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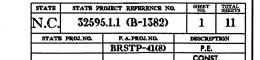
## STATE OF NORTH CAROLINA

DEPARTMENT OF TRANSPORTATION

**DIVISION OF HIGHWAYS** GEOTECHNICAL UNIT

## **STRUCTURE** SUBSURFACE INVESTIGATION

STATE PROJECT 32595.1.1 I.D. NO. **B-1382** F.A. PROJECT BRSTP-41 (8) COUNTY SAMPSON PROJECT DESCRIPTION \_BRIDGE 12 ON NC 41 OVER BLACK RIVER AT -L-**STATION** 18+07.50

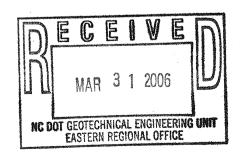


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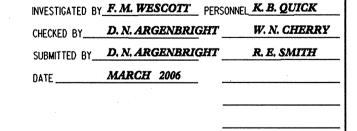
GENERAL SOIL AND ROCK STRATA DESCRIPTIONS AND INDICATED BOUNDARIES ARE BASED ON A
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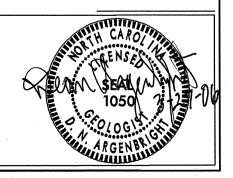
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DRAWN BY: C. M. KENT

### NORTH CAROLINA DEPARTMENT OF TRANSPORTATION

### DIVISION OF HIGHWAYS

GEOTECHNICAL ENGINEERING UNIT

### SUBSURFACE INVESTIGATION

### SOIL AND ROCK LEGEND, TERMS, SYMBOLS, AND ABBREVIATIONS

SOIL DESCRIPTION ROCK DESCRIPTION

HARD ROCK IS NON-COASTAL PLAIN MATERIAL THAT IF TESTED, WOULD YIELD SPT REFUSAL, AN INFERRED TERMS AND DEFINITIONS WELL GRADED - INDICATES A GOOD REPRESENTATION OF PARTICLE SIZES FROM FINE TO COARSE UNIFORM - INDICATES THAT SOIL PARTICLES ARE ALL APPROXIMATELY THE SAME SIZE. (ALSO SOIL IS CONSIDERED TO BE THE UNCONSOLIDATED, SEMI-CONSOLIDATED, OR WEATHERED EARTH MATERIALS THAT CAN BE PENETRATED WITH A CONTINUOUS FLIGHT POWER AUGER, AND YIELD LESS THAN 180 BLOWS PER FOOT ACCORDING TO STANDARD PENETRATION TEST (AASHTO 1206, ASTM D-1586). SOIL MARCH LINE INDICATES THE LEVEL AT WHICH NON-COSTAL PLAIN MATERIAL VOULD YELD SPT REFUSAL.

SPT REFUSAL IS PENETRATION BY A SPLIT SPOON SAMPLER EQUAL TO OR LESS THAN 0.1 FOOT PER 60 BLOWS. ALLUVIUM (ALLUV.) - SOILS THAT HAVE BEEN TRANSPORTED BY WATER. AQUIFER - A WATER BEARING FORMATION OR STRATA. GAP-GRADED - INDICATES A MIXTURE OF UNIFORM PARTICLES OF TWO OR MORE SIZES. IN NON-COASTAL PLAIN MATERIAL. THE TRANSITION BETWEEN SOIL AND ROCK IS OFTEN REPRESENTED BY A ZONE CLASSIFICATION IS BASED ON THE AASHTO SYSTEM, BASIC DESCRIPTIONS GENERALLY SHALL INCLUDE. ANGULARITY OF GRAINS OF WEATHERED POC ARENACEOUS - APPLIED TO ROCKS THAT HAVE BEEN DERIVED FROM SAND OR THAT CONTAIN SAND. CONSISTENCY, COLOR, TEXTURE, MOISTURE, AASHTO CLASSIFICATION, AND OTHER PERTINENT FACTORS SUCH ROCK MATERIALS ARE TYPICALLY DIVIDED AS FOLLOWS: AS MINERALOGICAL COMPOSITION, ANGULARITY, STRUCTURE, PLASTICITY, ETC. EXAMPLE: THE ANGULARITY OR ROUNDNESS OF SOIL GRAINS IS DESIGNATED BY THE TERMS: ANGULAR, ARGILLACEOUS - APPLIED TO ALL ROCKS OR SUBSTANCES COMPOSED OF CLAY MINERALS. VERY STIFF, GRAY, SUTY CLAY, MOIST WITH INTERBEDDED FINE SAND LIVERS, HIGHLY PLASTIC, A-7-6 SUBANGULAR, SUBROUNDED, OR ROUNDED. WEATHERED NON-COASTAL PLAIN MATERIAL THAT WOULD YIELD SPT N VALUES > 100 BLOWS PER FOOT IF TESTED. R HAVING A NOTABLE PROPORTION OF CLAY IN THEIR COMPOSITION, AS SHALE, SLATE, ETC. ARTESIAN - GROUND WATER THAT IS UNDER SUFFICIENT PRESSURE TO RISE ABOVE THE LEVEL SOIL LEGEND AND AASHTO CLASSIFICATION MINERALOGICAL COMPOSITION FINE TO COARSE GRAIN IGNEOUS AND METAMORPHIC POCK THAT WHICH IT IS ENCOUNTERED, BUT WHICH DOES NOT NECESSARILY RISE TO DR ABOVE THE GENERAL MINERAL NAMES SUCH AS QUARTZ, FELDSPAR, MICA, TALC, KAOLIN, ETC. ARE USED IN DESCRIPTIONS WHENEVER THEY ARE CONSIDERED OF SIGNIFICANCE. WOULD YIELD SPT REFUSAL IF TESTED. ROCK TYPE INCLUDES GRANITE, GROUND SURFACE. CLASS. (≤ 35% PASSING #200) > 35% PASSING \*200) CALCAREOUS (CALC.) - SOILS THAT CONTAIN APPRECIABLE AMOUNTS OF CALCIUM CARBONATE. ONEISS, GABBRO, SCHIST, ETC.
FINE TO COARSE GRAIN METAMORPHIC AND NON-COASTAL PLAIN
SEDIMENTARY ROCK THAT WOULD VEILD SPT REFUSAL IF TESTED, ROCK TYPE
INCLUDES PHYLLITE, SLATE, SANDSTONE, ETC.
COASTAL PLAIN SEDIMENTS CEMENTED INTO ROCK, BUT MAY NOT YIELD A-4 A-5 A-6 A-7 GROUP A-1 A-3 Δ-2 A-1, A-2 A-4, A-5 NON-CRYSTALLINE ROCK (NCR) COLLUVIUM - ROCK FRAGMENTS MIXED WITH SOIL DEPOSITED BY GRAVITY ON SLOPE OR AT BOTTOM CLASS. A-3 A-6, A-7 SLIGHTLY COMPRESSIBLE LIDUID LIMIT LESS THAN 31 MODERATELY COMPRESSIBLE LIQUID LIMIT FOUND TO 31-50 COASTAL PLAIN SEDIMENTARY ROCK SYMBOL <u>CORE RECOVERY (REC.)</u> - TOTAL LENGTH OF ALL MATERIAL RECOVERED IN THE CORE BARREL DIVIDED BY TOTAL LENGTH OF CORE RUN AND EXPRESSED AS A PERCENTAGE. HIGHLY COMPRESSIBLE SPT REFUSAL. ROCK TYPE INCLUDES LIMESTONE, SANDSTONE, CEMENTED PASSIN PERCENTAGE OF MATERIAL SILT-WEATHERING DIKE - A TABULAR BODY OF IGNEOUS ROCK THAT CUTS ACROSS THE STRUCTURE OF ADJACENT SILT - CLA CLAY SOILS ORGANIC MATERIAL PEAT OTHER MATERIAL BOCKS OR CUTS MASSIVE BOCK SOILS SOILS SOILS ROCK FRESH, CRYSTALS BRIGHT, FEW JOINTS MAY SHOW SLIGHT STAINING, ROCK RINGS UNDER HAMMER IF CRYSTALLINE. FRESH RACE OF ORGANIC MATTER 3 - 5% DIP - THE ANGLE AT WHICH A STRATUM OR ANY PLANAR FEATURE IS INCLINED FROM THE 2 - 32 1 - 10% LITTLE 10 - 20% VERY SLIGHT ROCK GENERALLY FRESH, JOINTS STAINED, SOME JOINTS MAY SHOW THIN CLAY COATINGS IF OPEN, (V SLI,) CRYSTALS ON A BROKEN SPECIMEN FACE SHINE BRIGHTLY, ROCK RINGS UNDER HAMMER BLOWS IF SOILS WITH MODERATELY ORGANIC 5 - 10% 12 - 20% ASTIC INDEX 6 MX DIP DIRECTION (DIP AZIMUTH) - THE DIRECTION OR BEARING OF THE HORIZONTAL TRACE OF IGHLY ORGANIC 35% AND ABOVE HE LINE OF DIP, MEASURED CLOCKWISE FROM NORTH. OF A CRYSTALLINE NATURE. GROUP INDEX а a 8 4 MX 8 MX 12 MX 16 MX No M MODERATE GROUND WATER FAULT - A FRACTURE OR FRACTURE ZONE ALONG WHICH THERE HAS BEEN DISPLACEMENT OF THE AMOUNTS OF ROCK GENERALLY FRESH, JOINTS STAINED AND DISCOLORATION EXTENDS INTO BOCK UP TO SOILS SUAL TYPES STONE FRAGS. SIDES RELATIVE TO ONE ANOTHER PARALLEL TO THE FRACTURE. SILTY OR CLAYEY WATER LEVEL IN BORE HOLE IMMEDIATELY AFTER DRILLING FINE SILTY 1 Inch open joints may contain clay, in granitoid rocks some occasional felospar Crystals are dull and discolored, crystalline rocks ring under hammer blows. CLAYEY (SLI.) OF MAJOR GRAVEL AND GRAVEL AND SAND SOILS MATTER FISSILE - A PROPERTY OF SPLITTING ALONG CLOSELY SPACED PARALLEL PLANES. TERIALS STATIC WATER LEVEL AFTER 24 HOURS SIGNIFICANT PORTIONS OF ROCK SHOW DISCOLORATION AND WEATHERING EFFECTS. IN MODERATE CEN. ROTING FLOAT - ROCK FRAGMENTS ON SURFACE NEAR THEIR ORIGINAL POSITION AND DISLODGED FROM PARENT MATERIAL. **∇**P₩ FAIR TO GRANITOID ROCKS, MOST FELDSPARS ARE DULL AND DISCOLORED, SOME SHOW CLAY, ROCK HAS DULL SOUND UNDER HAMMER BLOWS AND SHOWS SIGNIFICANT LOSS OF STRENGTH AS COMPARED PERCHED WATER, SATURATED ZONE, OR WATER BEARING STRATA EXCELLENT TO GOOD FAIR TO POOR POOR AS A HINSHITAR POOR Oull-LOOD PLAIN (FP) - LAND BORDERING A STREAM, BUILT OF SEDIMENTS DEPOSITED BY HE STREAM. SPRING OR SEER WITH FRESH ROCK. PI OF A-7-5 SUBGROUP IS ≤ LL - 30 ; PI OF A-7-6 SUBGROUP IS > LL - 30 ALL ROCK EXCEPT QUARTZ DISCOLORED OR STAINED. IN GRANITOID ROCKS, ALL FELDSPARS DULL MODERATEL Y CONSISTENCY OR DENSENESS MISCELLANEOUS SYMBOLS IND DISCOLORED AND A MAJORITY SHOW KAOLINIZATION. ROCK SHOWS SEVERE LOSS OF STRENGTH FORMATION (FM.) - A MAPPABLE GEOLOGIC UNIT THAT CAN BE RECOGNIZED AND TRACED IN RANGE OF STANDARD PENETRATION RESISTENCE MOD. SEV.) AND CAN BE EXCAVATED WITH A GEOLOGIST'S PICK. ROCK GIVES "CLUNK" SOUND WHEN STRUCK. COMPACTNESS OR ROADWAY EMBANKMENT (RE) PRIMARY SOIL TYPE DPT DNT TEST BORING IF TESTED. WOULD YIELD SPT REFUSAL DESIGNATIONS JOINT - FRACTURE IN ROCK ALONG WHICH NO APPRECIABLE MOVEMENT HAS OCCURRED. (N-VALUE) WITH SOIL DESCRIPTION ALL ROCK EXCEPT QUARTZ DISCOLORED OR STAINED, ROCK FABRIC CLEAR AND EVIDENT BUT REDUCED S - BULK SAMPLE SEVERE VERY LOOSE LEDGE - A SHELF-LIKE RIDGE OR PROJECTION OF ROCK WHOSE THICKNESS IS SMALL COMPARED TO  $\oplus$ IN STRENGTH TO STRONG SOIL. IN GRANITOID ROCKS ALL FELDSPARS ARE KAOLINIZED TO SOME EXTENT. SOME FRAGMENTS OF STRONG ROCK USUALLY REMAIN. GENERALLY SOIL SYMBOL AUGER BORING 4 TO 10 SS - SPLIT SPOON ITS LATERAL EXTENT. MEDIUM DENSE 10 TO 30 ARTIFICIAL FILL (AF) OTHER LENS - A BODY OF SOIL OR ROCK THAT THINS OUT IN ONE OR MORE DIRECTIONS. MATERIAL SAMPLE IF TESTED, YIELDS SPT N VALUES > 100 BPF DENSE CORE BORING (NON-COHESIVE) THAN ROADWAY EMBANKMENT VERY DENSE VERY SEVERE ALL ROCK EXCEPT QUARTZ DISCOLORED OR STAINED. ROCK FABRIC ELEMENTS ARE DISCERNIBLE BUT MOTILED (MOT.) - IRREGULARLY MARKED WITH SPOTS OF DIFFERENT COLORS. MOTTLING IN SOILS USUALLY INDICATES POOR AERATION AND LACK OF GOOD DRAINAGE. ST - SHELBY TUBE >50 RAMPI F THE MASS IS EFFECTIVELY REDUCED TO SOIL STATUS, WITH ONLY FRAGMENTS OF STRONG BOCK INFERRED SOIL BOUNDARY VERY COET (0.25 MONITORING WELL REMAINING. SAPROLITE IS AN EXAMPLE OF ROCK WEATHERED TO A DEGREE SUCH THA <u>PERCHED WATER</u> - WATER MAINTAINED ABOVE THE NORMAL GROUND WATER LEVEL BY THE PRESENCE OF AI INTERVENING IMPERVIOUS STRATUM. (MW) 2 TO 4 GENERALLY 0.25 TO 0.50 0.5 TO 1.0 RM - RESILIENT MODULUS VESTIGES OF THE ORIGINAL ROCK FABRIC REMAIN. IF TESTED, YIELDS SPT N VALUES ( 100 BPF INFERRED ROCK LINE MEDIUM STIFF SILT-CLAY 4 TO 8 PIEZOMETER ROCK REDUCED TO SOIL. ROCK FABRIC NOT DISCERNIBLE OR DISCERNIBLE ONLY IN SMALL AND SCATTERED CONCENTRATIONS. QUARTZ MAY BE PRESENT AS DIKES OR STRINGERS, SAPROLITE IS Δ STIFF COMPLETE RESIDUAL (RES.) SOIL - SOIL FORMED IN PLACE BY THE WEATHERING OF ROCK. INSTALLATION 1 TO 2 VERY STIFF RS - ROCK SAMPLE COHESTVE 15 TO 30 ROCK QUALITY DESIGNATION (ROD) - A MEASURE OF ROCK QUALITY DESCRIBED BY TOTAL LENGTH OF ROCK SEGMENTS EQUAL TO OR GREATER THAN 4 INCHES DIVIDED BY THE TOTAL LENGTH OF CORE RUN AN SLOPE INDICATOR HARD  $\bigcirc$ DIP & DIP DIRECTION OF RT - RECOMPACTED TRIAXIA INSTALLATION ROCK STRUCTURES ROCK HARDNESS TEXTURE OR GRAIN SIZ SAMPLE EXPRESSED AS A PERCENTAGE. SPT N-VALUE CBR - CALIFORNIA BEARING CANNOT BE SCRATCHED BY KNIFE OR SHARP PICK, BREAKING OF HAND SPECIMENS REQUIRES SAPROLITE (SAP.) - RESIDUAL SOIL THAT RETAINS THE RELIC STRUCTURE OR FABRIC OF THE VERY HARD SOUNDING ROD U.S. STD. SIEVE SIZE REF SPT REFUSAL RATIO SAMPLE SEVERAL HARD BLOWS OF THE GEOLOGIST'S PICK 0.075 0.42 SILL - AN INTRUSIVE BODY OF IGNEOUS ROCK OF APPROXIMATELY UNIFORM THICKNESS AND **ABBREVIATIONS** CAN BE SCRATCHED BY KNIFE OR PICK ONLY WITH DIFFICULTY, HARD HAMMER BLOWS REQUIRED RELATIVELY THIN COMPARED WITH ITS LATERAL EXTENT, THAT HAS BEEN EMPLACED PARALLEL TO THE BEDDING OR SCHISTOSITY OF THE INTRUDED ROCKS. CUVBCE COBBLE (COB.) TO DETACH HAND SPECIMEN. BOULDER GRAVE - MOISTURE CONTENT AR - AUGER REFUSAL HT. - HTGHI Y (BLDR.) (GR.) (\$1.) (CL.) MODERATE: Y CAN BE SCRATCHED BY KNIFE OR PICK. GOUGES OR GROOVER TO 0.25 INCHES DEEP CAN BE - BORING TERMINATED MED. - MEDIUM V - VERY SLICKENSIDE - POLISHED AND STRIATED SURFACE THAT RESULTS FROM FRICTION ALONG A FAULT OR CL. - CLAY MICA. - MICACEOUS VST - VANE SHEAR TEST EXCAVATED BY HARD BLOW OF A GEOLOGIST'S PICK, HAND SPECIMENS CAN BE DETACHED GRAIN MM 305 SIZE IN. 12 2.0 0.25 0.05 0.005 CPT - CONE PENETRATION TEST BY MODERATE BLOWS. MOD. - MODERATELY WEA. - WEATHERED STANDARD PENETRATION TEST (PENETRATION RESISTANCE) (SPT) - NUMBER OF BLOWS (N OR BPF) OF A 140 LB. HAMMER FALLING 30 INCHES REQUIRED TO PRODUCE A PENETRATION OF 1 FOOT INTO SOIL WITH A 2 INCH OUTSIDE DIAMETER SPLIT SPOON SAMPLER. SPT REFUSAL IS PENETRATION EQUAL TO OR LESS NP - NON PLASTIC CAN BE GROOVED OR GOUGED 0.05 INCHES DEEP BY FIRM PRESSURE OF KNIFE OR PICK POINT. CSE. - COARSE  $\gamma$  - UNIT WEIGHT MEDIUM SOIL MOISTURE - CORRELATION OF TERMS - DILATOMETER TEST ORG. - ORGANIC 7,- DRY UNIT WEIGHT HARD CAN BE EXCAVATED IN SMALL CHIPS TO PEICES I INCH MAXIMUM SIZE BY HARD BLOWS OF THI DPT - DYNAMIC PENETRATION TEST PMT - PRESSIREMETER TEST POINT OF A GEOLOGIST'S PICK. SOIL MOISTURE SCALE FIELD MOISTURE GUIDE FOR FIELD MOISTURE DESCRIPTION THAN 0.1 FOOT PER 60 BLOWS. - VOID RATIO CAN BE GROVED OR GOUGED READILY BY KNIFE OR PICK. CAN BE EXCAVATED IN FRAGMENTS (ATTERBERG LIMITS) SAP. - SAPROLITIC SOFT F - FINE SD - SAND SANDS <u>STRATA CORE RECOVERY (SREC.)</u> - TOTAL LENGTH OF STRATA MATERIAL RECOVERED DIVIDED BY TOTAL LENGTH OF STRATUM AND EXPRESSED AS A PERCENTAGE, FROM CHIPS TO SEVERAL INCHES IN SIZE BY MODERATE BLOWS OF A PICK POINT, SMALL, THIN FOSS. - FOSSILIFEROUS - SATURATED HIGHALLY LITCUITO, VERY WET HIGHALLY SL. - SILT, SILTY PIECES CAN BE BROKEN BY FINGER PRESSURE. FRAC - FRACTURED FRACTURES SLI. - SLIGHTLY (SAT.) FROM BELOW THE GROUND WATER TABLE STRATA ROCK QUALITY DESIGNATION (SROD) - A MEASURE OF ROCK QUALITY DESCRIBED BY CAN BE CARVED WITH KNIFE. CAN BE EXCAVATED READILY WITH POINT OF PICK. PIECES 1 INCH LIQUID LIMIT FRAGS. - FRAGMENTS TAL LENGTH OF ROCK SEGMENTS WITHIN A STRATUM EQUAL TO OR GREATER THAN 4 INCHES DIVIDED THE TOTAL LENGTH OF STRATA AND EXPRESSED AS A PERCENTAGE. TCR - TRICONE REFUSAL OR MORE IN THICKNESS CAN BE BROKEN BY FINGER PRESSURE, CAN BE SCRATCHED READILY BY LASTIC SEMISOLID: REQUIRES DRYING TO FINGERNAU RANGE - WET - (W) EQUIPMENT USED ON SUBJECT PROJECT TOPSOIL (TS.) - SURFACE SOILS USUALLY CONTAINING DRGANIC MATTER. ATTAIN OPTIMUM MOISTURE FRACTURE SPACING BEDDING PLASTIC LIMIT TERM THICKNESS **IERM** SPACING ADVANCING TOOLS DRILL UNITS: BENCH MARK: BM# IR.R. SPIKE IN 12" PINE TREE AT -BL- STATION 10+31, 60'L VERY THICKLY BEDDED > 4 FEET - MDIST - (M) SOLID; AT OR NEAR OPTIMUM MOISTURE VERY WIDE MORE THAN 10 FEET OPTIMUM MOISTURE AUTOMATIC \_\_\_ MANUAL CLAY BITS THICKLY REDDED 1.5 - 4 FFFT MOBILE B-\_\_\_ 3 TO 10 FEET SHRINKAGE LIMIT 0.16 - 1.5 FEET ELEVATION: 46.08 FT. MODERATELY CLOSE 1 TO 3 FEET 6 CONTINUOUS FLIGHT AUGER VERY THINLY REDDED 0.03 - 0.16 FFFT REQUIRES ADDITIONAL WATER TO CORE SIZE: 0.16 TO 1 FEET - DRY - (D) BK-51 THICKLY LAMINATED
THINLY LAMINATED NOTES: VERY CLOSE ATTAIN OPTIMUM MOISTURE 8" HOLLOW AUGERS LESS THAN DUE FEET \_\_\_-B\_\_\_\_ PLASTICI INDURATION HARD FACED FINGER BITS \_\_\_ CME-45C \_\_\_\_\_N\_\_\_\_ FOR SEDIMENTARY ROCKS, INDURATION IS THE HARDENING OF THE MATERIAL BY CEMENTING, HEAT, PRESSURE, ETC. PLASTICITY INDEX (PI DRY STRENGTH TUNG.-CARBIDE INSERTS \_\_-H\_\_\_\_ NONPLASTIC VERY LOW 0-5 \_\_ CME-550 RUBBING WITH FINGER FREES NUMEROUS GRAINS LOW PLASTICITY FRIABLE 6-15 SLIGHT CASING \_ W/ ADVANCER GENTLE BLOW BY HAMMER DISINTEGRATES SAMPLE. HAND TOOLS: MED. PLASTICITY MEDIUN 16-25 \_ PORTABLE HOIST TRICONE 215/6 STEEL TEETH HIGH PLASTICIT GRAINS CAN BE SEPARATED FROM SAMPLE WITH STEEL PROBE: POST HOLE DIGGER 26 OR MORE HIGH MODERATELY INDURATED BREAKS EASILY WHEN HIT WITH HAMMER \_\_ TRICONE \_\_\_ HAND AUGER \* TUNG.-CARB. OTHER CME-45B GRAINS ARE DIFFICULT TO SEPARATE WITH STEEL PROBE: SOLINDING ROD CORE BIT DESCRIPTIONS MAY INCLUDE COLOR OR COLOR COMBINATIONS (TAN, RED, YELLOW-BROWN, BLUE-GRAY). DIFFICULT TO BREAK WITH HAMMER. VANE SHEAR TEST MODIFIERS SUCH AS LIGHT, DARK, STREAKED, ETC. ARE USED TO DESCRIBE APPEARANCE. \_\_ OTHER\_ \_ OTHER SHARP HAMMER BLOWS REQUIRED TO BREAK SAMPLE; EXTREMELY INDURATED OTHER SAMPLE BREAKS ACROSS GRAINS.

PROJECT REFERENCE NO.

32595,LI(B-I382)

SHEET NO.

2





### STATE OF NORTH CAROLINA DEPARTMENT OF TRANSPORTATION

MICHAEL F. EASLEY GOVERNOR

LYNDO TIPPETT SECRETARY

March 29, 2006

STATE PROJECT:

32595.1.1 B-1382

F. A. PROJECT:

BRSTP-41(8)

COUNTY:

Sampson

**DESCRIPTION:** 

Bridge No. 12 on NC 41 (Tomahawk Rd) over Black River

SUBJECT:

Geotechnical Report - Bridge Foundation Investigation for

NC 41 (Tomahawk Rd) over Black River at -L- Station

18+07.50

### **Site Description**

The proposed bridge site is located at the existing NC 41 bridge over Black River approximately 4± miles east of Harrells. The replacement structure will be constructed along the existing NC 41 alignment. Based on the proposed design, the new structure will have four spans with a total length of 265 feet. The bents will have a skew of 90 degrees.

One Standard Penetration Test (SPT) boring was made at or near each proposed bent location to provide subsurface information relative to foundation design. The borings were made with ATV mounted CME-45B drill machine and were advanced by rotary drill methods using bentonite drilling fluid.

The bridge site is located in the Coastal Plain Physiographic Province and is underlain by Recent alluvial deposits, upland soils, and Cretaceous age soils of the Black Creek Formation. Topography at the site is nearly flat to gentle sloping. Elevations at the site range from 16± feet along the channel bed to 46± feet along the existing NC 41 roadway. Artesian flow was noted at the site in the borings at Bents 2, 3, 4, and End Bent 2. The hydraulic head was measured at an elevation of 28± feet. The natural ground water at End Bent 1 and Bent 1 occurs at an elevation of 36± feet. The true water levels east of the river will generally match the stream flow line. The surface of Black River was at an elevation of 23± feet.

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LOCATION: CENTURY CENTER COMPLEX ENTRANCE B-2 1020 BIRCH RIDGE DRIVE RALEIGH NC

### **Soil Description**

Subsurface conditions at the site are relatively uniform. Surficial alluvial soils generally consist of very soft to soft sandy silt (A-4) and very loose to loose sand (A-2-4) with trace to moderate amounts of organics as well as very loose to loose sand (A-1-b, A-2-4, A-3) east of the river. Tested organic samples within the silts and sands ranged from 0.8 to 5 percent. A 3± to 7± foot thick upland deposit was noted at End Bent 1 and Bent 1. The upland soils consist of very loose sand (A-1-b, A-3). Soils belonging to the Cretaceous age Black Creek Formation underlie the alluvial and upland deposits at elevations ranging from 12± to 30± feet. Soils of the Black Creek Formation consist of loose to very dense sand (A-2-4, A-2-6, A-3).

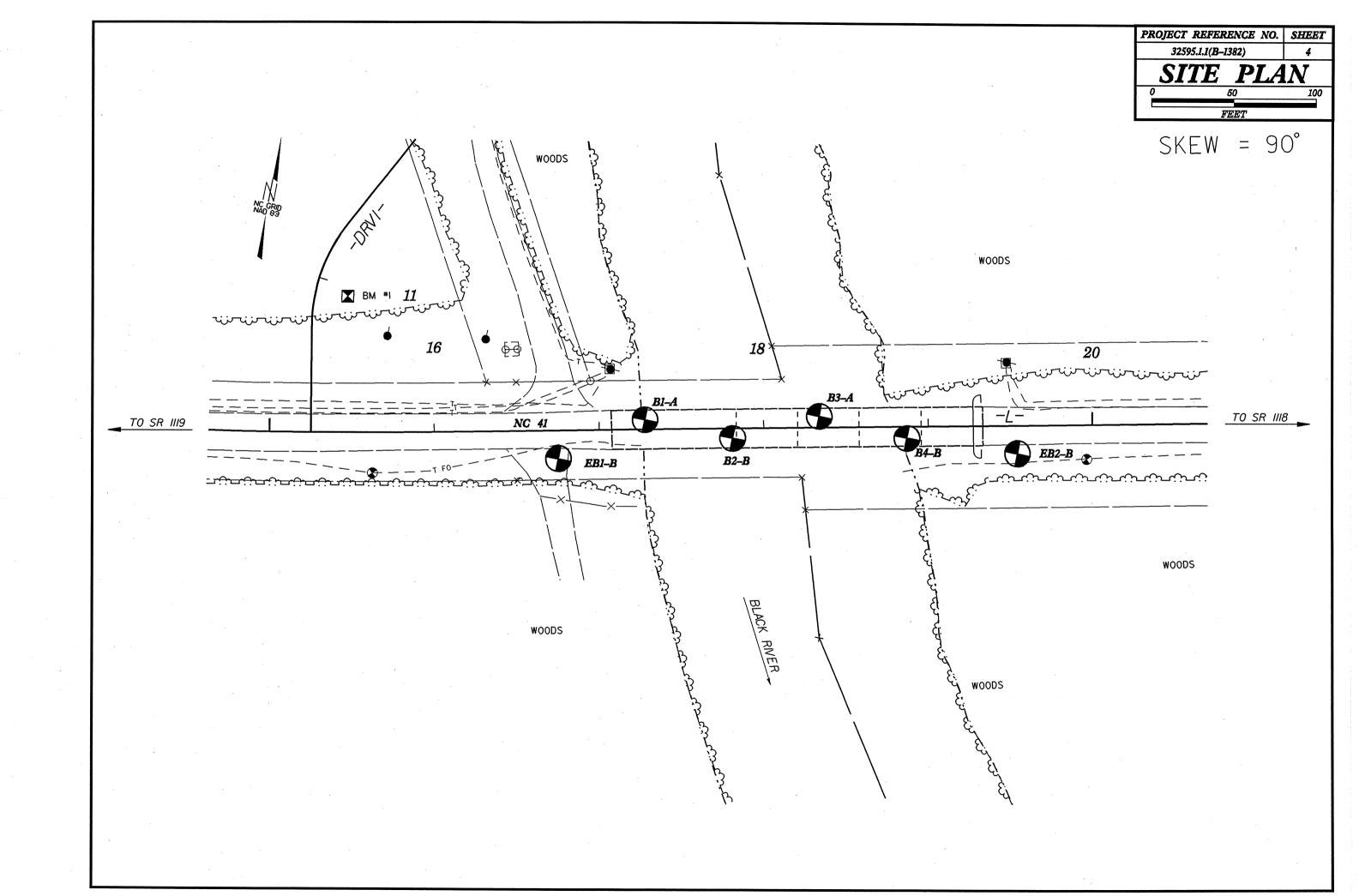
Based on the proposed design, the existing grade will be raised 1± foot at the proposed bridge site. The existing roadway embankment at the end bents consists of 3± to 7± feet of very loose to loose sand (A-3). The proposed end bent slopes will be mainly constructed within the existing embankment. Some additional fill will be required for construction of the end bent and side slopes. Borrow meeting Coastal Plain criteria is available in nearby areas.

The Geotechnical foundation report is based on the Bridge Survey and Hydraulic Design Report dated October 19, 2005. If significant changes are made in the design or location of the proposed structure, the subsurface information should be reviewed and modified as necessary.

Prepared By:

Fred M. Wescott III

Project Engineering Geologist



# NORTH CAROLINA DEPARTMENT OF TRANSPORTATION GEOTECHNICAL UNIT BORING LOG

PROJECT NO. 32595.1.1 **ID.** B-1382 COUNTY SAMPSON GEOLOGIST K. B. QUICK SITE DESCRIPTION BRIDGE NO. 12 ON NC 41 OVER BLACK RIVER GROUND WATER BORING LOCATION BORING NO. EBI-B 16+75 OFFSET 17' RT ALIGNMENT -L-OHR. N/A COLLAR ELEVATION 45.5' NORTHING 351265 **EASTING** 2218433 24 HR. 9.0' TOTAL DEPTH 60.1' DRILL MACHINE CME-45B DRILL METHOD ROTARY W/MUD HAMMER TYPE AUTOMATIC START DATE 11/08/05 COMPLETION DATE 11/08/05 SURFACE WATER DEPTH N/A DEPTH TO ROCK N/A DEPTH BLOW COUNT PEN. BLOWS PER FOOT SAMPLE. SOIL AND ROCK 100 NUMBER MOI. G 10.510.510.5(FT.) (FT.) DESCRIPTION 45.0 0.0 2 5 3 1.0 BROWN TAN SAND, MOIST TO SATURATED (ROADWAY EMBANKMENT) 2 2 1.0 4.1 SS-I 40.0 GRAY SAND. MOIST TO SATURATED Y 10 | 8 | 1.0 8.6 6 35.0 3 | 2 6 1.0 13.6 SS-2 30.0 7 | 12 | 1.0 18.6 | 5 SS-3 25.0 23.6 13 9 12 1.0 20.0 28.6 6 9 | 13 | 1.0 15.0 GREEN GRAY SAND. 7 | 10 | 1.0 SATURATED 33.6 5 SS-4 10.0 (BLACK CREEK FORMATION) 38.6 8 | 11 | 17 | 1.0 5.0 43.6 7 | 13 | 17 | 1.0 SS-5 0.0 48.6 6 12 25 1.0 -5.0 53.6 8 12 18 1.0 -10.0 58.6 7 8 12 1.0 ---X-20-SS-6 -15.0 BORING TERMINATED AT ELEVATION - J4.6 JN --20.0 MEDIUM DENSE SAND -25.0 -30.0

# NORTH CAROLINA DEPARTMENT OF TRANSPORTATION GEOTECHNICAL UNIT BORING LOG

										BOKI		·····	<u> </u>	SHEET 6	OF II
PROJECT SITE DESC		2595				B-1382	COUNT				GE	OLOGIST	K. B. QUICK		
BORING N						ON NC 41 <b>ATION</b> 17+				6′ LT	LAT	IGNMENT	' -L-	GROUN 0 HR.	I <b>D WATEI</b> N/A
COLLAR			38.5				1298	OFFO		EASTING		8480	<u> </u>	24 HR.	2.3′
TOTAL D	<b>EPTH</b> 73	3 <b>.</b> l′	D	RILL	MACH	INE CME-45B		DRILL M	ЕТНО	D ROTA			HAMMER TY		
START D						LETION DATE			SURF	ACE WATE	R DEP	TH N/A	DEPTH TO	ROCK	N/A
ELEV.	DEPTH (FT.)						PER 50		100	SAMPLE NUMBER	<b>V</b> /01	Ŏ.	SOIL AND DESCRIP		
38.5	0.0					<b></b>				. voimble v	/MOI.	Ğ			
35.0 -	2.4	- 1	2	2	1.0 1.0	<del>10</del> <del></del>			-	SS-28	▼		BROWN MOIST TO S		:D
30.0 -	7.4	3	3	4	1.0	7				SS-29			•		
25.0 -	.7	.5	6	1Ø.	1.0				-						
20.0	16.7	4	5	12	1.0					SS-30				· · ·	
15.0	21.7	8	9	14	1.0	X 23			_						
10.0 -	26.7		8	10	1.0	X18			_ _ _ _	66.71					
5.0	<u> </u>		14	16	1.0		60		_	SS-3I			GREEN GRAY		
0.0	36.7 -		5	7	1.0	- X 12			_ _ _ _				(BLACK CREEK		
-5.0	41.7		10	13	1.0	X23			_ _ _						
-10.0	46.7	7	10	13	1.0	23			- - -						
-15 <b>.</b> 0 <b>-</b>	51.7	6	8	15	1.0	23			_ _ _ _	SS-32					
-20 <b>.</b> 0 <b>-</b>	56.7	10	32	34	1.0			X66	- - - -			0 0 0 0 0 0 0 0			
-25 <b>.</b> 0	61.7	33	63	37	0.6				- <u>*</u>	SS-33	٠.	0 0 0 0 0 0 0 0			
-30 <b>.</b> 0	66.7	35	46	54	1.0			<u>- 100</u> +	- - - - -			0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0			
-35 <b>.</b> 0	71.7	26	50	50	0.9	PODIC		100+	╫			• • • • • • • • • • • • • • • • • • •			
-40.0						BORING T ELEVAI VERY I	:-3	4.6 710 7	1 1						

### NORTH CAROLINA DEPARTMENT OF TRANSPORTATION TO NORTH CAROLINA DEPARTMENT OF TRANSPORTATION GEOTECHNICAL UNIT BORING LOG

GEOTECHNICAL HNIT RORING LOG

	ECHNICAL UNII	BOKING LO	<b>এ</b>		@E(	DIECHNICAL	UNIT BORING	GLOG SHEET 7 OF 11
PROJECT NO. 32595.1.1 ID.	B-1382 COUNTY SAMPSO	ON <b>GEOLOGIST</b>	K. B. QUICK	PROJECT NO. 3	2595.1.1	<b>ID.</b> B-1382 <b>COUNT</b>	SAMPSON (	GEOLOGIST K. B. QUICK
	ON NC 41 OVER THE BLACK			SITE DESCRIPTION	BRIDGE NO	. 12 ON NC 41 OVER T	HE BLACK RIVER	GROUND WATER
BORING NO. B2-B BORING LO		<del></del>		BORING NO. B		LOCATION 18+34	OFFSET 7'LT	ALIGNMENT -L- 0 HR. N/A
	NORTHING	EASTING		COLLAR ELEVAT		NORTHING	EASTING	24 HR. ARTESIAN
		HOD ROTARY W/MUD		TOTAL DEPTH 5	9.5' DRILL	MACHINE CME-45B	DRILL METHOD ROTARY	W/MUD HAMMER TYPE AUTOMATIC
		RFACE WATER DEPTH N/A	DEPTH TO ROCK N/A	START DATE 12		COMPLETION DATE 12/07/		EPTH N/A DEPTH TO ROCK N/A
ELEV. DEPTHBLOW COUNT PEN		SAMPLE V	SOIL AND ROCK	11 F1 FV 1	BLOW COUNT			SOIL AND ROCK
(FT.) 0.510.510.5 (FT.	.] 0	NUMBER MOI. G	DESCRIPTION	(+1.)	0.510.510.51	(FT.) 0 25 50	75 IOO NUMBER MC	DI. G DESCRIPTION
17.7				19.1 <del>  0.0</del>	WOH 1 1	1.0	SS-34	BROWN TAN SAND, SATURATED
15.0 + 1.6   1   2   2   1.0	)	1 3 3 3 3 1 1 1	OWN TAN COARSE SANDY SILT	± 3.8		I.0   X3		(ALLUVIUM)
15.0 <u>T</u>		1 1 1	WITH TRACE AMOUNTS OF	15.0 + 3.8				BROWN SAND WITH TRACE AMOUNTS
+ 7,   2   5   2   4		998	ORGANICS, WET (ALLUVIUM)	$H = \Xi$				OF ORGANICS, SATURATED
10.0 + 7.1   3   5   9   1.0	'   <del>^</del> \ <sup>1</sup> 4			10.0 + 8.8	6   10   11	1.0	SS <b>-</b> 35	
	+							
5.0 + 11.7   5   8   13   1.0	)   <del>  X   2        </del>	SS-40		5.0 + 13	9 9 11	1.0     20		
16.7 3 4 6 1.0	)				5 5 7	1.0 - 12		
				0.0 +				
21.7 7 8 9 1.0		SS-40			22 27 32			
-5.0 + 21.1   /   8   3   1.0	`   <del></del>	1 1 10000	DEEN COAY CAND WITH THE	-5.0 + 23	22 21 32	1.0	59-   SS-36	
			REEN GRAY SAND WITH THIN	$\mathbb{I}$				GREEN GRAY SAND WITH THIN
$\begin{vmatrix} -10.0 + 26.7 \end{vmatrix} = 5 \begin{vmatrix} 7 \end{vmatrix} = 11 \begin{vmatrix} 1.0 \end{vmatrix}$	)   <del> X                             </del>	SS-4I	CLAY LAYERS, SATURATED	-10.0 + 28	6 9 14	1.0		ORLEN GRAT SARD WITH THIN
			(BLACK CREEK FORMATION)					CLAY LAYERS, SATURATED
-l5.0 ± 3l.7   8   15   29   l.0	)		IDEACK CREEK FORMATION		5 9 21	1.0   + \ 30		(BLACK CREEK FORMATION)
-i5.0 <u>+</u>				-15.0 + 33				(DLACK CREEK FORMATION)
36.7 13 18 20 1.0		SS-42			5 9 20	1.0		GREEN GRAY SAND WITH THIN  CLAY LAYERS, SATURATED  (BLACK CREEK FORMATION)
-20.0 + 36.1   13   18   20   1.0	`	33-42		-20 <b>.</b> 0 <del>   3</del> 8	3 3 20	1.0		
‡		33-42		ll				
-25.0 + 41.7   30   38   40   1.0				-25.0 + 43	21 39 58	1.0	97_ <b>X</b>	10 • • • • 1
-30.0 + 46.7   39   56   44   1.0	1				46 54	0.4   + +	<del>  </del>	
-30.0 <del> </del>				-30.0 +				
51.7 25 35 54 1.0		SS-43			15 35 54			
-35.0 + 51.1   25   35   54   1.0		33-43		-35.0 + 55	15   35   54	1.0	<del></del>	
		SS-43		$\mathbb{H}$				
-40.0 <u>+</u> 56.7 28 67 33 1.0	<u>                                     </u>			-40.0 + 58	30 33 45	1.0 []	X78	
	BORING TERMINATED AT					DODING TEST		
450	ELEVATION = 40.3' IN _					BORING TERMINA		
-45.0 +	- VERY DENSE SAND -			-45.0 +		E ELEVATION 1-4	1 11 1	
1 1 1 1						L - VERY DENSE	>AND	
-50.0 +				-50.0 +				
1				‡				
-55.0 +								
55.0				-55.0 +				
-60.0 +				-60.0 🛨				
		<u> </u>						

### north carolina department of transportation in north carolina department of transportation GEOTECHNICAL UNIT BORING LOG

PROJEC			2595	.1.1		ID.	3-1382		COUNT	ΓY	SAMP	102°	١	]	GEOL	OGIST	K. B. Q	UICK		
SITE DE							ON NC			BLA								G	ROUNI	D WATER
BORING		B4			ORIN		ATION	18+8			OFFS	ET	7′ RT			MENT	-L-		HR.	N/A
COLLAR				31.9			ORTHING		319	1 =			EASTIN		2186				HR.	4.8′
TOTAL							INE CME						D ROTA					R TYPE		
START							LETION I						ACE WAT		EPTH	N/A	L	H TO RO		/A
ELEV			1			PEN. (FT.)		OWS 25	50 			100	SAMPLE NUMBER	1 /	). G	7		AND R		
	‡							<del> </del>	-			-					*:			
31 <b>.</b> 9 30 <b>.</b> 0	#	0.0 2.1	WOH 1	WOF Ø	WOF 1	1.0 1.0	*0 *t						SS-44							
	+							ļ				-]		Y			BROWN S	AND, SAT	URATI	ED
25.0	+	7.1	2	1	2	1.0	<del>*3</del> -	‡ <u></u> -	_	 		-					(/	LLUVIUM	)	
	ŧ	1.					#===	<del>  :</del>	-			-11					200991 1400	\	, 000	
20.0	‡	II <b>.</b> 5	2	3	2	1.0	X 5 _	<u> </u>					SS-45				3ROWN MOI SAND	, SATURA		ANIC
15.0	+	6.5	4	6	7	1.0	- X 13	===				_			000		TAN SA	ND, SATL	IRATE	)
	ŧ						}	<del> </del> -	_			_	1.							
10.0	+	21.5	8	11	16	1.0		X 27			[	_	SS-46				•			
	‡.							<u> </u>				_								
5.0	+2	?6 <b>.</b> 5	10	14	18	1.0		X 3:	2											
	Ŧ.	71 F		_	1.0			#				-							•	
0.0	+	31.5	6	8	13	1.0						_					GREEN	GRAY S	SAND.	
-5.0	#3	6.5	6	9	14	1.0		23_				-	SS-47				SA	TURATED		
	‡							====				-					(DLACK C	סבבע בסו	DA A TIZ	) NI
-10.0	Ŧ,	41.5	6	8	12	1.0	x	20_				_					(BLACK CI	REEK FUI	KMA I I	יאג
-15.0	# 4	6.5	9	17	37	1.0			X 5	 54 -										
15.0	‡											-					4.1x			
-20.0	Į:	51.5	7	10	17	1.0		X27				_								
	‡		1				====	ļ ·				1								
-25.0	15	6.5	22	43	35	1.0		<del> </del>			¥ 78	-	SS-48							
	Ŧ							]			_/	-								
-30.0	+	SI <b>.</b> 5	31	46	41	1.0		+			<u>- x</u> =	37			• • • •					
75.0	Ŧ						BORI	NG TI	ЕВМІЙ	ATE	D _AT	-								
-35.0	Ŧ			٠.			AE	EVAT RY_D	ENSE	JI.H	ND	-]		٠						,
-40.0	土						<u> </u>	‡ <del>-</del>	-			-	4.							
70.0	‡							===				_								
-45.0	丰				,			<u> </u>				-								
		l					<u> </u>	<u> </u>				-				<u> </u>				

			(			ECHNICAL	UNIT	' BQR	ING		<b>G</b>	SHEET 8 OF II
PROJECT		2595				B-1382 <b>COUNT</b>			GEO	DLOGIST	K. B. QUI	
SITE DESC BORING N		<u>В</u> 2-В				ON NC 4I OVER						GROUND WATER
COLLAR I					<del></del>	ATION 19+54 NORTHING 351323	OFFS	ET 17' RT		GNMENT	`-L-	0 HR. N/A
TOTAL DE						IINE CME-45B	DDIII MI	ETHOD ROT	1G 2218		HAMMED	TYPE AUTOMATIC
START DA						LETION DATE 11/10/1		SURFACE WAT				TO ROCK N/A
	DEPTH							SAMPLE	1 /	<u> </u>		ND ROCK
ELEV.	(FT.)	0.5	10.5	10.5	(FT.)	0 25 50	75	IOO NUMBER	1 / 1	Ö		RIPTION
43.0	+							-				
-	0.0	1	1	2	1.0			_		_:::		
40.0 -	4.0	2	1	3	1.0			-   <sub>SS-20</sub>				I SAND, MOIST
	†							-   00 20		<b>-:::</b>	(ROADWAY	EMBANKMENT)
35 <b>.</b> 0 –	8.5	1	2	3	1.0			_				·
- -	_	,						_				
30.0	13.5	2	3	3	ا ,			-				
-	+ 13.3		٦	٦	1.0		=====	_				
25.0 -	<u> </u>							_		TAI	N SAND, MOIS	T TO SATURATED
.23.0	18.5	1	1	1	1.0			SS-2I				
	-							_			(ALL	UVIUM)
20.0	23.5	5	4	5	1.0	- <u> </u>   <del>-</del>   <u></u> -		_  SS-22				
	Ē							_		• • • •	•	
15.0	28.5	3	4	6	1.0			-   SS-23				•
_	-							_				
10.0	33.5	7	10	15	1.0	75		-   -   SS-24				
_	[ 33 <b>.</b> 3	•			'•'			_  33-24				
5.0 -	38.5	9	10	_,	ا ,			_			*	
=	20.5	7	16	21	1.0			_     .				
-	-							_				
0.0	43.5	4	7	11	1.0	[		-			CDEEN C	RAY SAND,
											GREEN G	KAT SANU,
-5 <b>.</b> 0 -	48.5	6	8	13	1.0			_  SS-25			SATU	RATED
-								_			(DLACK CDE	TV FORMATIONS
-10.0	53.5	6	8	14	1.0	22		-			IDLACK CREE	EK FORMATION)
-								_				
-I5.0 <del>-</del>	58 <b>.</b> 5	10	21	41	1.0			_				
_	- 50.5	10	21	41	1.0			_				
-20.0 -	-							-				
-20 <b>.</b> 0 —	63.5	5	8	12	1.0	20.X		_  SS-26				
-	-							_				
-25 <b>.</b> 0 -	68.5	13	30	60	1.0		30 x	SS- <u>2</u> 7				•
-		٠.						-				
-30.0	73.5	18	39	44	1.0			<u>-    </u>				
-	-				$\vdash$	- BORING -TERMIN				***		
-35.0						E ELEVATION :-	32.07INI			ŀ		
	<u> </u>			L		VERY-DENSE	- SAND -					

B-1382 Bridge No. 12 on NC 41 over Black River

HOLE # SAMPLE # PASS 10 PASS 40 PASS 200 CSESAND FINESAND SI CL LL PI CLASS DEPTH MOIST. ORG.  EB1-B SS-1 93 56 10 62.4 28.6 3.4 5.6 16 NP A-3(0) 4.1-5.6  SS-2 97 69 21 56.6 22.2 7.6 13.6 33 6 A-2-4(0) 13.6-15.1  SS-3 92 60 14 67.8 18.2 4.4 9.6 24 NP A-2-4(0) 18.6-20.1  SS-4 98 93 21 43.6 3.6 41.2 11.6 20 NP A-2-4(0) 33.6-35.1  SS-5 100 92 18 61.2 21.8 7.4 9.6 19 NP A-2-4(0) 43.6-45.1  SS-6 90 88 23 12.0 62.8 9.6 15.6 18 NP A-2-4(0) 58.6-60.1	
SS-2       97       69       21       56.6       22.2       7.6       13.6       33       6       A-2-4(0)       13.6-15.1         SS-3       92       60       14       67.8       18.2       4.4       9.6       24       NP       A-2-4(0)       18.6-20.1         SS-4       98       93       21       43.6       3.6       41.2       11.6       20       NP       A-2-4(0)       33.6-35.1         SS-5       100       92       18       61.2       21.8       7.4       9.6       19       NP       A-2-4(0)       43.6-45.1	
SS-3 92 60 14 67.8 18.2 4.4 9.6 24 NP A-2-4(0) 18.6-20.1 SS-4 98 93 21 43.6 3.6 41.2 11.6 20 NP A-2-4(0) 33.6-35.1 SS-5 100 92 18 61.2 21.8 7.4 9.6 19 NP A-2-4(0) 43.6-45.1	
SS-5 100 92 18 61.2 21.8 7.4 9.6 19 NP A-2-4(0) 43.6-45.1	
SS-6 90 88 23 12.0 62.8 9.6 15.6 18 NP A-2-4(0) 58.6-60.1	
<b>EB2-B</b> SS-20 100 99 10 21.8 68.7 3.8 5.7 14 NP A-3(0) 4.0-5.5	
SS-21 100 100 23 8.3 71.7 10.3 9.7 19 NP A-2-4(0) 18.5-20.0	
SS-22 100 87 3 73.3 24.2 0.8 1.6 14 NP A-3(0) 23.5-25.0	
SS-23 NOT ENOUGH SAMPLE 28.5-30.0 27.5	
SS-24 100 99 12 28.3 60.0 6.1 5.7 15 NP A-2-4(0) 33.5-35.0	
SS-25 100 99 13 10.5 77.2 8.7 3.6 14 NP A-2-4(0) 48.5-50.0	
SS-26 100 85 30 41.2 31.9 11.1 15.8 23 NP A-2-4(0) 63.5-65.0	
SS-27 100 100 5 8.3 87.3 0.8 3.6 14 NP A-3(0) 68.5-70.0	
<b>B1-A</b> SS-28 88 46 11 64.7 24.8 0.5 10.0 20 NP A-1-b(0) 1.0-1.5	
SS-29 100 69 23 57.9 20.5 0.5 21.2 38 19 A-2-6(1) 7.4-8.9	
SS-30 98 91 28 14.6 58.8 7.5 19.2 29 3 A-2-4(0) 16.7-18.2	
SS-31 100 99 14 24.2 62.9 1.8 11.1 23 NP A-2-4(0) 31.7-33.2	
SS-32 100 96 26 19.6 57.3 4.0 19.2 30 6 A-2-4(0) 51.7-53.2	
SS-33 100 99 9 16.5 75.6 0.8 7.0 24 NP A-3(0) 61.7-62.8	
<b>B3-A</b> SS-34 98 55 4 63.1 32.9 1.5 5.1 25 NP A-3(0) 1.0-1.5 (0.8)	
SS-35 100 98 14 25.5 61.9 1.5 11.1 21 NP A-2-4(0) 8.8-10.3	
SS-36 100 99 10 25.3 66.0 0.7 8.1 23 NP A-3(0) 23.0-24.5	
SS-37 100 96 24 21.8 55.8 4.2 18.1 26 NP A-2-4(0) 38.0-39.5	
SS-38 100 100 11 9.7 81.3 0.0 9.1 23 NP A-2-4(0) 53.0-54.5	
DOD 00.00 07 77 54 00.0 444 47.0 50.04 ND 44/0\ 4.004	
<b>B2-B</b> SS-39 97 77 51 33.3 14.1 47.6 5.0 24 NP A-4(0) 1.6-3.1	
SS-40 100 98 13 29.1 58.6 0.2 12.1 21 NP A-2-4(0) 11.7-13.2	
SS-41 100 99 27 20.4 54.1 3.3 22.2 28 4 A-2-4(0) 26.7-28.2	
SS-42 100 99 12 12.0 76.7 1.2 10.1 26 NP A-2-4(0) 36.7-38.2	
SS-43 100 100 10 6.8 84.3 1.9 7.1 19 NP A-3(0) 51.7-53.2	
<b>B4-B</b> SS-44 100 99 21 10.4 72.4 5.1 12.1 25 NP A-2-4(0) 1.0-1.5	
SS-45 92 91 20 12.9 68.4 6.6 12.1 NOT ENOUGH 11.5-13.0 5.1	
SS-46 100 98 14 30.1 57.0 4.8 8.1 23 NP A-2-4(0) 21.5-23.0	
SS-47 100 99 16 9.5 75.4 3.0 12.1 27 NP A-2-4(0) 36.5-38.0	
SS-48 100 98 8 8.3 85.3 2.4 4.0 24 NP A-3(0) 56.5-58.0	

32595.1.1 B-1382 Sampson Co. Bridge No. 12 on NC 41 over Black River



**View Looking East Toward End Bent 2** 

**CONTENTS:** SHEET DESCRIPTION

TITLE SHEET

TEST SITE PLAN

SCOUR REPORT

SOIL TEST RESULTS

SITE PHOTOGRAPH

STRUCTURE INVENTORY REPORT

**LEGEND** 

**PROFILE** BORE LOGS

DRAWN BY: C.M. KENT

### STATE OF NORTH CAROLINA

### DEPARTMENT OF TRANSPORTATION

**DIVISION OF HIGHWAYS** GEOTECHNICAL UNIT

## **STRUCTURE** SUBSURFACE INVESTIGATION

STATE PROJECT 32595.1.1 I.D. NO. <b>B-1382</b>
F.A. PROJECT_ <i>BRSTP-41 (8)</i>
COUNTY <b>SAMPSON</b>
PROJECT DESCRIPTION BRIDGE 26 ON
NC 41 (TOMAHAWK RD) OVER
BLACK RIVER OVERFLOW AT -L-
STATION 38+37

STATE	STATE P	ROJECT REFERENCE NO.	SHEET NO.	TOT
N.C.	3259	5.1.1 (B-1382)	1	1
STATE	PROJ. NO.	F. A. PROJ. NO.	DESCRIP	TION
		BRSTP-41(8)	P.E.	
			CONS	7

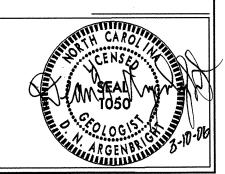
### **CAUTION NOTICE**

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INVESTIGATED BY F. M. WESCOTT PERSONNEL K. B. QUICK D. N. ARGENBRIGHT W. N. CHERRY SUBMITTED BY\_\_\_\_D. N. ARGENBRIGHT R. E. SMITH MARCH 2006



NOTE - THE INFORMATION CONTAINED HEREIN IS NOT IMPLIED OR GUARANTEED BY THE N. C. DEPARTMENT OF TRANSPORTATION AS BEING ACCURATE NOR IT IS CONSIDERED TO BE PART OF THE PLANS. SPECIFICATIONS, OR CONTRACT FOR THE PROJECT.

NOTE - BY HAVING REQUESTED THIS INFORMATION THE CONTRACTOR SPECIFICALLY WAIVES ANY CLAIMS FOR INCREASED COMPENSATION OR EXTENSION OF TIME BASED ON DIFFERENCES BETWEEN THE CONDITIONS INDICATED HEREIN AND THE ACTUAL CONDITIONS AT THE PROJECT SITE.

### NORTH CAROLINA DEPARTMENT OF TRANSPORTATION

### DIVISION OF HIGHWAYS

### GEOTECHNICAL ENGINEERING UNIT

### SUBSURFACE INVESTIGATION

### SOIL AND ROCK LEGEND, TERMS, SYMBOLS, AND ABBREVIATIONS

ROCK DESCRIPTION

HARD ROCK IS NON-COASTAL PLAIN MATERIAL THAT IF TESTED, WOULD YIELD SPT REFUSAL, AN INFERRED ROCK LINE INDICATES THE LEVEL AT WHICH NON-COASTAL PLAIN MATERIAL WOULD YIELD SPT REFUSAL.

SPT REFUSAL IS PENETRATION BY A SPLIT SPOON SAMPLER EQUAL TO OR LESS THAN 0.1 FOOT PER 60 BLOWS. SOIL DESCRIPTION TERMS AND DEFINITIONS VELL GRADED - INDICATES A GOOD REPRESENTATION OF PARTICLE SIZES FROM FINE TO COARSE UNIFORM - INDICATES THAT SOIL PARTICLES ARE ALL APPROXIMATELY THE SAME SIZE. (ALSO SOIL IS CONSIDERED TO BE THE UNCONSOLIDATED, SEMI-CONSOLIDATED, OR WEATHERED EARTH MATERIALS ALLUVIUM (ALLUV.) - SOILS THAT HAVE BEEN TRANSPORTED BY WATER. D- INDICATES A MIXTURE OF UNIFORM PARTICLES OF TWO OR MORE SIZES. THAT CAN BE PENETRATED WITH A CONTINUOUS FLIGHT POWER AUGER, AND YIELD LESS THAN AQUIFER - A WATER BEARING FORMATION OR STRATA. 100 blows per foot according to standard penetration test (aashto t206, astm D-1586). Soil Classification is based on the aashto system. Basic descriptions generally shall include: IN NON-COASTAL PLAIN MATERIAL. THE TRANSITION BETWEEN SOIL AND ROCK IS OFTEN REPRESENTED BY A ZONE ARENACEOUS - APPLIED TO ROCKS THAT HAVE BEEN DERIVED FROM SAND OR THAT CONTAIN SAND. ANGULARITY OF GRAINS CONSISTENCY, COLOR, TEXTURE, MOISTURE, AGNITO CLASSIFICATION, AND OTHER PERTINENT FACTORS SUCH AS MINERALOGICAL COMPOSITION, ANGULARITY, STRUCTURE, PLASTICITY, ETC. EXAMPLE: ROCK MATERIALS ARE TYPICALLY DIVIDED AS FOLLOWS. THE ANGULARITY OR ROUNDNESS OF SOIL GRAINS IS DESIGNATED BY THE TERMS: ANGULAR, ARGILLACEOUS - APPLIED TO ALL ROCKS OR SUBSTANCES COMPOSED OF CLAY MINERALS. R HAVING A NOTABLE PROPORTION OF CLAY IN THEIR COMPOSITION, AS SHALE, SLATE, ETC WEATHERED NON-COASTAL PLAIN MATERIAL THAT WOULD YIELD SPT N VALUES > 100 BLOWS PER FOOT IF TESTED. VERY STIFF, GRAY, SUTY CLAY, WORST WITH INTERBEDDED FINE SAND LAYERS, HIGHLY PLASTIC, 4-7-SUBANGULAR, SUBROUNDED, OR ROUNDED. ARTESIAN - GROUND WATER THAT IS UNDER SUFFICIENT PRESSURE TO RISE ABOVE THE LEVEL SOIL LEGEND AND AASHTO CLASSIFICATION MINERALOGICAL COMPOSITION FINE TO COARSE GRAIN IGNEOUS AND METAMORPHIC ROCK THA AT WHICH IT IS ENCOUNTERED, BUT WHICH DOES NOT NECESSARILY RISE TO OR ABOVE THE CRYSTALLINE ROCK (CR) MINERAL NAMES SUCH AS QUARTZ, FELDSPAR, MICA, TALC, KAOLIN, ETC. ARE USED IN DESCRIPTIONS WHENEVER THEY ARE CONSIDERED OF SIGNIFICANCE. GENERAL GRANULAR MATERIALS SILT-CLAY MATERIALS WOULD YIELD SPT REFUSAL IF TESTED, ROCK TYPE INCLUDES GRANITE. ORGANIC MATERIALS ( < 35% PASSING #200) > 35% PASSING #2001 CALCAREOUS (CALC.) - SOILS THAT CONTAIN APPRECIABLE AMOUNTS OF CALCIUM CARBONATE. FINE TO COARSE GRAIN METAMORPHIC AND NON-COASTAL PLAIN SEDIMENTARY ROCK THAT WOULD YEILD SPT REFUSAL IF TESTED, ROCK TYPE A-4 A-5 A-6 A-NON-CRYSTALLINE ROCK (NCR) COLLUVIUM - ROCK FRAGMENTS MIXED WITH SOIL DEPOSITED BY GRAVITY ON SLOPE OR AT BOTTOM GROUP A-1, A-2 A-4, A-5 CLASS. A-6, A-7 INCLUDES PHYLLITE, SLATE, SANDSTONE, ETC.
COASTAL PLAIN SEDIMENTS CEMENTED INTO ROCK, BUT MAY NOT YIELD SLIGHTLY COMPRESSIBLE LIQUID LIMIT LESS THAN 31 LIQUID LIMIT EQUAL TO 31-50 MODERATELY COMPRESSIBLE DASTAL PLAIN DIMENTARY ROCK CORE RECOVERY (REC.) - TOTAL LENGTH OF ALL MATERIAL RECOVERED IN THE CORE BARREL DIVIDED BY TOTAL LENGTH OF CORE RUN AND EXPRESSED AS A PERCENTAGE. SYMBOL HIGHLY COMPRESSIBLE LIQUID LIMIT GREATER THAN 50 SPT REFUSAL. ROCK TYPE INCLUDES LIMESTONE, SANDSTONE, CEMENTED PASSIN PERCENTAGE OF MATERIAL SILT-DIKE - A TABULAR BODY OF IGNEOUS ROCK THAT CUTS ACROSS THE STRUCTURE OF ADJACENT \* 10 MUCK. CLAY SOILS ORGANIC MATERIAL ROCKS OR CUTS MASSIVE ROCK OTHER MATERIAL SOILS SOILS SOILS ROCK FRESH, CRYSTALS BRIGHT, FEW JOINTS MAY SHOW SLIGHT STAINING, ROCK RINGS UNDER FRESH DIP - THE ANGLE AT WHICH A STRATUM OR ANY PLANAR FEATURE IS INCLINED FROM THE RACE OF ORGANIC MATTER 3 - 5% 2 - 3% HAMMER IF CRYSTALLINE. ITTLE ORGANIC MATTER HORIZONTAL. LITTLE 10 - 20% TIME LODINGE 48 MX 41 MN 48 MX 41 MN 40 MX 41 MN 48 MX 41 MN NP 18 MX 18 MX 11 MN 11 MN 18 MX 18 MX 11 MN VERY SLIGH ROCK GENERALLY FRESH, JOINTS STAINED, SOME JOINTS MAY SHOW THIN CLAY COATINGS IF OPEN, DIP DIRECTION (DIP AZIMUTH) - THE DIRECTION OR BEARING OF THE HORIZONTAL TRACE OF SOILS WITH MODERATELY ORGANIC 5 - 10% 12 - 20% HIGHLY ORGANIO V SLI.) CRYSTALS ON A BROKEN SPECIMEN FACE SHINE BRIGHTLY, ROCK RINGS UNDER HAMMER BLOWS IF THE LINE OF DIP. MEASURED CLOCKWISE FROM NORTH. OF A CRYSTALLINE NATURE. GROUP INDEX 0 4 MX 8 MX 12 MX 16 MX No M MODERATE 0 0 GROUND WATER FAULT - A FRACTURE OR FRACTURE ZONE ALONG WHICH THERE HAS BEEN DISPLACEMENT OF THE AMOUNTS OF ROCK GENERALLY FRESH, JOINTS STAINED AND DISCOLORATION EXTENDS INTO ROCK UP 1 USUAL TYPES STONE FRAGS. FINE SOILS SIDES RELATIVE TO ONE ANOTHER PARALLEL TO THE FRACTURE. WATER LEVEL IN BORE HOLE IMMEDIATELY AFTER DRILLING I INCH. OPEN JOINTS MAY CONTAIN CLAY. IN GRANITOID ROCKS SOME OCCASIONAL FELDSPAR SILTY OR CLAYEY SILTY CLAYEY (SLI.) SOILS SOILS MATTER CRYSTALS ARE DULL AND DISCOLORED. CRYSTALLINE ROCKS RING UNDER HAMMER BLOWS. FISSILE - A PROPERTY OF SPLITTING ALONG CLOSELY SPACED PARALLEL PLANES. ▼... STATIC WATER LEVEL AFTER 24 HOURS MATERIALS MODERATE SIGNIFICANT PORTIONS OF ROCK SHOW DISCOLORATION AND WEATHERING EFFECTS. IN FLOAT - ROCK FRAGMENTS ON SURFACE NEAR THEIR ORIGINAL POSITION AND DISLODGED FROM GEN. RATING  $\nabla_{PW}$ GRANITOID ROCKS, MOST FELDSPARS ARE DULL AND DISCOLORED, SOME SHOW CLAY, ROCK HAS PARENT MATERIAL. POOR PERCHED WATER, SATURATED ZONE, OR WATER BEARING STRATA (LCOM) EXCELLENT TO GOOD FAIR TO POOR AS A UNSHITARI POOR DULL SOUND UNDER HAMMER BLOWS AND SHOWS SIGNIFICANT LOSS OF STRENGTH AS COMPARED FLOOD PLAIN (FP) - LAND BORDERING A STREAM, BUILT OF SEDIMENTS DEPOSITED BY OM-SPRING OR SEEP PI OF A-7-5 SUBGROUP IS ≤ LL - 30 ; PI OF A-7-6 SUBGROUP IS > LL - 30 HODERATEL ALL ROCK EXCEPT QUARTZ DISCOLORED OR STAINED, IN GRANITOID ROCKS, ALL FELDSPARS DULL MISCELLANEOUS SYMBOLS CONSISTENCY OR DENSENESS SEVERE AND DISCOLORED AND A MAJORITY SHOW KAOLINIZATION, ROCK SHOWS SEVERE LOSS OF STRENGTH FORMATION (FM.) - A MAPPABLE GEOLOGIC UNIT THAT CAN BE RECOGNIZED AND TRACED IN: RANGE OF STANDARD ENETRATION RESISTENCE AND CAN BE EXCAVATED WITH A GEOLOGIST'S PICK. ROCK GIVES "CLUNK" SOUND WHEN STRUCK. SPT CPT
DPT DPT TEST BORING PRIMARY SOIL TYPE ROADWAY EMBANKMENT (RE) IF TESTED, WOULD YIELD SPT REFUSAL JOINT - FRACTURE IN ROCK ALONG WHICH NO APPRECIABLE MOVEMENT HAS OCCURRED WITH SOIL DESCRIPTION DESIGNATIONS ALL ROCK EXCEPT QUARTZ DISCOLORED OR STAINED, ROCK FABRIC CLEAR AND EVIDENT BUT REDUCED SEVERE S - BULK SAMPLE LEDGE - A SHELF-LIKE RIDGE OR PROJECTION OF ROCK WHOSE THICKNESS IS SMALL COMPARED TO VERY LOOSE STRENGTH TO STRONG SOIL. IN GRANITOID ROCKS ALL FELDSPARS ARE KAOLINIZED TO SOME GENERALLY SOIL SYMBOL AUGER BORING (SEV.) ITS LATERAL EXTENT. SS - SPLIT SPOON EXTENT. SOME FRAGMENTS OF STRONG ROCK USUALLY REMAIN. MEDIUM DENSE 10 TO 30 ARTIFICIAL FILL (AF) OTHER IF TESTED, YIELDS SPT N VALUES > 100 BPF LENS - A BODY OF SOIL OR ROCK THAT THINS OUT IN ONE OR MORE DIRECTIONS MATERIAL DENSE CORE BORING (NON-COHESIVE) THAN ROADWAY EMBANKMENT MOTTLED (MOT.) - IRREGULARLY MARKED WITH SPOTS OF DIFFERENT COLORS. MOTTLING IN VERY SEVERE ALL ROCK EXCEPT QUARTZ DISCOLORED OR STAINED, ROCK FABRIC ELEMENTS ARE DISCERNIBLE BU VERY DENSE ST - SHELBY TUBE >50 THE MASS IS EFFECTIVELY REDUCED TO SOIL STATUS, WITH ONLY FRAGMENTS OF STRONG ROCK REMAINING. SAPROLITE IS AN EXAMPLE OF ROCK WEATHERED TO A DEGREE SUCH THAT ONLY MINOF OILS USUALLY INDICATES POOR AERATION AND LACK OF GOOD DRAINAGE. INFERRED SOU ROUNDARY SAMPLE VEDY COE <u>ERCHED WATER</u> - WATER MAINTAINED ABOVE THE NORMAL GROUND WATER LEVEL BY THE PRESENCE OF AN MONITORING WELL (0.25 € 2 TO 4 RM - RESILIENT MODULUS 0.25 TO 0.50 0.5 TO 1.0 GENERALLY VESTIGES OF THE ORIGINAL ROCK FABRIC REMAIN. IF TESTED, YIELDS SPT N VALUES < 100 BPF ITERVENING IMPERVIOUS STRATUM. INFERRED ROCK LINE SILT-CLAY MEDIUM STIFF 4 TO 8 PIEZOMETER SAMPLE. Δ ROCK REDUCED TO SOIL. ROCK FABRIC NOT DISCERNIBLE, OR DISCERNIBLE ONLY IN SMALL AND RESIDUAL (RES.) SOIL - SOIL FORMED IN PLACE BY THE WEATHERING OF ROCK. INSTALLATION MATERIAL 1 TO 2 ALLUVIAL SOIL BOUNDAR RS - ROCK SAMPLE SCATTERED CONCENTRATIONS, QUARTZ MAY BE PRESENT AS DIKES OR STRINGERS, SAPROLITE IS VERY STIFF COHESIVE 15 TO 3Ø ROCK QUALITY DESIGNATION (ROD) - A MEASURE OF ROCK QUALITY DESCRIBED BY TOTAL LENGTH OF ROCK SEGMENTS EQUAL TO OR GREATER THAN 4 INCHES DIVIDED BY THE TOTAL LENGTH OF CORE RUN AN SLOPE INDICATOR  $\bigcirc$ RT - RECOMPACTED TRIAXIA DIP & DIP DIRECTION OF ROCK HARDNESS ROCK STRUCTURES EXPRESSED AS A PERCENTAGE. TEXTURE OR GRAIN SIZ - SPT N-VALUE SAPROLITE (SAP.) - RESIDUAL SOIL THAT RETAINS THE RELIC STRUCTURE OR FABRIC OF THE CBR - CALIFORNIA BEARING CANNOT BE SCRATCHED BY KNIFE OR SHARP PICK, BREAKING OF HAND SPECIMENS REQUIRES SOUNDING ROD U.S. STD. SIEVE SIZE 60 (REF)- SPT REFUSAL RATIO SAMPLE SEVERAL HARD BLOWS OF THE GEOLOGIST'S PICK. 0.42 0.25 0.075 SILL - AN INTRUSIVE BODY OF IGNEOUS ROCK OF APPROXIMATELY UNIFORM THICKNESS AND CAN BE SCRATCHED BY KNIFE OR PICK ONLY WITH DIFFICULTY, HARD HAMMER BLOWS REQUIRED ABBREVIATIONS RELATIVELY THIN COMPARED WITH ITS LATERAL EXTENT, THAT HAS BEEN EMPLACED PARALLEL TO THE BEDDING OR SCHISTOSITY OF THE INTRUDED ROCKS. COBBLE AR - AUGER REFUSAL SILT HI. - HIGHLY # - MOISTURE CONTENT SAND SAND MODERATELY CAN BE SCRATCHED BY KNIFE OR PICK. GOUGES OR GROOVES TO 0.25 INCHES DEEP CAN BE (RLDR.) (COR.) (CR ) (SL ) (CL.) - BORING TERMINATED MED. - MEDIUM V - VERY SLICKENSIDE - POLISHED AND STRIATED SURFACE THAT RESULTS FROM FRICTION ALONG A FAULT OR TED BY HARD BLOW OF A GEOLOGIST'S PICK. HAND SPECIMENS CAN BE DETACHED CL. - CLAY MICA. - MICACEOUS VST - VANE SHEAR TEST 305 12 2.0 0.25 0.05 0.005 MM IN. CPT - CONE PENETRATION TEST BY MODERATE BLOWS. MOD. - MODERATELY WEA. - WEATHERED STANDARD PENETRATION TEST (PENETRATION RESISTANCE) (SPT) - NUMBER OF BLOWS (N OR BPF) OF A 140 LB, HAMMER FALLING 30 INCHES REQUIRED TO PRODUCE A PENETRATION OF 1 FOOT INTO SOIL WITH A 2 INCH OUTSIDE DIAMETER SPLIT SPOON SAMPLER, SPT REFUSAL IS PENETRATION EQUAL TO OR LESS SIZE MEDIUM CAN BE GROOVED OR GOUGED 0.05 INCHES DEEP BY FIRM PRESSURE OF KNIFF OR PICK POINT. CSE. - COARSE NP - NON PLASTIC 7 - UNIT WEIGHT DILATOMETER TEST CAN BE EXCAVATED IN SMALL CHIPS TO PEICES I INCH MAXIMUM SIZE BY HARD BLOWS O SOIL MOIS 7- DRY UNIT WEIGHT JRE - CORRELATION OF TERMS POINT OF A GEOLOGIST'S PICK. DPT - DYNAMIC PENETRATION TEST PMT - PRESSUREMETER TEST SOIL MOISTURE SCALE FIELD MOISTURE THAN 0.1 FOOT PER 60 BLOWS. GUIDE FOR FIELD MOISTURE DESCRIPTION SOFT CAN BE GROVED OR GOUGED READILY BY KNIFE OR PICK. CAN BE EXCAVATED IN FRAGMENTS <u>Strata core recovery (srec.)</u> - total length of strata material recovered divided by total length of stratum and expressed as a percentage, F - FINE SD. - SAND, SANDY FROM CHIPS TO SEVERAL INCHES IN SIZE BY MODERATE BLOWS OF A PICK POINT. SMALL, THIN HISHALLY I TOUTO: VERY WET, USUALLY FOSS. - FOSSILIFEROUS SL. - SILT, SILTY PIECES CAN BE BROKEN BY FINGER PRESSURE. SATURATED FRAC. - FRACTURED, FRACTURES SIT - SITCHTLY STRATA ROCK QUALITY DESIGNATION (SROD) - A MEASURE OF ROCK QUALITY DESCRIBED BY FROM BELOW THE GROUND WATER TABLE CAN BE CARVED WITH KNIFE, CAN BE EXCAVATED READILY WITH POINT OF PICK, PIECES 1 INCH (SAT.) STRAIN MOCK CALL'IT DESIGNATION SALOY. IN RESIDIE OF NOCK CALL'IT DESIGNADED BY TOTAL LENGTH OF ROCK SEGMENTS WITHIN A STRATUM EQUAL TO OR GREATER THAN 4 INCHES DIVIDED BY THE TOTAL LENGTH OF STRATA AND EXPRESSED AS A PERCENTAGE. FRAGS. - FRAGMENTS TCR - TRICONE REFUSAL LIQUID LIMIT SOFT OR MORE IN THICKNESS CAN BE BROKEN BY FINGER PRESSURE, CAN BE SCRATCHED READILY BY FINGERNAIL SEMISOLID: REQUIRES DRYING TO ANGE - WET - (W) TOPSOIL (TS.) - SURFACE SOILS LISUALLY CONTAINING DRIGANIC MATTER. EQUIPMENT USED ON SUBJECT PROJECT ATTAIN OPTIMUM MOISTURE FRACTURE SPACING PLASTIC LIMIT THICKNESS TFRM BENCH MARK: BM # 2 -- R.R. SPIKE SET IN 6" OAK TREE AT -BL-TERM ADVANCING TOOLS: HAMMER TYPE: SPACING DRILL UNITS: VERY THICKLY BEDDED > 4 FEET VERY WIDE MORE THAN 10 FEET - MOIST - (M SOLID: AT OR NEAR OPTIMUM MOISTURE AUTOMATIC MANUAL STA, 40+30,195' LT OPTIMUM MOISTURE THICKLY BENDED 1.5 - 4 FFFT CLAY BITS 3 TO 10 FEET MOBILE B-\_\_\_ 0.16 - 1.5 FEET ELEVATION: 39.25 SL \_ SHRINKAGE LIMIT THINLY BEDDED MODERATELY CLOSE 1 TO 3 FEET VERY THINLY REDDED 0.03 - 0.16 FFFT 6º CONTINUOUS FLIGHT AUGER CORE SIZE: 0.16 TO 1 FEET REQUIRES ADDITIONAL WATER TO CLOSE NOTES: - DRY - (D) BK-51 THICKLY LAMINATED 0.008 - 0.03 FEET VERY CLOSE LESS THAN 0.16 FEET ATTAIN OPTIMUM MOISTURE R\* HOLLOW AUGERS THINLY LAMINATED < 0.008 FFFT \_\_\_\_ INDURATION PLASTICIT HARD FACED FINGER BITS CME-45C \_\_\_\_n-OR SEDIMENTARY ROCKS, INDURATION IS THE HARDENING OF THE MATERIAL BY CEMENTING, HEAT, PRESSURE, ETC. PLASTICITY INDEX (PI) DRY STRENGTH TUNG.-CARBIDE INSERTS \_\_\_-H\_\_\_ NONPLASTIC VERY LOW CME-550 RUBBING WITH FINGER FREES NUMEROUS GRAINS: CASING W/ ADVANCER LOW PLASTICITY 6-15 SLIGHT GENTLE BLOW BY HAMMER DISINTEGRATES SAMPLE. HAND TOOLS: MED. PLASTICITY MEDIUM PORTABLE HOIST TRICONE 215/6 STEEL TEETH POST HOLF DIGGER GRAINS CAN BE SEPARATED FROM SAMPLE WITH STEEL PROBE-HIGH PLASTICIT MODERATELY INDURATED 26 OR MORE BREAKS EASILY WHEN HIT WITH HAMMER, TRICONE \* TUNG.-CARB. HAND AUGER OTHER CME 45B GRAINS ARE DIFFICULT TO SEPARATE WITH STEEL PROBE; INDURATED SOUNDING ROD CORE BIT DESCRIPTIONS MAY INCLUDE COLOR OR COLOR COMBINATIONS (TAN. RED. YELLOW-BROWN, BLUE-GRAY). DIFFICULT TO BREAK WITH HAMMER. VANE SHEAR TEST OTHER MODIFIERS SUCH AS LIGHT, DARK, STREAKED, ETC. ARE USED TO DESCRIBE APPEARANCE. OTHER SHARP HAMMER BLOWS REQUIRED TO BREAK SAMPLE; EXTREMELY INDURATED OTHER SAMPLE BREAKS ACROSS GRAINS.

REVISED 03/07/05

PROJECT REFERENCE NO. SHEET NO. 32595.1.1 (B-1382) 2



## STATE OF NORTH CAROLINA DEPARTMENT OF TRANSPORTATION

MICHAEL F. EASLEY
GOVERNOR

LYNDO TIPPETT SECRETARY

March 10, 2006

STATE PROJECT:

32595.1.1 B-1382

F. A. PROJECT:

BRSTP-41(8)

COUNTY:

Sampson

**DESCRIPTION:** 

Bridge No. 26 on NC 41 (Tomahawk Rd) over Black River

Overflow

SUBJECT:

Geotechnical Report - Bridge Foundation Investigation for

NC 41 (Tomahawk Rd) over Black River Overflow at -L-

Station 38+37

### **Site Description**

The proposed bridge site is located at the existing NC 41 over Black River Overflow approximately 4± miles east of Harrells. The replacement structure will be constructed along the existing NC 41 alignment. Based on the proposed design, the new structure will have four spans with a total length of 200 feet. The bents will have a skew of 120 degrees.

One Standard Penetration Test (SPT) boring was made at or near each proposed bent location to provide subsurface information relative to foundation design. The borings were made with ATV mounted CME-45B drill machine and were advanced by rotary drill methods using bentonite drilling fluid.

The bridge site is located in the Coastal Plain Physiographic Province and is underlain by Recent alluvial deposits and Cretaceous age soils of the Black Creek Formation. Topography at the site is nearly flat to gentle sloping. Elevations at the site range from 29± feet along the channel bed to 43± feet along the existing NC 41 roadway. Artesian flow was noted in all borings drilled at the site. The hydrostatic head elevations ranged from 33± feet to 34± feet within the bore holes. The true water levels will generally match the stream flow line. The surface of Black River Overflow was at an elevation of 31± feet.

### MAILING ADDRESS:

NC DEPARTMENT OF TRANSPORTATION GEOTECHNICAL ENGINEERING UNIT 1589 MAIL SERVICE CENTER RALEIGH NC 27699-1589 TELEPHONE: 919-250-4088 FAX: 919-250-4237

WEBSITE: WWW.DOH.DOT.STATE.NC.US

LOCATION:
CENTURY CENTER COMPLEX
ENTRANCE B-2
1020 BIRCH RIDGE DRIVE
RALEIGH NC

### **Soil Description**

Subsurface conditions at the site are relatively uniform. Surficial alluvial soils generally consist of  $2\pm$  to  $6\pm$  feet of very soft to stiff silt (A-4) and very loose sand (A-2-4) with trace to moderate amounts of organics underlain by loose to dense sand (A-1-b, A-2-4, A-3). Tested organic samples within the silts and sands ranged from 4 to 14 percent. Soils belonging to the Cretaceous age Black Creek Formation underlie the alluvial deposits at elevations ranging from  $23\pm$  to  $27\pm$  feet. Soils of the Black Creek Formation consist of  $6\pm$  to  $16\pm$  feet of very loose to very dense sand (A-1-a, A-1-b, A-2-4, A-3) underlain by  $3\pm$  to  $13\pm$  feet of stiff to very stiff clayey silt (A-4) and sandy silty clay (A-7-5, A-7-6). The cohesive deposits are underlain at elevations ranging from  $4\pm$  to  $10\pm$  feet by medium dense to very dense sand and clayey sand (A-2-4, A-2-6). Approximately 4 feet of calcareous sandstone was encountered within the sands in boring EB2-A at an elevation of  $-1\pm$  feet.

Based on the proposed design, the existing grade will be raised 1± foot at the proposed bridge site. The existing roadway embankment at the end bents consists of 11± feet of very loose to medium dense sand (A-2-4, A-3). The proposed end bent slopes will be mainly constructed within the existing embankment. Some additional fill will be required for construction of the end bent and side slopes. Borrow meeting Coastal Plain criteria is available in nearby areas.

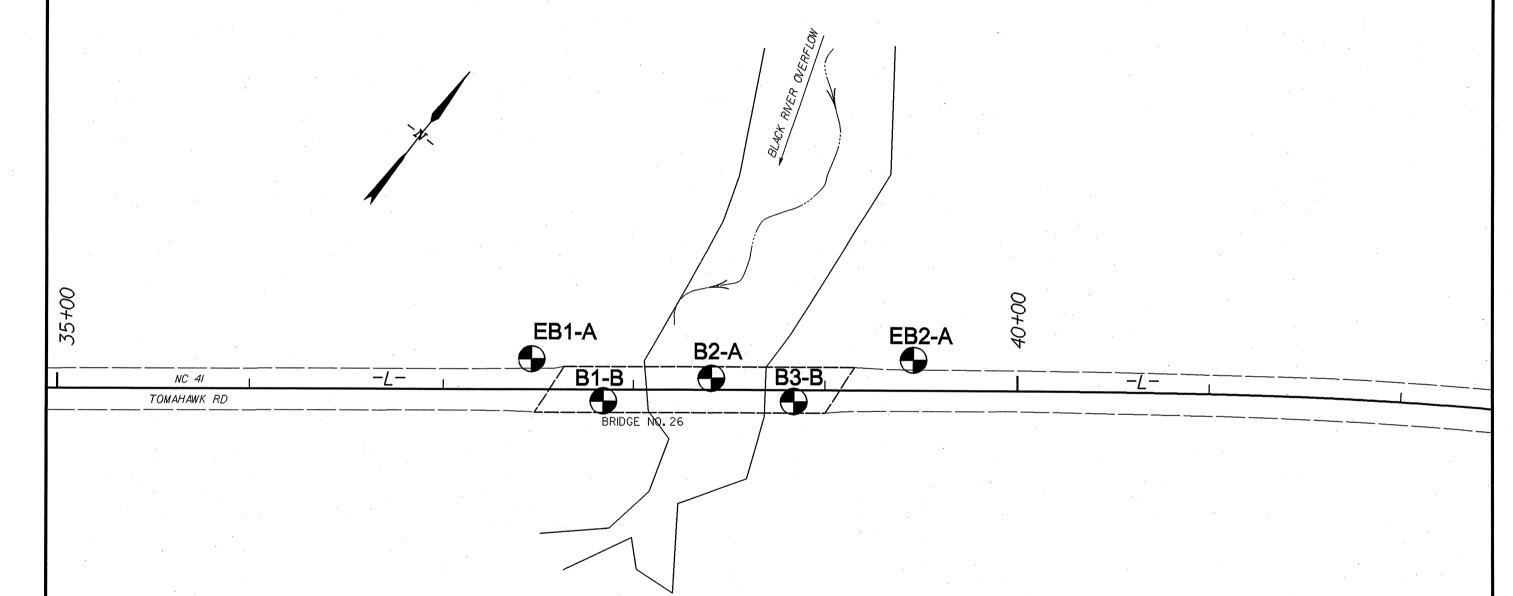
The Geotechnical foundation report is based on the Bridge Survey and Hydraulic Design Report dated August 2, 2005. If significant changes are made in the design or location of the proposed structure, the subsurface information should be reviewed and modified as necessary.

Prepared By:

Fred M. Wescott III

Project Engineering Geologist

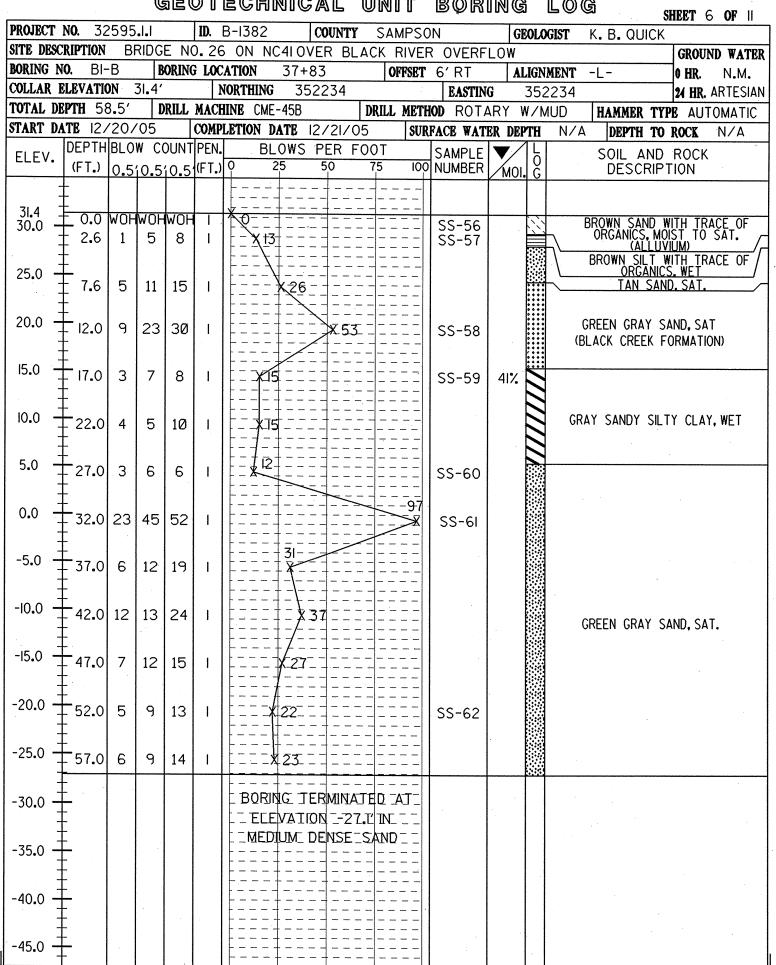
	TEST	SITE	PI	AN		
				0 50 FEET	 ECT REFERENCE NO. 32595.1.1 (B-1382)	SHEET 4



### NORTH CAROLINA DEPARTMENT OF TRANSPORTATION || NORTH CAROLINA DEPARTMENT OF TRANSPORTATION GEOTECHNICAL UNIT BORING LOG

PROJECT NO. 32595.1.1 **ID.** B-1382 COUNTY SAMPSON GEOLOGIST K. B. QUICK SITE DESCRIPTION BRIDGE NO. 26 ON NC4IOVER BLACK RIVER OVERFLOW GROUND WATER BORING NO. EBI-A 37+46 BORING LOCATION OFFSET 16'LT ALIGNMENT -L-O HR. N.M. COLLAR ELEVATION 42.7' NORTHING 352228 **EASTING** 2220225 24 HR. 12.4 TOTAL DEPTH 60.0' DRILL MACHINE CME-45B DRILL METHOD ROTARY W/MUD HAMMER TYPE AUTOMATIC START DATE 11/9/05 COMPLETION DATE 11/9/05 SURFACE WATER DEPTH N/A DEPTH TO ROCK N/A DEPTHIBLOW COUNTIPEN. BLOWS PER FOOT SAMPLE SOIL AND ROCK 100 NUMBER MOI. G (FT.) 0.510.510.5(FT.) 75 DESCRIPTION 42.7 0.0 40.0 2 2 4.0 4 SS-7 GRAY TAN SAND, MOIST (ROADWAY EMBANKMENT) 35.0 8.5 5 7 10 BROWN MOD. ORGANIC SILT (ALLUVIUM) Y 30.0 13.5 | 10 | 15 | 19 SS-8 GRAY SAND. SAT. 25.0 18.5 | 12 | 19 | 33 | X-52-GRAY SAND, SAT. 20.0 (BLACK CREEK FORMATION) 23.5 9 18 | 21 SS-9 15.0 6 8 4 SS-IO 25% GRAY CLAYEY SILT. WET 5 6 9 7 5 11 SS-II 0.0 43.5 5 15 26 -5.0 5 12 | 13 GREEN GRAY SAND, SAT. -10.053.5 9 12 | 17 <u>X-29</u>-SS-I2 -15.0 58.5 8 11 13 BORING JERMINATED AT -20.0 \_ Edevairon --17.82 IN MEDIUM DENSE SAND -25.0 -30.0 -35.0

# GEOTECHNICAL UNIT BORING LOG



# NORTH CAROLINA DEPARTMENT OF TRANSPORTATION ONTH CAROLINA GEOTECHNICAL UNIT BORING LOG GEOTECH

PROJECT NO. 32595.1.1 **ID.** B-1382 COUNTY SAMPSON GEOLOGIST K. B. QUICK SITE DESCRIPTION BRIDGE NO. 26 ON NC41 OVER BLACK RIVER OVERFLOW GROUND WATER BORING LOCATION BORING NO. B2-A 38+40 OFFSET 6'LT ALIGNMENT 0 HR. N.M. COLLAR ELEVATION 29.1' 352278 2220305 24 HR. ARTESIAN NORTHING EASTING TOTAL DEPTH 61.2' DRILL MACHINE CME-45B DRILL METHOD ROTARY W/MUD HAMMER TYPE AUTOMATIC START DATE 1/4/06 COMPLETION DATE 1/5/06 SURFACE WATER DEPTH 1.8' DEPTH TO ROCK N/A DEPTHIBLOW COUNTIPEN. BLOWS PER FOOT SAMPLE SOIL AND ROCK DESCRIPTION 100 NUMBER 10.510.510.5(FT.) /MOL 29.1 BROWN MODERATELY ORGANIC SS-63 SILT. WET (ALLUVIUM) 25.0 BROWN TAN SAND, SAT. 5.3 | 5 | 10 | 10 SS-64 GRAY SAND. SAT 20.0 9.5 | 13 | 19 | 26 (BLACK CREEK FORMATION) 15.0 14.5 6 3 9 SS-65 20% GRAY SANDY SILTY CLAY, WET 19.5 4 8 l 7 24.7 4 6 8 29.7 8 12 20 1 SS-66 -5.0 10 16 34.7 6 -10.0 <sup>-</sup>39.7 8 | 11 | 20 | X 31\_ -15.0 ± 44.7 9 12 18 GREEN GRAY SAND, SAT. X 30 -20.0 ¥ 49.7 8 12 | 18 **Х З**О. SS-67 -25.0 T 54.7 7 8 | 12 -30.0 丁59.7 10 15 46 BORING JERMINATED AT -35.0 ELEVATION --32 L - -ON VERY DENSE SAND -40.0 -45.0 -50.0

## NORTH CAROLINA DEPARTMENT OF TRANSPORTATION

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PROJECT		2595		<del>-</del> 11/		3-1382	COUN		AMPS(			OLO	GIST	K.B.QUICK	
SITE DES	CRIPTION B3	-B				ON NC4 ATION 3	8+83			OVERF		ION	MENT	1	GROUND WATE
	ELEVATION DE LE CONTRE LE					ORTHING	35229!		offæl	EASTING			MEN 1 20346	<u>-L-</u>	O HR. N.M. 24 Hr. Artesia
	DEPTH 59					INE CME-4			I. METI	HOD ROTA				HAMMER TY	PE AUTOMATIC
	DATE 12/					ETION DAT				FACE WATI				<del></del>	ROCK N/A
ELEV.	DEPTH	BLO	W CO			BLO	NS PER			SAMPLE		1Ľ		SOIL AND	
	(FT.)	0.5	10.5°	10.5	(FT.)	0 25	50	75	100	NUMBER	MOL	. Ğ		DESCRIP	
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05.0	‡	}								,				(ALLUV	IUM)
25.0	± 8.8	12	17	17		}	X 34 -			SS-50			•		
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20.0	<b>‡</b> 13.2	5	11	28			X39=						(E	BLACK CREEK	FORMATION)
	<u>‡</u>						/					::::			
15.0	18.2	5	5	12		X17	] -			SS-5I	31%				
	‡					===							GR	AY SANDY SIL	TY CLAY, WET
10.0	± 23.2	8	17	18		+	\ \ \ \ \ \ \ \ \ \ \ \ \ \ \ \ \ \ \			SS-52				***************************************	
	± 20.2	ľ	1			[]				33 32				GREEN GRAY	SAND, SAT.
5.0	± 28.2	3	6	10	,					CC 57		11	·····		
•	+ 20.2	٥		שו						SS-53		H	. (	GRAY CLAYEY	SAND, SAT.
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0.0	<del>+</del> 33.2	6	10	19		<i> </i>	. 29. <sub> </sub> _								
-5.0	<u> </u>					E = = = # =									
J.0	38.2	6	10	13		X-2	3			SS-54					
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		ina depar' 'Echnical (		TRANSPORTATION NG LOG SUPPT : AF II
PROJECT NO. 32595.I.I		B-1382 COUNTY		Sheet o ar ii
		6 ON NC4IOVER BLA	SAMPSON CK RIVER OVERFI	GEOLOGIST K. B. QUICK OW GROUND WATE
BORING NO. EB2-A	BORING LOC	CATION 39+46	OFFSET 16'LT	ALIGNMENT -L- 0 HR. N.M.
COLLAR ELEVATION 42	<del></del>	NORTHING 2220382	EASTING	352350 <b>24 HR.</b> ARTESIA
TOTAL DEPTH 65.1' START DATE 11/9/05		HINE CME-45B PLETION DATE 11/9/05	DRILL METHOD ROTAF	
ELEV. DEPTH BLOW (FT.) 0.510	COUNT PEN	BLOWS PER FO	OOT SAMPLE	
42.7				
40.0 + 0.0 1	1 2 1	1-3		
40.0	4 3 1	\[\\\\\\\\\\\\\\\\\\\\\\\\\\\\\\\\\\\\	SS-I3	TAN SAND, MOIST (ROADWAY EMBANKMENT)
35.0 + 0.6   3				EMBANKMENT)
8.6 2 3	3   7   1	- <del>-</del> <del>-</del>		▼ 🗒
30.0 + 17.6   3   6				
30.0   13.6   3   3	3   3   1	1-46		GRAY SAND, MOIST TO SAT. (ALLUVIUM)
25.0 + 10.6   2				000 000 000 000
18.6 3 6	6   7   1	X13++	SS-14	000 000 000
20.0 = 37.6				GRAY SAND, SAT. (BLACK CREEK FORMATION)
$\begin{vmatrix} 20.0 & \pm 23.6 & 4 & 1 \\ \pm & 23.6 & 4 & 1 \end{vmatrix}$	1   11	22		000
15.0 = 20.6 = 5	_   _   .			GRAY SAND, SAT.  COOK COOK COOK COOK COOK COOK COOK CO
$\begin{vmatrix} 13.0 & \pm 28.6 \\ \pm 28.6 \end{vmatrix}$ 5	5   6   1		SS-15	000 000 000 000 000
10.0 + 77.6 7				GRAY SANDY CLAY, WET
33.6 7 10	0 16 1	\\\ <del>\\\\\\\\\\\\\\\\\\\\</del>	-   -   SS-16	51111 511151 5211, 1121
5.0 + 70   5   5				
38.6 5 5	5   8   1		SS-17	GREEN GRAY SAND, SAT.
0.0 + 43.6 16 5	39 41 0 7		<u>I</u> QQ,	
\ \frac{1}{2}   \frac{1}{2}  \frac{1}{2}  \frac{1}{2}  \fra	, ,   +1   0.1			CALCAREOUS SANDSTONE
-5.0 + 48.6 5 6	6 8 1			CALCAREOUS SANDSTONE
		14		
-10.0 + 53.6 8 1	2 15 1		-   -   SS-18	
-I5.0 <del>+</del> 58.6 8 8	9   11   1	<del> </del>	 	GREEN GRAY SAND, SAT.
	´   <del>` </del>   '	<del></del>		
$\begin{vmatrix} -20.0 & + & 63.6 & 6 & 1 \end{vmatrix}$	1   17   I	X28		
‡ # 1 · 1				
-25.0 +		BORING TERMINA		
		ELEVATION -2		
-30.0 +		- IN-MEDIUM DENS	t-SANU   	
<u> </u>				
-35.0 +				
		<del></del>		

### **GEOTECHNICAL UNIT FIELD SCOUR REPORT**

PROJECT: 32595.1.1 ID: B-1382 COUNTY: Sampson
DESCRIPTION(1): Bridge No. 26 on NC 41over the Black River Overflow
INFORMATION ON EXISTING BRIDGE  Information obtained from:  □ microfilm (Reel:Pos:) □ other:
BR. NO.: 26 BR. LENGTH: 150.5' NO. BENTS: 7 NO. BENTS IN: CHANNEL: 2 FLOODPLAIN: 5
FOUNDATION TYPE:Timber piles
EVIDENCE OF SCOUR(2):
ABUTMENTS OR END BENT SLOPES: None Noted
INTERIOR BENTS: None noted
CHANNEL BED: None noted
CHANNEL BANKS: None noted
EXISTING SCOUR PROTECTION:
TYPE(3): Concrete end wall
EXTENT(4): 8 feet from outside edge of bridge
EFFECTIVENESS(5): Appears satisfactory
OBSTRUCTIONS(6) (DAMS,DEBRIS,ETC.): None Noted
DESIGN INFORMATION
CHANNEL BED MATERIAL(7): Fine to coarse sand and silt with trace to moderate amounts of organics
(SS-56, SS-63)
CHANNEL BANK MATERIAL(8): Fine to coarse sand (SS-8) and moderately organic silt
CHANNEL BANK COVER(9): Wooded and grasses
FLOOD PLAIN WIDTH(10): 600 +/- feet
FLOOD PLAIN COVER(11): Wooded

SHEET 9

DESIGN INFORMATION CONT.
STREAM IS:DEGRADINGEQUILIBRIUM (12)
OTHER OBSERVATIONS AND COMMENTS:
CHANNEL MIGRATION TENDENCY (13): Northeast toward end bent 2
GEOTECHNICALLY ADJUSTED SCOUR ELEVATIONS(14):
Geotechnical analysis agrees with the Hydrualic Unit's estimate of scour at this site. The
proposed scour at this site is at elevations of 26 to 27 +/- feet.
REPORTED BY: And MWSWT4 DATE: 3-10-06

### INSTRUCTIONS

- (1) GIVE THE DESCRIPTION OF THE SPECIFIC SITE, INCLUDING ROUTE NUMBER AND BODY OF WATER CROSSED.
- NOTE ANY EVIDENCE OF SCOUR AT THE EXISTING END BENTS OR ABUTMENTS (UNDERMINING, SLOUGHING, SCOUR LOCATIONS, DEGRADATIONS, ETC.)
- 3) NOTE ANY EXISTING SCOUR PROTECTION (RIR RAP, ETC.)
- 4) DESCRIBE THE EXTENT OF ANY EXISTING SCOUR PROTECTION.
- (5) DESCRIBE WHETHER OR NOT THE SCOUR PROTECTION APPEARS TO BE WORKING.
- (6) NOTE ANY DAMS, FALLEN TREES, DEBRIS AT BENTS, ETC.
- (7) DESCRIBE THE CHANNEL BED MATERIAL BASED ON OBSERVATION AND/OR SAMPLES.
- (8) DESCRIBE THE CHANNEL BANK MATERIAL BASED ON OBSERVATION AND/OR SAMPLES.
- 9) DESCRIBE THE BANK COVERING (GRASS, TREES, RIP RAP, NONE, ETC.)
- 10) GIVE THE APPROXIMATE FLOOD PLAIN WIDTH (ESTIMATE).
- 1) DESCRIBE THE FLOOD PLAIN COVERING (GRASS, TREES, CROPS, ETC.)
- (12) CHECK THE APPROPRIATE SPACE AS TO WHETHER THE STREAM IS DEGRADING OR AGGRADING.
- (13) DESCRIBE THE POTENTIAL OF THE BODY OF WATER TO MIGRATE LATERALLY DURING THE LIFE OF THE BRIDGE (APPROXIMATELY 100 YEARS).
- (14) GIVE THE GEOTECHNICALLY ADJUSTED SCOUR ELEVATION EXPECTED OVER THE LIFE OF THE BRIDGE (APPROXIMATELY 100 YEARS). THIS CAN BE GIVEN AS AN ELEVATION RANGE ACROSS THE SITE, OR ON A BENT BY BENT BASIS WHERE VARIATIONS EXIST. DISCUSS THE RELATIONSHIP BETWEEN THE HYDRAULICS THEORETICAL SCOUR AND THE GEOTECHNICALLY ADJUSTED SCOUR ELEVATION. IF THE GEOTECHNICALLY ADJUSTED SCOUR ELEVATION IS DEPENDENT ON SCOUR COUNTER MEASURES, EXPLAIN. (RIPRAP ARMORING ON SLOPES, ETC.) THE GEOTECHNICALLY ADJUSTED SCOUR ELEVATION IS BASED ON THE ERODABILITY OF MATERIALS WITH CONSIDERATION FOR JOINTING, FOLIATION, BEDDING ORIENTATION AND FREQUENCY, CORE RECOVERY PERCENTAGE, PERCENTAGE RQD, DIFFERENTIAL WEATHERING, SHEAR STRENGTH, OBSERVATIONS AT EXISTING STRUCTURES, OTHER TESTS DEEMED APPROPRIATE, AND OVERALL GEOLOGIC CONDITIONS AT THE SITE.

B-1382 Bridge No. 26 on NC 41 over Black River Overflow

HOLE#	SAMPLE#	PASS 10	PASS 40	<b>PASS 200</b>	CSESAND	FINESAND	SI	CL	LL	PI	CLASS	DEPTH	MOIST.	ORG.
EB1-A	SS-7	92	70	7	52.3	41.2	2.8	3.6	14	NP	A-3(0)	4.0-5.5		
	SS-8	90	48	24	72.9	22.8	2.6	1.6	15	NP	A-1-b(0)	13.5-15.0		•
	SS-9	89	68	- 13	63.6	23.2	5.5	7.7	14	NP	A-2-4(0)	23.5-25.0		
	SS-10	86	85	65	3.2	22.6	14.3	59.8	22	NP	A-4(0)	28.5-30.0	24.9	
	SS-11	99	80	15	42.2	43.2	4.8	9.7	14	NP	A-2-4(0)	38.5-40.0		
	SS-12	88	86	16	21.8	64.0	4.4	9.7	18	NP	A-2-4(0)	53.5-55.0		•
EB2-A	SS-13	98	85	15	30.3	56.8	5.3	7.7	16	NP	A-2-4(0)	4.1-5.6		
	SS-14	33	31	18	13.7	40.4	10.3	35.6	18	NP	A-1-b(0)	18.6-20.1		
	<b>SS-1</b> 5	18	17	12	10.9	25.9	19.6	43.6	22	NP	A-1-a(0)	28.6-30.1	•	
	SS-16	100	97	20	43.6	38.0	8.7	9.7	13	NP	A-2-4(0)	34.0-35.1		
	SS-17	85	76	17	34.5	46.1	7.7	11.7	13	NP	A-2-4(0)	38.6-40.1		
	SS-18	84	81	17	30.1	52.3	7.9	9.7	15	NP	A-2-4(0)	53.6-55.1		
	SS-19 (	25	24	14	16.8	38.4	17.0	27.9	27	7	A-2-4(0)	58.6-60.1		
B3-B	SS-49	93	82	20	36.1	45.2	8.6	10.1	27	NP	A-2-4(0)	1.0-2.2		6.7
	SS-50	100	72	10	61.9	29.7	3.4	5.0	23	NP	A-3(0)	8.8-10.3		
	SS-51	90	80	42	29.1	26.8	9.8	34.3		33	A-7-6(8)	18.2-19.7	30.8	
	SS-52	97	89	25	49.0	28.2	6.7	16.1		10	A-2-4(0)	23.2-24.7		
	SS-53	100	98	26	18.3	57.1	4.4	20.2	31	11	A-2-6(0)	28.2-29.7		
	SS-54	100	73	13	55.4	33.4	4.1	7.1	21	NP	A-2-4(0)	38.2-39.7		
	SS-55	100	97	13	13.0	75.7	3.2	8.1	27	NP	A-2-4(0)	53.2-54.7		
B1-B	SS-56	100	87	14	32.2	56.4	2.3	9.1	21	NP	A-2-4(0)	1.0-1.5		
	SS-57	100	92	64	15.3	22.8	54.8	7.1	28	8	A-4(3)	2.6-3.1		4.0
	SS-58	98	68	10	70.2	21.4	3.3	5.0		NP	A-3(0)	12.0-13.5		
	SS-59	100	98	75	7.7	23.8	18.1	50.5		54	A-7-6(42)	17.0-18.5	41.2	
	SS-60	100	90	20	31.4	49.7	4.7	14.1			• • •	27.0-28.5		
	SS-61	100	94	17	25.8	61.2	4.9	8.1				32.0-33.5		
	SS-62	100	91	12	28.0	60.8	3.1	8.1	26	NP	A-2-4(0)	52.0-53.5		
B2-A	SS-63	85	71	51	24.9	17.0					A-4(0)	2.3-3.3		14.3
	SS-64	96	49	9	73.8	18.2	3.3	4.7			A-1-b(0)	,		
	SS-65	100	94	60	17.2	25.9	15.7	41.1	53	17	A-7-5(10)	14.5-15.5	20.4	
•	SS-66	100	90	12	41.3	47.6	6.4	4.7	24	NP	A-2-4(0)	29.7-31.2		
	SS-67	100	96	11	16.0	73.6	5.8	4.7	24	NP	A-2-4(0)	49.7-51.2		

32595.1.1 B-1382 Sampson Co. Bridge No. 26 on NC 41 over Black River Overflow



**View Looking West Toward End Bent 1**