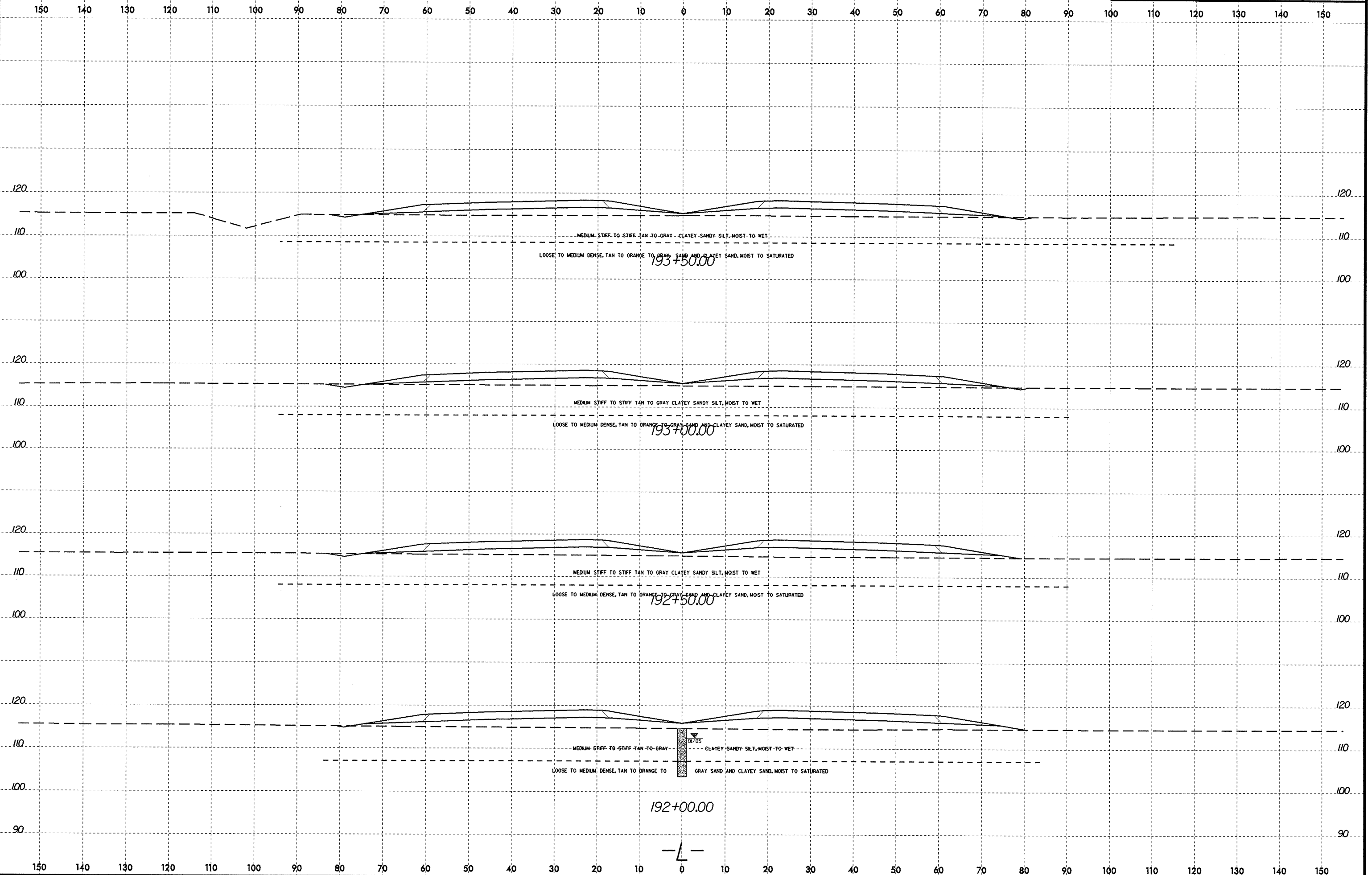
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③

01/05

190+00.00

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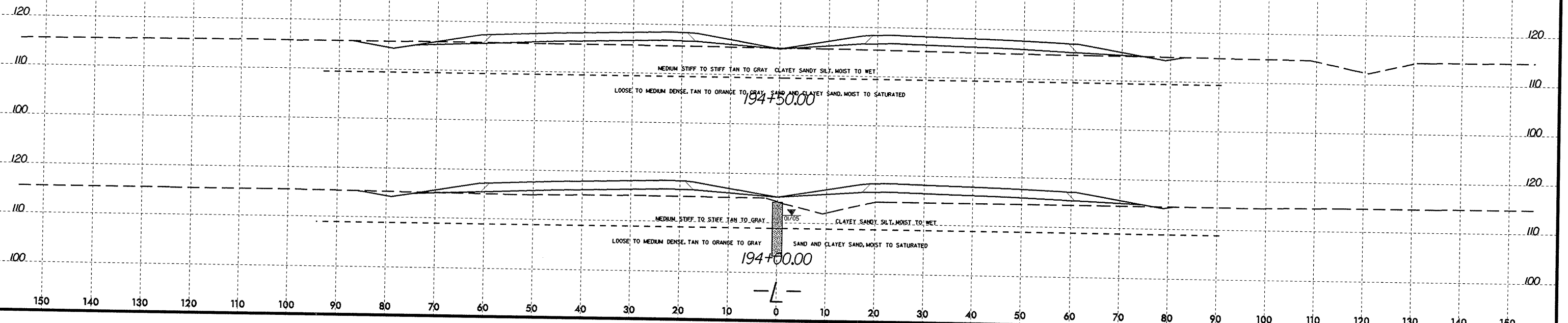


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KEmiller

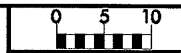
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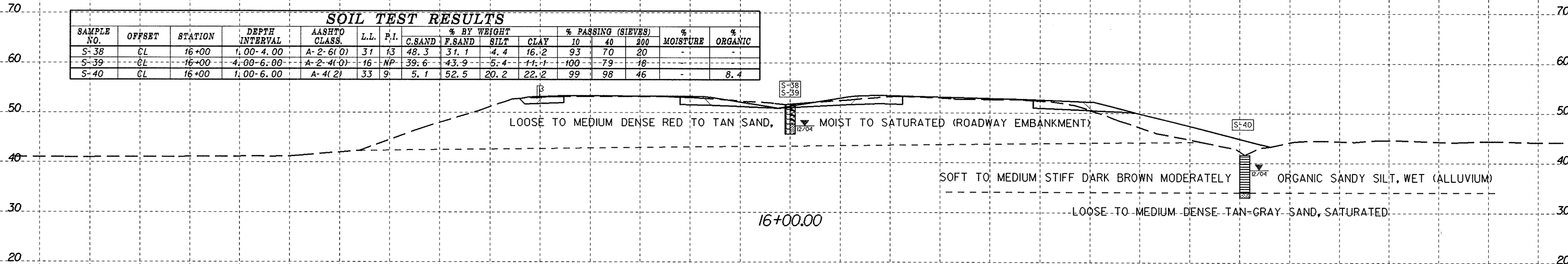


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150 140 130 120 110 100 90 80 70 60 50 40 30 20 10 0 10 20 30 40 50 60 70 80 90 100 110 120 130 140 150

SOIL TEST RESULTS															
SAMPLE NO.	OFFSET	STATION	DEPTH INTERVAL	AASHTO CLASS.	L.L.	P.I.	% BY WEIGHT				% PASSING (SIEVES)			% MOISTURE	% ORGANIC
							C.SAND	F.SAND	SILT	CLAY	10	40	200		
S-38	CL	16+00	1.00-4.00	A-2-6(0)	31	13	48.3	31.1	4.4	16.2	93	70	20	-	-
S-39	CL	16+00	4.00-6.00	A-2-4(0)	16	NP	39.6	43.9	5.4	11.1	100	79	18	-	-
S-40	CL	16+00	1.00-6.00	A-4(2)	33	9	5.1	52.5	20.2	22.2	99	98	46	-	8.4



LOOSE TO MEDIUM DENSE RED TO TAN SAND, MOIST TO SATURATED (ROADWAY EMBANKMENT)

SOFT TO MEDIUM STIFF DARK BROWN MODERATELY ORGANIC SANDY SILT, WET (ALLUVIUM)

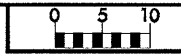
LOOSE TO MEDIUM DENSE TAN-GRAY SAND, SATURATED

16+00.00

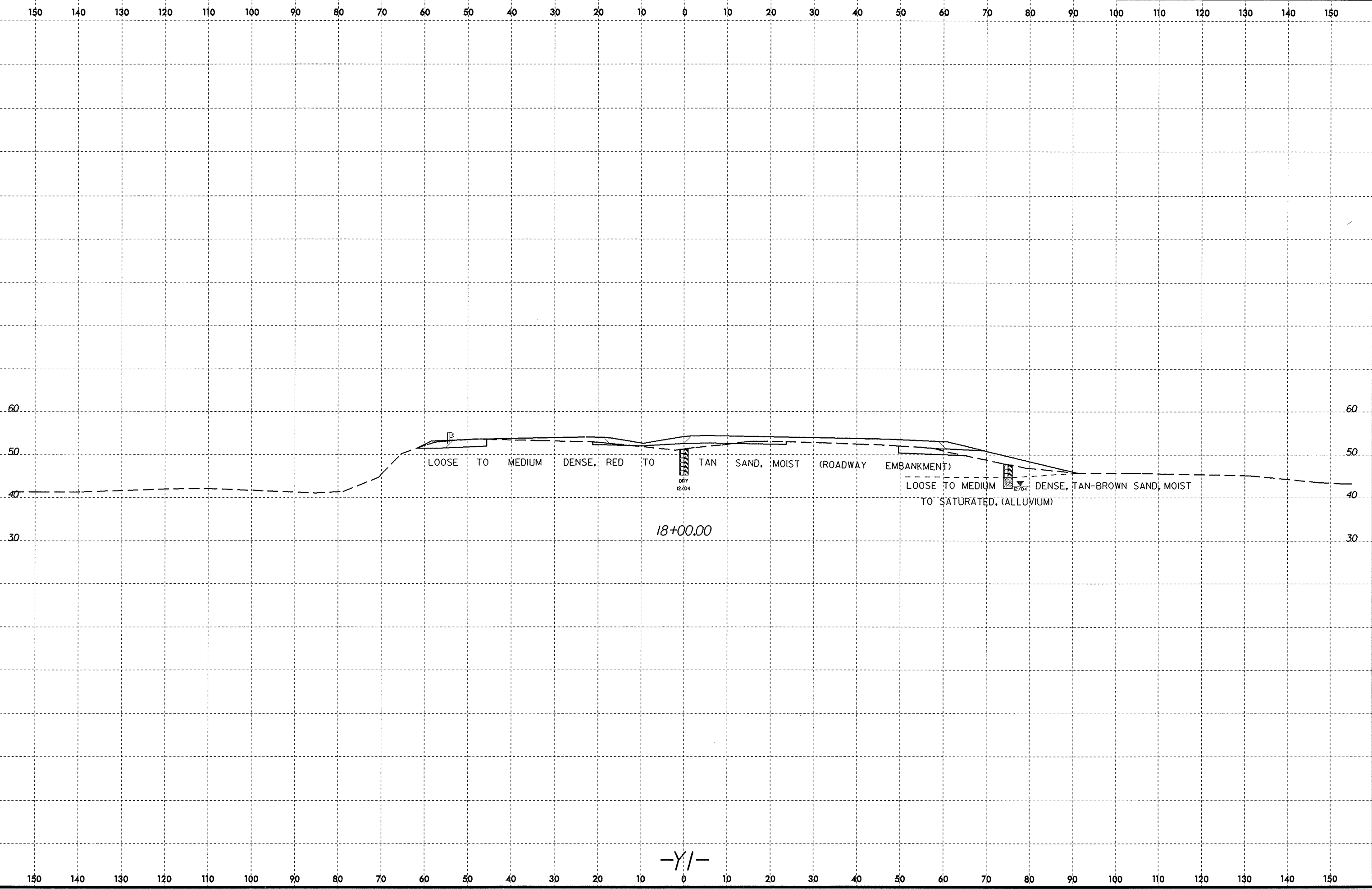
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8/23/99



PROJ. REFERENCE NO.	SHEET NO.
R-2719A	101



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 SonMiller At 6/22/08

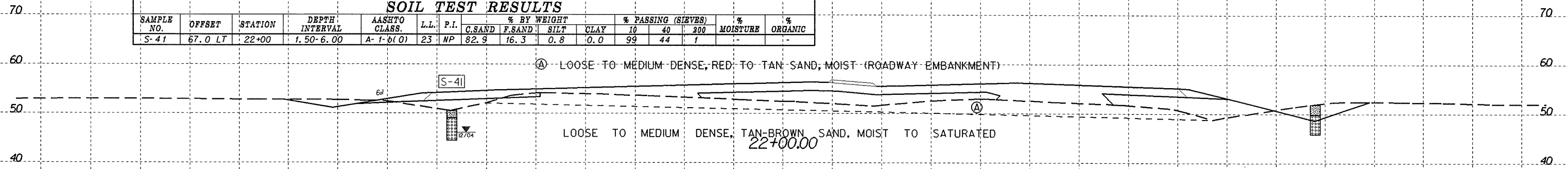
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8/23/99



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SOIL TEST RESULTS															
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							C.SAND	F.SAND	SILT	CLAY	10	40	200		
S-41	67.0 LT	22+00	1.50-6.00	A-1-b(0)	23	NP	82.9	16.3	0.8	0.0	99	44	1	-	-



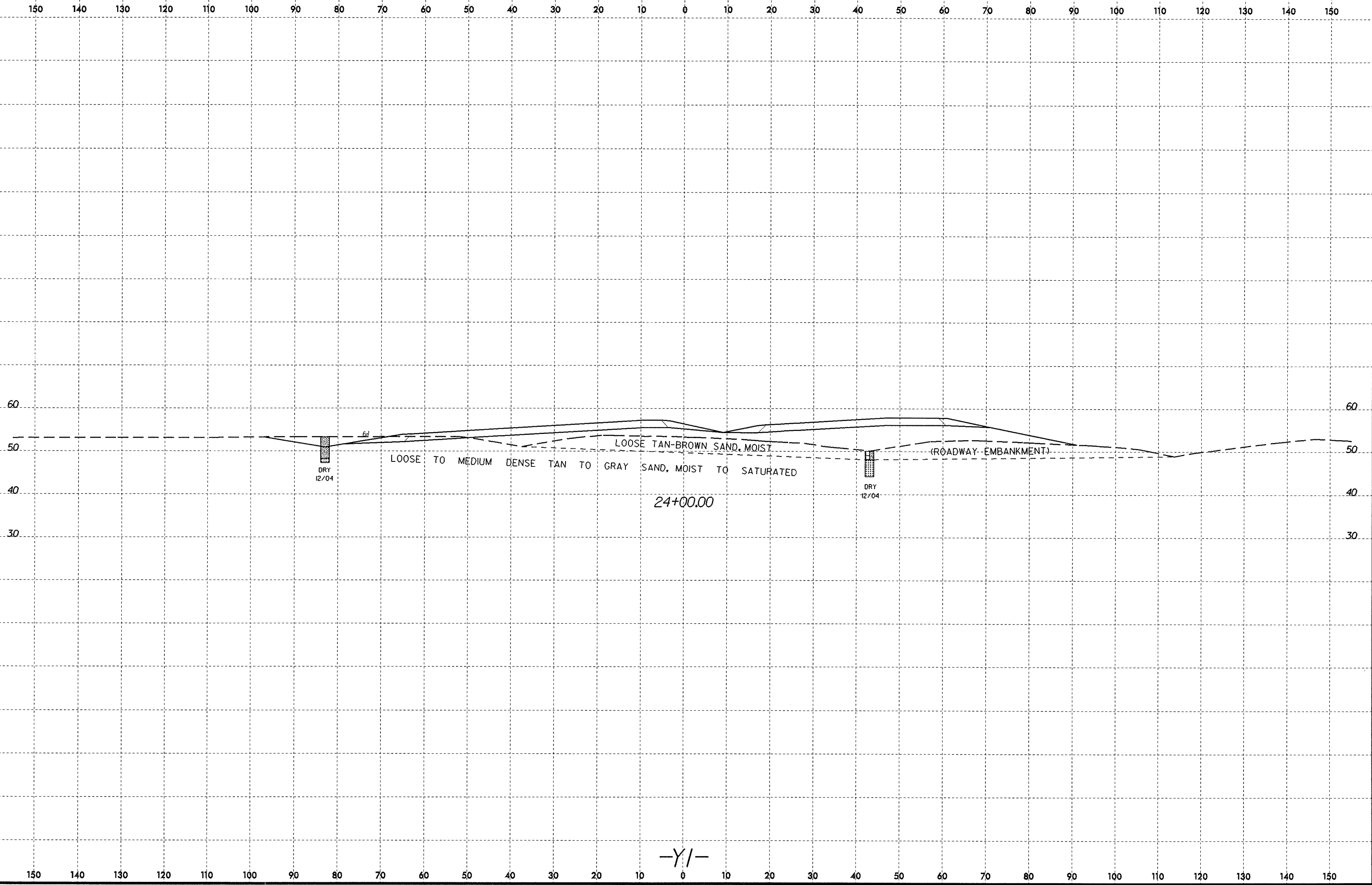
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8/23/99

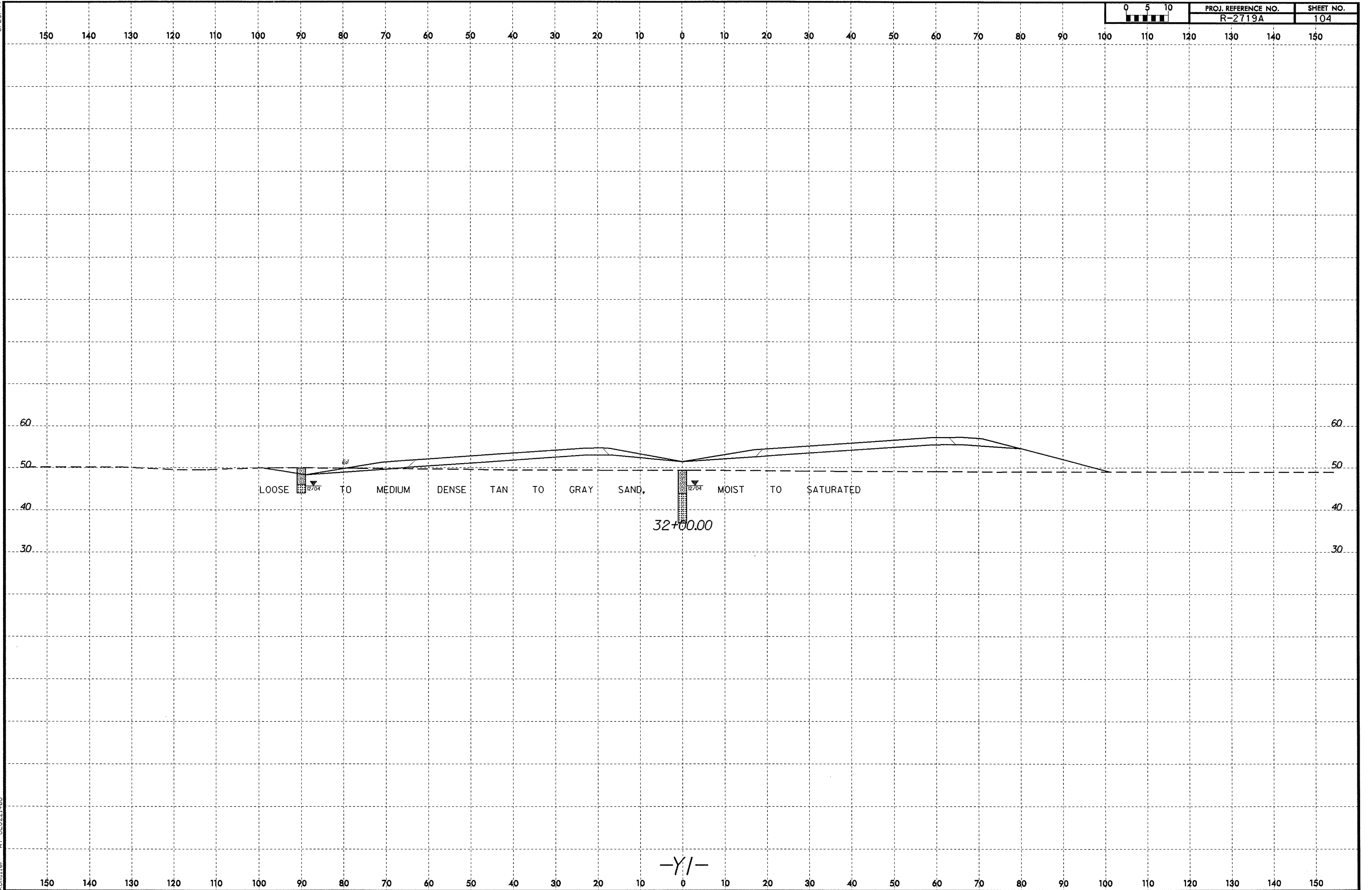
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	R-2719A	103



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James

0 5 10	PROJ. REFERENCE NO.	SHEET NO.
	R-2719A	104



LOOSE TO MEDIUM DENSE TAN TO GRAY SAND, MOIST TO SATURATED

32+00.00

-Y/-

150 140 130 120 110 100 90 80 70 60 50 40 30 20 10 0 10 20 30 40 50 60 70 80 90 100 110 120 130 140 150

70
60
50
40
30

70
60
50
40
30

SOIL TEST RESULTS

SAMPLE NO.	OFFSET	STATION	DEPTH INTERVAL	AASHTO CLASS.	L.I.	P.I.	% BY WEIGHT				% PASSING (SIEVES)			% MOISTURE	% ORGANIC
							C.SAND	F.SAND	SILT	CLAY	10	40	200		
S-1	89.0 LT	34+00	1.00-2.00	A-2(4)0	18	NP	35.5	40.9	7.5	16.1	99	85	25	-	-
S-2	89.0 LT	34+00	2.00-4.00	A-2(4)0	22	3	37.7	38.1	5.1	19.1	100	83	26	-	-
S-3	89.0 LT	34+00	4.00-6.00	A-3(0)	2#	NP	60.9	35.6	0.4	3.0	100	89	4	-	-

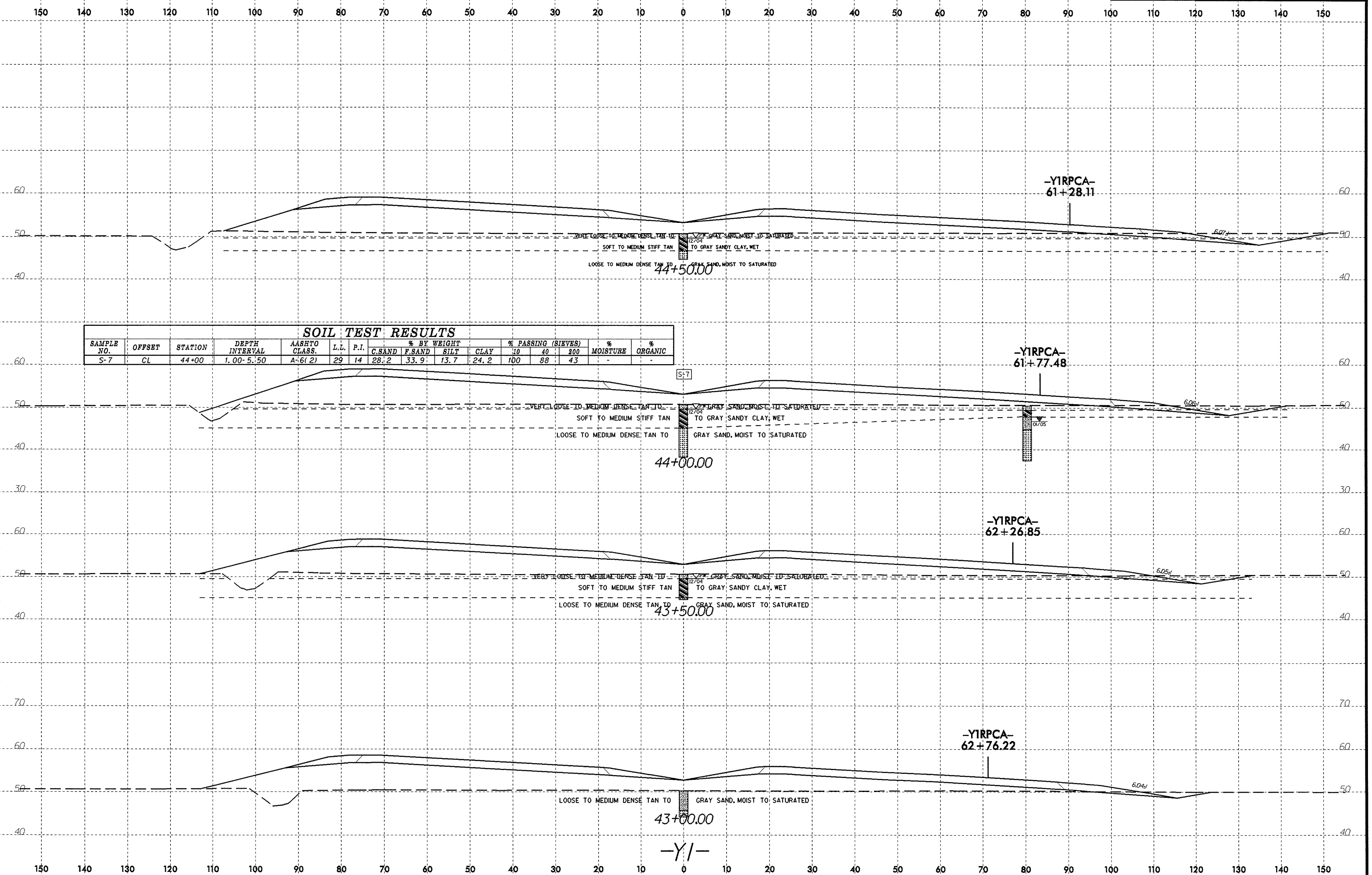
LOOSE TO MEDIUM DENSE TAN TO GRAY SAND, MOIST TO SATURATED

34+00.00

-Y/-

150 140 130 120 110 100 90 80 70 60 50 40 30 20 10 0 10 20 30 40 50 60 70 80 90 100 110 120 130 140 150

8/23/99



SOIL TEST RESULTS

SAMPLE NO.	OFFSET	STATION	DEPTH INTERVAL	AASHTO CLASS.	L.L.	P.I.	% BY WEIGHT				% PASSING (SIEVES)			% MOISTURE	% ORGANIC
							C.SAND	F.SAND	SILT	CLAY	#10	#40	#200		
S-7	CL	44+00	1.00-5.50	A-6(2)	29	14	28.2	33.9	13.7	24.2	100	88	43	-	-

44+50.00

44+00.00

43+50.00

43+00.00

-YI-

-YIRPCA-61+28.11

-YIRPCA-61+77.48

-YIRPCA-62+26.85

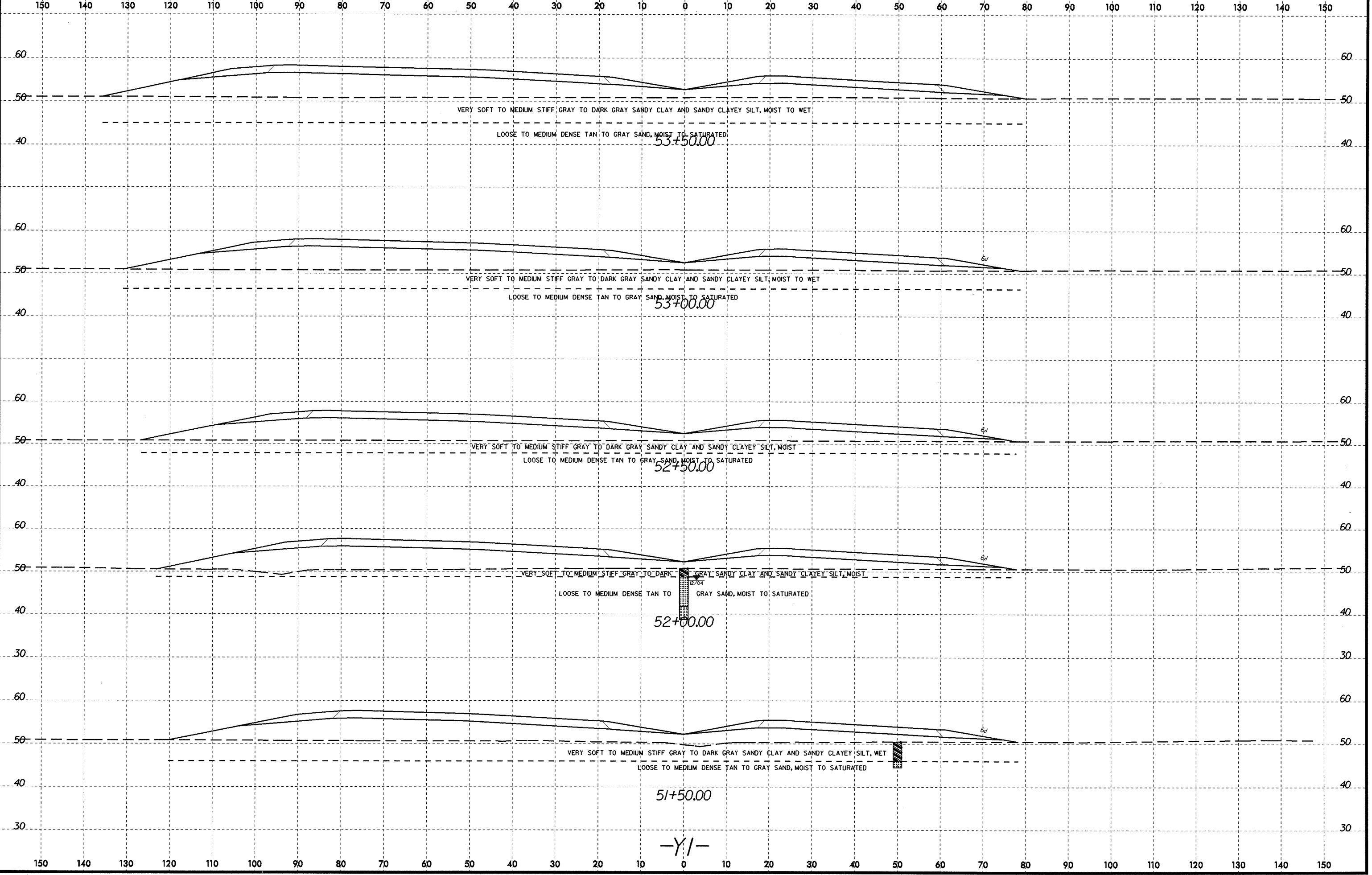
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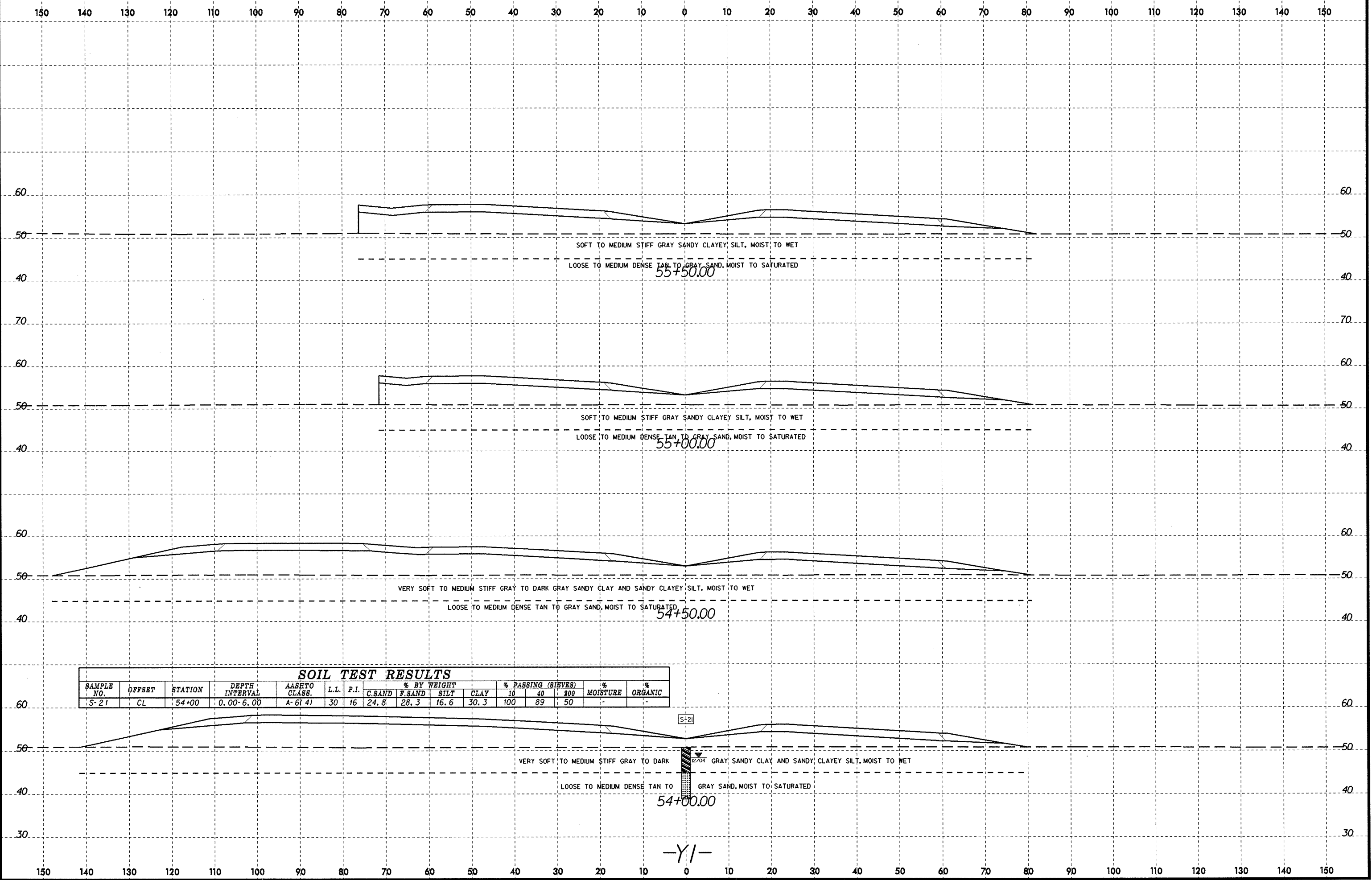
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PROJ. REFERENCE NO.	SHEET NO.
R-2719A	107



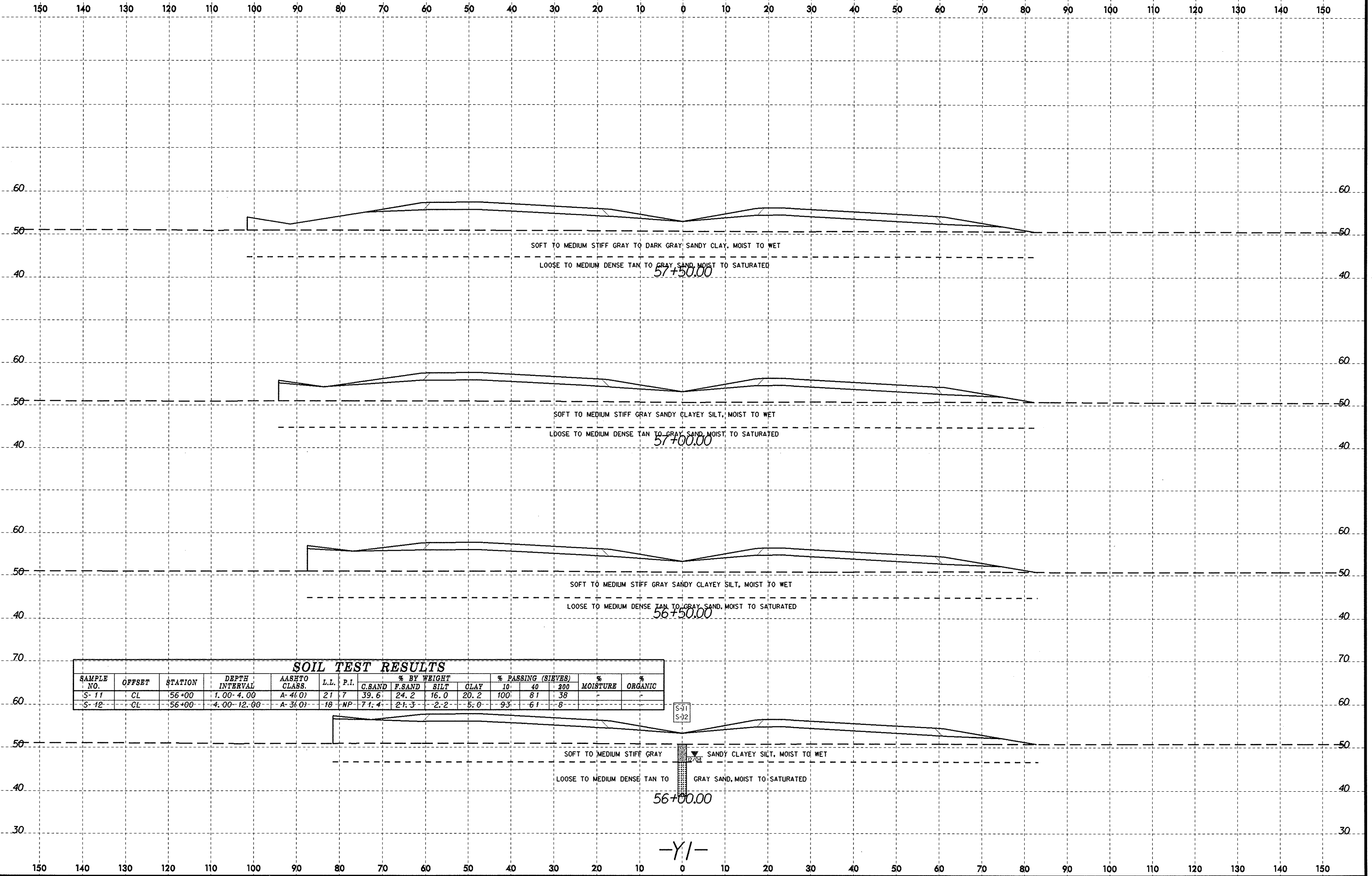
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SOIL TEST RESULTS

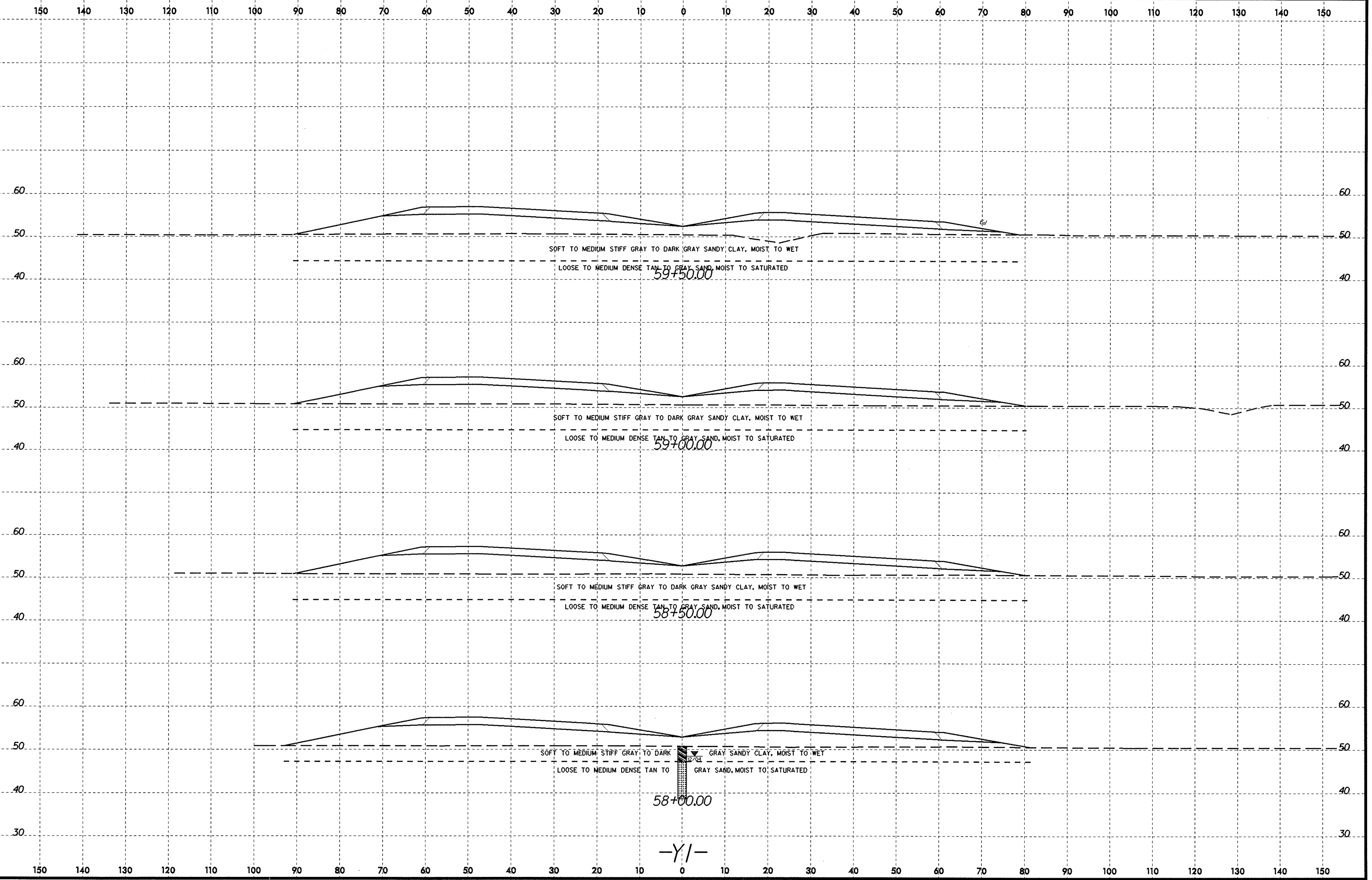
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							C.SAND	F.SAND	SILT	CLAY	10	40	200		
S-21	CL	54+00	0.00-6.00	A-6(4)	30	16	24.8	28.3	16.6	30.3	100	89	50	-	-

8/23/99
 30-NOV-2005 09:53
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 kmiller



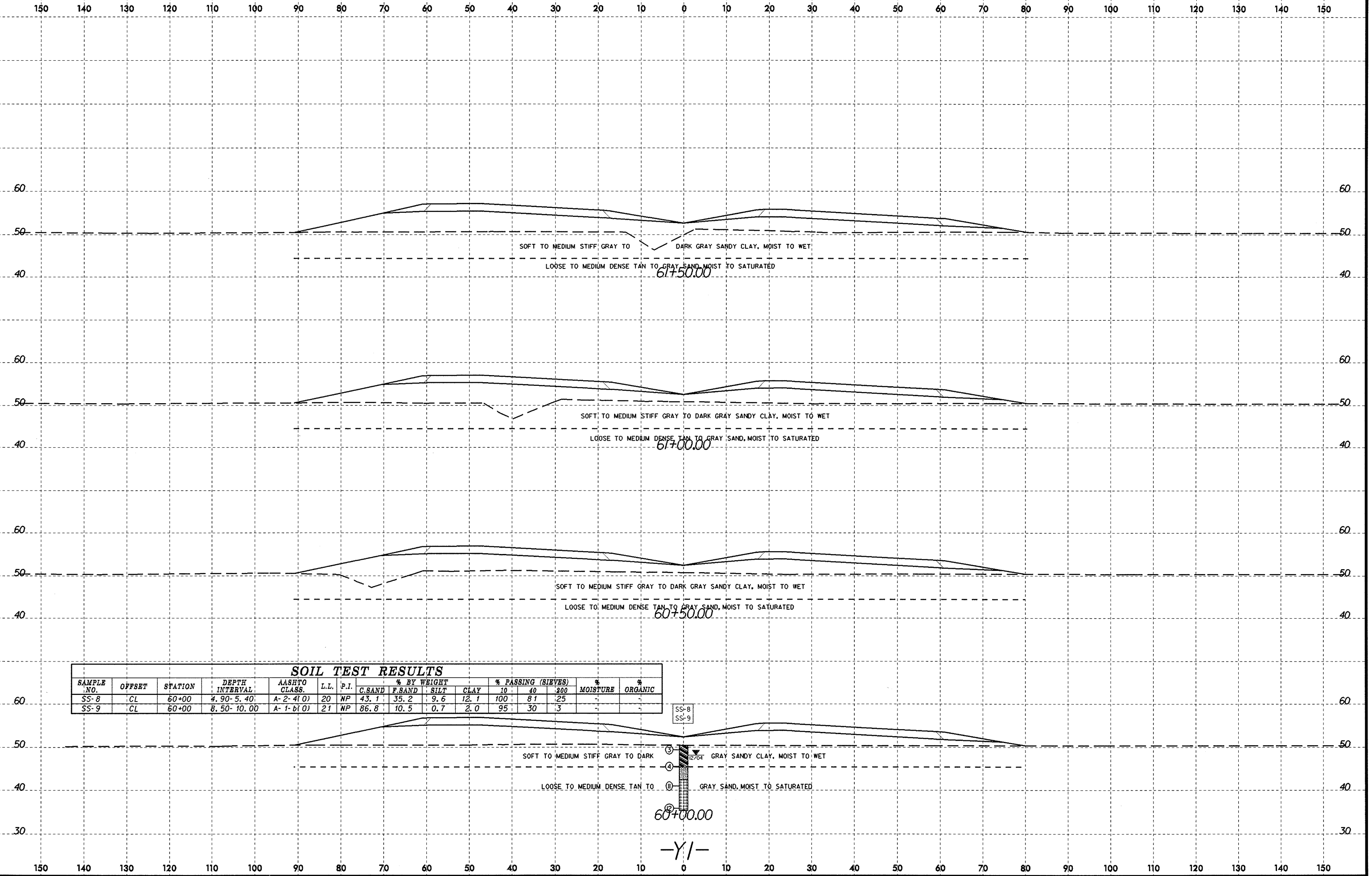
SOIL TEST RESULTS															
SAMPLE NO.	OFFSET	STATION	DEPTH INTERVAL	AASHTO CLASS.	L.L.	P.I.	% BY WEIGHT				% PASSING (SIEVES)			% MOISTURE	% ORGANIC
							C.SAND	F.SAND	SILT	CLAY	10	40	200		
S-11	CL	56+00	1.00-4.00	A-4(0)	21	7	39.6	24.2	16.0	20.2	100	81	38		
S-12	CL	56+00	4.00-12.00	A-3(0)	18	NP	71.4	21.3	2.2	5.0	93	61	8		

S-11
S-12



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SOIL TEST RESULTS

SAMPLE NO.	OFFSET	STATION	DEPTH INTERVAL	AASHTO CLASS.	L.L.	P.I.	% BY WEIGHT				% PASSING (SIEVES)			% MOISTURE	% ORGANIC
							C.SAND	F.SAND	SILT	CLAY	10	40	200		
SS-8	CL	60+00	4.90-5.40	A-2-4(0)	20	WP	43.1	35.2	9.6	12.1	100	81	25	-	-
SS-9	CL	60+00	8.50-10.00	A-1-b(0)	21	WP	86.8	10.5	0.7	2.0	95	30	3	-	-

SS-8
SS-9

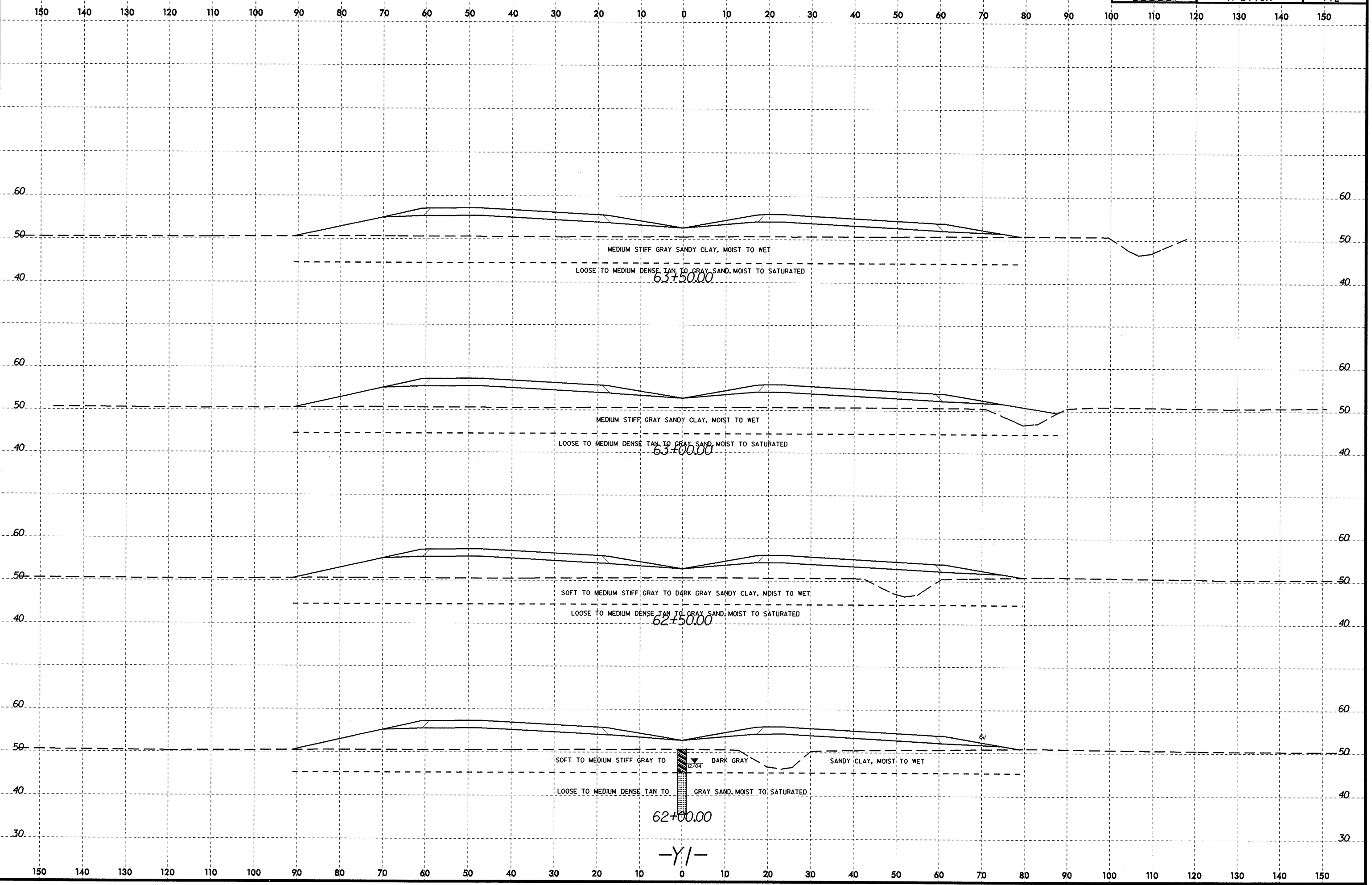
60+00.00
-Y/-

8/23/99



PROJ. REFERENCE NO.
R-2719A

SHEET NO.
112



MEDIUM STIFF GRAY SANDY CLAY, MOIST TO WET

LOOSE TO MEDIUM DENSE TAN TO GRAY SAND, MOIST TO SATURATED

63+50.00

MEDIUM STIFF GRAY SANDY CLAY, MOIST TO WET

LOOSE TO MEDIUM DENSE TAN TO GRAY SAND, MOIST TO SATURATED

63+00.00

SOFT TO MEDIUM STIFF GRAY TO DARK GRAY SANDY CLAY, MOIST TO WET

LOOSE TO MEDIUM DENSE TAN TO GRAY SAND, MOIST TO SATURATED

62+50.00

SOFT TO MEDIUM STIFF GRAY TO

DARK GRAY

SANDY CLAY, MOIST TO WET

LOOSE TO MEDIUM DENSE TAN TO

GRAY SAND, MOIST TO SATURATED

62+00.00

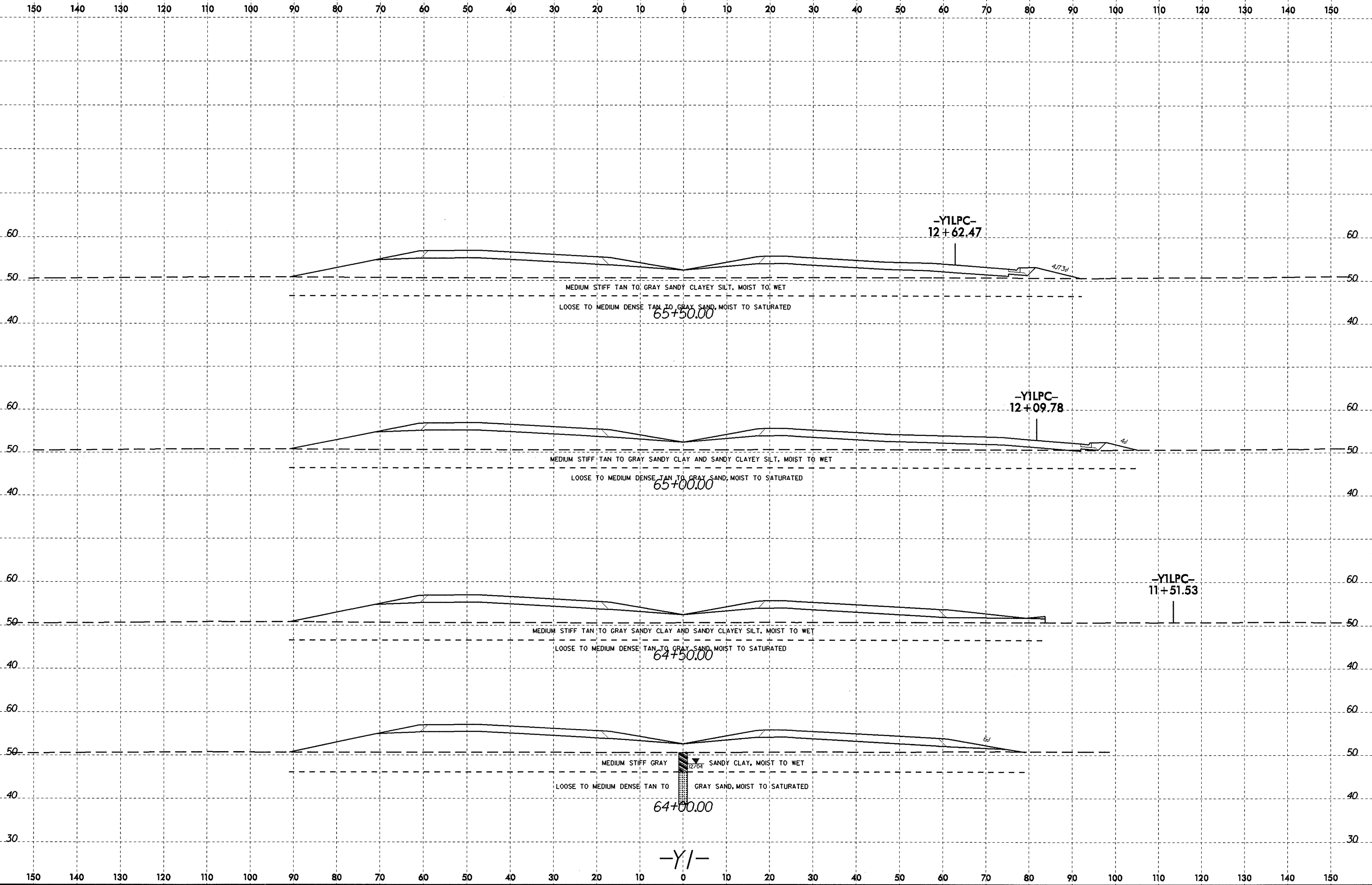
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Author: kmiller

8/23/99

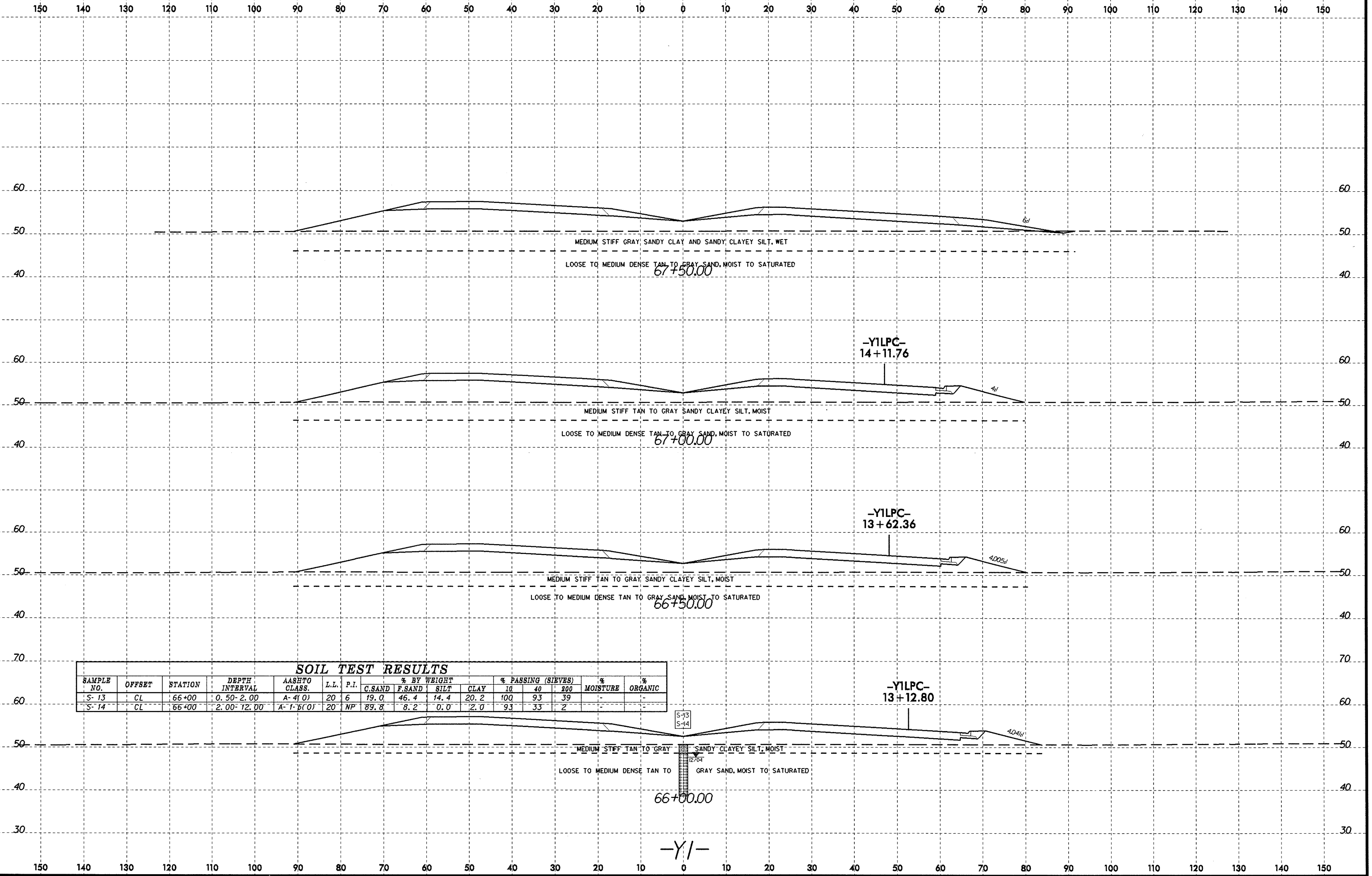


PROJ. REFERENCE NO.	SHEET NO.
R-2719A	113



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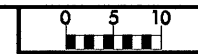


SOIL TEST RESULTS

SAMPLE NO.	OFFSET	STATION	DEPTH INTERVAL	AASHTO CLASS.	L.L.	P.I.	% BY WEIGHT				% PASSING (SIEVES)			% MOISTURE	% ORGANIC
							C.SAND	F.SAND	SILT	CLAY	10	40	200		
S-13	CL	66+00	0.50-2.00	A-4(0)	20	6	19.0	46.4	14.4	20.2	100	93	39	-	-
S-14	CL	66+00	2.00-12.00	A-1-B(0)	20	NP	89.8	8.2	0.0	2.0	93	33	2	-	-

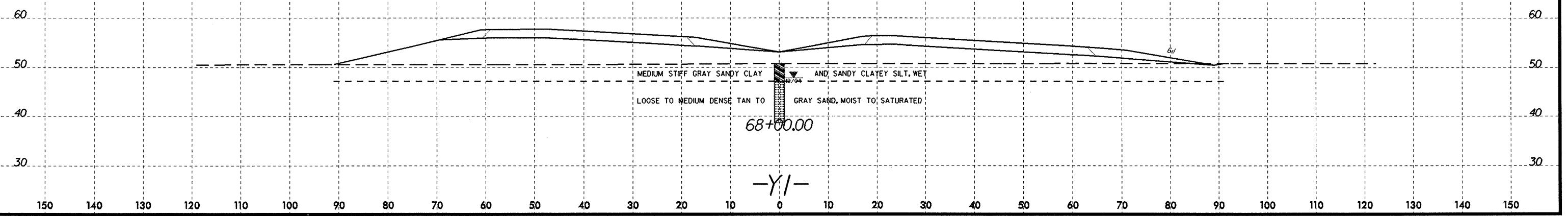
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8/23/99



PROJ. REFERENCE NO.	SHEET NO.
R-2719A	115

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68+00.00

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8/23/99

0 5 10

PROJ. REFERENCE NO.
R-2719A

SHEET NO.
116

150 140 130 120 110 100 90 80 70 60 50 40 30 20 10 0 10 20 30 40 50 60 70 80 90 100 110 120 130 140 150

SOIL TEST RESULTS															
SAMPLE NO.	OFFSET	STATION	DEPTH INTERVAL	AASHTO CLASS.	L.L.	P.I.	% BY WEIGHT				% PASSING (SIEVES)			% MOISTURE	% ORGANIC
							C.SAND	F.SAND	SILT	CLAY	10	40	200		
S-27	CL	108+00	13.50-17.80	A-2-4(0)	22	NP	68.6	16.0	4.3	11.2	100	72	16	-	-

S-27

VERY LOOSE TO MEDIUM DENSE TAN TO GRAY SAND, MOIST TO SATURATED

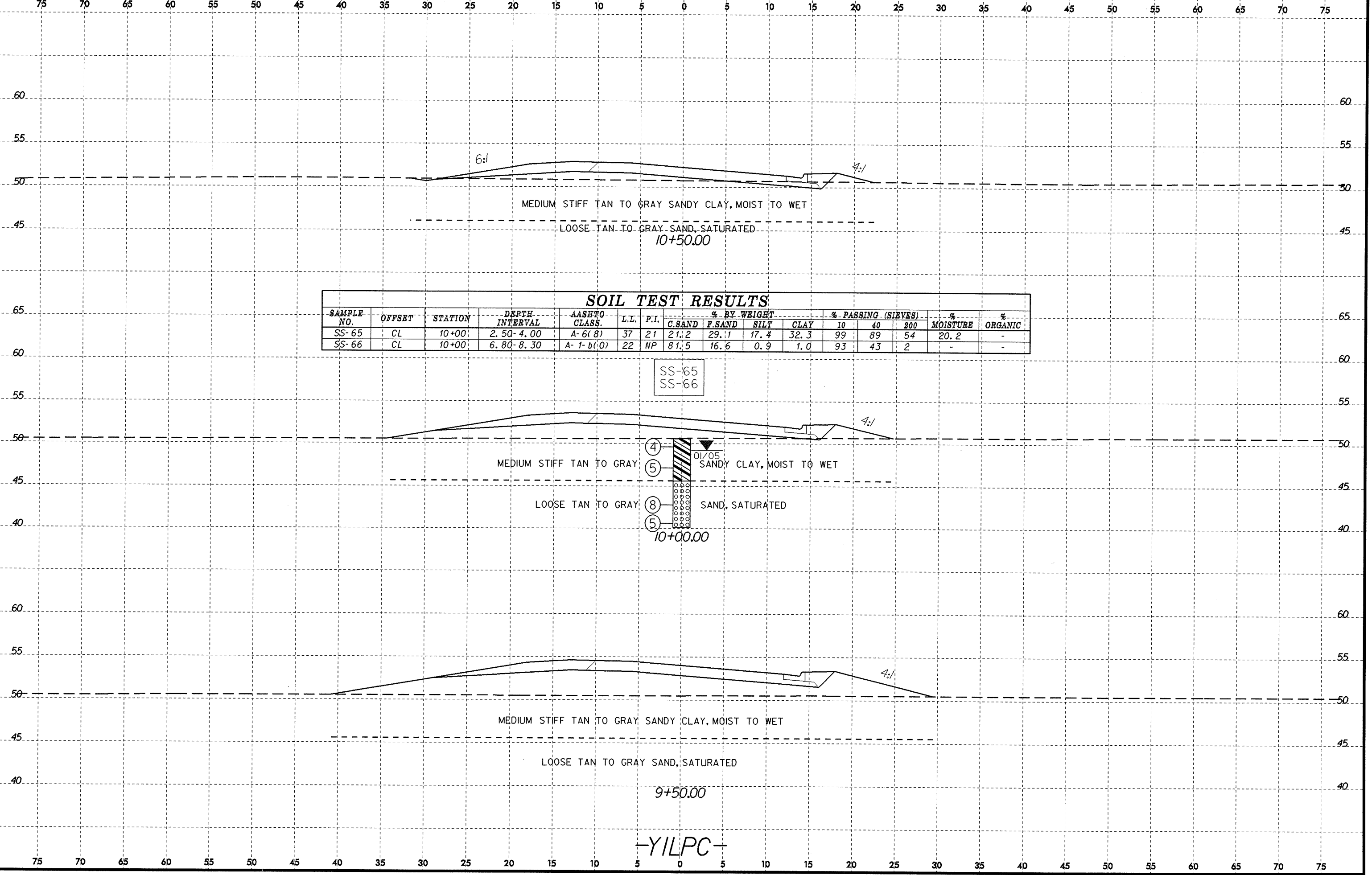
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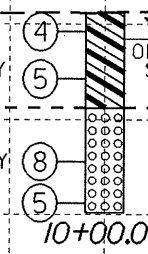
8/23/99



SOIL TEST RESULTS

SAMPLE NO.	OFFSET	STATION	DEPTH INTERVAL	AASHTO CLASS.	L.L.	P.I.	% BY WEIGHT				% PASSING (SIEVES)			% MOISTURE	% ORGANIC
							C.SAND	F.SAND	SILT	CLAY	10	40	200		
SS-65	CL	10+00	2.50-4.00	A-6(8)	37	21	21.2	29.1	17.4	32.3	99	89	54	20.2	-
SS-66	CL	10+00	6.80-8.30	A-1-b(0)	22	NP	81.5	16.6	0.9	1.0	93	43	2	-	-

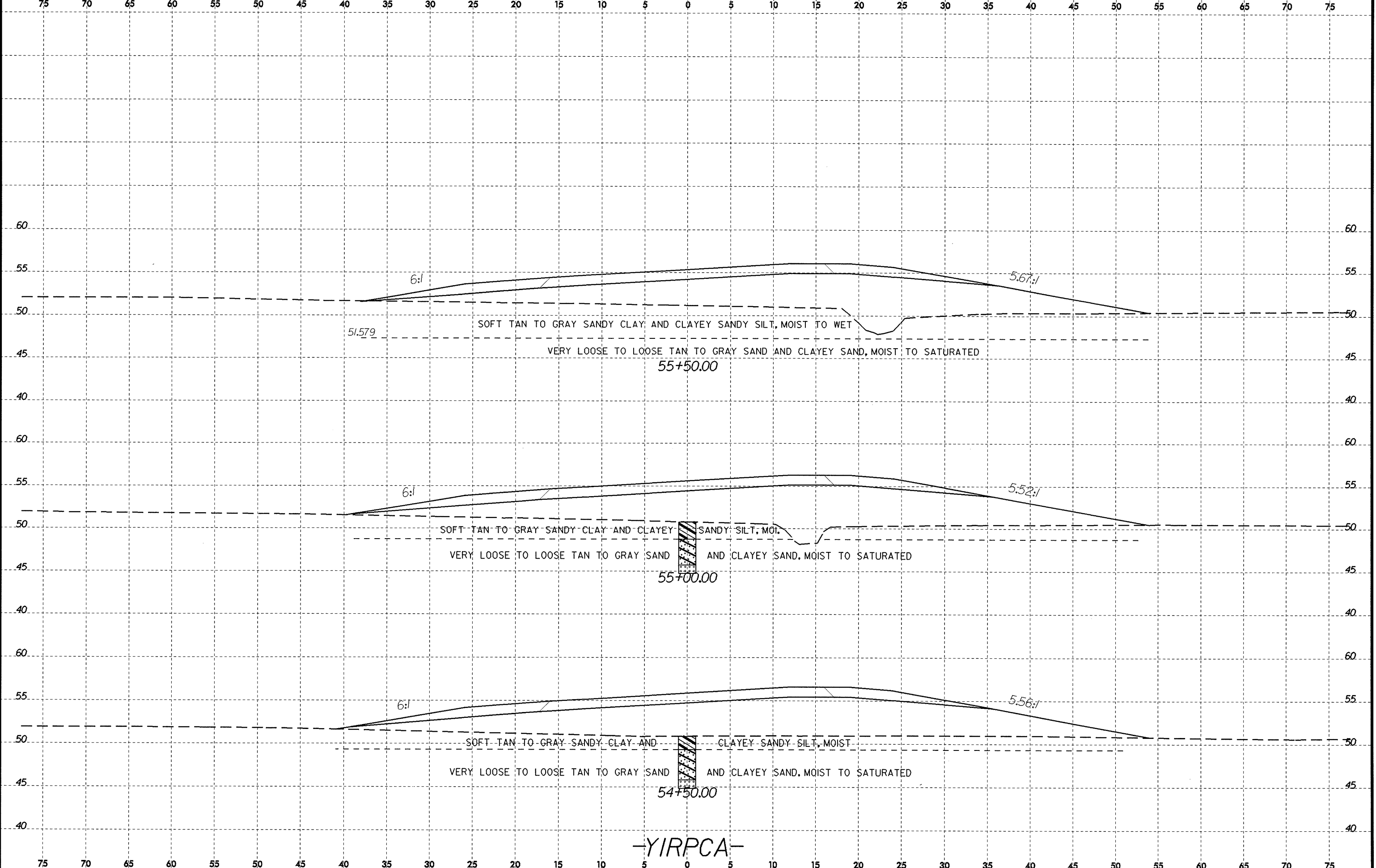
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SS-66



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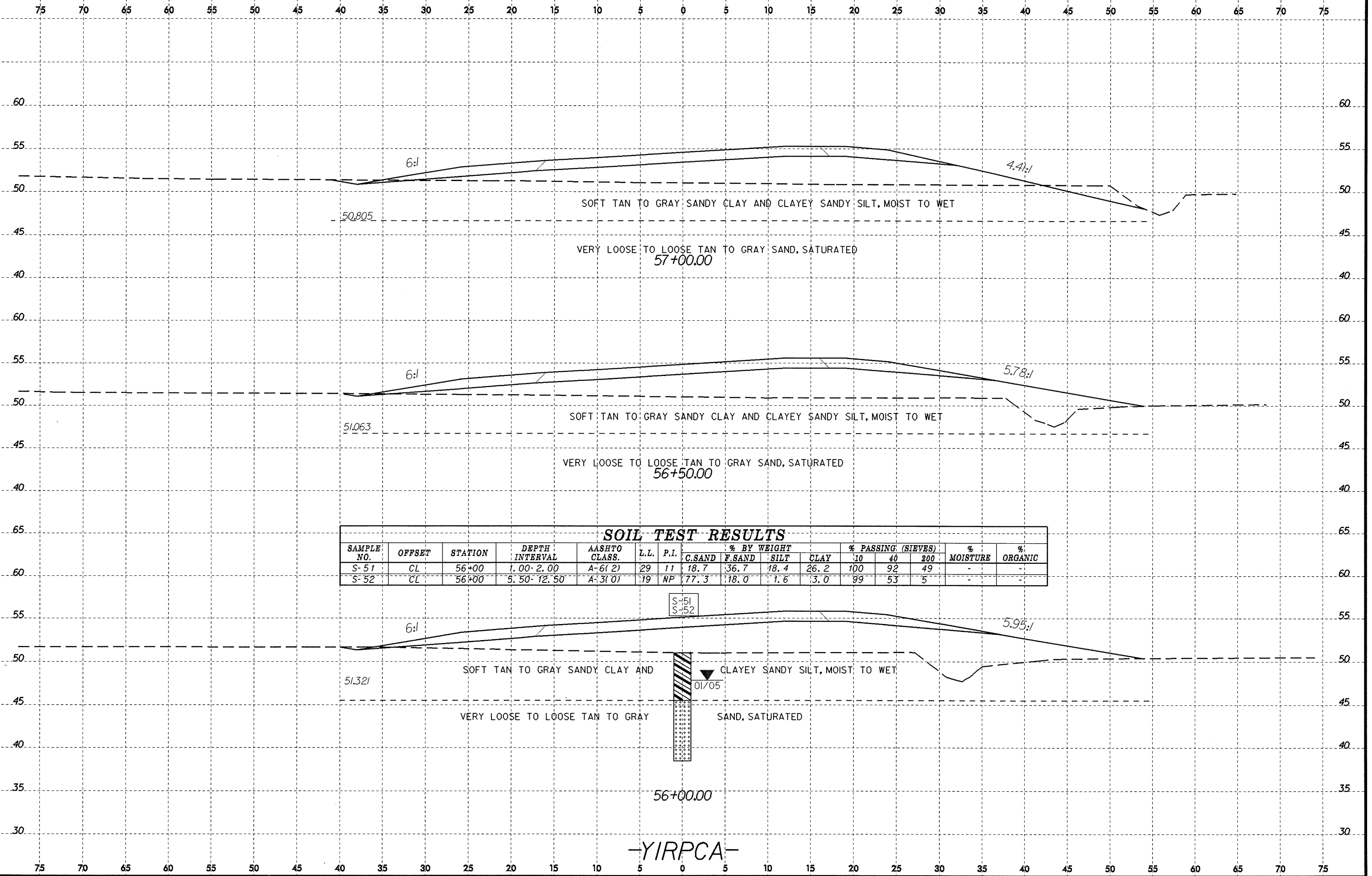
8/23/99



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Somiller AT 05/22/08

8/23/99
 10-NOV-2005 11:03
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 Kbmiller

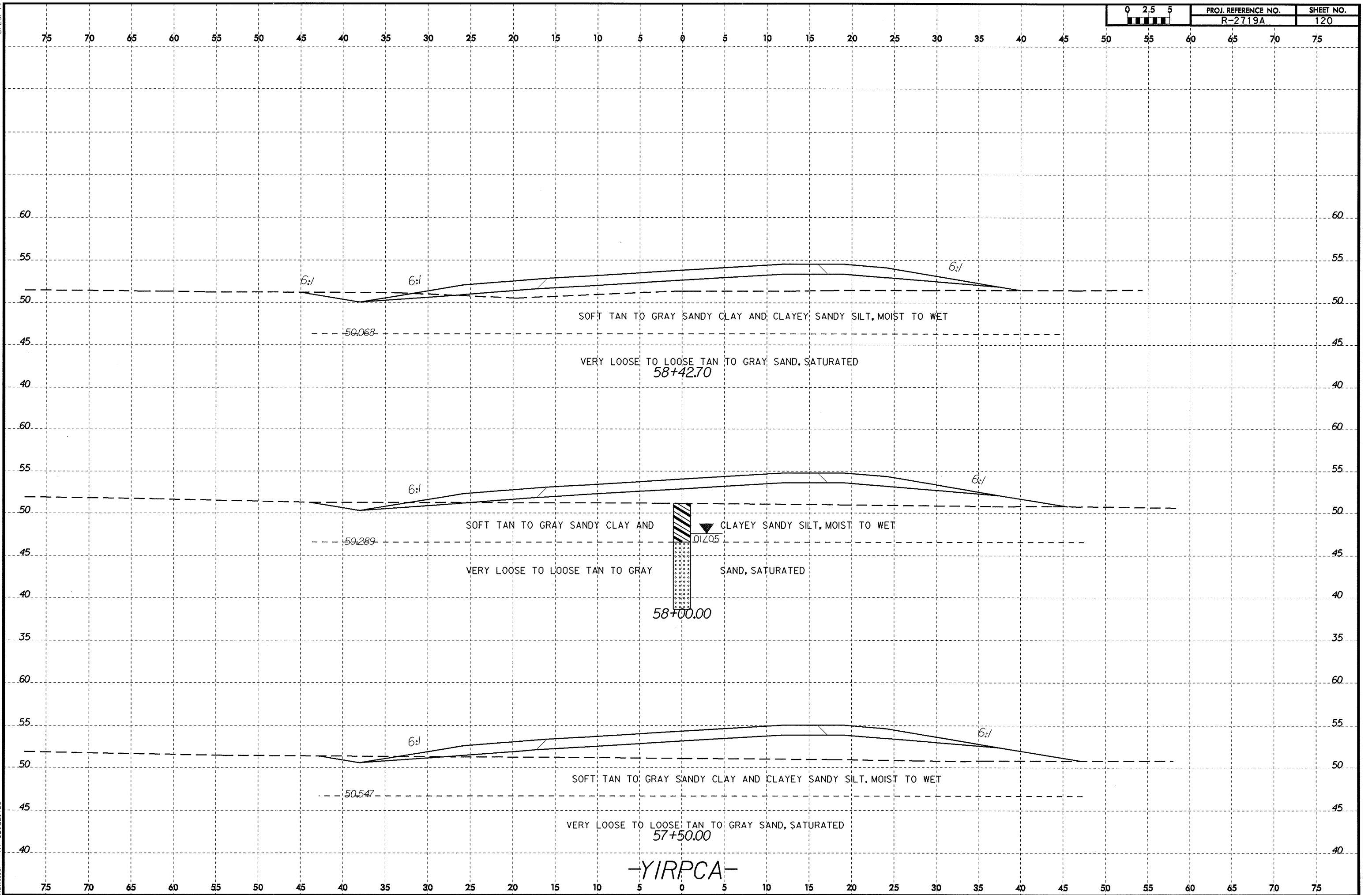


SOIL TEST RESULTS

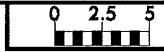
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							C.SAND	F.SAND	SILT	CLAY	10	40	200		
S-51	CL	56+00	1.00-2.00	A-6(2)	29	11	18.7	36.7	18.4	26.2	100	92	49	-	-
S-52	CL	56+00	5.50-12.50	A-3(0)	19	NP	77.3	18.0	1.6	3.0	99	53	5	-	-

-YIRPCA-

8/23/99
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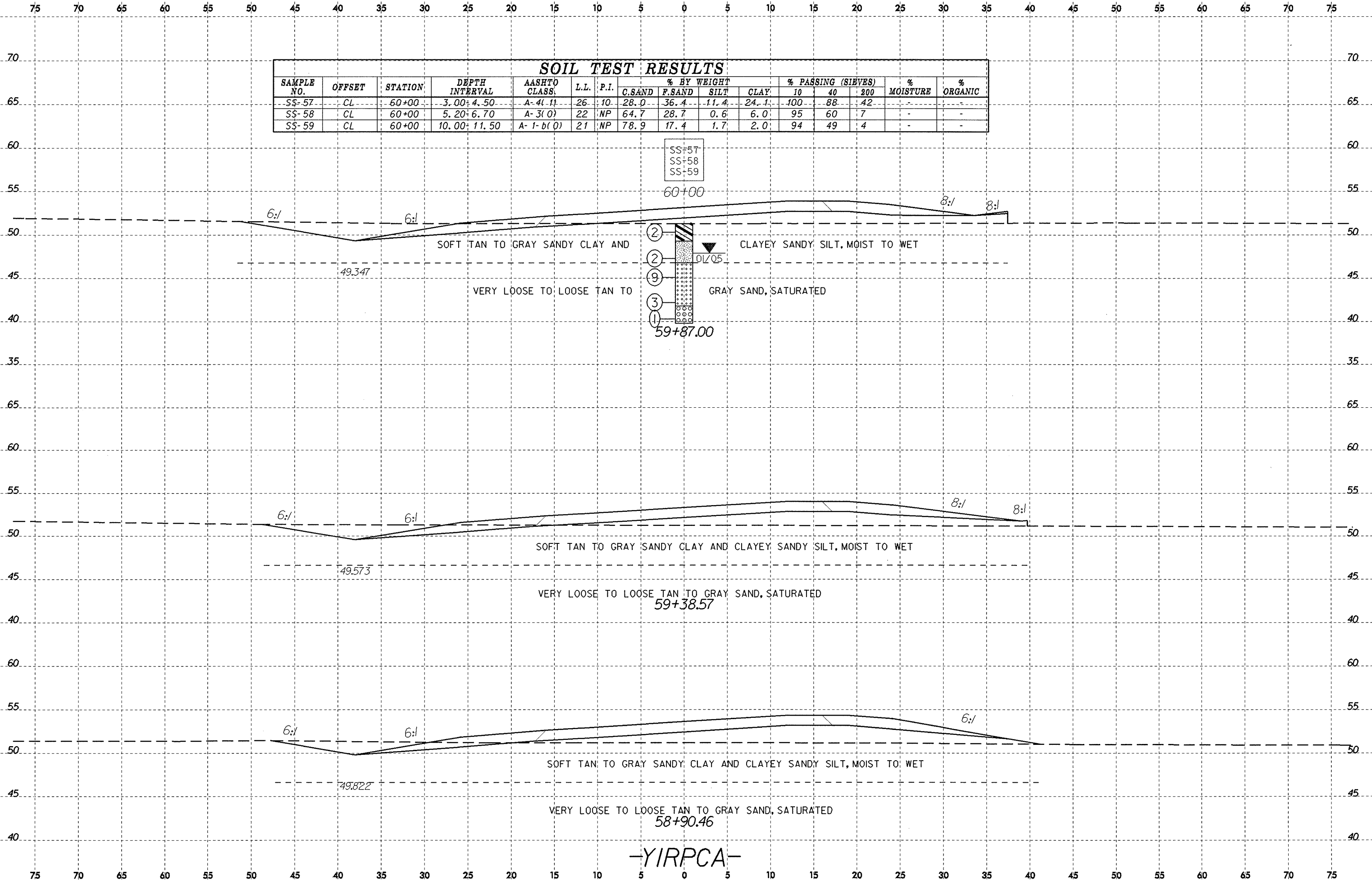


8/23/99



SOIL TEST RESULTS

SAMPLE NO.	OFFSET	STATION	DEPTH INTERVAL	AASHTO CLASS.	L.L.	P.I.	% BY WEIGHT				% PASSING (SIEVES)			% MOISTURE	% ORGANIC
							C.SAND	F.SAND	SILT	CLAY	10	40	200		
SS-57	CL	60+00	3.00-4.50	A-4(1)	26	10	28.0	36.4	11.4	24.1	100	88	42	-	-
SS-58	CL	60+00	5.20-6.70	A-3(0)	22	NP	64.7	28.7	0.6	6.0	95	60	7	-	-
SS-59	CL	60+00	10.00-11.50	A-1-b(0)	21	NP	78.9	17.4	1.7	2.0	94	49	4	-	-

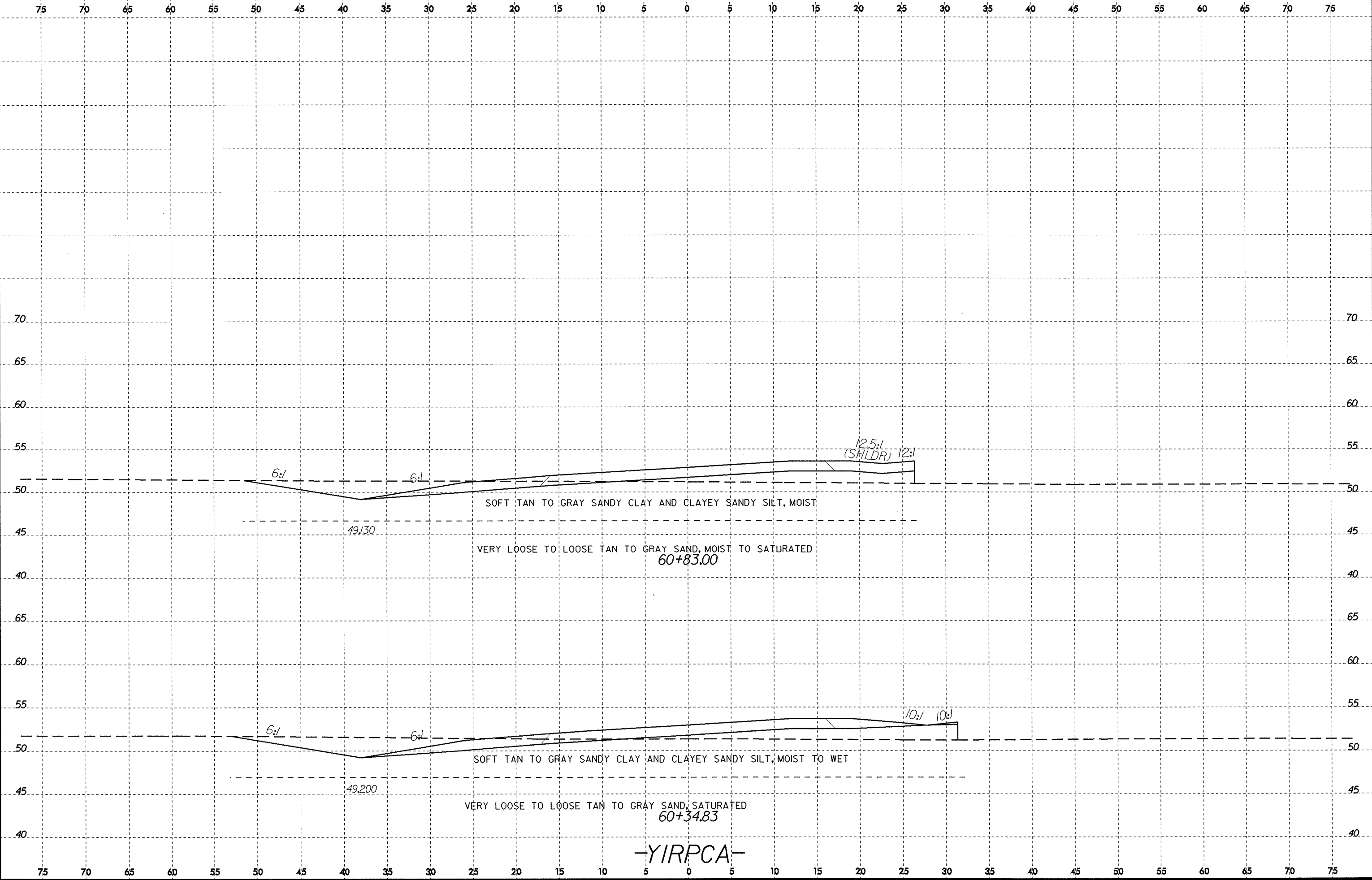


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0 2.5 5 [Scale Bar]	PROJ. REFERENCE NO. R-2719A	SHEET NO. 122
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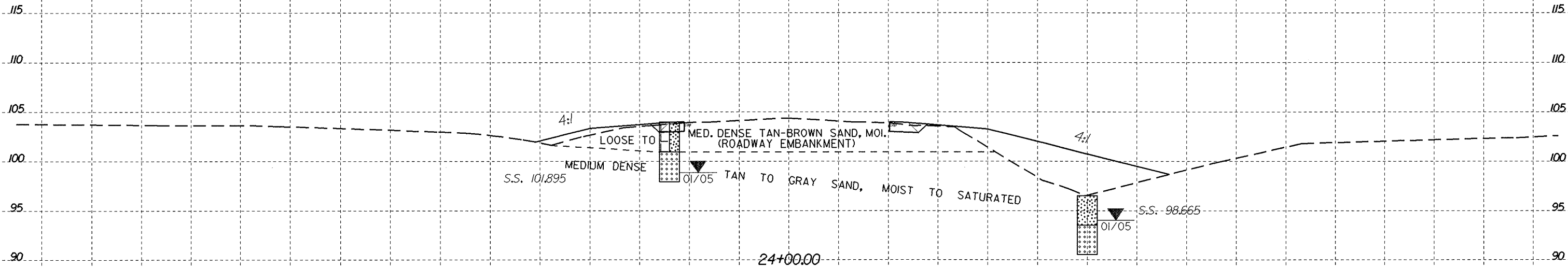
8/23/99



PROJ. REFERENCE NO.
R-2719A

SHEET NO.
123

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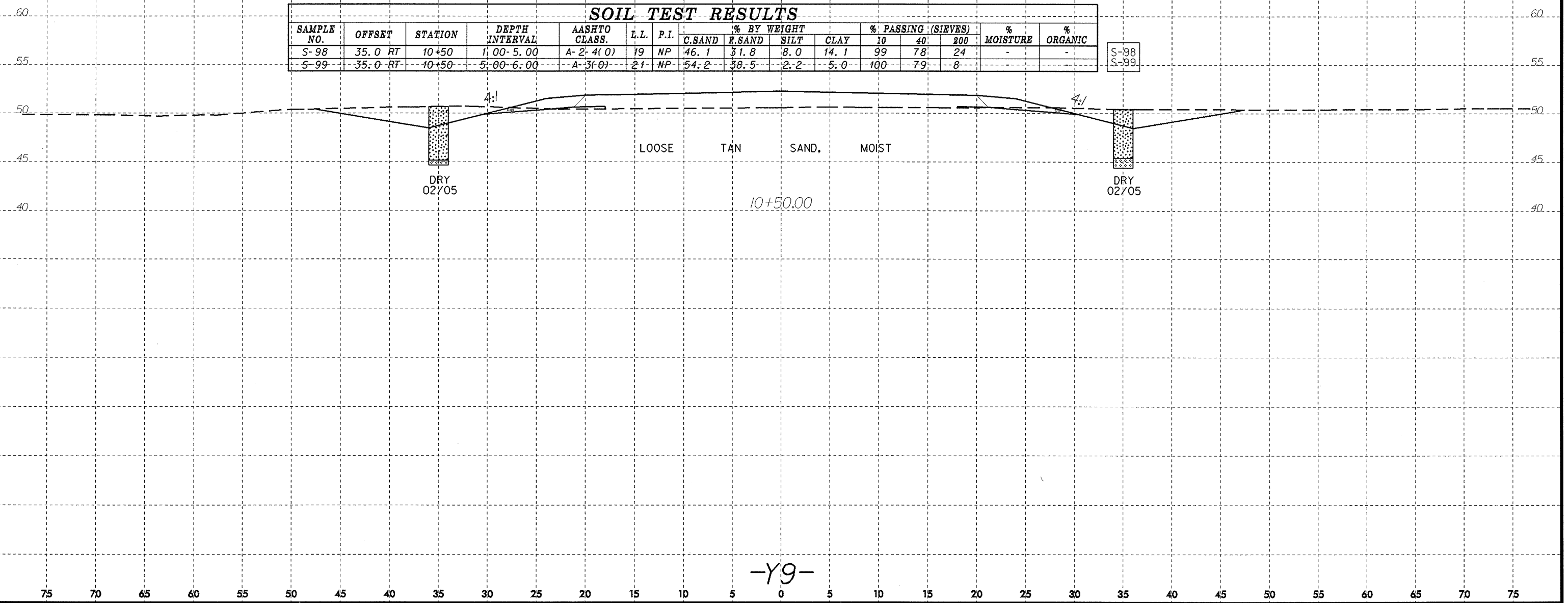
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8/23/99

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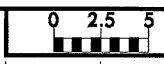
SOIL TEST RESULTS																
SAMPLE NO.	OFFSET	STATION	DEPTH INTERVAL	AASHTO CLASS.	L.L.	P.I.	% BY WEIGHT				% PASSING (SIEVES)			% MOISTURE	% ORGANIC	
							C.SAND	F.SAND	SILT	CLAY	10	40	200			
S-98	35.0 RT	10+50	1.00-5.00	A-2-4(0)	19	NP	46.1	31.8	8.0	14.1	99	78	24	-	-	
S-99	35.0 RT	10+50	5.00-6.00	A-3(0)	21	NP	54.2	38.5	2.2	5.0	100	79	8	-	-	

S-98
S-99

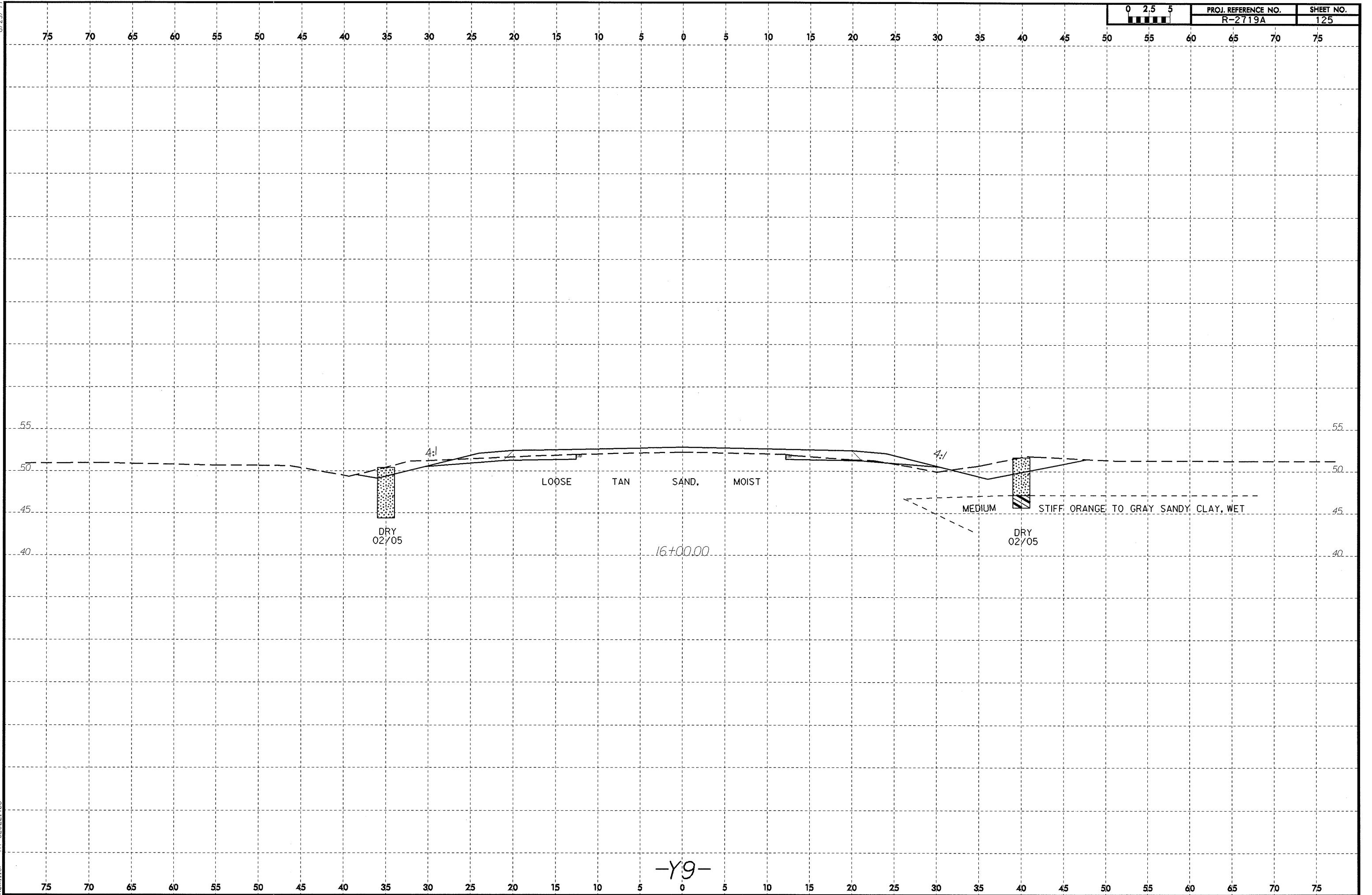


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8/23/99



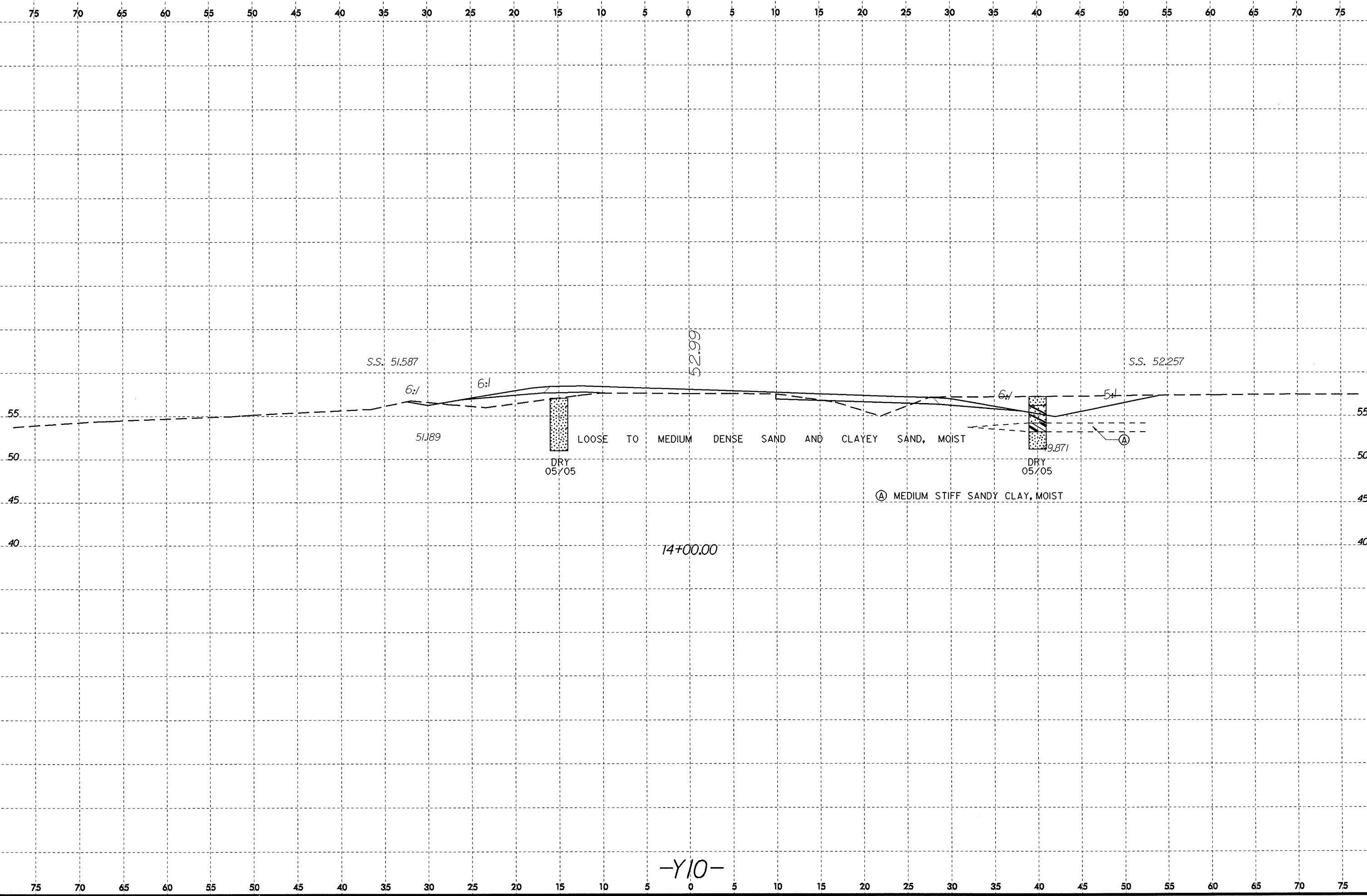
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R-2719A	125



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 kmiller

-Y9-

8/23/99



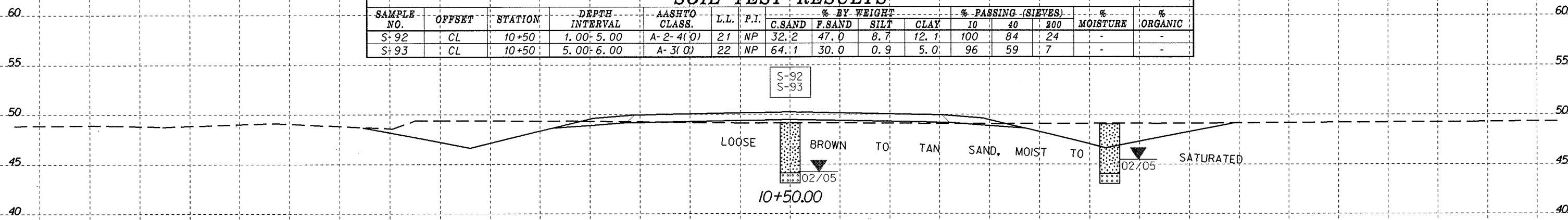
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Miller AT GEJ221408

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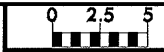
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SOIL TEST RESULTS															
SAMPLE NO.	OFFSET	STATION	DEPTH INTERVAL	AASHTO CLASS.	L.L.	P.T.	% BY WEIGHT				% PASSING (SIEVES)			% MOISTURE	% ORGANIC
							C.SAND	F.SAND	SILT	CLAY	10	40	200		
S-92	CL	10+50	1.00-5.00	A-2-4(0)	21	NP	32.2	47.0	8.7	12.1	100	84	24	-	-
S-93	CL	10+50	5.00-6.00	A-3(0)	22	NP	64.1	30.0	0.9	5.0	96	59	7	-	-



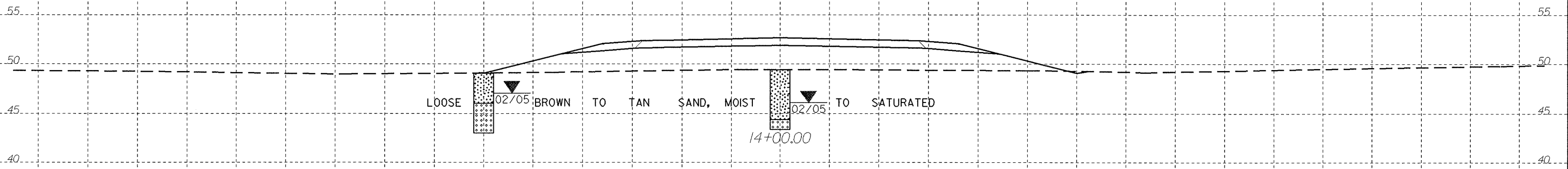
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8/23/99



PROJ. REFERENCE NO.	SHEET NO.
R-2719A	128

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LOOSE 02/05 BROWN TO TAN SAND, MOIST 02/05 TO SATURATED

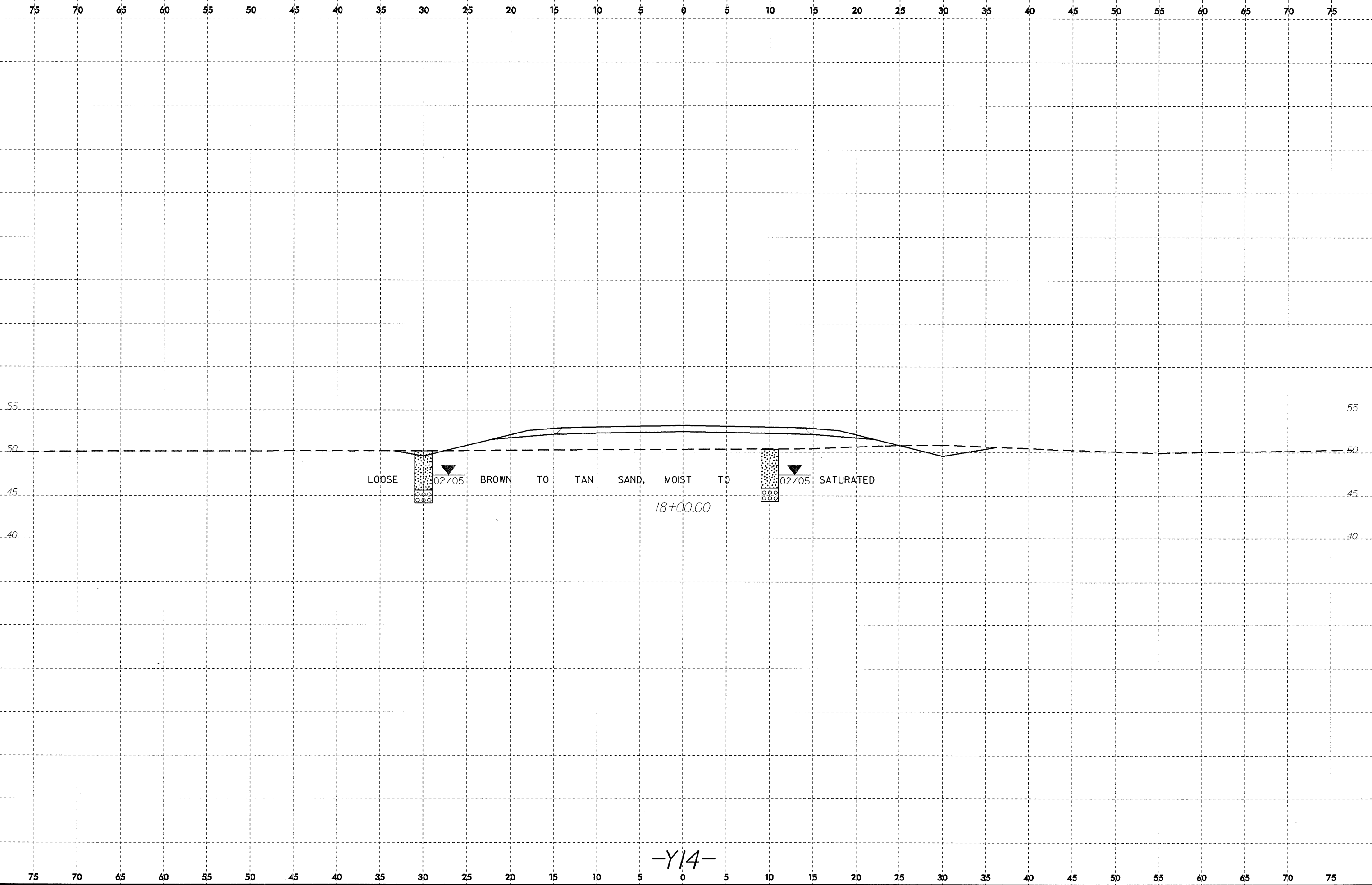
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 8/23/99

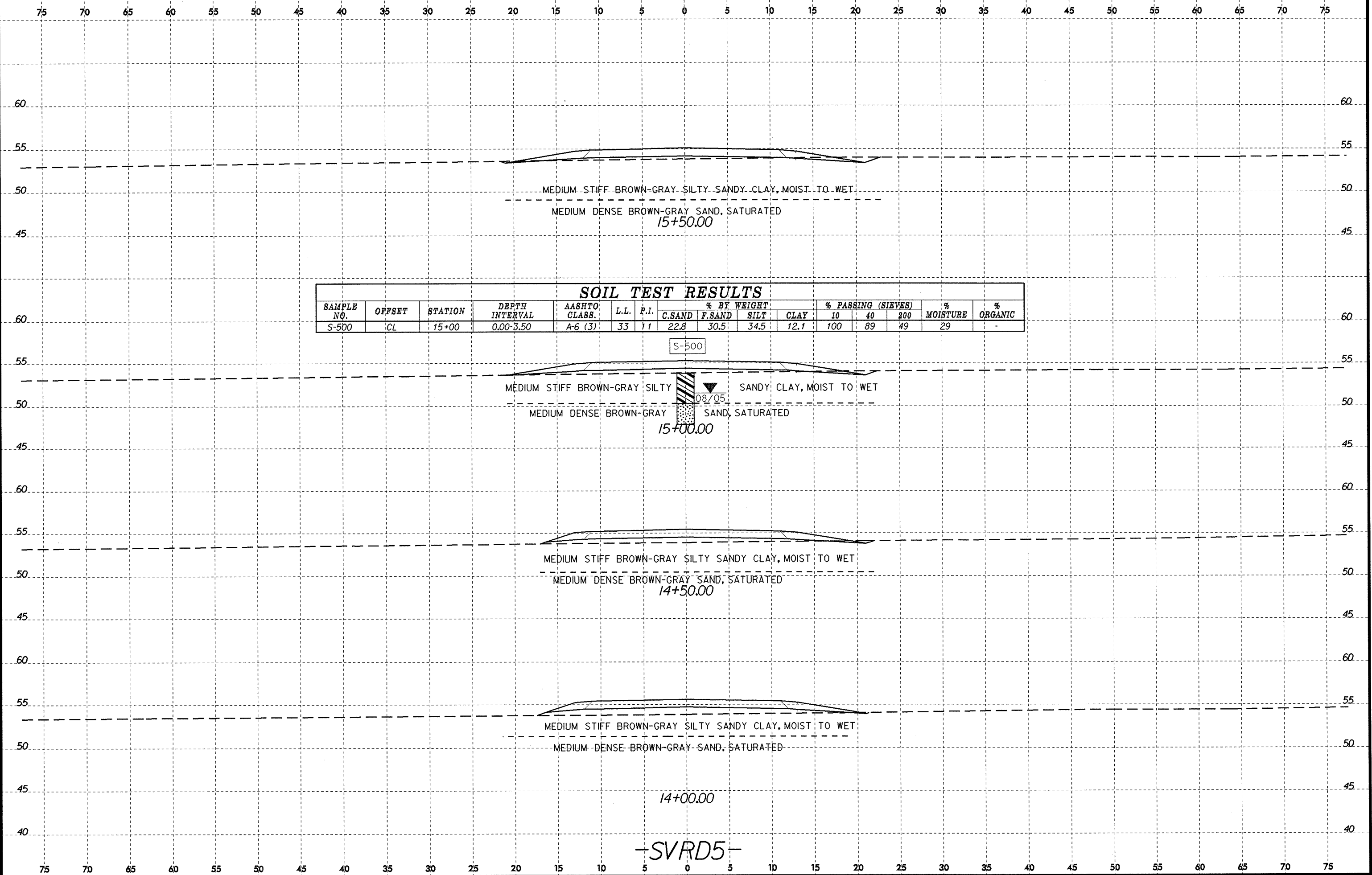
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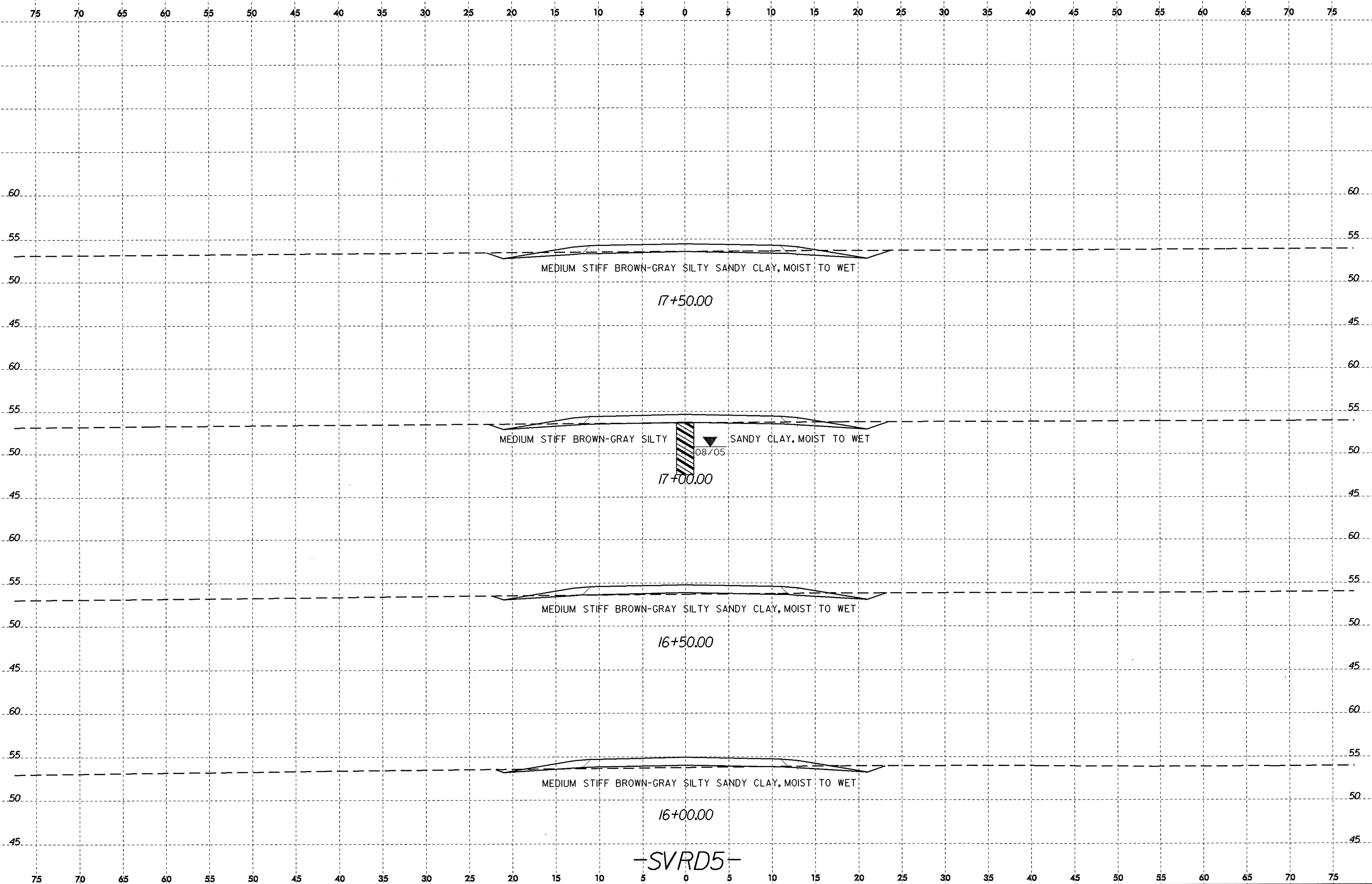
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SOIL TEST RESULTS															
SAMPLE NO.	OFFSET	STATION	DEPTH INTERVAL	AASHTO CLASS.	L.L.	P.I.	% BY WEIGHT				% PASSING (SIEVES)			% MOISTURE	% ORGANIC
							C.SAND	F.SAND	SILT	CLAY	10	40	200		
S-500	CL	15+00	0.00-3.50	A-6 (3)	33	11	22.8	30.5	34.5	12.1	100	89	49	29	-

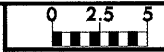
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8/23/99



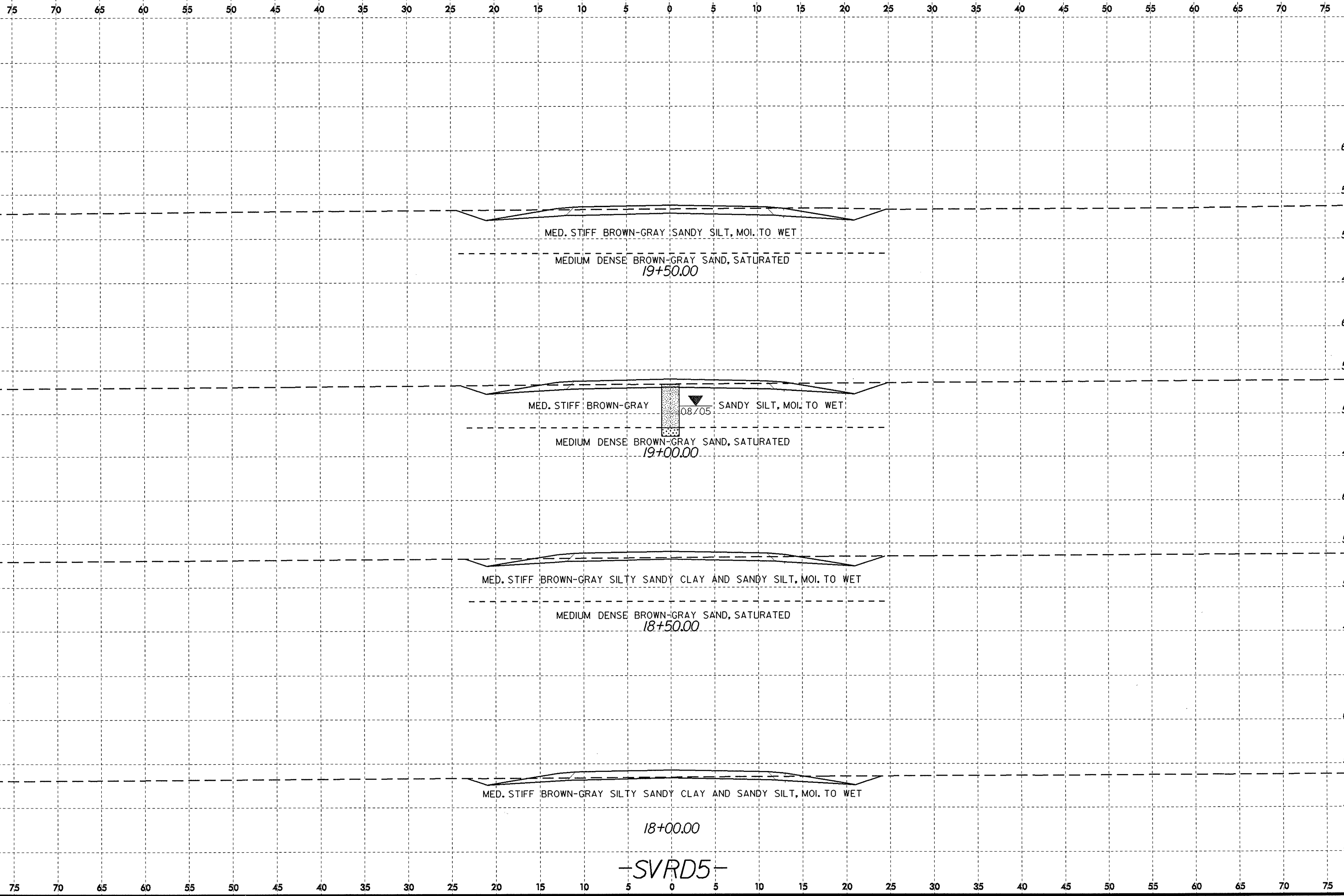
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8/23/99



PROJ. REFERENCE NO.
R-2719A

SHEET NO.
132



MED. STIFF BROWN-GRAY SANDY SILT, MOI. TO WET

MEDIUM DENSE BROWN-GRAY SAND, SATURATED
19+50.00

MED. STIFF BROWN-GRAY SANDY SILT, MOI. TO WET

MEDIUM DENSE BROWN-GRAY SAND, SATURATED
19+00.00

08/05

MED. STIFF BROWN-GRAY SILTY SANDY CLAY AND SANDY SILT, MOI. TO WET

MEDIUM DENSE BROWN-GRAY SAND, SATURATED
18+50.00

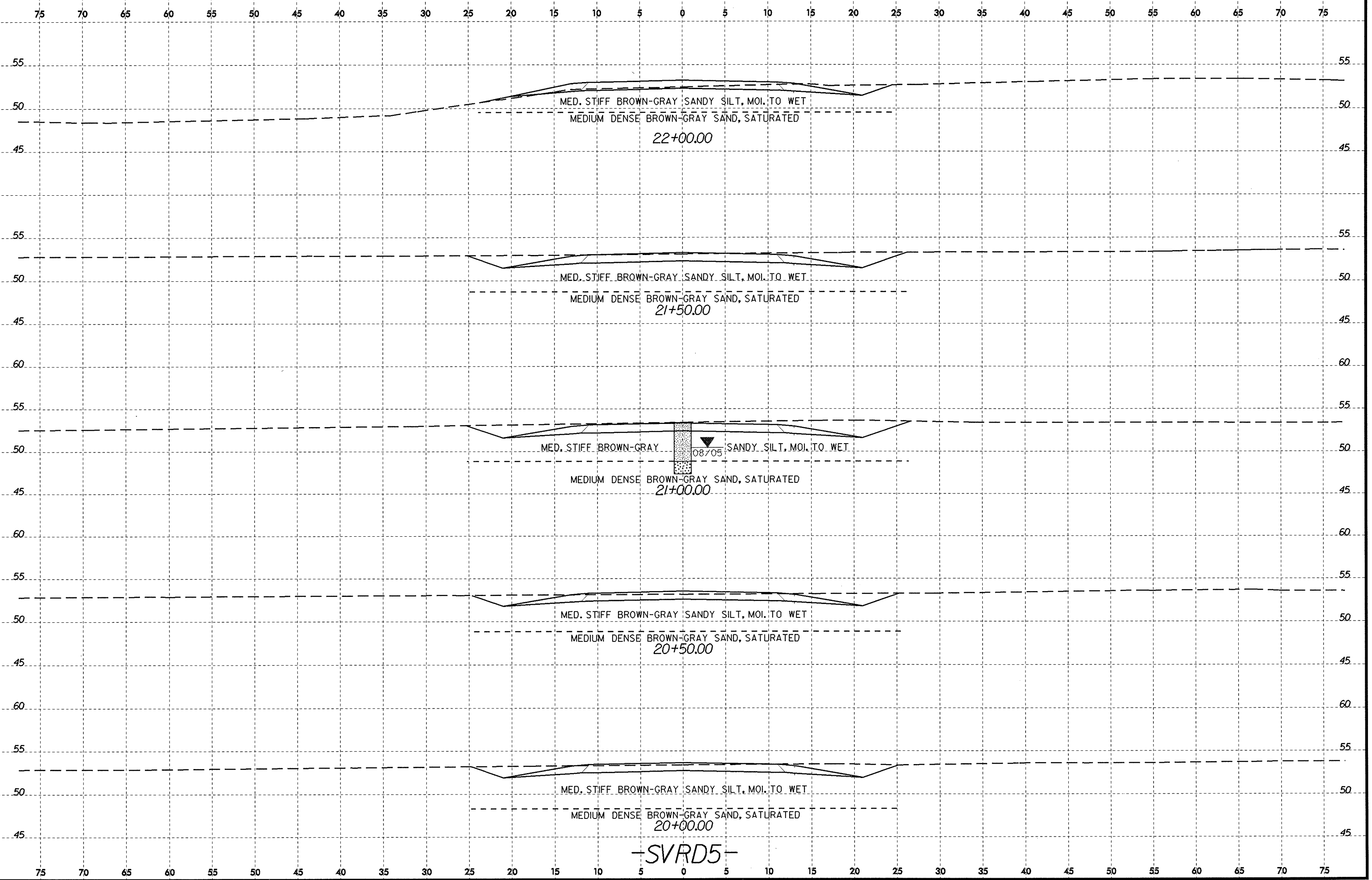
MED. STIFF BROWN-GRAY SILTY SANDY CLAY AND SANDY SILT, MOI. TO WET

18+00.00

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kbriller

8/23/99
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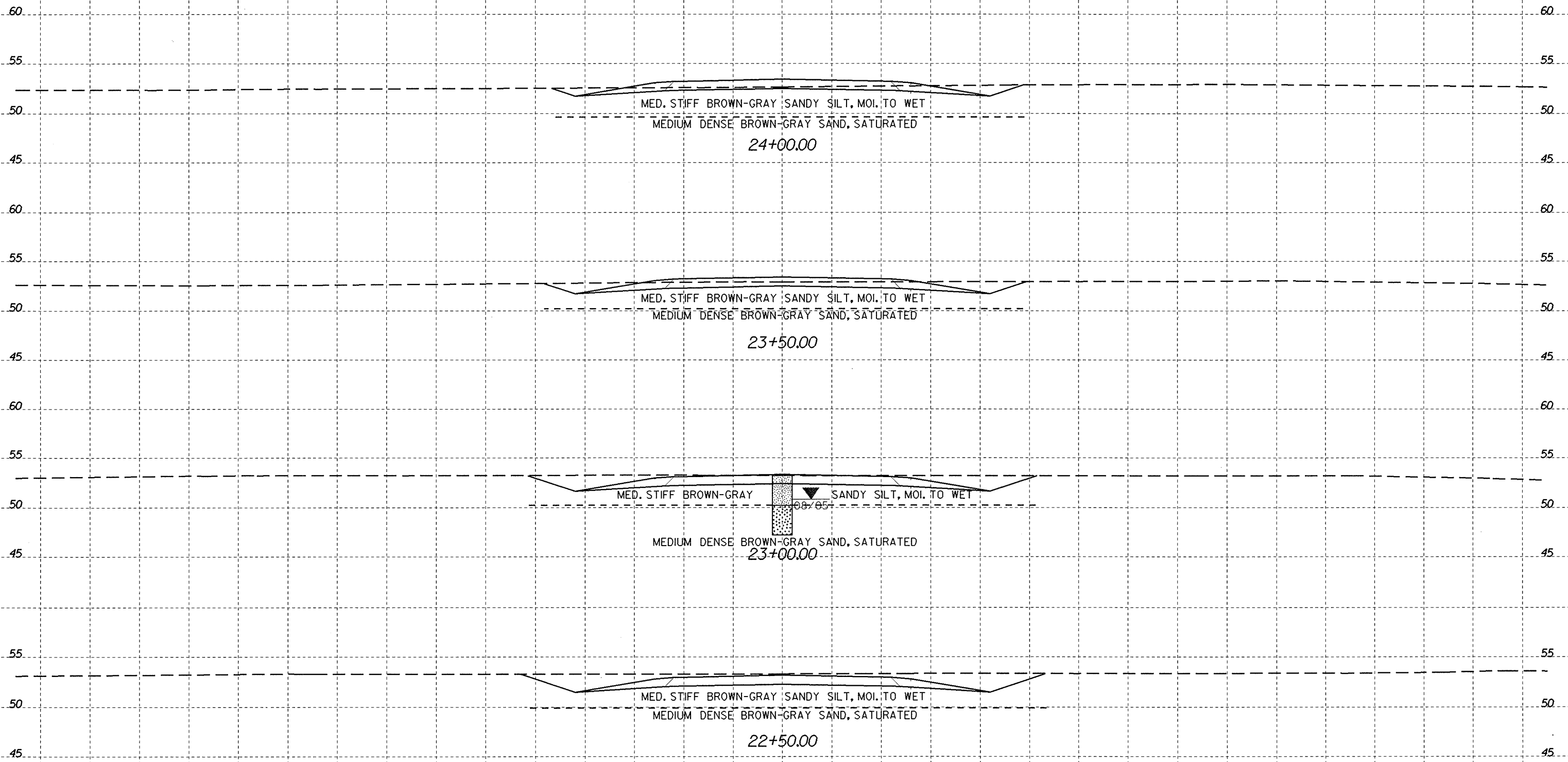
8/23/99



PROJ. REFERENCE NO.
R-2719A

SHEET NO.
134

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MED. STIFF BROWN-GRAY SANDY SILT, MOI. TO WET
MEDIUM DENSE BROWN-GRAY SAND, SATURATED
24+00.00

MED. STIFF BROWN-GRAY SANDY SILT, MOI. TO WET
MEDIUM DENSE BROWN-GRAY SAND, SATURATED
23+50.00

MED. STIFF BROWN-GRAY SANDY SILT, MOI. TO WET
MEDIUM DENSE BROWN-GRAY SAND, SATURATED
23+00.00

MED. STIFF BROWN-GRAY SANDY SILT, MOI. TO WET
MEDIUM DENSE BROWN-GRAY SAND, SATURATED
22+50.00

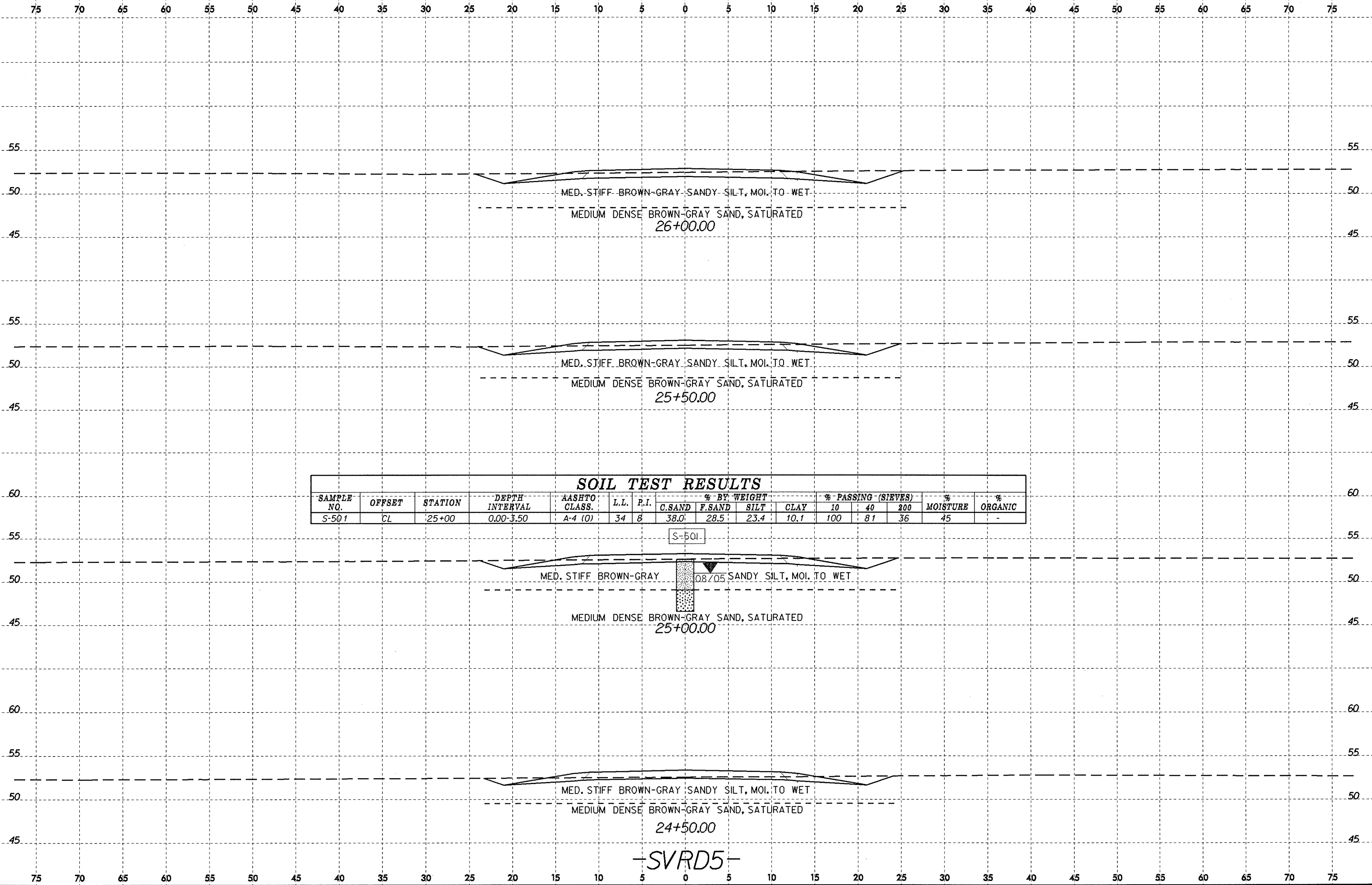
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KMiller AT 06/22/08

8/23/99



PROJ. REFERENCE NO. R-2719A SHEET NO. 135



SOIL TEST RESULTS

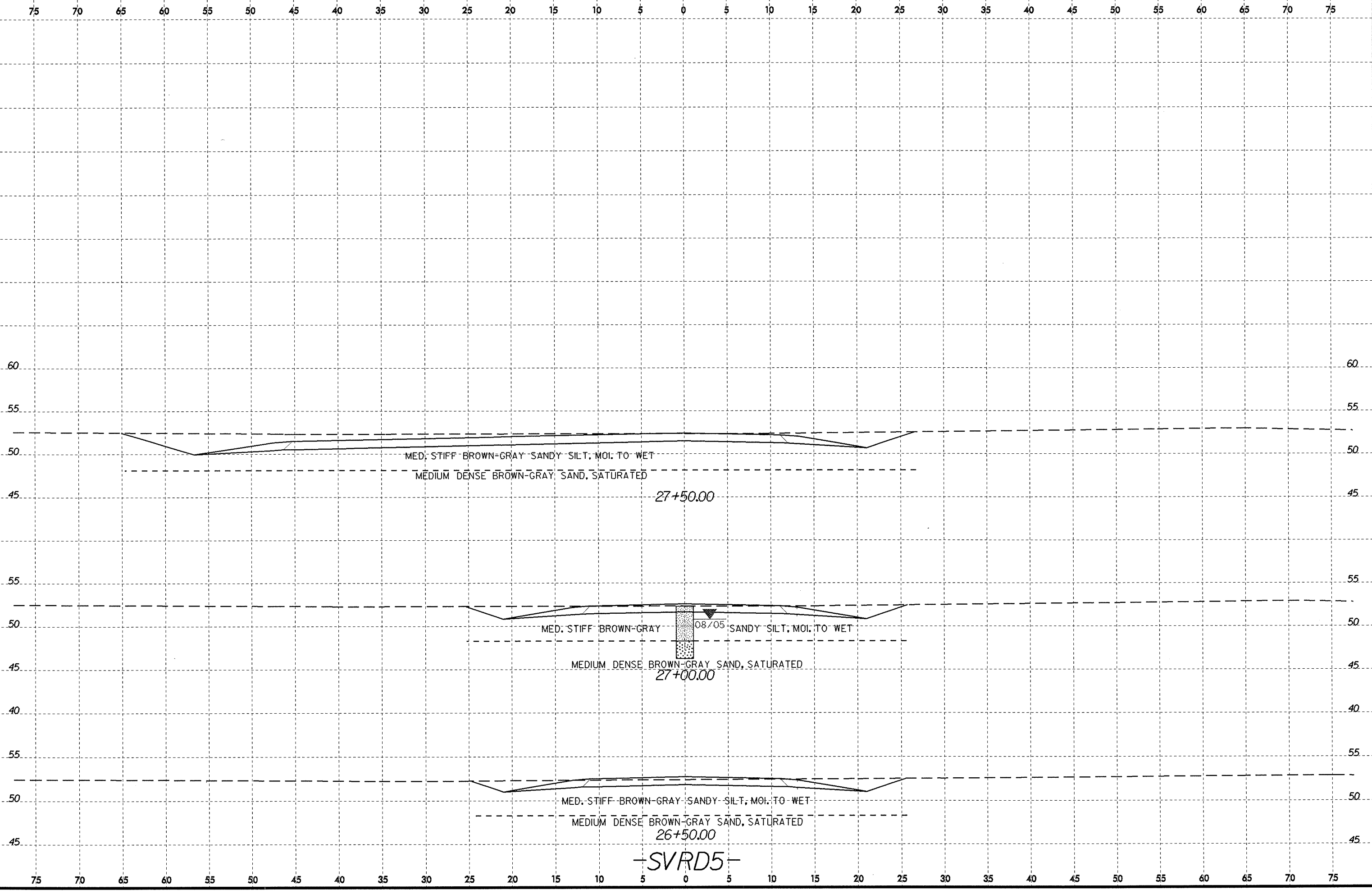
SAMPLE NO.	OFFSET	STATION	DEPTH INTERVAL	AASHTO CLASS.	L.L.	P.I.	% BY WEIGHT				% PASSING (SIEVES)			% MOISTURE	% ORGANIC
							C.SAND	F.SAND	SILT	CLAY	10	40	200		
S-501	CL	25+00	0.00-3.50	A-4 (0)	34	8	38.0	28.5	23.4	10.1	100	81	36	45	-

S-501

-SVRD5-

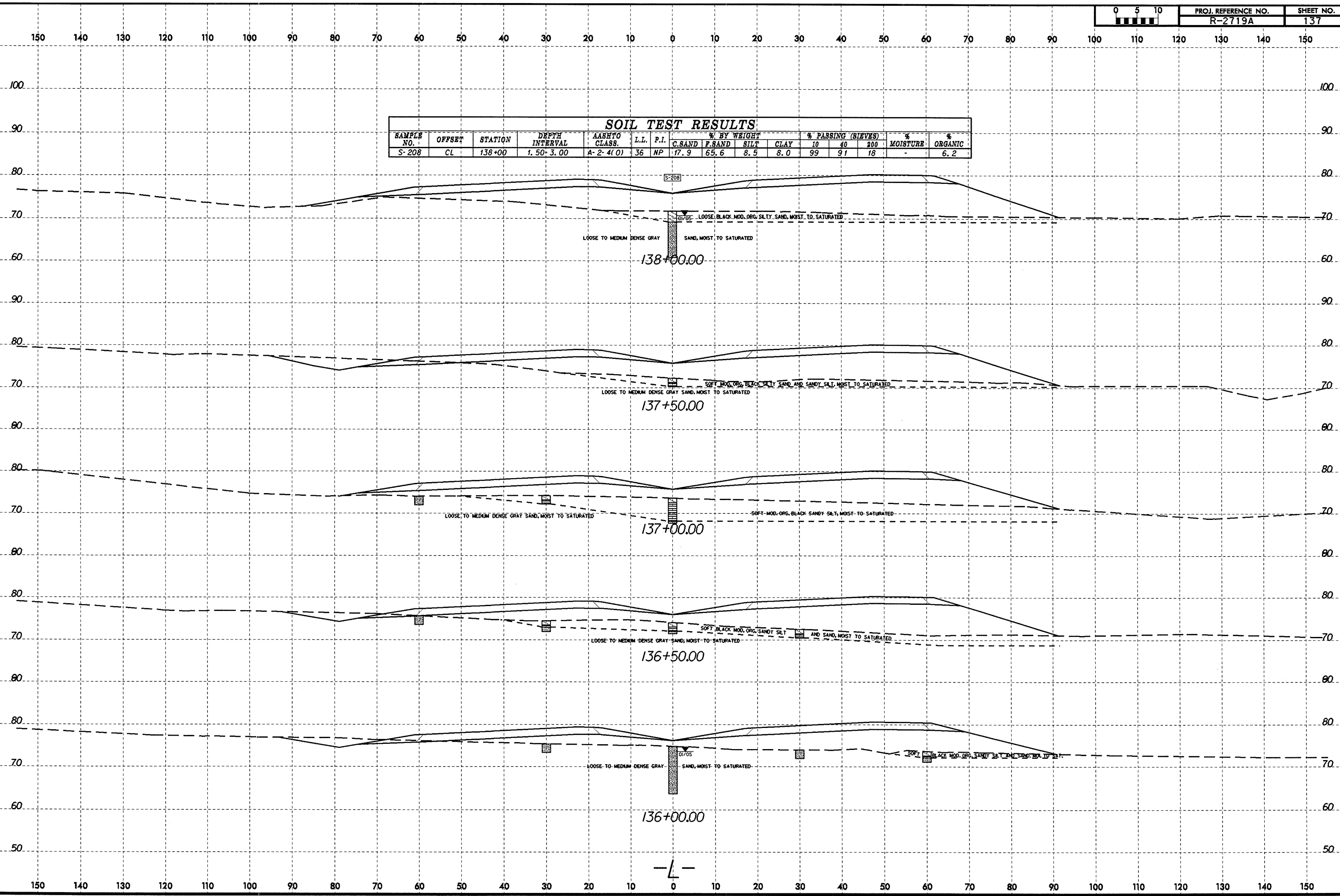
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8/23/99
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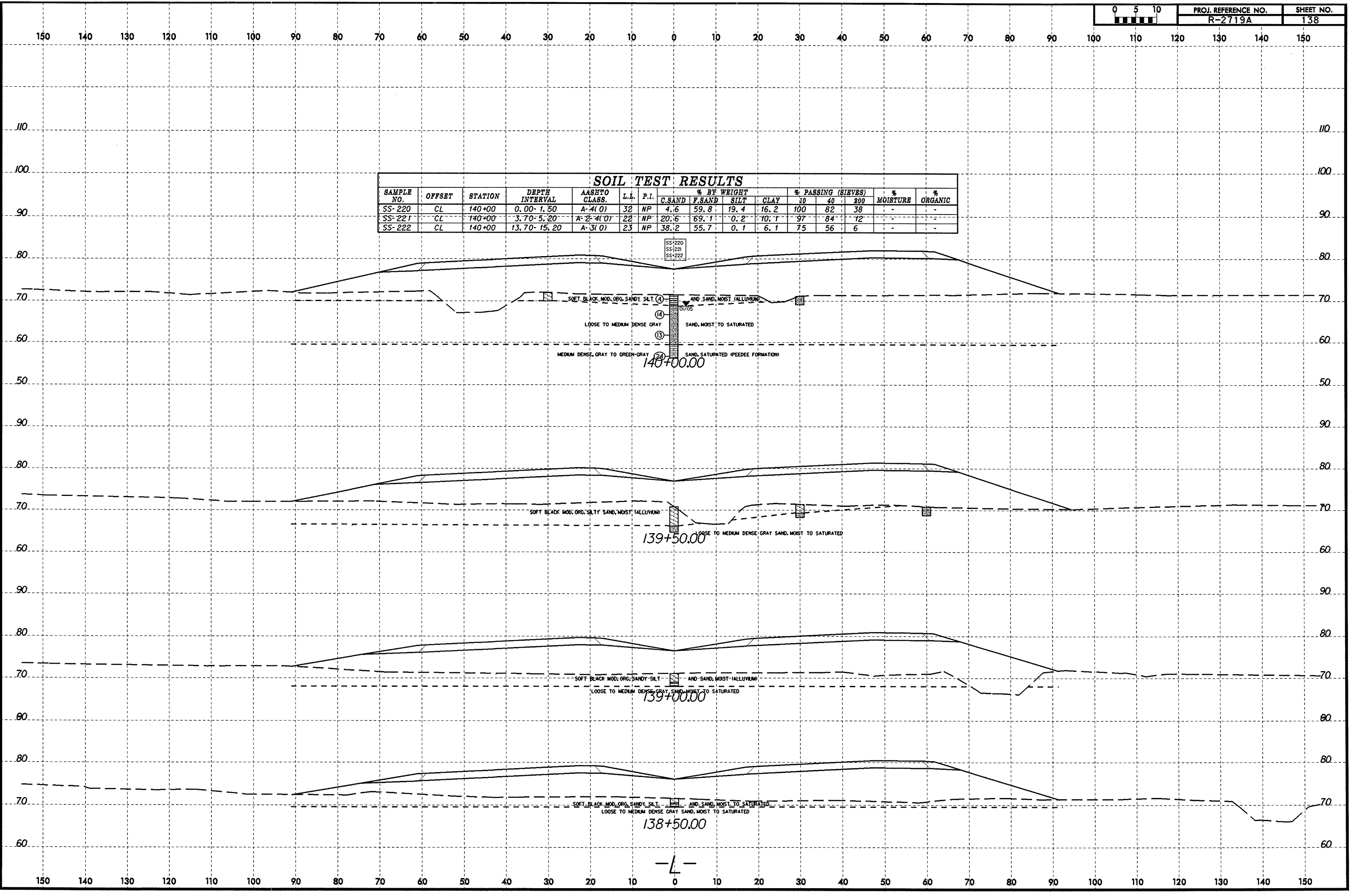


—SVRD5—

SOIL TEST RESULTS															
SAMPLE NO.	OFFSET	STATION	DEPTH INTERVAL	AASHTO CLASS.	L.L.	P.I.	% BY WEIGHT				% PASSING (SIEVES)			% MOISTURE	% ORGANIC
							C.SAND	F.SAND	SILT	CLAY	10	40	200		
S-208	CL	138+00	1.50-3.00	A-2-4(O)	36	NP	17.9	65.6	8.5	8.0	99	91	18	-	6.2



SOIL TEST RESULTS															
SAMPLE NO.	OFFSET	STATION	DEPTH INTERVAL	AASHTO CLASS.	L.L.	P.I.	% BY WEIGHT				% PASSING (SIEVES)			% MOISTURE	% ORGANIC
							C.SAND	F.SAND	SILT	CLAY	10	40	200		
SS-220	CL	140+00	0.00-1.50	A-4(0)	32	NP	4.6	59.8	19.4	16.2	100	82	38	-	-
SS-221	CL	140+00	3.70-5.20	A-2-4(0)	22	NP	20.6	69.7	0.2	70.1	97	84	12	-	-
SS-222	CL	140+00	13.70-15.20	A-3(0)	23	NP	38.2	55.7	0.1	6.1	75	56	6	-	-



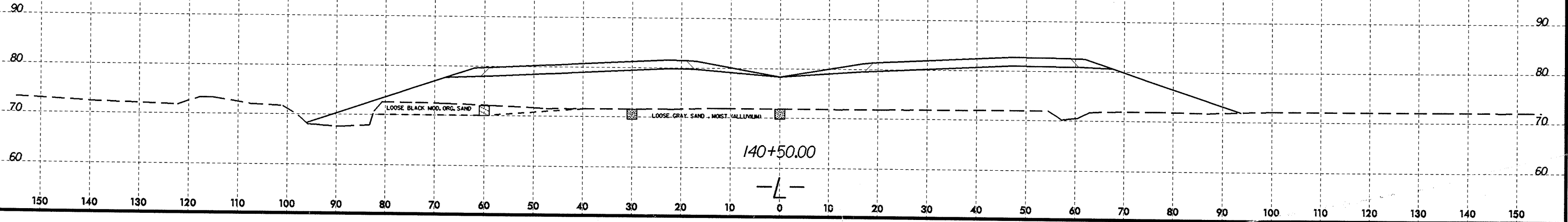
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8/23/99



PROJ. REFERENCE NO. R-2719A	SHEET NO. 139
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09-DEC-2005 14:51
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