

NORTH CAROLINA DEPARTMENT OF TRANSPORTATION
DIVISION OF HIGHWAYS
GEOTECHNICAL ENGINEERING UNIT



PROJECT REFERENCE NO. 34461.13 (R-2554BA) SHEET NO. 2

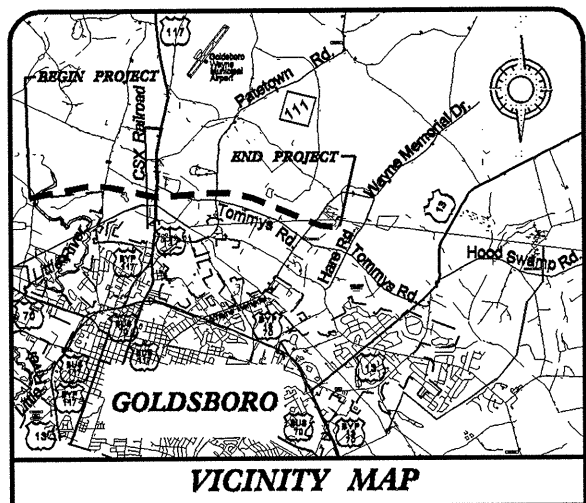
SUBSURFACE INVESTIGATION

SOIL AND ROCK LEGEND, TERMS, SYMBOLS, AND ABBREVIATIONS

SOIL DESCRIPTION				GRADATION				ROCK DESCRIPTION				TERMS AND DEFINITIONS											
SOIL IS CONSIDERED TO BE THE UNCONSOLIDATED, SEMI-CONSOLIDATED, OR WEATHERED EARTH MATERIALS THAT CAN BE PENETRATED WITH A CONTINUOUS FLIGHT POWER AUGER, AND YIELD LESS THAN 100 BLOWS PER 30 CM ACCORDING TO STANDARD PENETRATION TEST (ASTM D-1586). SOIL CLASSIFICATION IS BASED ON THE AASHTO SYSTEM. BASIC DESCRIPTIONS GENERALLY SHALL INCLUDE: CONSISTENCY, COLOR, TEXTURE, MOISTURE, AASHTO CLASSIFICATION, AND OTHER PERTINENT FACTORS SUCH AS MINERALOGICAL COMPOSITION, ANGULARITY, STRUCTURE, PLASTICITY, ETC. EXAMPLE: <i>VERY STIFF, GRAY, SILTY CLAY, MOST WITH INTERBEDDED FINE SAND LAYERS, HIGHLY PLASTIC, A-7-6</i>				WELL GRADED - INDICATES A GOOD REPRESENTATION OF PARTICLE SIZES FROM FINE TO COARSE. UNIFORM - INDICATES THAT SOIL PARTICLES ARE ALL APPROXIMATELY THE SAME SIZE. (ALSO POORLY GRADED) GAP-GRADED - INDICATES A MIXTURE OF UNIFORM PARTICLES OF TWO OR MORE SIZES. ANGULARITY OF GRAINS THE ANGULARITY OR ROUNDNESS OF SOIL GRAINS IS DESIGNATED BY THE TERMS: ANGULAR, SUBANGULAR, SUBROUNDED, OR ROUNDED.				HARD ROCK IS NON-COASTAL PLAIN MATERIAL THAT IF TESTED, WOULD YIELD SPT REFUSAL. AN INFERRED ROCK LINE INDICATES THE LEVEL AT WHICH NON-COASTAL PLAIN MATERIAL WOULD YIELD SPT REFUSAL. SPT REFUSAL IS PENETRATION BY A SPLIT SPOON SAMPLER EQUAL TO OR LESS THAN 3 CM PER 60 BLOWS. IN NON-COASTAL PLAIN MATERIAL, THE TRANSITION BETWEEN SOIL AND ROCK IS OFTEN REPRESENTED BY A ZONE OF WEATHERED ROCK. ROCK MATERIALS ARE TYPICALLY DIVIDED AS FOLLOWS: WEATHERED ROCK (WR) CRYSTALLINE ROCK (CR) NON-CRYSTALLINE ROCK (NCR) COASTAL PLAIN SEDIMENTARY ROCK (CP)				ALLUVIUM (ALLUV.) - SOILS THAT HAVE BEEN TRANSPORTED BY WATER. AQUIFER - A WATER BEARING FORMATION OR STRATA. ARENACEOUS - APPLIED TO ROCKS THAT HAVE BEEN DERIVED FROM SAND OR THAT CONTAIN SAND. ARGILLACEOUS - APPLIED TO ALL ROCKS OR SUBSTANCES COMPOSED OF CLAY MINERALS, OR HAVING A NOTABLE PROPORTION OF CLAY IN THEIR COMPOSITION, AS SHALE, SLATE, ETC. ARTESIAN - GROUND WATER THAT IS UNDER SUFFICIENT PRESSURE TO RISE ABOVE THE LEVEL AT WHICH IT IS ENCOUNTERED, BUT WHICH DOES NOT NECESSARILY RISE TO OR ABOVE THE GROUND SURFACE. CALCAREOUS (CALC.) - SOILS THAT CONTAIN APPRECIABLE AMOUNTS OF CALCIUM CARBONATE. COLLUVIUM - ROCK FRAGMENTS MIXED WITH SOIL DEPOSITED BY GRAVITY ON SLOPE OR AT BOTTOM OF SLOPE. CORE RECOVERY (REC.) - TOTAL LENGTH OF ALL MATERIAL RECOVERED IN THE CORE BARREL DIVIDED BY TOTAL LENGTH OF CORE RUN AND EXPRESSED AS A PERCENTAGE. DIKE - A TABULAR BODY OF IGNEOUS ROCK THAT CUTS ACROSS THE STRUCTURE OF ADJACENT ROCKS OR CUTS MASSIVE ROCK. DIP - THE ANGLE AT WHICH A STRATUM OR ANY PLANAR FEATURE IS INCLINED FROM THE HORIZONTAL. DIP DIRECTION (DIP AZIMUTH) - THE DIRECTION OR BEARING OF THE HORIZONTAL TRACE OF THE LINE OF DIP, MEASURED CLOCKWISE FROM NORTH. FAULT - A FRACTURE OR FRACTURE ZONE ALONG WHICH THERE HAS BEEN DISPLACEMENT OF THE SIDES RELATIVE TO ONE ANOTHER PARALLEL TO THE FRACTURE. FISSILE - A PROPERTY OF SPLITTING ALONG CLOSELY SPACED PARALLEL PLANES. FLOAT - ROCK FRAGMENTS ON SURFACE NEAR THEIR ORIGINAL POSITION AND DISLODGED FROM PARENT MATERIAL. FLOOD PLAIN (FP) - LAND BORDERING A STREAM, BUILT OF SEDIMENTS DEPOSITED BY THE STREAM. FORMATION (FM) - A MAPPABLE GEOLOGIC UNIT THAT CAN BE RECOGNIZED AND TRACED IN THE FIELD. JOINT - FRACTURE IN ROCK ALONG WHICH NO APPRECIABLE MOVEMENT HAS OCCURRED. LEDGE - A SHELF-LIKE RIDGE OR PROJECTION OF ROCK WHOSE THICKNESS IS SMALL COMPARED TO ITS LATERAL EXTENT. LENS - A BODY OF SOIL OR ROCK THAT THINS OUT IN ONE OR MORE DIRECTIONS. MOTTLED (MOT.) - IRREGULARLY MARKED WITH SPOTS OF DIFFERENT COLORS. MOTTLING IN SOILS USUALLY INDICATES POOR AERATION AND LACK OF GOOD DRAINAGE. PERCHED WATER - WATER MAINTAINED ABOVE THE NORMAL GROUND WATER LEVEL BY THE PRESENCE OF AN INTERVENING IMPERVIOUS STRATUM. RESIDUAL (RES.) SOIL - SOIL FORMED IN PLACE BY THE WEATHERING OF ROCK. ROCK QUALITY DESIGNATION (RQD) - A MEASURE OF ROCK QUALITY DESCRIBED BY TOTAL LENGTH OF ROCK SEGMENTS EQUAL TO OR GREATER THAN 10 CM DIVIDED BY THE TOTAL LENGTH OF CORE RUN AND EXPRESSED AS A PERCENTAGE. SAPROLITE (SAP.) - RESIDUAL SOIL THAT RETAINS THE RELIC STRUCTURE OR FABRIC OF THE PARENT ROCK. SILL - AN INTRUSIVE BODY OF IGNEOUS ROCK OF APPROXIMATELY UNIFORM THICKNESS AND RELATIVELY THIN COMPARED WITH ITS LATERAL EXTENT, THAT HAS BEEN EMPLACED PARALLEL TO THE BEDDING OR SCHISTOSITY OF THE INTRODUCED ROCKS. SLICKENSIDE - POLISHED AND STRIATED SURFACE THAT RESULTS FROM FRICTION ALONG A FAULT OR SLIP PLANE. STANDARD PENETRATION TEST (PENETRATION RESISTANCE) (SPT) - NUMBER OF BLOWS IN OF A 63.5 KG HAMMER FALLING 0.76 M REQUIRED TO PRODUCE A PENETRATION OF 30 CM INTO SOIL WITH A 5 CM OUTSIDE DIAMETER SPLIT SPOON SAMPLER. SPT REFUSAL IS PENETRATION EQUAL TO OR LESS THAN 3 CM PER 60 BLOWS. STRATA CORE RECOVERY (SCREC) - TOTAL LENGTH OF STRATA MATERIAL RECOVERED DIVIDED BY TOTAL LENGTH OF STRATUM AND EXPRESSED AS A PERCENTAGE. STRATA ROCK QUALITY DESIGNATION (SRQD) - A MEASURE OF ROCK QUALITY DESCRIBED BY TOTAL LENGTH OF ROCK SEGMENTS WITHIN A STRATUM EQUAL TO OR GREATER THAN 10 CM DIVIDED BY THE TOTAL LENGTH OF STRATA AND EXPRESSED AS A PERCENTAGE. TOPSOIL (TS) - SURFACE SOILS USUALLY CONTAINING ORGANIC MATTER.											
SOIL LEGEND AND AASHTO CLASSIFICATION				MINERALOGICAL COMPOSITION				WEATHERING															
GENERAL CLASS. GRANULAR MATERIALS (<= 35% PASSING #200) SILT-CLAY MATERIALS (> 35% PASSING #200) ORGANIC MATERIALS				MINERAL NAMES SUCH AS QUARTZ, FELDSPAR, MICA, TALC, KAOLIN, ETC. ARE USED IN DESCRIPTIONS WHENEVER THEY ARE CONSIDERED OF SIGNIFICANCE.				FRESH VERY SLIGHT (V SLI.) SLIGHT (SLI.) MODERATE (MOD.) MODERATELY SEVERE (MOD. SEV.) SEVERE (SEV.) VERY SEVERE (V SEV.) COMPLETE															
GROUP CLASS. A-1, A-2, A-3, A-4, A-5, A-6, A-7, A-8, A-9, A-10, A-11, A-12, A-13, A-14, A-15, A-16, A-17, A-18, A-19, A-20, A-21, A-22, A-23, A-24, A-25, A-26, A-27, A-28, A-29, A-30, A-31, A-32, A-33, A-34, A-35, A-36, A-37, A-38, A-39, A-40, A-41, A-42, A-43, A-44, A-45, A-46, A-47, A-48, A-49, A-50, A-51, A-52, A-53, A-54, A-55, A-56, A-57, A-58, A-59, A-60, A-61, A-62, A-63, A-64, A-65, A-66, A-67, A-68, A-69, A-70, A-71, A-72, A-73, A-74, A-75, A-76, A-77, A-78, A-79, A-80, A-81, A-82, A-83, A-84, A-85, A-86, A-87, A-88, A-89, A-90, A-91, A-92, A-93, A-94, A-95, A-96, A-97, A-98, A-99, A-100				COMPRESSIBILITY SLIGHTLY COMPRESSIBLE MODERATELY COMPRESSIBLE HIGHLY COMPRESSIBLE				ROCK FRESH, CRYSTALS BRIGHT, FEW JOINTS MAY SHOW SLIGHT STAINING. ROCK RINGS UNDER HAMMER IF CRYSTALLINE. ROCK GENERALLY FRESH, JOINTS STAINED, SOME JOINTS MAY SHOW THIN CLAY COATINGS IF OPEN. CRYSTALS ON A BROKEN SPECIMEN FACE SHINE BRIGHTLY. ROCK RINGS UNDER HAMMER BLOWS IF OF A CRYSTALLINE NATURE. ROCK GENERALLY FRESH, JOINTS STAINED AND DISCOLORATION EXTENDS INTO ROCK UP TO 1 INCH. OPEN JOINTS MAY CONTAIN CLAY. IN GRANITOID ROCKS SOME OCCASIONAL FELDSPAR CRYSTALS ARE DULL AND DISCOLORED. CRYSTALLINE ROCKS RING UNDER HAMMER BLOWS. SIGNIFICANT PORTIONS OF ROCK SHOW DISCOLORATION AND WEATHERING EFFECTS. IN GRANITOID ROCKS, MOST FELDSPARS ARE DULL AND DISCOLORED, SOME SHOW CLAY. ROCK HAS DULL SOUND UNDER HAMMER BLOWS AND SHOWS SIGNIFICANT LOSS OF STRENGTH AS COMPARED WITH FRESH ROCK. ALL ROCK EXCEPT QUARTZ DISCOLORED OR STAINED. IN GRANITOID ROCKS, ALL FELDSPARS DULL AND DISCOLORED AND A MAJORITY SHOW KAOLINIZATION. ROCK SHOWS SEVERE LOSS OF STRENGTH AND CAN BE EXCAVATED WITH A GEOLOGIST'S PICK. ROCK GIVES "CLUNK" SOUND WHEN STRUCK. IF TESTED, WOULD YIELD SPT REFUSAL. ALL ROCK EXCEPT QUARTZ DISCOLORED OR STAINED. ROCK FABRIC CLEAR AND EVIDENT BUT REDUCED IN STRENGTH TO STRONG SOIL. IN GRANITOID ROCKS ALL FELDSPARS ARE KAOLINIZED TO SOME EXTENT. SOME FRAGMENTS OF STRONG ROCK USUALLY REMAIN. IF TESTED, YIELDS SPT N VALUES > 100 BLOWS PER 30 CM. ALL ROCK EXCEPT QUARTZ DISCOLORED OR STAINED. ROCK FABRIC ELEMENTS ARE DISCERNIBLE BUT THE MASS IS EFFECTIVELY REDUCED TO SOIL STATUS, WITH ONLY FRAGMENTS OF STRONG ROCK REMAINING. SAPROLITE IS AN EXAMPLE OF ROCK WEATHERED TO A DEGREE SUCH THAT ONLY MINOR VESTIGES OF THE ORIGINAL ROCK FABRIC REMAIN. IF TESTED, YIELDS SPT N VALUES < 100 BLOWS PER 30 CM. ROCK REDUCED TO SOIL. ROCK FABRIC NOT DISCERNIBLE, OR DISCERNIBLE ONLY IN SMALL AND SCATTERED CONCENTRATIONS. QUARTZ MAY BE PRESENT AS DIKES OR STRINGERS. SAPROLITE IS ALSO AN EXAMPLE.				PERCENTAGE OF MATERIAL ORGANIC MATERIAL GRANULAR SOILS SILT-CLAY SOILS OTHER MATERIAL TRACE OF ORGANIC MATTER 2 - 3% 3 - 5% TRACE 1 - 10% LITTLE ORGANIC MATTER 3 - 5% 5 - 12% LITTLE 10 - 20% MODERATELY ORGANIC 5 - 10% 12 - 20% SOME 20 - 35% HIGHLY ORGANIC >10% >20% HIGHLY 35% AND ABOVE				WEATHERING ROCK FRESH, CRYSTALS BRIGHT, FEW JOINTS MAY SHOW SLIGHT STAINING. ROCK RINGS UNDER HAMMER IF CRYSTALLINE. ROCK GENERALLY FRESH, JOINTS STAINED, SOME JOINTS MAY SHOW THIN CLAY COATINGS IF OPEN. CRYSTALS ON A BROKEN SPECIMEN FACE SHINE BRIGHTLY. ROCK RINGS UNDER HAMMER BLOWS IF OF A CRYSTALLINE NATURE. ROCK GENERALLY FRESH, JOINTS STAINED AND DISCOLORATION EXTENDS INTO ROCK UP TO 1 INCH. OPEN JOINTS MAY CONTAIN CLAY. IN GRANITOID ROCKS SOME OCCASIONAL FELDSPAR CRYSTALS ARE DULL AND DISCOLORED. CRYSTALLINE ROCKS RING UNDER HAMMER BLOWS. SIGNIFICANT PORTIONS OF ROCK SHOW DISCOLORATION AND WEATHERING EFFECTS. IN GRANITOID ROCKS, MOST FELDSPARS ARE DULL AND DISCOLORED, SOME SHOW CLAY. ROCK HAS DULL SOUND UNDER HAMMER BLOWS AND SHOWS SIGNIFICANT LOSS OF STRENGTH AS COMPARED WITH FRESH ROCK. ALL ROCK EXCEPT QUARTZ DISCOLORED OR STAINED. IN GRANITOID ROCKS, ALL FELDSPARS DULL AND DISCOLORED AND A MAJORITY SHOW KAOLINIZATION. ROCK SHOWS SEVERE LOSS OF STRENGTH AND CAN BE EXCAVATED WITH A GEOLOGIST'S PICK. ROCK GIVES "CLUNK" SOUND WHEN STRUCK. IF TESTED, WOULD YIELD SPT REFUSAL. ALL ROCK EXCEPT QUARTZ DISCOLORED OR STAINED. ROCK FABRIC CLEAR AND EVIDENT BUT REDUCED IN STRENGTH TO STRONG SOIL. IN GRANITOID ROCKS ALL FELDSPARS ARE KAOLINIZED TO SOME EXTENT. SOME FRAGMENTS OF STRONG ROCK USUALLY REMAIN. IF TESTED, YIELDS SPT N VALUES > 100 BLOWS PER 30 CM. ALL ROCK EXCEPT QUARTZ DISCOLORED OR STAINED. ROCK FABRIC ELEMENTS ARE DISCERNIBLE BUT THE MASS IS EFFECTIVELY REDUCED TO SOIL STATUS, WITH ONLY FRAGMENTS OF STRONG ROCK REMAINING. SAPROLITE IS AN EXAMPLE OF ROCK WEATHERED TO A DEGREE SUCH THAT ONLY MINOR VESTIGES OF THE ORIGINAL ROCK FABRIC REMAIN. IF TESTED, YIELDS SPT N VALUES < 100 BLOWS PER 30 CM. ROCK REDUCED TO SOIL. ROCK FABRIC NOT DISCERNIBLE, OR DISCERNIBLE ONLY IN SMALL AND SCATTERED CONCENTRATIONS. QUARTZ MAY BE PRESENT AS DIKES OR STRINGERS. SAPROLITE IS ALSO AN EXAMPLE.							
PI OF A-7-5 SUBGROUP IS <= LL - 30 ; PI OF A-7-6 SUBGROUP IS > LL - 30				GROUND WATER WATER LEVEL IN BORE HOLE IMMEDIATELY AFTER DRILLING STATIC WATER LEVEL AFTER 24 HOURS PERCHED WATER, SATURATED ZONE, OR WATER BEARING STRATA SPRING OR SEEP																			
CONSISTENCY OR DENSENESS				MISCELLANEOUS SYMBOLS																			
PRIMARY SOIL TYPE COMPACTNESS OR CONSISTENCY RANGE OF STANDARD PENETRATION RESISTANCE (N-VALUE) RANGE OF UNCONFINED COMPRESSIVE STRENGTH (KN/M ²)				ROADWAY EMBANKMENT (RE) WITH SOIL DESCRIPTION SOIL SYMBOL ARTIFICIAL FILL (AF) OTHER THAN ROADWAY EMBANKMENT INFERRED SOIL BOUNDARY INFERRED ROCK LINE ALLUVIAL SOIL BOUNDARY DIP & DIP DIRECTION OF ROCK STRUCTURES SOUNDING ROD				SPT TEST BORING AUGER BORING CORE BORING MONITORING WELL PIEZOMETER INSTALLATION SLOPE INDICATOR INSTALLATION SPT N-VALUE SPT REFUSAL				SAMPLE DESIGNATIONS S - BULK SAMPLE SS - SPLIT SPOON SAMPLE ST - SHELBY TUBE SAMPLE RS - ROCK SAMPLE RT - RECOMPACTED TRIAXIAL SAMPLE CBR - CALIFORNIA BEARING RATIO SAMPLE											
GENERAL GRANULAR MATERIAL (NON-COHESIVE) GENERAL SILT-CLAY MATERIAL (COHESIVE)				ROADWAY EMBANKMENT (RE) WITH SOIL DESCRIPTION SOIL SYMBOL ARTIFICIAL FILL (AF) OTHER THAN ROADWAY EMBANKMENT INFERRED SOIL BOUNDARY INFERRED ROCK LINE ALLUVIAL SOIL BOUNDARY DIP & DIP DIRECTION OF ROCK STRUCTURES SOUNDING ROD				SPT TEST BORING AUGER BORING CORE BORING MONITORING WELL PIEZOMETER INSTALLATION SLOPE INDICATOR INSTALLATION SPT N-VALUE SPT REFUSAL				SAMPLE DESIGNATIONS S - BULK SAMPLE SS - SPLIT SPOON SAMPLE ST - SHELBY TUBE SAMPLE RS - ROCK SAMPLE RT - RECOMPACTED TRIAXIAL SAMPLE CBR - CALIFORNIA BEARING RATIO SAMPLE											
TEXTURE OR GRAIN SIZE				ABBREVIATIONS				ROCK HARDNESS															
U.S. STD. SIEVE SIZE OPENING (MM) 4 10 40 60 200 270 4.75 2.00 0.42 0.25 0.075 0.053				AR - AUGER REFUSAL BT - BORING TERMINATED CL - CLAY CPT - CONE PENETRATION TEST CSE - COARSE DMT - DILATOMETER TEST DPT - DYNAMIC PENETRATION TEST e - VOID RATIO F - FINE FOSS. - FOSSILIFEROUS FRAC. - FRACTURED, FRACTURES FRAGS. - FRAGMENTS				HI. - HIGHLY MED. - MEDIUM MICA. - MICACEOUS MOD. - MODERATELY NP - NON PLASTIC ORG. - ORGANIC PMT - PRESSUREMETER TEST SAP. - SAPROLITIC SD. - SAND, SANDY SL. - SILT, SILTY SLI. - SLIGHTLY TCR - TRICONE REFUSAL				VERY HARD HARD MODERATELY HARD MEDIUM HARD SOFT VERY SOFT				CANNOT BE SCRATCHED BY KNIFE OR SHARP PICK. BREAKING OF HAND SPECIMENS REQUIRES SEVERAL HARD BLOWS OF THE GEOLOGIST'S PICK. CAN BE SCRATCHED BY KNIFE OR PICK ONLY WITH DIFFICULTY. HARD HAMMER BLOWS REQUIRED TO DETACH HAND SPECIMEN. CAN BE SCRATCHED BY KNIFE OR PICK. GOUGES OR GROOVES TO 6 MM DEEP CAN BE EXCAVATED BY HARD BLOW OF A GEOLOGIST'S PICK. HAND SPECIMENS CAN BE DETACHED BY MODERATE BLOWS. CAN BE GROOVED OR GOUGED 13 MM DEEP BY FIRM PRESSURE OF KNIFE OR PICK POINT. CAN BE EXCAVATED IN SMALL CHIPS TO PEICES 25 MM MAXIMUM SIZE BY HARD BLOWS OF THE POINT OF A GEOLOGIST'S PICK. CAN BE GROOVED OR GOUGED READILY BY KNIFE OR PICK. CAN BE EXCAVATED IN FRAGMENTS FROM CHIPS TO SEVERAL INCHES IN SIZE BY MODERATE BLOWS OF A PICK POINT. SMALL, THIN PIECES CAN BE BROKEN BY FINGER PRESSURE. CAN BE CARVED WITH KNIFE. CAN BE EXCAVATED READILY WITH POINT OF PICK. PIECES 25 MM OR MORE IN THICKNESS CAN BE BROKEN BY FINGER PRESSURE. CAN BE SCRATCHED READILY BY FINGERNAIL.							
BOULDER (BLDR.) COBBLE (COB.) GRAVEL (GRAV.) COARSE SAND (CSE, SD.) FINE SAND (F SD.) SILT (SL) CLAY (CL)				HI. - HIGHLY MED. - MEDIUM MICA. - MICACEOUS MOD. - MODERATELY NP - NON PLASTIC ORG. - ORGANIC PMT - PRESSUREMETER TEST SAP. - SAPROLITIC SD. - SAND, SANDY SL. - SILT, SILTY SLI. - SLIGHTLY TCR - TRICONE REFUSAL				VERY HARD HARD MODERATELY HARD MEDIUM HARD SOFT VERY SOFT															
GRAIN SIZE MM 305 75 2.0 0.25 0.05 0.005 IN. 12 3				AR - AUGER REFUSAL BT - BORING TERMINATED CL - CLAY CPT - CONE PENETRATION TEST CSE - COARSE DMT - DILATOMETER TEST DPT - DYNAMIC PENETRATION TEST e - VOID RATIO F - FINE FOSS. - FOSSILIFEROUS FRAC. - FRACTURED, FRACTURES FRAGS. - FRAGMENTS				HI. - HIGHLY MED. - MEDIUM MICA. - MICACEOUS MOD. - MODERATELY NP - NON PLASTIC ORG. - ORGANIC PMT - PRESSUREMETER TEST SAP. - SAPROLITIC SD. - SAND, SANDY SL. - SILT, SILTY SLI. - SLIGHTLY TCR - TRICONE REFUSAL				VERY HARD HARD MODERATELY HARD MEDIUM HARD SOFT VERY SOFT											
SOIL MOISTURE - CORRELATION OF TERMS				EQUIPMENT USED ON SUBJECT PROJECT				FRACTURE SPACING				BEDDING											
SOIL MOISTURE SCALE (ATTERBERG LIMITS) FIELD MOISTURE DESCRIPTION GUIDE FOR FIELD MOISTURE DESCRIPTION				DRILL UNITS: MOBILE B- BK-51 CME-45C CME-550 PORTABLE HOIST CME-45B				TERM SPACING VERY WIDE MORE THAN 3 M WIDE 3 TO 10 M MODERATELY CLOSE 30 TO 100 CM CLOSE 5 TO 30 CM VERY CLOSE LESS THAN 5 CM				TERM THICKNESS VERY THICKLY BEDDED > 1 M THICKLY BEDDED 0.5 - 1 M THINLY BEDDED 0.05 - 0.5 M VERY THINLY BEDDED 10 - 50 MM THICKLY LAMINATED 2.5 - 10 MM THINLY LAMINATED < 2.5 MM				BENCH MARK: ELEVATION: M							
LL - LIQUID LIMIT PL - PLASTIC LIMIT OM - OPTIMUM MOISTURE SL - SHRINKAGE LIMIT				ADVANCING TOOLS: CLAY BITS 152mm CONTINUOUS FLIGHT AUGER 203mm HOLLOW AUGERS HARD FACED FINGER BITS TUNG-CARBIDE INSERTS CASING w/ ADVANCER TRICONE 75mm STEEL TEETH TRICONE mm TUNG-CARB. CORE BIT				HAMMER TYPE: AUTOMATIC MANUAL CORE SIZE: B N H HAND TOOLS: POST HOLE DIGGER HAND AUGER SOUNDING ROD VANE SHEAR TEST				INDURATION: RUBBING WITH FINGER FREES NUMEROUS GRAINS; GENTLE BLOW BY HAMMER DISINTEGRATES SAMPLE. GRAINS CAN BE SEPARATED FROM SAMPLE WITH STEEL PROBE; BREAKS EASILY WHEN HIT WITH HAMMER. GRAINS ARE DIFFICULT TO SEPARATE WITH STEEL PROBE; DIFFICULT TO BREAK WITH HAMMER. SHARP HAMMER BLOWS REQUIRED TO BREAK SAMPLE; SAMPLE BREAKS ACROSS GRAINS.				NOTES: Approximate Limits of organic soils Approximate Limits of flood plain soils							
PLASTICITY INDEX (PI) DRY STRENGTH VERY LOW SLIGHT MEDIUM HIGH				X CME-45B				FRIABLE MODERATELY INDURATED INDURATED EXTREMELY INDURATED															
NONPLASTIC LOW PLASTICITY MED. PLASTICITY HIGH PLASTICITY				X CME-45B				FRIABLE MODERATELY INDURATED INDURATED EXTREMELY INDURATED															
COLOR DESCRIPTIONS MAY INCLUDE COLOR OR COLOR COMBINATIONS (TAN, RED, YELLOW-BROWN, BLUE-GRAY). MODIFIERS SUCH AS LIGHT, DARK, STREAKED, ETC. ARE USED TO DESCRIBE APPEARANCE.				X CME-45B				FRIABLE MODERATELY INDURATED INDURATED EXTREMELY INDURATED															

CONTRACT: PROJECT: R-2554BA

See Sheet 1-A For Index of Sheets
See Sheet 1-B For Conventional Symbols



RW PLANS

STATE OF NORTH CAROLINA
DIVISION OF HIGHWAYS
WAYNE COUNTY

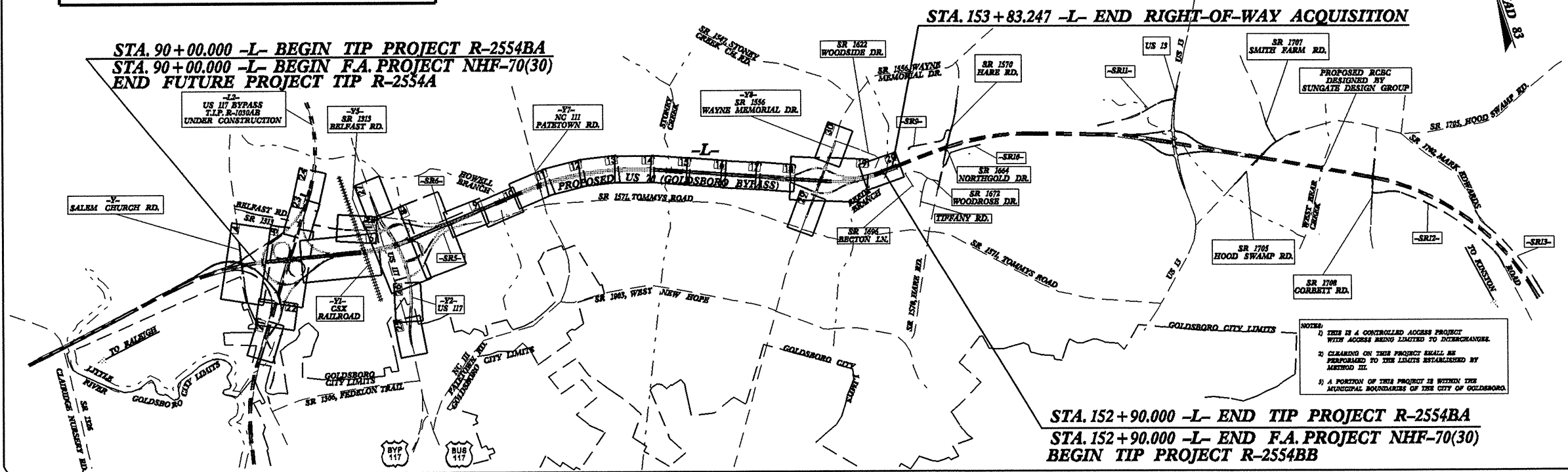
**LOCATION: US 70 (GOLDSBORO BYPASS)
FROM EAST OF SR 1300 (SALEM CHURCH ROAD)
TO EAST OF SR 1556 (WAYNE MEMORIAL DRIVE)**

**TYPE OF WORK: GRADING, PAVING, DRAINAGE,
STRUCTURES, CULVERTS, SIGNING, AND SIGNALS**

ALL DIMENSIONS IN THESE PLANS ARE IN METERS UNLESS OTHERWISE SHOWN

STATE	STATE PROJECT REFERENCE NO.	SHEET	TOTAL
N.C.	R-2554BA	2A	84
STATE PROGRAM	F.A. PROGRAM	DESCRIPTION	
34461.1.3	NHF-70(30)	PE	
34461.2.7		RW	

**PRELIMINARY PLANS
DO NOT USE FOR CONSTRUCTION**



**STA. 90+00.000 -L- BEGIN TIP PROJECT R-2554BA
STA. 90+00.000 -L- BEGIN F.A. PROJECT NHF-70(30)
END FUTURE PROJECT TIP R-2554A**

STA. 153+83.247 -L- END RIGHT-OF-WAY ACQUISITION

**STA. 152+90.000 -L- END TIP PROJECT R-2554BA
STA. 152+90.000 -L- END F.A. PROJECT NHF-70(30)
BEGIN TIP PROJECT R-2554BB**

NOTES:
1) THIS IS A CONTROLLED ACCESS PROJECT WITH ACCESS BEING LIMITED TO OVERCROSSINGS.
2) CLEARING ON THIS PROJECT SHALL BE PERFORMED TO THE LIMITS ESTABLISHED BY METHOD III.
3) A PORTION OF THIS PROJECT IS WITHIN THE MUNICIPAL BOUNDARIES OF THE CITY OF GOLDSBORO.

GRAPHIC SCALE

PLANS: 1" = 100'

PROFILE (HORIZONTAL): 1" = 100'

PROFILE (VERTICAL): 1" = 20'

DESIGN DATA

ADT 2008	=	20,728
ADT 2028	=	29,048
DHV	=	10 %
D	=	55 %
T	=	10 % *
V	=	110 km/h
* TTST	6% DUAL	4%

PROJECT LENGTH

Length Roadway TIP Project R-2554BA	5.833 km
*Length Structure TIP Project R-2554BA	0.457 km
TOTAL LENGTH TIP Project R-2554BA	6.290 km (3.91 MILES)

*USED EBL -L- STATIONS FOR STRUCTURE LENGTHS

PLANS PREPARED BY:
RUMMEL, KLEPPER & KAHL, LLP
consulting engineers
5800 FARRINGTON PLACE, SUITE 105
RALEIGH, NORTH CAROLINA 27609
(919) 878-9550

DIVISION OF HIGHWAYS
2002 STANDARD SPECIFICATIONS

RIGHT OF WAY DATE: MAY, 2005
LETTING DATE: MAY, 2007

B. KEITH SKINNER, P.E.
PROJECT ENGINEER

MICHAEL T. MERRITT, P.E.
PROJECT DESIGN ENGINEER

Scott Blevins, P.E.
Project Engineer-Project Services

NCDOT CONTACT: Project Engineer-Project Services

HYDRAULICS ENGINEER

SIGNATURE

ROADWAY DESIGN

SIGNATURE

**DIVISION OF HIGHWAYS
STATE OF NORTH CAROLINA**

SIGNATURE

STATE DESIGN ENGINEER

**DEPARTMENT OF TRANSPORTATION
FEDERAL HIGHWAY ADMINISTRATION**

APPROVED

DIVISION ADMINISTRATOR

DATE



STATE OF NORTH CAROLINA
DEPARTMENT OF TRANSPORTATION

MICHAEL F. EASLEY
GOVERNOR

LYNDO TIPPETT
SECRETARY

January 16, 2007

STATE PROJECT: 34461.1.3 R-2554BA
F.A. PROJECT NHF-70(30)
COUNTY: Wayne
DESCRIPTION: US 70 (Goldsboro Bypass) from East of SR 1300
(Salem Church Rd.) To East of SR 1556
(Wayne Memorial Drive)

SUBJECT: Geotechnical Report – Inventory

Project Description

The project consists of constructing a four lane divided facility along a new location. The project begins approximately 1 kilometer west of Salem Church Road and proceeds 6.3 kilometers in an easterly direction to a point east of SR 1556 (Wayne Memorial Drive). The western terminus ties in with future project R-2554A and the eastern terminus ties in with future project R-2554BB. The geotechnical investigation of subsurface conditions was confined to the corridor of proposed new construction.

The following survey lines were investigated for this project:

<u>Line</u>	<u>Station</u>
-L-	90+00 to 152+90
-Y1-	10+00 to 12+00
-Y2-	10+87 to 26+35
-Y2A-	10+20 to 11+51
-Y2B-	10+00 to 11+49
-Y5-	11+08 to 13+05
-Y7-	10+60 to 12+80
-Y8-	12+00 to 19+95
-L2RPA-	0+00 to 6+55
-L2RPB-	0+00 to 2+20
-L2RPC-	4+40 to 9+17
-L2RPD-	0+00 to 5+10

-L2RPBD-	0+00 to 17+94
-L2RPDB-	0+00 to 9+27
-L2LPB-	0+00 to 4+88
-L2LPD-	0+00 to 4+98
-Y2RPA-	10+00 to 16+65
-Y2RPD-	10+00 to 16+57
-Y2LPA-	10+00 to 14+16
-Y2LPD-	10+00 to 14+93
-Y8RPA-	10+00 to 14+71
-Y8RPB-	10+00 to 14+78
-Y8RPC-	10+00 to 14+40
-Y8RPD-	10+00 to 15+27
-SR5-	10+09 to 18+02
-SR6-	10+00 to 11+77
-SR6-	14+38 to 19+30
-Y8DET-	10+44 to 17+05

Areas of Special Geotechnical Interest

- 1) A high water table, seasonal high ground water or the potential for ground water related construction problems occur along the majority of the project.
- 2) The entire project contains cohesive soils except in the following areas. Typically, the cohesive soils exhibit medium to high plasticity indices, relatively high moisture contents, 50 percent or more passing the 75um sieve and or very soft to soft consistency

<u>Line</u>	<u>Station±</u>
-L-	129+20 to 131+20
-L-	133+40 to 142+80
-L-	147+30 to 152+20
-Y8-	12+00 to 13+70
-L2RPDB-	9+27 to 16+80
-Y8RPA-	10+00 to 14+71
-Y8RPC-	13+40 to 13+80
-Y8RPD-	10+00 to 12+10
-SR5-	13+40 to 15+30
-SR5-	16+30 to 18+02

- 3) The following sections contain recent alluvial soils typically consisting of very soft to soft cohesive silts and clays or very loose to loose sand:

<u>Line</u>	<u>Station (+)</u>
-L-	94+37 to 94+55
-L-	98+50 to 98+94
-L-	114+45 to 115+50
-L-	131+65 to 132+95
-L2RPBD-	13+85 to 14+36
-L2RPBD-	16+65 to 17+94
-L2LPD-	0+95 to 2+40

- 4) The following areas contain soft organic deposits that have the potential to cause embankment stability and/or long term settlement problems:

<u>Line</u>	<u>Station(±)</u>
-L-	141+11 to 141+89
-L-	152+43 to 152+90

- 5) Water wells located within or in close proximity to the proposed right-of-way were noted at the following locations:

<u>Line</u>	<u>Station (+)</u>
-L-	109+47, 53 m Rt.
-L-	111+50, 51 m Rt.
-L-	146+70, 16 m Rt.
-L-	151+40, 18 m Rt.
-L-	151+86, 15 m Lt.
-Y2-	14+50, 31 m Rt.
-Y2-	23+20, 55 m Rt.
-Y2RPA-	14+30, centerline
-Y5-	11+23, 35 m Rt.
-Y7_	10+15, 20 m Lt.
-Y8-	12+40, 19 m Rt.
-Y8-	13+22, 13 m Rt.
-Y8-	15+30, 12 m Rt.
-Y8-	15+42, 10 m Rt.
-Y8-	15+72, 12 m Rt.
-Y8-	16+39, 15 m Rt.
-Y8-	17+90, 65 m Rt.
-Y8-	18+15, 64 m Lt.
-Y8RPA-	13+60, 13 m Rt.
-Y8RPD-	14+70, 20 m Lt.
-SR5-	13+70, 15 m Rt.

Additional water wells not listed above may be present within the proposed right-of-way.

- 6) Farm ponds located within proposed right-of-way were noted at the following locations:

<u>Line</u>	<u>Station (+)</u>
-SR5-	15+90 to 16+18

Physiography and Geology

The project is located within the Middle Coastal Plain Physiographic Province. Topography is gently sloping to level and typical of the region. Elevations range from 29± to 40± meters.

The geology of the project basically consists of Pleistocene to Recent age fluvial and coastal plain sediments overlying marine to marginal marine sediments of the Pliocene age Yorktown Formation and deltaic sediments of the Upper Cretaceous age Black Creek Formation. The boundary between the Pleistocene to Recent coastal plain sediments and underlying Yorktown Formation sediments is generally difficult to determine due to a similar composition and degree of weathering. Therefore, no distinction is made between the two on the accompanying soil profile sheets. The Black Creek Formation was primarily encountered in deep borings along the project at elevations typically ranging between 21 and 28 meters. The project is drained by Howell Branch, Stoney Creek, and Reedy Branch, all within the Neuse River drainage basin.

Ground Water

Ground water data was collected primarily from late fall of 2002 to summer of 2003 as well as the spring of 2005 and 2006 during above average rainfall conditions. Typically, ground water levels were measured at depths of 0.5 to 2.0± meters below the natural ground surface in nearly level to level interstream areas and 2 to 3 meters or more below the natural ground surface in moderately to well drained upland areas. Ground water levels should fall 1± meters or more during dry summer conditions. Flood plain segments and very poorly drained upland areas typically exhibit water levels near or at the natural ground surface.

Soils

Soils occurring along the project are derived from marine and fluvial sediments deposited in the geologic past. Based on origin and occurrence, soils encountered during the investigation are separated into four major categories. The categories are surficial upland soils, coastal plain and Yorktown Formation soils, Black Creek Formation soils and flood plain soils.

Surficial upland soils include rootmat topsoil and generally the top 1 to 2 meters of the soil profile. Topsoil typically consists of 0.2 to 0.4 meters of very loose silty sand (A-2-4) or very soft to soft clayey fine sandy silt (A-4) with little or no organic content. Engineering properties of the topsoil range from fair to poor for the silt to good for the granular material. Rootmat along the project primarily occurs in wooded areas and is typically 0.1 to 0.2 meters thick. The rootmat is generally slightly organic and exhibits

poor engineering properties. Surficial soils underlying the topsoil typically consist of loose to medium dense fine to coarse sand (A-2-4), clayey sand (A-2-4, A-2-6), medium stiff to stiff silty fine sandy clay (A-6, A-7-6) or medium stiff clayey sandy silt (A-4). The granular material generally exhibits good to excellent engineering properties. However, the cohesive soils generally exhibit fair to poor engineering properties which include relatively high moisture contents, medium high plasticity indices and 50 percent or more passing the 75um sieve. These soils have the potential to cause subgrade stability problems.

Flood plain soils typically consist of very loose to loose silty sand (A-2-4) and very soft to soft clayey fine sandy silt (A-4) and sandy clay (A-6). Organic contents of the soils within the roadway portion of the project generally range from non-organic to slightly organic. Engineering properties of flood plain soils range from good for the granular material to poor for the very soft to soft saturated silts and organic deposits. Bridges are proposed for the major flood plain areas.

The Pleistocene and Yorktown Formations generally underlie the surficial soils at a depth of 1 to 2 meters. Soils within this formation typically consist of beds of very loose to medium dense sand or clayey sand (A-2-4, A-3, A-1-b, A-2-6) and very soft to medium stiff silty sandy clay (A-6, A-7-6) or clayey sandy silt (A-4). The granular soils typically exhibit good to excellent engineering properties. However, the cohesive soils generally have poor engineering properties due to a soft to very soft consistency, relatively high moisture contents, medium to high plasticity indices and 50 percent or more passing the 75 um sieve. These soils have the potential to cause subgrade stability problems or embankment stability/settlement problems.

The Black Creek Formation underlies the Yorktown Formation and the alluvial soils. Cohesive soils within this formation typically consist of beds of very stiff to hard silty and fine sandy clay (A-6, A-7-5, A-7-6) or clayey fine sandy silt (A-4). These soils exhibit fair to poor engineering properties. Granular soils within this formation typically consist of beds of medium dense to very dense fine to coarse sand (A-2-4, A-3, A-1-b). These soils exhibit good to excellent engineering properties. Due to its depth of occurrence, the Black Creek Formation should not affect the roadway portion of this project.

California Bearing Ratio (CBR) Samples

A bulk sample was taken at the following location and submitted for CBR testing:

<u>Sample No.</u>	<u>Station</u>	<u>Depth (m)</u>
CBR-1	-L- 78+10	0.3-3.3

Undisturbed Samples

Undisturbed (Shelby Tube) samples were taken at the following locations and submitted for testing:

<u>Sample No.</u>	<u>Station</u>	<u>Depth (m)</u>	<u>Test</u>
ST-1	-L2RPDB- Sta. 6+15	7.53 – 8.13	Triaxial CU, Con0solidation
ST-3	-L2RPBD Sta. 10+60	8.80 – 9.40	Triaxial CU, Consolidation
ST-4	-L- Sta. 93+60	8.97 – 9.57	Triaxial CU, Consolidation
ST-5	-L2RPDB- Sta. 12+00	7.50 – 8.10	Triaxial CU
ST-10	-L2RPDB- Sta. 11+40, 20 m Lt.	9.30 – 9.75	Consolidation
ST-12	-L2RPBD- Sta. 7+69.5 3.5 m RT	2.14-2.62	Consolidation

Culvert at -Y2- Sta. 12+60

Natural ground elevations range from 35.60± meters at the bottom of the ditch to 37.00 ± meters along the adjacent floodplain. Borings completed in the vicinity show approximately 1.5± meters of soft to stiff alluvial sandy silt (A-4) underlain by very loose to medium dense alluvial sand (A-3). During this investigation groundwater was measured at an elevation of 35.20± meters.

Prepared by:

J.L Stone, L.G.
Project Geologist

NWW/JLS

EARTHWORK BALANCE SHEET

R-2554BA

COMPUTED BY: ASH DATE: 11/16/2004
 CHECKED BY: MTM DATE: 11/16/2004
 REVISED BY: BJM/ASH DATE: 11/14/2007



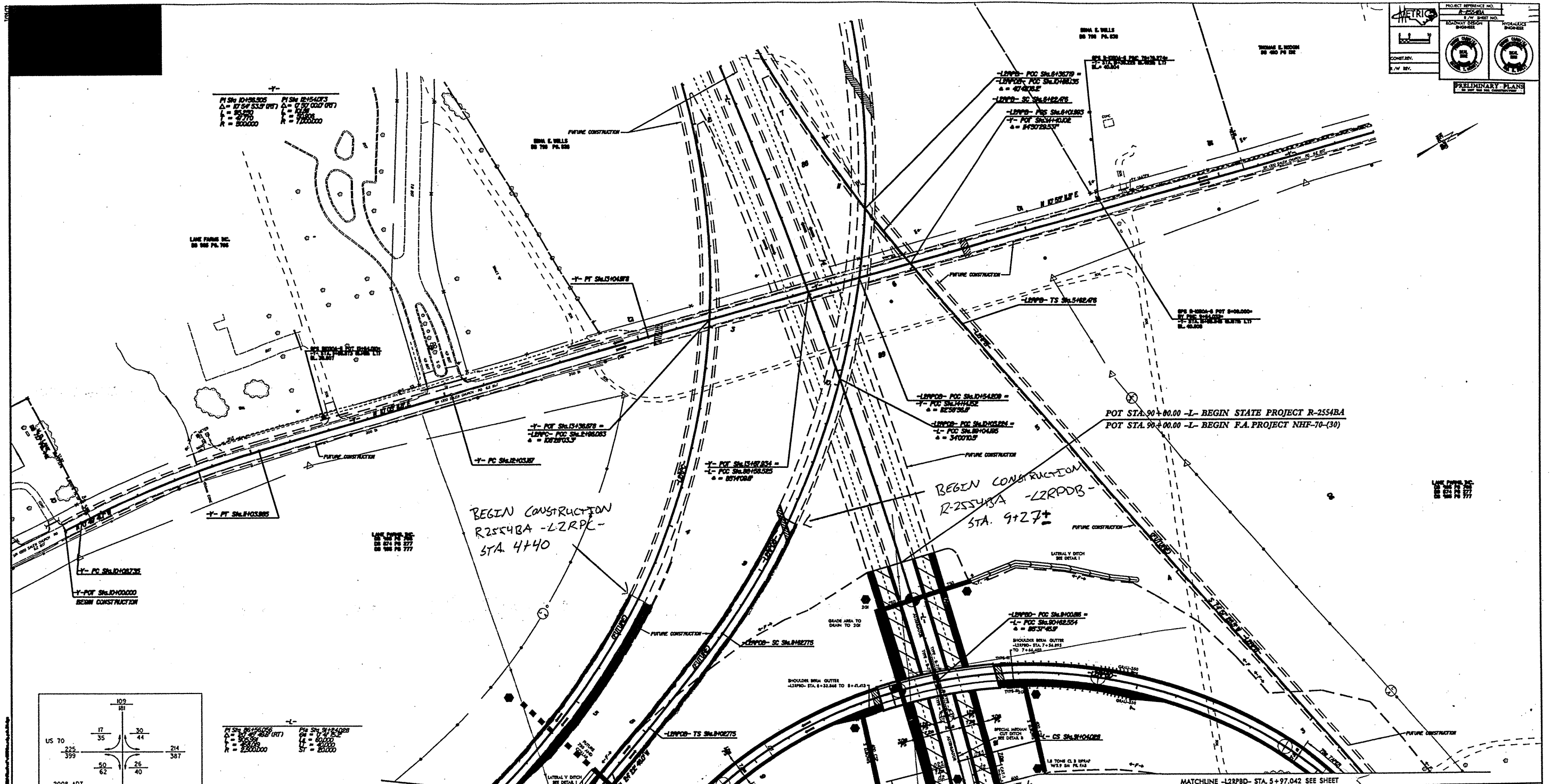
SHEET _____ OF _____

LINE	STATION	STATION	TOTAL EXCAV. (UNCL.)	ROCK EXCAV.	UNDERCUT EXCAV.	UNSUIT. EXCAV.	SUITABLE EXCAV.	TOTAL EMB.	ROCK EMB.	EARTH EMB.	EMBANKMENT [+25%]	BORROW	SUITABLE WASTE	UNSUIT. WASTE	TOTAL WASTE
SUMMARY #1															
DUAL BRIDGES															
L	90+00	92+85						96,710		96,710	120,888	120,888			
L2	81+80	84+09 (LT)	2,013			201	1,812	87		87	109		1,703	201	1,904
L2	85+20	92+00 (CL)						2,127		2,127	2,659	2,659			
L2	86+60	90+80 (CL)	489			49	440	141		141	176		264	49	313
L2	96+70	102+10 (LT)	420			42	378	2,274		2,274	2,843	2,843		42	42
L2RPB	3+04	5+06						20,518		20,518	25,648	25,648			
L2LPB	0+95	4+89	1,140			114	1,026	15,266		15,266	19,083	18,057		114	114
L2RPC	4+29	9+17	14,229		660	142	14,087	30,177		30,177	37,721	23,634		802	802
L2RPBD	0+00.00	7+72	897			90	807	109,908		109,908	137,385	136,578		90	90
END BRIDGE BEGN BRIDGE															
L2RPBD	8+30	10+76						123,962		123,962	154,953	154,953			
L2RPDB	6+12	8+60						135,867		135,867	169,834	169,834			
END BRIDGE															
SUBTOTALS:			19,188		660	638	18,550	537,037		137,123	671,299	654,716	1,967	1,298	3,265
SUMMARY #2															
END BRIDGE BEGN BRIDGE															
L	93+58	100+33						380,730		380,730	475,913	475,913			
L2	84+80	88+00 (RT)	34			3	31	46		46	58	27		3	3
L2	90+00	93+20 (RT)	877			88	789	283		283	354		435	88	523
L2	95+25	97+95 (RT)	604			60	544	291		291	364		180	60	240
L2RPA	1+18	6+56	586			59	527	38,764		38,764	48,455	47,928		59	59
L2LPD	0+76	4+98	706		400	71	635	42,843		42,843	53,554	52,919		471	471
L2RPD	1+30	3+86						42,455		42,455	53,069	53,069			
END BRIDGE															
L2RPBD	11+38	16+84						109,630		109,630	137,038	137,038			
L2RPDB	0+00	5+00	32,395		1,276	3,240	29,156	13,665		13,665	17,081	12,075	4,516	16,590	
SUBTOTALS:			35,202		1,676	3,521	31,681	628,707		628,707	785,886	766,894	12,689	5,197	17,886
SUMMARY #3															
END BRIDGE BEGN BRIDGE															
L	100+88	103+58						206,874		206,874	258,593	258,593			
Y2B	10+20	11+40	36		960	4	32	1,626		1,626	2,033	2,001		964	964
Y5	11+08	12+95	316			32	284	638		638	798	514		32	32
EY2	14+87	15+16	204			20	184	110		110	138		46	20	66
EY2	16+54	16+84	197			20	177	31		31	39		138	20	158
SUBTOTALS:			753		960	75	678	209,279		209,248	261,601	261,107	184	1,035	1,219
SUMMARY #4															
END BRIDGE BEGN BRIDGE															
L	104+04	114+20	3,511			351	3,160	126,887		126,887	158,609	155,449		351	351
Y2	10+87	26+35	8,999		6,220	900	8,099	28,102		28,102	35,128	27,029		7,120	7,120
Y2A	10+20	11+51	1,048		620	105	943	649		649	811		132	725	857
Y2RPA	12+20	16+40	3,593		2,970	359	3,234	4,524		4,524	5,655	2,421		3,329	3,329
Y2LPA	10+82	12+39						8,306		8,306	10,383	10,383			
Y2RPD	11+58	16+40	848		412	85	763	23,708		23,708	29,635	28,872		497	497
Y2LPD	10+97	12+39						18,873		18,873	23,591	23,591			
SR5	10+10	12+90	5			1	4	5,718		5,718	7,148	7,144		1	1
SR5	17+80	18+02	26			3	23	29		29	36	13		3	3
SR5	20+60	21+00	50			5	45						45	5	50
SR5	22+16	22+45	24			2	22	31		31	39	17		2	2
SR6	10+20	11+77	265			27	239						239	27	265
SR6	19+00	19+28	47			5	42	2		2	3		39	5	44
SUBTOTALS:			18,416		10,222	1,842	16,574	216,829		216,829	271,038	254,919	455	12,064	12,519

LINE	STATION	STATION	TOTAL EXCAV. (UNCL.)	ROCK EXCAV.	UNDERCUT EXCAV.	UNSUIT. EXCAV.	SUITABLE EXCAV.	TOTAL EMB.	ROCK EMB.	EARTH EMB.	EMBANKMENT [+25%]	BORROW	SUITABLE WASTE	UNSUIT. WASTE	TOTAL WASTE
SUMMARY #5															
END BRIDGE BEGN BRIDGE															
L	115+32	118+78						83,719		83,719	104,649	104,649			
SUBTOTALS:								83,719		83,719	104,649	104,649			
SUMMARY #6															
END BRIDGE															
L	119+10	128+20	18,580		3,000	1,858	16,722	132,232		132,232	165,290	148,568		4,858	4,858
SUBTOTALS:			18,580		3,000	1,858	16,722	132,232		132,232	165,290	148,568		4,858	4,858
SUMMARY #7															
END BRIDGE BEGN BRIDGE															
L	128+20	131+49	17,229					17,229		17,229	15,506	10,943		1,827	1,723
SUBTOTALS:			17,229					17,229		17,229	15,506	10,943		1,827	1,723
SUMMARY #8															
END BRIDGE															
L	133+25	142+00	1,334					1,334		1,334	1,201	71,904		133	133
SUBTOTALS:			1,334					1,334		1,334	1,201	71,904		133	133
SUMMARY #9															
END CONST.															
L	142+00	152+90	53,090					53,090		53,090	34,133		10,424	450	10,874
END BRIDGE															
Y8	12+00	15+56	279					279		279	251	52,094		28	28
END BRIDGE															
Y8	16+18	19+95	547					547		547	492	41,559		55	55
Y8RPA	12+47	14+60	1,285					1,285		1,285	1,157	17,867		129	129
Y8RPB	12+00	14+60	5,344		1,150	534	4,810	10,157		10,157	4,810	10,157		1,684	1,684
Y8RPC	12+02	14+20	1,126					1,126		1,126	1,013	13,415		113	113
Y8RPD	12+27	14+80	54					54		54	49	25,575		5	5
Y8DET	10+00	17+06	820					820		820	738	3,055		82	82
SUBTOTALS:			62,545		1,600	946	61,600	197,855		197,855	166,776	247,320	196,145	10,424	2,546
TOTALS:			173,247		18,118	10,735	162,512	2,088,505		2,088,505	2,610,642	2,475,677	27,546	28,853	56,399
EST. SHOULDER MATERIAL:								30,000		30,000	37,500	37,500			
ADDITIONAL UNDERCUT (EMBANKMENT STABILITY):								500		500	625	625		500	500
ADDITIONAL UNDERCUT (SUBGRADE STABILITY):								4,000		4,000	5,000	5,000		4,000	4,000
ADDITIONAL UNDERCUT (GRADE POINT):								5,900		5,900	7,375	7,375		5,900	5,900
WASTE IN LIEU OF BORROW:												-27,546	-27,546		-27,546
REMOVAL OF DETOURS:			3,055					3,055		3,055			3,055		3,055
PROJECT TOTALS:			176,302		28,518	10,735	165,567	2,128,905		2,128,905	2,661,142	2,498,630	3,055	39,253	42,308
EST. FOR REPL. TOPSOIL ON BORROW PITS												124,932			
GRAND TOTALS:			176,302					176,302		176,302	2,661,142	2,498,630	3,055	39,253	42,308
SAY:			180,000					180,000		180,000	2,665,000				45,000

-L-, -L2-, -L2LPB-, -L2LPD-, -L2RPA-, -L2RPBD-, -L2RPC-, -L2RPDB-, -Y-, -Y2-, -Y2A-, -Y2RPA-, -Y8-, -Y8RPA-, -Y8RPB-, -Y8RPC-, -Y8RPD-, -YDET-, -SR5-, & -SR6- PAVEMENT STRUCTURE VOLUME = 15,300 m3
 DDE = 56,400 m3

Note: Earthwork quantities are calculated by the Roadway Design Unit. These earthwork quantities are based in part on subsurface data provided by the Geotechnical Engineering Unit.



PI SH 1048205 PI SH 1048203
 $\Delta = 17.54$ $\Delta = 17.54$
 $L = 277.0$ $L = 277.0$
 $R = 250000$ $R = 250000$

PI SH 9124028 PI SH 9124028
 $\Delta = 17.54$ $\Delta = 17.54$
 $L = 277.0$ $L = 277.0$
 $R = 250000$ $R = 250000$

PROJECTED TRAFFIC VOLUMES

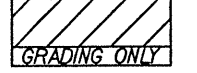
109	181
17	30
35	44
225	214
399	387
60	26
62	40
137	204

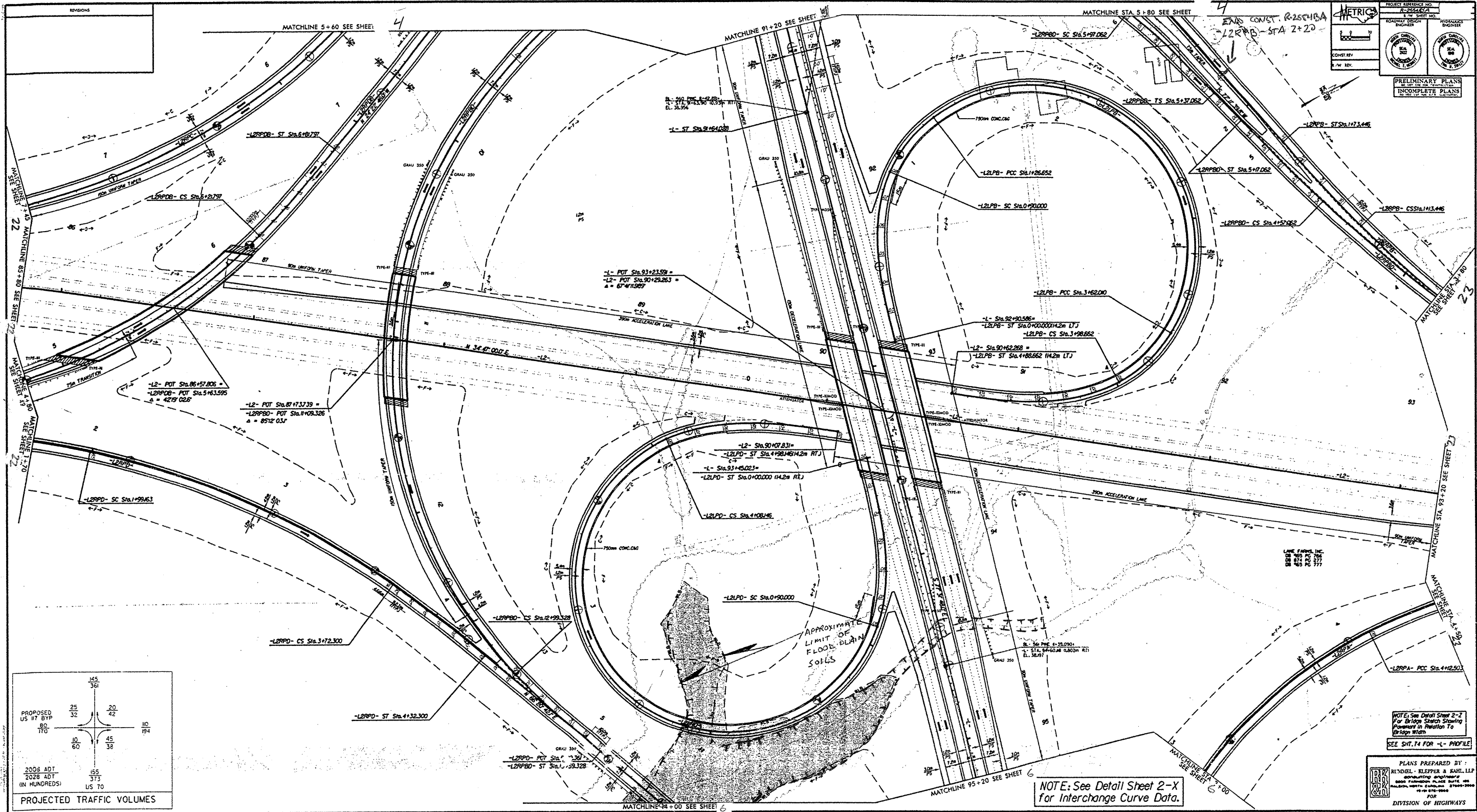
2008 ADT
 2028 ADT
 (BY HUNDREDS)
 US RT BYPASS

NOTE: See Detail Sheet E-11 for Interchange Curve Data.
 NOTE: See Detail Sheet E-12 for Bridge Slab Stopping Power in Relation To Bridge Width.
 SEE SH. 33 FOR -L- PROFILE
 SEE SH. 47 FOR -LWPD- PROFILE
 SEE SH. 48 FOR -LWPC- PROFILE
 SEE SH. 52 & 53 FOR -LWPD- PROFILE
 SEE SH. 56 FOR -LWPD- PROFILE

PLANS PREPARED BY:
 HENRIK & KATH, LLP
 CONSULTING ENGINEERS
 1000 ...
 DIVISION OF HIGHWAYS

DATE: _____
 CHECKED BY: _____





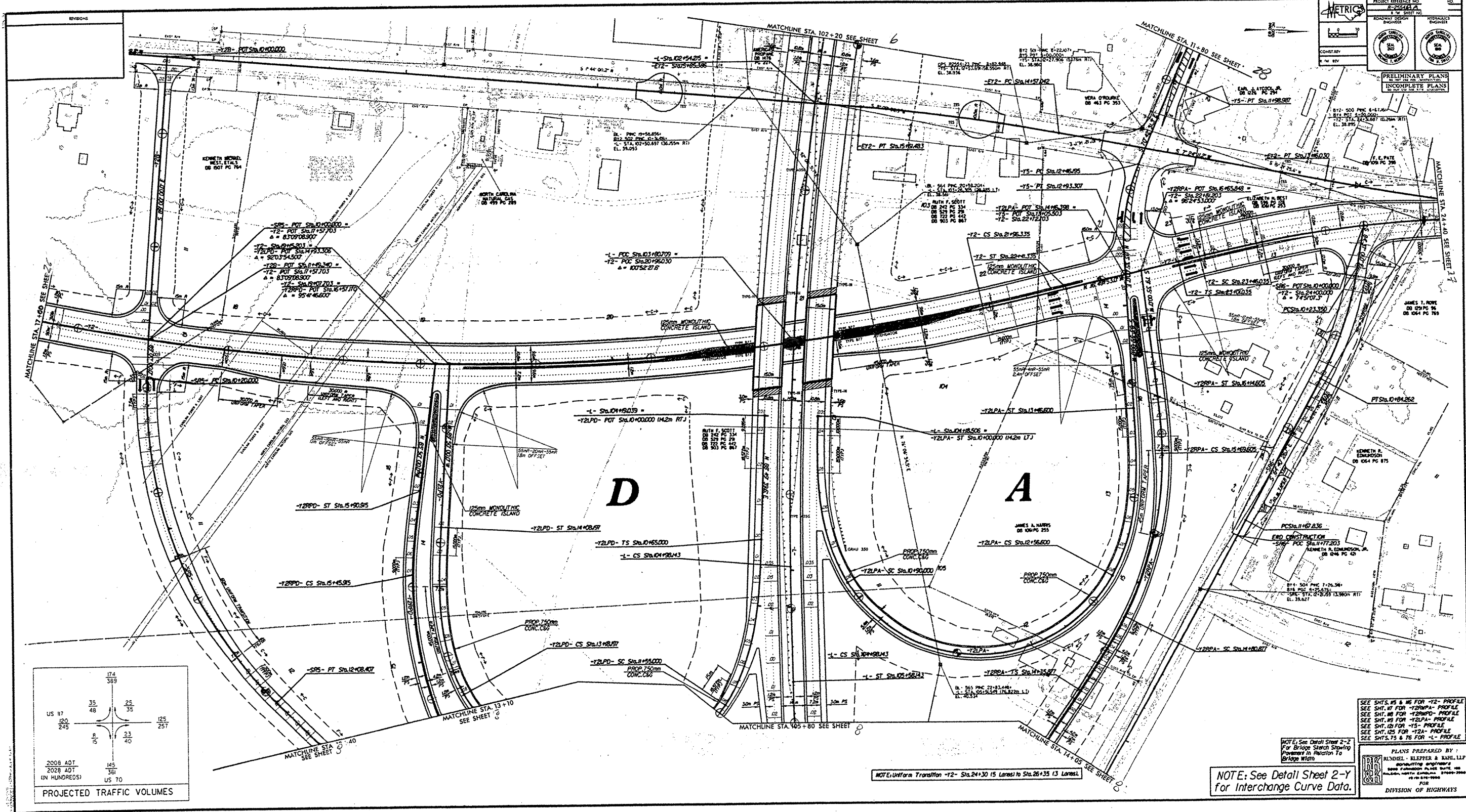
PROJECTED TRAFFIC VOLUMES			
2028 ADT	155	155	155
2028 ADT (IN HUNDREDS)	373	373	373
	US 70		

NOTE: See Detail Sheet 2-2 for Bridge Section Showing Pavement in Relation to Bridge Wym

SEE SH. 14 FOR -L- PROFILE

PLANS PREPARED BY:
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 Consulting Engineers
 6000 FARMERSBURGH BLVD. SUITE 100
 FARMERSBURGH, NY 11735
 (845) 336-1000
 FOR
 DIVISION OF HIGHWAYS

PROJECT REFERENCE NO. 12-2221A
 1. SHEET NO.
 ROADWAY DESIGN
 HIGHWAYS
 CONTRACTOR
 METRIC
 PRELIMINARY PLANS
 INCOMPLETE PLANS



PROJECTED TRAFFIC VOLUMES

US 17	174	389	
35	48	25	35
120	245		125
	15	23	40
		140	381
		140	381
		174	389
			125

2008 ADT
 2028 ADT
 (IN HUNDREDS)

SEE SHTS. 15 & 16 FOR -12- PROFILE
 SEE SHT. 17 FOR -12RPA- PROFILE
 SEE SHT. 18 FOR -12RPA- PROFILE
 SEE SHT. 19 FOR -12LPA- PROFILE
 SEE SHT. 20 FOR -15- PROFILE
 SEE SHT. 21 FOR -12A- PROFILE
 SEE SHTS. 25 & 26 FOR -1- PROFILE

NOTE: See Detail Sheet 2-Y
 for Interchange Curve Data.

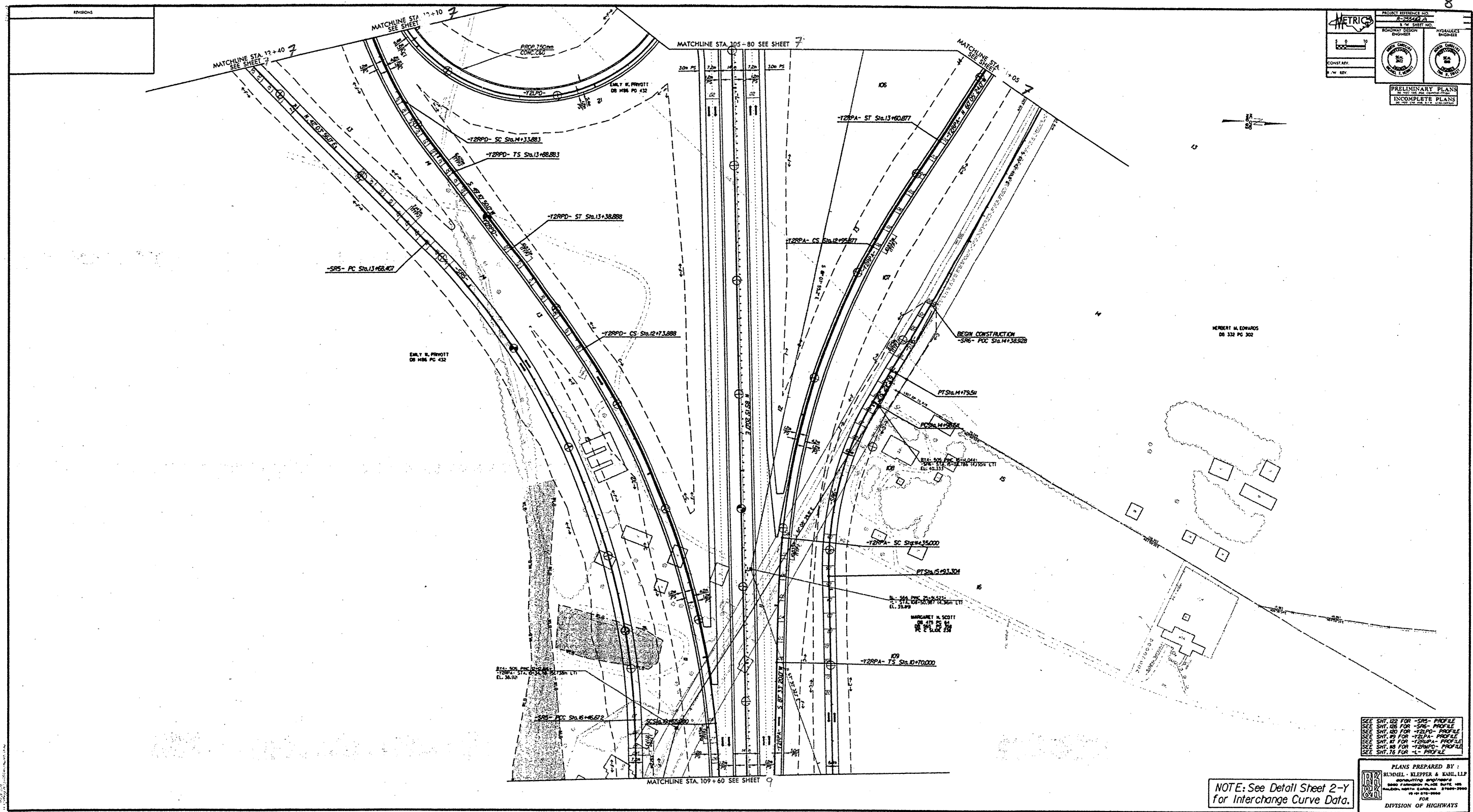
NOTE: Uniform Transition -12- Sta. 24+30 15 Lanes to Sta. 26+35 (3 Lanes)

PLANS PREPARED BY:
 RUMMEL, KLEPPER & KAHL, LLP
 Consulting Engineers
 10000 North Central Expressway, Suite 200
 Dallas, Texas 75243
 (214) 343-1000
 FOR
 DIVISION OF HIGHWAYS

PROJECT REFERENCE NO. 12-2562-A
 PLAN SHEET NO. 8
 ROADWAY DESIGN ENGINEER
 HYDRAULIC ENGINEER

CONST. REV. 1
 11/11/10

PRELIMINARY PLANS
 INCOMPLETE PLANS



SEE SHT. 122 FOR -SR5- PROFILE
 SEE SHT. 125 FOR -SR5- PROFILE
 SEE SHT. 120 FOR -T2RPA- PROFILE
 SEE SHT. 89 FOR -T2RPA- PROFILE
 SEE SHT. 87 FOR -T2RPA- PROFILE
 SEE SHT. 88 FOR -T2RPA- PROFILE
 SEE SHT. 76 FOR -L- PROFILE

PLANS PREPARED BY:
 RUMMEL - KLEPPER & KAHL, LLP
 8800 FARMERSBURG PL. SUITE 100
 HUNTSVILLE, ALABAMA 35894-2900
 256-876-9900

NOTE: See Detail Sheet 2-Y
 for Interchange Curve Data.

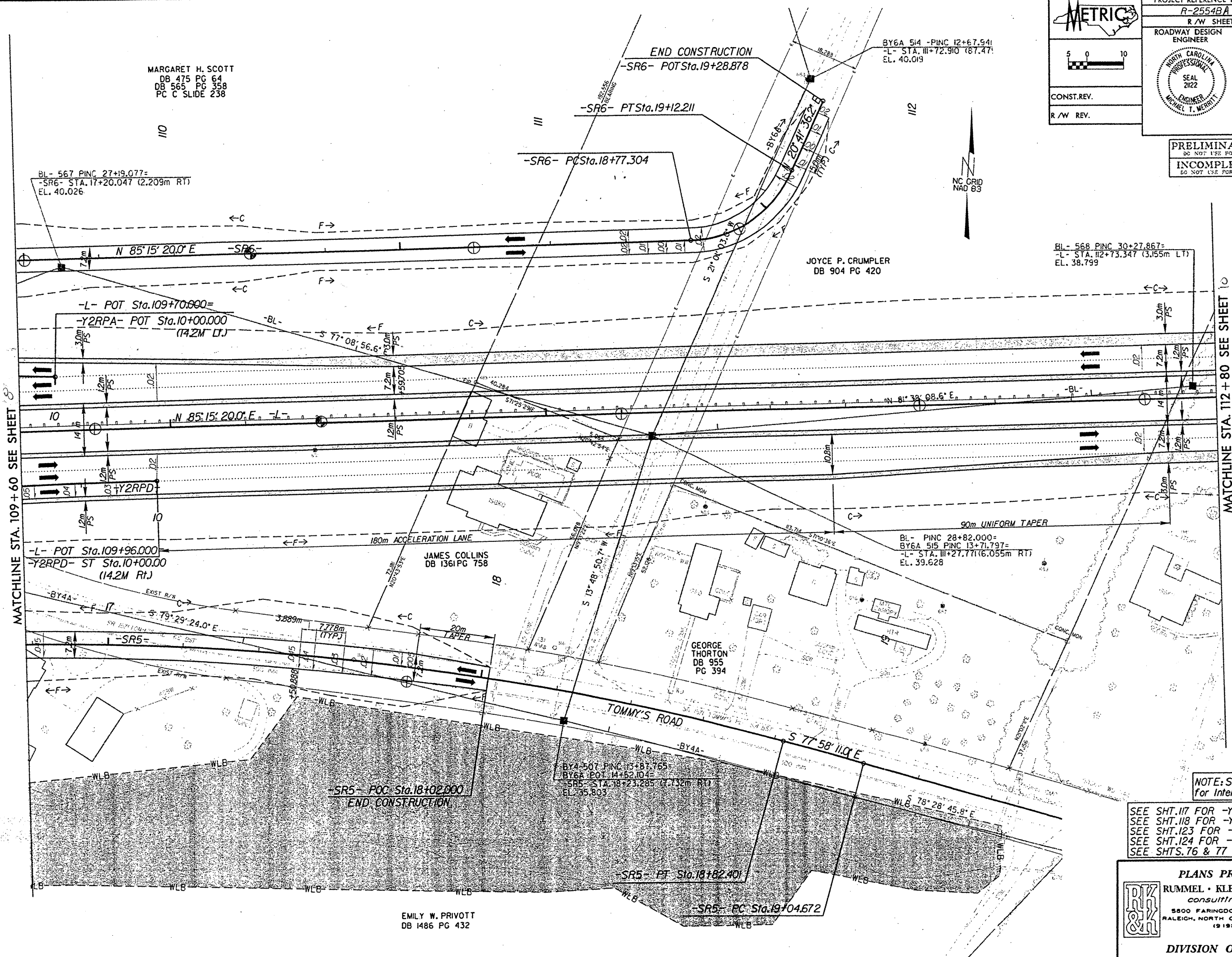
METRIC

CONST. REV.
R/W REV.

PROJECT REFERENCE NO. R-2554BA	SHEET NO.
R/W SHEET NO.	
ROADWAY DESIGN ENGINEER MICHAEL T. MERRITT	HYDRAULICS ENGINEER TIM H. SWEITZ

PRELIMINARY PLANS
DO NOT USE FOR CONSTRUCTION.
INCOMPLETE PLANS
DO NOT USE FOR R/W ACQUISITION

MARGARET H. SCOTT
DB 475 PG 64
DB 565 PG 358
PC C SLIDE 238



REVISIONS

MATCHLINE STA. 109+60 SEE SHEET 18

MATCHLINE STA. 112+80 SEE SHEET 10

NOTE: See Detail Sheet 2-Y for Interchange Curve Data.

SEE SHT. 117 FOR -Y2RPA- PROFILE
SEE SHT. 118 FOR -Y2RPD- PROFILE
SEE SHT. 123 FOR -SR5- PROFILE
SEE SHT. 124 FOR -SR6- PROFILE
SEE SHTS. 76 & 77 FOR -L- PROFILE

PLANS PREPARED BY :

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consulting engineers
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RALEIGH, NORTH CAROLINA 27609-3960
(919) 878-9560
FOR
DIVISION OF HIGHWAYS

EMILY W. PRIVOTT
DB 1486 PG 432

JOYCE P. CRUMPLER
DB 904 PG 420

JAMES COLLINS
DB 1361 PG 758

GEORGE THORNTON
DB 955 PG 394

BY4-501 PINC 13+81.765
BY6A POT 14+52.104
SR5- STA. 18+23.285 (1.132m RT)
EL. 35.803

BL- PINC 28+82.000=
BY6A 515 PINC 13+71.797=
-L- STA. 11+27.771 (6.055m RT)
EL. 39.628

BL- 568 PINC 30+27.867=
-L- STA. 112+73.347 (3.155m LT)
EL. 38.799

BL- 567 PINC 27+19.077=
-SR6- STA. 17+20.047 (2.209m RT)
EL. 40.026

CAREY R. HAM
 DB 1621 PG 708

NC GRID
 NAD 83

-TI- 103 PINC 5+77.965=
 -L- STA. 114+27.108 (59.987m LT)
 ELEV.= 33.131
 NAIL SET

-TI-104 PINC 6+10.417=
 -L- STA. 114+42.770 (31.565, LT)
 ELEV.=33.226
 NAIL SET

BL- 569 PINC 31+84.201=
 -L- STA. 114+29.666 (0.937m LT)
 EL. 36.786

BL POT 32+07.513
 -TI-105 PINC 6+42.852=
 -L- STA. 114+52.977 (0.779m LT)
 ELEV.=33.367
 NAIL SET

BL- 570 PINC 32+62.972=
 -L- STA. 115+08.434 (0.408m LT)
 EL. 33.203

BL- 571 PINC 33+44.122=
 -L- STA. 115+89.581 (0.428m RT)
 EL. 37.656

-TI-106 PINC 6+81.459=
 -L- STA. 114+49.465 (37.671m RT)
 ELEV.= 33.092
 NAIL SET

-TI- 107 PINC 7+30.605=
 -L- STA. 114+32.632 (83.834m RT)
 ELEV.= 34.999
 NAIL SET

BY4- 508 PINC 15+95.311=
 -SR5- STA. 20.31.770 (4.681m RT)
 EL. 35.108

-SR5- PC Sta.20+55.017

-SR5- PT Sta.21+15.247

-SR5- POT Sta.22+76.518

BEGIN CONSTRUCTION
 -SR5- POC Sta.20+60.000

WILLIAM W. WHITE
 DB 1137 PG 620

-TI- 108 POT 8+11.687=
 -L- STA. 114+09.242 (161.469m RT)
 ELEV.= 34.071
 NAIL SET

BY4 509 PINC 18+15.292=
 -L- STA. 115+19.717 (67.727m RT)
 EL. 36.978

NOTE: See Detail Sheet 2-Y
 for Interchange Curve Data.

NOTE: See Detail Sheet 2-Z
 For Bridge Sketch Showing
 Pavement In Relation To
 Bridge Width

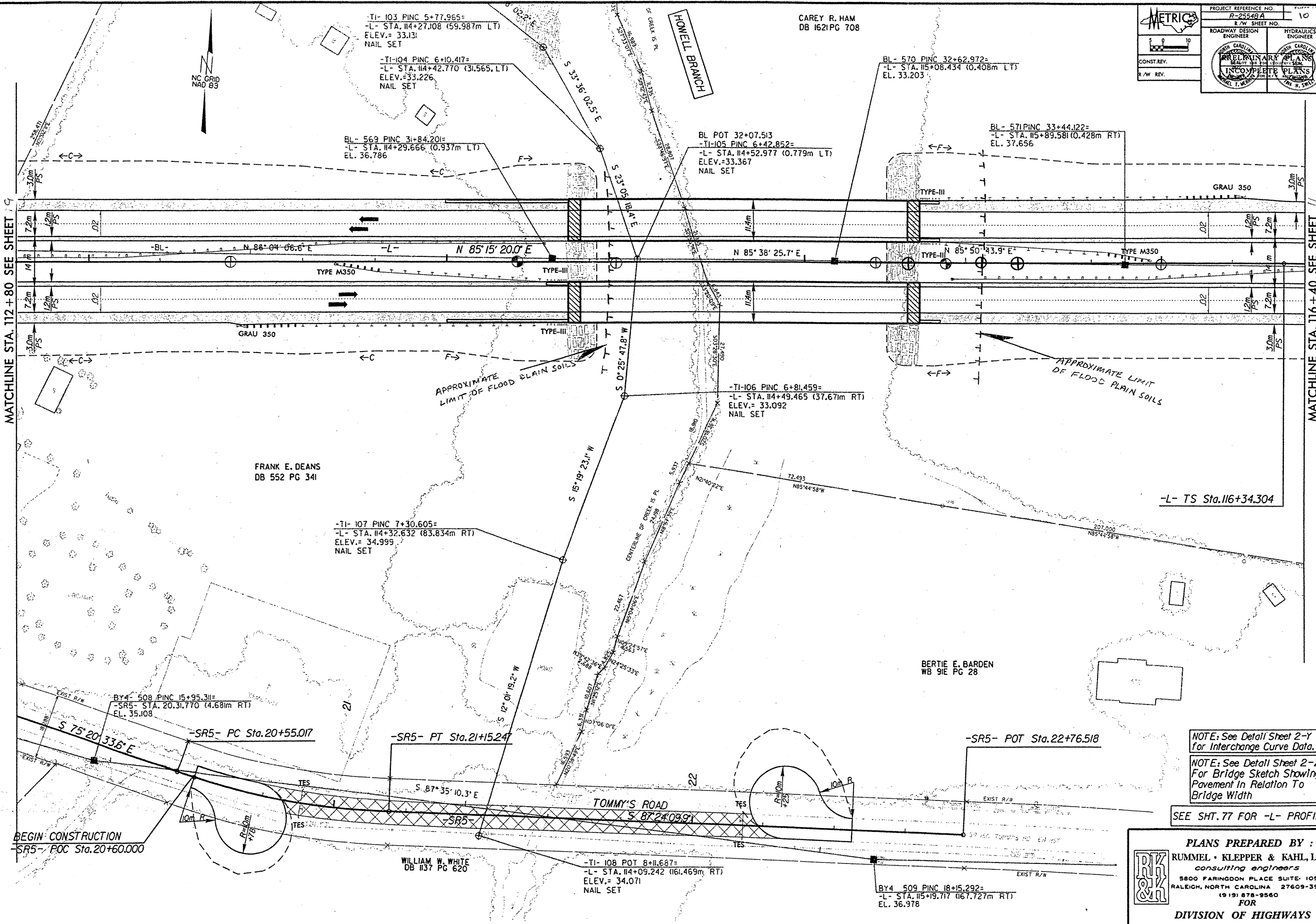
SEE SHT. 77 FOR -L- PROFILE

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REVISIONS

MATCHLINE STA. 112+80 SEE SHEET 9

MATCHLINE STA. 116+40 SEE SHEET 11



METRIC

PROJECT REFERENCE NO. R-2554BA SHEET NO. 11

R/W SHEET NO.

ROADWAY DESIGN ENGINEER
 SEAL 2122
 MICHAEL I. VERRILL

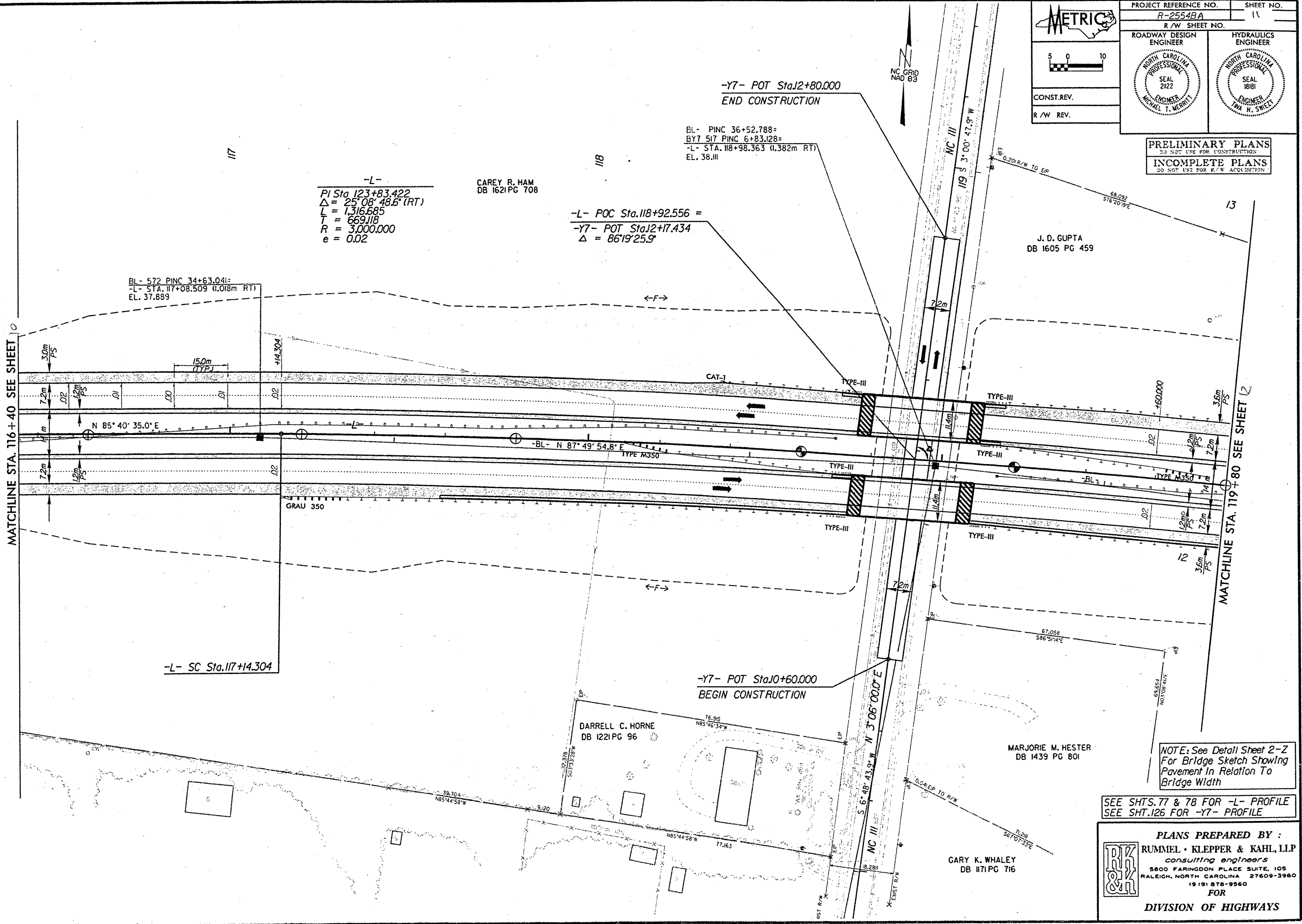
HYDRAULICS ENGINEER
 SEAL 18181
 TWA H. SWEET

CONST. REV.

R/W REV.

PRELIMINARY PLANS
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INCOMPLETE PLANS
 DO NOT USE FOR R/W ACQUISITION



REVISIONS

MATCHLINE STA. 116+40 SEE SHEET 10

MATCHLINE STA. 119+80 SEE SHEET 12

NOTE: See Detail Sheet 2-Z For Bridge Sketch Showing Pavement In Relation To Bridge Width

SEE SHTS. 77 & 78 FOR -L- PROFILE
 SEE SHT. 126 FOR -Y7- PROFILE

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 1919 878-9560

FOR

DIVISION OF HIGHWAYS

GARY K. WHALEY
 DB 1171 PG 716

MARJORIE M. HESTER
 DB 1439 PG 801

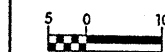
DARRELL C. HORNE
 DB 1221 PG 96

J. D. GUPTA
 DB 1605 PG 459

CAREY R. HAM
 DB 1621 PG 708



PROJECT REFERENCE NO. R-2554B A	SHEET NO. 12
R/W SHEET NO.	
ROADWAY DESIGN ENGINEER NORTH CAROLINA PROFESSIONAL SEAL 2022 MICHAEL T. MERRITT	HYDRAULICS ENGINEER NORTH CAROLINA PROFESSIONAL SEAL 18181 H. SWIEZY



CONST. REV.
R/W REV.

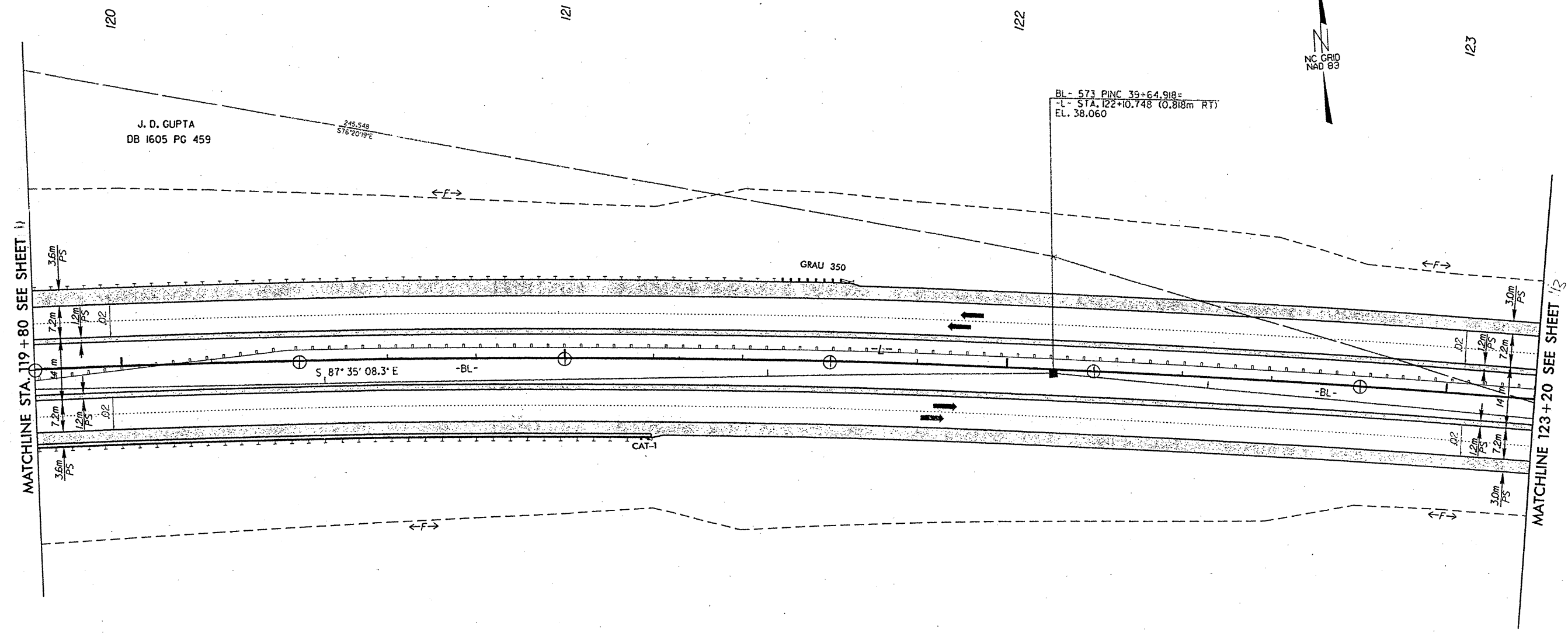
PRELIMINARY PLANS
DO NOT USE FOR CONSTRUCTION
INCOMPLETE PLANS
DO NOT USE FOR R/W ACQUISITION



HUBERT C. MATHEWS
DB 582 PG 536

J. D. GUPTA
DB 1605 PG 459

BL- 573 PINC 39+64.918=
-L- STA. 122+10.748 (0.818m RT)
EL. 38.060






J. D. GUPTA
DB 1605 PG 459

-L-
PI Sta 123+83.422
 $\Delta = 25^\circ 08' 48.6" (RT)$
 $L = 1,316.685$
 $T = 669.118$
 $R = 3,000.000$
 $e = 0.02$

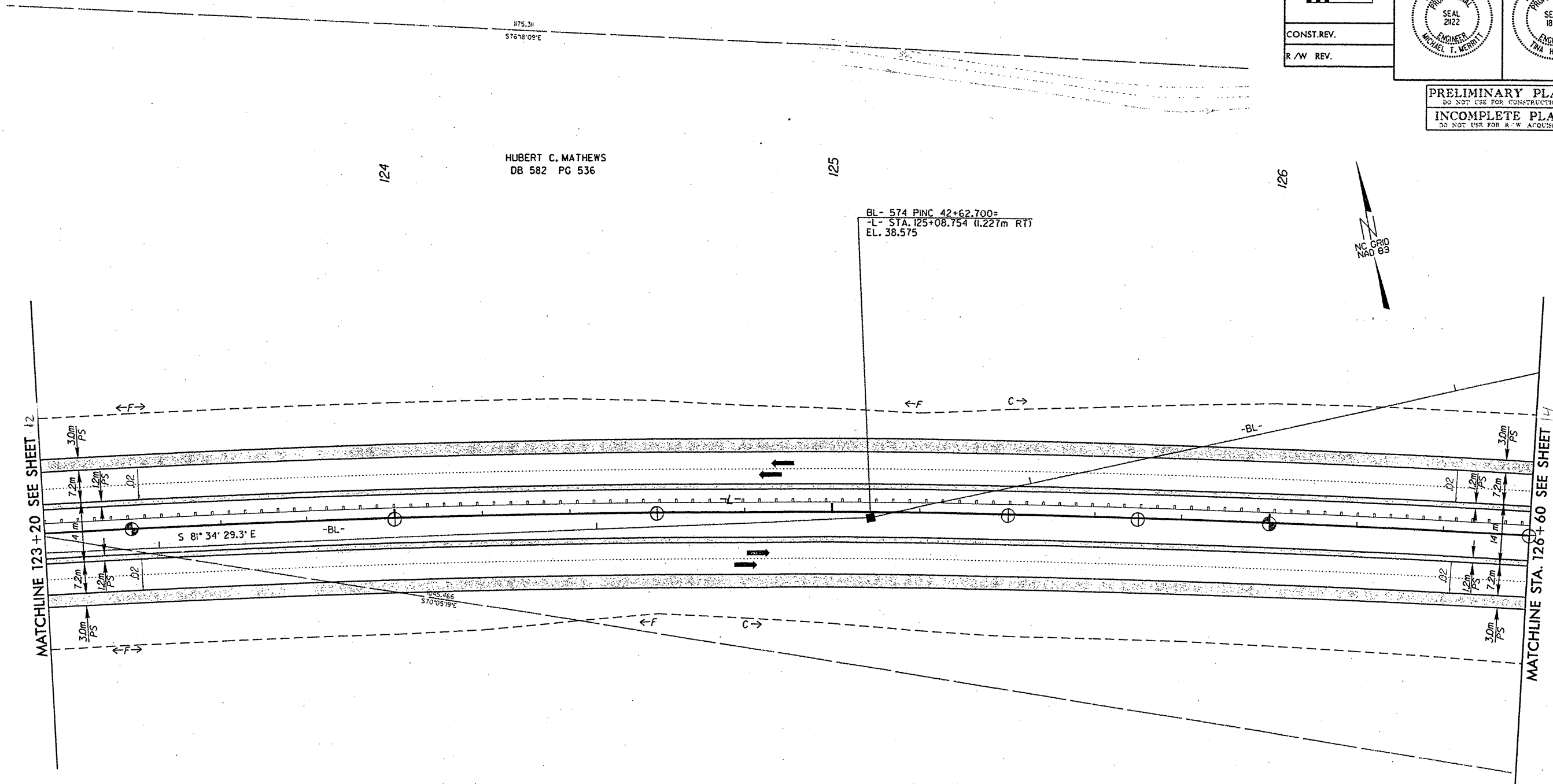
SEE SHT. 78 FOR -L- PROFILE

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DIVISION OF HIGHWAYS

REVISIONS

 5 0 10 CONST. REV. R/W REV.	PROJECT REFERENCE NO. R-2554BA	SHEET NO. 13
	R/W SHEET NO.	
ROADWAY DESIGN ENGINEER 	HYDRAULICS ENGINEER 	

PRELIMINARY PLANS
DO NOT USE FOR CONSTRUCTION
INCOMPLETE PLANS
DO NOT USE FOR R/W ACQUISITION



HUBERT C. MATHEWS
DB 582 PG 536

BL- 574 PINC 42+62.700=
-L- STA. 125+08.754 (1.227m RT)
EL. 38.575



MATCHLINE 123+20 SEE SHEET 12


MATCHLINE STA. 126+60 SEE SHEET 14

J. D. GUPTA
DB 1605 PG 459

-L-
PI Sta 123+83.422
 $\Delta = 25^\circ 08' 48.6''$ (RT)
L = 1,316.685
T = 669.118
R = 3,000.000
e = 0.02


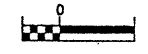
SEE SHTS. 78 & 79 FOR -L- PROFILE

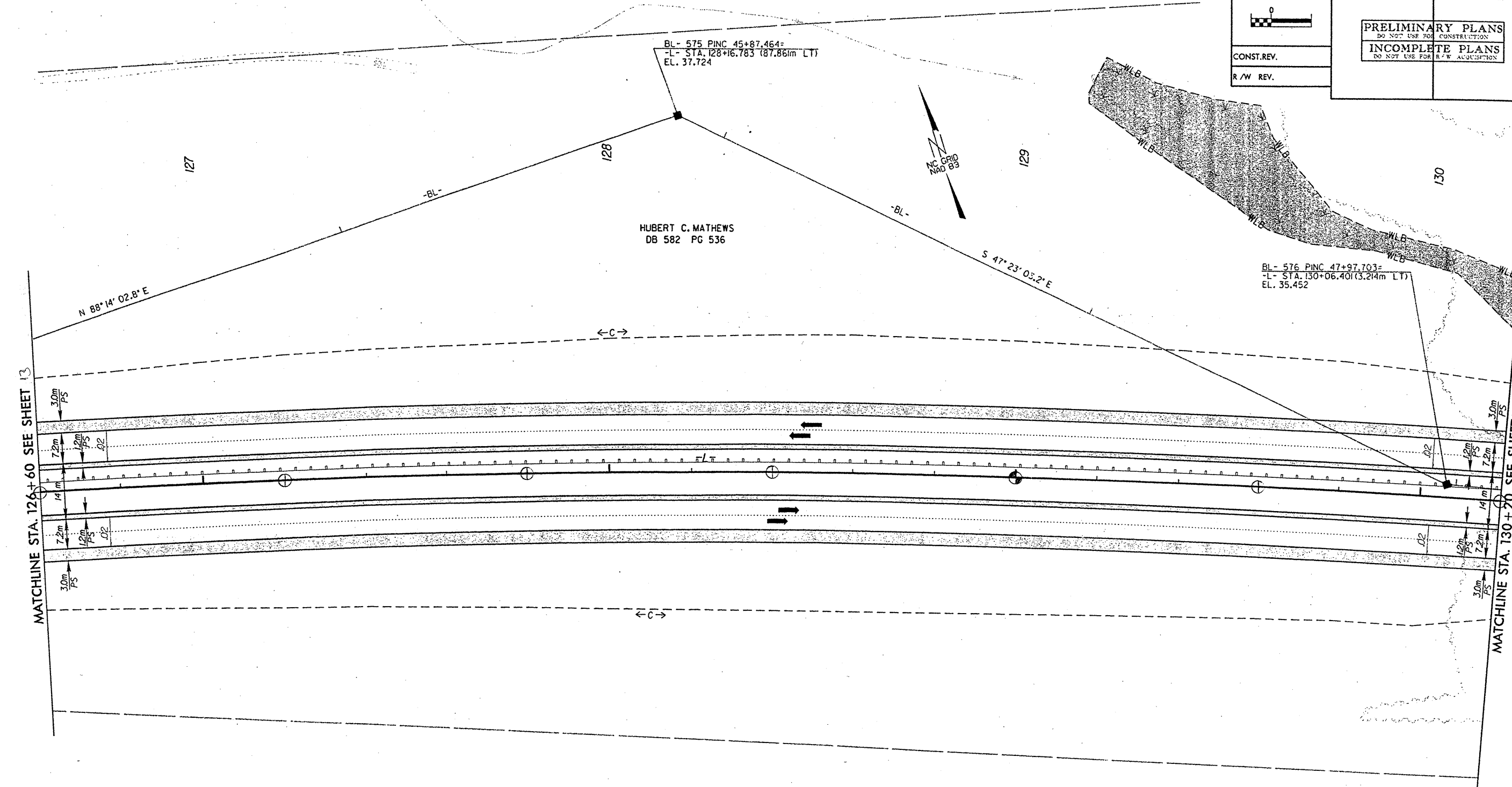
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	PROJECT REFERENCE NO.	SHEET NO.
	R-2554BA	14
	R/W SHEET NO.	
	ROADWAY DESIGN ENGINEER	HYDRAULICS ENGINEER
CONST. REV.	PRELIMINARY PLANS <small>DO NOT USE FOR CONSTRUCTION</small>	
R/W REV.	INCOMPLETE PLANS <small>DO NOT USE FOR R/W ACQUISITION</small>	




REVISIONS

-L-
 PI Sta 123+83.422
 $\Delta = 25^{\circ} 08' 48.6" (RT)$
 $L = 1316.685$
 $T = 669.118$
 $R = 3,000.000$
 $e = 0.02$

J. D. GUPTA
 DB 1605 PG 459

SEE SHT. 79 FOR -L- PROFILE

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 FOR
DIVISION OF HIGHWAYS

METRIC

PROJECT REFERENCE NO. R-2554BA SHEET NO. 15

R/W SHEET NO.

ROADWAY DESIGN ENGINEER HYDRAULICS ENGINEER

CONST. REV. R/W REV.

SEAL 2122 MICHAEL T. MERRITT

SEAL 18181 TWA H. SWICLY

PRELIMINARY PLANS
DO NOT USE FOR CONSTRUCTION

INCOMPLETE PLANS
DO NOT USE FOR R/W ACQUISITION

HUBERT C. MATHEWS
DB 582 PG 536

RUTH H. EXUM
DB 1486 PG 40

J. D. GUPTA
DB 1605 PG 459

SHERMAN E. BEST
DB 1534 PG 253

BL - 577 PINC 49+51.111=
-L- STA. 131+59.729 (0.203m LT)
ELEV. 30.557

-T2-201 POT 5+00.000=
-L- STA. 133+06.077 (47.042m LT)
ELEV. = 30.236
NAIL SET

-T2-202 PINC 5+29.043=
-L- STA. 132+94.509 (20.403m LT)
ELEV. = 29.974
NAIL SET

BL - 578 PINC 51+18.114=
-L- STA. 133+26.726 (1.655m LT)
ELEV. 30.803

BL POT 50+92.788
-T2-203 PINC 5+19.233=
-L- STA. 133+01.410 (1.429m LT)
ELEV. = 29.966
NAIL SET

-T2-204 PINC 5+79.387=
-L- STA. 132+96.775 (28.366m RT)
ELEV. = 30.455
NAIL SET

-T2-205 6+16.223=
-L- STA. 132+86.313 (63.686m RT)
ELEV. = 30.555
NAIL SET

-T2-206 POT 6+40.398=
-L- STA. 132+77.403 (86.158m RT)
ELEV. = 29.198
NAIL SET

NOTE: See Detail Sheet 2-Z
For Bridge Sketch Showing
Pavement in Relation To
Bridge Width

SEE SHTS. 79 & 80 FOR -L- PROFILE

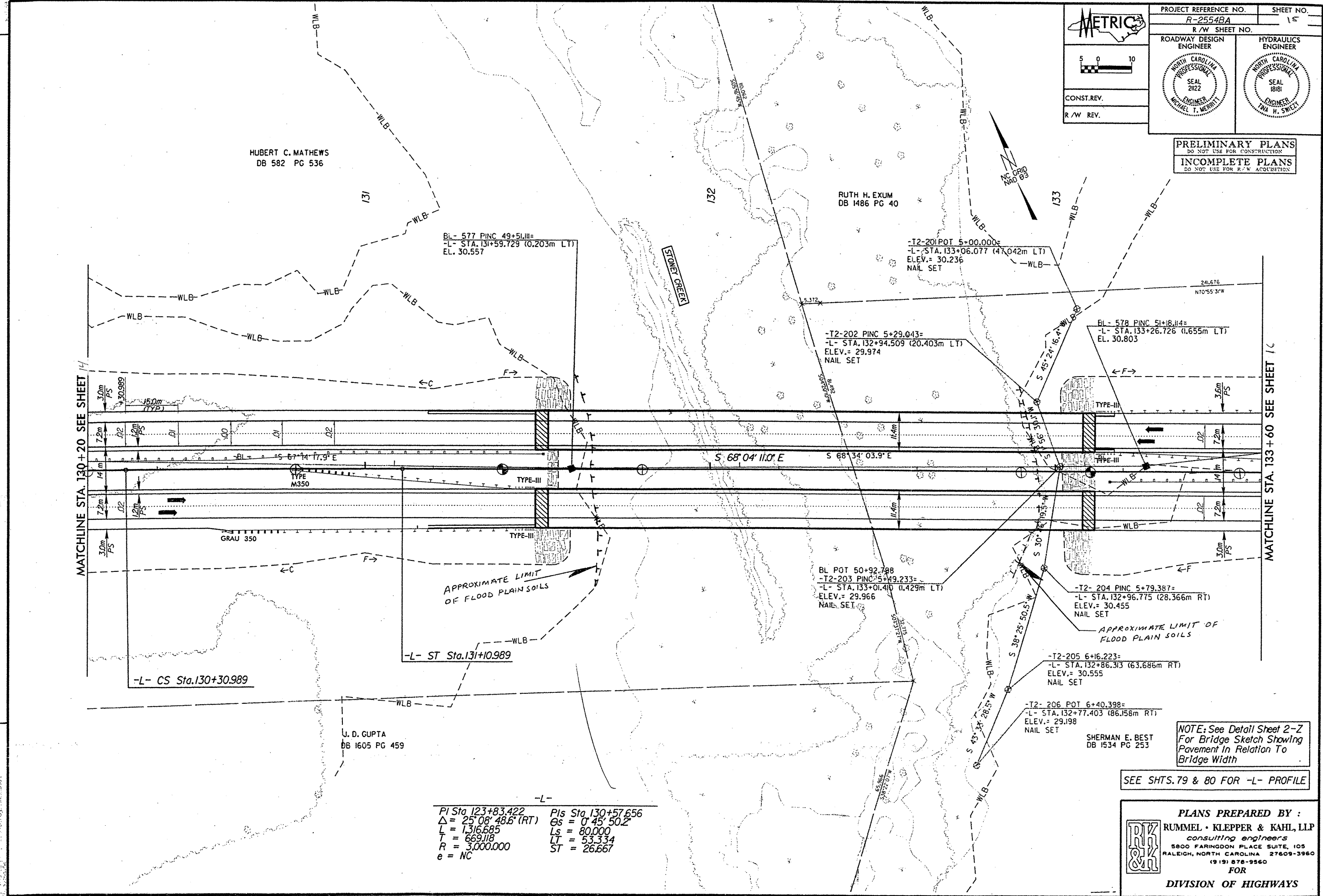
-L-




PI Sta 123+83.422	PIs Sta 130+57.656
$\Delta = 25^{\circ} 08' 48.6" (RT)$	$\Theta_s = 0^{\circ} 45' 50.2"$
$L = 1,316.685$	$L_s = 80,000$
$T = 669.118$	$LT = 53.334$
$R = 3,000,000$	$ST = 26.667$
$e = NC$	

MATCHLINE STA. 130+20 SEE SHEET 14

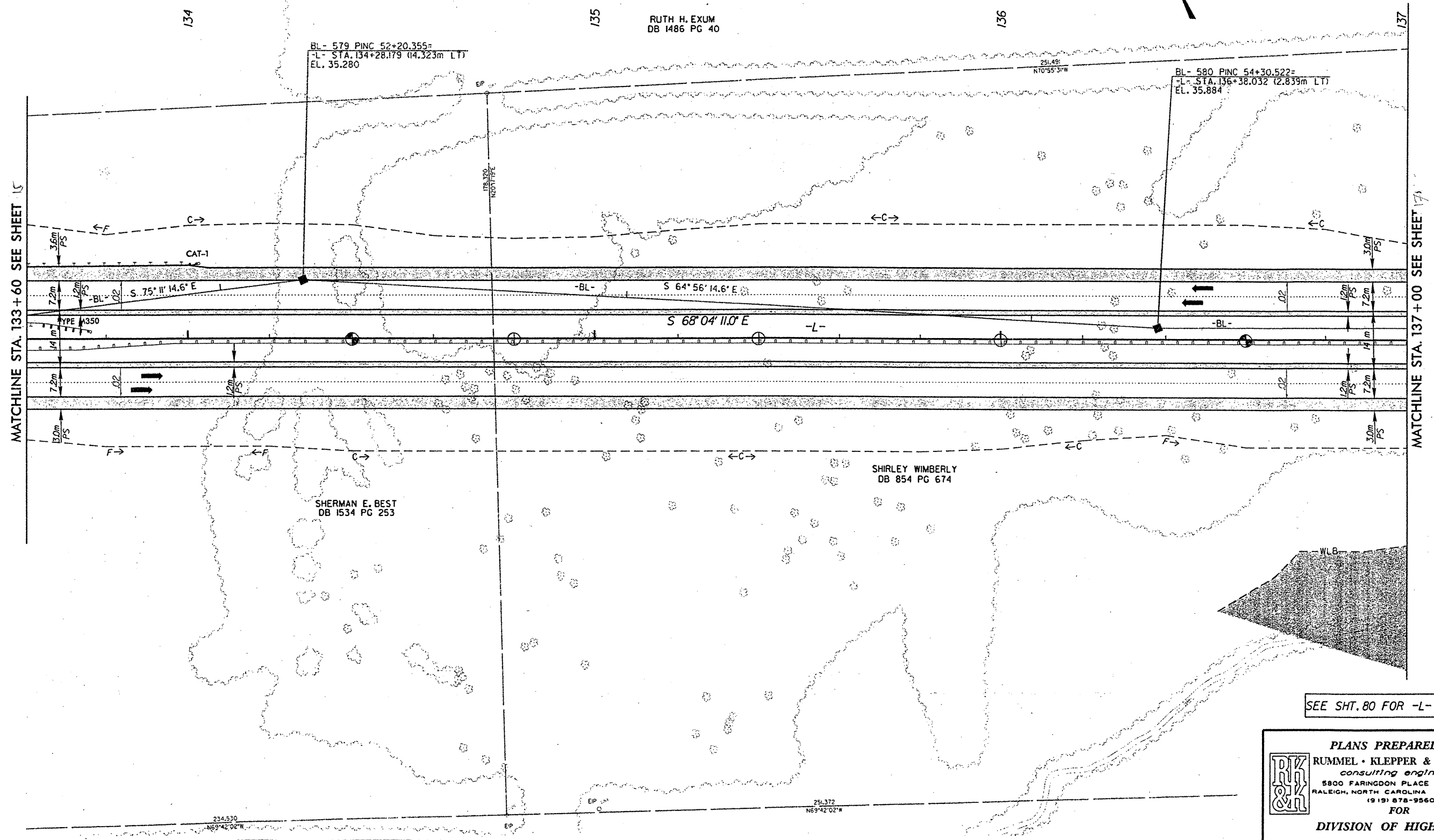
MATCHLINE STA. 133+60 SEE SHEET 1C

REVISIONS



 5 0 10	PROJECT REFERENCE NO. R-2554BA	SHEET NO. 16
	R/W SHEET NO.	
CONST. REV.	ROADWAY DESIGN ENGINEER 	HYDRAULICS ENGINEER 
R/W REV.		

PRELIMINARY PLANS
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INCOMPLETE PLANS
DO NOT USE FOR R. W. ACQUISITION




MATCHLINE STA. 133 + 60 SEE SHEET 15

MATCHLINE STA. 137 + 00 SEE SHEET 17

SEE SHT. 80 FOR -L- PROFILE

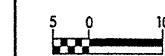
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 FOR
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PROJECT REFERENCE NO. R-2554BA SHEET NO. 17
R/W SHEET NO.



CONST. REV.
R/W REV.

ROADWAY DESIGN ENGINEER



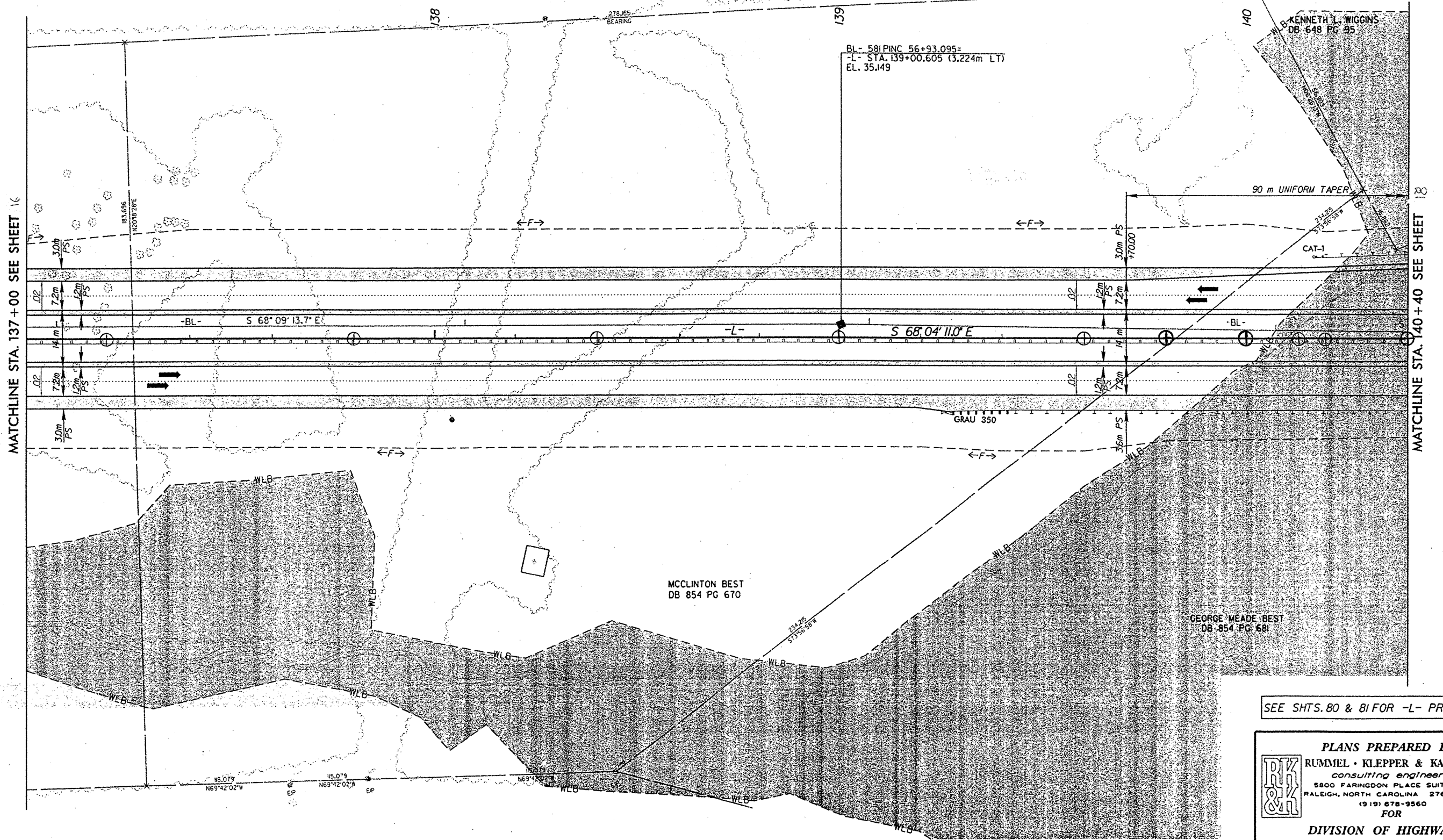
HYDRAULICS ENGINEER



PRELIMINARY PLANS
DO NOT USE FOR CONSTRUCTION
INCOMPLETE PLANS
DO NOT USE FOR R/W ACQUISITION



REUBEN HAMILTON
DB 1486 PG 43




REVISIONS

MCCLINTON BEST
DB 854 PG 670

GEORGE MEADE BEST
DB 854 PG 681

SEE SHTS. 80 & 81 FOR -L- PROFILE

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 FOR
 DIVISION OF HIGHWAYS

METRIC

PROJECT REFERENCE NO. R-25548A SHEET NO. 18

R/W SHEET NO.

ROADWAY DESIGN ENGINEER
HYDRAULICS ENGINEER

SEAL 2022
SEAL 18181

CONST. REV.
R/W REV.

ENGINEER MICHAEL T. MERRITT
ENGINEER TIM H. SWEET

PRELIMINARY PLANS
DO NOT USE FOR CONSTRUCTION

INCOMPLETE PLANS
DO NOT USE FOR R/W ACQUISITION

-Y8RPB-

BL- 583 PINC 61+38.866=
L- STA. 142+99.758 (109.694m LT)
EL. 38.068

Pls Sta 10+43.348
Os = 4° 39' 19.0"
Ls = 65.000
LT = 43.348
ST = 21.680

PI Sta 11+19.212
Δ = 15° 26' 11.5" (LT)
L = 107.767
T = 54.212
R = 400.000
e = 0.08

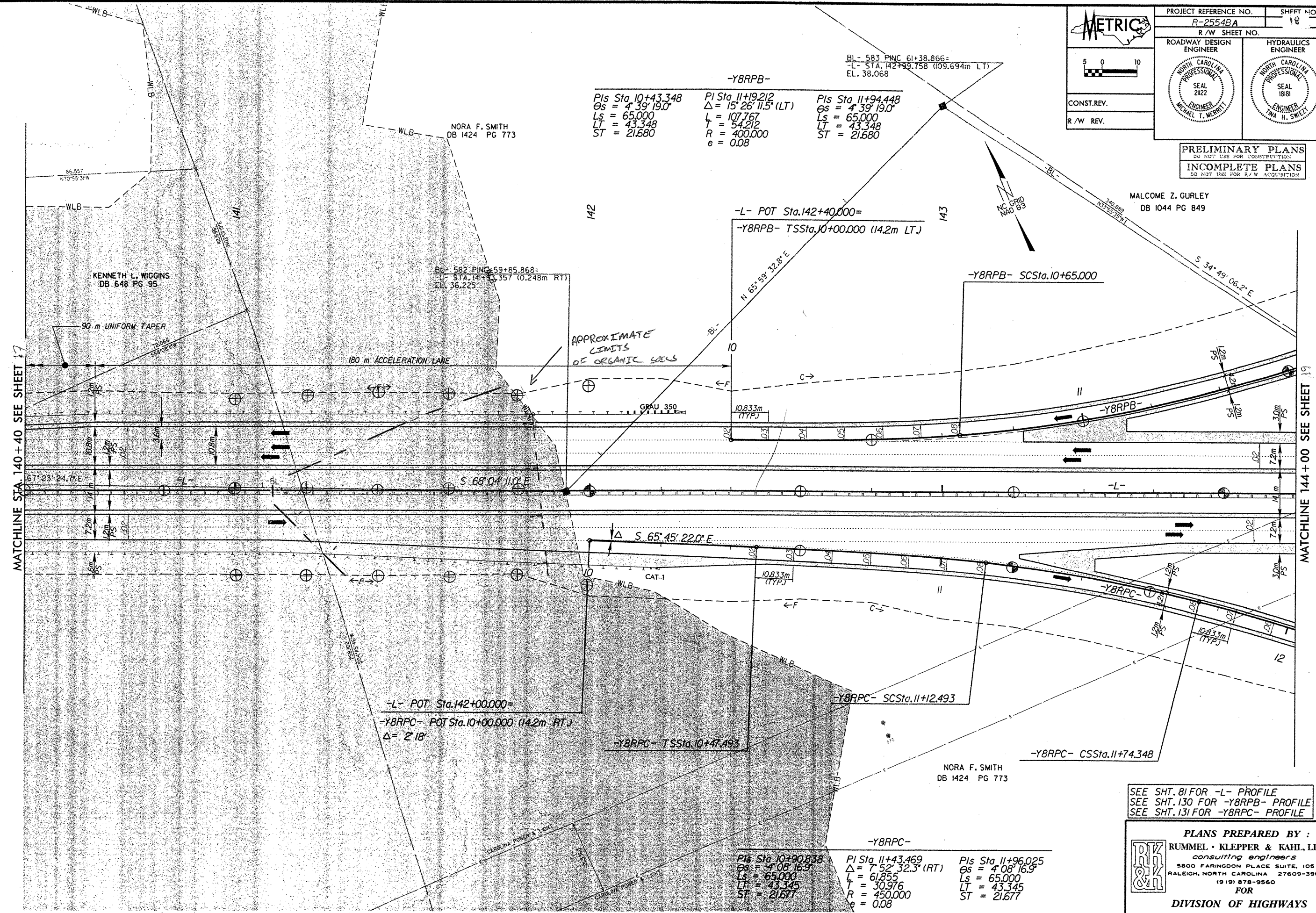
Pls Sta 11+94.448
Os = 4° 39' 19.0"
Ls = 65.000
LT = 43.348
ST = 21.680

NORA F. SMITH
DB 1424 PG 773

KENNETH L. WIGGINS
DB 648 PG 95

BL- 582 PINC 59+85.868=
L- STA. 141+33.357 (0.248m RT)
EL. 36.225

MALCOLM Z. GURLEY
DB 1044 PG 849



REVISIONS

MATCHLINE STA. 140+40 SEE SHEET 17

MATCHLINE 144+00 SEE SHEET 19

-L- POT Sta. 142+00.000=
-Y8RPC- POT Sta. 10+00.000 (14.2m RT)
Δ = 2° 18'

-Y8RPC- TSS Sta. 10+47.493

-Y8RPC- SCSta. 11+12.493

-Y8RPC- CSS Sta. 11+74.348

NORA F. SMITH
DB 1424 PG 773

-Y8RPC-

Pls Sta 10+90.838
Os = 4° 08' 16.9"
Ls = 65.000
LT = 43.345
ST = 21.677

PI Sta 11+43.469
Δ = 7° 52' 32.3" (RT)
L = 61.855
T = 30.976
R = 450.000
e = 0.08

Pls Sta 11+96.025
Os = 4° 08' 16.9"
Ls = 65.000
LT = 43.345
ST = 21.677

SEE SHT. 81 FOR -L- PROFILE
SEE SHT. 130 FOR -Y8RPB- PROFILE
SEE SHT. 131 FOR -Y8RPC- PROFILE

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FOR
DIVISION OF HIGHWAYS

METRIC

PROJECT REFERENCE NO. R-2554BA SHEET NO. 21
 R/W SHEET NO.

ROADWAY DESIGN ENGINEER
 SEAL 2122
 ENGINEER MICHAEL T. WERRILL

HYDRAULICS ENGINEER
 SEAL 1818
 ENGINEER TINA H. SWEET

CONST. REV.
 R/W REV.

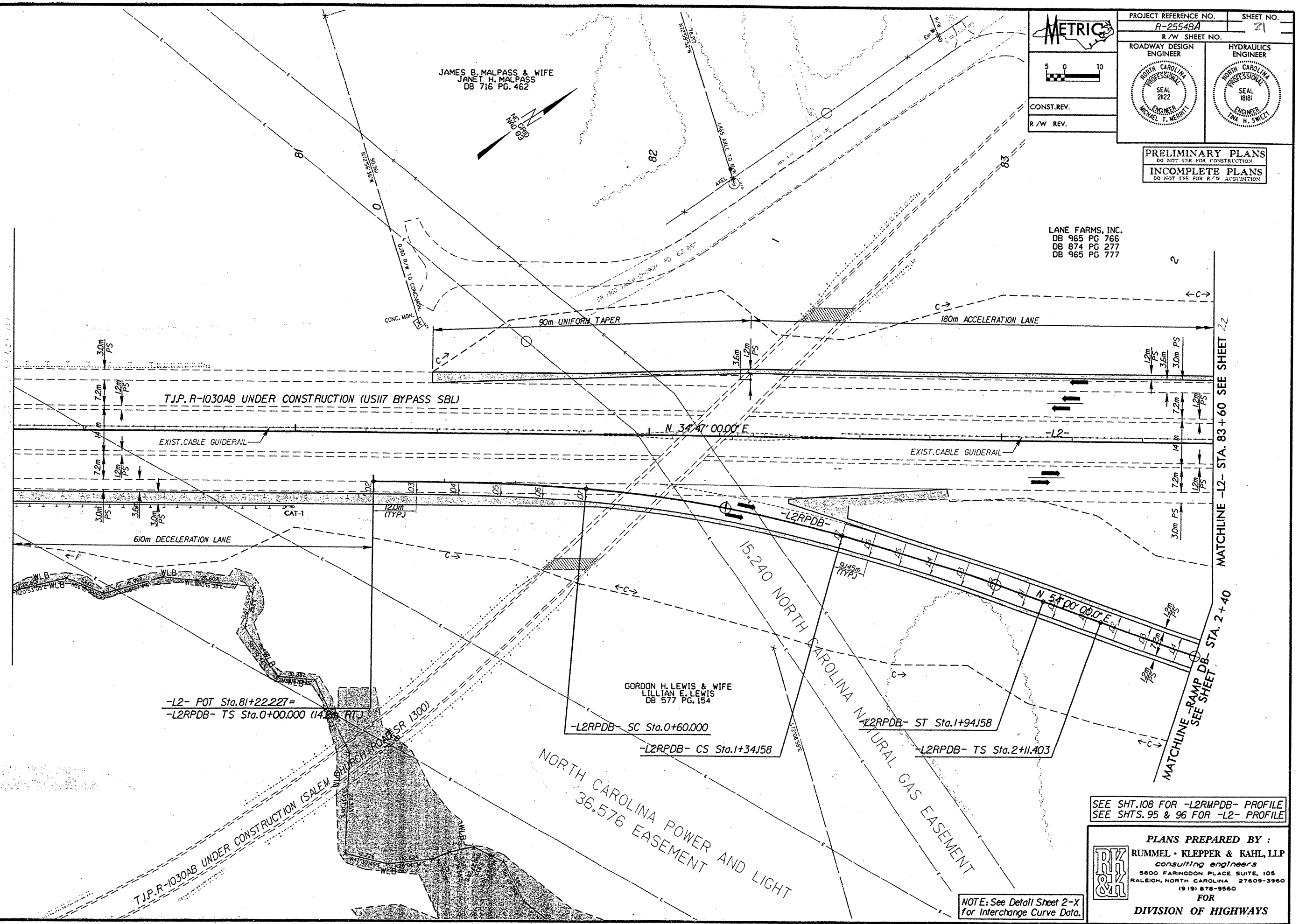
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INCOMPLETE PLANS
 DO NOT USE FOR R/W ACQUISITION

LANE FARMS, INC.
 DB 965 PG 766
 DB 874 PG 277
 DB 965 PG 777

JAMES B. MALPASS & WIFE
 JANET H. MALPASS
 DB 716 PG. 462

GORDON H. LEWIS & WIFE
 LILLIAN E. LEWIS
 DB 577 PG. 154

REVISIONS






SEE SHT. 108 FOR -L2RMPDB- PROFILE
 SEE SHTS. 95 & 96 FOR -L2- PROFILE

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 (919) 878-9560
 FOR
DIVISION OF HIGHWAYS

NOTE: See Detail Sheet 2-X
 for Interchange Curve Data.


MATCHLINE -L2- STA. 83+60 SEE SHEET Z2
 MATCHLINE -RAMP DB- STA. 2+40 SEE SHEET Z1

 5 0 10	PROJECT REFERENCE NO. R-2554BA	SHEET NO. 22
	R/W SHEET NO.	
CONST. REV.	ROADWAY DESIGN ENGINEER 	HYDRAULICS ENGINEER 
R/W REV.		

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INCOMPLETE PLANS
DO NOT USE FOR R/W ACQUISITION



GORDON H. LEWIS & WIFE
LILLIAN E. LEWIS
DB 577 PG. 154

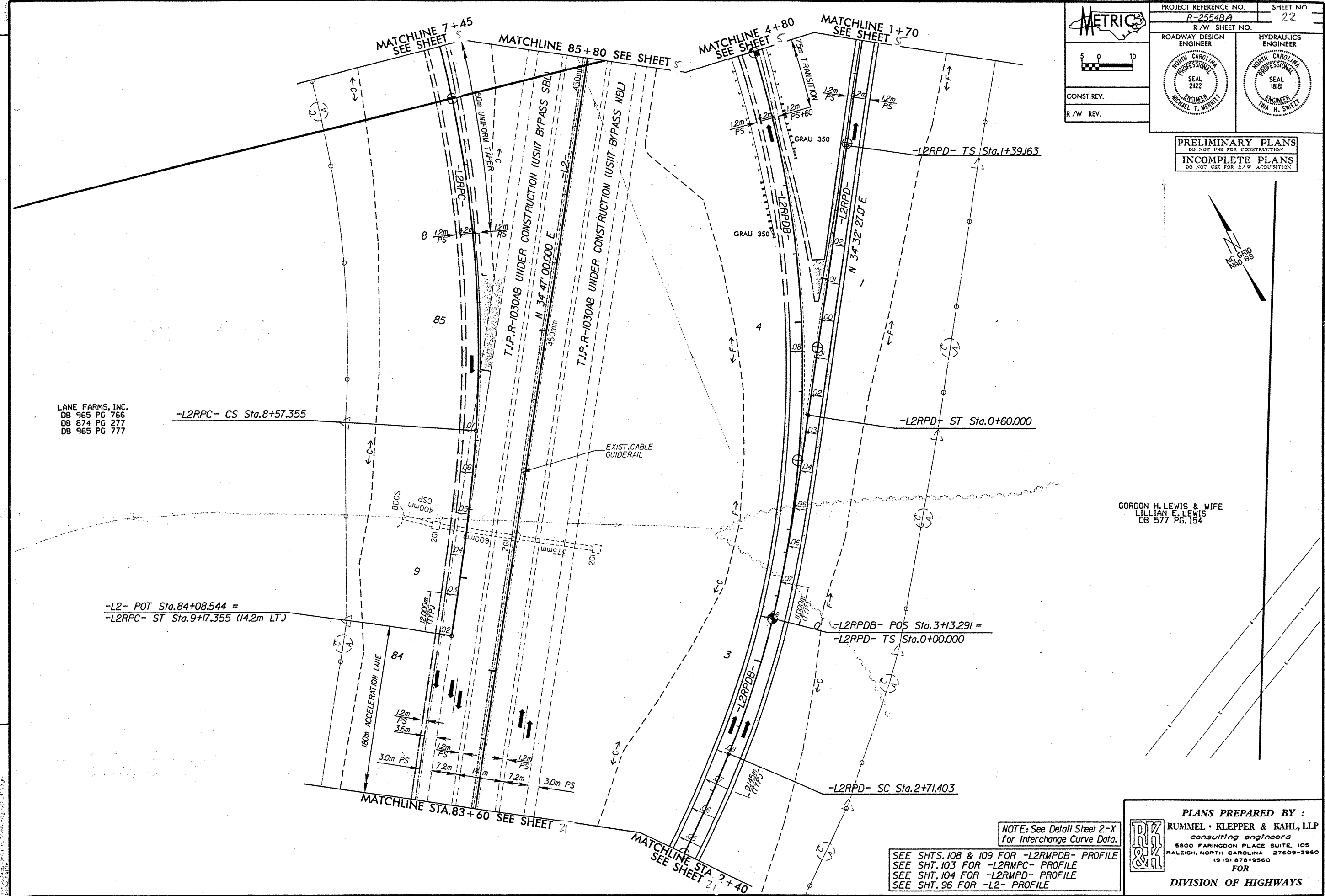
PLANS PREPARED BY :

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consulting engineers
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 FOR
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NOTE: See Detail Sheet 2-X
for Interchange Curve Data.

SEE SHTS. 108 & 109 FOR -L2RMPDB- PROFILE
 SEE SHT. 103 FOR -L2RMPD- PROFILE
 SEE SHT. 104 FOR -L2RMPD- PROFILE
 SEE SHT. 96 FOR -L2- PROFILE

REVISIONS

LANE FARMS, INC.
DB 965 PG 766
DB 874 PG 277
DB 965 PG 777



METRIC

PROJECT REFERENCE NO. R-2554BA SHEET NO. 23
 R/W SHEET NO.

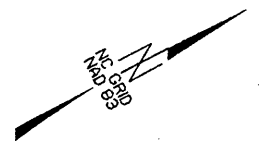
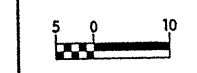
ROADWAY DESIGN ENGINEER
 HYDRAULICS ENGINEER

SEAL 2122
 SEAL 18181

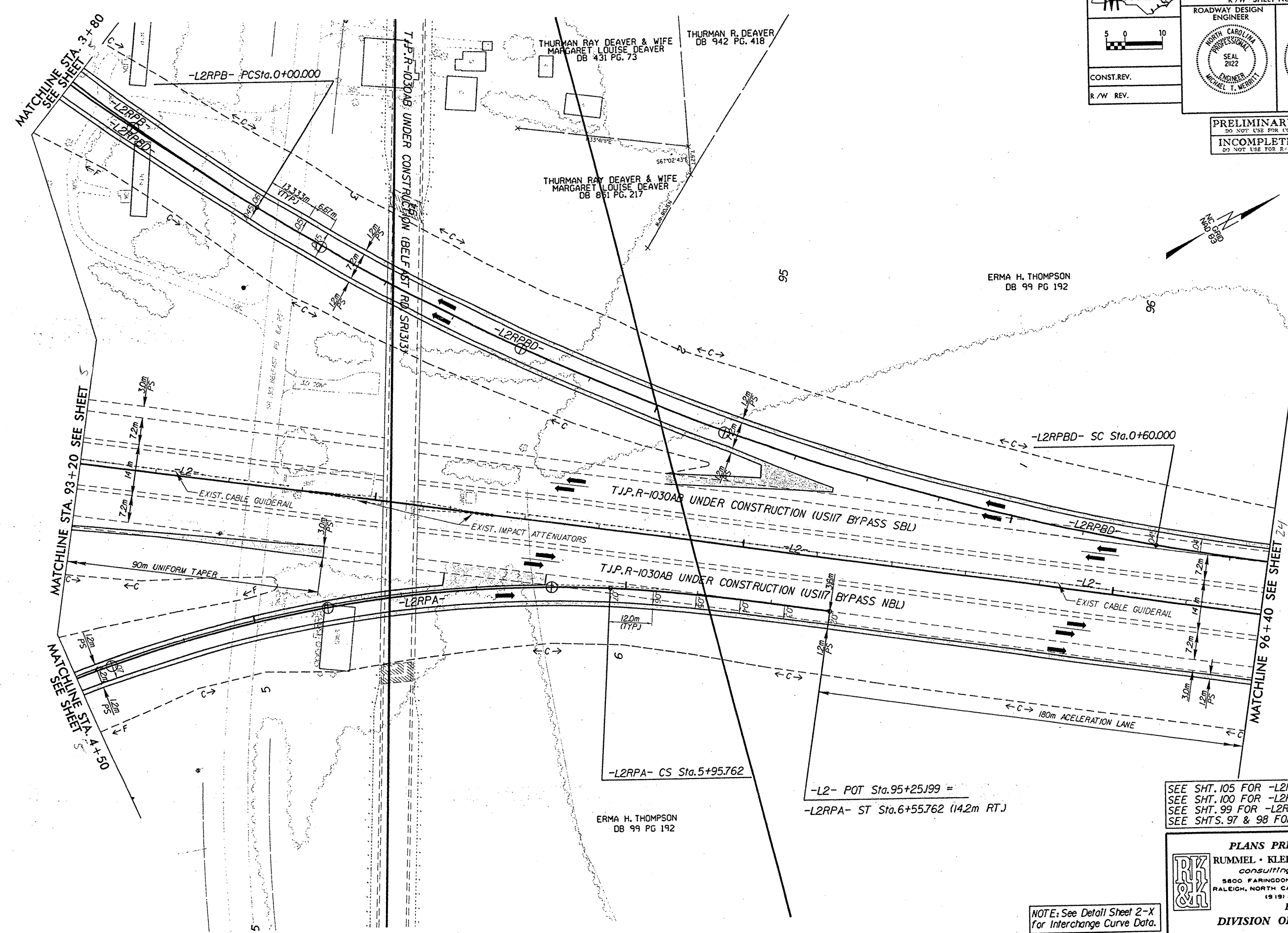
ENGINEER MICHAEL T. MERRILL
 ENGINEER TWA H. SWILEY

CONST. REV.
 R/W REV.

PRELIMINARY PLANS
 DO NOT USE FOR CONSTRUCTION
INCOMPLETE PLANS
 DO NOT USE FOR R/W ACQUISITION



REVISIONS



SEE SHT. 105 FOR -L2RMPBD- PROFILE
 SEE SHT. 100 FOR -L2RMPB- PROFILE
 SEE SHT. 99 FOR -L2RMPA- PROFILE
 SEE SHTS. 97 & 98 FOR -L2- PROFILE

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consulting engineers
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 FOR
DIVISION OF HIGHWAYS

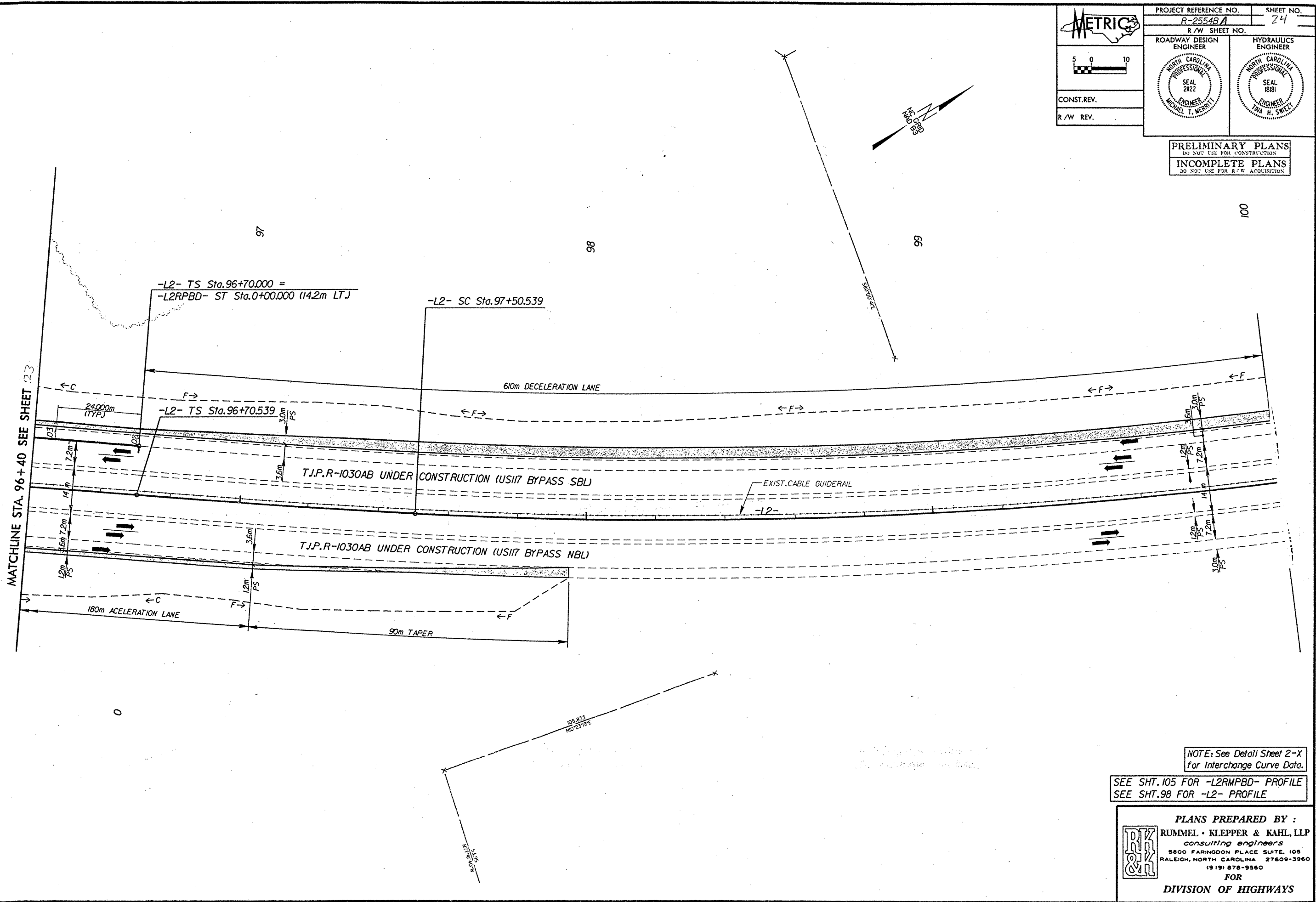
NOTE: See Detail Sheet 2-X
 for Interchange Curve Data.

METRIC

CONST. REV.
R/W REV.

PROJECT REFERENCE NO. R-2554B A	SHEET NO. 24
R/W SHEET NO.	
ROADWAY DESIGN ENGINEER MICHAEL T. MERRITT	HYDRAULICS ENGINEER TINA H. SWIETLY

PRELIMINARY PLANS
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INCOMPLETE PLANS
DO NOT USE FOR R-W ACQUISITION






REVISIONS

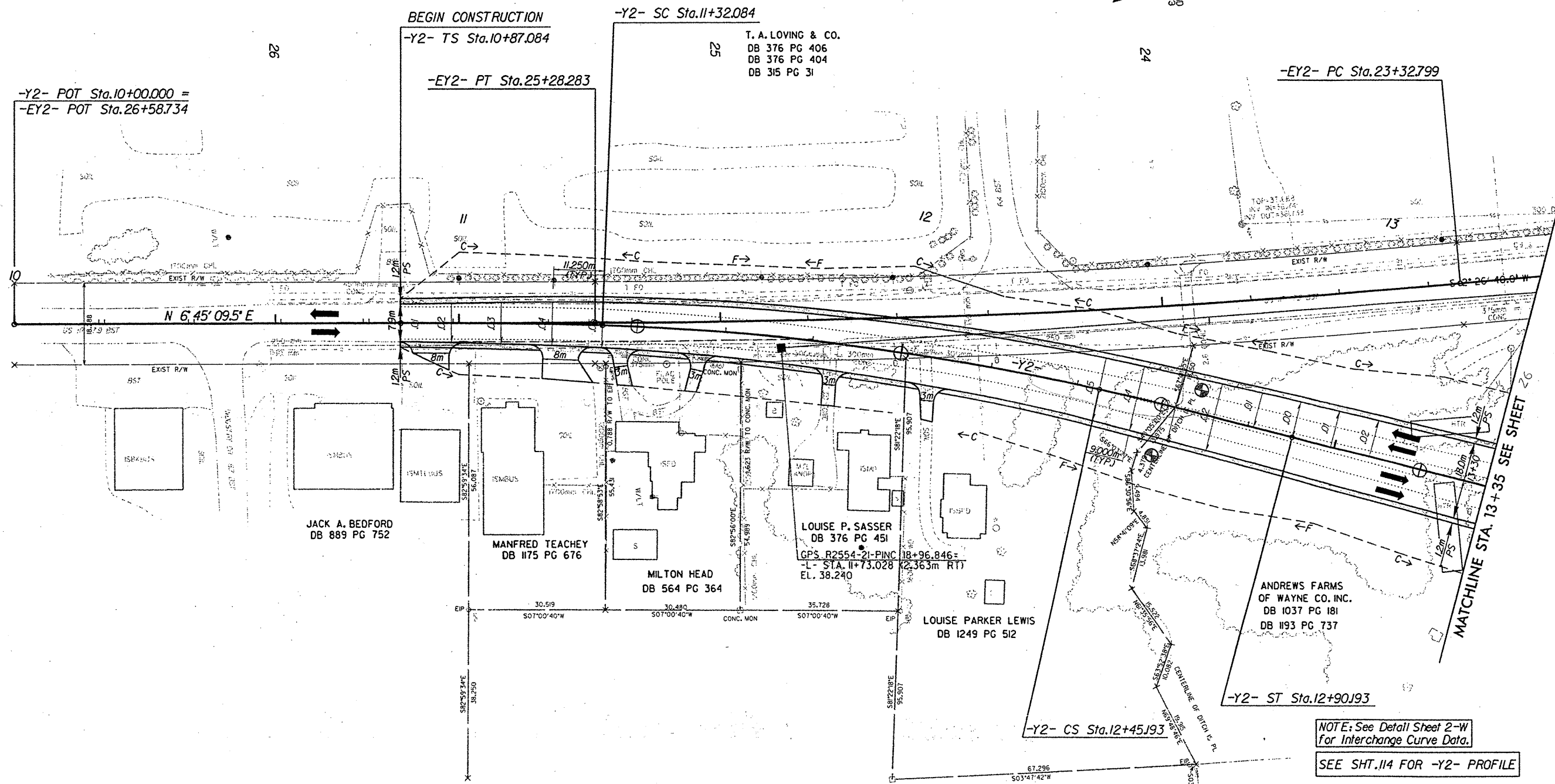
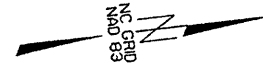
NOTE: See Detail Sheet 2-X for Interchange Curve Data.

SEE SHT. 105 FOR -L2RMPBD- PROFILE
SEE SHT. 98 FOR -L2- PROFILE

PLANS PREPARED BY :
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consulting engineers
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 (919) 878-9560
 FOR
DIVISION OF HIGHWAYS

 5 0 10 CONST. REV. R/W REV.	PROJECT REFERENCE NO. R-25548A	SHEET NO. 25
	R/W SHEET NO.	
ROADWAY DESIGN ENGINEER  MICHAEL T. MERRITT	HYDRAULICS ENGINEER  TWA H. SWITZ	

PRELIMINARY PLANS
DO NOT USE FOR CONSTRUCTION
INCOMPLETE PLANS
DO NOT USE FOR R/W ACQUISITION




REVISIONS

NOTE: 240m Uniform Transition -Y2- Sta. 10+90 (2 lanes) to Sta. 13+30 (5 lanes).

NOTE: See Detail Sheet 2-W for Interchange Curve Data.

SEE SHT. 114 FOR -Y2- PROFILE

PLANS PREPARED BY :



RUMMEL • KLEPPER & KAHL, LLP
consulting engineers
 5800 FARINGDON PLACE SUITE 105
 RALEIGH, NORTH CAROLINA 27609-3960
 (919) 878-9560
 FOR
 DIVISION OF HIGHWAYS

METRIC

PROJECT REFERENCE NO. R-2554BA SHEET NO. 26
 R/W SHEET NO.

ROADWAY DESIGN ENGINEER
 MICHAEL T. METRITZ
 SEAL 2122

HYDRAULICS ENGINEER
 TMA H. SWIETZ
 SEAL 18181

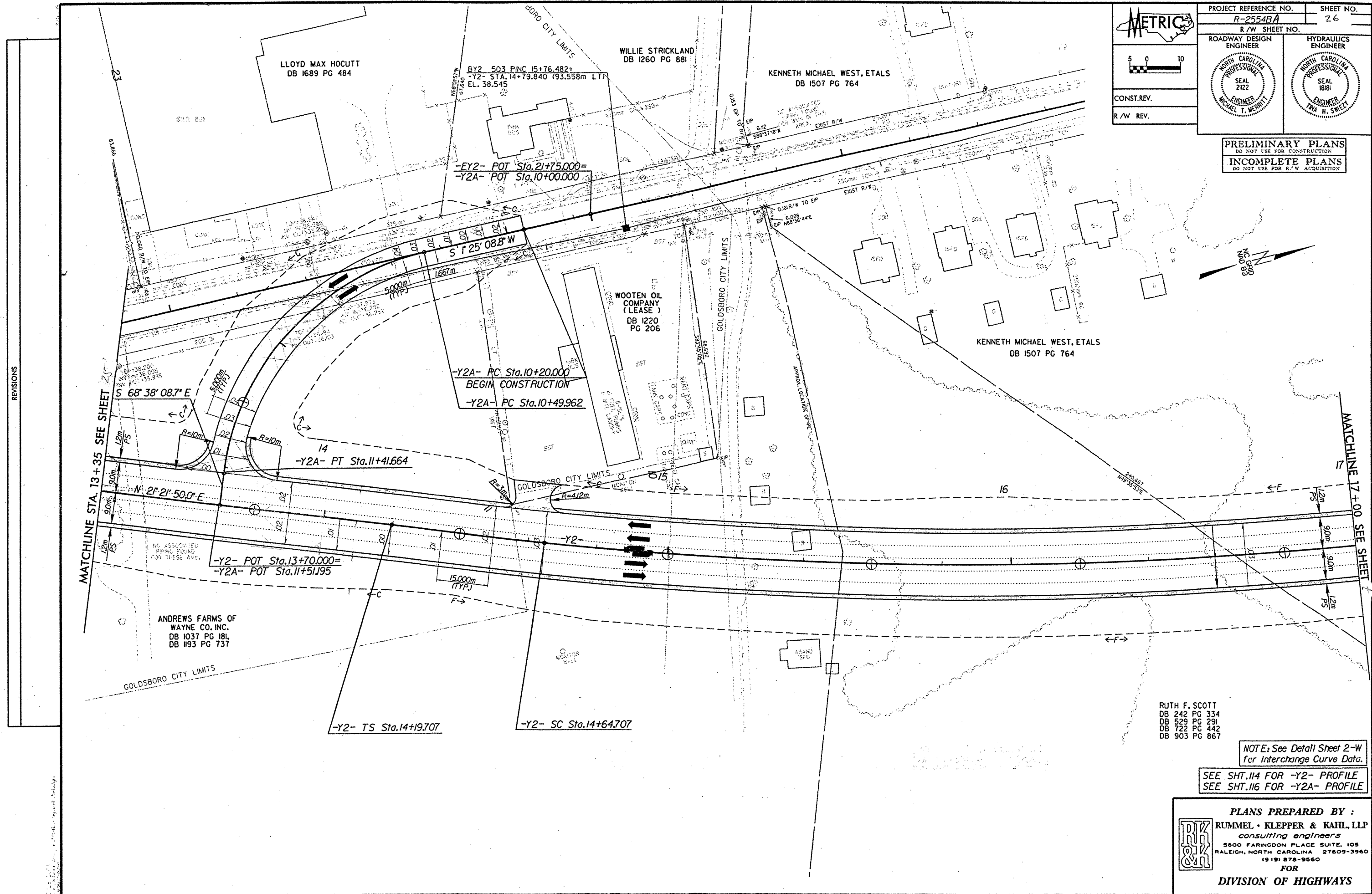
CONST. REV.
 R/W REV.

5 0 10

North Arrow

PRELIMINARY PLANS
 DO NOT USE FOR CONSTRUCTION

INCOMPLETE PLANS
 DO NOT USE FOR R/W ACQUISITION



REVISIONS

MATCHLINE STA. 13+35 SEE SHEET 25

MATCHLINE 17+00 SEE SHEET 27

RUTH F. SCOTT
 DB 242 PG 334
 DB 529 PG 291
 DB 722 PG 442
 DB 903 PG 867

NOTE: See Detail Sheet 2-W for Interchange Curve Data.
 SEE SHT. 114 FOR -Y2- PROFILE
 SEE SHT. 116 FOR -Y2A- PROFILE

PLANS PREPARED BY :

RUMMEL • KLEPPER & KAHL, LLP
 consulting engineers
 5800 FARRINGTON PLACE SUITE 105
 RALEIGH, NORTH CAROLINA 27609-3990
 (919) 878-9560

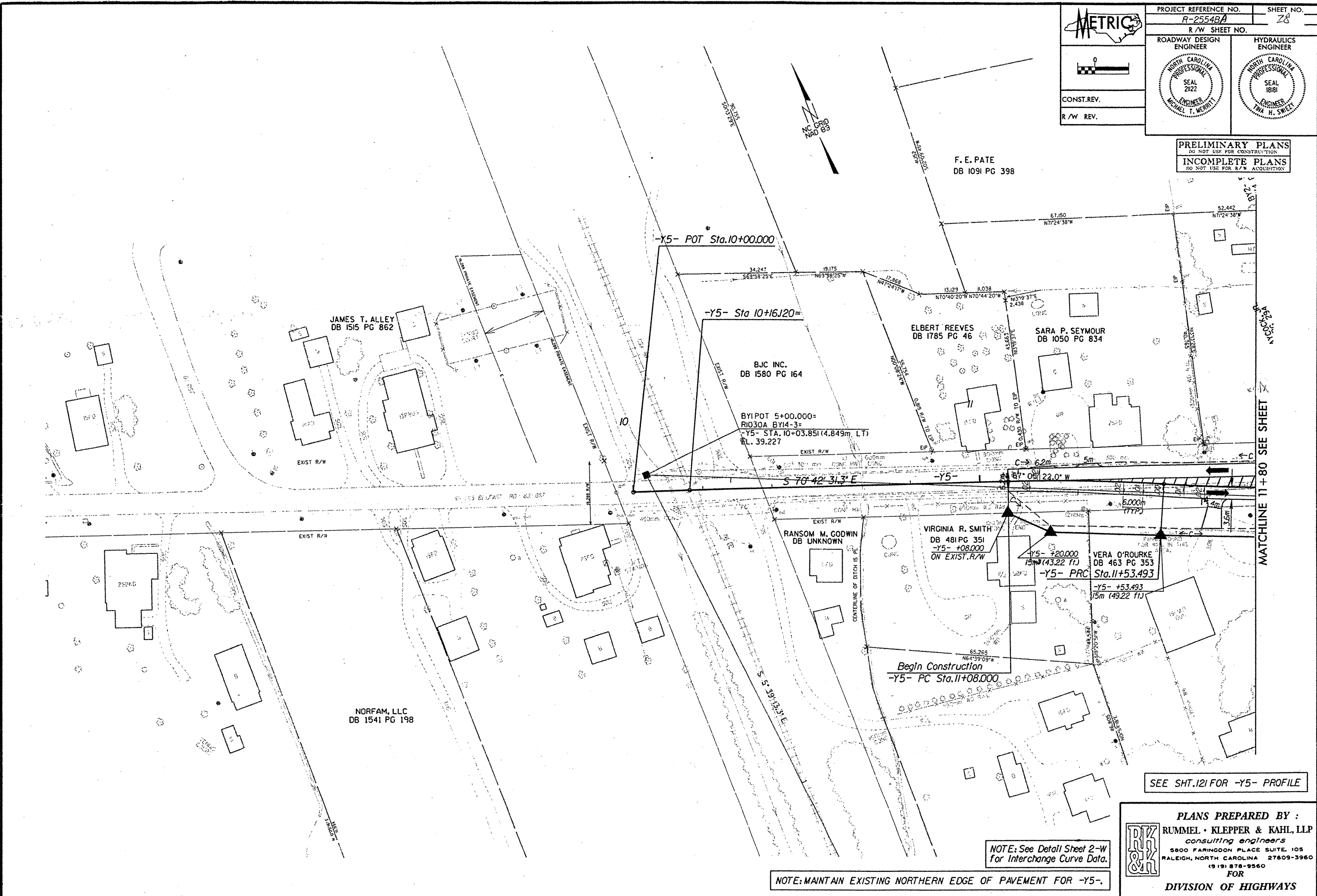
FOR
DIVISION OF HIGHWAYS



PROJECT REFERENCE NO. R-2554BA	SHEET NO. 28
R/W SHEET NO.	
ROADWAY DESIGN ENGINEER	HYDRAULICS ENGINEER
CONST. REV.	
R/W REV.	

PRELIMINARY PLANS
DO NOT USE FOR CONSTRUCTION
INCOMPLETE PLANS
DO NOT USE FOR R/W ACQUISITION

REVISIONS



MATCHLINE 11+80 SEE SHEET 28

SEE SHT. 121 FOR -Y5- PROFILE

NOTE: See Detail Sheet 2-W for Interchange Curve Data.

NOTE: MAINTAIN EXISTING NORTHERN EDGE OF PAVEMENT FOR -Y5-.

PLANS PREPARED BY :

RUMMEL • KLEPPER & KAHL, LLP
consulting engineers
 5800 FARMINGTON PLACE SUITE 105
 RALEIGH, NORTH CAROLINA 27609-3960
 (919) 878-9560
 FOR
DIVISION OF HIGHWAYS

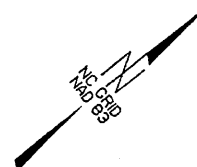
METRIC

CONST. REV.
R/W REV.

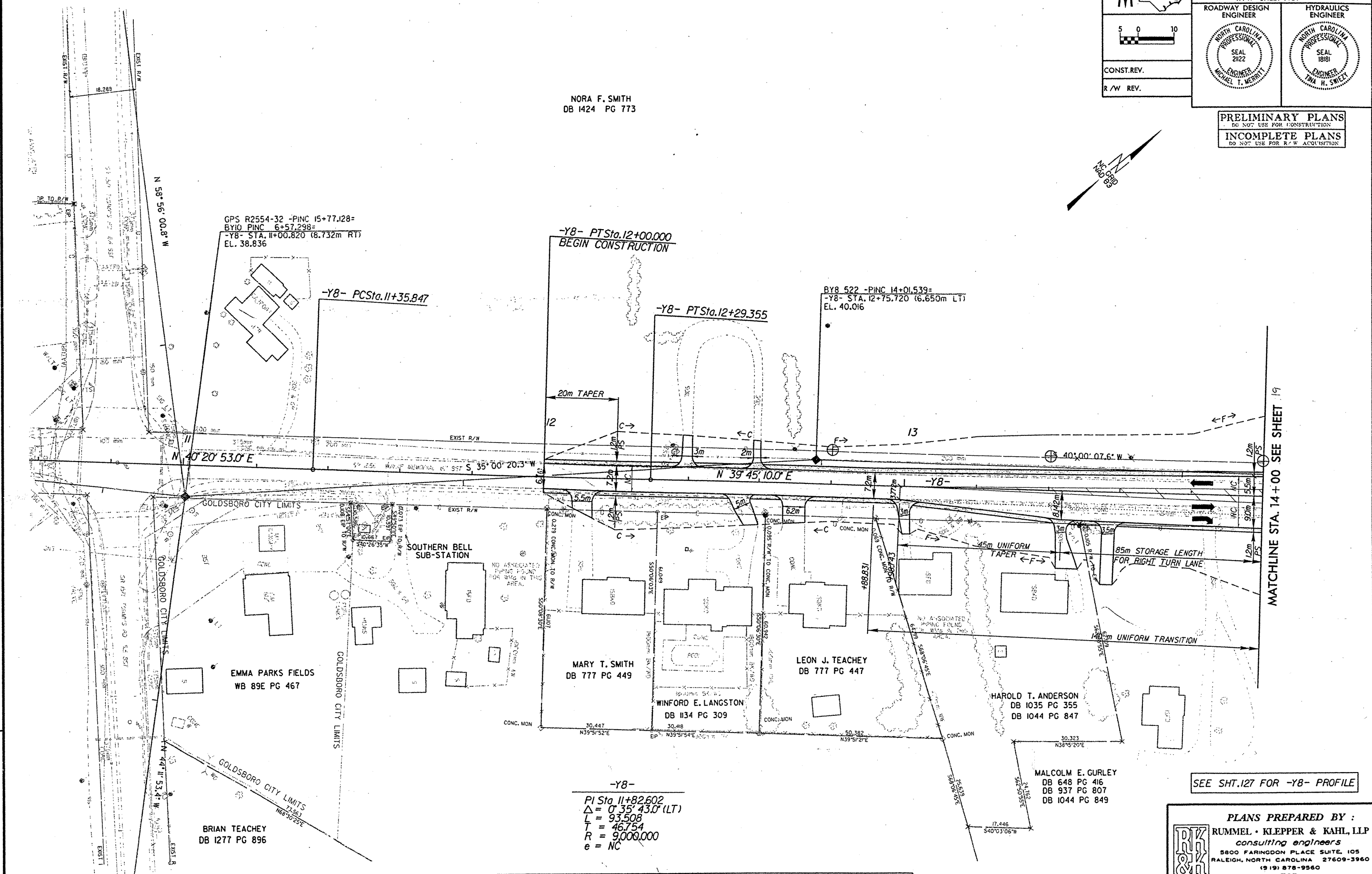
PROJECT REFERENCE NO. R-2554BA	SHEET NO. 29
R/W SHEET NO.	
ROADWAY DESIGN ENGINEER	HYDRAULICS ENGINEER

PRELIMINARY PLANS
DO NOT USE FOR CONSTRUCTION
INCOMPLETE PLANS
DO NOT USE FOR R/W ACQUISITION

NORA F. SMITH
DB 1424 PG 773



REVISIONS



-Y8-
PI Sta 11+82.602
 $\Delta = 0^{\circ} 35' 43.0''$ (LT)
 $L = 93.508$
 $T = 46.754$
 $R = 9,000,000$
 $e = NC$

NOTE: 140m Uniform Transition -Y8- Sta. 12+88.913 (2 Lanes) to Sta. 14+28.831 (3 Lanes).

MATCHLINE STA. 14+00 SEE SHEET 19

SEE SHT. 127 FOR -Y8- PROFILE

PLANS PREPARED BY :
RUMMEL • KLEPPER & KAHL, LLP
consulting engineers
5800 FARINGDON PLACE SUITE 105
RALEIGH, NORTH CAROLINA 27609-3960
(919) 878-9560
FOR
DIVISION OF HIGHWAYS

METRIC

PROJECT REFERENCE NO. R-2554BA SHEET NO. 30
 R/W SHEET NO.

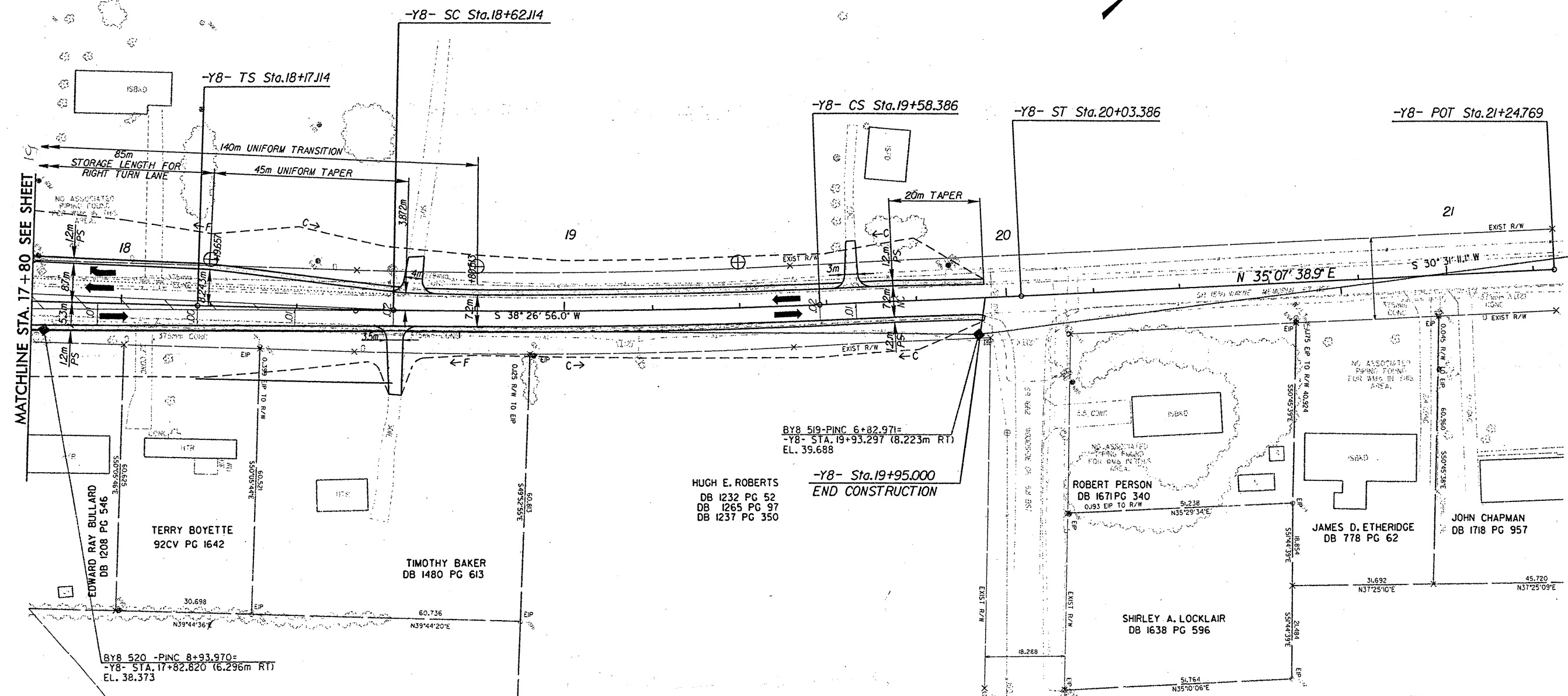
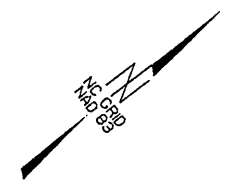
ROADWAY DESIGN ENGINEER
 NORTH CAROLINA PROFESSIONAL SEAL 2122
 MICHAEL T. WENNITZ

HYDRAULICS ENGINEER
 NORTH CAROLINA PROFESSIONAL SEAL 18181
 W. H. SWIFT

CONST. REV.
 R/W REV.

JOAN CRUMPLER JOYNER
 DB 1551 PG 491

PRELIMINARY PLANS
 DO NOT USE FOR CONSTRUCTION
INCOMPLETE PLANS
 DO NOT USE FOR R/W ACQUISITION



REVISIONS

MATCHLINE STA. 17+80 SEE SHEET 19

NOTE: 140m Uniform Transition -Y2- Sta. 17+40.513 (3 Lanes) to Sta. 18+80.513 (2 Lanes).

-Y8-

PIs Sta 18+47.114 Gs = 0'44'12.0" Ls = 45.000 LT = 30.000 ST = 15.000	PI Sta 19+10.262 Δ = 3'09'07.1 (LT) L = 96.272 T = 48.148 R = 1,750.000 e = 0.02	PIs Sta 19+73.386 Gs = 0'44'12.0" Ls = 45.000 LT = 30.000 ST = 15.000
---	---	---

SEE SHTS. 127 & 128 FOR -Y8- PROFILE

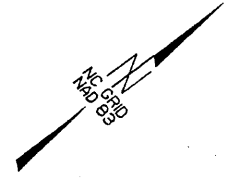
PLANS PREPARED BY :
 RUMMEL • KLEPPER & KAHL, LLP
 consulting engineers
 5800 FARMINGTON PLACE SUITE 105
 RALEIGH, NORTH CAROLINA 27609-3560
 (919) 878-9560
 FOR
 DIVISION OF HIGHWAYS

METRIC

CONST. REV.
R/W REV.

PROJECT REFERENCE NO. R-2554BA	SHEET NO. 31
R/W SHEET NO.	
ROADWAY DESIGN ENGINEER MICHAEL T. MERRITT	HYDRAULICS ENGINEER TINA H. SWETZ

PRELIMINARY PLANS
DO NOT USE FOR CONSTRUCTION
INCOMPLETE PLANS
DO NOT USE FOR R/W ACQUISITION



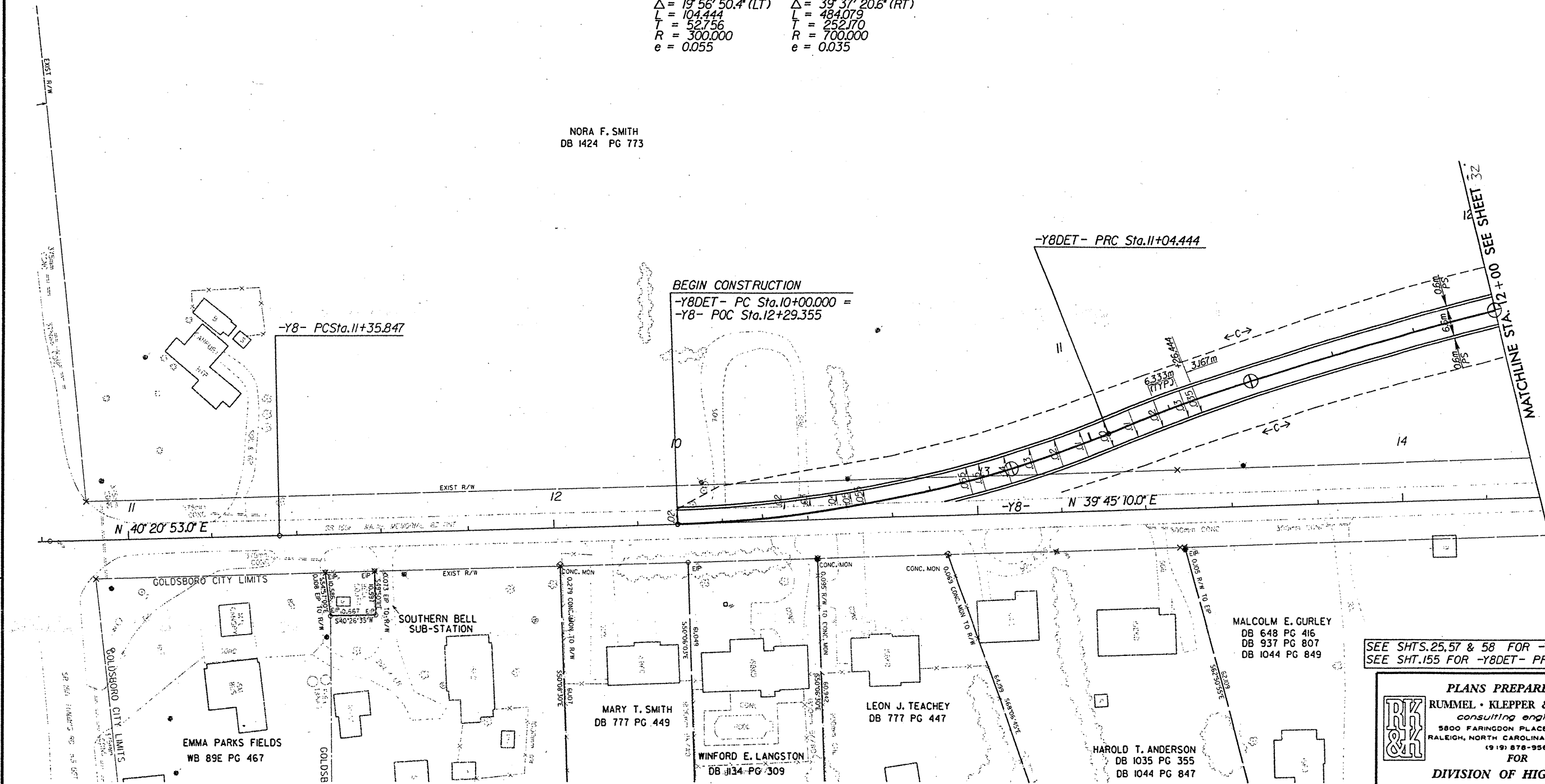
Σ1

-Y8DET-

PI Sta 10+52.756	PI Sta 13+56.614
$\Delta = 19^{\circ} 56' 50.4" (LT)$	$\Delta = 39^{\circ} 37' 20.6" (RT)$
L = 104.444	L = 484.079
T = 52.756	T = 252.170
R = 300.000	R = 700.000
e = 0.055	e = 0.035

NORA F. SMITH
DB 1424 PG 773

REVISIONS



SEE SHTS. 25, 57 & 58 FOR -Y8- DESIGN
SEE SHT. 155 FOR -Y8DET- PROFILE

PLANS PREPARED BY :

RUMMEL • KLEPPER & KAHL, LLP
consulting engineers
5800 FARINGDON PLACE, SUITE 105
RALEIGH, NORTH CAROLINA 27609-3960
(919) 878-9560
FOR
DIVISION OF HIGHWAYS

MALCOLM E. GURLEY
DB 648 PG 416
DB 937 PG 807
DB 1044 PG 849




MARY T. SMITH
DB 777 PG 449

LEON J. TEACHEY
DB 777 PG 447

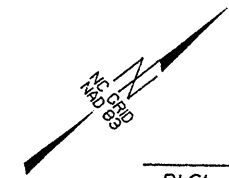
WINFORD E. LANGSTON
DB 1134 PG 309

HAROLD T. ANDERSON
DB 1035 PG 355
DB 1044 PG 847

EMMA PARKS FIELDS
WB 89E PG 467

 5 0 10	PROJECT REFERENCE NO. R-2554B/A	SHEET NO. 32
	R/W SHEET NO.	
CONST. REV.	ROADWAY DESIGN ENGINEER	HYDRAULICS ENGINEER
R/W REV.	 MICHAEL T. MERRITT	 W. H. SWETZ

PRELIMINARY PLANS
DO NOT USE FOR CONSTRUCTION
INCOMPLETE PLANS
DO NOT USE FOR R/W ACQUISITION

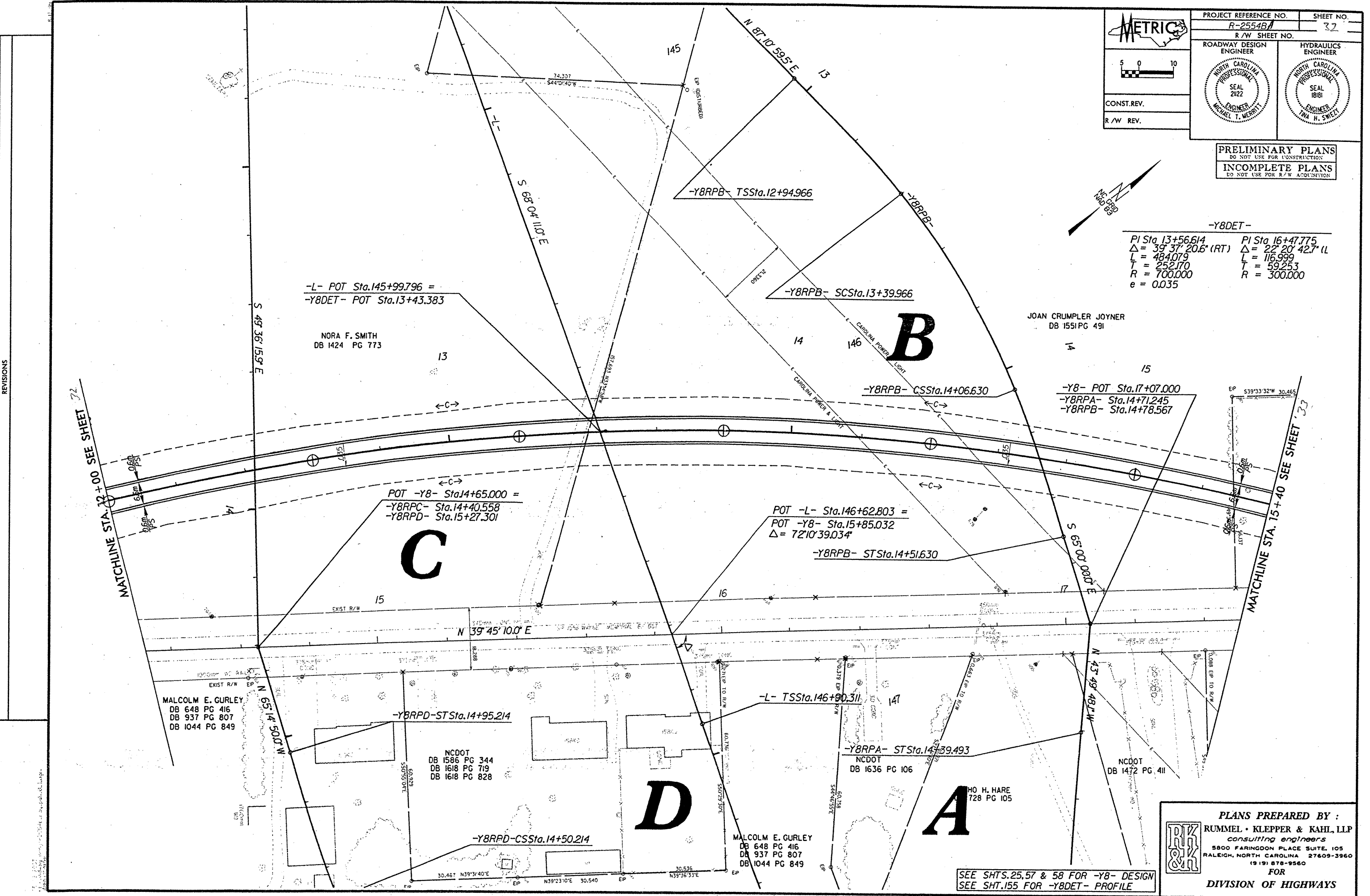


-Y8DET-


PI Sta 13+56.614	PI Sta 16+47.775
$\Delta = 39^{\circ} 37' 20.6''$ (RT)	$\Delta = 22^{\circ} 20' 42.7''$ (L)
L = 484.079	L = 116.999
T = 252.170	T = 59.253
R = 700.000	R = 300.000
e = 0.035	

JOAN CRUMPLER JOYNER
DB 1551 PG 491

-Y8- POT Sta.17+07.000
-Y8RPA- Sta.14+71.245
-Y8RPB- Sta.14+78.567

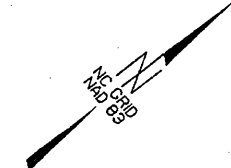


SEE SHTS. 25, 57 & 58 FOR -Y8- DESIGN
SEE SHT. 155 FOR -Y8DET- PROFILE

PLANS PREPARED BY :

RUMMEL · KLEPPER & KAHL, LLP
consulting engineers
 5800 FARRINGTON PLACE SUITE 105
 RALEIGH, NORTH CAROLINA 27609-3960
 (919) 878-9560
 FOR
DIVISION OF HIGHWAYS

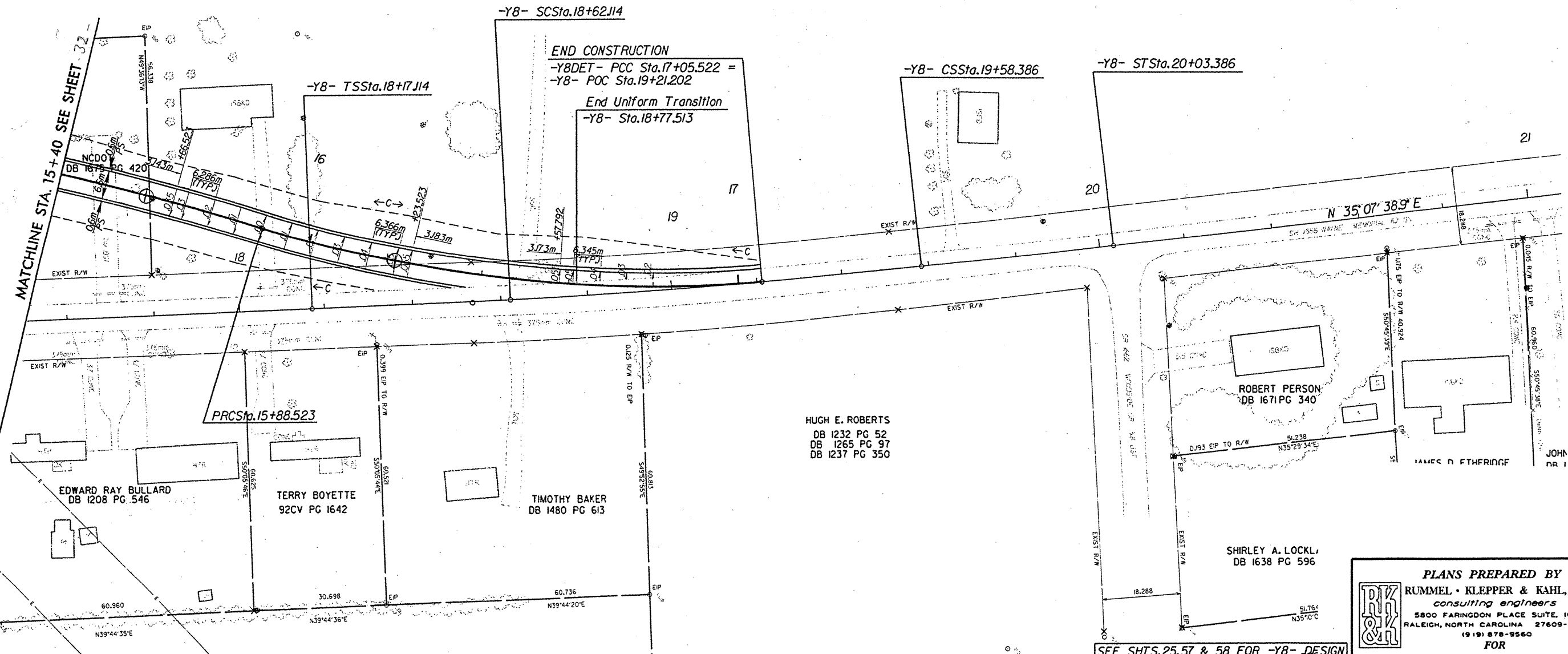
 5 0 10	PROJECT REFERENCE NO. R-2554BA	SHEET NO. 33
	R/W SHEET NO.	
	ROADWAY DESIGN ENGINEER NORTH CAROLINA PROFESSIONAL SEAL 2022 MICHAEL T. MERRITT	HYDRAULICS ENGINEER NORTH CAROLINA PROFESSIONAL SEAL 1818 TINA H. SWIEZ
CONST. REV.		
R/W REV.		

PRELIMINARY PLANS
DO NOT USE FOR CONSTRUCTION
INCOMPLETE PLANS
DO NOT USE FOR R/W ACQUISITION



-Y8DET-
 PI Sta 16+47.775
 $\Delta = 22^\circ 20' 42.7" (LT)$
 $L = 116.999$
 $T = 59.253$
 $R = 300.000$
 $e = 0.055$

JOAN CRUMPLER JOYNER
DB 1551 PG 491

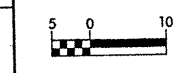


REVISIONS

SEE SHTS. 25, 57 & 58 FOR -Y8- DESIGN
SEE SHT. 155 FOR -Y8DET- PROFILE

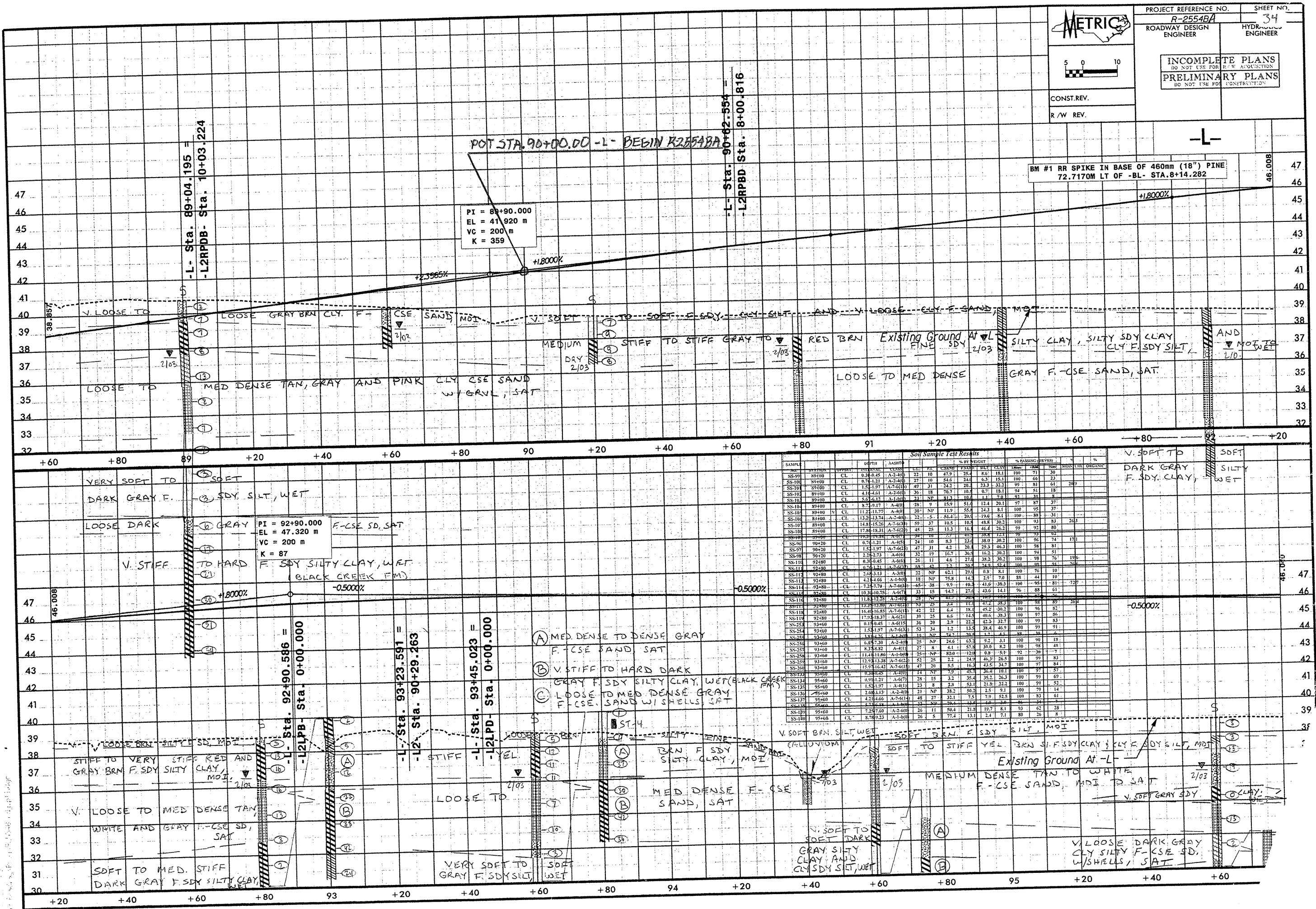
PLANS PREPARED BY :

RUMMEL • KLEPPER & KAHL, LLP
consulting engineers
 5800 FARINGDON PLACE SUITE 105
 RALEIGH, NORTH CAROLINA 27609-3960
 (919) 878-9560
 FOR
 DIVISION OF HIGHWAYS



INCOMPLETE PLANS
 DO NOT USE FOR ACQUISITION
 PRELIMINARY PLANS
 DO NOT USE FOR CONSTRUCTION

CONST. REV.
 R/W REV.



PI = 89+90.000
 EL = 41.920 m
 VC = 200 m
 K = 359

PI = 92+90.000
 EL = 47.320 m
 VC = 200 m
 K = 87

Soil Sample Test Results

SAMPLE NO.	STATION	DEPTH	CLASS	LL	PL	CS	FS	SH	CLAY	W	U	MOI	ORGANIC
SS-99	89+00	CL 0.34-0.45	A-2-4(0)	22	10	47.9	25.2	8.6	18.1	100	71	38	
SS-100	89+00	CL 0.74-1.21	A-2-4(0)	27	10	54.6	24.1	6.3	15.1	100	66	23	
SS-101	89+00	CL 1.54-1.97	A-2-4(0)	29	31	24.2	20.1	23.3	32.2	99	81	64	200
SS-102	89+00	CL 4.14-4.61	A-2-4(0)	36	18	70.7	10.2	0.7	18.1	94	51	18	
SS-103	89+00	CL 5.6-6.13	A-1-1(0)	23	NP	81.3	10.0	1.1	7.0	92	30	8	
SS-104	89+00	CL 8.7-9.17	A-4(0)	28	9	15.9	51.0	12.4	20.1	92	83	37	
SS-105	89+00	CL 11.2-11.77	A-4(0)	20	NP	11.9	55.4	24.1	8.1	100	95	37	
SS-106	89+00	CL 12.2-13.24	A-2-4(0)	22	5	54.3	20.1	19.6	8.1	100	88	31	
SS-107	89+00	CL 14.8-15.26	A-7-6(3)	59	37	10.5	10.5	48.8	30.2	100	93	83	203
SS-108	89+00	CL 17.84-18.31	A-7-6(2)	45	25	11.3	16.1	46.4	26.2	99	92	80	
SS-109	89+00	CL 19.2-19.34	A-8(0)	24	10	7.7	41.8	38.8	12.1	96	92	83	
SS-96	90+20	CL 0.74-1.21	A-1-1(0)	24	10	8.3	22.4	38.0	30.2	100	96	74	171
SS-97	90+20	CL 1.54-1.97	A-7-6(2)	47	11	4.2	20.1	29.3	46.3	100	98	81	
SS-98	90+20	CL 2.28-2.73	A-4-6(2)	32	19	16.7	36.9	16.2	30.2	100	94	51	
SS-110	92+80	CL 0.34-0.45	A-6(0)	26	11	4.6	27.0	38.2	30.2	100	98	76	196
SS-111	92+80	CL 0.74-1.21	A-7-6(3)	28	42	7.7	20.4	34.9	42.4	100	90	34	266
SS-112	92+80	CL 2.68-3.13	A-3(0)	22	NP	62.1	29.0	0.8	8.1	100	76	10	
SS-113	92+80	CL 4.2-4.66	A-1-1(0)	18	NP	75.8	14.7	2.7	7.0	88	44	10	
SS-114	92+80	CL 7.7-7.79	A-7-6(3)	45	38	9.9	49.3	41.6	38.3	100	95	81	727
SS-115	92+80	CL 10.38-10.75	A-6-7(3)	33	15	14.7	27.0	43.6	14.1	96	88	61	
SS-116	92+80	CL 11.8-12.25	A-2-4(0)	28	NP	41.7	20.4	10.9	16.6	100	84	26	
SS-117	92+80	CL 13.2-13.20	A-7-6(2)	53	25	3.4	11.1	47.2	38.3	100	95	89	204
SS-118	92+80	CL 16.44-16.85	A-7-6(1)	45	22	6.4	18.1	45.2	30.2	100	96	82	
SS-119	92+80	CL 17.2-18.37	A-6-7(1)	39	25	6.6	14.5	40.6	38.3	100	97	86	
SS-120	93+60	CL 0.11-0.45	A-6(1)	36	20	2.9	22.2	42.3	32.7	100	99	83	
SS-121	93+60	CL 1.54-1.97	A-7-6(3)	53	34	1.2	13.5	38.4	46.9	100	99	91	
SS-122	93+60	CL 1.84-1.26	A-1-1(0)	19	NP	74.7	20.6	1.7	4.1	88	30	18	
SS-123	93+60	CL 6.85-7.30	A-2-4(0)	25	NP	24.6	63.2	9.2	3.1	100	90	18	
SS-124	93+60	CL 8.37-8.82	A-4(1)	27	NP	4.1	57.8	30.0	8.2	100	98	48	
SS-125	93+60	CL 11.8-13.86	A-1-1(0)	25	NP	82.0	12.0	0.8	5.7	92	30	7	
SS-126	93+60	CL 12.9-13.38	A-7-6(2)	62	26	2.2	24.9	46.3	26.5	100	99	83	
SS-127	93+60	CL 15.9-16.43	A-7-6(1)	47	20	5.5	16.3	43.5	34.7	100	97	84	
SS-128	95+60	CL 0.34-0.45	A-4(0)	14	NP	7.9	45.4	36.4	10.1	100	99	57	
SS-129	95+60	CL 0.91-1.21	A-6(7)	28	15	3.2	35.4	35.2	36.2	100	99	52	
SS-130	95+60	CL 1.54-1.97	A-3(1)	23	8	2.5	53.1	21.3	32.2	100	99	14	
SS-131	95+60	CL 2.68-3.13	A-2-4(0)	21	NP	38.2	50.3	2.5	9.1	100	79	14	
SS-132	95+60	CL 4.2-4.66	A-7-6(1)	48	27	32.1	7.5	7.9	52.5	100	83	61	
SS-133	95+60	CL 6.28-6.18	A-1-1(0)	33	NP	70.7	13.4	4.0	3.0	86	34	8	
SS-134	95+60	CL 7.24-7.60	A-2-4(0)	26	11	50.4	21.8	19.7	8.1	93	62	28	
SS-135	95+60	CL 8.78-9.23	A-1-1(0)	26	5	77.4	13.1	2.4	7.1	80	26	8	

- (A) MED DENSE TO DENSE GRAY F-CSE SAND, SAT
- (B) V. STIFF TO HARD DARK GRAY F. SDY SILTY CLAY, WET (BLACK CREEK FM)
- (C) LOOSE TO MED DENSE GRAY F-CSE SAND W/ SHELLS, SAT

V. LOOSE TO MED DENSE TAN, WHITE AND GRAY F-CSE SD, SAT

V. LOOSE TO MED DENSE TAN, WHITE AND GRAY F-CSE SD, SAT

Soil Sample Test Results														
SAMPLE NO.	STATION	DEPTH	CLASS.	LL	PL	USAND	FSAND	SLT	CLAY	% PASSING	NO. 20	NO. 40	NO. 60	NO. 100
SS-194	98+20	CL	0.30-0.45	A-4(2)	21	8	17.2	31.0	26.7	24.3	100	89	58	
SS-195	98+20	CL	0.76-1.51	A-6(8)	31	17	13.0	28.7	25.1	32.3	100	92	64	
SS-196	98+20	CL	2.68-3.13	A-2-4(0)	28	7	77.0	11.0	0.9	11.1	97	44	13	
SS-197	98+20	CL	5.73-6.28	A-1-1(0)	23	NP	77.1	18.4	1.5	3.0	85	34	5	
SS-198	98+20	CL	8.25-8.53	A-2-4(0)	28	NP	66.4	8.7	0.5	4.0	99	61	17	
SS-199	98+20	CL	10.30-11.75	A-1-1(0)	24	NP	78.5	12.7	2.7	6.1	79	36	8	
SS-200	98+20	CL	13.35-14.80	A-7-6(36)	36	35	1.8	8.7	41.0	48.5	100	99	93	
SS-232	100+40	CL	0.15-0.45	A-2-7(4)	73	45	69.6	5.2	1.7	23.5	97	36	25	
SS-233	100+40	CL	1.52-1.97	A-4(1)	22	0	89.3	21.2	18.1	31.4	100	70	44	
SS-234	100+40	CL	3.80-4.25	A-4(0)	19	NP	92.6	19.5	2.8	8.2	93	64	37	
SS-235	100+40	CL	6.83-7.28	A-2-4(0)	25	NP	15.9	56.4	16.6	11.2	100	94	23	
SS-236	100+40	CL	9.88-10.33	A-4(0)	22	4	18.9	46.3	22.6	12.3	100	94	36	
SS-237	100+40	CL	12.92-13.37	A-2-4(0)	22	NP	26.9	55.9	4.9	12.3	83	69	15	
SS-238	100+40	CL	14.45-14.90	A-7-6(27)	30	27	2.2	9.8	34.9	53.1	100	99	92	
S-176	101+20	20M LT	0.36-1.70	A-7-6(10)	49	32	36.8	20.0	14.9	28.3	99	78	46	
S-177	101+20	20M LT	1.98-2.43	A-2-4(0)	24	NP	11.7	67.7	8.5	12.1	99	96	22	
S-178	101+20	20M LT	2.46-2.91	A-2-4(0)	27	7	81.0	9.8	3.1	6.1	85	32	8	
S-179	101+20	20M LT	5.79-7.30	A-2-4(0)	21	NP	11.7	67.7	8.5	12.1	99	96	22	
S-180	101+20	20M LT	7.30-11.31	A-2-4(0)	21	NP	5.3	74.5	10.1	10.1	100	99	21	
S-181	101+20	20M LT	11.31-12.00	A-6(4)	36	18	3.2	81.5	25.1	20.2	96	95	50	
SS-471	102+20	22M LT	0.75-1.10	A-6(6)	30	17	19.8	31.4	29.8	28.1	100	90	57	
SS-472	102+20	22M LT	2.58-3.03	A-4(0)	24	4	7.9	63.8	12.1	16.2	100	96	37	
SS-473	102+20	22M LT	5.58-6.03	A-1-1(0)	21	NP	80.9	13.1	2.9	3.0	76	24	6	
SS-474	102+20	22M LT	7.08-7.53	A-2-4(0)	20	NP	20.3	53.1	9.5	8.1	100	92	19	
SS-475	102+20	22M LT	11.58-12.03	A-2-4(0)	27	NP	19.8	69.9	6.3	4.0	100	93	12	
SS-476	102+20	22M LT	16.08-16.53	A-2-4(0)	23	NP	12.2	72.8	7.9	7.1	96	94	17	

METRIC
 ROADWAY DESIGN ENGINEER
 HYDRAULICS ENGINEER

PROJECT REFERENCE NO. **R-25548A**
 SHEET NO. **35**

5 0 10

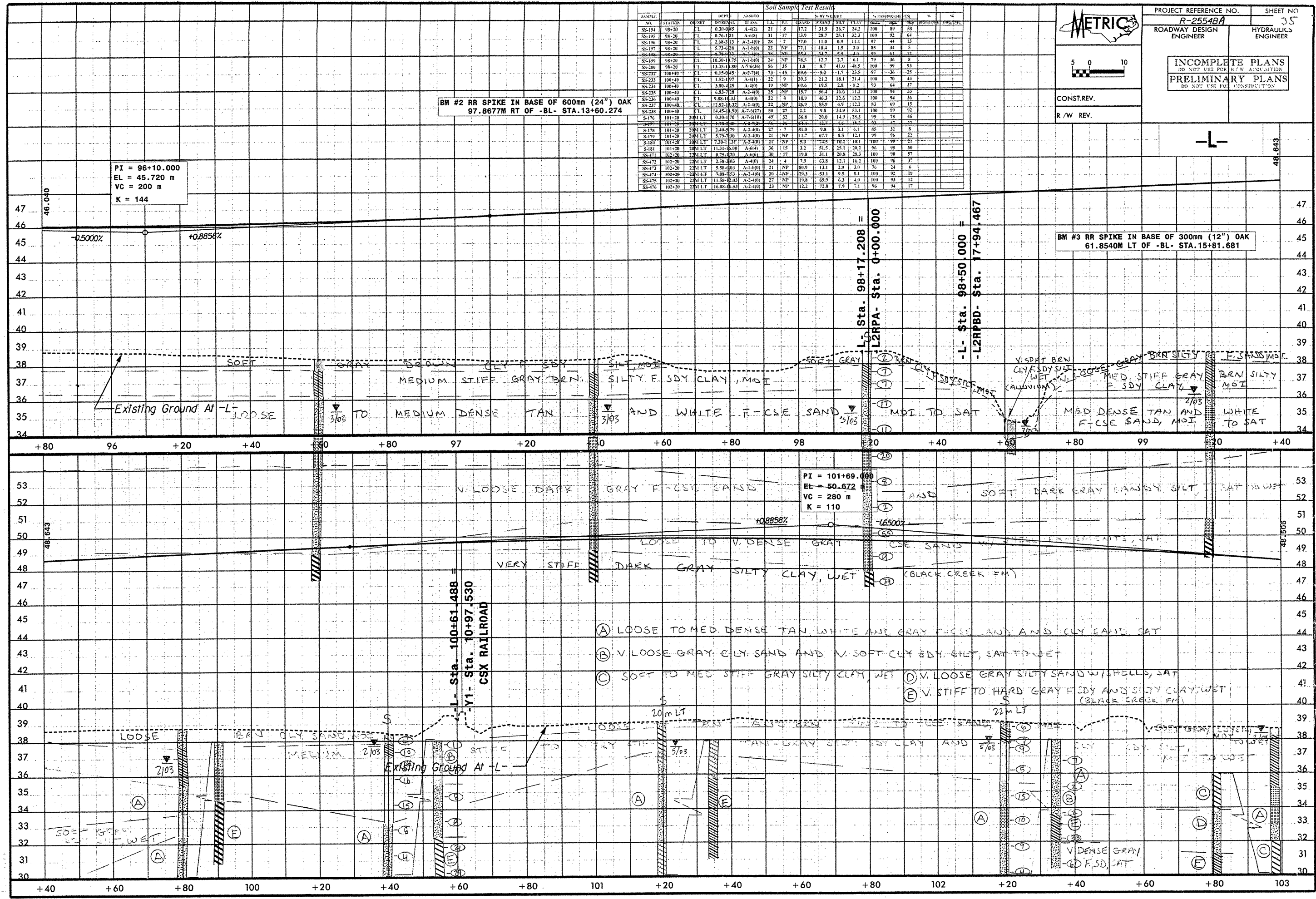
CONST. REV.
 R/W REV.

INCOMPLETE PLANS
 DO NOT USE FOR R/W ACQUISITION
 PRELIMINARY PLANS
 DO NOT USE FOR CONSTRUCTION

BM #2 RR SPIKE IN BASE OF 600mm (24") OAK
 97.8677M RT OF -BL- STA.13+60.274

PI = 96+10.000
 EL = 45.720 m
 VC = 200 m
 K = 144

BM #3 RR SPIKE IN BASE OF 300mm (12") OAK
 61.8540M LT OF -BL- STA.15+81.681

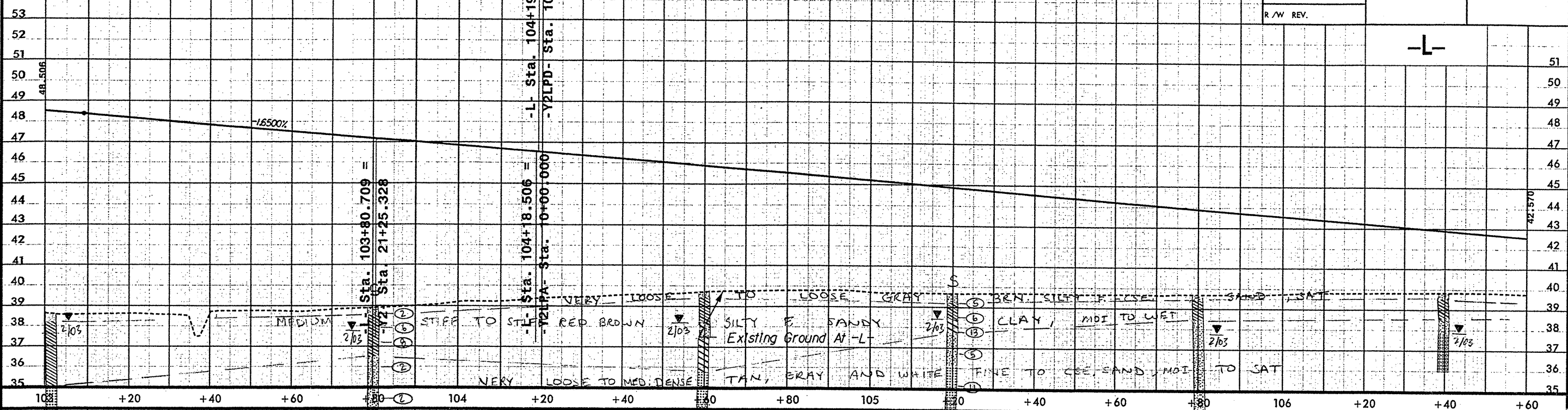




INCOMPLETE PLANS
 DO NOT USE FOR R/W ACQUISITION
 PRELIMINARY PLANS
 DO NOT USE FOR CONSTRUCTION

CONST. REV.
 R/W REV.

-L-



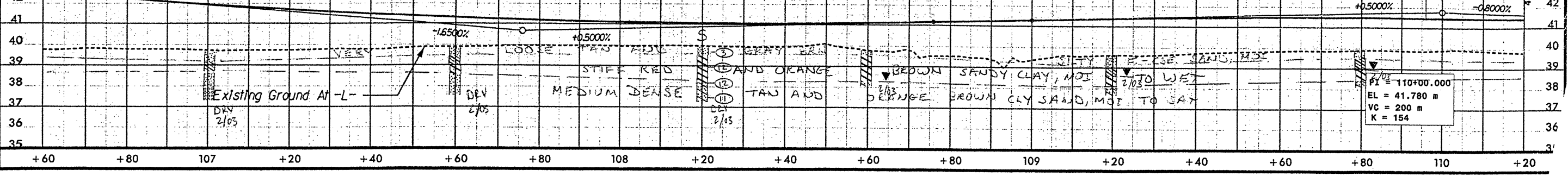
Soil Sample Test Results

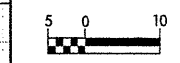
SAMPLE NO.	STATION	DEPTH (M)	CLASS	LL	PL	% BY WEIGHT				% PASSING (SIEVES)			% MOISTURE	% ORGANIC
						CLAY	FINE SAND	SILT	COARSE SAND	75µm	475µm	2mm		
SS 81	103+80	CL	A-2(4)	18	NP	25.1	48.5	17.0	8.1	100	85	2		
SS 83	103+80	CL	A-2(4)	32	16	22.6	36.6	14.5	26.3	100	85	4		
SS 84	103+80	CL	A-2(4)	28	7	52.3	31.0	2.5	14.2	94	54	1	11.2	
SS 87	103+80	CL	A-2(4)	24	NP	20.7	62	0.1	3.0	95	32	3		
SS 88	103+80	CL	A-2(4)	24	NP	18.1	45.9	8.3	31.1	100	97	4		
SS 89	103+80	CL	A-2(4)	21	NP	22.0	55.6	5.3	17.1	81	71	1		
SS 90	103+80	CL	A-2(4)	24	NP	38.1	53.7	5.2	3.0	100	86	1		
SS 91	103+80	CL	A-2(4)	24	NP	22.4	38.4	20.3	18.9	100	98	4		
SS 77	105+20	CL	A-2(4)	33	17	22.5	33.5	14.3	29.7	100	86	5		
SS 78	105+20	CL	A-2(4)	30	31	12.1	34.9	12.5	40.4	100	95	5		
SS 79	105+20	CL	A-2(4)	29	6	20.4	59.0	4.4	16.2	100	92	2		
SS 80	105+20	CL	A-2(4)	17	NP	62.0	20.5	2.4	5.1	100	73	2		
SS 82	105+20	CL	A-2(4)	24	NP	54.1	28	1.9	5.1	95	56	2		
SS 59	108+20	CL	A-2(4)	16	NP	32.7	43.9	13.3	10.1	100	80	5		
SS 60	108+20	CL	A-2(4)	59	33	29.5	20.3	9.7	40.5	97	76	5		
SS 61	108+20	CL	A-2(4)	34	14	40.8	21.2	2.1	16.0	100	64	1		

BM #4 RR SPIKE IN BASE OF 460mm (18") POPLAR
 90.9079 RT OF -BL- STA. 25+29.607

PI = 107+75.810
 EL = 40.659 m
 VC = 200 m
 K = 93

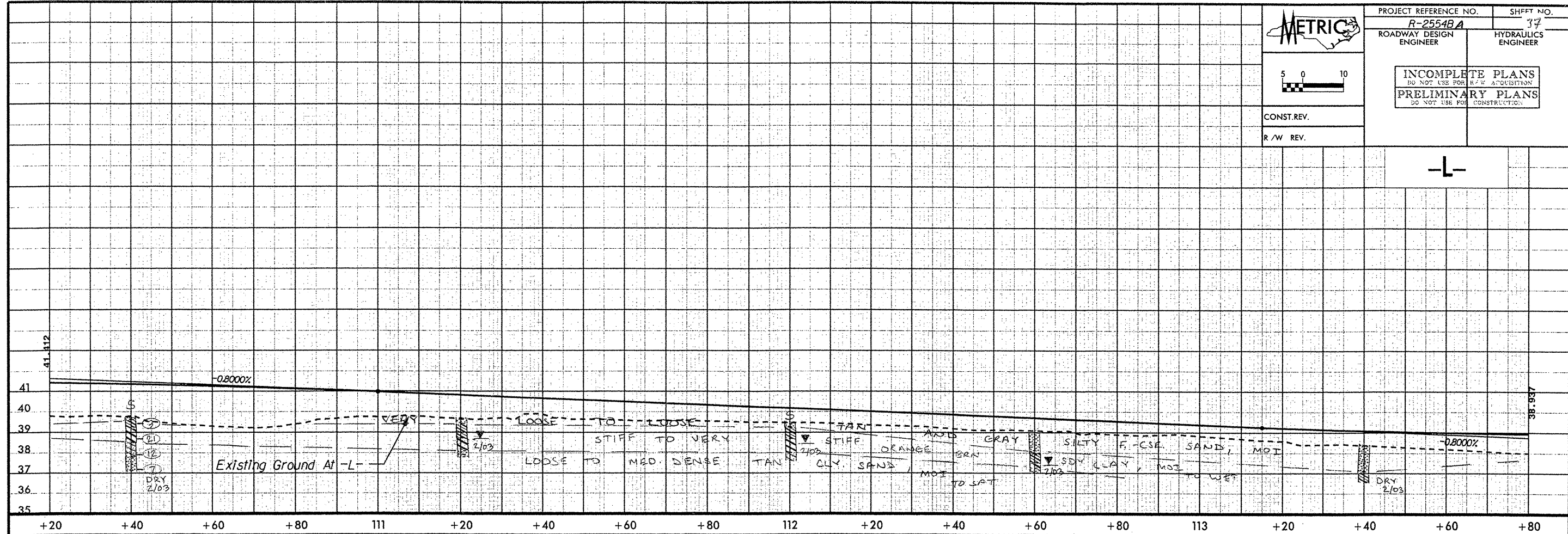
PI = 110+00.000
 EL = 41.780 m
 VC = 200 m
 K = 154





CONST. REV.
 R/W REV.

-L-



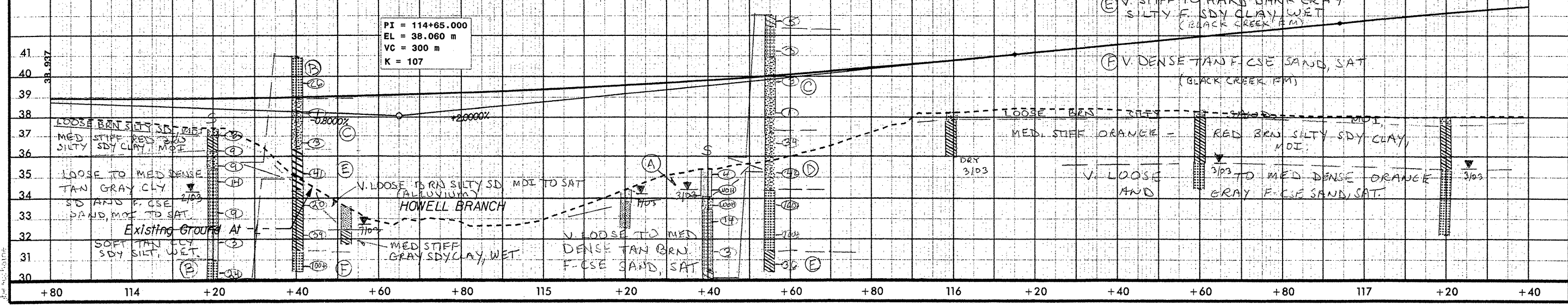
Soil Sample Test Results

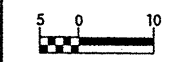
SAMPLE NO.	STATION	DEPTH (m)	AASHTO CLASS	U _c	GRAVIMETRIC				% PASSING (SIEVES)			MOISTURE ORGANIC
					W _t	CS	FS	SLT	CLAY	2.0mm	75µm	
SS-170	10+40	CL	0.76-1.21 A-7.6(6)	54	26	45.1	11.5	4.0	36.4	95	91	62
SS-171	10+40	CL	1.52-1.97 A-2.6(0)	34	14	54.9	24.3	1.6	17.4	98	62	19
SS-172	10+40	CL	2.28-2.93 A-2.6(0)	24	NP	58.8	39.9	1.7	3.1	100	67	11
S-46	12+00	CL	0.30-0.76 A-6(4)	31	16	26.3	3.0	13.1	30.4	100	86	47
S-47	12+00	CL	0.76-1.52 A-7.6(10)	41	28	29.8	21.7	7.8	33.8	99	81	30
SS-162	14+20	CL	0.76-1.21 A-6(2)	33	15	37.5	24.3	12.1	26.4	100	77	43
SS-163	14+20	CL	1.52-1.97 A-2.6(0)	23	7	46.3	31.9	3.7	18.1	95	71	22
SS-164	14+20	CL	2.28-2.93 A-4(3)	23	NP	14.4	4.1	19.4	22.1	100	93	50
SS-165	14+20	CL	6.37-7.32 A-3(0)	13	NP	42.1	52.6	1.3	4.0	100	96	4
SS-166	14+20	CL	11.43-11.8 A-2.6(0)	21	NP	23.8	53.8	14.7	8.0	94	82	33
SS-167	14+20	CL	12.96-13.4 A-2.6(0)	30	NP	47.5	41.1	9.3	2.0	100	71	19
SS-168	14+20	CL	14.49-14.9 A-7.6(25)	50	42	5.2	29.9	21.8	20.1	98	93	66
SS-301	15+40	CL	0.76-1.21 A-6(0)	23	NP	37.1	0.6	19.2	11.1	100	86	30
SS-302	15+40	CL	1.52-1.97 A-2.6(0)	30	NP	52.6	29.4	16.0	4.0	95	60	23
SS-303	15+40	CL	2.28-2.97 A-1.8(0)	2	NP	79.0	1.4	0.6	5.0	97	141	6
SS-304	15+40	CL	3.04-3.41 A-2.6(0)	4	NP	80.6	1.1	1.2	2.0	95	116	7
SS-305	15+40	CL	6.30-7.34 A-1.8(0)	2	NP	79.6	11.2	2.2	7.0	94	49	9
SS-306	15+40	CL	8.41-8.86 A-2.6(0)	3	NP	23.2	6.2	10.5	2.0	99	138	19
SS-307	15+40	CL	14.54-14.9 A-3(0)	2	NP	49.6	30.0	3.6	4.8	99	70	1
SS-308	15+40	CL	17.58-18.0 A-6(12)	33	22	7.4	38.5	18.0	44.1	100	96	66

- (A) V. SOFT TO SOFT BRN CLY SDY SILT, MOI TO WET (ALLUVIUM)
- (B) MED. DENSE TAN F-CSE SAND, SAT
- (C) VERY LOOSE DARK GRAY SILTY F-CSE SAND, SAT
- (D) DENSE DARK GRAY F-CSE SAND, SAT (BLACK CREEK FM)
- (E) V. STIFF TO HARD DARK GRAY SILTY F. SDY CLAY, WET (BLACK CREEK FM)
- (F) V. DENSE TAN F-CSE SAND, SAT (BLACK CREEK FM)

BM #5 RR SPIKE IN BASE OF 360mm (14") OAK
 36.8291 RT OF -BL- STA. 31+74.508

PI = 114+65.000
 EL = 38.060 m
 VC = 300 m
 K = 107



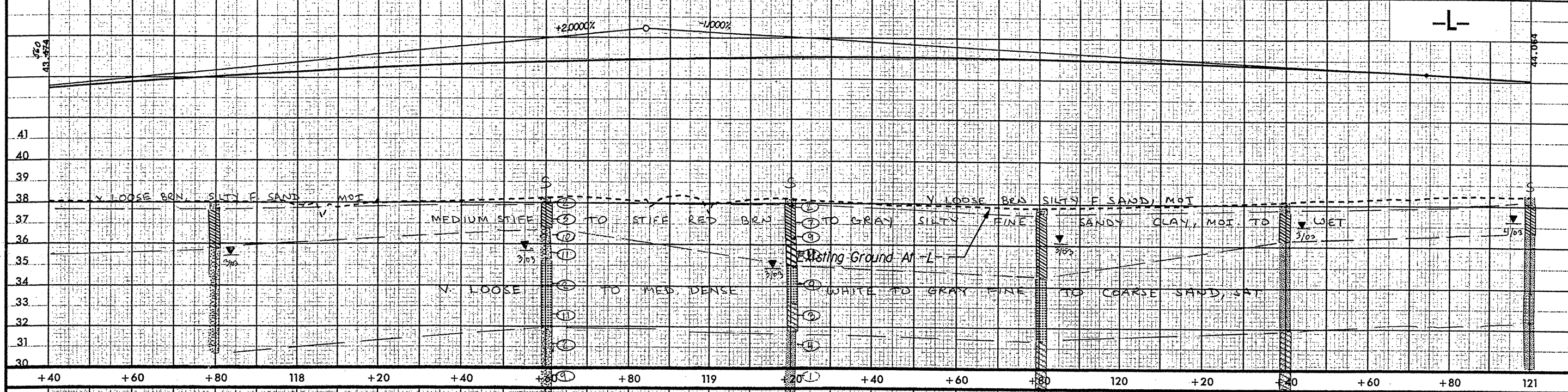


INCOMPLETE PLANS
 DO NOT USE FOR ACQUISITION
 PRELIMINARY PLANS
 DO NOT USE FOR CONSTRUCTION

CONST. REV.
 R/W REV.

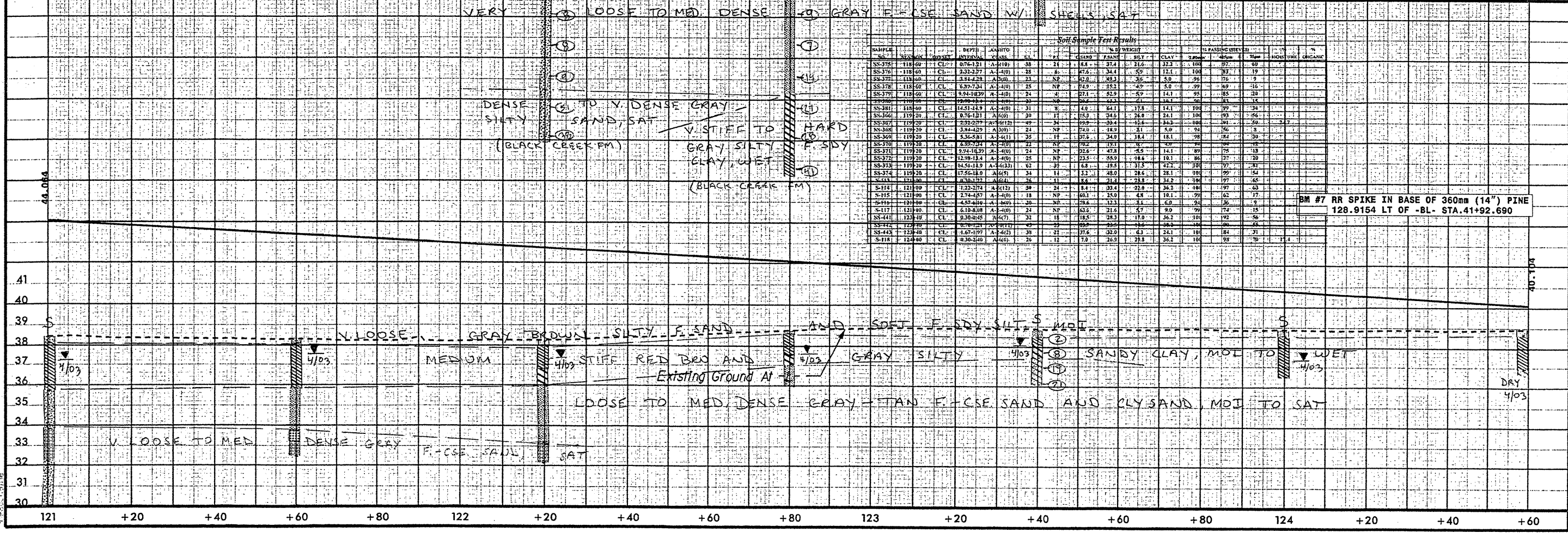
BM #6 RR SPIKE IN BASE OF 660mm (26") TWIN OAK
 176.2625 LT OF -BL- STA. 35+55.047

PI = 118+84.000
 EL = 46.440 m
 VC = 288 m
 K = 123.81

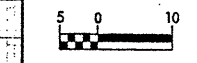


Soil Sample Test Results

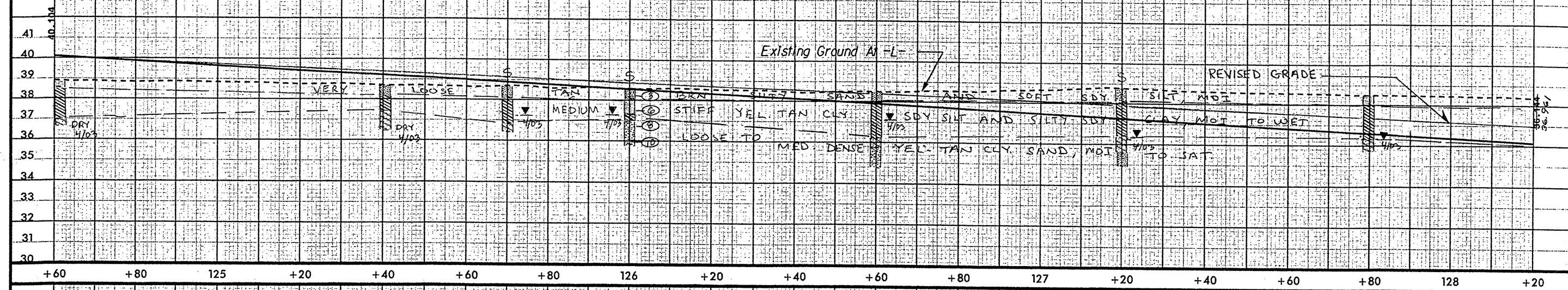
SAMPLE NO.	DEPTH (M)	LABORATORY	WATER CONTENT (%)	LIQUID LIMIT (%)	PLASTICITY INDEX	FLUIDITY	WATER CONTENT (%)	LIQUID LIMIT (%)	PLASTICITY INDEX	FLUIDITY	WATER CONTENT (%)	LIQUID LIMIT (%)	PLASTICITY INDEX	FLUIDITY
SS-375	118.60	CL	0.76-1.21	A(10)	38	NP	8.1	37.2	21.8	32.3	108	97.2	99	
SS-376	118.60	CL	3.23-3.77	A(10)	38	NP	12.4	37.7	25.1	11.6	92	92	19	
SS-377	118.60	CL	3.84-4.28	A(10)	33	NP	12.0	49.3	37.3	5.0	96	76	9	
SS-378	118.60	CL	6.39-7.34	A(10)	25	NP	74.9	152	47	5.0	99	69	16	
SS-379	118.60	CL	9.94-10.39	A(10)	24	NP	27.1	62.9	35.9	14.1	95	85	20	
SS-380	119.20	CL	0.99-1.24	A(10)	38	NP	11.8	24.1	12.3	11.1	100	63	12	
SS-381	119.20	CL	14.51-14.9	A(10)	31	NP	14.8	24.1	11.3	14.1	100	70	14	
SS-382	119.20	CL	0.76-1.21	A(10)	38	NP	75.3	34.6	24.0	24.1	108	93	56	
SS-383	119.20	CL	2.32-2.77	A(10)	49	NP	10.9	33.4	23.4	14.3	104	84	50	
SS-384	119.20	CL	3.84-4.28	A(10)	24	NP	74.0	18.9	3.1	5.0	94	56	3	
SS-385	119.20	CL	5.35-5.81	A(10)	25	NP	10.9	24.0	14.1	18.1	98	84	30	
SS-386	119.20	CL	6.87-7.34	A(10)	24	NP	6.2	19.1	6.7	3.0	97	74	15	
SS-387	119.20	CL	9.94-10.39	A(10)	24	NP	22.6	47.8	25.5	14.1	97	75	13	
SS-388	119.20	CL	12.98-13.4	A(10)	25	NP	23.5	55.9	32.4	10.1	86	27	20	
SS-389	119.20	CL	14.51-14.9	A(10)	62	NP	6.5	19.5	31.3	47.2	90	97	31	
SS-390	119.20	CL	17.66-18.0	A(10)	31	NP	11.1	23.7	48.0	24.6	28.1	100	99	
SS-414	121.80	CL	0.30-0.32	A(6)	26	NP	8.6	31.4	23.8	31.2	98	97	65	
S-114	121.80	CL	1.22-2.74	A(12)	34	NP	8.1	33.4	22.9	36.3	94	97	63	
S-115	121.80	CL	2.74-4.57	A(10)	18	NP	66.1	25.0	4.8	10.1	99	62	17	
S-116	121.80	CL	4.57-4.10	A(10)	30	NP	29.6	12.3	3.1	6.0	94	36	9	
S-117	121.80	CL	6.18-6.08	A(10)	24	NP	62.0	21.8	4.7	6.0	99	74	15	
SS-411	123.40	CL	0.30-0.45	A(6)	31	NP	18.5	28.1	11.0	36.2	100	192	56	
SS-412	123.40	CL	0.76-0.81	A(6)	45	NP	25.9	25	14.2	38.3	100	80	64	
SS-443	123.40	CL	1.67-1.97	A(6)	39	NP	37.6	32.0	6.3	24.1	100	84	91	
S-118	124.80	CL	0.30-0.40	A(6)	26	NP	7.0	26.3	29.8	36.2	100	93	70	



BM #7 RR SPIKE IN BASE OF 360mm (14") PINE
 128.9154 LT OF -BL- STA. 41+92.690



-L-



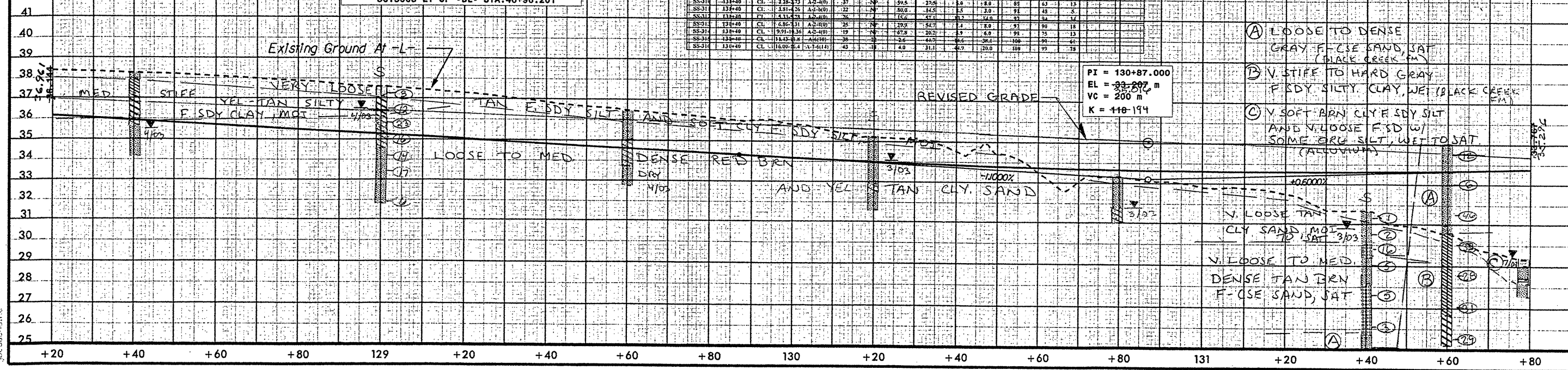
Soil Sample Test Results

SAMPLE NO.	STATION	DEPTH	CLASS	LL	PL	USAND	FSAND	SILT	CLAY	2.0mm	75um	4.75mm	MOISTURE	ORGANIC
SS-19	125+00	CL 0.60-1.20	A-4(4)	31	17	14.7	14.4	66.3	26.1	100	96	43		
SS-44	125+00	CL 0.30-0.45	A-2(40)	15	8	23.3	54.6	20.1	3.8	100	31	23		
SS-44	125+00	CL 0.76-1.21	A-4(0)	19	8	21.7	33.6	36.6	14.1	100	93	39		
SS-44	125+00	CL 1.52-1.97	A-2(40)	25	10	28.5	39.8	11.6	20.1	100	81	34		
SS-44	125+00	CL 1.20-1.30	A-4(4)	33	8	33.0	30.6	30.4	30.5	100	88	44		
SS-44	125+00	CL 0.30-0.45	A-4(5)	23	8	13.5	29.7	38.6	28.1	100	95	61		
SS-44	125+00	CL 0.30-0.45	A-1(50)	41	28	13.9	29.9	28.0	38.2	100	95	61		
SS-44	125+00	CL 1.52-1.97	A-2(40)	27	8	28.4	34.5	12.3	10.7	100	89	29		
SS-48	125+00	CL 3.05-5.50	A-2(40)	27	8	75.2	11.0	1.8	11.1	98	46	14		
SS-101	130+20	CL 0.30-0.45	A-2(40)	24	8	46.1	37.5	1.3	8.8	100	76	19		
SS-102	130+20	CL 2.27-2.65	A-2(40)	38	19	36.5	18.5	2.9	25.1	91	70	28		
SS-103	130+20	CL 1.26-1.66	A-2(40)	22	8	12.4	34.6	3.9	11.7	100	96	25		
SS-309	134+40	CL 0.76-1.21	A-2(40)	16	8	13.7	35.9	36.6	116.0	91	81	35		
SS-311	134+40	CL 2.28-2.73	A-2(40)	37	18	59.5	29.8	1.0	8.8	89	63	13		
SS-311	134+40	CL 1.51-1.26	A-4(50)	122	18	80.0	14.5	1.5	3.0	91	48	5		
SS-312	134+40	CL 1.51-1.26	A-4(50)	36	17	17.6	47.1	31.2	14.0	93	94	14		
SS-311	134+40	CL 1.51-1.26	A-4(50)	25	18	29.9	54.7	1.4	7.0	91	90	18		
SS-311	134+40	CL 0.76-1.21	A-2(40)	19	8	17.8	30.2	1.9	4.8	94	75	13		
SS-311	134+40	CL 1.44-1.83	A-4(50)	38	20	3.5	44.7	36.4	39.4	100	89	40		
SS-311	134+40	CL 116.00-2.4	A-1(61)	43	18	4.0	31.1	40.9	120.0	109	99	78		

BM #8 RR SPIKE IN BASE OF 600mm (24") OAK
 50.5008 LT OF -BL- STA. 46+96.201

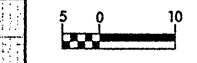
PI = 130+87.000
 EL = 33.207 m
 VC = 200 m
 K = +110-194

- Ⓐ LOOSE TO DENSE GRAY F-CSE SAND, SAT (BLACK CREEK FM)
- Ⓑ V. STIFF TO HARD GRAY F. SDY SILTY CLAY, WET (BLACK CREEK FM)
- Ⓒ V. SOFT BRN CLY F. SDY SILT AND V. LOOSE F. SDY W/ SOME GRN SILT, WET TO SAT (ALUMINUM)





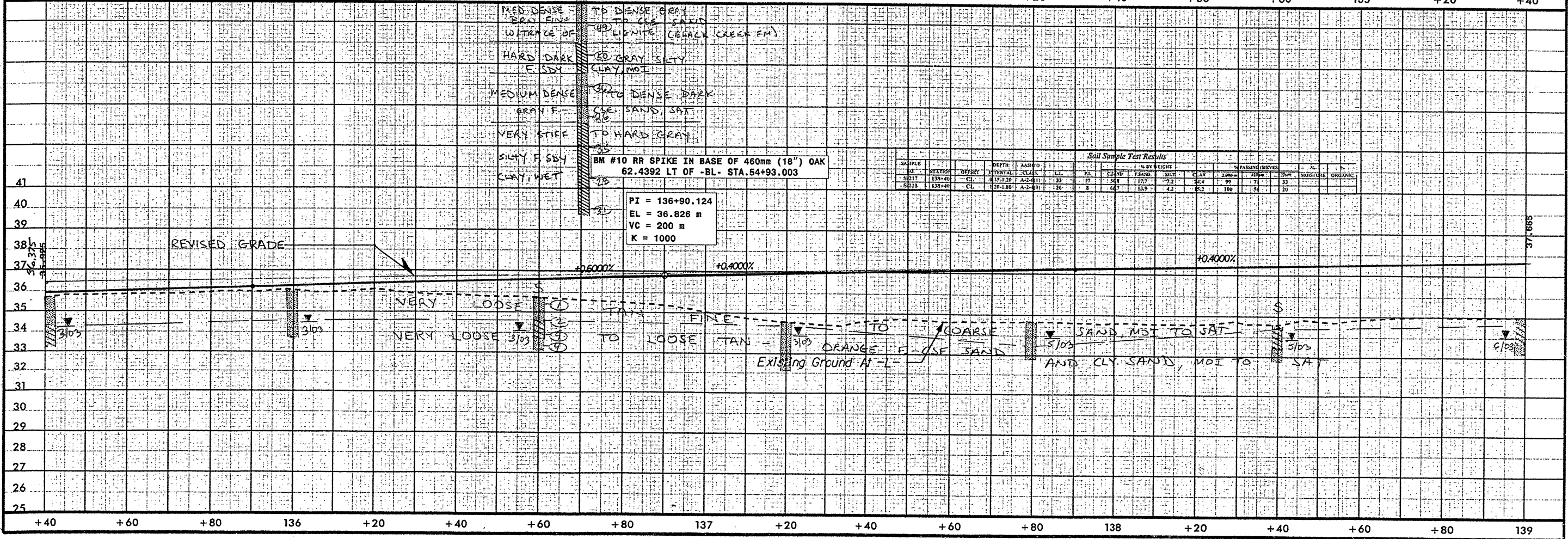
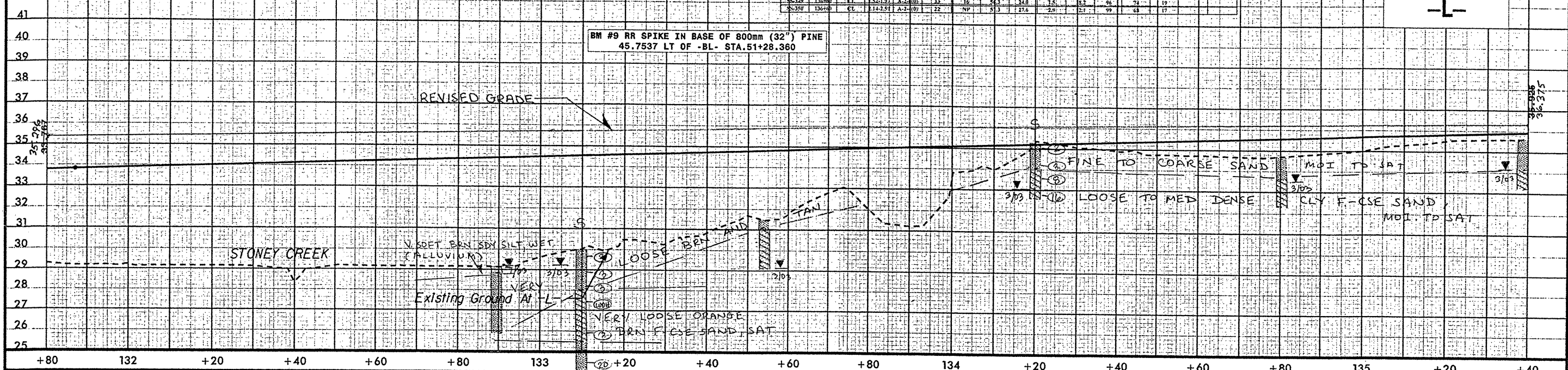
INCOMPLETE PLANS
DO NOT USE FOR R/W ACQUISITION
PRELIMINARY PLANS
DO NOT USE FOR CONSTRUCTION



CONST. REV.
R/W REV.

Soil Sample Test Results

SAMPLE NO.	STATION	DEPTH INTERVAL	AASHTO CLASS.	LL	PI	% BY WEIGHT				% PASSING (SIEVES)			MOISTURE	ORGANIC
						CLAY	FINE SAND	COARSE SAND	SILT	NO. 10	NO. 40	NO. 200		
S-331	132+00	CL 1.26-1.51	A-2(1)	26	2	58.5	27.2	1.7	1.7	99	81	18		
S-332	132+10	CL 1.31-1.74	A-2(1)	24	18	64.9	23.3	4.7	1.7	98	81	20		
S-333	132+20	CL 1.36-1.51	A-2(1)	26	NP	54.8	29.4	7.8	1.0	98	77	13		
S-334	132+30	CL 1.40-1.47	A-2(1)	33	10	1.1	30.1	31.4	4.3	100	94	6		
S-342	133+00	CL 1.92-10.37	A-2(1)	20	NP	23.6	55.1	2.1	0.1	100	91	28		
S-343	133+10	CL 1.26-13.4	A-2(1)	32	18	0.5	37.9	31.4	0.3	100	97	74		
S-344	133+20	CL 1.20-11.64	A-2(1)	47	21	3.1	20.2	24.2	2.2	100	98	66		
S-345	134+20	CL 1.15-8.48	A-2(1)	15	NP	68.8	40.2	6.3	1.0	100	88	13		
S-346	134+30	CL 1.51-1.91	A-2(1)	33	18	4.0	27.0	6.8	2.2	99	79	30		
S-347	134+40	CL 1.29-2.74	A-2(1)	31	14	44.9	31.3	1.5	0.2	100	87	23		
S-348	136+00	CL 1.15-8.48	A-2(1)	14	NP	47.6	37.9	3.4	0.1	100	81	17		
S-349	136+10	CL 1.51-1.74	A-2(1)	33	18	5.3	24.0	1.5	0.2	96	72	19		
S-350	136+20	CL 1.40-2.31	A-2(1)	22	NP	51.3	27.6	2.9	0.1	99	68	17		



Soil Sample Test Results

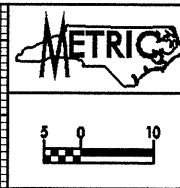
SAMPLE NO.	STATION	DEPTH INTERVAL	AASHTO CLASS.	LL	PI	% BY WEIGHT				% PASSING (SIEVES)			MOISTURE	ORGANIC
						CLAY	FINE SAND	COARSE SAND	SILT	NO. 10	NO. 40	NO. 200		
S-217	136+40	CL 1.15-1.20	A-2(1)	13	7	66.7	17.7	7.2	0.4	99	71	33		
S-218	138+40	CL 1.20-1.80	A-2(1)	26	8	66.7	13.9	4.3	0.2	100	56	20		

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Soil Sample Test Results

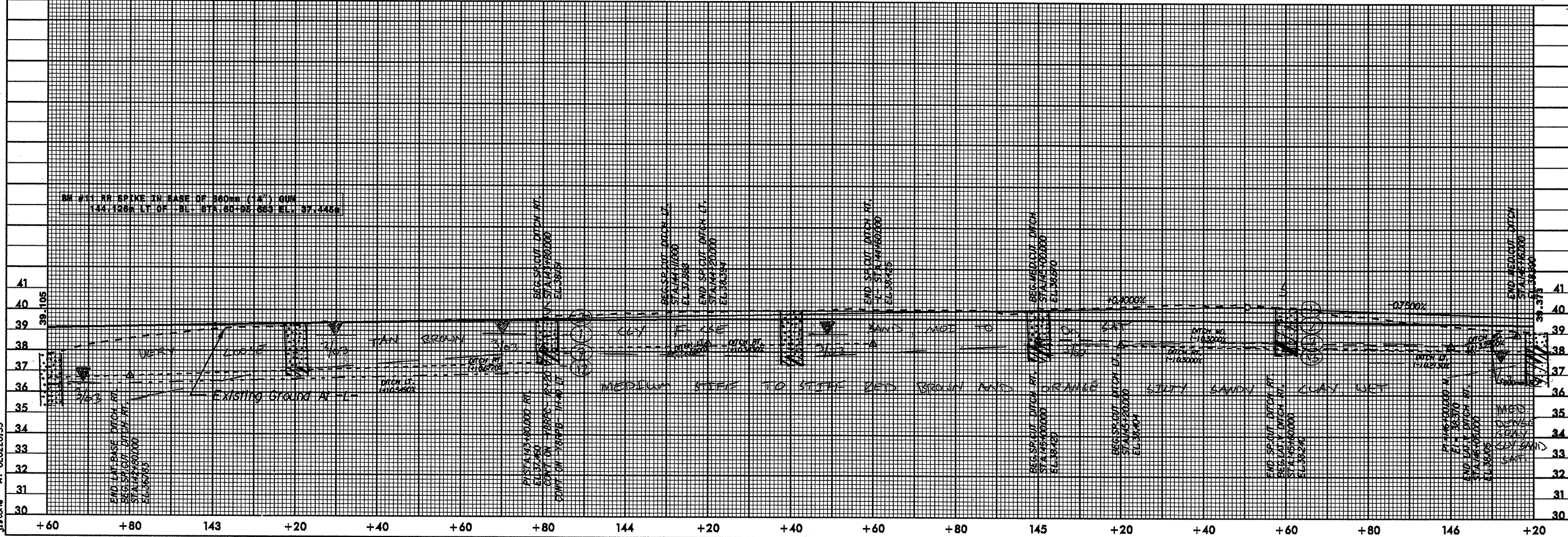
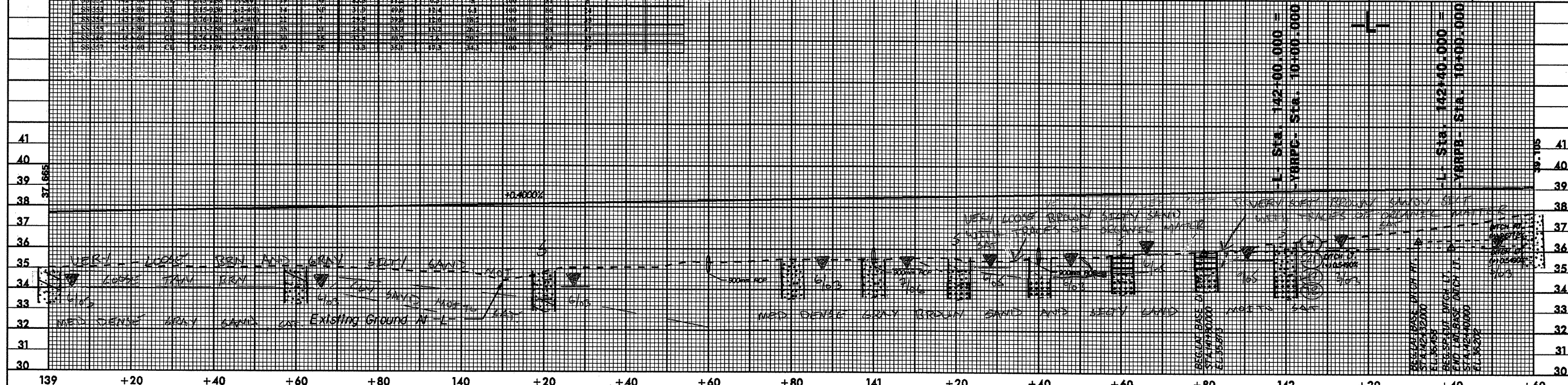
DATE	STATION	TEST	CLASS	% BY WEIGHT				% BY VOLUME					
				W	L	P	LL	W	L	P	LL		
10/10/00	139+20	101	CL	22.0	11.0	67.0	11.0	11.0	78.0	100	100	0	0
10/10/00	139+40	102	CL	22.0	11.0	67.0	11.0	11.0	78.0	100	100	0	0
10/10/00	139+60	103	CL	22.0	11.0	67.0	11.0	11.0	78.0	100	100	0	0
10/10/00	139+80	104	CL	22.0	11.0	67.0	11.0	11.0	78.0	100	100	0	0
10/10/00	140+20	105	CL	22.0	11.0	67.0	11.0	11.0	78.0	100	100	0	0
10/10/00	140+40	106	CL	22.0	11.0	67.0	11.0	11.0	78.0	100	100	0	0
10/10/00	140+60	107	CL	22.0	11.0	67.0	11.0	11.0	78.0	100	100	0	0
10/10/00	140+80	108	CL	22.0	11.0	67.0	11.0	11.0	78.0	100	100	0	0
10/10/00	141+20	109	CL	22.0	11.0	67.0	11.0	11.0	78.0	100	100	0	0
10/10/00	141+40	110	CL	22.0	11.0	67.0	11.0	11.0	78.0	100	100	0	0
10/10/00	141+60	111	CL	22.0	11.0	67.0	11.0	11.0	78.0	100	100	0	0
10/10/00	141+80	112	CL	22.0	11.0	67.0	11.0	11.0	78.0	100	100	0	0
10/10/00	142+20	113	CL	22.0	11.0	67.0	11.0	11.0	78.0	100	100	0	0
10/10/00	142+40	114	CL	22.0	11.0	67.0	11.0	11.0	78.0	100	100	0	0
10/10/00	142+60	115	CL	22.0	11.0	67.0	11.0	11.0	78.0	100	100	0	0
10/10/00	142+80	116	CL	22.0	11.0	67.0	11.0	11.0	78.0	100	100	0	0



PROJECT REFERENCE NO. R-2554BA
 ROADWAY DESIGN ENGINEER
 SHEET NO. 41
 HYDRA-ENGINEER

PRELIMINARY PLANS
 DO NOT USE FOR CONSTRUCTION

CONST. REV.
 R/W REV.



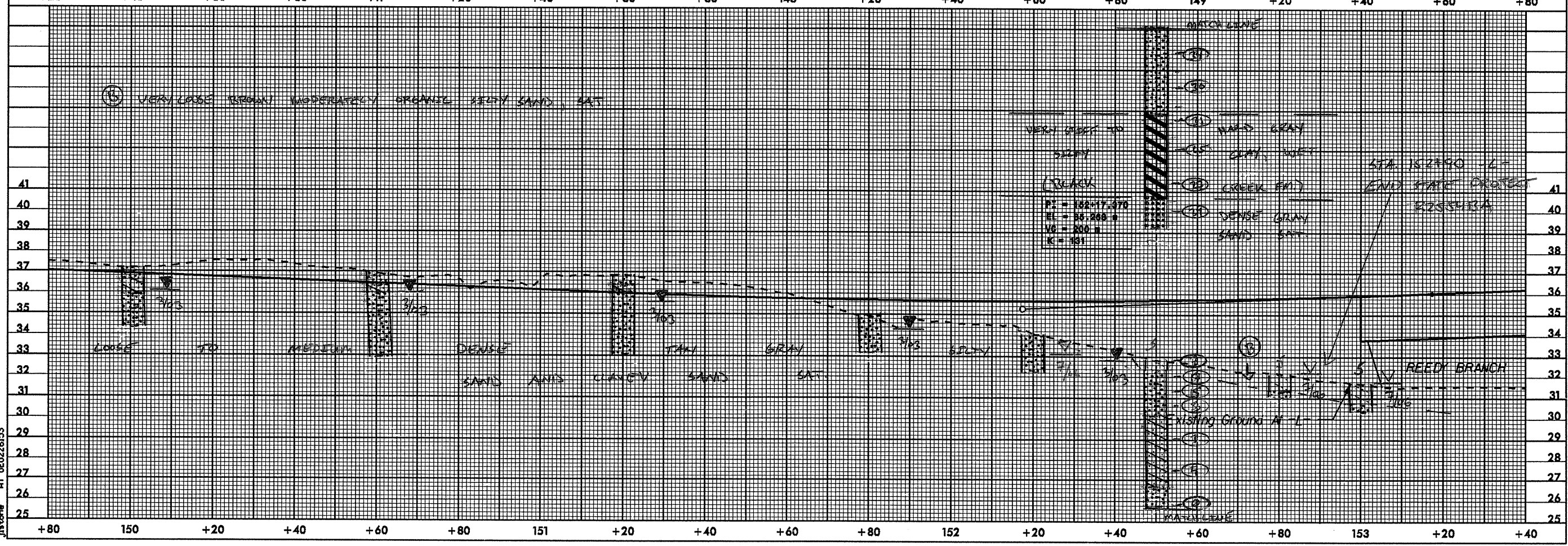
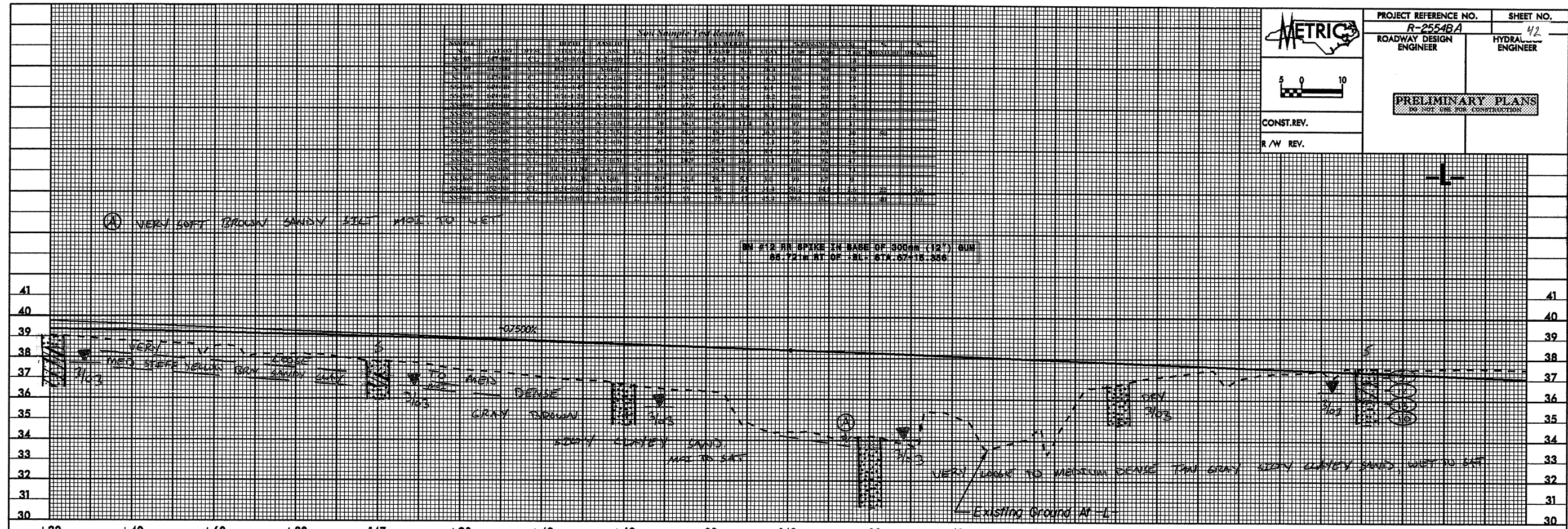
BM #11 RR SPIKE IN BASE OF 360mm (14") GUM
 144+126K LY OF - BL. STA. 60-00 363 EL. 37.445m



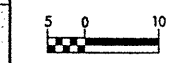
PRELIMINARY PLANS
 FOR THE PROPOSED CONSTRUCTION

CONST. REV.
 R / W REV.

Soil Sample Test Results										
NO.	DATE	DEPTH	TEST	RESULT	TEST	RESULT	TEST	RESULT	TEST	RESULT
1	10/25/06	0-12"	WATER CONTENT	22.5	LIQUID LIMIT	45	PLASTICITY INDEX	22.5	UNSATURATED SHREDS	0
2	10/25/06	12-24"	WATER CONTENT	22.5	LIQUID LIMIT	45	PLASTICITY INDEX	22.5	UNSATURATED SHREDS	0
3	10/25/06	24-36"	WATER CONTENT	22.5	LIQUID LIMIT	45	PLASTICITY INDEX	22.5	UNSATURATED SHREDS	0
4	10/25/06	36-48"	WATER CONTENT	22.5	LIQUID LIMIT	45	PLASTICITY INDEX	22.5	UNSATURATED SHREDS	0
5	10/25/06	48-60"	WATER CONTENT	22.5	LIQUID LIMIT	45	PLASTICITY INDEX	22.5	UNSATURATED SHREDS	0
6	10/25/06	60-72"	WATER CONTENT	22.5	LIQUID LIMIT	45	PLASTICITY INDEX	22.5	UNSATURATED SHREDS	0
7	10/25/06	72-84"	WATER CONTENT	22.5	LIQUID LIMIT	45	PLASTICITY INDEX	22.5	UNSATURATED SHREDS	0
8	10/25/06	84-96"	WATER CONTENT	22.5	LIQUID LIMIT	45	PLASTICITY INDEX	22.5	UNSATURATED SHREDS	0
9	10/25/06	96-108"	WATER CONTENT	22.5	LIQUID LIMIT	45	PLASTICITY INDEX	22.5	UNSATURATED SHREDS	0
10	10/25/06	108-120"	WATER CONTENT	22.5	LIQUID LIMIT	45	PLASTICITY INDEX	22.5	UNSATURATED SHREDS	0



15-AUG-2006 14:43
 I:\viro\greenville_investigation\tr-2554ba\cadd\original\roadway\pro\r-2554b-rdy-pl.dgn
 jstone AT GEO226153



INCOMPLETE PLANS
 DO NOT USE FOR R/W ACQUISITION
 PRELIMINARY PLANS
 DO NOT USE FOR CONSTRUCTION

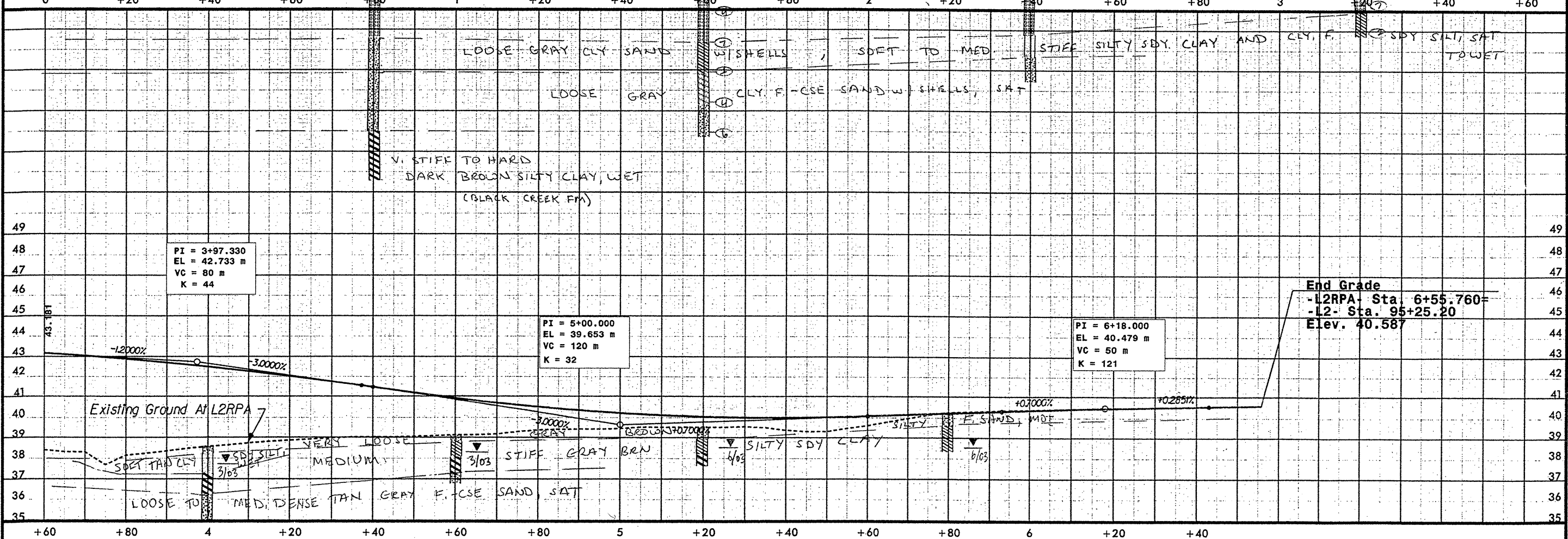
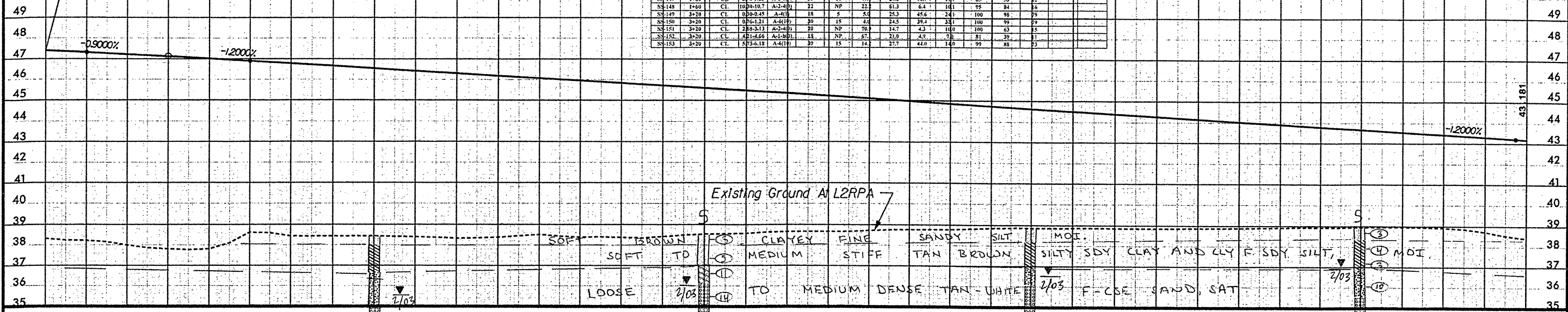
CONST. REV.
 R/W REV.

L2RPA

Begin Grade
 -L2RPA- Sta. 0+00.000=
 -L- Sta. 98+17.208
 Elev. 47.411

PI = 0+30.000
 EL = 47.141 m
 VC = 40 m
 K = 133

SAMPLE NO.	STATION	DEPTH INTERVAL	AASHTO CLASS	LL	PI	% BY WEIGHT				% PASSING (SIEVES)			Moisture	% ORGANIC
						CS&SD	FS&SD	SILT	CLAY	20mm	425µ	75µ		
SS-141	1+60	CL	0B0-0.45 A-4(0)	13	NP	18	82	27	100	100	96	85		
SS-142	1+60	CL	0B1-1.21 A-4(0)	20	7	22	37.6	19.6	202	100	93	85		
SS-143	1+60	CL	1B2-1.97 A-2(40)	25	11	32	44.9	8.6	232	100	97	81		
SS-144	1+60	CL	2B3-2.92 A-1(60)	35	5	48	42	4.9	242	100	96	76		
SS-145	1+60	CL	5B3-6.18 A-1(60)	20	NP	77	16.4	2.7	30	97	88	7		
SS-146	1+60	CL	7B3-7.70 A-6(1)	30	13	30	38.8	22.2	80	97	80	36		
SS-147	1+60	CL	8B3-9.23 A-2(40)	30	11	49	28.5	10.7	101	85	83	21		
SS-148	1+60	CL	10B3-10.7 A-2(40)	22	NP	22	81.3	6.4	101	95	84	86		
SS-149	3+20	CL	0B0-0.45 A-4(0)	18	5	51	25.3	45.6	241	100	98	79		
SS-150	3+20	CL	0P6-1.21 A-6(1)	30	15	21	24.5	39.4	331	100	99	79		
SS-151	3+20	CL	2B3-3.13 A-2(40)	20	NP	70	14.7	4.3	100	100	63	85		
SS-152	3+20	CL	4B1-4.66 A-1(60)	18	NP	67	21.9	4.9	70	81	39	11		
SS-153	3+20	CL	5B3-6.18 A-6(1)	30	15	14	27.7	44.0	140	99	88	23		



PI = 3+97.330
 EL = 42.733 m
 VC = 80 m
 K = 44

PI = 5+00.000
 EL = 39.653 m
 VC = 120 m
 K = 32

PI = 6+18.000
 EL = 40.479 m
 VC = 50 m
 K = 121

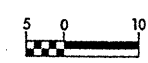
End Grade
 -L2RPA- Sta. 6+55.760=
 -L2- Sta. 95+25.20
 Elev. 40.587

11/20/07
 R. Williams, Project Engineer, 2010/07/07
 11/20/07

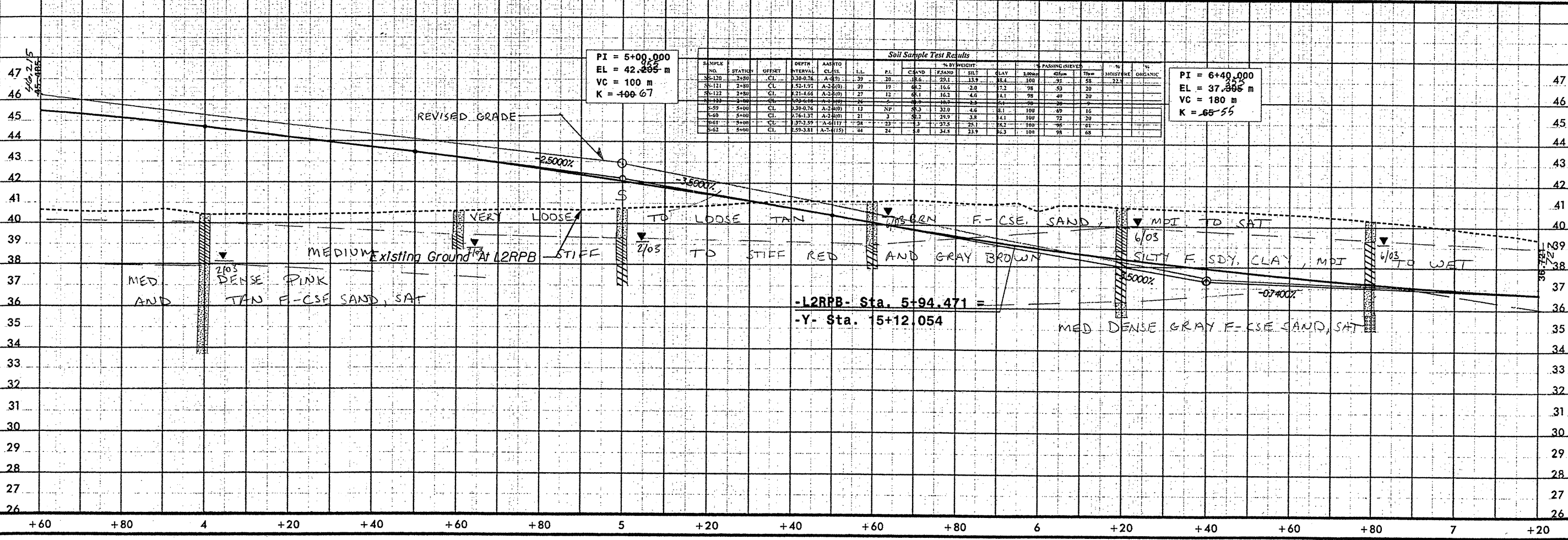
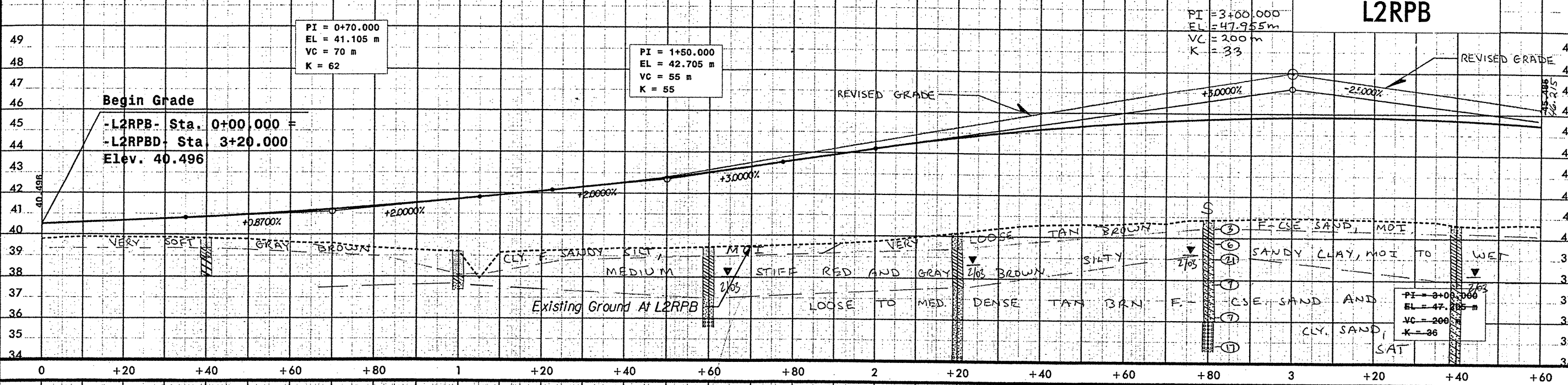


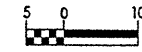
INCOMPLETE PLANS DO NOT USE FOR R/W ACQUISITION PRELIMINARY PLANS DO NOT USE FOR CONSTRUCTION

CONST. REV. R/W REV.



L2RPB





INCOMPLETE PLANS
DO NOT USE FOR R.W. ACQUISITION
PRELIMINARY PLANS
DO NOT USE FOR CONSTRUCTION

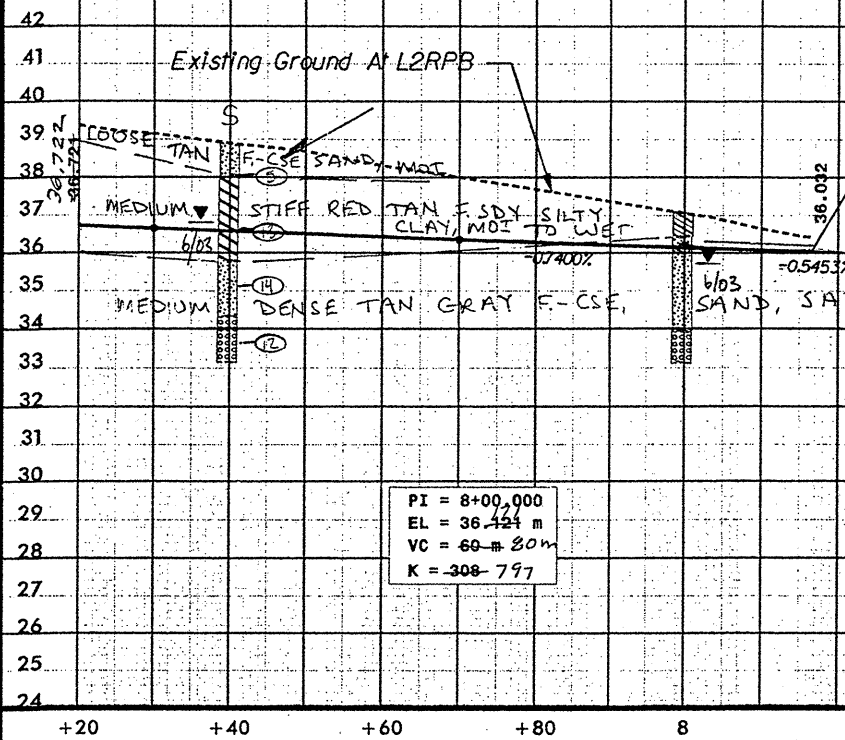
CONST.REV.
R/W REV.

L2RPB

End Grade

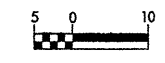
L2RPB Sta. 8+16.590 =
L2RPB Sta. 12+79.917
Elev. 31.563

Existing Ground At L2RPB



SAMPLE NO.	STATION	DEPTH	CLASS	LIQUIDITY		SLY WEIGHT			% PASSING (SIEVES)			MOISTURE %	ORGANIC %	
				LL	PL	SS	SP	CLAY	4.75mm	75um	20um			
SS-561	7+40	CL	0.80-1.01	A-3-A(15)	41	20	16.3	26.9	39.4	100	92	63		
SS-562	7+40	CL	2.29-2.74	A-2-A(32)	60	30	1.9	21.7	26.7	50.7	99	79		
SS-563	7+40	CL	3.81-4.26	A-2-A(0)	29	7	63.8	22.7	13	11.3	93	51	14	
SS-564	7+40	CL	5.33-5.78	A-1-B(0)	22	2	84.1	10.1	6.7	5.1	98	34	6	

15-000000
 15-000000
 15-000000
 15-000000

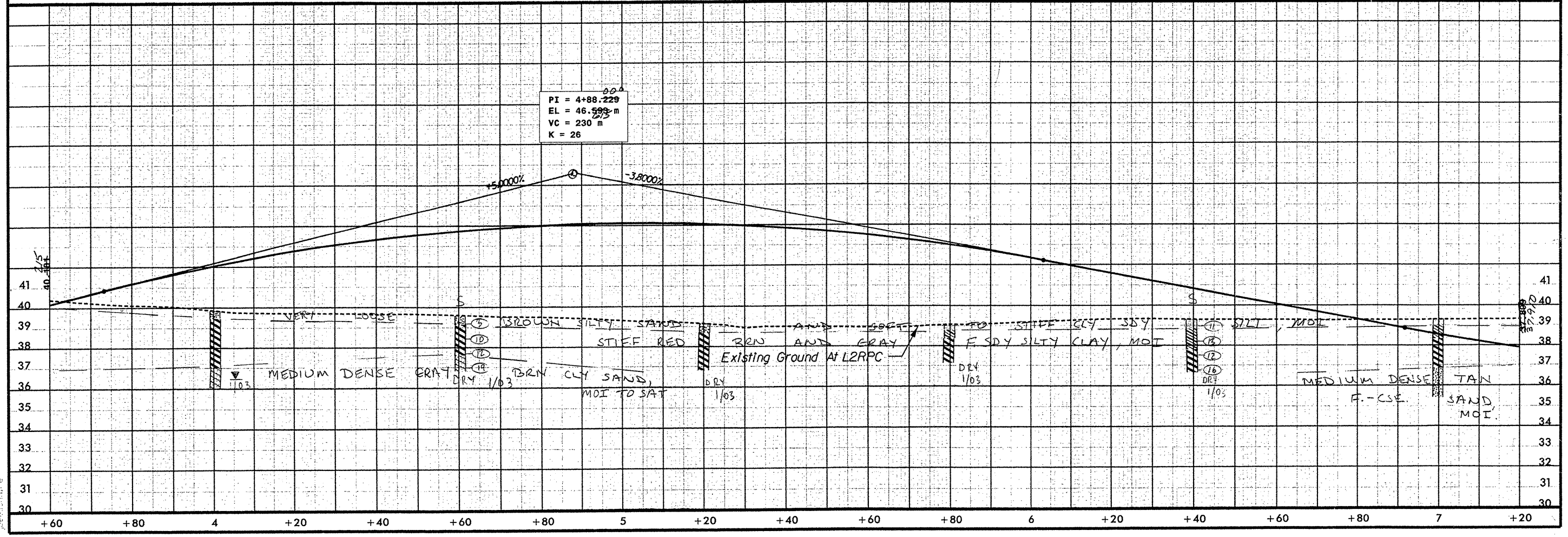
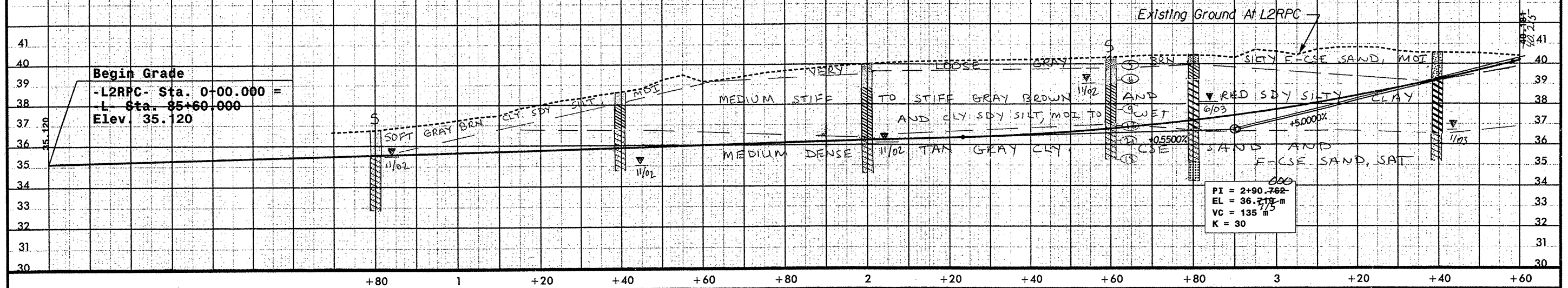


INCOMPLETE PLANS
 DO NOT USE FOR R/W ACQUISITION
 PRELIMINARY PLANS
 DO NOT USE FOR CONSTRUCTION

CONST. REV.
 R/W REV.

Soil Sample Test Results														
SAMPLE NO.	STATION	DEPTH	CLASS	L.L.	P.L.	C.E.	F.SAND	SILT	CLAY	% PASSING SIEVES			MOISTURE	ORGANIC
										200	425	75		
S-7	0+80	CL	A-4(0)	16	3	24	33.5	23.0	18.1	100	84	42		
S-8	0+80	CL	A-4(0)	19	5	39	34.1	18.6	18.2	98	79	36		
S-9	0+80	CL	A-2(4)	39	22	68	12.1	2.5	20.2	97	57	24		
S-10	0+80	CL	A-2(4)	36	19	78	11.9	4.1	11.1	90	37	14		
S-11	2+60	CL	A-4(0)	31	6	48	29.6	19.9	18.1	100	75	28		
S-25	2+60	CL	A-6(0)	35	19	9	20.8	37.0	38.3	100	94	76	22.1	
S-26	2+60	CL	A-2(4)	40	23	71	11.7	3.7	19.1	92	44	16		
S-91	4+60	CL	A-7(6)	32	24	109	21.1	29.7	38.3	100	94	74	20.8	
S-92	4+60	CL	A-7(6)	75	48	9	18.3	18.0	4.4	100	95	83		
S-93	4+60	CL	A-2(4)	29	11	41	31.7	4.3	28.2	100	81	27		
S-94	6+40	CL	A-6(0)	35	21	4	21.3	38.4	38.3	100	98	81		
S-95	6+40	CL	A-7(6)	59	39	3	16.9	28.9	59.4	99	97	84		

L2RPC

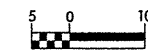


11/15/2003
 in Bannock Proj 505510.rdw
 10/15/2003



PROJECT REFERENCE NO. SHEET NO.

R-2554BA 47
ROADWAY DESIGN ENGINEER HYDRAULICS ENGINEER



INCOMPLETE PLANS
DO NOT USE FOR R/W ACQUISITION
PRELIMINARY PLANS
DO NOT USE FOR CONSTRUCTION

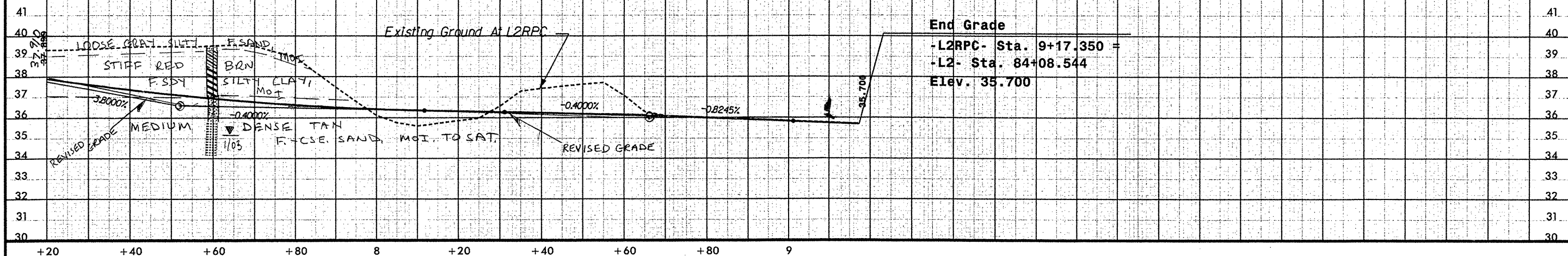
CONST. REV.

R/W REV.

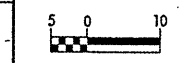
L2RPC

7+52.000
PI = 7+51.720
EL = 36.580 m
VC = 120 m
K = 35

PI = 8+66.120
EL = 36.422 m
VC = 70 m
K = 165-166

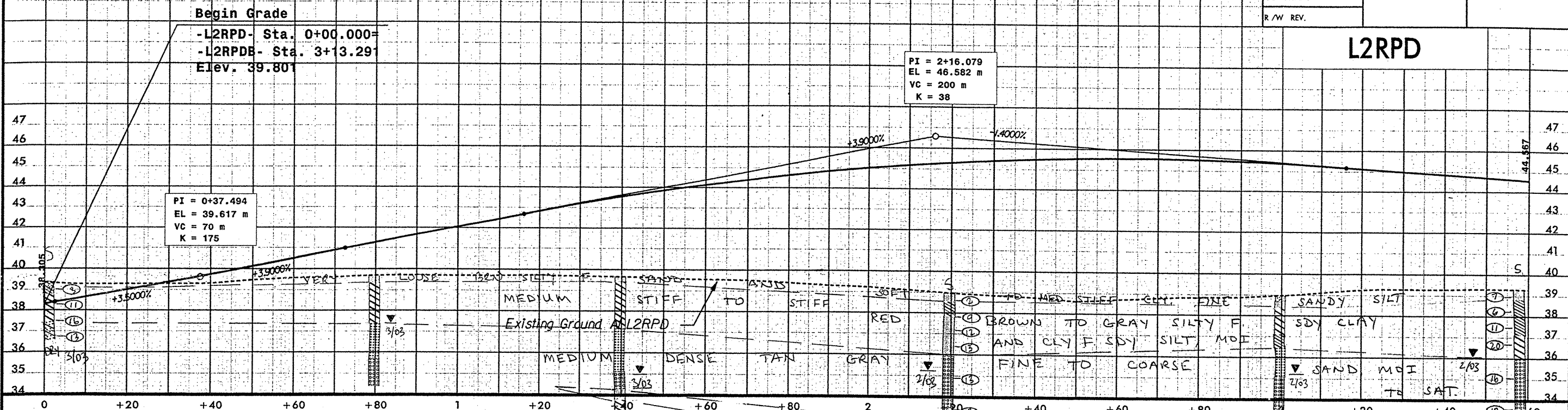


Vertical Curve Data



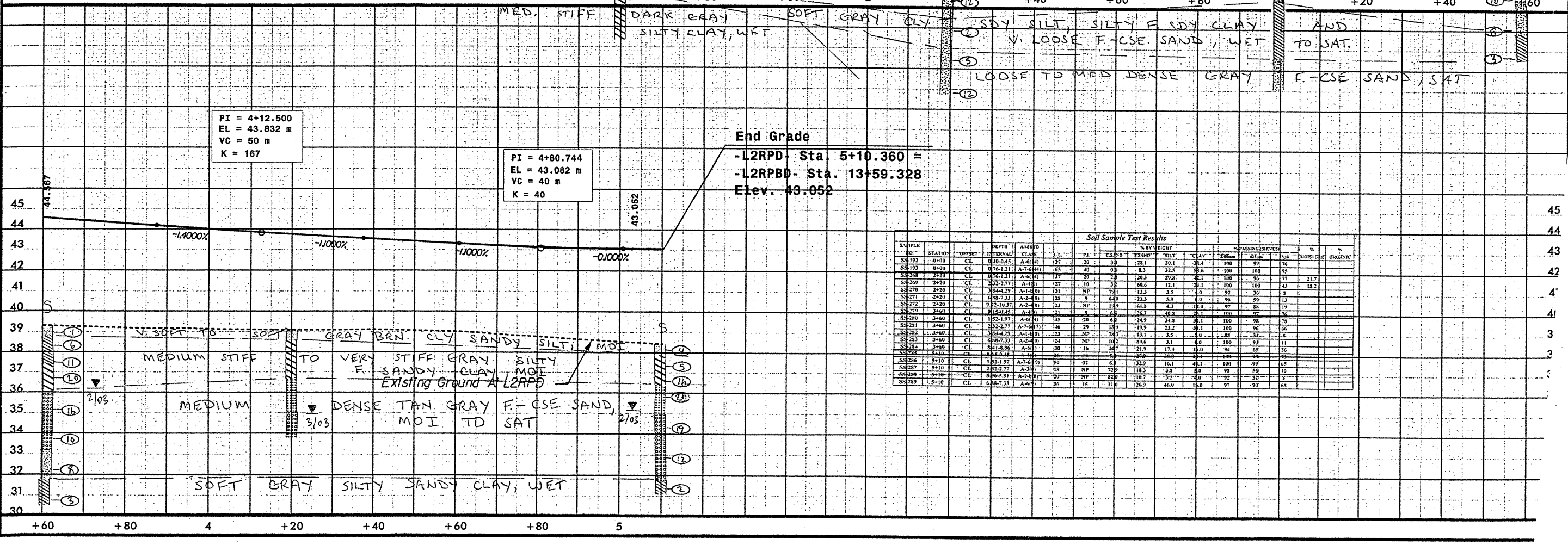
CONST. REV.
 R/W REV.

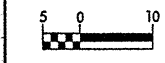
L2RPD



Soil Sample Test Results

SAMPLE NO.	STATION	DEPTH (m)	ASSIGN CLASS	LL	PI	CS NO.	% SAND	% SILT	% CLAY	W _L	W _p	U _c	U _g	U _o	% MOISTURE	% ORGANIC
SS192	0+00	0.30-0.45	A-6(4)	137	20	38	28.1	30.1	38.4	100	95	76				
SS193	0+00	0.76-1.21	A-7(6)	65	40	03	8.3	32.5	58.6	100	100	95				
SS198	2+20	0.61-1.21	A-6(4)	137	20	28	28.3	29.8	42.1	100	100	43	18.2			
SS269	2+20	1.32-2.77	A-6(4)	77	19	32	69.6	12.1	21.1	100	100	43	18.2			
SS270	2+20	3.84-4.29	A-1(8)	121	NP	791	13.3	3.5	4.0	92	30	3				
SS271	2+20	6.88-7.33	A-2(8)	128	9	648	23.3	3.9	4.0	96	59	13				
SS272	2+20	9.22-10.37	A-2(8)	23	NP	199	61.8	4.3	18.9	97	88	19				
SS279	2+60	0.15-0.45	A-6(4)	75	2	88	25.6	40.8	33.6	100	95	76				
SS280	3+60	1.52-1.97	A-6(4)	135	20	62	24.9	34.8	38.1	100	95	79				
SS281	3+60	2.32-2.77	A-7(6)	46	29	189	19.9	33.2	38.1	100	96	66				
SS282	3+60	3.08-4.29	A-1(8)	23	NP	783	13.1	3.5	4.0	83	34	6				
SS283	3+60	6.88-7.33	A-2(8)	124	NP	102	89.6	3.1	4.0	100	93	11				
SS284	3+60	8.41-8.86	A-6(4)	130	16	42	21.9	17.4	18.9	94	65	35				
SS285	3+60	8.86-9.31	A-6(4)	36	16	42	21.9	17.4	18.9	94	65	35				
SS286	5+10	1.52-1.97	A-7(6)	50	32	63	33.9	16.1	49.1	100	99	65				
SS287	5+10	2.32-2.77	A-3(8)	118	NP	329	18.3	3.8	4.0	95	55	10				
SS288	5+10	2.77-3.22	A-3(8)	79	NP	829	10.7	3.2	4.0	92	32	9				
SS289	5+10	6.88-7.33	A-6(4)	36	15	118	12.9	46.9	11.0	97	90	68				





INCOMPLETE PLANS
 DO NOT USE FOR ACQUISITION
 PRELIMINARY PLANS
 DO NOT USE FOR CONSTRUCTION

L2RPBD

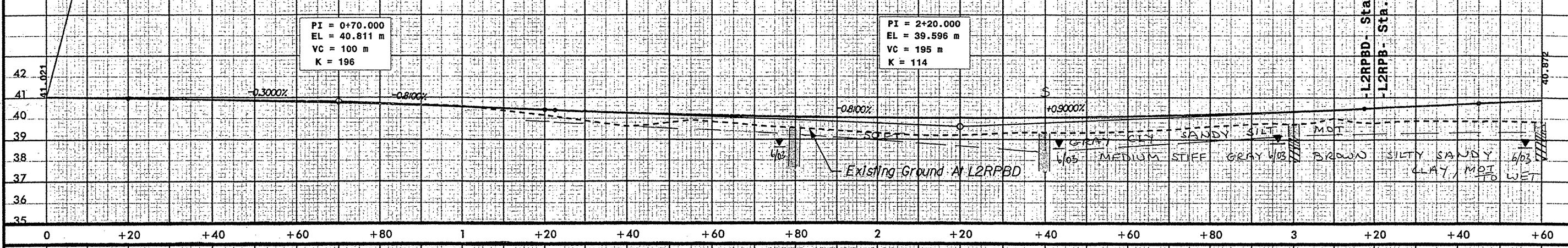
Begin Grade
 -L2RPBD- Sta. 0+00.000 =
 -L2- Sta. 96+70.000
 Elev. 41.021

Soil Sample Test Results															
SAMPLE NO.	STATION	OFFSET	DEPTH	BASIS TO	CLASS	LL	PI	BY WEIGHT			% PASSING (SIEVES)			MOISTURE	ORGANIC
								CBAND	FSAND	LSILT	CLAY	4.75mm	75µm		
S-219	2+40	CL	0.30-0.91	A-4(0)	16	NP	17.7	31.8	13.5	12.7	80	89	83		
S-220	2+40	CT	0.31-1.80	A-4(0)	17	NP	31.1	48.0	26.6	8.0	80	97	44		

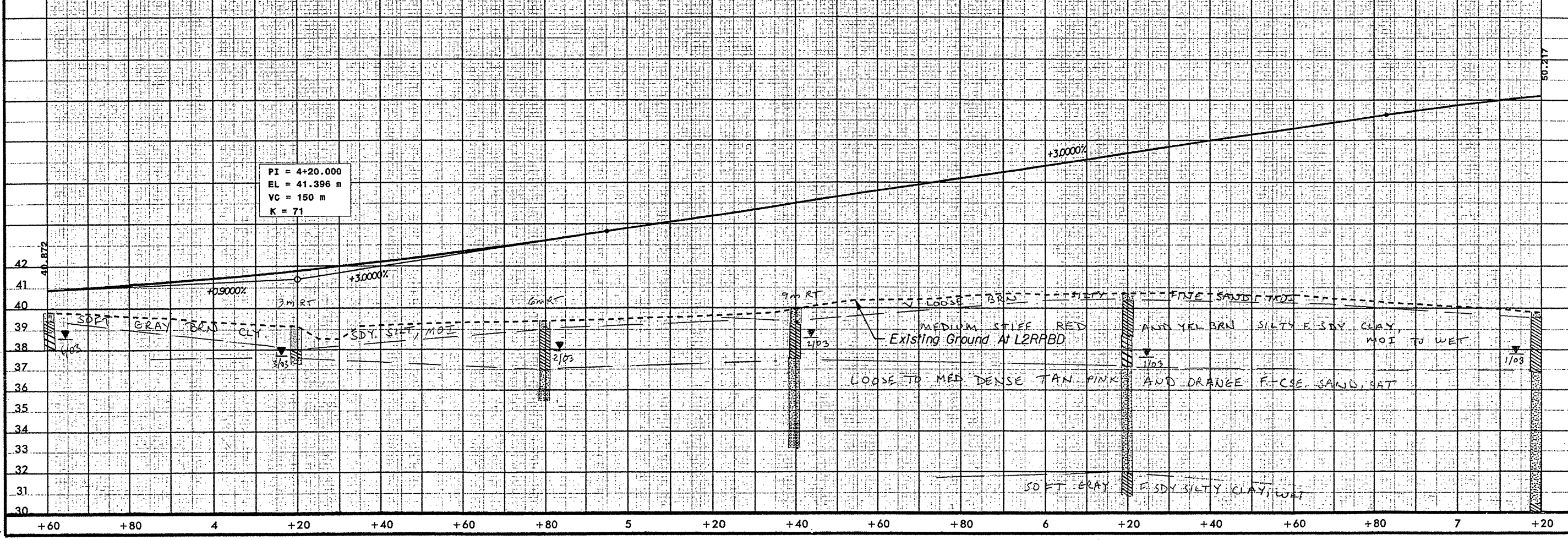
PI = 0+70.000
 EL = 40.811 m
 VC = 100 m
 K = 196

PI = 2+20.000
 EL = 39.596 m
 VC = 195 m
 K = 114

-L2RPBD- Sta. 3+20.000 =
 -L2RPB- Sta. 0+00.000



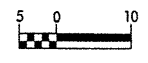
PI = 4+20.000
 EL = 41.396 m
 VC = 150 m
 K = 71



11-04-2002
 J. J. Jones, 301075-05, 1:25P, 466/F1
 geobu@bairns

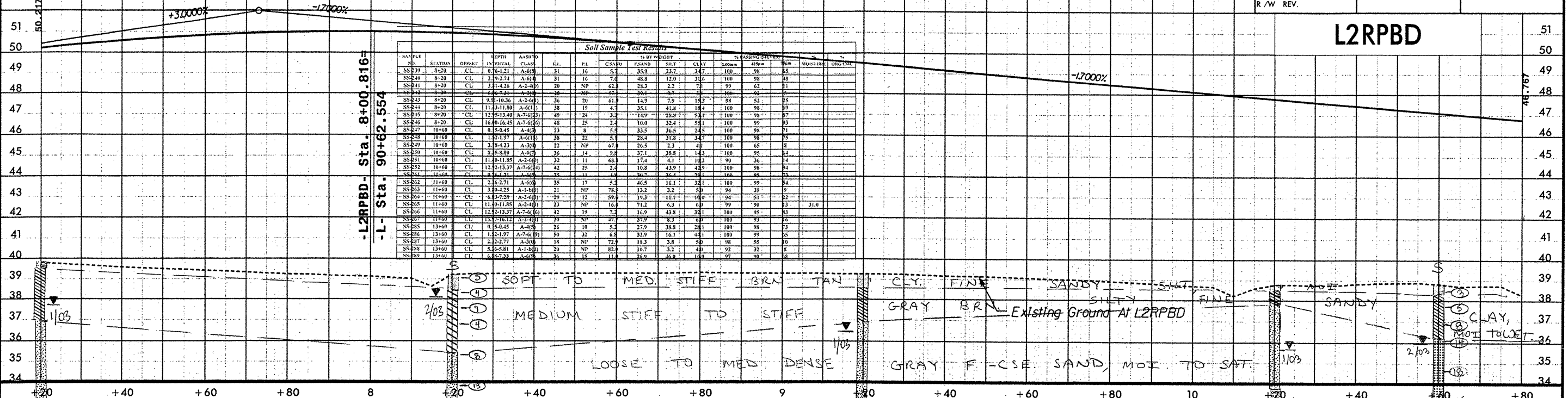


PI = 7+73.000
 EL = 51.986 m
 VC = 180 m
 K = 38

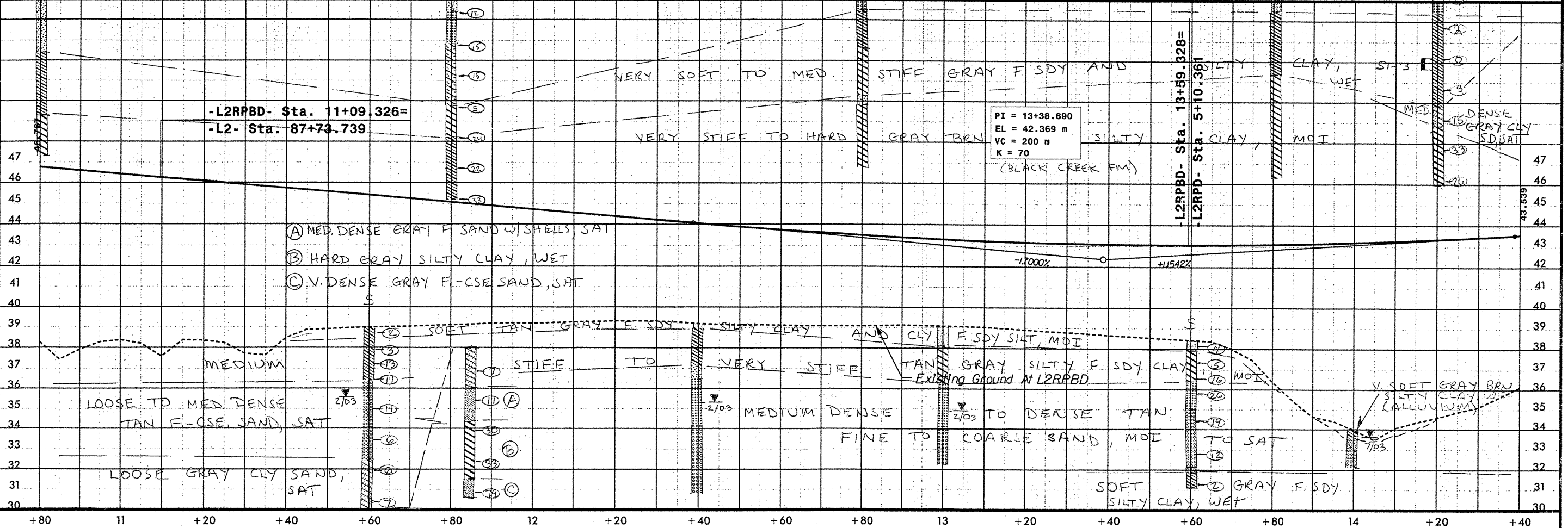


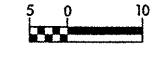
L2RPBD

SAMPLE NO.	STATION	DEPTH INTERVAL	ASHD CLASS	LL	PL	% BY WEIGHT				% PASSING SIEVES				MOISTURE	ORG. MAT.
						CSRD	FSAND	SILT	CLAY	4.75mm	75µm	15µm	7.5µm		
SS-239	8+20	CL 0.6-1.21	A-6(0)	31	16	5.7	35.9	23.7	34.7	100	98	75			
SS-240	8+20	CL 2.19-2.74	A-6(0)	31	16	7.0	48.8	12.0	34.6	100	98	48			
SS-241	8+20	CL 3.11-4.26	A-2-4(0)	20	NP	62.1	28.5	2.2	7.1	99	62	31			
SS-242	8+20	CL 4.67-6.08	A-2-4(0)	20	NP	72.1	26.2	1.7	1.0	99	72	31			
SS-243	8+20	CL 9.35-10.36	A-2-4(0)	36	20	61.9	14.9	7.9	15.3	98	52	25			
SS-244	8+20	CL 11.43-11.80	A-6(1)	38	19	4.7	35.1	41.8	18.4	100	98	29			
SS-245	8+20	CL 12.55-13.40	A-7-6(2)	49	24	3.4	14.9	28.8	53.1	100	98	27			
SS-246	8+20	CL 16.48-16.45	A-7-6(0)	48	25	2.4	10.0	32.4	55.1	100	99	23			
SS-247	10+60	CL 0.5-0.65	A-4(3)	23	8	5.4	33.5	36.5	24.6	100	98	21			
SS-248	10+60	CL 1.52-1.97	A-6(1)	38	22	5.1	28.4	31.8	34.7	100	98	25			
SS-249	10+60	CL 3.18-4.23	A-3(0)	22	NP	67.8	26.5	2.3	4.1	100	65	8			
SS-250	10+60	CL 3.35-8.80	A-6(0)	36	14	3.8	37.1	38.8	14.3	100	92	24			
SS-251	10+60	CL 11.40-11.85	A-2-4(0)	32	11	68.1	17.4	4.1	10.2	90	36	14			
SS-252	10+60	CL 12.13-13.37	A-7-6(1)	42	25	2.4	10.8	43.9	43.9	100	98	24			
SS-253	11+60	CL 0.7-1.21	A-6(0)	35	17	4.4	30.2	36.4	28.8	100	88	23			
SS-254	11+60	CL 2.16-2.71	A-6(0)	35	17	5.2	46.5	16.1	32.1	100	99	24			
SS-255	11+60	CL 3.40-4.25	A-1-1(0)	21	NP	78.2	13.2	3.2	5.3	94	39	9			
SS-256	11+60	CL 6.43-7.38	A-2-4(0)	29	12	59.1	19.3	11.7	10.0	94	51	22			
SS-257	11+60	CL 11.04-11.85	A-2-4(0)	23	NP	16.4	71.2	6.3	6.3	99	90	23	31.0		
SS-258	11+60	CL 12.12-13.37	A-7-6(0)	42	19	7.4	16.9	43.8	31.1	100	95	21			
SS-259	11+60	CL 15.71-16.12	A-2-4(0)	20	NP	47.7	27.9	8.3	16.1	100	75	16			
SS-258	13+60	CL 0.5-0.45	A-4(5)	26	10	5.4	27.9	38.8	28.1	100	98	23			
SS-256	13+60	CL 1.52-1.97	A-7-6(0)	50	32	6.8	32.9	16.1	44.1	100	99	25			
SS-258	13+60	CL 3.52-3.71	A-3(0)	18	NP	72.1	18.3	3.8	5.0	98	55	10			
SS-258	13+60	CL 4.16-6.81	A-1-1(0)	20	NP	82.4	10.7	3.2	4.1	93	32	7			
SS-259	13+60	CL 6.82-7.33	A-6(0)	36	18	11.4	26.2	46.0	16.0	97	50	8			



- (A) MED. DENSE GRAY F SAND W/ SHELLS, SAT
- (B) HARD GRAY SILTY CLAY, WET
- (C) V. DENSE GRAY F-CSE SAND, SAT





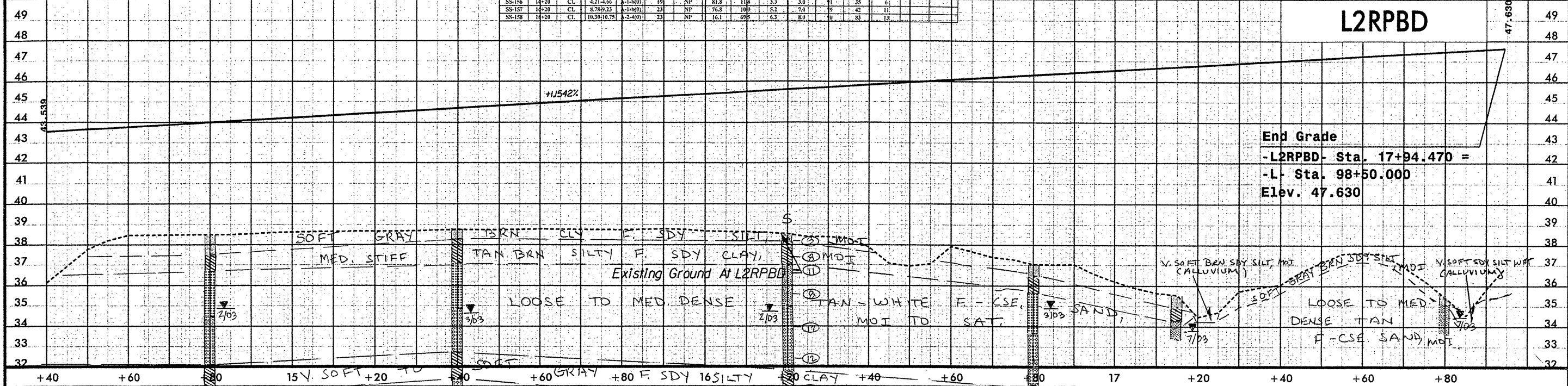
CONST. REV.
 R/W REV.

Soil Sample Test Results															
SAMPLE NO.	STATION	OFFSET	DEPTH INTERVAL	CLASS	L.I.	P.L.	% BY WEIGHT				% PASSING (SIEVES)			MOISTURE %	ORGANIC %
							C.SAND	F.SAND	SILT	CLAY	20#	40#	75#		
SS-154	14+20	CL	0.76-1.21	A-6(3)	25	12	8.6	42.8	26.8	22.1	100	98	56		
SS-155	14+20	CL	1.53-1.97	A-2-4(0)	22	NP	47.4	37.1	3.4	12.0	100	89	16		
SS-156	14+20	CL	4.21-5.66	A-2-4(0)	19	NP	81.8	11.8	3.3	3.0	100	35	61		
SS-157	14+20	CL	8.74-9.23	A-2-4(0)	21	NP	76.5	10.9	5.3	7.0	100	42	11		
SS-158	14+20	CL	10.30-10.75	A-2-4(0)	23	NP	16.1	69.5	6.3	8.0	100	33	13		

L2RPBD

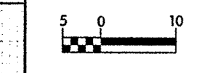
47.630

End Grade
 -L2RPBD- Sta. 17+94.470 =
 -L- Sta. 98+50.000
 Elev. 47.630



SOFT GRAY SILTY CLAY
 MED. STIFF
 TAN BRN SILTY F SDY CLAY
 Existing Ground At L2RPBD
 LOOSE TO MED DENSE
 TAN-WHITE F-CSE MDT TO SAT SAND
 V. SOFT BRN SDY SILT MDT (ALLUVIUM)
 SOFT BRN SDY SILT MDT
 LOOSE TO MED DENSE TAN F-CSE SAND MDT
 V. SOFT BRN SDY SILT MDT (ALLUVIUM)
 V. SOFT BRN SDY SILT MDT (ALLUVIUM)
 SAND V. LOOSE CLY F SD W/ SHELLS, WET TO SAT
 GRAY F-CSE SD W/ SHELLS SAT
 V. STIFF DARK GRAY BRN. CLAY, WET (BLACK CREEK FM)
 V. STIFF SILTY DARK GRAY BRN. CLAY, WET (BLACK CREEK FM)

DATE: 10/20/05
 FILE: 2554B-Roadway-Design-Drawings-10/20/05.dwg
 DRAWN: JMB



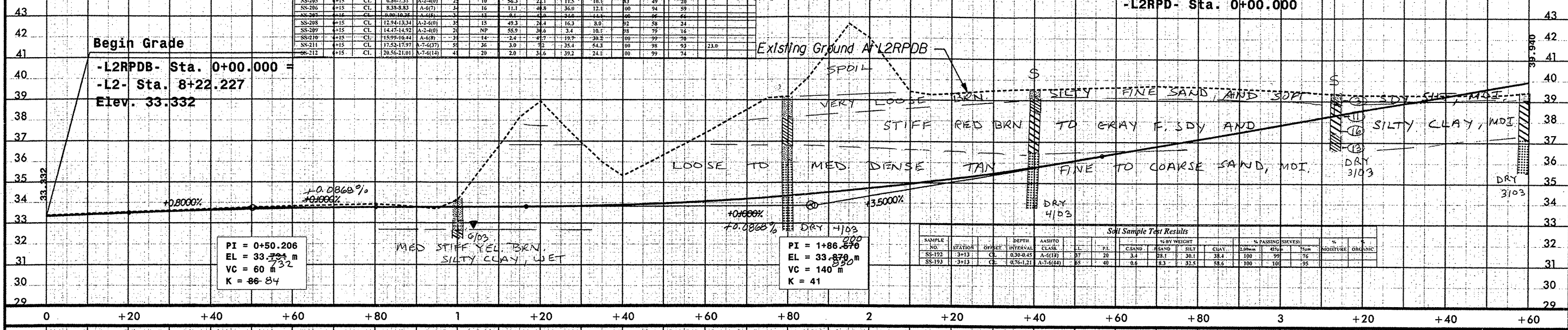
INCOMPLETE PLANS
 DO NOT USE FOR R/W ACQUISITION
 PRELIMINARY PLANS
 DO NOT USE FOR CONSTRUCTION

CONST. REV.
 R/W REV.

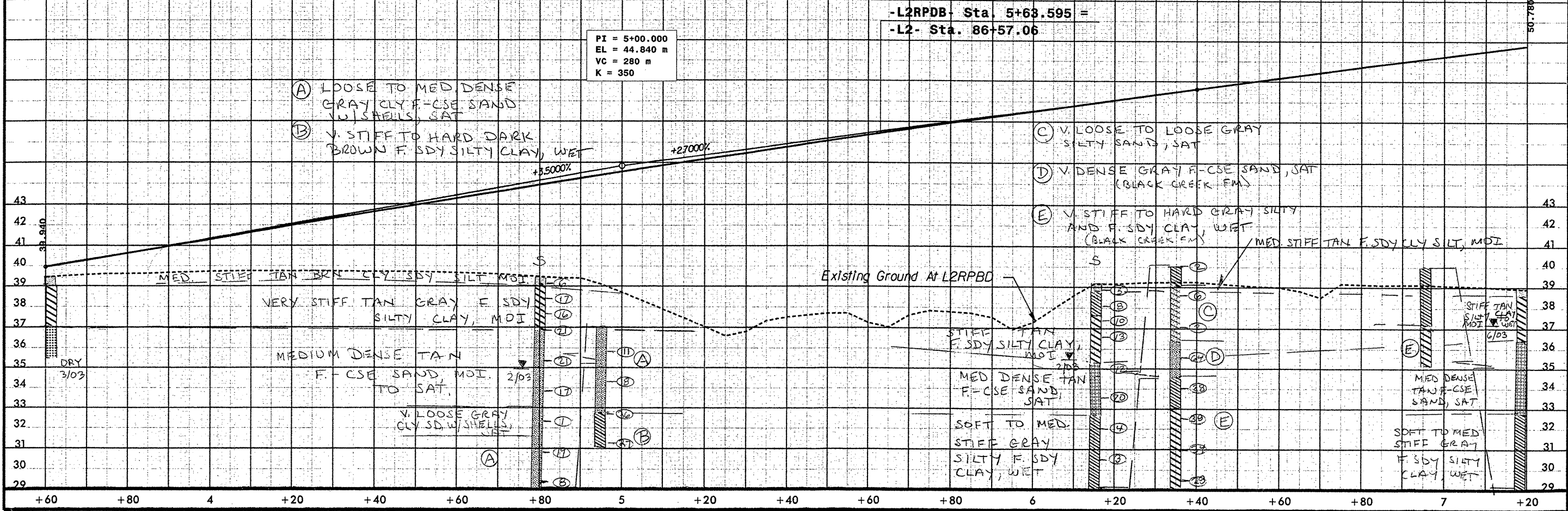
L2RPDB

Soil Sample Test Results														
SAMPLE NO.	STATION	DEPTH INTERVAL	AASHTO CLASS.	L	PL	% BY WEIGHT				% PASSING (SIEVES)			% MOISTURE	% ORGANIC
						CSAND	FSAND	SILT	CLAY	200µm	425µm	75µm		
SS-170	+40	CL 0.31-1.07	A-6(13)	4	24	8.3	32.3	24.2	33.3	100	97	64		
SS-171	+40	CL 1.07-3.05	A-7(650)	7	47	0.6	14.5	32.3	52.5	100	100	92		
SS-273	+80	CL 0.76-1.21	A-7(617)	4	22	2.2	24.3	35.4	38.1	100	99	79		
SS-274	+80	CL 1.21-2.40	A-7(617)	4	22	2.2	24.3	35.4	38.1	100	99	79		
SS-275	+80	CL 2.40-3.39	A-2(40)	2	8	50.4	34.9	6.7	8.0	95	60	17		
SS-276	+80	CL 3.39-10.39	A-2(40)	2	7	22.9	5.6	9.5	16.0	91	80	24		
SS-277	+80	CL 12.98-13.43	A-2(40)	2	8	33.3	42.5	9.1	14.8	87	69	22		
SS-278	+80	CL 12.54-14.96	A-6(15)	3	21	8.2	21.1	33.6	36.1	100	96	79		
SS-281	+15	CL 0.30-0.45	A-6(15)	3	10	6.0	21.8	35.0	33.2	100	97	77		
SS-282	+15	CL 0.76-1.21	A-6(9)	2	14	4.0	23.5	35.3	36.2	100	98	80		
SS-283	+15	CL 2.24-2.73	A-7(631)	5	35	3.8	19.1	26.8	50.3	100	98	85		
SS-284	+15	CL 3.31-4.26	A-1(50)	20	NP	81.3	13.1	1.6	4.0	98	44	6		
SS-285	+15	CL 6.86-7.31	A-2(40)	2	10	56.3	22.1	11.5	10.1	83	49	20		
SS-286	+15	CL 8.38-8.83	A-6(7)	3	15	11.1	48.8	36.0	12.1	100	94	59		
SS-287	+15	CL 9.90-10.35	A-6(8)	3	14	6.1	48.8	31.6	13.5	100	96	64		
SS-288	+15	CL 12.94-13.34	A-2(40)	3	15	49.3	24.4	16.3	8.0	92	58	24		
SS-289	+15	CL 14.47-14.92	A-2(40)	2	NP	55.9	34.6	3.4	10.1	98	79	16		
SS-290	+15	CL 15.99-16.44	A-6(8)	3	14	2.4	47.7	19.7	20.2	100	99	70		
SS-291	+15	CL 17.53-17.97	A-7(637)	5	35	3.0	2	35.4	54.3	100	98	93	23.0	
SS-292	+15	CL 20.56-21.01	A-7(614)	4	20	2.0	34.6	39.2	24.1	100	99	74		

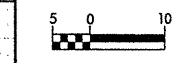
-L2RPDB- Sta. 3+13.291 =
 -L2RPD- Sta. 0+00.000



-L2RPDB- Sta. 5+63.595 =
 -L2- Sta. 86+57.06



L2RPDB

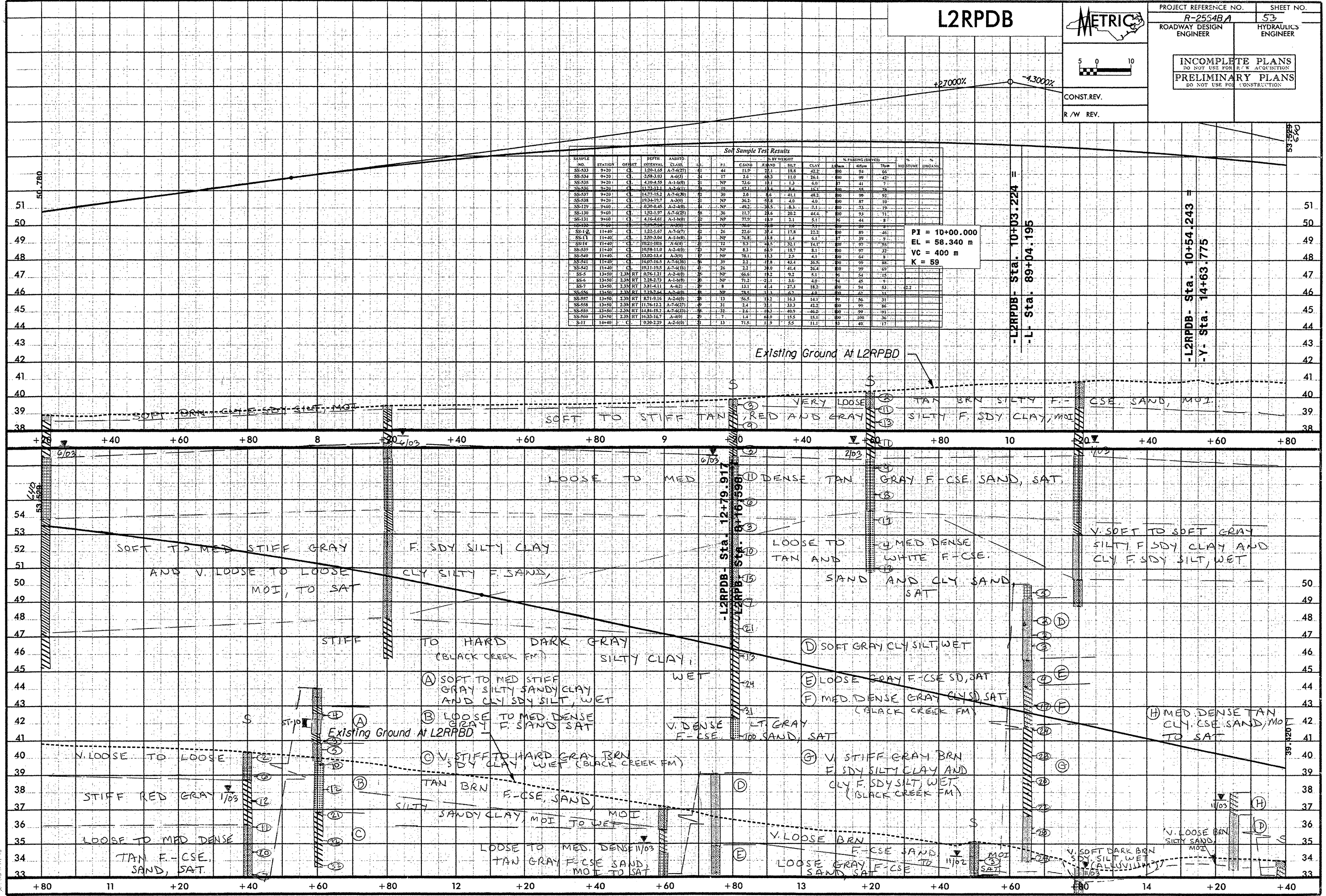


INCOMPLETE PLANS
 DO NOT USE FOR R/W ACQUISITION
PRELIMINARY PLANS
 DO NOT USE FOR CONSTRUCTION

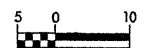
CONST. REV.
 R/W REV.

Soil Sample Test Results														
SAMPLE NO.	STATION	DEPTH	DEPTH INTERVAL	CLASS	LI	PL	CLAY	FINE SAND	SILT	CLAY	20um	45um	75um	% ORGANIC
SS-53	9+20	C	1.20-1.65	A-7(60)	41	44	11.9	3.1	18.8	42.2	80	94	99	42
SS-54	9+20	C	2.58-3.03	A-6(3)	34	17	2.6	66.3	11.0	26.1	80	99	99	42
SS-55	9+20	C	4.10-4.55	A-1(60)	1	NP	73.6	19.1	1.3	6.0	97	41	71	
SS-56	9+20	C	11.72-12.1	A-2(60)	20	19	57.1	18.4	2.4	12.1	80	94	98	
SS-57	9+20	C	14.77-15.2	A-7(60)	41	39	2.8	6.6	41.1	48.2	80	99	92	
SS-58	9+20	C	19.34-19.7	A-3(6)	31	NP	36.2	55.8	4.0	4.0	80	97	10	
SS-129	9+60	C	0.30-0.45	A-2(60)	41	NP	49.2	38.5	8.3	7.1	80	73	19	
SS-130	9+60	C	1.52-1.97	A-7(625)	28	36	11.7	21.6	20.2	44.4	80	93	71	
SS-131	9+60	C	4.16-4.61	A-1(60)	22	NP	77.9	19.9	2.1	5.1	76	44	8	
SS-132	9+60	C	20.99-21.4	A-7(60)	41	NP	29.6	37.4	17.8	22.1	80	89	46	
SS-12	11+40	C	1.22-1.67	A-7(67)	22	26	23.6	39.9	3.9	1.9	80	99	89	
SS-11	11+40	C	2.59-3.04	A-1(60)	33	NP	76.8	18.8	1.4	6.1	97	39	9	
SS-14	11+40	C	10.21-10.6	A-6(9)	12	12	5.3	48.5	32.1	14.1	100	97	55	
SS-539	11+40	C	10.58-11.0	A-2(60)	20	NP	8.1	68.9	18.7	8.1	80	97	32	
SS-60	11+40	C	13.23-13.7	A-3(6)	17	NP	78.1	18.3	2.5	4.1	80	64	8	
SS-541	11+40	C	15.07-16.5	A-7(635)	36	39	2.2	11.8	43.1	35.5	80	99	88	
SS-542	11+40	C	19.11-19.5	A-7(616)	41	26	2.2	39.0	41.4	26.4	80	99	69	
SS-5	13+80	2.3M RT	0.76-1.21	A-2(60)	35	NP	66.6	19.2	9.2	5.1	86	54	15	
SS-6	13+80	2.3M RT	2.28-2.73	A-1(60)	20	NP	71.2	21.1	3.6	4.0	94	45	9	
SS-7	13+80	2.3M RT	3.81-4.11	A-6(3)	29	8	15.1	4.4	27.8	18.2	80	94	53	12
SS-56	13+80	2.3M RT	7.85-7.8	A-2(60)	20	NP	78.5	18.3	4.2	4.0	80	99	91	
SS-567	13+80	2.3M RT	8.71-9.16	A-2(60)	20	13	56.5	18.2	16.3	14.1	9	56	31	
SS-568	13+80	2.3M RT	11.76-12.2	A-7(625)	49	31	2.4	7.1	33.3	43.2	80	99	86	
SS-569	13+80	2.3M RT	14.81-15.3	A-7(625)	38	32	2.6	16.3	40.9	46.2	80	99	91	
SS-569	13+80	2.3M RT	16.33-16.7	A-1(60)	22	7	4.4	6.0	15.5	15.1	80	100	36	
S-11	14+40	C	0.30-2.29	A-2(60)	31	13	71.8	19.9	5.5	11.1	93	40	17	

PI = 10+00.000
 EL = 58.340 m
 VC = 400 m
 K = 59



11/20/2013
 R. Brown, Proj. Engineer
 Branch Name



PROJECT REFERENCE NO. R-2554BA SHEET NO. 54

ROADWAY DESIGN ENGINEER HYDRAULICS ENGINEER

INCOMPLETE PLANS
DO NOT USE FOR R/W ACQUISITION
PRELIMINARY PLANS
DO NOT USE FOR CONSTRUCTION

CONST.REV.

R/W REV.

L2RPDBPI = 15+60.000
EL = 34.260 m
VC = 170 m
K = 45

Soil Sample Test Results

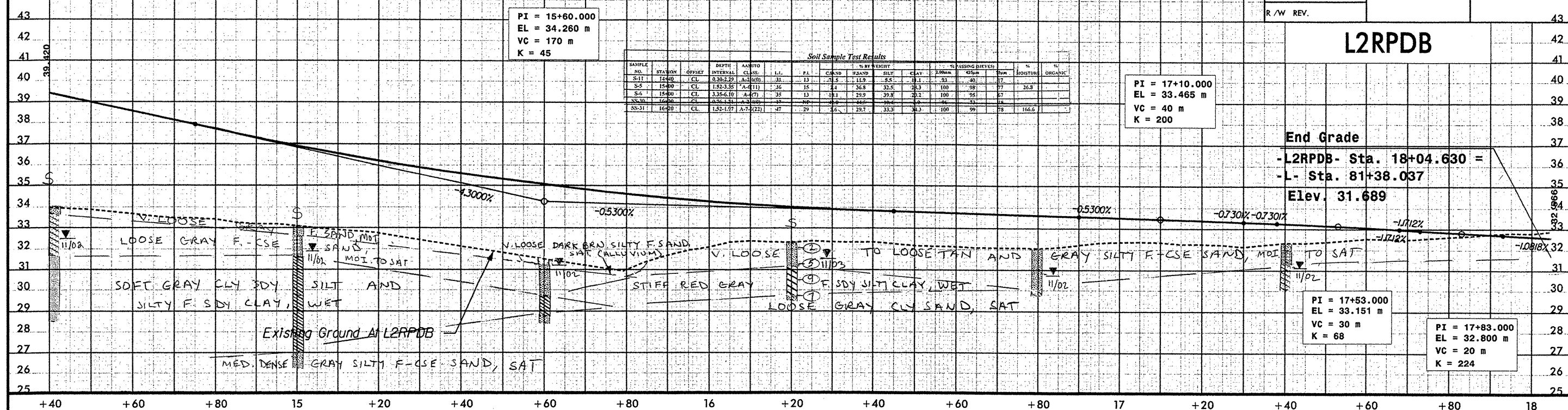
SAMPLE NO.	STATION	OFFSET	DEPTH INTERVAL	CLASS.	L.L.	P.I.	% BY WEIGHT		% PASSING SIEVES		SHR	MOISTURE	% ORGANIC
							CSAND	FSAND	SILT	CLAY			
S-11	14+40	CL	0.30-2.29	A-2(0)	31	13	5.5	11.9	5.5	91	40	17	
S-S	15+00	CL	1.52-3.35	A-4(1)	36	15	4.1	36.8	32.5	28.3	100	98	26.8
S-6	16+00	CL	3.35-6.10	A-7(2)	35	13	10.1	29.9	39.8	23.2	100	95	27
SS-30	16+00	CL	3.35-6.10	A-7(2)	35	13	10.1	29.9	39.8	23.2	100	95	27
SS-31	16+00	CL	1.52-1.97	A-7(2)	47	29	6.6	29.7	33.3	34.3	100	99	166.6

PI = 17+10.000
EL = 33.465 m
VC = 40 m
K = 200**End Grade**

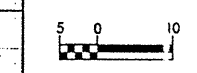
-L2RPDB- Sta. 18+04.630 =

-L- Sta. 81+38.037

Elev. 31.689

PI = 17+53.000
EL = 33.151 m
VC = 30 m
K = 68PI = 17+83.000
EL = 32.800 m
VC = 20 m
K = 224

1/21/05
 J. J. Gibson
 P.E.
 METRIC
 1425 South Hwy 138
 Austin, TX 78748

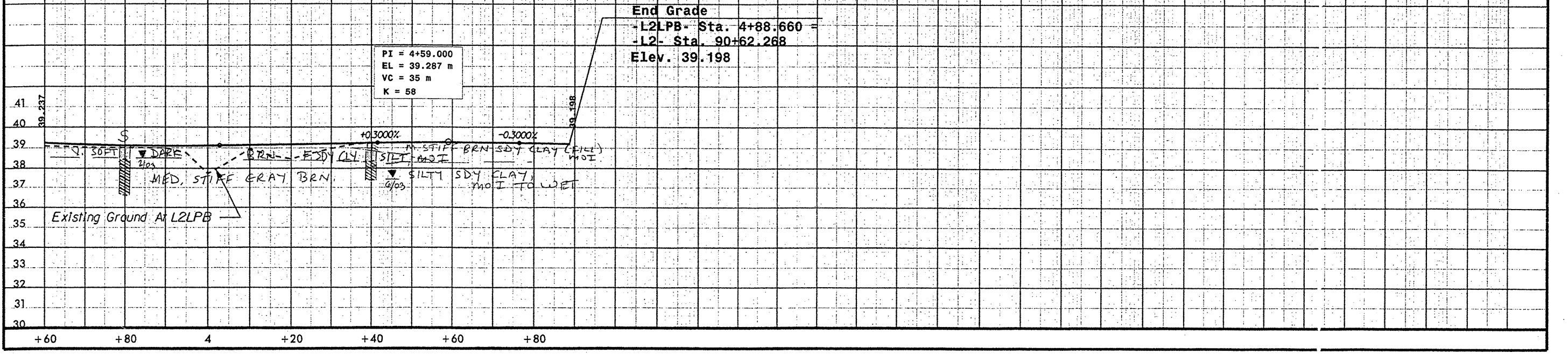
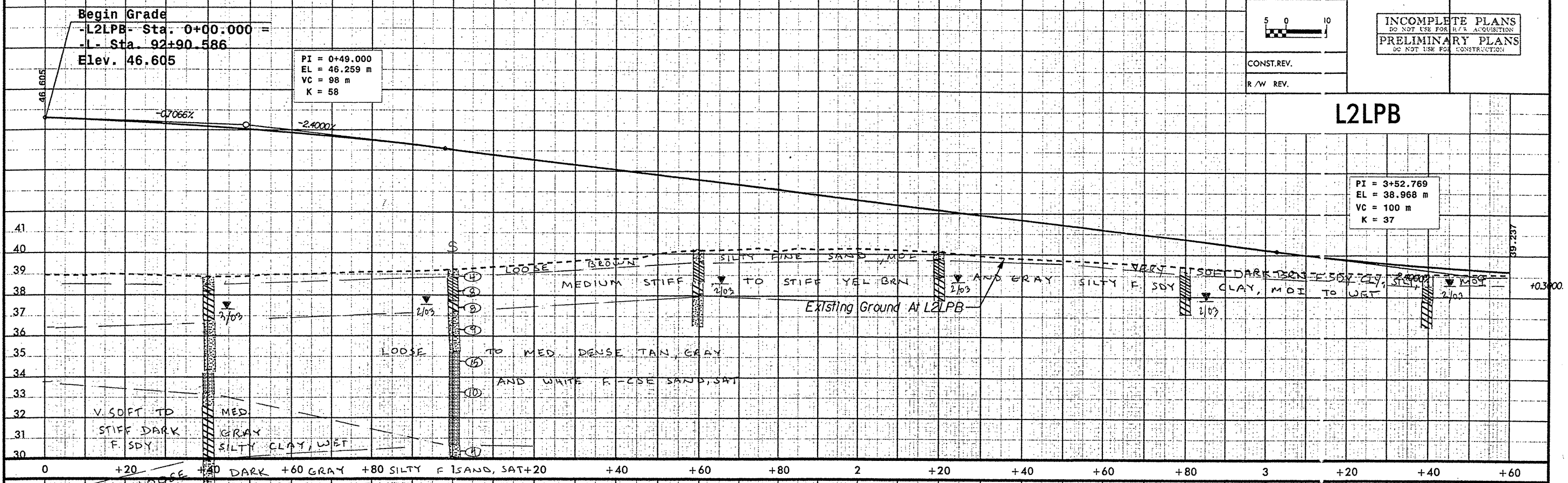


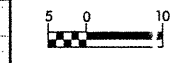
INCOMPLETE PLANS
 DO NOT USE FOR ACQUISITION
 PRELIMINARY PLANS
 DO NOT USE FOR CONSTRUCTION

CONST. REV.
 R/W REV.

L2LPB

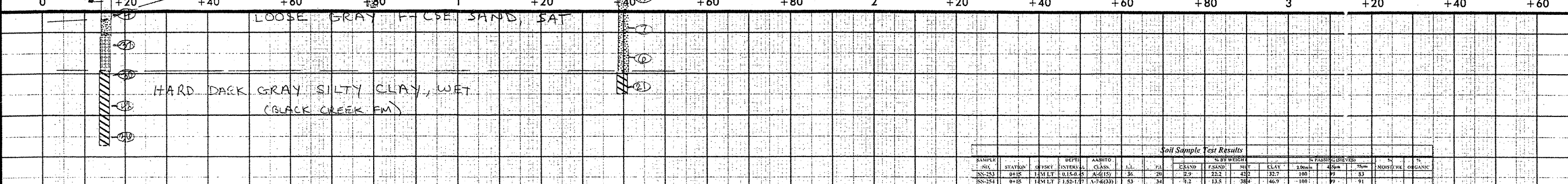
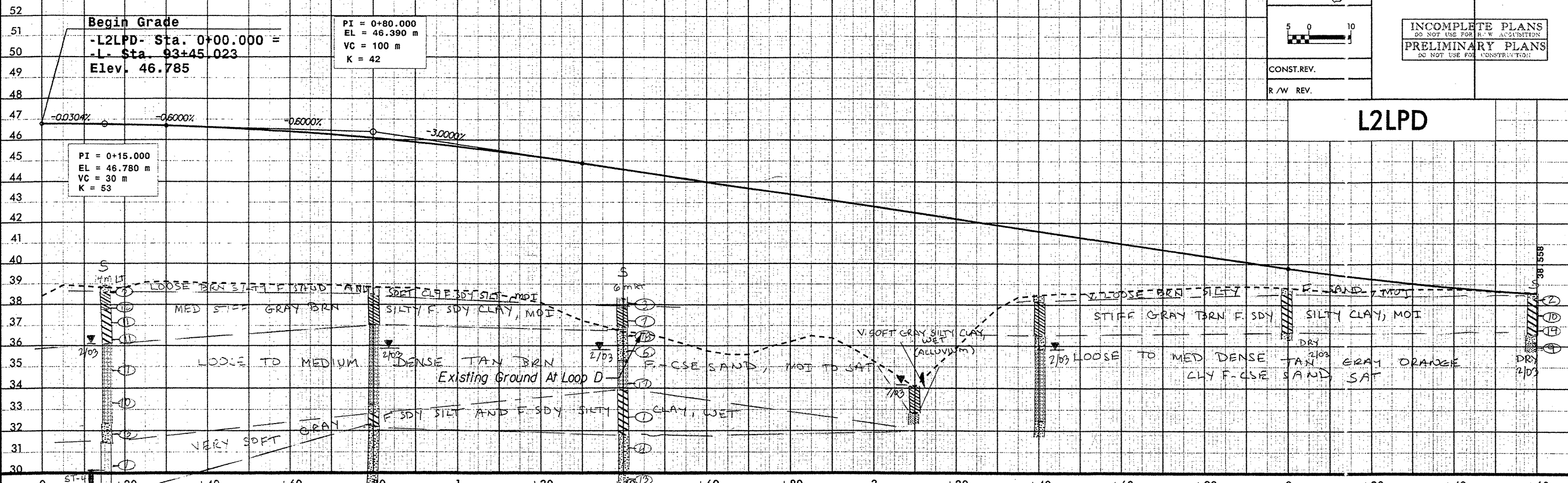
PI = 3+52.769
 EL = 38.968 m
 VC = 100 m
 K = 37





CONST. REV.
 R/W REV.

L2LPD



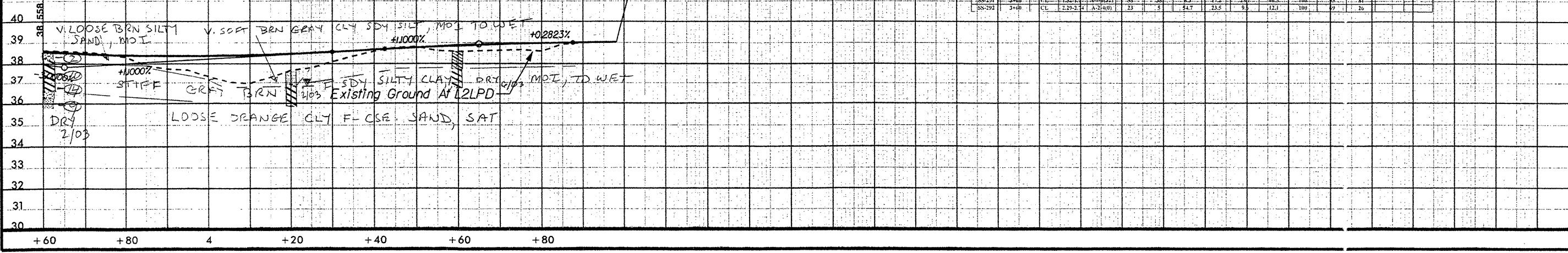
PI = 3+65.000
 EL = 37.840 m
 VC = 130 m
 K = 32

PI = 4+65.000
 EL = 38.940 m
 VC = 45 m
 K = 55

End Grade
 -L2LPD- Sta. 4+98.146 =
 -L2- Sta. 90+07.831
 Elev. 39.034

Soil Sample Test Results

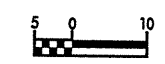
SAMPLE NO.	STATION	DEPTH	CLASS.	L.L.	P.L.	% BY WEIGHT			% PASSING (SIEVES)			MOISTURE	ORGANIC
						CLAY	F.SAND	SILT	200µ	45µ	75µ		
SS-243	0+15	1.51-1.77	A-1(1)	36	70	2.9	23.2	42.0	33.7	100	89	83	
SS-244	0+15	1.52-1.77	A-2(1)	53	24	6.2	13.5	38.4	46.9	100	99	91	
SS-255	0+15	1.51-1.76	A-1(1)	19	NP	74.2	20.5	1.1	4.1	88	89	6	
SS-256	0+15	1.51-1.76	A-1(1)	35	NP	31.6	63.7	0.2	4.1	100	91	15	
SS-257	0+15	1.51-1.76	A-1(1)	27	8	6.1	57.8	30.0	18.2	100	88	48	
SS-258	0+15	1.51-1.76	A-1(1)	25	NP	82.0	12.0	0.1	5.1	92	81	7	
SS-259	0+15	1.52-1.77	A-2(1)	52	25	2.2	24.9	46.3	26.5	100	99	85	
SS-260	0+15	1.51-1.76	A-2(1)	47	20	8.5	16.3	43.5	34.7	100	87	84	
SS-293	0+140	0.15-0.5	A-1(1)	16	NP	4.4	44.4	33.0	18.2	100	88	60	
SS-294	0+140	0.76-1.1	A-1(1)	27	19	2.2	39.2	30.5	28.2	100	99	72	
SS-295	0+140	1.52-1.77	A-2(1)	24	5	2.6	67.9	7.3	22.1	100	80	33	
SS-296	0+140	3.81-4.6	A-1(1)	20	NP	83.4	12.0	0.1	4.0	78	72	4	
SS-297	0+140	5.33-5.9	A-2(1)	46	16	31.6	15.9	36.5	16.1	100	84	60	58.9
SS-298	0+140	6.85-7.0	A-2(1)	27	NP	62.0	19.9	8.0	10.1	94	81	18	24.9
SS-299	0+140	8.06-10.8	A-2(1)	21	NP	18.8	52.5	4.8	27.0	95	86	13	
SS-300	0+140	12.95-14.6	A-2(1)	47	21	4.0	12.3	47.5	35.2	100	97	89	
SS-299	3+60	0.15-0.5	A-2(1)	47	28	8.1	19.6	20.9	48.5	100	97	80	
SS-291	3+60	1.52-1.77	A-2(1)	53	38	8.3	17.3	24.1	46.5	100	85	81	
SS-292	3+60	2.29-2.4	A-2(1)	23	5	54.7	23.5	9.3	12.1	100	87	26	



6/09/09



PROJECT REFERENCE NO. R-2554B A	SHEET NO. 57
ROADWAY DESIGN ENGINEER	HYDRAULICS ENGINEER



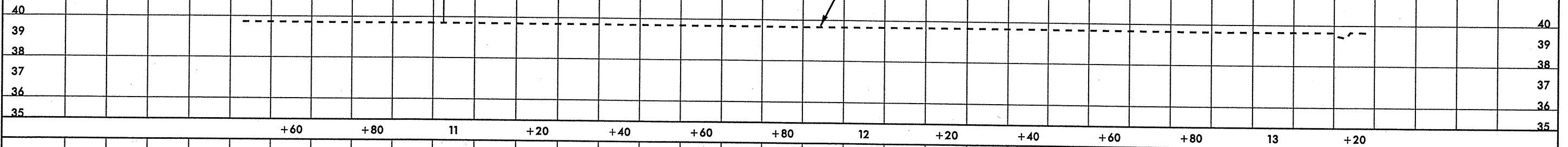
INCOMPLETE PLANS
DO NOT USE FOR R/W ACQUISITION
PRELIMINARY PLANS
DO NOT USE FOR CONSTRUCTION

CONST. REV.
R/W REV.

-Y1-

-Y- Sta. 10+97.530 =
-L- Sta. 100+61.488

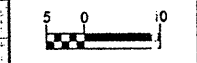
Existing Ground At -Y1-
(CSX RAILROAD)



14-SEP-2005 09:00
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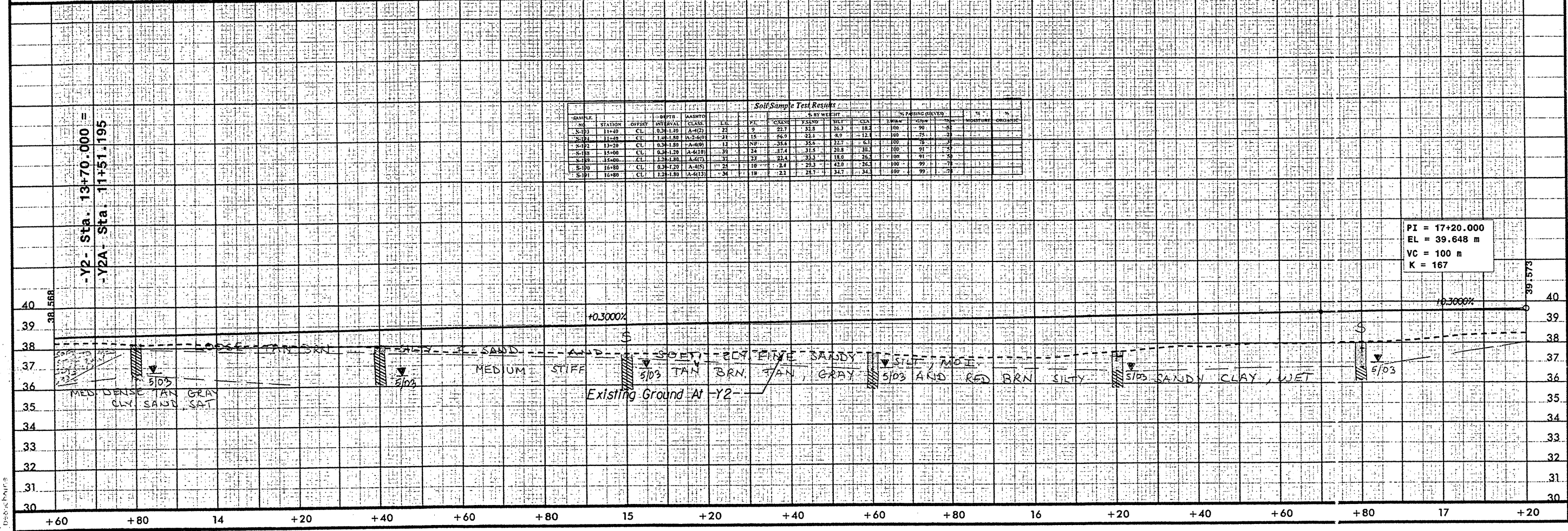
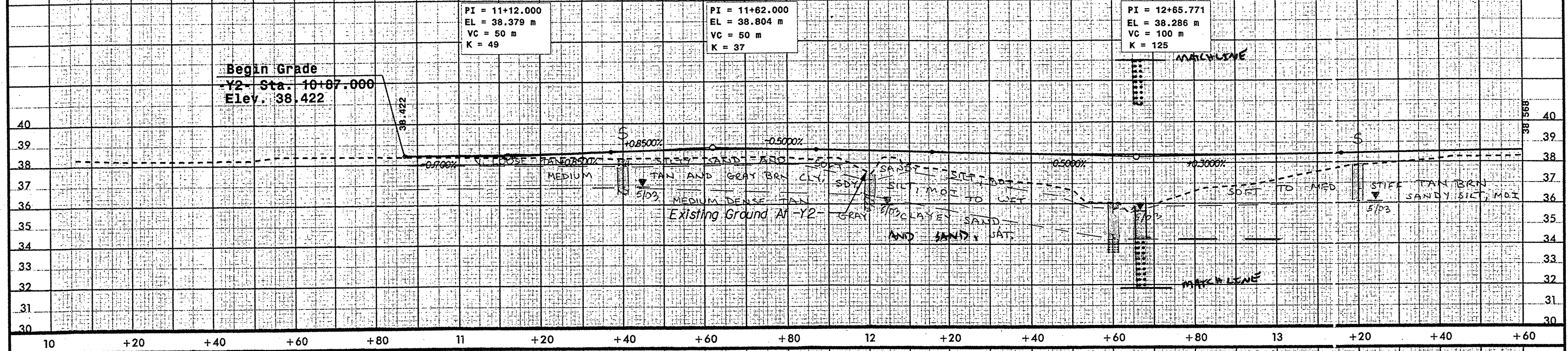


PROJECT REFERENCE NO. R-25548A SHEET NO. 5B
 ROADWAY DESIGN ENGINEER HYDRAULICS ENGINEER
 INCOMPLETE PLANS DO NOT USE FOR R/W ACQUISITION
 PRELIMINARY PLANS DO NOT USE FOR CONSTRUCTION



CONST. REV.
 R/W REV.

-Y2-



Soil Sample Test Results

SAMPLE	STATION	DEPTH	LAB	CLASS	% BY WEIGHT				% PASSING SIEVES		MOISTURE	ORGANIC	
					W	P	CS	FS	NO. 10	NO. 20			
S-101	11+40	CL	0.3-1.40	A-4(2)	22	9	23.7	31.8	26.3	18.2	100	90	5
S-102	11+40	CL	1.40-1.80	A-2-4(0)	21	15	56.9	23.1	8.9	12	100	75	2
S-103	13+20	CL	0.3-1.80	A-4(0)	12	NP	25.6	25.6	22.1	6.1	100	6	5
S-104	15+00	CL	0.3-1.20	A-6(10)	30	24	27.4	31.5	20.8	30	100	91	5
S-105	15+00	CL	1.20-1.80	A-6(1)	30	25	22.4	33.3	18.0	26	100	91	5
S-106	16+80	CL	0.3-1.20	A-4(5)	25	10	2.4	29.3	42.0	26	100	99	7
S-107	16+80	CL	1.20-1.80	A-6(13)	34	18	2.1	28.7	34.7	34	100	99	7

5/14/00
 J. J. Jones
 2005-05-19
 2005-05-19
 2005-05-19

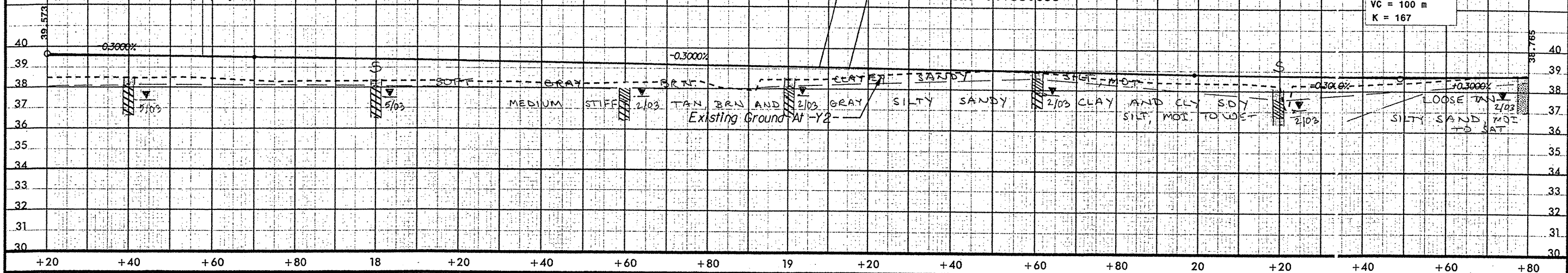
-Y2-

PI = 17+20.000
 EL = 39.648 m
 VC = 100 m
 K = 167

-Y2- Sta. 17+57.703 =
 -SR5- Sta. 10+00.000 =
 -Y2B- Sta. 11+49.340 =

-Y2- Sta. 19+07.703
 -Y2RPD- Sta. 16+47.523
 -Y2- Sta. 19+15.203
 -Y2LPD- Sta. 14+83.699

PI = 20+49.000
 EL = 38.661 m
 VC = 100 m
 K = 167



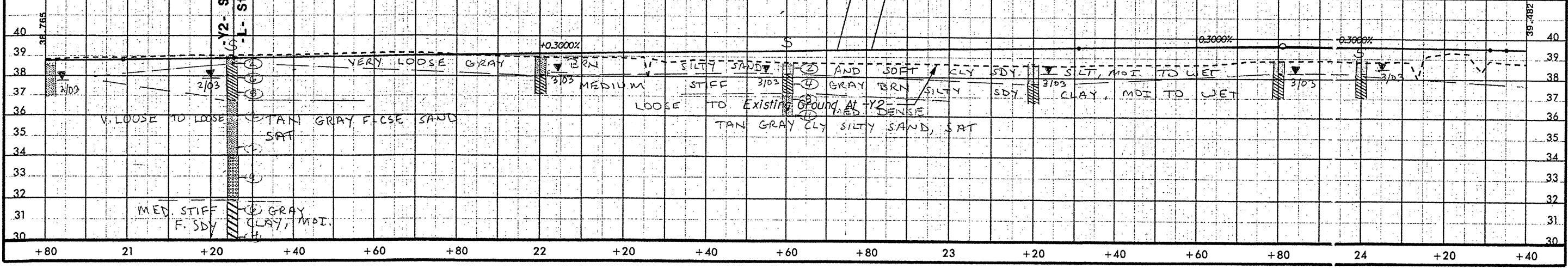
Soil Sample Test Results

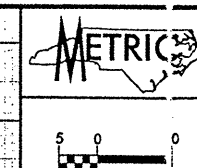
SAMPLE NO.	STATION	DEPTH	TESTS	PI	% BY WEIGHT			% PASSING (SIEVES)		LIQ. LIMIT	PLASTICITY INDEX	UNIFORMITY COEFFICIENT
					CLAY	SILT	SAND	NO. 20	NO. 40			
S-74	20+20	CL	0.30-0.40 A-4(1)	21	8	24.1	25.3	32.2	15.4	100	8	57
S-75	20+20	CL	0.46-0.70 A-6(1)	20	12	19.0	22.2	26.1	32.7	100	8	65
S-76	20+20	CL	0.76-1.23 A-8(10)	14	20	17.6	21.8	25.9	34.7	100	8	66
S-77	20+20	CL	1.25-1.81 A-4(1)	23	10	22.4	27.6	33.4	26.5	100	8	58
SS-182	21+25	CL	0.30-0.45 A-3-4(1)	15	NP	24.5	48.5	17.0	1.1	100	8	30
SS-183	21+25	CL	0.76-1.23 A-6(1)	12	16	22.6	26.6	14.5	26.3	100	8	67
SS-184	21+25	CL	1.68-3.11 A-2-4(1)	28	7	52.3	31.0	2.5	24.1	94	5	17
SS-185	21+25	CL	1.21-1.66 A-2-8(1)	23	NP	37.7	51.5	5.7	8.1	100	8	11
SS-186	21+25	CL	3.74-6.19 A-1-8(1)	24	NP	90.7	6.2	0.1	3.0	98	3	3
SS-187	21+25	CL	1.25-1.70 A-2-6(1)	12	27	16.5	49.9	8.3	22.3	100	9	44
SS-188	21+25	CL	10.30-10.5 A-2-4(1)	22	1	24.2	52.1	13.5	10.1	98	4	25
SS-189	21+25	CL	13.35-13.8 A-2-4(1)	27	NP	22.0	55.6	5.3	17.2	81	7	19
SS-190	21+25	CL	14.87-15.8 A-2-4(1)	24	NP	38.1	53.7	5.2	3.0	100	8	11
SS-412	22+60	CL	0.30-0.45 A-4(1)	16	12	38.2	25.7	20.9	12.1	98	7	39
SS-421	22+60	CL	0.76-1.23 A-6(2)	15	12	34.1	27.7	20.0	25.2	99	7	45
SS-422	22+60	CL	1.52-1.97 A-2-4(1)	10	5	44.4	31.7	11.7	22.1	99	7	29
S-78	24+00	CL	0.30-0.61 A-4(1)	19	4	21.4	39.3	42.1	16.1	100	9	71
S-79	24+00	CL	0.61-1.33 A-6(10)	31	16	1.8	30.6	35.9	32.3	100	9	22.9

PI = 23+81.000
 EL = 39.657 m
 VC = 100 m
 K = 167

-Y2- Sta. 21+25.038 =
 -L- Sta. 103+80.709 =

-Y2- Sta. 22+72.703
 -Y2LPA- Sta. 14+06.637 =
 -Y5- Sta. 13+05.503
 -Y2- Sta. 22+81.203
 -Y2RPA- Sta. 16+56.187





CONST. REV.
 R / W REV.

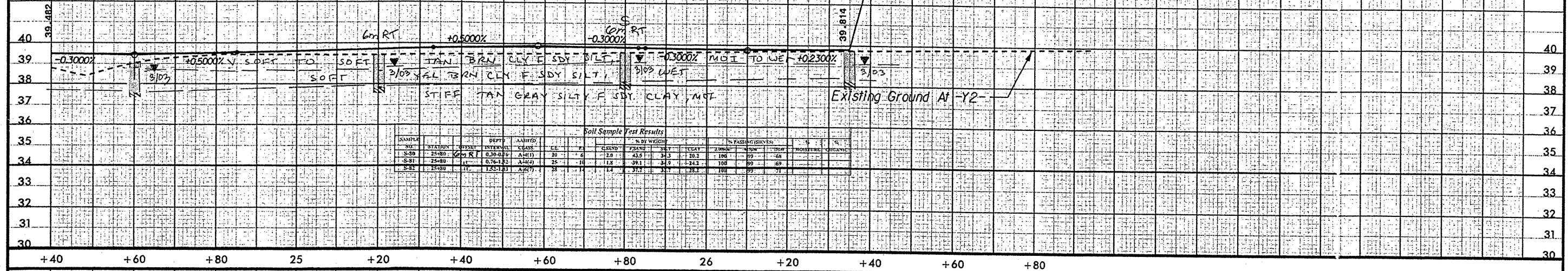
-Y2-

PI = 24+60.000
 EL = 39.420 m
 VC = 50 m
 K = 63

PI = 25+58.271
 EL = 39.912 m
 VC = 50 m
 K = 63

PI = 26+10.000
 EL = 39.757 m
 VC = 50 m
 K = 94

End Grade
 -Y2- Sta. 26+35.000
 Elev. 39.814

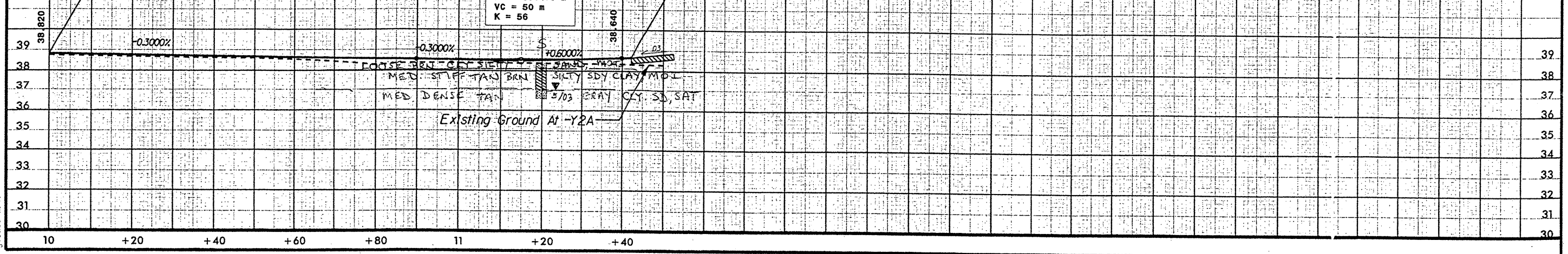


-Y2A-

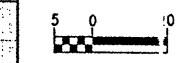
Begin Grade
 -Y2A- Sta. 10+00.000
 Elev. 38.820

PI = 11+14.850
 EL = 38.475 m
 VC = 50 m
 K = 56

End Grade
 -Y2A- Sta. 11+42.190 =
 -Y2- Sta. 13+70.000
 Elev. 38.640



11-04-042
 R. J. ...
 J. ...

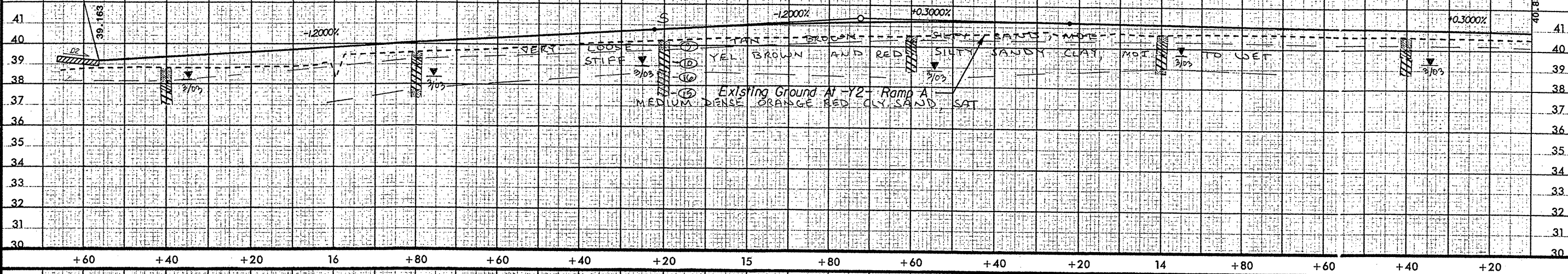


CONST. REV.
R/W REV.

-Y2- RAMP A

End Grade
 -Y2RPA- Sta. 16+56.187
 -Y2- Sta. 22+80.124
 Elev. 39.163

PI = 14+72.347
 EL = 41.369 m
 VC = 100 m
 K = 67



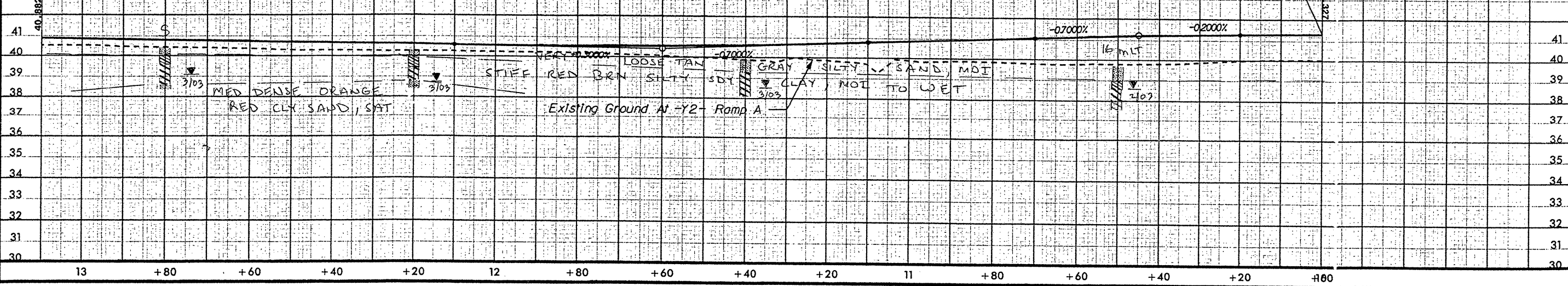
Soil Sample Test Results

SAMP. NO.	STATION	DEPTH	SANDS	CLAY	% BY WEIGHT		% PASSING (SIYES)					MOISTURE	PLASTICITY
					LL	PL	NO. 20	NO. 40	NO. 60	NO. 100	NO. 200		
S-97	12+80	CL	0.30-0.76	A-6(9)	39	2	28.6	39.7	63.4	36.3	100	38	86
S-98	12+80	CL	0.76-4.3	A-7-4(16)	61	0	17.0	27.3	10.8	44.4	100	21	60
SS-41	15+20	CL	0.30-0.76	A-7-6(9)	36	2	21.5	20.5	7.7	38.3	91	30	52
SS-42	15+20	CL	0.76-4.3	A-7-6(9)	52	2	10.0	19.4	1.3	32.3	91	22	41
SS-43	15+20	CL	2.25-4.3	A-2-6(0)	35	0	46.9	16.6	5.5	12.1	94	63	17

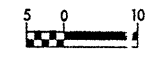
Begin Grade
 -Y2RPA- Sta. 10+00.000
 -L- Sta. 109+70.000
 Elev. 41.327

PI = 11+60.000
 EL = 40.432 m
 VC = 100 m
 K = 100

PI = 10+45.000
 EL = 41.237 m
 VC = 50 m
 K = 100



10-04-2010
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 10-04-2010



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 PRELIMINARY PLANS
 DO NOT USE FOR CONSTRUCTION

CONST. REV.
 R/W REV.

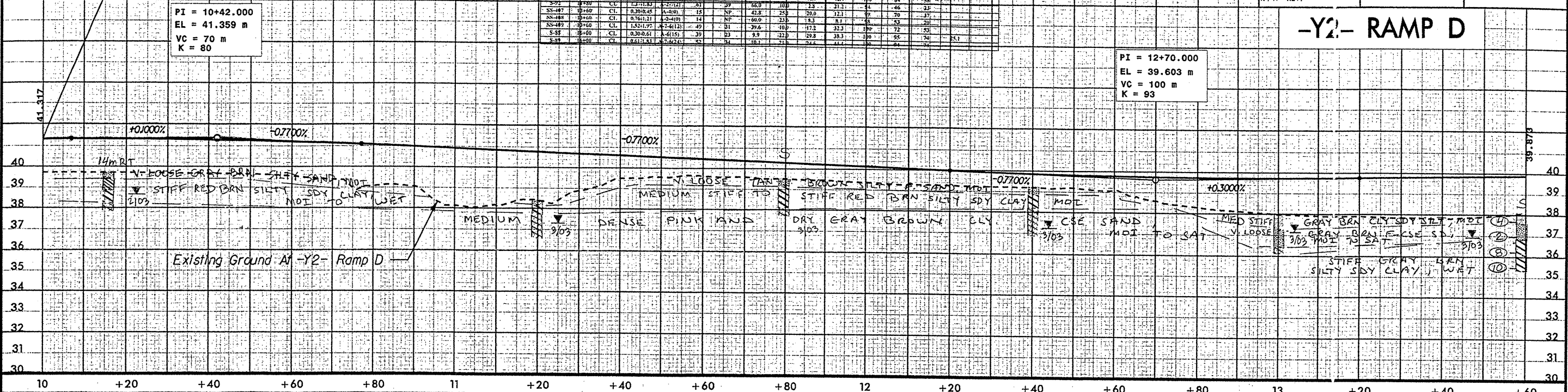
Begin Grade
 -Y2RPD- Sta. 10+00.000
 L Sta. 109+96.000
 Elev. 41.317

PI = 10+42.000
 EL = 41.359 m
 VC = 70 m
 K = 80

Soil Sample Test Results														
SAMPLE NO.	STATION	DEPTH	PERCENT	SAND	SILT	CLAY	LIQUIDITY		PLASTICITY		MOISTURE			
							W	U	PI	SH				
S-79	10+40	CL	0.300	77-81(5)	49	28	23.6	198	14.2	42.4	100	86	62	23.2
S-91	10+88	CL	0.761	77-81(5)	84	31	27.9	178	12.9	42.4	100	84	58	
S-92	10+90	CL	1.371	77-81(5)	81	39	66.0	100	12.9	42.4	100	84	58	
SS-107	10+60	CL	0.300	A-1(0)	15	NP	41.5	250	26.0	12.1	79	79	17	
SS-108	10+60	CL	0.761	A-2-1(0)	14	NP	60.0	23.5	18.3	8.1	79	79	17	
SS-109	10+60	CL	1.521	A-7-61(2)	49	B1	39.6	100	17.2	32.3	100	72	53	
S-38	10+60	CL	0.300	A-1(0)	39	B3	9.9	222	29.8	38.3	100	95	74	25.1
S-39	10+60	CL	0.611	A-7-61(2)	52	M	10.1	21	24.6	44.1	100	84	58	

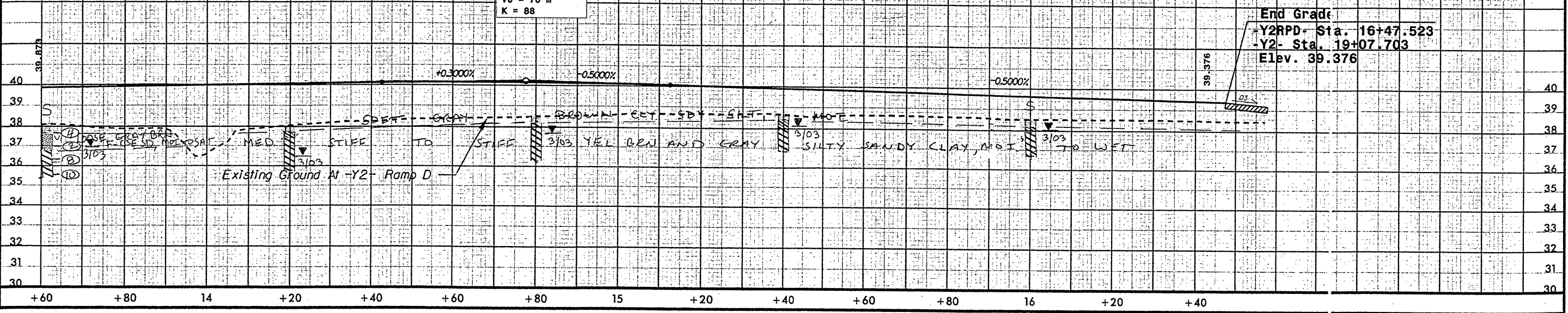
-Y2- RAMP D

PI = 12+70.000
 EL = 39.603 m
 VC = 100 m
 K = 93

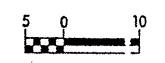


PI = 14+77.527
 EL = 40.226 m
 VC = 70 m
 K = 88

End Grade
 -Y2RPD- Sta. 16+47.523
 -Y2- Sta. 19+07.703
 Elev. 39.376



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INCOMPLETE PLANS
 DO NOT USE FOR ACQUISITION
PRELIMINARY PLANS
 DO NOT USE FOR CONSTRUCTION

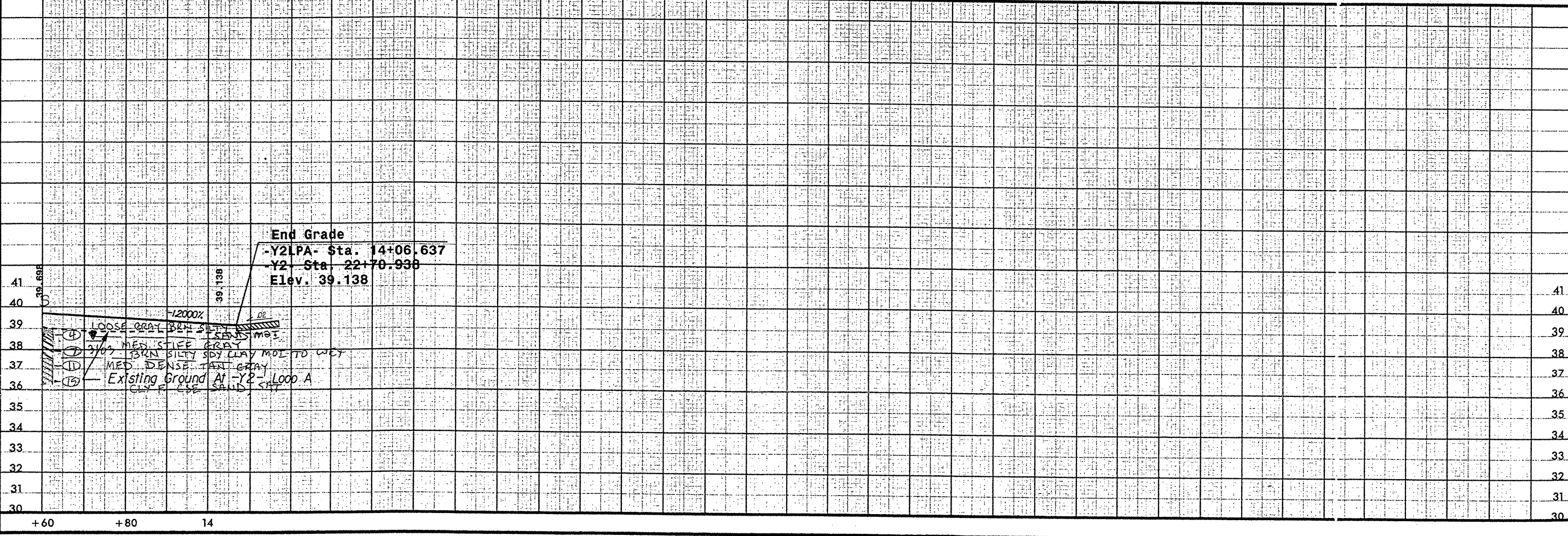
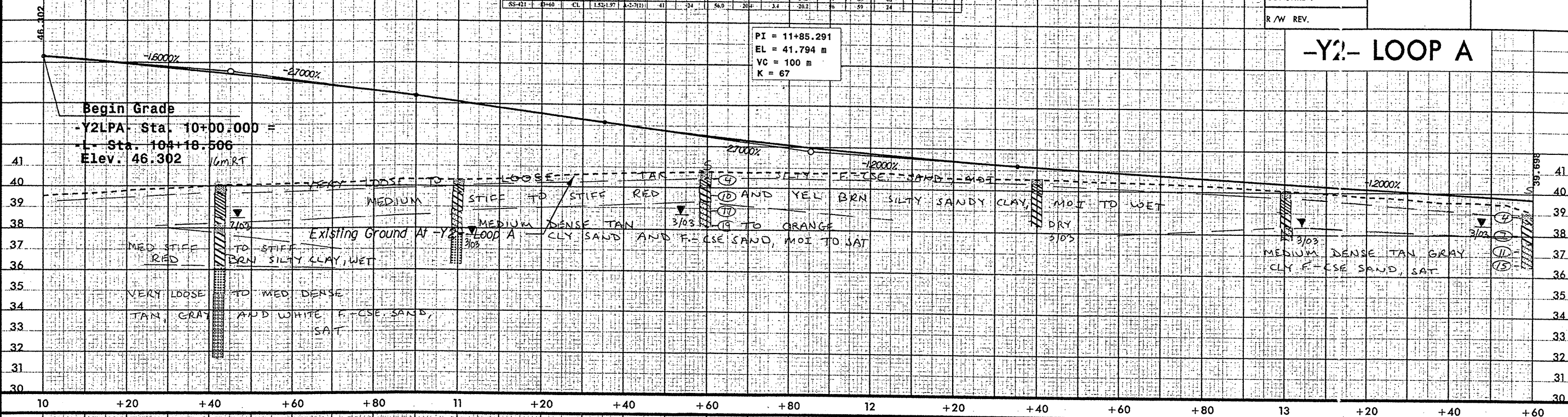
CONST. REV.
 R/W REV.

SAMPLE NO.	STATION	DEPTH	CLASS	LI	PL	BY WEIGHT		BY FINNING (YES)		MOISTURE %	ORGANIC %	
						SAND	SILT	CLAY	200µ			
SS-416	0+60	CL	0.30-0.45	44	20	30.5	25.5	13.7	30.3	83	78	4.1
SS-417	0+60	CL	0.76-1.21	43	22	35.8	23.0	10.5	30.3	83	73	4.4
SS-418	0+60	CL	1.35-1.97	43-7(1)	25	32.1	22.9	7.9	32.1	85	61	3.5
SS-419	0+60	CL	0.30-0.45	44-6(3)	28	31.5	29.1	30.0	16.4	24.2	75	41
SS-420	0+60	CL	0.76-1.21	43-7(1)	27	25.7	20.5	11.5	34.3	79	55	4.8
SS-421	0+60	CL	1.35-1.97	43-7(1)	41	34.0	20.4	3.4	20.2	86	59	2.4

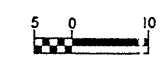
PI = 10+45.000
 EL = 45.582 m
 VC = 90 m
 K = 82

PI = 11+85.291
 EL = 41.794 m
 VC = 100 m
 K = 67

-Y2- LOOP A



4/11/2017
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 2554B.dwg



INCOMPLETE PLANS DO NOT USE FOR R/W ACQUISITION PRELIMINARY PLANS DO NOT USE FOR CONSTRUCTION

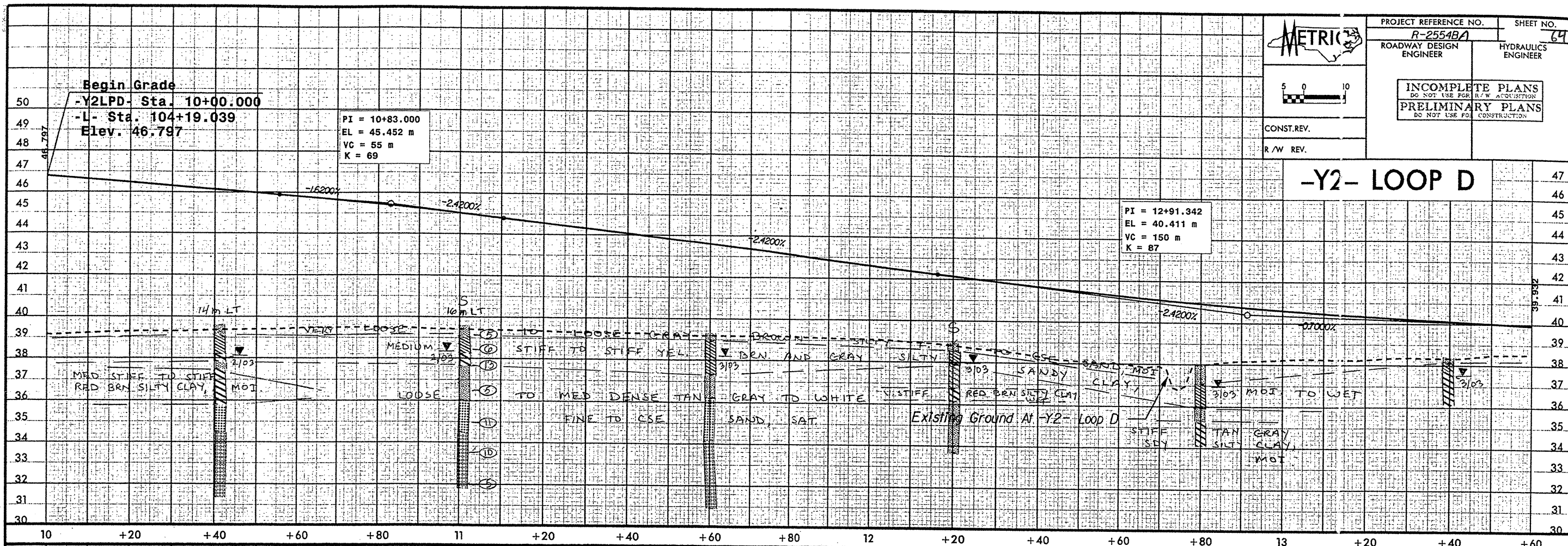
CONST. REV.
R/W REV.

-Y2- LOOP D

Begin Grade
-Y2LPD- Sta. 10+00.000
-L- Sta. 104+19.039
Elev. 46.797

PI = 10+83.000
EL = 45.452 m
VC = 55 m
K = 69

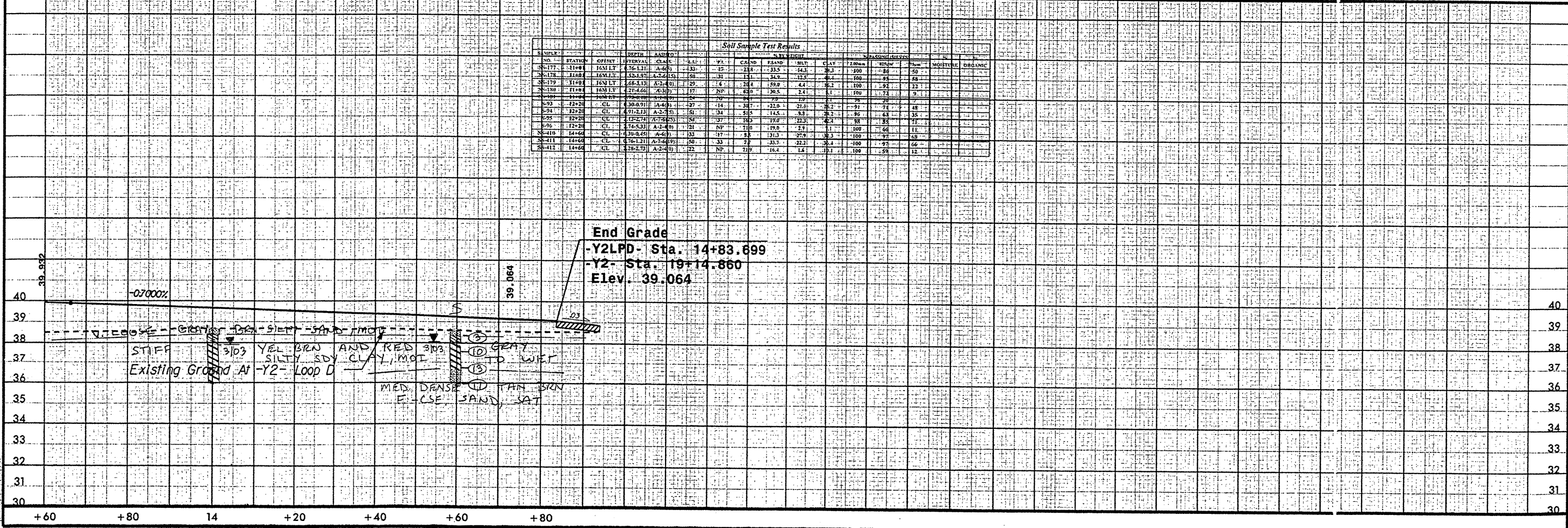
PI = 12+91.342
EL = 40.411 m
VC = 150 m
K = 87

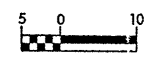


Soil Sample Test Results

SAMPLE NO.	STATION	DEPTH (m)	CLASS	W	L	P	U	FL	FR	CLAY	PI	LI	PL	SH	SH _{min}	SH _{max}	MOI	DR
SS-177	11+08	1.0-1.1	A-6	33	15	23	33.5	64.3	2.3	100	80	50	50	50	50	50	50	50
SS-178	11+08	1.0-1.1	A-6	33	15	23	33.5	64.3	2.3	100	80	50	50	50	50	50	50	50
SS-179	11+08	1.0-1.1	A-6	33	15	23	33.5	64.3	2.3	100	80	50	50	50	50	50	50	50
SS-180	11+08	1.0-1.1	A-6	33	15	23	33.5	64.3	2.3	100	80	50	50	50	50	50	50	50
SS-181	11+08	1.0-1.1	A-6	33	15	23	33.5	64.3	2.3	100	80	50	50	50	50	50	50	50
SS-182	11+08	1.0-1.1	A-6	33	15	23	33.5	64.3	2.3	100	80	50	50	50	50	50	50	50
SS-183	11+08	1.0-1.1	A-6	33	15	23	33.5	64.3	2.3	100	80	50	50	50	50	50	50	50
SS-184	11+08	1.0-1.1	A-6	33	15	23	33.5	64.3	2.3	100	80	50	50	50	50	50	50	50
SS-185	11+08	1.0-1.1	A-6	33	15	23	33.5	64.3	2.3	100	80	50	50	50	50	50	50	50
SS-186	11+08	1.0-1.1	A-6	33	15	23	33.5	64.3	2.3	100	80	50	50	50	50	50	50	50
SS-187	11+08	1.0-1.1	A-6	33	15	23	33.5	64.3	2.3	100	80	50	50	50	50	50	50	50
SS-188	11+08	1.0-1.1	A-6	33	15	23	33.5	64.3	2.3	100	80	50	50	50	50	50	50	50
SS-189	11+08	1.0-1.1	A-6	33	15	23	33.5	64.3	2.3	100	80	50	50	50	50	50	50	50
SS-190	11+08	1.0-1.1	A-6	33	15	23	33.5	64.3	2.3	100	80	50	50	50	50	50	50	50
SS-191	11+08	1.0-1.1	A-6	33	15	23	33.5	64.3	2.3	100	80	50	50	50	50	50	50	50
SS-192	11+08	1.0-1.1	A-6	33	15	23	33.5	64.3	2.3	100	80	50	50	50	50	50	50	50
SS-193	11+08	1.0-1.1	A-6	33	15	23	33.5	64.3	2.3	100	80	50	50	50	50	50	50	50
SS-194	11+08	1.0-1.1	A-6	33	15	23	33.5	64.3	2.3	100	80	50	50	50	50	50	50	50
SS-195	11+08	1.0-1.1	A-6	33	15	23	33.5	64.3	2.3	100	80	50	50	50	50	50	50	50
SS-196	11+08	1.0-1.1	A-6	33	15	23	33.5	64.3	2.3	100	80	50	50	50	50	50	50	50
SS-197	11+08	1.0-1.1	A-6	33	15	23	33.5	64.3	2.3	100	80	50	50	50	50	50	50	50
SS-198	11+08	1.0-1.1	A-6	33	15	23	33.5	64.3	2.3	100	80	50	50	50	50	50	50	50
SS-199	11+08	1.0-1.1	A-6	33	15	23	33.5	64.3	2.3	100	80	50	50	50	50	50	50	50
SS-200	11+08	1.0-1.1	A-6	33	15	23	33.5	64.3	2.3	100	80	50	50	50	50	50	50	50
SS-410	14+66	1.0-1.1	A-6	33	15	23	33.5	64.3	2.3	100	80	50	50	50	50	50	50	50
SS-411	14+66	1.0-1.1	A-6	33	15	23	33.5	64.3	2.3	100	80	50	50	50	50	50	50	50
SS-412	14+66	1.0-1.1	A-6	33	15	23	33.5	64.3	2.3	100	80	50	50	50	50	50	50	50

End Grade
-Y2LPD- Sta. 14+83.699
-Y2- Sta. 19+14.860
Elev. 39.064





INCOMPLETE PLANS
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CONST. REV.
R/W REV.

Soil Sample Test Results												
SAMPLE NO.	STATION	DEPTH (M)	CLASS.	LIQ. LIM.	PL. LIM.	% BY WEIGHT			% PASSING (SIEVES)			
S-97	12+60	1.0	CL	25	17	W	LL	PL	CLAY	NO. 10	NO. 40	NO. 60
S-100	12+60	1.0	CL	25	17	15.3	23.5	24	18.2	100	95	60
						93	22.5	23	14.4	100	95	70

-Y5-

Begin Grade
-Y5- Sta. 11+08.000
Elev. 39.116

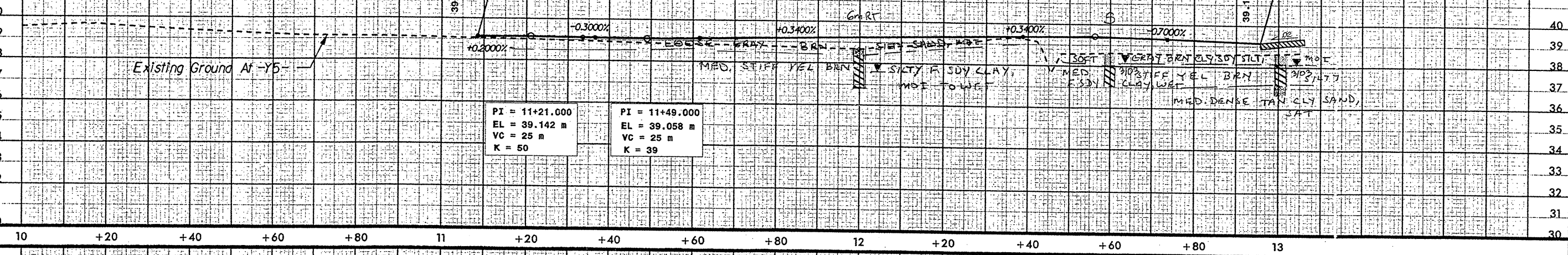
PI = 12+56.230
EL = 39.423 m
VC = 35 m
K = 34

End Grade
-Y5- Sta. 12+95.740
-Y2- Sta. 22+74.467
Elev. 39.146

Existing Ground At -Y5-

PI = 11+21.000
EL = 39.142 m
VC = 25 m
K = 50

PI = 11+49.000
EL = 39.058 m
VC = 25 m
K = 39



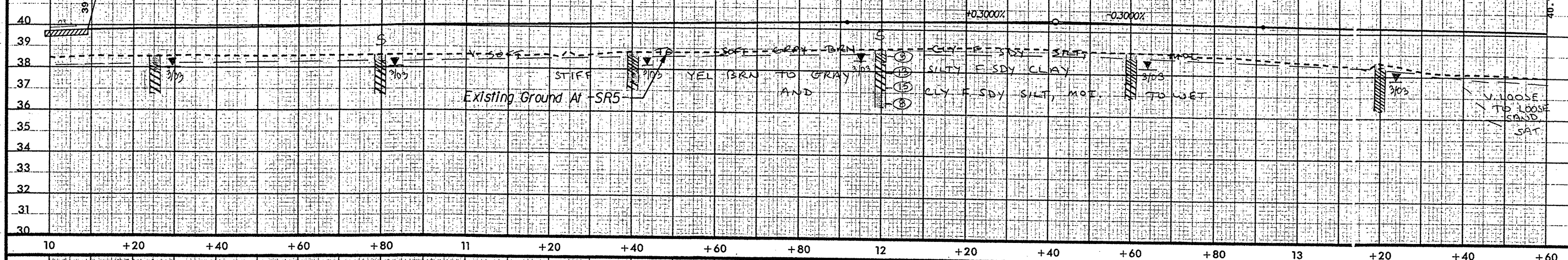


CONST. REV.
 R/W REV.

-SR5-

PI = 12+41.632
 EL = 40.496 m
 VC = 100 m
 K = 167

Begin Grade
 -SR5- Sta. 10+09.064 =
 -Y2- Sta. 17+57.703
 Elev. 39.798

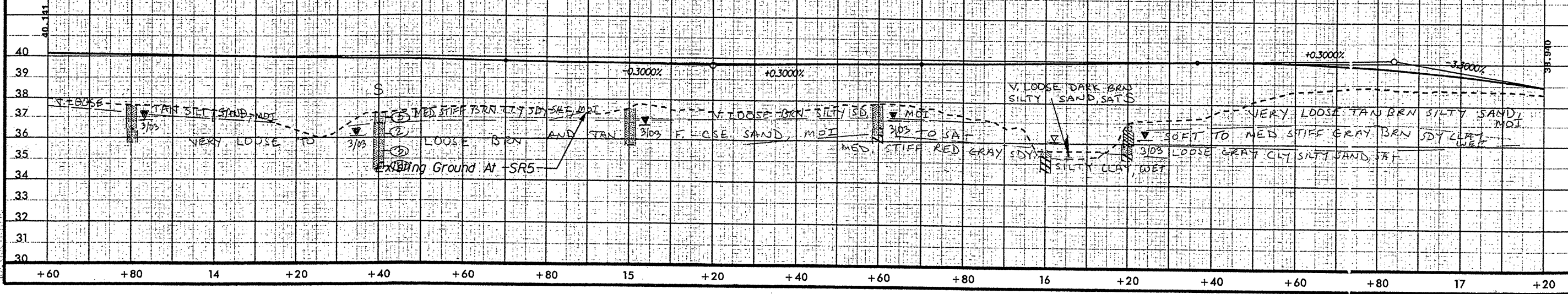


Soil Sample Test Results

SAMPLE NO.	STATION	DEPTH (M)	CLASS	W (%)	L (%)	% BY WEIGHT				PASSING SIEVES		PI	LI
						CLAY	SILT	FINE SAND	COARSE SAND	NO. 20	NO. 40		
S-36	10+90	0.30-0.61	A-6 (1)	134	19	48	132.7	30.2	10.3	100	98	69	31.5
S-37	10+90	0.61-1.23	A-7-6 (3)	161	42	28	124.6	24.2	10.4	100	99	77	31.5
S-401	12+00	0.15-0.30	A-6 (1)	128	14	40	130.1	43.6	10.2	100	92	48	31.5
S-405	12+00	0.30-0.61	A-7-6 (3)	149	31	40	130.1	43.6	10.2	100	92	48	31.5
S-406	12+00	0.61-1.23	A-7-6 (3)	149	31	40	130.1	43.6	10.2	100	92	48	31.5
S-407	12+00	1.23-1.85	A-7-6 (3)	149	31	40	130.1	43.6	10.2	100	92	48	31.5
S-408	12+00	1.85-2.47	A-7-6 (3)	149	31	40	130.1	43.6	10.2	100	92	48	31.5
S-409	12+00	2.47-3.09	A-7-6 (3)	149	31	40	130.1	43.6	10.2	100	92	48	31.5
S-410	12+00	3.09-3.71	A-7-6 (3)	149	31	40	130.1	43.6	10.2	100	92	48	31.5
S-411	12+00	3.71-4.33	A-7-6 (3)	149	31	40	130.1	43.6	10.2	100	92	48	31.5
S-412	12+00	4.33-4.95	A-7-6 (3)	149	31	40	130.1	43.6	10.2	100	92	48	31.5
S-413	12+00	4.95-5.57	A-7-6 (3)	149	31	40	130.1	43.6	10.2	100	92	48	31.5
S-414	12+00	5.57-6.19	A-7-6 (3)	149	31	40	130.1	43.6	10.2	100	92	48	31.5
S-415	12+00	6.19-6.81	A-7-6 (3)	149	31	40	130.1	43.6	10.2	100	92	48	31.5
S-416	12+00	6.81-7.43	A-7-6 (3)	149	31	40	130.1	43.6	10.2	100	92	48	31.5
S-417	12+00	7.43-8.05	A-7-6 (3)	149	31	40	130.1	43.6	10.2	100	92	48	31.5
S-418	12+00	8.05-8.67	A-7-6 (3)	149	31	40	130.1	43.6	10.2	100	92	48	31.5
S-419	12+00	8.67-9.29	A-7-6 (3)	149	31	40	130.1	43.6	10.2	100	92	48	31.5
S-420	12+00	9.29-9.91	A-7-6 (3)	149	31	40	130.1	43.6	10.2	100	92	48	31.5
S-421	12+00	9.91-10.53	A-7-6 (3)	149	31	40	130.1	43.6	10.2	100	92	48	31.5
S-422	12+00	10.53-11.15	A-7-6 (3)	149	31	40	130.1	43.6	10.2	100	92	48	31.5
S-423	12+00	11.15-11.77	A-7-6 (3)	149	31	40	130.1	43.6	10.2	100	92	48	31.5
S-424	12+00	11.77-12.39	A-7-6 (3)	149	31	40	130.1	43.6	10.2	100	92	48	31.5
S-425	12+00	12.39-13.01	A-7-6 (3)	149	31	40	130.1	43.6	10.2	100	92	48	31.5

PI = 15+20.000
 EL = 39.661 m
 VC = 100 m
 K = 167

PI = 16+84.000
 E. = 40.153 m
 V. = 95 m
 K = 26

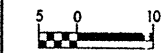


11-04-2002
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PROJECT REFERENCE NO. SHEET NO.

R-2554B/1 62
ROADWAY DESIGN ENGINEER HYDRAULICS ENGINEER



INCOMPLETE PLANS
DO NOT USE FOR ACQUISITION
PRELIMINARY PLANS
DO NOT USE FOR CONSTRUCTION

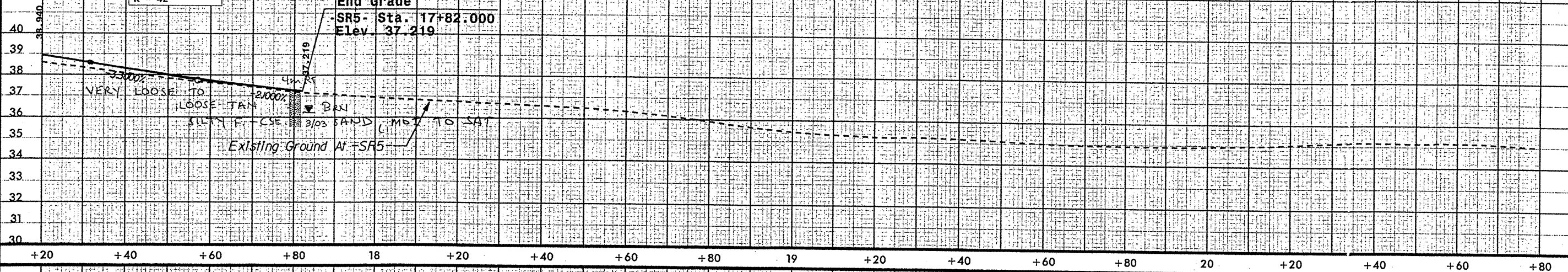
CONST. REV.

R/W REV.

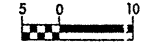
-SR5-

PI = 17+57.000
EL = 37.744 m
VC = 50 m
K = 42

End Grade
-SR5- Sta. 17+82.000
Elev. 37.219



11/17/00
P:\projects\2000\2554B\2554B.dwg
2554B.dwg



-SR6-

Begin Grade
 -SR6- Sta. 10+09.329
 Elev. 39.124

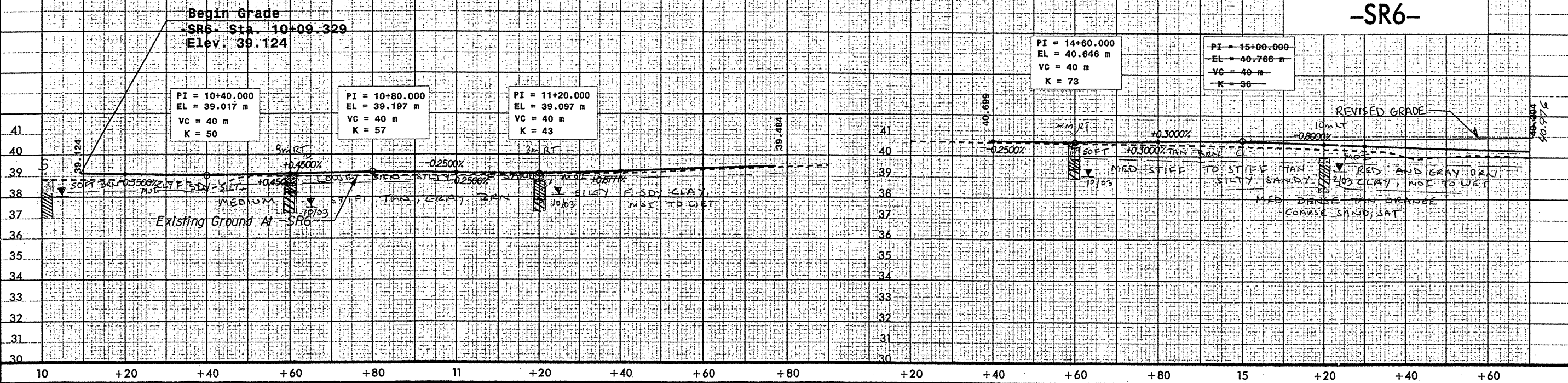
PI = 10+40.000
 EL = 39.017 m
 VC = 40 m
 K = 50

PI = 10+80.000
 EL = 39.197 m
 VC = 40 m
 K = 57

PI = 11+20.000
 EL = 39.097 m
 VC = 40 m
 K = 43

PI = 14+60.000
 EL = 40.646 m
 VC = 40 m
 K = 73

PI = 15+00.000
 EL = 40.766 m
 VC = 40 m
 K = 96

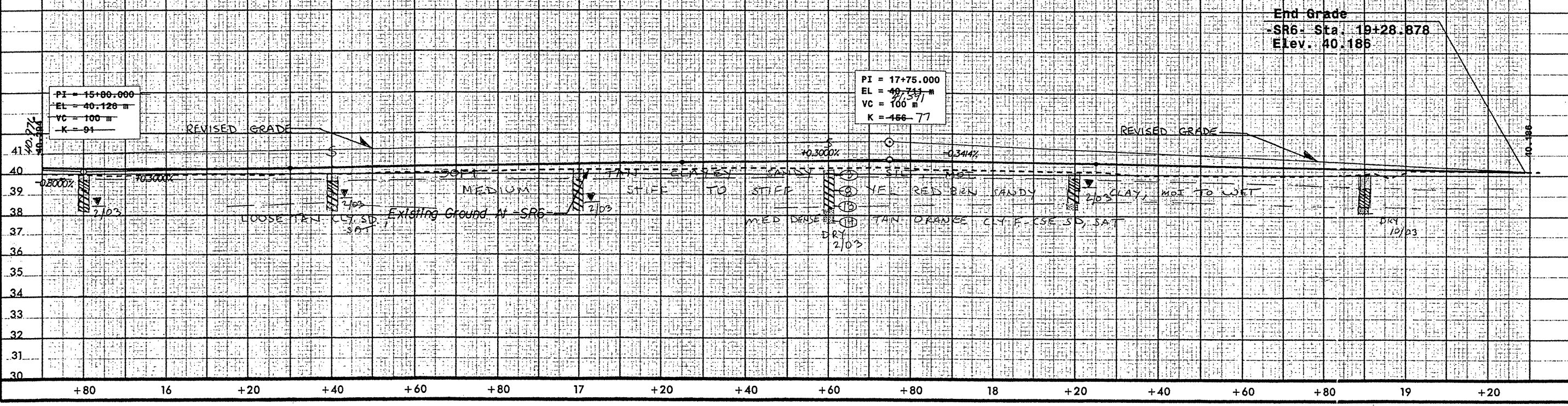


Soil Sample Test Results												
SAMPLE NO.	DEPTH	TEST	RESULT	UNIT	TEST	RESULT	UNIT	TEST	RESULT	UNIT	TEST	RESULT
S-79	10+00	CL	0.380-0.61	A-1(0)	19	14	2.4	130	42.1	16.1	100	95
S-79	10+00	CL	0.61-1.83	A-1(0)	31	116	1.8	130	33.3	32.3	100	92
S-69	10+10	CL	0.380-0.46	A-1(0)	15	NP	15.7	55.3	116.7	12.2	100	31
S-70	10+10	CL	0.460-0.76	A-1(0)	29	116	13.1	114.7	38.8	8.9	100	82
S-71	10+10	CL	0.761-1.32	A-1(20)	27	137	9.2	130	5.4	49.0	100	76
S-72	10+10	CL	1.321-1.83	A-1(20)	43	22	48.0	290	0.6	22.4	100	61
SS-173	17+60	CL	0.380-0.45	A-1(0)	16	NP	11.3	55.4	21.1	11.1	100	155
SS-174	17+60	CL	0.451-0.71	A-1(0)	55	29	6.1	70	11.6	6.2	100	78
SS-175	17+60	CL	0.711-0.97	A-1(0)	42	20	25.5	134	13.8	34.4	100	84
SS-176	17+60	CL	0.971-1.73	A-1(0)	25	14	41.9	245	0.6	18.1	100	46

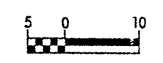
End Grade
 -SR6- Sta. 19+28.878
 Elev. 40.186

PI = 15+00.000
 EL = 40.126 m
 VC = 100 m
 K = 94

PI = 17+75.000
 EL = 40.733 m
 VC = 100 m
 K = 466-77



11/04/2002
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INCOMPLETE PLANS
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PRELIMINARY PLANS
 DO NOT USE FOR CONSTRUCTION

CONST. REV.
 R / W REV.

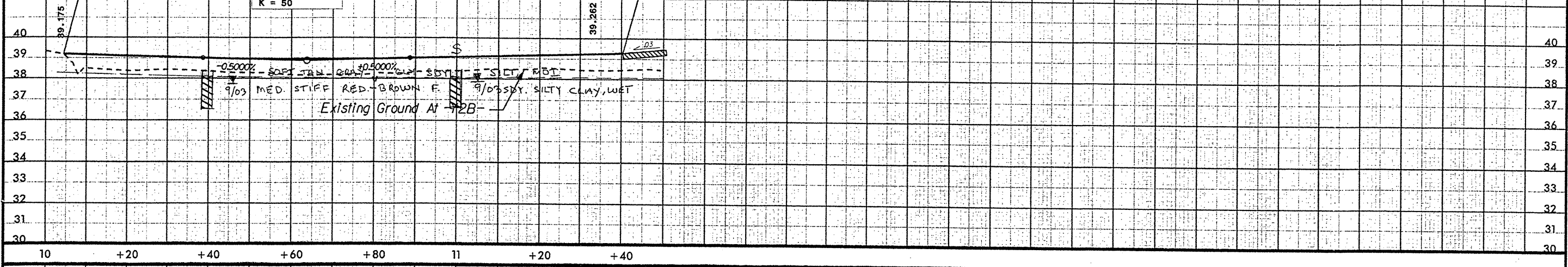
Soil Sample Test Results													
SAMPLE NO.	STATION	DEPTH	CLASS	LIQUID LIMIT (%)	PLASTICITY INDEX (%)	% BY WEIGHT				% PASSING (NO. 20)		MOISTURE (%)	ORGANIC (%)
						GRAVEL	SAND	SILT	CLAY	75 μm	425 μm		
S-258	11+00	CL	A-7-6(H)	55	8	6.8	26.8	36.3	44.3	100	97	80	
S-259	11+00	CL	A-7-6(C)	63	30	4.0	21.3	55.3	50.4	100	95	81	

Begin Grade
 -Y2B- Sta. 10+04.340 =
 -Y2- Sta. 17+57.703
 Elev. 39.175

PI = 10+63.666
 EL = 38.879 m
 VC = 50 m
 K = 50

End Grade
 -Y2B- Sta. 11+40.310 =
 -EY2- Sta. 17+56.938
 Elev. 39.262

-Y2B-

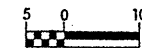


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PROJECT REFERENCE NO. SHEET NO.

R-2554B A 70
ROADWAY DESIGN ENGINEER HYDRAULICS ENGINEER



INCOMPLETE PLANS
DO NOT USE FOR R/W ACQUISITION
PRELIMINARY PLANS
DO NOT USE FOR CONSTRUCTION

CONST. REV.

R/W REV.

-Y7-

Existing Ground At -Y7-
(PATETOWN RD)

(MINIMUM RESURFACING)

-Y7- Sta. 12+17.434 =
-L- Sta. 118+92.556

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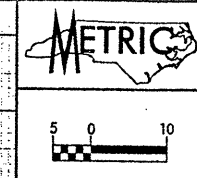
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+80 11 +20 +40 +60 +80 12 +20 +40 +60 +80 13 +20 +40 +60

Vertical curve data table

Soil Sample Test Results

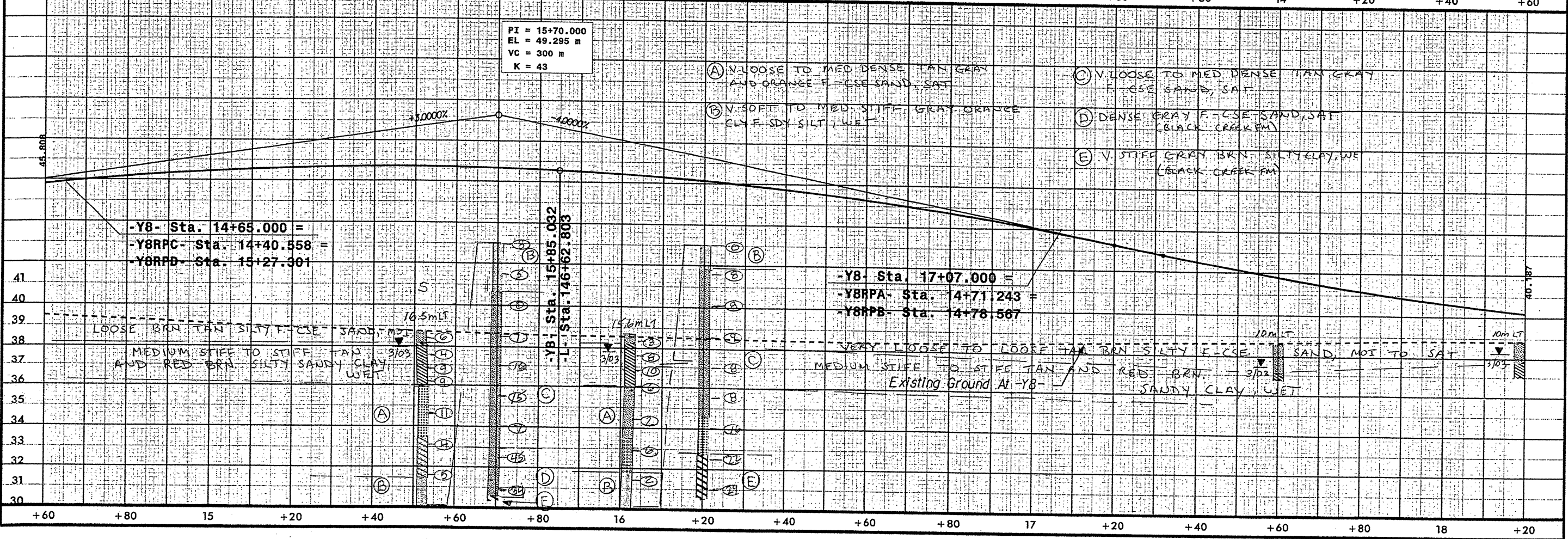
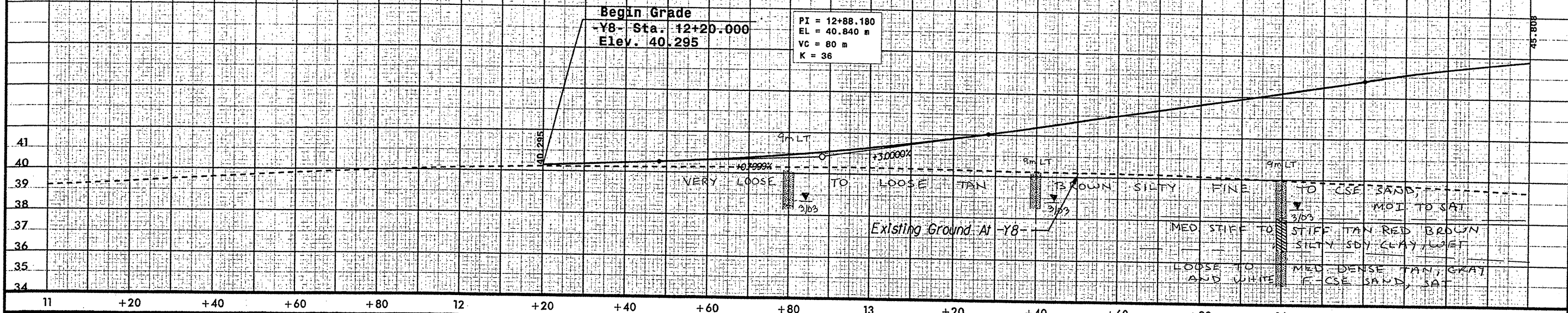
SAMPLE NO.	SYMBOL	DEPTH	APPROX. INTERVAL	APPROX. CL.	F.T.	% D			% PASSING (SIEVES)			MOISTURE	PLASTICITY	
						LIQ.	PL.	SH.	NO. 20	NO. 40	NO. 60			
SS-327	15-82	16.5M LT	0.15-0.45	A-4(0)	16	NP	22.2	24.9	14.8	8.0	100	93	28	
SS-328	15-82	16.5M LT	0.76-1.21	A-6(5)	31	17	21.4	34.7	17.8	26.1	100	90	49	210
SS-329	15-82	16.5M LT	2.32-2.77	A-6(3)	33	19	24.0	39.7	16.3	26.1	100	90	39	
SS-330	15-82	16.5M LT	3.84-4.29	A-6(0)	39	NP	20.3	21.5	12	7.9	97	84	9	
SS-331	15-82	16.5M LT	4.85-5.32	A-6(0)	36	20	25.1	33	15	10.1	98	82	12	
SS-332	15-82	16.5M LT	6.42-6.87	A-6(0)	36	NP	27.3	27.8	12.8	19.1	100	100	42	
SS-333	15-82	16.5M LT	7.47-7.92	A-6(0)	23	NP	27.5	26.5	16.6	11.6	98	87	18	
SS-334	15-82	16.5M LT	8.98-9.43	A-6(0)	22	NP	24.2	24.1	14.1	15.1	93	82	30	
SS-335	15-82	16.5M LT	10.51-11.0	A-6(0)	27	NP	22.2	23.2	15.5	13.1	100	98	21	
SS-336	15-82	16.5M LT	12.08-12.53	A-6(0)	25	NP	25.3	24.5	21.2	8.0	100	93	13	
SS-337	15-82	16.5M LT	13.65-14.1	A-6(0)	24	NP	25.9	22.2	20	8.0	100	91	16	
SS-338	16-82	15.6M LT	0.76-1.21	A-6(0)	32	16	19.4	41.1	9.6	30.1	100	85	43	
SS-339	16-82	15.6M LT	2.32-2.77	A-6(2)	32	16	25.1	38.7	8.2	28.1	100	89	40	
SS-340	16-82	15.6M LT	3.84-4.29	A-6(0)	20	NP	23.0	25.1	2.9	9.0	100	96	13	
SS-341	16-82	15.6M LT	5.36-5.81	A-6(0)	24	31	24.3	15.0	4.8	6.0	99	43	10	
SS-342	16-82	15.6M LT	6.88-7.33	A-6(0)	20	25	23.6	24.1	11.9	22.0	100	99	45	245
SS-343	16-82	15.6M LT	8.40-8.85	A-6(0)	23	NP	22.2	21.6	6.3	8.0	100	99	17	
SS-344	16-82	15.6M LT	9.92-10.37	A-6(0)	23	NP	22.2	21.6	6.3	8.0	100	99	17	
SS-345	16-82	15.6M LT	11.44-11.89	A-6(0)	23	NP	22.2	21.6	6.3	8.0	100	99	17	
SS-346	16-82	15.6M LT	12.96-13.41	A-6(0)	23	NP	22.2	21.6	6.3	8.0	100	99	17	
SS-347	16-82	15.6M LT	14.48-14.93	A-6(0)	19	NP	22.2	21.6	6.3	8.0	100	99	17	
SS-348	16-82	15.6M LT	16.00-16.45	A-6(0)	23	NP	22.2	21.6	6.3	8.0	100	99	17	
SS-349	16-82	15.6M LT	17.52-17.97	A-6(0)	23	NP	22.2	21.6	6.3	8.0	100	99	17	
SS-350	16-82	15.6M LT	19.04-19.49	A-6(0)	23	NP	22.2	21.6	6.3	8.0	100	99	17	
SS-351	16-82	15.6M LT	20.56-21.01	A-6(0)	23	NP	22.2	21.6	6.3	8.0	100	99	17	
SS-352	16-82	15.6M LT	22.08-22.53	A-6(0)	23	NP	22.2	21.6	6.3	8.0	100	99	17	
SS-353	16-82	15.6M LT	23.60-24.05	A-6(0)	23	NP	22.2	21.6	6.3	8.0	100	99	17	
SS-354	16-82	15.6M LT	25.12-25.57	A-6(0)	23	NP	22.2	21.6	6.3	8.0	100	99	17	
SS-355	16-82	15.6M LT	26.64-27.09	A-6(0)	23	NP	22.2	21.6	6.3	8.0	100	99	17	
SS-356	16-82	15.6M LT	28.16-28.61	A-6(0)	23	NP	22.2	21.6	6.3	8.0	100	99	17	



PROJECT REFERENCE NO. **R-2554B A** SHEET NO. **71**
 ROADWAY DESIGN ENGINEER HYDRAULICS ENGINEER
INCOMPLETE PLANS
 DO NOT USE FOR ACQUISITION
PRELIMINARY PLANS
 DO NOT USE FOR CONSTRUCTION

CONST.REV.
R/W REV.

-Y8-



10/24/2008
 03:00pm
 2008/10/24 10:24 AM
 10/24/2008

METRIC

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CONST.REV.
R/W REV.

PROJECT REFERENCE NO. **R-2554BA** SHEET NO. **72**

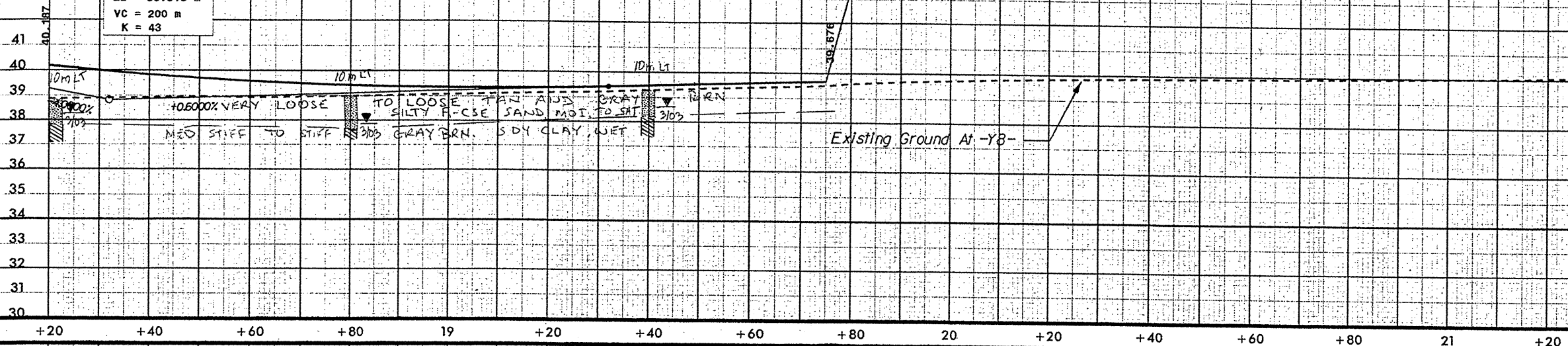
ROADWAY DESIGN ENGINEER HYDRAULICS ENGINEER

INCOMPLETE PLANS
DO NOT USE FOR R/W ACQUISITION
PRELIMINARY PLANS
DO NOT USE FOR CONSTRUCTION

-Y8-

PI = 18+31.927
EL = 38.818 m
VC = 200 m
K = 43

End Grade
-Y8- Sta. 19+75.00
Elev. 39.676



1/15/02
 J. J. JENSEN
 J. J. JENSEN
 J. J. JENSEN



ROADWAY DESIGN ENGINEER HYDRAULICS ENGINEER

INCOMPLETE PLANS
DO NOT USE FOR ACQUISITION
PRELIMINARY PLANS
DO NOT USE FOR CONSTRUCTION

CONST. REV.
R/W REV.

-Y8- Ramp A

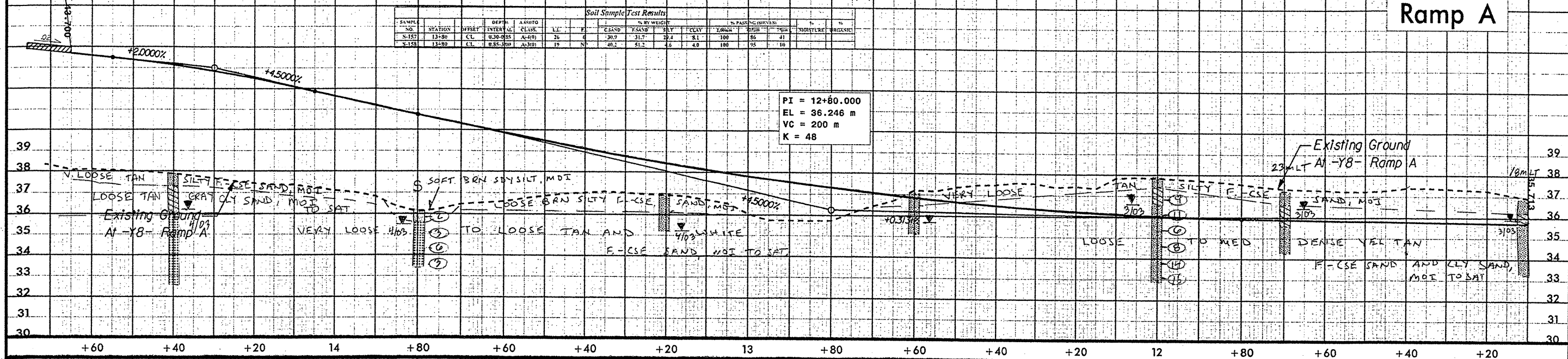
END
Begin Grade
-Y8RPA- Sta. 14+65.210 =
-Y8- Sta. 17+06.325
Elev. 43.700

PI = 14+30.000
EL = 42.996 m
VC = 50 m
K = 20

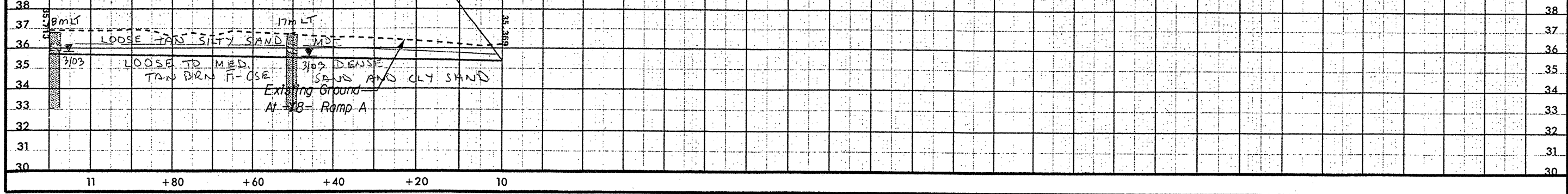
Soil Sample Test Results

SAMPLE NO.	STATION	OFFSET	DEPTH	ALSHO	FLDS	LL	P	ESAND	FSAND	SLY	PCLY	Y ₂₀₀	U ₂₀₀	U ₄₀	U ₁₀	U ₂	U ₁	U _{0.75}
S-157	13+80	CL	0.30-0.85	A-310	25	6	30.0	31.7	22.4	81	100	86	41					
S-158	13+90	CL	0.85-1.00	A-310	19	NP	46.2	51.2	4.6	6.0	100	95	18					

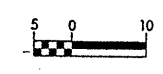
PI = 12+80.000
EL = 36.246 m
VC = 200 m
K = 48



BEGIN
End Grade
-Y8RPA- Sta. 10+00.000 =
-L- Sta. 151+70.000
Elev. 35.369



1:24.2002
C:\Users\1001\My Documents\Bachar



INCOMPLETE PLANS
 DO NOT USE FOR R/W ACQUISITION
 PRELIMINARY PLANS
 DO NOT USE FOR CONSTRUCTION

CONST. REV.
 R/W REV.

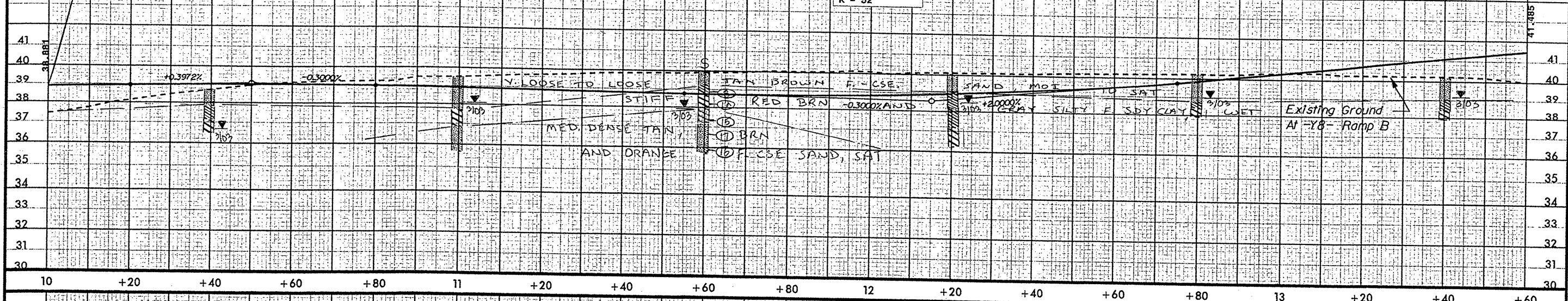
Soil Sample Test Results											
SAMPL NO.	STATION	DEPTH	CLASS	LL	PL	CSNO	FSNO	CLAY	ALP	SHRINKAGE (MM)	%
SS-42	11+60	CL	0.76-0.77 A-6(7)	38	22	18.2	37.2	26.1	100	52	50
SS-43	11+60	CL	1.52-0.97 A-7-6(10)	43	28	11.5	46.0	30.2	100	57	55
SS-44	11+60	CL	2.28-1.73 A-2-4(1)	24	18	19.7	54.3	18.1	100	37	35
SS-45	11+60	CL	3.05-2.50 A-2-4(1)	24	18	19.7	54.3	18.1	100	37	35
SS-46	11+60	CL	0.30-0.45 A-2-4(1)	14	10	11.1	31.6	10.1	100	76	15
SS-47	11+60	CL	0.76-1.21 A-6(7)	39	25	17.1	38.0	16.5	100	71	39
SS-48	11+60	CL	1.52-0.97 A-7-6(10)	41	28	17.9	38.4	16.5	100	95	48
SS-49	11+60	CL	2.28-1.73 A-2-4(1)	35	23	11.1	46.0	14.7	100	71	39
SS-50	11+60	CL	3.05-2.50 A-2-4(1)	21	16	11.1	46.0	10.3	100	99	20

Begin Grade
 -Y8RPB- Sta. 10+00.000
 -L- Sta. 142+40.000
 Elev. 38.881

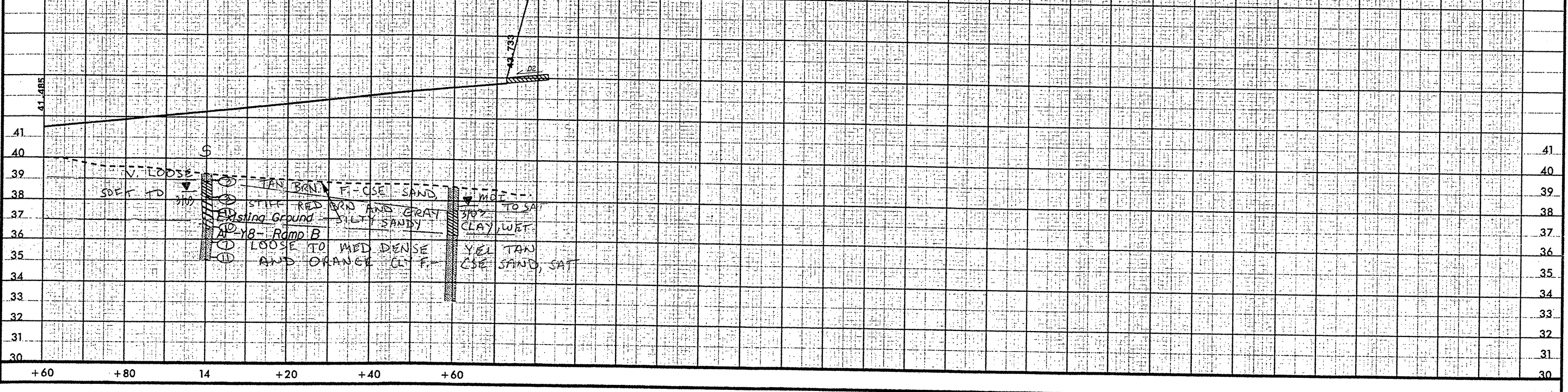
PI = 10+50.000
 EL = 39.080 m
 VC = 60 m
 K = 86

PI = 12+15.000
 EL = 38.585 m
 VC = 120 m
 K = 52

-Y8-
 Ramp B



End Grade
 -Y8RPB- Sta. 14+72.417
 -Y8- Sta. 17+05.420
 Elev. 43.733



11/24/2000
 R. Gomez/30413/10/10/2554BA/14
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INCOMPLETE PLANS
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 PRELIMINARY PLANS
 DO NOT USE FOR CONSTRUCTION

CONST. REV.
 R/W REV.

**-Y8-
Ramp C**

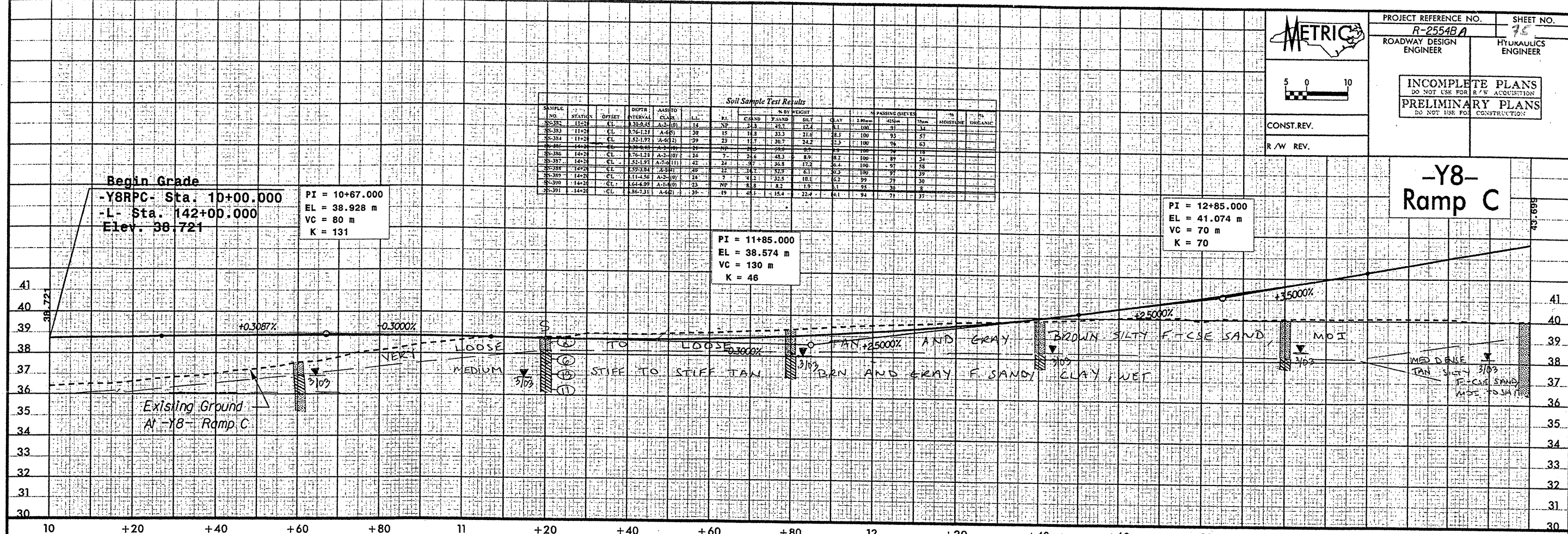
Soil Sample Test Results

SAMPLE NO.	STATION	DEPTH	INTERVAL	AASHTO CLASS	LL	PL	C SAND	F SAND	SILT	CLAY	100µm	425µm	75µm	% MOISTURE	% ORGANIC
SS-382	11+20	CL	30-45	A-2(0)	14	11	21.3	42.1	12.4	8.1	100	93	57		
SS-383	11+25	CL	176-127	A-4(5)	38	15	16.3	33.3	21.6	8.1	100	93	57		
SS-384	11+30	CL	52-177	A-4(2)	29	23	11.7	30.7	24.2	21.3	100	96	63		
SS-385	11+35	CL	39-84	A-4(2)	29	23	11.7	30.7	24.2	21.3	100	96	63		
SS-386	14+21	CL	176-127	A-2(0)	14	7	21.6	42.3	8.9	8.1	100	97	58		
SS-387	14+22	CL	52-177	A-7(0)(1)	42	24	37	36.8	17.2	6.4	100	87	24		
SS-388	14+23	CL	176-127	A-4(2)	29	11	16.7	32.9	6.1	8.1	100	97	58		
SS-389	14+24	CL	11-4-36	A-2(0)	24	7	4.2	32.5	10.1	6.2	99	79	30		
SS-390	14+25	CL	144-97	A-1(0)	12.3	7.0	8.3	8.2	1.9	1.1	94	30	8		
SS-391	14+26	CL	186-731	A-4(2)	30	19	4.1	18.4	22.4	4.1	94	71	37		

Begin Grade
 -Y8RPC- Sta. 10+00.000 PI = 10+67.000
 -L- Sta. 142+00.000 EL = 38.928 m
 Elev. 38.721 VC = 80 m
 K = 131

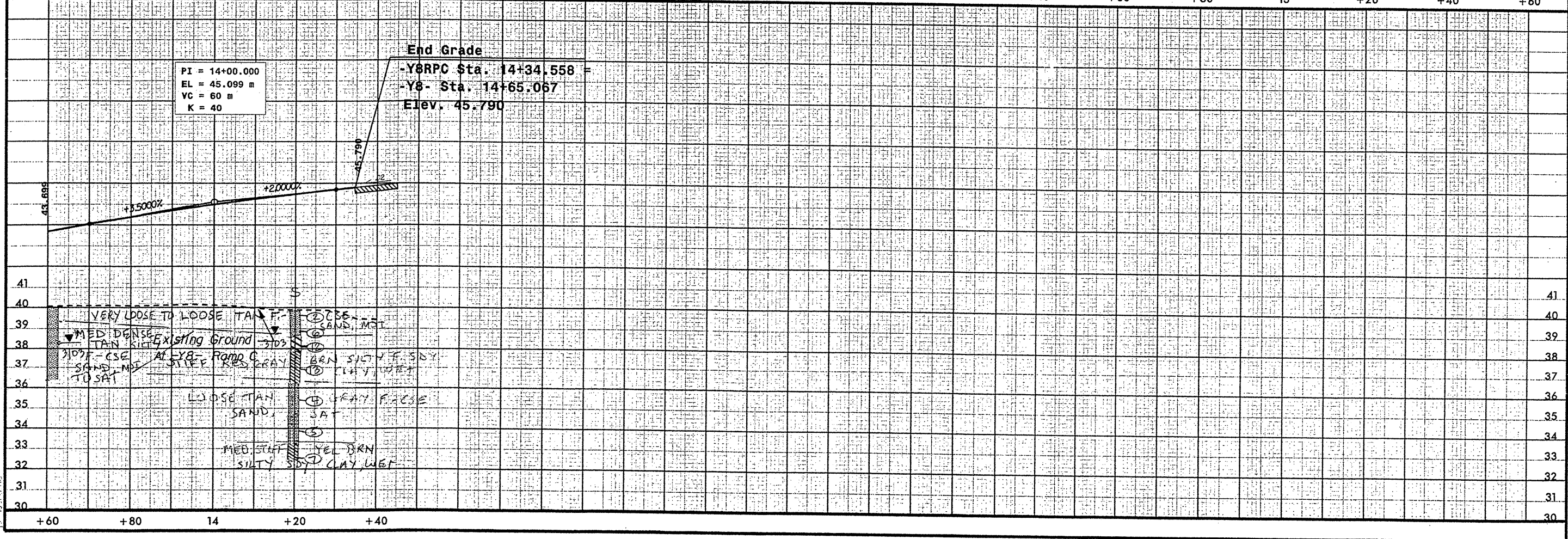
PI = 11+85.000
 EL = 38.574 m
 VC = 130 m
 K = 46

PI = 12+85.000
 EL = 41.074 m
 VC = 70 m
 K = 70

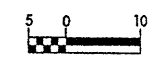


PI = 14+00.000
 EL = 45.099 m
 VC = 60 m
 K = 40

End Grade
 -Y8RPC Sta. 14+34.558 =
 -Y8- Sta. 14+65.067
 Elev. 45.790



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INCOMPLETE PLANS DO NOT USE FOR R/W ACQUISITION PRELIMINARY PLANS DO NOT USE FOR CONSTRUCTION

CONST. REV. R/W REV.

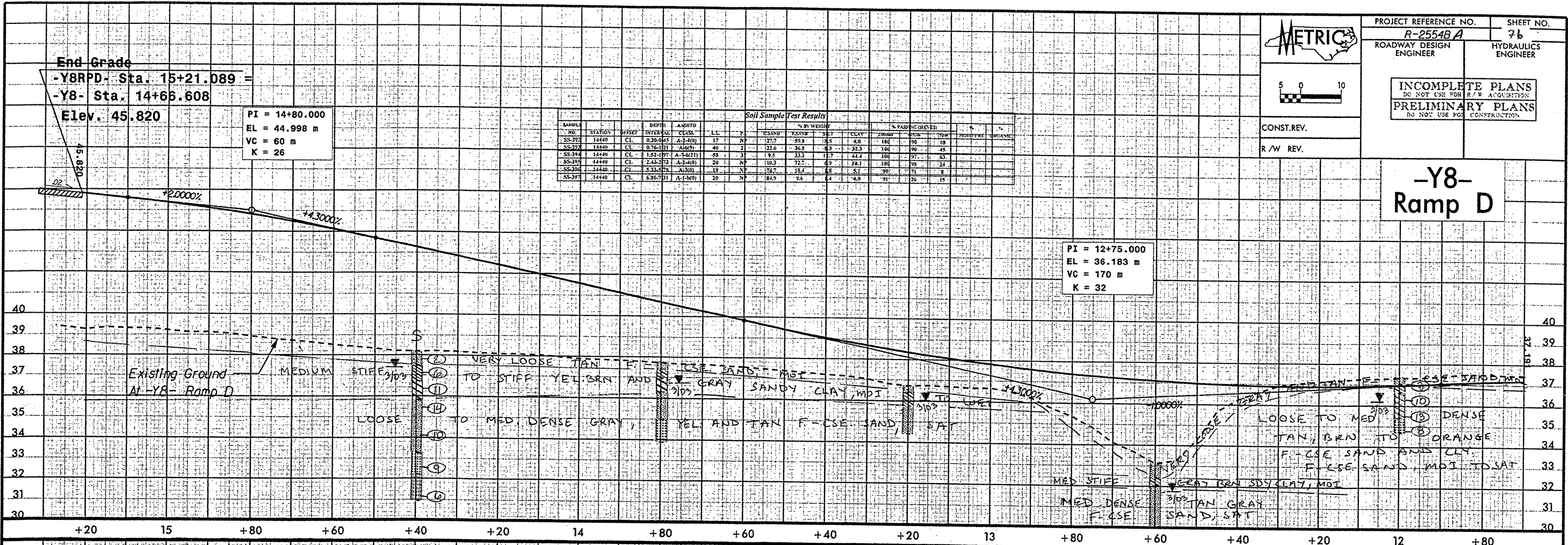
-Y8- Ramp D

End Grade
-Y8RPD- Sta. 15+21.089 =
-Y8- Sta. 14+66.608
Elev. 45.820

PI = 14+80.000
EL = 44.998 m
VC = 60 m
K = 26

Soil Sample Test Results														
SAMPLE NO.	STATION	DEPTH	ANALYST	CLASS.	LL	PL	% BY WEIGHT				% PASSING (SIEVES)			
							CLAY	SLT	SAND	FINE SAND	NO. 20	NO. 40	NO. 60	NO. 100
SS-392	14440	CL	0.30-0.45	A-1-100	17	NP	27.7	89.8	18.5	4.0	100	90	18	
SS-393	14440	CL	0.76-1.21	A-1-100	40	NP	27.6	36.3	4.3	33.3	100	50	45	
SS-394	14440	CL	1.62-1.97	A-1-100	48	NP	19.5	33.3	11.7	24.4	100	97	25	
SS-395	14440	CL	2.43-3.73	A-1-100	20	NP	10.3	72.7	4.9	19.1	100	98	28	
SS-396	14440	CL	5.33-6.78	A-1-100	19	NP	14.7	18.4	4.8	5.1	99	91	8	
SS-397	14440	CL	6.36-7.01	A-1-100	20	NP	18.9	9.6	4.3	4.0	91	26	15	

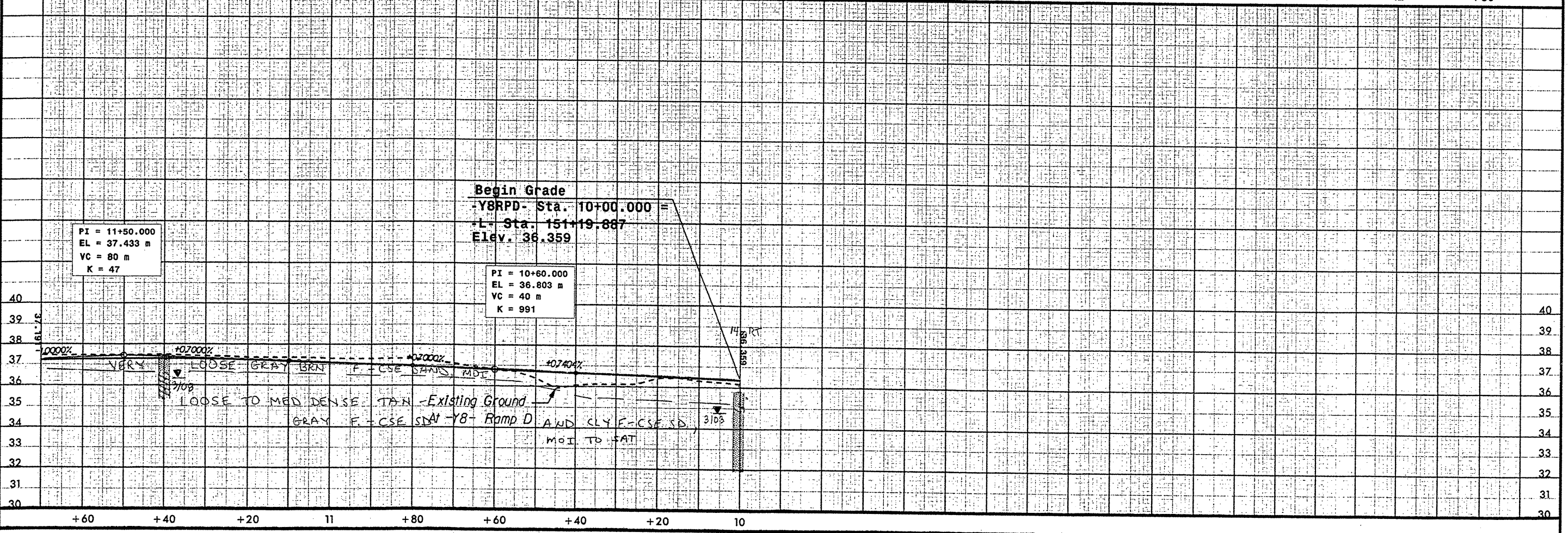
PI = 12+75.000
EL = 36.183 m
VC = 170 m
K = 32



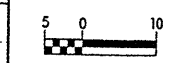
PI = 11+50.000
EL = 37.433 m
VC = 80 m
K = 47

Begin Grade
-Y8RPD- Sta. 10+00.000 =
-L- Sta. 151+19.887
Elev. 36.359

PI = 10+60.000
EL = 36.803 m
VC = 40 m
K = 991



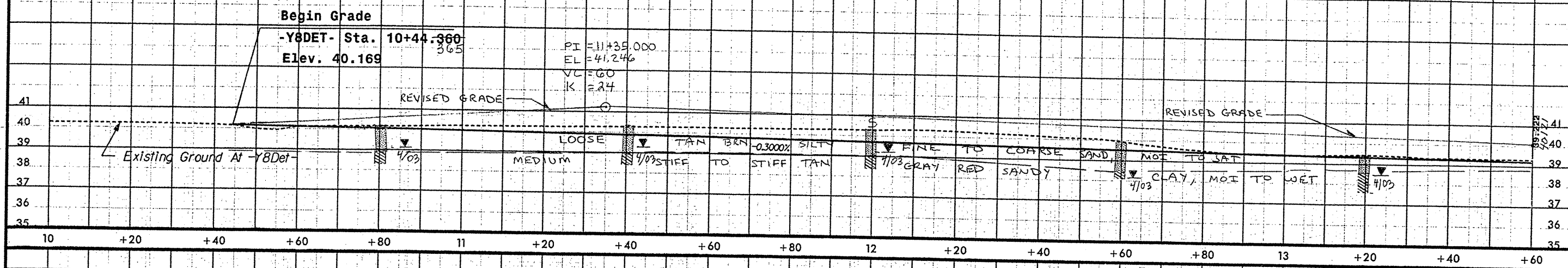
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INCOMPLETE PLANS
DO NOT USE FOR ACQUISITION
PRELIMINARY PLANS
DO NOT USE FOR CONSTRUCTION

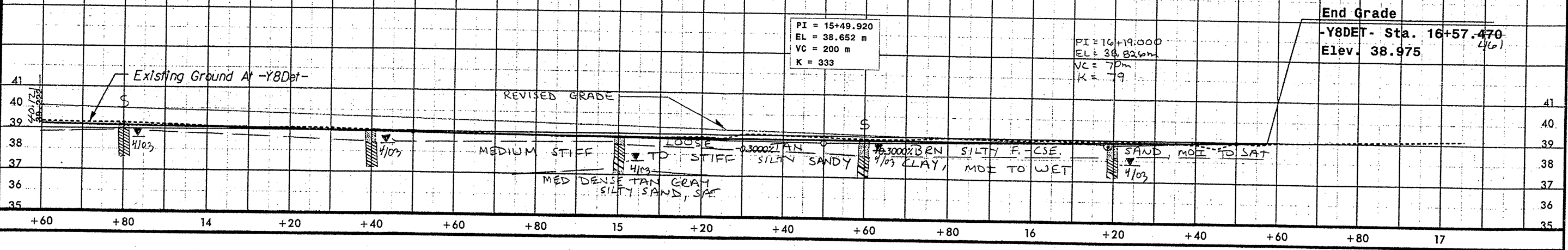
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R/W REV.

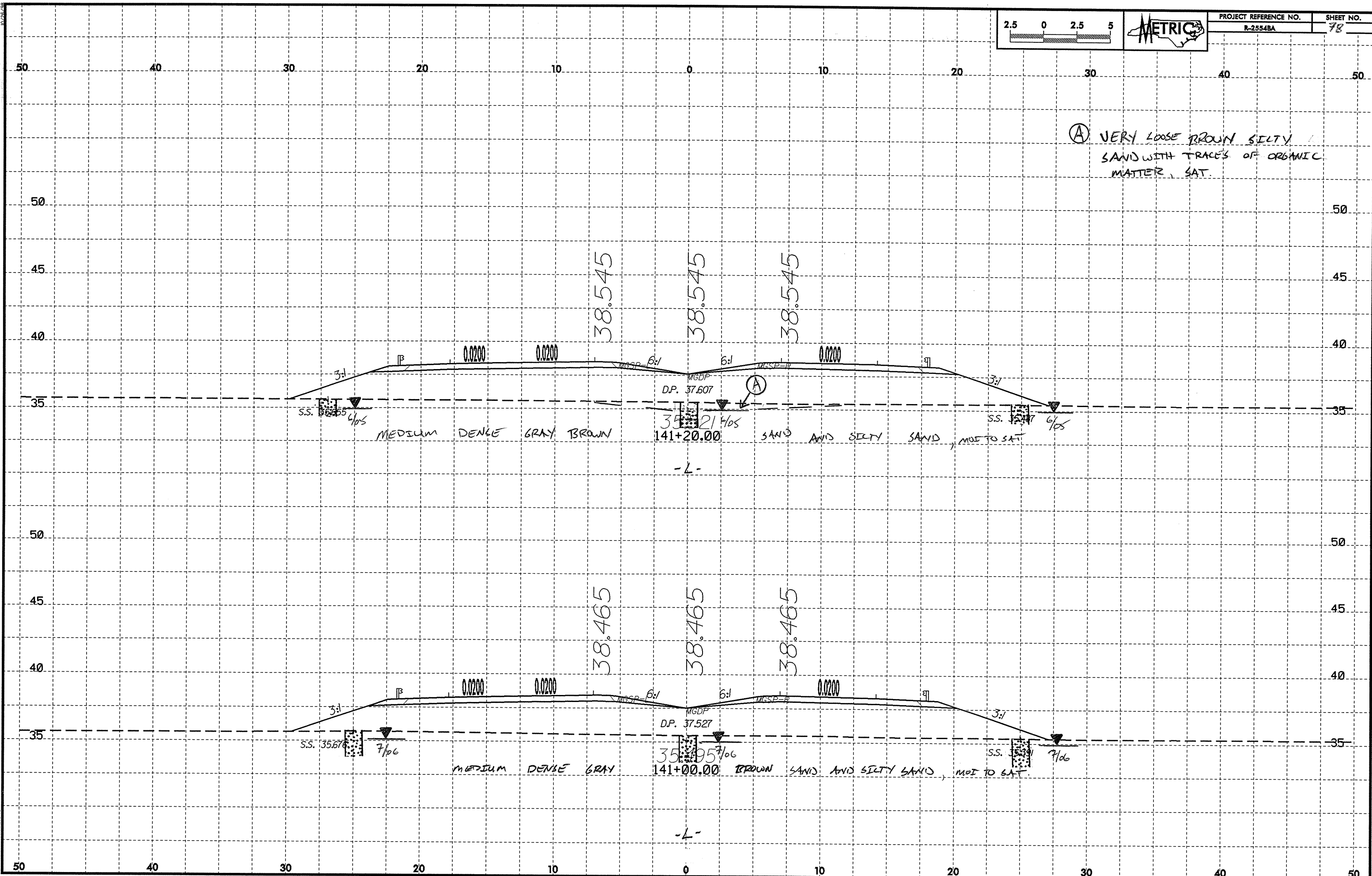
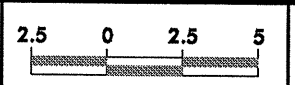
-Y8DET-



Soil Sample Test Results

SAMPLE NO.	STATION	DEPTH INTERVAL	AASHTO CLASS	LL		PI		% BY WEIGHT				% PASS (SIEVES)			MOISTURE	ORGANIC
				U	P	U	P	CSAND	FSAND	SILT	CLAY	200µm	75µm	75µm		
S-154	12+00	CL 0.30-1.00	A-2(40)	15	NP	32.5	53.7	11.8	2.0	100	87	15				
S-154	12+00	CL 1.20-1.80	A-6(5)	25	8	27.4	40.8	10.1	22.2	109	33	36				
S-154	13+80	CL 0.30-1.00	A-6(5)	35	8	21.2	35.3	8.2	30.3	109	92	47				
S-154	13+80	CL 1.10-1.80	A-6(8)	39	20	16.1	36.3	10.3	28.1	109	95	52				
S-154	15+60	CL 0.00-0.30	A-2(40)	14	NP	31.1	61.6	8.4	1.0	100	36	32				
S-154	15+60	CL 0.90-1.40	A-6(0)	32	6	14.1	44.6	11.1	30.2	100	95	43				
S-154	15+60	CL 1.40-1.80	A-2(41)	44	30	11.9	41.6	11.3	24.2	109	96	51				





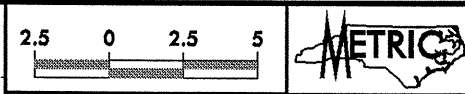
Ⓐ VERY LOOSE BROWN SILTY SAND WITH TRACES OF ORGANIC MATTER, SAT.

s.s. 35265 4/05
 MEDIUM DENSE GRAY BROWN
 35221 4/05
 SAND AND SILTY SAND, MOD TO SAT
 s.s. 35247 6/05

-L-

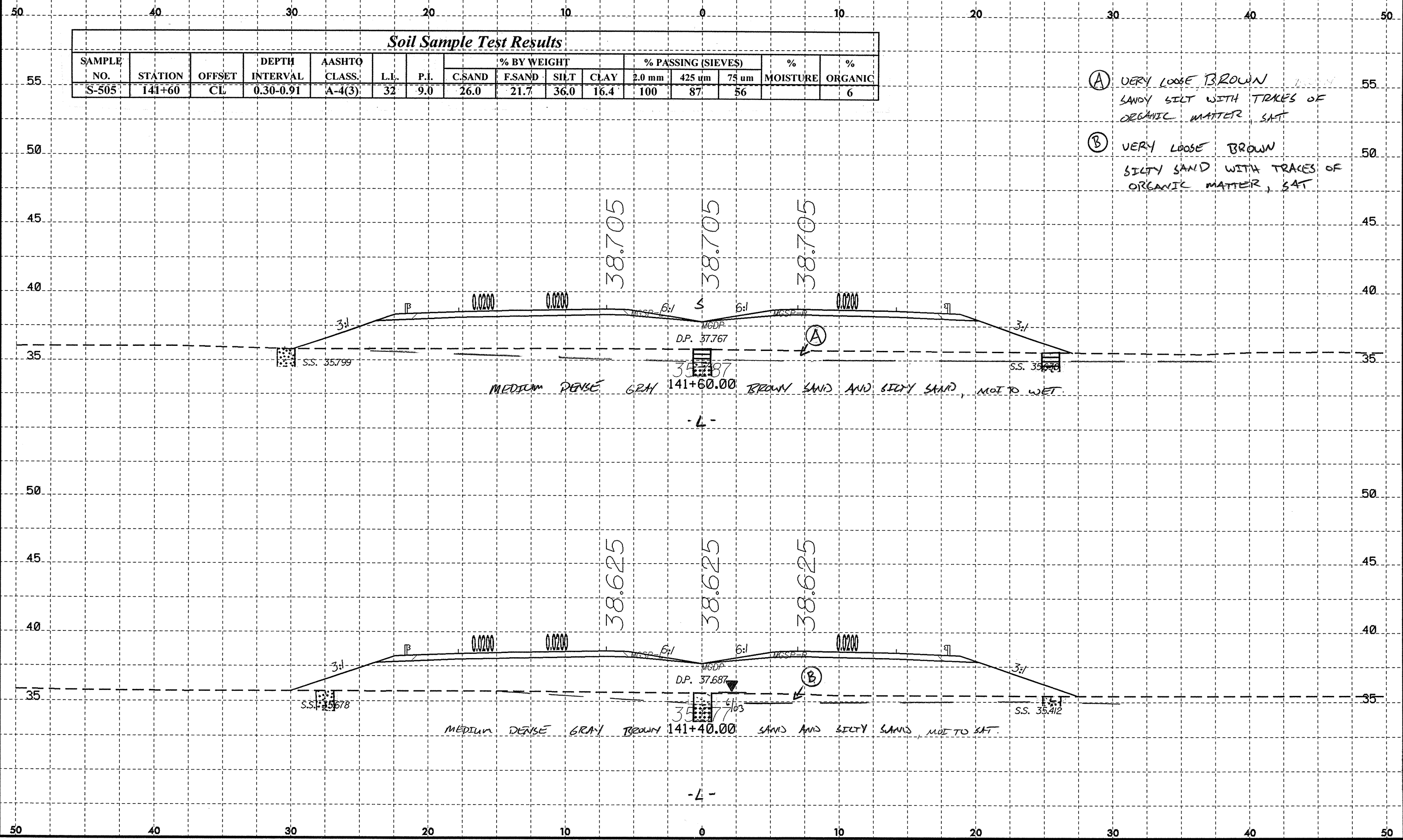
s.s. 35266 7/06
 MEDIUM DENSE GRAY
 35295 7/06
 BROWN SAND AND SILTY SAND, MOD TO SAT
 s.s. 35291 7/06

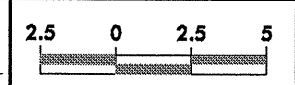
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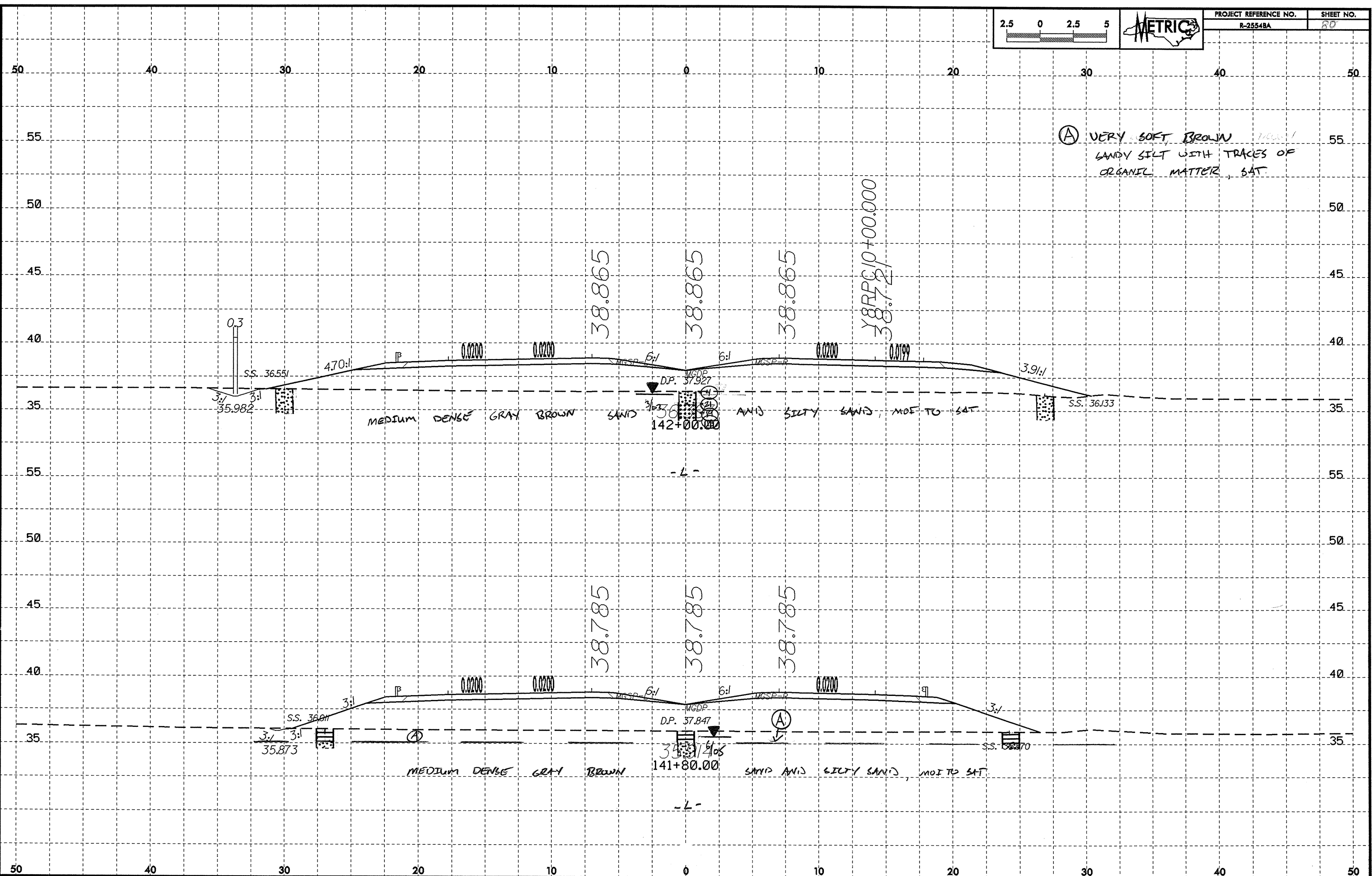
Soil Sample Test Results															
SAMPLE NO.	STATION	OFFSET	DEPTH INTERVAL	AASHTO CLASS.	L.L.	P.I.	% BY WEIGHT				% PASSING (SIEVES)			% MOISTURE	% ORGANIC
							C.SAND	F.SAND	SILT	CLAY	2.0 mm	425 um	75 um		
S-505	141+60	CL	0.30-0.91	A-4(3)	32	9.0	26.0	21.7	36.0	16.4	100	87	56		6

- Ⓐ VERY LOOSE BROWN SANDY SILT WITH TRACES OF ORGANIC MATTER, SAT
- Ⓑ VERY LOOSE BROWN SILTY SAND WITH TRACES OF ORGANIC MATTER, SAT

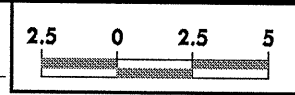




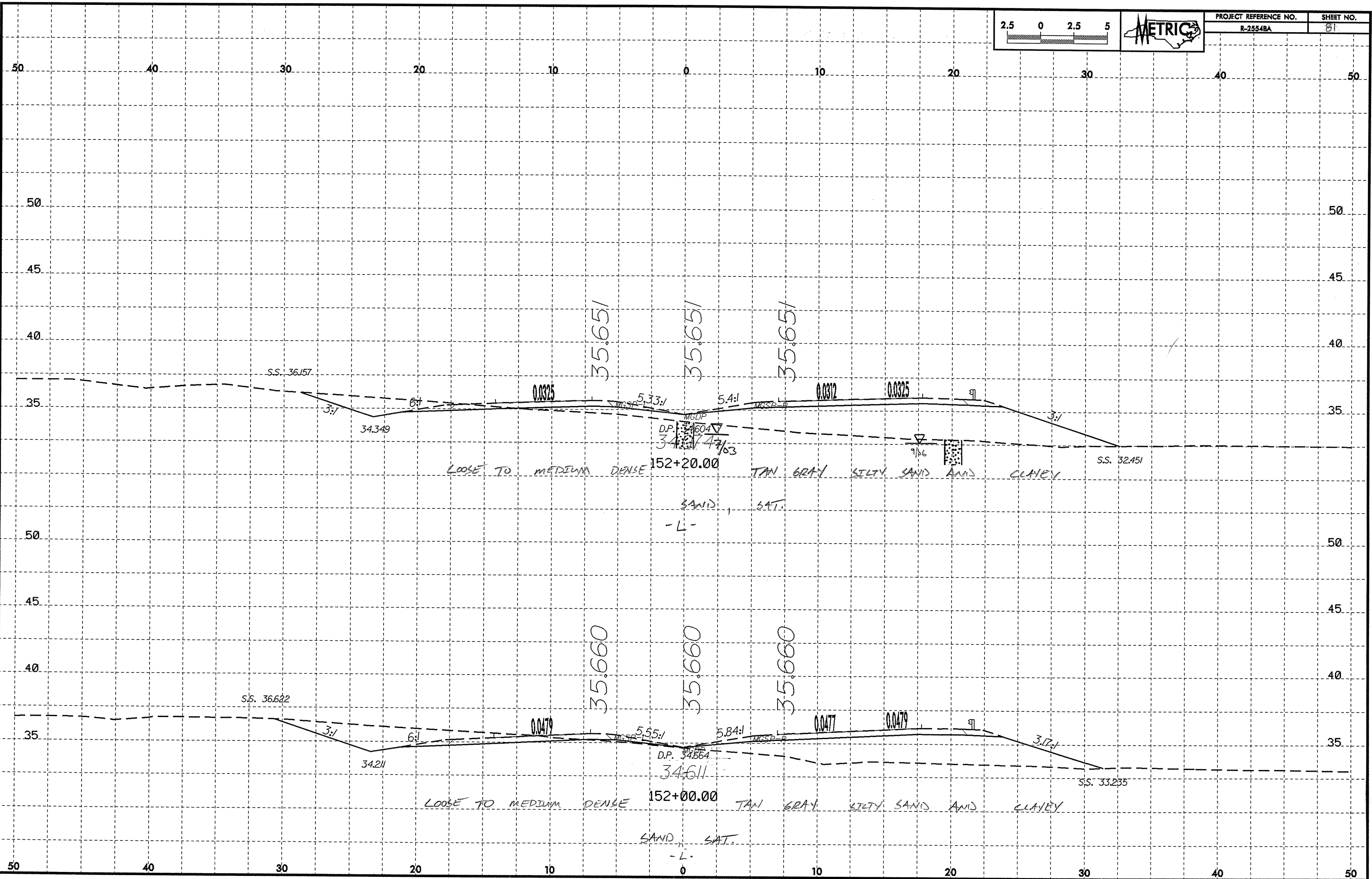
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R-2554BA	80



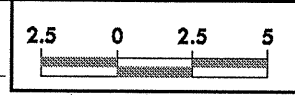
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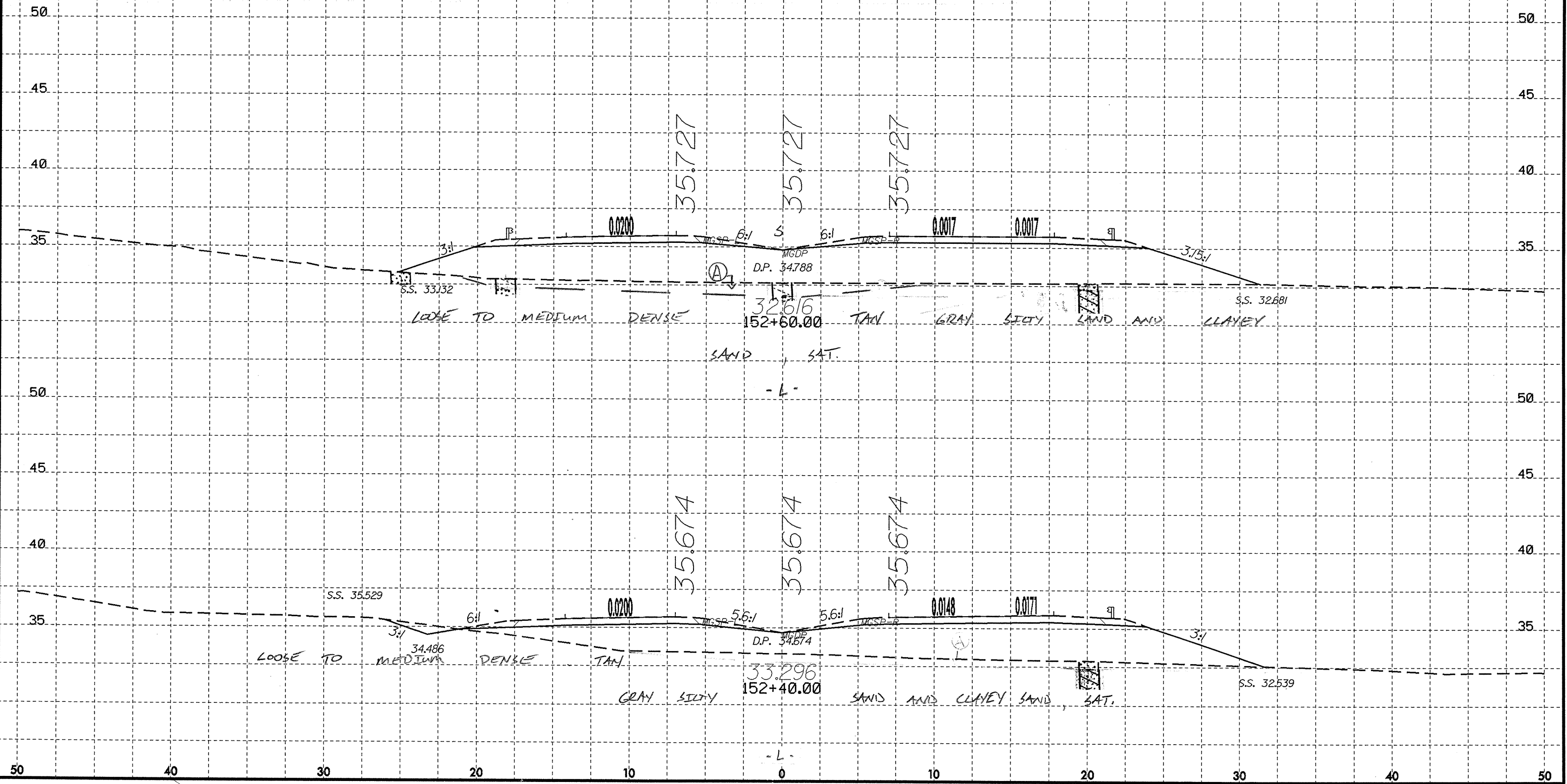


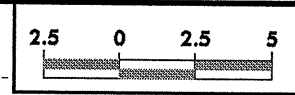
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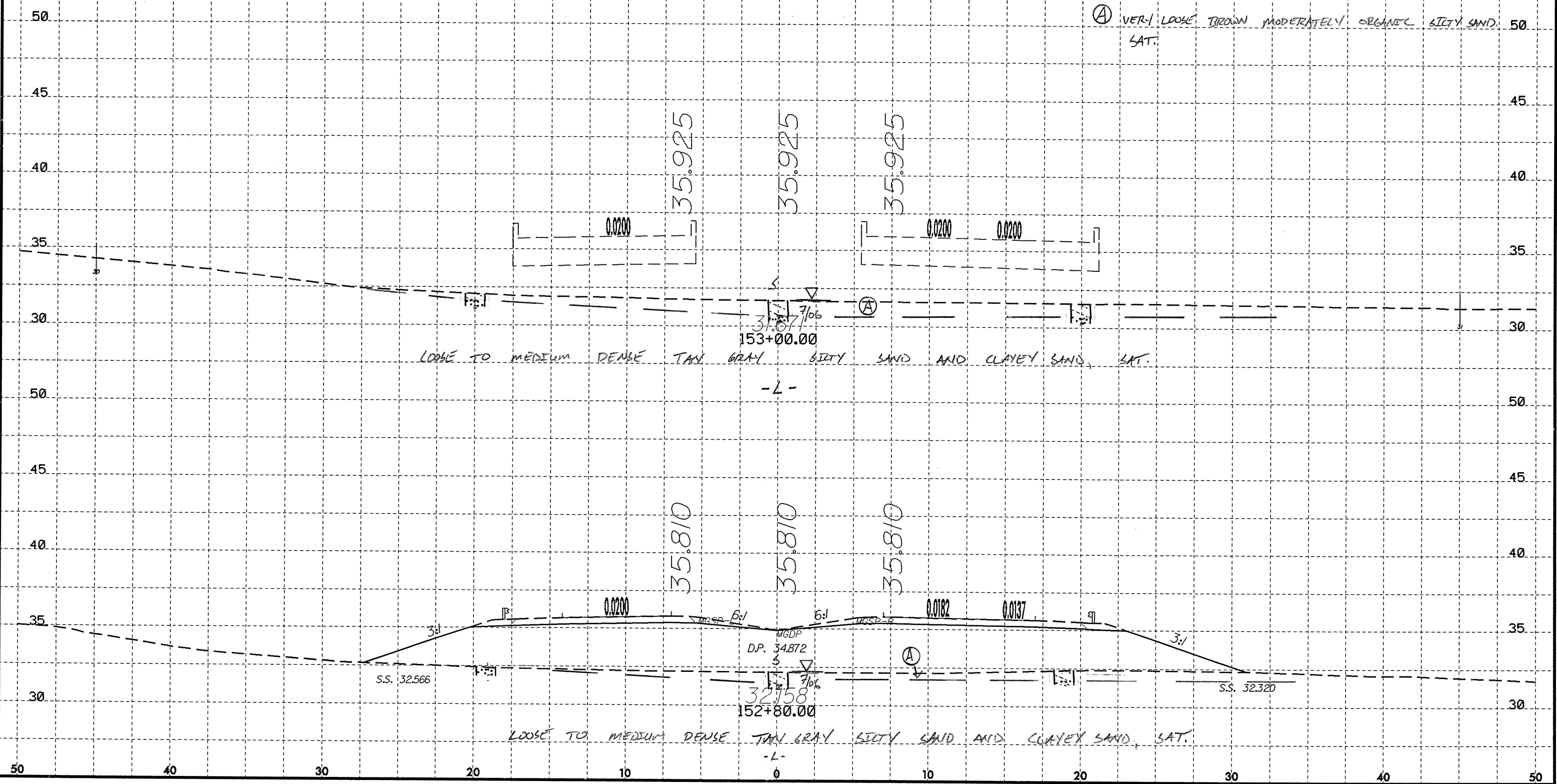
Soil Sample Test Results															
SAMPLE NO.	STATION	OFFSET	DEPTH INTERVAL	AASHTO CLASS.	L.L.	P.I.	% BY WEIGHT				% PASSING (SIEVES)			% MOISTURE	% ORGANIC
							C.SAND	F.SAND	SILT	CLAY	2.0 mm	425 um	75 um		
S-900	152+60	CL	0.31-0.61	A-2-4(0)	28	NP	31.4	51.2	14.8	2.6	99	86	21	22	6

Ⓐ VERY LOOSE BROWN MODERATELY ORGANIC SILTY SAND, SAT.

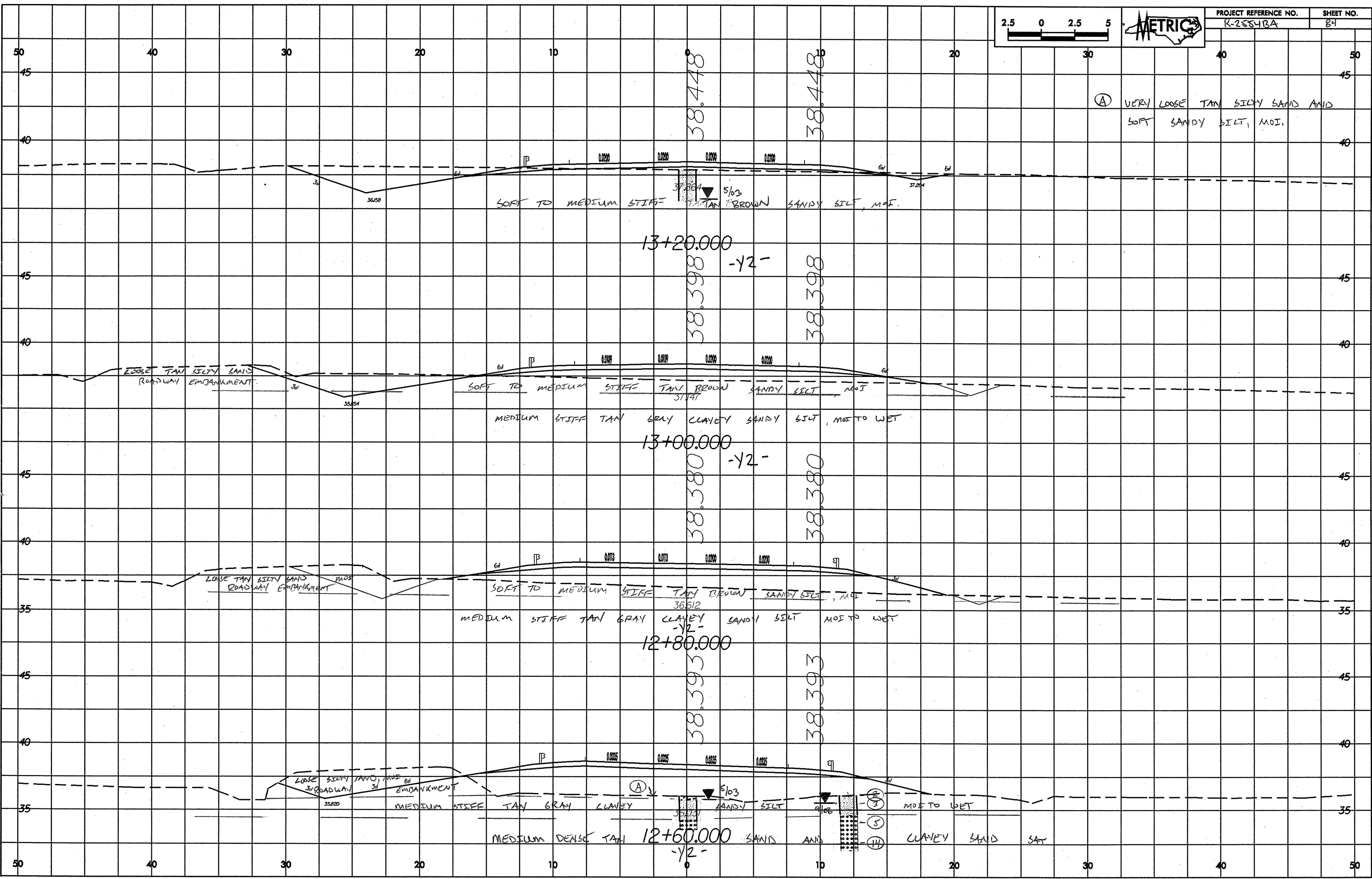
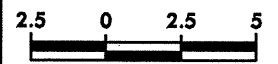




Soil Sample Test Results															
SAMPLE NO.	STATION	OFFSET	DEPTH INTERVAL	AASHTO CLASS.	L.L.	P.I.	% BY WEIGHT				% PASSING (SIEVES)			% MOISTURE	% ORGANIC
							C.SAND	F.SAND	SILT	CLAY	2.0 mm	425 um	75 um		
S-901	153+00	CL	0.31-0.61	A-2-4(0)	23	NP	45.4	39.8	10.2	4.6	99	75	17	40	10



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