NOTE: SEE SHEET IA FOR PLAN SHEET LAYOUT AT TIME OF INVESTIGATION

**CONTENTS** 

**STATION** 

PLAN PROFILE XSECT

SOIL SAMPLE DATA PAGE 5 STATE OF NORTH CAROLINA

DEPARTMENT OF TRANSPORTATION DIVISION OF HIGHWAYS GEOTECHNICAL ENGINEERING UNIT

## ROADWAY SUBSURFACE INVESTIGATION

PROJ. REFERENCE NO. 33615.1.1 (B-4274)

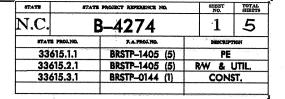
\_\_ F.A. PROJ. *BRSTP-1405(5*)

COUNTY SCOTLAND

PROJECT DESCRIPTION BRIDGE 14 ON NC 144 OVER

BIG SHOE HEEL CREEK

## **INVENTORY**

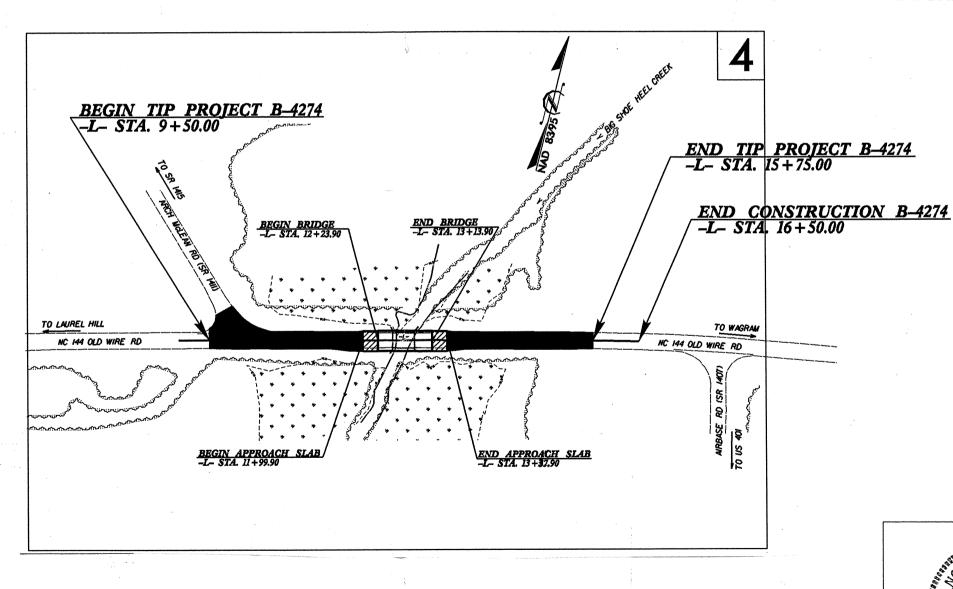


#### CAUTION NOTICE

THE SUBSURFACE INFORMATION AND THE SUBSURFACE INVESTIGATION ON WHICH IT IS BASED WERE MADE FOR THE PURPOSE OF STUDY, PLANNING, AND DESIGN, AND NOT FOR CONSTRUCTION OR PAY PURPOSES. THE VARIOUS FIELD BORING LOGS, ROCK CORES, AND SOIL TEST DATA AVAILABLE MAY BE REVIEWED OR INSPECTED IN RALEIGH BY CONTACTING THE N.C. OEPERTHENT OF TRANSPORTATION, GEOTECHNICAL ENGINEERING UNIT AT (919) 250-4088. NEITHER THE SUBSURFACE PLANS AND REPORTS, NOR THE FIELD BORING LOGS, ROCK CORES, OR SOIL TEST DATA ARE PART OF THE CONTRACT.

CENERAL SOIL AND ROCK STRATA DESCRIPTIONS AND INDICATED BOUNDARIES ARE BASED ON A GEOTECHNICAL INTERPRETATION OF ALL AVAILABLE SUBSURFACE DATA AND MAY NOT NECESSARILY REFLECT THE ACTUAL SUBSURFACE CONDITIONS BETWEEN BORINGS OR BETWEEN SAMPLED STRATA WITHIN THE BORFHOLE. THE LABORATORY SAMPLE DATA AND THE IN STITU (IN-PLACED IEST DATA CAN BE RELIED ON ONLY TO THE DEGREE OF RELIABILITY INHERENT IN THE STANDARD TEST METHOD. THE OSSERVED WATER LEVELS OR SOIL MOISTURE CONDITIONS INDICATED IN THE SUBSURFACE OF SOIL MOISTURE CONDITIONS ARE AS RECORDED AT THE TIME OF THE INVESTIGATION, THESE WATER LEVELS OR SOIL MOISTURE CONDITIONS ARE AS RECORDED AT THE TIME OF THE INVESTIGATION, THESE WATER LEVELS OR SOIL MOISTURE CONDITIONS MAY VARY CONSIDERABLY WITH TIME ACCORDING TO CLIMATIC CONDITIONS INCLUDING TEMPERATURES DESCRIPTIONS AND WATER MONACHINATIC EACTORINATIC CONDITIONS INCLUDING TEMPERATURES, PRECIPITATION, AND WIND, AS WELL AS OTHER NON-CLIMATIC FACTORS.

THE BIDDER OR CONTRACTOR IS CAUTIONED THAT DETAILS SHOWN ON THE SUBSURFACE PLANS THE BIDDER OR CONTRACTOR IS CAUTIONED THAT DETAILS SHOWN ON THE SUBSURFACE PLANS ARE PRELIMMARY ONLY AND IN MANY CASES THE FINAL DESIGN DETAILS ARE DIFFERENT, FOR BIDDING AND CONSTRUCTION PURPOSES, REFER TO THE CONSTRUCTION PLANS AND DOCUMENTS FOR FINAL DESIGN INFORMATION ON THIS PROJECT, THE DEPARTMENT DOES NOT WARRANT OR ADMANCE THE SUFFICIENCY OR ACCURACY OF THE INVESTIGATION MAD, NOR THE INTERPRETATIONS MADE, OR OPINION OF THE DEPARTMENT AS TO THE TYPE OF MATERIALS AND CONDITIONS TO BE ENCOUNTERED. THE BIDDER OR CONTRACTOR IS CAUTIONED TO MAKE SUCH INDEPENDENT USBURFACE INVESTIGATIONS AS HE DEEMS NECESSARY TO SATISFY HIMSELF AS TO CONDITIONS TO BE ENCOUNTERED ON THIS PROJECT. THE CONTRACTOR SHALL HAVE NO CLAIM FOR ADDITIONAL COMPENSATION OR FOR AN EXTENSION OF TIME FOR ANY REASON RESULTING FROM THE ACTUAL CONDITIONS ENCOUNTERED AT THE SITE DIFFERING FROM THOSE INDICATED IN THE SUBSURFACE INFORMATION.



PERSONNEL LITTLE

MURRAY

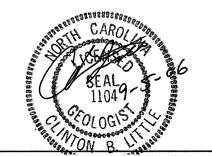
**ESTEP** HARPER

INVESTIGATED BY LITTLE

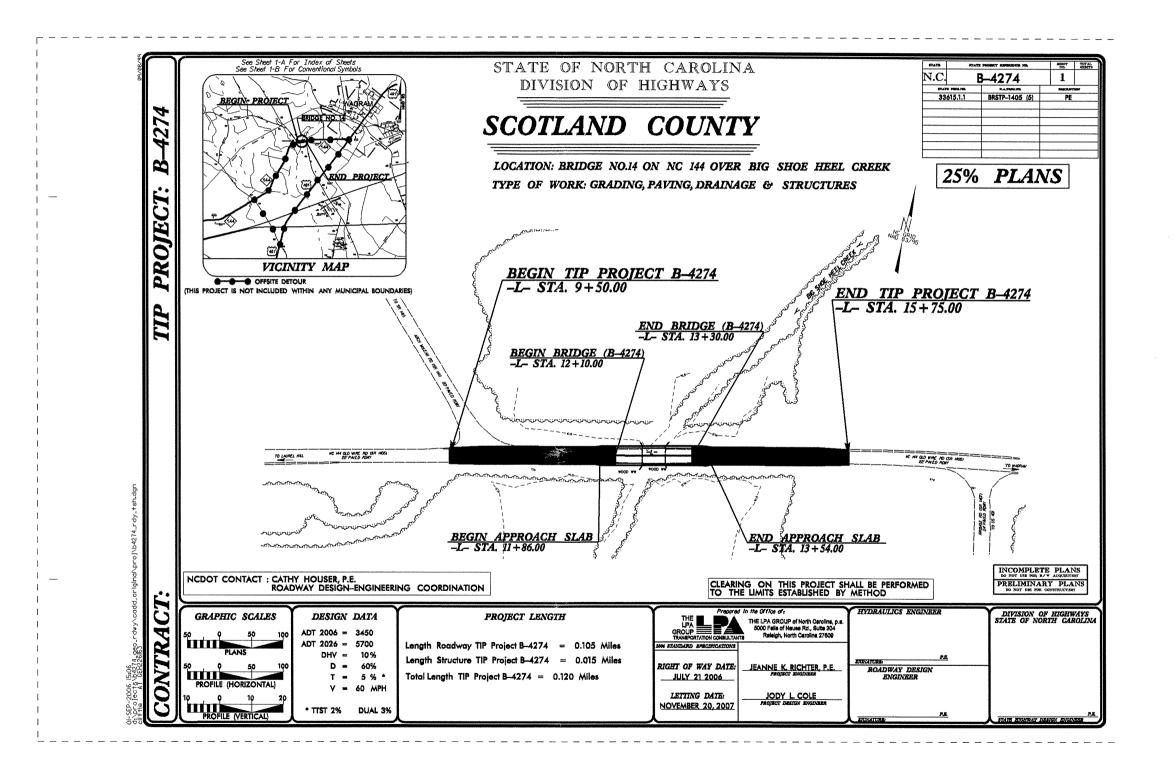
McCLURE

SUBMITTED BY\_ LITTLE

SEPTEMBER 2006



C201896



#### NORTH CAROLINA DEPARTMENT OF TRANSPORTATION

### DIVISION OF HIGHWAYS

GEOTECHNICAL ENGINEERING UNIT

## SUBSURFACE INVESTIGATION

Column   C	SOIL AND ROCK LEGEND, TERMS, SYMBOLS, AND ABBREVIATIONS											
### 1985   1985	SOIL DESCRIPTION			TERMS AND DEFINITIONS								
Column   C	SOIL IS CONSIDERED TO BE THE UNCONSOLIDATED, SEMI-CONSOLIDATED, OR WEATHERED EARTH MATERIALS THAT CAN BE PENETRATED WITH A CONTINUOUS FLIGHT POWER AUGER, AND VIELD LESS THAN 180 BLOWS PER FOOT ACCORDING TO STANDARD PENETRATION TEST (AASHTO 126, ASTM D-1560, SOIL CLASSIFICATION IS BASED ON THE AASHTO SYSTEM, BASIC DESCRIPTIONS GENERALLY SHALL INCLUDE. CONSISTENCY, COLOR, TEXTURE, AND OUSTURE, ARSHTO CLASSIFICATION, AND OTHER PERTINENT FACTORS SUCH AS MINERAL GOOGLAL COMPOSITION, AND LABITY, STRUCTURE, PLASTICITY, ETC. EXAMPLE:	UNIFORM - INDICATES THAT SOIL PARTICLES ARE ALL APPROXIMATELY THE SAME SIZE. (ALSO PODRLY GRADED)  OAP-GRADED - INDICATES A MIXTURE OF UNIFORM PARTICLES OF TWO OR MORE SIZES.  ANGULARITY OF GRAINS  THE ANGULARITY OR ROUNDNESS OF SOIL GRAINS IS DESIGNATED BY THE TERMS, ANGULAR,	ROCK LINE INDICATES THE LEVEL AT WHICH NON-COASTAL PLAIN MATERIAL WOULD YIELD SPT REFUSAL.  SPT REFUSAL IS PENETRATION BY A SPLIT SPOON SAMPLER BOUAL TO OR LESS THAN 0.1 FOOT PER 60 BLOWS.  IN NON-COASTAL PLAIN MATERIAL. THE TRANSITION BETWEEN SOIL AND ROCK IS OFTEN REPRESENTED BY A ZONG OF WEATHERED ROCK.  ROCK MATERIALS ARE TYPICALLY DIVIDED AS FOLLOWS:  WEATHERED  NON-COASTAL PLAIN MATERIAL THAT WOULD YIELD SPT N YALUES > 100	AQUIFER - A WATER BEARING FORMATION OR STRATA.  ARENACEOUS - APPLIED TO ROCKS THAT HAVE BEEN DERIVED FROM SAND OR THAT CONTAIN SAND.  ARGILLACEOUS - APPLIED TO ALL ROCKS OR SUBSTANCES COMPOSED OF CLAY MINERALS,  OR HAVING A NOTABLE PROPORTION OF CLAY IN THEIR COMPOSITION, AS SHALE, SLATE, ETC.								
The contract   Contr		MINERALOGICAL COMPOSITION	55, 55, 51, 51, 50, 51, 51, 51, 51, 51, 51, 51, 51, 51, 51	AT WHICH IT IS ENCOUNTERED, BUT WHICH DOES NOT NECESSARILY RISE TO OR ABOVE THE								
Column   C	GENERAL GRANULAR MATERIALS SILT-CLAY MATERIALS ODCANG MATERIALS	WHENEVER THEY ARE CONSIDERED OF SIGNIFICANCE.	WOULD YIELD SPT REFUSAL IF TESTED, ROCK TYPE INCLUDES GRANITE, GREISS, GABRON, SCHISTE, ETC.  GREISS, GABRON, SCHISTE, ETC.									
The column   The		SLIGHTLY COMPRESSIBLE LIQUID LIMIT LESS THAN 31	ROCK (NCR) SEDIMENTARY ROCK THAT WOULD YEILD SPT REFUSAL IF TESTED. ROCK TYPE INCLUDES PHYLLITE, SLATE, SANDSTONE, ETC.	OF SLOPE.								
The column	555560000000000000000000000000000000000	HIGHLY COMPRESSIBLE LIQUID LIMIT GREATER THAN 50		LENGTH OF CORE RUN AND EXPRESSED AS A PERCENTAGE.								
1.00   1.00	# 10 50 MX GRANULAR CLAY PEAT	ODCANIC MATERIAL GRANULAR SILT - CLAY		ROCKS OR CUTS MASSIVE ROCK.								
Secretary   West   We	* 200 15 HX 25 HX 10 HX 35 HX 35 HX 35 HX 35 HX 35 HX 36 HN	TRACE OF ORGANIC MATTER 2 - 3% 3 - 5% TRACE 1 - 10%	HAMMER IF CRYSTALLINE.									
The contract of the contract	PLASTIC INDEX 6 MX NP 18 MX 10 MX 11 MN 11 MN 10 MX 10 MX 11 MN 11 MN LITTLE OR HIGHLY	MDDERATELY ORGANIC         5 - 10%         12 - 20%         SOME         20 - 35%           HIGHLY ORGANIC         20%         20%         HIGHLY         35%         AND ABOVE	(V SLI.) CRYSTALS ON A BROKEN SPECIMEN FACE SHINE BRIGHTLY. ROCK RINGS UNDER HAMMER BLOWS IF	THE LINE OF DIP, MEASURED CLOCKWISE FROM NORTH.								
Warrier   Warr	USUAL TYPES STONE FRAGS. FINE SILTY OR CLAYEY SILTY CLAYEY ORGANIC		(SLI,) 1 INCH, OPEN JOINTS MAY CONTAIN CLAY, IN GRANITOID ROCKS SOME OCCASIONAL FELDSPAR	SIDES RELATIVE TO ONE ANOTHER PARALLEL TO THE FRACTURE.								
Second Continue	OF MAJUR UNREVEL, AND SAND GRAVEL AND SAND SOILS SOILS MATTER	——————————————————————————————————————	MODERATE SIGNIFICANT PORTIONS OF ROCK SHOW DISCOLORATION AND WEATHERING EFFECTS. IN	FLOAT - ROCK FRAGMENTS ON SURFACE NEAR THEIR ORIGINAL POSITION AND DISLODGED FROM								
PROPERTY   CONTINUES   CONTI	AS A EXCELLENT TO GOOD FAIR TO POOR POOR UNSUITAB	LE CONTRACTOR OF THE CONTRACTO	DULL SOUND UNDER HAMMER BLOWS AND SHOWS SIGNIFICANT LOSS OF STRENGTH AS COMPARED WITH FRESH ROCK.	FLOOD PLAIN (FP) - LAND BORDERING A STREAM, BUILT OF SEDIMENTS DEPOSITED BY								
Part   Company	CONSISTENCY OR DENSENESS		SEVERE AND DISCOLORED AND A MAJORITY SHOW KAOLINIZATION, ROCK SHOWS SEVERE LOSS OF STRENGTH	FORMATION (FM.) - A MAPPABLE GEOLOGIC UNIT THAT CAN BE RECOGNIZED AND TRACED IN								
Second Column   Col	PRIMARY SOIL TYPE COMPACTNESS OR PENETRATION RESISTENCE COMPRESSIVE STRENGTH	ROADWAY EMBANKMENT (RE)  POPT DOT TEST BORING  DESIGNATION  SAMPLE  DESIGNATION  DESIGNATION	S IF TESTED, WOULD YIELD SPT REFUSAL	JOINT - FRACTURE IN ROCK ALONG WHICH NO APPRECIABLE MOVEMENT HAS OCCURRED.								
## STITICAL FLA SPYCHOSE STATE AND SPECIAL SPYCHOSE STATE AND SPYCHOS	CENERALLY VERY LOOSE (4	S - BULK SAMPLE  AUGER BORING	(SEV.) IN STRENGTH TO STRONG SOIL. IN GRANITOID ROCKS ALL FELDSPARS ARE KADLINIZED TO SOME	LEDGE " A SHELF-LIKE RIDGE OR PROJECTION OF ROCK WHOSE THICKNESS IS SMALL COMPARED TO								
## ## ## ## ## ## ## ## ## ## ## ## ##	MATERIAL (NDN-COHESIVE)  GENSE  WERY DENSE  MATERIAL  DENSE  MON-COHESIVE)  N/A  N/A  N/A  N/A  N/A  N/A  N/A  N/	ARTIFICIAL FILL (AF) OTHER THAN ROADWAY EMBANKMENT  INFERRED SOIL BOUNDARY  MY MONITORING VELL  SAMPLE  SAMPLE  SAMPLE	IF TESTED, YIELDS SPT N VALUES > 180 BPF  VERY SEVERE ALL ROCK EXCEPT QUARTZ DISCOLORED OR STAINED. ROCK FABRIC ELEMENTS ARE DISCERNIBLE BUT (V SEV.) THE MASS IS EFFECTIVELY REDUCED TO SOIL STATUS, WITH ONLY FRAGMENTS OF STRONG ROCK REMAININD, SAPROLITE IS AN EXAMPLE OF ROCK WEATHERED TO A DEGREE SUCH THAT ONLY MINOR	MOTILED (MOT.) - IRREGULARLY MARKED WITH SPOTS OF DIFFERENT COLORS MOTILING IN SOILS USUALLY INDICATES POOR AERATION AND LACK OF GOOD DRAINAGE.  PERCHED WATER - WATER MAINTAINED ABOVE THE NORMAL GROUND WATER LEVEL BY THE PRESENCE OF AN								
TEXTURE OR GRAIN SIZE	SILT-CLAY         MEDIUM STIFF         4 TO 8         0.5 TO 1.0           MATERIAL         STIFF         8 TO 15         1 TO 2	INFERRED ROCK LINE  ALLUVIAL SOIL BOUNDARY  TYPE ALLUVIAL SOIL BOUNDARY  ALLUVIAL SOIL BOUNDARY  ALLUVIAL SOIL BOUNDARY	TRIAXIAL COMPLETE ROCK REDUCED TO SOIL. ROCK FABRIC NOT DISCERNIBLE, OR DISCERNIBLE ONLY IN SMALL AND SCATTERED CONCENTRATIONS, QUARTZ MAY BE PRESENT AS DIKES OR STRINGERS, SAPROLITE IS	RESIDUAL (RES.) SOIL - SOIL FORMED IN PLACE BY THE WEATHERING OF ROCK.								
STREET	HARD >30 >4	25/025 DIP & DIP DIRECTION OF INSTALLATION CBR - CALIFORNIA E	EARING POCK HADDNESS	ROCK SEGMENTS EQUAL TO OR GREATER THAN 4 INCHES DIVIDED BY THE TOTAL LENGTH OF CORE RUN AND								
### APP 240 8.47 8.29 8.47 8.29 8.47 8.29 8.47 8.29 8.47 8.29 8.47 8.29 8.47 8.29 8.47 8.29 8.47 8.29 8.47 8.29 8.47 8.29 8.49 8.29 8.49 8.49 8.40 8.40 8.40 8.40 8.40 8.40 8.40 8.40												
SOLUTION OF LINE COURSE OWNER OF THE COURSE OWNER OW	OPENING (MM) 4.76 2.00 0.42 0.25 0.075 0.053	ABBREVIATIONS	HARD CAN BE SCRATCHED BY KNIFE OR PICK ONLY WITH DIFFICULTY, HARD HAMMER BLOWS REQUIRED TO DETACH HAND SPECIMEN.	RELATIVELY THIN COMPARED WITH ITS LATERAL EXTENT, THAT HAS BEEN EMPLACED PARALLEL								
SST. MOSTURE - CORRELATION OF TERMS  SOIL MOSTURE - CORRELATION OF THE MOSTURE - CORR	BOULDER CDBBLE GRAVEL SAND SAND SIL1 CLAY (BLDR.) (COB.) (GR.) (CSE. SD.) (F SD.) (SL.) (CL.)	BT - BORING TERMINATED MED MEDIUM V - VERY	MODERATELY CAN BE SCRATCHED BY KNIFE OR PICK, GOUGES OR GROOVES TO 0.25 INCHES DEEP CAN BE HARD EXCAVATED BY HARD BLOW OF A GEOLOGIST'S PICK, HAND SPECIMENS CAN BE DETACHED	SLICKENSIDE - POLISHED AND STRIATED SURFACE THAT RESULTS FROM FRICTION ALONG A FAULT OR								
SOL MOISTURE - CORRELATION OF TERMS  SOL MOISTURE SCALE FIELD MOISTURE FIELD MOIS		CSE COARSE NP - NON PLASTIC 7 - UNIT WEIGHT	MEDIUM CAN BE GROOVED OR GOUGED 0.05 INCHES DEEP BY FIRM PRESSURE OF KNIFE OR PICK POINT.	STANDARD PENETRATION TEST (PENETRATION RESISTANCE) (SPT) - NUMBER OF BLOWS (N OR BPF) OF								
STREAM CORE CLIMITS IN DESCRIPTION OF THE PROPERTY OF STREAM PROPERTY	COLL MOTOTINE COME. STELD MOTOTINE	DPT - DYNAMIC PENETRATION TEST PMT - PRESSUREMETER TEST	POINT OF A GEOLOGIST'S PICK.	A 2 INCH OUTSIDE DIAMETER SPLIT SPOON SAMPLER, SPT REFUSAL IS PENETRATION EQUAL TO OR LESS								
CAN BE CANYED WITH MORE CAN BE EXAMATED REQUITY HIS PORT OF PICK, PECES I NICH PRASS, FRANCHERS SLI. SLIGHTLY SOFT ON HORE IN THICKNESS CENTRATED REGIOUS WITH FORT OF PICK, PECES I NICH PRASS, FRANCHERS ST. STRICT SOFT ON HORE IN THICKNESS CENTRATED REGIOUS WITH FORT OF PICK, PECES I NICH PRASS, FRANCHERS ST. STRICT SOFT ON HORE IN THICKNESS CENTRATED REGIOUS WITH FORT OF PICK, PECES I NICH PRASS, FRANCHERS ST. STRICT SOFT ON HORE IN THICKNESS CENTRATED REGIOUS WITH FORT OF PICK, PECES I NICH PRASS, FRANCHERS ST. STRICT SOFT ON HORE IN THICKNESS CENTRATED REGIOUS WITH FORT OF PICK, PECES I NICH PRASS, FRANCHERS ST. STRICT SOFT ON HORE IN THICKNESS CENTRATED REGIOUS WITH FORT OF PICK, PECES I NICH PRASS, FRANCHERS ST. STRICT SOFT ON HORE IN THICKNESS CENTRATED REGION AND EXPRESSED AS A ROWARD AND AND AND AND AND AND AND AND AND AN	(ATTERBERG LIMITS) DESCRIPTION BOIDE FOR FIELD MOISTONE DESCRIPTION	F - FINE SD SAND, SANDY	FROM CHIPS TO SEVERAL INCHES IN SIZE BY MODERATE BLOWS OF A PICK POINT. SMALL, THIN	OF STRATUM AND EXPRESSED AS A PERCENTAGE.								
Set/SOLIDe RECOURSE DRYING TO ATTAIN OPTIMEM MOISTURE  ONLY OF THINK MOISTURE  ONLY OF THINK MOISTURE  SHRINGCE CHARGE OF THINK OF THINK MOISTURE  ONLY OF THINK MOISTURE  SHRINGCE CHARGE CHARGE COLES DRYING TO SUBJECT PROJECT  FRACTURE SPACING  IEM SECULDS  IEM SEC	(SAT.) FROM BELOW THE GROUND WATER TABLE		SOFT OR MORE IN THICKNESS CAN BE BROKEN BY FINGER PRESSURE. CAN BE SCRATCHED READILY BY	TOTAL LENGTH OF ROCK SEGMENTS WITHIN A STRATUM EQUAL TO OR GREATER THAN 4 INCHES DIVIDED BY THE								
PLASTICITY  PROBLE HIGH  PORTABLE HOIST  PROBLE HIGH  PORTABLE HOIST  PROBLE HORRE WITH HORRER PROBLE WITH STEEL PROBE;  PROBLE HORRE WITH HORRER WITH HORRER  PROBLE HORRE WITH HORRER  PROBLE HORRER  PROBLE HORRE  PROBLE HORRER  PR	RANGE - WET - (W) SEMISOLIDE REGULES DEFINE TO ATTAIN OPTIMUM MOISTURE	EQUIPMENT USED ON SUBJECT PROJECT	FRACTURE SPACING BEDDING									
ON OPTIMUM MOISTURE  SULTATION PRIAM MOISTURE  SULTATION PRESSURE FEET THINLY EBEDED OLIG 1 FEET THINLY EBEDED OLIG 1 FEET THINLY LARIMSTED OLIG	PLC + FLASTIC LIMI	DRILL UNITS:   HOVERCING TOOLST	VERY THICKLY REPORT > 4 FEET	BENCH MARK: _								
REQUIRES ADDITIONAL WATER TO ATTAIN OPTIMUM MOISTURE  PLASTICITY  BY CIONE SIZE:  CORE SIZE:  CLOSE  D.6 TO 1 FEET  THINLY LAMINATED  A.609 - 0.03 FEET	UM T OF LINOR MOISTORE	MOBILE B-	WIDE 3 TO 10 FEET THICKLY BEDDED 1.5 - 4 FEET THINLY BEDDED 0.16 - 1.5 FEET	ELEVATION: _ FT.								
PLASTICITY  PLASTICITY INDEX (P) D DRY STRENGTH  NONPLASTIC  PLASTICITY INDEX (P) D DRY STRENGTH  VELOW PLASTICITY  PLASTICITY INDEX (P) D DRY STRENGTH  VELOW PLASTICITY  POST HOLICAL TO SEPARATE WITH STEEL PROBE;  PLASTICITY  PLASTIC	REDUIRES ADDITIONAL WATER TO	6° CONTINUOUS FLIGHT AUGER CORE SIZE:	CLOSE 0.16 TO 1 FEET VERY THINLY BEDDED 0.003 - 0.16 FEET THICKLY LAMINATED 0.008 - 0.03 FEET	NOTES:								
PLASTICITY INDEX (PI) DRY STRENGTH  NONPLASTIC  0-5 VERY LOW PLASTICITY 10-25 MEDIUM HIGH PLASTICITY 26 OR MORE HIGH  DESCRIPTIONS MAY INCLUDE COLOR OR COLOR COMBINATIONS (TAN, RED, YELLOW-BROWN, BLUE-GRAY).  TUNG,-CARBIDE INSERTS  TUNG,-CARBIDE INSERTS  TUNG,-CARBIDE INSERTS  TUNG,-CARBIDE INSERTS  TUNG,-CARBIDE INSERTS  CASING W/ ADVANCER HAND TOOLS; HAND TOOLS; PORTABLE HOIST  TRICONE STEEL TEETH POST HOLD DIGGER HAND TOOLS; PORTABLE HOIST  TRICONE STEEL TEETH POST HOLD DIGGER HAND TOOLS; PORTABLE HOIST ORALL DIGGER HAND TOOLS; PORTABLE HORN SAMPLE WITH STEEL PROBE; DIFFICULT TO BREAK WITH HAMMER.	PLASTICITY											
NOPLASTICE 0-5 VERY LOW LOW PLASTICITY 6-15 SLIGHT  LOW PLASTICITY 16-25 MEDIUM HIGH PLASTICITY 26 OR MORE HIGH HIGH PLASTICITY 26 OR MORE HIGH  COLOR  DESCRIPTIONS MAY INCLUDE COLOR OR COLOR COMBINATIONS (TAN, RED, YELLOW-BROWN, BLUE-GRAY).    VAME SHEAR TEST   VANE SHEAR TEST		TUNG-CARBIDE INSERTS	<b>!</b>									
MED. PASTICITY  16-25  MEDIUM HIGH HIGH PASTICITY 16-25  MEDIUM HIGH PASTICITY 16-25  MEDIUM HIGH PORTABLE HOIST  TRICONE STEEL TEETH POST HOLE DIGGER HODERATELY INDURATED GRAINS CAN BE SEPARATED FROM SAMPLE WITH STEEL PROBE; BREAKS EASILY WHEN HIT WITH HAMMER.  DESCRIPTIONS MAY INCLUDE COLOR OR COLOR COMBINATIONS (TAN, RED, YELLOW-BROWN, BLUE-GRAY).  TRICONE STEEL TEETH POST HOLE DIGGER HODERATELY INDURATED GRAINS CAN BE SEPARATED FROM SAMPLE WITH STEEL PROBE; DIFFICULT TO SEPARATE WITH STEEL PROBE; DIFFICULT TO BREAK WITH HAMMER.	LOW PLASTICITY 6-15 SLIGHT	X CME-550										
COLOR  DESCRIPTIONS MAY INCLUDE COLOR OR COLOR COMBINATIONS (TAN, RED, YELLOW-BROWN, BLUE-GRAY).  TRICONE 'TUNG,-CARB.	MED. PLASTICITY 16-25 MEDIUM	PORTABLE HOIST TRICONE STEEL TEETH POST HOLE DIGGE										
DESCRIPTIONS MAY INCLUDE COLOR OR COLOR COMBINATIONS (TAN, RED, YELLOW-BROWN, BLUE-GRAY).	COLOR		INDURATED GRAINS ARE DIFFICULT TO SEPARATE WITH STEEL PROBE;									
		L_  CURE BII	DIFFICULT TO BREAK WITH HAMMER.									

PROJECT REFERENCE NO. 33615.1.1

SHEET NO.



# STATE OF NORTH CAROLINA DEPARTMENT OF TRANSPORTATION

MICHAEL F. EASLEY
GOVERNOR

LYNDO TIPPETT SECRETARY

September 1, 2006

STATE PROJECT:

33615.1.1 (B-4274)

FEDERAL PROJECT:

BRSTP-1405(5)

COUNTY:

Scotland

DESCRIPTION:

Bridge 14 on NC 144 over Big Shoe Heel Creek

SUBJECT:

Geotechnical Report - Inventory

#### PROJECT DESCRIPTION

The project is located on NC 144 (Old Wire Road, SR 1405) just north of US 401 near the town of Wagram. The project will replace the bridge over Big Shoe Heel Creek. This report addresses the roadway approaches from Station 9+50 to 15+75 –L-.

The Geotechnical field investigation was conducted in August of 2006. Four Standard Penetration Test borings were performed through the existing pavement with a CME 550 drill machine using 8" hollow stem augers and an automatic drop hammer. Representative soil samples were collected and tested for grain size and Atterburg limits. Three samples were also tested for organic content.

#### AREAS OF SPECIAL GEOTECHNICAL INTEREST

Organic Soils: The recent alluvial soils contain layers of organic sands. The organic content of the three samples tested were approximately 5%, 20%, and 28%. Granular soils with organic content greater than 10% are considered muck and are generally unsuitable for highway construction. These soils were encountered at depths of six to eight feet below the existing roadway grade. The layer thickness was between two and eight feet. Some of the split spoon samples contained significant quantities of wood.

<u>Alluvial Soils</u>: Alluvial soils were encountered in all borings, immediately below roadway embankment fill. The upper portion consists of medium dense sand and clayey sand overlying organic sands as discussed above.

<u>Groundwater:</u> Groundwater levels will be seasonally variable. Zero hour readings in the test borings were at six feet and three feet. Groundwater is likely to be present within three feet below the proposed grade during wet weather periods.

<u>Wetlands</u>: Wetlands are mapped immediately adjacent to the roadway, on both sides, for most of the length of the project. These areas contain standing surface water or very shallow groundwater and muck soils.

#### PHYSIOGRAPHY AND GEOLOGY

The project is located in the coastal plain region of North Carolina. It is within the floodplain of Big Shoe Heel Creek. The floodplain contains recent sediments (alluvium) consisting of alluvial sands and granular organic soils as previously discussed. The overall thickness of recent sediments is about twelve feet. Below the sediments are coastal plain soils of the Middendorf Formation. Our test borings terminated with minimal penetration into the Middendorf soils. They were sampled as white medium dense coarse sands.

#### **SOIL PROPERTIES**

Roadway Fill Soils

These soils occur across the site. They consist of three feet of tan loose moist sand.

Alluvial Soils

Again, the recent alluvial deposit averages twelve feet thick with loose to medium dense saturated sand and clayey sand over organic sands. The two borings on the east side encountered a basal sand and coarse sand layer three feet thick.

Coastal Plain Soils

Coastal Plain soils were encountered below the recent alluvium at depths of 12 to 15 feet below the existing roadway grade. They consisted of white medium dense coarse sand.

Respectfully submitted

Clint Little

Regional Geological Engineer

## **EARTHWORK BALANCE SHEET**

Volumes in Cubic Yards

PROJECT TIP # B-4274

COUNTY Scotland

DATE 6/3/2008

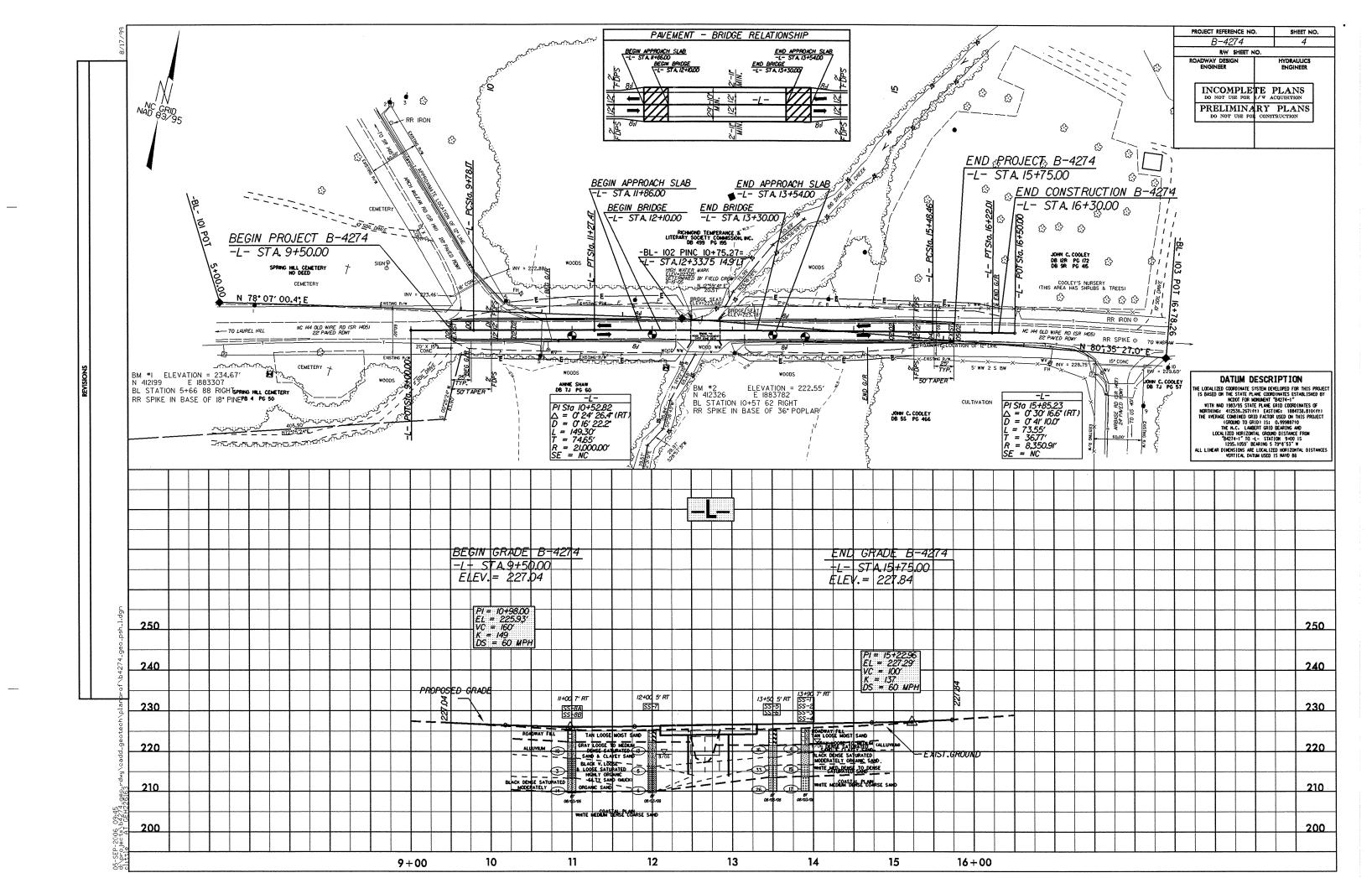
SHEET 1 OF 1

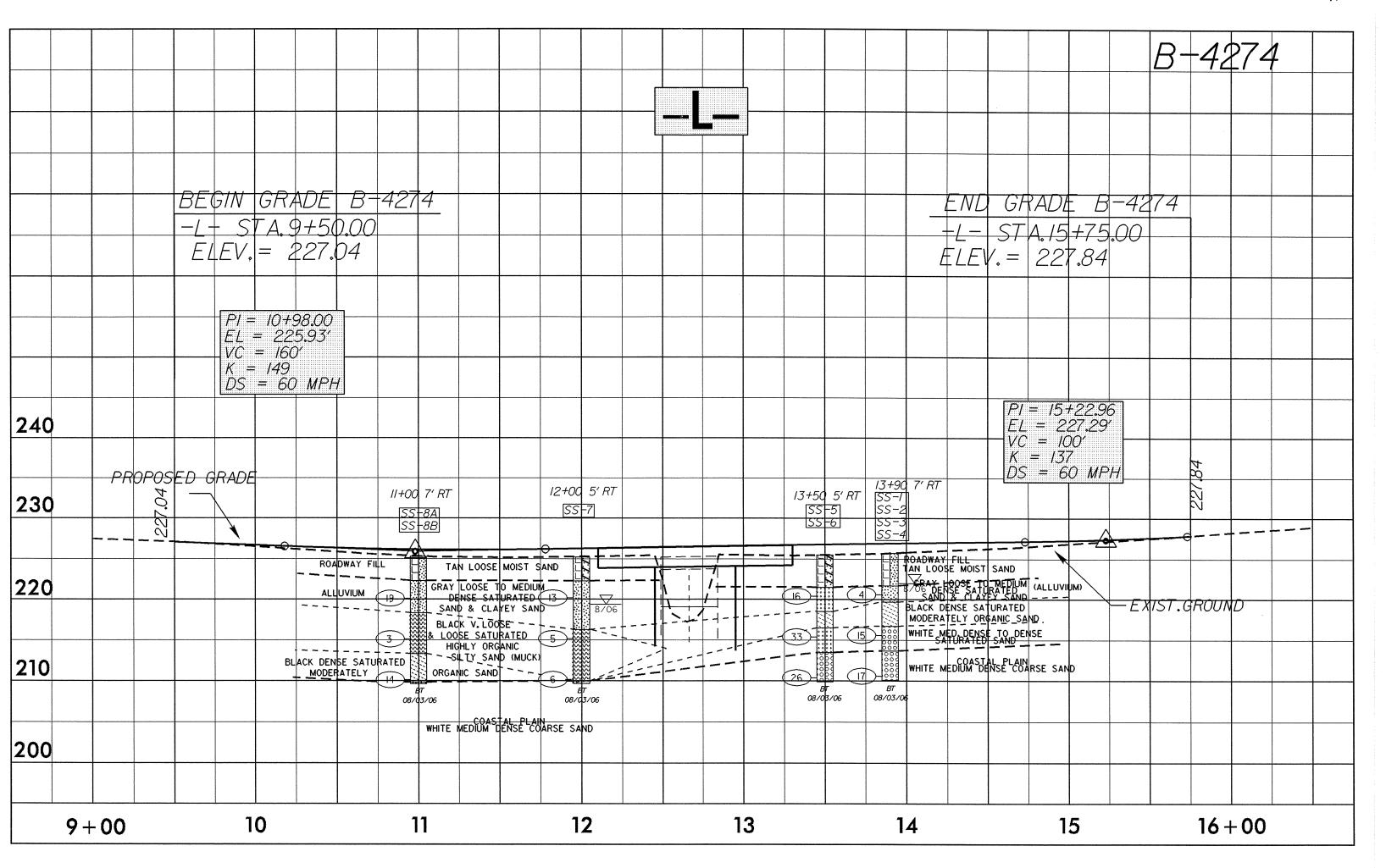
LINE	STATION	STATION	TOTAL EXCAV. (UNCL.)	ROCK EXCAV.	UNDERCUT EXCAV.	UNSUIT. EXCAV.	SUITABLE EXCAV.	TOTAL EMB.	ROCK EMB.	UNDERCUT EMB.	EARTH EMB.	EMBANK. 15%	BORROW	SUITABLE WASTE	UNSUIT. WASTE	TOTAL WASTE
-L-	9+50.00	12+23.90 BEGIN BRIDGE	68	0	0	0	68	268	0	0	268	308	240	0	0	0
-L-	13+13.90 END BRIDGE	15+75.00	110	0	0	. 0	110	230	0	0	230	265	155	0	0	0
															·	
PROJECT SUBTOTAL		178	0	0	0	178	498	0	0	498	573	395	0	0	0	
ADDITIONAL UNDERCUT SHOULDER MATERIAL																
WASTE IN LIEU OF BORROW LOSS DUE TO CLEARING & GRUBBING										,						
PROJECT TOTAL		178	0	0	0	178	498	0	0	498	573	395	0	0	0	
EST 5%	TO REPLACE BORROW	TOP SOIL ON PIT											20			
GRAND TOTAL		178										415				
SAY			200									-	450			

EST. UNDERCUT EXCAVATION = 400 C.Y.

EST. SELECT GRANULAR FILL = 750 C.Y.

<sup>\*</sup> EARTHWORK QUANTITIES ARE CALCULATED BY THE ROADWAY DESIGN UNIT. THESE EARTHWORK QUANTITIES ARE BASED IN PART ON SUBSURFACE DATA PROVIDED BY THE GEOTECHNICAL ENGINEERING UNIT.





#### NORTH CAROLINA DEPARTMENT OF TRANSPORTATION DIVISION OF HIGHWAY MATERIALS & TESTS UNIT SOILS LABORATORY

#### NORTH CAROLINA DEPARTMENT OF TRANSPORTATION DIVISION OF HIGHWAY MATERIALS & TESTS UNIT **SOILS LABORATORY**

T. I. P. No.	B-4274  REPORT ON SAMPLES OF SOILS FOR QUALITY								B-4274								
									REPORT O	N SAMI	PLES OF	SOILS FOR QUALITY					
Project	3361511	County	SCOTLA	ND	Owner			<b>Project</b> 3361511			County	SCOTLAND		Owner			
	8/4/06		8/14/06		<b>Reported</b> 8/16/06			Date: Sample	8/4/06			8/14/06		Reported 8/16/06			
												C C MURRAY					
	N WAINAINA			- -			pecifications	Sampled from Submitted by N WAINAINA					- -	1995 Standard Specifications			
732143 TO 732 9/1/06	177		a a-					732143 TO 73 9/1/06	32177								
			ST RESU									ST RESU		_	-		
Proj. Sample N		SS-1	SS-2	SS-3	SS-4	SS-5	SS-6	Proj. Sample			SS-7	SS-8A	SS-8B		<u> </u>		
Lab. Sample N		732143	732144	732145	732146	732147	732148	Lab. Sample			732149	732150	732151			<u> </u>	
Retained #4 S			99	4	100	99	97	Retained #4		% %	97	- 00	1 05		<b>-</b>		
Passing #10 S Passing #40 S			61	81 30	44	59	55	Passing #10 Passing #40		<del>%</del>	63	96	95 75	<del> </del>	<del> </del>	-	
Passing #200		13	17	3	15	7	5	Passing #20		%	21	29	30	<del> </del>	<u> </u>		
1 000011B W 2 0 0			NO. 10 FR				L			, ,		NO. 10 FR	<u> </u>			<u> </u>	
SOIL MORTA	AP - 100%	MIINUS	NO. 10 FR	ACTION				SOIL MORT	NO. IU FR	ACTION	T	T					
Coarse Sand		54.0	60.5	84.0	75.9	66.3	69.5		nd Ret - #60	%	55.1	47.1	40.3	-	<del> </del>	<del> </del>	
Fine Sand R			25.0	12.9	10.4	29.2	27.0		Ret - #270	%	26.6	25.6	31.6	+	+		
Silt 0.05 - 0			7.5	1.1	4.6	1.5	1.5		0.005 mm	%	6.2	19.2	20.0			<u> </u>	
Clay < 0.00:			7.0	2.0	9.1	3.0	2.0	Clay < 0.0		%	12.1	8.1	8.1				
Passing #40 S			-	-	_	-	-	Passing #40		%	-	-	-				
Passing #200	Sieve %	_	-	-	_	_		Passing #20	0 Sieve	%	-	_	-				
		1 1	T	T -^		10									<del></del>		
L. L.		19	26	20	22	19	23	L. L.			16	62	46	<del> </del>		-	
P. I. AASHTO Clas	asification	NP	NP A-2-4(0)	NP A-1-b(0)	4 A-1-b(0)	NP A-3(0)	NP A-3(0)	P. I. AASHTO C	loggifi og ti om		NP A-2-4(0)	NP A-2-5(0)	NP				
Station	SSITICATION	A-2-4(0) 13+90	13+90	13+90	13+90	13+50	A-3(0) 13+50	Station	iassinçation		12+00	11+00	A-2-5(0) 11+00		<del> </del>	-	
OFFSET	· · · · · · · · · · · · · · · · · · ·	7 RT	7 RT	7 RT	7 RT	5 RT	5 RT	OFFSET			5 RT	7 RT	7 RT		<del> </del>		
ALIGNMENT	٦	L	L	L	L	L	L	ALIGNMEN	ĪT		L	L	L	<del> </del>			
Depth (Ft)		4.10	6.00	9.10	14.10	4.10	9.10	Depth (Ft)	i <del>T</del>	· · · · · · · · · · · · · · · · · · ·	4.10	7.00	9.20				
	to	5.60	9.00	10.60	15.60	5.60	10.60			to	5.60	9.00	10.70				
%ORGANIC			5.2					%ORGANIC				19.7	27.7				
cc: C C MURI Soils File																	
						Soils Engir	neer								Soils Engi	ineer	