BEGIN PROJECT END PROJECT VICINITY MAP DETOUR ROUTE NEAREST SHIPPING POINT: WAGRAM ON LAURINBURG SOUTHERN RR APPROX. 1.5 MILES FROM PROJECT

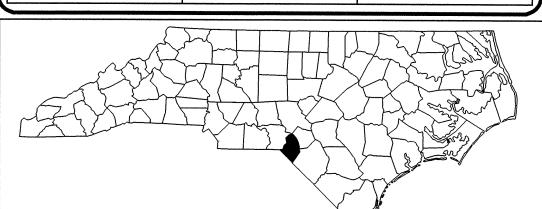
STRUCTURE

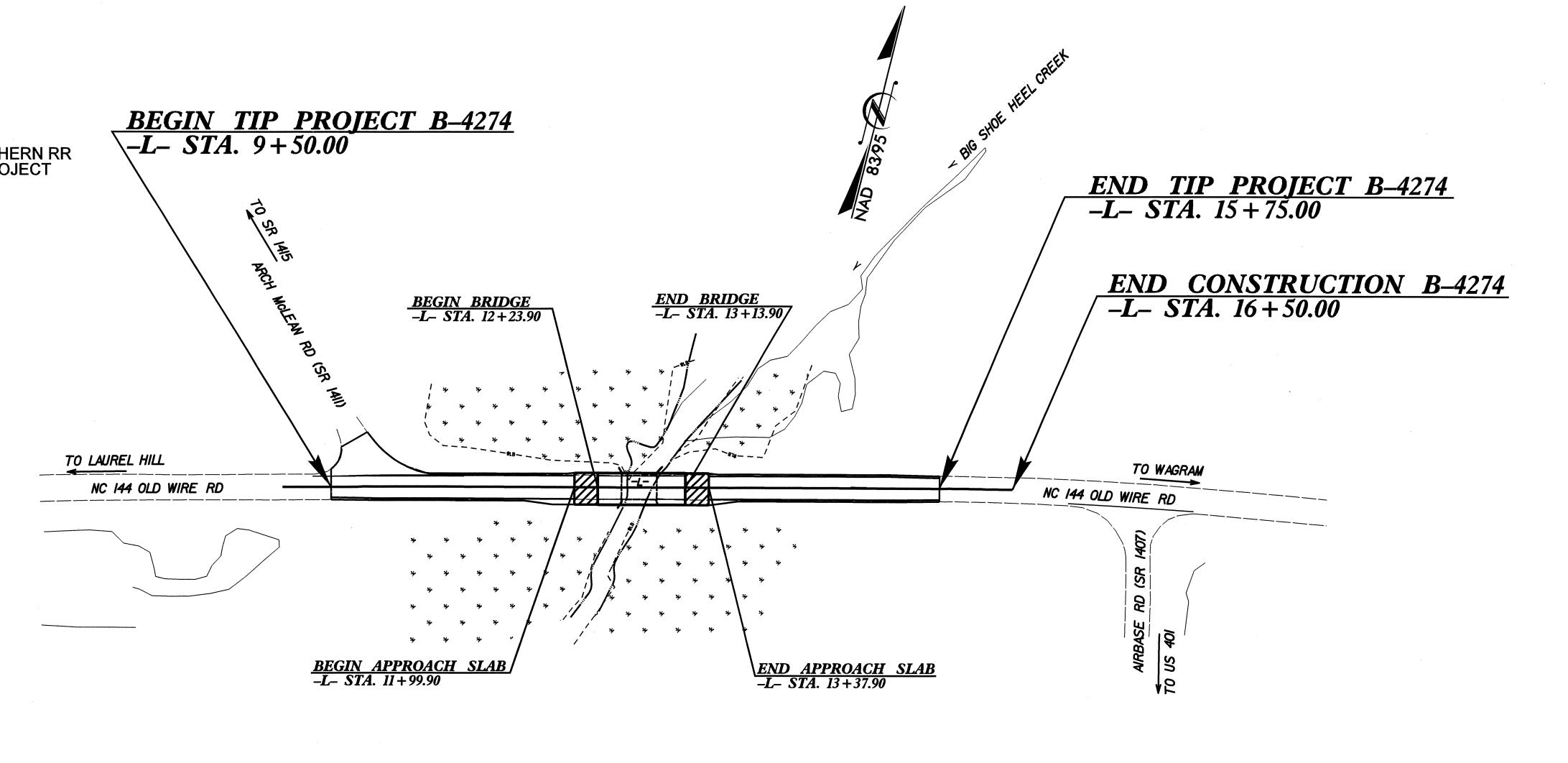
STATE OF NORTH CAROLINA DIVISION OF HIGHWAYS

SCOTLAND COUNTY

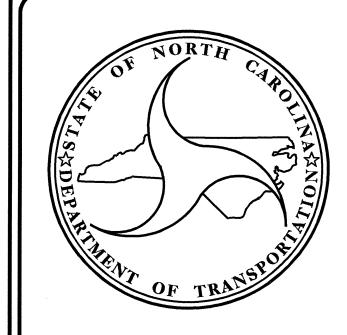
LOCATION: BRIDGE NO. 14 OVER BIG SHOE HEEL CREEK ON NC 144 TYPE OF WORK: GRADING, PAVING, DRAINAGE & STRUCTURE

STATE	STATE PROJECT REFERENCE NO.	SHEET TOTAL NO. SHEETS
N.C.	B-4274	
STATE PROJ.NO	o. F. A. PROJ. NO.	DESCRIPTION
33615.1.1	BRSTP-1405 (5)	PE
33615.2.1	BRSTP-1405 (5)	R/W & UTIL
33615.3.1	BRSTP-0144 (1)	CONST.





** DESIGN EXCEPTION REQUIRED FOR BRIDGE WIDTH.



DESIGN DATA

ADT 2008 = 3676

ADT 2028 = 5926

DHV =

60 MPH * TTST 2% DUAL 3%

FUNC. = RURAL MAJOR CLASS COLLECTOR

PROJECT LENGTH

LENGTH OF ROADWAY TIP PROJECT B-4274 = 0.101 MI.

LENGTH OF STRUCTURE TIP PROJECT B-4274 = 0.017 MI.

TOTAL LENGTH OF TIP PROJECT B-4274 = 0.118 MI.

Prepared in the Office of:

DIVISION OF HIGHWAYS 1000 BIRCH RIDGE DR. RALEIGH, NC 27610

2006 STANDARD SPECIFICATIONS

LETTING DATE: **AUGUST 19, 2008**

N. N. BULLOCK, PE PROJECT ENGINEER

D. R. CALHOUN, PE PROJECT DESIGN ENGINEER

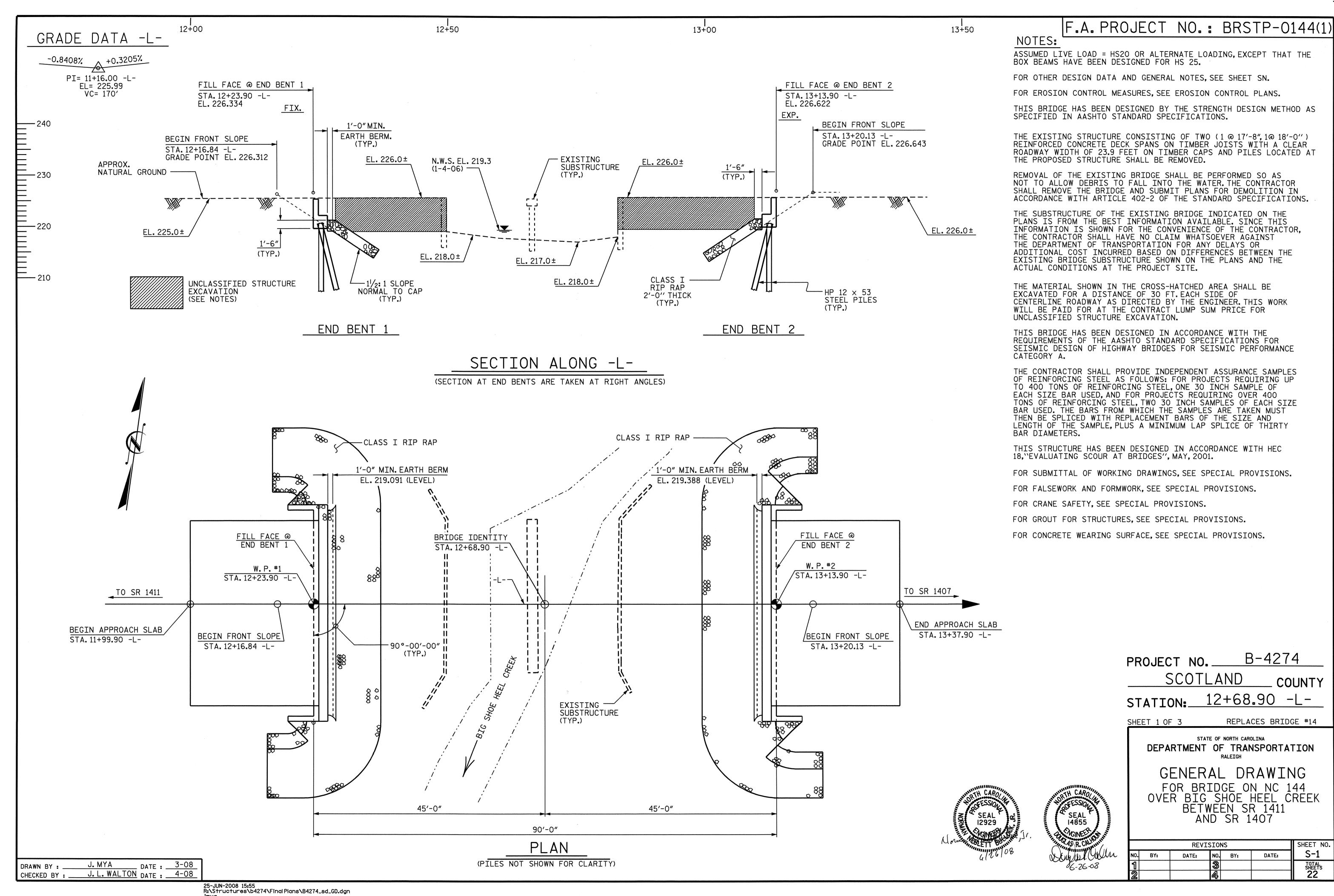
STRUCTURE DESIGN UNIT

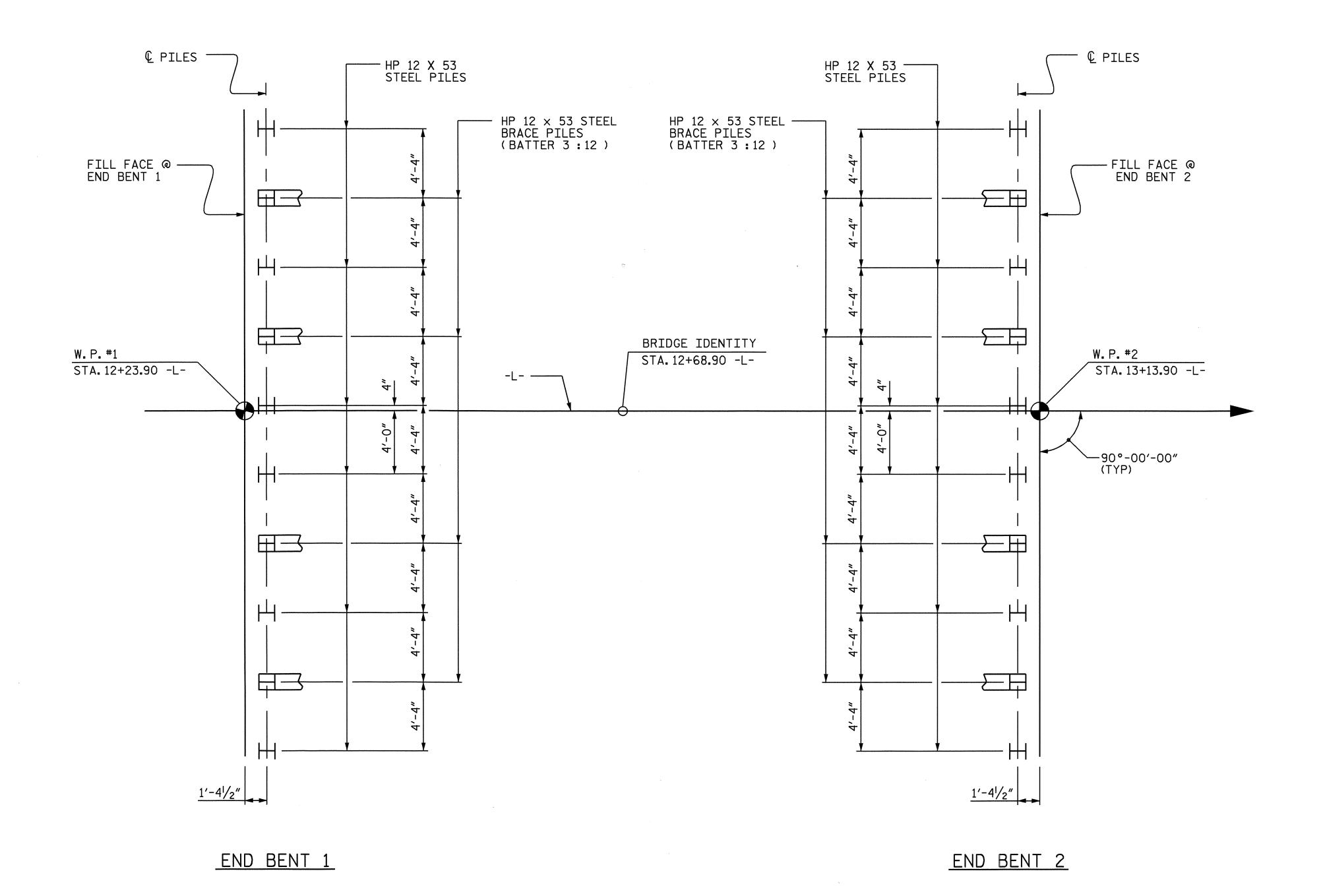
DIVISION OF HIGHWAYS STATE OF NORTH CAROLINA

STATE DESIGN ENGINEER

DEPARTMENT OF TRANSPORTATION FEDERAL HIGHWAY ADMINISTRATION

APPROVED
DIVISION ADMINISTRATOR





FOUNDATION LAYOUT

(DIMENSIONS LOCATING END BENT PILES ARE SHOWN TO CENTERLINE OF PILES)

FOUNDATION NOTES:

THE ALLOWABLE BEARING CAPACITY FOR PILES AT END BENT 1 AND 2 IS 60 TONS PER PILE.

DRIVE PILES AT END BENT 1 AND 2 TO A REQUIRED BEARING CAPACITY OF 120 TONS PER PILE. THE REQUIRED BEARING CAPACITY IS EQUAL TO THE ALLOWABLE BEARING CAPACITY WITH A MINIMUM FACTOR OF SAFETY OF TWO.

TESTING PILES WITH THE PILE DRIVING ANALYZER (PDA) DURING DRIVING, RE-STRIKING OR RE-DRIVING MAY BE REQUIRED. THE ENGINEER WILL DETERMINE THE NEED FOR PDA TESTING. SEE PILE DRIVING ANALYZER SPECIAL PROVISION.



PROJECT NO. B-4274

SCOTLAND COUNTY

STATION: 12+68.90 -L-

SHEET 2 OF 3

STATE OF NORTH CAROLINA

DEPARTMENT OF TRANSPORTATION

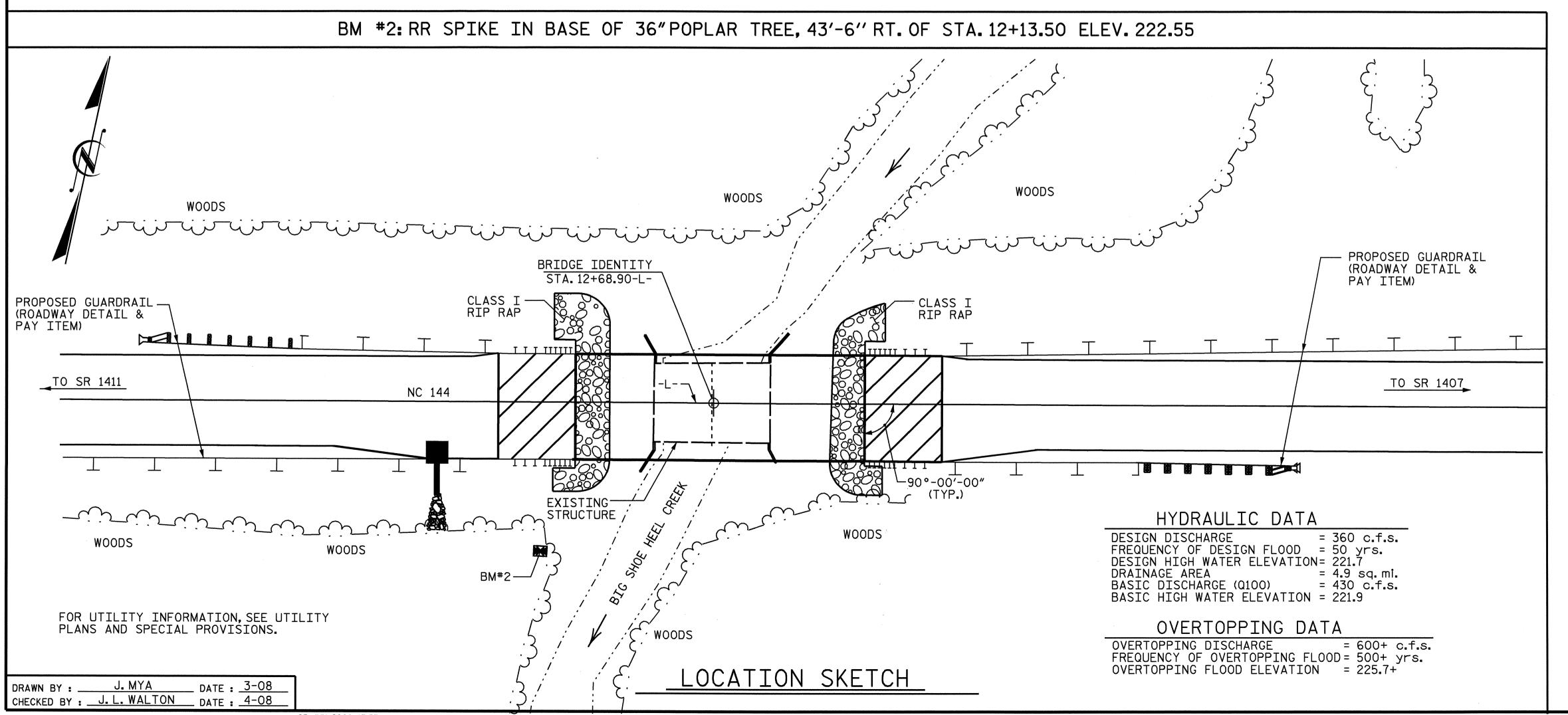
RALEIGH

GENERAL DRAWING
FOR BRIDGE ON NC 144
OVER BIG SHOE HEEL CREEK
BETWEEN SR 1411
AND SR 1407

		SHEET NO.				
NO.	BY:	DATE:	NO.	BY:	DATE:	S-2
1			3			TOTAL SHEETS
2			4			22

DRAWN BY: J. MYA DATE: 3-08
CHECKED BY: J. L. WALTON DATE: 4-08

						То	TAL BIL	OF MA	TERIAL —										The state of the s
	REMOVAL OF EXISTING STRUCTURE	PDA TESTING	PDA ASSISTANCE	UNCLASSIFIED STRUCTURE EXCAVATION	CONCRETE WEARING SURFACE	GROOVING BRIDGE FLOORS	CLASS A CONCRETE	BRIDGE APPROACH SLABS	REINFORCING STEEL	HP STE	12 X 53 EL PILES	TWO BAR METAL RAIL	1'-2'' X 2'-11 ¹ / ₂ '' CONCRETE PARAPET	RIP RAP CLASS I (2'-0" THICK)	FILTER FABRIC FOR DRAINAGE	ELASTOMERIC BEARINGS	EVAZOTE JOINT SEALS	3'-0' PRES CON BOX	"X 3'-3" TRESSED NCRETE BEAMS
	LUMP SUM	EA.	EA.	LUMP SUM	SQ. FEET	SQ. FEET	CU. YDS.	LUMP SUM	LBS.	NO.	LIN.FT.	LIN.FT.	LIN.FT.	TONS	SQ. YDS.	LUMP SUM	LUMP SUM		LIN. FT.
SUPERSTRUCTURE					2908	4084		LUMP SUM				160.50	175.50			LUMP SUM		12	1053.0
END BENT 1				LUMP SUM			19.2		2894	10	550	—		89	99				
END BENT 2				LUMP SUM			19.2		2894	10	750			67	74				
							÷												
TOTAL	LUMP SUM	2	2	LUMP SUM	2908	4084	38.4	LUMP SUM	5788	20	1300	160.50	175.50	156	173	LUMP SUM	LUMP SUM	12	1053.0



PROJECT NO. B-4274

SCOTLAND COUNTY

STATION: 12+68.90 -L-

SHEET 3 OF 3

DEPARTMENT OF TRANSPORTATION
RALEIGH

GENERAL DRAWING
FOR BRIDGE ON SR NC 144 OVER
BIG SHOE HEEL CREEKCREEK
BETWEEN SR 1411 AND SR 1407

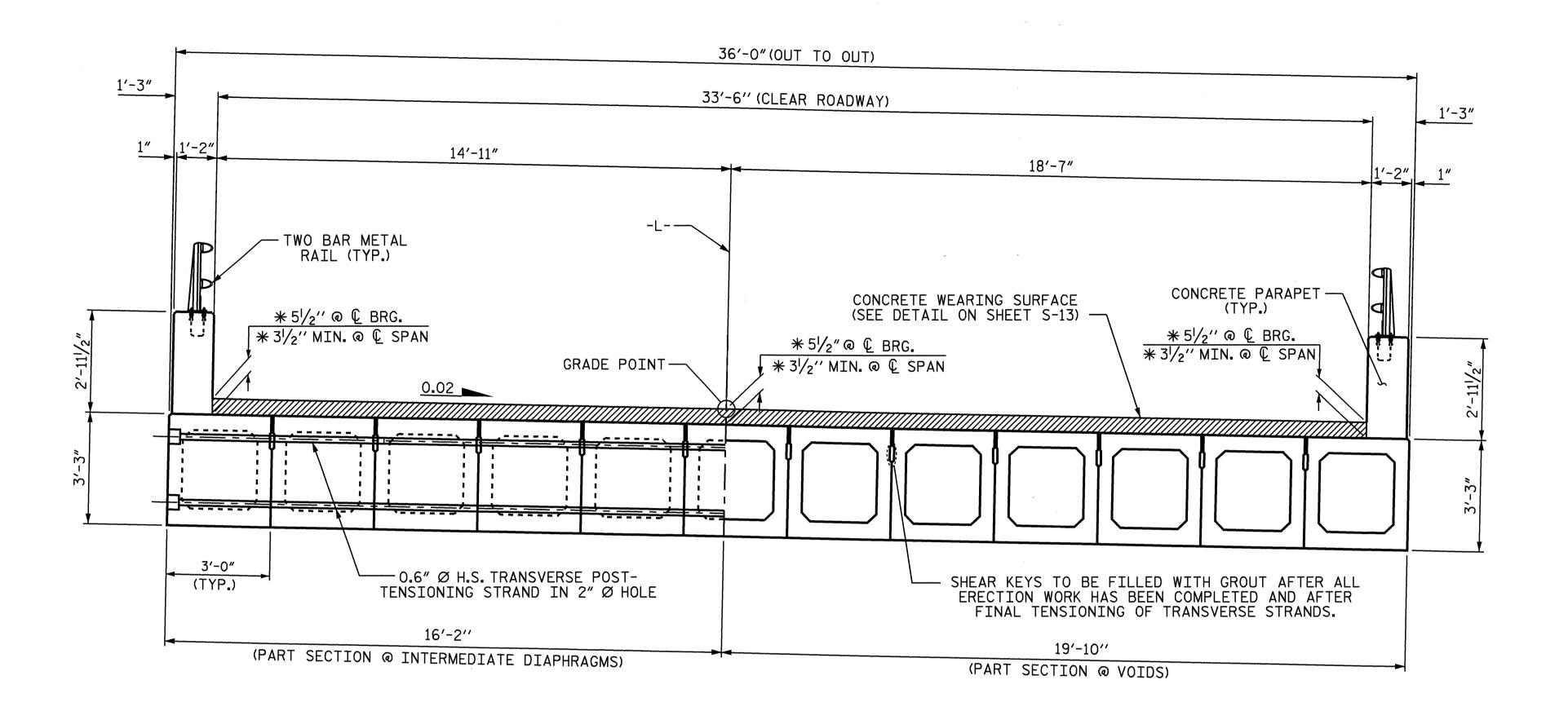
REVISIONS

SHEET NO.

S-3

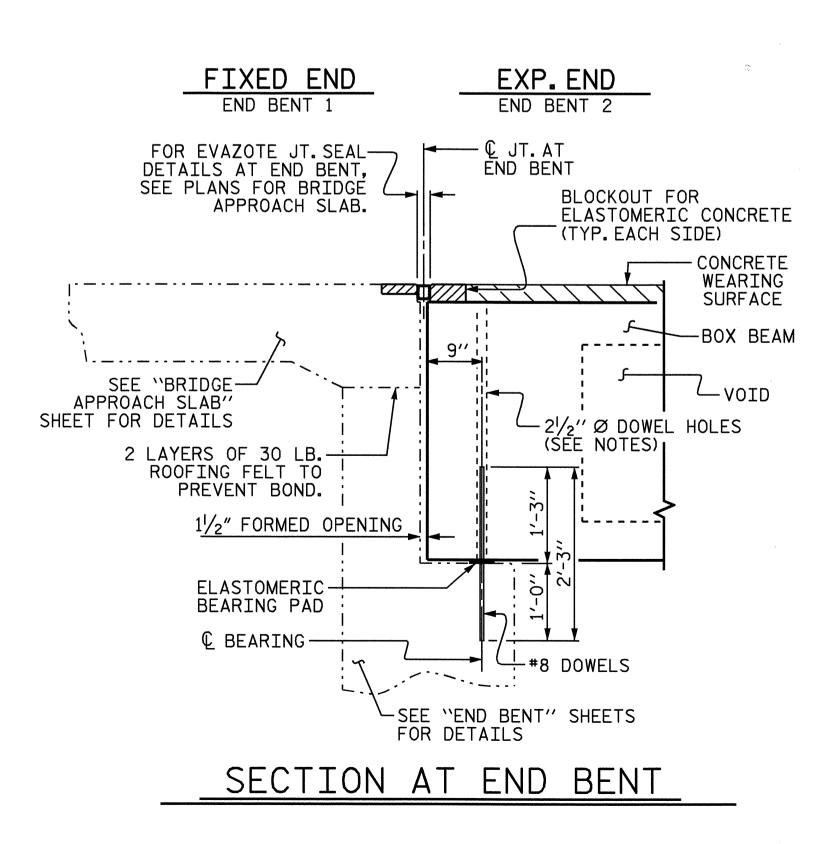
TOTAL
SHEETS
22

25-JUN-2008 15:55
R:\Structures\b4274\FinalPlans\B4274_sd_GD.dgn



TYPICAL SECTION

* BASED ON PREDICTED CAMBER AND THEORETICAL GRADE LINE ELEVATION



25-JUN-2008 15:55
R:\Structures\b4274\FinalPlans\B4204_sd_TS.dgn

NOTES

ALL PRESTRESSING STRANDS SHALL BE 7-WIRE LOW RELAXATION GRADE 270 STRANDS AND SHALL CONFORM TO AASHTO M203 EXCEPT FOR SAMPLING REQUIREMENTS WHICH SHALL BE IN ACCORDANCE WITH THE STANDARD SPECIFICATIONS.

ALL REINFORCING STEEL CAST WITH THE BOX BEAM SECTIONS SHALL BE GRADE 60 AND SHALL BE INCLUDED IN THE UNIT PRICE BID FOR PRESTRESSED CONCRETE BOX BEAMS.

RECESSES FOR TRANSVERSE STRANDS SHALL BE GROUTED AFTER THE TENSIONING OF THE STRANDS.

THE $2^{1}/_{2}$ " Ø DOWEL HOLES AT FIXED ENDS OF BOX BEAM SECTIONS SHALL BE FILLED WITH NON-SHRINK GROUT. THE $2^{1}/_{2}$ " Ø DOWEL HOLES AT EXPANSION ENDS OF BOX BEAM SECTIONS SHALL BE FILLED WITH JOINT SEALER MATERIAL TO $1^{1}/_{2}$ " ABOVE THE TOP OF DOWELS AND THEN FILLED WITH GROUT.

THE JOINT SEALER MATERIAL SHALL CONFORM TO THE REQUIREMENTS OF TYPE SL LOW MODULUS SILICONE SEALANT. SEE SECTION 1028 OF THE STANDARD SPECIFICATIONS.

THE TRANSFER OF LOAD FROM THE ANCHORAGES TO THE BOX BEAM UNIT SHALL BE DONE WHEN THE CONCRETE HAS REACHED A COMPRESSIVE STRENGTH OF NOT LESS THAN 4800 PSI.

PRESTRESSING STRANDS SHALL BE CUT FLUSH WITH THE BOX BEAM UNIT ENDS.

APPLY EPOXY PROTECTIVE COATING TO BOX BEAM UNIT ENDS.

PLACEMENT OF THE CONCRETE WEARING SURFACE SHALL OCCUR AFTER CASTING THE CONCRETE PARAPET. THE COST OF THE REINFORCING STEEL CAST WITH THE CONCRETE WEARING SURFACE SHALL BE INCLUDED IN THE UNIT PRICE BID FOR CONCRETE WEARING SURFACE. FOR CONCRETE WEARING SURFACE, SEE SPECIAL PROVISIONS.

THE LOCATION OF THE VOID DRAINS MAY BE SHIFTED SLIGHTLY WHERE NECESSARY TO CLEAR PRESTRESSING STRANDS OR TRANSVERSE REINFORCING

FOR PRESTRESSED CONCRETE MEMBERS, SEE SPECIAL PROVISIONS.

PROJECT NO. B-4274

SCOTLAND COUNTY

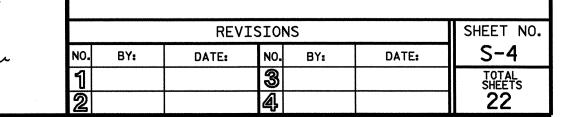
STATION: 12+68.90 -L-

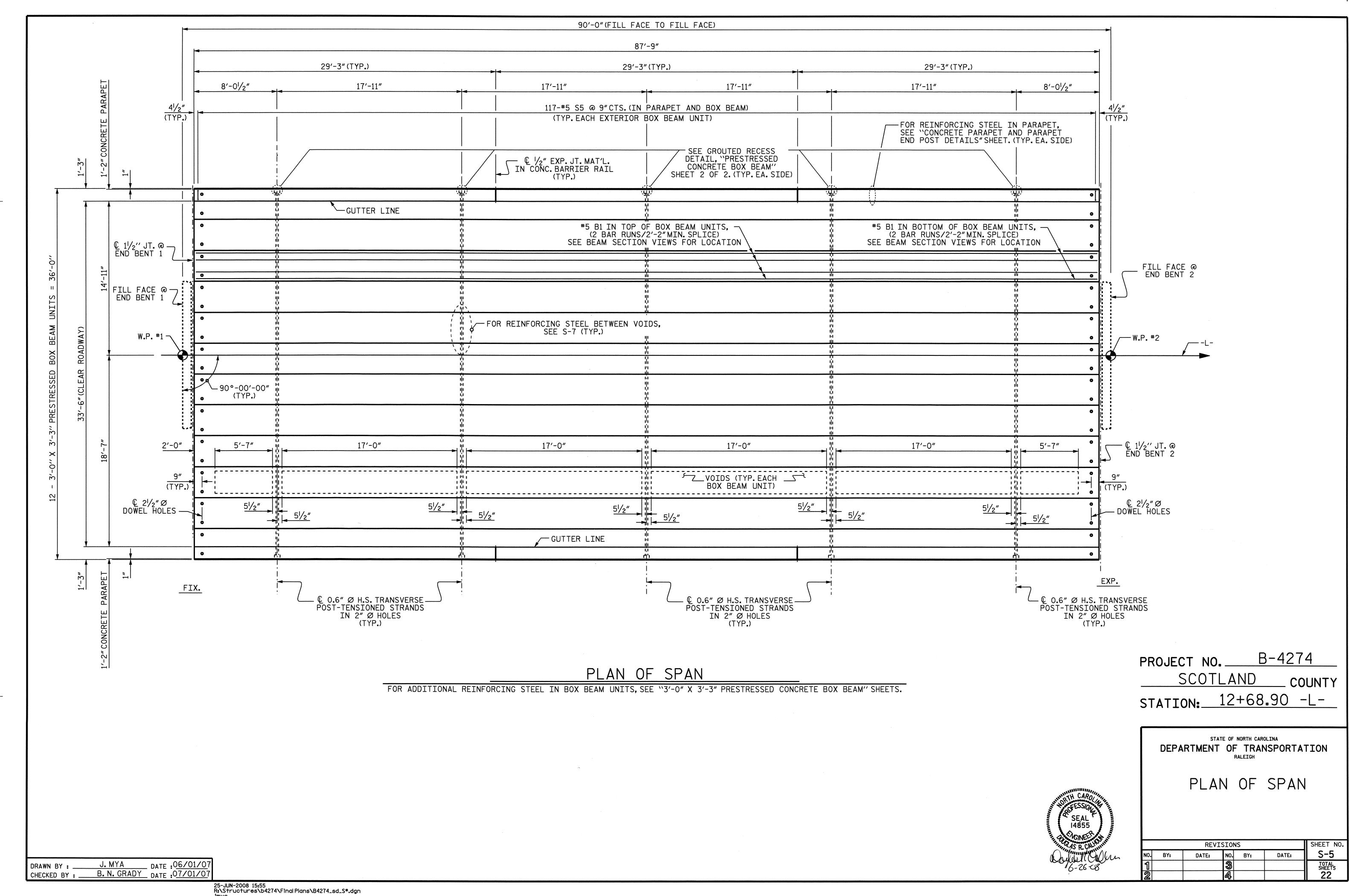
STATE OF NORTH CAROLINA

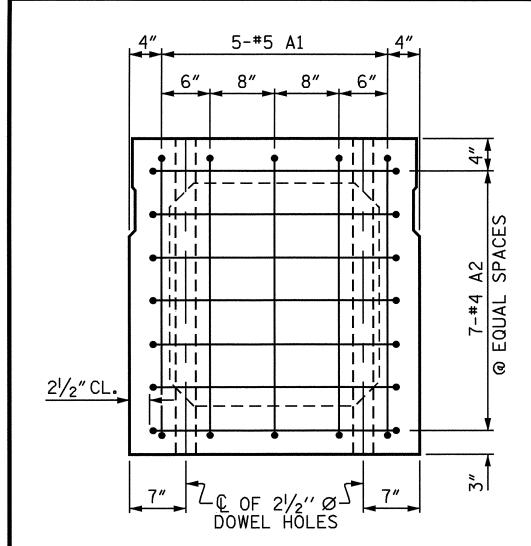
DEPARTMENT OF TRANSPORTATION

RALEIGH

3'-0" x 3'-3" PRESTRESSED CONCRETE BOX BEAM UNIT

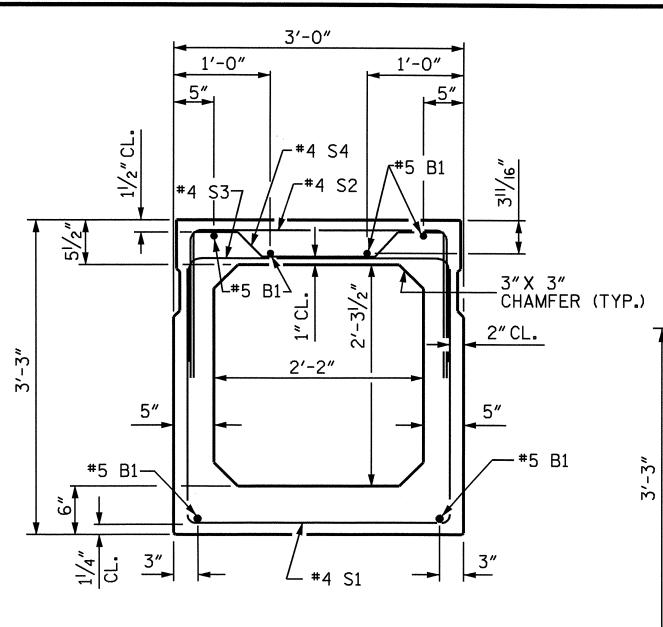




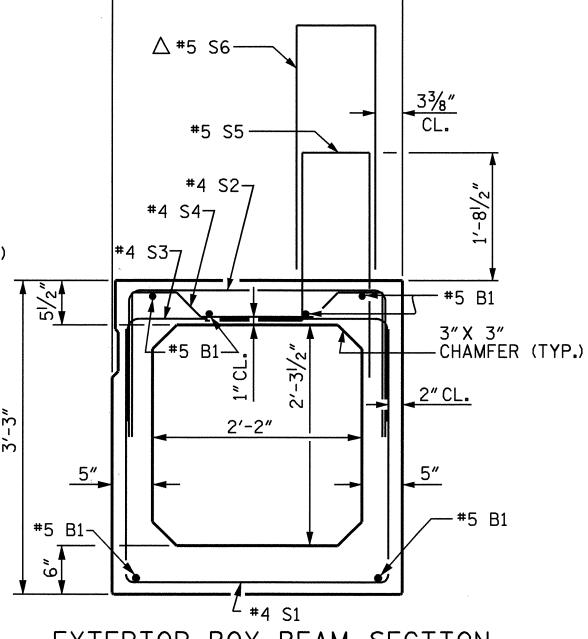


END ELEVATION

SHOWING PLACEMENT OF #5 & #4 "A" BARS
AND LOCATION OF DOWEL HOLES.
(INTERIOR BOX BEAM SECTION SHOWN-EXTERIOR
SECTION SIMILAR EXCEPT SHEAR KEY LOCATION.
STRAND LAYOUT NOT SHOWN.)



INTERIOR BOX BEAM SECTION (STRAND LAYOUT NOT SHOWN)

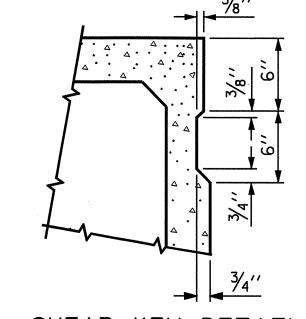


3'-0"

EXTERIOR BOX BEAM SECTION (STRAND LAYOUT NOT SHOWN)

(STRAND LAYOUT NOT S

△ FOR S6 BARS IN PARAPET, SEE
SHEET S-8

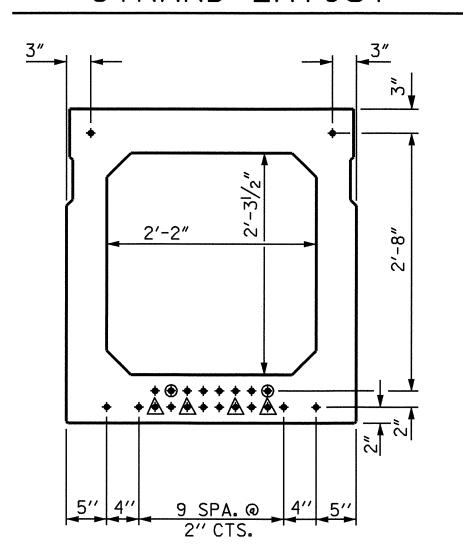


SHEAR KEY DETAIL

NOTE: OMIT SHEAR KEY ON OUTSIDE FACE
OF EXTERIOR BOX BEAMS.

GRADE 270 STRANDS O.6" Ø L.R. AREA (SQUARE INCHES) ULTIMATE STRENGTH (LBS. PER STRAND) APPLIED PRESTRESS (LBS. PER STRAND) 43,950

O.6" Ø LOW RELAXATION STRAND LAYOUT



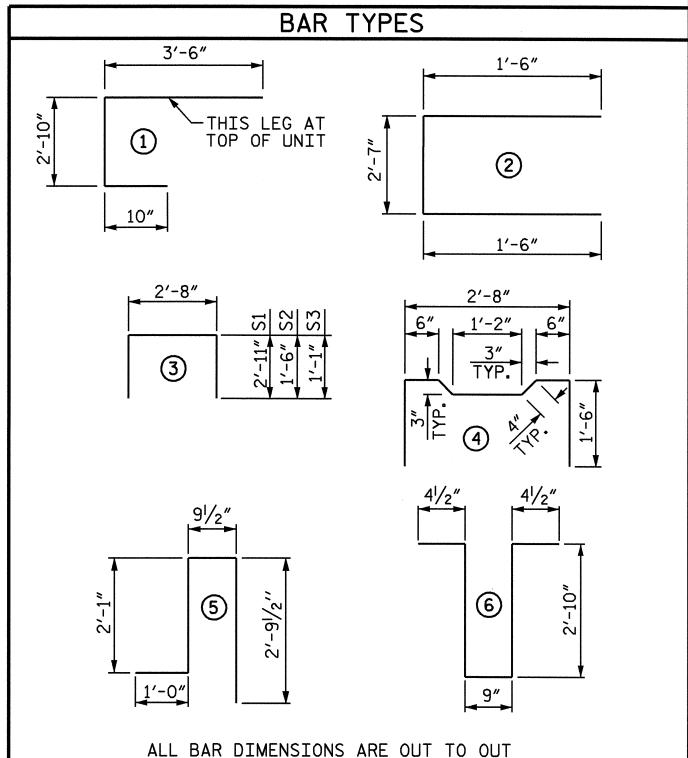
TYPICAL STRAND LOCATION

(22 STRANDS REQUIRED)

(INTERIOR BOX BEAM SECTION SHOWN-EXTERIOR SECTION SIMILAR EXCEPT SHEAR KEY LOCATION)

DEBONDING LEGEND

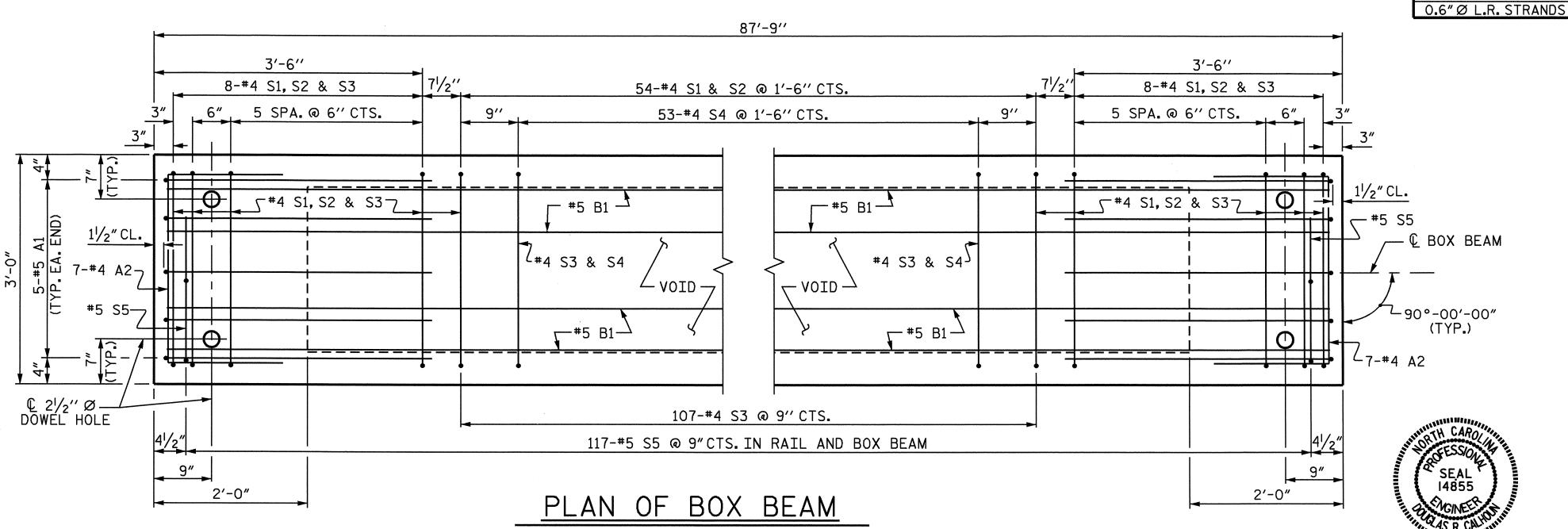
- FULLY BONDED STRANDS
- STRANDS DEBONDED FOR 4'-0"
 FROM END OF GIRDER
- STRANDS DEBONDED FOR 6'-0"
 FROM END OF GIRDER



BILL OF	MATERIAL	FOR ONE	BOX BEAM	SECTION

				EXTERI	OR UNIT	INTERI	OR UNIT
BAR	NUMBER	SIZE	TYPE	LENGTH	WEIGHT	LENGTH	WEIGHT
A1	10	#5	1	7′-2″	75	7′-2″	75
A2	44	#4	2	5′-7″	164	5′-7″	164
B1	12	#5	STR	44'-10"	561	44'-10"	561
K1	15	#4	6	7′-2″	72	7′-2″	72
K2	10	#4	STR	2′-7″	17	2′-7″	17
S1	70	#4	3	8′-6″	397	8′-6″	397
S2	70	#4	3	5′-8″	265	5′-8″	265
S3	123	#4	3	4'-10"	397	4'-10"	397
S4	53	#4	4	5′-10″	207	5′-10″	207
* S5	117	#5	5	6′-8″	814		
REINF	ORCING S	STEEL			2155 LBS.		2155 LBS.

No. 22



ASSEMBLED BY: J. MYA
CHECKED BY: B. N. GRADY

DATE: 6/01/07
DATE: 7/01/07

DRAWN BY: TLA 5/05
CHECKED BY: GM 6/05

ADDED 7/11/05
REV. 5/1/06

TLA/GM

EXTERIOR UNIT SHOWN, INTERIOR UNIT SIMILAR EXCEPT OMIT #5 S5 BARS.
FOR LOCATION OF DIAPHRAGMS, SEE PLAN OF SPANS.
FOR REINFORCING STEEL IN DIAPHRAGMS, SEE DIAPHRAGM DETAILS. SHEET 2 OF 2.

PROJECT NO. B-4274

SCOTLAND COUNTY

STATION: 12+68.90 -L-

814 LBS.

17.2 CU. YDS.

No. 22

17.4 CU. YDS.

SHEET 1 OF 2

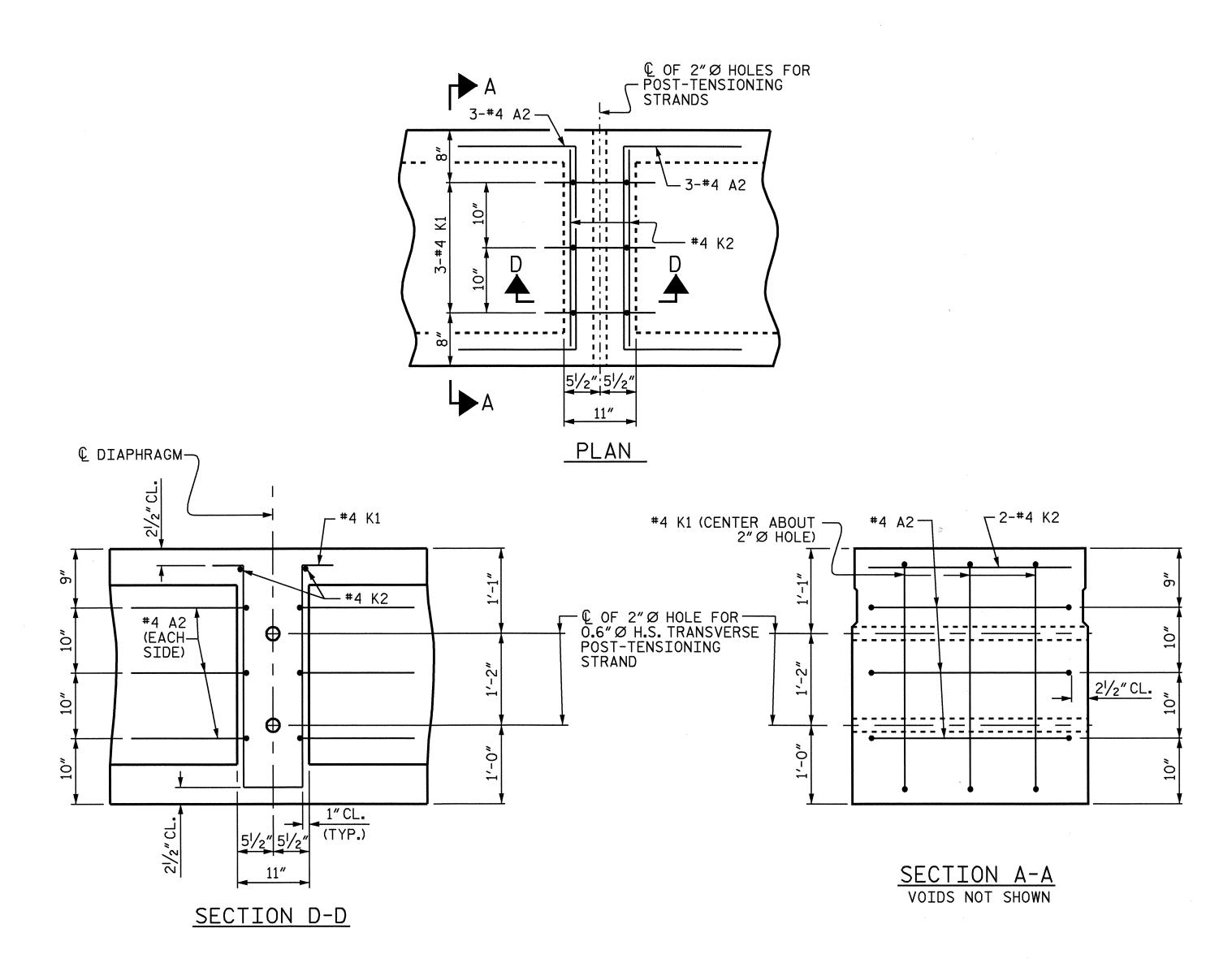
* EPOXY COATED REINF. STEEL

6000 P.S.I. CONCRETE

DEPARTMENT OF TRANSPORTATION
RALEIGH
STANDARD

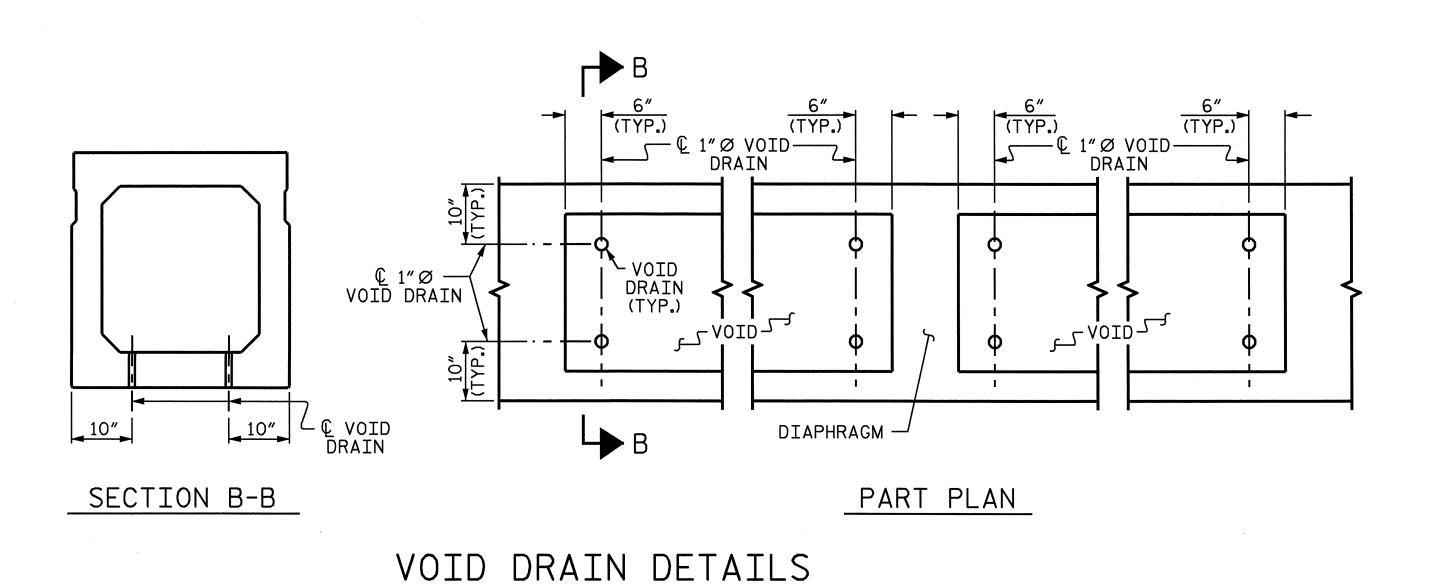
3'-0" X 3'-3"
PRESTRESSED CONCRETE
BOX BEAM UNIT

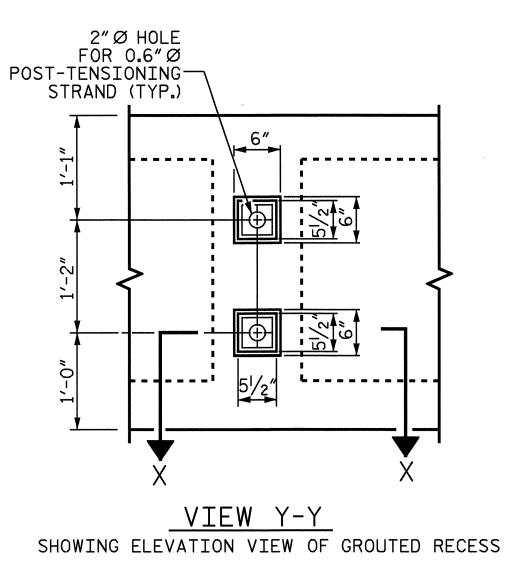
		SHEET NO.				
0.	BY:	DATE:	NO.	BY:	DATE:	S-6
1			3			TOTAL SHEETS
2			4			22

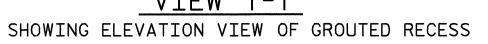


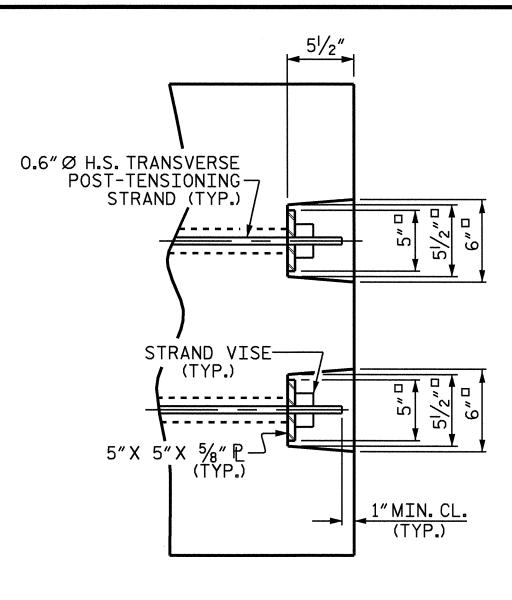
DOUBLE DIAPHRAGM DETAILS

#4 "S" BARS NOT SHOWN. #4 "S" BARS MAY BE SHIFTED SLIGHTLY TO CLEAR 2" Ø HOLE.

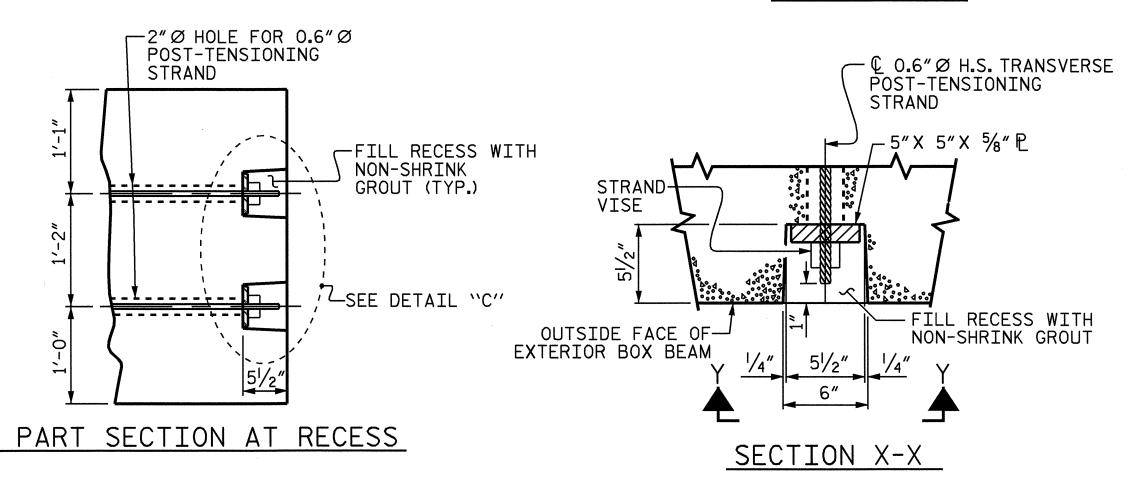








DETAIL "C"



GROUTED RECESS DETAIL AT END OF POST-TENSIONED STRANDS EXTERIOR BOX BEAM

DEAD L	OAD DEFLE	CTION	AND	CAMBER
			3′-0	″× 3′−3″
			0.6"Ø	L.R. STRAND
			(SPAN A
CAMBER (E	BEAM ALONE IN	PLACE)		2 ¹³ / ₁₆ "
DEFLECTION CONCRETE V	I DUE TO VEARING SURFAC	E ¥		3/4"
FINAL CAM	BER	f		21/16"

PROJECT NO. B-4274SCOTLAND COUNTY STATION: 12+68.90 -L-

SHEET 2 OF 2

STATE OF NORTH CAROLINA DEPARTMENT OF TRANSPORTATION STANDARD 3'-0" X 3'-3" PRESTRESSED CONCRETE BOX BEAM UNIT

				·
		SHEET NO.		
).	BY:	S-7		
		3		TOTAL SHEETS
2		4		22

25-JUN-2008 15:55 R:\Structures\b4274\FinalPlans\B4274_sd_G#.dgn

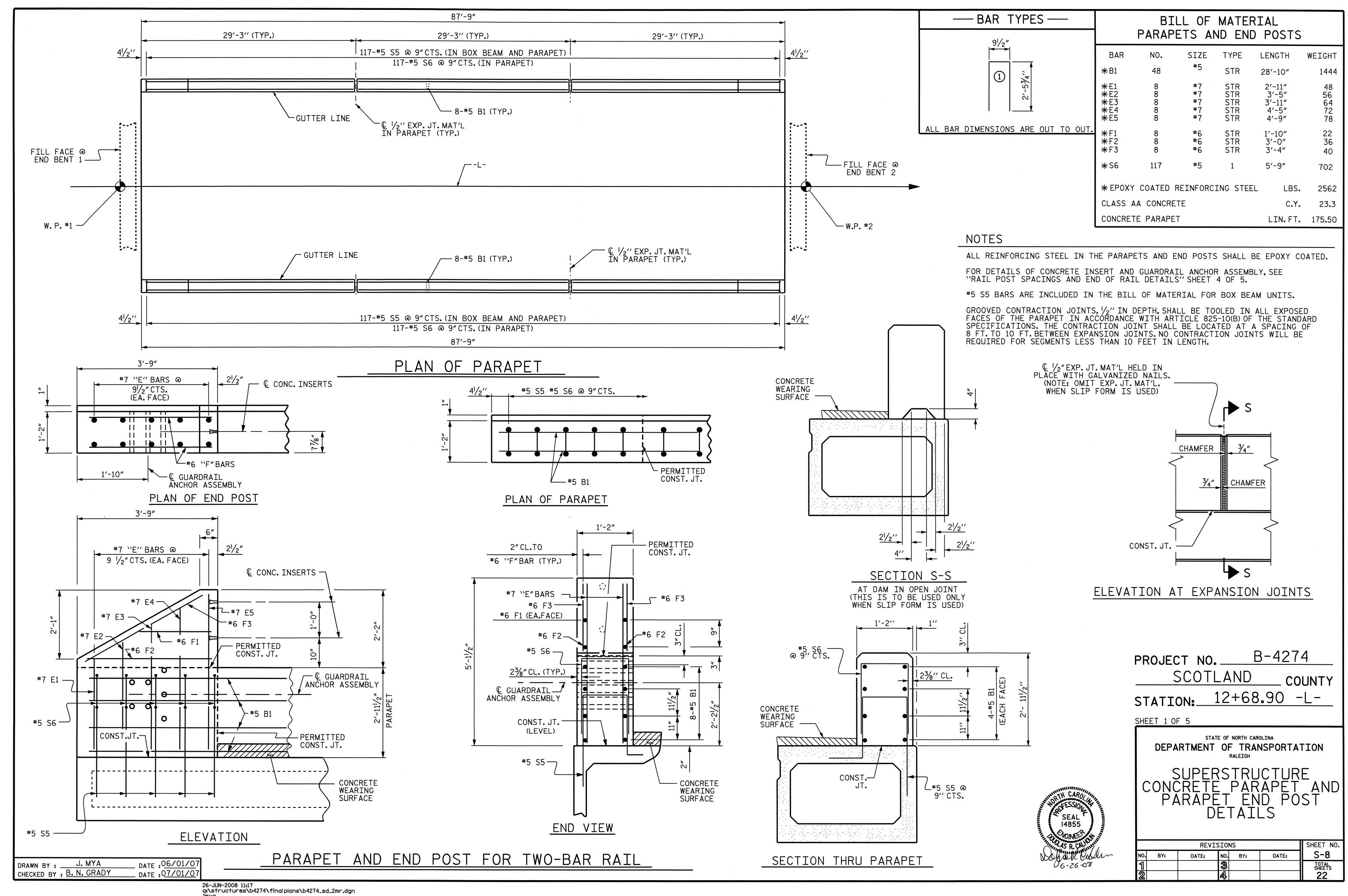
ASSEMBLED BY : J. MYA CHECKED BY : B. N. GRADY

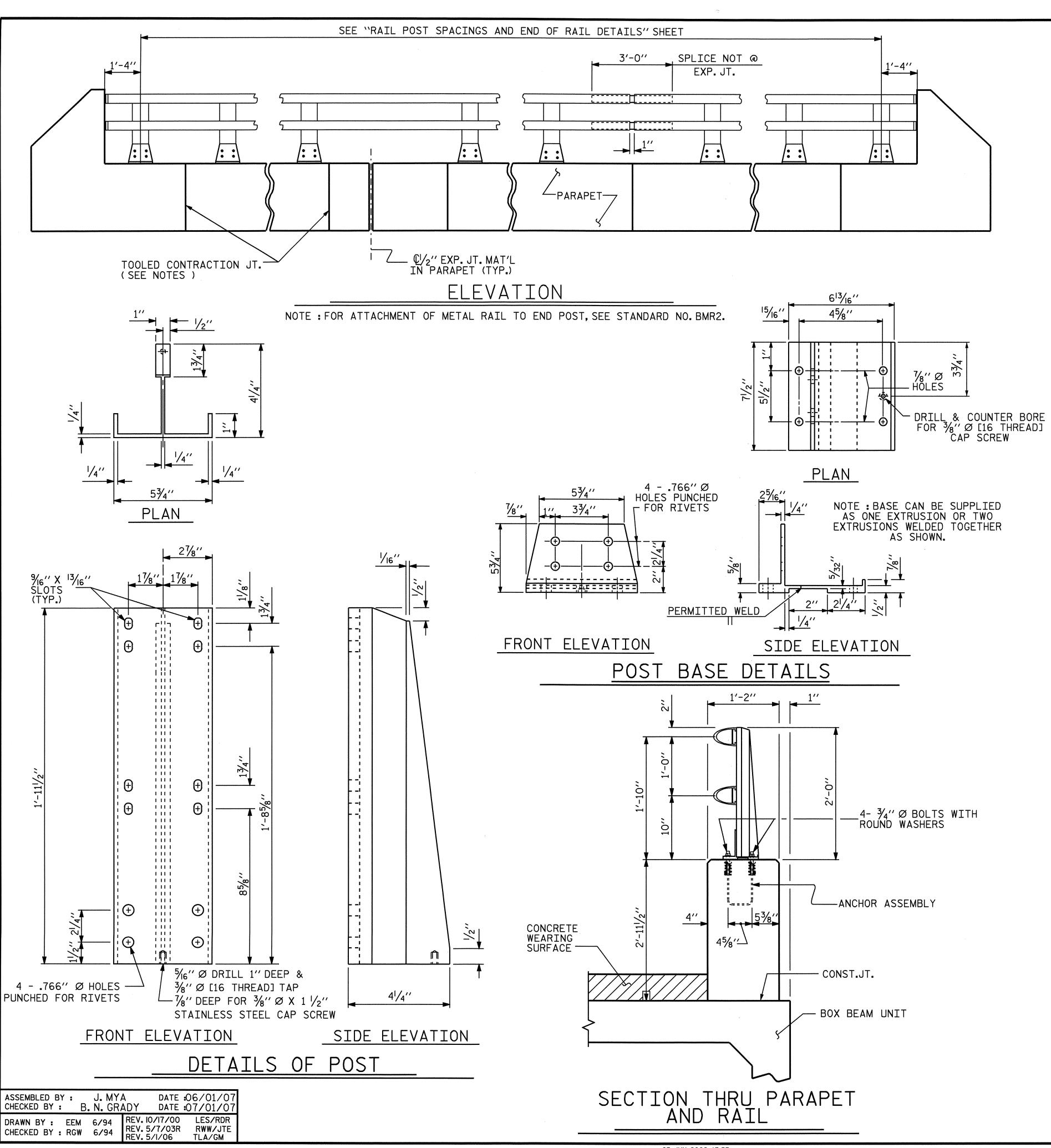
DRAWN BY: TLA 5/05 CHECKED BY: GM 6/05

DATE : 6/07 DATE : 7/01/07

ADDED 7/II/05 REV.5/I/06 TLA/GM

(DIMENSIONS SHOWN ARE TYPICAL FOR EACH VOID)





NOTES

AT THE CONTRACTOR'S OPTION, METAL RAIL MAY BE EITHER ALUMINUM OR GALVANIZED STEEL IN ACCORDANCE WITH THE REQUIREMENTS OF THE GENERAL NOTES AND THE FOLLOWING SPECIFICATIONS FOR THE ALTERNATE MATERIALS; HOWEVER, THE CONTRACTOR WILL BE REQUIRED TO USE THE SAME RAIL MATERIAL ON ALL STRUCTURES ON THE PROJECT FOR WHICH METAL RAIL IS DESIGNATED.

ALUMINUM RAILS

MATERIAL FOR POSTS, BASES AND RAILS, EXPANSION BARS AND CLAMP BARS SHALL BE ASTM B-221 ALLOY 6061-T6. MATERIAL FOR RIVETS SHALL BE ASTM B316 ALLOY 6061-T6. RIVETS SHALL BE STANDARD BUTTON HEAD AND CONE POINT COLD DRIVEN AS PER DRAWING.

THE BASE OF RAIL POSTS, OR ANY OTHER ALUMINUM SURFACE IN CONTACT WITH CONCRETE SHALL BE THOROUGHLY COATED WITH AN ALUMINUM IMPREGNATED CAULKING COMPOUND OF APPROVED QUALITY.

MATERIAL FOR SHIMS TO BE ASTM B209 ALLOY 6061-T6.

GALVANIZED STEEL RAILS

MATERIAL AND GALVANIZING ARE TO CONFORM TO THE FOLLOWING SPECIFICATIONS:

POST, POST BASES, RAILS, EXPANSION BARS AND CLAMP BARS: AASHTO M270 GRADE 36 STRUCTURAL STEEL - GALVANIZED TO AASHTO M111.

RIVETS: RIVETS SHALL MEET THE REQUIREMENTS OF ASTM A502 FOR GRADE 1 RIVETS.

THE CUT ENDS OF GALVANIZED STEEL RAILING, AFTER GRINDING SMOOTH SHALL BE GIVEN TWO COATS OF ZINC RICH PAINT MEETING THE REQUIREMENTS OF FEDERAL SPECIFICATION MIL-P-26915 USAF TYPE 1, OR OF FEDERAL SPECIFICATIONS TT-P-641.

SHIMS: SHIMS SHALL MEET THE REQUIREMENTS OF ASTM A570 FOR GRADE 33 OR A611 FOR GRADE C AND SHALL BE GALVANIZED IN ACCORDANCE WITH AASHTO M111.

RAIL CAPS: RAIL CAPS SHALL MEET THE REQUIREMENTS OF ASTM A570 FOR GRADE 33 OR A611 FOR GRADE C AND SHALL BE GALVANIZED IN ACCORDANCE WITH AASHTO M111.

GENERAL NOTES

RAILING SHALL BE CONTINUOUS FROM END POST TO END POST OF BRIDGE. EACH JOINT IN RAIL LENGTH SHALL BE SPLICED AS DETAILED. PANEL LENGTHS OF RAIL SHALL BE ATTACHED TO A MINIMUM OF THREE POSTS.

FOR END OF RAIL TO CLEAR FACE OF CONCRETE END POST DIMENSION, SEE STANDARD NO. BMR2.

CAP SCREWS SHALL BE ASTM F593 ALLOY 305 STAINLESS STEEL. WASHERS SHALL MEET THE REQUIREMENTS OF ASTM F844 EXCEPT THEY SHALL BE MADE FROM ALLOY 304 STAINLESS STEEL.

CERTIFIED MILL REPORTS ARE REQUIRED FOR RAILS AND POSTS. SHOP INSPECTION IS NOT REQUIRED. METAL RAIL POSTS SHALL BE SET NORMAL TO CURB GRADE.

METHOD OF MEASUREMENT FOR METAL RAILS: FOR LENGTH OF METAL RAILS TO BE PAID FOR, SEE THE STANDARD SPECIFICATIONS.

CURVED RAIL USAGE: WHERE RAILS ARE TO BE USED ON BRIDGES ON HORIZONTAL AND/OR VERTICAL CURVATURE THE CONTRACTOR MAY, AT HIS OPTION, HAVE THE REQUIRED CURVATURE IN THE RAIL FORMED IN THE SHOP OR IN THE FIELD. IN EITHER EVENT, THE RAIL SHALL CONFORM WITHOUT BUCKLING OR KINKING TO THE REQUIRED CURVATURE IN A UNIFORM MANNER ACCEPTABLE TO THE ENGINEER.

TO INSURE FUTURE IDENTIFICATION OF THE FABRICATOR, A PERMANENT IDENTIFYING MARK SHALL BE PLACED ON EACH POST. THE METHOD OF MARKING AND LOCATION SHALL BE SUCH THAT IT DOES NOT DETRACT FROM THE APPEARANCE OF THE POST, BUT REMAINS VISIBLE AFTER RAIL PLACEMENT.

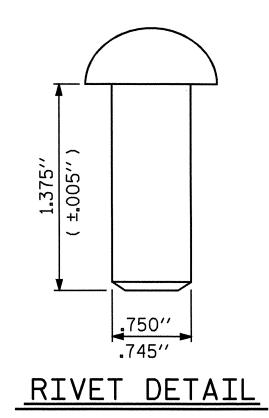
SHIMS SHALL BE USED AS NECESSARY FOR POST ALIGNMENT.

ALLOY 6351-T5 MAY BE SUBSTITUTED FOR ALLOY 6061-T6 WHERE APPLICABLE.

MINOR VARIATIONS IN DETAILS OF METAL RAIL WILL BE CONSIDERED. DETAILS OF SUCH VARIATIONS, IF DESIRED, SHALL BE SUBMITTED FOR APPROVAL.

GROOVED CONTRACTION JOINTS, $\frac{1}{2}$ " IN DEPTH, SHALL BE TOOLED IN ALL EXPOSED FACES OF THE PARAPET IN ACCORDANCE WITH ARTICLE 825-10(B) OF THE STANDARD SPECIFICATIONS. THE CONTRACTION JOINT SHALL BE LOCATED AT A SPACING OF 8FT. TO 10FT. BETWEEN EXPANSION JOINTS. NO CONTRACTION JOINTS WILL BE REQUIRED FOR SEGMENTS LESS THAN 10 FEET IN LENGTH.

PAY LENGTH = 160.50 LIN. FT.

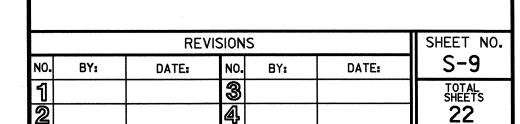


PROJECT NO. ____B-4274 ____SCOTLAND ___ county station: 12+68.90 -L-

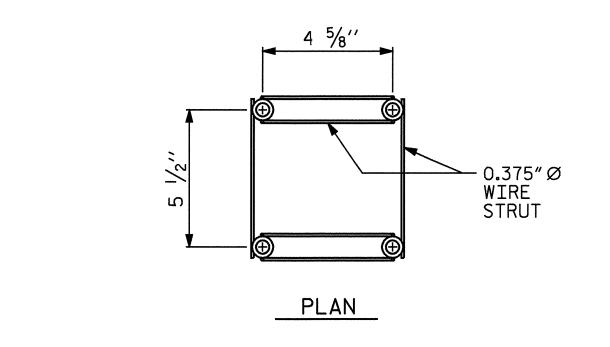
SHEET 2 OF 5

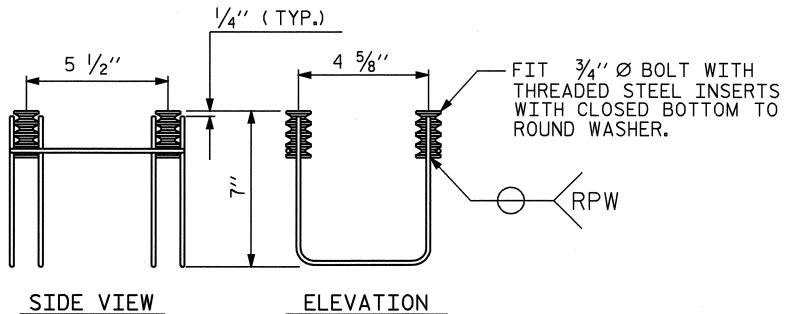
DEPARTMENT OF TRANSPORTATION
RALEIGH
STANDARD

2 BAR METAL RAIL



25-JUN-2008 15:55 R:\Structures\b4274\Final Plans\B4274_sd_2MR.dgn





MINIMUM LENGTH OF THREADS IN INSERT (FERRULE): $1\frac{3}{4}$ "

4-BOLT METAL RAIL ANCHOR ASSEMBL'

(30 ASSEMBLIES REQUIRED)

NOTES

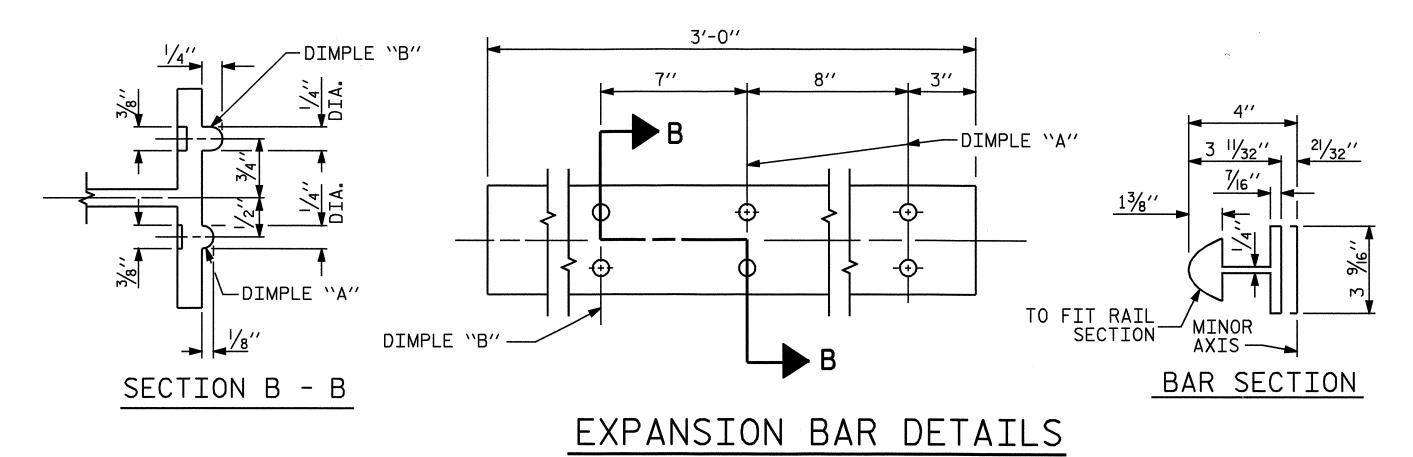
STRUCTURAL CONCRETE ANCHOR ASSEMBLY

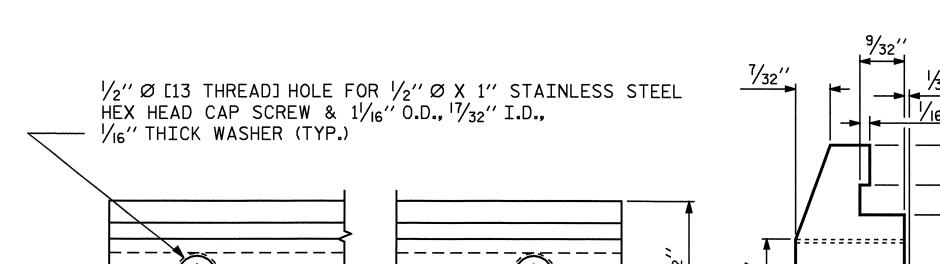
THE STRUCTURAL CONCRETE ANCHOR ASSEMBLY SHALL CONSIST OF THE FOLLOWING COMPONENTS:

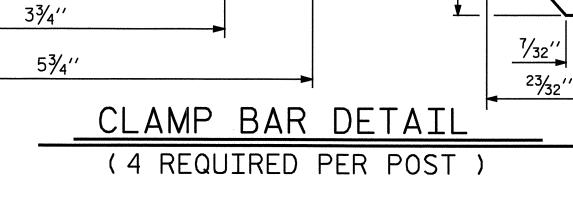
- A. FERRULES SHALL BE MADE FROM STEEL MEETING THE REQUIREMENTS OF AASHTO M169, GRADE 12L14 AND SHALL HAVE A MINIMUM LENGTH OF THREADS OF 2" FOR 3/4" FERRULES.
- B. 4 $\frac{3}{4}$ " Ø X 2 $\frac{1}{2}$ " BOLTS WITH WASHERS.BOLTS SHALL CONFORM TO THE REQUIREMENTS OF ASTM A307. BOLTS AND WASHERS SHALL BE GALVANIZED. AT THE CONTRACTOR'S OPTION, STAINLESS STEEL BOLTS AND WASHERS MAY BE USED AS AN ALTERNATE FOR THE 3/4" Ø X 21/2" GALVANIZED BOLTS AND WASHERS. THEY SHALL CONFORM TO OR EXCEED THE MECHANICAL REQUIREMENTS OF ASTM A307. THE USE OF THIS ALTERNATE SHALL BE APPROVED BY THE ENGINEER.
- C. WIRE STRUT SHOWN IN THE CONCRETE ANCHOR ASSEMBLY DETAIL IS THE MINIMUM ALLOWABLE SIZE AND SHALL HAVE A MINIMUM TENSILE STRENGTH OF 100,000 PSI. AS AN OPTION, A 7_6 " Ø WIRE STRUT WITH A MINIMUM TENSILE STRENGTH OF 90,000 PSI IS ACCEPTABLE.
- D. THE METAL RAIL ANCHOR ASSEMBLIES TO BE HOT DIPPED GALVANIZED TO CONFORM TO REQUIREMENTS OF AASHTO M111.
- E. THE COST OF THE METAL RAIL ANCHOR ASSEMBLY WITH BOLTS AND WASHERS COMPLETE IN PLACE SHALL BE INCLUDED IN THE PRICE BID FOR LINEAR FEET OF METAL RAIL.
- F. BOLTS TO BE TIGHTENED ONE-HALF TURN WITH A WRENCH FROM A FINGER-TIGHT POSITION.

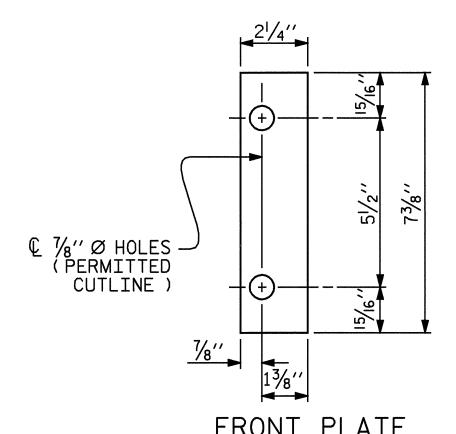
THE CONTRACTOR MAY USE ADHESIVELY ANCHORED ANCHOR BOLTS IN PLACE OF THE METAL RAIL ANCHOR ASSEMBLY. LEVEL ONE FIELD TESTING IS REQUIRED, AND THE YIELD LOAD OF THE 3/4" Ø BOLT IS 10 KIPS. FOR ADHESIVELY ANCHORED ANCHOR BOLTS OR DOWELS, SEE SPECIAL PROVISIONS.

WHEN ADHESIVELY ANCHORED ANCHOR BOLTS ARE USED, BOLTS SHALL MEET THE REQUIREMENTS OF ASTM F593 ALLOY 304 STAINLESS STEEL WITH MINIMUM 75,000 PSI ULTIMATE STRENGTH. NUTS SHALL MEET THE REQUIREMENTS OF ASTM F594 ALLOY 304 STAINLESS STEEL AND WASHERS SHALL MEET THE REQUIREMENTS OF ASTM F844 EXCEPT THEY SHALL BE MADE FROM ALLOY 304 STAINLESS STEEL.

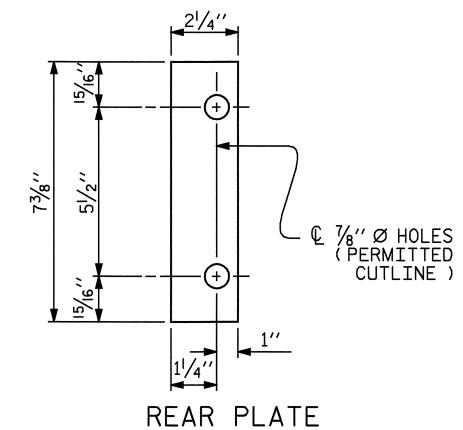






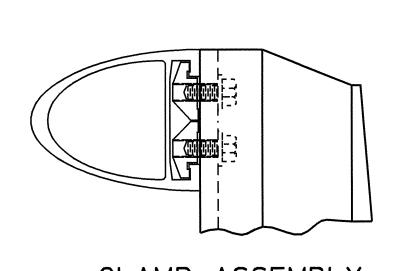




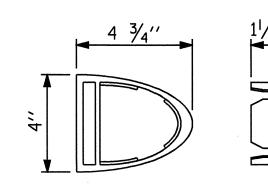


SHIM DETAILS

SHIMS MAY BE CUT ALONG PERMITTED CUTLINE OR SLOTTED TO EDGE OF PLATE TO FACILITATE PLACEMENT.



CLAMP ASSEMBLY



RAIL CAP



RAIL SECTION B-4274 PROJECT NO.____ SCOTLAND COUNTY

STATION: 12+68.90 -L-

← MINOR AXIS

/ SEMI-ELLIPSE

MAJOR AXIS

SHEET 3 OF 5

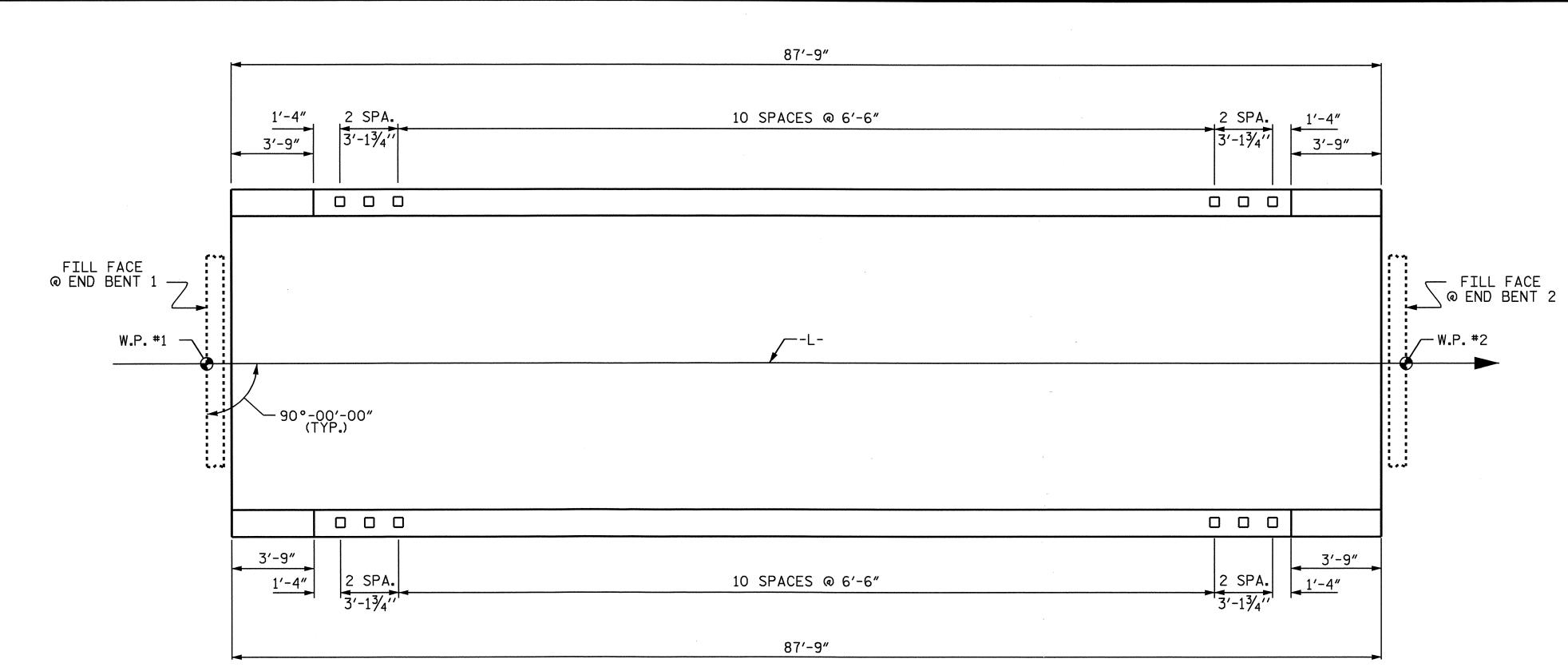
STATE OF NORTH CAROLINA DEPARTMENT OF TRANSPORTATION

STANDARD

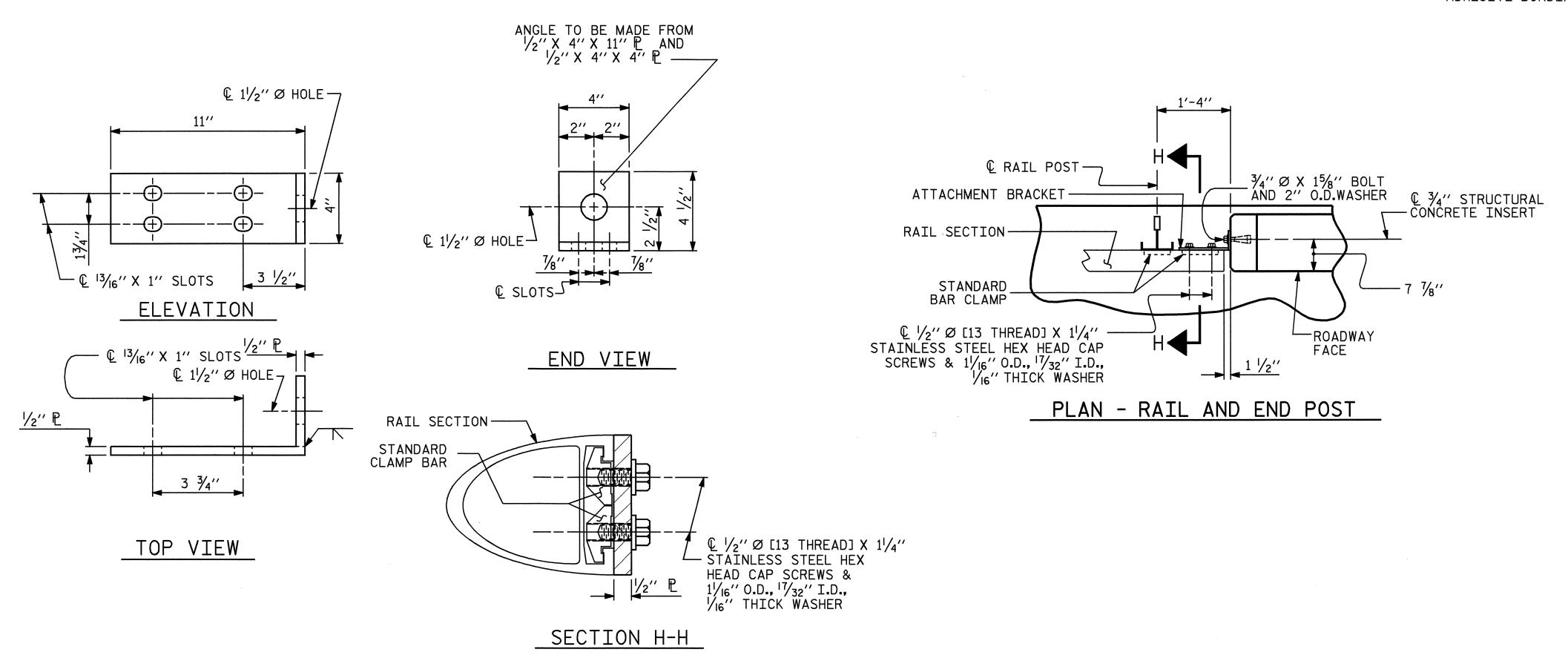
2 BAR METAL RAIL

		SHEET NO.				
NO.	BY:	DATE:	NO.	BY:	DATE:	S-I0
1			3			TOTAL SHEETS
2			4			∥ 22

ASSEMBLED BY: J.B. WILSON DATE: 7/01/06 CHECKED BY: B.N. GRADY DATE: 7/01/07 DRAWN BY: EEM 6/94 REV. 2/6/97 EEM/RGW REV. 8/16/99 MAB/LES REV. 5/1/06R KMM/GM



PLAN OF RAIL POST SPACINGS



<u>DETAILS FOR ATTACHING METAL RAIL TO END POST</u>

NOTES

STRUCTURAL CONCRETE INSERT

- THE STRUCTURAL CONCRETE INSERT ASSEMBLY SHALL CONSIST OF THE FOLLOWING COMPONENTS:
- A. FERRULES SHALL BE MADE FROM STEEL MEETING THE REQUIREMENTS OF AASHTO M169, GRADE 12L14 AND SHALL HAVE A MINIMUM LENGTH OF THREADS OF 11/2".
- B. 1 $\frac{3}{4}$ " Ø X $1\frac{5}{8}$ " BOLT WITH WASHER. BOLT SHALL CONFORM TO THE REQUIREMENTS OF ASTM A307. BOLT AND WASHER SHALL BE GALVANIZED. (AT THE CONTRACTOR'S OPTION, STAINLESS STEEL BOLT AND WASHER MAY BE USED AS AN ALTERNATE FOR THE $\frac{3}{4}$ " Ø X $1\frac{5}{8}$ " GALVANIZED BOLT AND WASHER. THEY SHALL CONFORM TO OR EXCEED THE MECHANICAL REQUIREMENTS OF ASTM A307. THE USE OF THIS ALTERNATE SHALL BE APPROVED BY THE ENGINEER.)
- C. WIRE STRUT SHOWN IN THE CONCRETE INSERT ASSEMBLY DETAIL IS THE MINIMUM ALLOWABLE SIZE AND SHALL HAVE A MINIMUM TENSILE STRENGTH OF 100,000 PSI. AS AN OPTION, A γ_6 " Ø WIRE STRUT WITH A MINIMUM TENSILE STRENGTH OF 90,000 PSI IS ACCEPTABLE.

METAL RAIL TO END POST CONNECTION

- THE METAL RAIL TO END POST CONNECTION SHALL CONSIST OF THE FOLLOWING COMPONENTS:
- A. $\frac{1}{2}$ " PLATES SHALL CONFORM TO AASHTO M270 GRADE 36 AND SHALL BE GALVANIZED AFTER FABRICATION.
- B. 3/11 STRUCTURAL CONCRETE INSERT SHALL HAVE A WORKING LOAD SHEAR CAPACITY OF 4800 LBS. THE FERRULES SHALL ENGAGE A 34"Ø X 158" BOLT WITH 2" O.D. WASHER IN PLACE. THE 34"Ø X 158" BOLT SHALL HAVE N. C. THREADS.
- C. CAP SCREWS FOR RAIL ATTACHMENT TO ANGLE SHALL CONFORM TO THE REQUIREMENTS OF ASTM F593 ALLOY 305 STAINLESS STEEL. CAP SCREWS TO BE CENTERED IN SLOTS AT 60°F.
- D. STANDARD CLAMP BARS (SEE METAL RAIL SHEET).
- E. $\frac{1}{2}$ " Ø PIPE SLEEVES (IF REQUIRED) TO BE GALVANIZED.

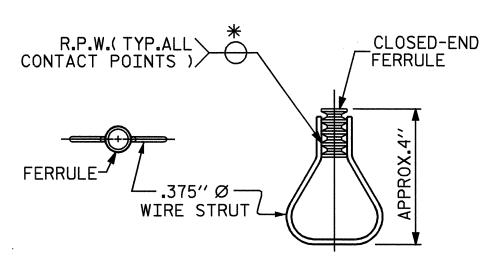
THE COST OF THE STANDARD CLAMP BARS AND CAP SCREWS USED IN THE METAL RAIL TO END POST CONNECTION SHALL BE INCLUDED IN THE UNIT CONTRACT PRICE BID FOR LINEAR FEET OF 1 OR 2 BAR METAL RAILS.

THE $\frac{3}{4}$ " STRUCTURAL CONCRETE INSERT WITH BOLT SHALL BE ASSEMBLED IN THE SHOP.

SEAL 14855

THE COST OF THE $\frac{3}{4}$ " STRUCTURAL CONCRETE INSERT ASSEMBLY, AND THE $\frac{1}{2}$ " PLATES COMPLETE IN PLACE SHALL BE INCLUDED IN THE VARIOUS PAY ITEMS.

THE CONTRACTOR, AT HIS OPTION, MAY USE AN ADHESIVE BONDING SYSTEM IN LIEU OF THE STRUCTURAL CONCRETE INSERT EMBEDDED IN THE END POST. IF THE ADHESIVE BONDING SYSTEM IS USED, THE 3/4" Ø X 15/8" BOLT WITH WASHER SHALL BE REPLACED WITH A $\frac{3}{4}$ " Ø X $6\frac{1}{2}$ " BOLT AND 2" O.D. WASHER. ALL SPECIFICATIONS THAT APPLY TO THE 34" Ø X 158" BOLT SHALL APPLY TO THE 34" Ø X 6 1/2" BOLT. FIELD TESTING OF THE ADHESIVE BONDING SYSTEM IS NOT REQUIRED.



ELEVATION

* EACH WELDED ATTACHMENT OF WIRE TO FERRULE SHALL DEVELOP THE TENSILE STRENGTH OF THE WIRE.

PROJECT NO. B-4274 SCOTLAND

COUNTY 12+68.90 -L-

SHEET 4 OF 5

DEPARTMENT OF TRANSPORTATION RALEIGH STANDARD

STATE OF NORTH CAROLINA

RAIL POST SPACINGS

END OF RAIL DETAILS FOR ONE OR TWO BAR METAL RAILS

SHEET NO. **REVISIONS** S-II DATE: NO. BY: BY: TOTAL SHEETS 22

25-JUN-2008 15:55 R:\Structures\b4274\Final Plans\B4274_sd_2MR.dgn

ASSEMBLED BY : J. MYA

CHECKED BY : B. N. GRADY

DRAWN BY: FCJ 1/88

CHECKED BY : CRK 3/89

DATE : 06/01/07

LES/RDR

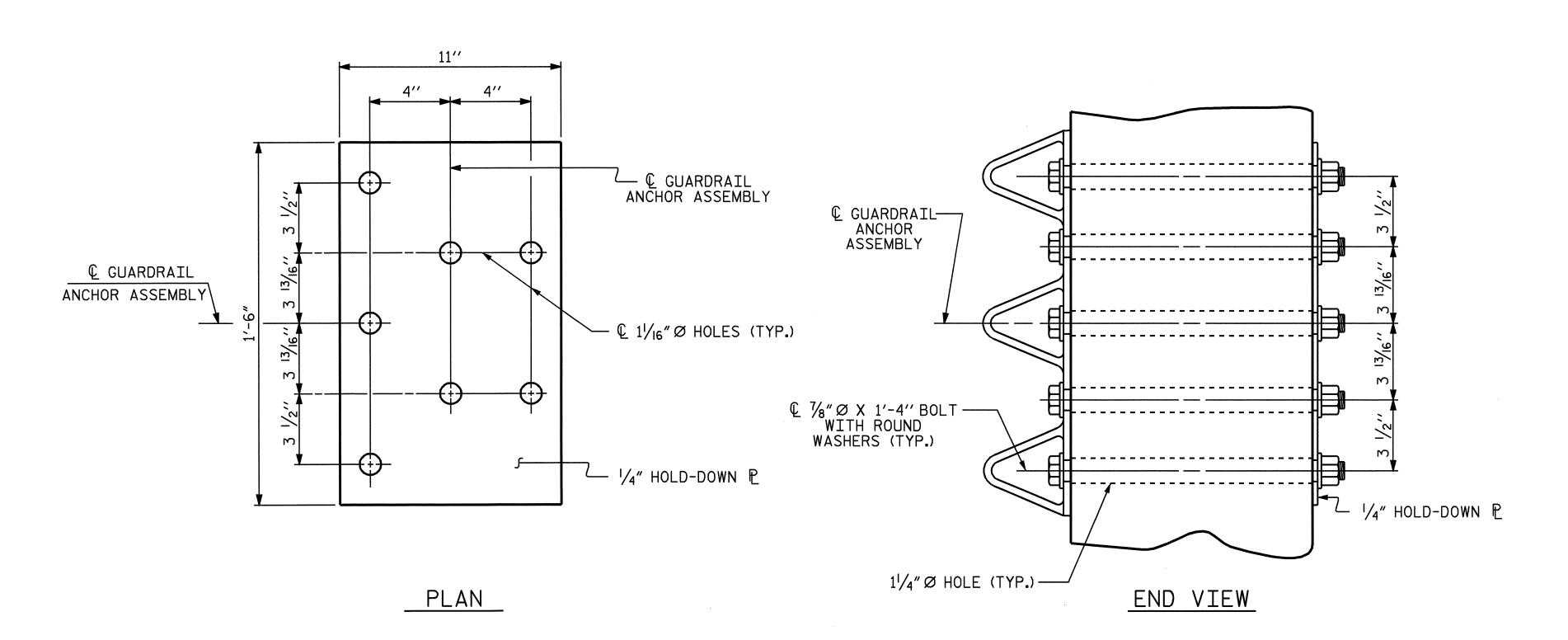
RWW/JTE

TLA/GM

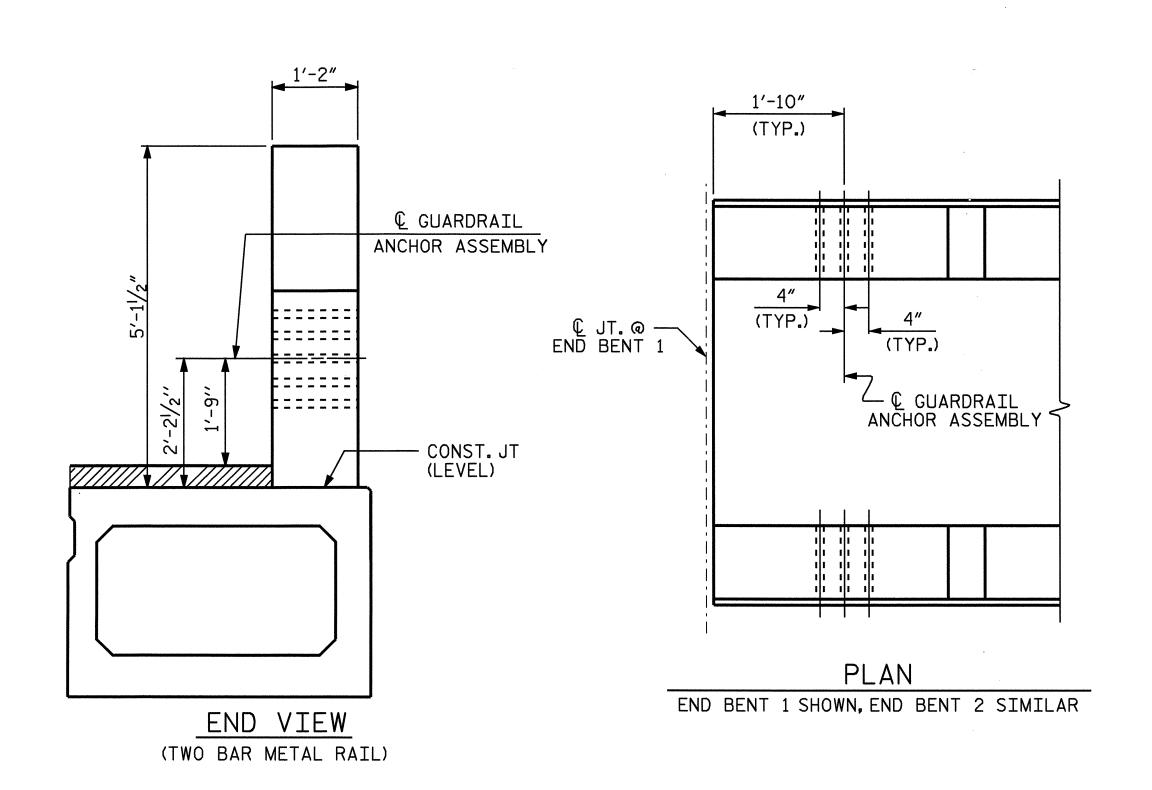
DATE : 7/01/07

REV. 10/17/00

REV. 5/7/03 REV. 5/1/06



GUARDRAIL ANCHOR ASSEMBLY DETAILS



LOCATION OF GUARDRAIL ANCHOR AT END POST

ASSEMBLED BY : J,MYA	DATE : 06/01/07		
CHECKED BY : B. N. GRADY	DATE : 7/01/07		
DRAWN BY: EEM 6/94 CHECKED BY: RGW 6/94	REV. 10/17/00 RWW/LES REV. 5/7/03 RWW/JTE REV. 5/1/06 TLA/GM		

NOTES

THE GUARDRAIL ANCHOR ASSEMBLY SHALL CONSIST OF A $1/4^{\prime\prime}$ HOLD DOWN PLATE AND 7 - $1/8^{\prime\prime}$ Ø BOLTS WITH NUTS AND WASHERS.

THE HOLD-DOWN PLATE SHALL CONFORM TO AASHTO M270 GRADE 36.AFTER FABRICATION, THE HOLD-DOWN PLATE SHALL BE HOT-DIP GALVANIZED IN ACCORDANCE WITH AASHTO M111.

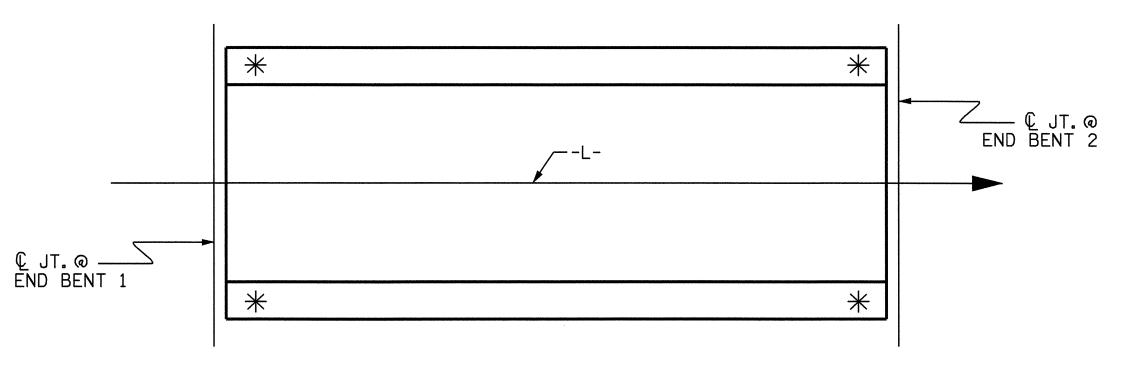
BOLTS SHALL CONFORM TO THE REQUIREMENTS OF ASTM A307 AND NUTS SHALL CONFORM TO THE REQUIREMENTS OF AASHTO M291. BOLTS, NUTS AND WASHERS SHALL BE GALVANIZED. AT THE CONTRACTOR'S OPTION, STAINLESS STEEL BOLTS, NUTS AND WASHERS MAY BE USED AS AN ALTERNATE FOR THE 1/8" Ø GALVANIZED BOLTS, NUTS AND WASHERS. THEY SHALL CONFORM TO OR EXCEED THE MECHANICAL REQUIREMENTS OF ASTM A307. THE USE OF THIS ALTERNATE SHALL BE APPROVED BY THE ENGINEER.

AFTER INSTALLATION, THE EXPOSED THREAD OF THE BOLT SHALL BE BURRED WITH A SHARP POINTED TOOL.

THE COST OF THE GUARDRAIL ANCHOR ASSEMBLIES WITH BOLTS, NUTS AND WASHERS COMPLETE IN PLACE, SHALL BE INCLUDED IN THE VARIOUS PAY ITEMS.

THE VERTICAL REINFORCING BARS MAY BE SHIFTED SLIGHTLY IN THE END POST TO CLEAR ASSEMBLY BOLTS.

THE 1 $\frac{1}{4}$ " Ø HOLES SHALL BE FORMED OR DRILLED WITH A CORE BIT. IMPACT TOOLS WILL NOT BE PERMITTED. ANY CONCRETE DAMAGED BY THIS WORK SHALL BE REPAIRED TO THE SATISFACTION OF THE ENGINEER.



SKETCH SHOWING POINTS OF ATTACHMENT

*LOCATION OF GUARDRAIL ATTACHMENT

PROJECT NO. B-4274

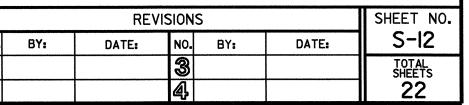
SCOTLAND COUNTY

STATION: 12+68.90 -L-

SHEET 5 OF 5

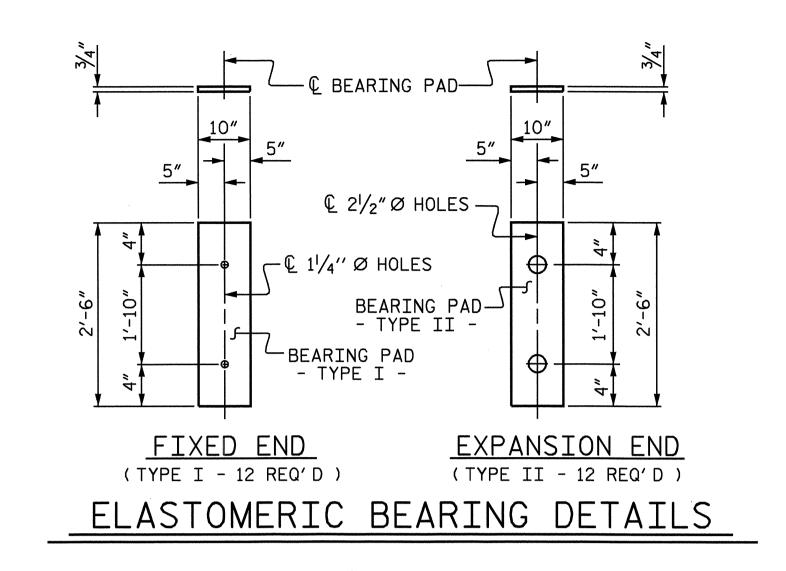
DEPARTMENT OF TRANSPORTATION
RALEIGH
STANDARD

GUARDRAIL ANCHORAGE DETAILS FOR METAL RAILS

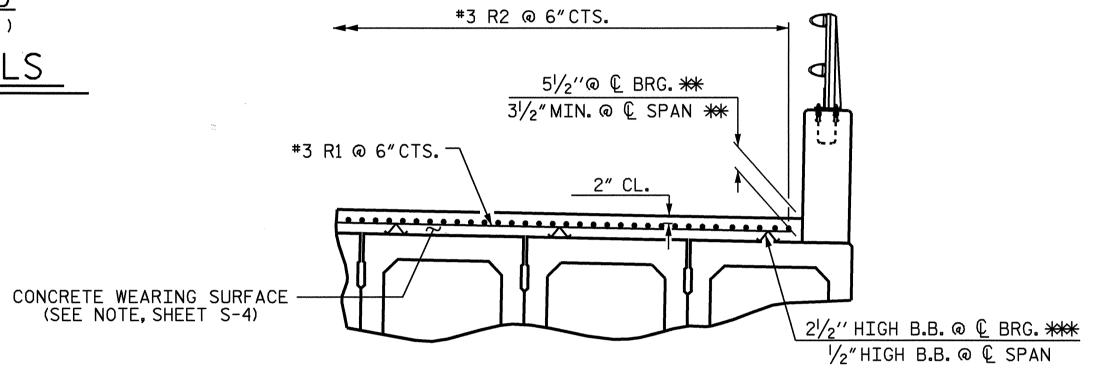


STD. NO. BMR8

25-JUN-2008 15:55 R:\Structures\b4274\FinalPlans\B4274_sd_2MR.dgn Imva



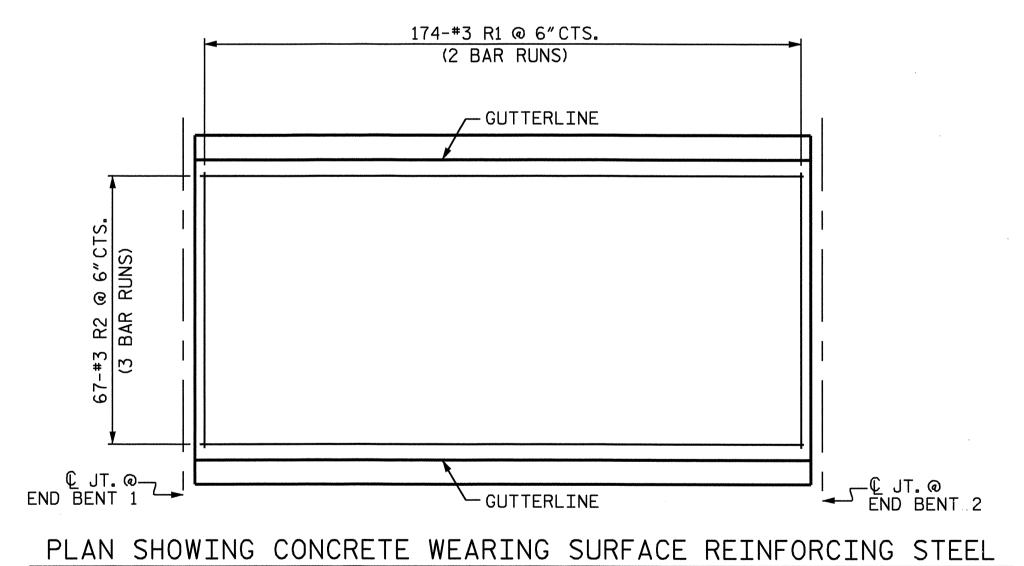
BOX E	BOX BEAM UNITS REQUIRED								
	NUMBER	LENGTH	TOTAL LENGTH						
UNIT TYPE									
INTERIOR	10	87′-9″	877′-6′′						
EXTERIOR	2	87′-9″	175′-6″						
TOTAL	12	87′-9″	1053′-0′′						



GROOVING BRIDGE FLOORS APPROACH SLABS <u>1446</u> SQ.FT. 2638 SQ.FT. BRIDGE DECK TOTAL 4084 SQ.FT.

REINFORCING FOR CONCRETE WEARING SURFACE

** BASED ON PREDICTED FINAL CAMBER AND THEORETICAL GRADE LINE ELEVATIONS *** BEAM BOLSTERS SHALL BE PLACED AT 2'-0" MAXIMUM SPACINGS.



BILL OF MATERIAL FOR CONCRETE WEARING SURFACE							
BAR	NO.	SIZE	TYPE	LENGTH	WEIGHT		
 ₩R1	348	#3	STR	17′-3′′	2257		
*R2	201	#3	STR	30'-0''	2267		
] .			
* EPOXY CO	DATED RE	INFORCI	NG STEEL		4524		
CONCRETE	E WEARIN	G SURFAC	CE	SQ.FT.	2908		

SPLICE LEN	IGTH CHART
BAR SIZE	EPOXY COATED
#3	1′-3″

B-4274 PROJECT NO. B-4274

SCOTLAND COUNTY

STATION: 12+68.90 -L-

STATE OF NORTH CAROLINA DEPARTMENT OF TRANSPORTATION
RALEIGH

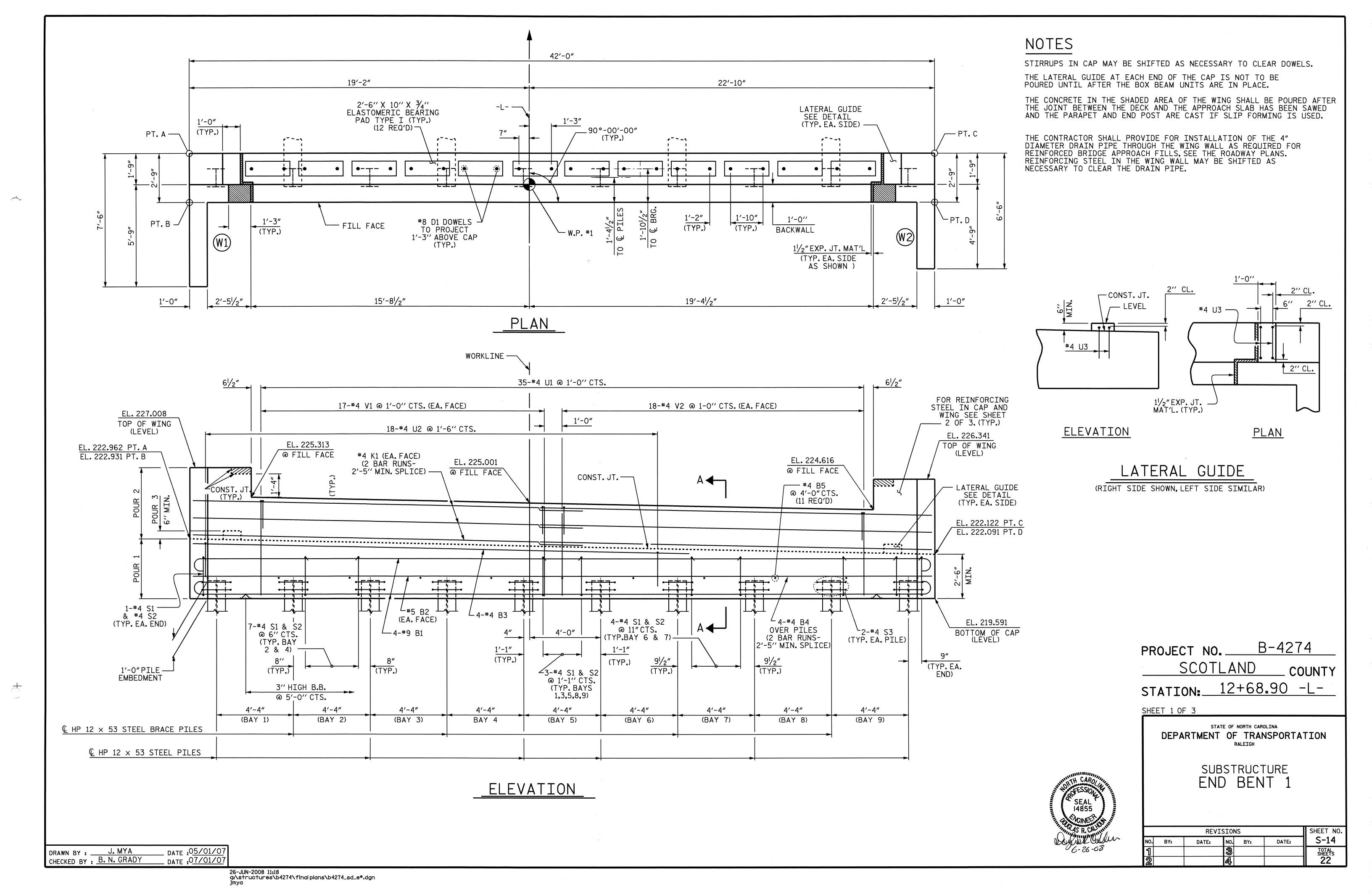
STANDARD

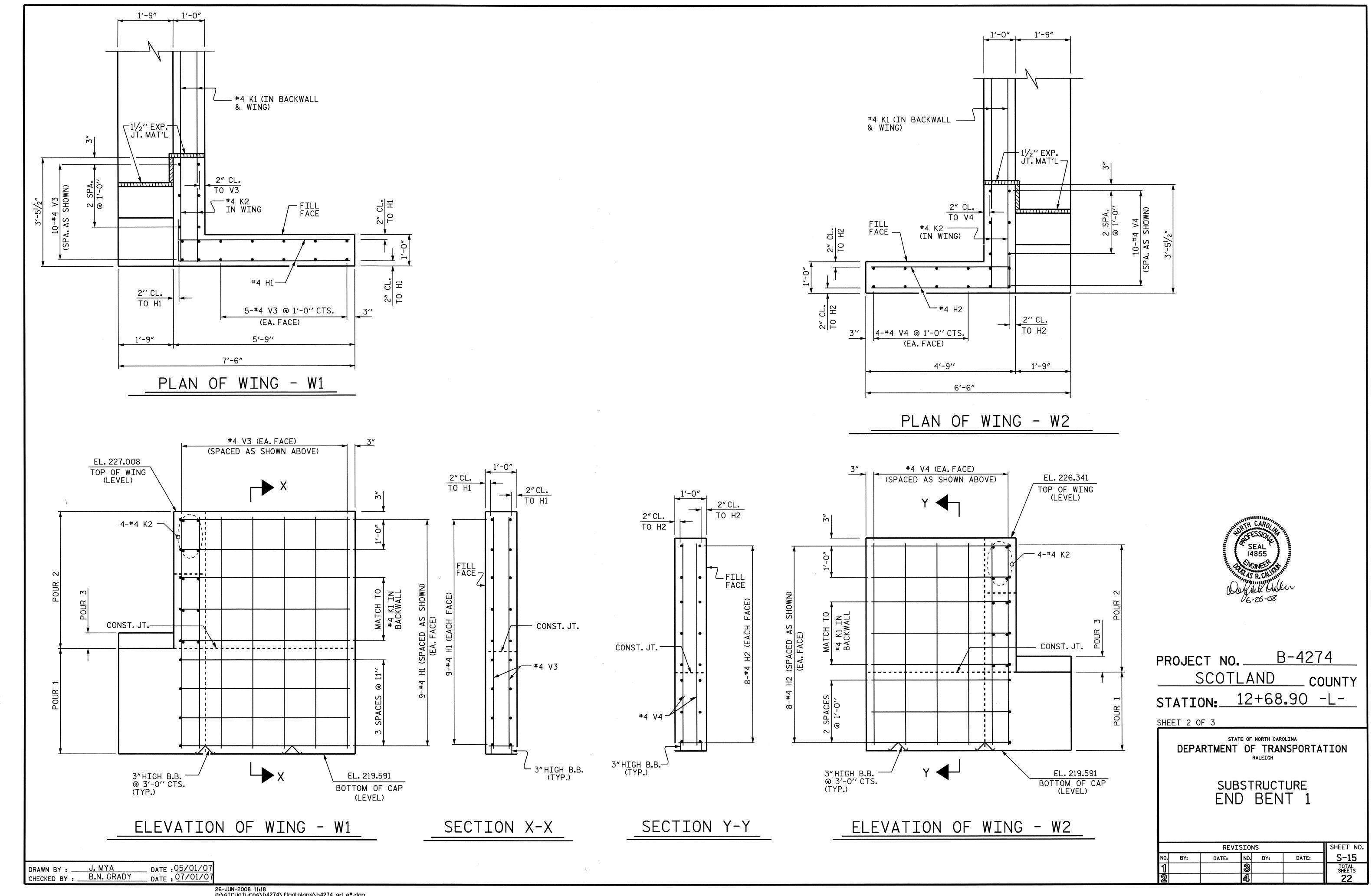
3'-0" X 3'-3" PRESTRESSED CONCRETE BOX BEAM UNIT DETAILS

STITE OF	OF ESSION	William Control
William Work	³⁵ SEAL ⁷ 14855	Walter Walter Control of the Control
	SUCINEES	
11/1/1	QAS R. CAL	Authority
Old	wille	28 28

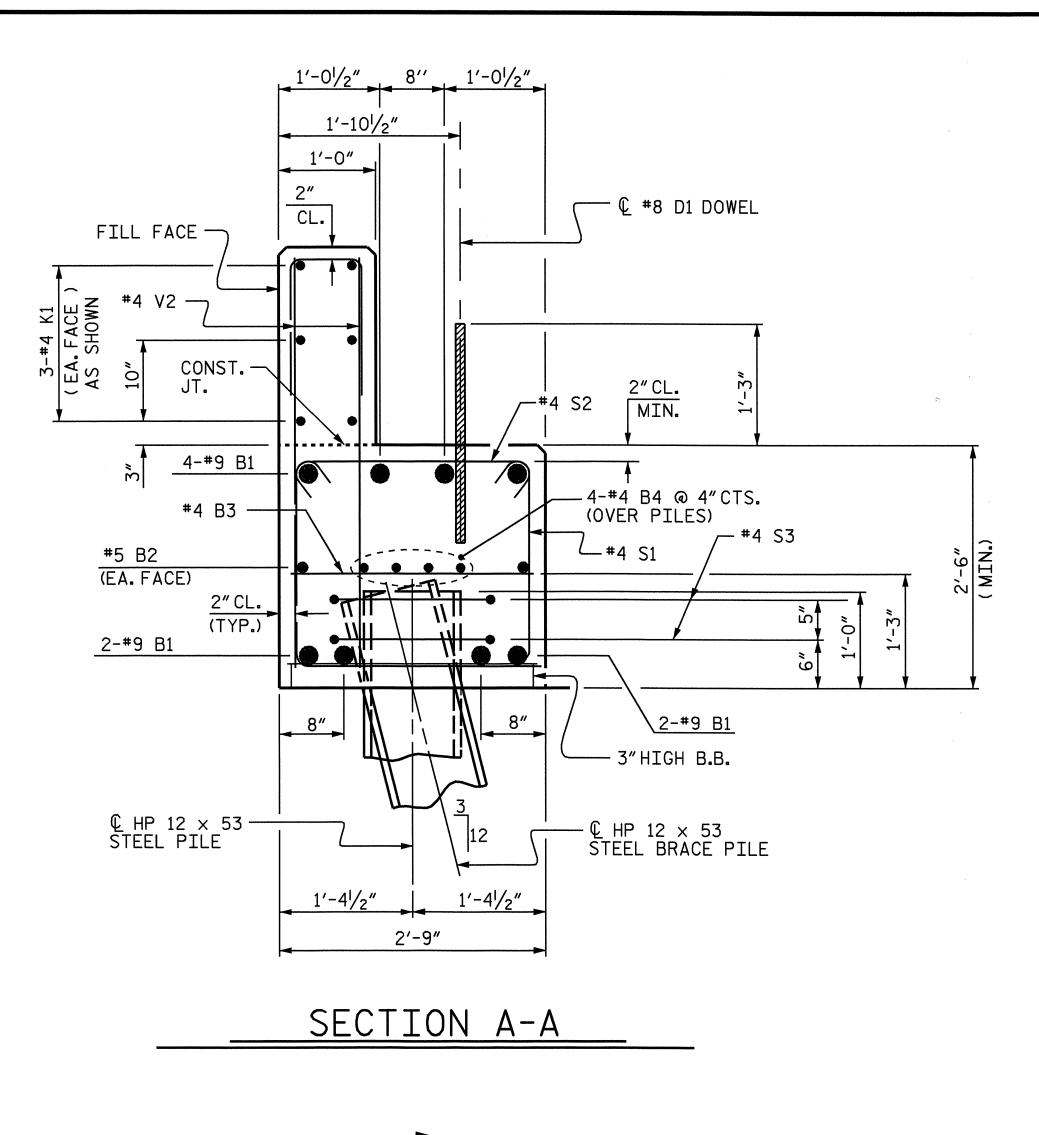
	REV	ISION	S		SHEET NO.
BY:	DATE:	NO.	BY:	DATE:	S-13
		3			TOTAL SHEETS
		4			22

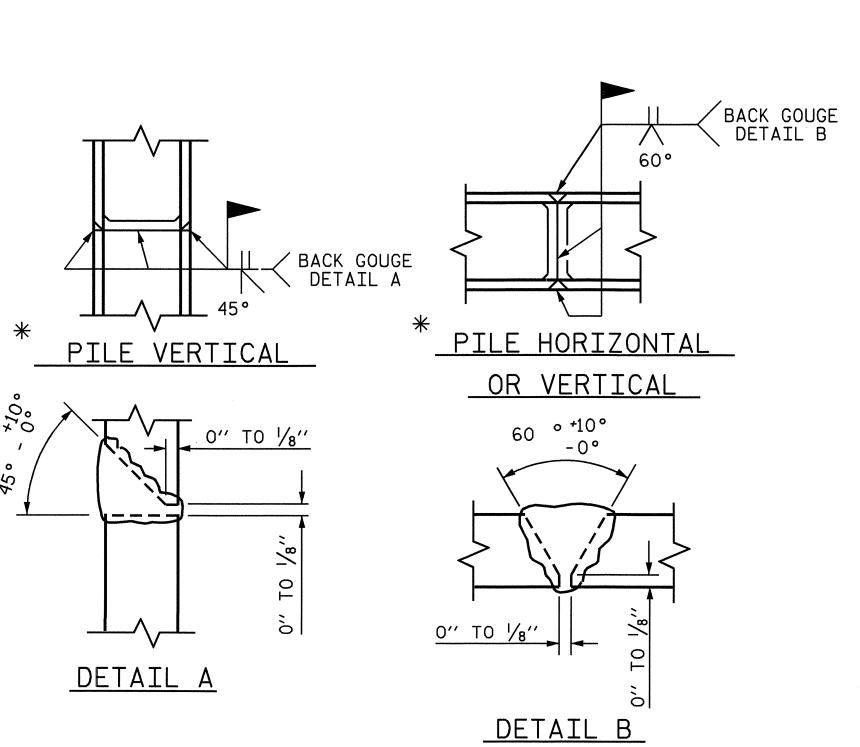
ASSEMBLED BY : J.MYA CHECKED BY : B.N. GRADY DATE : 6/01/07 DATE : 7/01/07 ADDED 7/II/05R REV.5/I/06 TLA/GM DRAWN BY: TLA 5/05 CHECKED BY: GM 6/05





26-JUN-2008 11:18 q:\structures\b4274\finalplans\b4274_sd_e#.dgn





*POSITION OF PILE DURING WELDING.

___ DATE .05/01/07 ___ DATE .07/01/07

J. MYA

CHECKED BY : B.N. GRADY

PILE SPLICE DETAILS

BAR TYPES 2'-5" 1'-5" 5'-5" 4'-5'' ___1'-3'' LAP 1'-8" Ø 2'-5" NO.: 10

END BENT 1 SIZE TYPE LENGTH WEIGHT 87 28'-0'' 22'-1'' 118 STR 18 STR 144 H2 54 5'-1'' STR 22'-1'' 177 STR 3'-1'' 16 193 87 86 114 STR 112 STR 95 77 #4 STR LBS 2894 REINFORCING STEEL CLASS A CONCRETE BREAKDOWN POUR 1 (CAP & LOWER C.Y. PART OF WINGS) 13.5 POUR 2 (BACKWALL & UPPER PART OF WINGS) C.Y. 5.6 POUR 3 (LATERAL GUIDES) C.Y. 0.1 C.Y. TOTAL 19.2 HP 12 X 53 STEEL PILES:

LIN. FT.: 550

BILL OF MATERIAL

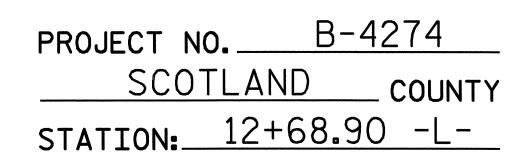
ALL BAR DIMENSIONS ARE OUT TO OUT. MINIMUM OF 3- ONE CUBIC FOOT BAGS OF #78M STONE. BAGS SHALL BE OF POROUS FABRIC, SECURELY TIED. 6"(MIN.)PIPE FOR DRAINAGE GRADE TO DRAIN TOE OF SLOPE-

BAGGED STONE AND PIPE SHALL BE PLACED IMMEDIATELY AFTER COMPLETION OF END BENT EXCAVATION. PIPE MAY BE EITHER CONCRETE, CORRUGATED STEEL, CORRUGATED ALUMINUM ALLOY, OR CORRUGATED PLASTIC. PERFORATED PIPE WILL NOT BE ALLOWED.

BAGGED STONE SHALL REMAIN IN PLACE UNTIL THE ENGINEER DIRECTS THAT IT BE REMOVED. THE CONTRACTOR SHALL REMOVE AND DISPOSE OF SILT ACCUMULATIONS AT BAGGED STONE WHEN SO DIRECTED BY THE ENGINEER. BAGS SHALL BE REMOVED AND REPLACED WHENEVER THE ENGINEER DETERMINES THAT THEY HAVE DETERIORATED AND LOST THEIR EFFECTIVENESS.

NO SEPARATE PAYMENT WILL BE MADE FOR THIS WORK AND THE ENTIRE COST OF THIS WORK SHALL BE INCLUDED IN THE UNIT CONTRACT PRICE BID FOR THE SEVERAL PAY ITEMS.

TEMPORARY DRAINAGE AT END BENT



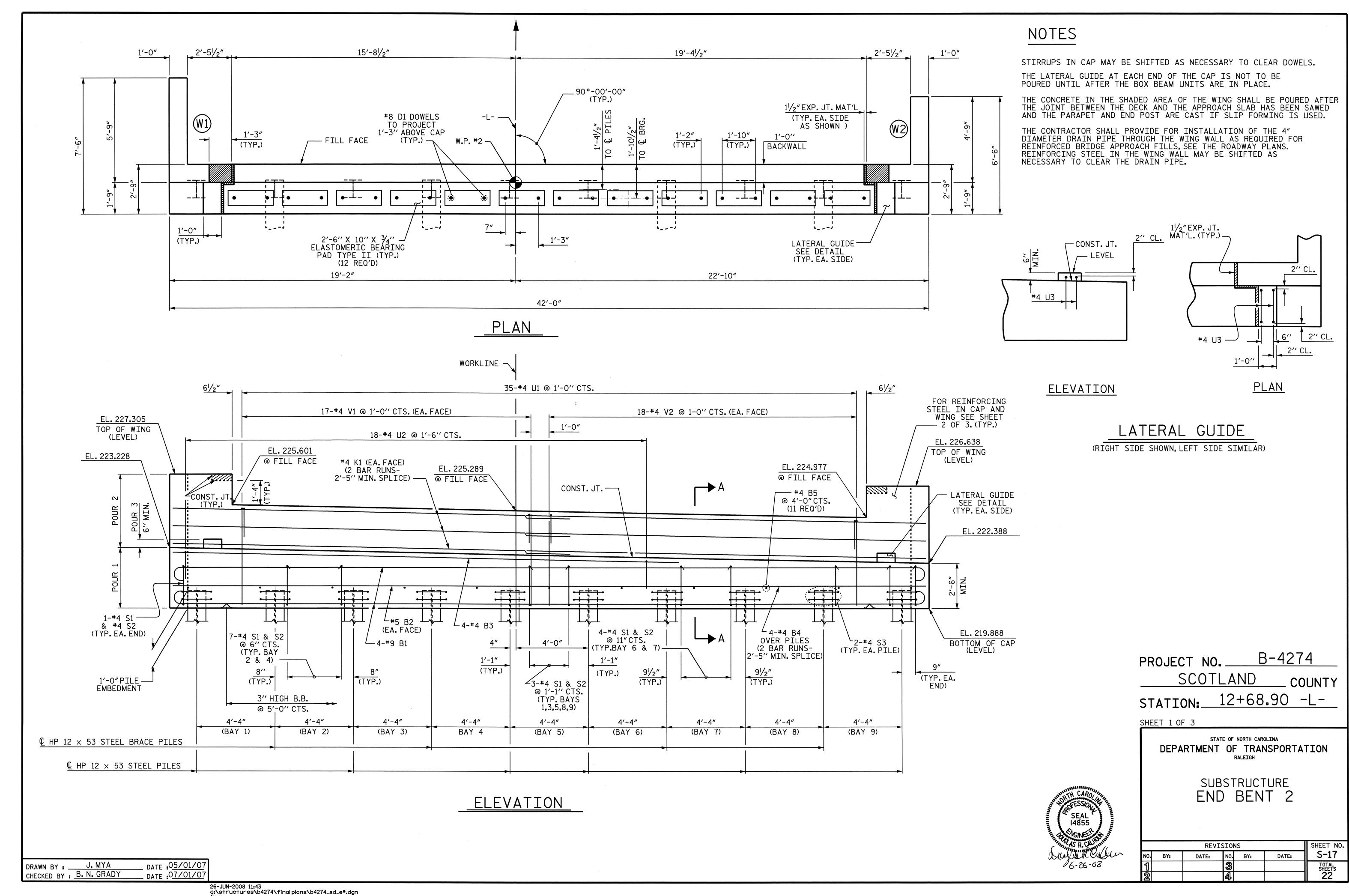
SHEET 3 OF 3

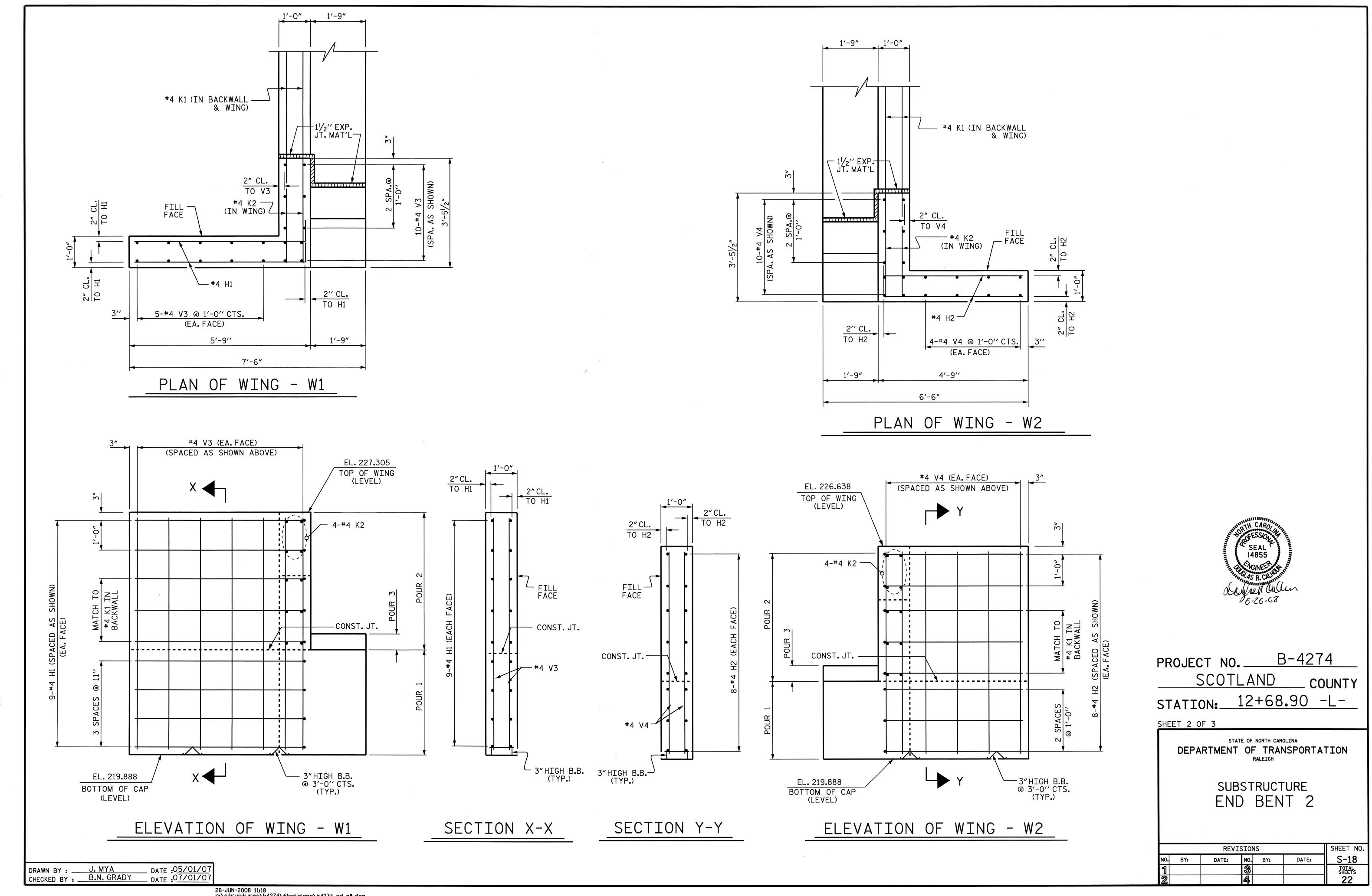
STATE OF NORTH CAROLINA DEPARTMENT OF TRANSPORTATION

> SUBSTRUCTURE END BENT 1

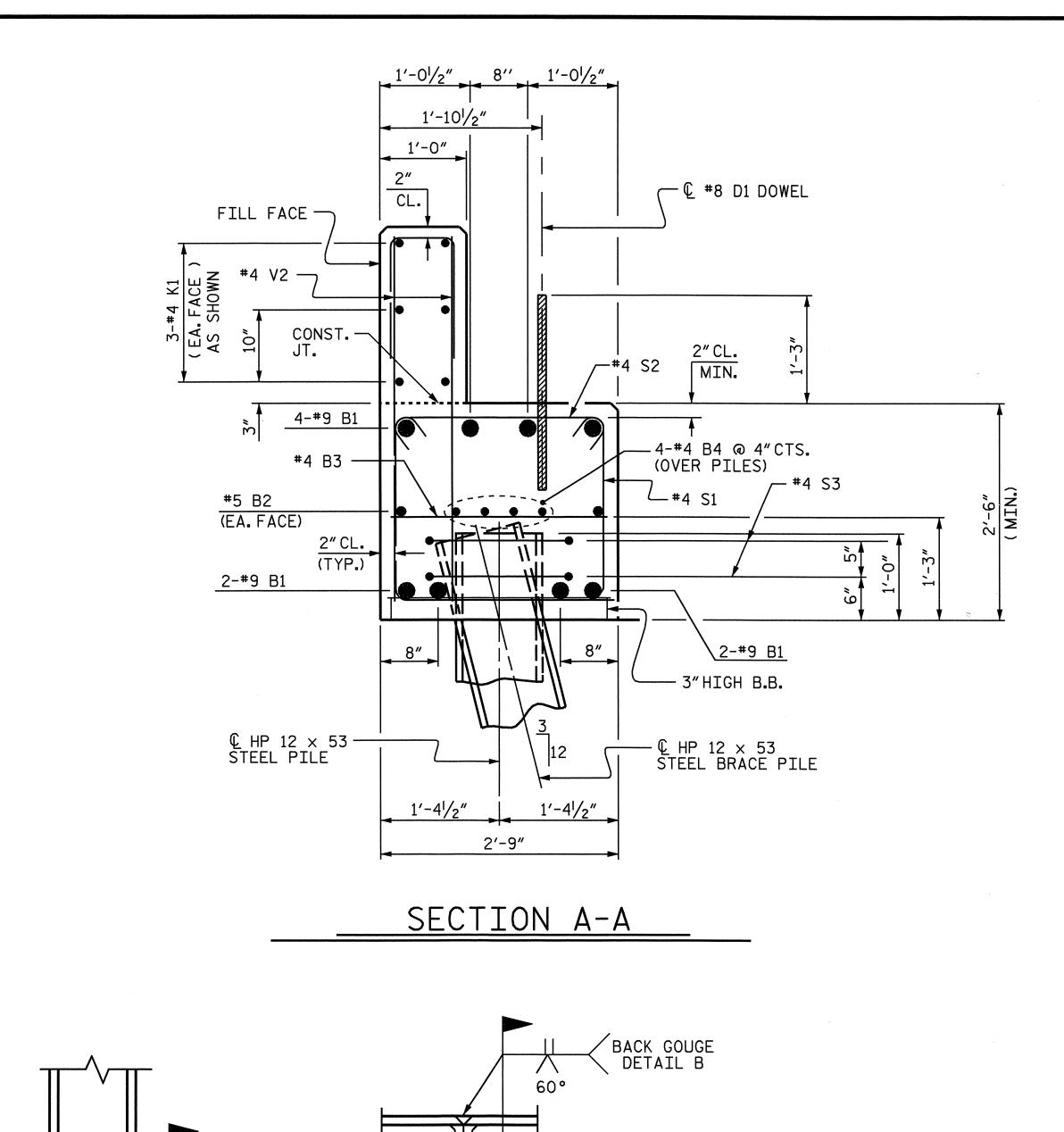


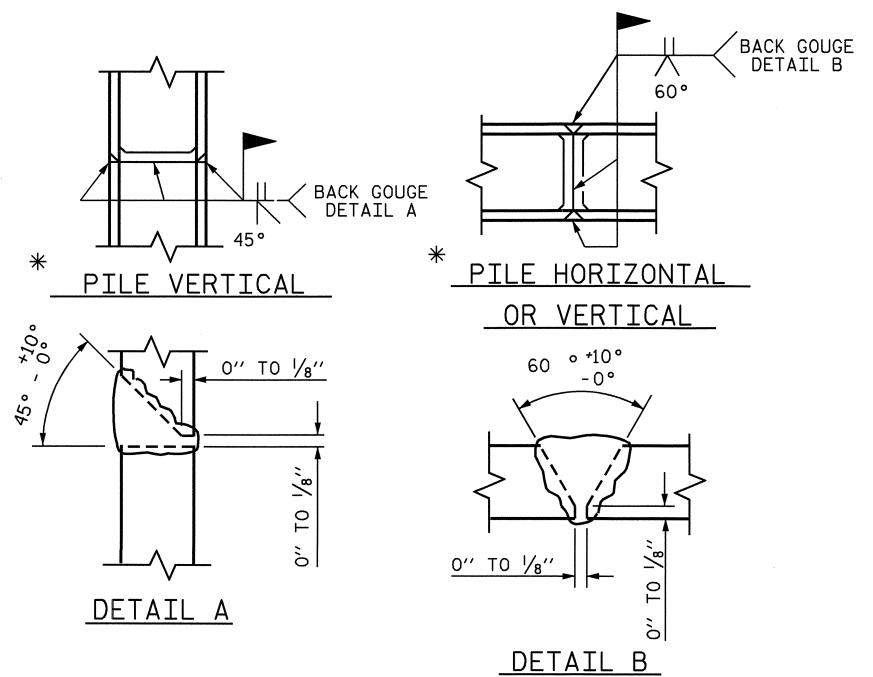
		REV:	ISION	S		SHEET NO.
NO.	BY:	DATE:	NO.	BY:	DATE:	S-16
1			3			TOTAL SHEETS
2			4,			22





26-JUN-2008 11:18 q:\structures\b4274\finalplans\b4274_sd_e#.dgn





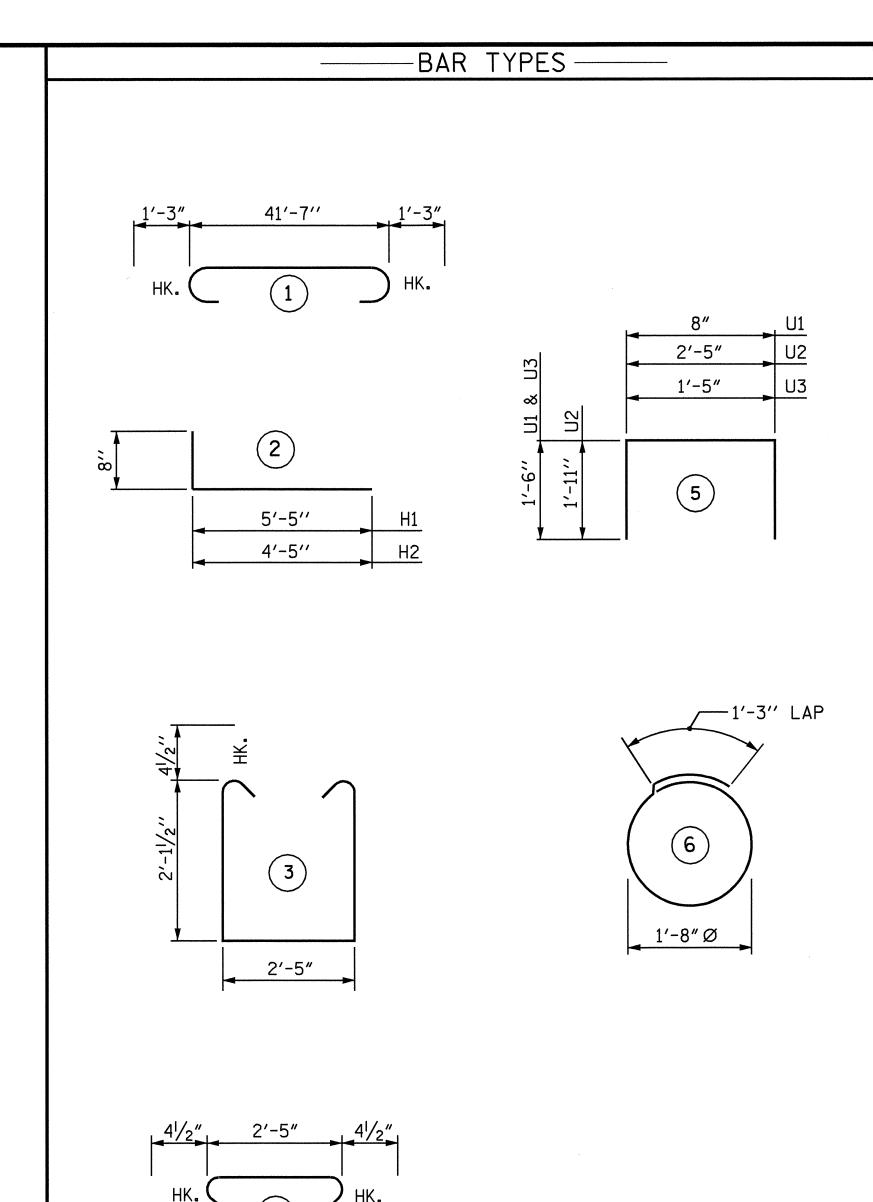
* POSITION OF PILE DURING WELDING.

__ DATE .05/01/07 __ DATE .07/01/07

J. MYA

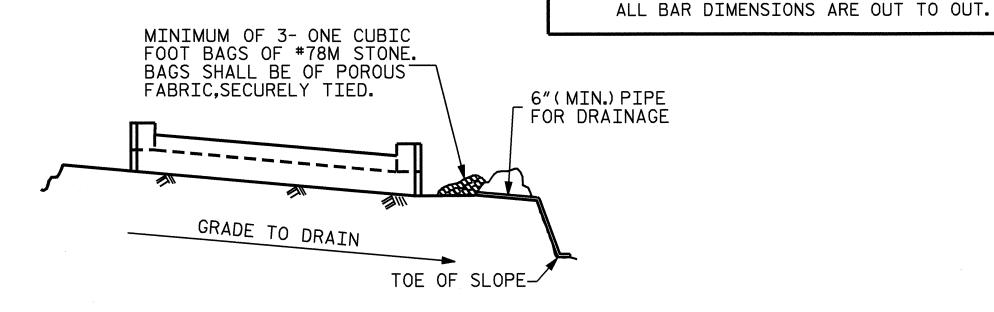
CHECKED BY : B.N. GRADY

PILE SPLICE DETAILS



		END	BEN		
BAR		SIZE	TYPE		WEIGHT
B1 B2	8 2	#9 #5	1 STR	44'-1'' 41'-8''	1199 87
B3	4	#4	STR	28'-0''	75
B4	8	#4	STR		118
B5	11	#4	STR	2′-5′′	18
D1	24	#8	STR	2'-3''	144
H1 H2	18 16	#4 #4	2 2	6′-1′′ 5′-1′′	73 54
			_		
K1 K2	12 8	#4 #4	STR	22′-1′′ 3′-1′′	177 16
		•	STR		16
S1	39 70	#4 #4	3	7′-5′′	193
S2 S3	39 20	#4 #4	4 6	3′-2′′ 6′-6′′	82 87
U1	35	#4	5	3′-8′′	86
U2	18	#4	5	6′-3′′	75
U3	4	#4	5	4'-5''	12
V1	34	#4	STR	5′-0′′	114
٧2	36	#4	STR	4'-8''	112
V3 V4				7'-1'' 6'-5''	95 77
REI	NFORCING	SILL	L	LBS	2894
CLA	SS A CON	ICRETE	BREAK	DOWN	
	UR 1 (CAF		WER	C.Y.	17 E
	RT OF WI		0 IID		13 . 5
	UR 2 (BA) RT OF WI		. & UP	C.Y.	5.6
PO	UR 3 (LA	TERAL	GUIDES	S) C.Y.	0.1
ТОТ	AL			C.Y.	19.2
HP	12 X 53	STEEL	PILES	8	
NC	.: 10			LIN.FT.:	750

BILL OF MATERIAL

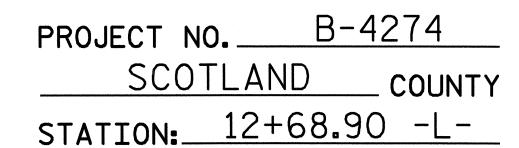


BAGGED STONE AND PIPE SHALL BE PLACED IMMEDIATELY AFTER COMPLETION OF END BENT EXCAVATION. PIPE MAY BE EITHER CONCRETE, CORRUGATED STEEL, CORRUGATED ALUMINUM ALLOY, OR CORRUGATED PLASTIC. PERFORATED PIPE WILL NOT BE ALLOWED.

BAGGED STONE SHALL REMAIN IN PLACE UNTIL THE ENGINEER DIRECTS THAT IT BE REMOVED. THE CONTRACTOR SHALL REMOVE AND DISPOSE OF SILT ACCUMULATIONS AT BAGGED STONE WHEN SO DIRECTED BY THE ENGINEER. BAGS SHALL BE REMOVED AND REPLACED WHENEVER THE ENGINEER DETERMINES THAT THEY HAVE DETERIORATED AND LOST THEIR EFFECTIVENESS.

NO SEPARATE PAYMENT WILL BE MADE FOR THIS WORK AND THE ENTIRE COST OF THIS WORK SHALL BE INCLUDED IN THE UNIT CONTRACT PRICE BID FOR THE SEVERAL PAY ITEMS.

TEMPORARY DRAINAGE AT END BENT

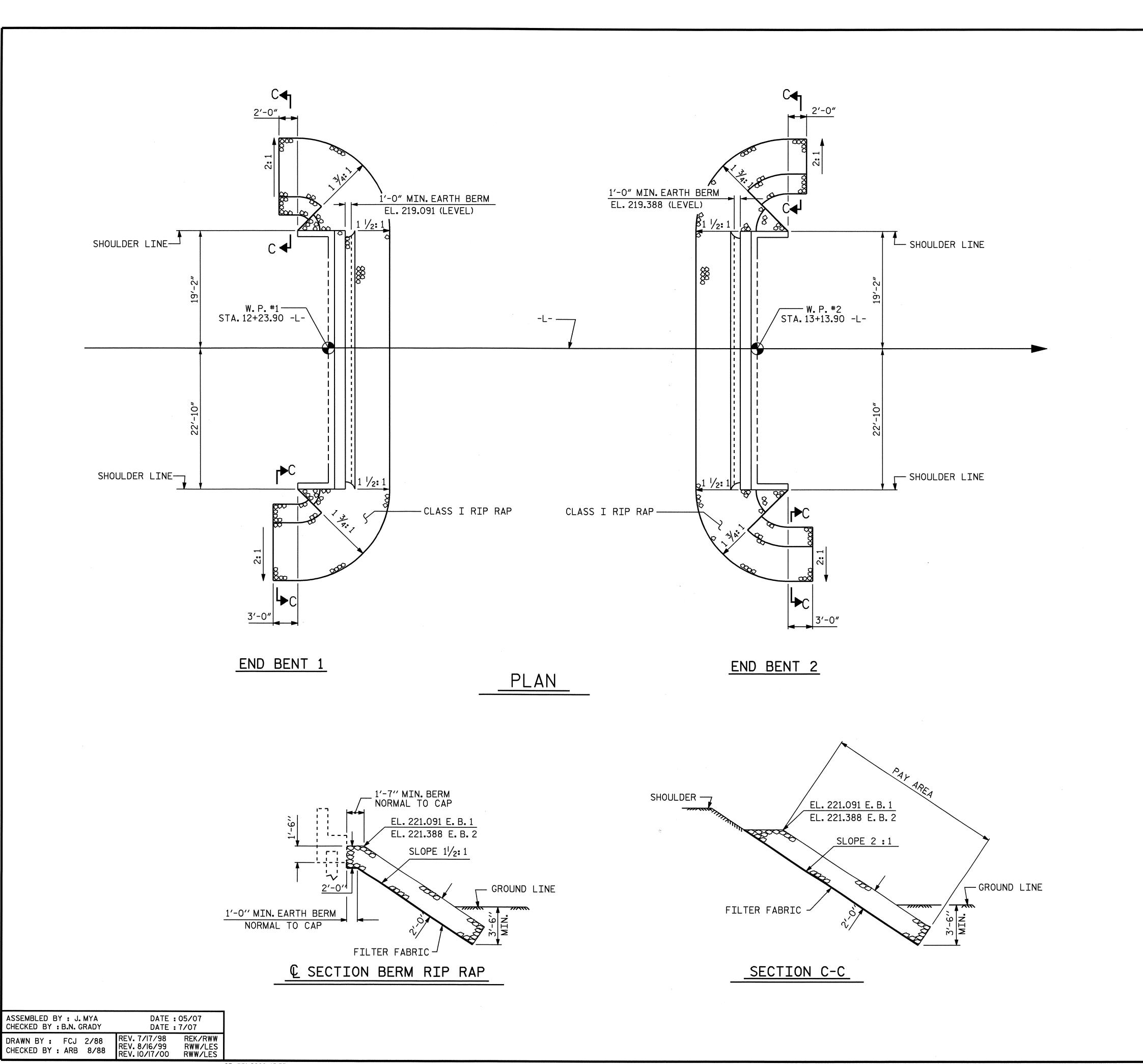


SHEET 3 OF 3

STATE OF NORTH CAROLINA DEPARTMENT OF TRANSPORTATION

> SUBSTRUCTURE END BENT 2

SHEET NO. REVISIONS S-19 DATE: DATE: BY: NO. BY: TOTAL SHEETS 22

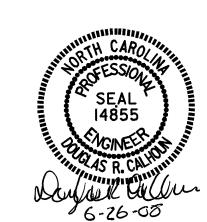


ESTIMATED QUANTITIES						
BRIDGE @ RIP RAP CLASS I FILTER FABRIC FOR DRAINAGE						
TONS	SQUARE YARDS					
89	99					
67	74					
	RIP RAP CLASS I (2'-0" THICK) TONS 89					

PROJECT NO. B-4274

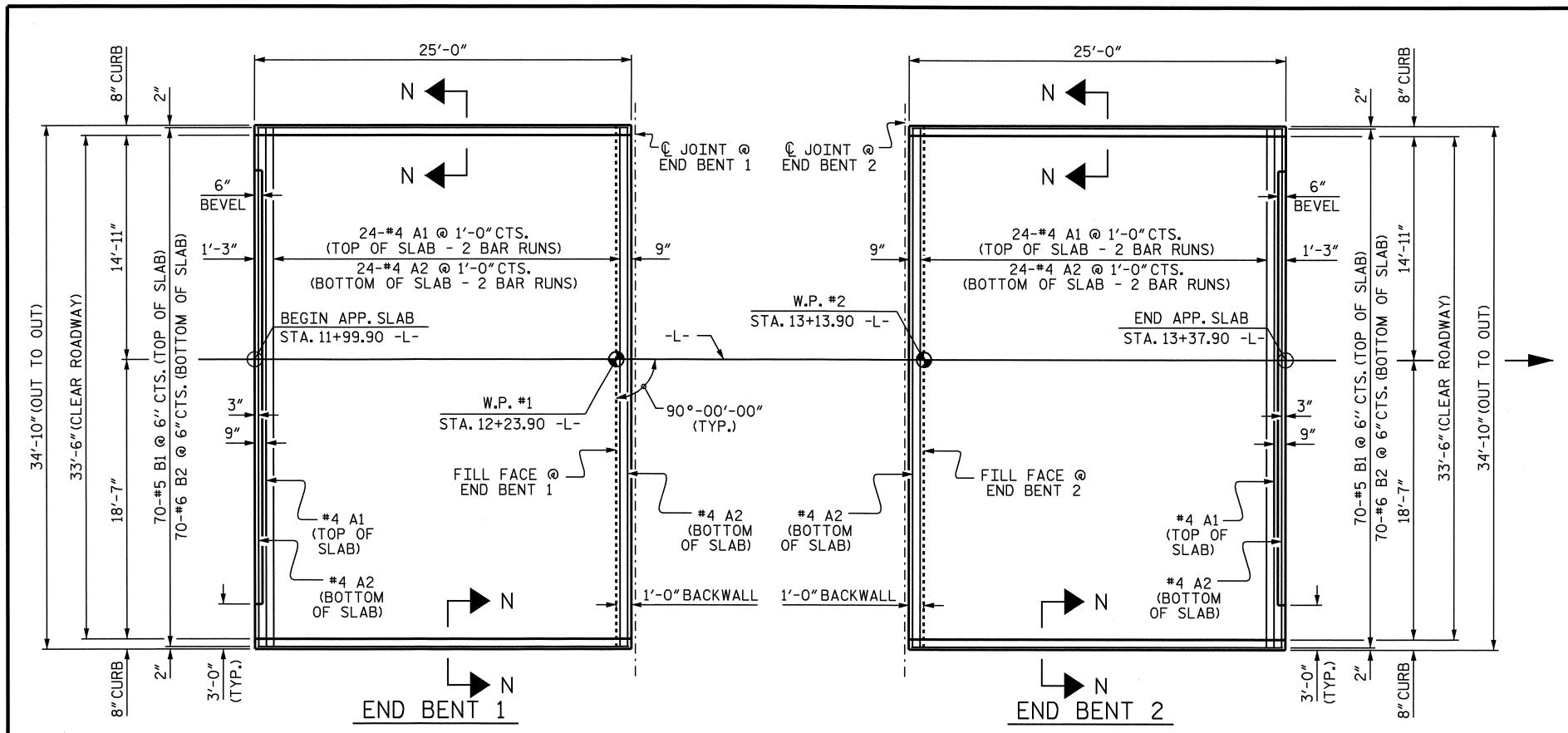
SCOTLAND COUNTY

STATION: 12+68.90 -L-



	= R:	IP R	RAP) DI	ETAI	LS—
		REV:	ISION:	S		SHEET NO
NO.	BY:	REV	ISION:	S BY:	DATE:	SHEET NO

SKEW 90°



PLAN OF APPROACH SLAB

--- #6 B2

LIMITS OF REINFORCED
BRIDGE APPROACH FILL
(ROADWAY PAY ITEM, SEE NOTES)-

FABRIC —

(TYP.)

†2:1 SLOPE -

4"Ø CORRUGATED

DRAINAGE PIPE ---

PERFORATED

SECTION THRU SLAB

51/4" CONTINUOUS HIGH CHAIR UPPER (CHCU)

@ 3'-0"CTS. ACROSS SLAB

-#4 A1

— #4 A2

6"COMP. A.B.C.-

SELECT MATERIAL

NORMAL TO END BENT

DATE:05/07 DATE:07/07

REV. 7/I0/OI LES/RDR REV. 5/7/O3R RWW/JTE REV. 5/I/O6R KMM/GM

ROADWAY

ASSEMBLED BY : J. MYA CHECKED BY : B. N. GRADY

DRAWN BY: EEM 3/95 CHECKED BY : VAP 3/95

APPROVED WIRE BAR

SUPPORTS @ 3'-0"CTS.

NOTES

APPROACH SLAB SHALL NOT BE CONSTRUCTED PRIOR TO COMPLETION OF THE BRIDGE DECK.

FOR REINFORCED BRIDGE APPROACH FILL INCLUDING FABRIC, IMPERMEABLE GEOMEMBRANE, 4" Ø DRAINAGE PIPE, #78M STONE, AND SELECT MATERIAL. SEE ROADWAY PLANS.

AREA BETWEEN THE WINGWALL AND APPROACH SLAB SHALL BE GRADED TO DRAIN THE WATER AWAY FROM THE FILL FACE OF THE BRIDGE AND SHALL BE PAVED. SEE ROADWAY PLANS.

THE 6"COMP. A.B.C. SHALL BE FLUSH WITH THE ROADWAY END OF THE APPROACH SLAB AND SHALL EXTEND 1'-0"OUTSIDE EACH EDGE OF THE APPROACH SLAB.

THE CONTRACTOR MAY USE 4"TYPE B-25.0B ASPHALT CONCRETE BASE COURSE IN LIEU OF 6"COMP. A.B.C. IF THIS OPTION IS USED, THE BASE COURSE SHALL BE FLUSH WITH THE ROADWAY END OF THE APPROACH SLAB, AND THE WIDTH SHALL BE THE SAME AS THAT OF THE APPROACH SLAB.

THE CONTRACTOR MAY USE 5"CLASS "A" CONCRETE BASE IN LIEU OF 6" COMP. A.B.C. IF THIS OPTION IS USED, THE CONCRETE BASE SHALL BE FLUSH WITH THE ROADWAY END OF THE APPROACH SLAB, AND THE WIDTH SHALL BE THE SAME AS THAT OF THE APPROACH SLAB. THE CONCRETE SHALL BE FINISHED TO A SMOOTH SURFACE AND A LAYER OF 30 LB ROOFING FELT SHALL BE PLACED BETWEEN THE CONCRETE BASE AND THE APPROACH SLAB TO PREVENT BOND. THE APPROACH SLAB SHALL NOT BE CAST UNTIL THE CONCRETE BASE HAS REACHED AN AGE OF THREE CURING DAYS.

THE JOINT SHALL BE SAWED PRIOR TO THE CASTING OF THE PARAPET AND END POST.

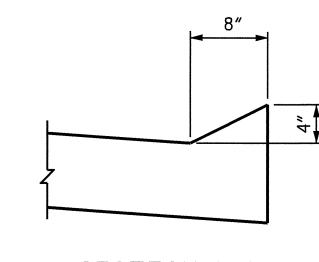
FOR EVAZOTE JOINT SEALS, SEE SPECIAL PROVISIONS.

THE NOMINAL UNCOMPRESSED SEAL WIDTH OF THE EVAZOTE JOINT SEAL SHALL BE 21/2".

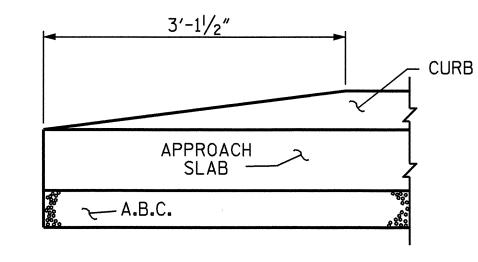
FOR ELASTOMERIC CONCRETE, SEE SPECIAL PROVISIONS.

BILL OF MATERIAL						
FOR ONE APPROACH SLAB (2 REQ'D)						
BAR	NO.	SIZE	TYPE	LENGTH	WEIGHT	
∗ A1	50	#4	STR	18′-3″	610	
A2	52	#4	STR	18'-2"	631	
∗ B1	70	#5	STR	23′-10″	1740	
B2	70	#6	STR	24'-8"	2593	
REINFORCING STEEL LBS. 3224						
*EPOXY COATED REINFORCING STEEL LBS. 2350						
CLASS	SAA	CONCRE	TE	C. Y.	32.9	

SPLICE	LENGTH CHART
BAR	MIN. SPLICE
#4 A1	2′-0″
#4 A2	1'-9"



SECTION N-N



END OF CURB WITHOUT SHOULDER BERM GUTTER

(OMIT TAPER WHEN SHOULDER BERM GUTTER IS REQUIRED)



B-4274 PROJECT NO._ SCOTLAND _ COUNTY 12+68.90 -L-STATION:

SHEET 1 OF 2

STATE OF NORTH CAROLINA DEPARTMENT OF TRANSPORTATION

STANDARD BRIDGE APPROACH SLAB FOR FLEXIBLE PAVEMENT

		REV]	SION	IS		SHEET NO.
NO.	BY:	DATE:	NO.	BY:	DATE:	S-21
1			3			TOTAL SHEETS
2			4			22

25-JUN-2008 15:54 R:\Structures\b4274\FinalPlans\B4274_sd_AS.dgn

€ JOINT—

† SAWED OPENING FOR

FORMED

OPENING

JOINT SEAL

SEE JOINT SEAL DETAILS

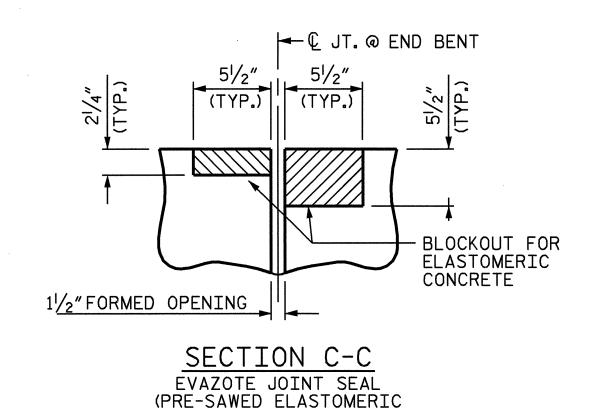
ON "BRIDGE APPROACH SLAB DETAILS" SHEET.

2 LAYERS OF 30 LB. ROOFING FELT TO PREVENT BOND

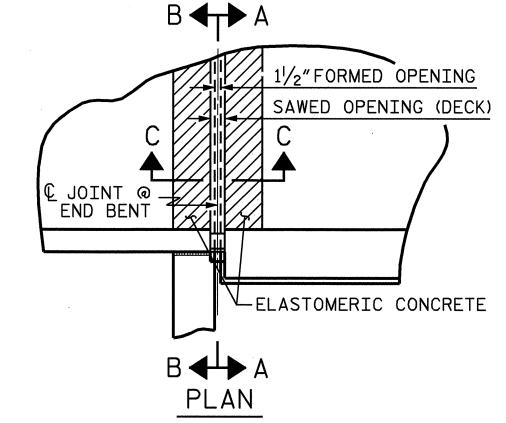
-#78M STONE

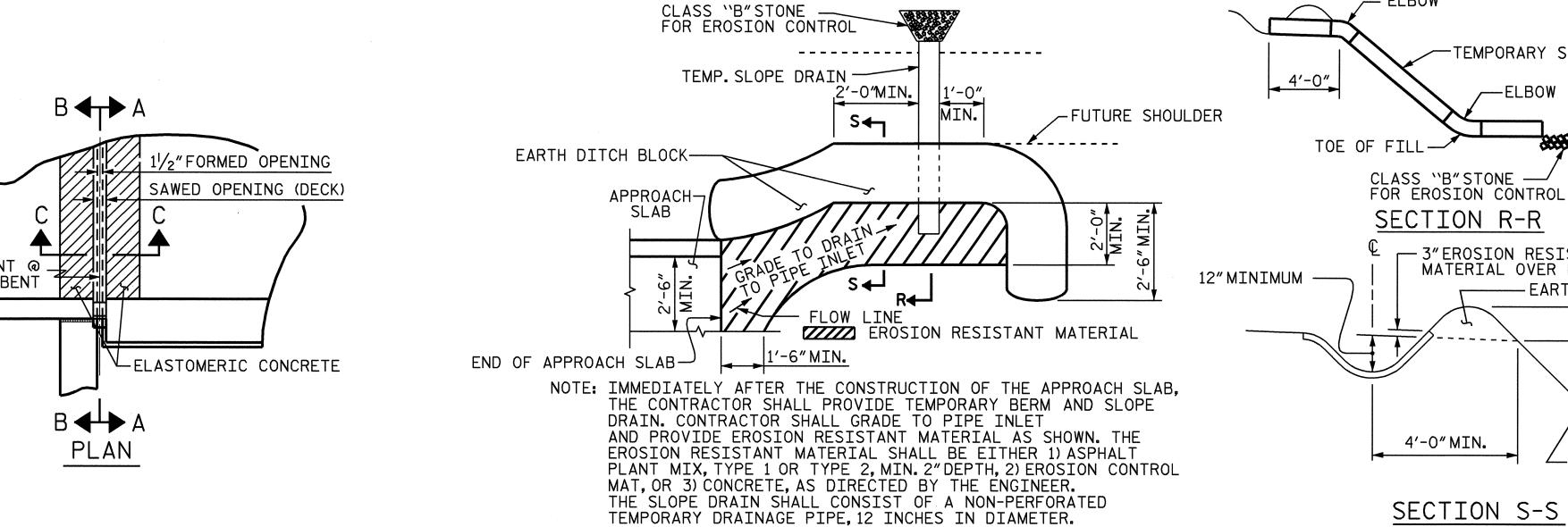
·IMPERMEABLE GEOMEMBRANE

STD. NO. BAS4



CONCRETE DIMENSIONS)

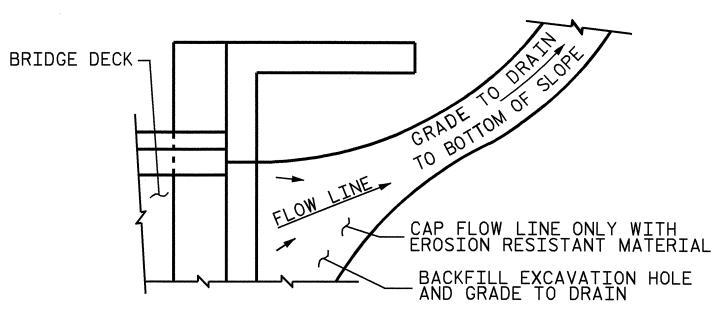




PLAN VIEW

TEMPORARY BERM AND SLOPE DRAIN DETAILS

(TO BE USED WHEN SHOULDER BERM GUTTER IS REQUIRED)



NOTE: IF THE APPROACH SLAB IS NOT CONSTRUCTED IMMEDIATELY AFTER THE BACKFILLING OF THE END BENT EXCAVATION, GRADE TO DRAIN TO THE BOTTOM OF THE SLOPE AND PROVIDE EROSION RESISTANT MATERIAL, SUCH AS FIBERGLASS ROVING OR AS DIRECTED BY THE ENGINEER TO PREVENT SOIL EROSION AND TO PROTECT THE AREA ADJACENT TO THE STRUCTURE. THE CONTRACTOR WILL BE REQUIRED TO REMOVE THESE MATERIALS PRIOR TO CONSTRUCTION OF THE APPROACH SLAB.

TEMPORARY DRAINAGE DETAIL

B-4274 PROJECT NO._ SCOTLAND COUNTY 12+68.90 -L-STATION:

-TEMPORARY SLOPE DRAIN

-EARTH DITCH BLOCK

- FILL SLOPE

-ELBOW

-3"EROSION RESISTANT

MATERIAL OVER PIPE

4'-0" MIN.

SECTION S-S

SHEET 2 OF 2

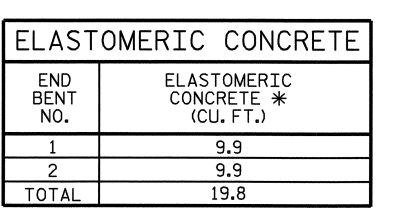
STANDARD

STATE OF NORTH CAROLINA DEPARTMENT OF TRANSPORTATION

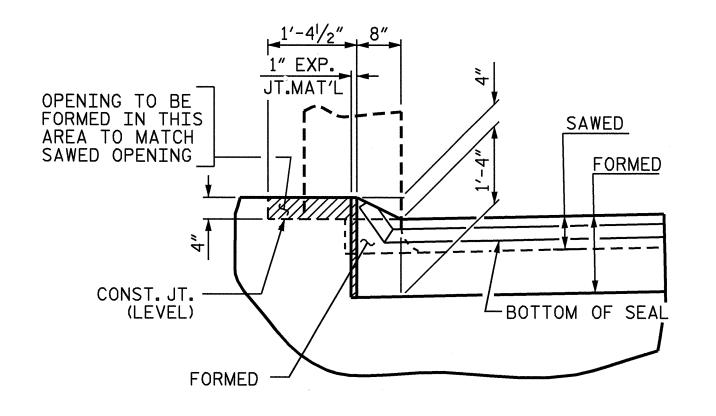
> BRIDGE APPROACH SLAB DETAILS

		REV]	SION	S		SHEET NO.
۶.	BY:	DATE:	NO.	BY:	DATE:	S-22
			3			TOTAL SHEETS
2			4			22

← C JT.@ END BENT 2¹/₈" @ 45° F 2" @ 60° F ← C JT. @ END BENT 1¾′′@90°F 2"SAWED OPENING FOR EVAZOTE JOINT SEAL SAWED OPENING FOR BEVEL AS SHOWN FROM' EVAZOTE JOINT SEAL GUTTER TO GUTTER BEVEL AS SHOWN FROM GUTTER TO GUTTER OPENING TO BE FORMED IN THIS ELASTOMERIC CONCRETE AREA TO MATCH SAWED OPENING CONST. JT. ELASTOMERIC (LEVEL) CONCRETE PARAPET -- ELASTOMERIC CONCRETE 11/2" FORMED OPENING ELASTOMERIC 1/2"FORMED OPENING SECTION C-C 3"MIN.(WILL EXCEED CONCRETE **EVAZOTE JOINT SEAL** 3"IF SEAL DEPTH IS SECTION C-C LARGER THAN 3" (FIXED) EVAZOTE JOINT SEAL (EXPANSION) RADIUS OF SAW BLADE SECTION A-A



* BASED ON THE MINIMUM BLOCKOUT SHOWN.



JOINT SEAL DETAILS @ END BENT

EVAZOTE JOINT SEAL TO BE CUT, HEAT WELDED AND TURNED AS SHOWN IN DETAILS ABOVE.

SECTION B-B

THE JOINT SHALL BE SAWED PRIOR TO THE CASTING OF THE PARAPET.

ASSEMBLED BY : J. MY	DATE : 05/01/07		
CHECKED BY : B. N. GR	DATE : 07/07/07		
DRAWN BY: FCJ CHECKED BY: ARB	11/88 11/88	REV. 10/17/00 REV. 5/7/03 REV. 5/1/06R	RWW/LES RWW/JTE MAA/KMM

STD. NO. BAS10

STANDARD NOTES

DESIGN DATA:

---- A.A.S.H.T.O. (CURRENT) SPECIFICATIONS ---- SEE PLANS LIVE LOAD ---- SEE A.A.S.H.T.O. IMPACT ALLOWANCE STRESS IN EXTREME FIBER OF STRUCTURAL STEEL - AASHTO M270 GRADE 36 20.000 LBS. PER SQ. IN.

- AASHTO M270 GRADE 50W - 27,000 LBS. PER SQ. IN.

- AASHTO M270 GRADE 50 - 27,000 LBS. PER SQ. IN.

REINFORCING STEEL IN TENSION

GRADE 60 - - 24,000 LBS. PER SQ. IN.

1.200 LBS. PER SQ. IN. CONCRETE IN COMPRESSION

---- SEE A.A.S.H.T.O. CONCRETE IN SHEAR

STRUCTURAL TIMBER - TREATED OR

1.800 LBS. PER SQ. IN. UNTREATED - EXTREME FIBER STRESS 17070 4,411 0511 05111 05111

COMPRESSION PERPENDICULAR TO GRAIN OF TIMBER ----

EQUIVALENT FLUID PRESSURE OF EARTH

375 LBS. PER SQ. IN.

30 LBS. PER CU. FT. ----

(MINIMUM)

MATERIAL AND WORKMANSHIP:

EXCEPT AS MAY OTHERWISE BE SPECIFIED ON PLANS OR IN THE SPECIAL PROVISIONS. ALL MATERIAL AND WORKMANSHIP SHALL BE IN ACCORDANCE WITH THE 2002 STANDARD SPECIFICATIONS "FOR ROADS AND STRUCTURES" OF THE N. C. DEPARTMENT OF TRANSPORTATION.

STEEL SHEET PILING FOR PERMANENT OR TEMPORARY APPLICATIONS SHALL BE HOT ROLLED.

CONCRETE:

UNLESS OTHERWISE REQUIRED ON PLANS, CLASS A CONCRETE SHALL BE USED FOR ALL PORTIONS OF ALL STRUCTURES WITH THE EXCEPTION THAT: CLASS AA CONCRETE SHALL BE USED IN BRIDGE SUPERSTRUCTURES, ABUTMENT BACKWALLS, AND APPROACH SLABS; CLASS B CONCRETE SHALL BE USED FOR SLOPE PROTECTION AND RIP RAP; AND CLASS S SHALL BE USED FOR UNDERWATER FOOTING SEALS.

CONCRETE CHAMFERS:

UNLESS OTHERWISE NOTED ON THE PLANS, ALL EXPOSED CORNERS ON STRUCTURES SHALL BE CHAMFERED 3/4" WITH THE FOLLOWING EXCEPTIONS: TOP CORNERS OF CURBS MAY BE ROUNDED TO 1-1/2" RADIUS WHICH IS BUILT INTO CURB FORMS: CORNERS OF TRANSVERSE FLOOR EXPANSION JOINTS SHALL BE ROUNDED WITH A 1/4"FINISHING TOOL UNLESS OTHERWISE REQUIRED ON PLANS: AND CORNERS OF EXPANSION JOINTS IN THE ROADWAY FACES AND TOPS OF CURBS AND SIDEWALKS SHALL BE ROUNDED TO A 1/4"RADIUS WITH A FINISHING STONE OR TOOL UNLESS OTHERWISE REQUIRED ON PLANS.

DOWELS:

DOWELS WHEN INDICATED ON PLANS AS FOR CULVERT EXTENSIONS, SHALL BE EMBEDDED AT LEAST 12" INTO THE OLD CONCRETE AND GROUTED INTO PLACE WITH 1:2 CEMENT MORTAR.

ALLOWANCE FOR DEAD LOAD DEFLECTION, SETTLEMENT, ETC. IN CASTING SUPERSTRUCTURES:

BRIDGES SHALL BE BUILT ON THE GRADE OR VERTICAL CURVE SHOWN ON PLANS. SLABS, CURBS AND PARAPETS SHALL CONFORM TO THE GRADE OR CURVE. ALL DIMENSIONS WHICH ARE GIVEN IN SECTION AND ARE AFFECTED BY DEAD LOAD DEFLECTIONS ARE DIMENSIONS AT CENTER LINE OF BEARING UNLESS OTHERWISE NOTED ON PLANS. IN SETTING FORMS FOR STEEL BEAM BRIDGES AND PRESTRESSED CONCRETE GIRDER BRIDGES, ADJUSTMENTS SHALL BE MADE DUE TO THE DEAD LOAD DEFLECTIONS FOR THE ELEVATIONS SHOWN. WHERE BLOCKS ARE SHOWN OVER BEAMS FOR BUILDING UP TO THE SLAB, THE VERTICAL DIMENSIONS OF THE BLOCKS SHALL BE ADJUSTED BETWEEN BEARINGS TO COMPENSATE FOR DEAD LOAD DEFLECTIONS, VERTICAL CURVE ORDINATE, AND ACTUAL BEAM CAMBER, WHERE BOTTOM OF SLAB IS IN LINE WITH BOTTOM OF TOP FLANGES, DEPTH OF SLAB BETWEEN BEARINGS SHALL BE ADJUSTED TO COMPENSATE FOR DEAD LOAD DEFLECTION, VERTICAL CURVE ORDINATE, AND ACTUAL BEAM CAMBER.

IN SETTING FALSEWORK AND FORMS FOR REINFORCED CONCRETE SPANS, AN ALLOWANCE SHALL BE MADE FOR DEAD LOAD DEFLECTIONS. SETTLEMENT OF FALSEWORK. AND PERMANENT CAMBER WHICH SHALL BE PROVIDED FOR IN ADDITION TO THE ELEVATIONS SHOWN, AFTER REMOVAL OF THE FALSEWORK, THE FINISHED STRUCTURES SHALL CONFORM TO THE PROFILE AND ELEVATIONS SHOWN ON THE PLANS AND CONSTRUCTION ELEVATIONS FURNISHED BY THE ENGINEER.

DETAILED DRAWINGS FOR FALSEWORK OR FORMS FOR BRIDGE SUPERSTRUCTURE AND ANY STRUCTURE OR PARTS OF A STRUCTURE AS NOTED ON THE PLANS SHALL BE SUBMITTED TO THE ENGINEER FOR APPROVAL BEFORE CONSTRUCTION OF THE FALSEWORK OR FORMS IS STARTED.

REINFORCING STEEL:

ALL REINFORCING STEEL SHALL BE DEFORMED WITH THE EXCEPTION OF #2 BARS WHICH MAY BE FABRICATED FROM COLD DRAWN STEEL WIRE. DIMENSIONS RELATIVE TO PLACEMENT OF REINFORCING ARE TO CENTERS OF BARS UNLESS OTHERWISE INDICATED IN THE PLANS. DIMENSIONS ON BAR DETAILS ARE TO CENTERS OF BARS OR ARE OUT TO OUT AS INDICATED ON PLANS.

WIRE BAR SUPPORTS SHALL BE PROVIDED FOR REINFORCING STEEL WHERE INDICATED ON THE PLANS. WHEN BAR SUPPORT PIECES ARE PLACED IN CONTINUOUS LINES, THEY SHALL BE SO PLACED THAT THE ENDS OF THE SUPPORTING WIRES SHALL BE LAPPED TO LOCK LEGS ON ADJOINING PIECES.

STRUCTURAL STEEL:

AT THE CONTRACTOR'S OPTION, HE MAY SUBSTITUTE 7/8" Ø SHEAR STUDS FOR THE $\frac{3}{4}$ " Ø STUDS SPECIFIED ON THE PLANS. THIS SUBSTITUTION SHALL BE MADE AT THE RATE OF 3 - 7/8" Ø STUDS FOR 4 - 3/4" Ø STUDS, AND STUD SPACING CHANGES SHALL BE MADE AS NECESSARY TO PROVIDE THE SAME EQUIVALENT NUMBER OF 7/8" STUDS ALONG THE BEAM AS SHOWN FOR 3/4" Ø STUDS BASED ON THE RATIO OF 3 - 7/8" Ø STUDS FOR 4 - 3/4" Ø STUDS. STUDS OF THE LENGTH SPECIFIED ON THE PLANS MUST BE PROVIDED. THE MAXIMUM SPACING SHALL BE 2'-0".

EXCEPT AT THE INTERIOR SUPPORTS OF CONTINUOUS BEAMS WHERE THE COVER PLATE IS IN CONTACT WITH BEARING PLATE, THE CONTRACTOR MAY, AT HIS OPTION, SUBSTITUTE FOR THE COVER PLATES DESIGNATED ON THE PLANS COVER PLATES OF THE EQUIVALENT AREA PROVIDED THESE PLATES ARE AT LEAST 5/16" IN THICKNESS AND DO NOT EXCEED A WIDTH EQUAL TO THE FLANGE WIDTH LESS 2"OR A THICKNESS EQUAL TO 2 TIMES THE FLANGE THICKNESS. THE SIZE OF FILLET WELDS SHALL CONFORM TO THE REQUIREMENTS OF THE CURRENT ANSI/AASHTO/AWS "BRIDGE WELDING CODE". ELECTROSLAG WELDING WILL NOT BE PERMITTED.

PLACEMENT OF BEAM OR GIRDER MEMBERS ON TRUCKS FOR HAULING SHALL BE DONE IN COMPLIANCE WITH LIMITS SHOWN ON SKETCHES PROVIDED TO THE MATERIALS AND TEST UNIT APPROVED BY THE STRUCTURE DESIGN UNIT DATED MAY 8, 1991. THESE SKETCHES PRIMARILY LIMIT THE UNSUPPORTED CANTILEVER LENGTH OF MEMBERS. WHEN THE CONTRACTOR WISHES TO PLACE MEMBERS ON TRUCKS NOT IN ACCORDANCE WITH THESE LIMITS, TO SHIP BY RAIL, TO ATTACH SHIPPING RESTRAINTS TO THE MEMBERS OR TO INVERT MEMBERS. HE SHALL SUBMIT A SKETCH FOR APPROVAL PRIOR TO SHIPPING. SEE ALSO ARTICLE 1072-11.

WITH THE SOLE EXCEPTION OF EDGES AT SURFACES WHICH BEAR ON OTHER SURFACES, ALL SHARP EDGES AND ENDS OF SHAPES AND PLATES SHALL BE SLIGHTLY ROUNDED BY SUITABLE MEANS TO A RADIUS OF APPROXIMATELY 1/16 INCH OR EQUIVALENT FLAT SURFACE AT A SUITABLE ANGLE PRIOR TO PAINTING, GALVANIZING, OR METALLIZING.

HANDRAILS AND POSTS:

METAL STANDARDS AND FACES OF THE CONCRETE END POSTS FOR THE METAL RAIL SHALL BE SET NORMAL TO THE GRADE OF THE CURB, UNLESS OTHERWISE SHOWN ON PLANS. THE METAL RAIL AND TOPS OF CONCRETE POSTS USED WITH THE ALUMINUM RAIL SHALL BE BUILT PARALLEL TO THE GRADE OF THE CURB. METAL HANDRAILS SHALL BE IN ACCORDANCE WITH THE PLANS. RAILS SHALL BE AS MANUFACTURED FOR BRIDGE RAILING. CASTINGS SHALL BE OF A UNIFORM APPEARANCE. FINS AND OTHER DEFORMATIONS RESULTING FROM CASTING OR OTHERWISE SHALL BE

REMOVED IN A MANNER SO THAT A UNIFORM COLORING OF THE COMPLETED CASTING SHALL

NOT BE ACCEPTED. CERTIFIED MILL REPORTS ARE REQUIRED FOR METAL RAILS AND POSTS.

SPECIAL NOTES:

GENERALLY, IN CASE OF DISCREPANCY, THIS STANDARD SHEET OF NOTES SHALL GOVERN OVER THE SPECIFICATIONS, BUT THE REMAINDER OF THE PLANS SHALL GOVERN OVER NOTES HEREON, AND SPECIAL PROVISIONS SHALL GOVERN OVER ALL. SEE SPECIFICATIONS ARTICLE 105-4.

BE OBTAINED. CASTINGS WITH DISCOLORATIONS OR OF NON-UNIFORM COLORING WILL

STD. NO. SN