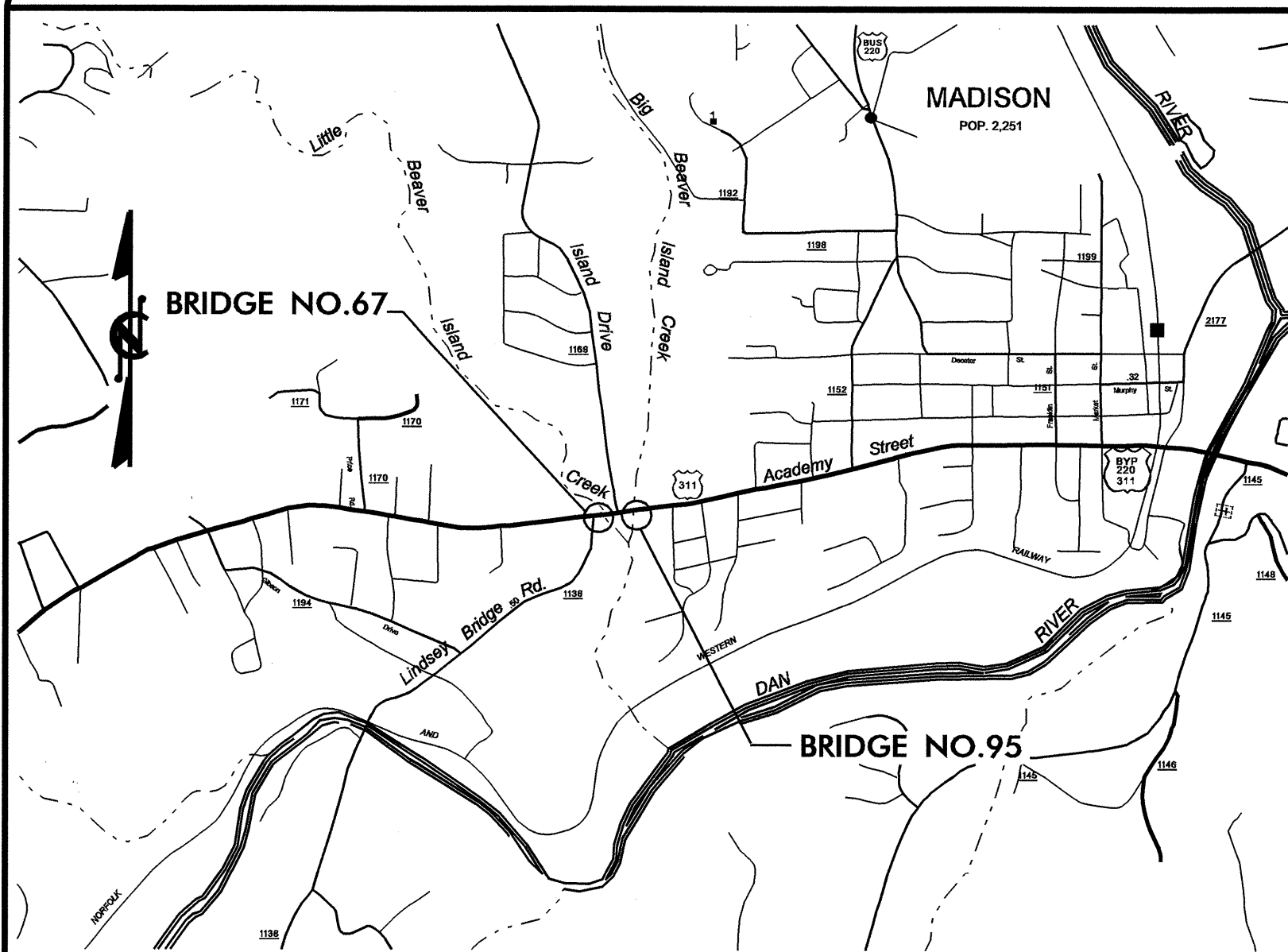


CONTRACT: C201855 TIP PROJECT: B-4252



VICINITY MAP

STATE OF NORTH CAROLINA
DIVISION OF HIGHWAYS

ROCKINGHAM COUNTY

LOCATION: BRIDGE NO. 95 OVER BIG BEAVER ISLAND CREEK ON US 311 AND BRIDGE NO. 67 OVER LITTLE BEAVER ISLAND CREEK ON US 311

TYPE OF WORK: GRADING, DRAINAGE, PAVING, STRUCTURES, & SIGNALS

STATE	STATE PROJECT REFERENCE NO.	SHEET NO.	TOTAL SHEETS
N.C.	B-4252		
STATE PROJ. NO.	F.A. PROJ. NO.	DESCRIPTION	
33594.1.1	BRSTP-311(14)	P.E.	
33594.2.1	BRSTP-311(14)	ROW & UTILITIES	
33594.3.1	BRSTP-311(22)	CONST.	

**STA. 12+75.00 -L-
BEGIN TIP PROJECT B-4252**

**END BRIDGE
-L- STA. 28+19.00**

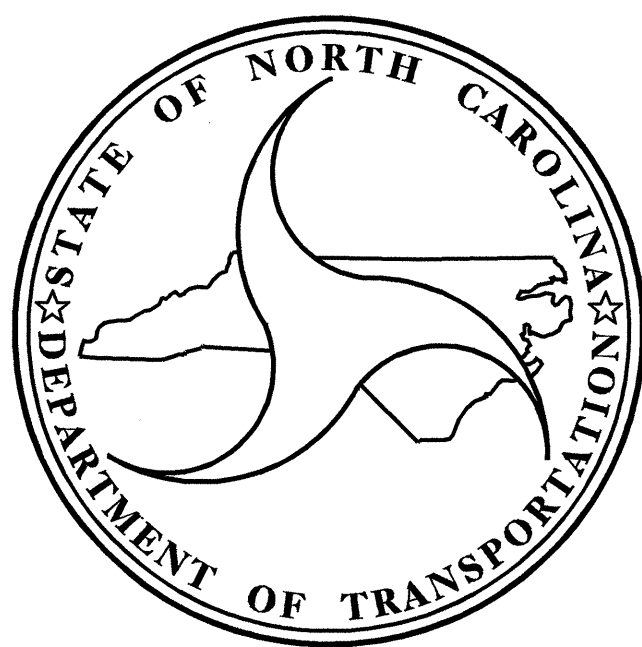
**BEGIN BRIDGE
-L- STA. 27+19.00**

**BEGIN BRIDGE
-L- STA. 31+11.50**

**END BRIDGE
-L- STA. 32+11.50**

**STA. 41+50.00 -L-
END TIP PROJECT B-4252**

STRUCTURES



DESIGN DATA

ADT 2008 =	14160
ADT 2028 =	20560
DHV =	10 %
D =	65 %
T =	3 % *
V =	50 MPH

* TTST 1 % DUAL 2 %

PROJECT LENGTH

LENGTH ROADWAY TIP PROJECT B-4252 =	0.507 MI.
LENGTH STRUCTURE TIP PROJECT B-4252 =	0.038 MI.
TOTAL LENGTH TIP PROJECT B-4252 =	0.545 MI.

Prepared In the Office of:

DIVISION OF HIGHWAYS

2006 STANDARD SPECIFICATIONS

RIGHT OF WAY DATE: JUNE 15, 2007	B. C. HUNT, P. E. PROJECT ENGINEER
LETTING DATE : August 19, 2008	T. G. PAYNE, P. E. PROJECT DESIGN ENGINEER

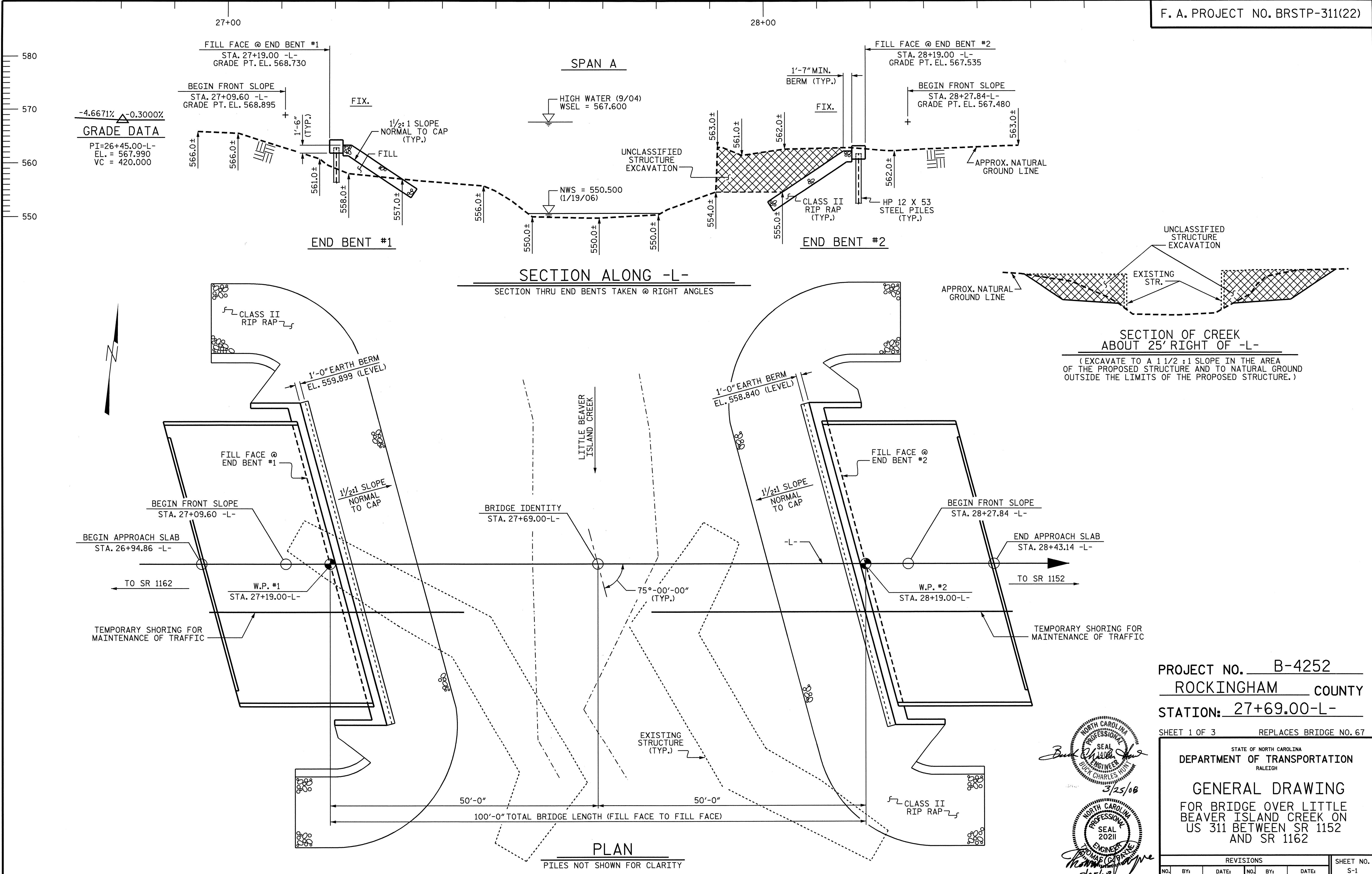
STRUCTURE DESIGN UNIT
1000 BIRCH RIDGE DR.
RALEIGH, N.C. 27610

DIVISION OF HIGHWAYS
STATE OF NORTH CAROLINA

P.E.
STATE DESIGN ENGINEER

DEPARTMENT OF TRANSPORTATION
FEDERAL HIGHWAY ADMINISTRATION

APPROVED _____
DIVISION ADMINISTRATOR DATE



GRADE DATA
 -4.6671% Δ -0.3000%
 PI=26+45.00-L-
 EL. = 567.990
 VC = 420.000

SECTION ALONG -L-
 SECTION THRU END BENTS TAKEN @ RIGHT ANGLES

SECTION OF CREEK ABOUT 25' RIGHT OF -L-

(EXCAVATE TO A 1 1/2 : 1 SLOPE IN THE AREA OF THE PROPOSED STRUCTURE AND TO NATURAL GROUND OUTSIDE THE LIMITS OF THE PROPOSED STRUCTURE.)

PLAN

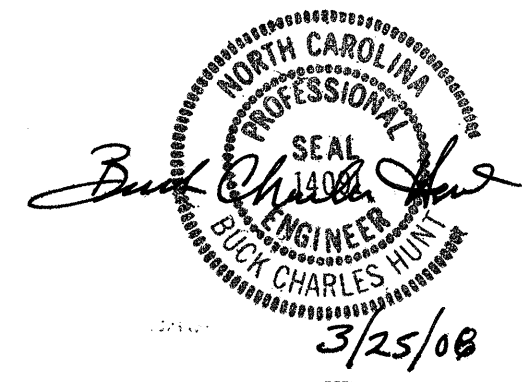
PILES NOT SHOWN FOR CLARITY

PROJECT NO. B-4252
ROCKINGHAM COUNTY
 STATION: 27+69.00-L-

SHEET 1 OF 3 REPLACES BRIDGE NO. 67

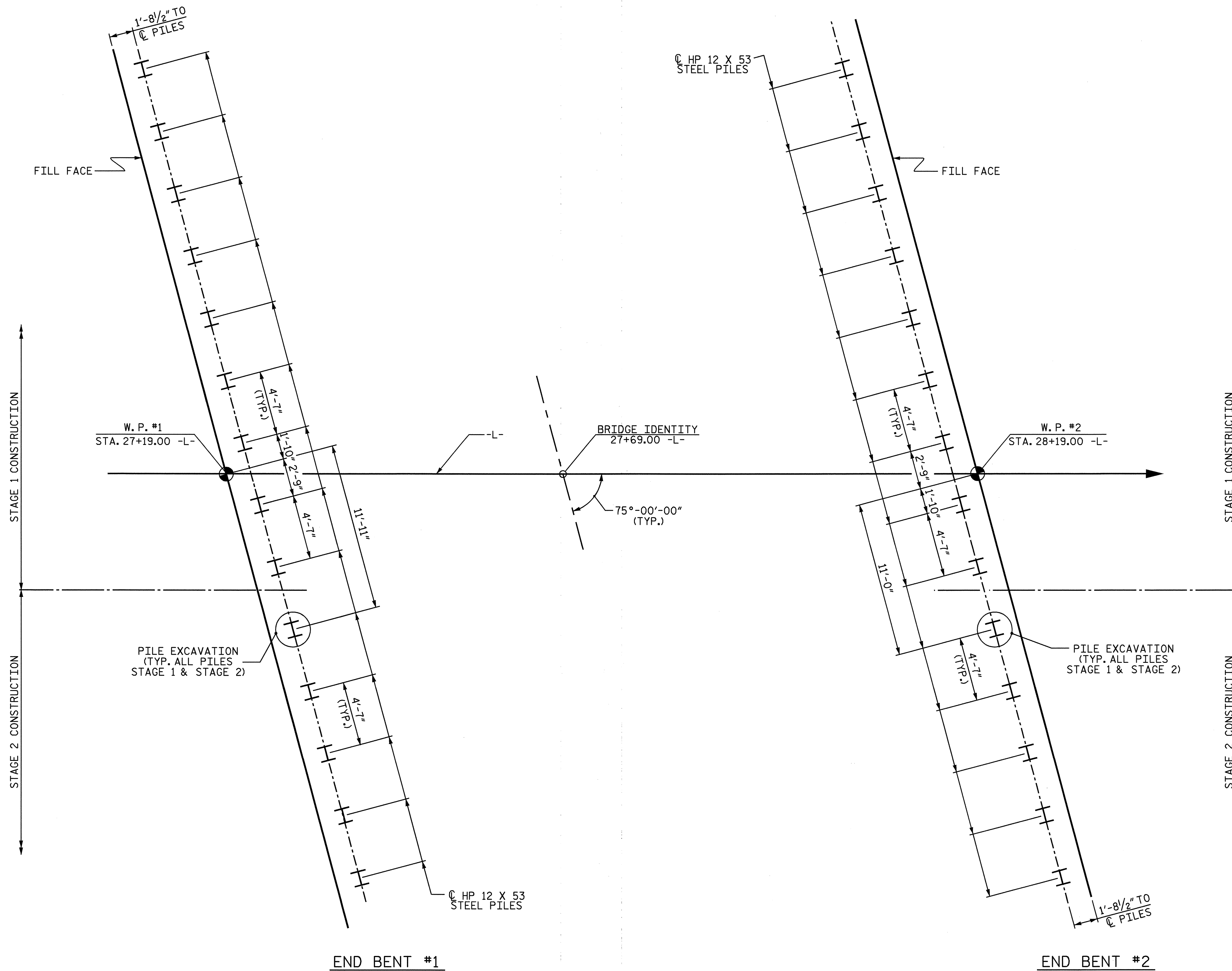
STATE OF NORTH CAROLINA
 DEPARTMENT OF TRANSPORTATION
 RALEIGH

GENERAL DRAWING
 FOR BRIDGE OVER LITTLE
 BEAVER ISLAND CREEK ON
 US 311 BETWEEN SR 1152
 AND SR 1162



REVISIONS						SHEET NO.
NO.	BY:	DATE:	NO.	BY:	DATE:	S-1
1			3			TOTAL SHEETS
2			4			70

DRAWN BY : R. G. EMERSON DATE : 01/08
 CHECKED BY : K. D. LAYNE DATE : 01/08



FOUNDATION LAYOUT

DIMENSIONS LOCATING PILES ARE SHOWN TO THE CENTERLINE OF PILES.

NOTES: DRIVE PILES AT END BENT #1 AND END BENT #2 TO A REQUIRED BEARING CAPACITY OF 100 TONS PER PILE. THE REQUIRED BEARING CAPACITY IS EQUAL TO THE ALLOWABLE BEARING CAPACITY WITH A MINIMUM FACTOR OF SAFETY OF TWO.

THE ALLOWABLE BEARING CAPACITY FOR PILES AT END BENT #1 AND END BENT #2 IS 50 TONS PER PILE.

DRIVE PILES AT END BENT #1 AND END BENT #2 TO A TIP ELEVATION NO HIGHER THAN 545.000.

PILE EXCAVATION IS REQUIRED TO INSTALL PILES AT END BENT #1 AND END BENT #2. EXCAVATE HOLES TO ELEVATION 545.000. SEE PILE EXCAVATION FOR INTEGRAL ABUTMENT SPECIAL PROVISION.

PROJECT NO. B-4252
ROCKINGHAM COUNTY
 STATION: 27+69.00 -L-

SHEET 2 OF 3

STATE OF NORTH CAROLINA
 DEPARTMENT OF TRANSPORTATION
 RALEIGH

GENERAL DRAWING
 FOR BRIDGE OVER LITTLE
 BEAVER ISLAND CREEK ON
 US 311 BETWEEN SR 1152
 AND SR 1162

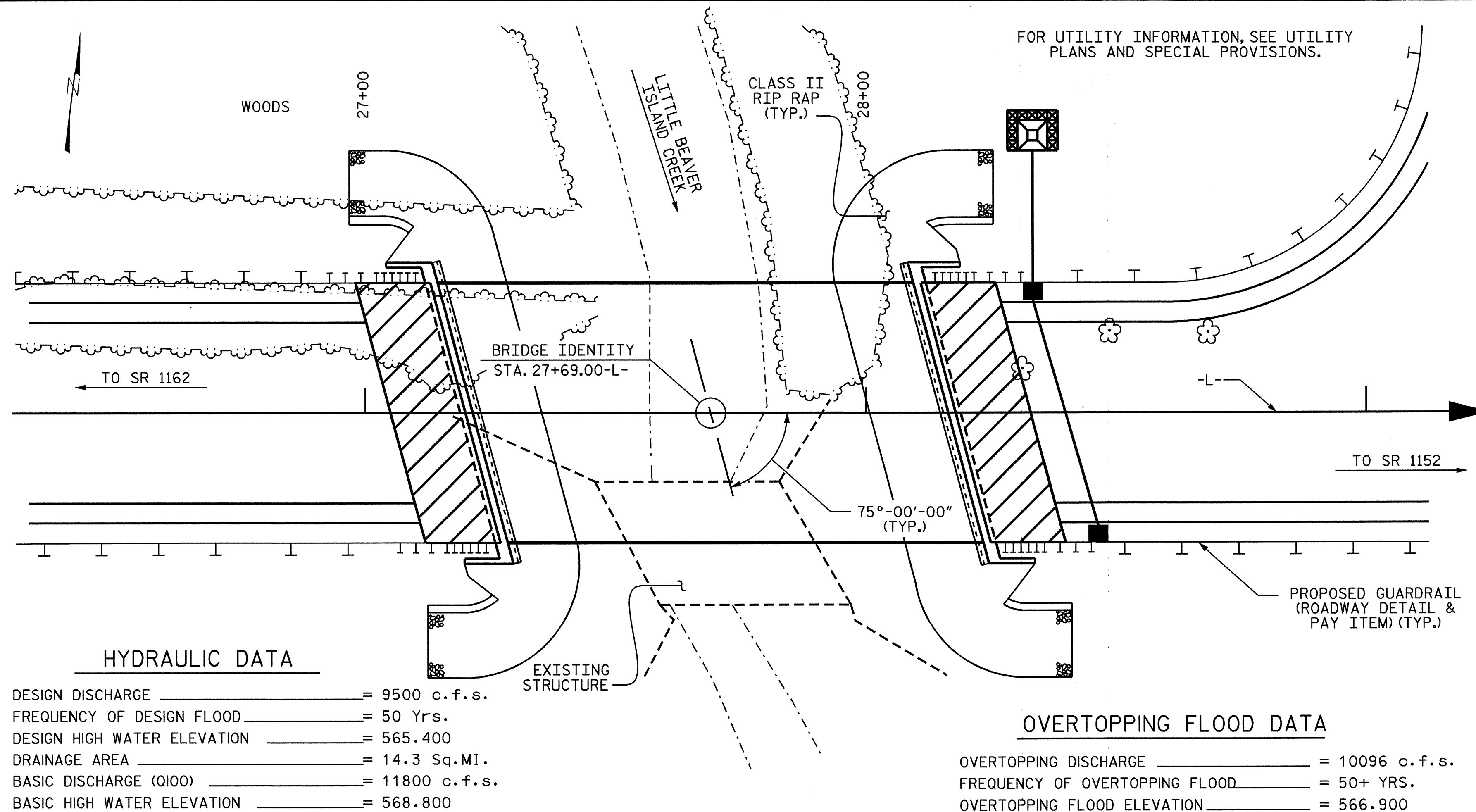


DRAWN BY : R. G. EMERSON DATE : 01/08
 CHECKED BY : K. D. LAYNE DATE : 01/08

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REVISIONS						SHEET NO.
NO.	BY:	DATE:	NO.	BY:	DATE:	S-2
1			3			TOTAL SHEETS
2			4			70

STR. #1



HYDRAULIC DATA

DESIGN DISCHARGE _____ = 9500 c.f.s.
 FREQUENCY OF DESIGN FLOOD _____ = 50 Yrs.
 DESIGN HIGH WATER ELEVATION _____ = 565.400
 DRAINAGE AREA _____ = 14.3 Sq.MI.
 BASIC DISCHARGE (Q100) _____ = 11800 c.f.s.
 BASIC HIGH WATER ELEVATION _____ = 568.800

OVERTOPPING FLOOD DATA

OVERTOPPING DISCHARGE _____ = 10096 c.f.s.
 FREQUENCY OF OVERTOPPING FLOOD _____ = 50+ YRS.
 OVERTOPPING FLOOD ELEVATION _____ = 566.900

LOCATION SKETCH

ASSUMED LIVE LOAD = HS 20 OR ALTERNATE LOADING, EXCEPT THAT THE GIRDERS HAVE BEEN DESIGNED FOR HS25.

THIS BRIDGE HAS BEEN DESIGNED IN ACCORDANCE WITH THE REQUIREMENTS OF THE AASHTO STANDARD SPECIFICATIONS FOR SEISMIC DESIGN OF HIGHWAY BRIDGES FOR SEISMIC PERFORMANCE CATEGORY A.

THE CONTRACTOR SHALL PROVIDE INDEPENDENT ASSURANCE SAMPLES OF REINFORCING STEEL AS FOLLOWS: FOR PROJECTS REQUIRING UP TO 400 TONS OF REINFORCING STEEL, ONE 30 INCH SAMPLE OF EACH SIZE BAR USED, AND FOR PROJECTS REQUIRING OVER 400 TONS OF REINFORCING STEEL, TWO 30 INCH SAMPLE OF EACH SIZE BAR USED. THE BARS FROM WHICH THE SAMPLES ARE TAKEN MUST THEN BE SPLICED WITH REPLACEMENT BARS OF THE SIZE AND LENGTH OF THE SAMPLE, PLUS A MINIMUM LAP SPLICE OF THIRTY BAR DIAMETERS.

REMOVAL OF THE EXISTING BRIDGE SHALL BE PERFORMED SO AS NOT TO ALLOW DEBRIS TO FALL INTO THE WATER. THE CONTRACTOR SHALL REMOVE THE BRIDGE AND SUBMIT PLANS FOR DEMOLITION IN ACCORDANCE ARTICLE 402-2 OF THE STANDARD SPECIFICATIONS.

AFTER SERVING AS A TEMPORARY STRUCTURE, THE EXISTING STRUCTURE LOCATED AT THE PROPOSED SITE SHALL BE REMOVED. THE EXISTING STRUCTURE CONSISTS OF 9 LINES OF W21 X 62 I-BEAMS AT 3'-3" CENTERS IN ONE SPAN OF 37'-0". THE CLEAR ROADWAY WIDTH IS 25.9' AND THE DECK IS REINFORCED CONCRETE WITH AN ASPHALT WEARING SURFACE. THE SPAN IS SUPPORTED BY ABUTMENTS OF MASS CONCRETE ON PILES. THE EXISTING BRIDGE IS PRESENTLY NOT POSTED FOR LOAD LIMIT. SHOULD THE STRUCTURAL INTEGRITY OF THE BRIDGE FURTHER DETERIORATE DURING CONSTRUCTION OF THE PROPOSED STRUCTURE, A LOAD LIMIT MAY BE POSTED AND MAY BE REDUCED AS FOUND NECESSARY DURING THE LIFE OF THE PROJECT.

THIS BRIDGE HAS BEEN DESIGNED BY THE STRENGTH DESIGN METHOD AS SPECIFIED IN AASHTO STANDARD SPECIFICATIONS.

REMOVABLE FORMS MAY BE USED IN LIEU OF METAL STAY-IN-PLACE FORMS IN ACCORDANCE WITH ARTICLE 420-3 OF THE STANDARD SPECIFICATIONS.

THE MATERIAL SHOWN IN THE CROSS-HATCHED AREA ON SHEET 1 OF 3 SHALL BE EXCAVATED BETWEEN -L- AND 30± FEET RIGHT OF -L- AS DIRECTED BY THE ENGINEER. THIS WORK WILL BE MEASURED AND PAID FOR AT THE CONTRACT UNIT PRICE PER CUBIC YARD FOR UNCLASSIFIED STRUCTURE EXCAVATION.

THE SUBSTRUCTURE OF THE EXISTING BRIDGE INDICATED ON THE PLANS IS FROM THE BEST INFORMATION AVAILABLE. SINCE THIS INFORMATION IS SHOWN FOR THE CONVENIENCE OF THE CONTRACTOR, THE CONTRACTOR SHALL HAVE NO CLAIM WHATSOEVER AGAINST THE DEPARTMENT OF TRANSPORTATION FOR ANY DELAYS OR ADDITIONAL COST INCURRED BASED ON DIFFERENCES BETWEEN THE EXISTING BRIDGE SUBSTRUCTURE SHOWN ON THE PLANS AND THE ACTUAL CONDITIONS AT THE PROJECT SITE.

THE CLASS AA CONCRETE IN THE BRIDGE DECK SHALL CONTAIN FLY ASH OR GROUND GRANULATED BLAST FURNACE SLAG AT THE SUBSTITUTION RATE SPECIFIED IN ARTICLE 1024-1 AND IN ACCORDANCE WITH ARTICLES 1024-5 AND 1024-6 OF THE STANDARD SPECIFICATIONS. NO PAYMENT WILL BE MADE FOR THIS SUBSTITUTION AS IT IS CONSIDERED INCIDENTAL TO THE COST OF THE REINFORCED CONCRETE DECK SLAB.

INASMUCH AS THE PAINT SYSTEM ON THE EXISTING STRUCTURAL STEEL CONTAINS LEAD, THE CONTRACTOR'S ATTENTION IS DIRECTED TO ARTICLE 107-1 OF THE STANDARD SPECIFICATIONS. ANY COSTS RESULTING FROM COMPLIANCE WITH APPLICABLE STATE OR FEDERAL REGULATIONS, PERTAINING TO HANDLING OF MATERIALS CONTAINING LEAD BASED PAINT SHALL BE INCLUDED IN THE BID PRICE FOR "REMOVAL OF EXISTING STRUCTURE."

NEEDLE BEAMS WILL NOT BE ALLOWED UNLESS OTHERWISE CALLED FOR ON THE PLANS OR APPROVED BY THE ENGINEER.

FOR LIMITS OF TEMPORARY SHORING FOR MAINTENANCE OF TRAFFIC, SEE TRAFFIC CONTROL PLANS. FOR PAY ITEM FOR TEMPORARY SHORING FOR MAINTENANCE OF TRAFFIC, SEE ROADWAY PLANS.

THIS STRUCTURE HAS BEEN DESIGNED IN ACCORDANCE WITH HEC 18, "EVALUATING SCOUR AT BRIDGES", MAY, 2001.

FOR FALSEWORK AND FORMWORK, SEE SPECIAL PROVISIONS.

FOR GROUT FOR STRUCTURES, SEE SPECIAL PROVISIONS.

FOR EROSION CONTROL MEASURES, SEE EROSION CONTROL PLANS.

FOR OTHER DESIGN DATA AND GENERAL NOTES, SEE SHEET SN.

ALL STRUCTURAL STEEL SHALL BE AASHTO M270 GRADE 50W.

FOR SUBMITTAL OF WORKING DRAWINGS, SEE SPECIAL PROVISIONS.

FOR SHIPPING STEEL STRUCTURAL MEMBERS, SEE SPECIAL PROVISIONS.

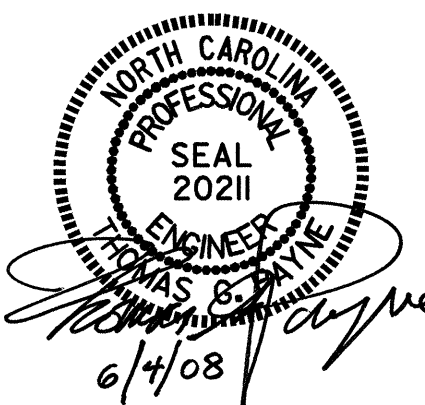
FOR CRANE SAFETY, SEE SPECIAL PROVISIONS.

TOTAL BILL OF MATERIAL

	REMOVAL OF EXISTING STRUCTURE	PILE EXCAVATION IN SOIL	PILE EXCAVATION NOT IN SOIL	UNCLASSIFIED STRUCTURE EXCAVATION	REINFORCED CONCRETE DECK SLAB	GROOVING BRIDGE FLOORS	CLASS A CONCRETE	BRIDGE APPROACH SLABS	REINFORCING STEEL	STRUCTURAL STEEL	HP 12 X 53 STEEL PILES		2 BAR METAL RAIL	1'-2" X 2'-6" CONCRETE PARAPET	RIP RAP CLASS II (2'-0" THICK)	FILTER FABRIC FOR DRAINAGE	EVAZOTE JOINT SEALS	ELECTRICAL CONDUIT SYSTEM
	LUMP SUM	LIN. FT.	LIN. FT.	CU. YDS.	SQ. FT.	SQ. FT.	CU. YDS.	LUMP SUM	LBS.	APPROX. LBS.	NO.	LIN. FT.	LIN. FT.	LIN. FT.	TONS	SQ. YDS.	LUMP SUM	LUMP SUM
SUPERSTRUCTURE	LUMP SUM			627	5,364	7,034				173,800			180.92	196.55			LUMP SUM	LUMP SUM
END BENT NO. 1		198	18				29.1		4,457		14	210			265	295		
END BENT NO. 2		165	35				28.4		4,389		14	210			300	335		
TOTAL	LUMP SUM	363	53	627	5,364	7,034	57.5	LUMP SUM	8,846	173,800	28	420	180.92	196.55	565	630	LUMP SUM	LUMP SUM

DRAWN BY : R. G. EMERSON DATE : 01/08
 CHECKED BY : K. D. LAYNE DATE : 01/08

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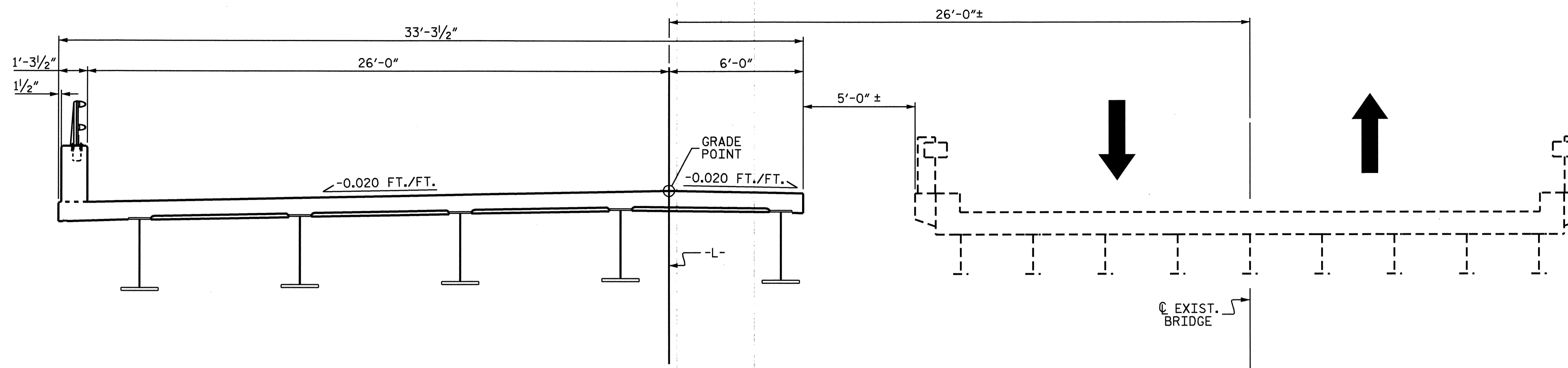
PROJECT NO. B-4252
 ROCKINGHAM COUNTY
 STATION: 27+69.00-L-

SHEET 3 OF 3

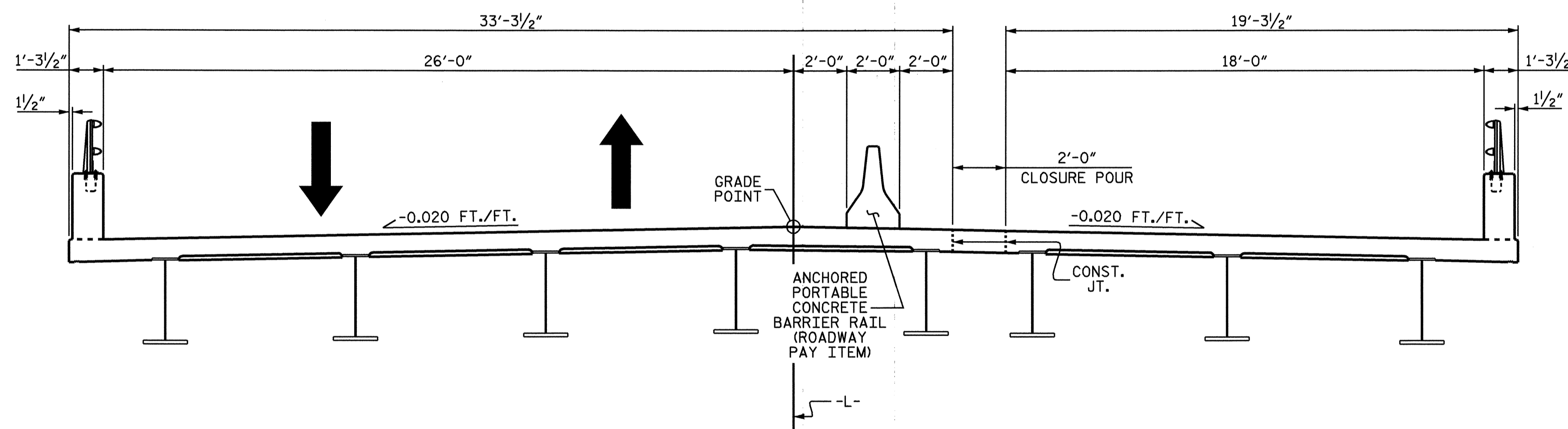
STATE OF NORTH CAROLINA
 DEPARTMENT OF TRANSPORTATION
 RALEIGH

GENERAL DRAWING
 FOR BRIDGE OVER LITTLE
 BEAVER ISLAND CREEK ON
 US 311 BETWEEN SR 1152
 AND SR 1162

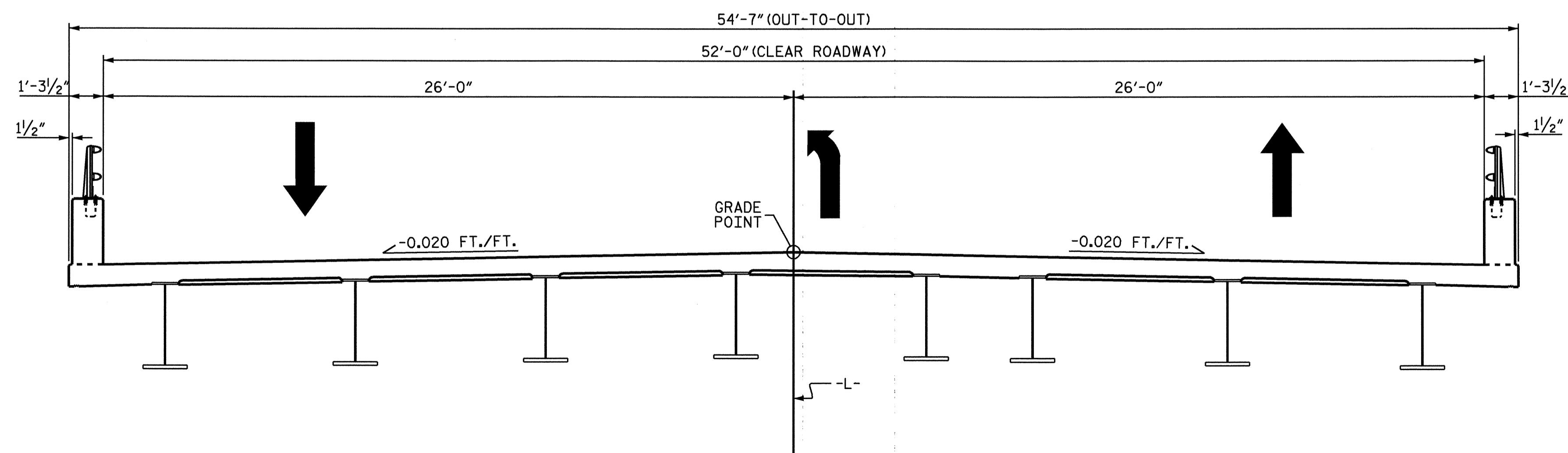
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NO.	BY:	DATE:	NO.	BY:	DATE:	S-3
1			3			TOTAL SHEETS 70
2			4			



STAGE 1



STAGE 2



FINAL TYPICAL SECTION

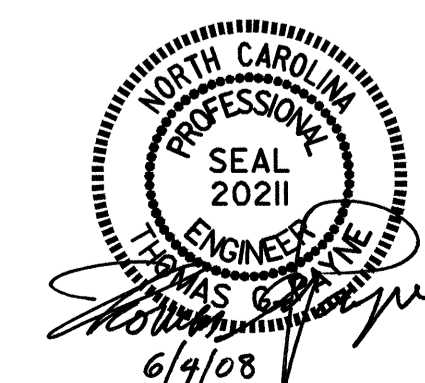
NOTES
 FOR PHASING AND MAINTENANCE OF TRAFFIC,
 SEE TRAFFIC CONTROL PLANS.
 SEE TRAFFIC CONTROL PLANS FOR LOCATION
 AND PAY LIMITS OF THE ANCHORED PORTABLE
 CONCRETE BARRIER RAIL.

PROJECT NO. B-4252
ROCKINGHAM COUNTY
 STATION: 27+69.00 -L-

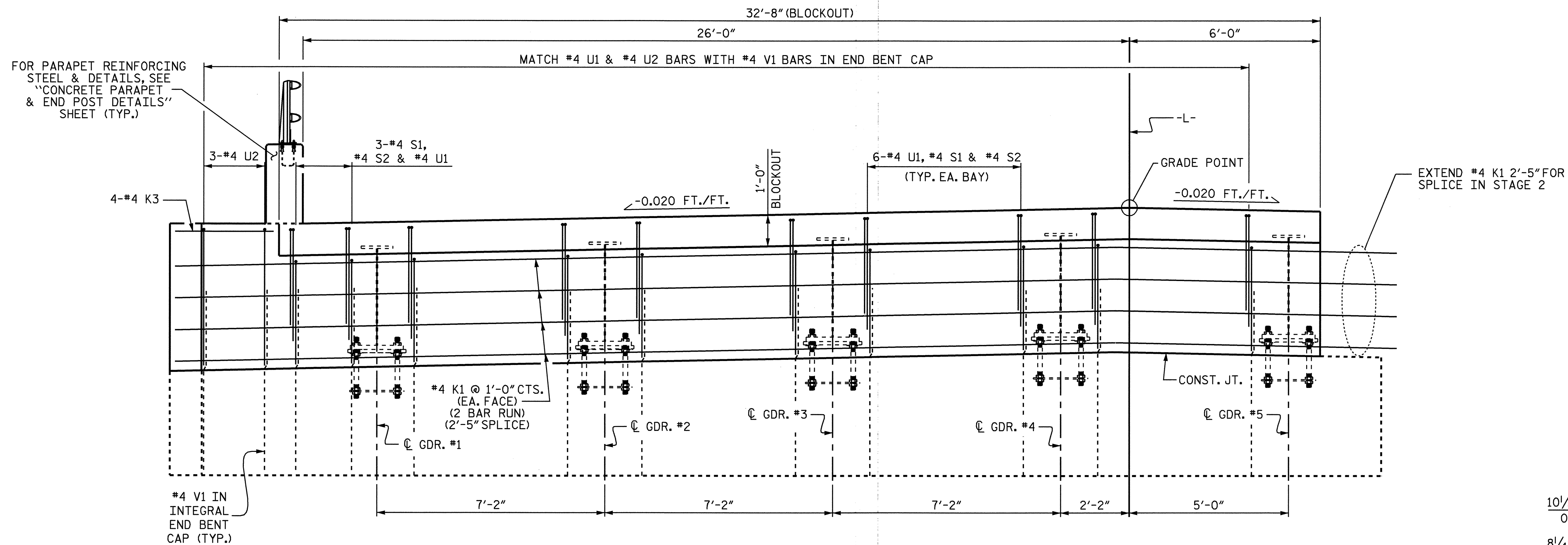
STATE OF NORTH CAROLINA
 DEPARTMENT OF TRANSPORTATION
 RALEIGH

CONSTRUCTION SEQUENCE

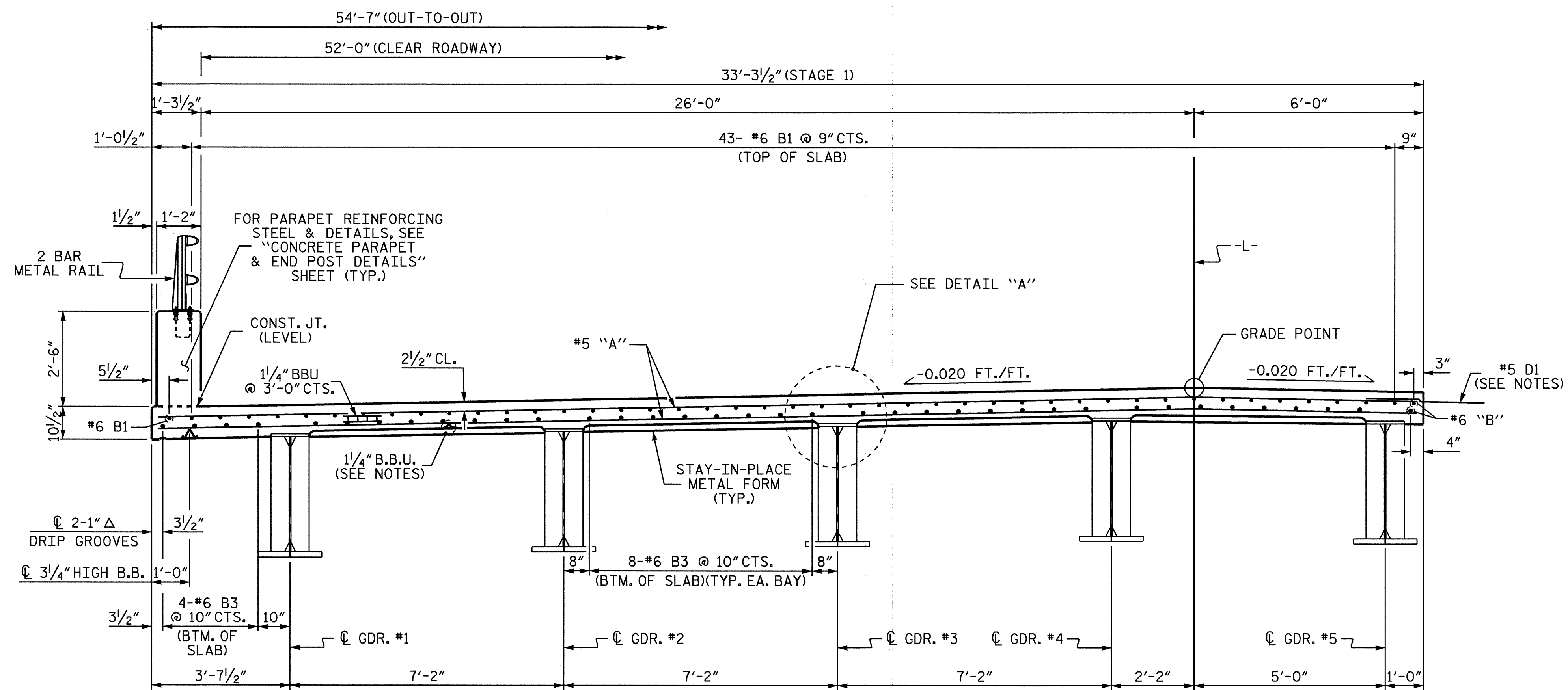
REVISIONS						SHEET NO.
NO.	BY:	DATE:	NO.	BY:	DATE:	S-4
1			3			TOTAL SHEETS
2			4			70



DRAWN BY : S. DOMBROWSKI DATE : 1/07
 CHECKED BY : M.K. BEARD DATE : 10/07



TYPICAL SECTION @ INTEGRAL END BENT
(DECK REINFORCEMENT NOT SHOWN FOR CLARITY)



TYPICAL SECTION @ INTEGRAL END BENT
(SHOWING DECK REINFORCEMENT, REINFORCEMENT IN ABUTMENT NOT SHOWN FOR CLARITY)

NOTES

PROVIDE 1/4" HIGH BEAM BOLSTERS UPPER AT 4'-0" CTS. ATOP THE METAL STAY-IN-PLACE FORMS TO SUPPORT THE BOTTOM MAT OF 'A' BARS. WHEN USING REMOVABLE FORMS, PROVIDE CONTINUOUS HIGH CHAIRS FOR METAL DECK (C.H.C.M.) @ 4'-0" CTS. WITH A HEIGHT TO SUPPORT THE BOTTOM MAT OF 'A' BARS A CLEAR DISTANCE OF 2 1/2" ABOVE THE TOP OF THE REMOVABLE FORM.

METAL STAY-IN-PLACE FORMS SHALL NOT BE WELDED TO BEAM OR GIRDER FLANGES IN THE ZONES REQUIRING CHARPY V-NOTCH TEST. SEE STRUCTURAL STEEL DETAIL SHEETS.

PARAPET IN THE SPAN SHALL NOT BE CAST UNTIL ALL SLAB CONCRETE IN THE SPAN HAS BEEN CAST AND HAS REACHED A MINIMUM COMPRESSIVE STRENGTH OF 3,000 PSI.

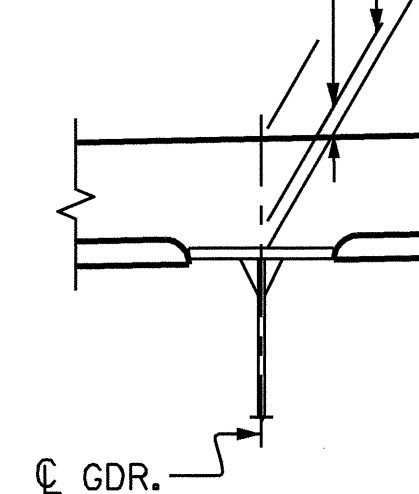
THE CONTRACTOR MAY, WHEN NECESSARY, PROPOSE A SCHEME FOR AVOIDING INTERFERENCE BETWEEN METAL STAY-IN-PLACE FORM SUPPORTS OR FORMS AND BEAM/GIRDER STIFFENERS OR CONNECTOR PLATES. THE PROPOSAL SHALL BE INDICATED, AS APPROPRIATE, ON EITHER THE STEEL WORKING DRAWINGS OR THE METAL STAY-IN-PLACE FORM WORKING DRAWINGS.

DOWELS SHALL BE PLACED IN THE SAME HORIZONTAL PLANE AS THE TOP SLAB REINFORCING STEEL.

10 1/4" TOP OF SLAB TO BOTTOM OF TOP FLANGE @ C GDR.

8 1/4" TOP OF SLAB TO TOP OF S.I.P. FORMS @ C GDR.

2" BUILD-UP @ C GDR.



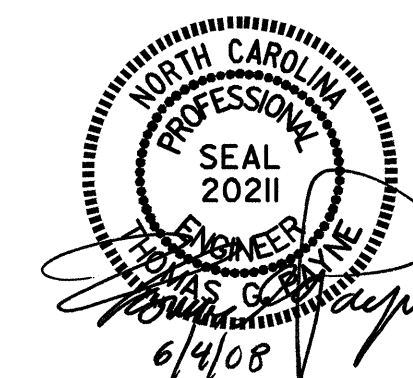
DETAIL "A"

PROJECT NO. B-4252
ROCKINGHAM COUNTY
STATION: 27+69.00 -L-

SHEET 1 OF 4

STATE OF NORTH CAROLINA
DEPARTMENT OF TRANSPORTATION
RALEIGH

SUPERSTRUCTURE
TYPICAL SECTION
(STAGE 1)

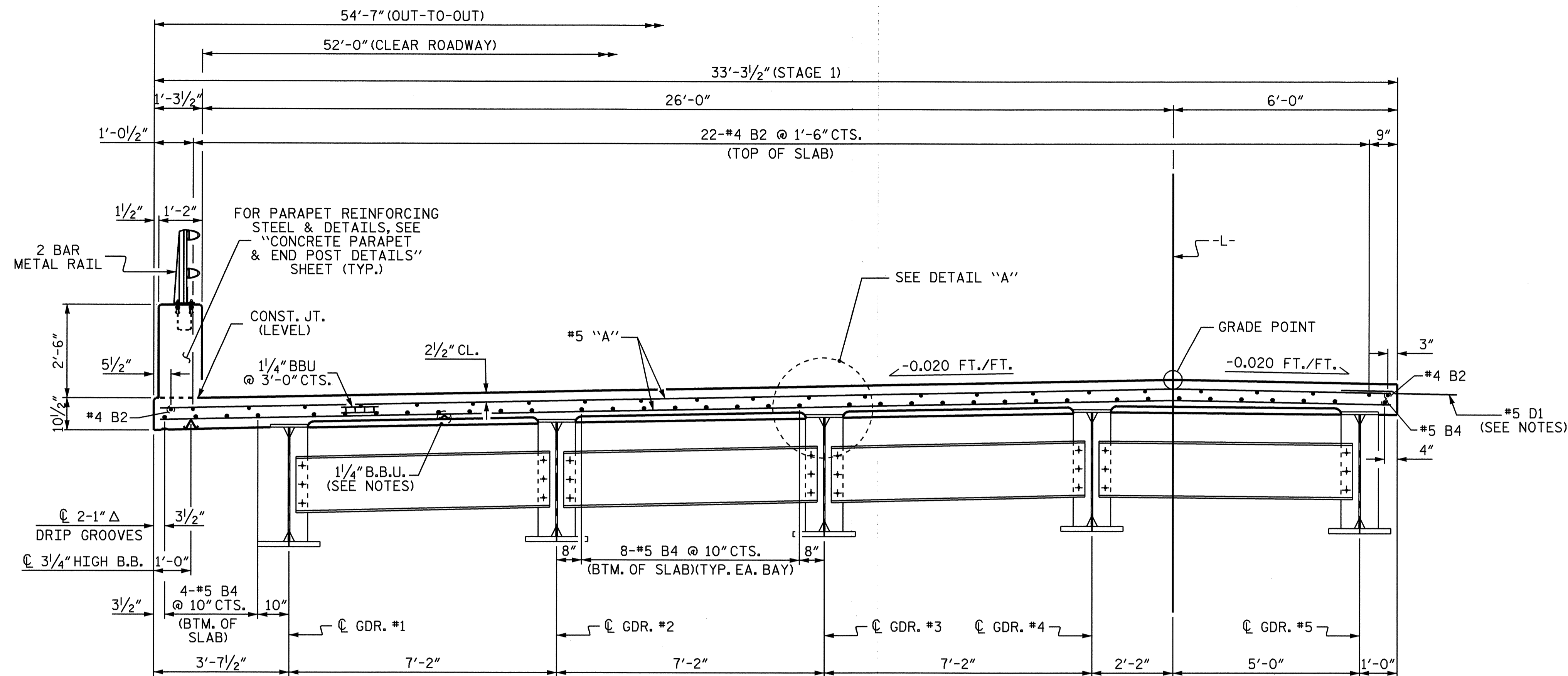


REVISIONS						SHEET NO. S-5 TOTAL SHEETS 70
NO.	BY:	DATE:	NO.	BY:	DATE:	
1			3			70
2			4			

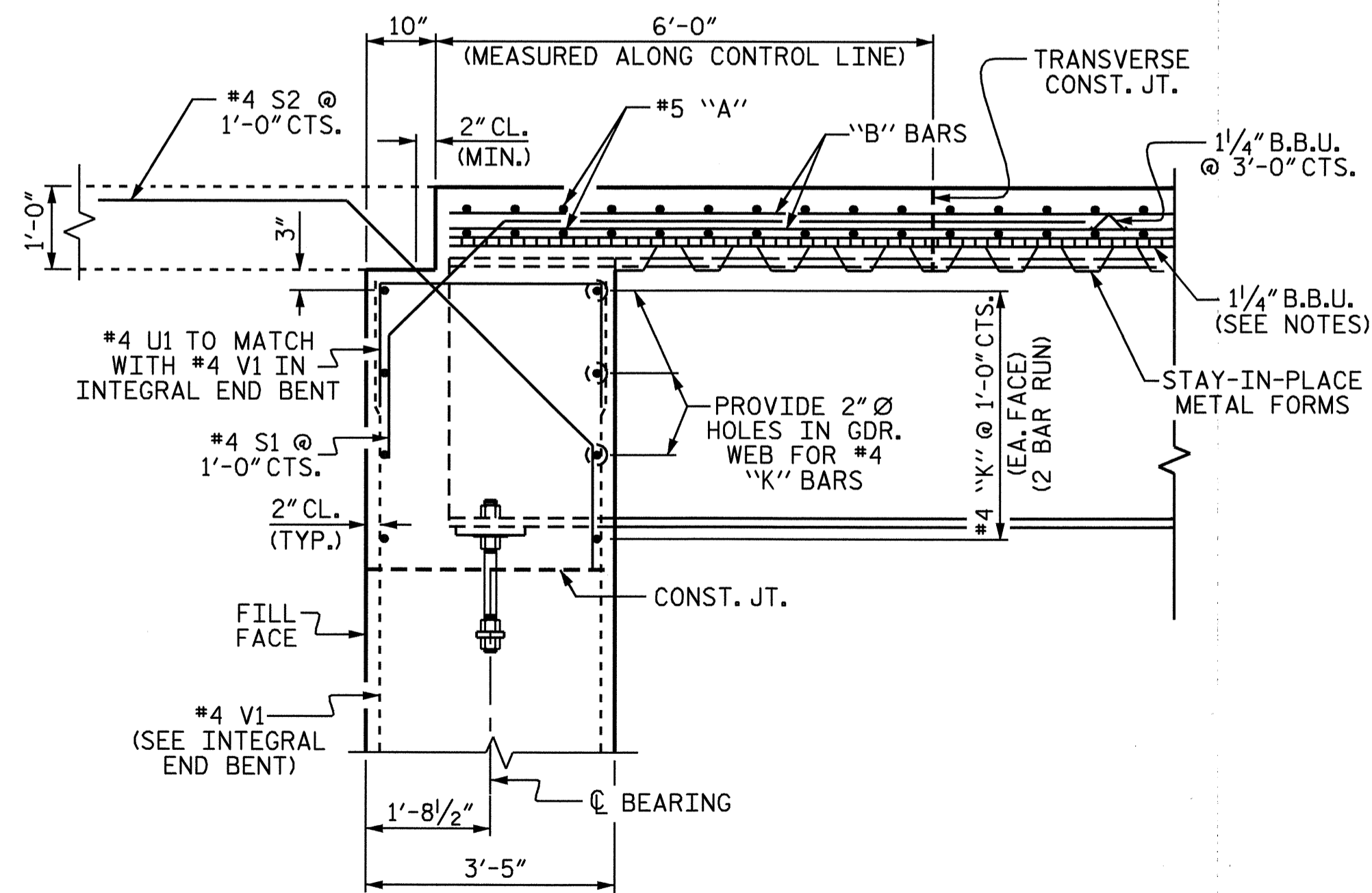
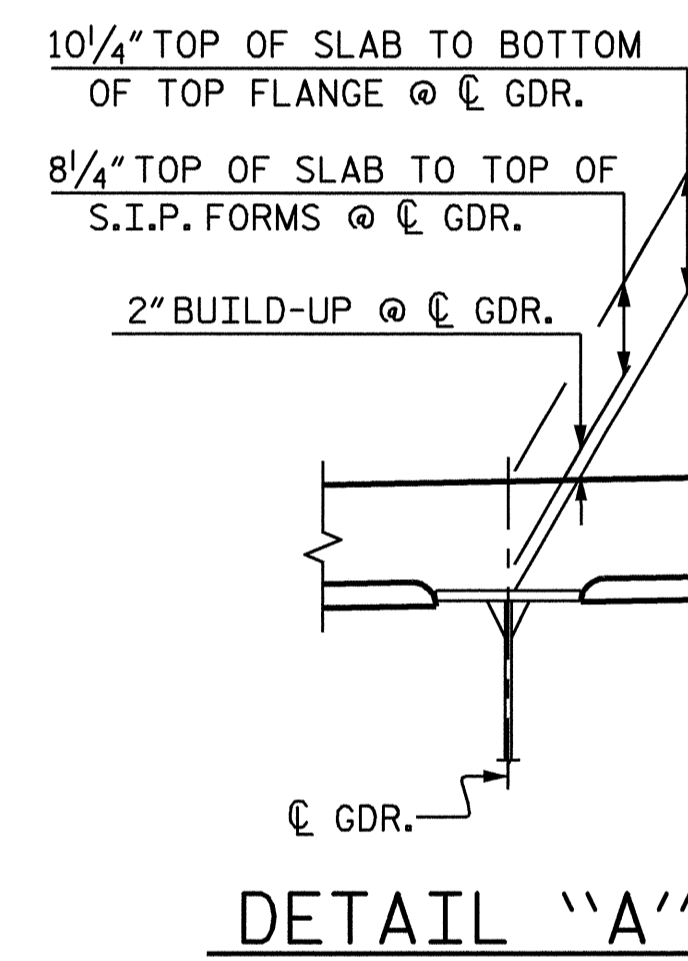
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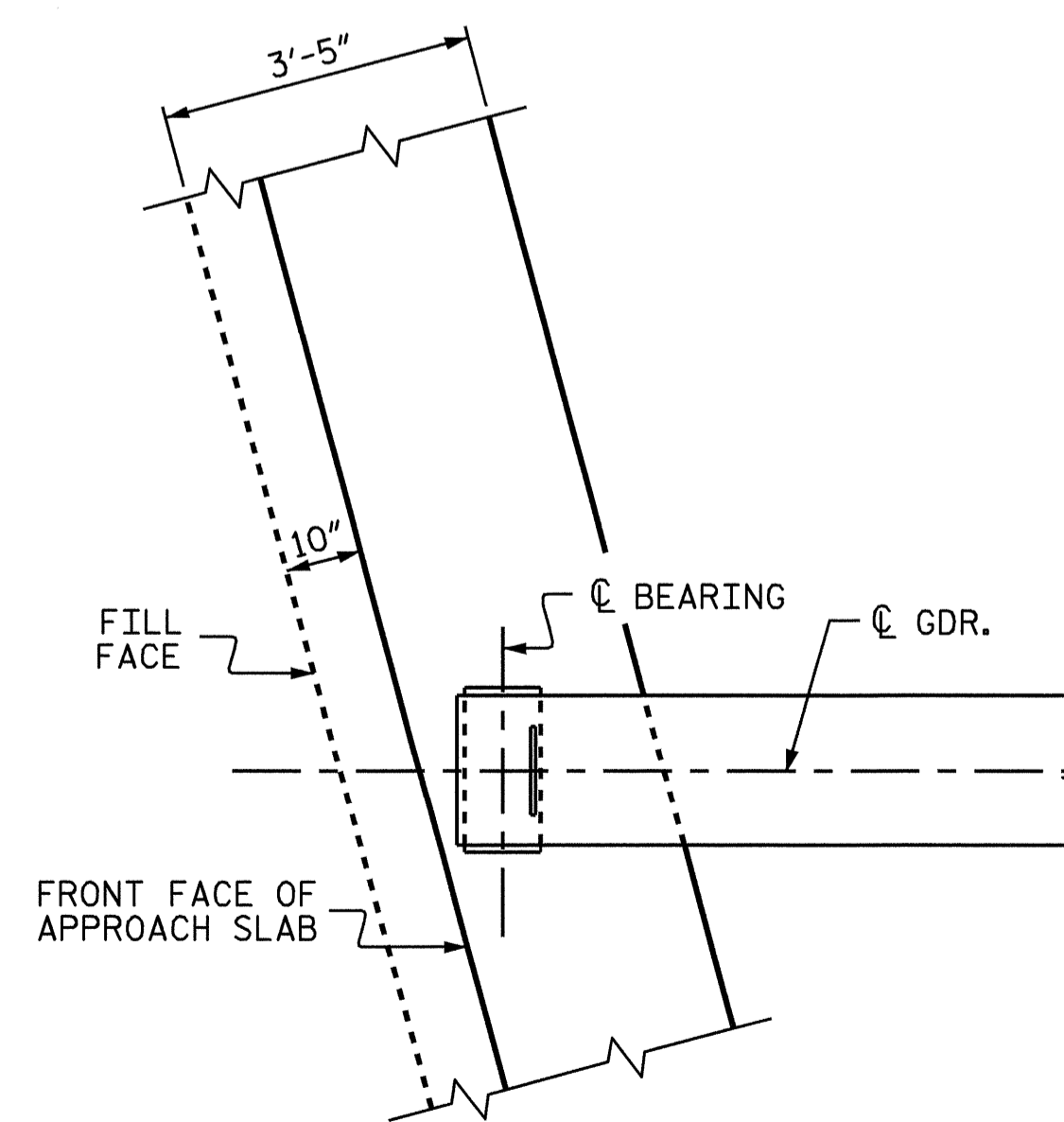
STR. #1



TYPICAL SECTION @ INTERMEDIATE DIAPHRAGM



SECTION THRU INTEGRAL END BENT



PLAN OF GIRDER @ END BENT #1

END BENT #1 SHOWN, END BENT #2 SIMILAR

PROJECT NO. B-4252
 ROCKINGHAM COUNTY
 STATION: 27+69.00 -L-

SHEET 2 OF 4

STATE OF NORTH CAROLINA
 DEPARTMENT OF TRANSPORTATION
 RALEIGH

SUPERSTRUCTURE
 TYPICAL SECTION
 (STAGE 1)

REVISIONS

NO.	BY:	DATE:	NO.	BY:	DATE:
1			3		
2			4		

SHEET NO. S-6

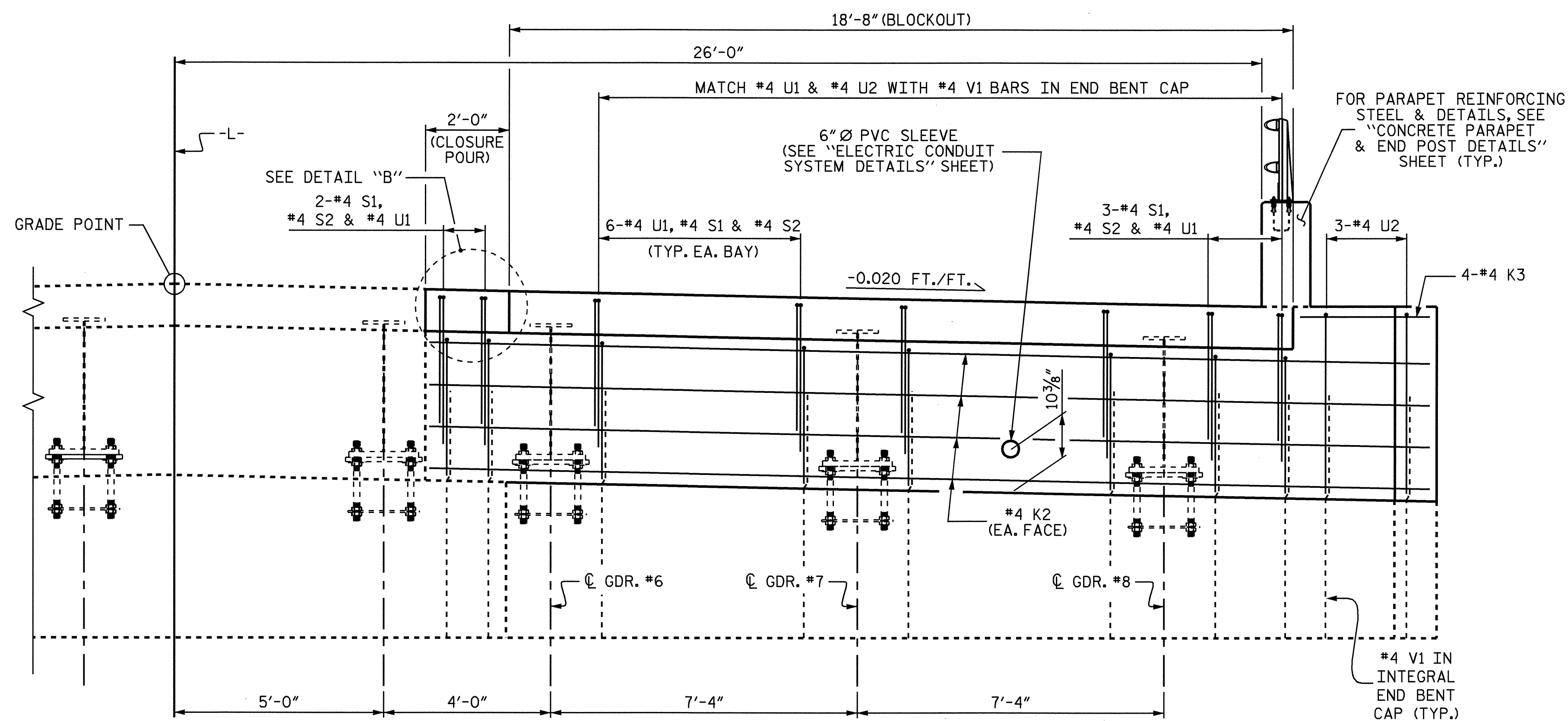
TOTAL SHEETS 70

DRAWN BY: S. DOMBROWSKI DATE: 12/06
 CHECKED BY: M.K. BEARD DATE: 10/07

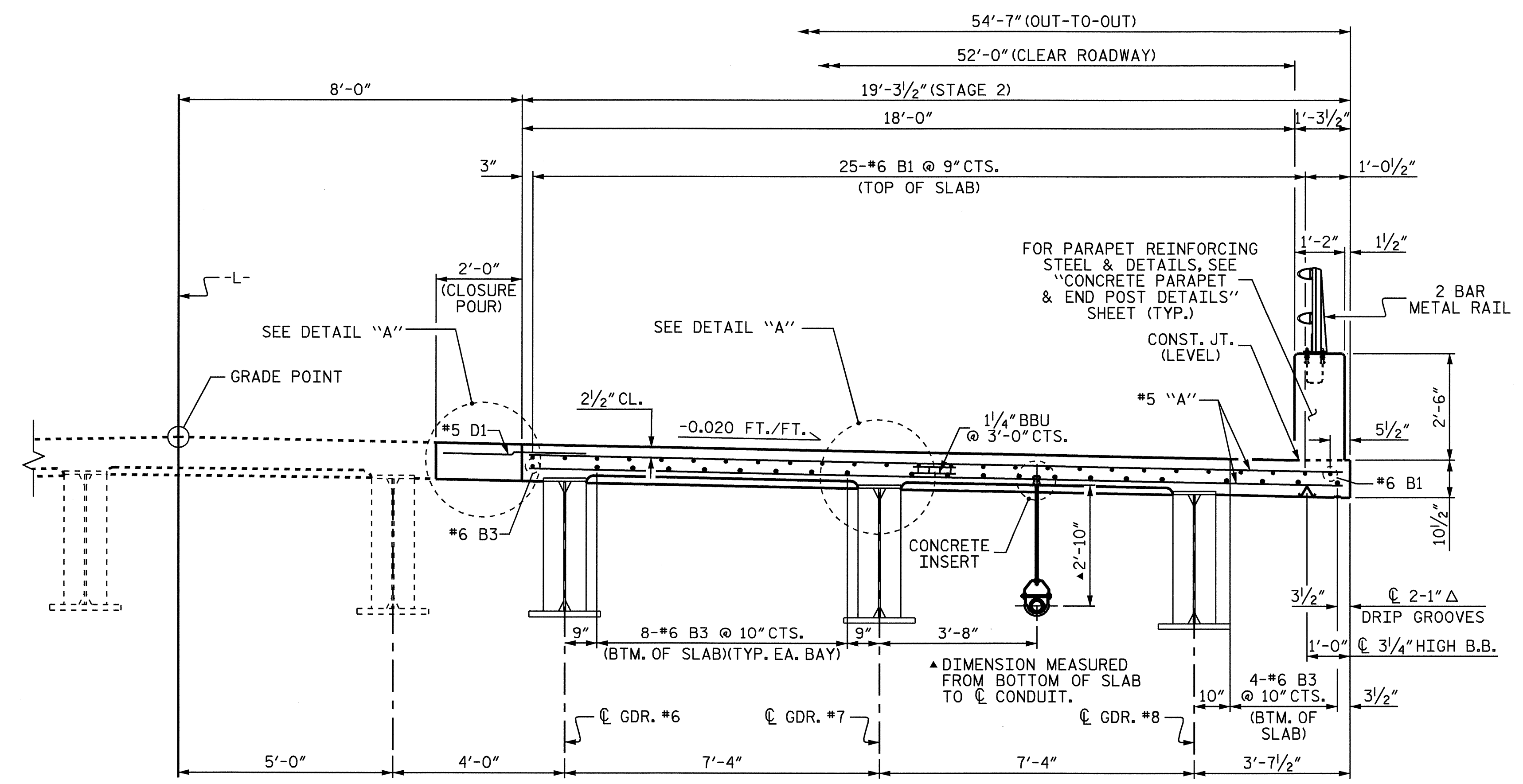
02-JUN-2008 13:48
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 tpsyne



STR. #1



TYPICAL SECTION @ INTEGRAL END BENT
(DECK REINFORCEMENT NOT SHOWN FOR CLARITY)



TYPICAL SECTION @ INTEGRAL END BENT
(SHOWING DECK REINFORCEMENT, REINFORCEMENT IN ABUTMENT NOT SHOWN FOR CLARITY)

NOTES

PROVIDE 1/4" HIGH BEAM BOLSTERS UPPER AT 4'-0" CTS. ATOP THE METAL STAY-IN-PLACE FORMS TO SUPPORT THE BOTTOM MAT OF 'A' BARS. WHEN USING REMOVABLE FORMS, PROVIDE CONTINUOUS HIGH CHAIRS FOR METAL DECK (C.H.C.M.) @ 4'-0" CTS. WITH A HEIGHT TO SUPPORT THE BOTTOM MAT OF 'A' BARS A CLEAR DISTANCE OF 2 1/2" ABOVE THE TOP OF THE REMOVABLE FORM.

METAL STAY-IN-PLACE FORMS SHALL NOT BE WELDED TO BEAM OR GIRDER FLANGES IN THE ZONES REQUIRING CHARTY V-NOTCH TEST. SEE STRUCTURAL STEEL DETAIL SHEETS.

PARAPET IN THE SPAN SHALL NOT BE CAST UNTIL ALL SLAB CONCRETE IN THE SPAN HAS BEEN CAST AND HAS REACHED A MINIMUM COMPRESSIVE STRENGTH OF 3,000 PSI.

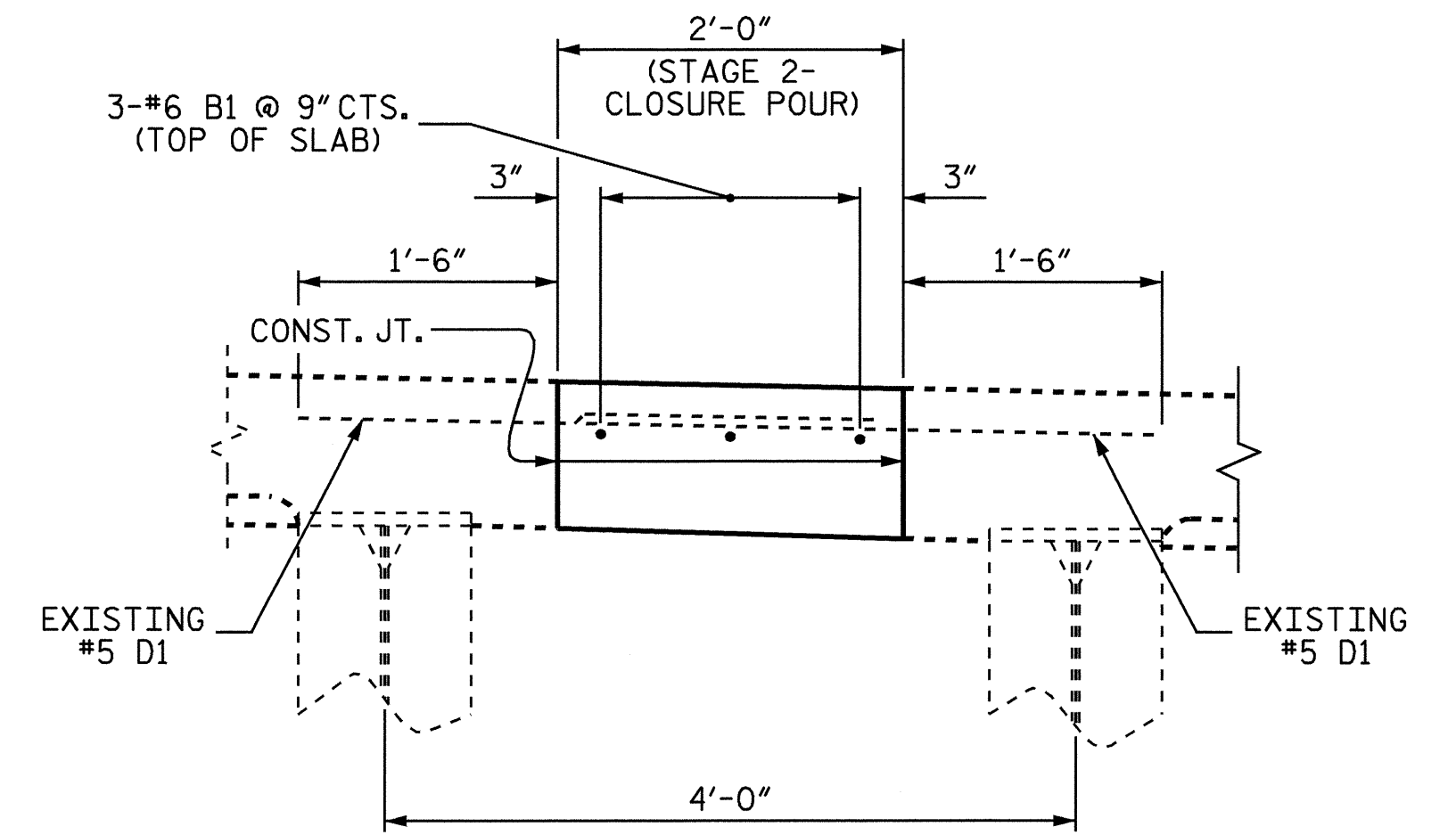
THE CONTRACTOR MAY, WHEN NECESSARY, PROPOSE A SCHEME FOR AVOIDING INTERFERENCE BETWEEN METAL STAY-IN-PLACE FORM SUPPORTS OR FORMS AND BEAM/GIRDER STIFFENERS OR CONNECTOR PLATES. THE PROPOSAL SHALL BE INDICATED, AS APPROPRIATE, ON EITHER THE STEEL WORKING DRAWINGS OR THE METAL STAY-IN-PLACE FORM WORKING DRAWINGS.

DOWELS SHALL BE PLACED IN THE SAME HORIZONTAL PLANE AS THE TOP SLAB REINFORCING STEEL.

FOR LOCATION AND DETAILS OF MULTI-CELL RACEWAY, HANGER ASSEMBLY, AND CONCRETE INSERTS, SEE "ELECTRICAL CONDUIT SYSTEM DETAILS" SHEET.

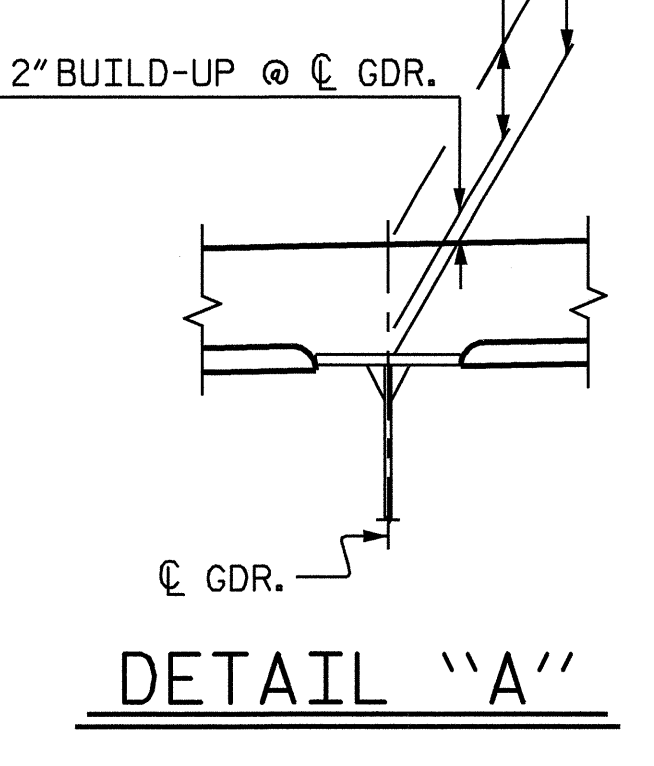
FOR ELECTRICAL CONDUIT SYSTEM, SEE SPECIAL PROVISIONS.

THE CONTRACTOR SHALL PROVIDE FOR INSTALLATION OF THE ELECTRICAL CONDUIT SYSTEM THROUGH THE INTEGRAL END BENT. REINFORCING STEEL MAY BE SHIFTED AS NECESSARY TO CLEAR THE CONDUIT.



DETAIL "B"

10 1/4" TOP OF SLAB TO BOTTOM OF TOP FLANGE @ C.G.R.
8 1/4" TOP OF SLAB TO TOP OF S.I.P. FORMS @ C.G.R.



DETAIL "A"

PROJECT NO. B-4252
ROCKINGHAM COUNTY
STATION: 27+69.00 -L-

SHEET 3 OF 4

STATE OF NORTH CAROLINA
DEPARTMENT OF TRANSPORTATION
RALEIGH

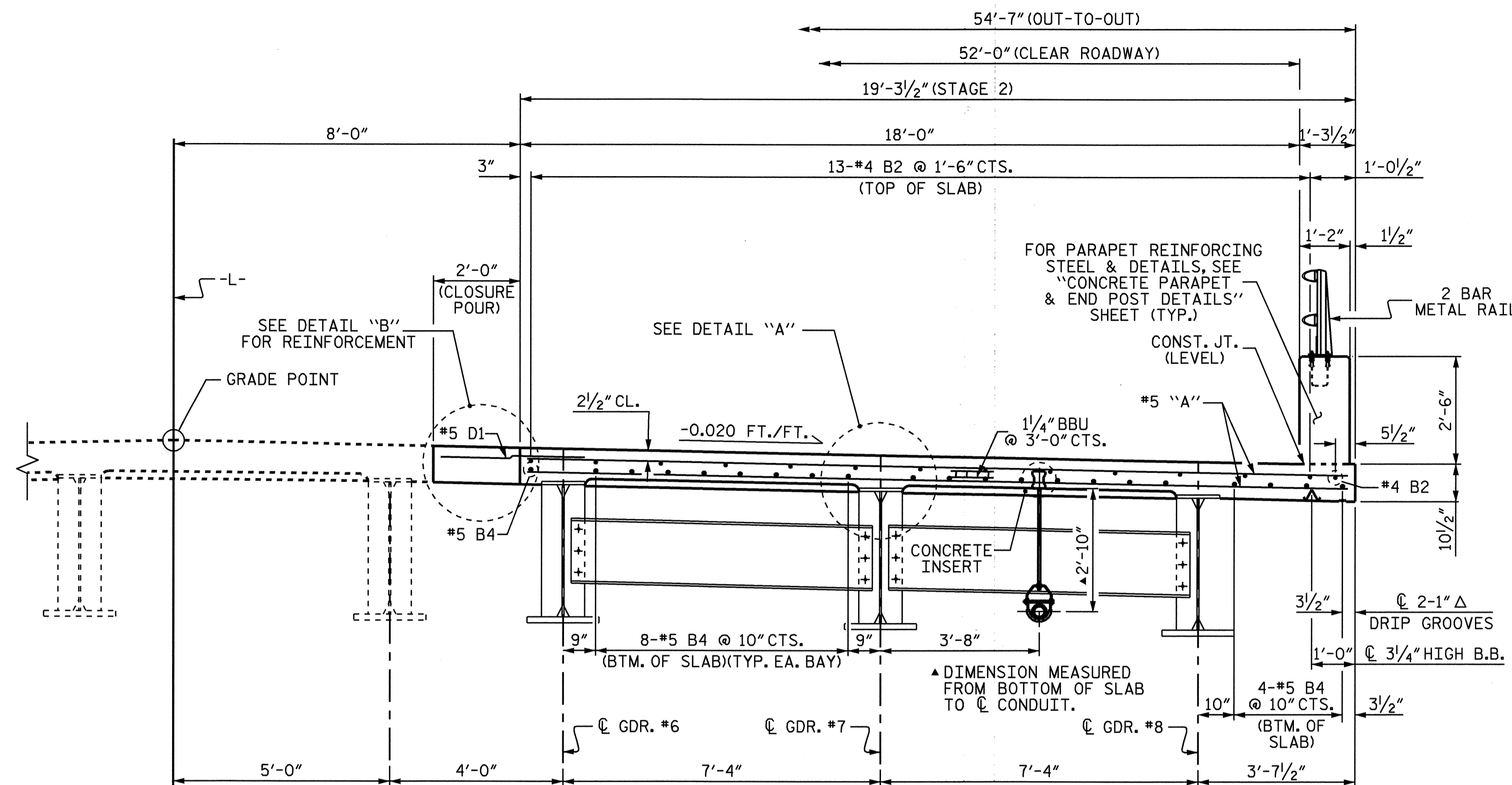
SUPERSTRUCTURE
TYPICAL SECTION
(STAGE 2)

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CHECKED BY: M.K. BEARD DATE: 10/07

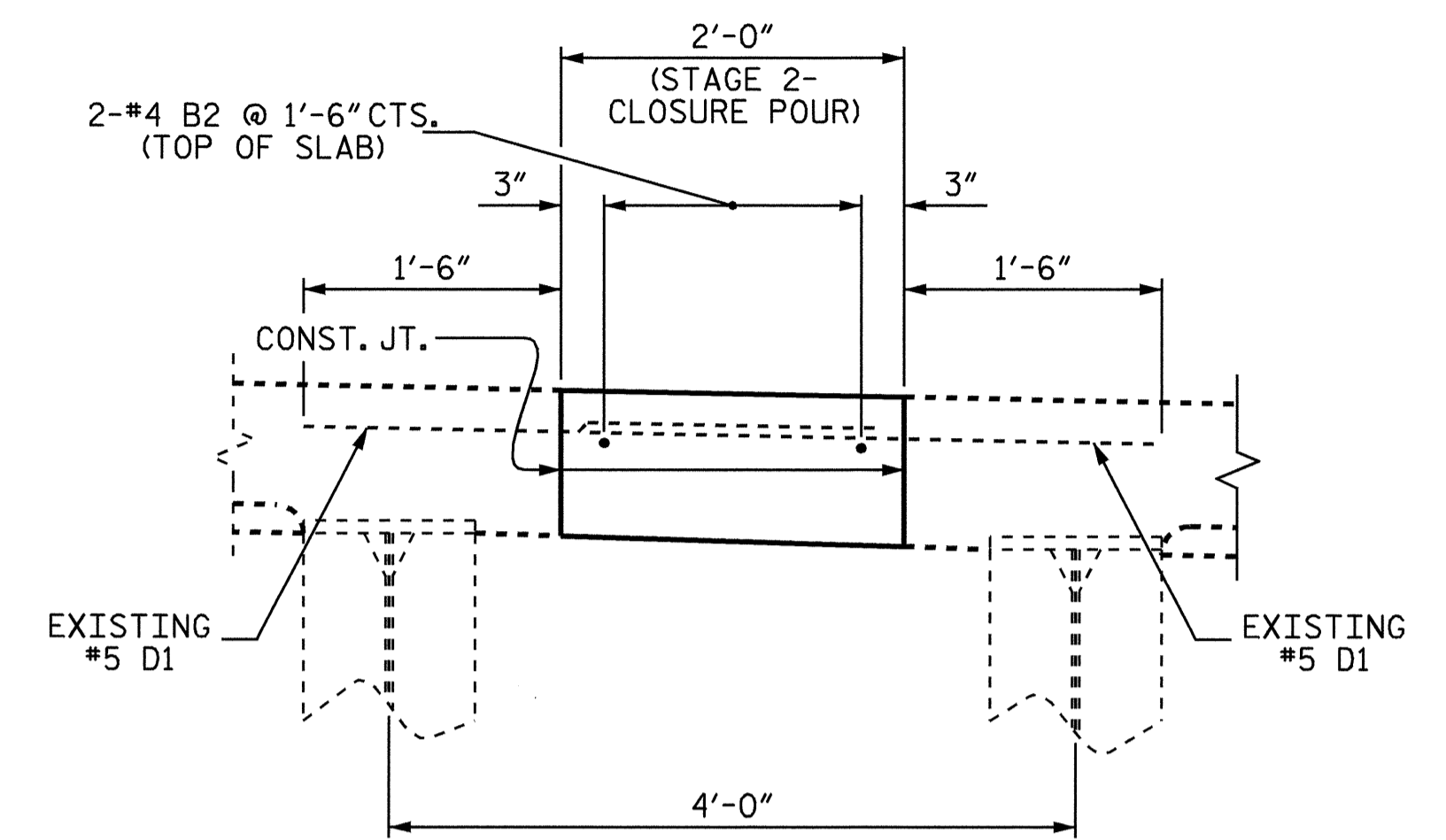
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tpayne



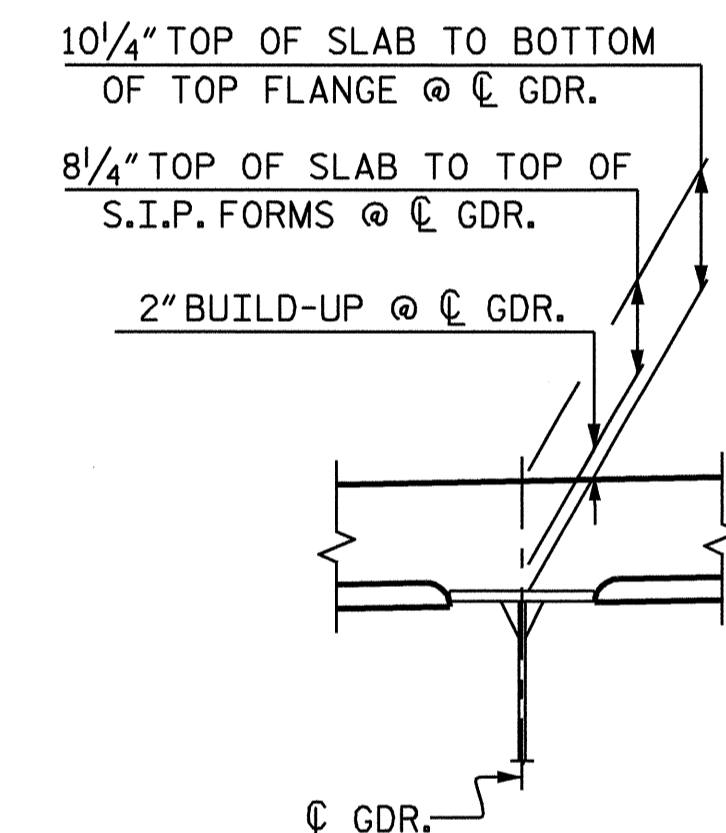
REVISIONS						SHEET NO.
NO.	BY:	DATE:	NO.	BY:	DATE:	
1			3			S-7
2			4			TOTAL SHEETS 70



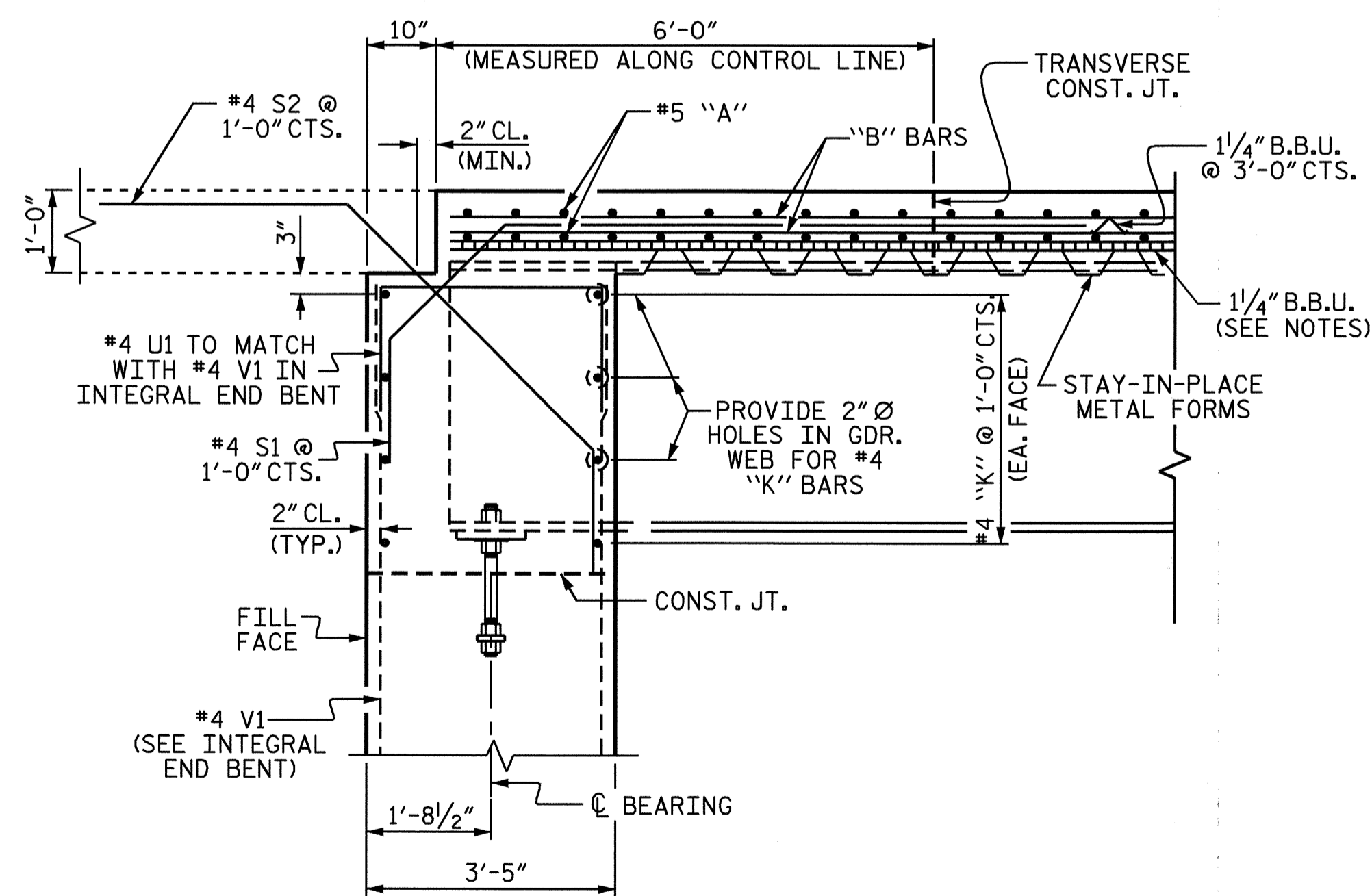
TYPICAL SECTION @ INTERMEDIATE DIAPHRAGM



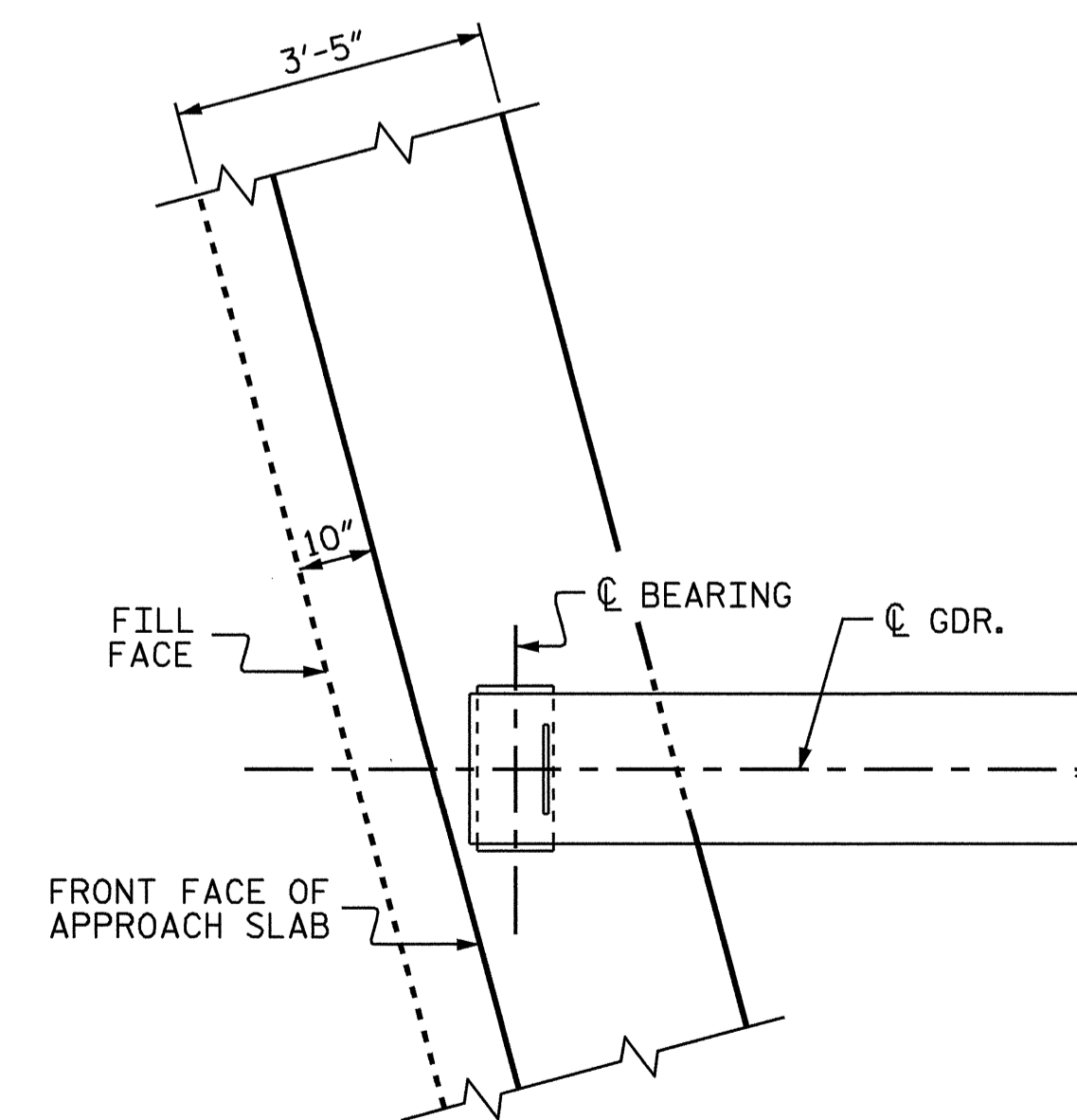
DETAIL "B"



DETAIL "A"



SECTION THRU INTEGRAL END BENT



PLAN OF GIRDER @ END BENT #1

END BENT #1 SHOWN, END BENT #2 SIMILAR

PROJECT NO. B-4252
 ROCKINGHAM COUNTY
 STATION: 27+69.00 -L-

SHEET 4 OF 4

STATE OF NORTH CAROLINA
 DEPARTMENT OF TRANSPORTATION
 RALEIGH

SUPERSTRUCTURE
 TYPICAL SECTION
 (STAGE 2)

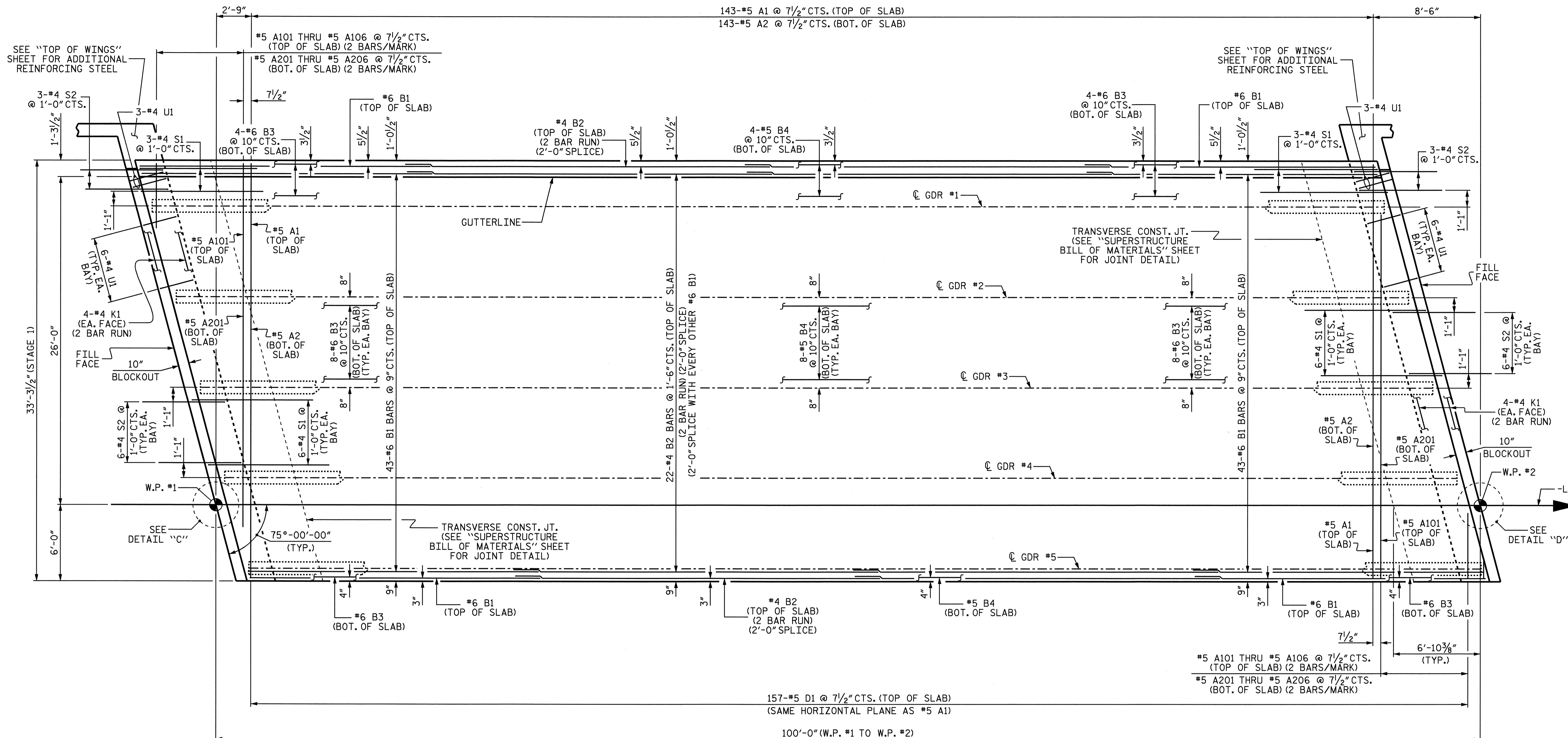
REVISIONS						SHEET NO.	
NO.	BY:	DATE:	NO.	BY:	DATE:	S-8	
1			3			TOTAL SHEETS	
2			4			70	



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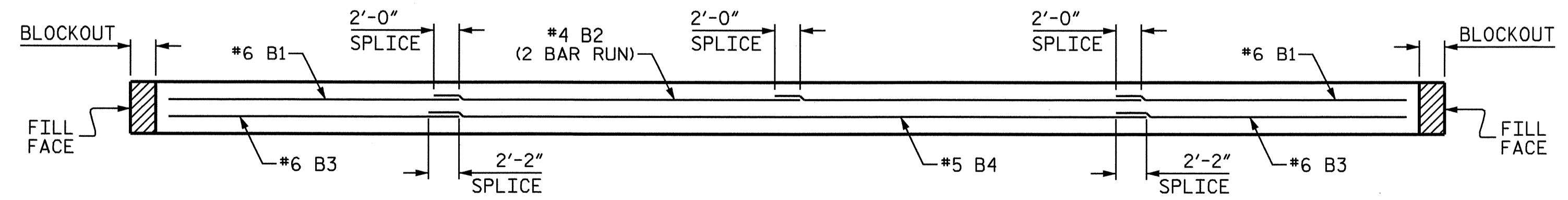
STR. #1



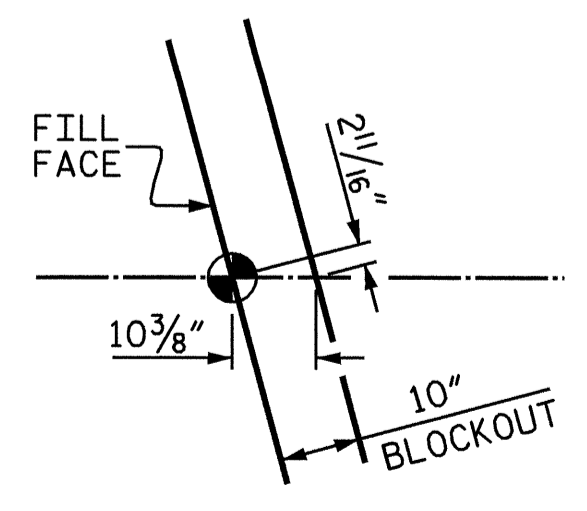
PLAN OF SPAN

PROJECT NO. B-4252
ROCKINGHAM COUNTY
 STATION: 27+69.00 -L-

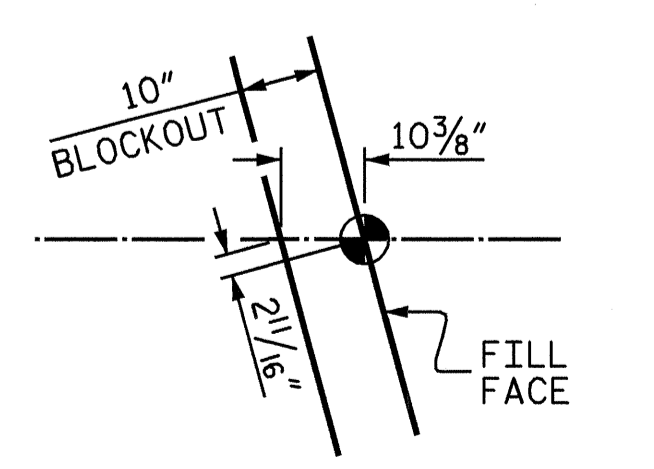
SHEET 1 OF 2



SCHEMATIC DIAGRAM OF SLAB



DETAIL "C"



DETAIL "D"

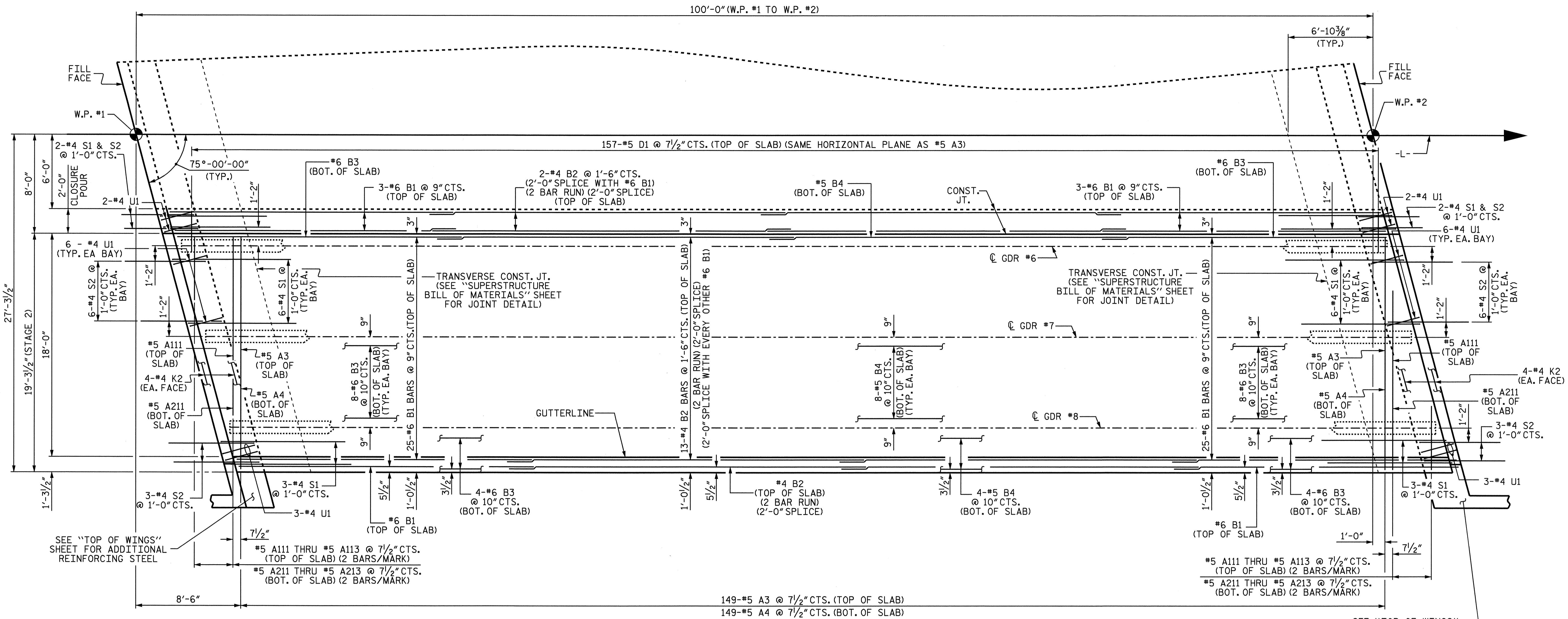
STATE OF NORTH CAROLINA
 DEPARTMENT OF TRANSPORTATION
 RALEIGH

**SUPERSTRUCTURE
 PLAN OF SPAN
 (STAGE 1)**



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NO.	BY:	DATE:	NO.	BY:	DATE:	S-9
1			3			TOTAL SHEETS
2			4			70



PLAN OF SPAN

PROJECT NO. B-4252

ROCKINGHAM COUNTY

STATION: 27+69.00 -L-

SHEET 2 OF 2

STATE OF NORTH CAROLINA
DEPARTMENT OF TRANSPORTATION
RALEIGH

SUPERSTRUCTURE
PLAN OF SPAN
(STAGE 2)

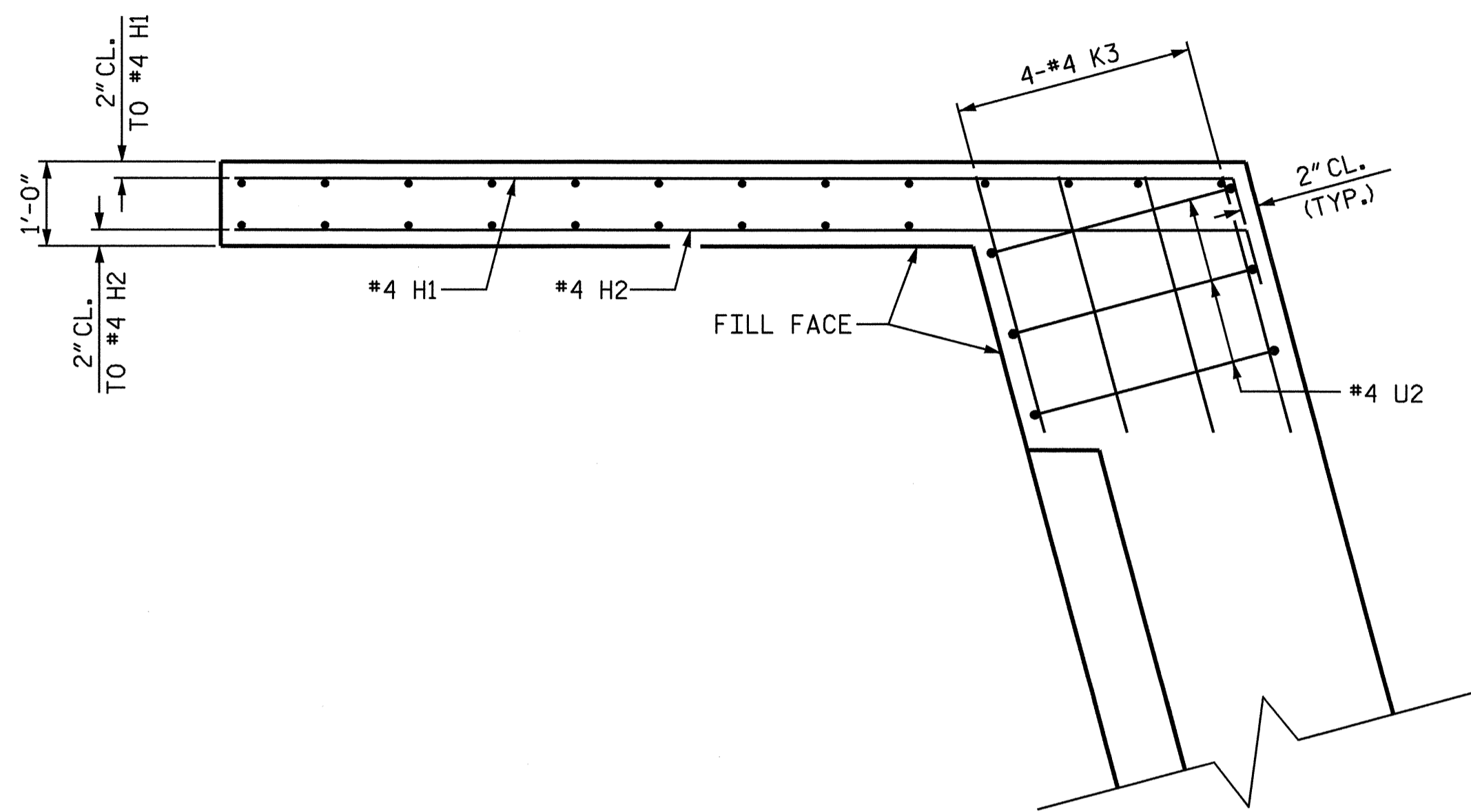


REVISIONS						SHEET NO.
NO.	BY:	DATE:	NO.	BY:	DATE:	S-10
1			3			TOTAL SHEETS
2			4			70

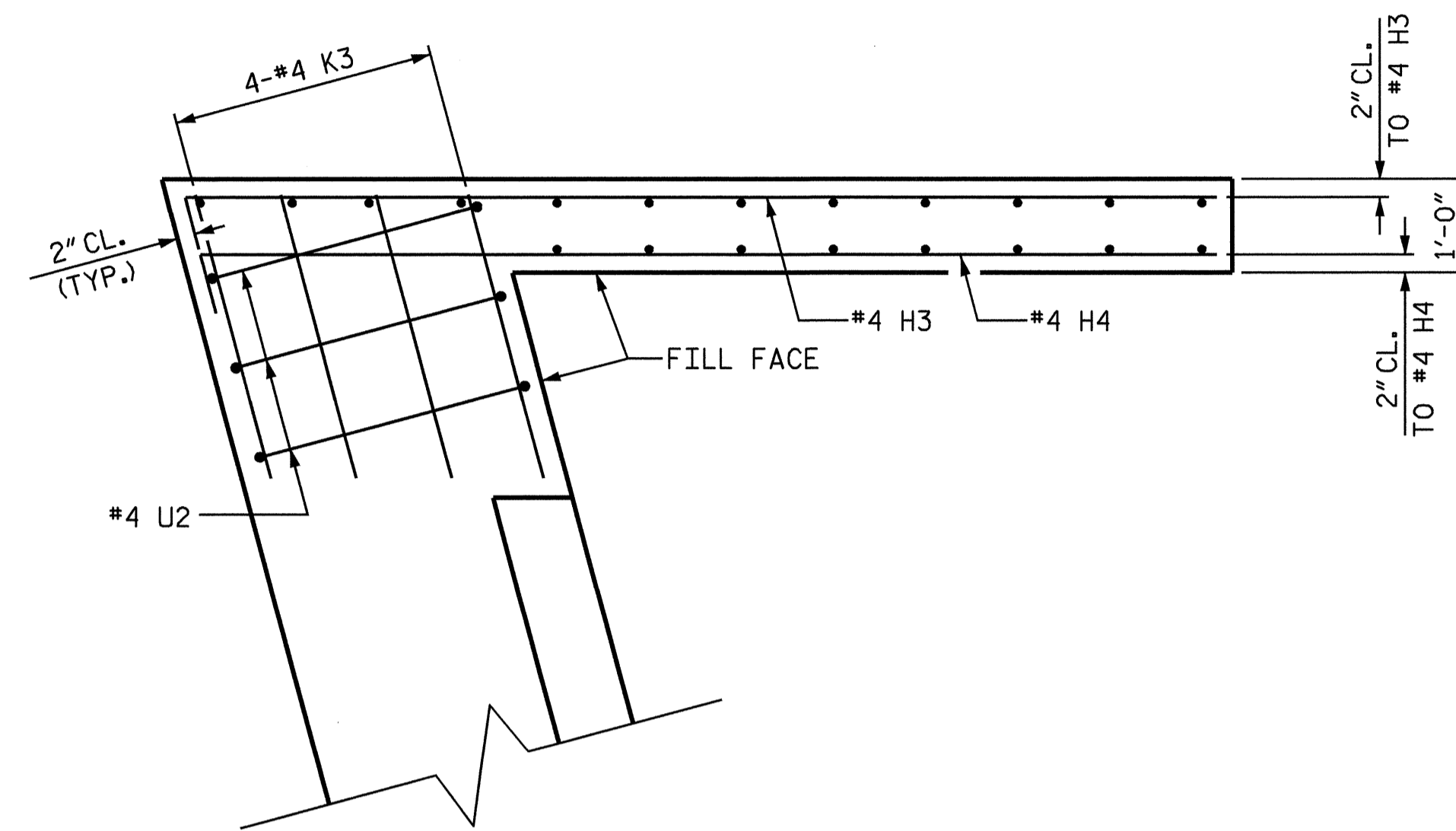
DRAWN BY : S. DOMBROWSKI DATE : 3/07
CHECKED BY : M.K. BEARD DATE : 10/07

20-MAR-2008 15:33
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klayne

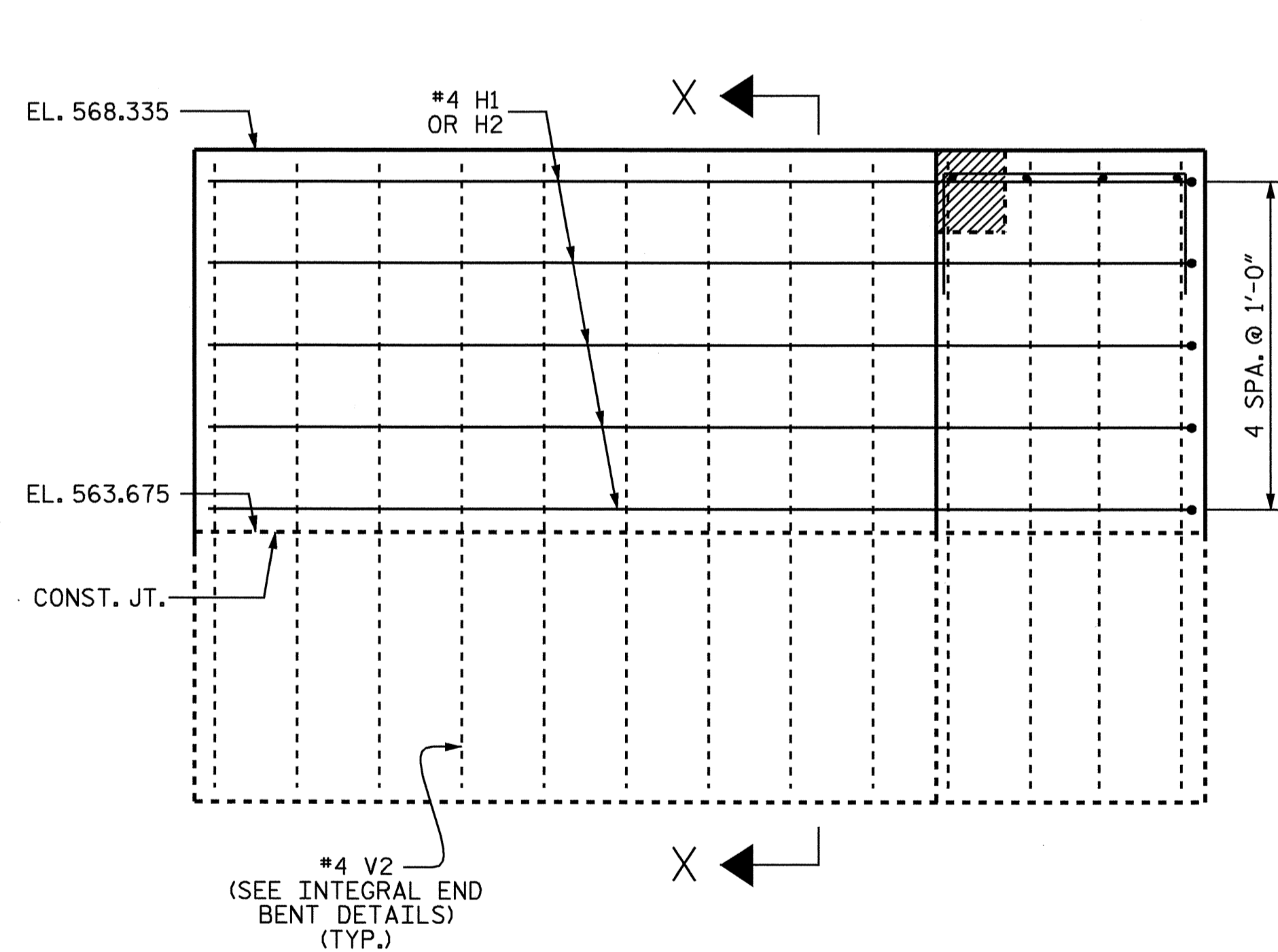
STR. #1



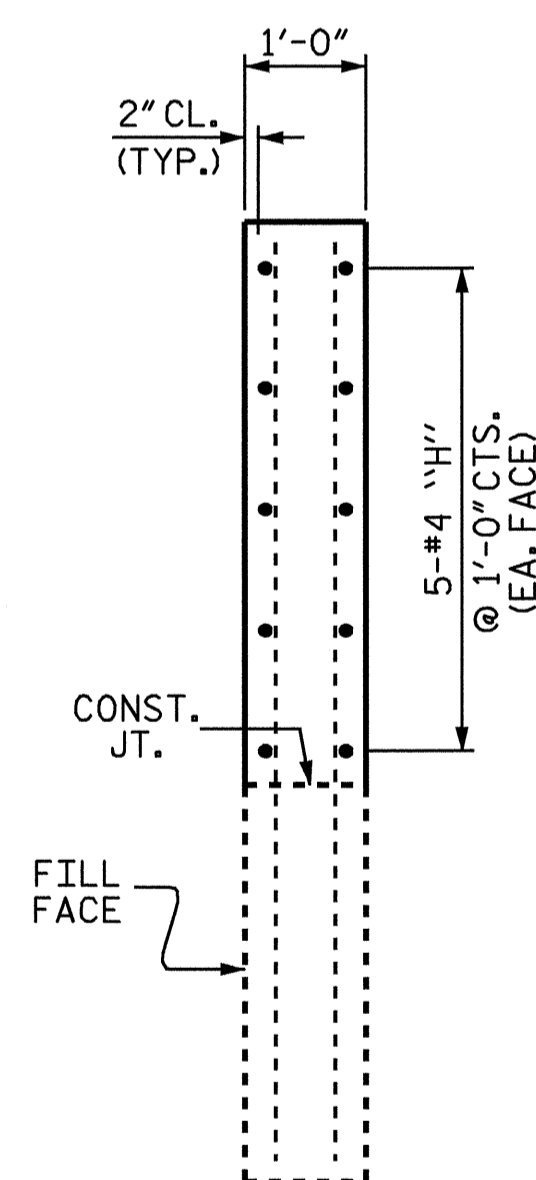
PLAN OF WING-W1
(STAGE 1)



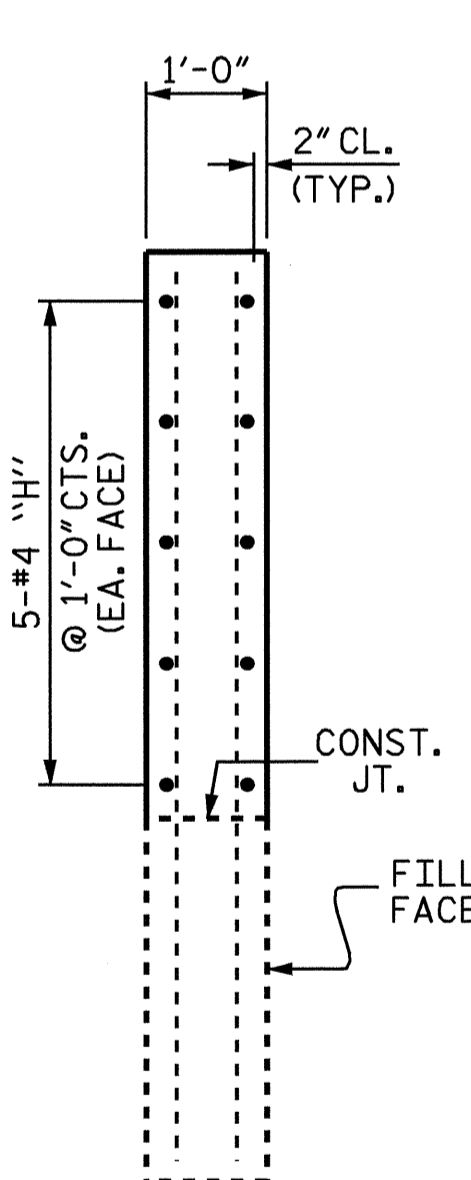
PLAN OF WING-W2
(STAGE 2)



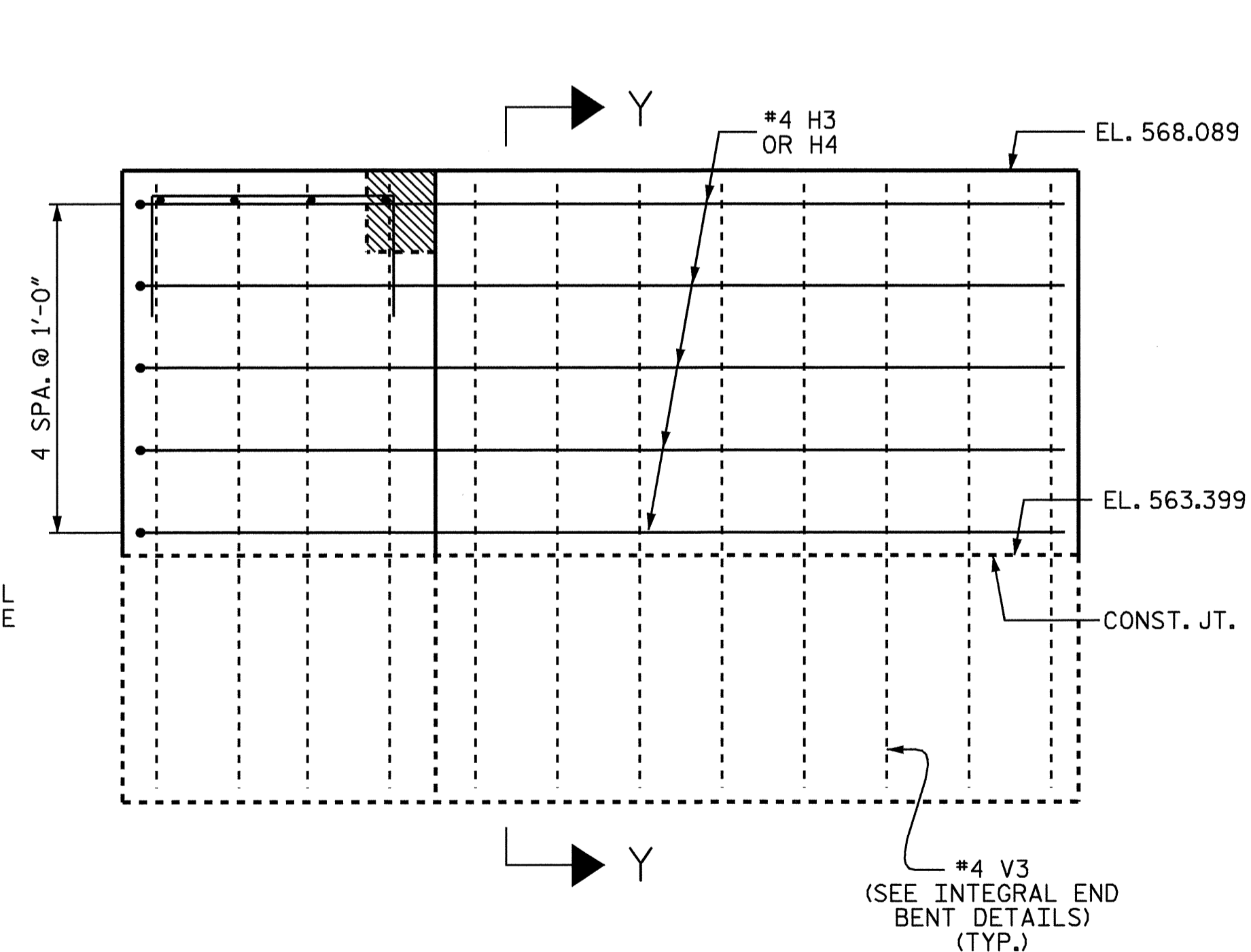
ELEVATION OF WING-W1
(STAGE 1)



SECTION X-X



SECTION Y-Y



ELEVATION OF WING-W2
(STAGE 2)

NOTE: LOWER PORTION OF WINGS
BUILT WITH INTEGRAL END BENTS.

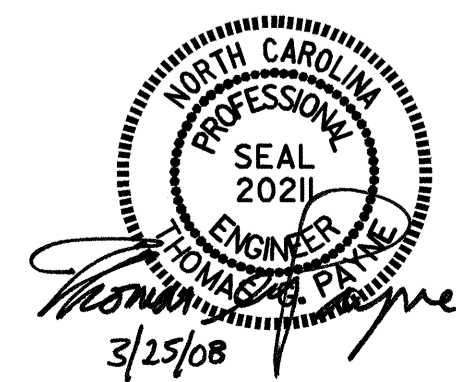
PROJECT NO. B-4252
ROCKINGHAM COUNTY
STATION: 27+69.00 -L-

SHEET 1 OF 2

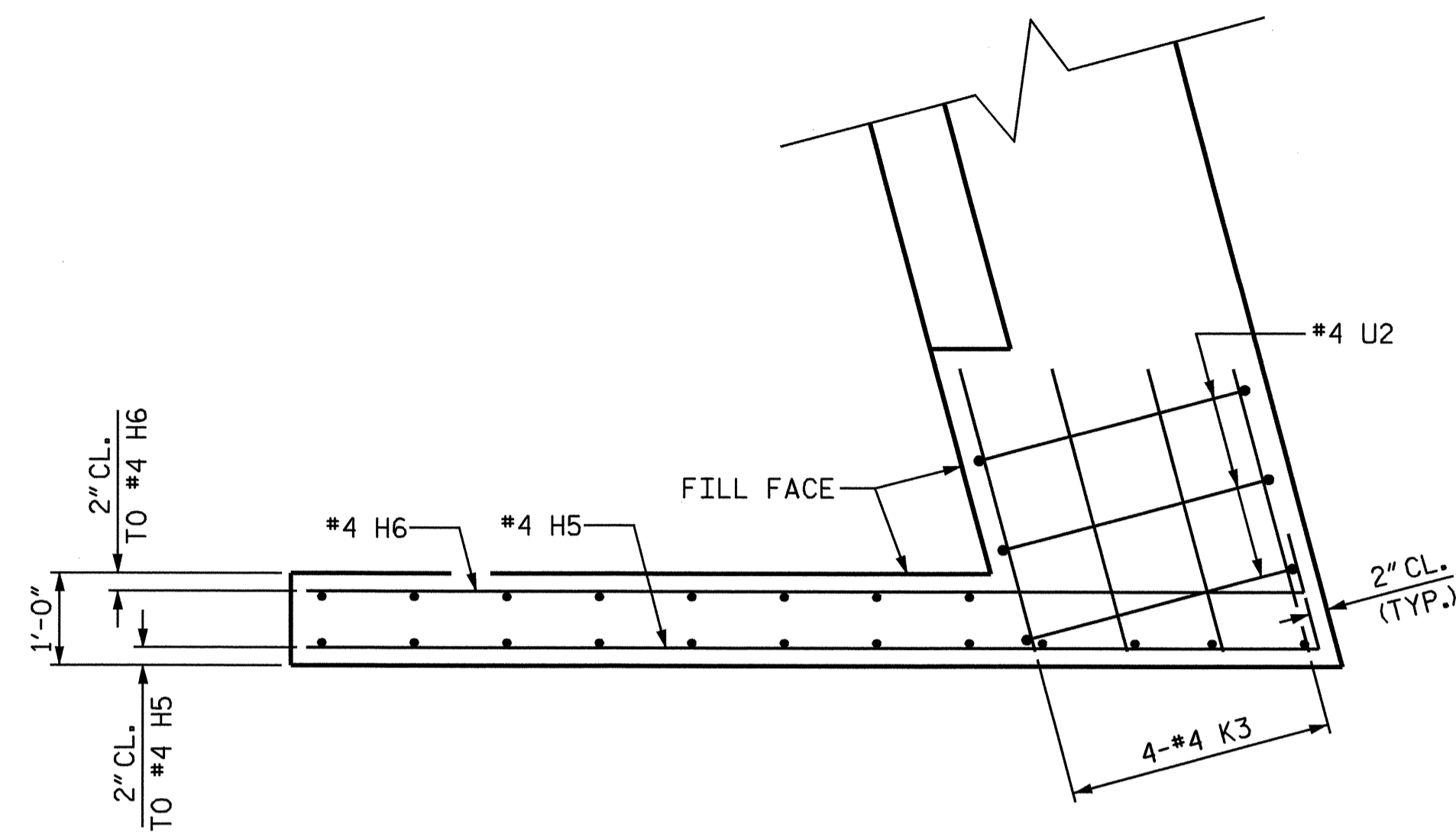
STATE OF NORTH CAROLINA
DEPARTMENT OF TRANSPORTATION
RALEIGH

SUPERSTRUCTURE
TOP OF WINGS
END BENT #1

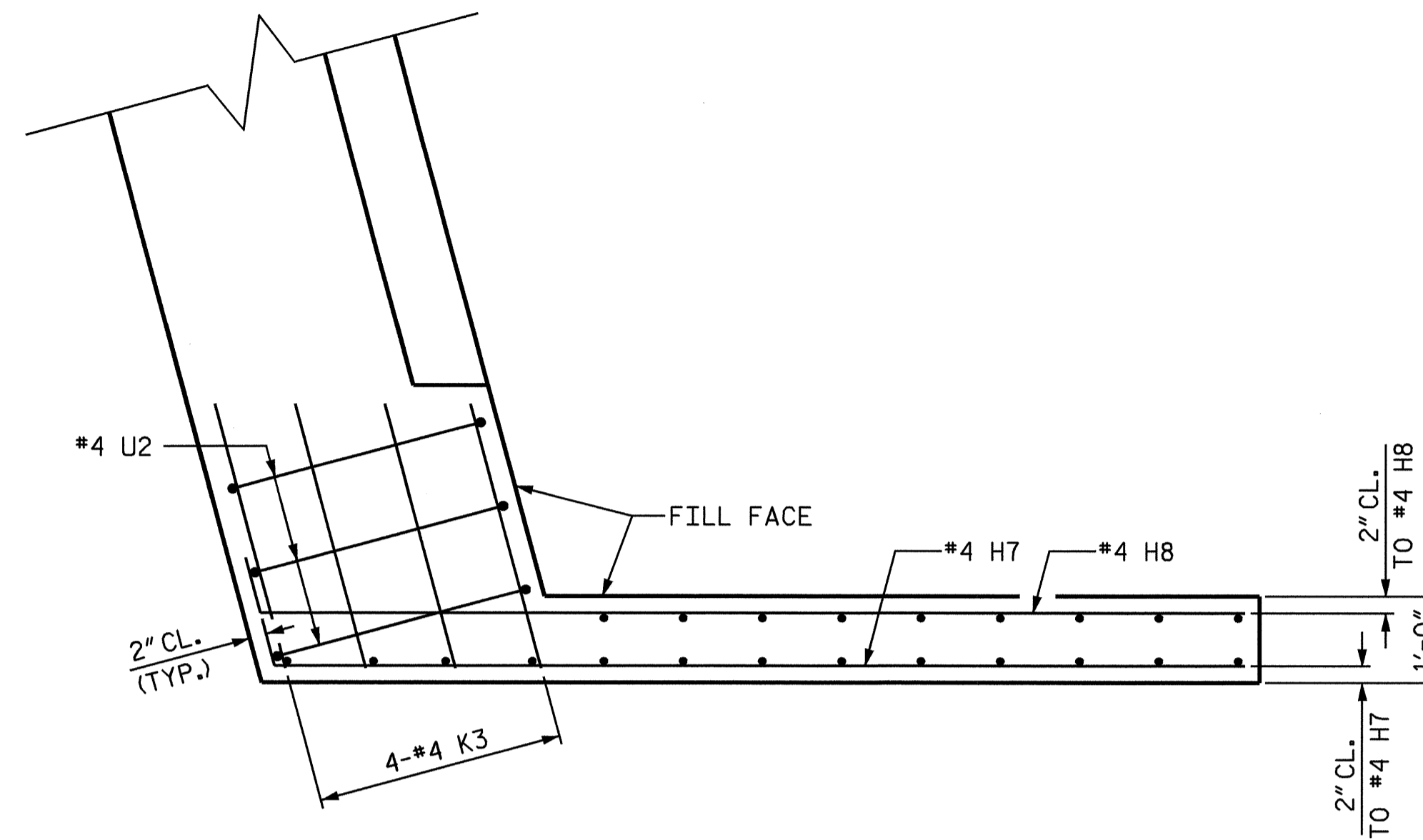
REVISIONS						SHEET NO.	
NO.	BY:	DATE:	NO.	BY:	DATE:	S-11	
1			3			TOTAL SHEETS 70	
2			4				



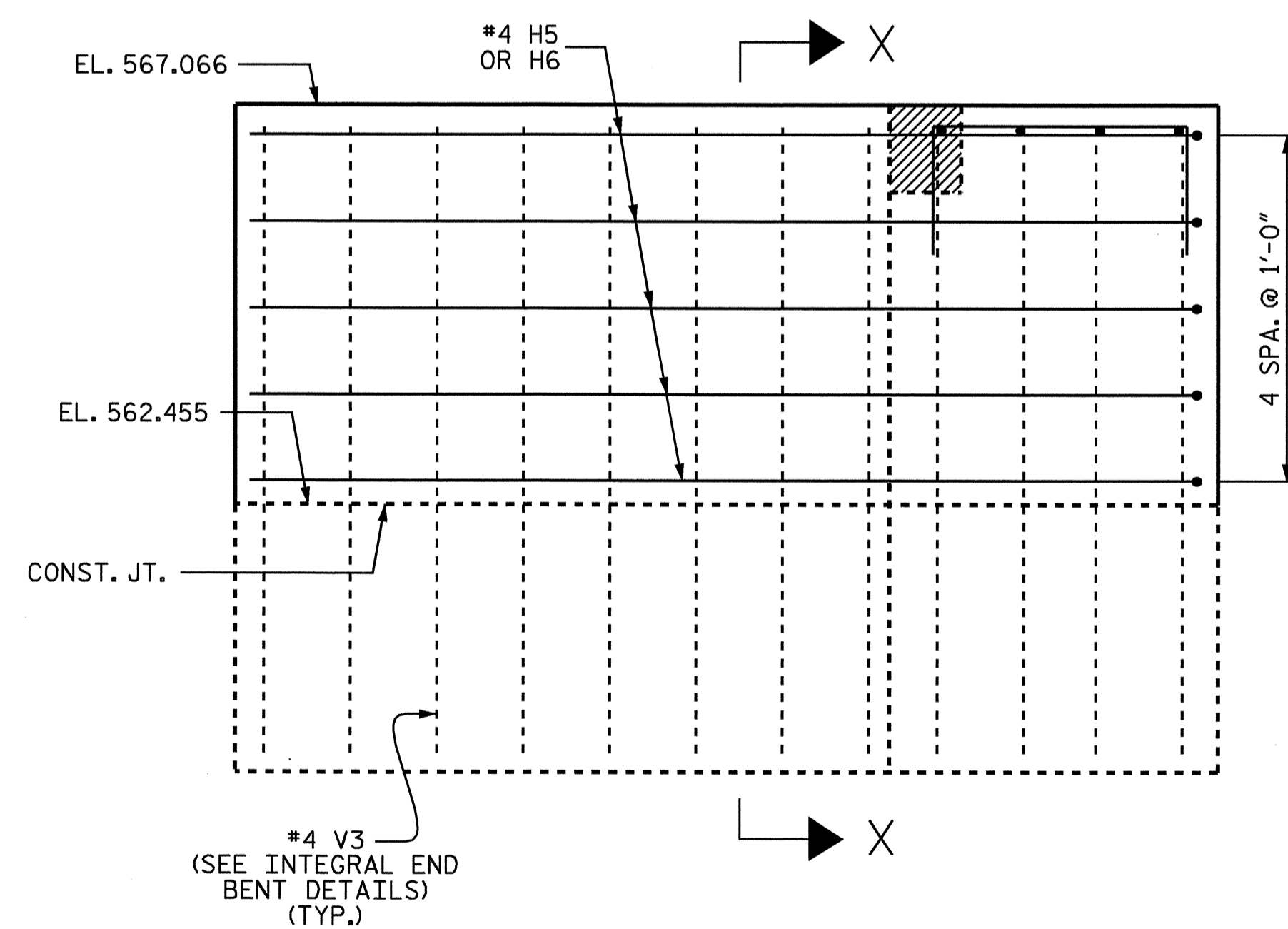
DRAWN BY : S. DOMBROWSKI DATE : 1/07
CHECKED BY : M.K. BEARD DATE : 10/07



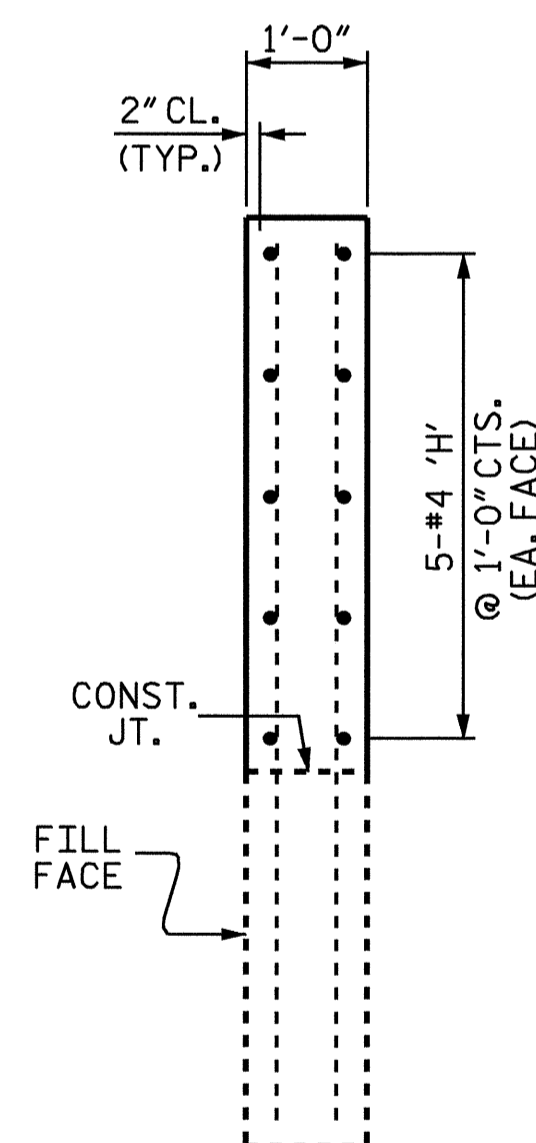
PLAN OF WING-W1
(STAGE 1)



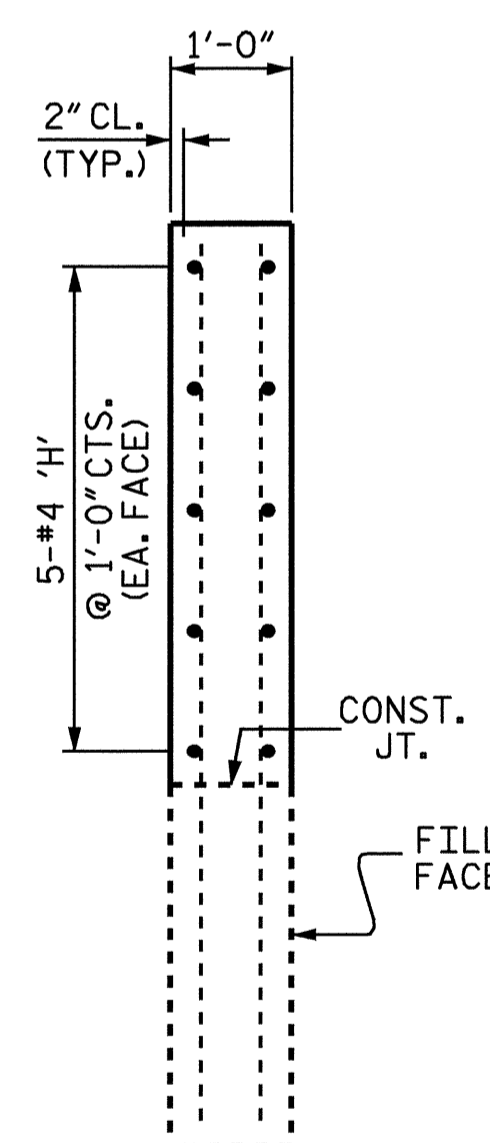
PLAN OF WING-W2
(STAGE 2)



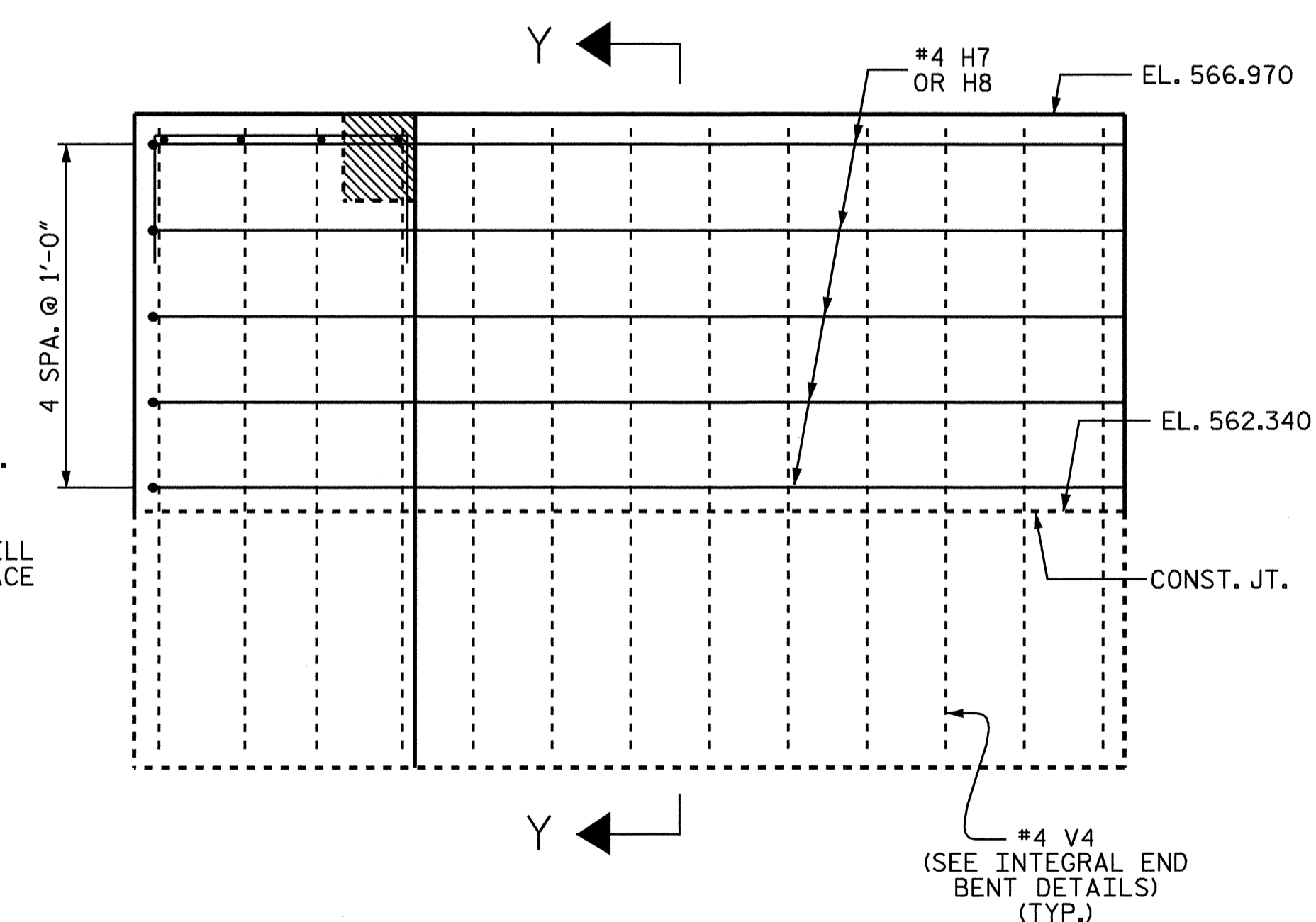
ELEVATION OF WING-W1
(STAGE 1)



SECTION X-X



SECTION Y-Y



ELEVATION OF WING-W2
(STAGE 2)

#4 V3
(SEE INTEGRAL END BENT DETAILS) (TYP.)

#4 V4
(SEE INTEGRAL END BENT DETAILS) (TYP.)

NOTE: LOWER PORTION OF WINGS BUILT WITH INTEGRAL END BENTS.

PROJECT NO. B-4252
ROCKINGHAM COUNTY
STATION: 27+69.00 -L-

SHEET 2 OF 2

STATE OF NORTH CAROLINA
DEPARTMENT OF TRANSPORTATION
RALEIGH

SUPERSTRUCTURE
TOP OF WINGS
END BENT #2

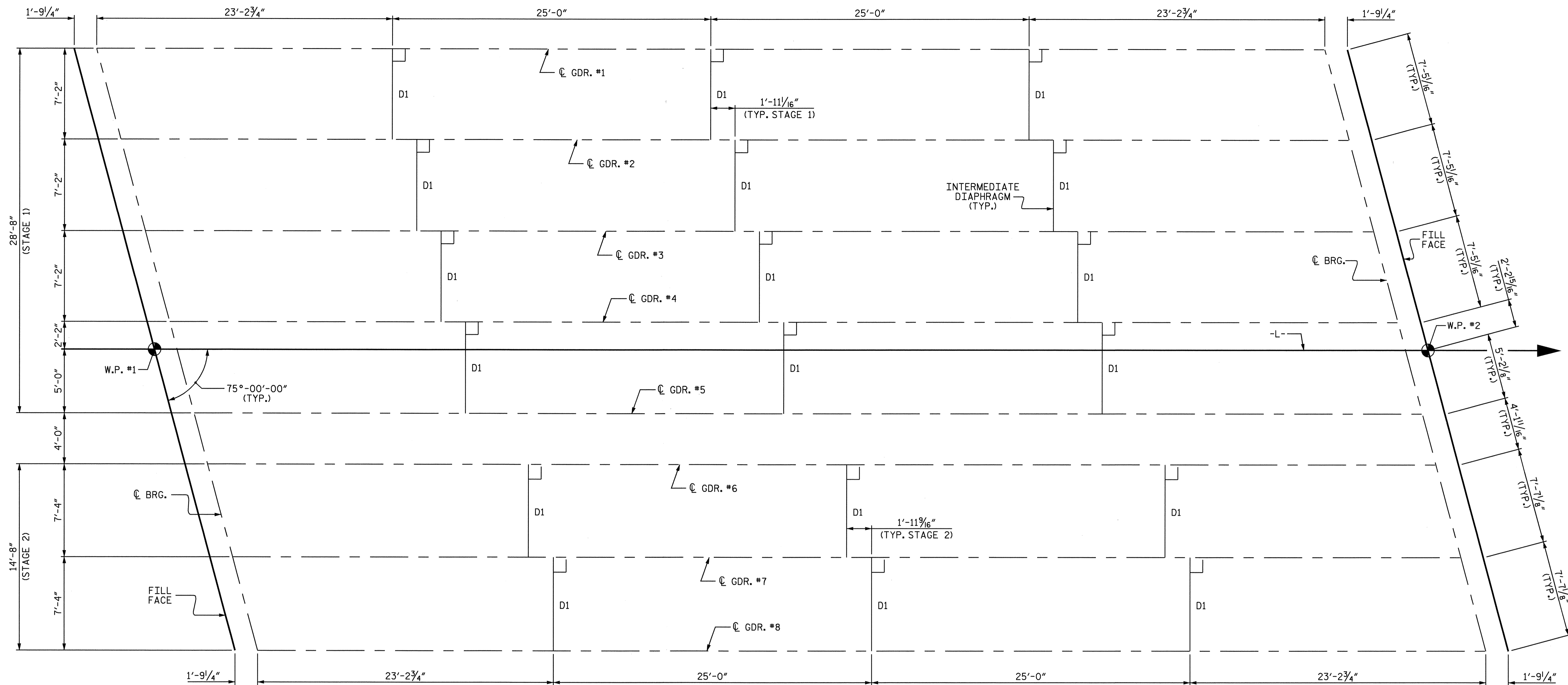


DRAWN BY : S. DOMBROWSKI DATE : 1/07
CHECKED BY : M.K. BEARD DATE : 10/07

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1			3			TOTAL SHEETS 70	
2			4				

STR. #1



FRAMING PLAN

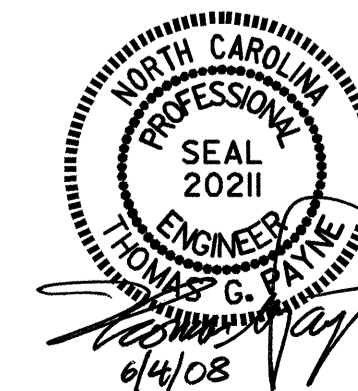
DEAD LOAD DEFLECTION TABLE FOR GIRDERS											
TENTH POINTS	0	.1	.2	.3	.4	.5	.6	.7	.8	.9	0
DEFLECTION DUE TO WEIGHT OF GIRDER	0.000	-0.028	-0.052	-0.072	-0.084	-0.088	-0.084	-0.072	-0.052	-0.028	0.000
DEFLECTION DUE TO WEIGHT OF SLAB *	0.000	-0.107	-0.200	-0.273	-0.319	-0.335	-0.319	-0.273	-0.200	-0.107	0.000
DEFLECTION DUE TO WEIGHT OF BARRIER RAIL	0.000	-0.009	-0.018	-0.024	-0.029	-0.030	-0.029	-0.024	-0.018	-0.009	0.000
TOTAL DEAD LOAD DEFLECTION	0.000	-0.144	-0.270	-0.369	-0.432	-0.453	-0.432	-0.369	-0.270	-0.144	0.000
VERTICAL CURVE ORDINATE	0.000	-0.044	-0.077	-0.102	-0.116	-0.121	-0.116	-0.102	-0.077	-0.044	0.000
REQUIRED CAMBER	0"	1 3/16"	2 5/16"	3 3/16"	3 3/16"	4"	3 3/16"	3 3/16"	2 5/16"	1 3/16"	0"

* INCLUDES SLAB, BUILDUPS & STAY-IN-PLACE FORMS. ALL VALUES ARE SHOWN IN FEET (DECIMAL FORM), EXCEPT "REQUIRED CAMBER", WHICH IS GIVEN IN INCHES (FRACTION FORM).

SIGN CONVENTION FOR DEAD LOAD DEFLECTION $\begin{matrix} + \\ \uparrow \\ 0 \\ \downarrow \\ - \end{matrix}$

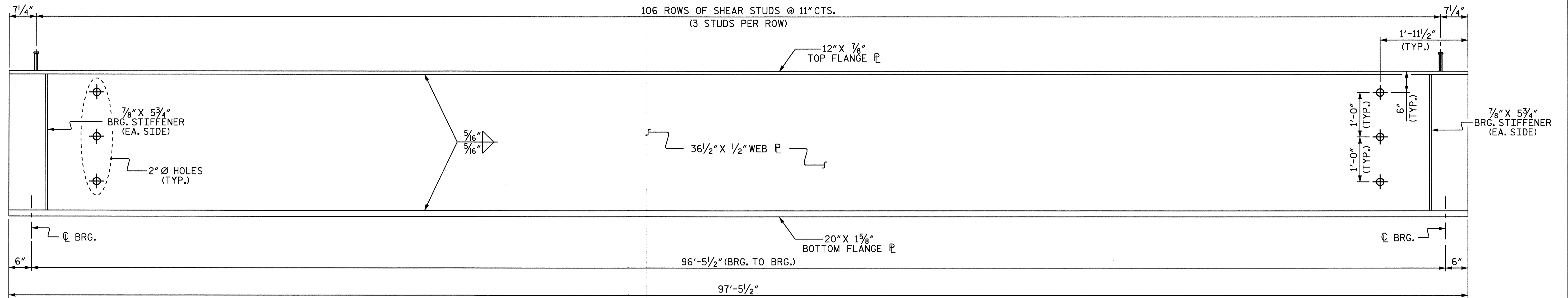
PROJECT NO. B-4252
ROCKINGHAM COUNTY
 STATION: 27+69.00 -L-

STATE OF NORTH CAROLINA
 DEPARTMENT OF TRANSPORTATION
 RALEIGH
 SUPERSTRUCTURE
 FRAMING PLAN
 AND
 DEAD LOAD DEFLECTIONS

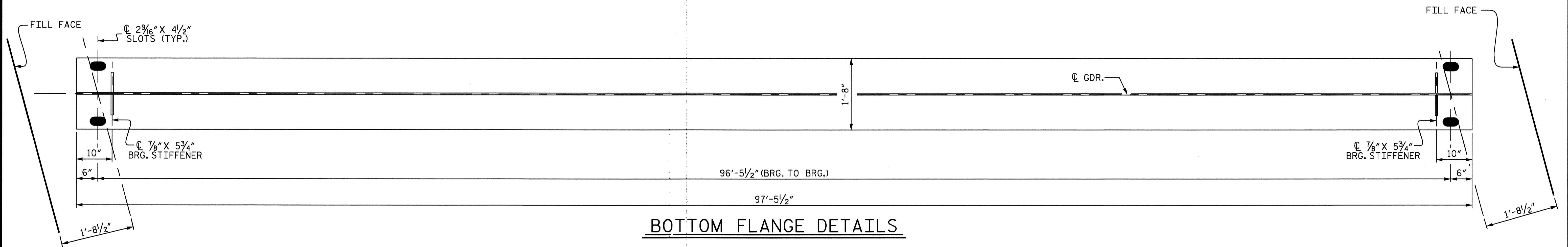


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NO.	BY:	DATE:	NO.	BY:	DATE:	S-13	
1			3			TOTAL SHEETS	
2			4			70	

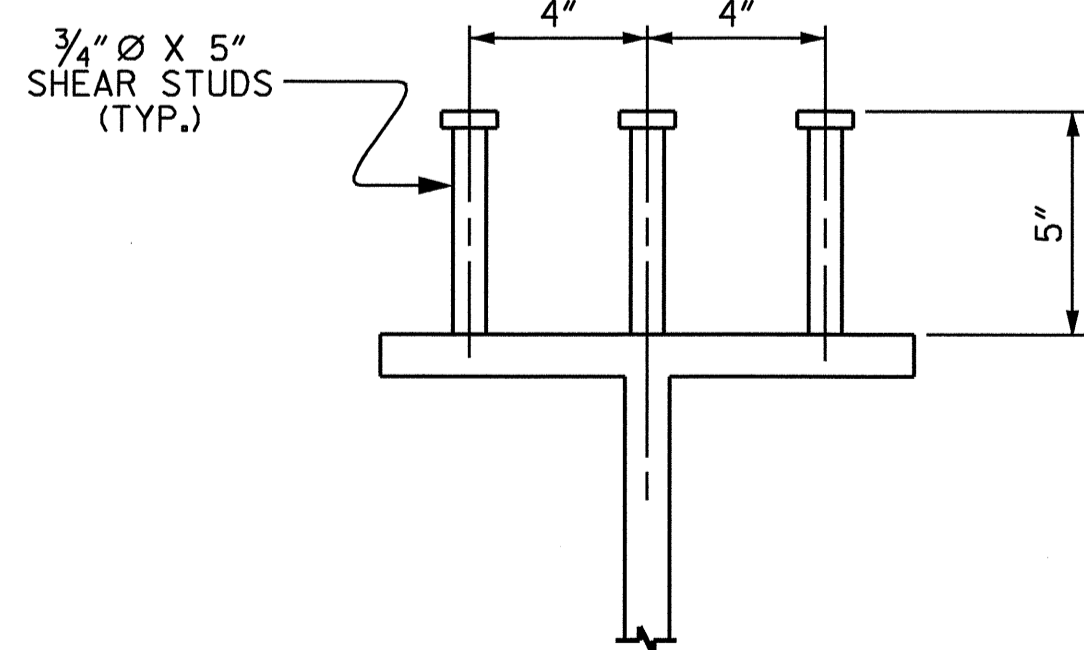
DRAWN BY: S. DOMBROWSKI DATE: 3/07
 CHECKED BY: M.K. BEARD DATE: 10/07



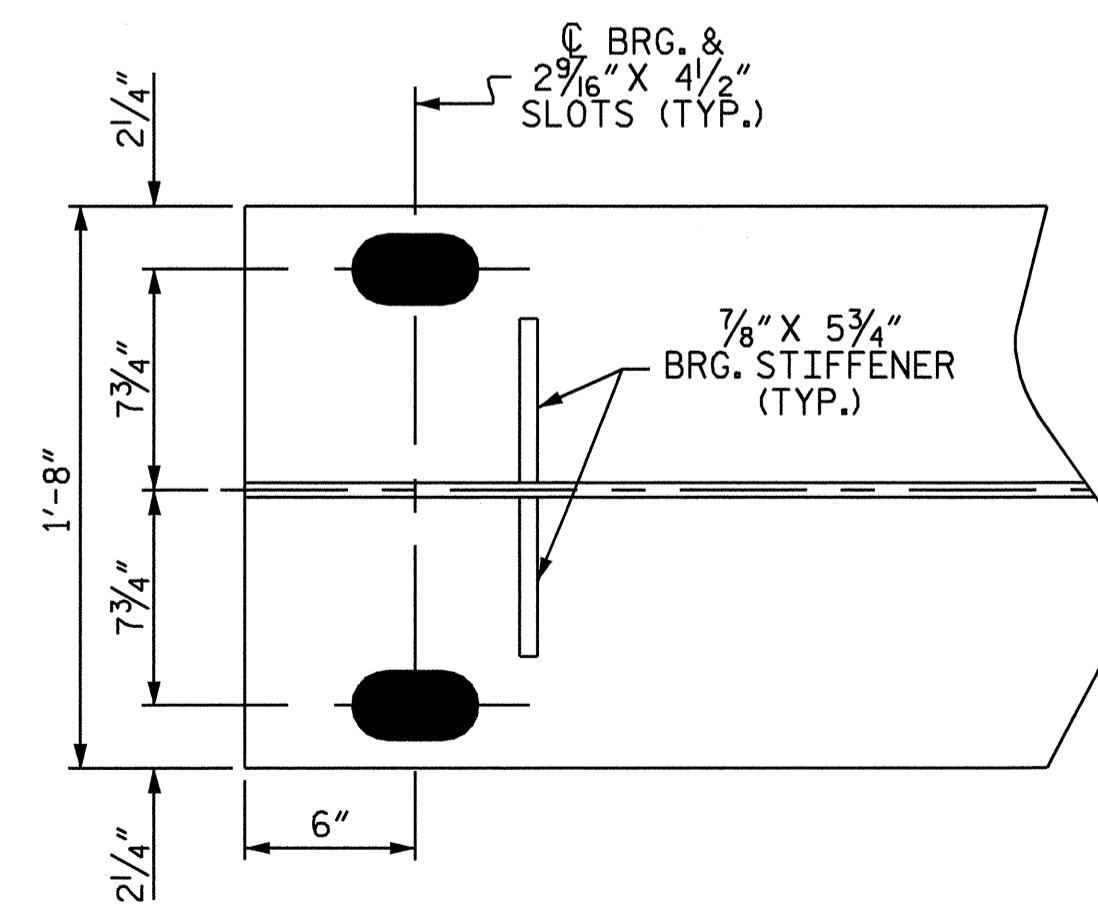
GIRDER ELEVATION



BOTTOM FLANGE DETAILS



SHEAR STUD DETAILS



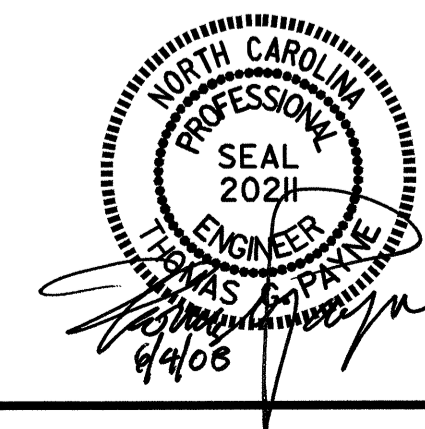
PARTIAL BOTTOM FLANGE DETAIL

PROJECT NO. B-4252
 ROCKINGHAM COUNTY
 STATION: 27+69.00 -L-

SHEET 1 OF 2

STATE OF NORTH CAROLINA
 DEPARTMENT OF TRANSPORTATION
 RALEIGH

SUPERSTRUCTURE
 STRUCTURAL STEEL



DRAWN BY : S. DOMBROWSKI DATE : 12/06
 CHECKED BY : M.K. BEARD DATE : 10/07

02-JUN-2008 13:50
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REVISIONS						SHEET NO.
NO.	BY:	DATE:	NO.	BY:	DATE:	S-14
1			3			TOTAL SHEETS
2			4			70

STR. #1

NOTES

ALL STRUCTURAL STEEL SHALL BE AASHTO M270 GRADE 50W.

ALL DIMENSIONS SHOWN ARE HORIZONTAL OR VERTICAL, UNLESS OTHERWISE NOTED.

BEARING STIFFENERS ARE TO BE PLACED NORMAL TO THE WEB OF THE GIRDER AND SHALL BE PLUMB.

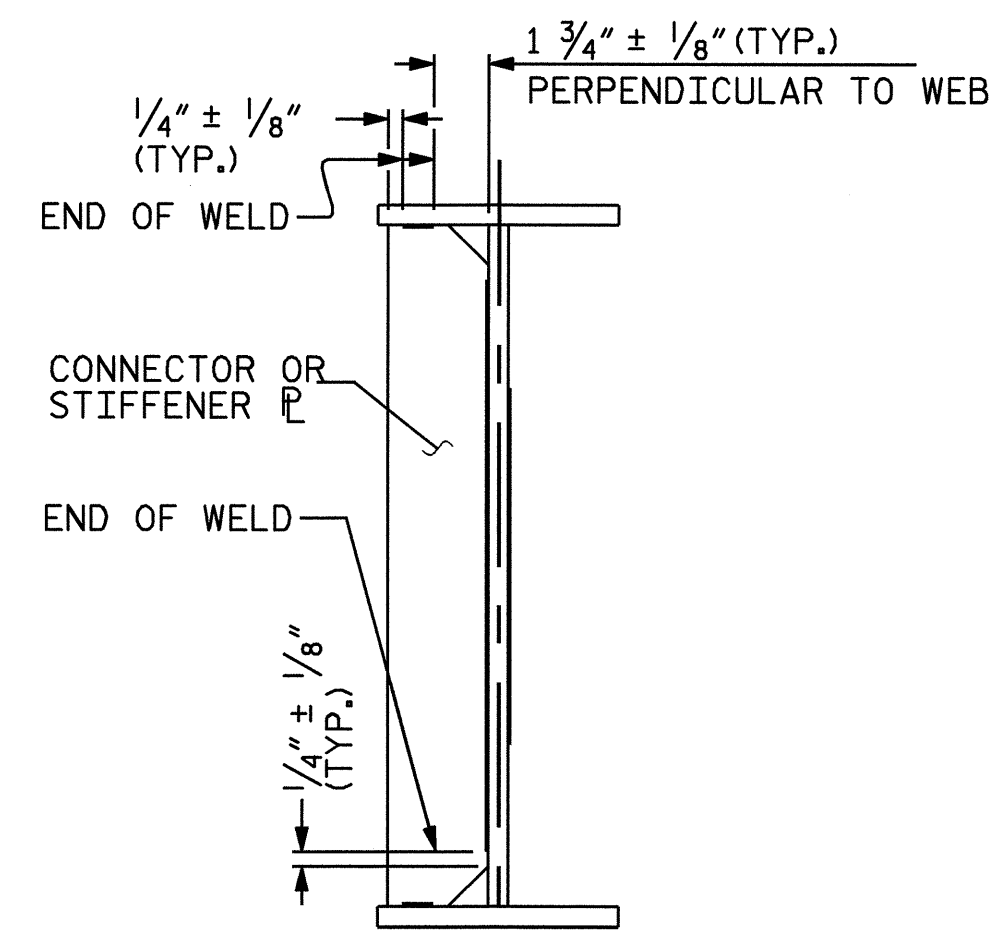
SHOP SPLICES ARE PERMITTED TO LIMIT THE MAXIMUM REQUIRED FLANGE PIECE LENGTHS TO 60 FEET AND WEB PIECE LENGTHS TO 45 FEET. PERMITTED FLANGE AND WEB SHOP SPLICES SHALL NOT BE LOCATED WITHIN 15 FEET OF MAXIMUM DEAD LOAD DEFLECTION. KEEP 2 FEET MINIMUM BETWEEN WEB AND FLANGE SHOP SPLICES. KEEP 6" MINIMUM BETWEEN CONNECTOR PLATE OR TRANSVERSE STIFFENER WELDS AND WEB OR FLANGE SHOP SPLICES.

STUDS ON GIRDERS MAY BE SHIFTED UP TO 1" IF NECESSARY TO CLEAR FLANGE SPLICE WELD.

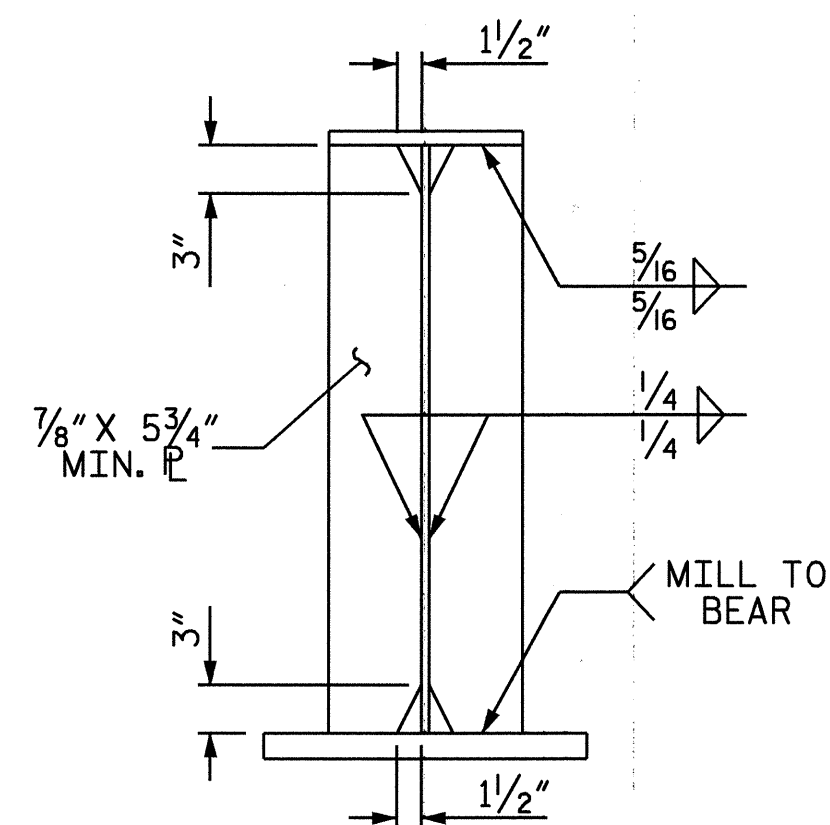
ALL SURFACES OF BEARING PLATES SHALL BE SMOOTH AND STRAIGHT.

ENDS OF GIRDERS SHALL BE PLUMB.

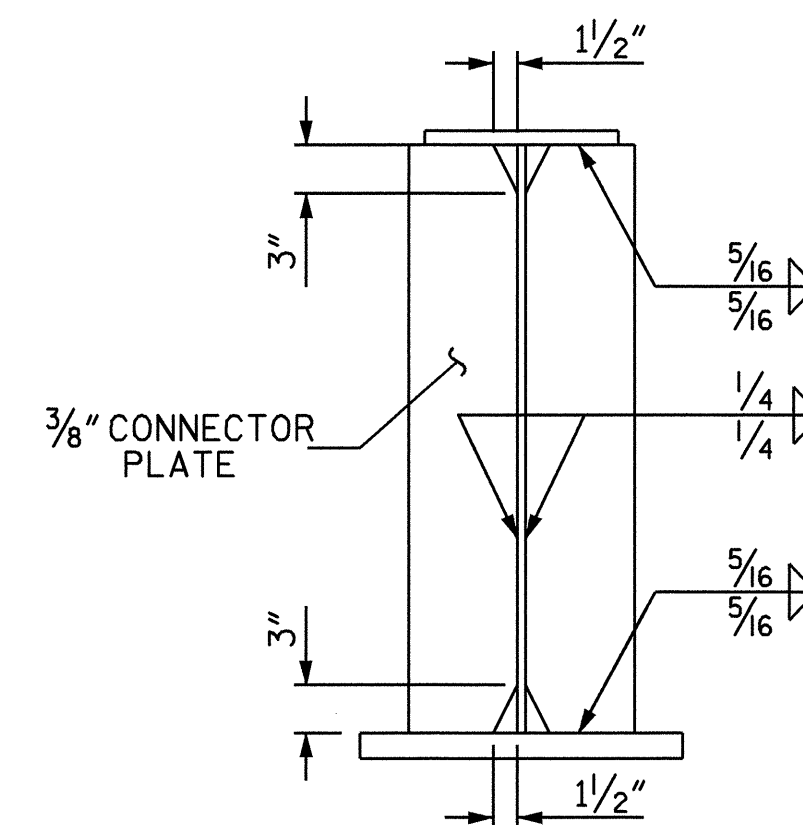
ALL SURFACES OF BEARING PLATES SHALL BE SMOOTH AND STRAIGHT.



TYPICAL STIFFENER OR CONNECTOR PLATE CONNECTIONS
WELD TERMINATION DETAILS

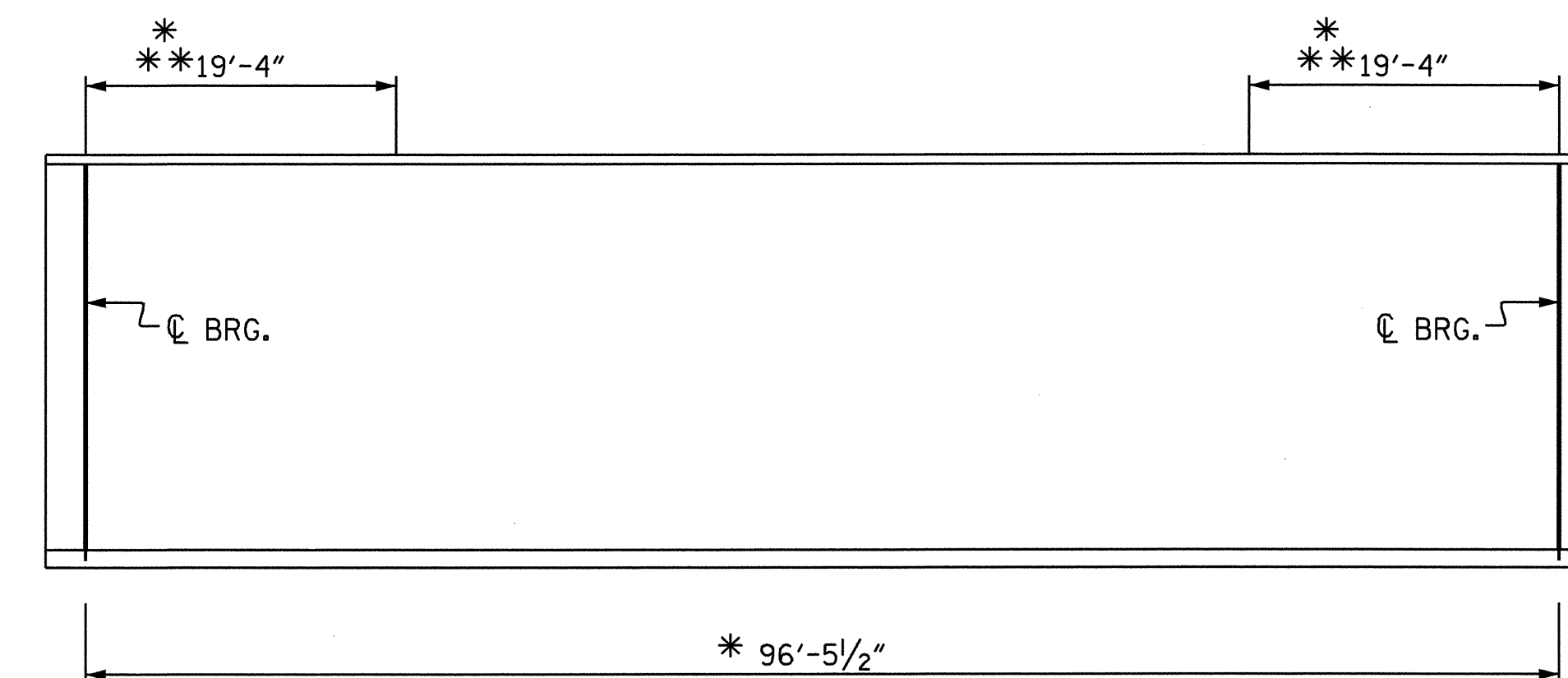


BEARING STIFFENER DETAIL
(AT END BENTS)



CONNECTOR PLATE DETAIL

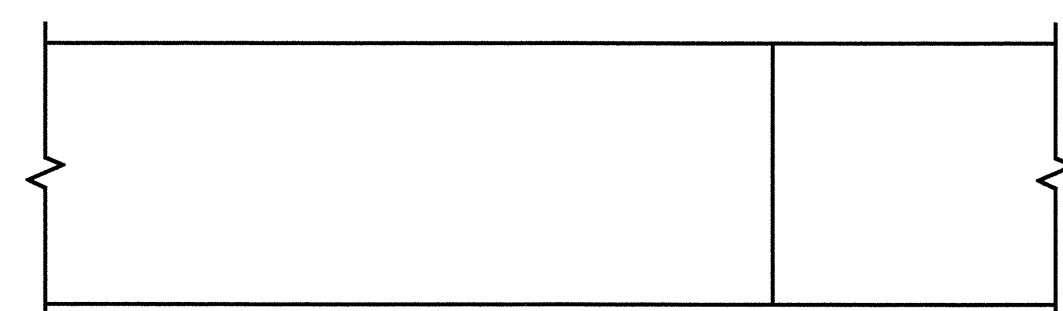
FOR "DEAD LOAD DEFLECTION TABLE FOR GIRDERS", SEE "SUPERSTRUCTURE BILL OF MATERIAL" SHEET



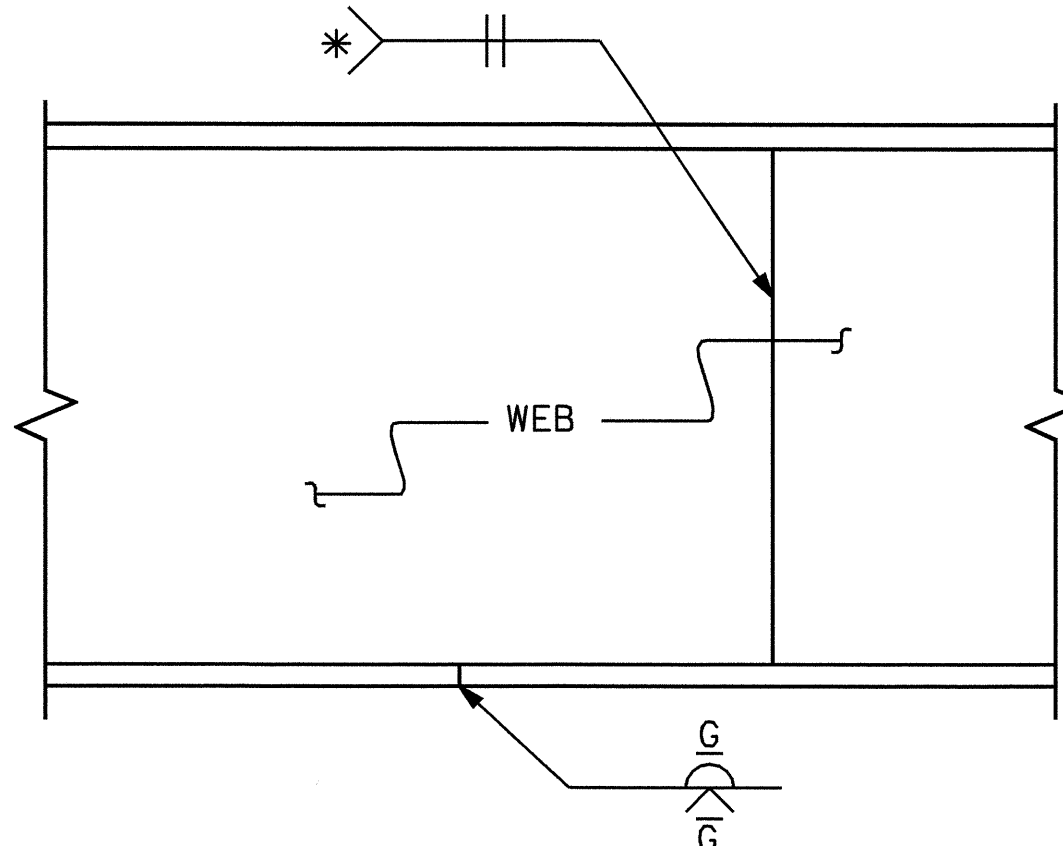
CHARPY V-NOTCH TEST FOR GIRDERS

* CHARPY V-NOTCH TESTS ARE REQUIRED FOR ALL TOP OR BOTTOM FLANGE PLATES WHICH FALL WITHIN THESE LIMITS, ALL WEB PLATES, AND ALL SPLICE PLATES. IF A PERMITTED SHOP FLANGE SPLICE IS NOT USED, CHARPY V-NOTCH TESTS WILL BE REQUIRED FOR THE ENTIRE FLANGE PLATE. FOR CHARPY V-NOTCH TESTS, SEE ARTICLE 1072-9 OF THE STANDARD SPECIFICATIONS.

* NO WELDING OF FORMS OR FALSEWORK TO THE TOP FLANGE WILL BE PERMITTED IN THIS REGION.

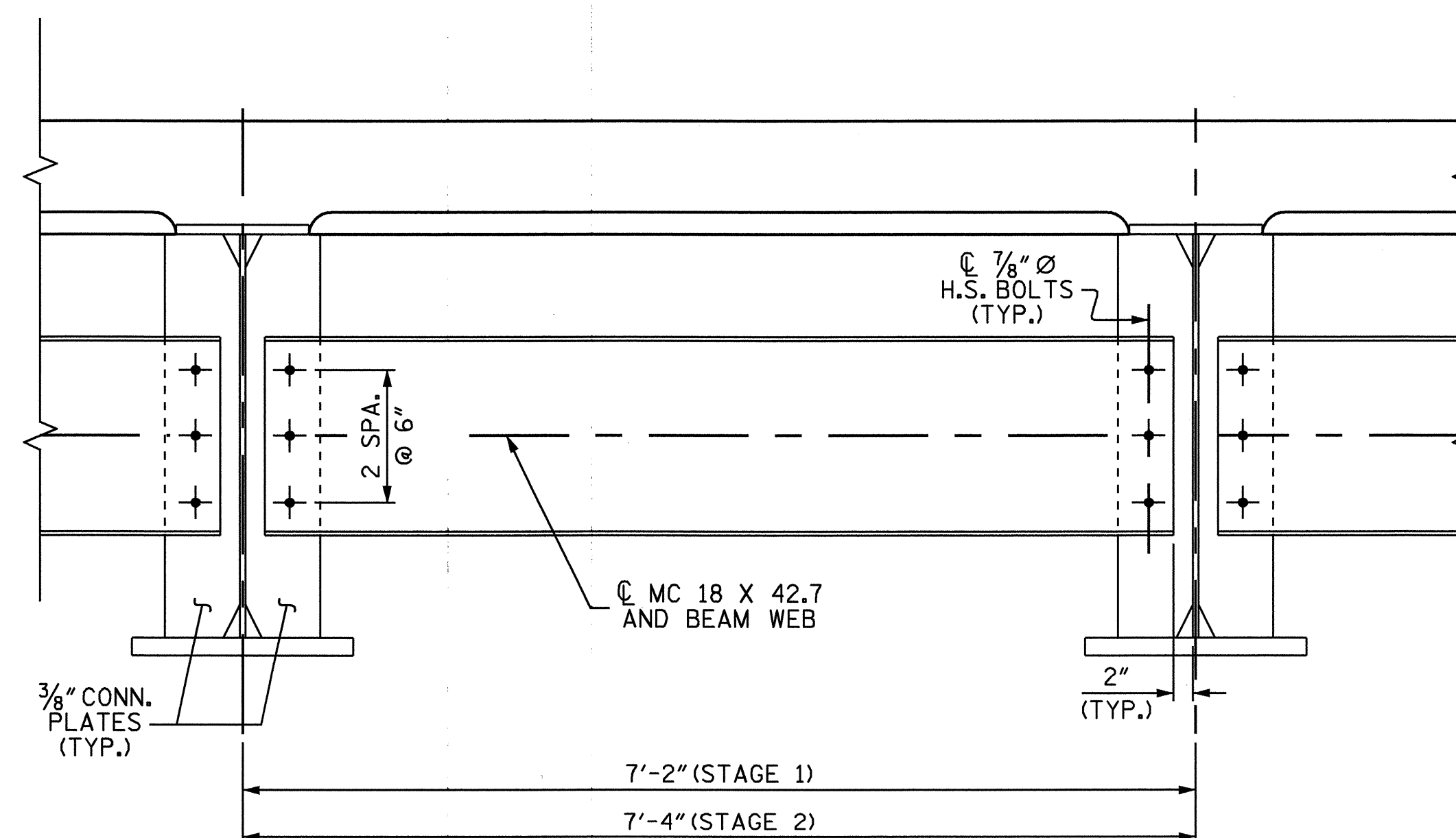


PLAN - FLANGE SPLICE
* GRIND SMOOTH AND FLUSH ON OUTER FACE OF EXTERIOR BEAMS /GIRDERS



ELEVATION

TYPICAL FLANGE AND WEB BUTT JOINT



INTERMEDIATE DIAPHRAGM (D1)

DRAWN BY : S. DOMBROWSKI DATE : 12/06
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23-MAY-2008 13:24
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PROJECT NO. B-4252
ROCKINGHAM COUNTY
STATION: 27+69.00 -L-

SHEET 2 OF 2

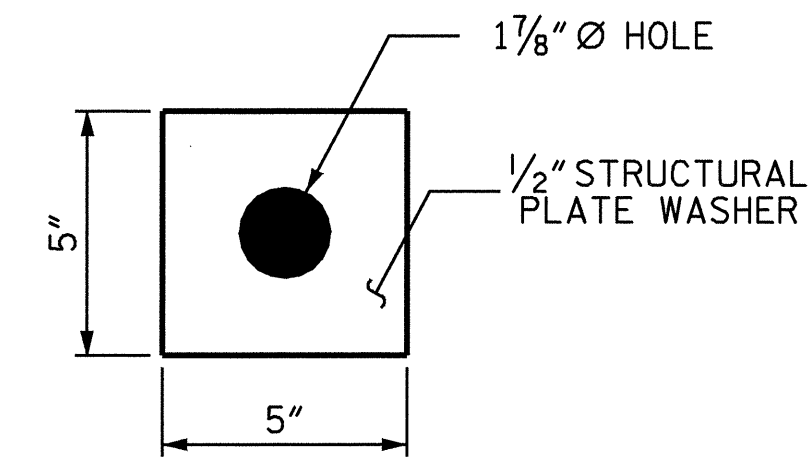
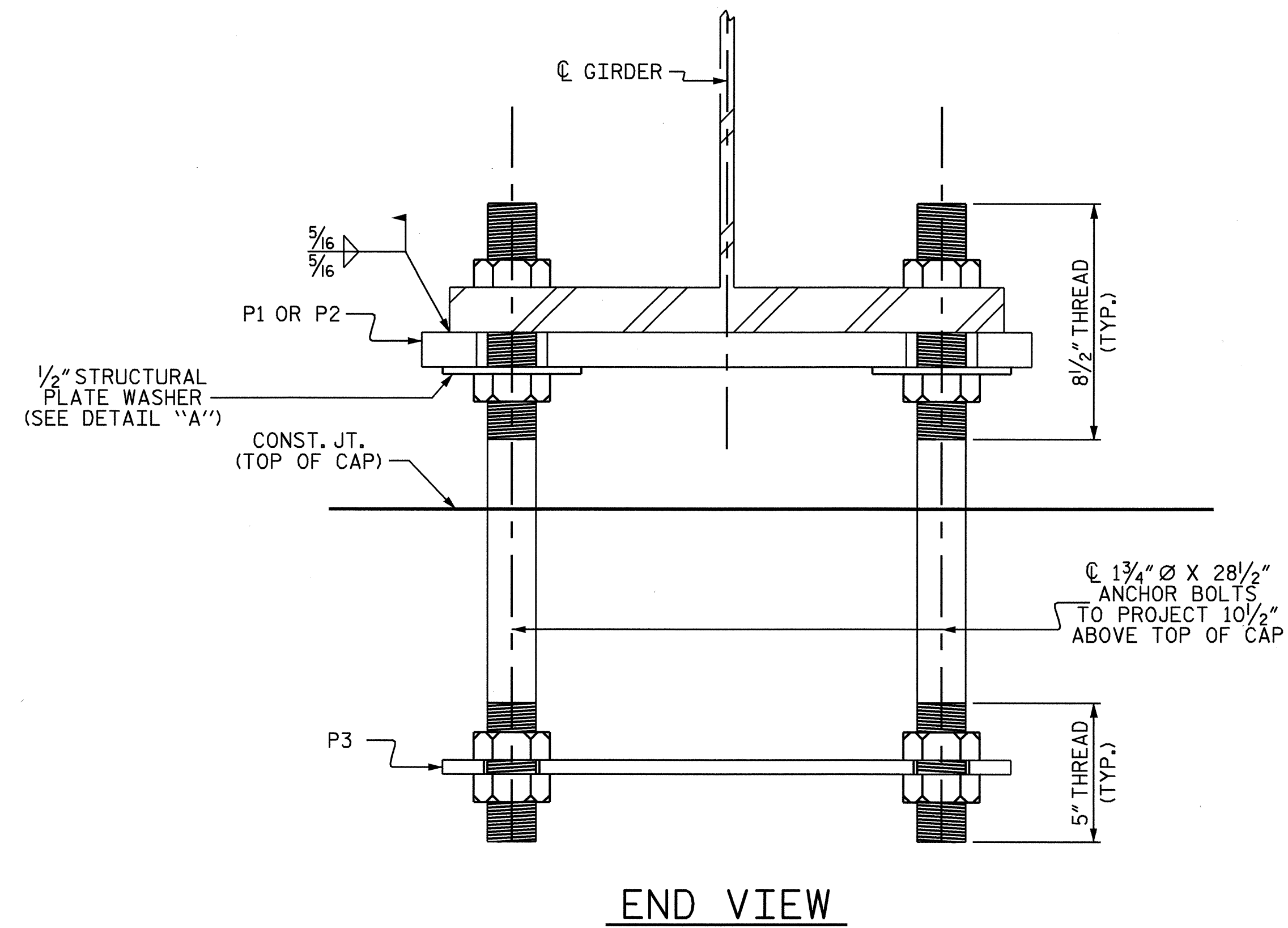
STATE OF NORTH CAROLINA
DEPARTMENT OF TRANSPORTATION
RALEIGH

SUPERSTRUCTURE
STRUCTURAL STEEL

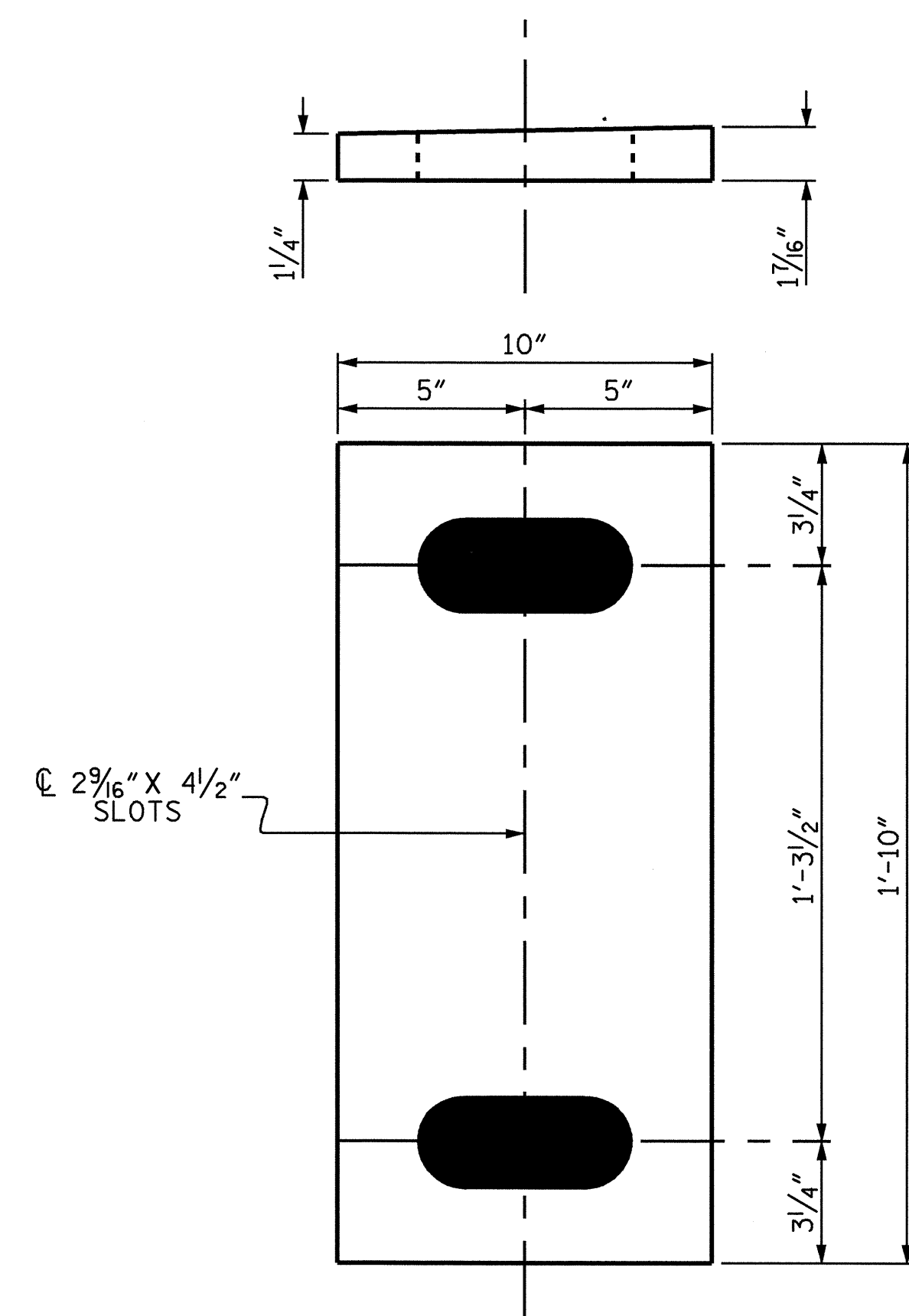
REVISIONS						SHEET NO.	
NO.	BY:	DATE:	NO.	BY:	DATE:	S-15	
1			3			TOTAL SHEETS	
2			4			70	

STR. #1

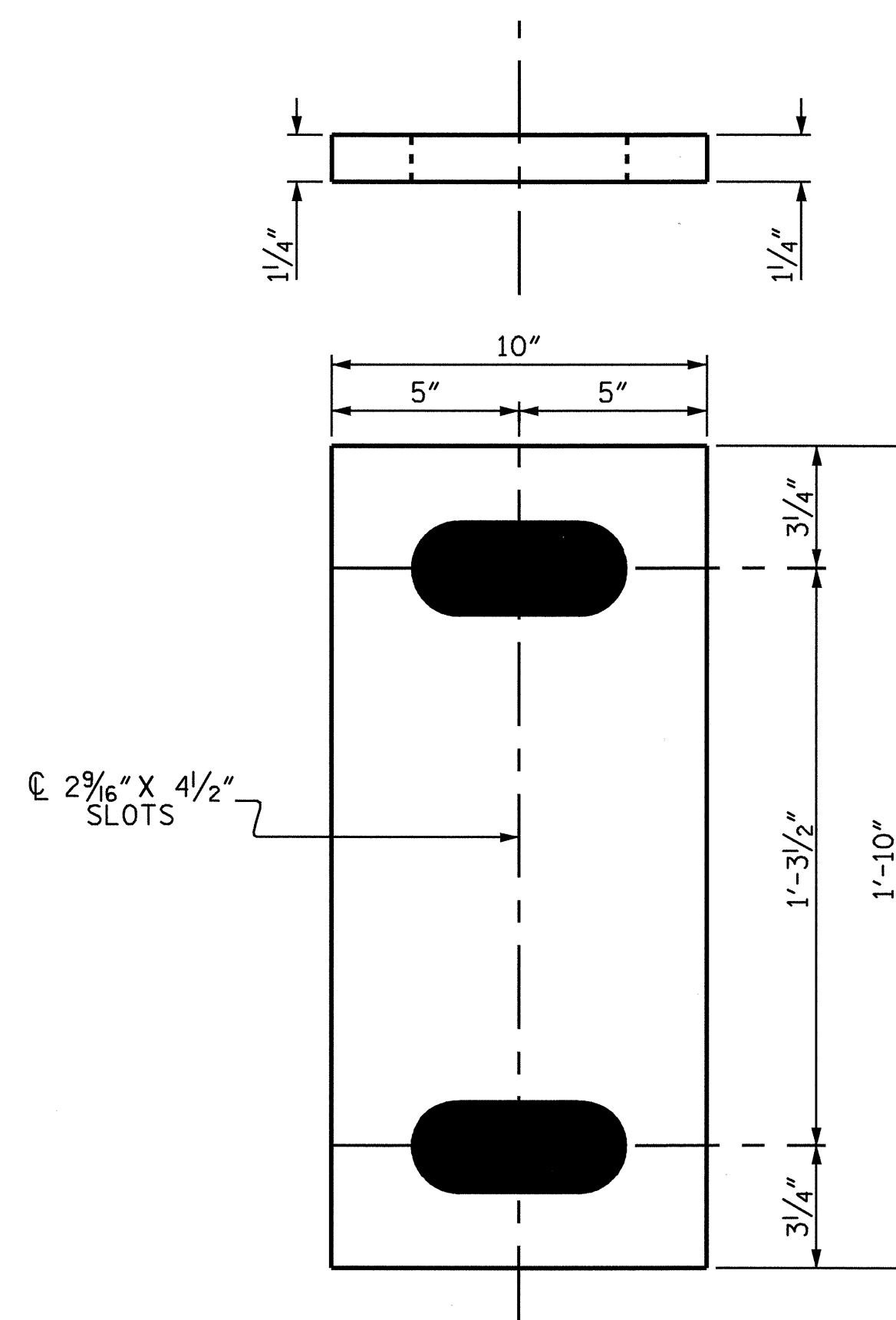
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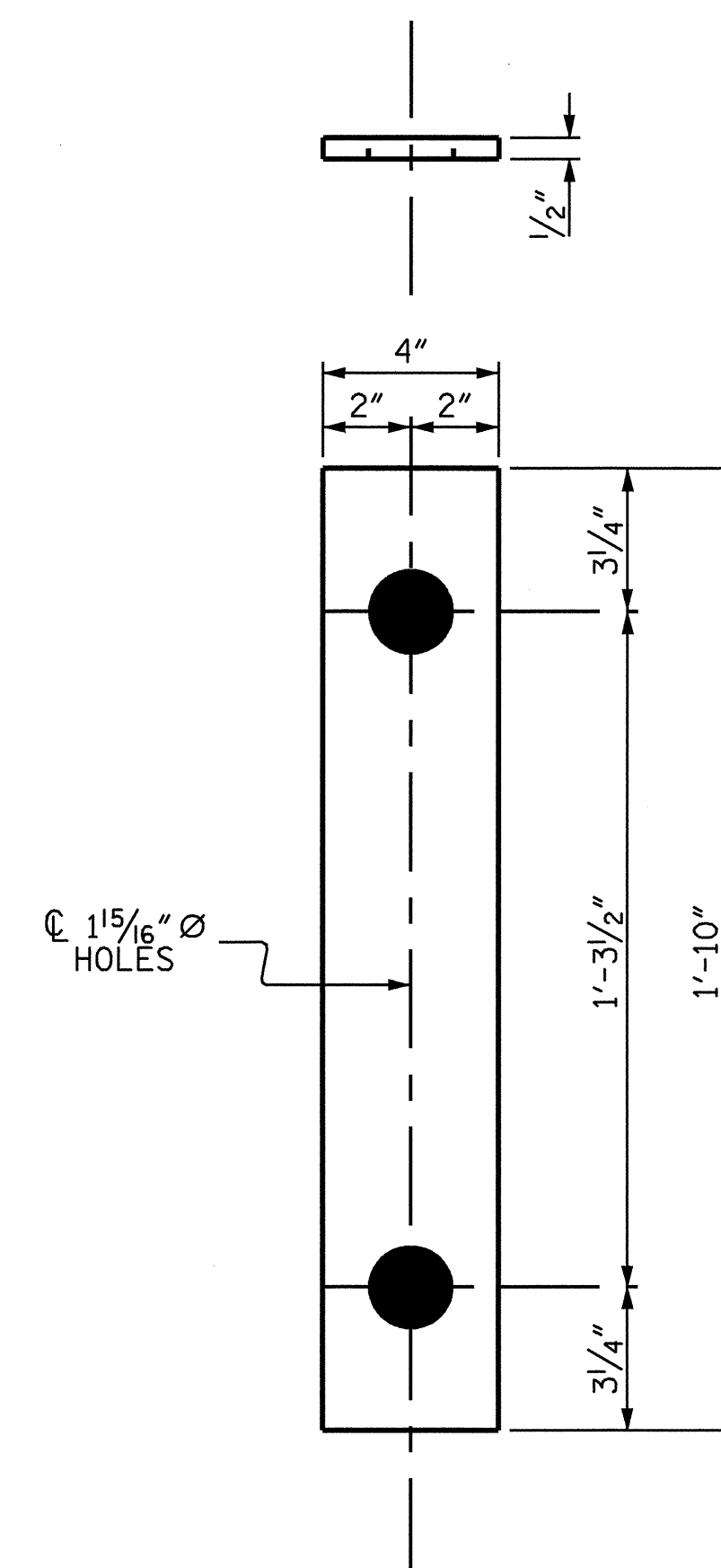
DETAIL "A"



LEVELING PLATE (P1)
(8 REQUIRED)



LEVELING PLATE (P2)
(8 REQUIRED)



ANCHORAGE PLATE (P3)
(16 REQUIRED)

NOTES

STRUCTURAL PLATE WASHERS SHALL BE AASHTO M270 GRADE 50W.

LEVELING PLATES AND ANCHORAGE PLATES SHALL BE AASHTO M270 GRADE 50W AND SHALL NOT BE GALVANIZED. ANCHOR BOLTS, NUTS AND WASHERS SHALL BE GALVANIZED IN ACCORDANCE WITH THE STANDARD SPECIFICATIONS.

ANCHOR BOLTS SHALL MEET THE REQUIREMENTS OF ASTM A449. NUTS SHALL MEET THE REQUIREMENTS OF AASHTO M291-DH OR AASHTO M292-2H. WASHERS SHALL MEET THE REQUIREMENTS OF AASHTO M293. SHOP DRAWINGS ARE NOT REQUIRED FOR ANCHOR BOLTS, NUTS AND WASHERS. SHOP INSPECTION IS REQUIRED.

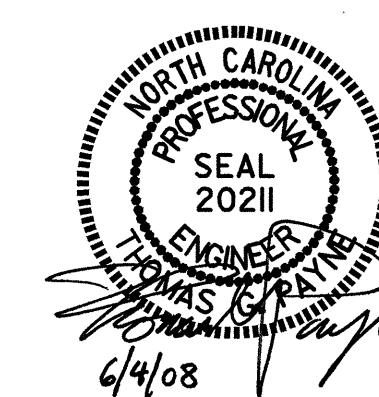
ALL SURFACES OF LEVELING PLATES SHALL BE SMOOTH AND STRAIGHT.

AT ALL FIXED POINTS OF SUPPORT AT INTEGRAL END BENT 1 AND 2, TOP NUTS FOR ANCHOR BOLTS ARE TO BE TIGHTENED FINGER TIGHT AND THEN BACKED OFF 1/2 TURN. THE THREAD OF THE NUT AND BOLT SHALL THEN BE BURRED WITH A SHARP POINTED TOOL.

PROJECT NO. B-4252
ROCKINGHAM COUNTY
 STATION: 27+69.00 -L-

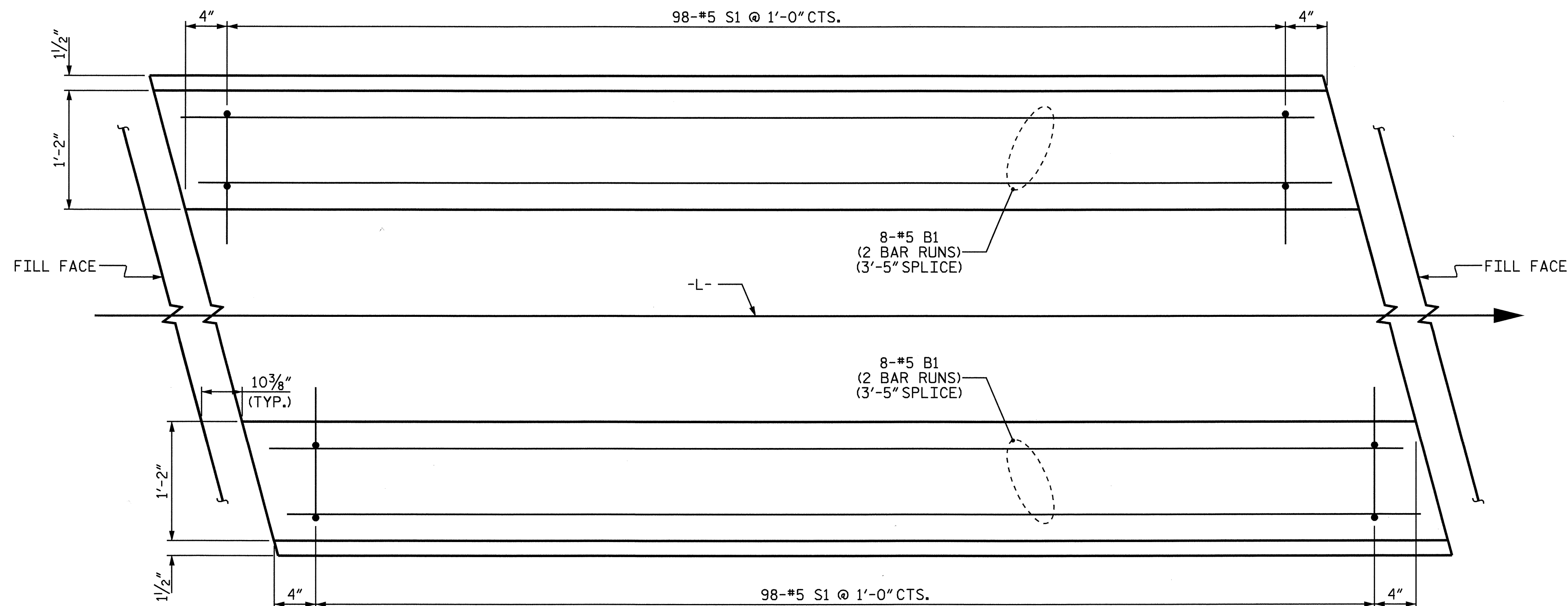
STATE OF NORTH CAROLINA
 DEPARTMENT OF TRANSPORTATION
 RALEIGH

SUPERSTRUCTURE
 LEVELING PLATE
 DETAILS



REVISIONS						SHEET NO.	
NO.	BY:	DATE:	NO.	BY:	DATE:	S-16	
1			3			TOTAL SHEETS	
2			4			70	

DRAWN BY : S. DOMBROWSKI DATE : 12/06
 CHECKED BY : M.K. BEARD DATE : 10/07



PLAN OF PARAPET

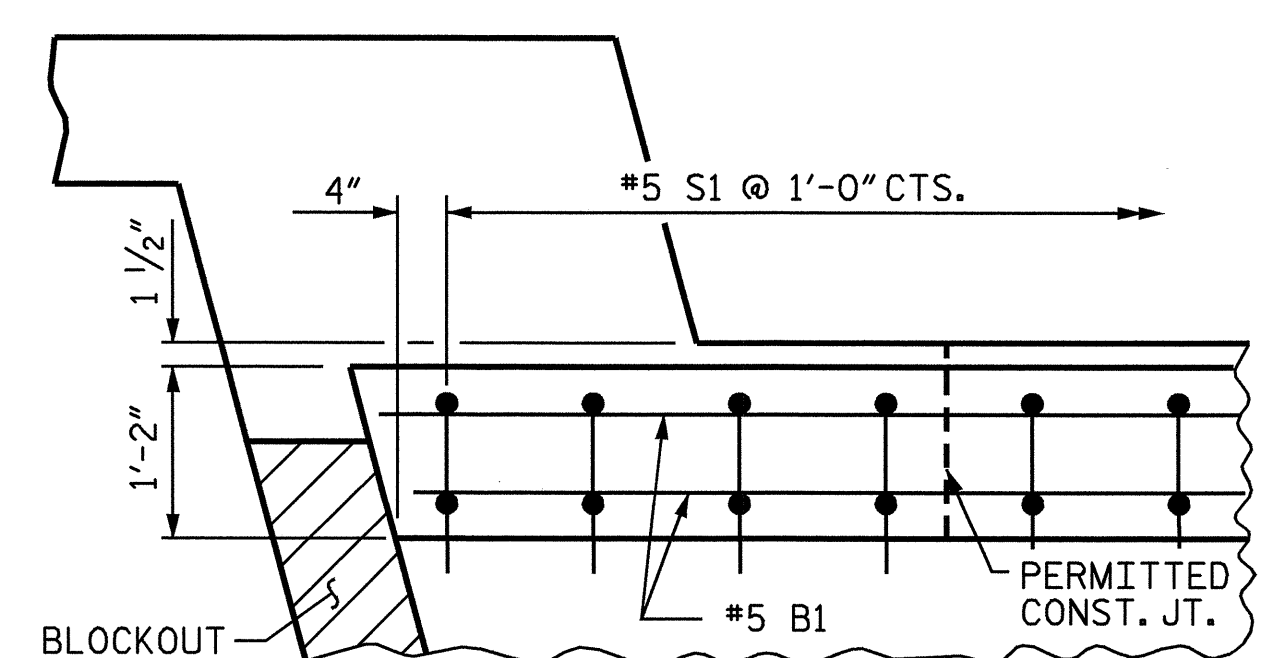
BAR TYPE		BILL OF MATERIAL				
PARAPETS & FOUR END POSTS						
BAR	NO.	SIZE	TYPE	LENGTH	WEIGHT	
*B1	32	#5	STR	50'-9"	1694	
*E1	8	#7	STR	2'-6"	41	
*E2	8	#7	STR	3'-0"	49	
*E3	8	#7	STR	3'-6"	57	
*E4	8	#7	STR	4'-0"	65	
*E5	8	#7	STR	4'-4"	71	
*F1	8	#6	STR	2'-0"	24	
*F2	4	#6	STR	3'-6"	21	
*F3	4	#6	STR	3'-3"	20	
*F4	4	#6	STR	3'-11"	24	
*F5	4	#6	STR	3'-8"	22	
*S1	196	#5	1	7'-0"	1431	

ALL BAR DIMENSIONS ARE OUT TO OUT

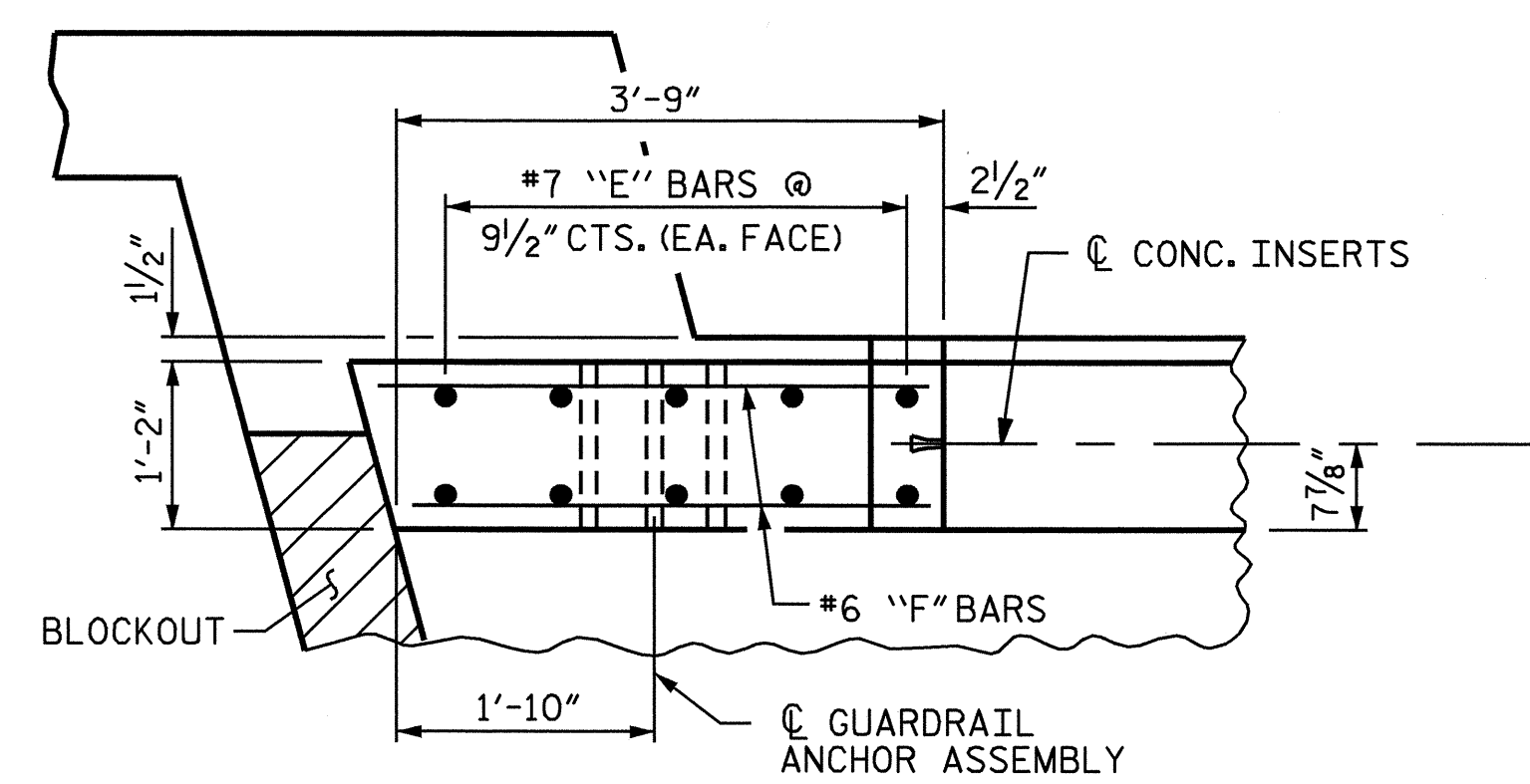
* EPOXY COATED REINFORCING STEEL	= 3519 LBS
CLASS "AA" CONCRETE	21.9 C.Y.
1'-2" X 2'-6" CONCRETE PARAPET	196.55 LIN.FT.

NOTES

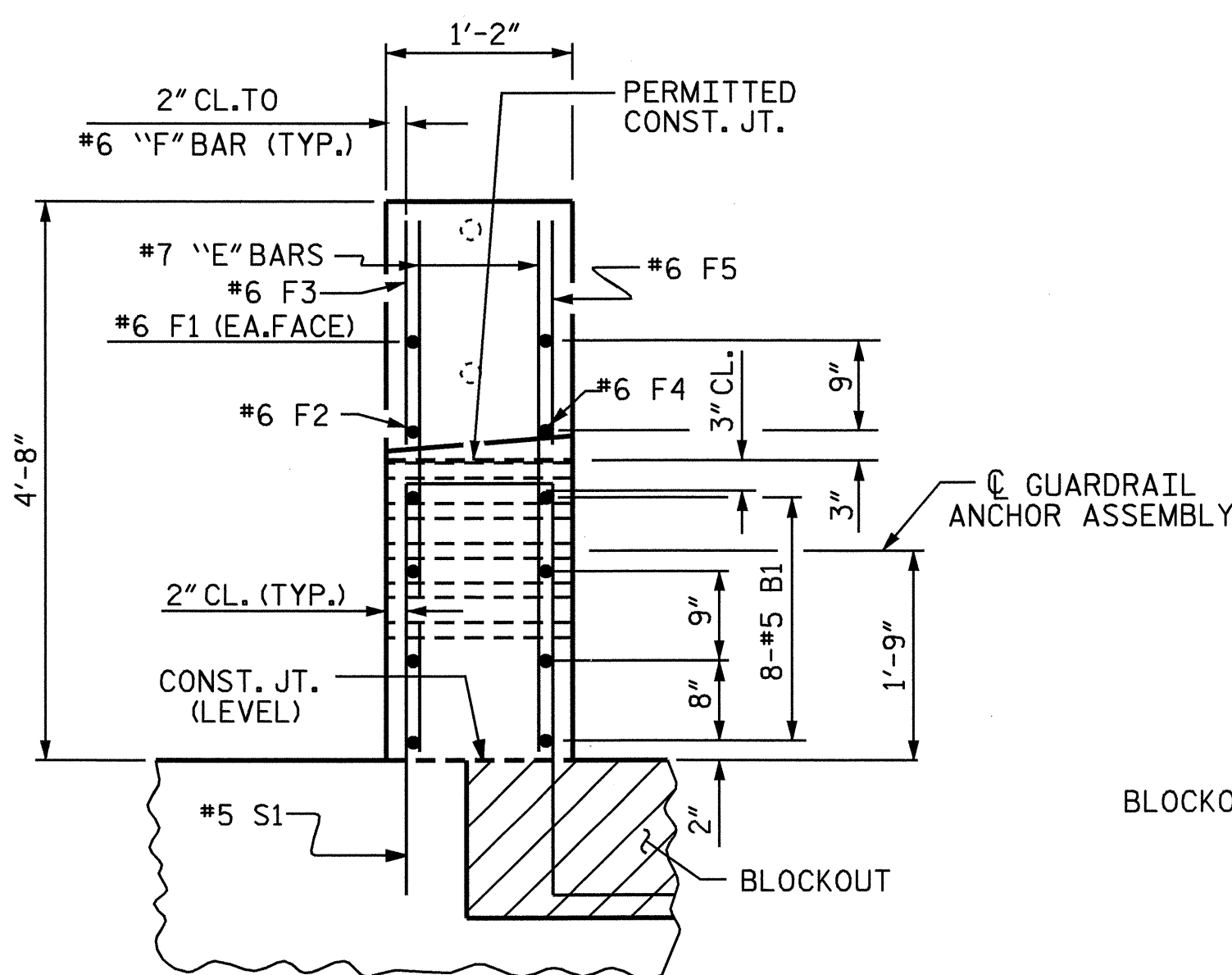
ALL REINFORCING STEEL IN PARAPET SHALL BE EPOXY COATED.
 FOR DETAIL OF CONCRETE INSERT AND GUARDRAIL ANCHOR ASSEMBLY, SEE "RAIL POST SPACING AND END OF RAIL DETAILS" SHEET.



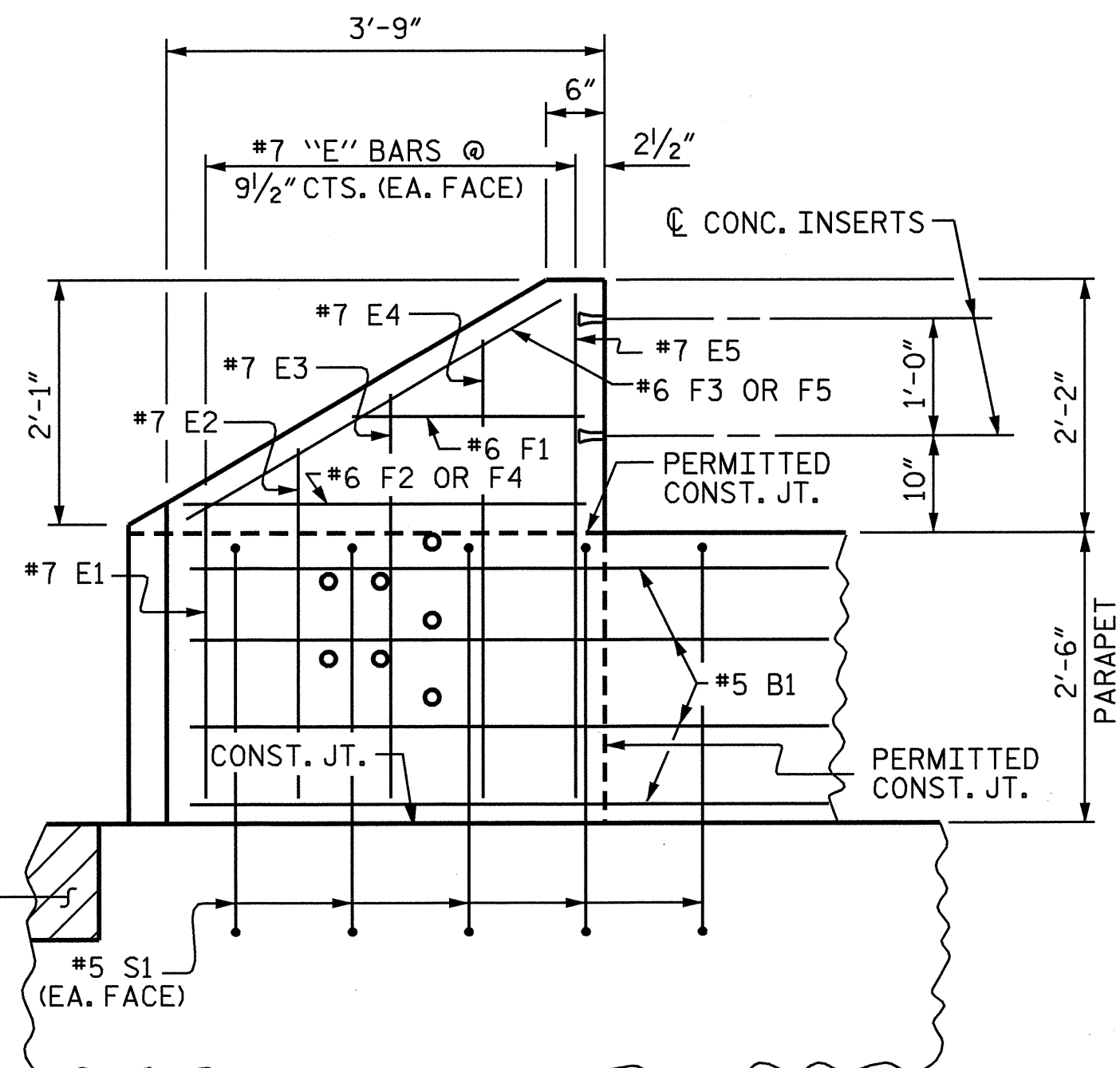
PLAN OF PARAPET



PLAN OF END POST



END VIEW



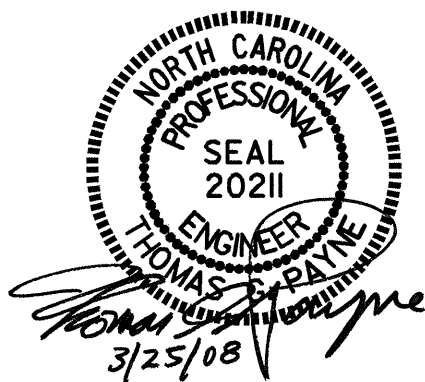
ELEVATION

PARAPET AND END POST FOR TWO BAR RAIL

PROJECT NO. B-4252
 ROCKINGHAM COUNTY
 STATION: 27+69.00 -L-

SHEET 1 OF 5

STATE OF NORTH CAROLINA
 DEPARTMENT OF TRANSPORTATION
 RALEIGH
 SUPERSTRUCTURE
 CONCRETE PARAPET
 AND END POST
 DETAILS
 FOR TWO BAR METAL RAILS



DRAWN BY : S. DOMBROWSKI DATE : 3/07
 CHECKED BY : M.K. BEARD DATE : 10/07

20-MAR-2008 15:33
 P:\Structures\plans\str1b-4252.sd.ss.dgn
 klayne

REVISIONS						SHEET NO.
NO.	BY:	DATE:	NO.	BY:	DATE:	S-17
1			3			TOTAL SHEETS 70
2			4			

STR.#1

NOTES

AT THE CONTRACTOR'S OPTION, METAL RAIL MAY BE EITHER ALUMINUM OR GALVANIZED STEEL IN ACCORDANCE WITH THE REQUIREMENTS OF THE GENERAL NOTES AND THE FOLLOWING SPECIFICATIONS FOR THE ALTERNATE MATERIALS; HOWEVER, THE CONTRACTOR WILL BE REQUIRED TO USE THE SAME RAIL MATERIAL ON ALL STRUCTURES ON THE PROJECT FOR WHICH METAL RAIL IS DESIGNATED.

ALUMINUM RAILS

MATERIAL FOR POSTS, BASES AND RAILS, EXPANSION BARS AND CLAMP BARS SHALL BE ASTM B-221 ALLOY 6061-T6. MATERIAL FOR RIVETS SHALL BE ASTM B316 ALLOY 6061-T6. RIVETS SHALL BE STANDARD BUTTON HEAD AND CONE POINT COLD DRIVEN AS PER DRAWING.

THE BASE OF RAIL POSTS, OR ANY OTHER ALUMINUM SURFACE IN CONTACT WITH CONCRETE SHALL BE THOROUGHLY COATED WITH AN ALUMINUM IMPREGNATED CAULKING COMPOUND OF APPROVED QUALITY.

MATERIAL FOR SHIMS TO BE ASTM B209 ALLOY 6061-T6.

GALVANIZED STEEL RAILS

MATERIAL AND GALVANIZING ARE TO CONFORM TO THE FOLLOWING SPECIFICATIONS:

POST, POST BASES, RAILS, EXPANSION BARS AND CLAMP BARS: AASHTO M270 GRADE 36 STRUCTURAL STEEL - GALVANIZED TO AASHTO M111.

RIVETS: RIVETS SHALL MEET THE REQUIREMENTS OF ASTM A502 FOR GRADE 1 RIVETS.

THE CUT ENDS OF GALVANIZED STEEL RAILING, AFTER GRINDING SMOOTH SHALL BE GIVEN TWO COATS OF ZINC RICH PAINT MEETING THE REQUIREMENTS OF FEDERAL SPECIFICATION MIL-P-26915 USAF TYPE 1, OR OF FEDERAL SPECIFICATIONS TT-P-641.

SHIMS: SHIMS SHALL MEET THE REQUIREMENTS OF ASTM A570 FOR GRADE 33 OR A611 FOR GRADE C AND SHALL BE GALVANIZED IN ACCORDANCE WITH AASHTO M111.

RAIL CAPS: RAIL CAPS SHALL MEET THE REQUIREMENTS OF ASTM A570 FOR GRADE 33 OR A611 FOR GRADE C AND SHALL BE GALVANIZED IN ACCORDANCE WITH AASHTO M111.

GENERAL NOTES

RAILING SHALL BE CONTINUOUS FROM END POST TO END POST OF BRIDGE. EACH JOINT IN RAIL LENGTH SHALL BE SPLICED AS DETAILED. PANEL LENGTHS OF RAIL SHALL BE ATTACHED TO A MINIMUM OF THREE POSTS.

FOR END OF RAIL TO CLEAR FACE OF CONCRETE END POST DIMENSION, SEE STANDARD NO. BMR2.

CAP SCREWS SHALL BE ASTM F593 ALLOY 305 STAINLESS STEEL. WASHERS SHALL MEET THE REQUIREMENTS OF ASTM F844 EXCEPT THEY SHALL BE MADE FROM ALLOY 304 STAINLESS STEEL.

CERTIFIED MILL REPORTS ARE REQUIRED FOR RAILS AND POSTS. SHOP INSPECTION IS NOT REQUIRED.

METAL RAIL POSTS SHALL BE SET NORMAL TO CURB GRADE.

METHOD OF MEASUREMENT FOR METAL RAILS: FOR LENGTH OF METAL RAILS TO BE PAID FOR, SEE THE STANDARD SPECIFICATIONS.

CURVED RAIL USAGE: WHERE RAILS ARE TO BE USED ON BRIDGES ON HORIZONTAL AND/OR VERTICAL CURVATURE THE CONTRACTOR MAY, AT HIS OPTION, HAVE THE REQUIRED CURVATURE IN THE RAIL FORMED IN THE SHOP OR IN THE FIELD. IN EITHER EVENT, THE RAIL SHALL CONFORM WITHOUT BUCKLING OR KINKING TO THE REQUIRED CURVATURE IN A UNIFORM MANNER ACCEPTABLE TO THE ENGINEER.

TO INSURE FUTURE IDENTIFICATION OF THE FABRICATOR, A PERMANENT IDENTIFYING MARK SHALL BE PLACED ON EACH POST. THE METHOD OF MARKING AND LOCATION SHALL BE SUCH THAT IT DOES NOT DETRACT FROM THE APPEARANCE OF THE POST, BUT REMAINS VISIBLE AFTER RAIL PLACEMENT.

SHIMS SHALL BE USED AS NECESSARY FOR POST ALIGNMENT.

ALLOY 6351-T5 MAY BE SUBSTITUTED FOR ALLOY 6061-T6 WHERE APPLICABLE.

MINOR VARIATIONS IN DETAILS OF METAL RAIL WILL BE CONSIDERED. DETAILS OF SUCH VARIATIONS, IF DESIRED, SHALL BE SUBMITTED FOR APPROVAL.

GROOVED CONTRACTION JOINTS, 1/2" IN DEPTH, SHALL BE TOOLED IN ALL EXPOSED FACES OF THE PARAPET IN ACCORDANCE WITH ARTICLE 825-10(B) OF THE STANDARD SPECIFICATIONS. THE CONTRACTION JOINT SHALL BE LOCATED AT A SPACING OF 8FT. TO 10FT. BETWEEN EXPANSION JOINTS. NO CONTRACTION JOINTS WILL BE REQUIRED FOR SEGMENTS LESS THAN 10 FEET IN LENGTH.

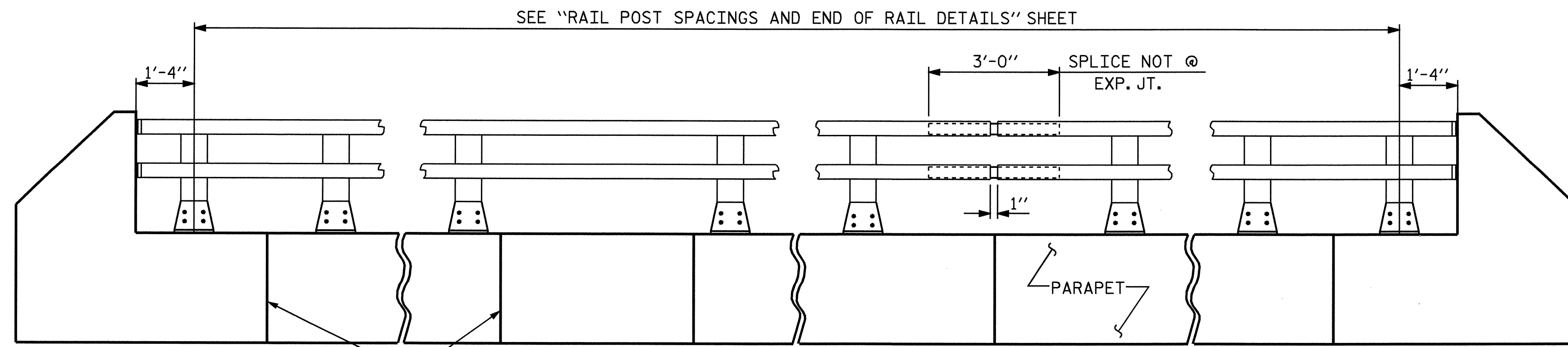
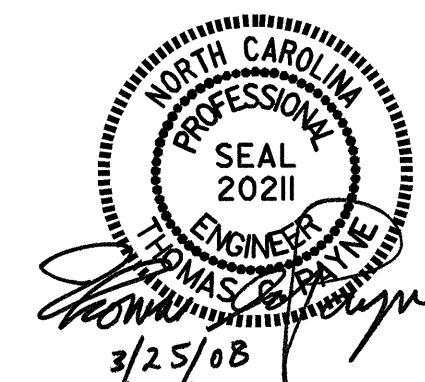
PAY LENGTH = 180.92 LIN. FT.

PROJECT NO. B-4252
ROCKINGHAM COUNTY
 STATION: 27+69.00 -L-

SHEET 2 OF 5

STATE OF NORTH CAROLINA
 DEPARTMENT OF TRANSPORTATION
 RALEIGH
 STANDARD
2 BAR METAL RAIL

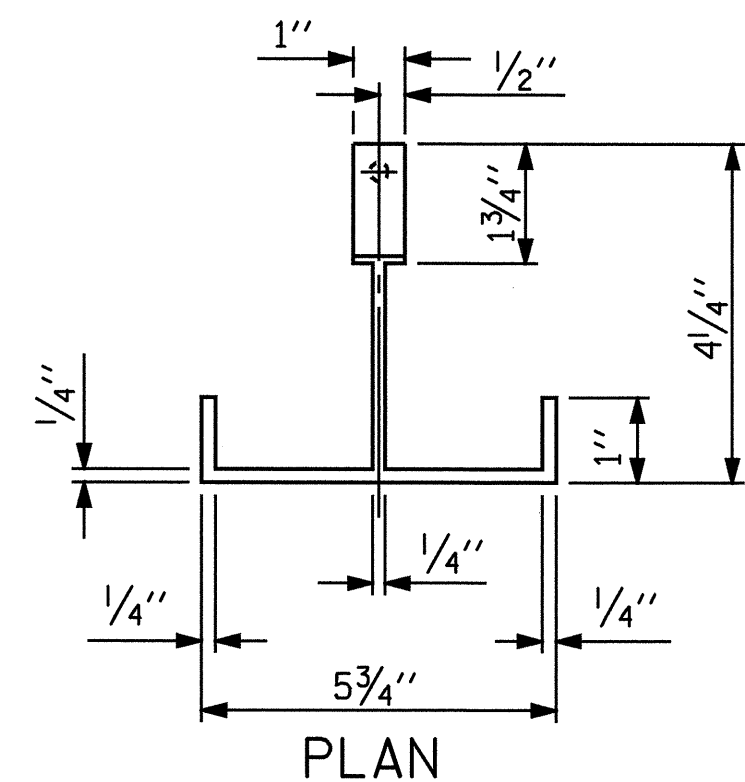
REVISIONS						SHEET NO.
NO.	BY:	DATE:	NO.	BY:	DATE:	S-18
1			3			TOTAL SHEETS
2			4			70



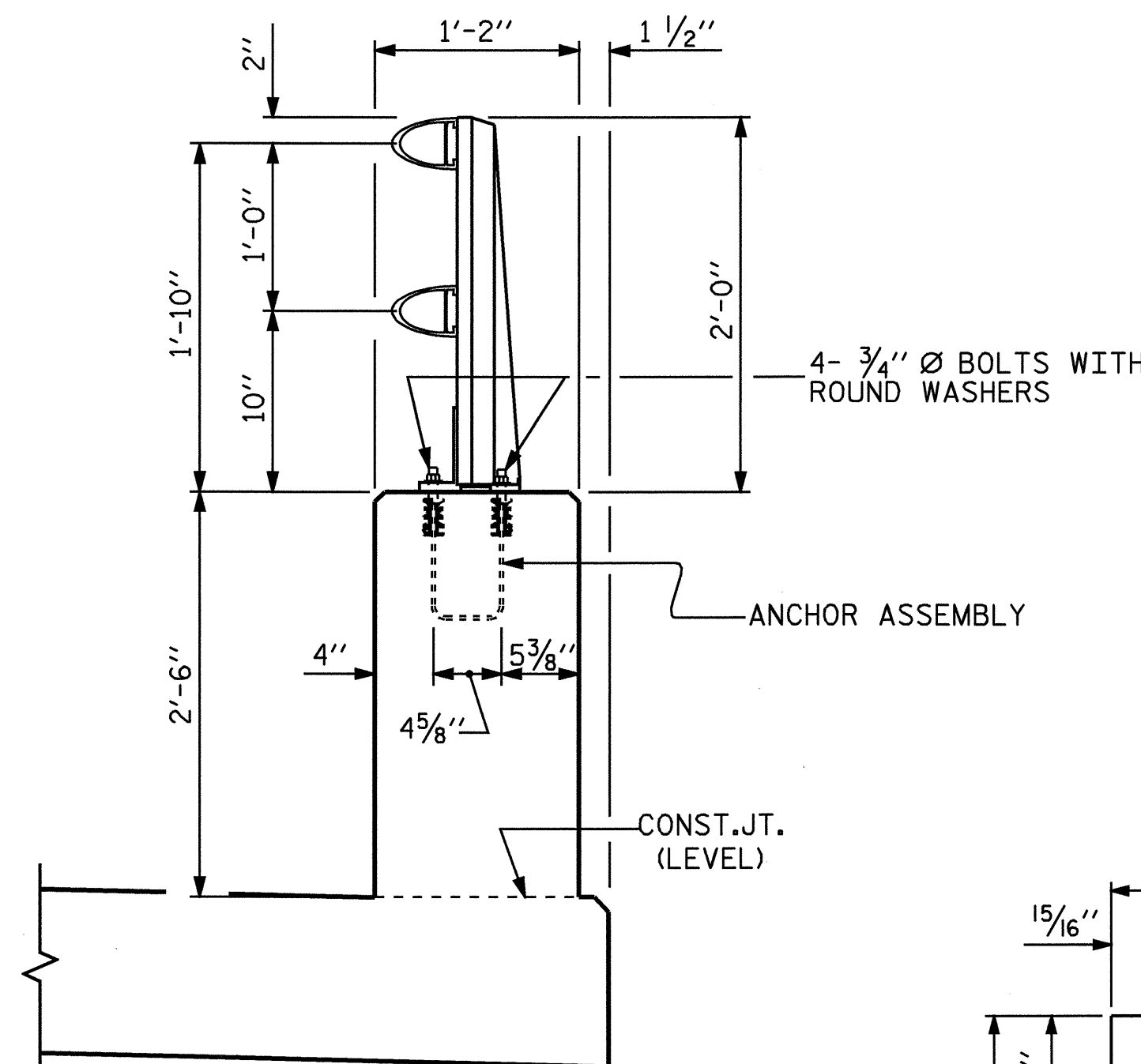
ELEVATION

TOOLED CONTRACTION JT. (SEE NOTES)

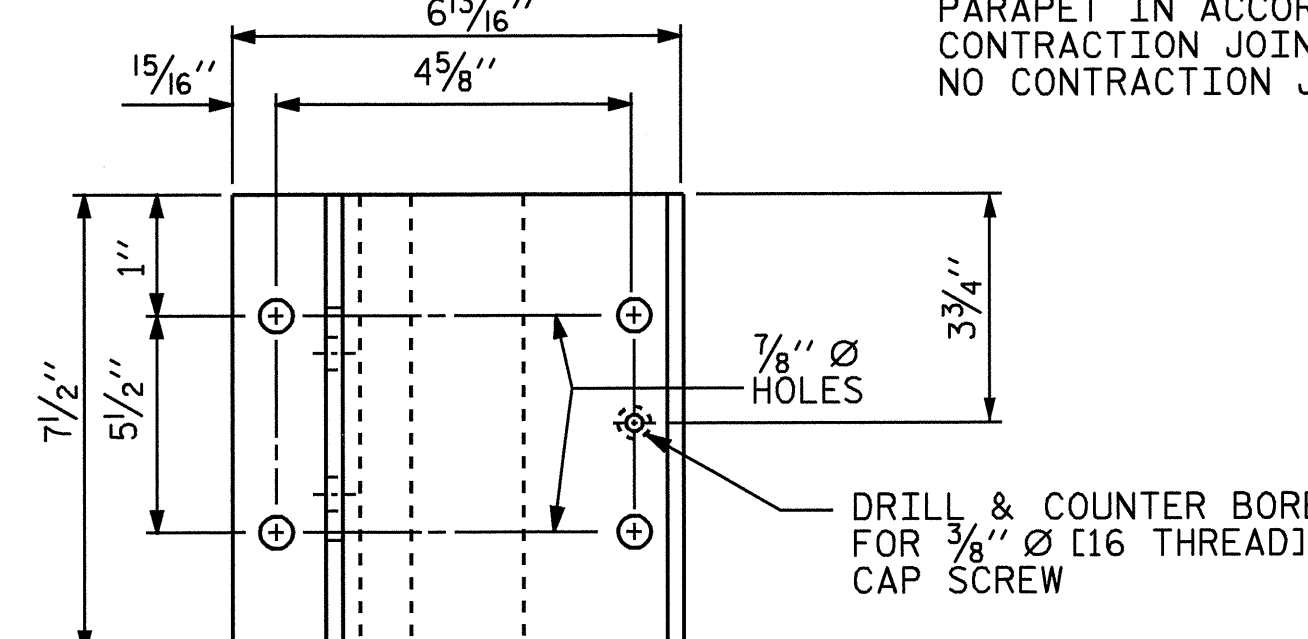
NOTE: FOR ATTACHMENT OF METAL RAIL TO END POST, SEE STANDARD NO. BMR2.



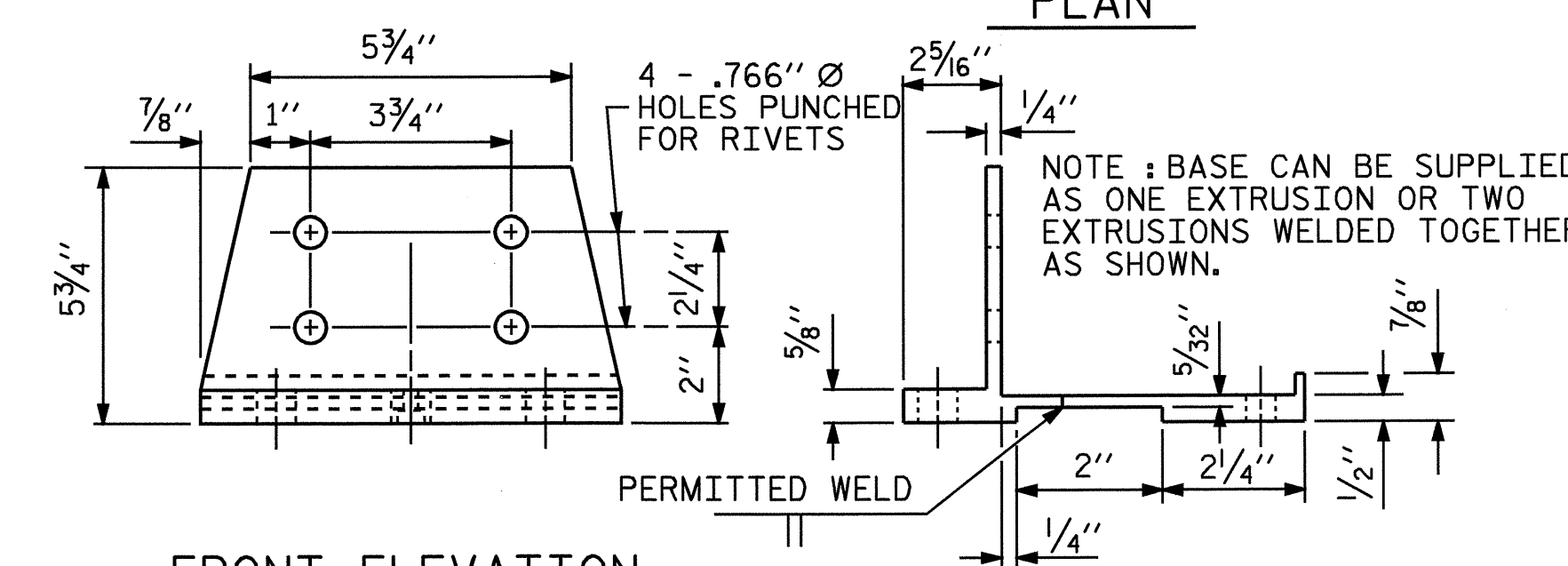
PLAN



SECTION THRU PARAPET AND RAIL

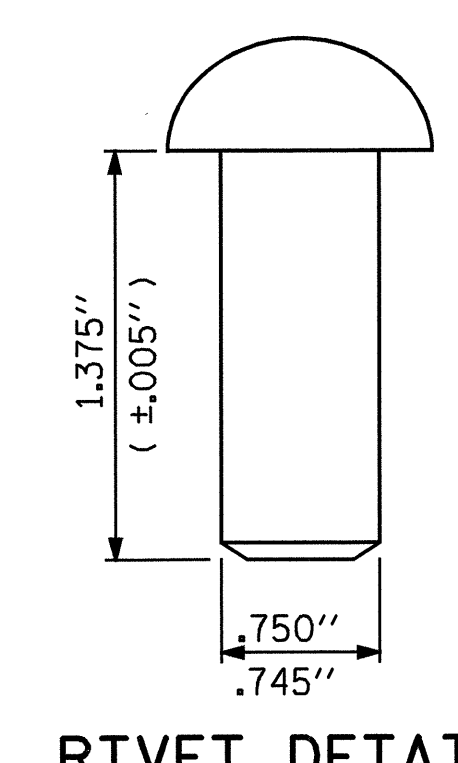


PLAN

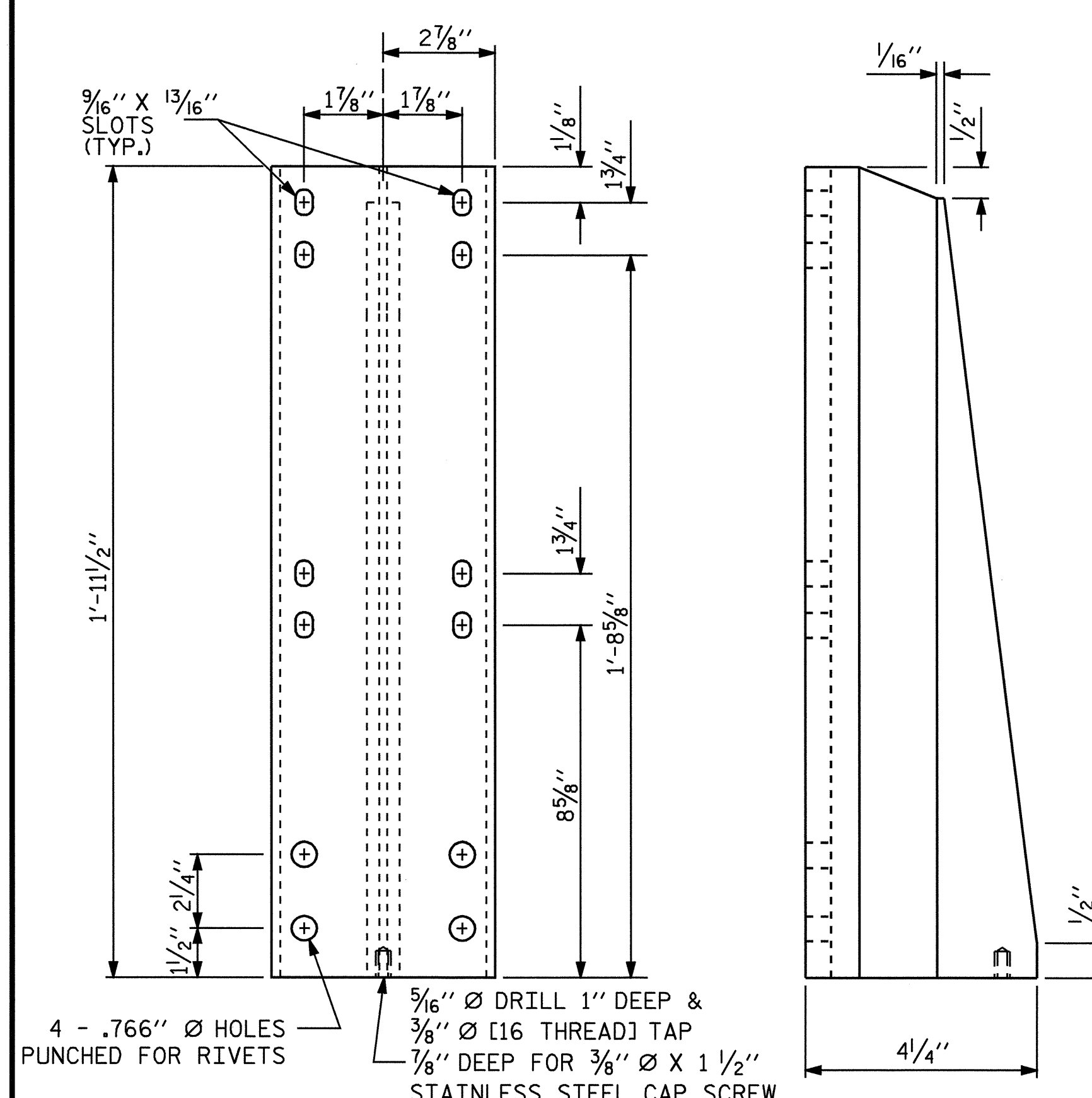


FRONT ELEVATION SIDE ELEVATION

POST BASE DETAILS



RIVET DETAIL



FRONT ELEVATION SIDE ELEVATION

DETAILS OF POST

ASSEMBLED BY : S. DOMBROWSKI	DATE : 7/07
CHECKED BY : M.K. BEARD	DATE : 10/07
DRAWN BY : EEM 6/94	REV. 10/17/00 LES/RDR
CHECKED BY : RGW 6/94	REV. 5/7/03R RWW/JTE
	REV. 5/1/06 TLA/GM

NOTES

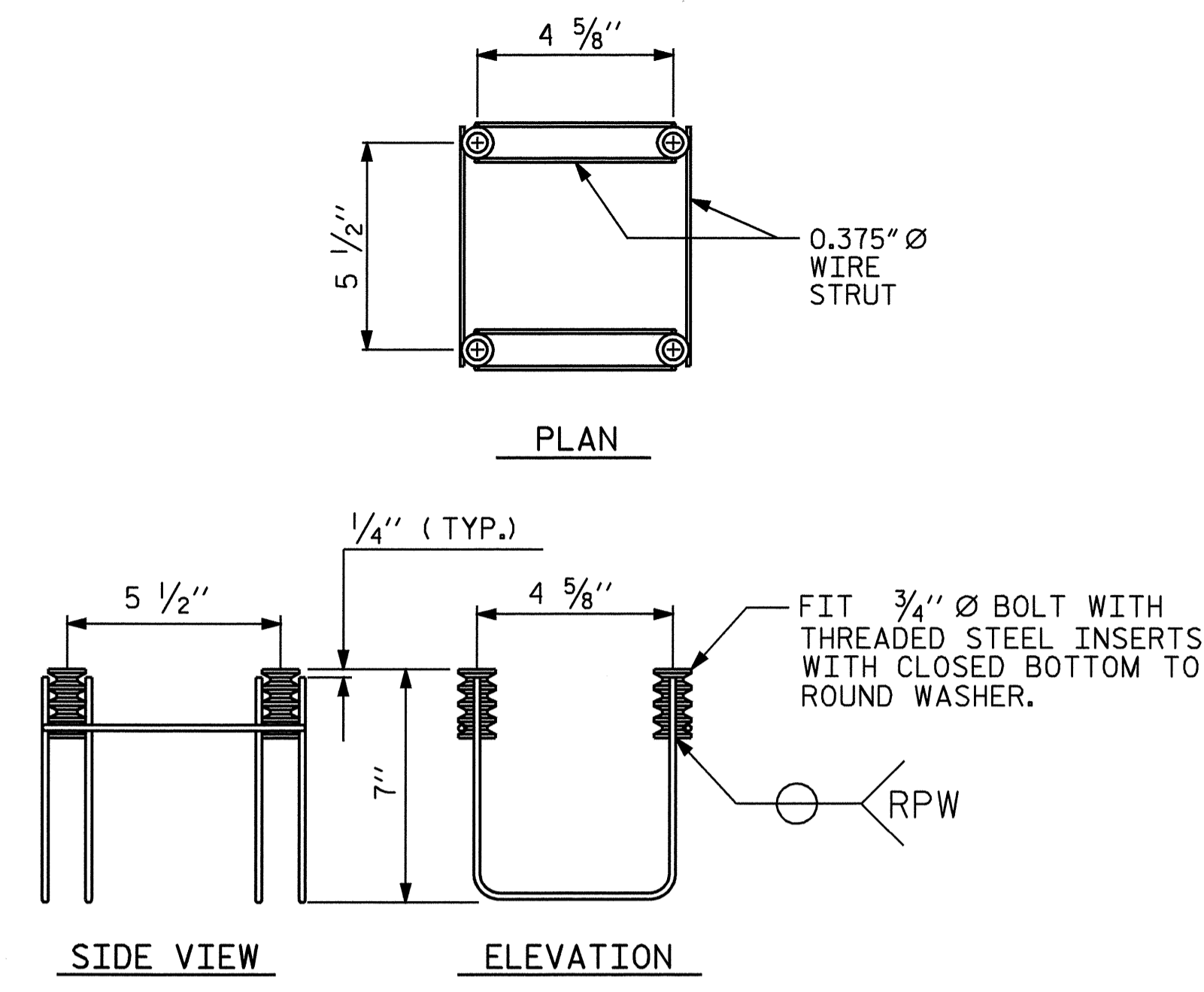
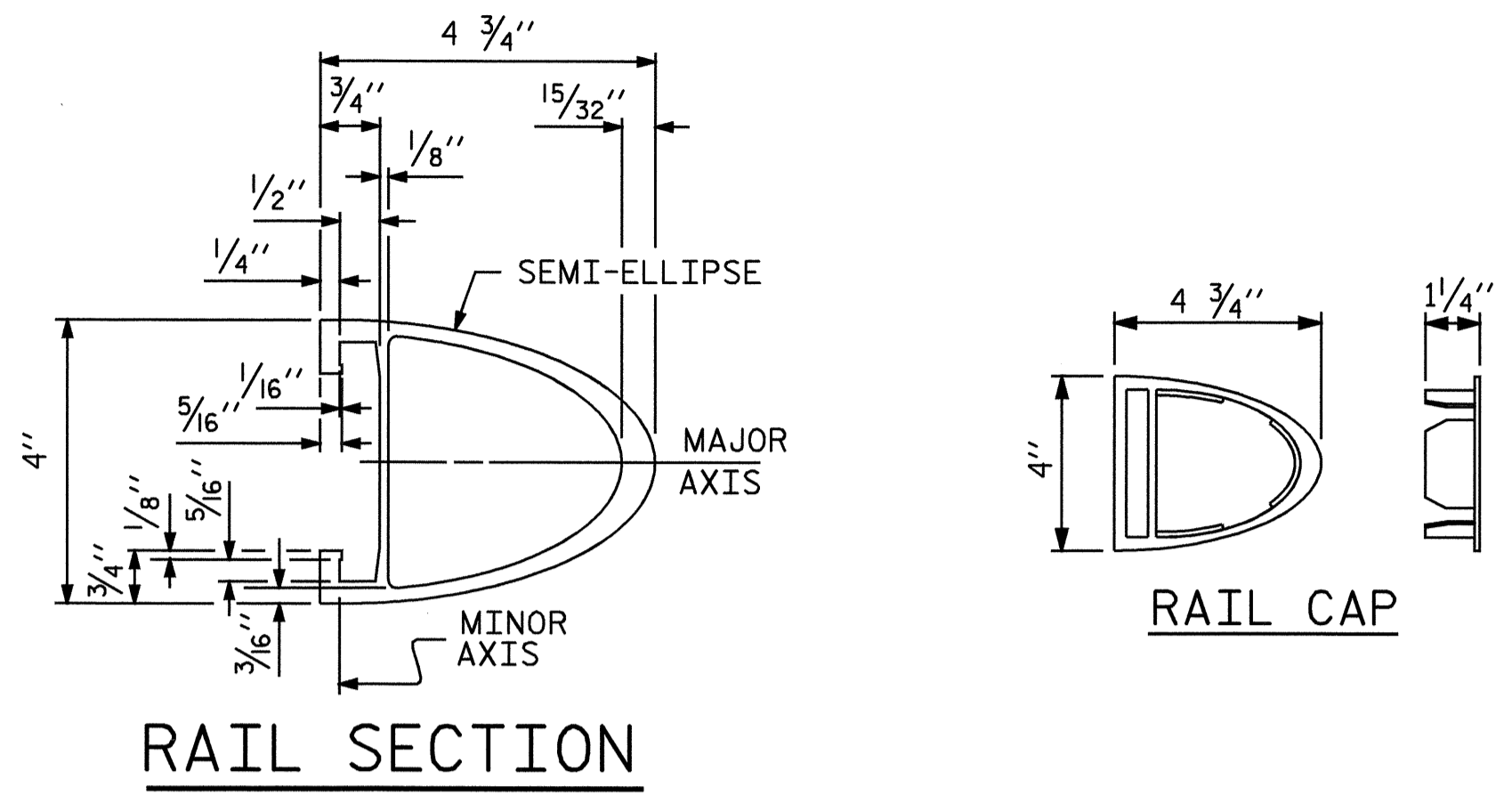
STRUCTURAL CONCRETE ANCHOR ASSEMBLY

THE STRUCTURAL CONCRETE ANCHOR ASSEMBLY SHALL CONSIST OF THE FOLLOWING COMPONENTS :

- A. FERRULES SHALL BE MADE FROM STEEL MEETING THE REQUIREMENTS OF AASHTO M169, GRADE 12L14 AND SHALL HAVE A MINIMUM LENGTH OF THREADS OF 2" FOR 3/4" FERRULES.
- B. 4 - 3/4" Ø X 2 1/2" BOLTS WITH WASHERS. BOLTS SHALL CONFORM TO THE REQUIREMENTS OF ASTM A307. BOLTS AND WASHERS SHALL BE GALVANIZED. AT THE CONTRACTOR'S OPTION, STAINLESS STEEL BOLTS AND WASHERS MAY BE USED AS AN ALTERNATE FOR THE 3/4" Ø X 2 1/2" GALVANIZED BOLTS AND WASHERS. THEY SHALL CONFORM TO OR EXCEED THE MECHANICAL REQUIREMENTS OF ASTM A307. THE USE OF THIS ALTERNATE SHALL BE APPROVED BY THE ENGINEER.
- C. WIRE STRUT SHOWN IN THE CONCRETE ANCHOR ASSEMBLY DETAIL IS THE MINIMUM ALLOWABLE SIZE AND SHALL HAVE A MINIMUM TENSILE STRENGTH OF 100,000 PSI. AS AN OPTION, A 7/16" Ø WIRE STRUT WITH A MINIMUM TENSILE STRENGTH OF 90,000 PSI IS ACCEPTABLE.
- D. THE METAL RAIL ANCHOR ASSEMBLIES TO BE HOT DIPPED GALVANIZED TO CONFORM TO REQUIREMENTS OF AASHTO M111.
- E. THE COST OF THE METAL RAIL ANCHOR ASSEMBLY WITH BOLTS AND WASHERS COMPLETE IN PLACE SHALL BE INCLUDED IN THE PRICE BID FOR LINEAR FEET OF METAL RAIL.
- F. BOLTS TO BE TIGHTENED ONE-HALF TURN WITH A WRENCH FROM A FINGER-TIGHT POSITION.

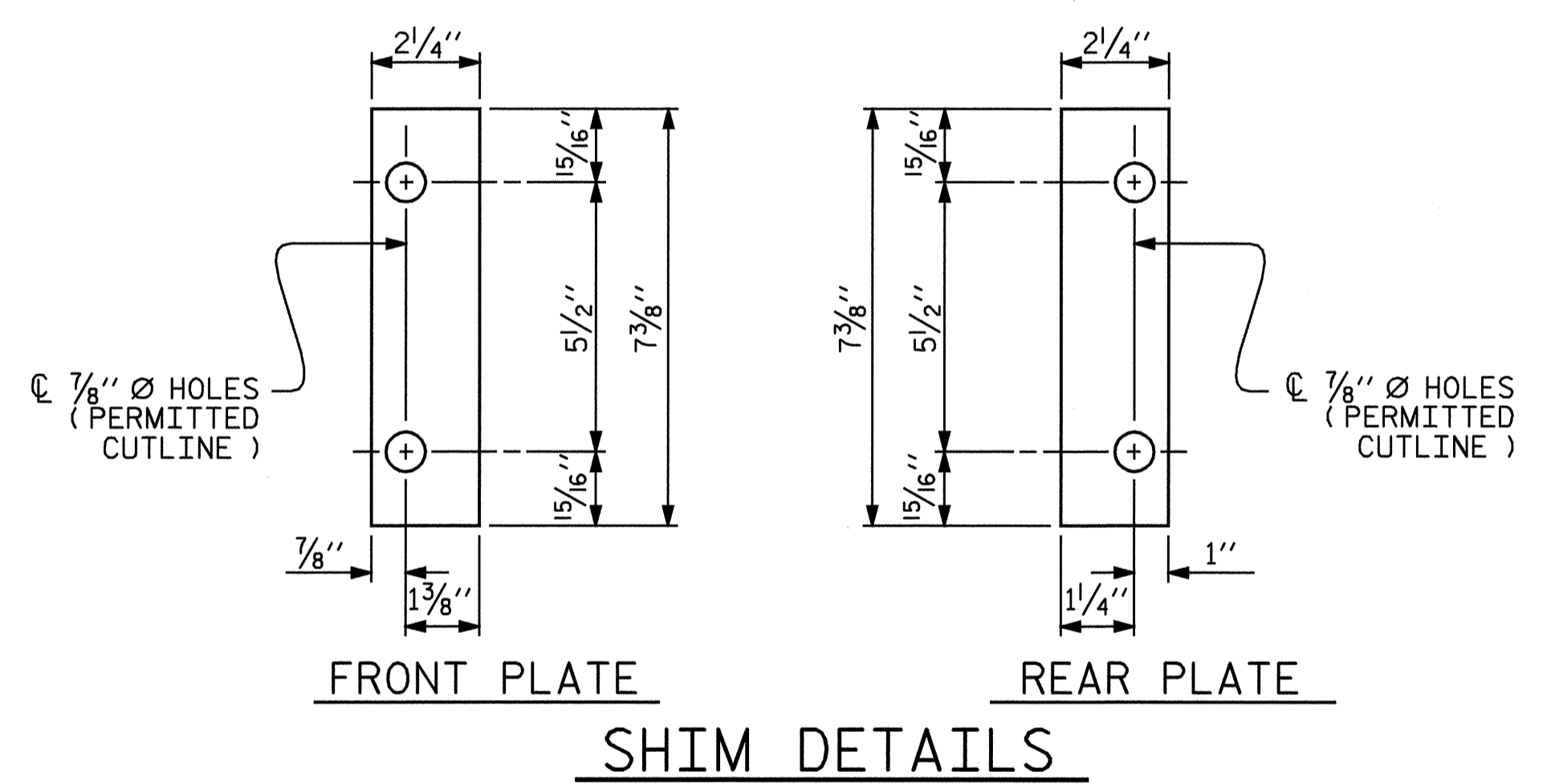
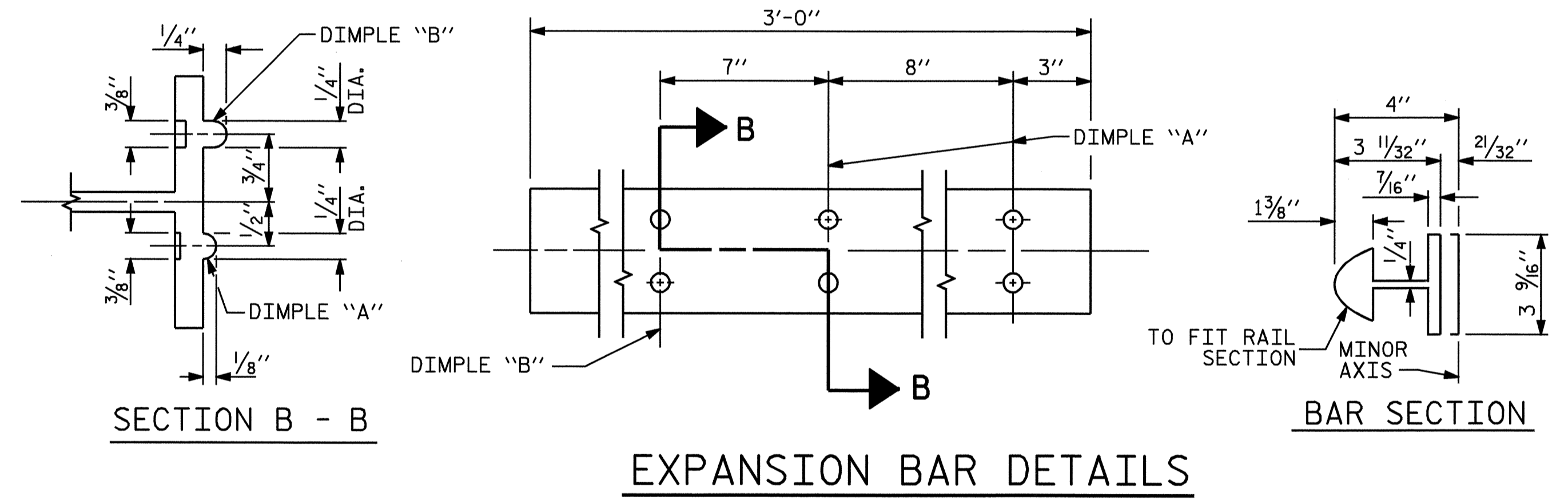
THE CONTRACTOR MAY USE ADHESIVELY ANCHORED ANCHOR BOLTS IN PLACE OF THE METAL RAIL ANCHOR ASSEMBLY. LEVEL ONE FIELD TESTING IS REQUIRED, AND THE YIELD LOAD OF THE 3/4" Ø BOLT IS 10 KIPS. FOR ADHESIVELY ANCHORED ANCHOR BOLTS OR DOWELS, SEE SPECIAL PROVISIONS.

WHEN ADHESIVELY ANCHORED ANCHOR BOLTS ARE USED, BOLTS SHALL MEET THE REQUIREMENTS OF ASTM F593 ALLOY 304 STAINLESS STEEL WITH MINIMUM 75,000 PSI ULTIMATE STRENGTH. NUTS SHALL MEET THE REQUIREMENTS OF ASTM F594 ALLOY 304 STAINLESS STEEL AND WASHERS SHALL MEET THE REQUIREMENTS OF ASTM F844 EXCEPT THEY SHALL BE MADE FROM ALLOY 304 STAINLESS STEEL.

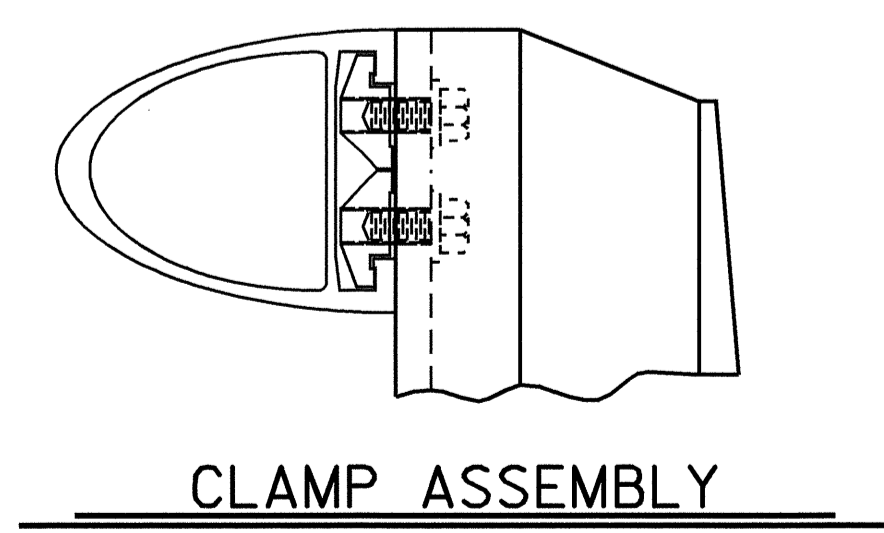
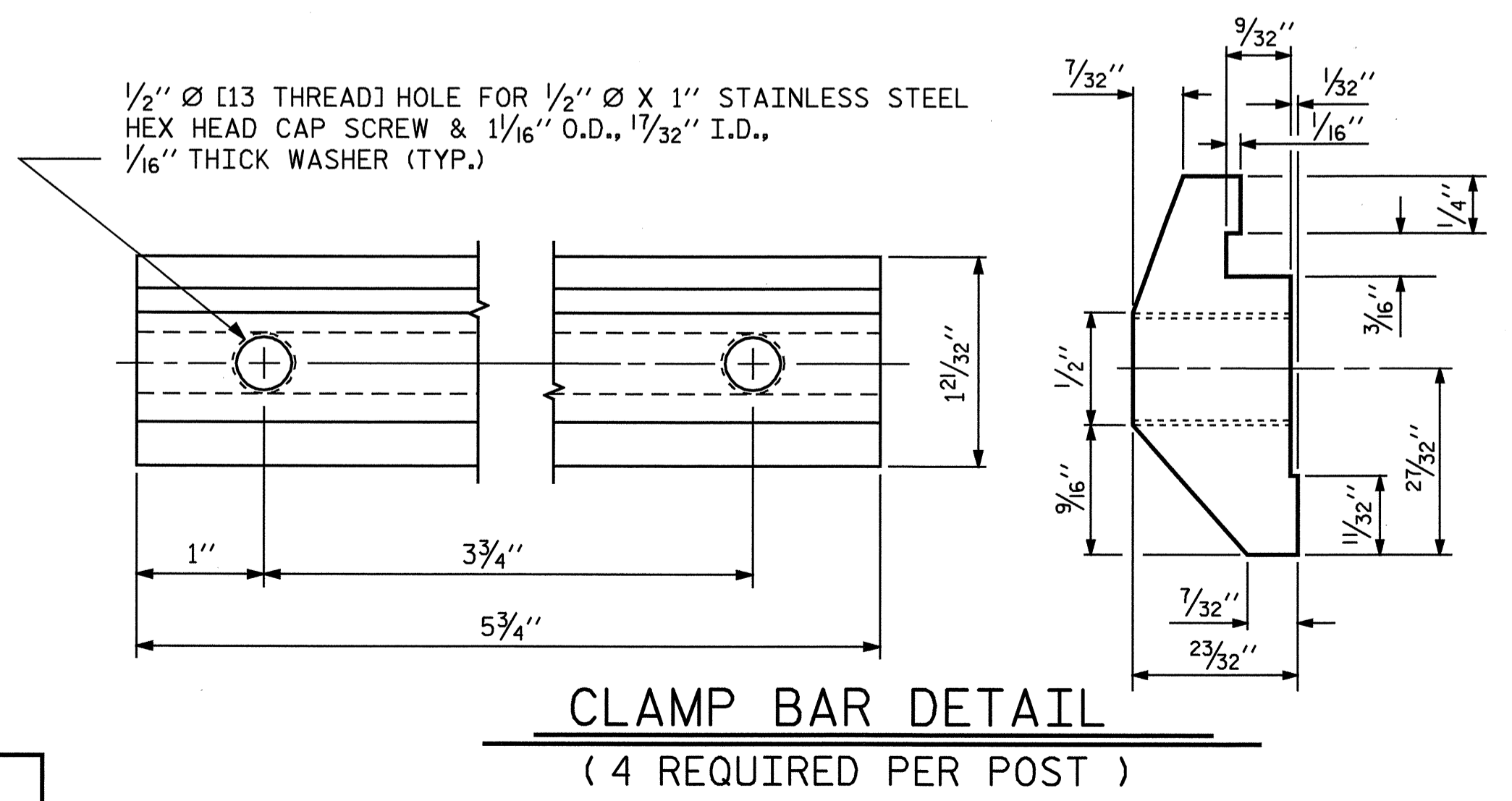


MINIMUM LENGTH OF THREADS IN INSERT (FERRULE) : 1 3/4"

4-BOLT METAL RAIL ANCHOR ASSEMBLY
 (32 ASSEMBLIES REQUIRED)



NOTE : SHIMS MAY BE CUT ALONG PERMITTED CUTLINE OR SLOTTED TO EDGE OF PLATE TO FACILITATE PLACEMENT.

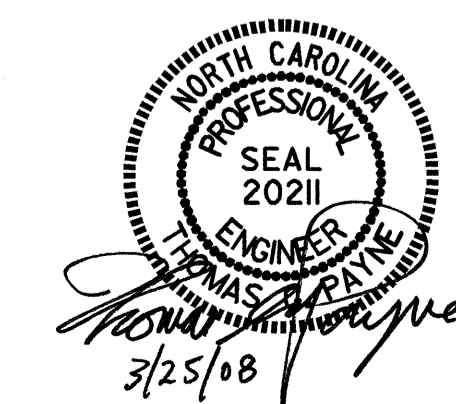


PROJECT NO. B-4252
ROCKINGHAM COUNTY
 STATION: 27+69.00 -L-

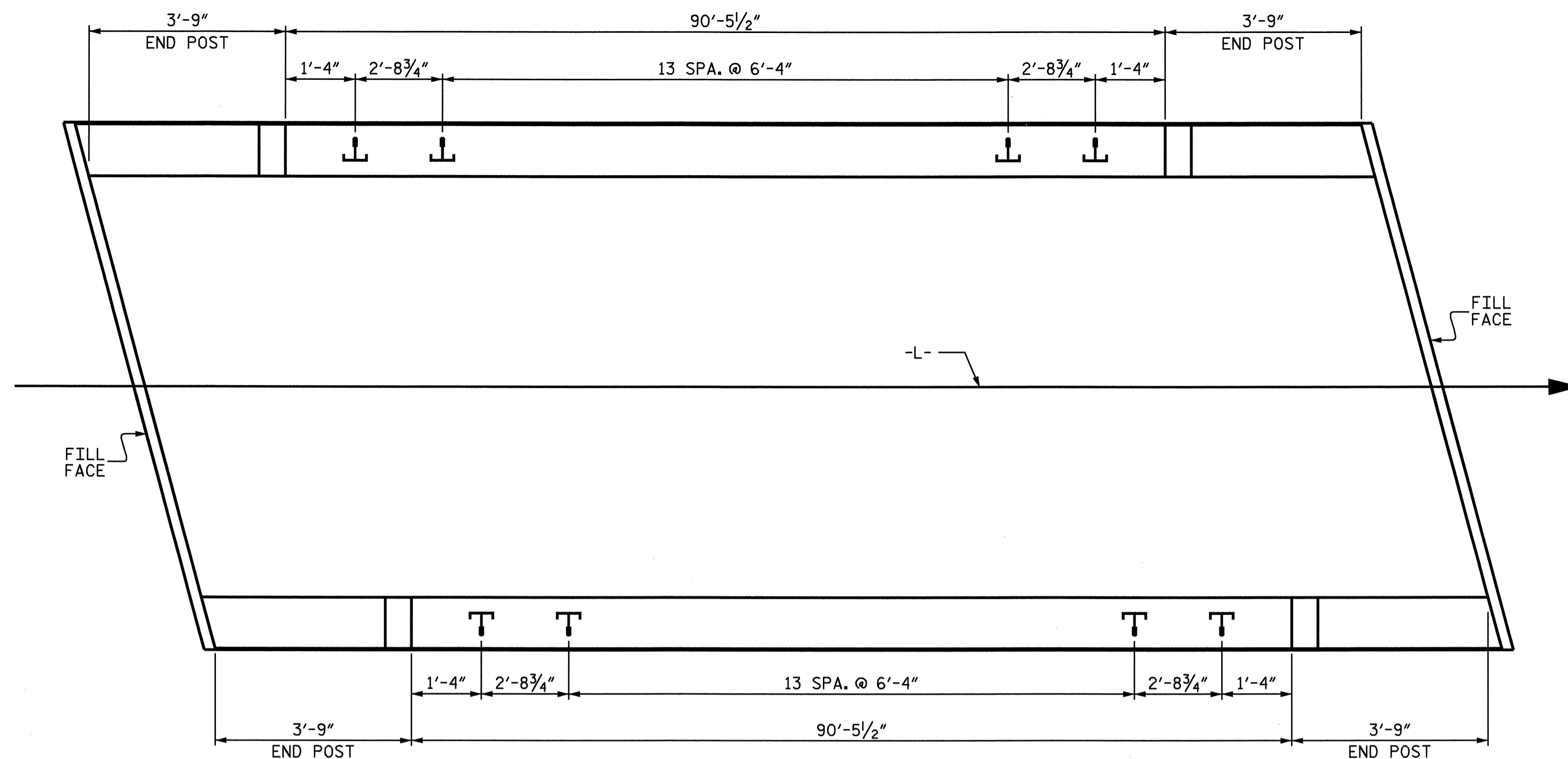
SHEET 3 OF 5

STATE OF NORTH CAROLINA
 DEPARTMENT OF TRANSPORTATION
 RALEIGH
 STANDARD
 2 BAR METAL RAIL

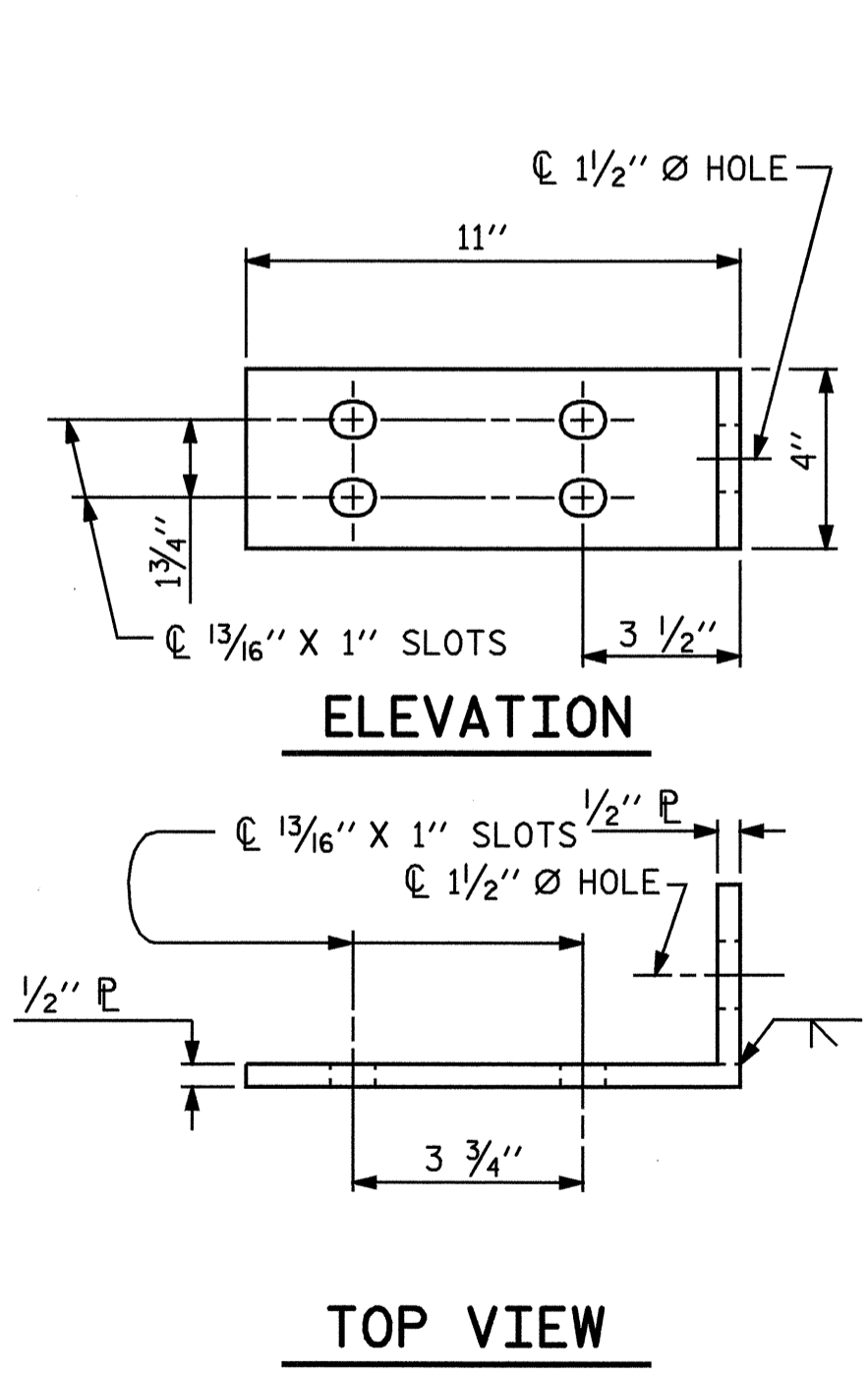
REVISIONS						SHEET NO.	
NO.	BY:	DATE:	NO.	BY:	DATE:	S-19	
1			3			TOTAL SHEETS 70	
2			4				



ASSEMBLED BY : S. DOMBROWSKI	DATE : 3/07
CHECKED BY : M.K. BEARD	DATE : 10/07
DRAWN BY : EEM 6/94	REV. 2/6/97 EEM/RGW
CHECKED BY : RGW 6/94	REV. 8/16/99 MAB/LES
	REV. 5/1/06R KMM/GM

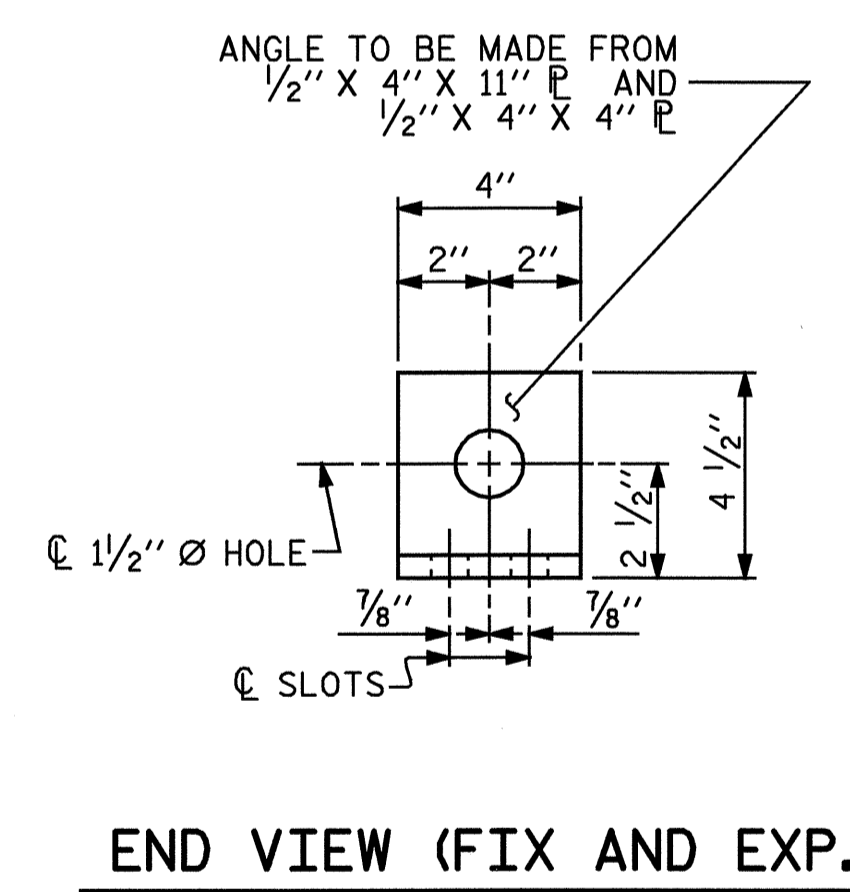


PLAN OF RAIL POST SPACINGS

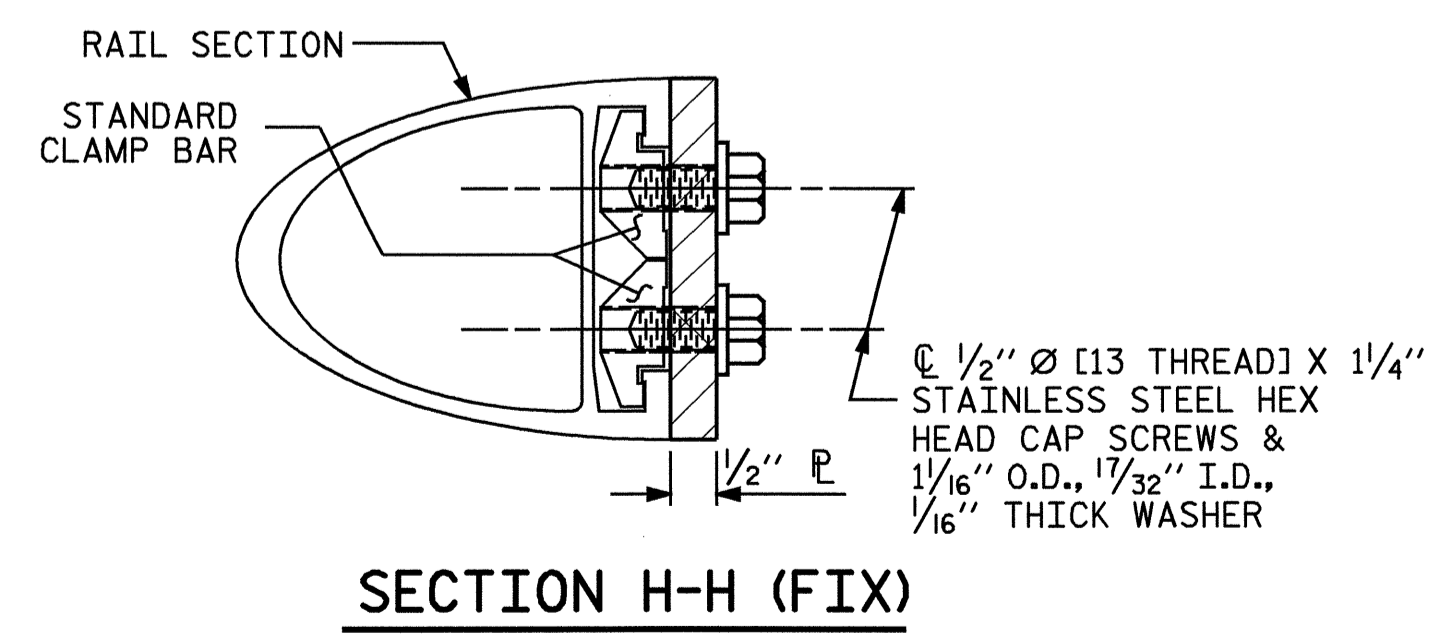


ELEVATION

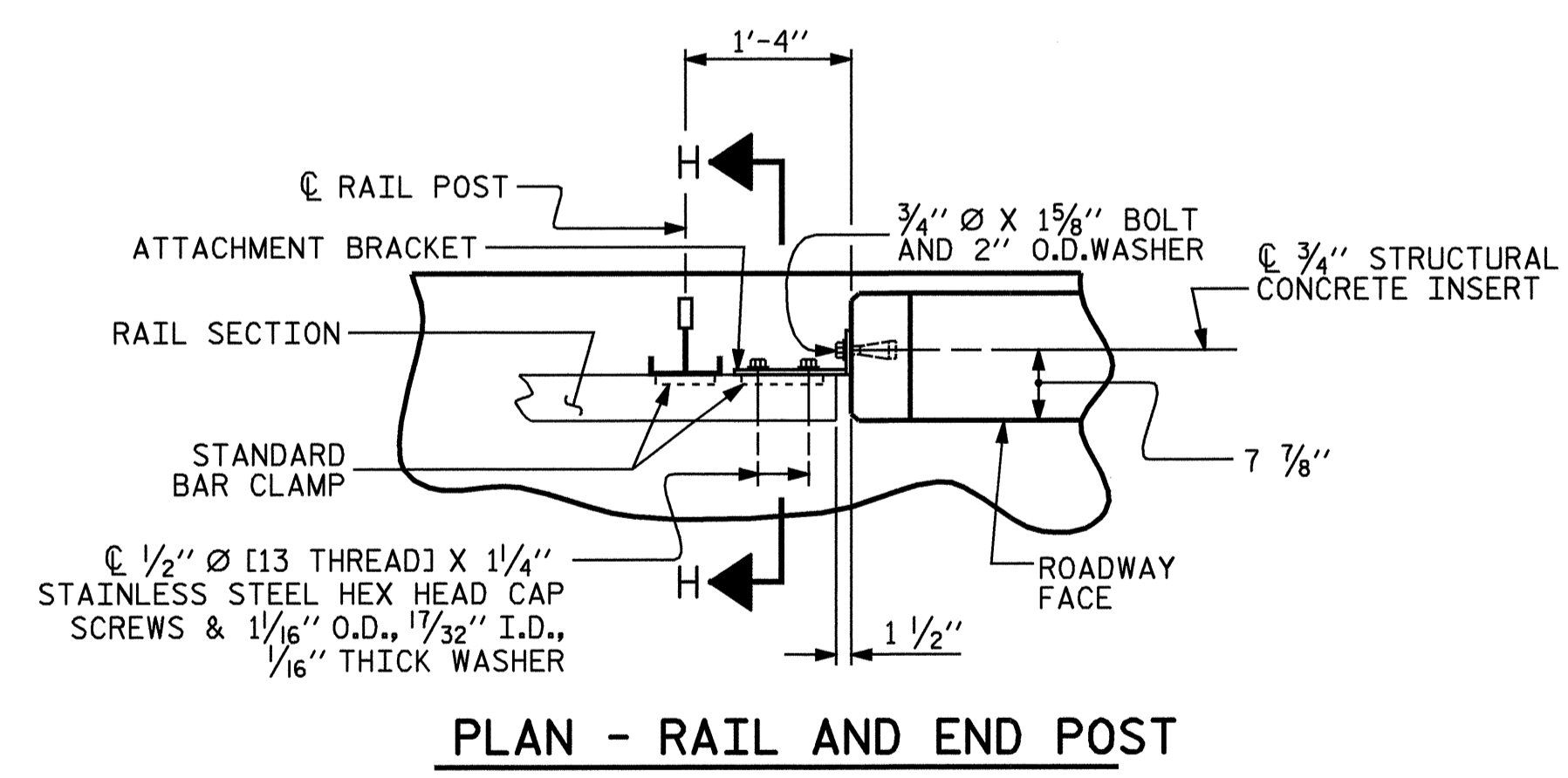
TOP VIEW



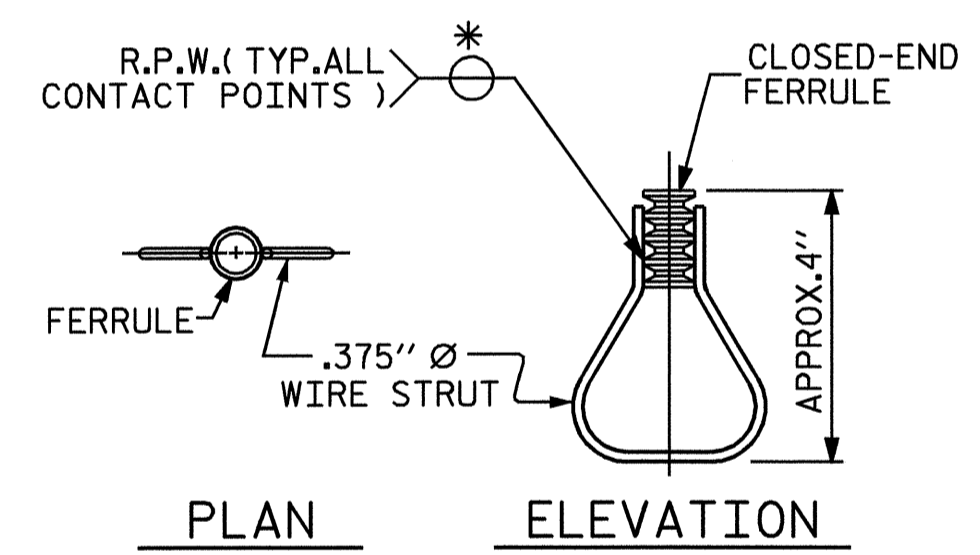
END VIEW (FIX AND EXP.)



SECTION H-H (FIX)



PLAN - RAIL AND END POST



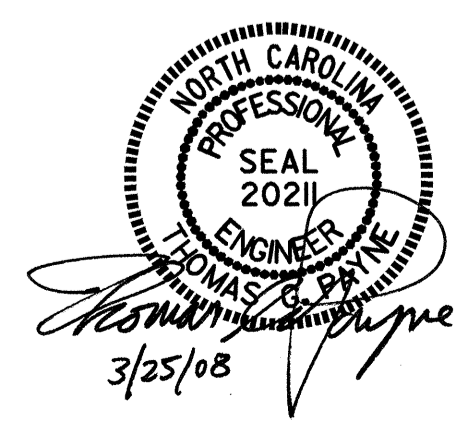
**PLAN ELEVATION
STRUCTURAL CONCRETE INSERT**

* EACH WELDED ATTACHMENT OF WIRE TO FERRULE SHALL DEVELOP THE TENSILE STRENGTH OF THE WIRE.

PROJECT NO. B-4252
ROCKINGHAM COUNTY
 STATION: 27+69.00 -L-

SHEET 4 OF 5

STATE OF NORTH CAROLINA
 DEPARTMENT OF TRANSPORTATION
 RALEIGH
 STANDARD
 RAIL POST SPACINGS
 AND
 END OF RAIL DETAILS
 FOR TWO BAR METAL RAILS



REVISIONS						SHEET NO.
NO.	BY:	DATE:	NO.	BY:	DATE:	S-20
1			3			TOTAL SHEETS 70
2			4			

ASSEMBLED BY : S. DOMBROWSKI	DATE : 7/07
CHECKED BY : M.K. BEARD	DATE : 10/07
DRAWN BY : FCJ 1/88	REV. 10/17/00 LES/RDR
CHECKED BY : CRK 3/89	REV. 5/7/03 RWW/JTE
	REV. 5/1/06 TLA/GM

DETAILS FOR ATTACHING METAL RAIL TO END POST

NOTES
 STRUCTURAL CONCRETE INSERT

THE STRUCTURAL CONCRETE INSERT ASSEMBLY SHALL CONSIST OF THE FOLLOWING COMPONENTS:

- FERRULES SHALL BE MADE FROM STEEL MEETING THE REQUIREMENTS OF AASHTO M169, GRADE 12L14 AND SHALL HAVE A MINIMUM LENGTH OF THREADS OF 1/2".
- 1 - 3/4" Ø X 1 5/8" BOLT WITH WASHER. BOLT SHALL CONFORM TO THE REQUIREMENTS OF ASTM A307. BOLT AND WASHER SHALL BE GALVANIZED. (AT THE CONTRACTOR'S OPTION, STAINLESS STEEL BOLT AND WASHER MAY BE USED AS AN ALTERNATE FOR THE 3/4" Ø X 1 5/8" GALVANIZED BOLT AND WASHER. THEY SHALL CONFORM TO OR EXCEED THE MECHANICAL REQUIREMENTS OF ASTM A307. THE USE OF THIS ALTERNATE SHALL BE APPROVED BY THE ENGINEER.)
- WIRE STRUT SHOWN IN THE CONCRETE INSERT ASSEMBLY DETAIL IS THE MINIMUM ALLOWABLE SIZE AND SHALL HAVE A MINIMUM TENSILE STRENGTH OF 100,000 PSI. AS AN OPTION, A 7/16" Ø WIRE STRUT WITH A MINIMUM TENSILE STRENGTH OF 90,000 PSI IS ACCEPTABLE.

NOTES
 METAL RAIL TO END POST CONNECTION

THE METAL RAIL TO END POST CONNECTION SHALL CONSIST OF THE FOLLOWING COMPONENTS:

- 1/2" PLATES SHALL CONFORM TO AASHTO M270 GRADE 36 AND SHALL BE GALVANIZED AFTER FABRICATION.
- 3/4" STRUCTURAL CONCRETE INSERT SHALL HAVE A WORKING LOAD SHEAR CAPACITY OF 4800 LBS. THE FERRULES SHALL ENGAGE A 3/4" Ø X 1 5/8" BOLT WITH 2" O.D. WASHER IN PLACE. THE 3/4" Ø X 1 5/8" BOLT SHALL HAVE N. C. THREADS.
- CAP SCREWS FOR RAIL ATTACHMENT TO ANGLE SHALL CONFORM TO THE REQUIREMENTS OF ASTM F593 ALLOY 305 STAINLESS STEEL. CAP SCREWS TO BE CENTERED IN SLOTS AT 60°F.
- STANDARD CLAMP BARS (SEE METAL RAIL SHEET).
- 1/2" Ø PIPE SLEEVES (IF REQUIRED) TO BE GALVANIZED.

THE COST OF THE STANDARD CLAMP BARS AND CAP SCREWS USED IN THE METAL RAIL TO END POST CONNECTION SHALL BE INCLUDED IN THE UNIT CONTRACT PRICE BID FOR LINEAR FEET OF 1 OR 2 BAR METAL RAILS.

THE 3/4" STRUCTURAL CONCRETE INSERT WITH BOLT SHALL BE ASSEMBLED IN THE SHOP.

THE COST OF THE 3/4" STRUCTURAL CONCRETE INSERT ASSEMBLY, AND THE 1/2" PLATES COMPLETE IN PLACE SHALL BE INCLUDED IN THE VARIOUS PAY ITEMS.

THE CONTRACTOR, AT HIS OPTION, MAY USE AN ADHESIVE BONDING SYSTEM IN LIEU OF THE STRUCTURAL CONCRETE INSERT EMBEDDED IN THE END POST. IF THE ADHESIVE BONDING SYSTEM IS USED, THE 3/4" Ø X 1 5/8" BOLT WITH WASHER SHALL BE REPLACED WITH A 3/4" Ø X 6 1/2" BOLT AND 2" O.D. WASHER. ALL SPECIFICATIONS THAT APPLY TO THE 3/4" Ø X 1 5/8" BOLT SHALL APPLY TO THE 3/4" Ø X 6 1/2" BOLT. FIELD TESTING OF THE ADHESIVE BONDING SYSTEM IS NOT REQUIRED.

NOTES

THE GUARDRAIL ANCHOR ASSEMBLY SHALL CONSIST OF A 1/4" HOLD DOWN PLATE AND 7 - 1/8" Ø BOLTS WITH NUTS AND WASHERS.

THE HOLD-DOWN PLATE SHALL CONFORM TO AASHTO M270 GRADE 36. AFTER FABRICATION, THE HOLD-DOWN PLATE SHALL BE HOT-DIP GALVANIZED IN ACCORDANCE WITH AASHTO M111.

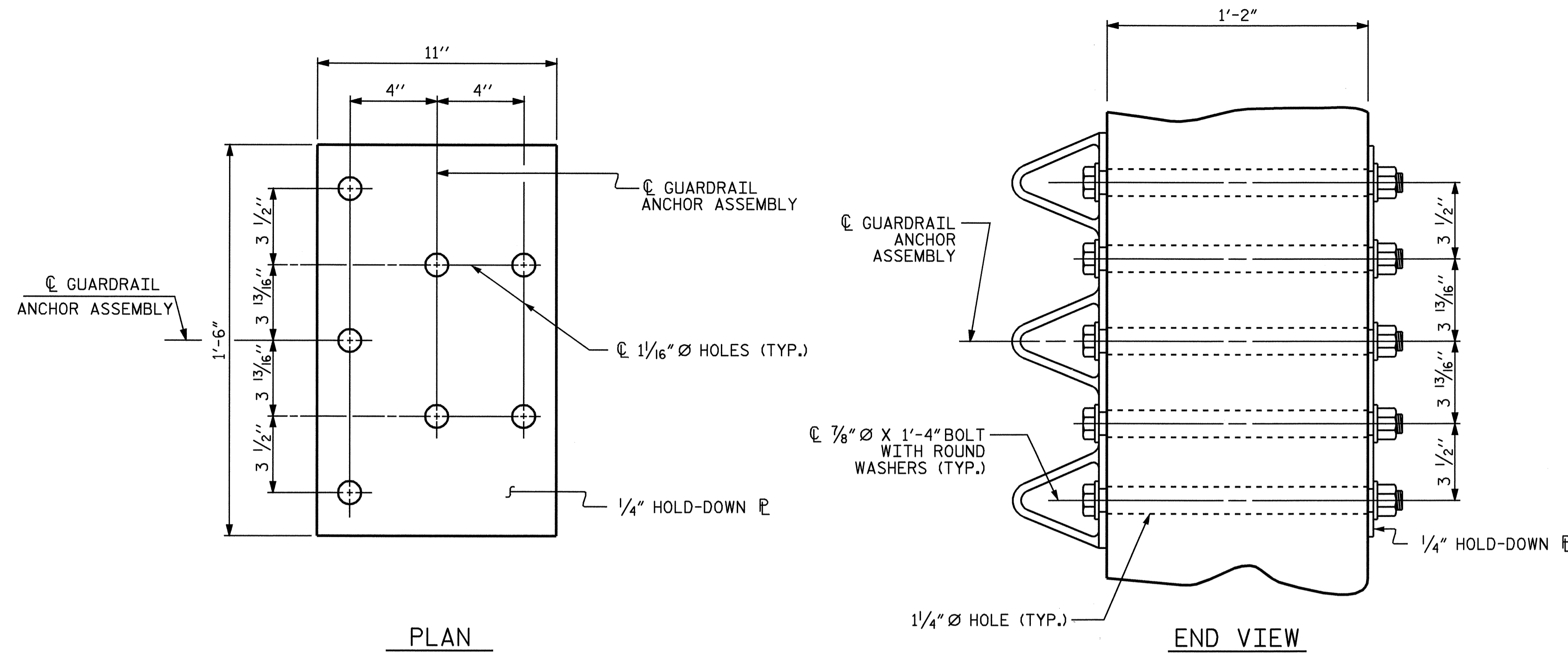
BOLTS SHALL CONFORM TO THE REQUIREMENTS OF ASTM A307 AND NUTS SHALL CONFORM TO THE REQUIREMENTS OF AASHTO M291. BOLTS, NUTS AND WASHERS SHALL BE GALVANIZED. AT THE CONTRACTOR'S OPTION, STAINLESS STEEL BOLTS, NUTS AND WASHERS MAY BE USED AS AN ALTERNATE FOR THE 1/8" Ø GALVANIZED BOLTS, NUTS AND WASHERS. THEY SHALL CONFORM TO OR EXCEED THE MECHANICAL REQUIREMENTS OF ASTM A307. THE USE OF THIS ALTERNATE SHALL BE APPROVED BY THE ENGINEER.

AFTER INSTALLATION, THE EXPOSED THREAD OF THE BOLT SHALL BE BURRED WITH A SHARP POINTED TOOL.

THE COST OF THE GUARDRAIL ANCHOR ASSEMBLIES WITH BOLTS, NUTS AND WASHERS COMPLETE IN PLACE, SHALL BE INCLUDED IN THE VARIOUS PAY ITEMS.

THE VERTICAL REINFORCING BARS MAY BE SHIFTED SLIGHTLY IN THE END POST TO CLEAR ASSEMBLY BOLTS.

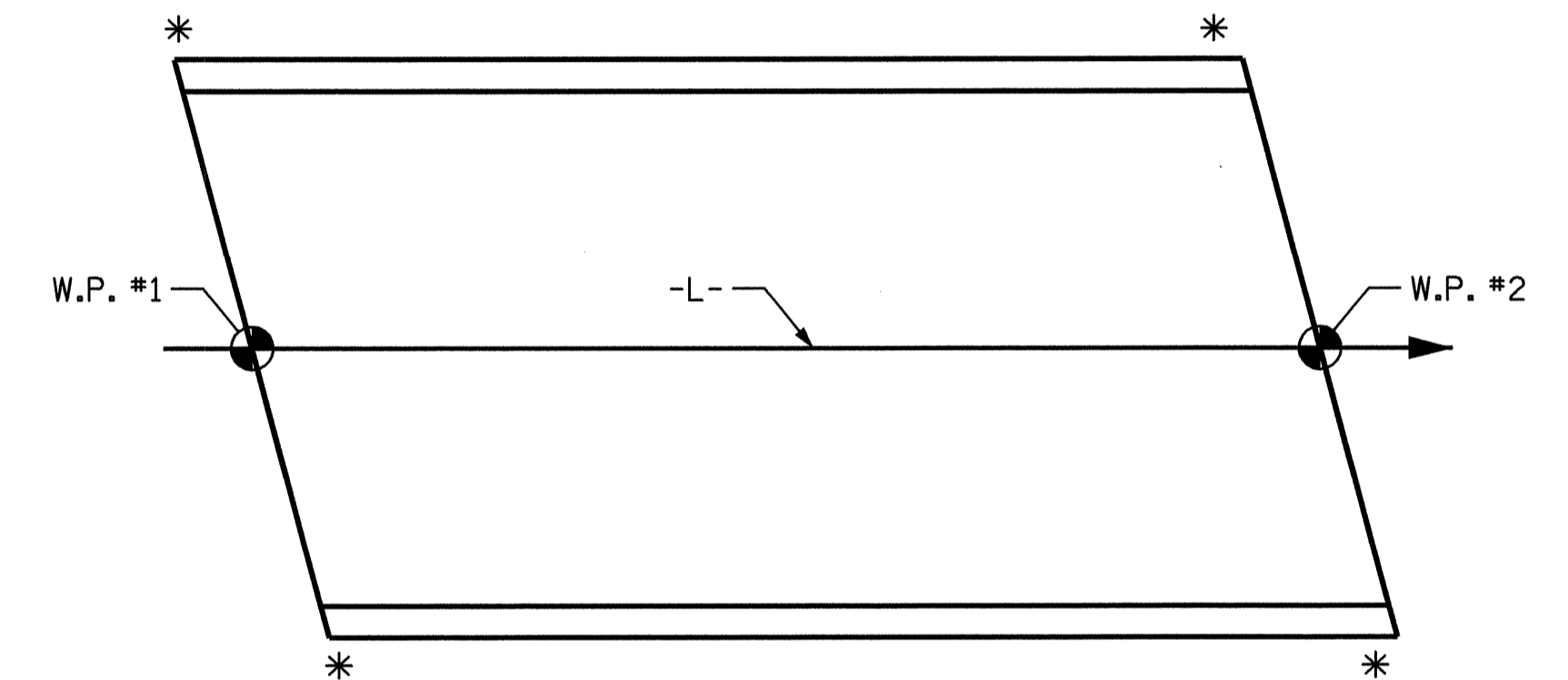
THE 1 1/4" Ø HOLES SHALL BE FORMED OR DRILLED WITH A CORE BIT. IMPACT TOOLS WILL NOT BE PERMITTED. ANY CONCRETE DAMAGED BY THIS WORK SHALL BE REPAIRED TO THE SATISFACTION OF THE ENGINEER.



PLAN

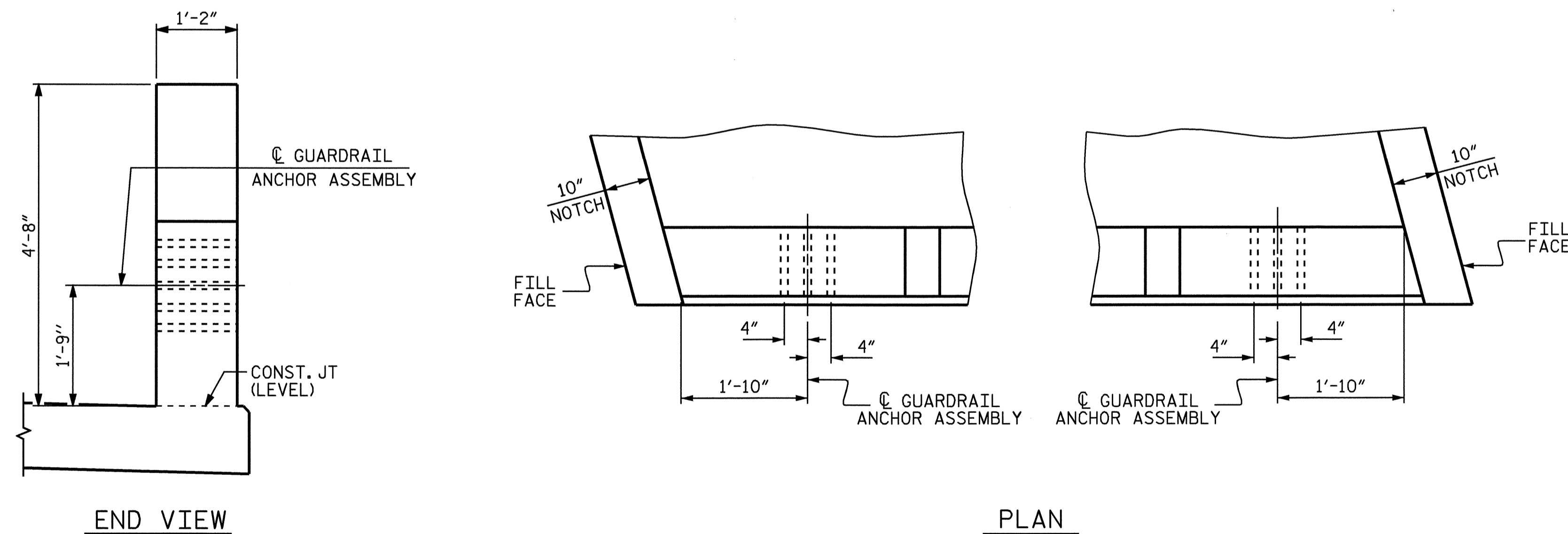
END VIEW

GUARDRAIL ANCHOR ASSEMBLY DETAILS



SKETCH SHOWING POINTS OF ATTACHMENT

* LOCATION OF GUARDRAIL ATTACHMENT



END VIEW

PLAN

LOCATION OF GUARDRAIL ANCHOR AT END POST

PROJECT NO. B-4252
ROCKINGHAM COUNTY
 STATION: 27+69.00 -L-

SHEET 5 OF 5

STATE OF NORTH CAROLINA
 DEPARTMENT OF TRANSPORTATION
 RALEIGH
 STANDARD
 GUARDRAIL ANCHORAGE
 DETAILS
 FOR METAL RAILS



REVISIONS						SHEET NO.	
NO.	BY:	DATE:	NO.	BY:	DATE:	S-21	
1			3			TOTAL SHEETS	
2			4			70	

ASSEMBLED BY : S. DOMBROWSKI	DATE : 7/07
CHECKED BY : M.K. BEARD	DATE : 10/07
DRAWN BY : EEM 6/94	REV. 10/17/00 RWW/LES
CHECKED BY : RGW 6/94	REV. 5/7/03 RWW/JTE
	REV. 5/1/06 TLA/GM

SUPERSTRUCTURE REINFORCING STEEL LENGTHS ARE BASED ON THE FOLLOWING MINIMUM SPLICE LENGTHS

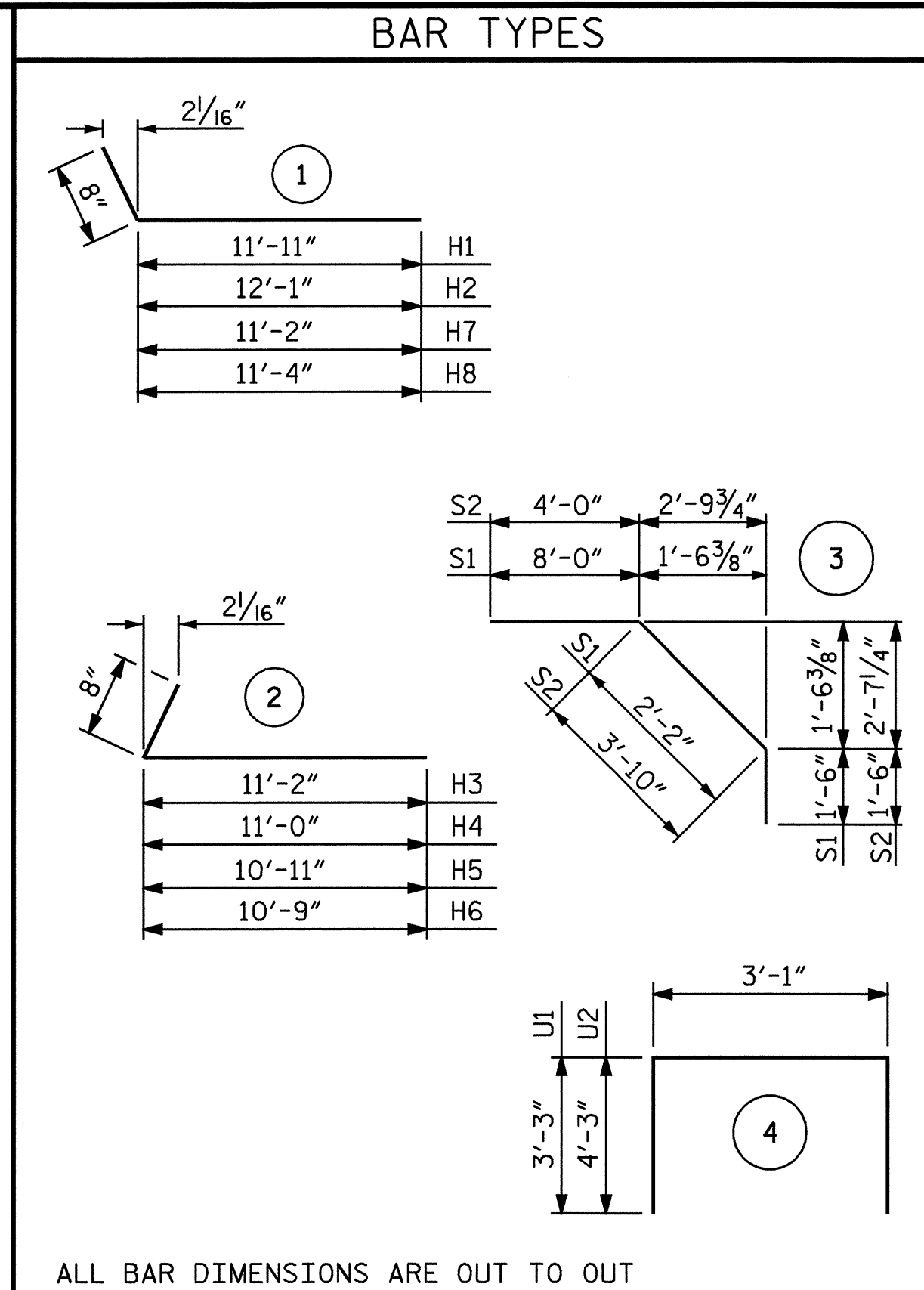
BAR SIZE	SUPERSTRUCTURE EXCEPT APPROACH SLABS, PARAPET, AND BARRIER RAIL		APPROACH SLABS		PARAPET AND BARRIER RAIL
	EPOXY COATED	UNCOATED	EPOXY COATED	UNCOATED	
#4	2'-0"	1'-9"	2'-0"	1'-9"	2'-9"
#5	2'-6"	2'-2"	2'-6"	2'-2"	3'-5"
#6	3'-0"	2'-7"	3'-10"	2'-7"	4'-4"
#7	5'-3"	3'-6"			
#8	6'-10"	4'-7"			

REINFORCING STEEL QUANTITIES			
	STAGE #1	STAGE #2	TOTAL
REINFORCING STEEL	10,849 LBS.	6,311 LBS.	17,160 LBS.

EPOXY COATED REINFORCING STEEL QUANTITIES			
	STAGE #1	STAGE #2	TOTAL
EPOXY COATED REINFORCING STEEL	10,665 LBS.	6,701 LBS.	17,366 LBS.
PARAPETS & END POSTS			3,519 LBS.
TOTAL			20,885 LBS.

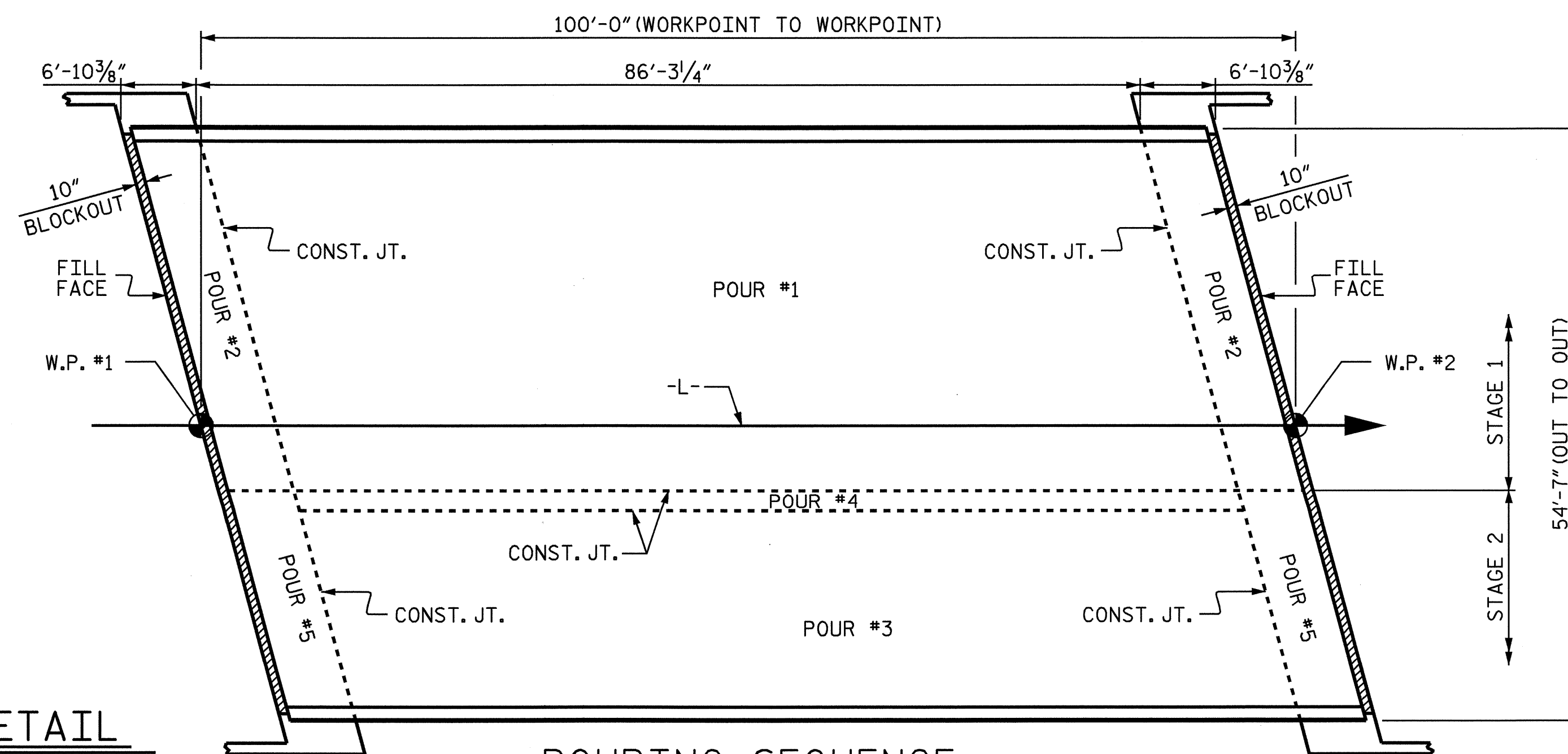
POUR SEQUENCE BREAKDOWN						
	POUR #1	POUR #2	POUR #3	POUR #4	POUR #5	TOTAL
STAGE 1	81.6	53.1				134.7
STAGE 2			47.1	5.0	31.9	84.0
TOTAL CONCRETE (CU.YD.)						218.7

NOTE: POUR #2 AND POUR #5 INCLUDE THE UPPER PORTION OF THE END BENT CAPS AND WINGS.

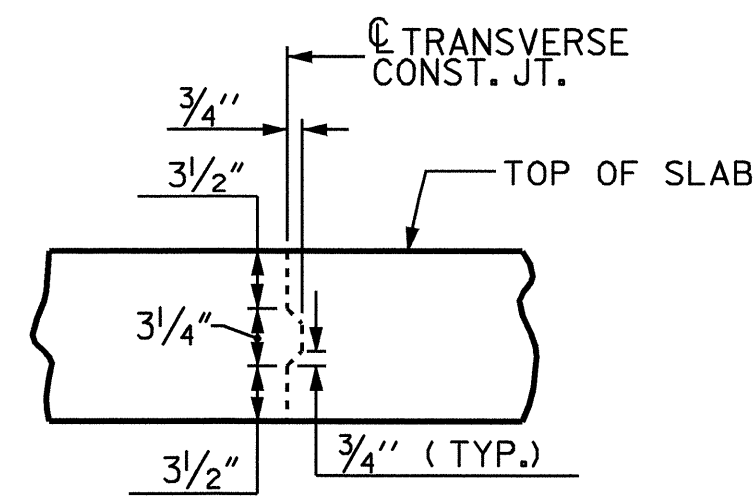


ALL BAR DIMENSIONS ARE OUT TO OUT

BILL OF MATERIAL STAGE 1						BILL OF MATERIAL STAGE 2					
BAR	NO.	SIZE	TYPE	LENGTH	WEIGHT	BAR	NO.	SIZE	TYPE	LENGTH	WEIGHT
* A1	143	#5	STR	32'-10"	4897	* A3	149	#5	STR	18'-10"	2927
A2	143	#5	STR	32'-10"	4897	A4	149	#5	STR	18'-10"	2927
* A101	4	#5	STR	28'-10"	120						
* A102	4	#5	STR	24'-2"	101						
* A103	4	#5	STR	19'-6"	81	* A111	4	#5	STR	14'-10"	62
* A104	4	#5	STR	14'-6"	60	* A112	4	#5	STR	10'-2"	42
* A105	4	#5	STR	9'-10"	41	* A113	4	#5	STR	5'-6"	23
* A106	4	#5	STR	5'-2"	22						
A201	4	#5	STR	28'-10"	120						
A202	4	#5	STR	24'-2"	101						
A203	4	#5	STR	19'-6"	81	A211	4	#5	STR	14'-10"	62
A204	4	#5	STR	14'-6"	60	A212	4	#5	STR	10'-2"	42
A205	4	#5	STR	9'-10"	41	A213	4	#5	STR	5'-6"	23
A206	4	#5	STR	5'-2"	22						
* B1	90	#6	STR	23'-0"	3109	* B1	58	#6	STR	23'-0"	2004
* B2	48	#4	STR	29'-0"	930	* B2	32	#4	STR	29'-0"	620
B3	74	#6	STR	23'-0"	2556	B3	42	#6	STR	23'-0"	1451
B4	37	#5	STR	56'-4"	2174	B4	21	#5	STR	56'-4"	1234
* D1	157	#5	STR	3'-4"	546	* D1	157	#5	STR	3'-4"	546
H1	5	#4	1	12'-7"	42	H3	5	#4	2	11'-10"	40
H2	5	#4	1	12'-9"	43	H4	5	#4	2	11'-8"	39
H5	5	#4	2	11'-7"	39	H7	5	#4	1	11'-10"	40
H6	5	#4	2	11'-5"	38	H8	5	#4	1	12'-0"	40
K1	16	#4	STR	21'-2"	226	K2	8	#4	STR	24'-8"	132
K3	8	#4	STR	3'-2"	17	K3	8	#4	STR	3'-2"	17
* S1	54	#4	3	11'-8"	421	* S1	34	#4	3	11'-8"	265
* S2	54	#4	3	9'-4"	337	* S2	34	#4	3	9'-4"	212
U1	54	#4	4	9'-7"	346	U1	34	#4	4	9'-7"	218
U2	6	#4	4	11'-7"	46	U2	6	#4	4	11'-7"	46
REINFORCING STEEL	= 10849 LBS.					REINFORCING STEEL	= 6311 LBS.				
* EPOXY COATED REINFORCING STEEL	= 10665 LBS.					* EPOXY COATED REINFORCING STEEL	= 6701 LBS.				



POURING SEQUENCE AND LAYOUT FOR COMPUTING AREA OF REINFORCED CONCRETE DECK SLAB (SQ. FT. = 5364)



TRANSVERSE CONSTRUCTION JOINT DETAIL

NOTE: REINFORCING STEEL IN SLAB NOT SHOWN. LONGITUDINAL REINFORCING STEEL SHALL BE CONTINUOUS THRU JOINT.

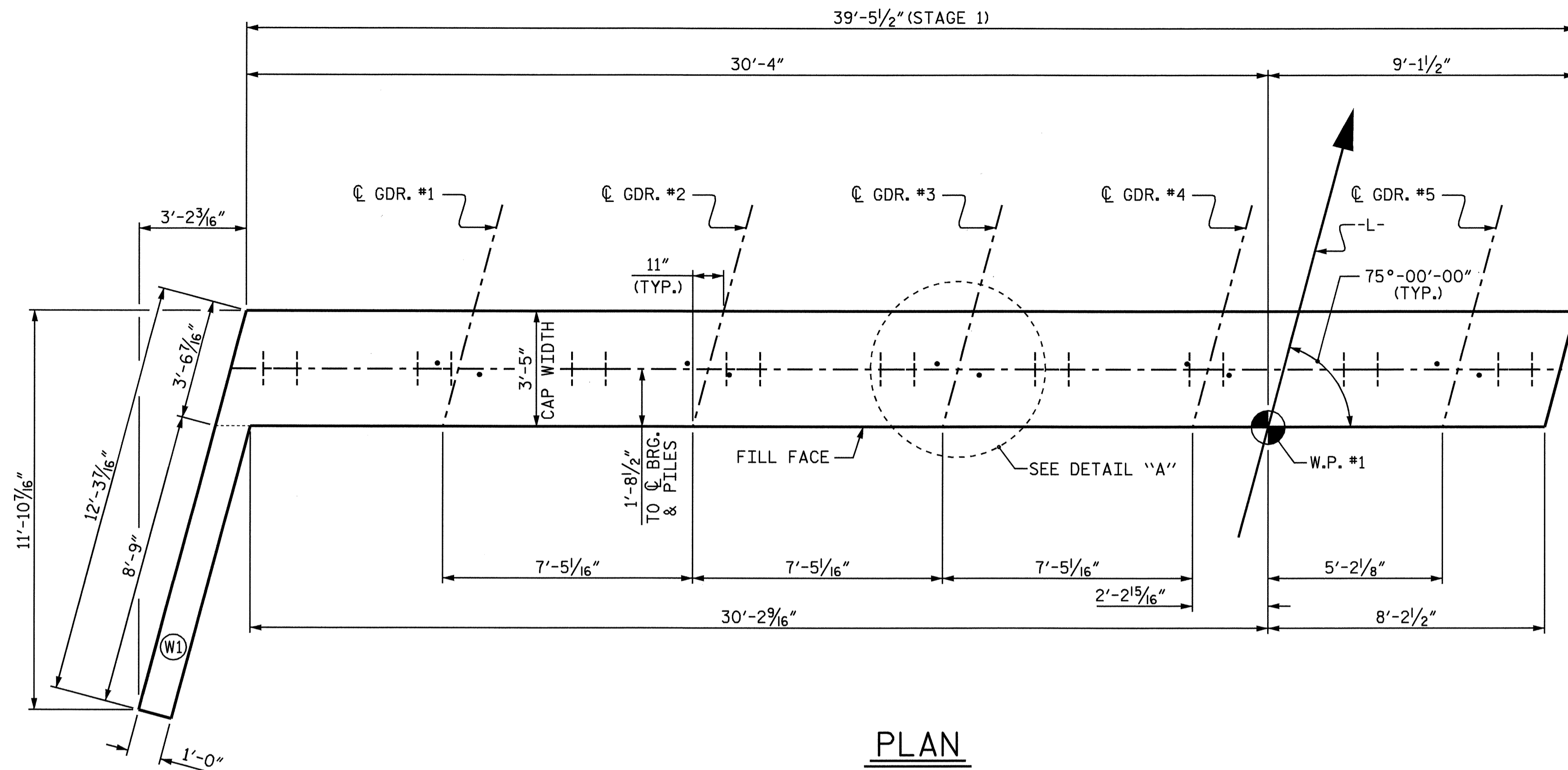
GROOVING BRIDGE FLOORS	
BRIDGE DECK	4799 SQ.FT.
APPROACH SLAB	2235 SQ.FT.
TOTAL	7034 SQ.FT.

PROJECT NO. B-4252
ROCKINGHAM COUNTY
 STATION: 27+69.00 -L-

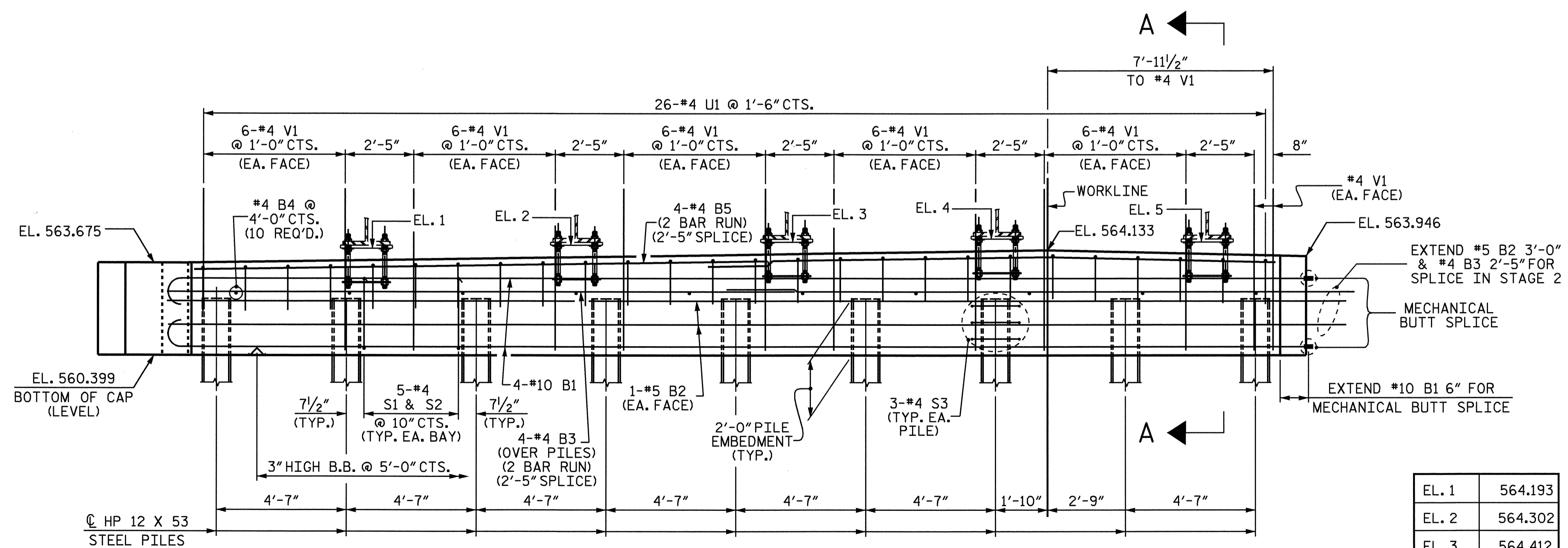
STATE OF NORTH CAROLINA DEPARTMENT OF TRANSPORTATION RALEIGH					
SUPERSTRUCTURE BILL OF MATERIAL					
REVISIONS					
NO.	BY:	DATE:	NO.	BY:	DATE:
1			3		
2			4		



DRAWN BY: S. DOMBROWSKI DATE: 3/07
 CHECKED BY: M.K. BEARD DATE: 10/07



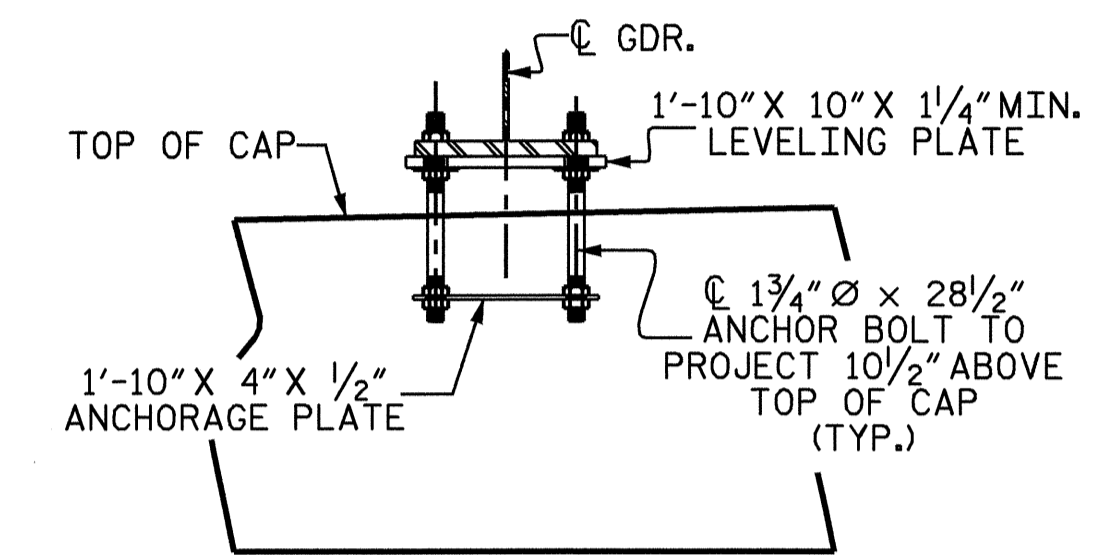
PLAN



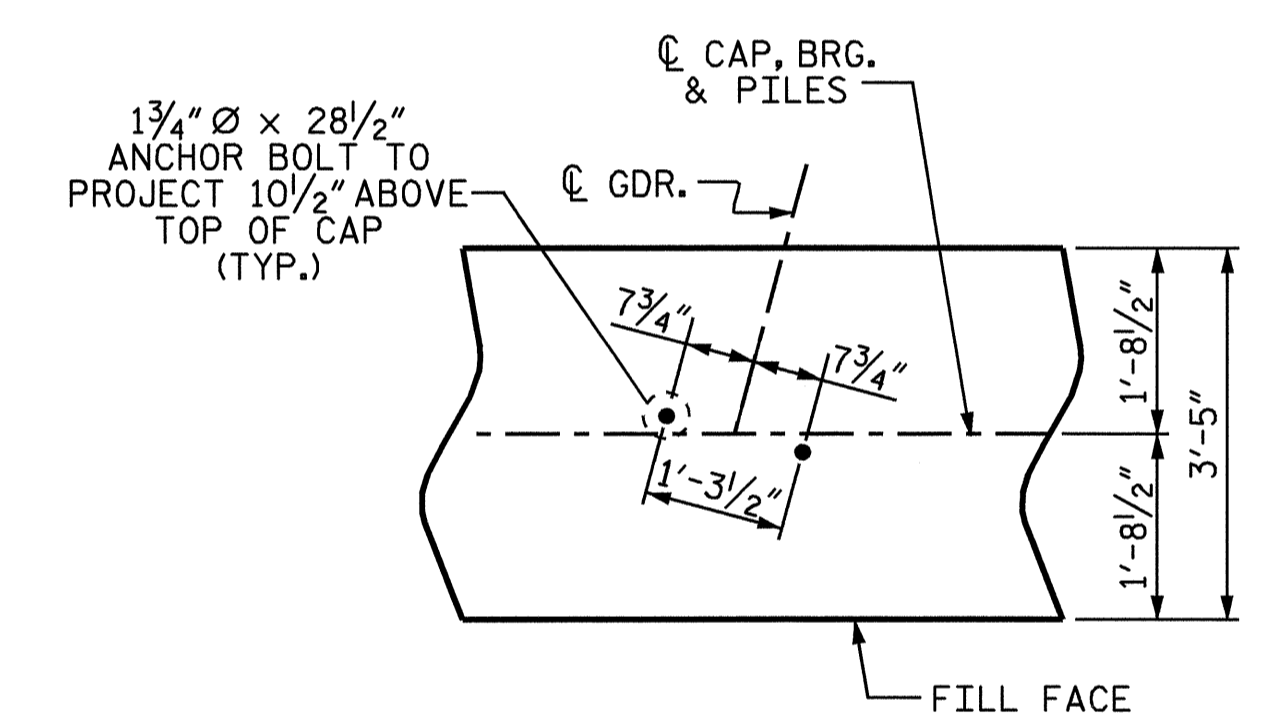
ELEVATION

NOTES

STIRRUPS IN CAP MAY BE SHIFTED AS NECESSARY TO CLEAR ANCHOR BOLTS.
 SEE SUPERSTRUCTURE SHEETS FOR UPPER PART OF INTEGRAL END BENT DETAILS.
 THE CONTRACTOR SHALL PROVIDE FOR INSTALLATION OF THE 4" DIAMETER DRAIN PIPE THROUGH THE WING WALL AS REQUIRED FOR REINFORCED BRIDGE APPROACH FILLS, SEE ROADWAY PLANS. REINFORCING STEEL IN THE WING WALL MAY BE SHIFTED AS NECESSARY TO CLEAR THE DRAIN PIPES.



ELEVATION VIEW



PLAN VIEW

DETAIL "A"

EL. 1	564.193
EL. 2	564.302
EL. 3	564.412
EL. 4	564.523
EL. 5	564.434

ELEVATIONS SHOWN ARE TO BOTTOM OF LEVELING PLATE

PROJECT NO. B-4252
 ROCKINGHAM COUNTY
 STATION: 27+69.00 -L-

SHEET 1 OF 4

STATE OF NORTH CAROLINA
 DEPARTMENT OF TRANSPORTATION
 RALEIGH

SUBSTRUCTURE
 INTEGRAL
 END BENT #1
 (STAGE 1)

REVISIONS						SHEET NO.	
NO.	BY:	DATE:	NO.	BY:	DATE:	S-23	
1			3			TOTAL SHEETS 70	
2			4				



DRAWN BY : S. DOMBROWSKI DATE : 1/07
 CHECKED BY : M.K. BEARD DATE : 2/08

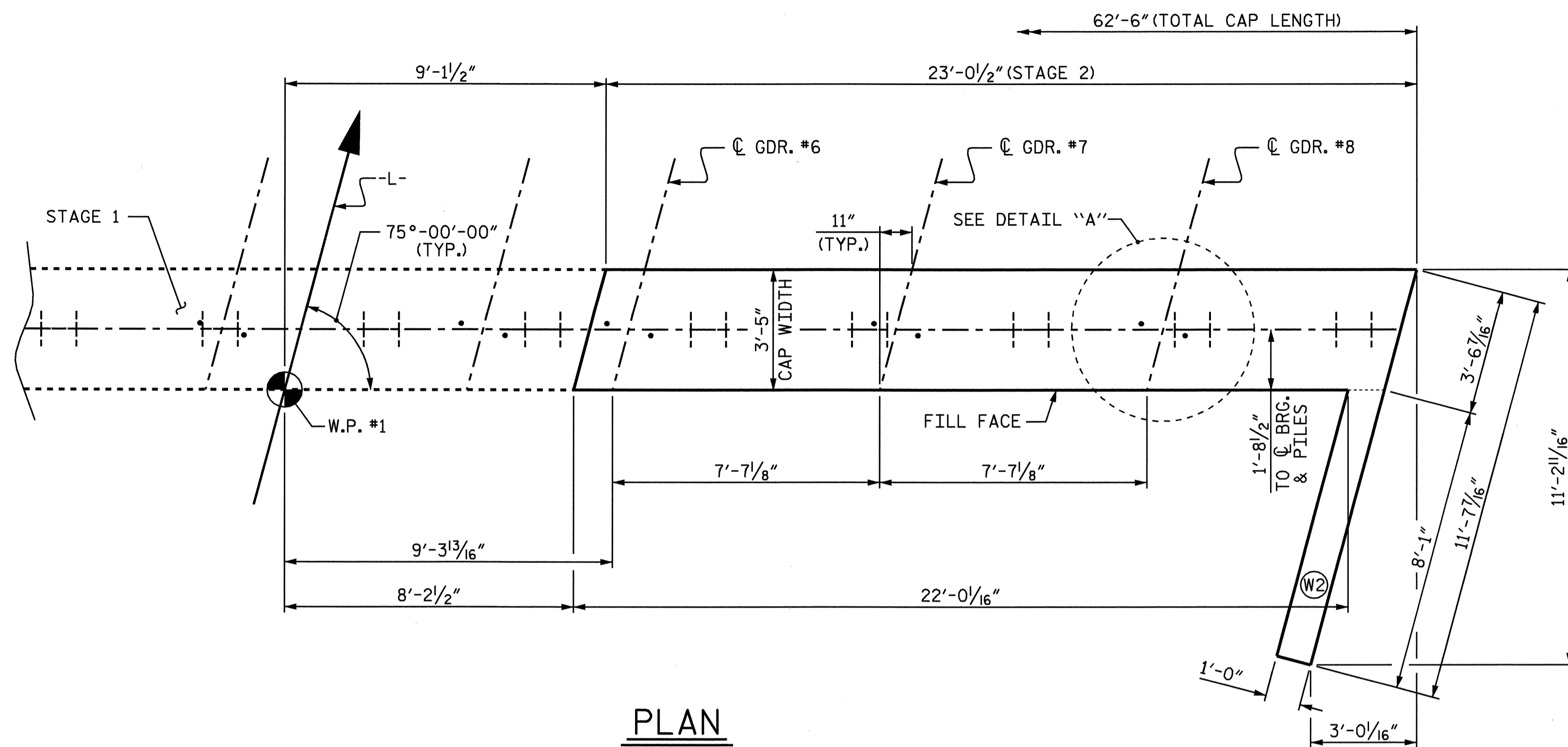
STR. #1

NOTES

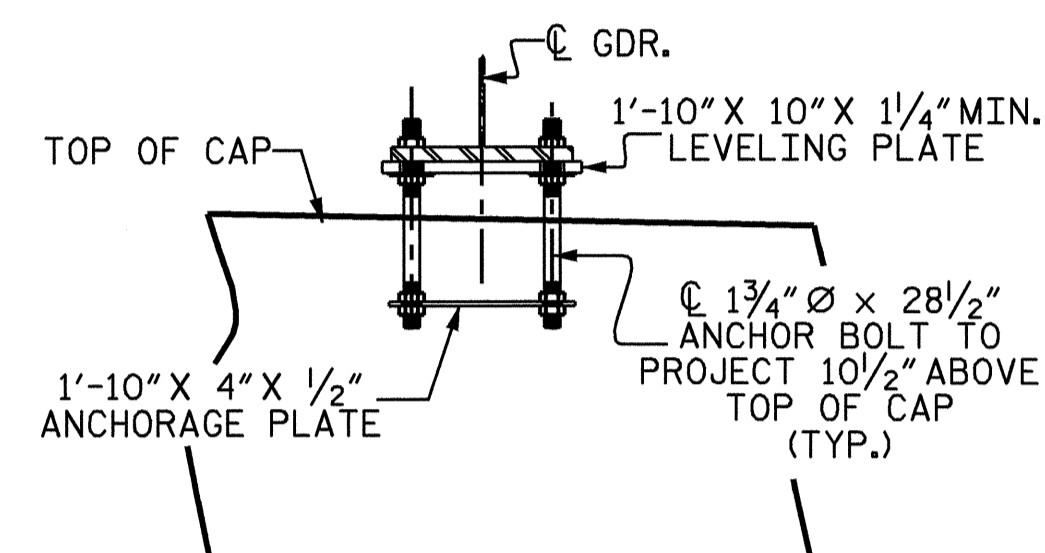
STIRRUPS IN CAP MAY BE SHIFTED AS NECESSARY TO CLEAR ANCHOR BOLTS.

SEE SUPERSTRUCTURE SHEETS FOR UPPER PART OF INTEGRAL END BENT DETAILS.

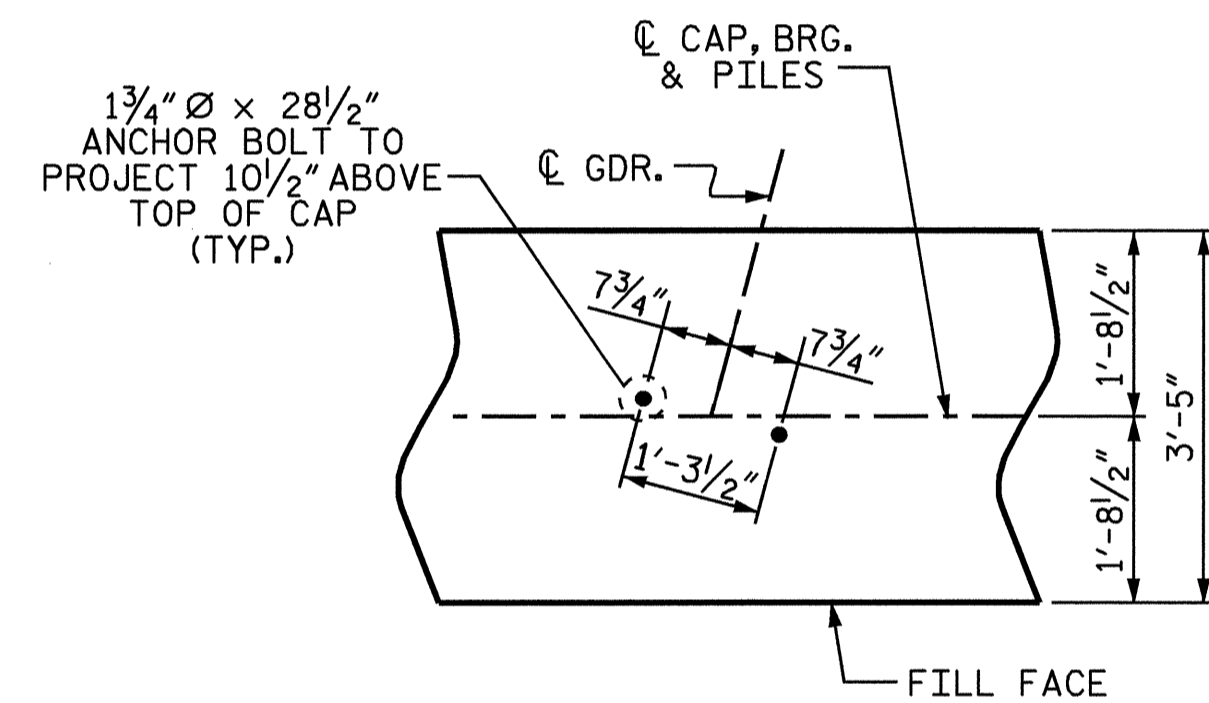
THE CONTRACTOR SHALL PROVIDE FOR INSTALLATION OF THE 4" DIAMETER DRAIN PIPE THROUGH THE WING WALL AS REQUIRED FOR REINFORCED BRIDGE APPROACH FILLS, SEE ROADWAY PLANS. REINFORCING STEEL IN THE WING WALL MAY BE SHIFTED AS NECESSARY TO CLEAR THE DRAIN PILES.



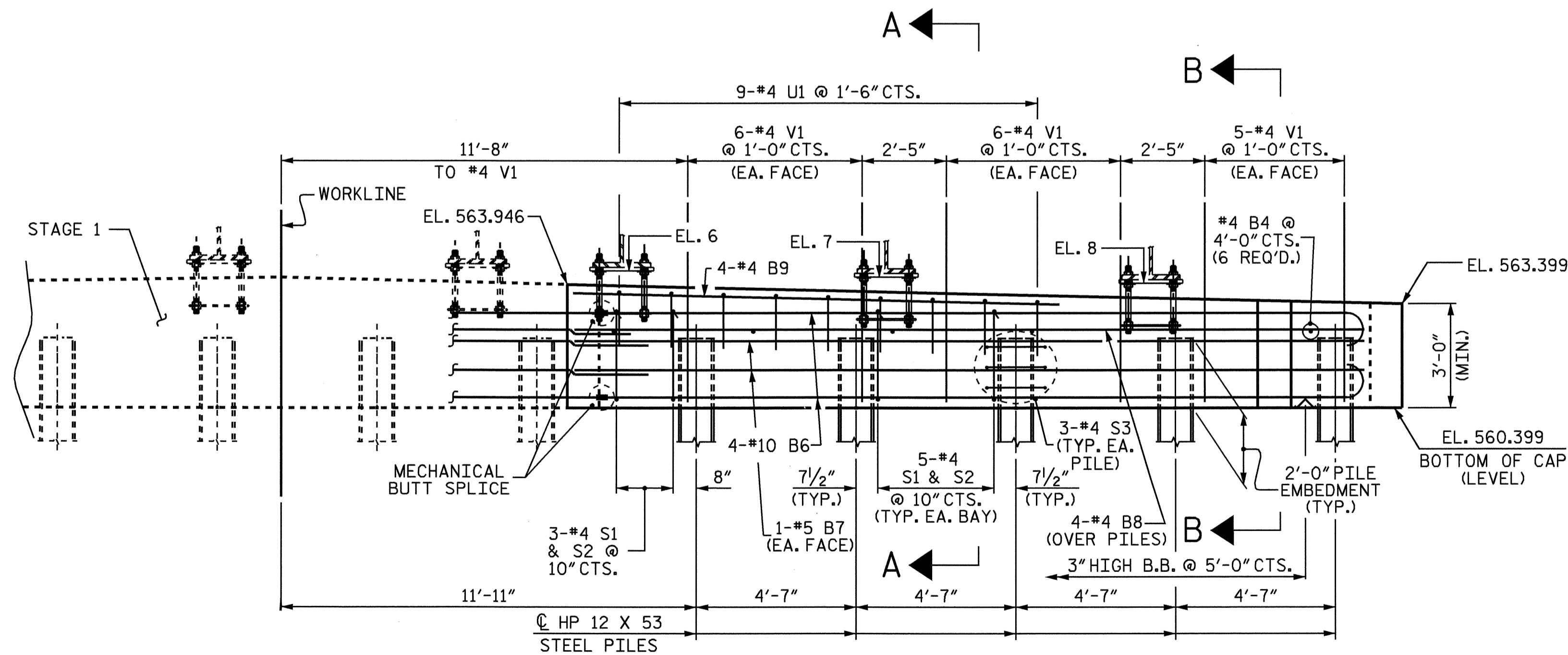
PLAN



ELEVATION VIEW



**PLAN VIEW
DETAIL "A"**



ELEVATION

EL. 6	564.336
EL. 7	564.156
EL. 8	563.977

ELEVATIONS SHOWN ARE TO BOTTOM OF LEVELING PLATE

PROJECT NO. B-4252
ROCKINGHAM COUNTY
 STATION: 27+69.00 -L-

SHEET 2 OF 4

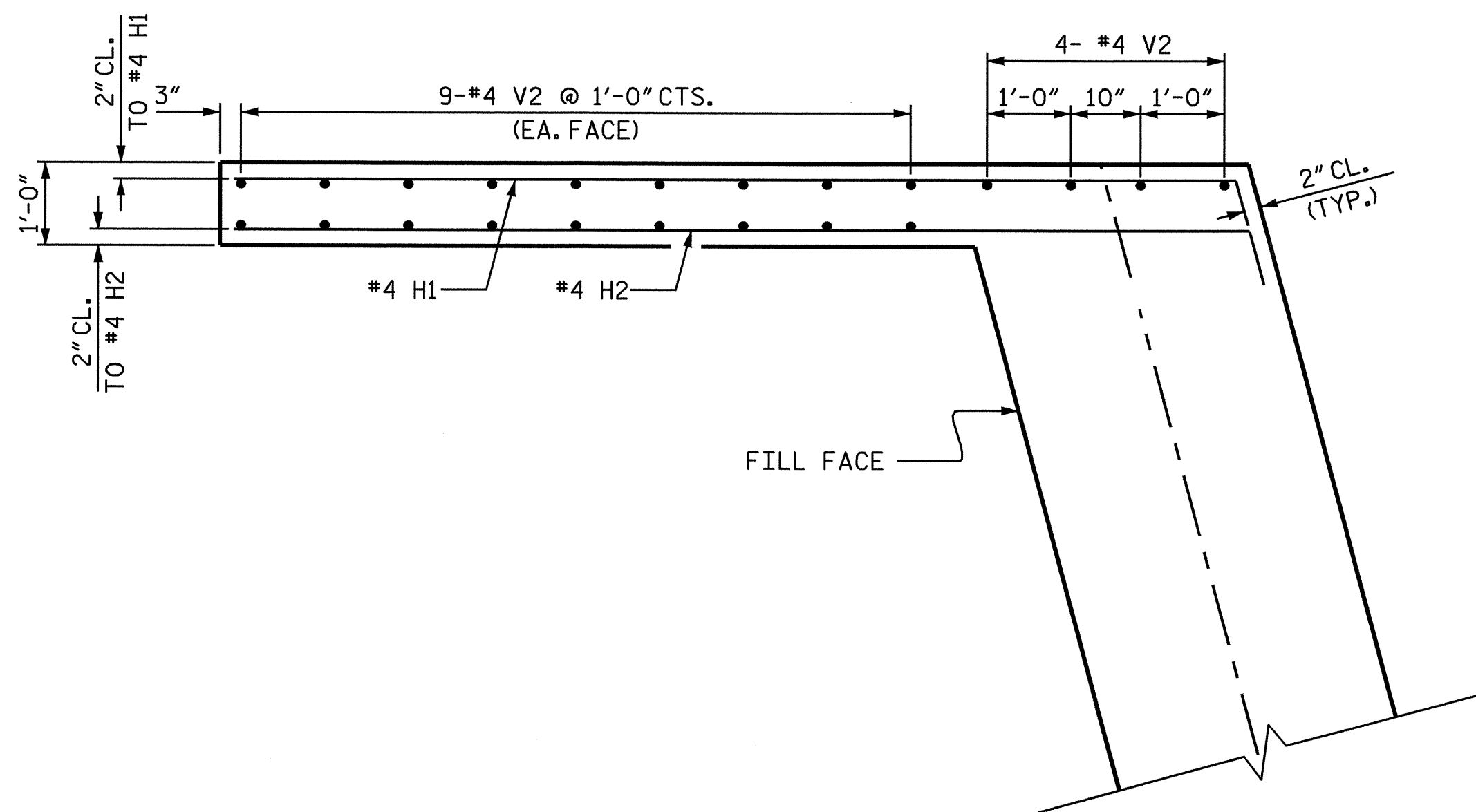
STATE OF NORTH CAROLINA
 DEPARTMENT OF TRANSPORTATION
 RALEIGH

**SUBSTRUCTURE
 INTEGRAL
 END BENT #1
 (STAGE 2)**

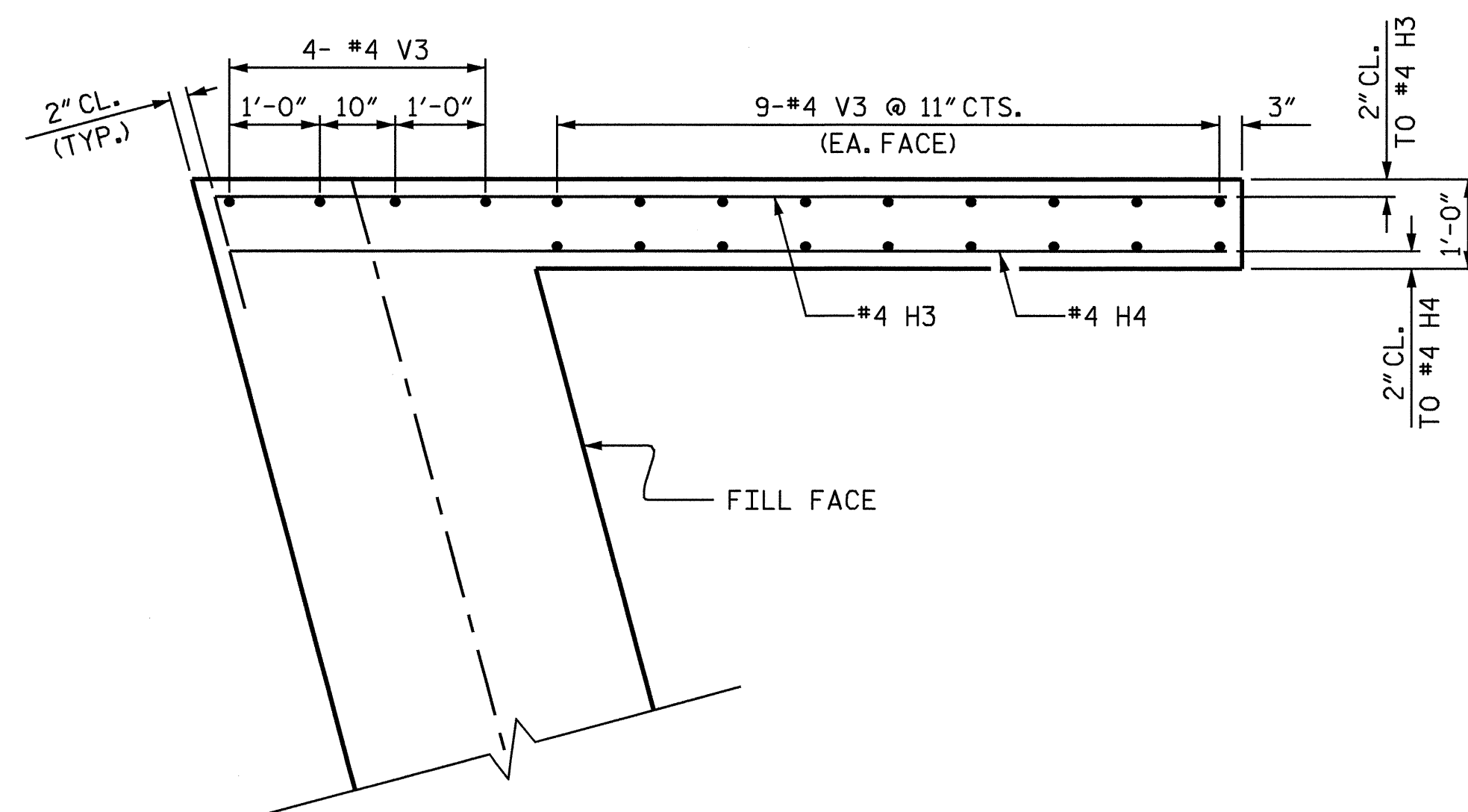


REVISIONS						SHEET NO. S-24
NO.	BY:	DATE:	NO.	BY:	DATE:	
1			3			TOTAL SHEETS 70
2			4			

DRAWN BY: S. DOMBROWSKI DATE: 1/07
 CHECKED BY: M.K. BEARD DATE: 2/08

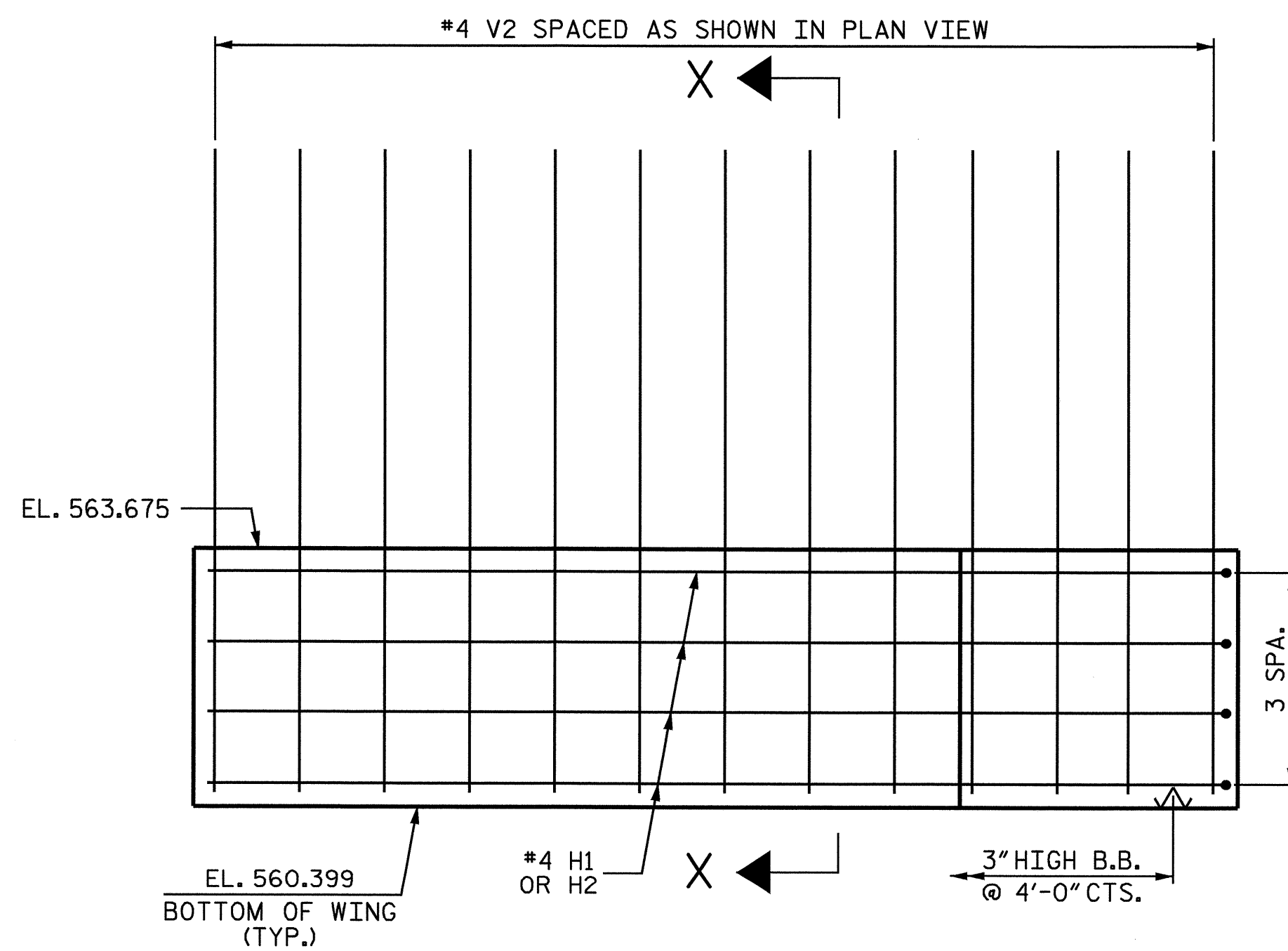


PLAN OF LEFT WING W1
(STAGE 1)

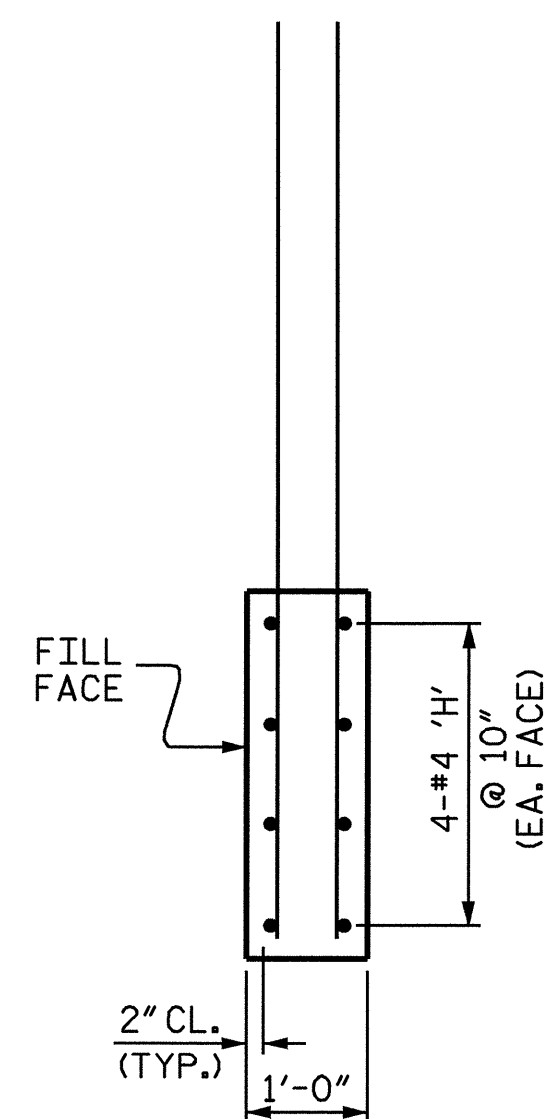


PLAN OF RIGHT WING W2
(STAGE 2)

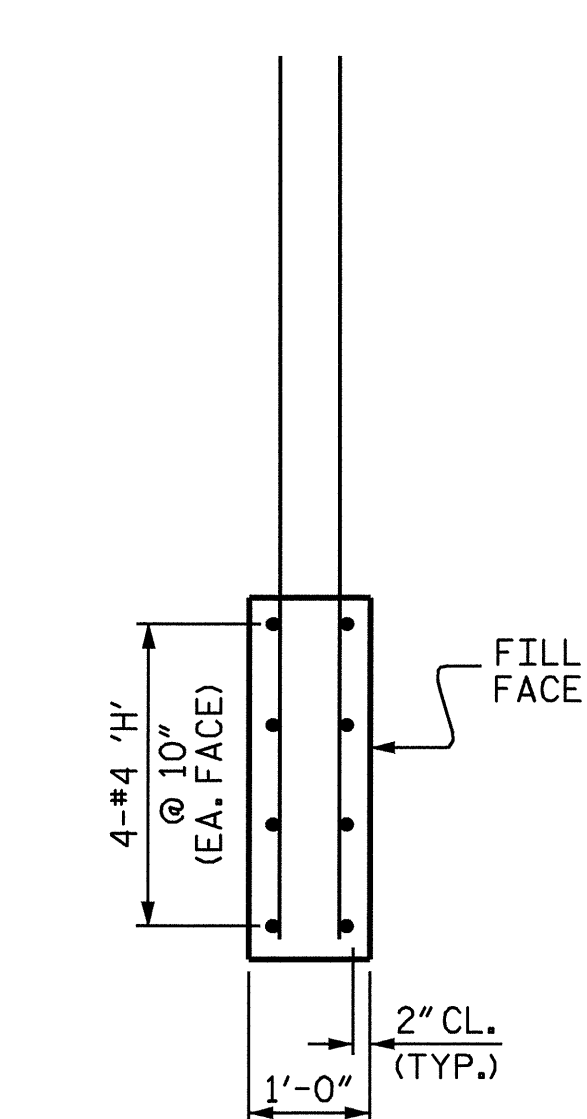
NOTE: THE UPPER PORTION OF THE WINGS SHALL BE POURED WITH THE SUPERSTRUCTURE. FOR DETAILS AND REINFORCING STEEL, SEE SUPERSTRUCTURE DETAILS.



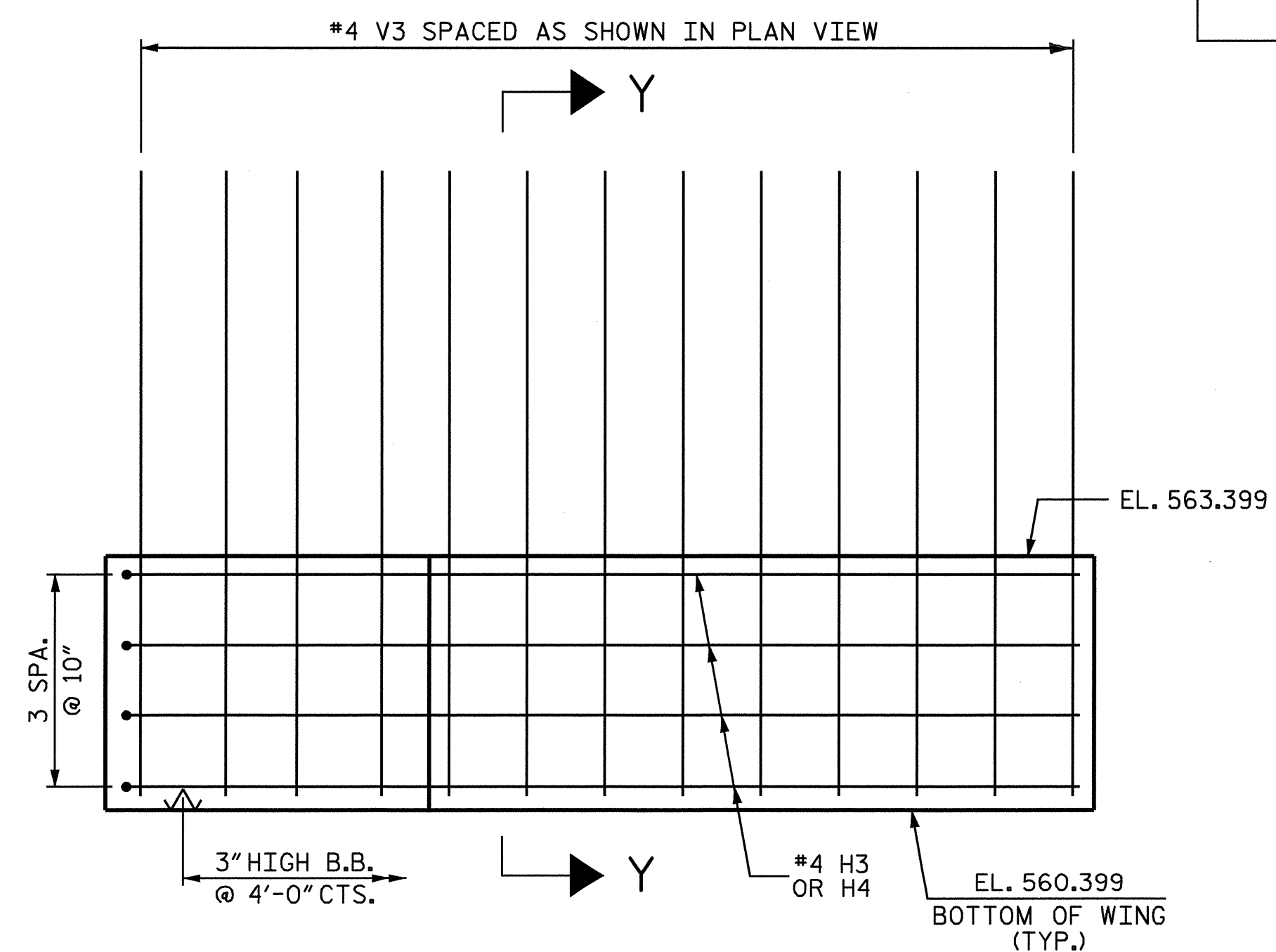
ELEVATION OF LEFT WING W1
(STAGE 1)



SECTION X-X



SECTION Y-Y



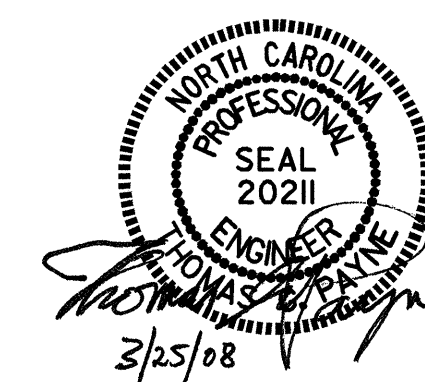
ELEVATION OF RIGHT WING W2
(STAGE 2)

PROJECT NO. B-4252
ROCKINGHAM COUNTY
STATION: 27+69.00 -L-

SHEET 3 OF 4

STATE OF NORTH CAROLINA
DEPARTMENT OF TRANSPORTATION
RALEIGH

SUBSTRUCTURE
INTEGRAL
END BENT #1

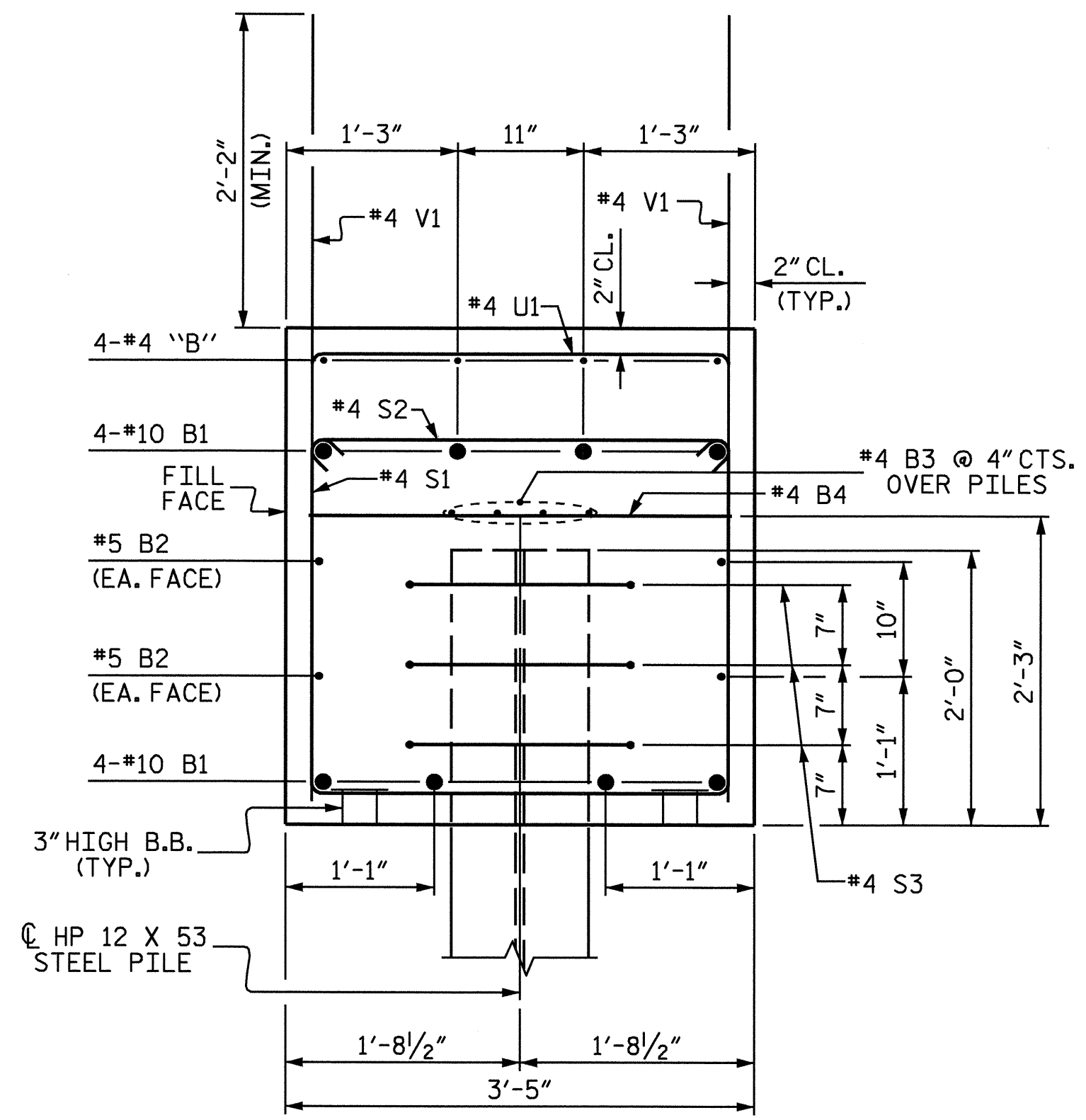


DRAWN BY : S. DOMBROWSKI DATE : 1/07
CHECKED BY : M.K. BEARD DATE : 2/08

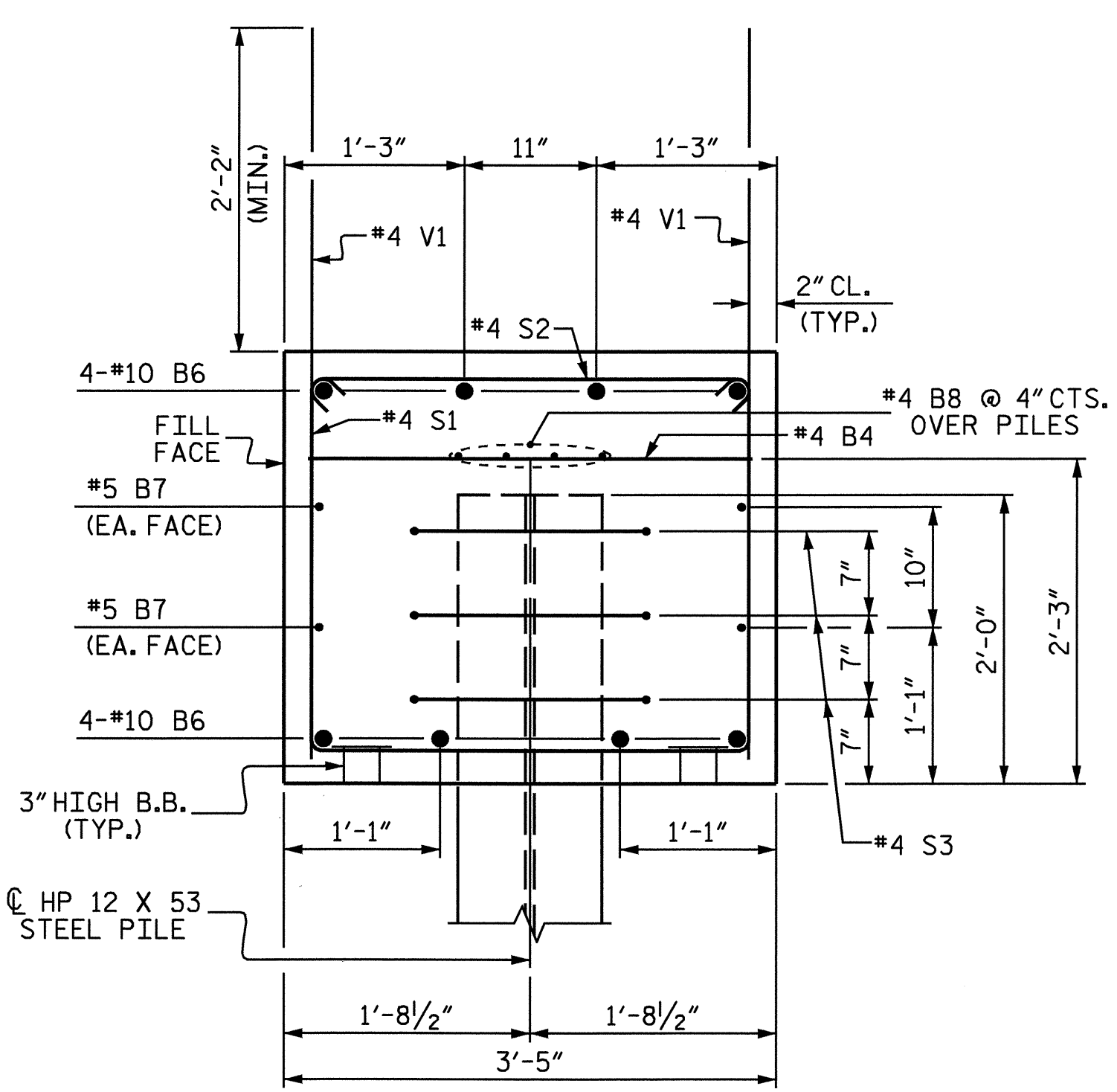
20-MAR-2008 15:32
P:\Structures\plans\str1B-4252.sd_Ebts_01.dgn
klayne

REVISIONS						SHEET NO.	
NO.	BY:	DATE:	NO.	BY:	DATE:	S-25	
1			3			TOTAL SHEETS	
2			4			70	

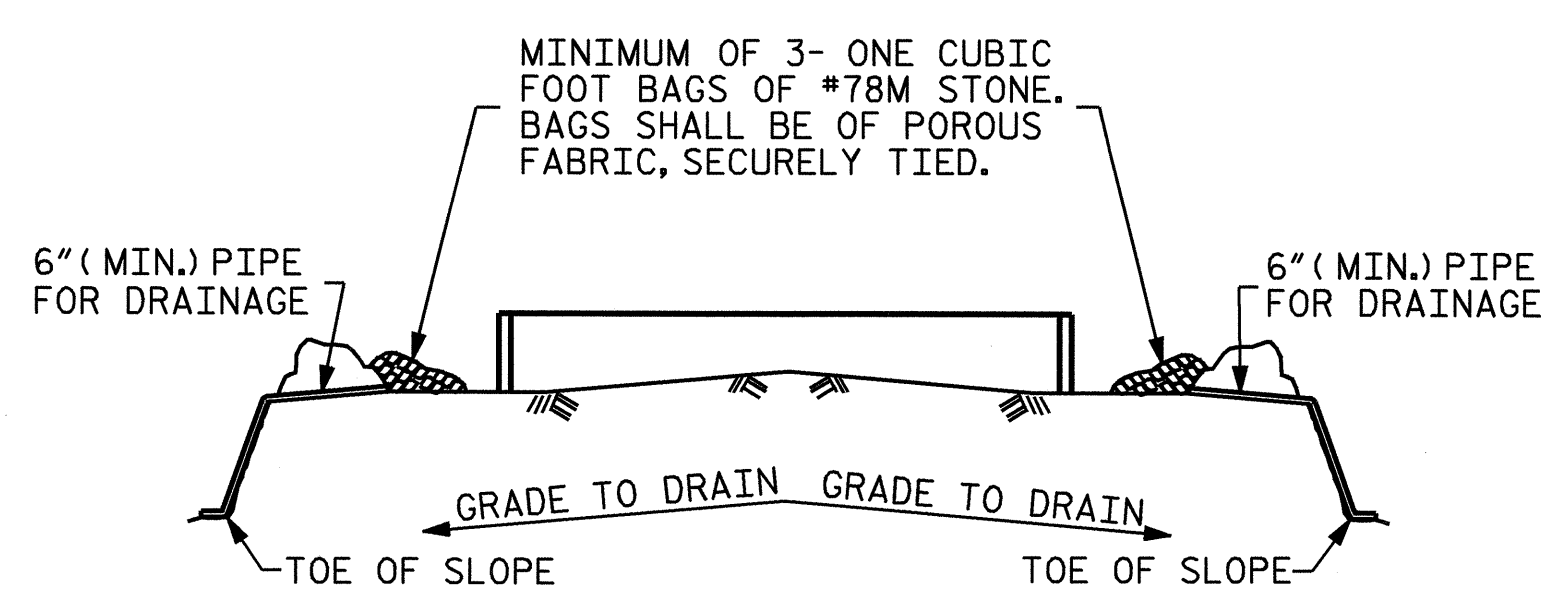
STR. #1



SECTION A-A



SECTION B-B

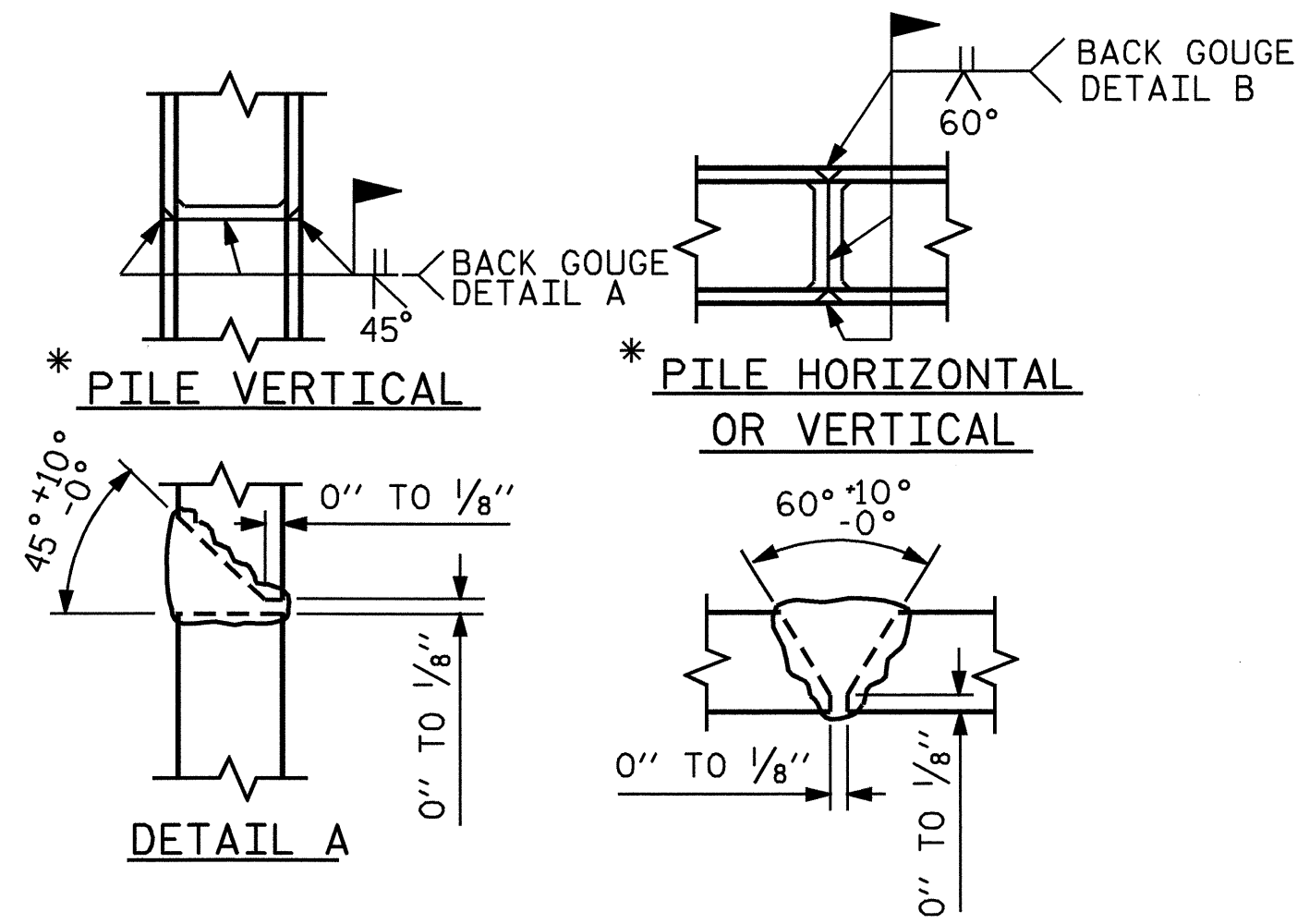


BAGGED STONE AND PIPE SHALL BE PLACED IMMEDIATELY AFTER COMPLETION OF END BENT EXCAVATION. PIPE MAY BE EITHER CONCRETE, CORRUGATED STEEL, CORRUGATED ALUMINUM ALLOY, OR CORRUGATED PLASTIC. PERFORATED PIPE WILL NOT BE ALLOWED.

BAGGED STONE SHALL REMAIN IN PLACE UNTIL THE ENGINEER DIRECTS THAT IT BE REMOVED. THE CONTRACTOR SHALL REMOVE AND DISPOSE OF SILT ACCUMULATIONS AT BAGGED STONE WHEN SO DIRECTED BY THE ENGINEER. BAGS SHALL BE REMOVED AND REPLACED WHENEVER THE ENGINEER DETERMINES THAT THEY HAVE DETERIORATED AND LOST THEIR EFFECTIVENESS.

NO SEPARATE PAYMENT WILL BE MADE FOR THIS WORK AND THE ENTIRE COST OF THIS WORK SHALL BE INCLUDED IN THE UNIT CONTRACT PRICE BID FOR THE SEVERAL PAY ITEMS.

TEMPORARY DRAINAGE AT END BENT

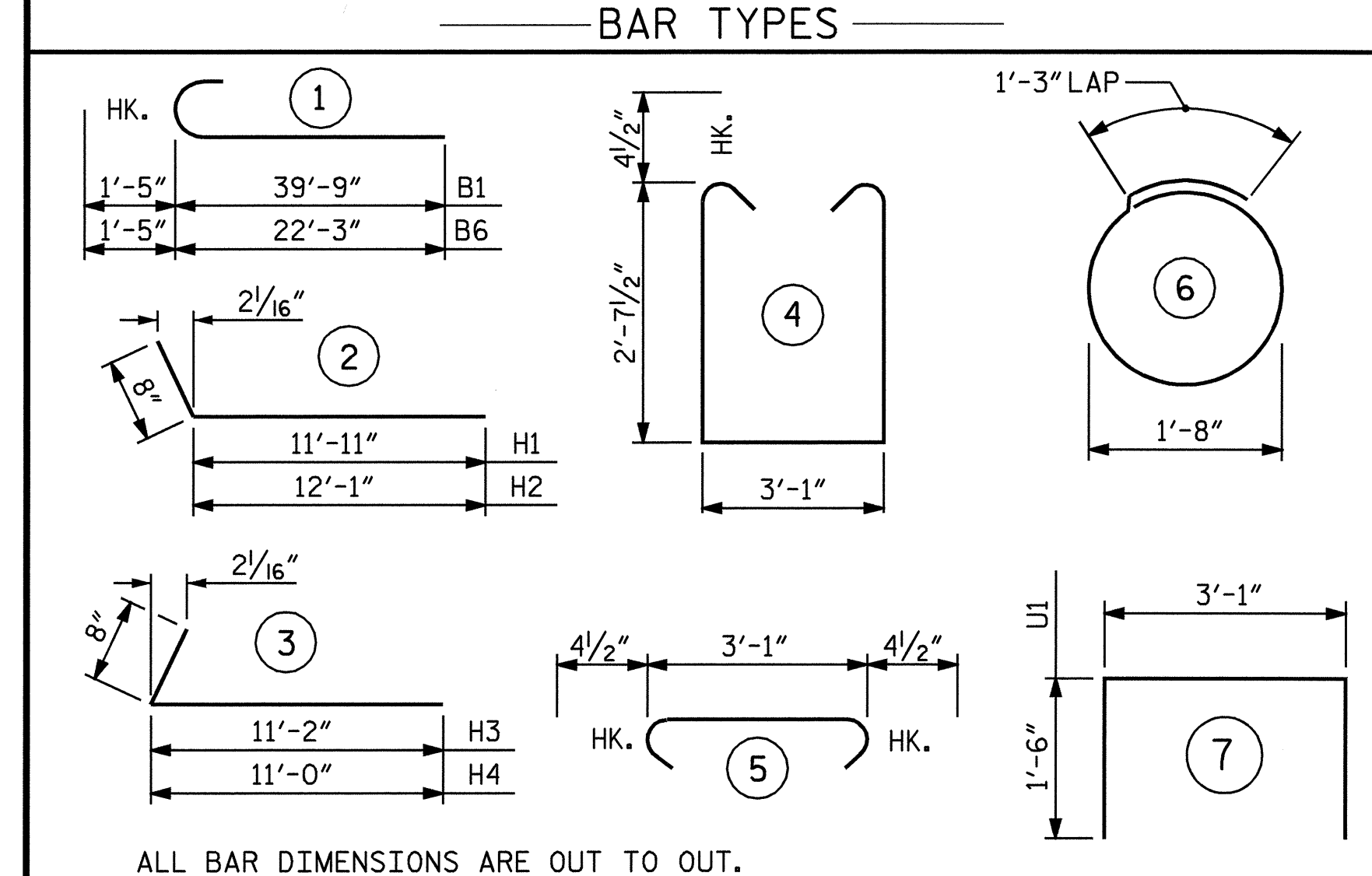


* POSITION OF PILE DURING WELDING.

PILE SPLICE DETAILS

BILL OF MATERIAL																													
INTEGRAL END BENT #1																													
STAGE 1							STAGE 2																						
BAR NO.	NO.	SIZE	TYPE	LENGTH	WEIGHT		BAR NO.	NO.	SIZE	TYPE	LENGTH	WEIGHT																	
B1	8	10	1	41'-2"	1417		B4	6	4	STR	3'-1"	12																	
B2	4	5	STR	42'-4"	177		B6	8	10	1	23'-8"	815																	
B3	8	4	STR	22'-2"	118		B7	4	5	STR	22'-10"	95																	
B4	10	4	STR	3'-1"	21		B8	4	4	STR	22'-10"	61																	
B5	8	4	STR	20'-3"	108		B9	4	4	STR	13'-8"	37																	
H1	4	4	2	12'-7"	34		H3	4	4	3	11'-10"	32																	
H2	4	4	2	12'-9"	34		H4	4	4	3	11'-8"	31																	
S1	40	4	4	9'-1"	243		S1	23	4	4	9'-1"	140																	
S2	40	4	5	3'-10"	102		S2	23	4	5	3'-10"	59																	
S3	27	4	6	6'-6"	117		S3	15	4	6	6'-6"	65																	
U1	26	4	7	6'-1"	106		U1	9	4	7	6'-1"	37																	
V1	64	4	STR	5'-9"	246		V1	34	4	STR	5'-9"	131																	
V2	22	4	STR	7'-7"	111		V3	22	4	STR	7'-4"	108																	
REINFORCING STEEL							LBS	2834							REINFORCING STEEL							LBS	1623						
CLASS 'A' CONCRETE							18.7 C.Y.							CLASS 'A' CONCRETE							10.4 C.Y.								
HP 12X53 STEEL PILES							NO. 9							HP 12X53 STEEL PILES							NO. 5								
							135 LIN.FT.														75 LIN.FT.								
PILE EXCAVATION IN SOIL							127 LIN.FT.							PILE EXCAVATION IN SOIL							71 LIN.FT.								
PILE EXCAV. NOT IN SOIL							12 LIN.FT.							PILE EXCAV. NOT IN SOIL							6 LIN.FT.								

TOTAL BILL OF MATERIAL																													
REINFORCING STEEL							LBS	4457							HP 12X53 STEEL PILES							NO. 14	210 LIN.FT.						
CLASS 'A' CONCRETE							29.1 C.Y.							PILE EXCAVATION IN SOIL							198 LIN.FT.								
														PILE EXCAV. NOT IN SOIL							18 LIN.FT.								



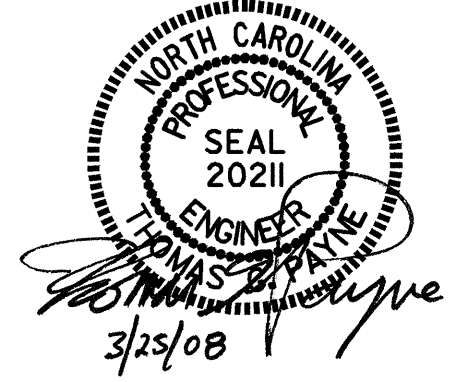
ALL BAR DIMENSIONS ARE OUT TO OUT.

PROJECT NO. B-4252
 ROCKINGHAM COUNTY
 STATION: 27+69.00 -L-

SHEET 4 OF 4

STATE OF NORTH CAROLINA
 DEPARTMENT OF TRANSPORTATION
 RALEIGH

SUBSTRUCTURE
 INTEGRAL
 END BENT #1



REVISIONS						SHEET NO.	
NO.	BY:	DATE:	NO.	BY:	DATE:	S-26	
1			3			TOTAL SHEETS 70	
2			4				

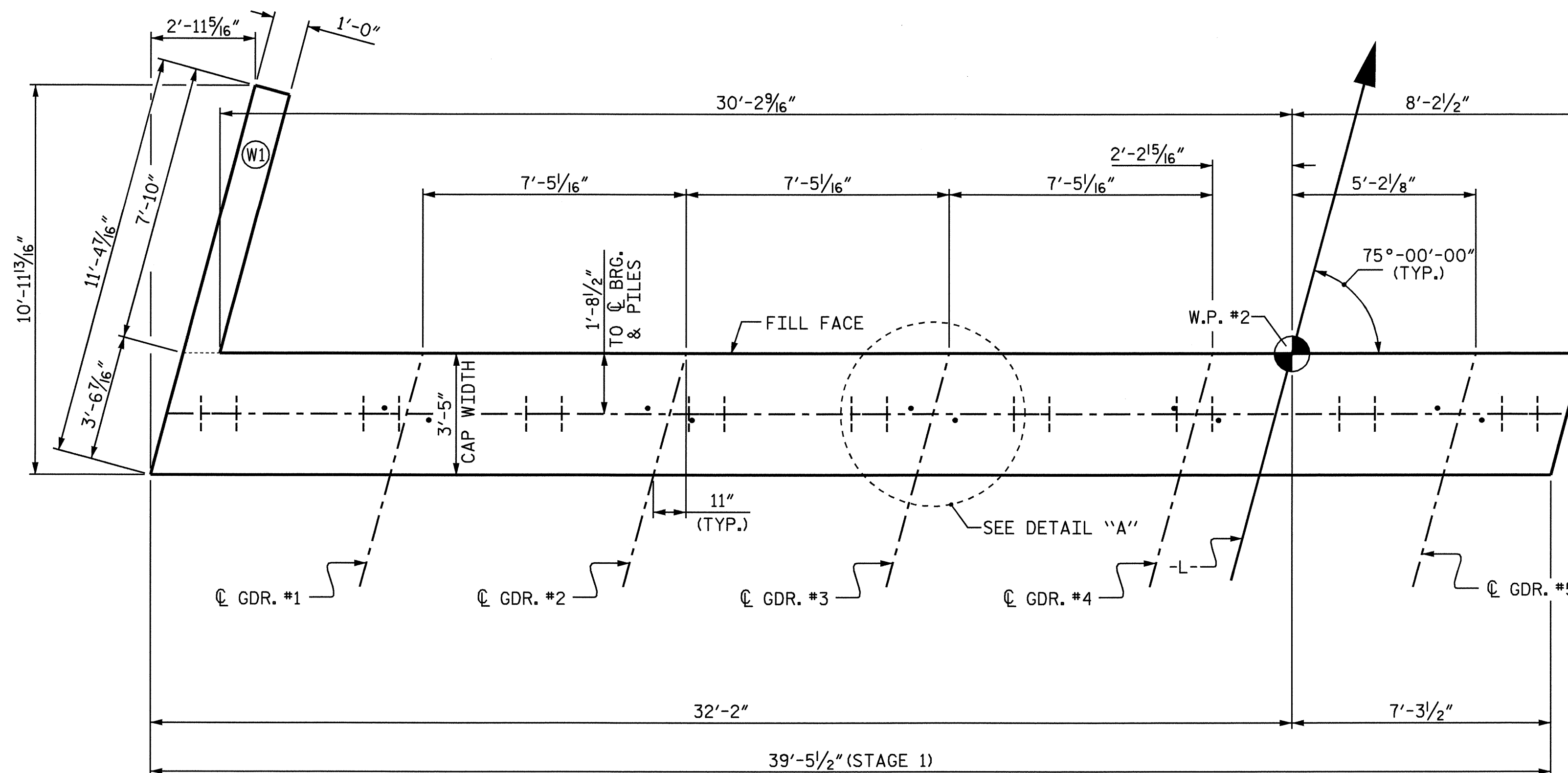
DRAWN BY: S. DOMBROWSKI DATE: 1/07
 CHECKED BY: M.K. BEARD DATE: 2/08

NOTES

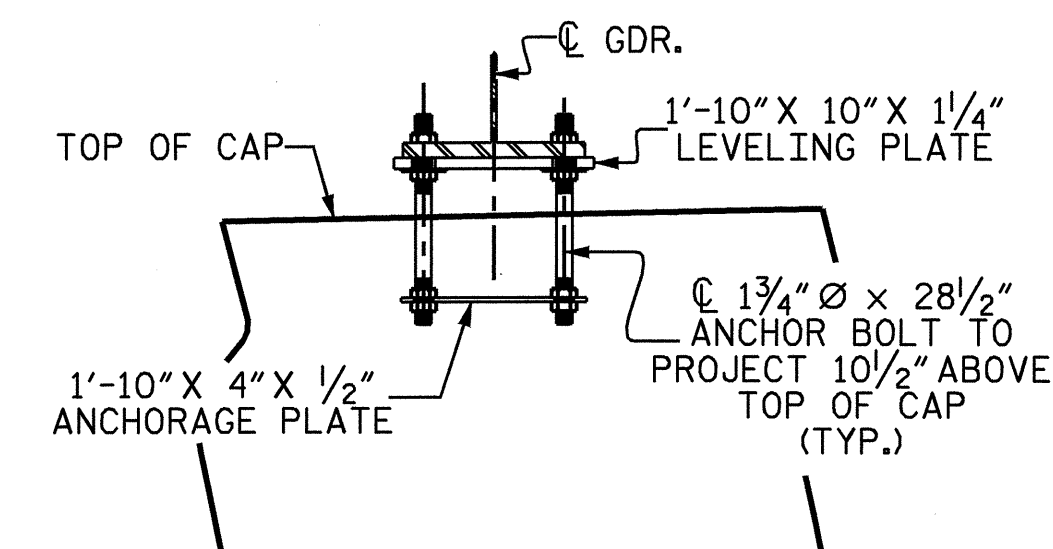
STIRRUPS IN CAP MAY BE SHIFTED AS NECESSARY TO CLEAR ANCHOR BOLTS.

SEE SUPERSTRUCTURE SHEETS FOR UPPER PART OF INTEGRAL END BENT DETAILS.

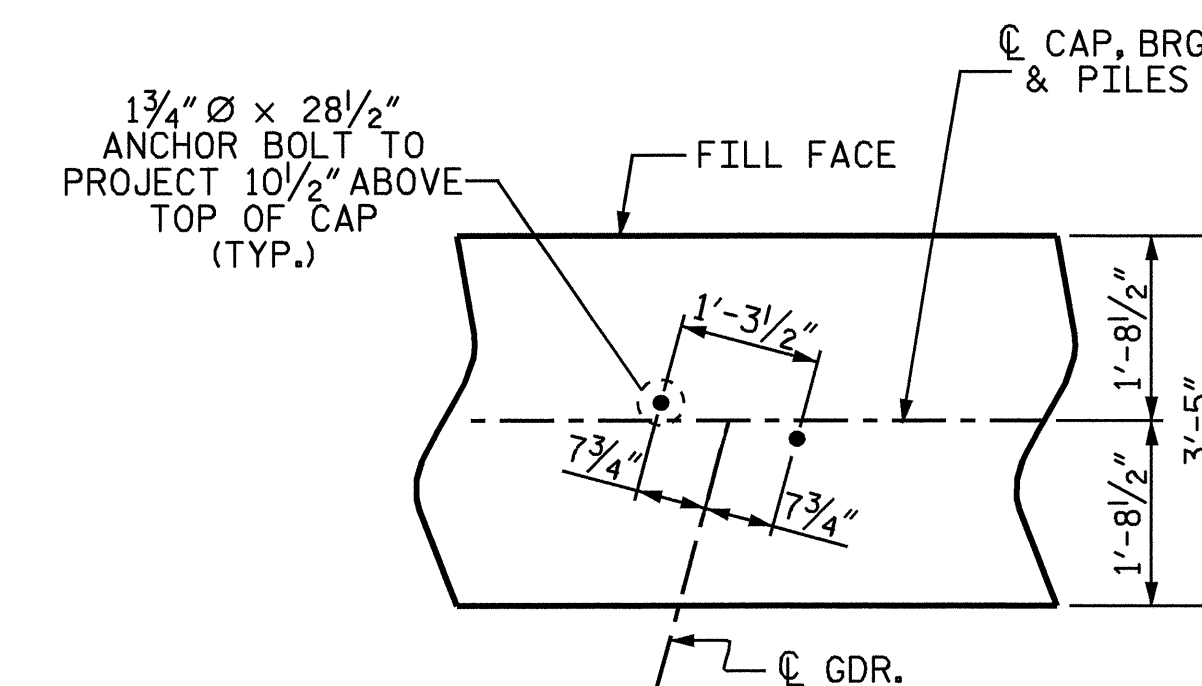
THE CONTRACTOR SHALL PROVIDE FOR INSTALLATION OF THE 4" DIAMETER DRAIN PIPE THROUGH THE WING WALL AS REQUIRED FOR REINFORCED BRIDGE APPROACH FILLS, SEE ROADWAY PLANS. REINFORCING STEEL IN THE WING WALL MAY BE SHIFTED AS NECESSARY TO CLEAR THE DRAIN PIPES.



PLAN

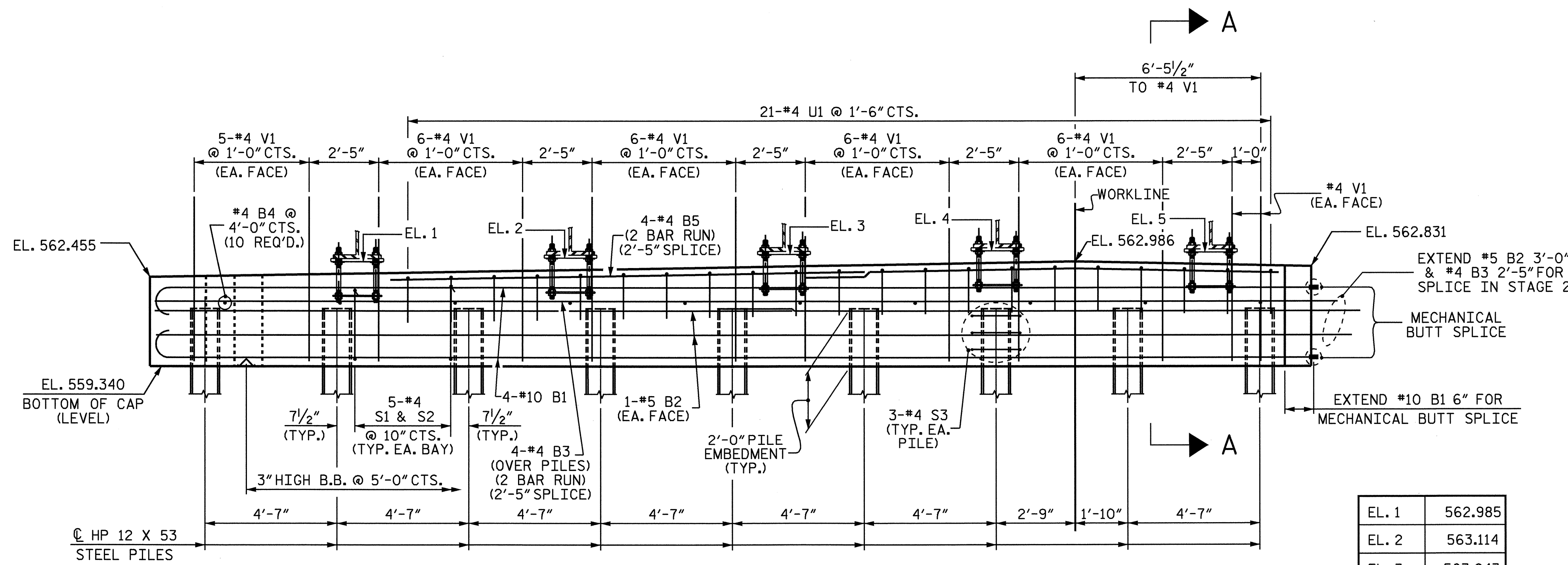


ELEVATION VIEW



PLAN VIEW

DETAIL "A"



ELEVATION

EL. 1	562.985
EL. 2	563.114
EL. 3	563.243
EL. 4	563.373
EL. 5	563.303

ELEVATIONS SHOWN ARE TO BOTTOM OF LEVELING PLATE

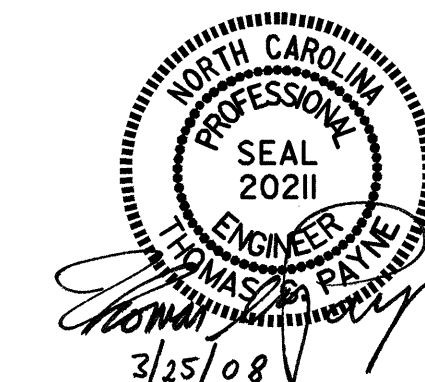
PROJECT NO. B-4252
 ROCKINGHAM COUNTY
 STATION: 27+69.00 -L-

SHEET 1 OF 4

STATE OF NORTH CAROLINA
 DEPARTMENT OF TRANSPORTATION
 RALEIGH

SUBSTRUCTURE
 INTEGRAL
 END BENT #2
 (STAGE 1)

REVISIONS						SHEET NO.
NO.	BY:	DATE:	NO.	BY:	DATE:	S-27
1			3			TOTAL SHEETS 70
2			4			



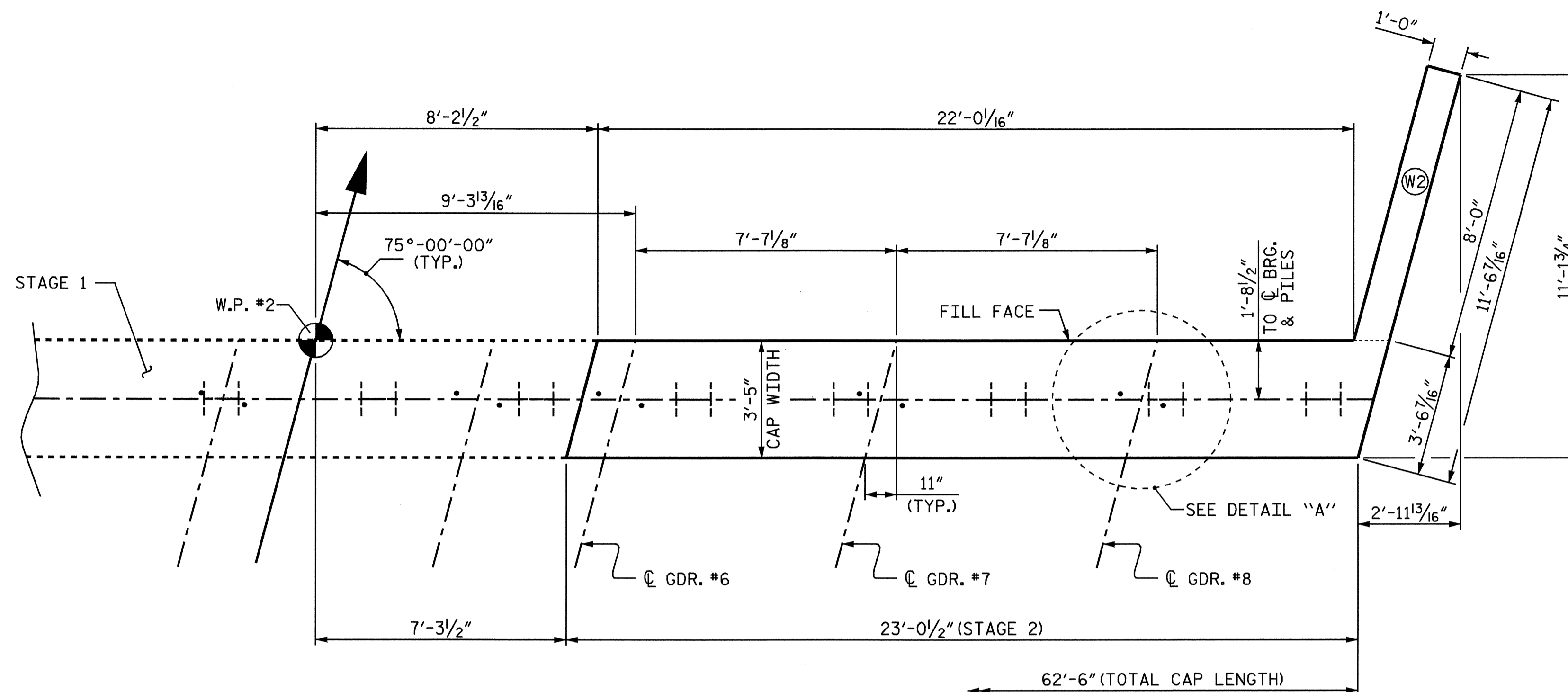
DRAWN BY: S. DOMBROWSKI DATE: 1/07
 CHECKED BY: M.K. BEARD DATE: 2/08

NOTES

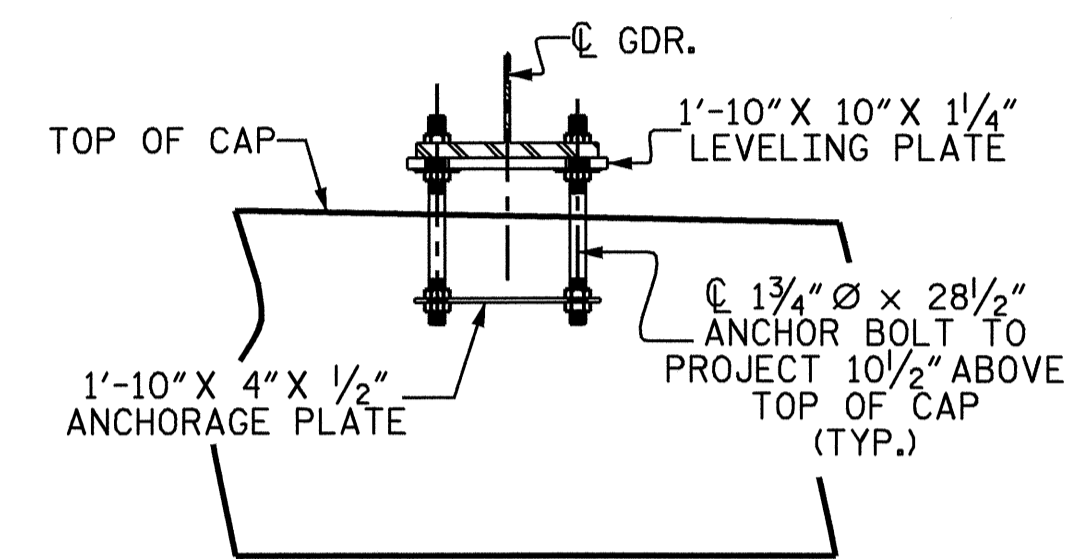
STIRRUPS IN CAP MAY BE SHIFTED AS NECESSARY TO CLEAR ANCHOR BOLTS.

SEE SUPERSTRUCTURE SHEETS FOR UPPER PART OF INTEGRAL END BENT DETAILS.

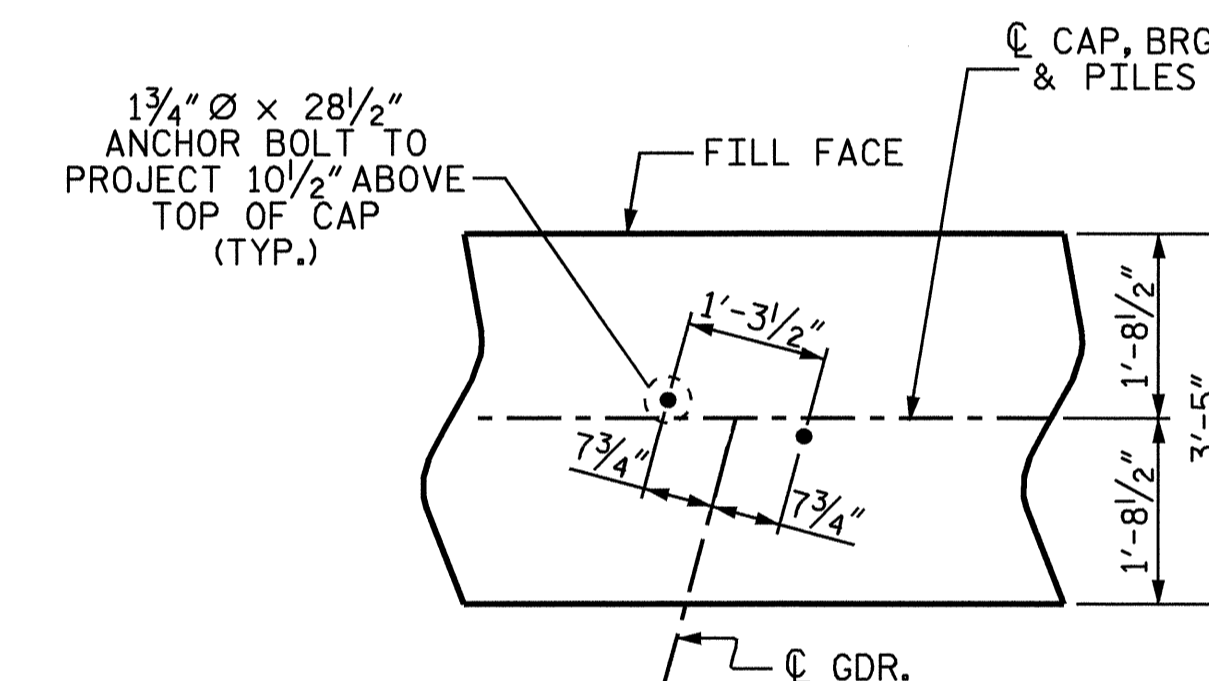
THE CONTRACTOR SHALL PROVIDE FOR INSTALLATION OF THE 4" DIAMETER DRAIN PIPE THROUGH THE WING WALL AS REQUIRED FOR REINFORCED BRIDGE APPROACH FILLS, SEE ROADWAY PLANS. REINFORCING STEEL IN THE WING WALL MAY BE SHIFTED AS NECESSARY TO CLEAR THE DRAIN PIPES.



PLAN

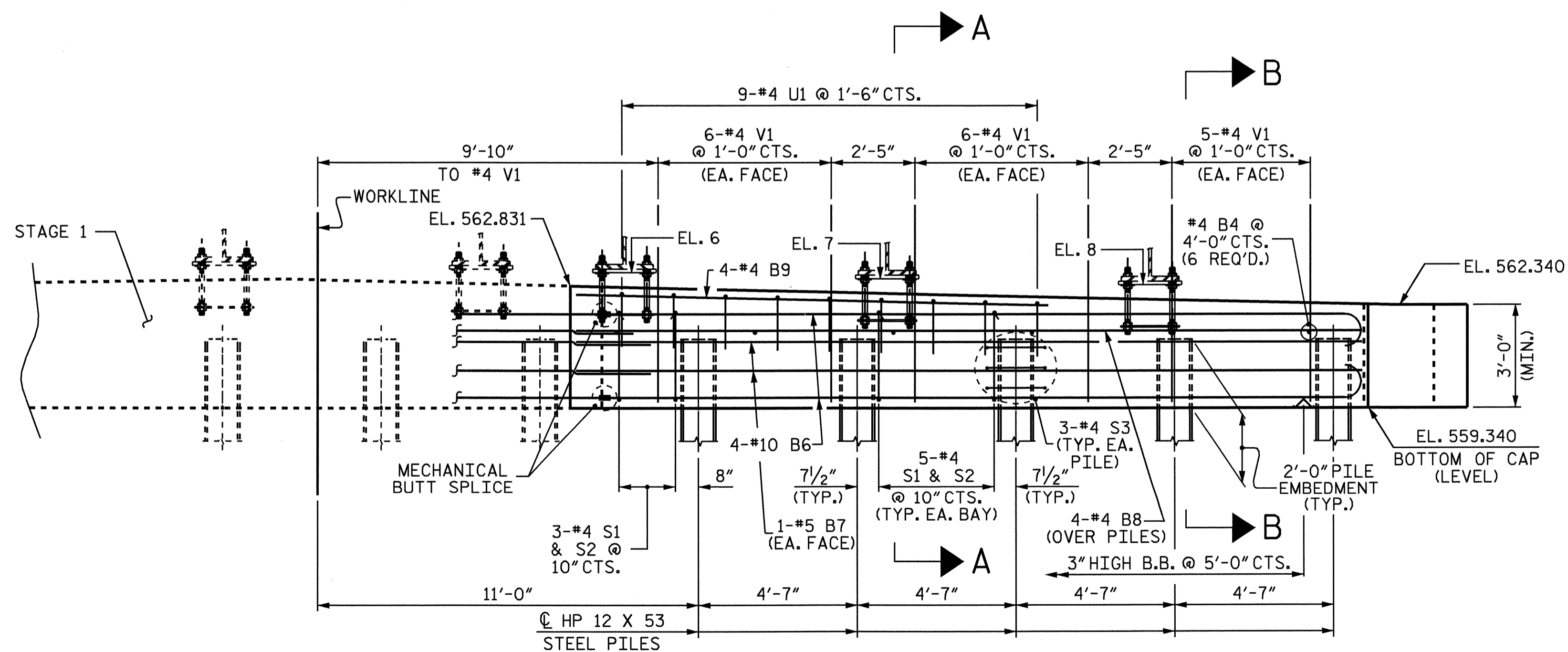


ELEVATION VIEW



PLAN VIEW

DETAIL "A"



ELEVATION

EL. 6	563.216
EL. 7	563.056
EL. 8	562.897

ELEVATIONS SHOWN ARE TO BOTTOM OF LEVELING PLATE

PROJECT NO. B-4252
ROCKINGHAM COUNTY
 STATION: 27+69.00 -L-

SHEET 2 OF 4

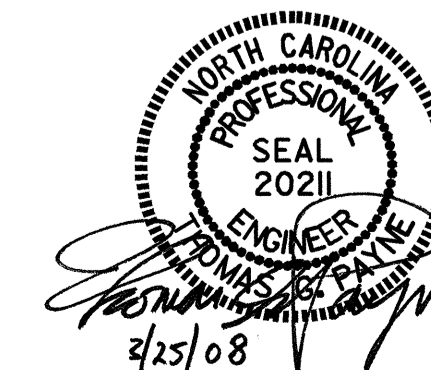
STATE OF NORTH CAROLINA
 DEPARTMENT OF TRANSPORTATION
 RALEIGH

SUBSTRUCTURE
 INTEGRAL
 END BENT #2
 (STAGE 2)

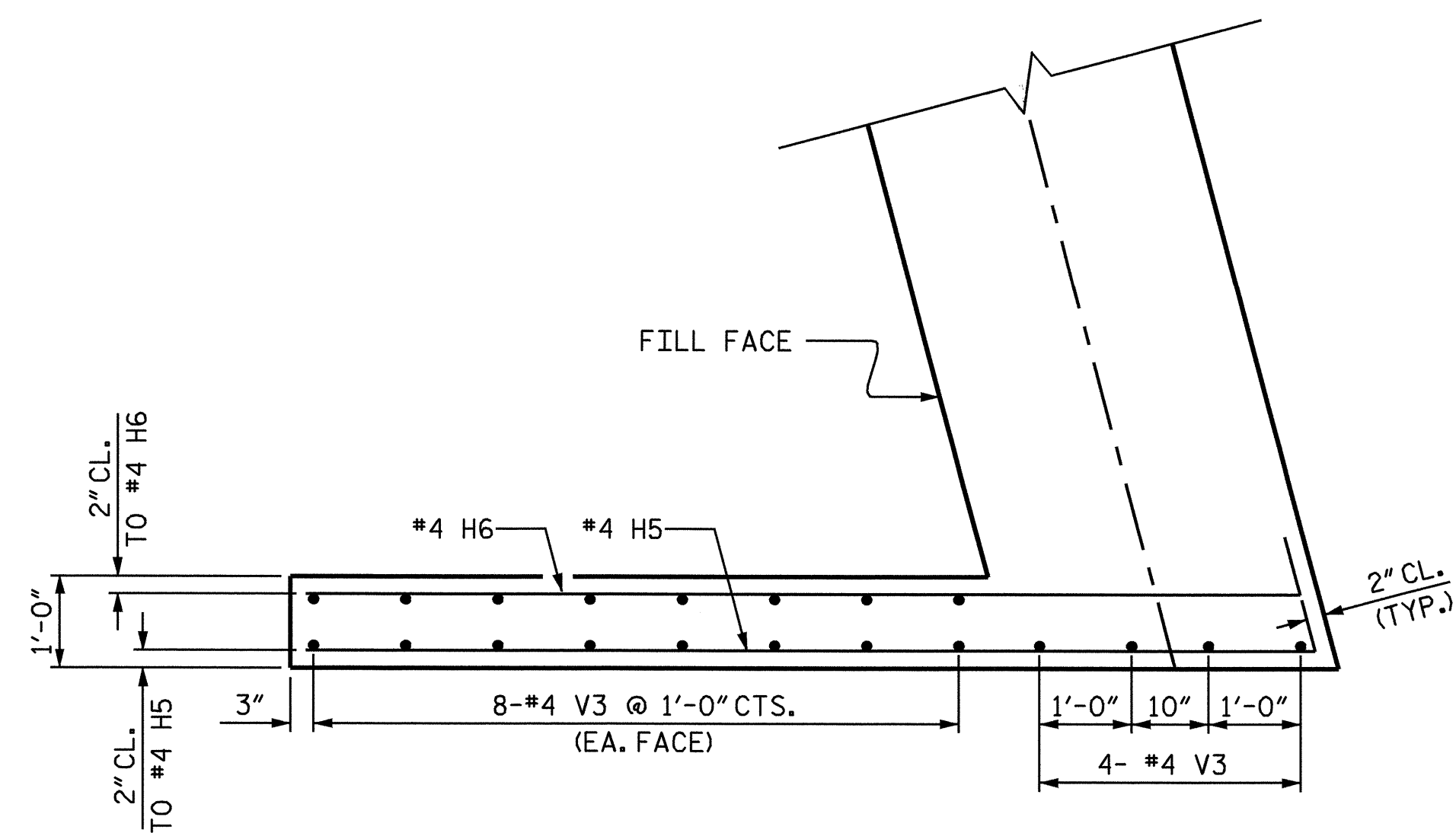
REVISIONS

NO.	BY:	DATE:	NO.	BY:	DATE:
1			3		
2			4		

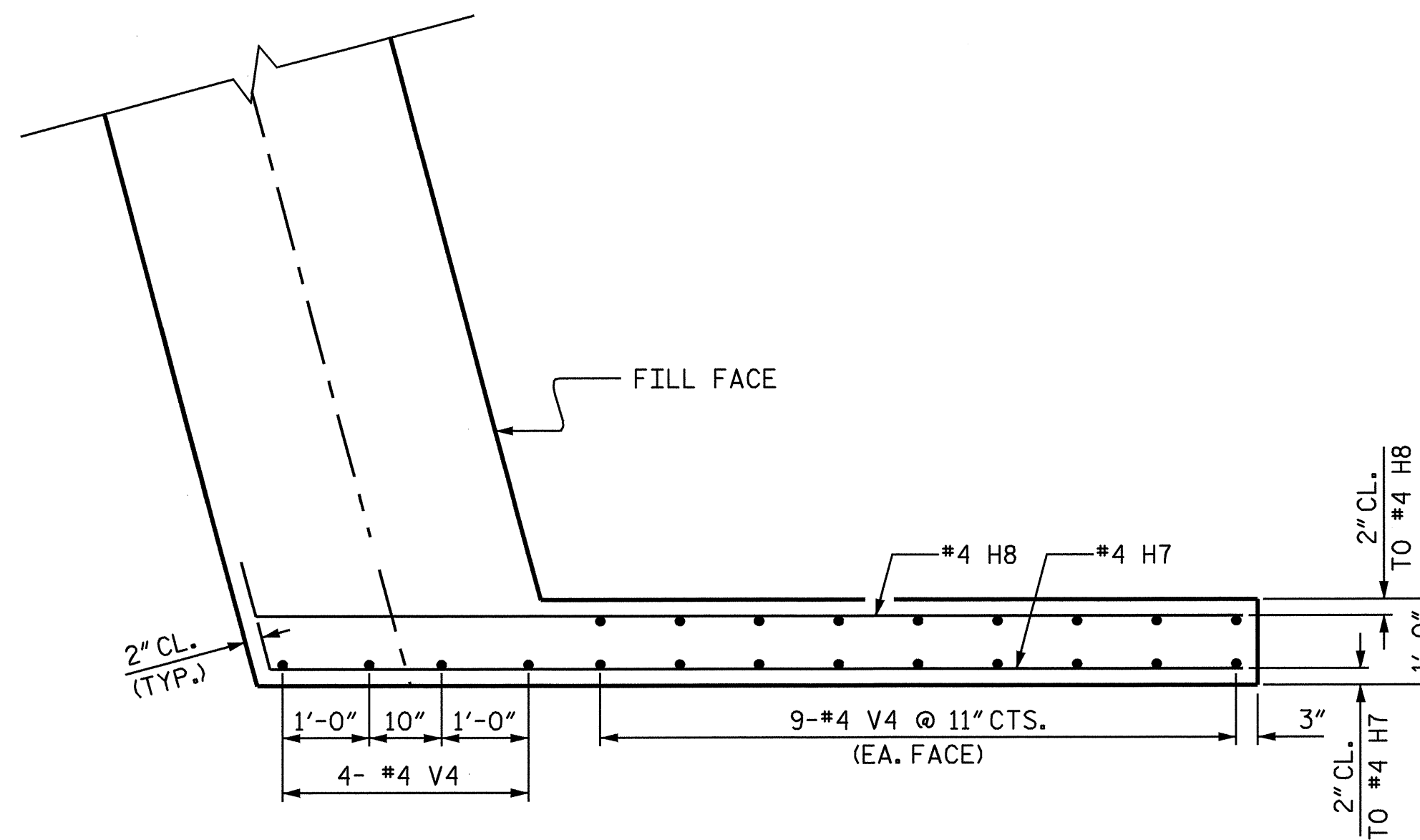
SHEET NO.	S-28
TOTAL SHEETS	70



DRAWN BY : S. DOMBROWSKI DATE : 1/07
 CHECKED BY : M.K. BEARD DATE : 2/08

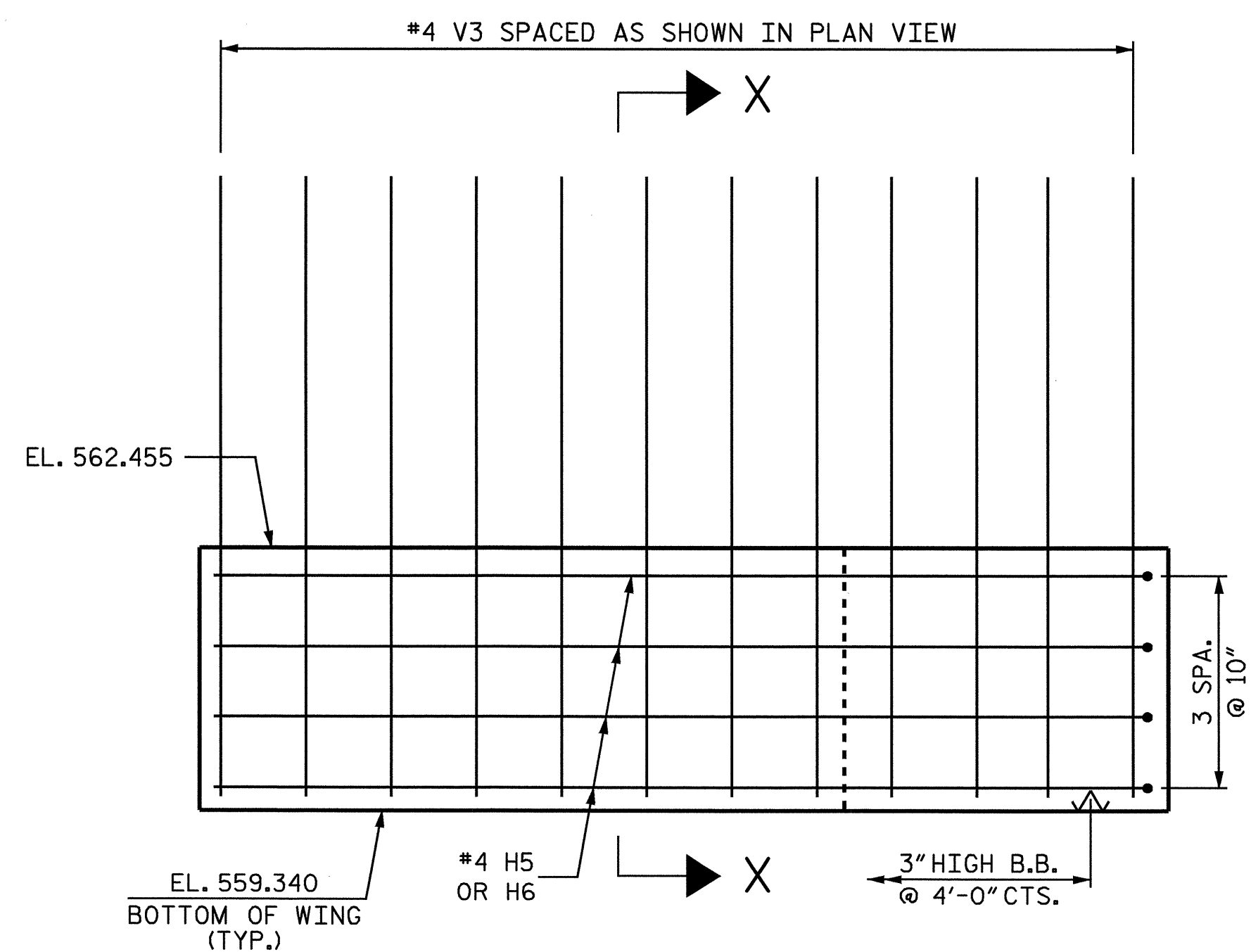


PLAN OF LEFT WING W1
(STAGE 1)

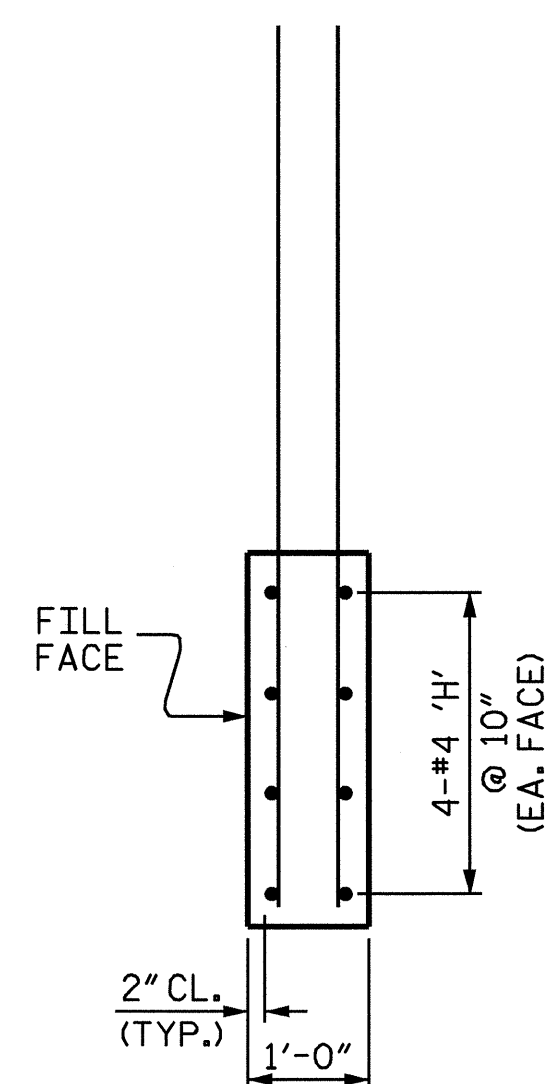


PLAN OF RIGHT WING W2
(STAGE 2)

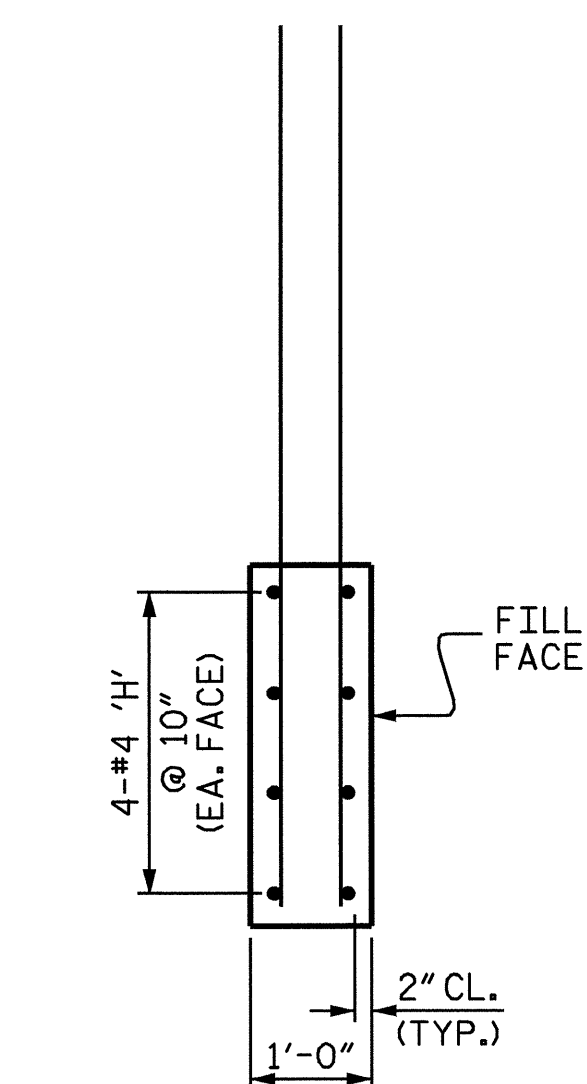
NOTE: THE UPPER PORTION OF THE WINGS SHALL BE POURED WITH THE SUPERSTRUCTURE. FOR DETAILS AND REINFORCING STEEL, SEE SUPERSTRUCTURE DETAILS.



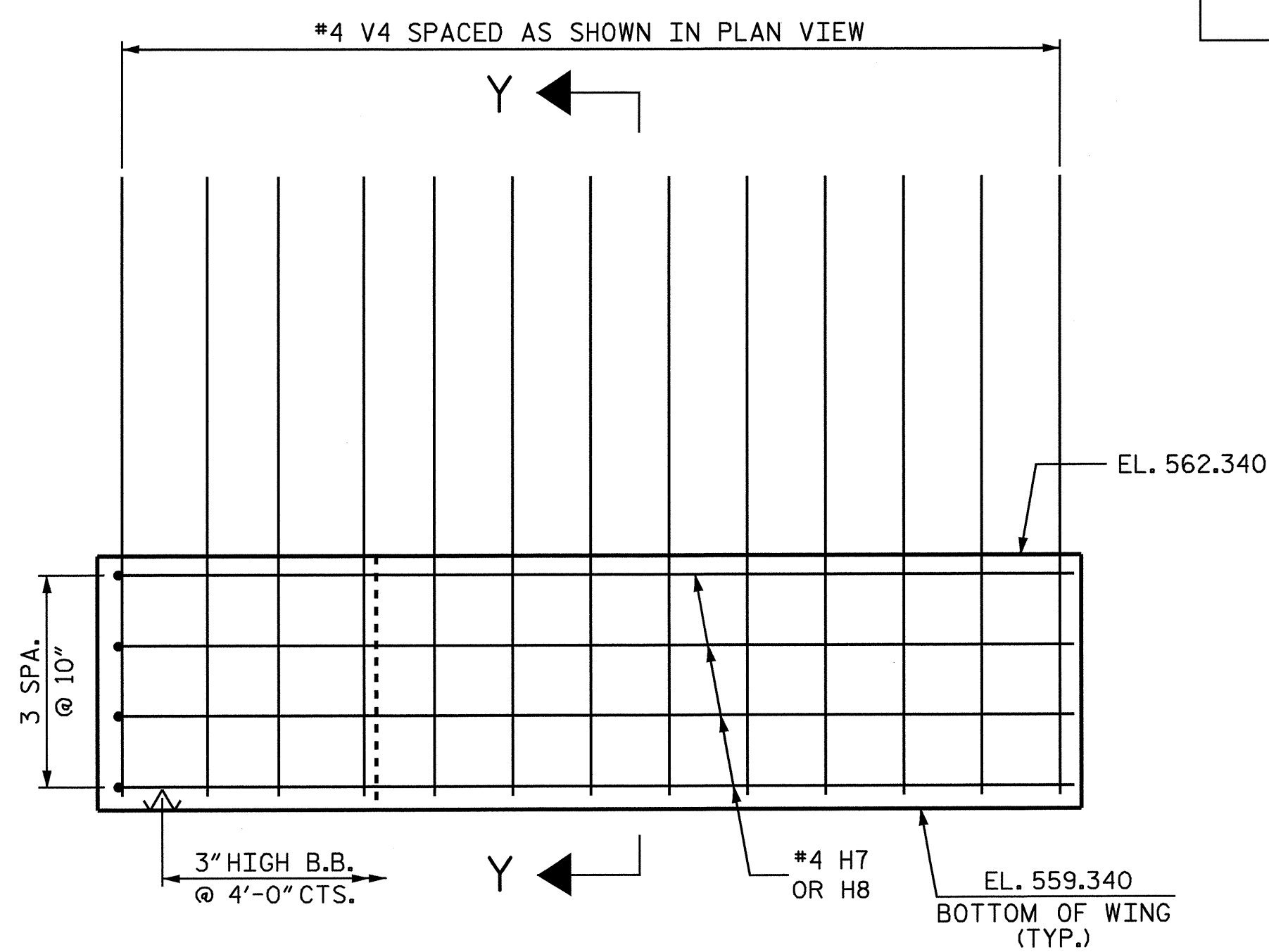
ELEVATION OF LEFT WING W1
(STAGE 1)



SECTION X-X



SECTION Y-Y

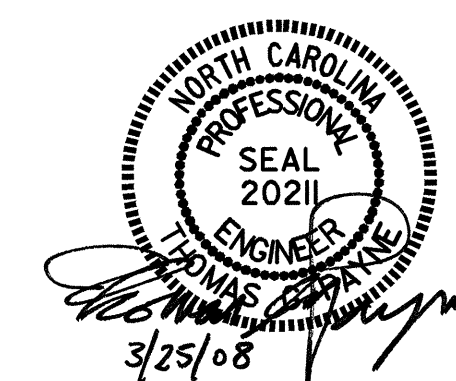


ELEVATION OF RIGHT WING W2
(STAGE 2)

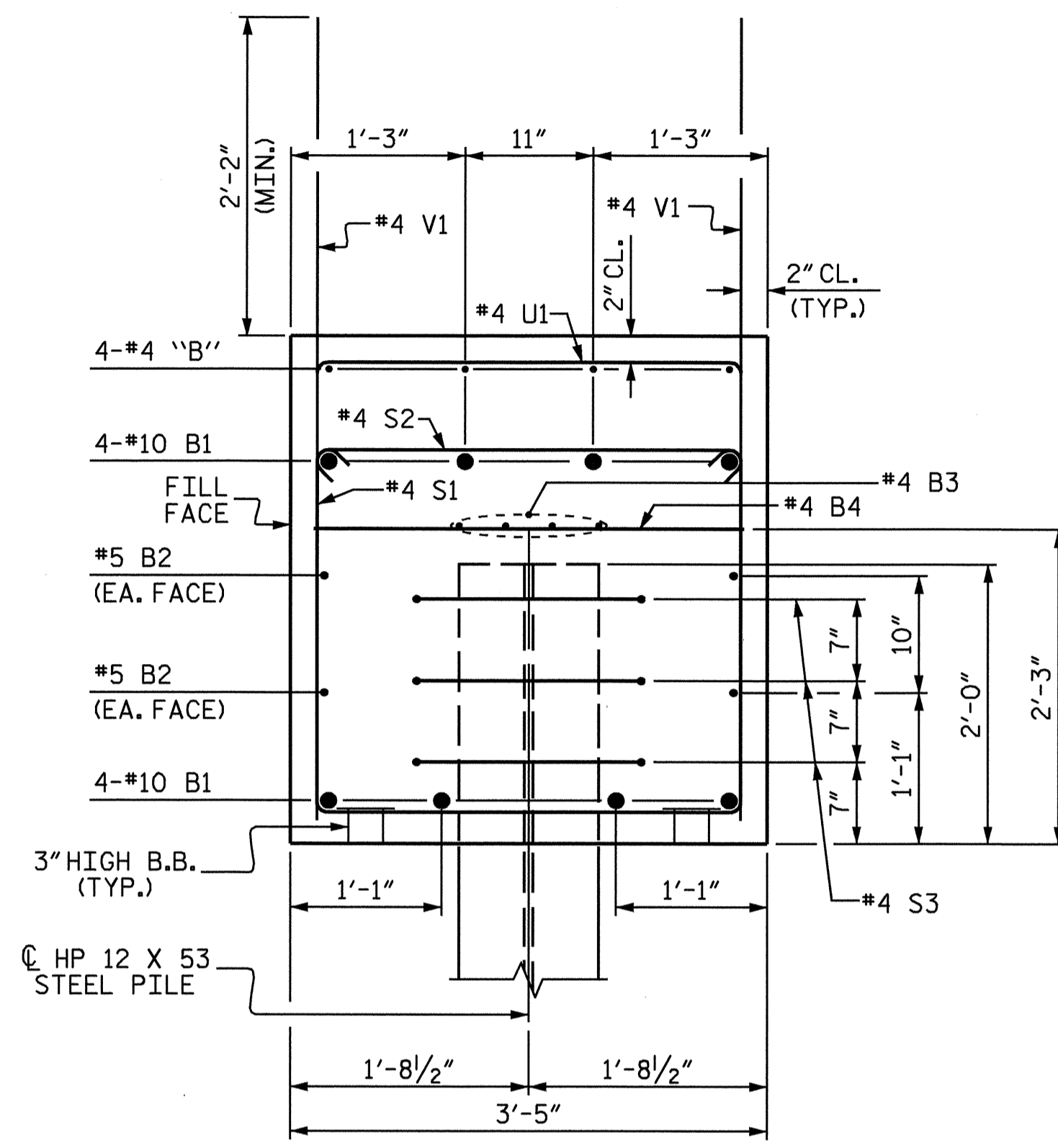
PROJECT NO. B-4252
ROCKINGHAM COUNTY
STATION: 27+69.00 -L-

SHEET 3 OF 4

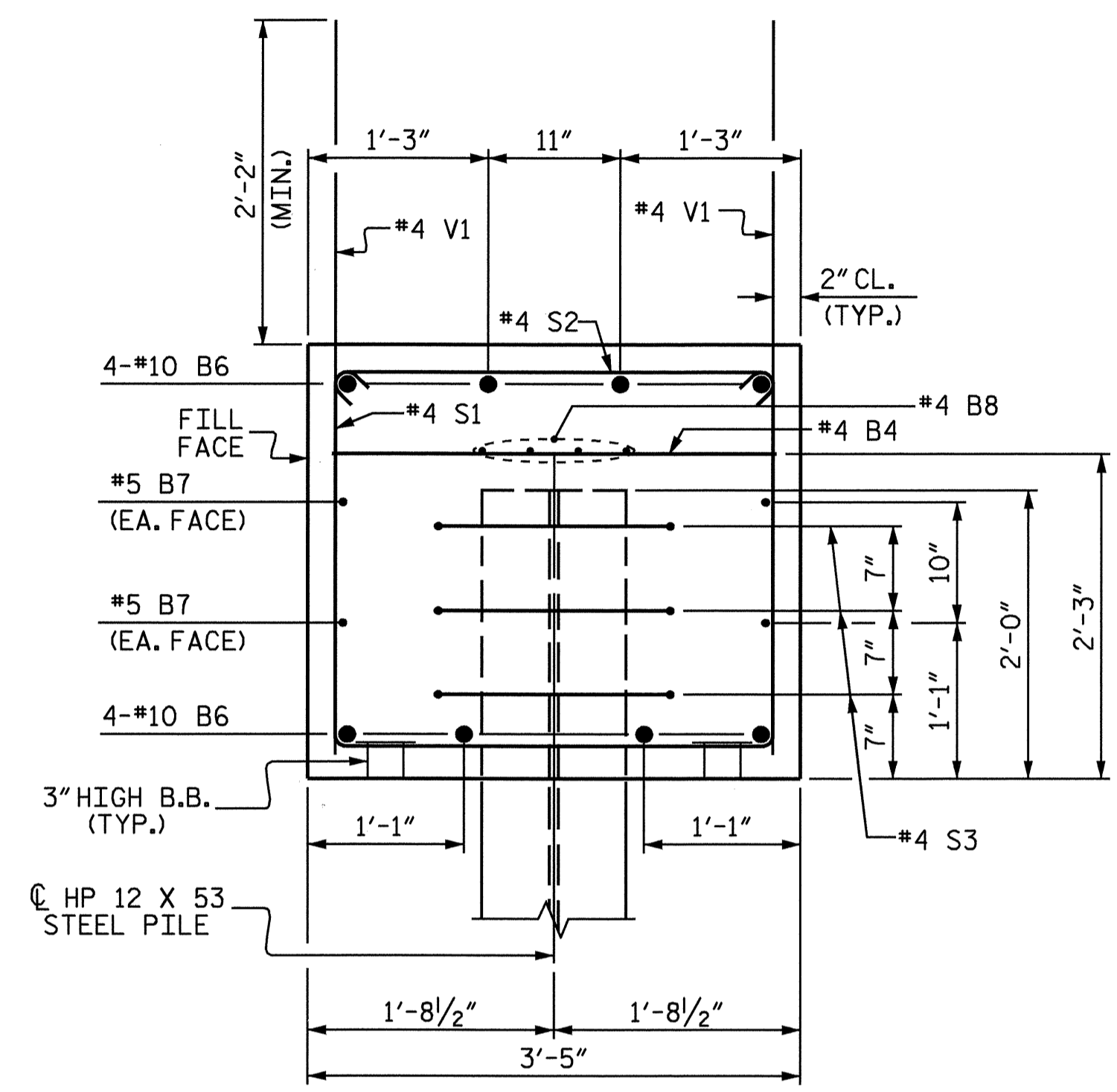
STATE OF NORTH CAROLINA DEPARTMENT OF TRANSPORTATION RALEIGH					
SUBSTRUCTURE INTEGRAL END BENT #2					
REVISIONS					
NO.	BY:	DATE:	NO.	BY:	DATE:
1			3		
2			4		
					SHEET NO. S-29
					TOTAL SHEETS 70



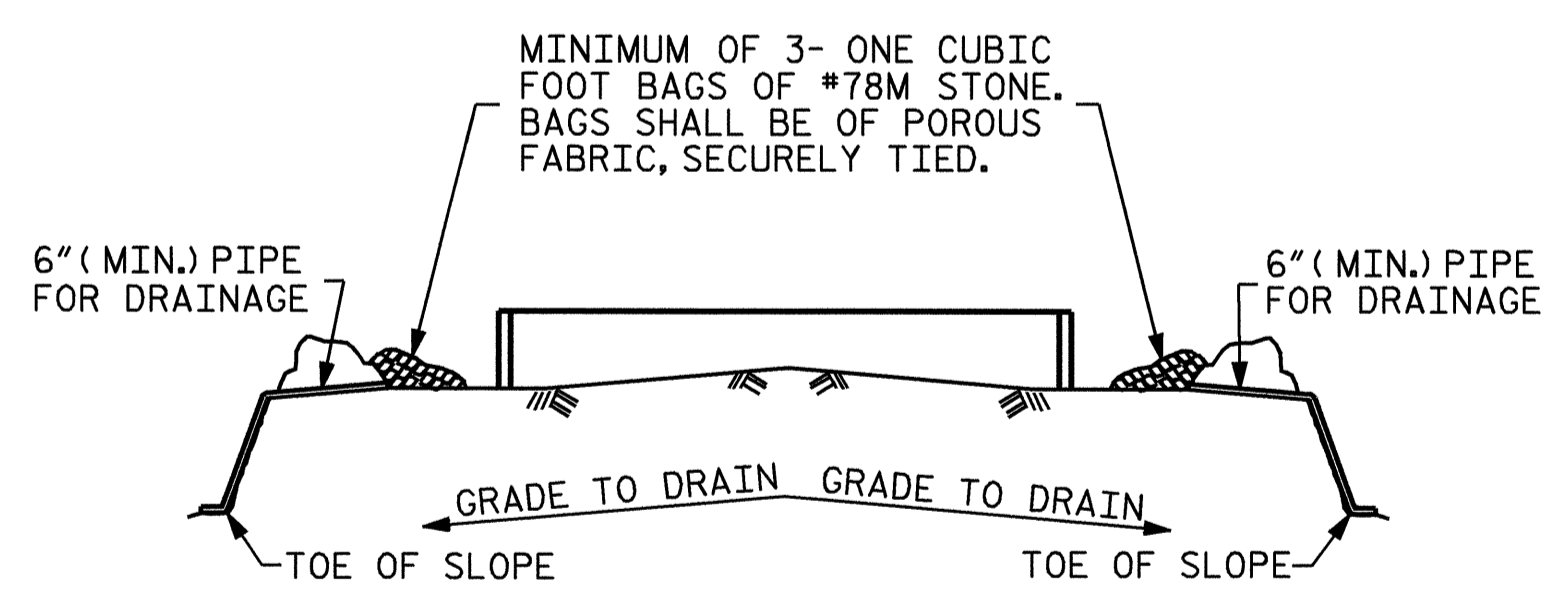
DRAWN BY : S. DOMBROWSKI DATE : 1/07
CHECKED BY : M.K. BEARD DATE : 2/08



SECTION A-A



SECTION B-B

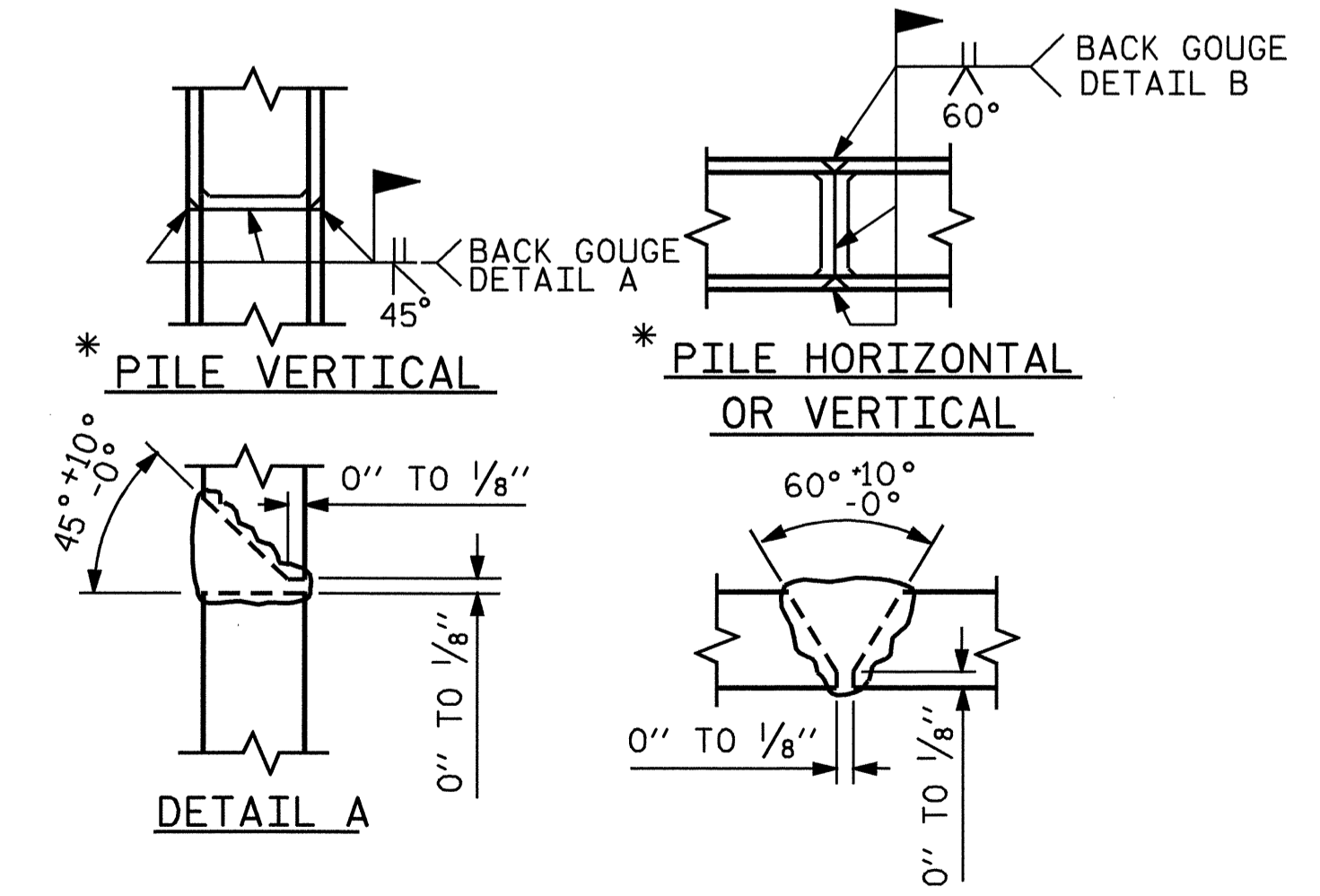


BAGGED STONE AND PIPE SHALL BE PLACED IMMEDIATELY AFTER COMPLETION OF END BENT EXCAVATION. PIPE MAY BE EITHER CONCRETE, CORRUGATED STEEL, CORRUGATED ALUMINUM ALLOY, OR CORRUGATED PLASTIC. PERFORATED PIPE WILL NOT BE ALLOWED.

BAGGED STONE SHALL REMAIN IN PLACE UNTIL THE ENGINEER DIRECTS THAT IT BE REMOVED. THE CONTRACTOR SHALL REMOVE AND DISPOSE OF SILT ACCUMULATIONS AT BAGGED STONE WHEN SO DIRECTED BY THE ENGINEER. BAGS SHALL BE REMOVED AND REPLACED WHENEVER THE ENGINEER DETERMINES THAT THEY HAVE DETERIORATED AND LOST THEIR EFFECTIVENESS.

NO SEPARATE PAYMENT WILL BE MADE FOR THIS WORK AND THE ENTIRE COST OF THIS WORK SHALL BE INCLUDED IN THE UNIT CONTRACT PRICE BID FOR THE SEVERAL PAY ITEMS.

TEMPORARY DRAINAGE AT END BENT

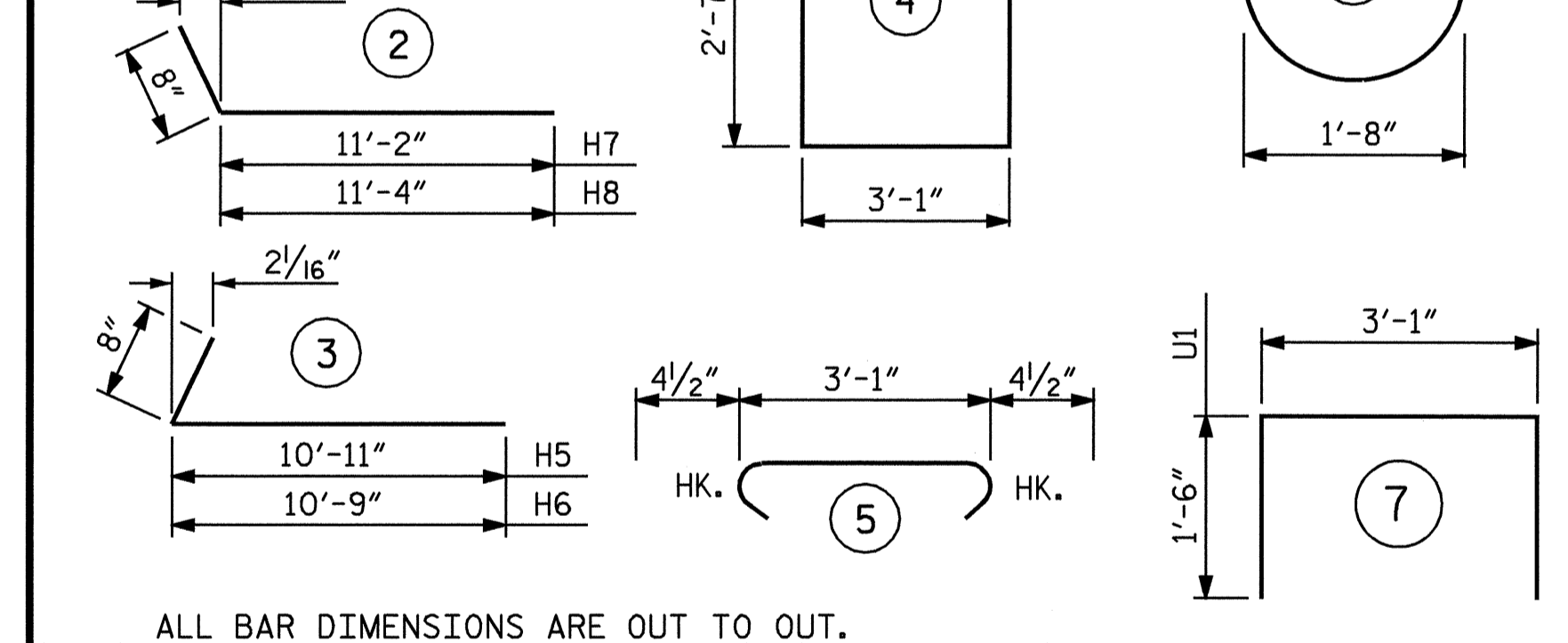


PILE SPLICE DETAILS
* POSITION OF PILE DURING WELDING.

BILL OF MATERIAL																													
INTEGRAL END BENT #2																													
STAGE 1							STAGE 2																						
BAR	NO.	SIZE	TYPE	LENGTH	WEIGHT	BAR	NO.	SIZE	TYPE	LENGTH	WEIGHT	BAR	NO.	SIZE	TYPE	LENGTH	WEIGHT												
B1	8	10	1	41'-2"	1417	B4	6	4	STR	3'-1"	12	B7	4	5	STR	22'-10"	95												
B2	4	5	STR	42'-4"	177	B6	8	10	1	23'-8"	815	B8	4	4	STR	22'-10"	61												
B3	8	4	STR	22'-2"	118	B7	4	5	STR	22'-10"	95	B9	4	4	STR	13'-8"	37												
B4	10	4	STR	3'-1"	21	B8	4	4	STR	22'-10"	61																		
B5	8	4	STR	16'-6"	88	B9	4	4	STR	13'-8"	37																		
H5	4	4	3	11'-7"	31	H7	4	4	2	11'-10"	32																		
H6	4	4	3	11'-5"	31	H8	4	4	2	12'-0"	32																		
S1	40	4	4	9'-1"	243	S1	23	4	4	9'-1"	140																		
S2	40	4	5	3'-10"	102	S2	23	4	5	3'-10"	59																		
S3	27	4	6	6'-6"	117	S3	15	4	6	6'-6"	65																		
U1	21	4	7	6'-1"	85	U1	9	4	7	6'-1"	37																		
V1	62	4	STR	5'-9"	238	V1	34	4	STR	5'-9"	131																		
V3	20	4	STR	7'-4"	98	V4	22	4	STR	7'-3"	107																		
REINFORCING STEEL							LBS	2766							REINFORCING STEEL							LBS	1623						
CLASS 'A' CONCRETE							18.0 C.Y.							CLASS 'A' CONCRETE							10.4 C.Y.								
HP 12X53 STEEL PILES							NO. 9							HP 12X53 STEEL PILES							NO. 5								
							135 LIN.FT.														75 LIN.FT.								
PILE EXCAVATION IN SOIL							106 LIN.FT.							PILE EXCAVATION IN SOIL							59 LIN.FT.								
PILE EXCAV. NOT IN SOIL							23 LIN.FT.							PILE EXCAV. NOT IN SOIL							12 LIN.FT.								

TOTAL BILL OF MATERIAL

REINFORCING STEEL	LBS	4389	HP 12X53 STEEL PILES	NO. 14	210 LIN.FT.
CLASS 'A' CONCRETE	28.4 C.Y.		PILE EXCAVATION IN SOIL	165 LIN.FT.	
			PILE EXCAV. NOT IN SOIL	35 LIN.FT.	



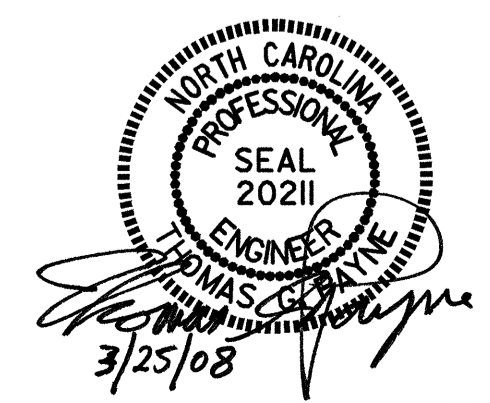
ALL BAR DIMENSIONS ARE OUT TO OUT.

PROJECT NO. B-4252
ROCKINGHAM COUNTY
 STATION: 27+69.00 -L-

SHEET 4 OF 4

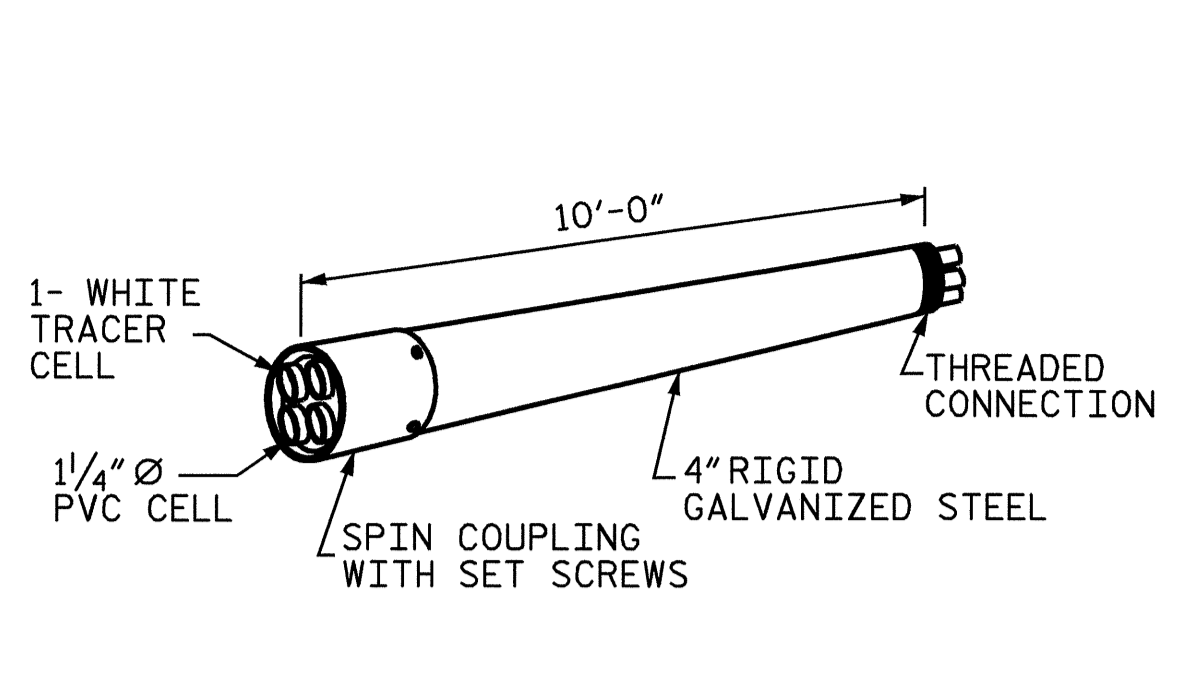
STATE OF NORTH CAROLINA
 DEPARTMENT OF TRANSPORTATION
 RALEIGH

SUBSTRUCTURE
 INTEGRAL
 END BENT #2

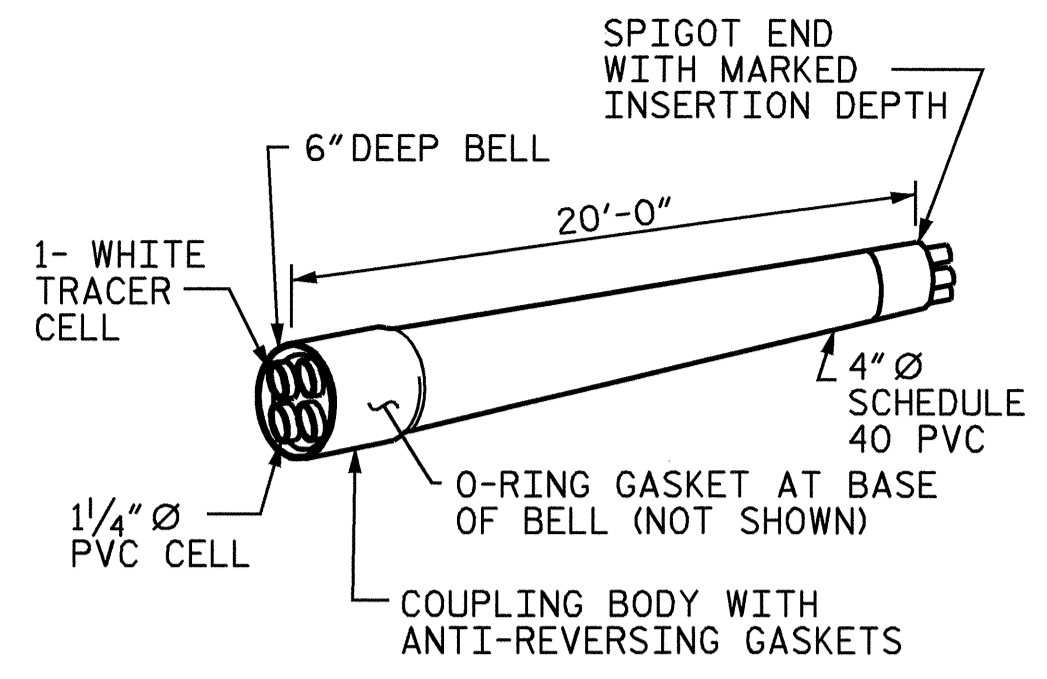


REVISIONS						SHEET NO.	
NO.	BY:	DATE:	NO.	BY:	DATE:	S-30	
1			3			TOTAL SHEETS	70
2			4				

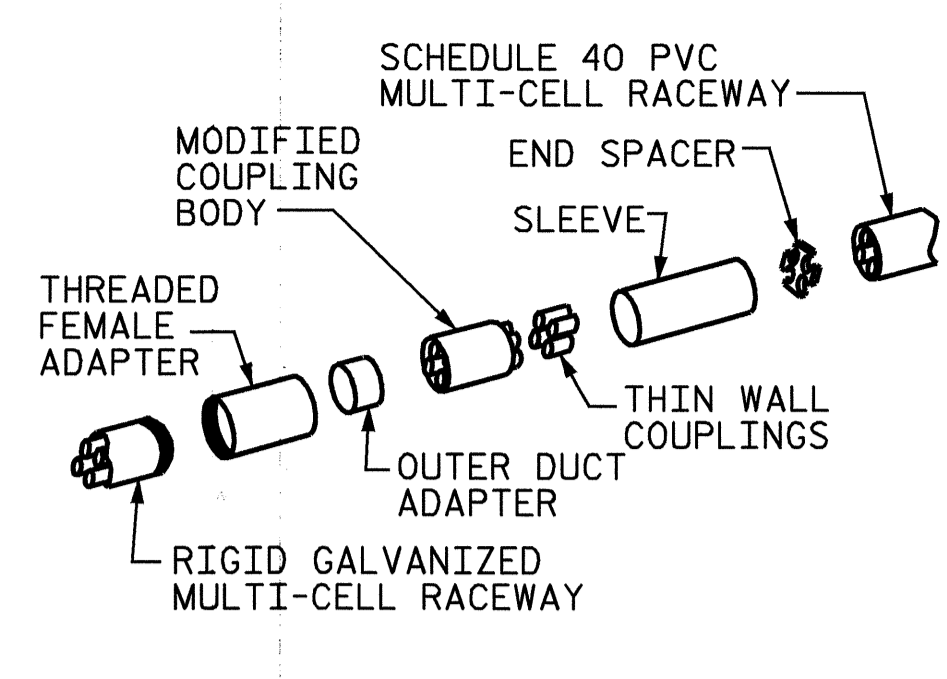
DRAWN BY : S. DOMBROWSKI DATE : 1/07
 CHECKED BY : M.K. BEARD DATE : 2/08



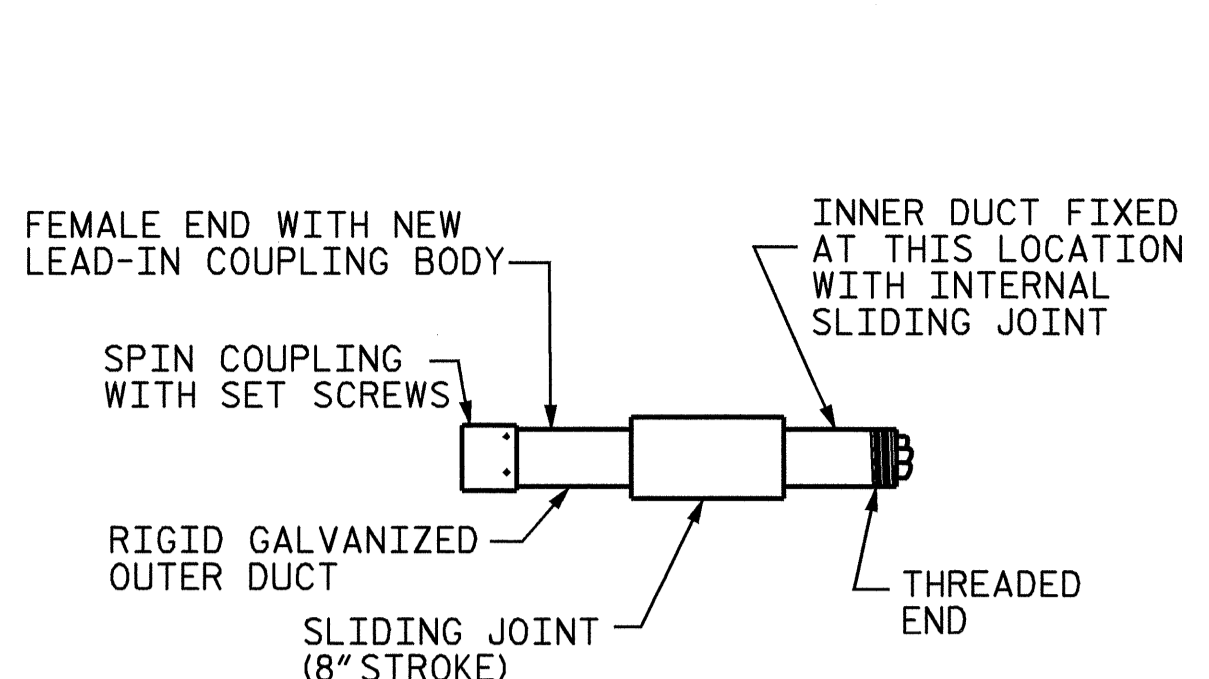
RIGID GALVANIZED (RGC) MULTI-CELL RACEWAY



SCHEDULE 40 PVC MULTI-CELL RACEWAY



TRANSITION ADAPTER



EXPANSION JOINT FITTING

NOTES

THE CONTRACTOR SHALL BE RESPONSIBLE FOR DETERMINING THE TOTAL QUANTITY OF CONDUIT NEEDED TO COMPLETE THE WORK AND THAT THE CONDUIT(S) ARE PLACED AT THE NOTED DIMENSION AND ABOVE THE BOTTOM OF THE GIRDER.

THE INSTALLATION OF THE CONDUIT SYSTEM SHALL BE PAID FOR AS LUMP SUM. THE PRICE SHALL INCLUDE ALL CONDUIT, HANGERS, STABILIZERS, EXPANSION JOINTS, CONCRETE INSERTS, PVC SLEEVES AND ALL NECESSARY HARDWARE TO COMPLETE THE WORK.

THE CONTRACTOR SHALL FIELD VERIFY THAT THE CONDUIT SYSTEM IS NOT IN CONFLICT WITH THE GUARDRAIL POSTS.

SEE DETAIL "C" FOR HANGER ASSEMBLY INSTALLATION.

INSTALL SLEEVES PARALLEL TO GIRDERS. SEE DETAIL "B" FOR SLEEVE INSTALLATION.

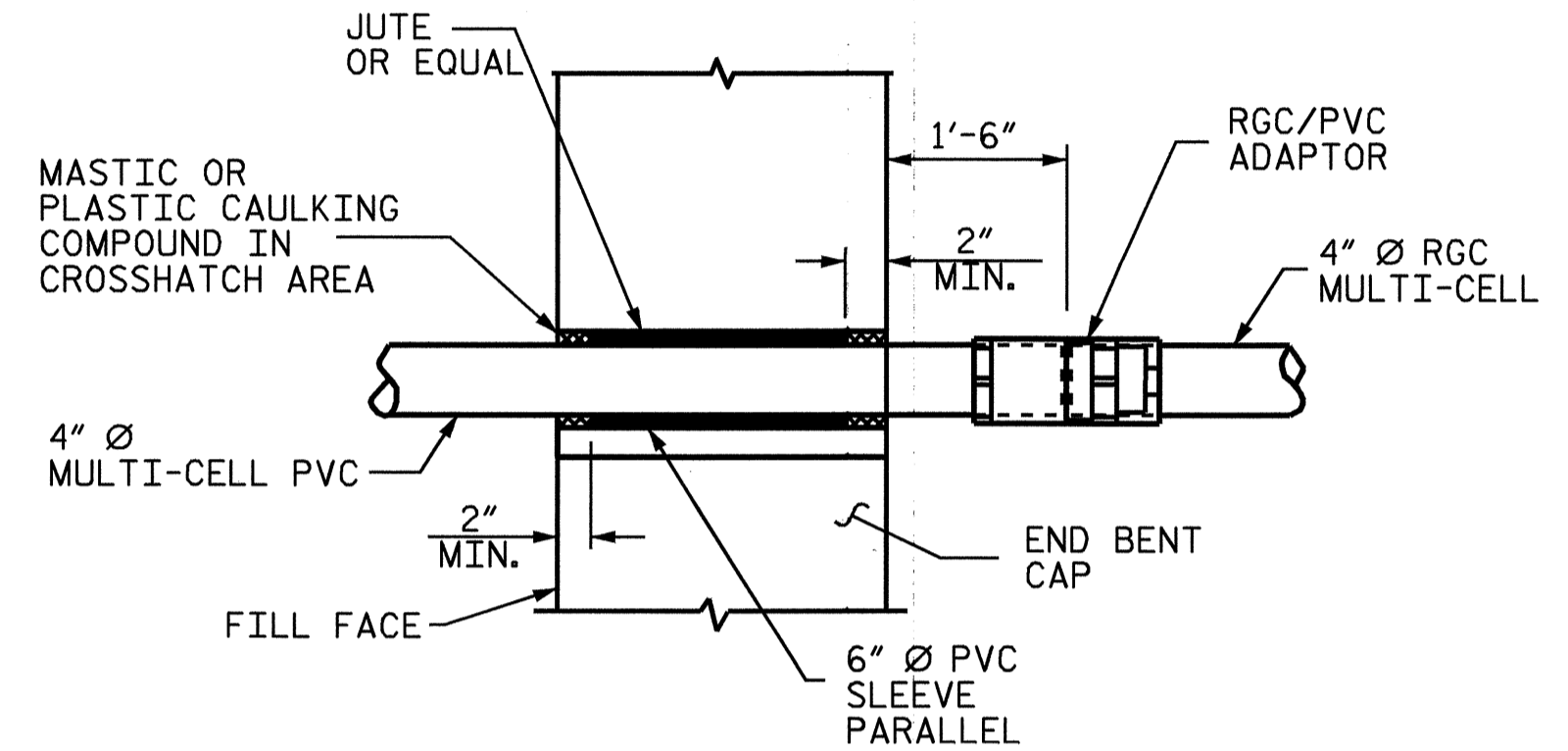
PROVIDE TRANSITION ADAPTOR (AND EXPANSION JOINT) FOR CONDUIT AT END BENT 1 AND END BENT 2.

INSTALL STABILIZER AT OR NEAR MIDSPAN OF SPAN A. STABILIZER CAN NOT BE USED INSTEAD OF A HANGER ASSEMBLY.

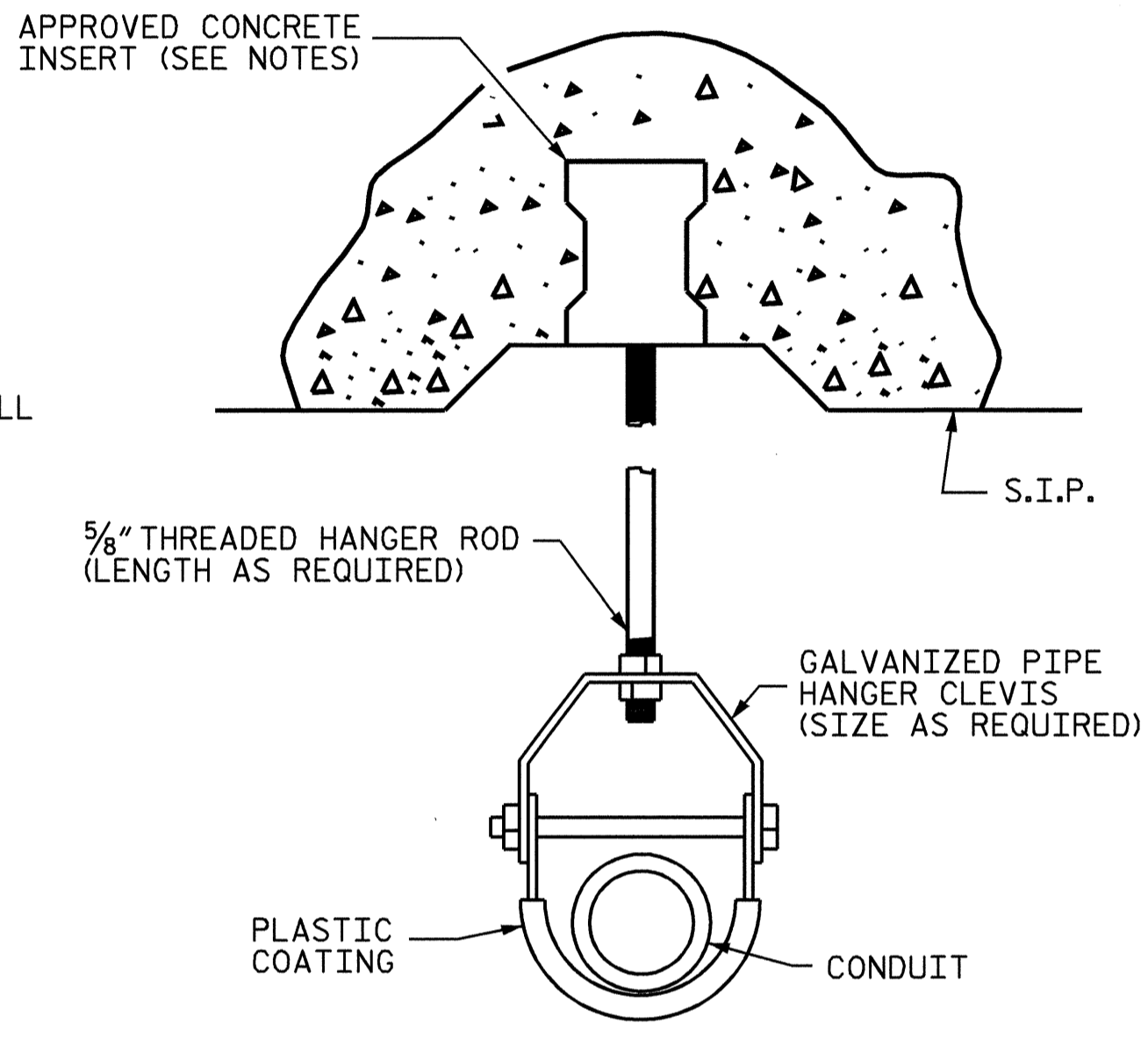
THE CONCRETE SCREW INSERT SHALL HAVE A ROD SIZE OF 5/8" AND A PULL FORCE OF 1260 lbs.

FOR ELECTRICAL CONDUIT SYSTEM, SEE SPECIAL PROVISIONS.

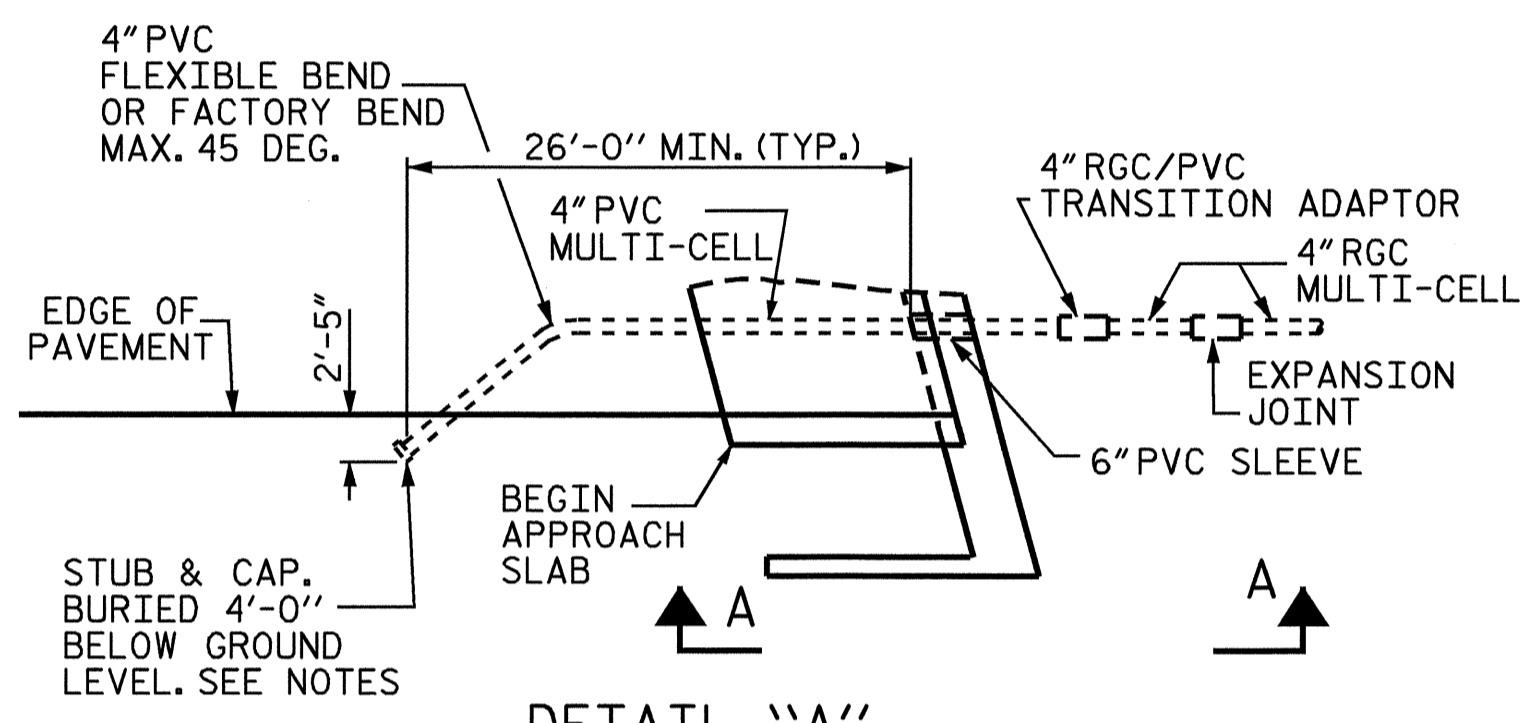
DETAIL "D"
4" MULTI-CELL COMPONENTS



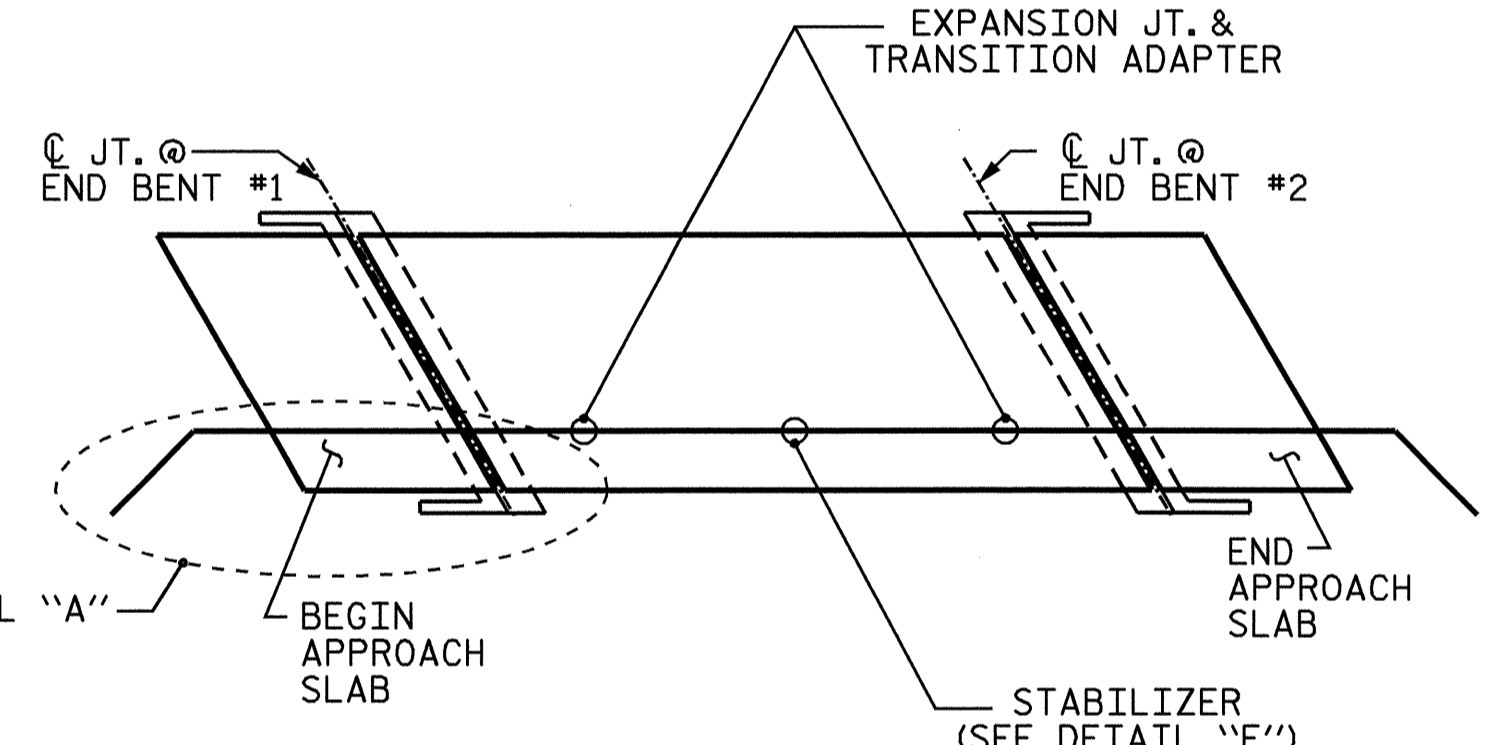
DETAIL "B"
PVC SLEEVE INSTALLATION & RGC/PVC ADAPTOR AT END BENT.



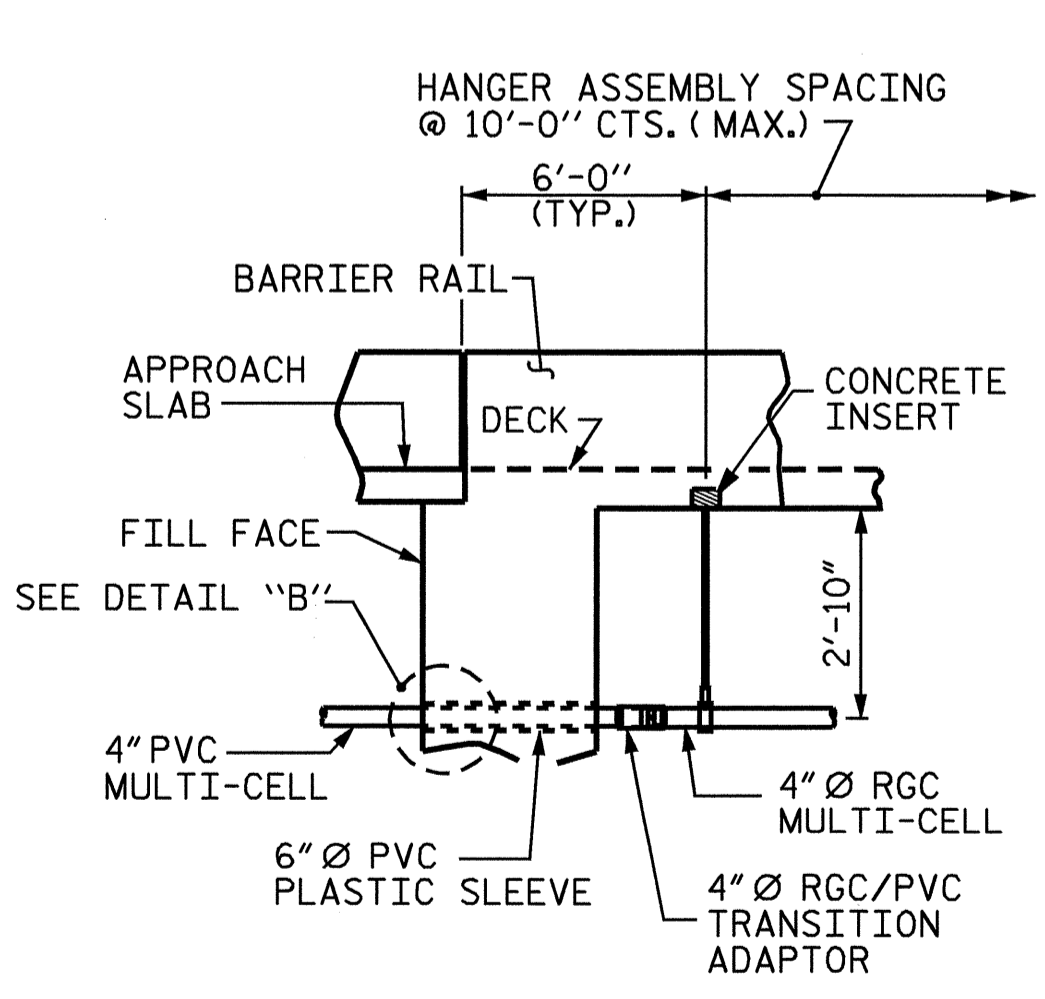
DETAIL "C"
HANGER ASSEMBLY



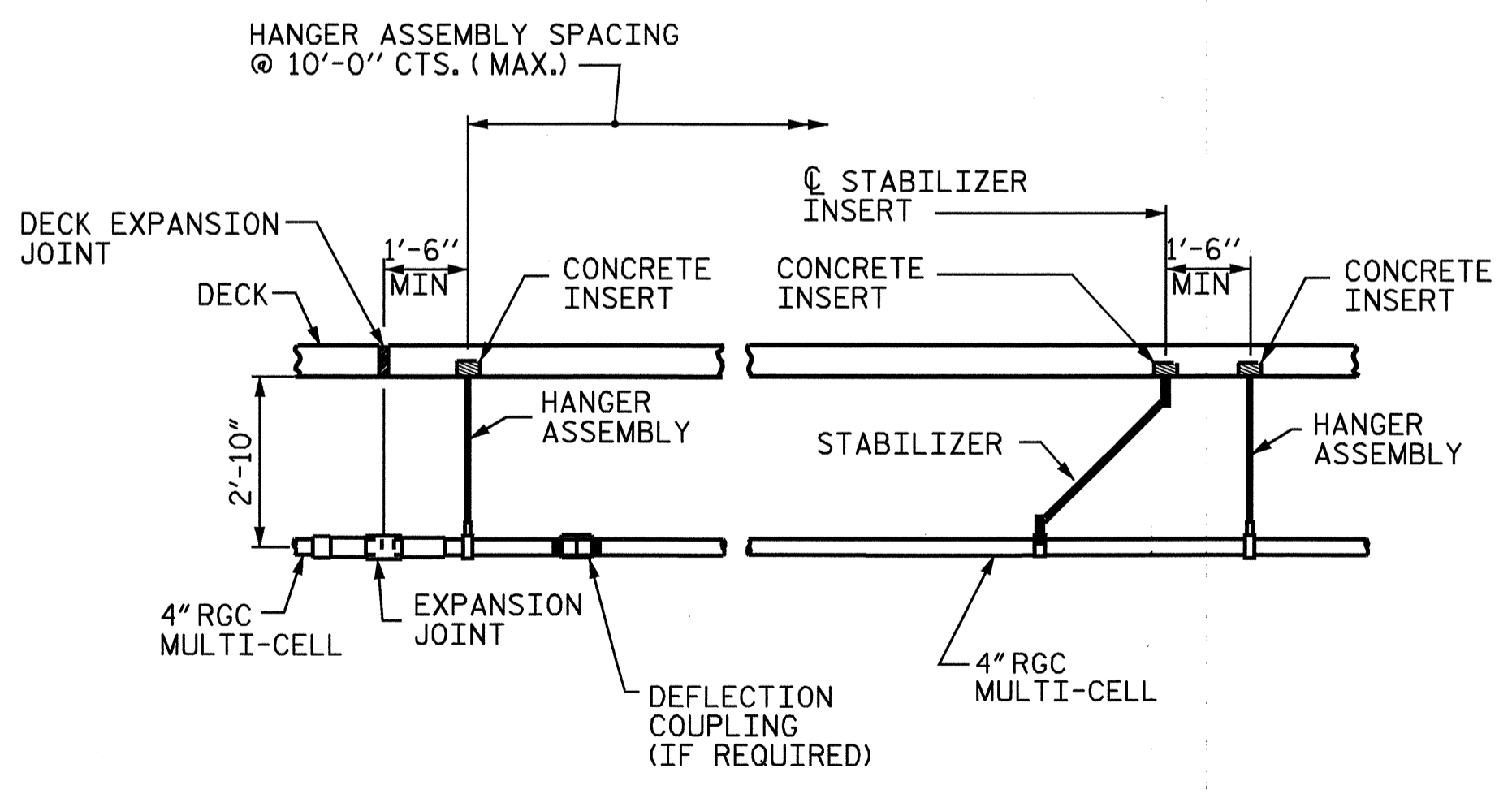
DETAIL "A"
TERMINATION OF CONDUIT AT WING WALL



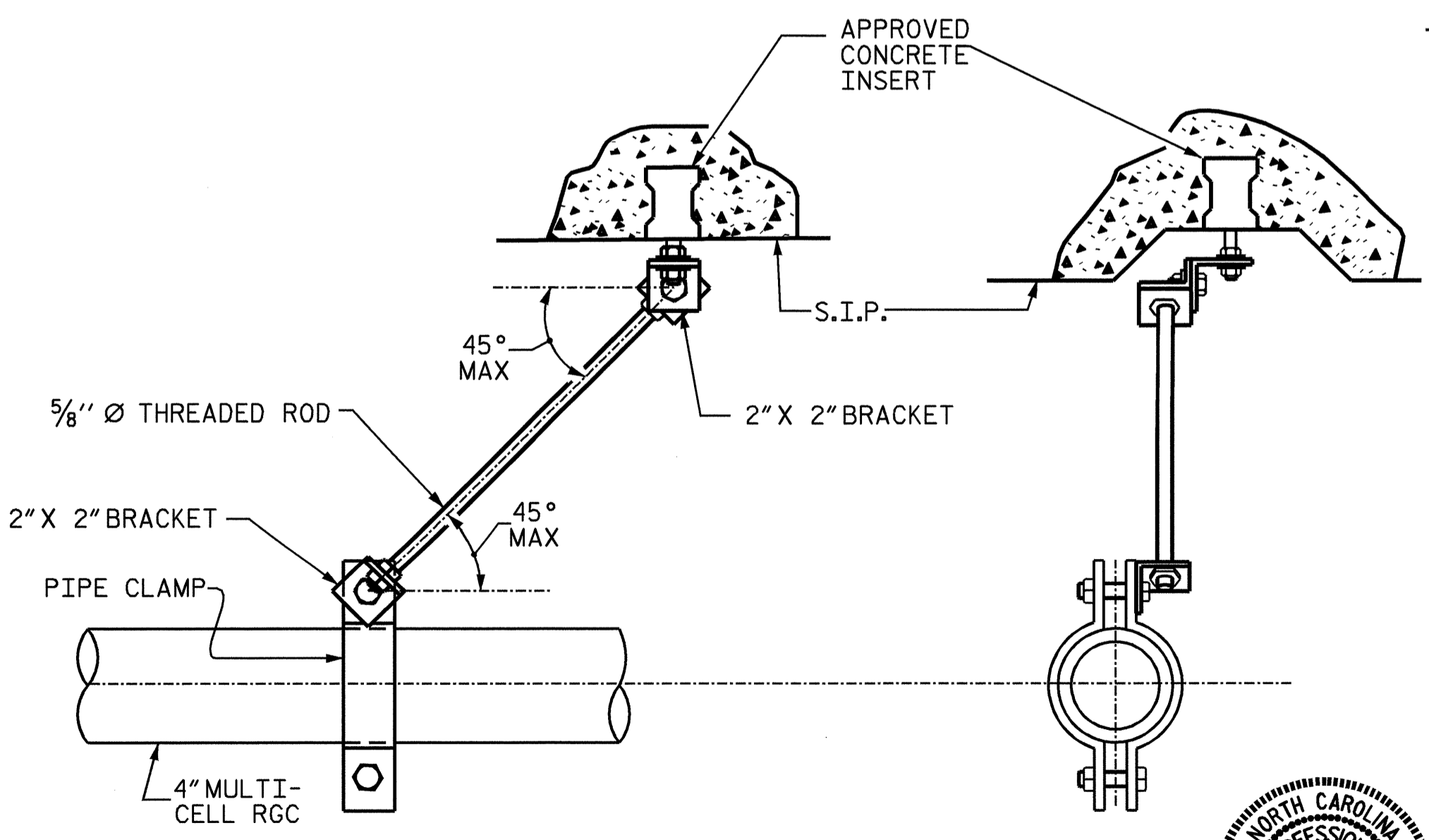
CONDUIT LAYOUT



VIEW A-A



VIEW B-B
STEEL OR CONCRETE GIRDERS

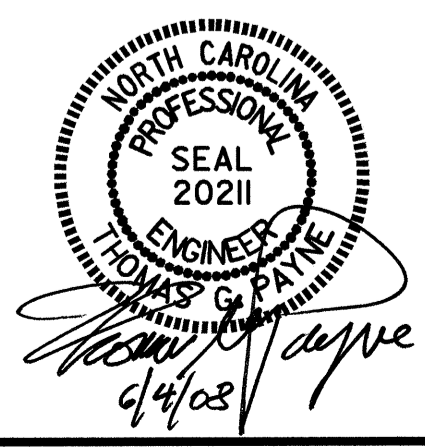


DETAIL "E"
STABILIZER

ELECTRIC CONDUIT DETAILS

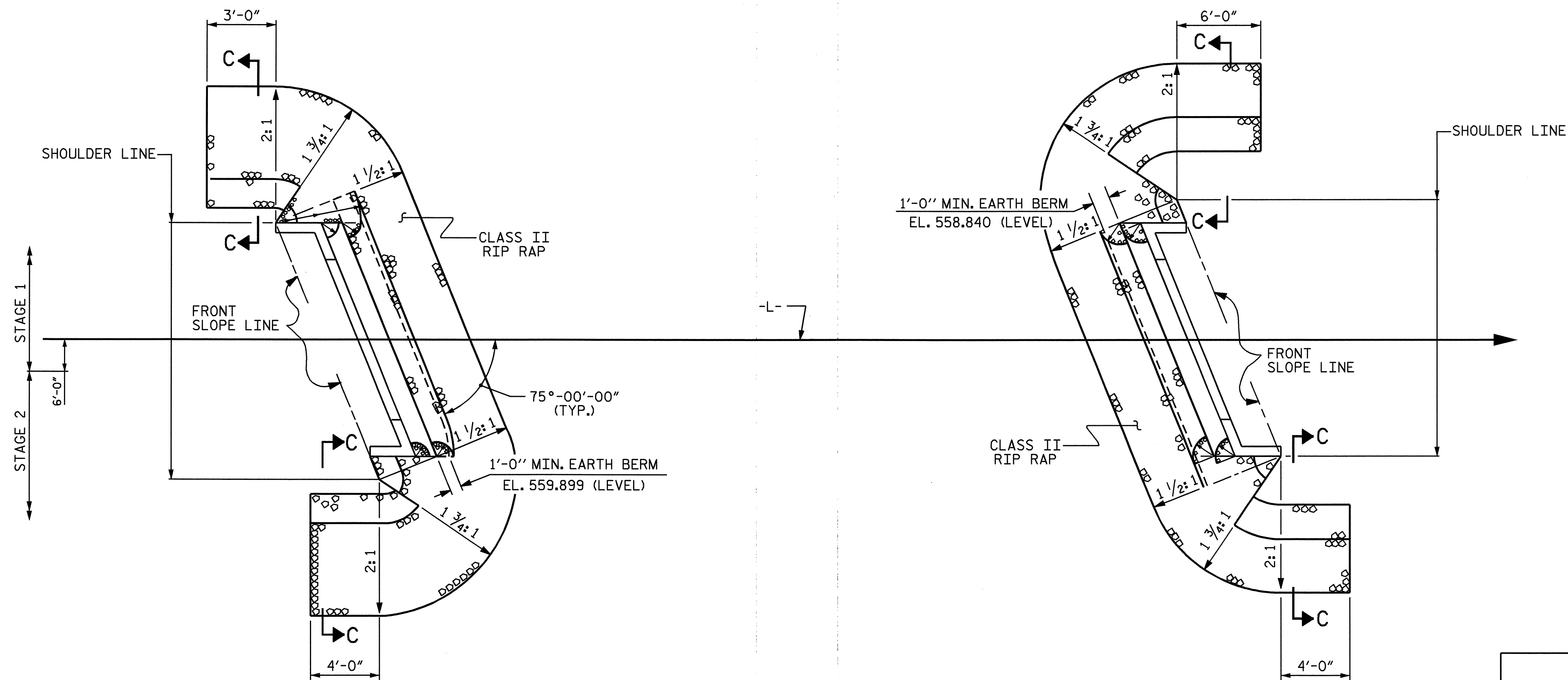
PROJECT NO. B-4252
ROCKINGHAM COUNTY
 STATION: 27+69.00 -L-

STATE OF NORTH CAROLINA
 DEPARTMENT OF TRANSPORTATION
 RALEIGH
 STANDARD
 ELECTRICAL CONDUIT
 SYSTEM DETAILS



ASSEMBLED BY : S. DOMBROWSKI	DATE : 12/07
CHECKED BY : M.K. BEARD	DATE : 1/08
DRAWN BY : RWW 2-4-03	REV. 5/1/06 TLA/GM
CHECKED BY : DBM 2-4-03	

REVISIONS						TOTAL SHEETS
NO.	BY:	DATE:	NO.	BY:	DATE:	70
1			3			
2			4			

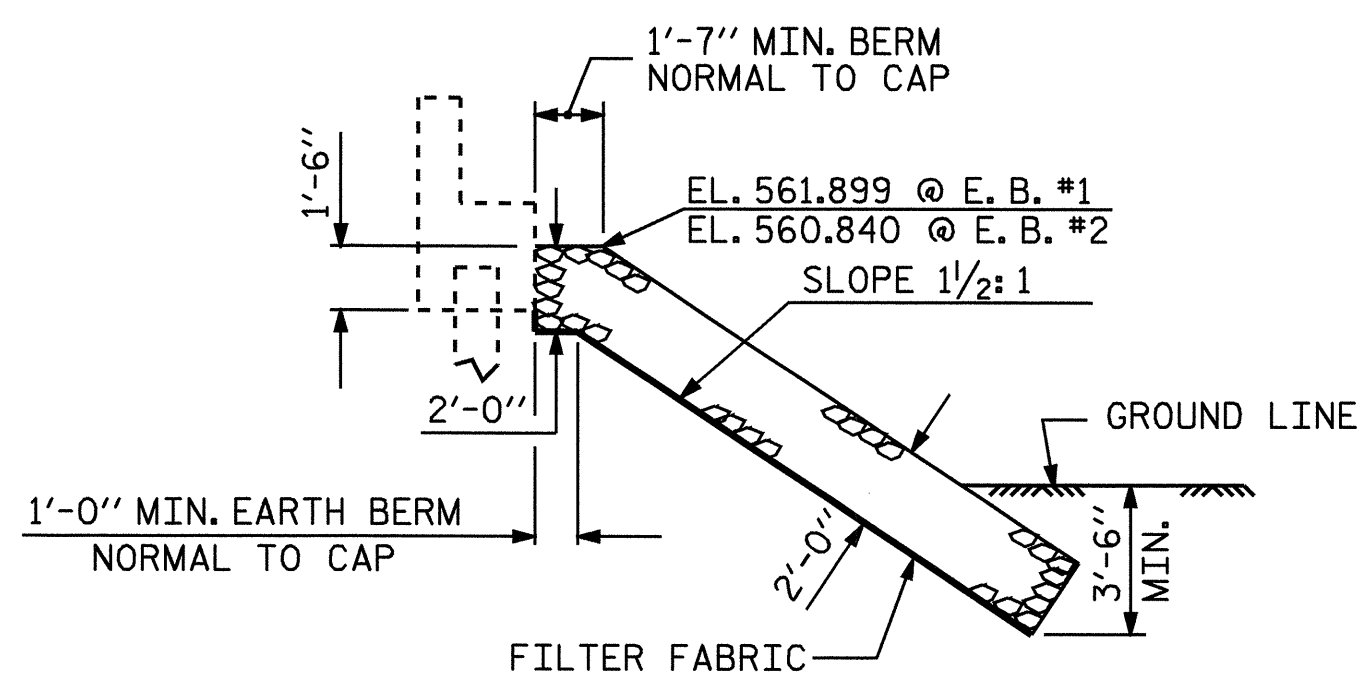


END BENT #1

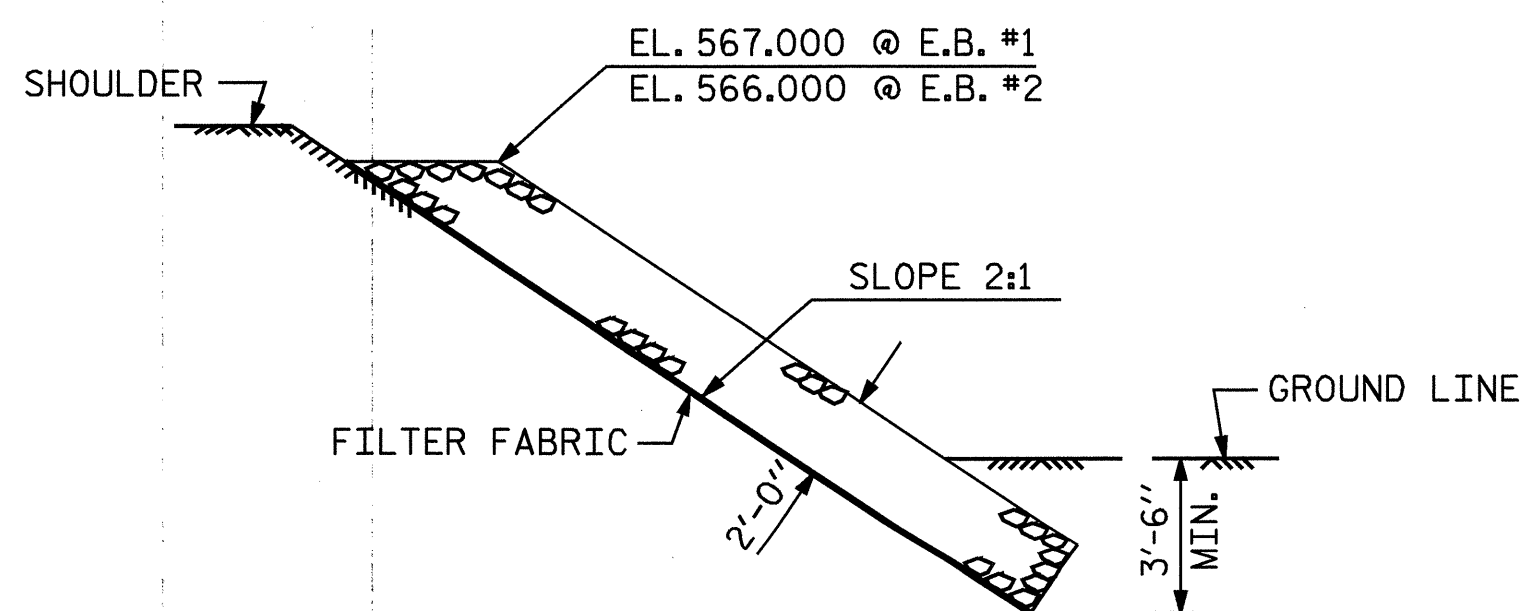
END BENT #2

PLAN OF RIP RAP

ESTIMATED QUANTITIES		
BRIDGE @ STA. 27+69.00 -L-	RIP RAP CLASS II (2'-0" THICK)	FILTER FABRIC FOR DRAINAGE
	TONS	SQUARE YARDS
END BENT #1	265	295
END BENT #2	300	335



SECTION
BERM RIP RAPPED



SECTION C-C

PROJECT NO. B-4252
ROCKINGHAM COUNTY
 STATION: 27+69.00 -L-

STATE OF NORTH CAROLINA
 DEPARTMENT OF TRANSPORTATION
 RALEIGH

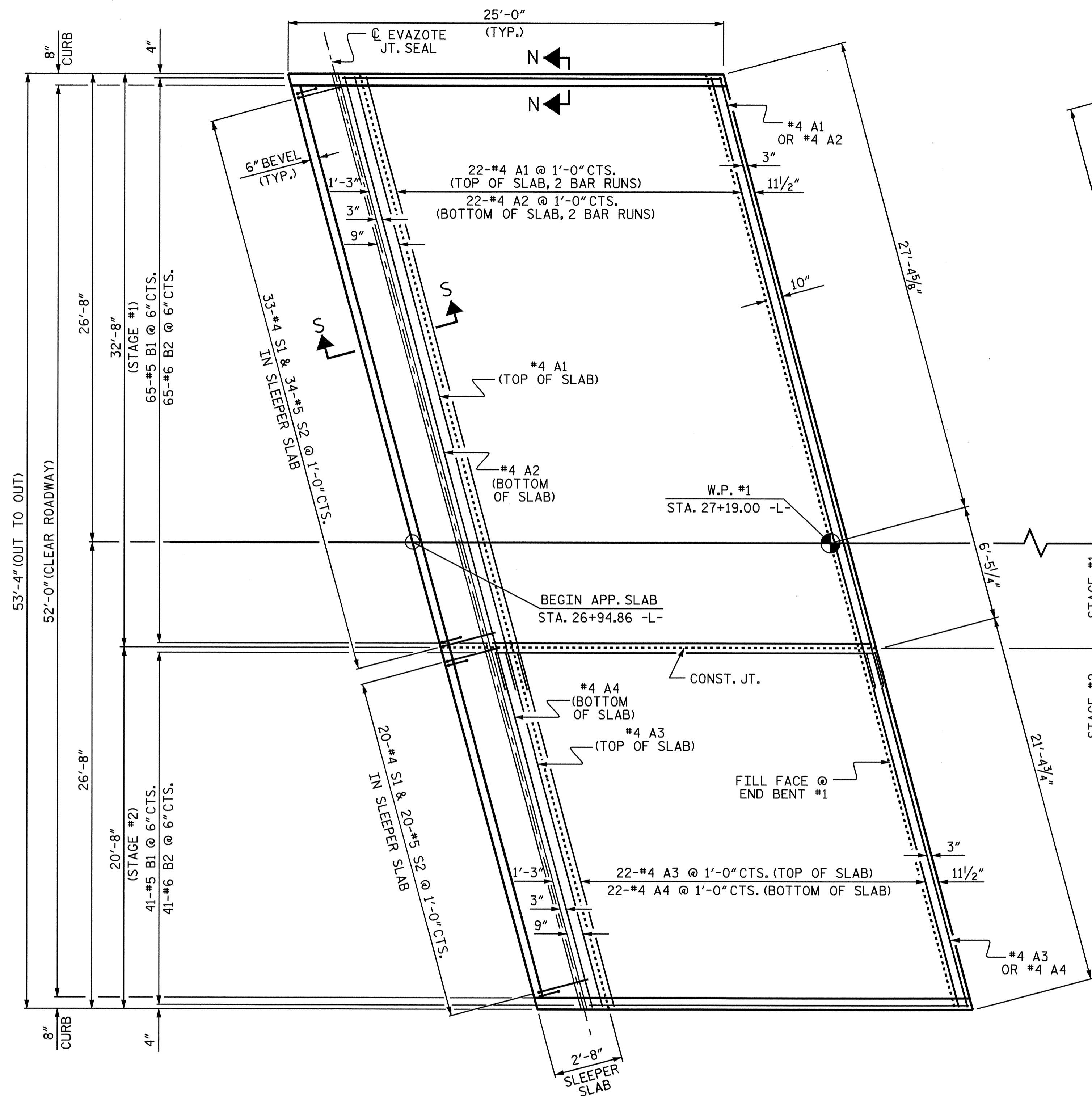
== RIP RAP DETAILS ==



ASSEMBLED BY : R. G. EMERSON DATE : 01/07
 CHECKED BY : M. K. BEARD DATE : 07/07
 DRAWN BY : REK 1/84 REV. 8/16/99 RWW/LES
 CHECKED BY : RDU 1/84 REV. 10/17/00 RWW/LES
 REV. 5/1/06 TLA/GM

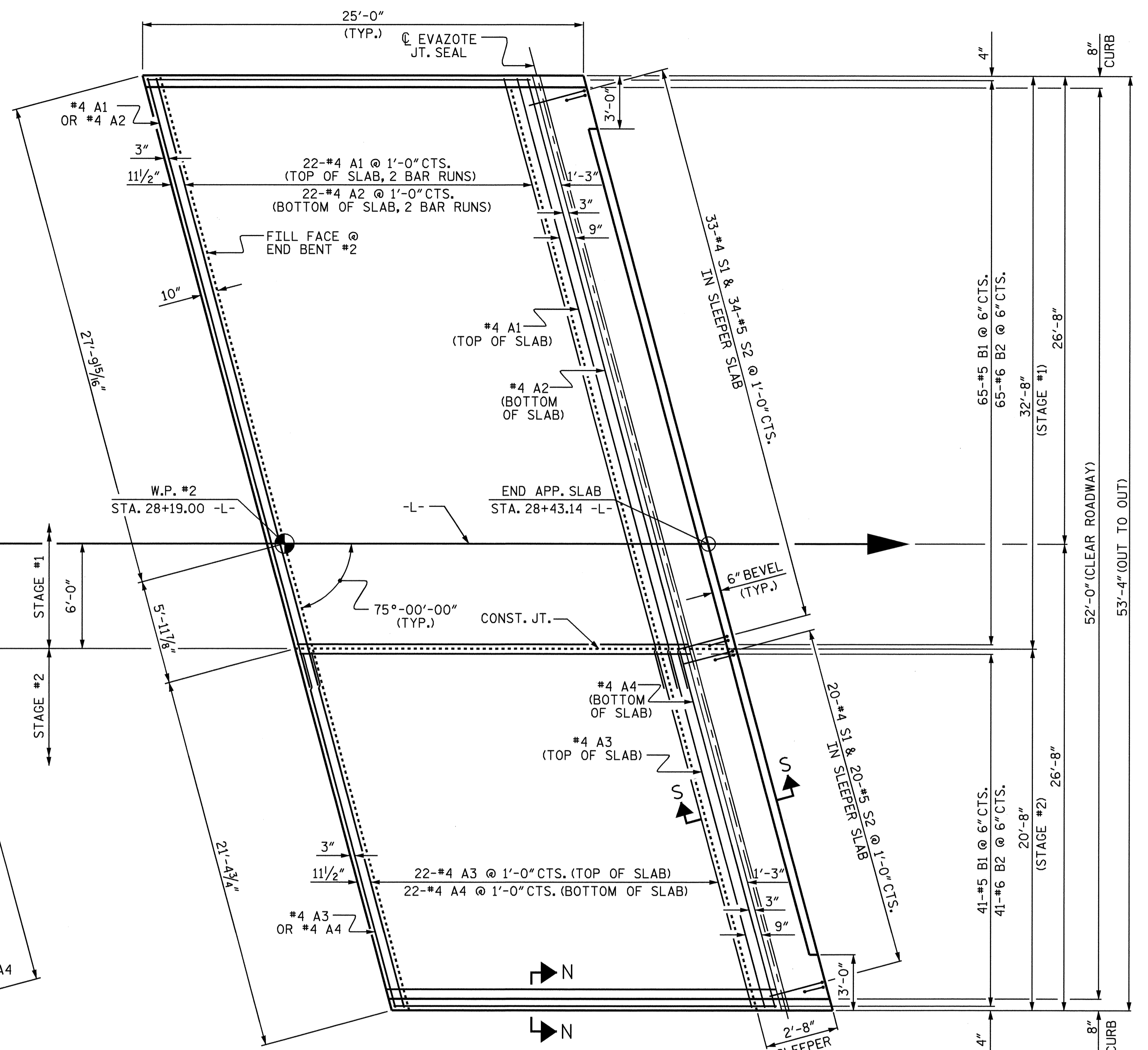
29-MAY-2008 13:23
 K:\Structures\plans\str1\B4252_sd.RR.dgn
 Klayne

REVISIONS						SHEET NO. S-32
NO.	BY:	DATE:	NO.	BY:	DATE:	
1			3			TOTAL SHEETS 70
2			4			

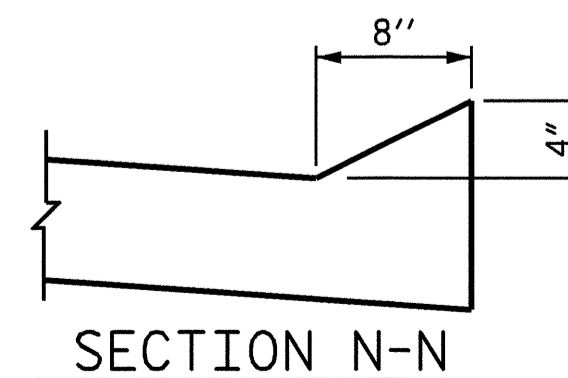


PLAN @ END BENT #1

#4 "A" BARS IN SLEEPER SLAB NOT SHOWN FOR CLARITY.



PLAN @ END BENT #2



EXTEND #4 A1 TOP BARS 2'-2" BEYOND CONST. JT. FOR 2'-0" SPLICE.
EXTEND #4 A2 BOTTOM BARS 1'-11" FOR 1'-9" SPLICE.

PROJECT NO. B-4252
ROCKINGHAM COUNTY
STATION: 27+69.00 -L-

SHEET 1 OF 3

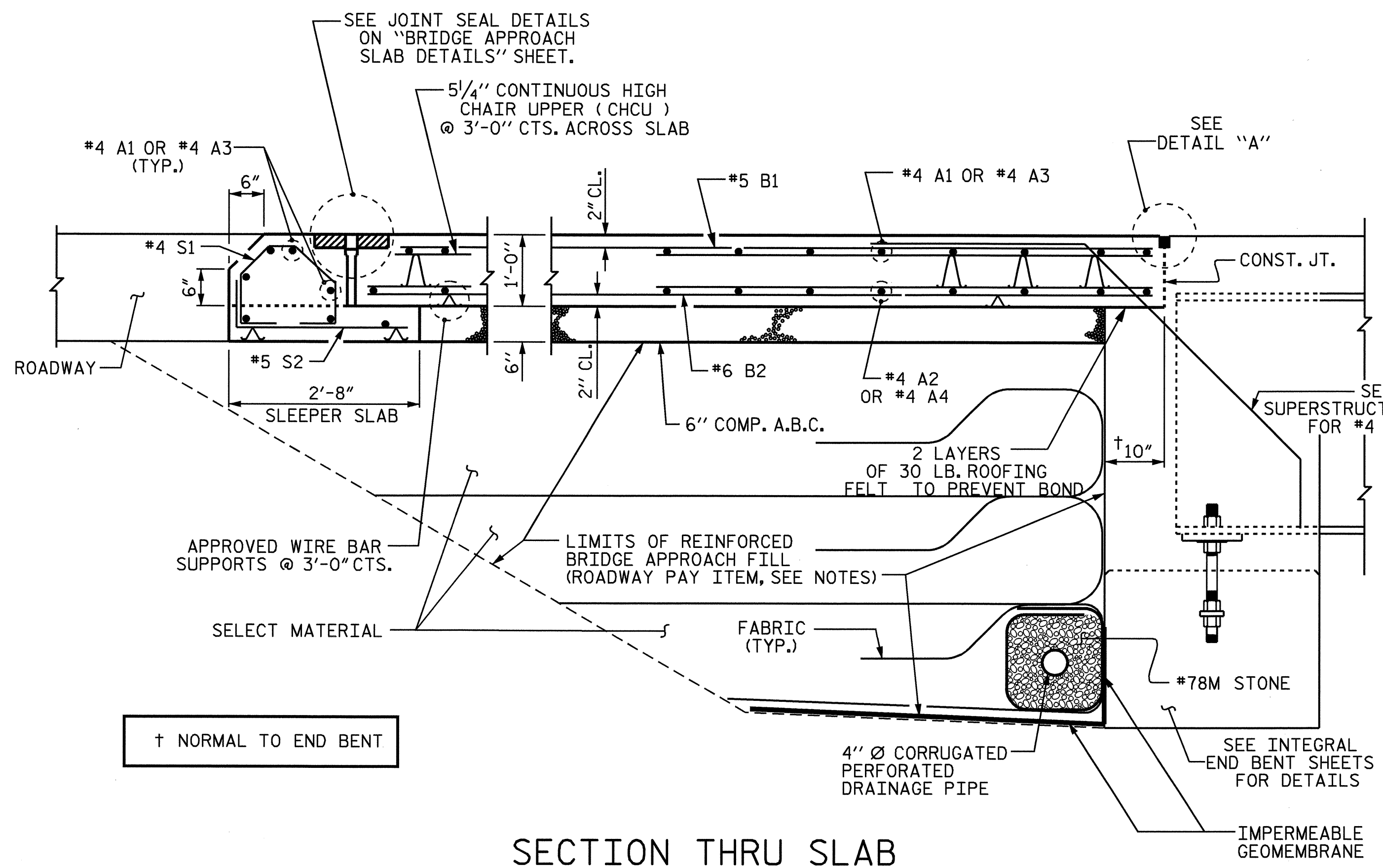


STATE OF NORTH CAROLINA
DEPARTMENT OF TRANSPORTATION
RALEIGH
STANDARD
BRIDGE APPROACH
SLAB FOR INTEGRAL
ABUTMENT
STAGE #1 & STAGE #2

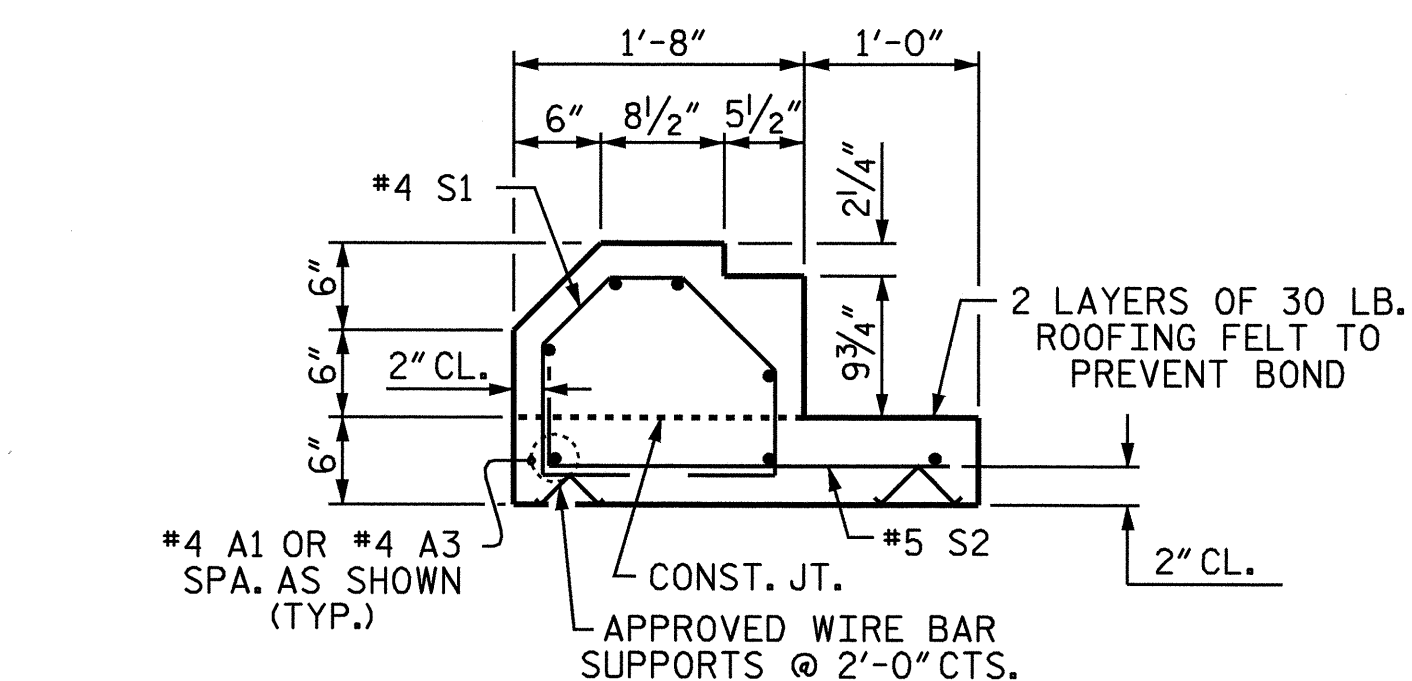
DRAWN BY : R. G. EMERSON DATE : 04/07
CHECKED BY : J. P. ADAMS DATE : 04/07

REVISIONS						SHEET NO.	
NO.	BY:	DATE:	NO.	BY:	DATE:	S-33	
1			3			TOTAL SHEETS	
2			4			70	

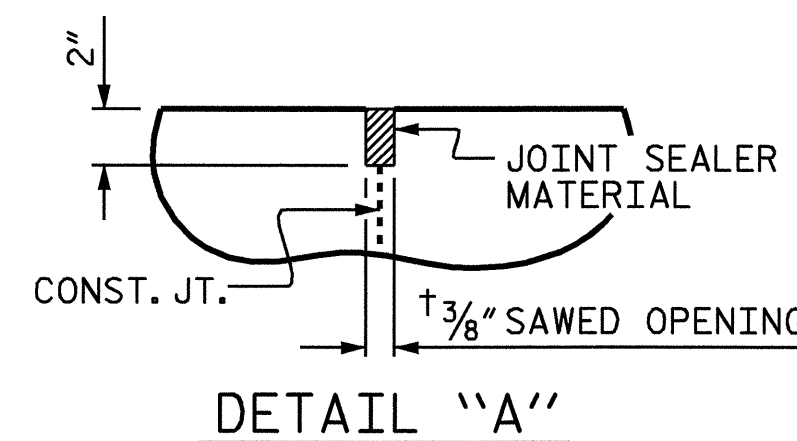
STR. #1



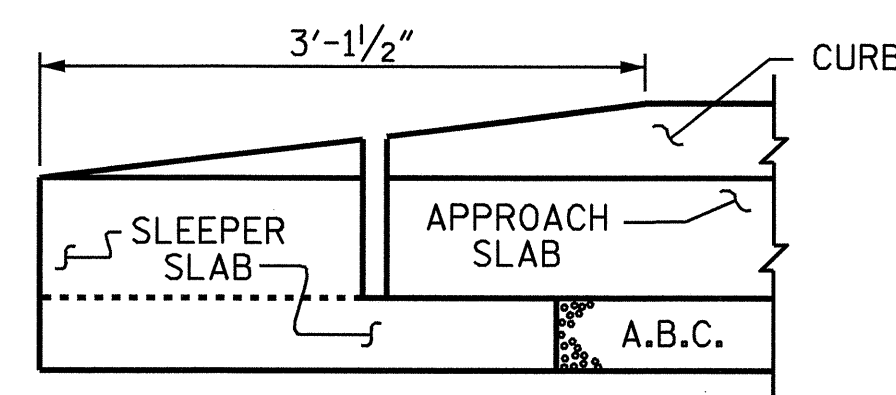
SECTION THRU SLAB



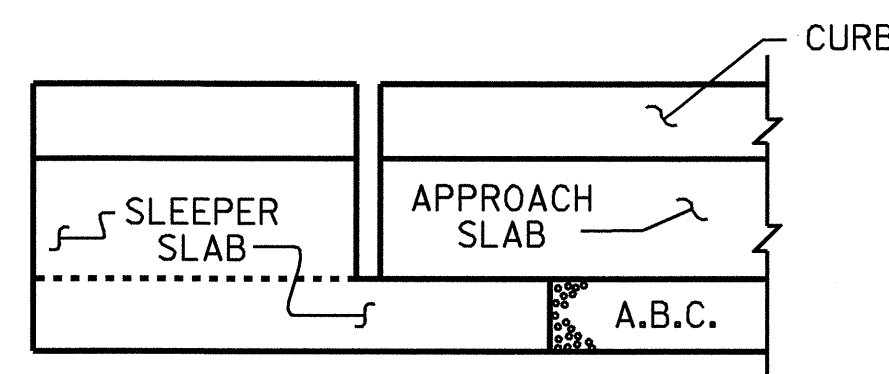
SECTION S-S
SHOWING SLEEPER SLAB



DETAIL "A"



END OF CURB WITHOUT SHOULDER BERM GUTTER



END OF CURB WITH SHOULDER BERM GUTTER

NOTES

APPROACH SLAB SHALL NOT BE CONSTRUCTED PRIOR TO COMPLETION OF THE BRIDGE DECK.

FOR REINFORCED BRIDGE APPROACH FILL INCLUDING FABRIC, IMPERMEABLE GEOMEMBRANE, 4" Ø DRAINAGE PIPE, #78M STONE, AND SELECT MATERIAL, SEE ROADWAY PLANS.

AREA BETWEEN THE WINGWALL AND APPROACH SLAB SHALL BE GRADED TO DRAIN THE WATER AWAY FROM THE FILL FACE OF THE BRIDGE AND SHALL BE PAVED. SEE ROADWAY PLANS.

THE 6" COMP. A.B.C. SHALL EXTEND 10'-0" BEYOND THE END OF THE APPROACH SLAB AND 1'-0" OUTSIDE OF EACH EDGE OF SLAB.

THE CONTRACTOR MAY USE 4" TYPE B-25.0B ASPHALT CONCRETE BASE COURSE IN LIEU OF 6" COMP. A.B.C. IF THIS OPTION IS USED, THE BASE COURSE SHALL EXTEND 1'-0" BEYOND THE END OF THE SLEEPER SLAB AND THE WIDTH SHALL BE THE SAME AS THAT OF THE APPROACH SLAB.

THE CONTRACTOR MAY USE 5" CLASS "A" CONCRETE BASE IN LIEU OF 6" COMP. A.B.C. IF THIS OPTION IS USED, THE CONCRETE BASE SHALL EXTEND 1'-0" BEYOND THE END OF THE SLEEPER SLAB AND THE WIDTH SHALL BE THE SAME AS THAT OF THE APPROACH SLAB. THE CONCRETE SHALL BE FINISHED TO A SMOOTH SURFACE AND A LAYER OF 30 LB ROOFING FELT SHALL BE PLACED BETWEEN THE CONCRETE BASE AND THE APPROACH SLAB TO PREVENT BOND. THE APPROACH SLAB SHALL NOT BE CAST UNTIL THE CONCRETE BASE HAS REACHED AN AGE OF THREE CURING DAYS.

THE VERTICAL JOINT ON THE RIGHT AND LEFT SIDE OF THE APPROACH SLAB AT THE ENDS OF THE EVAZOTE JOINT SHALL BE FILLED WITH SILICONE OR OTHER APPROVED MATERIAL IN ORDER TO PREVENT BACKFILL FROM ENTERING THE JOINT OPENING.

THE JOINT OPENING AT THE APPROACH SLAB/DECK INTERFACE SHALL BE SAWED NO MORE THAN 12 HOURS AFTER THE APPROACH SLAB IS CAST. THE JOINT SHALL BE CLEANED OF ALL DEBRIS BEFORE THE SEALANT IS APPLIED. THE JOINT SEALER MATERIAL SHALL CONFORM TO THE REQUIREMENTS OF TYPE SL LOW MODULUS SILICONE SEALANT.

WITH EVAZOTE JOINT SEAL

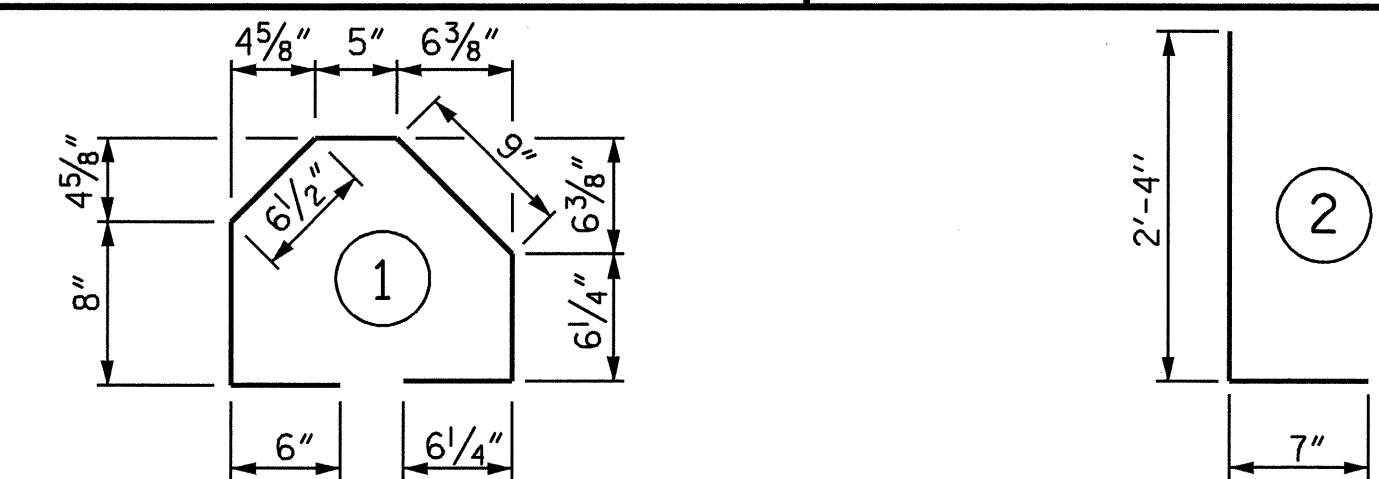
FOR EVAZOTE JOINT SEALS, SEE SPECIAL PROVISIONS.

THE NOMINAL UNCOMPRESSED SEAL WIDTH OF THE EVAZOTE JOINT SEAL SHALL BE 2 3/16".

FOR ELASTOMERIC CONCRETE, SEE SPECIAL PROVISIONS.

BILL OF MATERIAL

APP. SLAB @ END BENT #1 STAGE 1						APP. SLAB @ END BENT #2 STAGE 1					
BAR	NO.	SIZE	TYPE	LENGTH	WEIGHT	BAR	NO.	SIZE	TYPE	LENGTH	WEIGHT
* A1	60	#4	STR	18'-11"	758	* A1	60	#4	STR	18'-11"	758
A2	48	#4	STR	18'-8"	599	A2	48	#4	STR	18'-8"	599
* B1	65	#5	STR	22'-4"	1514	* B1	65	#5	STR	22'-4"	1514
B2	65	#6	STR	22'-9"	2221	B2	65	#6	STR	22'-9"	2115
* S1	33	#4	1	3'-11"	86	* S1	33	#4	1	3'-11"	86
S2	33	#5	2	2'-11"	100	S2	33	#5	2	2'-11"	100
REINFORCING STEEL						REINFORCING STEEL					
LBS. 2920						LBS. 2920					
* EPOXY COATED REINFORCING STEEL						* EPOXY COATED REINFORCING STEEL					
LBS. 2358						LBS. 2358					
CLASS AA CONCRETE						CLASS AA CONCRETE					
POUR #1 - SLAB & CURB						POUR #1 - SLAB & CURB					
C. Y. 29.2						C. Y. 29.2					
POUR #2 - SLEEPER SLAB						POUR #2 - SLEEPER SLAB					
C. Y. 3.5						C. Y. 3.5					
TOTAL						TOTAL					
C. Y. 32.7						C. Y. 32.7					
APP. SLAB @ END BENT #1 STAGE 2						APP. SLAB @ END BENT #2 STAGE 2					
BAR	NO.	SIZE	TYPE	LENGTH	WEIGHT	BAR	NO.	SIZE	TYPE	LENGTH	WEIGHT
* A3	30	#4	STR	21'-1"	423	* A3	30	#4	STR	21'-4"	428
A4	24	#4	STR	21'-1"	338	A4	24	#4	STR	21'-1"	338
* B1	41	#5	STR	22'-4"	955	* B1	41	#5	STR	22'-4"	955
B2	41	#6	STR	22'-9"	1401	B2	41	#6	STR	22'-9"	1401
* S1	20	#4	1	3'-11"	52	* S1	20	#4	1	3'-11"	52
S2	20	#5	2	2'-11"	61	S2	20	#5	2	2'-11"	61
REINFORCING STEEL						REINFORCING STEEL					
LBS. 1800						LBS. 1800					
* EPOXY COATED REINFORCING STEEL						* EPOXY COATED REINFORCING STEEL					
LBS. 1430						LBS. 1435					
CLASS AA CONCRETE						CLASS AA CONCRETE					
POUR #1 - SLAB & CURB						POUR #1 - SLAB & CURB					
C. Y. 18.5						C. Y. 18.5					
POUR #2 - SLEEPER SLAB						POUR #2 - SLEEPER SLAB					
C. Y. 2.2						C. Y. 2.2					
TOTAL						TOTAL					
C. Y. 20.7						C. Y. 20.7					



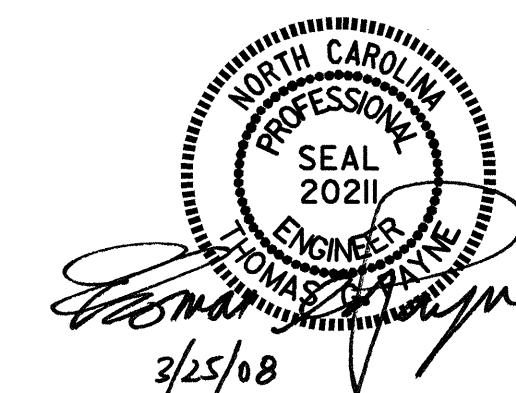
ALL BAR DIMENSIONS ARE OUT TO OUT

SPlice CHART	
#4 A1	2'-0"
#4 A2	1'-9"
#4 A3	2'-0"
#4 A4	1'-9"

PROJECT NO. B-4252
ROCKINGHAM COUNTY
 STATION: 27+69.00 -L-

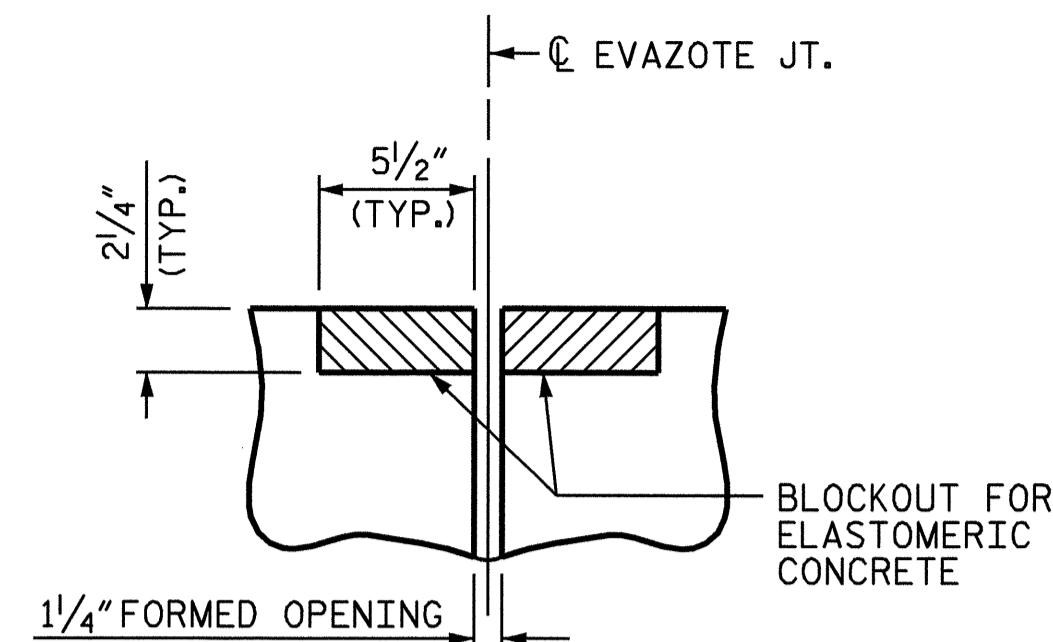
SHEET 2 OF 3

STATE OF NORTH CAROLINA
 DEPARTMENT OF TRANSPORTATION
 RALEIGH
 STANDARD
 BRIDGE
 APPROACH SLAB FOR
 INTEGRAL ABUTMENT

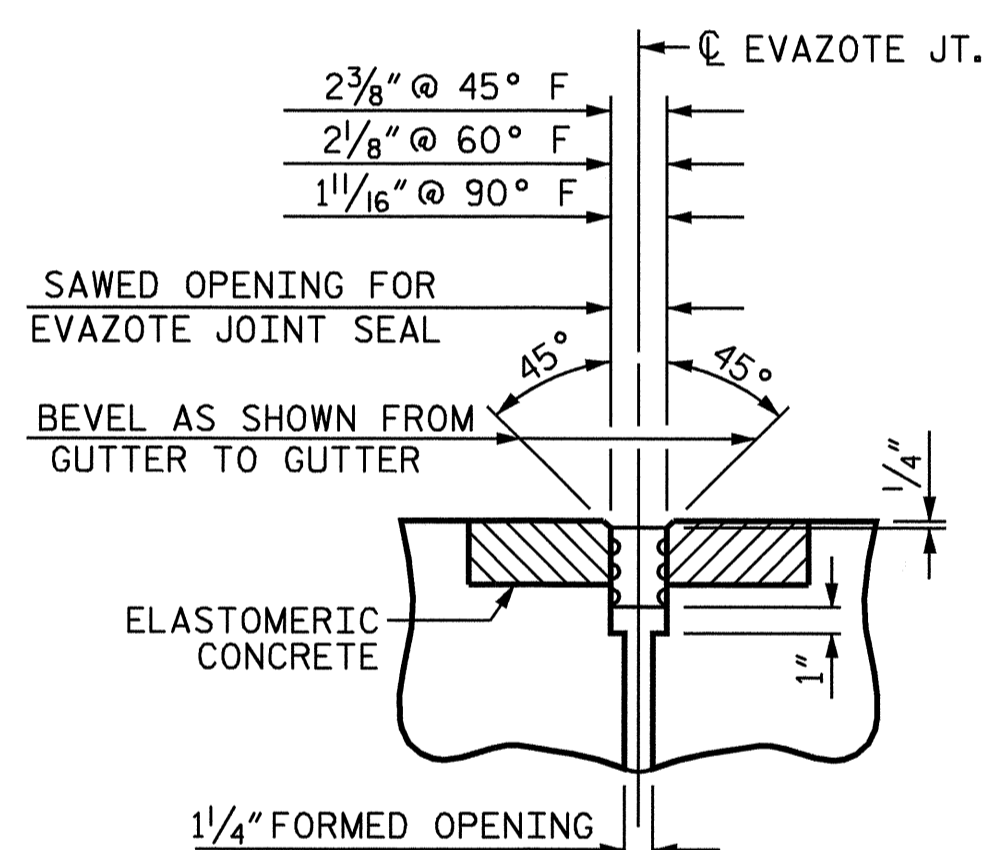


REVISIONS						SHEET NO.	
NO.	BY:	DATE:	NO.	BY:	DATE:	S-34	
1			3			TOTAL SHEETS	
2			4			70	

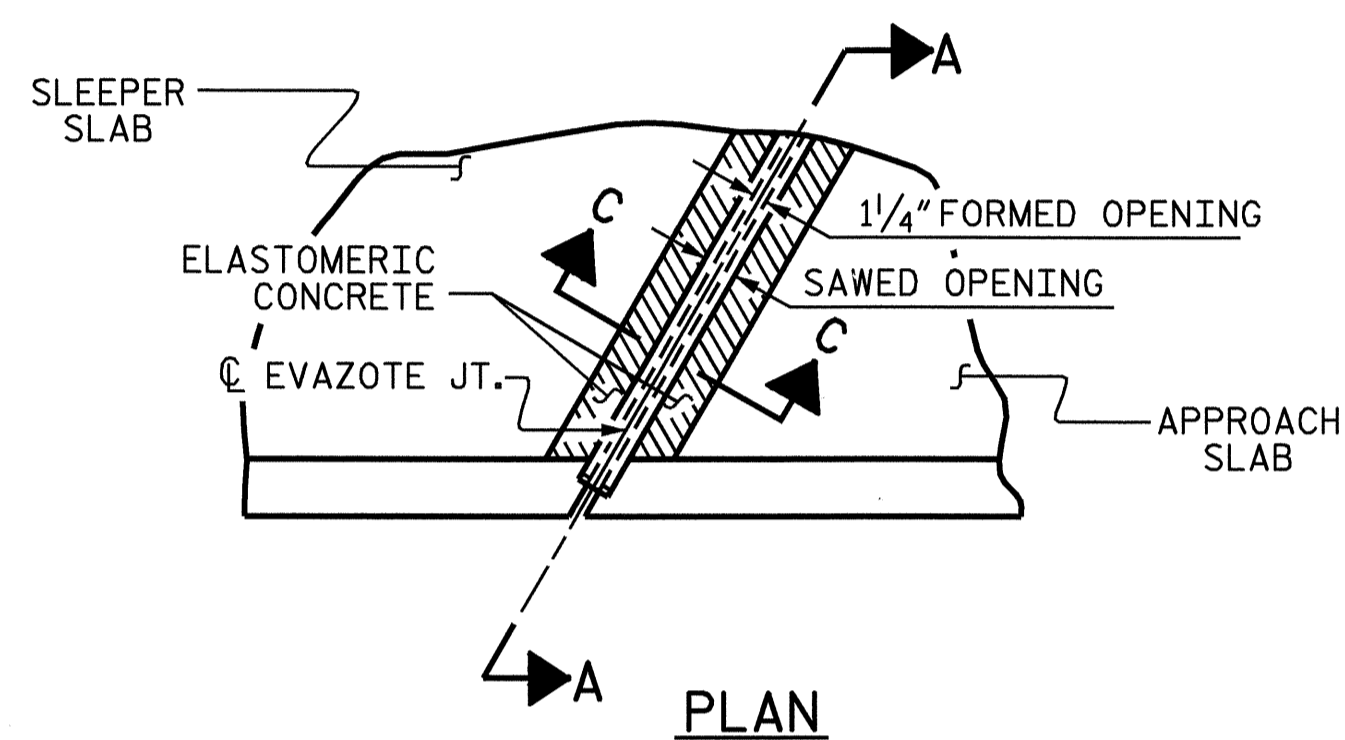
ASSEMBLED BY: R. G. EMERSON DATE: 03/07
 CHECKED BY: J. P. ADAMS DATE: 03/07
 DRAWN BY: TLA 10/05
 CHECKED BY: GM 5/06
 ADDED 5/1/06R KMM/GM



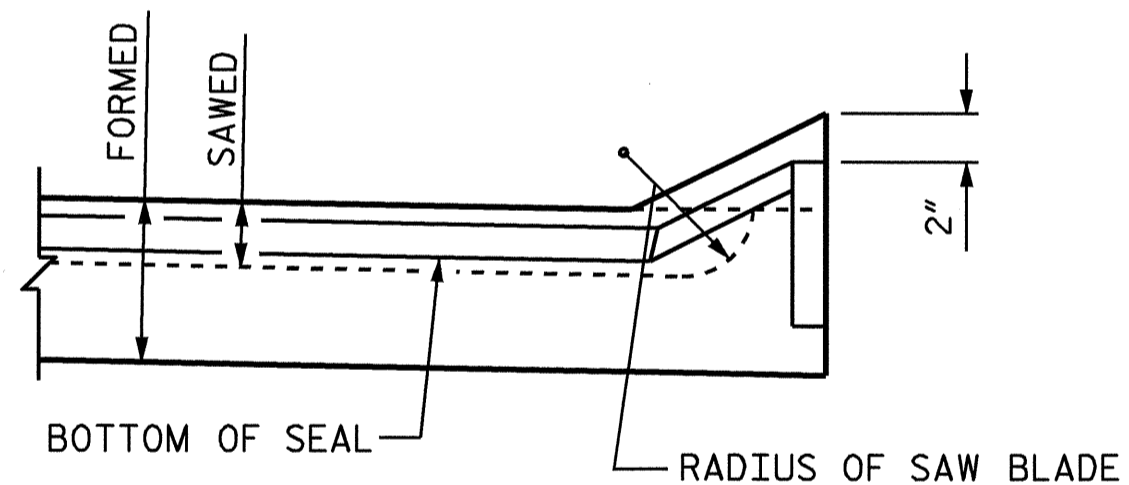
SECTION C-C
EVAZOTE JOINT SEAL
(PRE-SAWED ELASTOMERIC
CONCRETE DIMENSIONS)



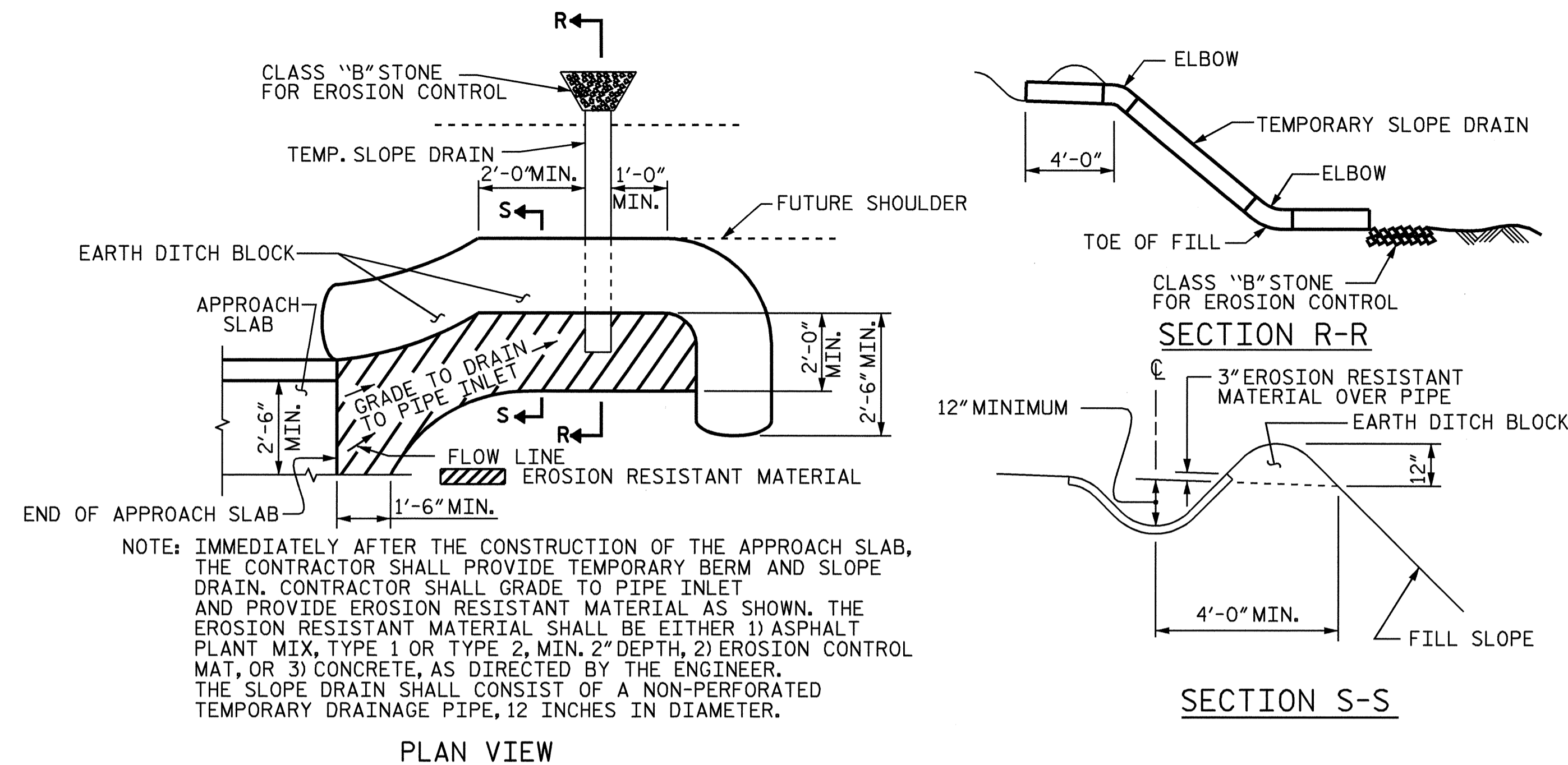
SECTION C-C
EVAZOTE JOINT SEAL



PLAN

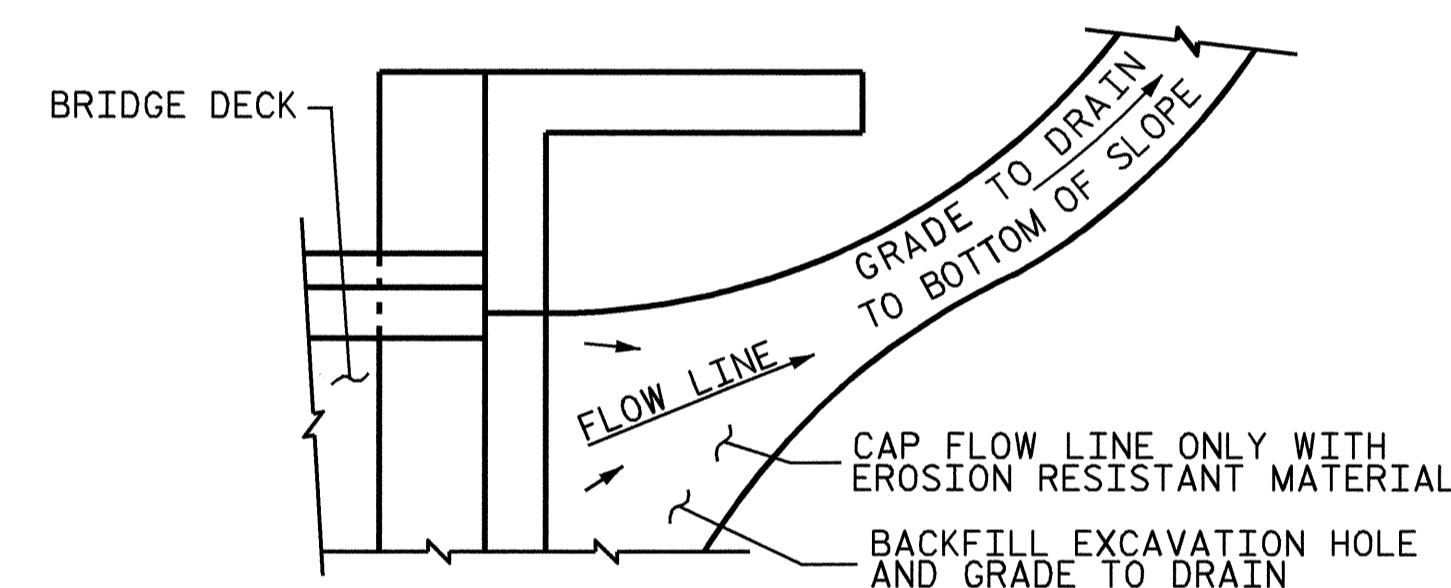


SECTION A-A



TEMPORARY BERM AND SLOPE DRAIN DETAILS

(TO BE USED WHEN SHOULDER BERM GUTTER IS REQUIRED)



NOTE: IF THE APPROACH SLAB IS NOT CONSTRUCTED IMMEDIATELY AFTER THE BACKFILLING OF THE END BENT EXCAVATION, GRADE TO DRAIN TO THE BOTTOM OF THE SLOPE AND PROVIDE EROSION RESISTANT MATERIAL, SUCH AS FIBERGLASS ROVING OR AS DIRECTED BY THE ENGINEER TO PREVENT SOIL EROSION AND TO PROTECT THE AREA ADJACENT TO THE STRUCTURE. THE CONTRACTOR WILL BE REQUIRED TO REMOVE THESE MATERIALS PRIOR TO CONSTRUCTION OF THE APPROACH SLAB.

TEMPORARY DRAINAGE DETAIL

ELASTOMERIC CONCRETE			
END BENT NO.	STAGE #1 (CU. FT.)	STAGE #2 (CU. FT.)	TOTAL ELASTOMERIC CONCRETE * (CU. FT.)
1	5.8	3.7	9.5
2	5.8	3.7	9.5
TOTAL	11.6	7.4	19.0

* BASED ON THE MINIMUM BLOCKOUT SHOWN.

PROJECT NO. B-4252
ROCKINGHAM COUNTY
STATION: 27+69.00 -L-

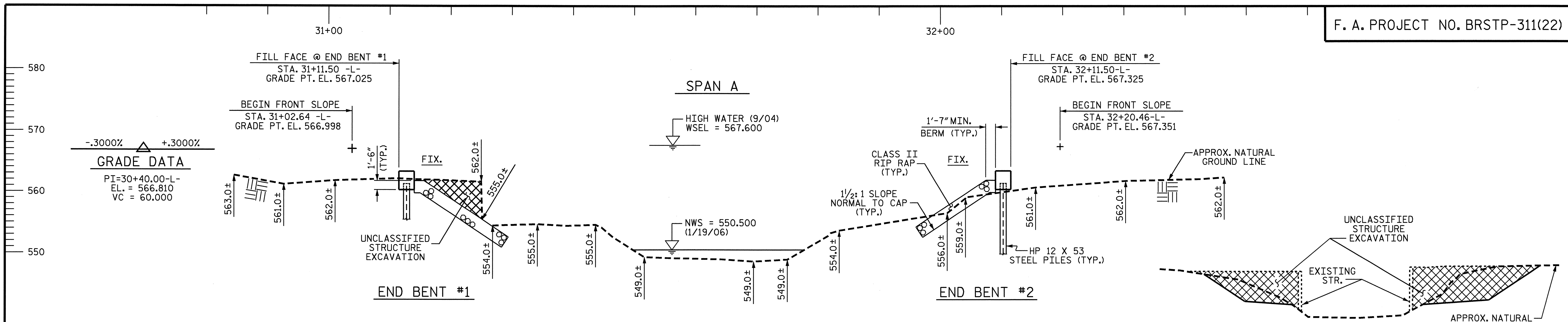
SHEET 3 OF 3

STATE OF NORTH CAROLINA
DEPARTMENT OF TRANSPORTATION
RALEIGH
STANDARD
BRIDGE APPROACH
SLAB DETAILS



REVISIONS						1988
NO.	BY:	DATE:	NO.	BY:	DATE:	SHEET NO.
1			3			S-35
2			4			TOTAL SHEETS 70

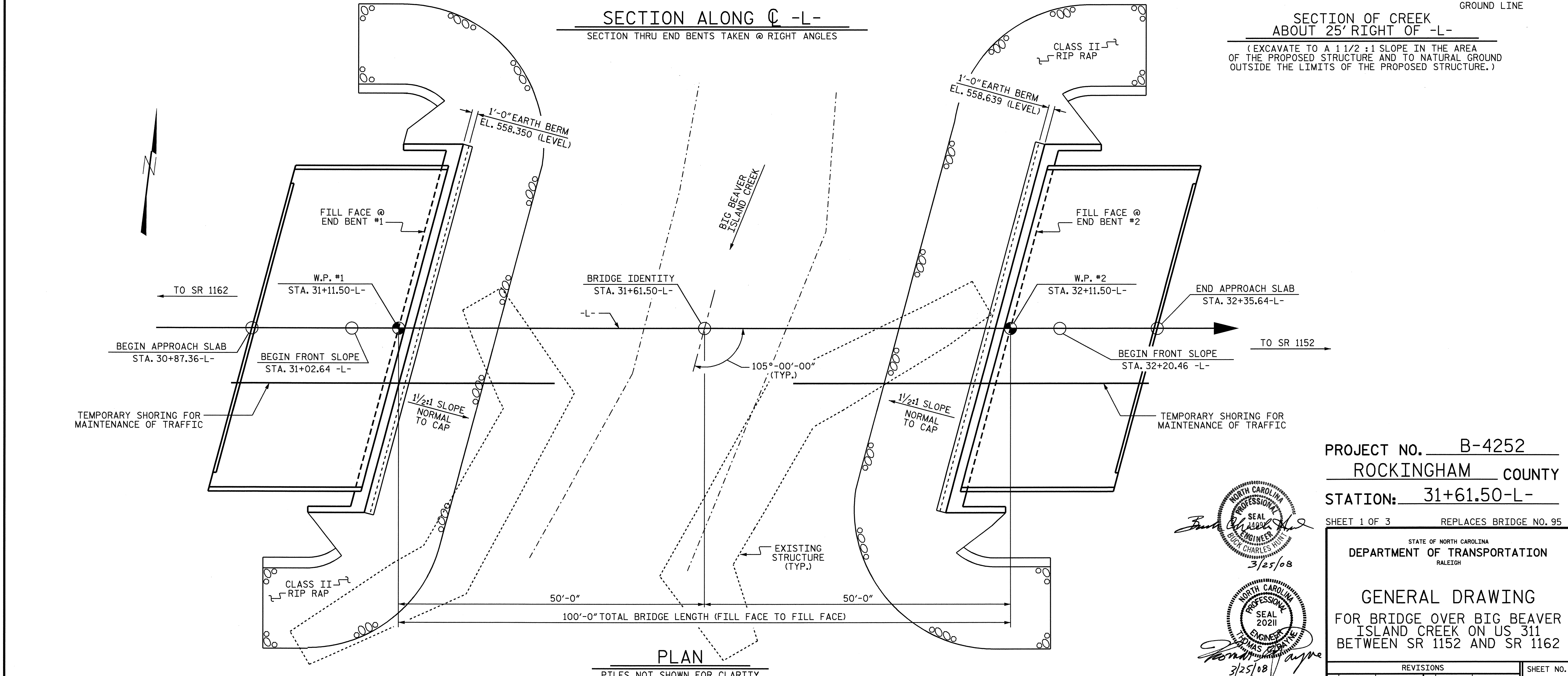
ASSEMBLED BY : R. G. EMERSON DATE : 03/07
CHECKED BY : J. P. ADAMS DATE : 03/07
DRAWN BY : FCJ 11/88 REV. 10/17/00 RWW/LES
CHECKED BY : ARB 11/88 REV. 5/1/03 RWW/JTE
REV. 5/1/06 TLA/GM



GRADE DATA
-0.3000% +0.3000%
PI=30+40.00-L-
EL. = 566.810
VC = 60.000

SECTION ALONG C -L-
SECTION THRU END BENTS TAKEN @ RIGHT ANGLES

SECTION OF CREEK ABOUT 25' RIGHT OF -L-
(EXCAVATE TO A 1 1/2 : 1 SLOPE IN THE AREA OF THE PROPOSED STRUCTURE AND TO NATURAL GROUND OUTSIDE THE LIMITS OF THE PROPOSED STRUCTURE.)



DRAWN BY : R. G. EMERSON DATE : 01/08
CHECKED BY : K. D. LAYNE DATE : 01/08

20-MAR-2008 15:34
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Klayne

PROFESSIONAL ENGINEER SEAL
NORTH CAROLINA
SEAL
2021
THOMAS
3/25/08

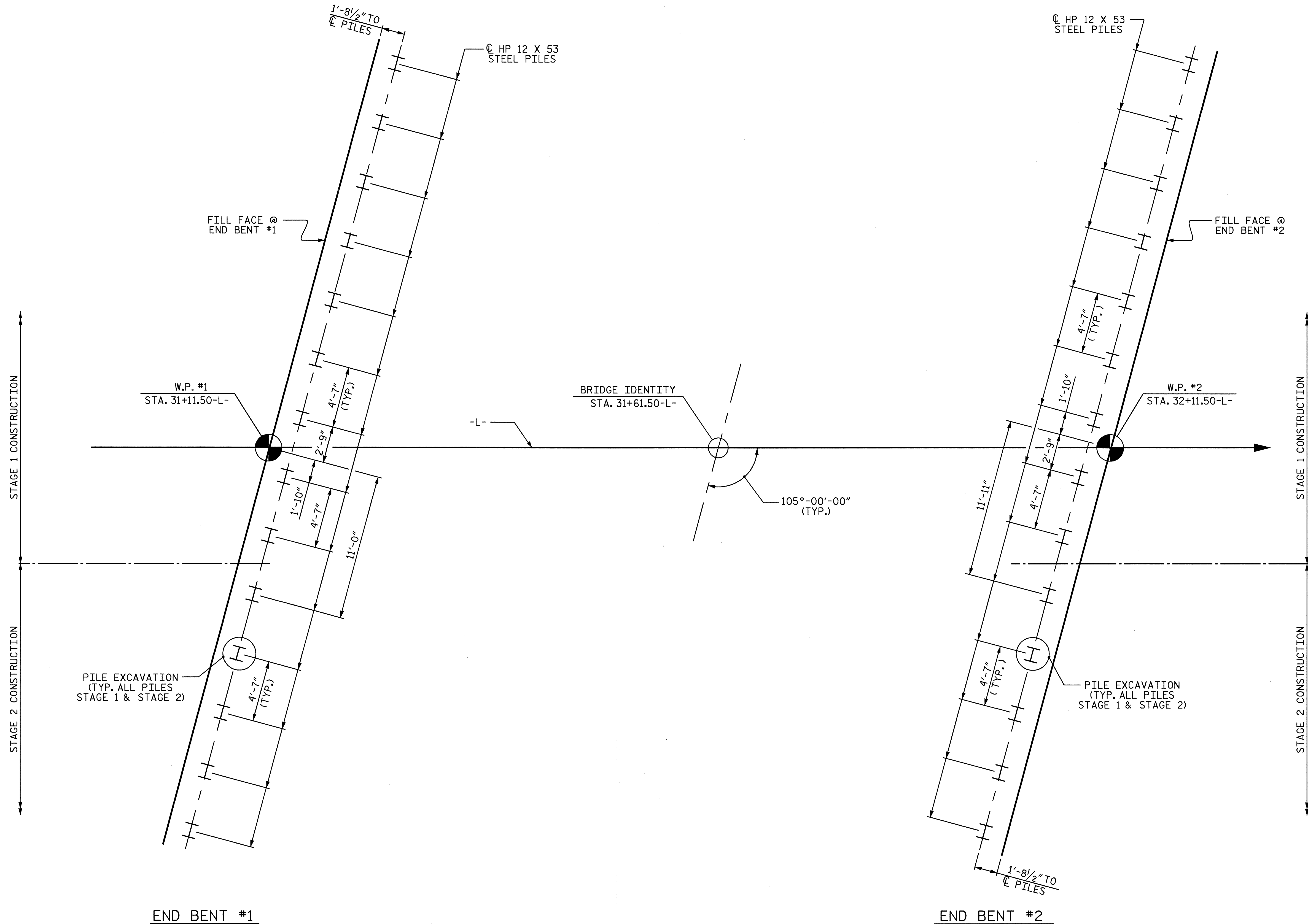
PROJECT NO. B-4252
ROCKINGHAM COUNTY
STATION: 31+61.50-L-
SHEET 1 OF 3 REPLACES BRIDGE NO. 95

STATE OF NORTH CAROLINA
DEPARTMENT OF TRANSPORTATION
RALEIGH

GENERAL DRAWING
FOR BRIDGE OVER BIG BEAVER
ISLAND CREEK ON US 311
BETWEEN SR 1152 AND SR 1162

REVISIONS						SHEET NO.
NO.	BY:	DATE:	NO.	BY:	DATE:	S-36
1			3			TOTAL SHEETS
2			4			70

STR.#2



FOUNDATION LAYOUT

DIMENSIONS LOCATING PILES ARE SHOWN TO THE CENTERLINE OF PILES.

NOTES: DRIVE PILES AT END BENT #1 AND END BENT #2 TO A REQUIRED BEARING CAPACITY OF 100 TONS PER PILE. THE REQUIRED BEARING CAPACITY IS EQUAL TO THE ALLOWABLE BEARING CAPACITY WITH A MINIMUM FACTOR OF SAFETY OF TWO.

THE ALLOWABLE BEARING CAPACITY FOR PILES AT END BENT #1 AND END BENT #2 IS 50 TONS PER PILE.

DRIVE PILES AT END BENT #1 AND END BENT #2 TO A TIP ELEVATION NO HIGHER THAN 544.000.

PILE EXCAVATION IS REQUIRED TO INSTALL PILES AT END BENT #1 AND END BENT #2. EXCAVATE HOLES TO ELEVATION 544.000. SEE PILE EXCAVATION FOR INTEGRAL ABUTMENT SPECIAL PROVISION.

DRAWN BY : R. G. EMERSON DATE : 01/08
 CHECKED BY : K. D. LAYNE DATE : 01/08

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 klayne

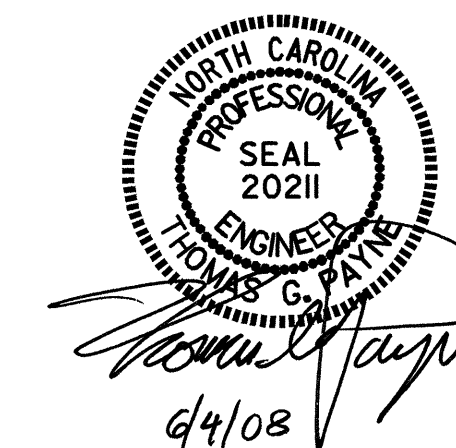
PROJECT NO. B-4252
ROCKINGHAM COUNTY
 STATION: 31+61.50 -L-

SHEET 2 OF 3

STATE OF NORTH CAROLINA
 DEPARTMENT OF TRANSPORTATION
 RALEIGH

GENERAL DRAWING

FOR BRIDGE OVER BIG BEAVER
 ISLAND CREEK ON US 311
 BETWEEN SR 1152 AND SR 1162

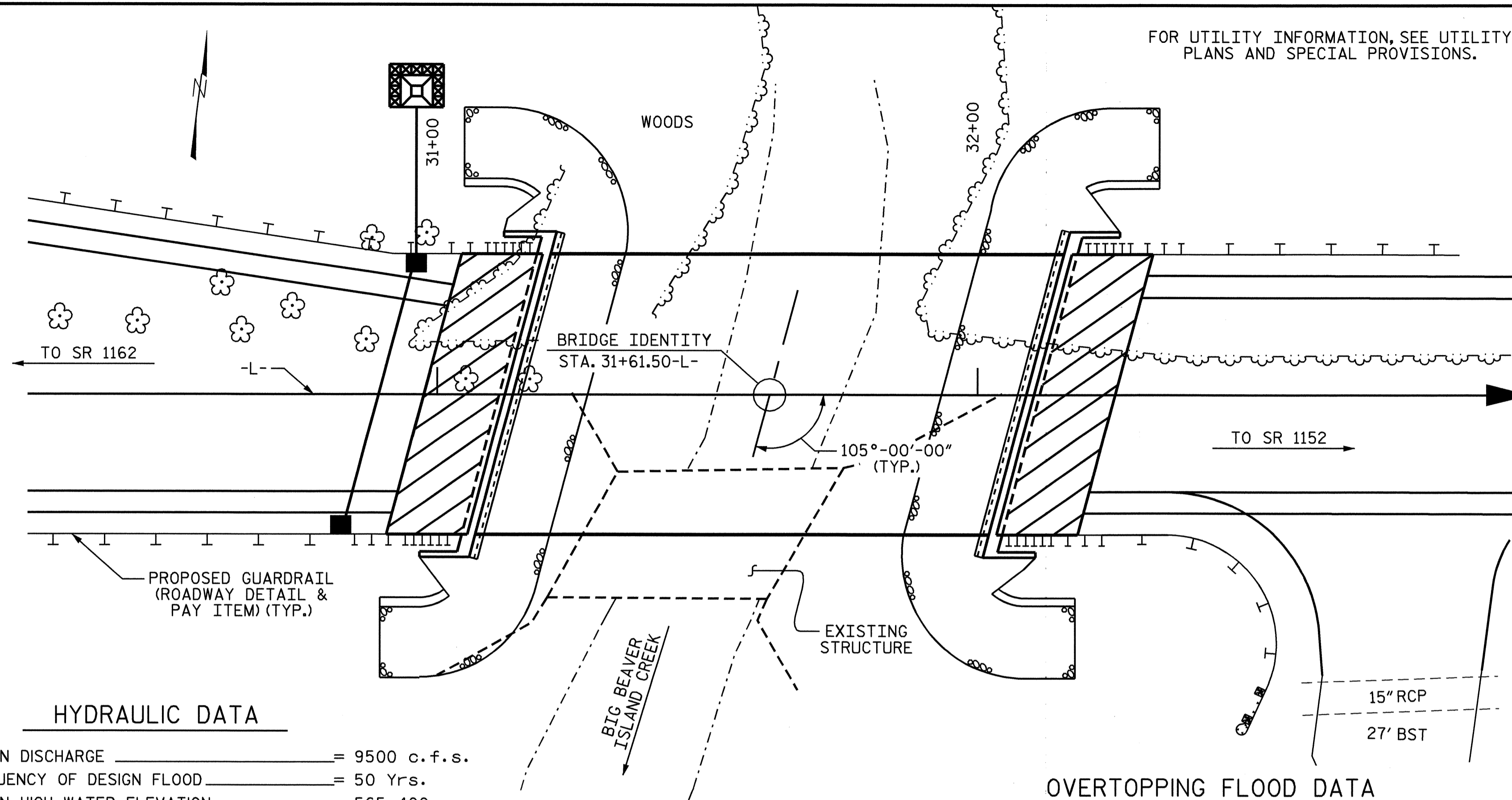


REVISIONS						SHEET NO.
NO.	BY:	DATE:	NO.	BY:	DATE:	S-37
1			3			TOTAL SHEETS 70
2			4			

STR. #2

BM#3 R/R SPIKE SET IN BASE OF 28" WALNUT 73.04 FT. LEFT OF STA. 25+85.70 -L- ELEV. 565.280

FOR UTILITY INFORMATION, SEE UTILITY PLANS AND SPECIAL PROVISIONS.



HYDRAULIC DATA

DESIGN DISCHARGE _____ = 9500 c. f. s.
 FREQUENCY OF DESIGN FLOOD _____ = 50 Yrs.
 DESIGN HIGH WATER ELEVATION _____ = 565.400
 DRAINAGE AREA _____ = 23.8 Sq. MI.
 BASIC DISCHARGE (Q100) _____ = 11800 c. f. s.
 BASIC HIGH WATER ELEVATION _____ = 568.800

OVERTOPPING FLOOD DATA

OVERTOPPING DISCHARGE _____ = 10096 c. f. s.
 FREQUENCY OF OVERTOPPING FLOOD _____ = 50+ YRS.
 OVERTOPPING FLOOD ELEVATION _____ = 566.900

LOCATION SKETCH

NOTES

- ASSUMED LIVE LOAD = HS 20 OR ALTERNATE LOADING, EXCEPT THAT THE GIRDERS HAVE BEEN DESIGNED FOR HS25.
- THIS BRIDGE HAS BEEN DESIGNED IN ACCORDANCE WITH THE REQUIREMENTS OF THE AASHTO STANDARD SPECIFICATIONS FOR SEISMIC DESIGN OF HIGHWAY BRIDGES FOR SEISMIC PERFORMANCE CATEGORY A.
- THE CONTRACTOR SHALL PROVIDE INDEPENDENT ASSURANCE SAMPLES OF REINFORCING STEEL AS FOLLOWS: FOR PROJECTS REQUIRING UP TO 400 TONS OF REINFORCING STEEL, ONE 30 INCH SAMPLE OF EACH SIZE BAR USED, AND FOR PROJECTS REQUIRING OVER 400 TONS OF REINFORCING STEEL, TWO 30 INCH SAMPLE OF EACH SIZE BAR USED. THE BARS FROM WHICH THE SAMPLES ARE TAKEN MUST THEN BE SPLICED WITH REPLACEMENT BARS OF THE SIZE AND LENGTH OF THE SAMPLE, PLUS A MINIMUM LAP SPLICE OF THIRTY BAR DIAMETERS.
- REMOVAL OF THE EXISTING BRIDGE SHALL BE PERFORMED SO AS NOT TO ALLOW DEBRIS TO FALL INTO THE WATER. THE CONTRACTOR SHALL REMOVE THE BRIDGE AND SUBMIT PLANS FOR DEMOLITION IN ACCORDANCE ARTICLE 402-2 OF THE STANDARD SPECIFICATIONS.
- AFTER SERVING AS A TEMPORARY STRUCTURE, THE EXISTING STRUCTURE LOCATED AT THE PROPOSED SITE SHALL BE REMOVED. THE EXISTING STRUCTURE CONSISTS OF 9 LINES OF W21 X 68 I-BEAMS AT 3'-3" CENTERS IN ONE SPAN OF 41'-0". THE CLEAR ROADWAY WIDTH IS 25.9' AND THE DECK IS REINFORCED CONCRETE WITH AN ASPHALT WEARING SURFACE. THE SPAN IS SUPPORTED BY ABUTMENTS OF MASS CONCRETE ON PILES. THE EXISTING BRIDGE IS PRESENTLY NOT POSTED FOR LOAD LIMIT. SHOULD THE STRUCTURAL INTEGRITY OF THE BRIDGE FURTHER DETERIORATE DURING CONSTRUCTION OF THE PROPOSED STRUCTURE, A LOAD LIMIT MAY BE POSTED AND MAY BE REDUCED AS FOUND NECESSARY DURING THE LIFE OF THE PROJECT.
- THIS BRIDGE HAS BEEN DESIGNED BY THE STRENGTH DESIGN METHOD AS SPECIFIED IN AASHTO STANDARD SPECIFICATIONS.
- REMOVABLE FORMS MAY BE USED IN LIEU OF METAL STAY-IN-PLACE FORMS IN ACCORDANCE WITH ARTICLE 420-3 OF THE STANDARD SPECIFICATIONS.
- THE MATERIAL SHOWN IN THE CROSS-HATCHED AREA ON SHEET 1 OF 3 SHALL BE EXCAVATED BETWEEN -L- AND 55± FEET RIGHT OF -L- AS DIRECTED BY THE ENGINEER. THIS WORK WILL BE MEASURED AND PAID FOR AT THE CONTRACT UNIT PRICE PER CUBIC YARD FOR UNCLASSIFIED STRUCTURE EXCAVATION.
- THE SUBSTRUCTURE OF THE EXISTING BRIDGE INDICATED ON THE PLANS IS FROM THE BEST INFORMATION AVAILABLE. SINCE THIS INFORMATION IS SHOWN FOR THE CONVENIENCE OF THE CONTRACTOR, THE CONTRACTOR SHALL HAVE NO CLAIM WHATSOEVER AGAINST THE DEPARTMENT OF TRANSPORTATION FOR ANY DELAYS OR ADDITIONAL COST INCURRED BASED ON DIFFERENCES BETWEEN THE EXISTING BRIDGE SUBSTRUCTURE SHOWN ON THE PLANS AND THE ACTUAL CONDITIONS AT THE PROJECT SITE.
- THE CLASS AA CONCRETE IN THE BRIDGE DECK SHALL CONTAIN FLY ASH OR GROUND GRANULATED BLAST FURNACE SLAG AT THE SUBSTITUTION RATE SPECIFIED IN ARTICLE 1024-1 AND IN ACCORDANCE WITH ARTICLES 1024-5 AND 1024-6 OF THE STANDARD SPECIFICATIONS. NO PAYMENT WILL BE MADE FOR THIS SUBSTITUTION AS IT IS CONSIDERED INCIDENTAL TO THE COST OF THE REINFORCED CONCRETE DECK SLAB.
- INASMUCH AS THE PAINT SYSTEM ON THE EXISTING STRUCTURAL STEEL CONTAINS LEAD, THE CONTRACTOR'S ATTENTION IS DIRECTED TO ARTICLE 107-1 OF THE STANDARD SPECIFICATIONS. ANY COSTS RESULTING FROM COMPLIANCE WITH APPLICABLE STATE OR FEDERAL REGULATIONS PERTAINING TO HANDLING OF MATERIALS CONTAINING LEAD BASED PAINT SHALL BE INCLUDED IN THE BID PRICE FOR "REMOVAL OF EXISTING STRUCTURE."
- NEEDLE BEAMS WILL NOT BE ALLOWED UNLESS OTHERWISE CALLED FOR ON THE PLANS OR APPROVED BY THE ENGINEER.
- FOR LIMITS OF TEMPORARY SHORING FOR MAINTENANCE OF TRAFFIC, SEE TRAFFIC CONTROL PLANS. FOR PAY ITEM FOR TEMPORARY SHORING FOR MAINTENANCE OF TRAFFIC, SEE ROADWAY PLANS.
- THIS STRUCTURE HAS BEEN DESIGNED IN ACCORDANCE WITH HEC 18, "EVALUATING SCOUR AT BRIDGES", MAY, 2001.
- FOR FALSEWORK AND FORMWORK, SEE SPECIAL PROVISIONS.
- FOR GROUT FOR STRUCTURES, SEE SPECIAL PROVISIONS.
- FOR EROSION CONTROL MEASURES, SEE EROSION CONTROL PLANS.
- FOR OTHER DESIGN DATA AND GENERAL NOTES, SEE SHEET SN.
- ALL STRUCTURAL STEEL SHALL BE AASHTO M270 GRADE 50W.
- FOR SUBMITTAL OF WORKING DRAWINGS, SEE SPECIAL PROVISIONS.
- FOR SHIPPING STEEL STRUCTURAL MEMBERS, SEE SPECIAL PROVISIONS.
- FOR CRANE SAFETY, SEE SPECIAL PROVISIONS.

TOTAL BILL OF MATERIAL

	REMOVAL OF EXISTING STRUCTURE	PILE EXCAVATION IN SOIL	PILE EXCAVATION NOT IN SOIL	UNCLASSIFIED STRUCTURE EXCAVATION	REINFORCED CONCRETE DECK SLAB	GROOVING BRIDGE FLOORS	CLASS A CONCRETE	BRIDGE APPROACH SLABS	REINFORCING STEEL	STRUCTURAL STEEL	HP 12 X 53 STEEL PILES		2 BAR METAL RAIL	1'-2" X 2'-6" CONCRETE PARAPET	RIP RAP CLASS II (2'-0" THICK)	FILTER FABRIC FOR DRAINAGE	EVAZOTE JOINT SEALS	ELECTRICAL CONDUIT SYSTEM
	LUMP SUM	LIN. FT.	LIN. FT.	CU. YDS.	SQ. FT.	SQ. FT.	CU. YDS.	LUMP SUM	LBS.	APPROX. LBS.	NO.	LIN. FT.	LIN. FT.	LIN. FT.	TONS	SQ. YDS.	LUMP SUM	LUMP SUM
SUPERSTRUCTURE	LUMP SUM			725	5,364	7,034				173,800			180.92	196.55			LUMP SUM	LUMP SUM
END BENT NO. 1		172	37				28.1		4,373		14	210			320	355		
END BENT NO. 2		167	44				28.0		4,374		14	210			260	285		
TOTAL	LUMP SUM	339	81	725	5,364	7,034	56.1	LUMP SUM	8,747	173,800	28	420	180.92	196.55	580	640	LUMP SUM	LUMP SUM

DRAWN BY : R. G. EMERSON DATE : 01/08
 CHECKED BY : K. D. LAYNE DATE : 01/08

29-MAY-2008 13:21
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PROJECT NO. B-4252
ROCKINGHAM COUNTY
 STATION: 31+61.50-L-

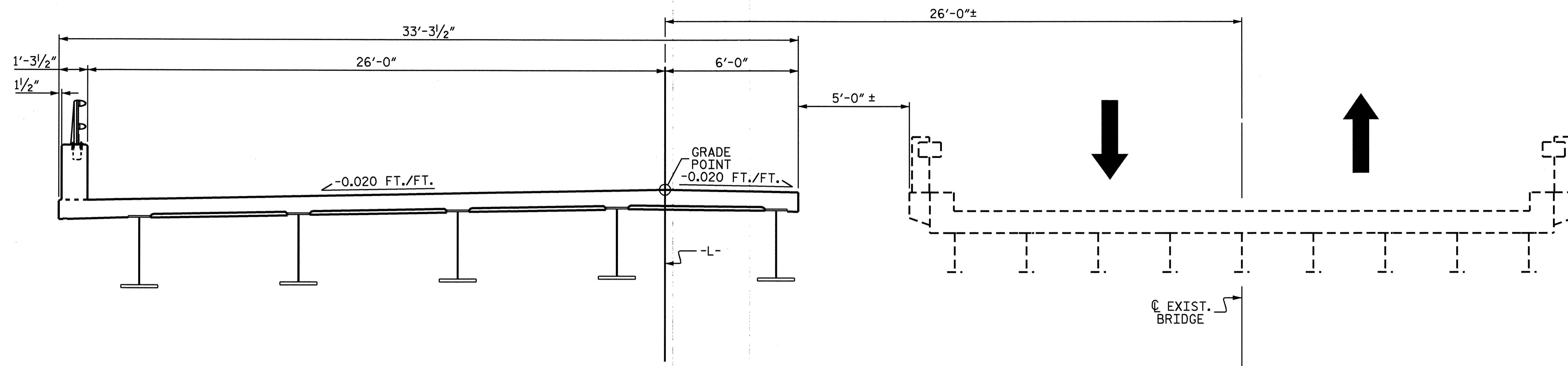
SHEET 3 OF 3

STATE OF NORTH CAROLINA
 DEPARTMENT OF TRANSPORTATION
 RALEIGH

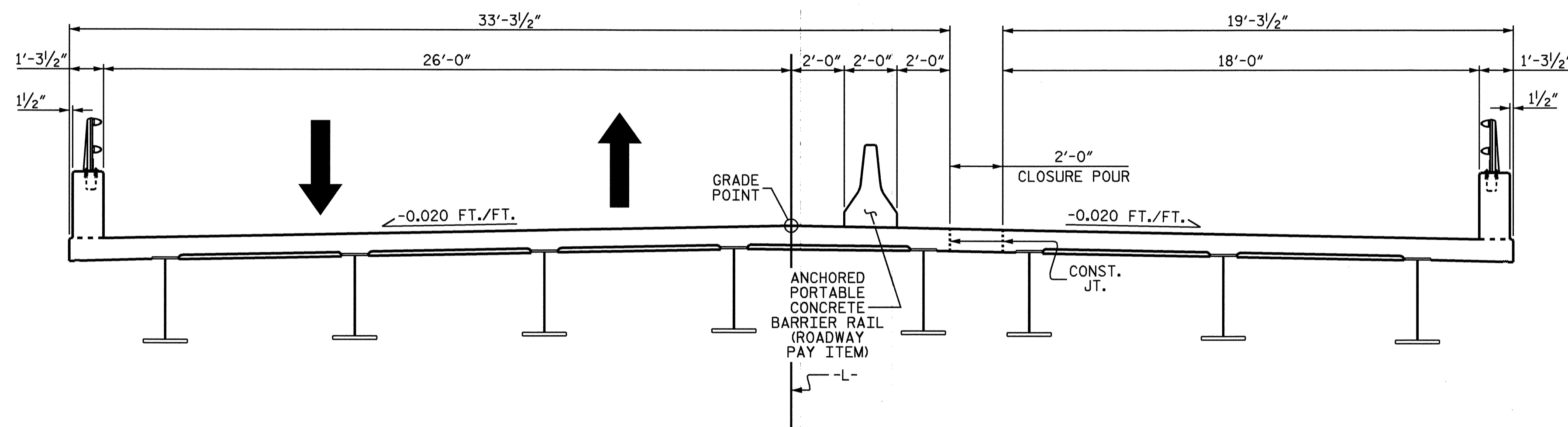
GENERAL DRAWING
 FOR BRIDGE OVER BIG
 BEAVER ISLAND CREEK ON
 US 311 BETWEEN SR 1152
 AND SR 1162

REVISIONS						SHEET NO.
NO.	BY:	DATE:	NO.	BY:	DATE:	S-38
1			3			TOTAL SHEETS 70
2			4			

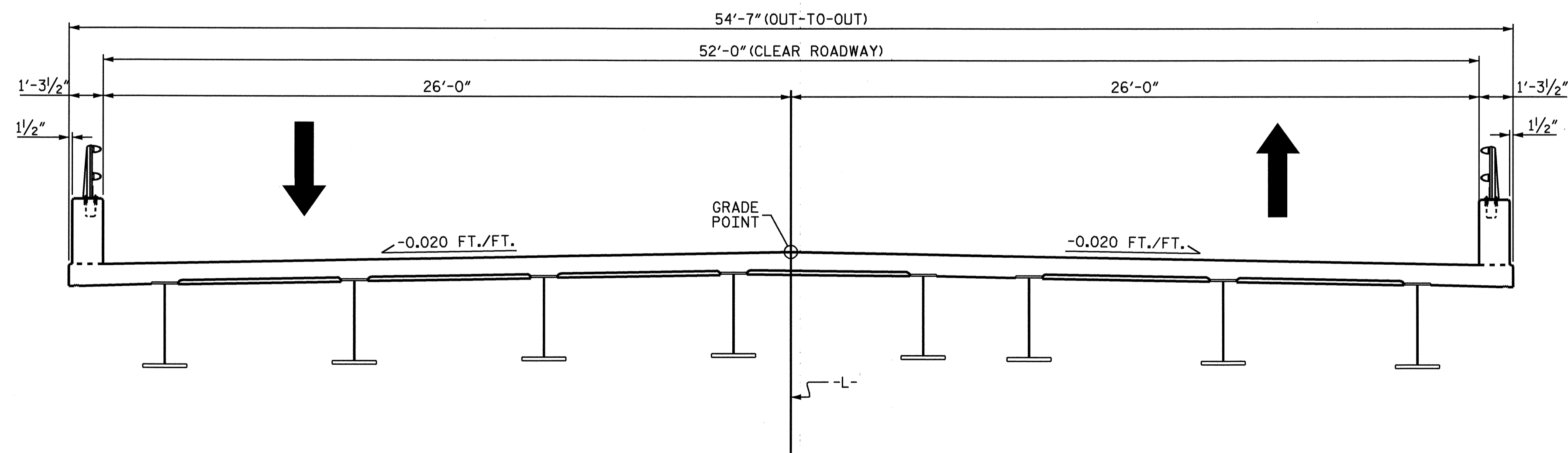
STR.#2



STAGE 1



STAGE 2



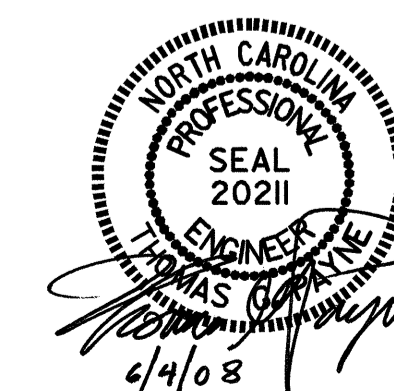
FINAL TYPICAL SECTION

NOTES
 FOR PHASING AND MAINTENANCE OF TRAFFIC,
 SEE TRAFFIC CONTROL PLANS.
 SEE TRAFFIC CONTROL PLANS FOR LOCATION
 AND PAY LIMITS OF THE ANCHORED PORTABLE
 CONCRETE BARRIER RAIL.

PROJECT NO. B-4252
ROCKINGHAM COUNTY
 STATION: 31+61.50 -L-

STATE OF NORTH CAROLINA
 DEPARTMENT OF TRANSPORTATION
 RALEIGH

**CONSTRUCTION
 SEQUENCE**

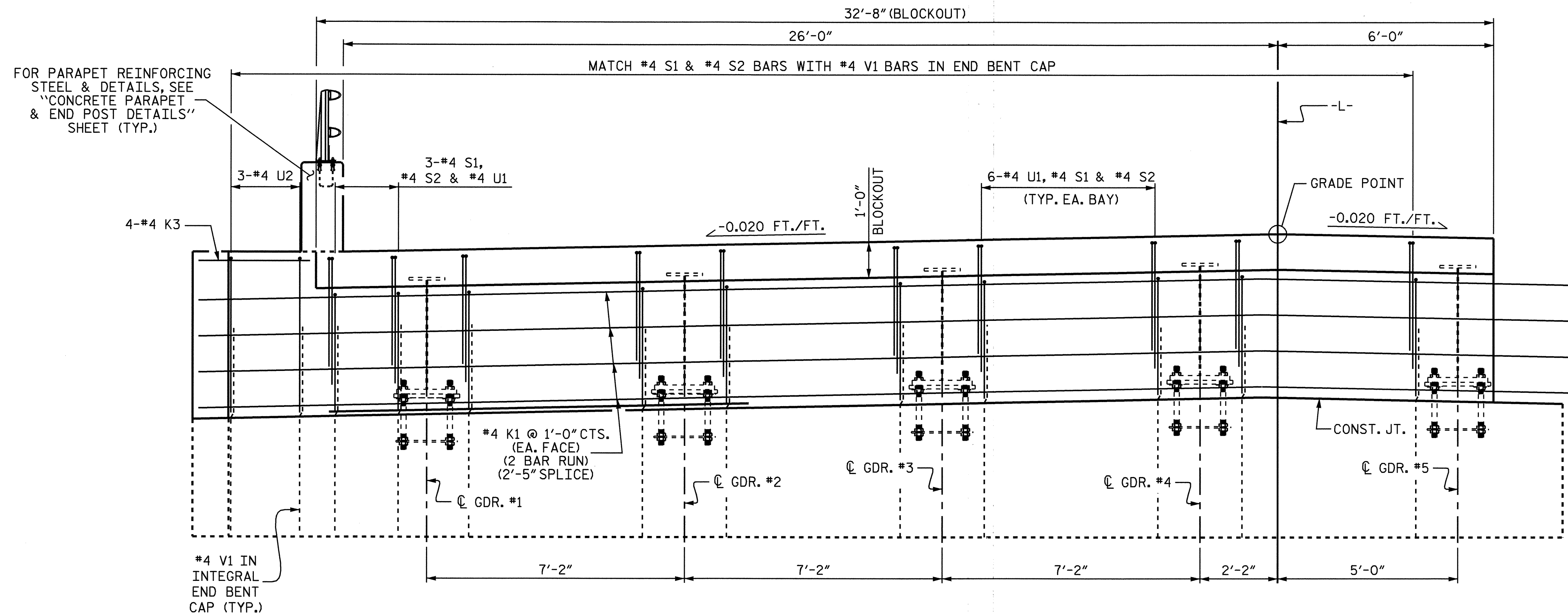


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 CHECKED BY : M.K. BEARD DATE : 10/07

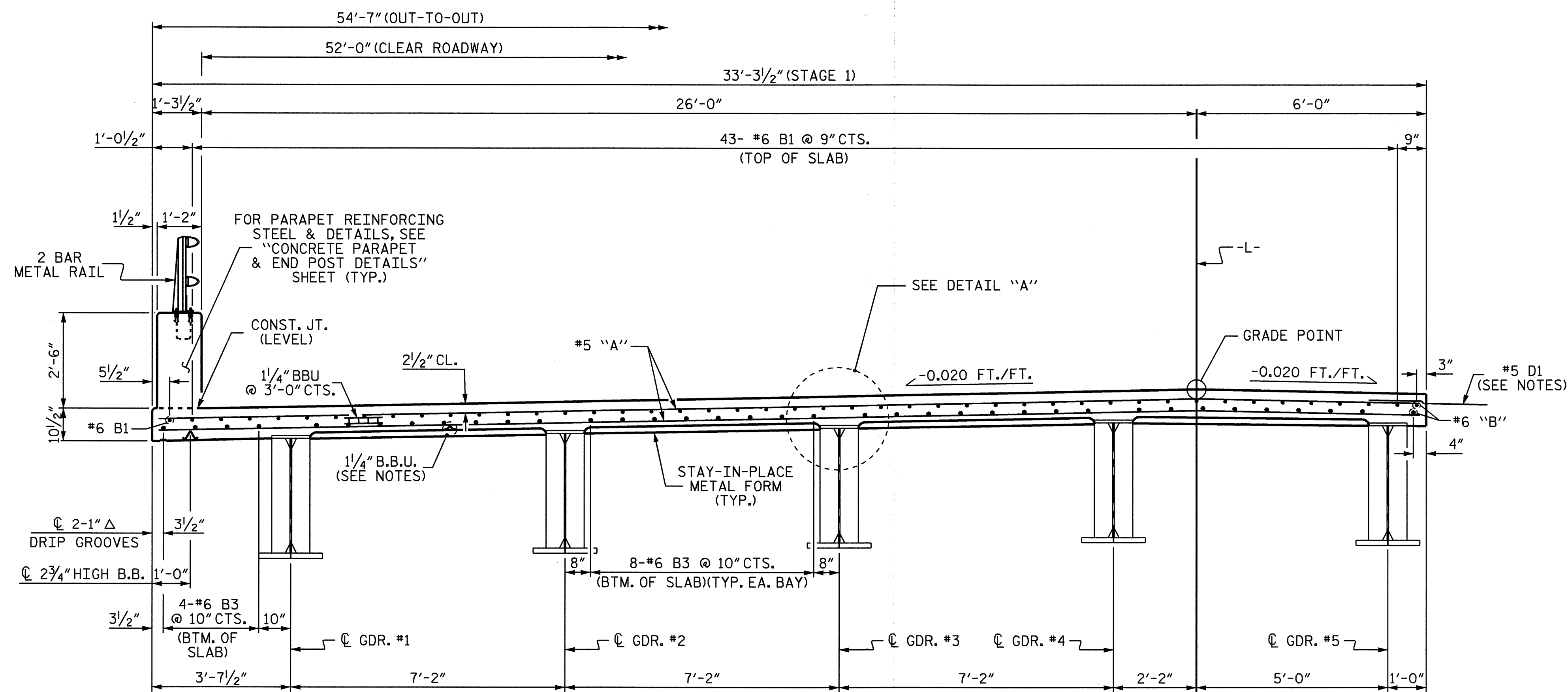
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REVISIONS						SHEET NO.
NO.	BY:	DATE:	NO.	BY:	DATE:	S-39
1			3			TOTAL SHEETS
2			4			70

STR. #2



TYPICAL SECTION @ INTEGRAL END BENT
(DECK REINFORCEMENT NOT SHOWN FOR CLARITY)



TYPICAL SECTION @ INTEGRAL END BENT
(SHOWING DECK REINFORCEMENT, REINFORCEMENT IN ABUTMENT NOT SHOWN FOR CLARITY)

NOTES

PROVIDE 1/4" HIGH BEAM BOLSTERS UPPER AT 4'-0" CTS. ATOP THE METAL STAY-IN-PLACE FORMS TO SUPPORT THE BOTTOM MAT OF 'A' BARS. WHEN USING REMOVABLE FORMS, PROVIDE CONTINUOUS HIGH CHAIRS FOR METAL DECK (C.H.C.M.) @ 4'-0" CTS. WITH A HEIGHT TO SUPPORT THE BOTTOM MAT OF 'A' BARS A CLEAR DISTANCE OF 2 1/2" ABOVE THE TOP OF THE REMOVABLE FORM.

METAL STAY-IN-PLACE FORMS SHALL NOT BE WELDED TO BEAM OR GIRDER FLANGES IN THE ZONES REQUIRING CHAMPY V-NOTCH TEST. SEE STRUCTURAL STEEL DETAIL SHEETS.

PARAPET IN THE SPAN SHALL NOT BE CAST UNTIL ALL SLAB CONCRETE IN THE SPAN HAS BEEN CAST AND HAS REACHED A MINIMUM COMPRESSIVE STRENGTH OF 3,000 PSI.

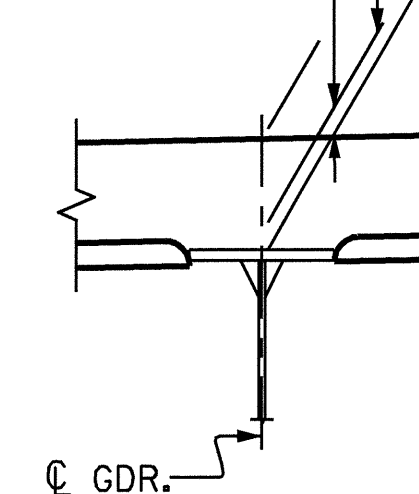
THE CONTRACTOR MAY, WHEN NECESSARY, PROPOSE A SCHEME FOR AVOIDING INTERFERENCE BETWEEN METAL STAY-IN-PLACE FORM SUPPORTS OR FORMS AND BEAM/GIRDER STIFFENERS OR CONNECTOR PLATES. THE PROPOSAL SHALL BE INDICATED, AS APPROPRIATE, ON EITHER THE STEEL WORKING DRAWINGS OR THE METAL STAY-IN-PLACE FORM WORKING DRAWINGS.

DOWELS SHALL BE PLACED IN THE SAME HORIZONTAL PLANE AS THE TOP SLAB REINFORCING STEEL.

10 1/4" TOP OF SLAB TO BOTTOM OF TOP FLANGE @ GDR.

8 1/4" TOP OF SLAB TO TOP OF S.I.P. FORMS @ GDR.

2" BUILD-UP @ GDR.



DETAIL "A"

PROJECT NO. B-4252
ROCKINGHAM COUNTY
STATION: 31+61.50 -L-

SHEET 1 OF 4

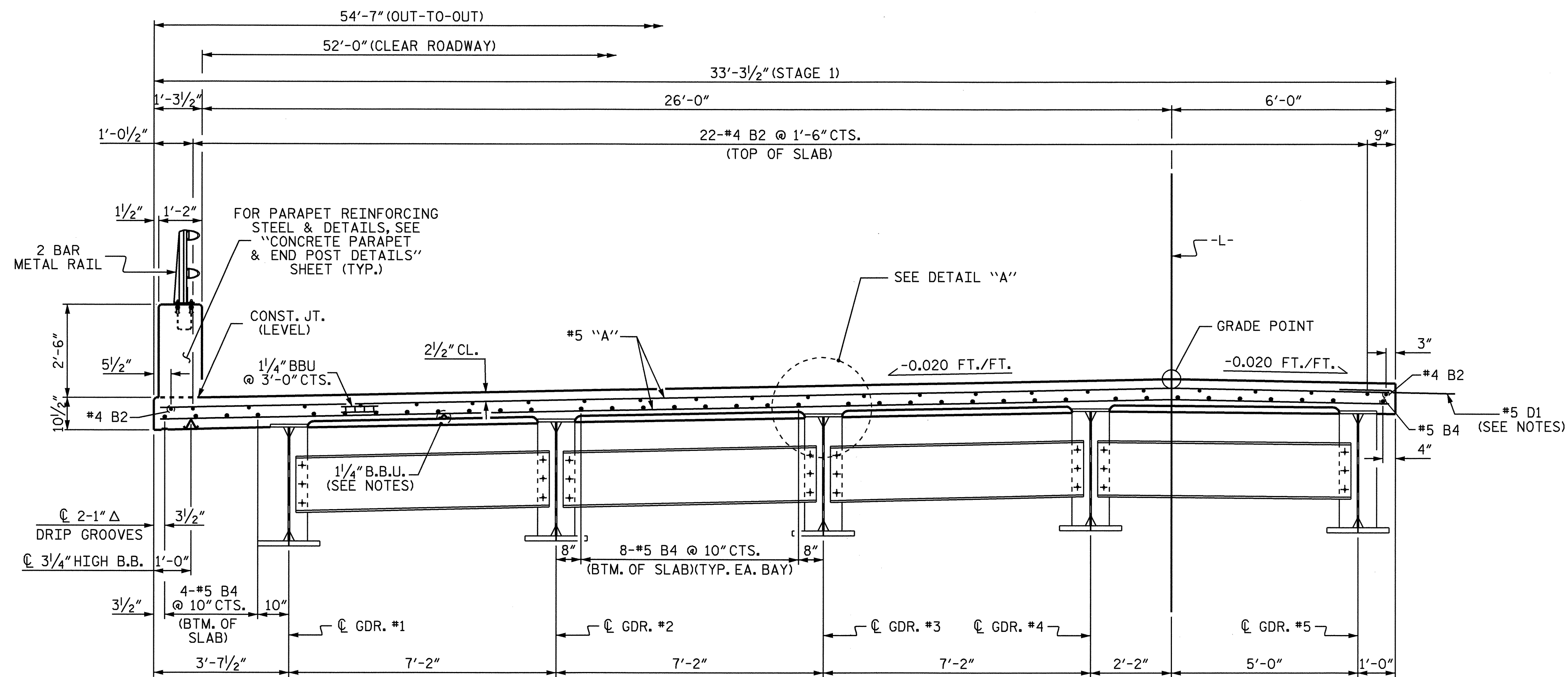
STATE OF NORTH CAROLINA
DEPARTMENT OF TRANSPORTATION
RALEIGH

SUPERSTRUCTURE
TYPICAL SECTION
(STAGE 1)

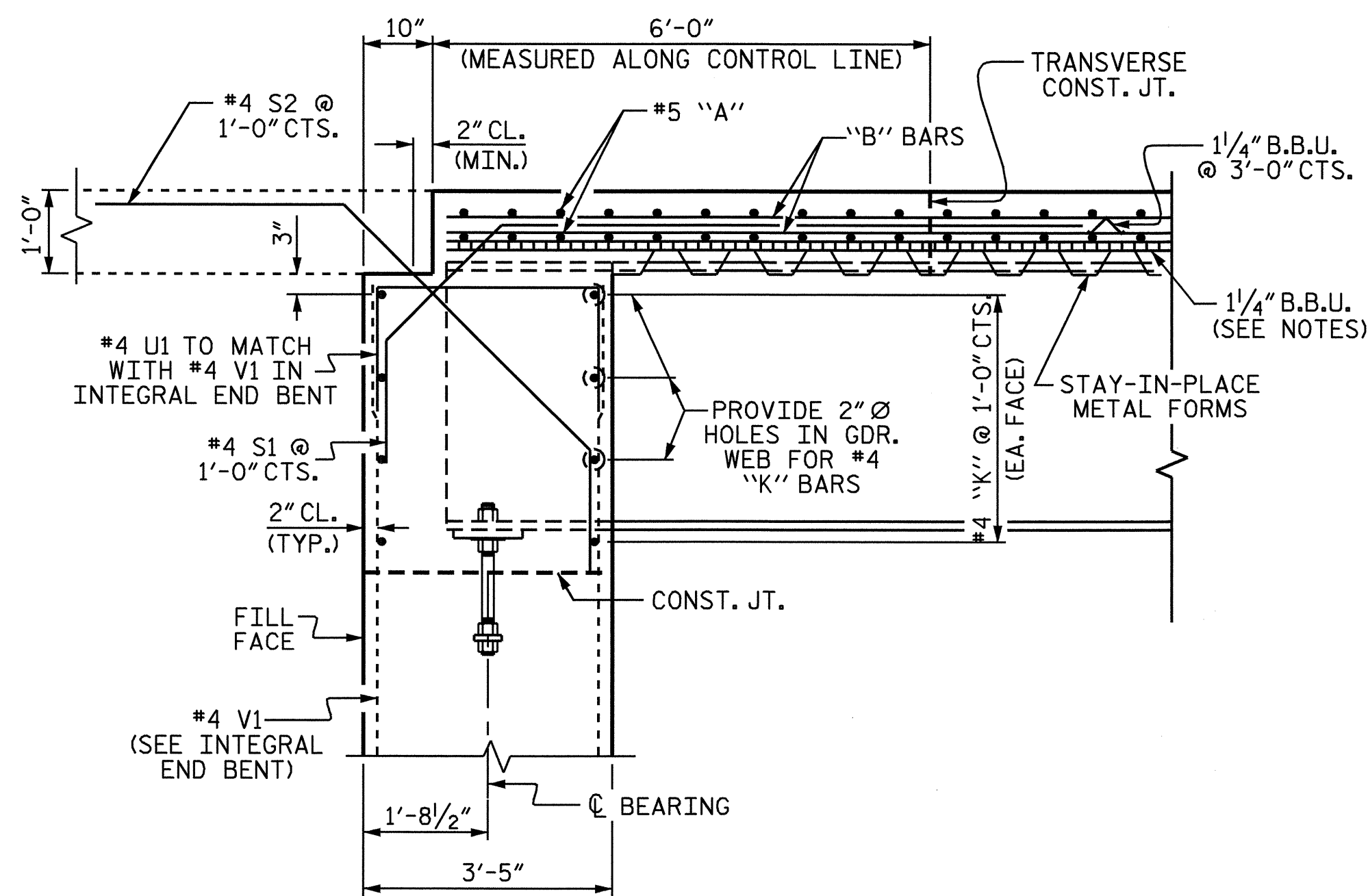
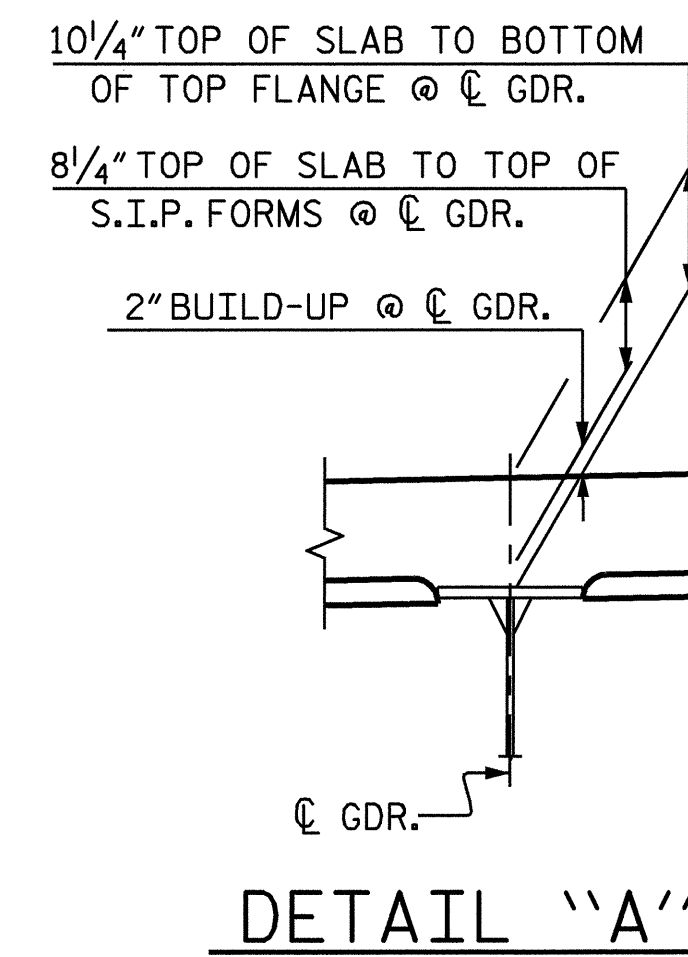
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NO.	BY:	DATE:	NO.	BY:	DATE:	S-40	
1			3			TOTAL	70
2			4				



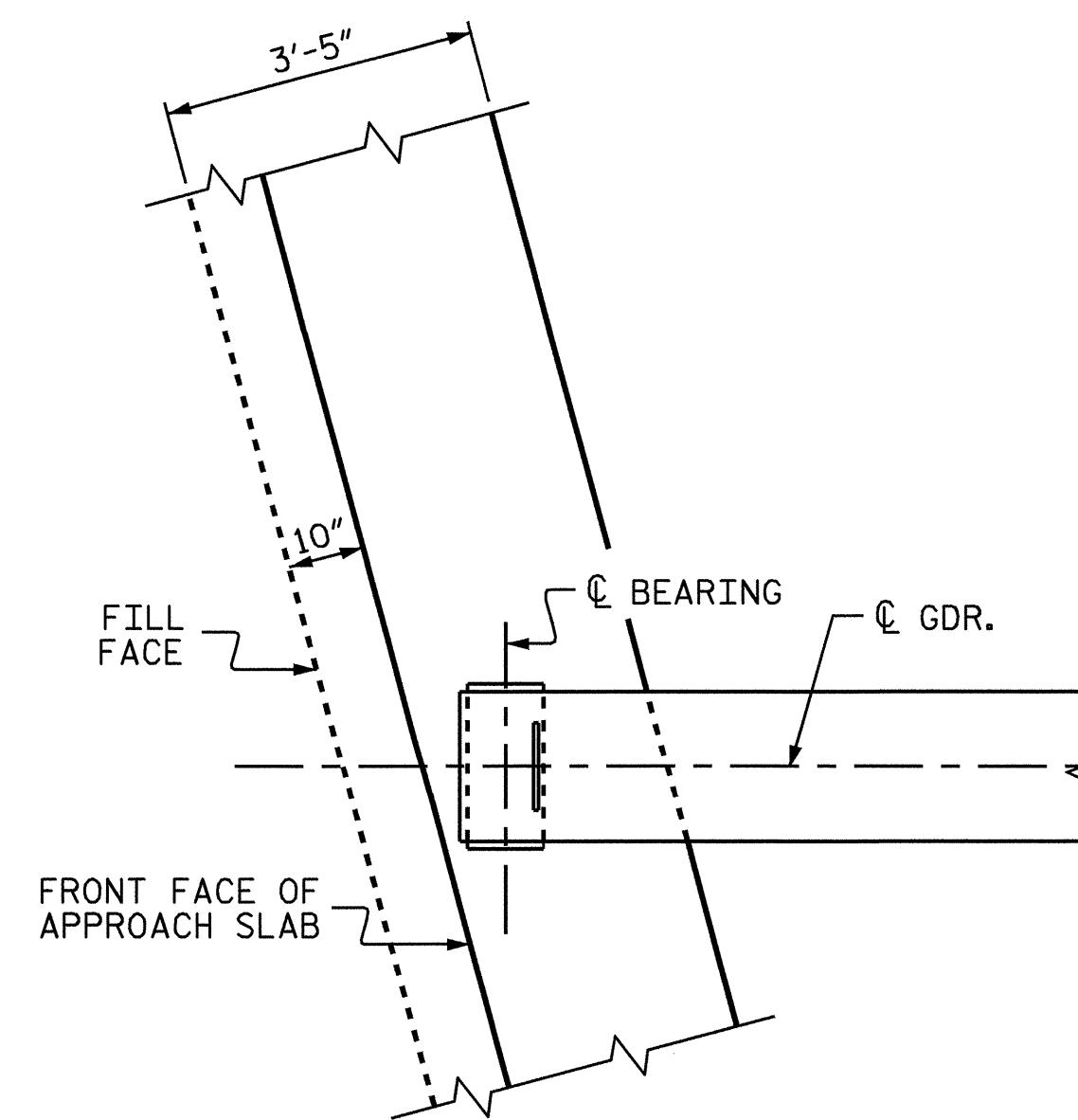
DRAWN BY : S. DOMBROWSKI DATE : 12/06
CHECKED BY : M.K. BEARD DATE : 10/07



TYPICAL SECTION @ INTERMEDIATE DIAPHRAGM



SECTION THRU INTEGRAL END BENT



PLAN OF GIRDER @ END BENT #1

END BENT #1 SHOWN, END BENT #2 SIMILAR

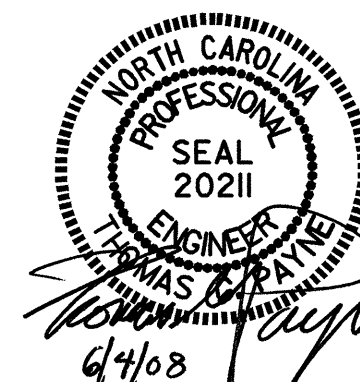
PROJECT NO. B-4252
ROCKINGHAM COUNTY
 STATION: 31+61.50 -L-

SHEET 2 OF 4

STATE OF NORTH CAROLINA
 DEPARTMENT OF TRANSPORTATION
 RALEIGH

SUPERSTRUCTURE
 TYPICAL SECTION
 (STAGE 1)

REVISIONS						SHEET NO.
NO.	BY:	DATE:	NO.	BY:	DATE:	S-41
1			3			TOTAL SHEETS
2			4			70



DRAWN BY: S. DOMBROWSKI DATE: 12/06
 CHECKED BY: M.K. BEARD DATE: 10/07

NOTES

PROVIDE 1/4" HIGH BEAM BOLSTERS UPPER AT 4'-0" CTS. ATOP THE METAL STAY-IN-PLACE FORMS TO SUPPORT THE BOTTOM MAT OF 'A' BARS. WHEN USING REMOVABLE FORMS, PROVIDE CONTINUOUS HIGH CHAIRS FOR METAL DECK (C.H.C.M.) @ 4'-0" CTS. WITH A HEIGHT TO SUPPORT THE BOTTOM MAT OF 'A' BARS A CLEAR DISTANCE OF 2/2" ABOVE THE TOP OF THE REMOVABLE FORM.

METAL STAY-IN-PLACE FORMS SHALL NOT BE WELDED TO BEAM OR GIRDER FLANGES IN THE ZONES REQUIRING CHARNY V-NOTCH TEST. SEE STRUCTURAL STEEL DETAIL SHEETS.

PARAPET IN THE SPAN SHALL NOT BE CAST UNTIL ALL SLAB CONCRETE IN THE SPAN HAS BEEN CAST AND HAS REACHED A MINIMUM COMPRESSIVE STRENGTH OF 3,000 PSI.

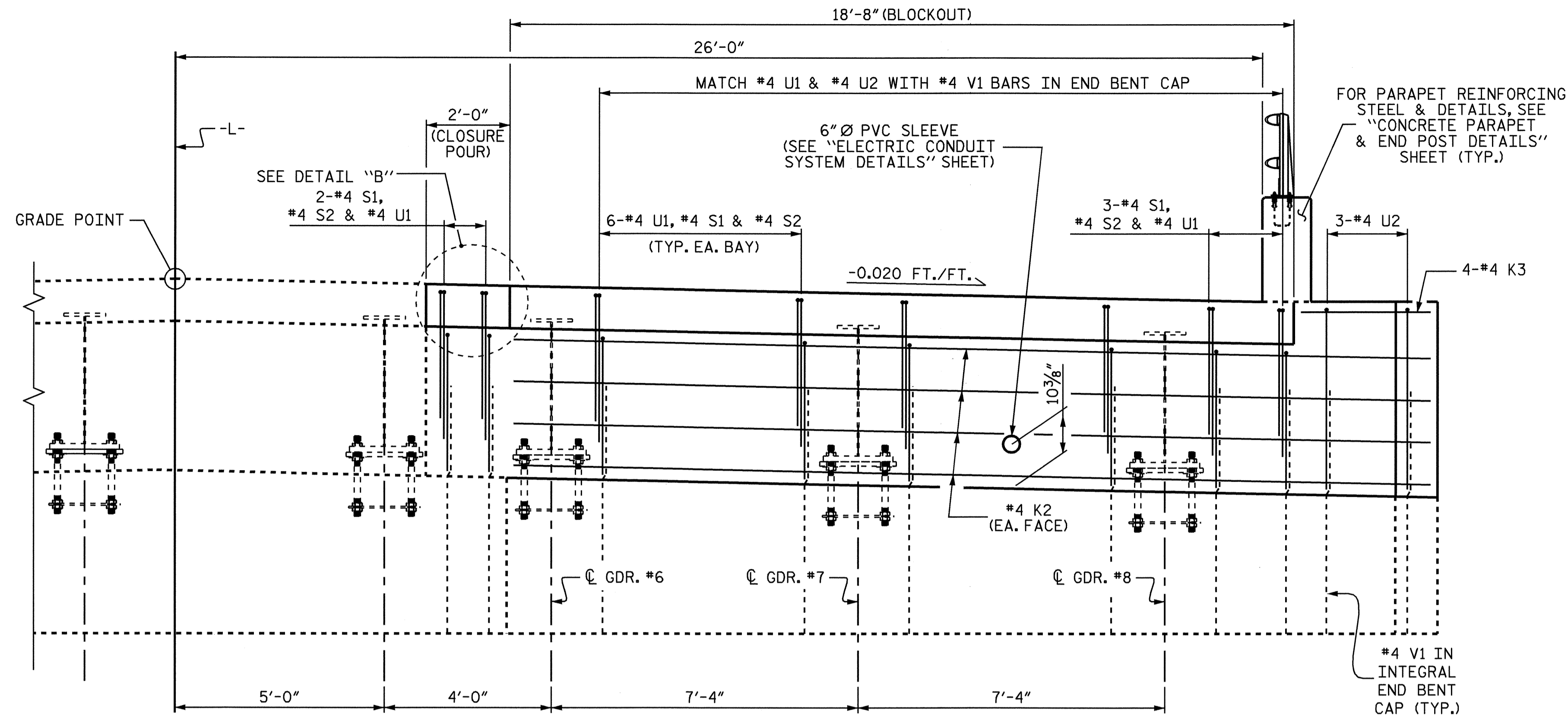
THE CONTRACTOR MAY, WHEN NECESSARY, PROPOSE A SCHEME FOR AVOIDING INTERFERENCE BETWEEN METAL STAY-IN-PLACE FORM SUPPORTS OR FORMS AND BEAM/GIRDER STIFFENERS OR CONNECTOR PLATES. THE PROPOSAL SHALL BE INDICATED, AS APPROPRIATE, ON EITHER THE STEEL WORKING DRAWINGS OR THE METAL STAY-IN-PLACE FORM WORKING DRAWINGS.

DOWELS SHALL BE PLACED IN THE SAME HORIZONTAL PLANE AS THE TOP SLAB REINFORCING STEEL.

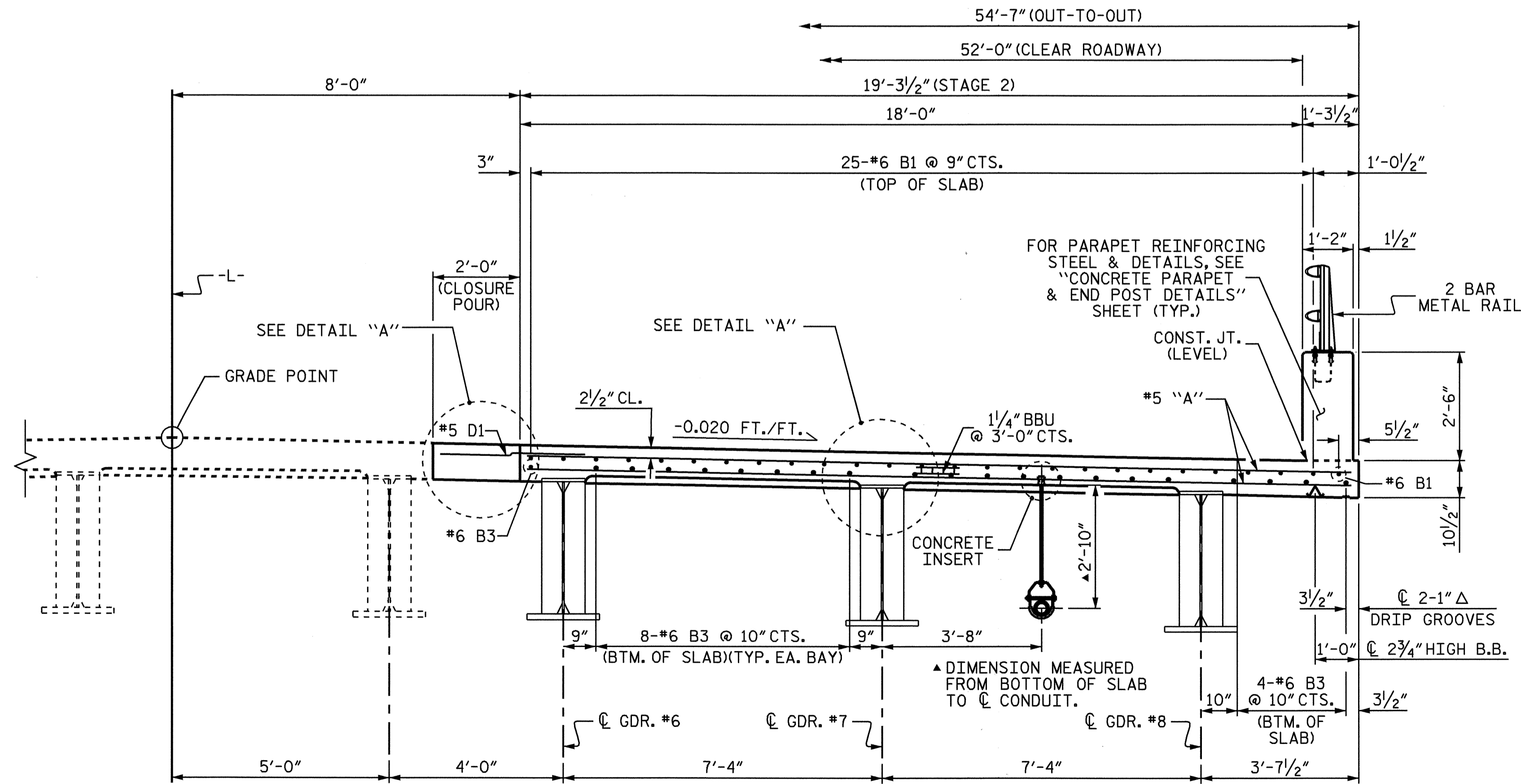
FOR LOCATION AND DETAILS OF MULTI-CELL RACEWAY, HANGER ASSEMBLY, AND CONCRETE INSERTS, SEE "ELECTRICAL CONDUIT SYSTEM DETAILS" SHEET.

FOR ELECTRICAL CONDUIT SYSTEM, SEE SPECIAL PROVISIONS.

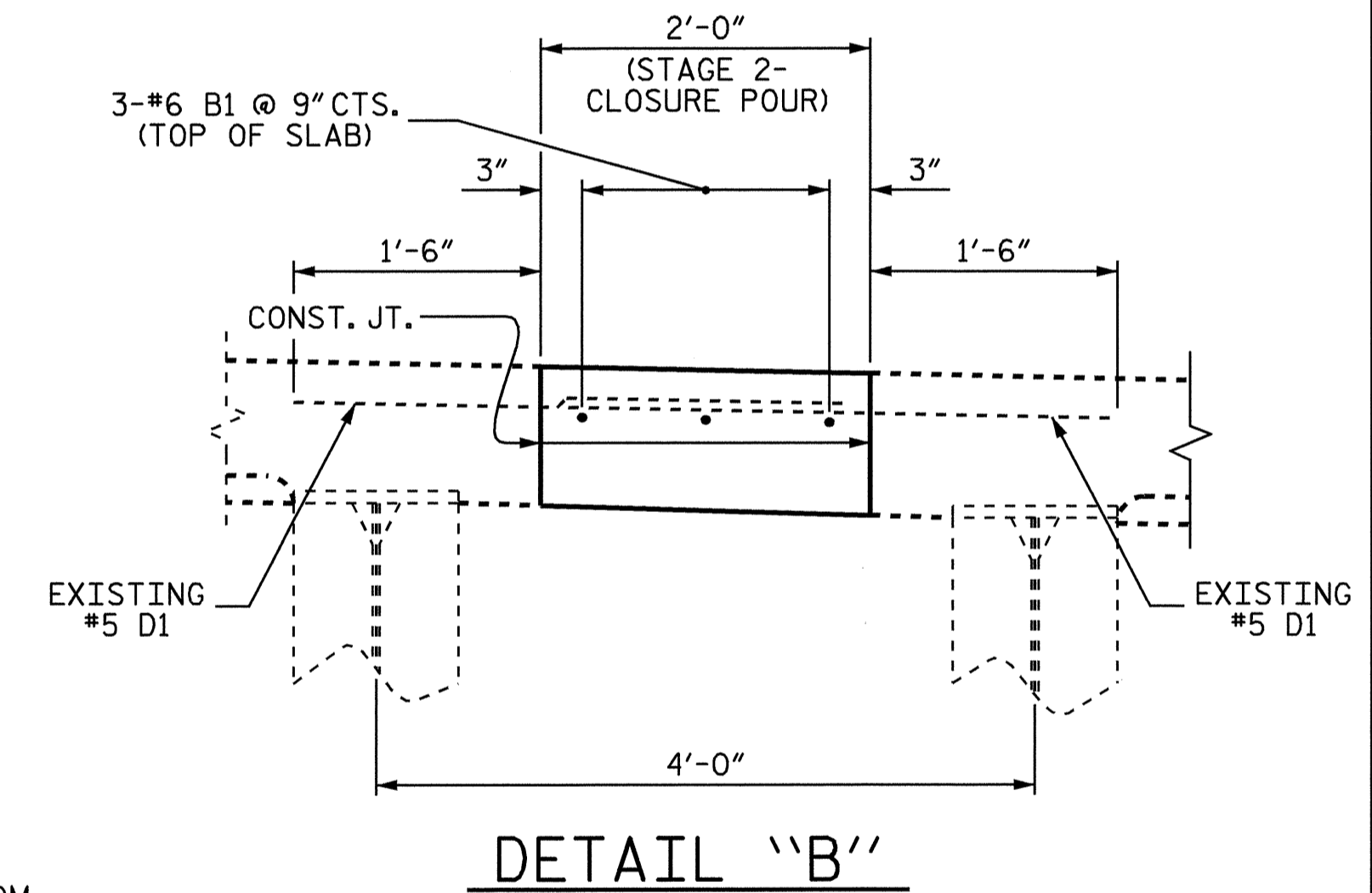
THE CONTRACTOR SHALL PROVIDE FOR INSTALLATION OF THE ELECTRICAL CONDUIT SYSTEM THROUGH THE INTEGRAL END BENT. REINFORCING STEEL MAY BE SHIFTED AS NECESSARY TO CLEAR THE CONDUIT.



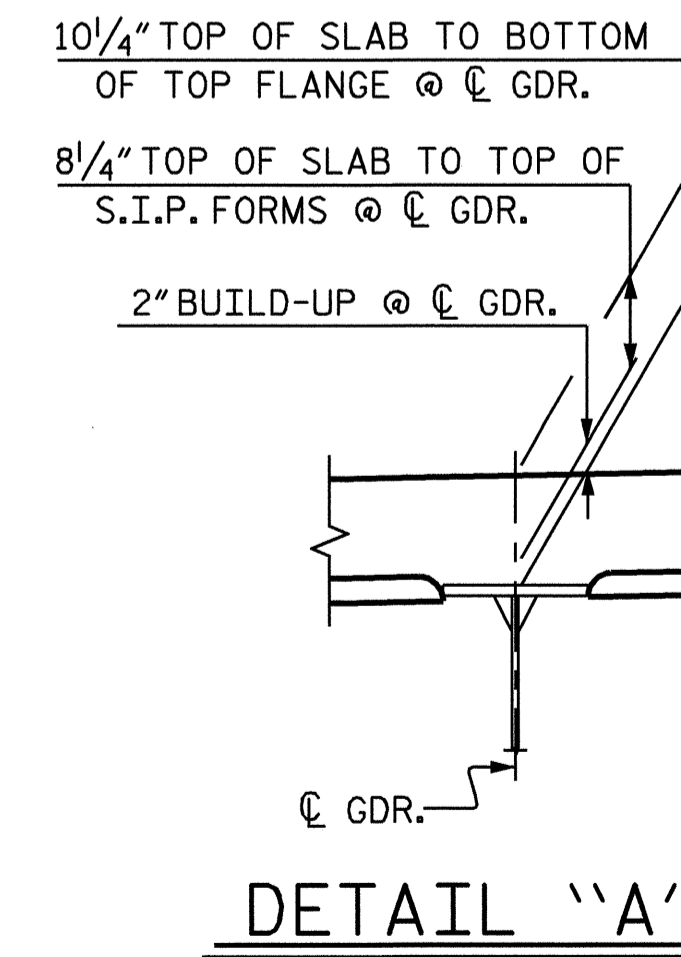
TYPICAL SECTION @ INTEGRAL END BENT
(DECK REINFORCEMENT NOT SHOWN FOR CLARITY)



TYPICAL SECTION @ INTEGRAL END BENT
(SHOWING DECK REINFORCEMENT, REINFORCEMENT IN ABUTMENT NOT SHOWN FOR CLARITY)



DETAIL "B"



DETAIL "A"

PROJECT NO. B-4252
ROCKINGHAM COUNTY
STATION: 31+61.50 -L-

SHEET 3 OF 4

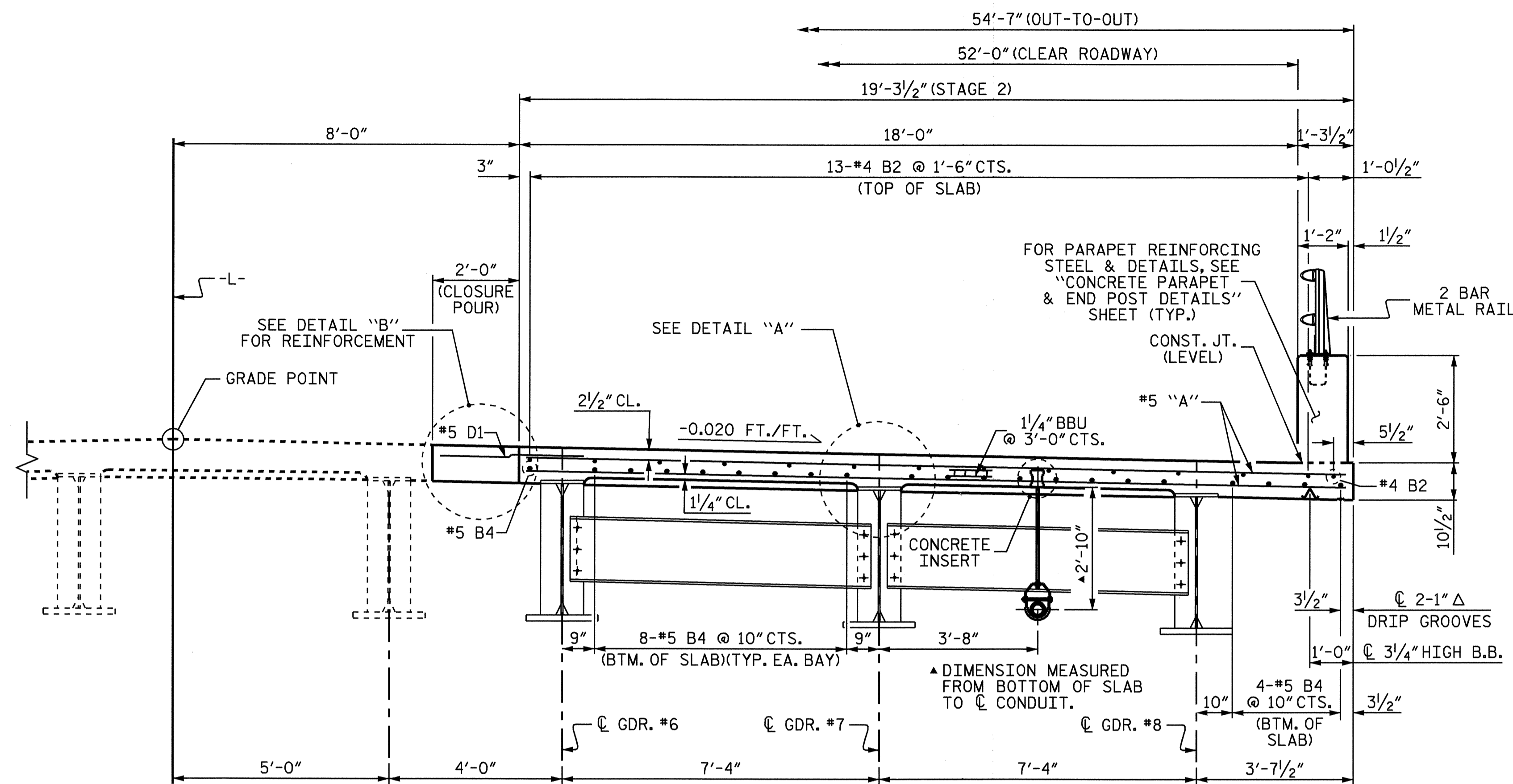
STATE OF NORTH CAROLINA
DEPARTMENT OF TRANSPORTATION
RALEIGH

SUPERSTRUCTURE
TYPICAL SECTION
(STAGE 2)

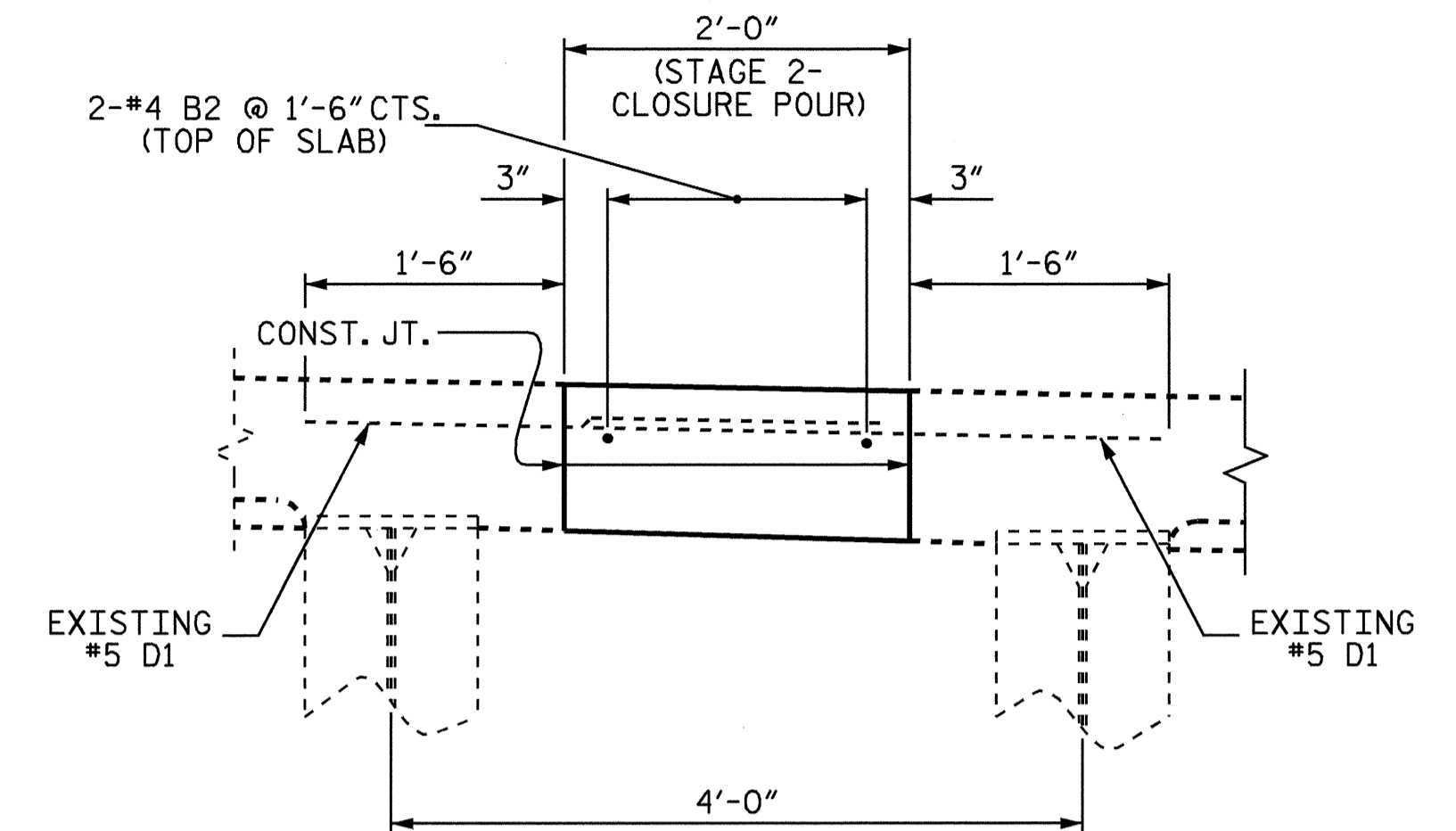
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2			4			



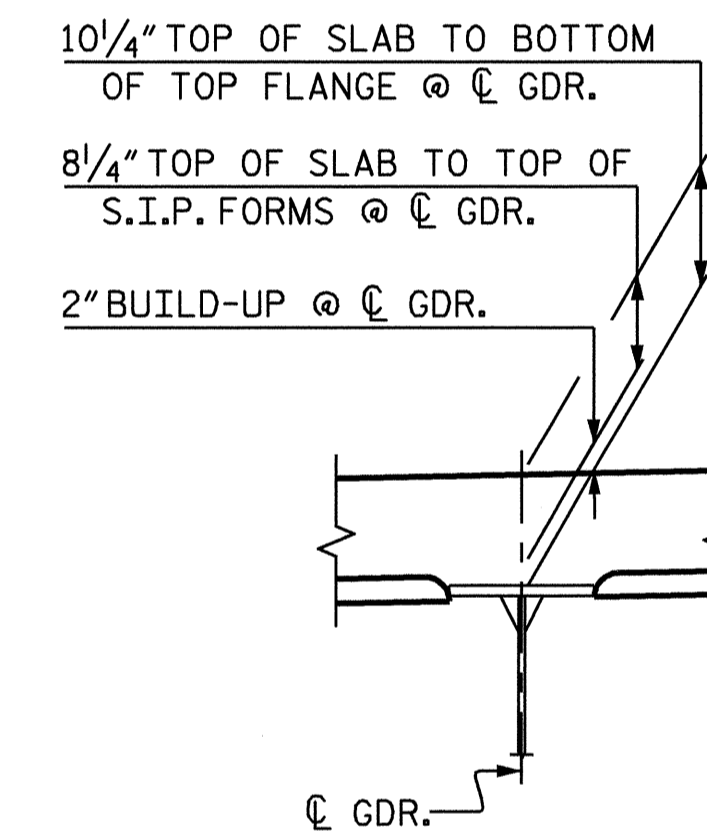
DRAWN BY : S. DOMBROWSKI DATE : 12/06
CHECKED BY : M.K. BEARD DATE : 10/07



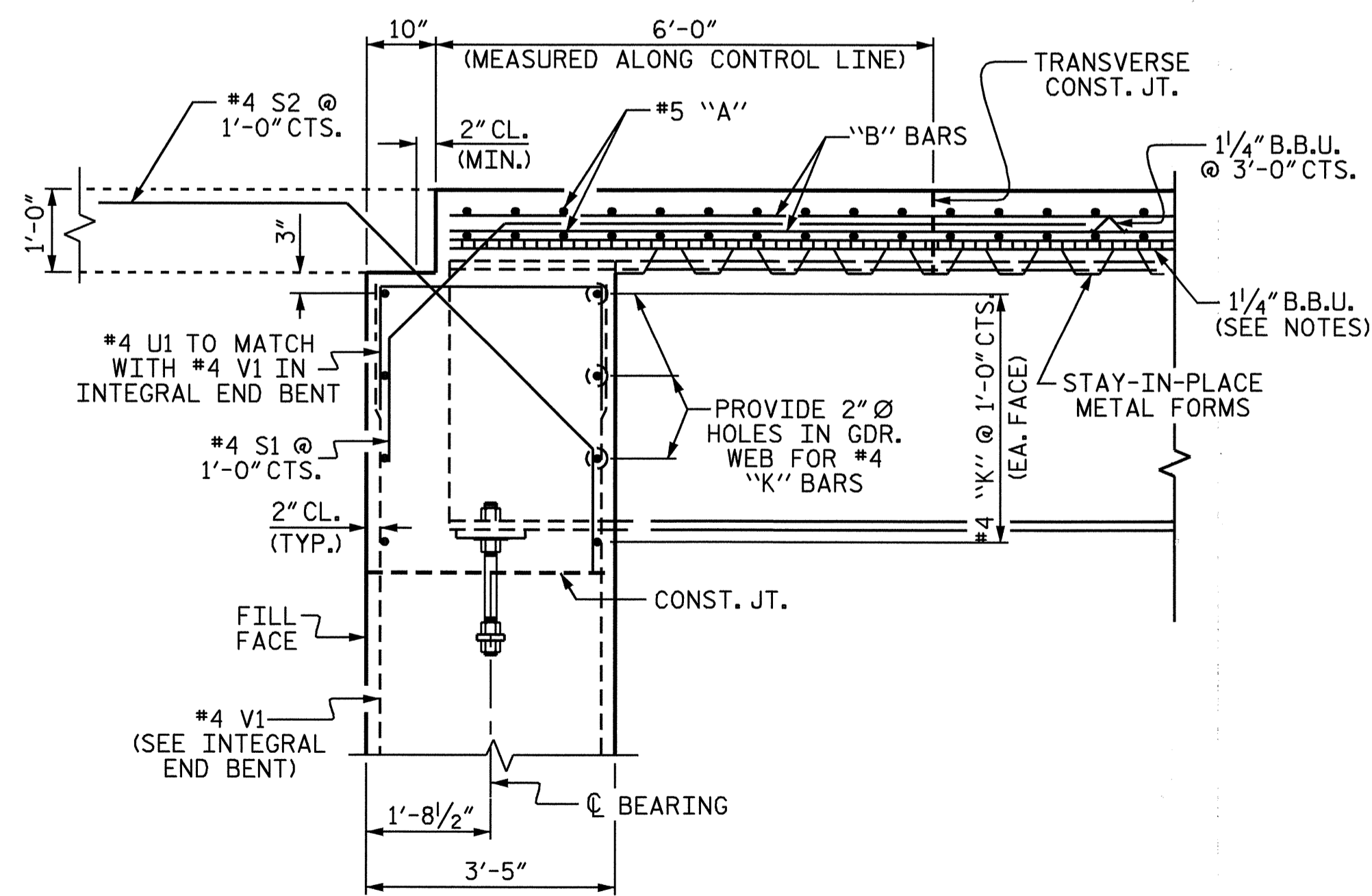
TYPICAL SECTION @ INTERMEDIATE DIAPHRAGM



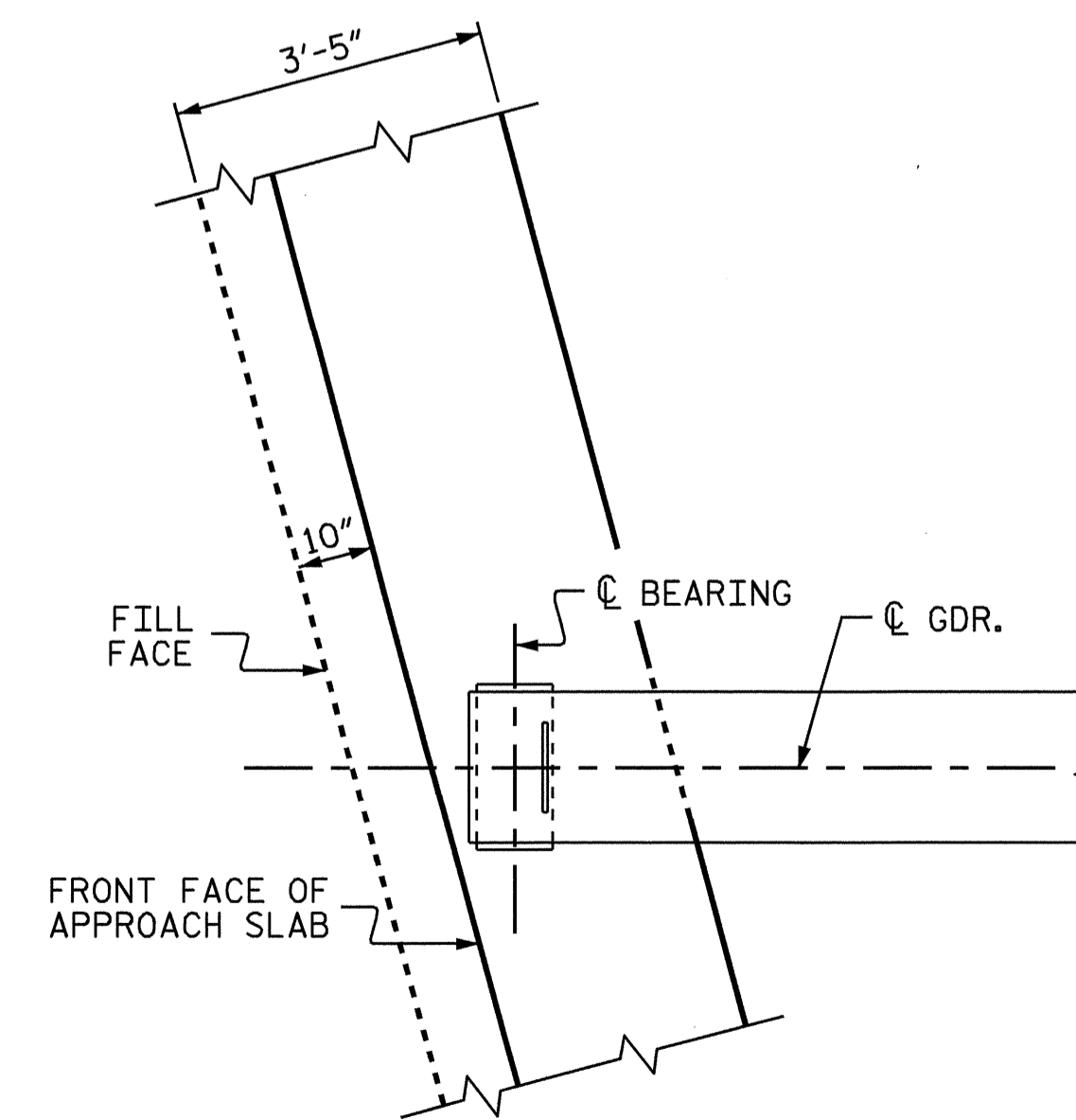
DETAIL "B"



DETAIL "A"



SECTION THRU INTEGRAL END BENT



PLAN OF GIRDER @ END BENT #1

END BENT #1 SHOWN, END BENT #2 SIMILAR

PROJECT NO. B-4252
 ROCKINGHAM COUNTY
 STATION: 31+61.50 -L-

SHEET 4 OF 4

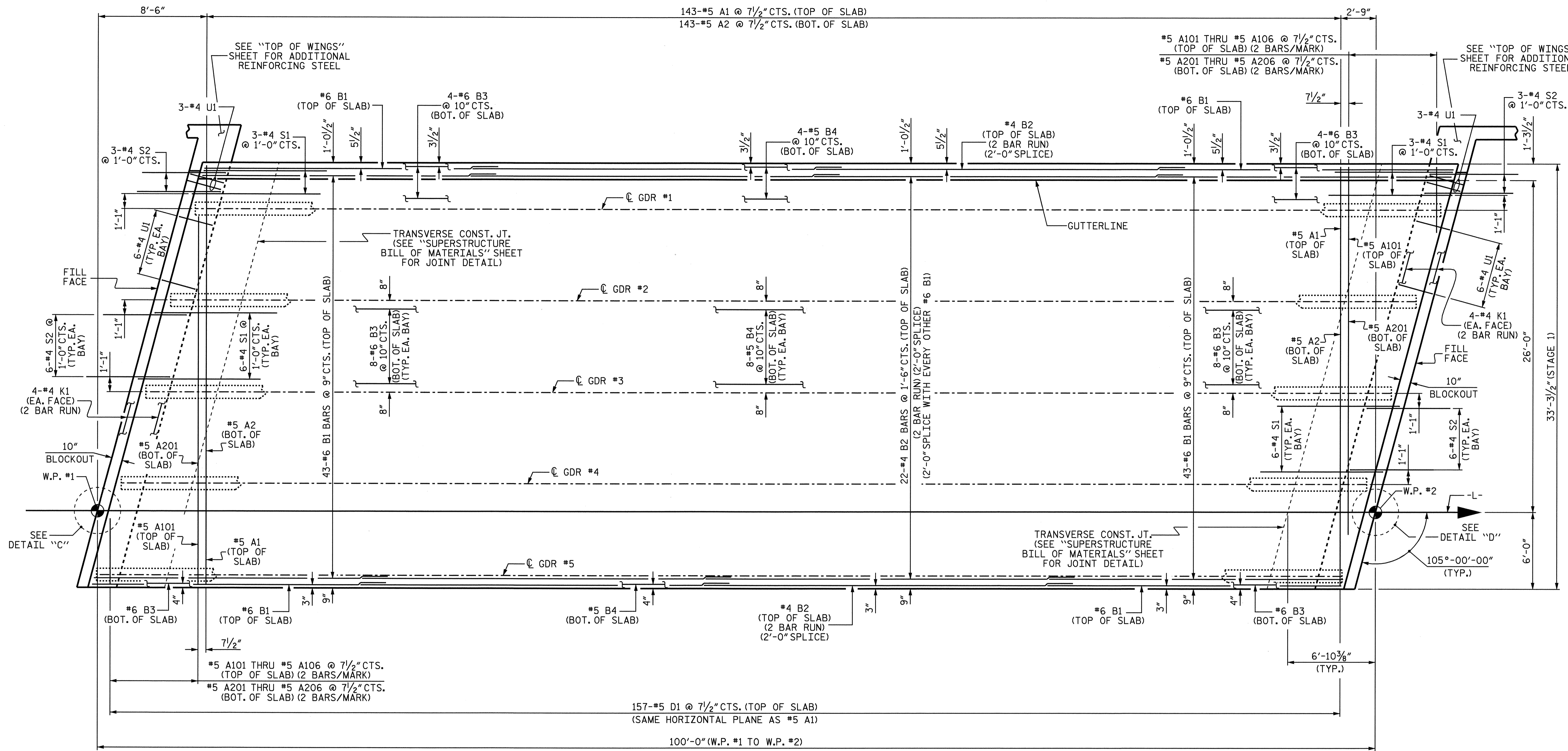
STATE OF NORTH CAROLINA
 DEPARTMENT OF TRANSPORTATION
 RALEIGH

SUPERSTRUCTURE
 TYPICAL SECTION
 (STAGE 2)

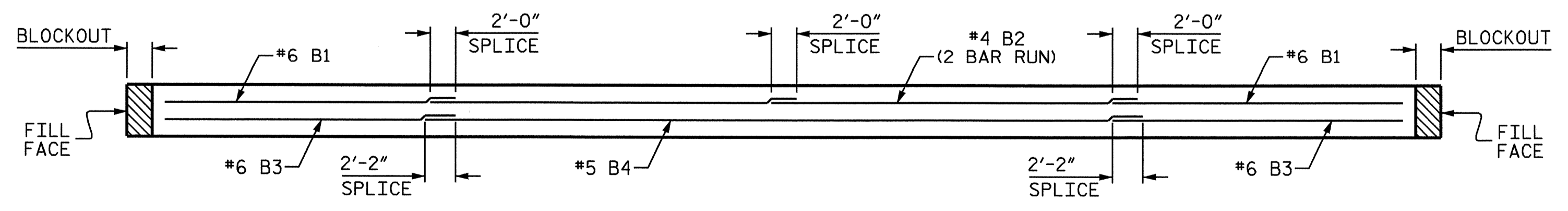


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NO.	BY:	DATE:	NO.	BY:	DATE:	S-43	
1			3			TOTAL SHEETS	
2			4			70	

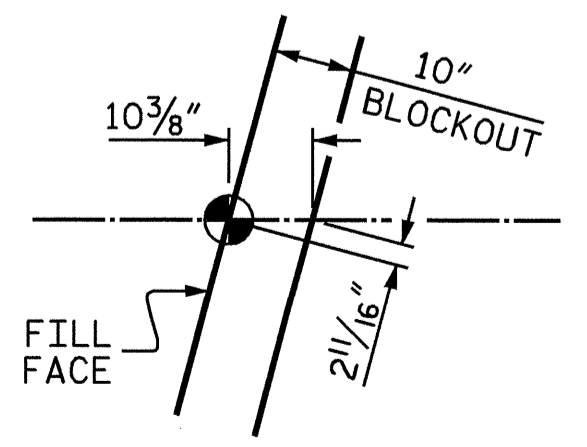
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 CHECKED BY: M.K. BEARD DATE: 10/07



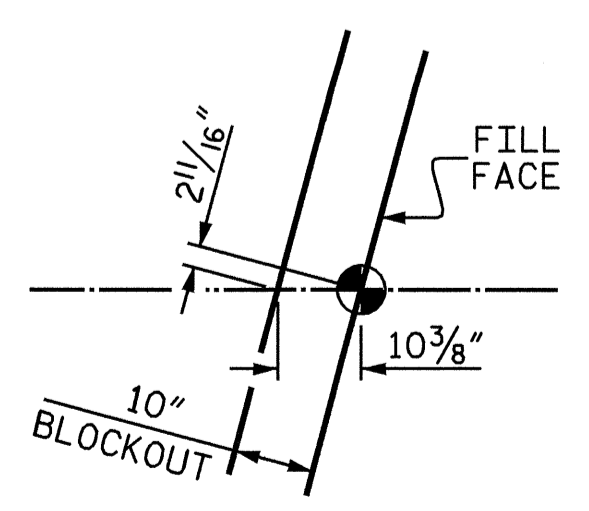
PLAN OF SPAN



SCHEMATIC DIAGRAM OF SLAB



DETAIL "C"



DETAIL "D"

PROJECT NO. B-4252
 ROCKINGHAM COUNTY
 STATION: 31+61.50 -L-

SHEET 1 OF 2

STATE OF NORTH CAROLINA
 DEPARTMENT OF TRANSPORTATION
 RALEIGH

SUPERSTRUCTURE
 PLAN OF SPAN
 (STAGE 1)



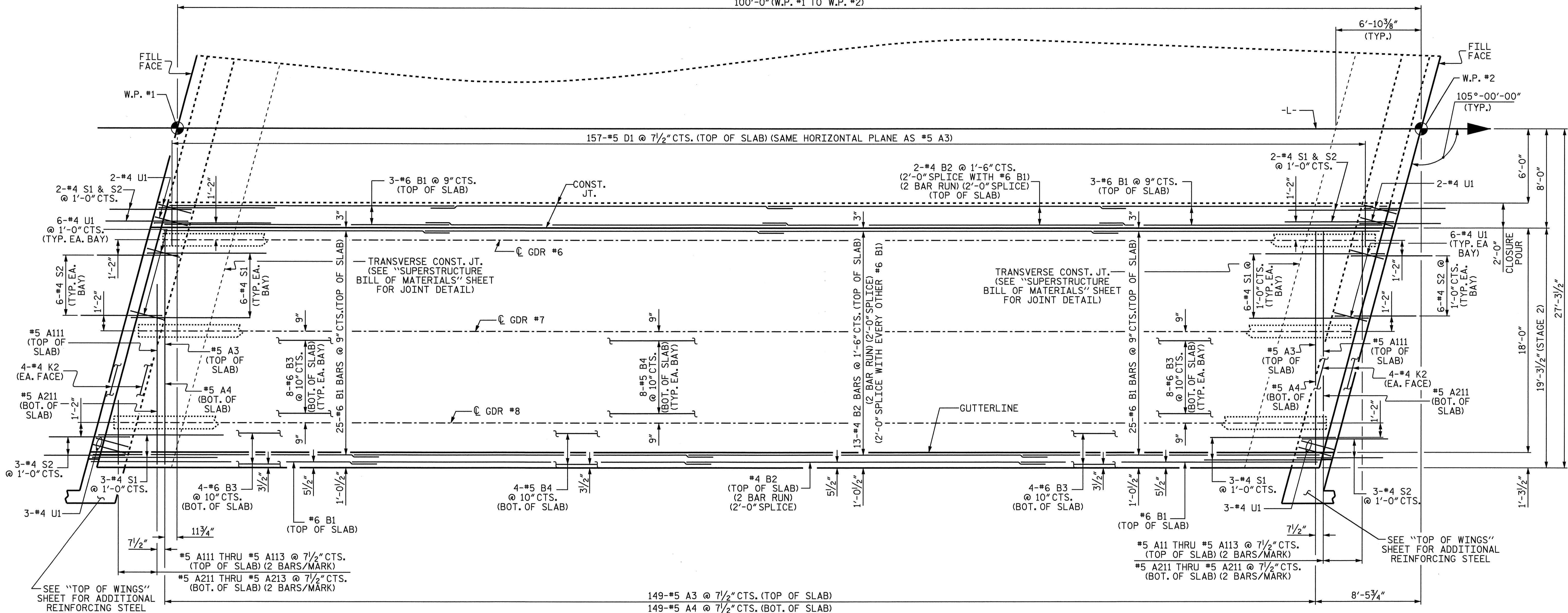
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NO.	BY:	DATE:	NO.	BY:	DATE:	S-44	
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2			4				

DRAWN BY: S. DOMBROWSKI DATE: 3/07
 CHECKED BY: M.K. BEARD DATE: 10/07

20-MAR-2008 15:35
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STR. #2

100'-0" (W.P. #1 TO W.P. #2)



PLAN OF SPAN

PROJECT NO. B-4252
ROCKINGHAM COUNTY
 STATION: 31+61.50 -L-

SHEET 2 OF 2

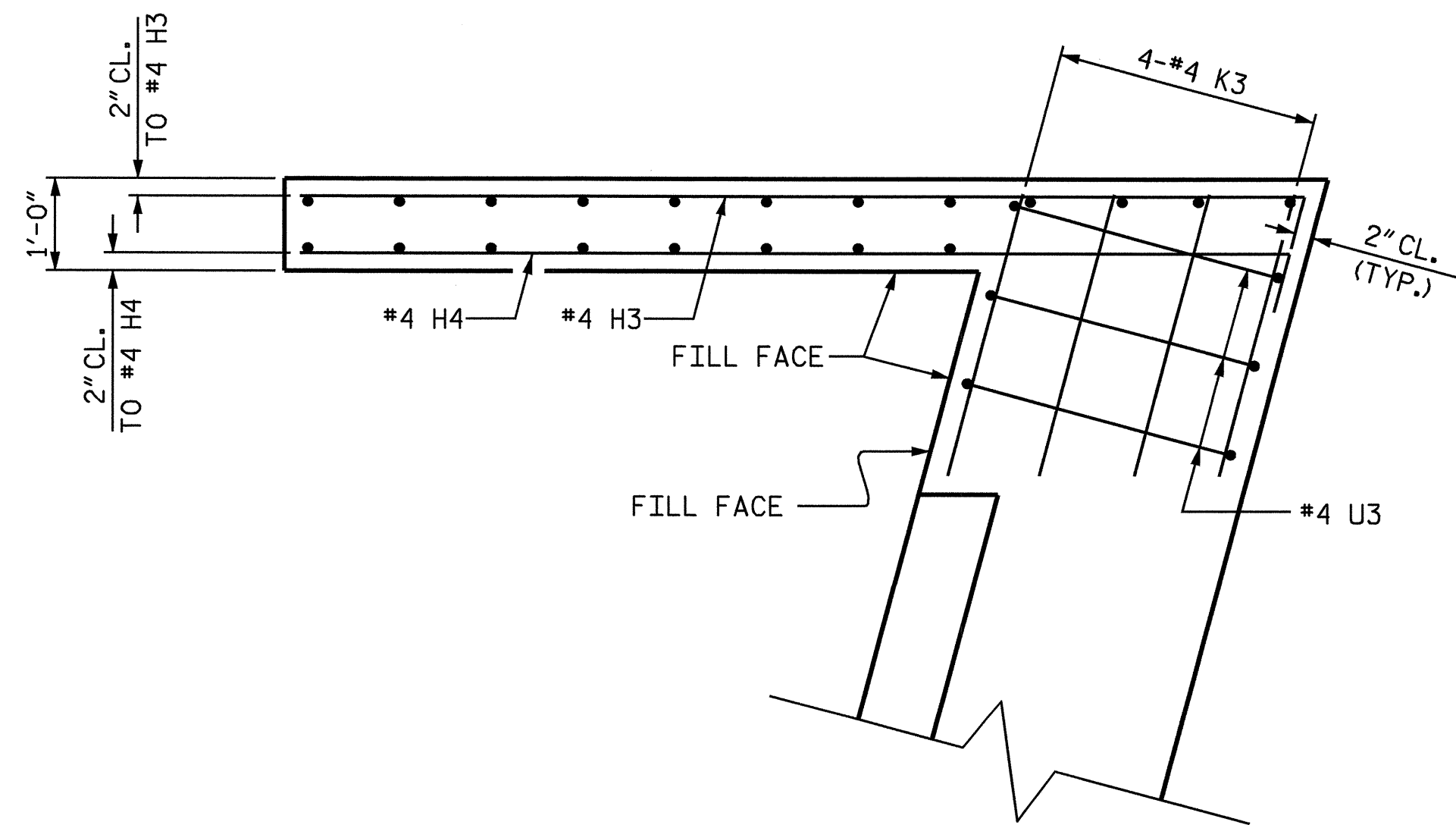
STATE OF NORTH CAROLINA
 DEPARTMENT OF TRANSPORTATION
 RALEIGH

SUPERSTRUCTURE
 PLAN OF SPAN
 (STAGE 2)

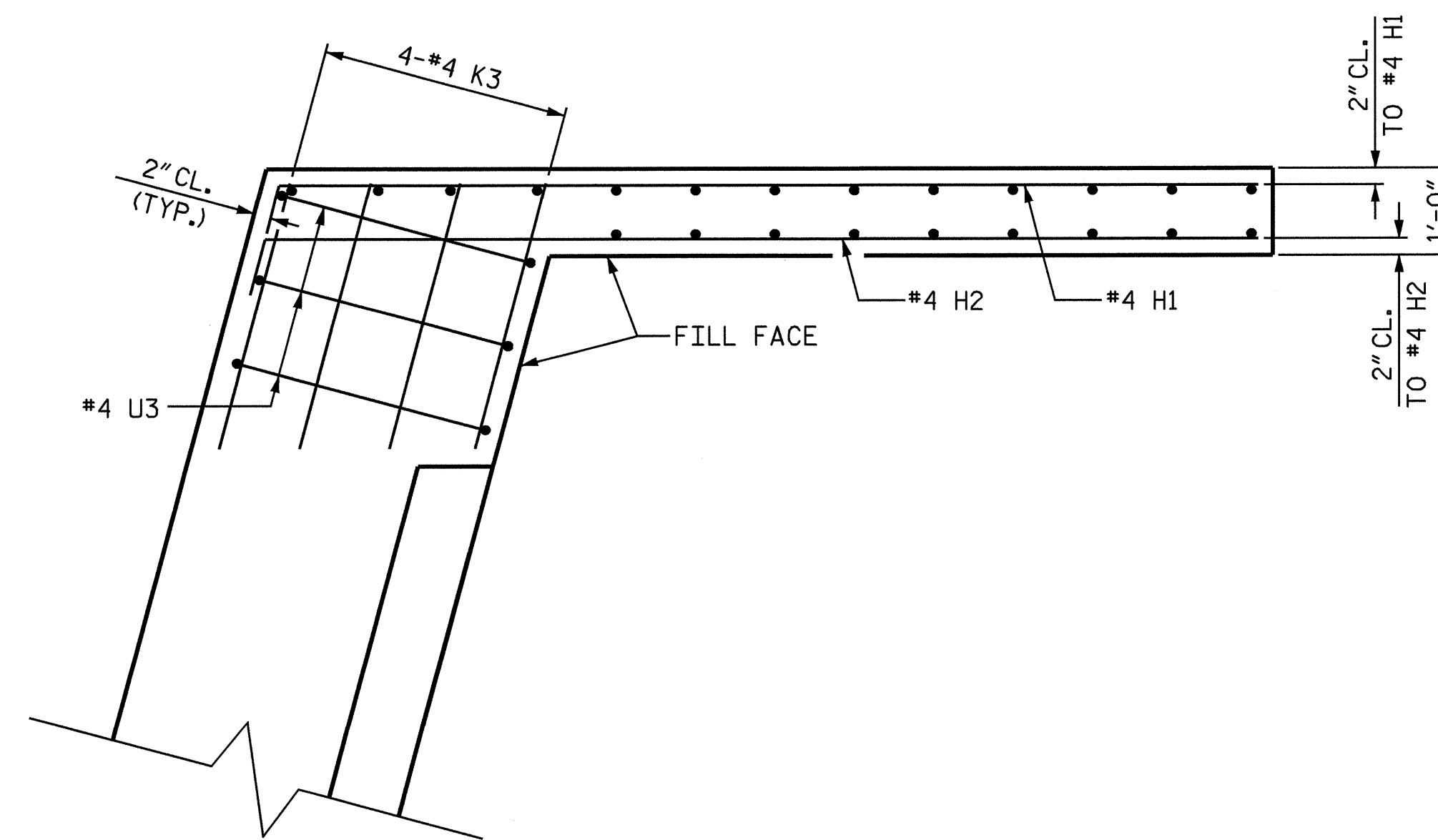
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NO.	BY:	DATE:	NO.	BY:	DATE:	S-45
1			3			TOTAL SHEETS
2			4			70



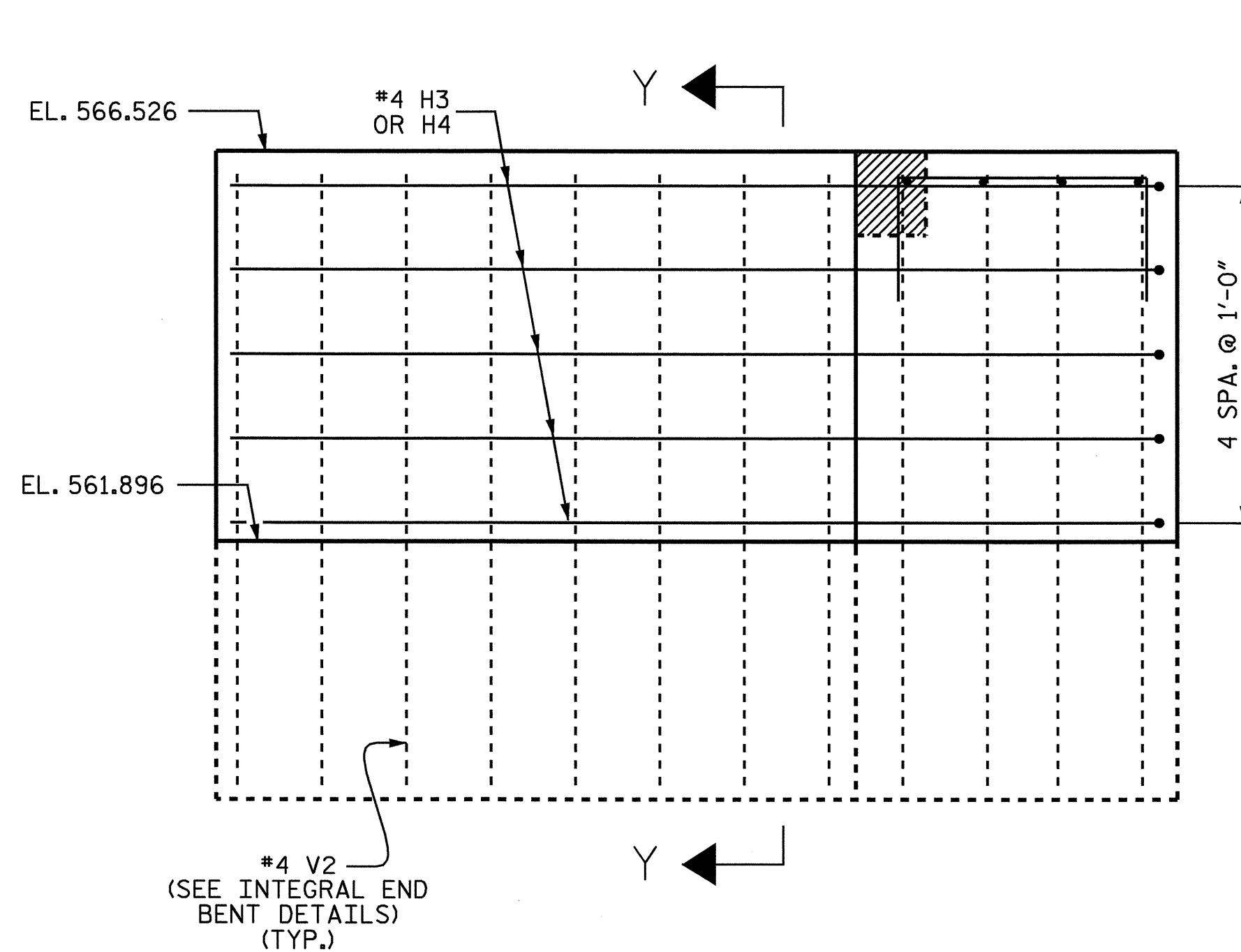
DRAWN BY : S. DOMBROWSKI DATE : 3/07
 CHECKED BY : M.K. BEARD DATE : 10/07



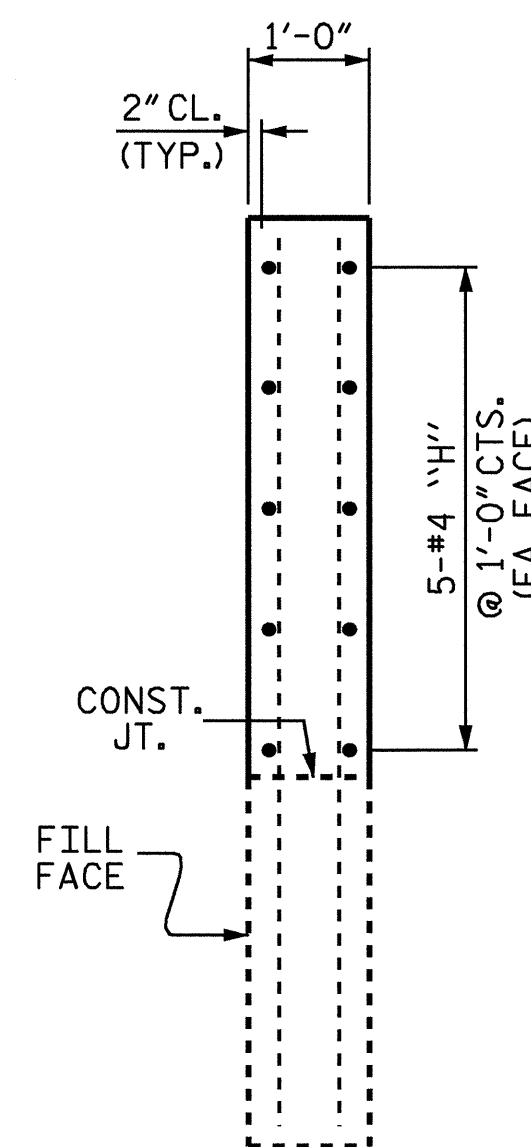
PLAN OF WING-W1
(STAGE 1)



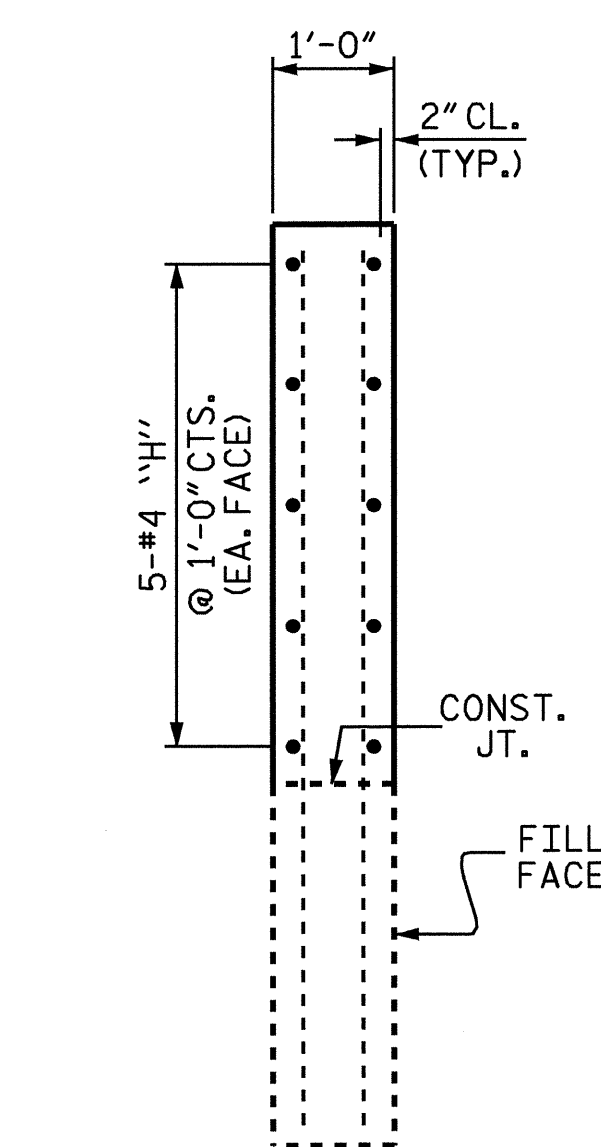
PLAN OF WING-W2
(STAGE 2)



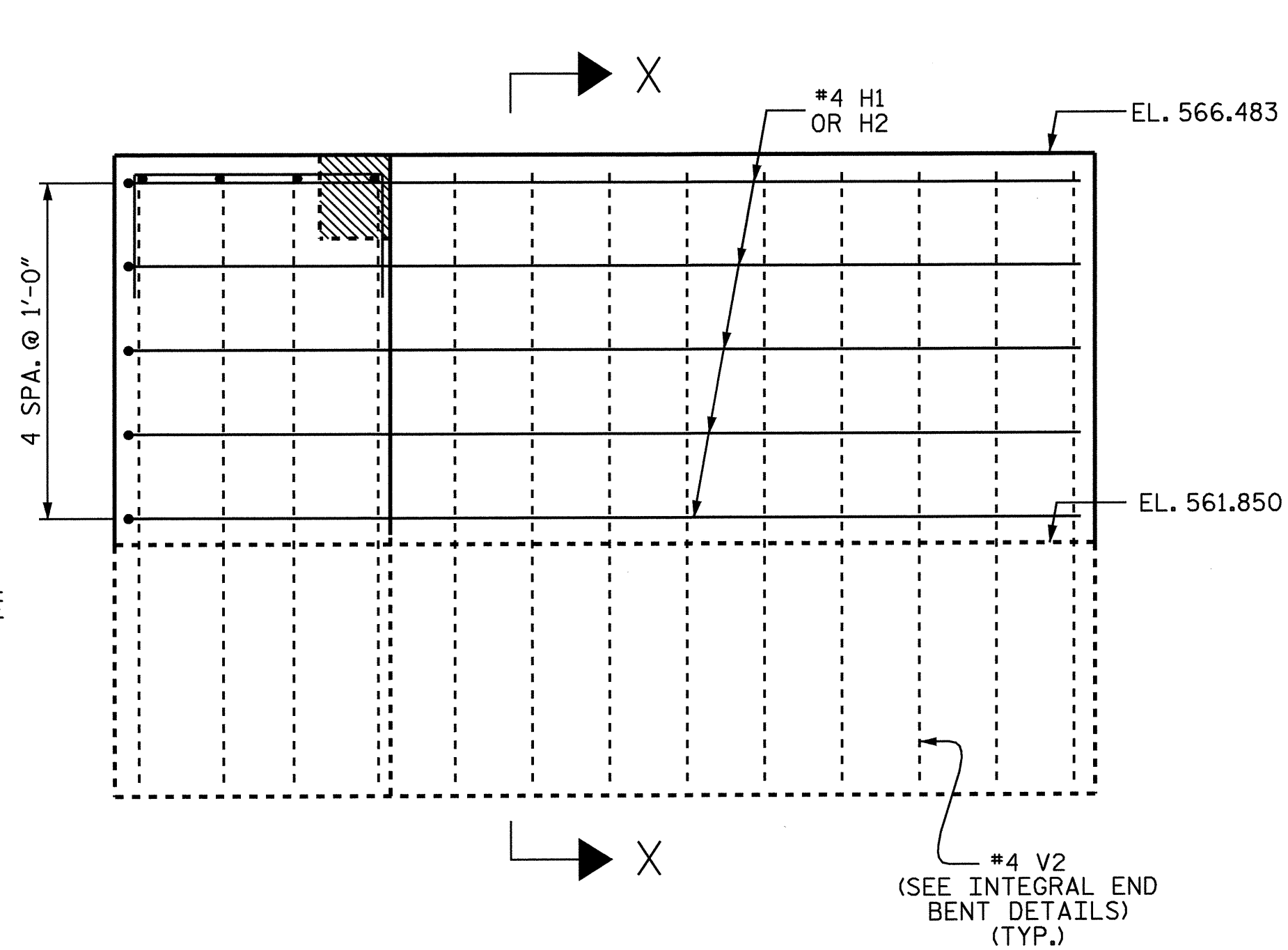
ELEVATION OF WING-W1
(STAGE 1)



SECTION X-X



SECTION Y-Y



ELEVATION OF WING-W2
(STAGE 2)

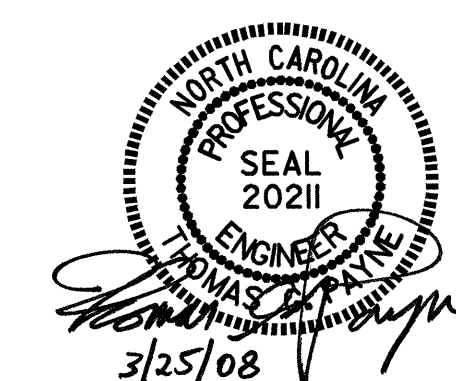
NOTE: LOWER PORTION OF WINGS
BUILT WITH INTEGRAL END BENTS.

PROJECT NO. B-4252
ROCKINGHAM COUNTY
STATION: 31+61.50 -L-

SHEET 1 OF 2

STATE OF NORTH CAROLINA
DEPARTMENT OF TRANSPORTATION
RALEIGH

SUPERSTRUCTURE
TOP OF WINGS
END BENT #1

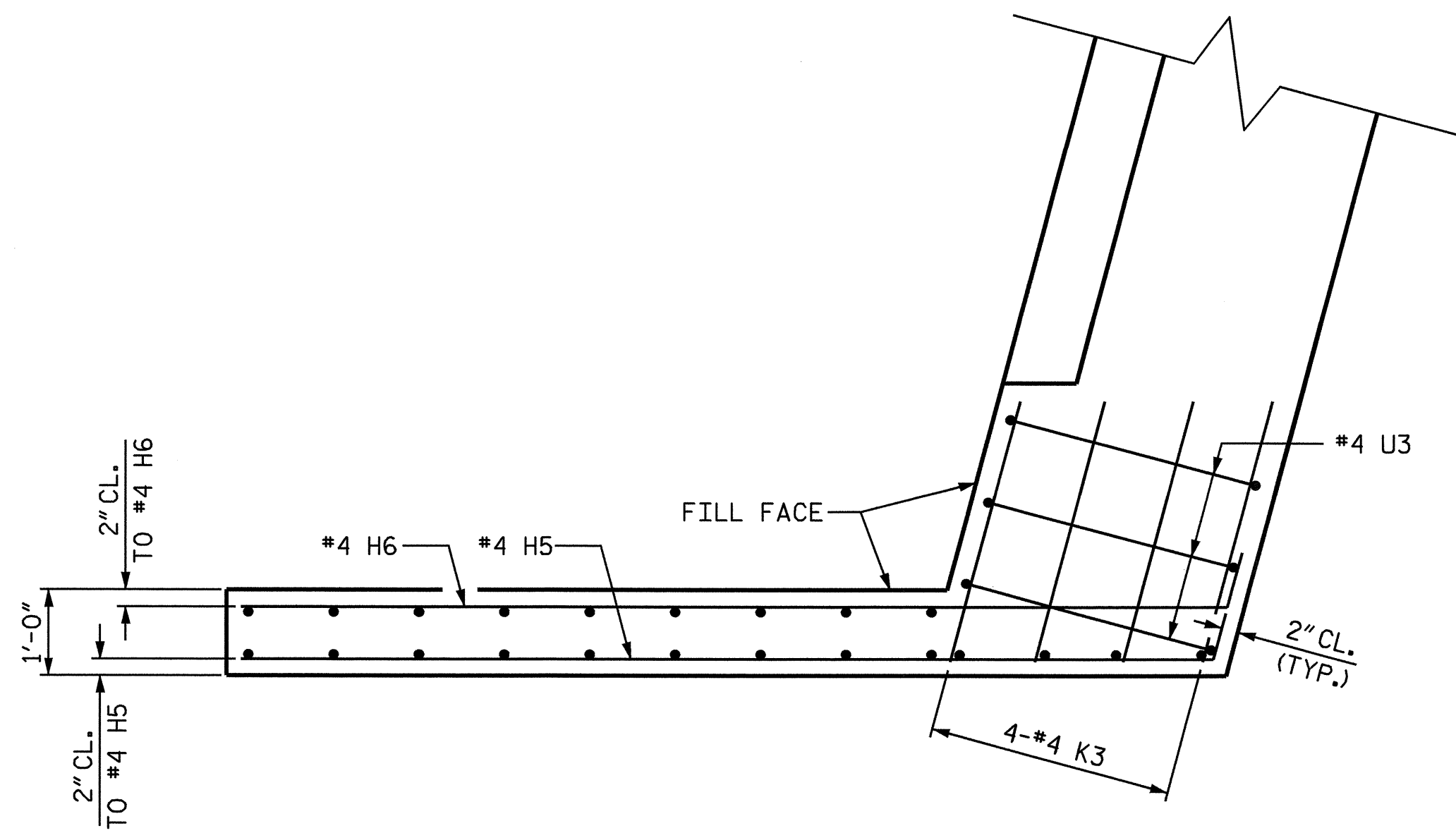


DRAWN BY: S. DOMBROWSKI DATE: 1/07
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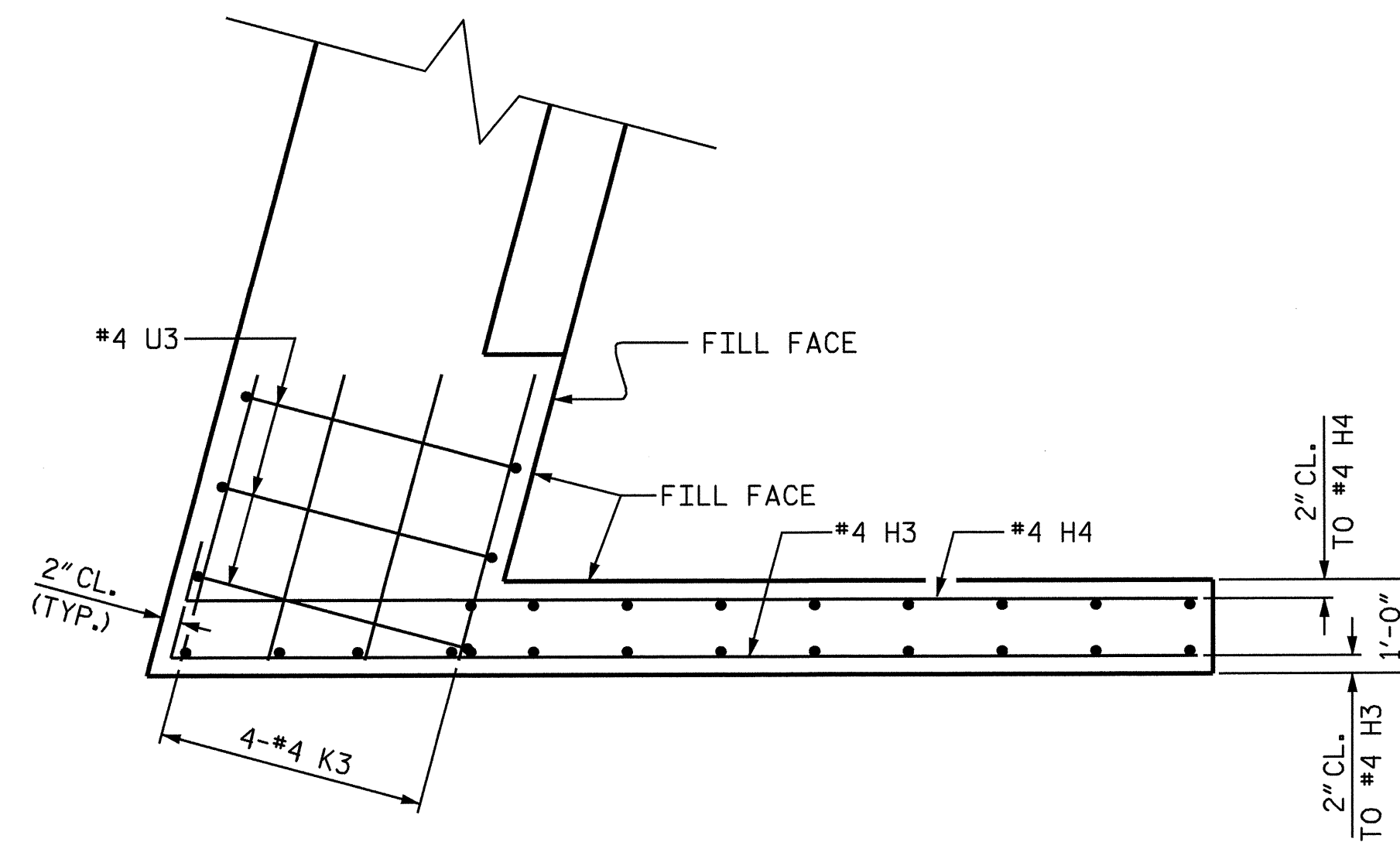
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REVISIONS						SHEET NO.	
NO.	BY:	DATE:	NO.	BY:	DATE:	S-46	
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2			4				

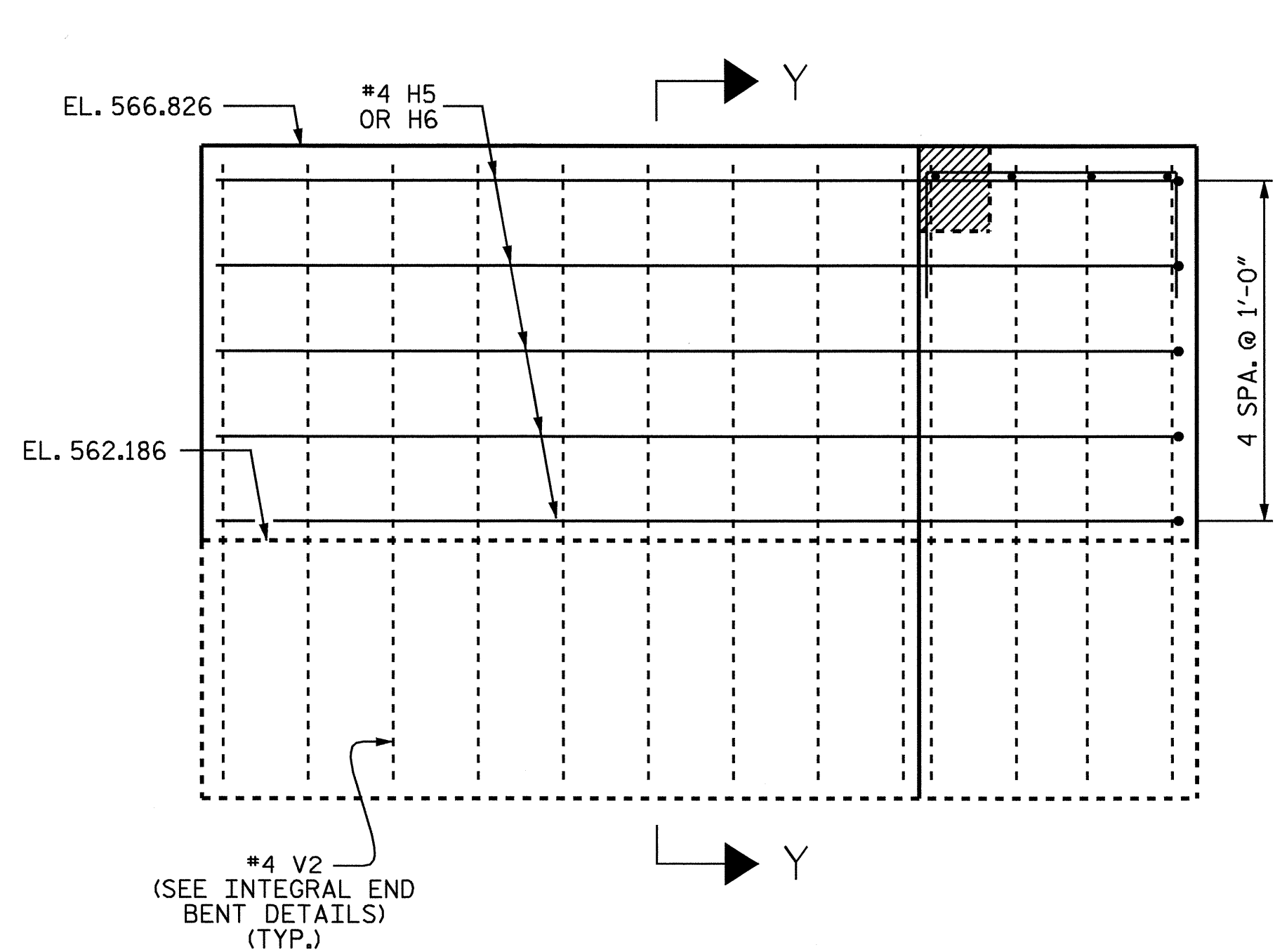
STR.#2



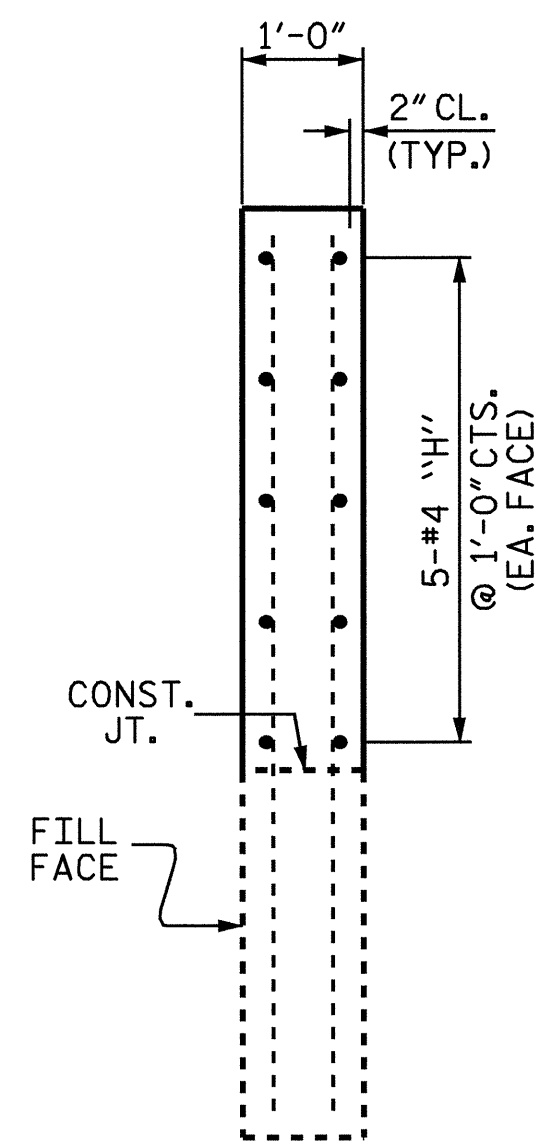
PLAN OF WING-W1
(STAGE 1)



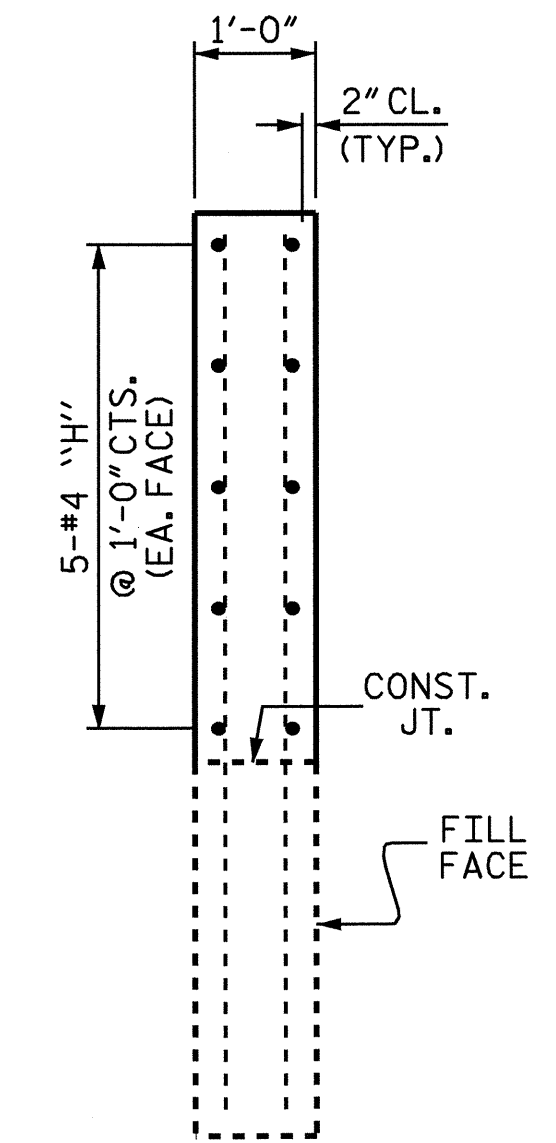
PLAN OF WING-W2
(STAGE 2)



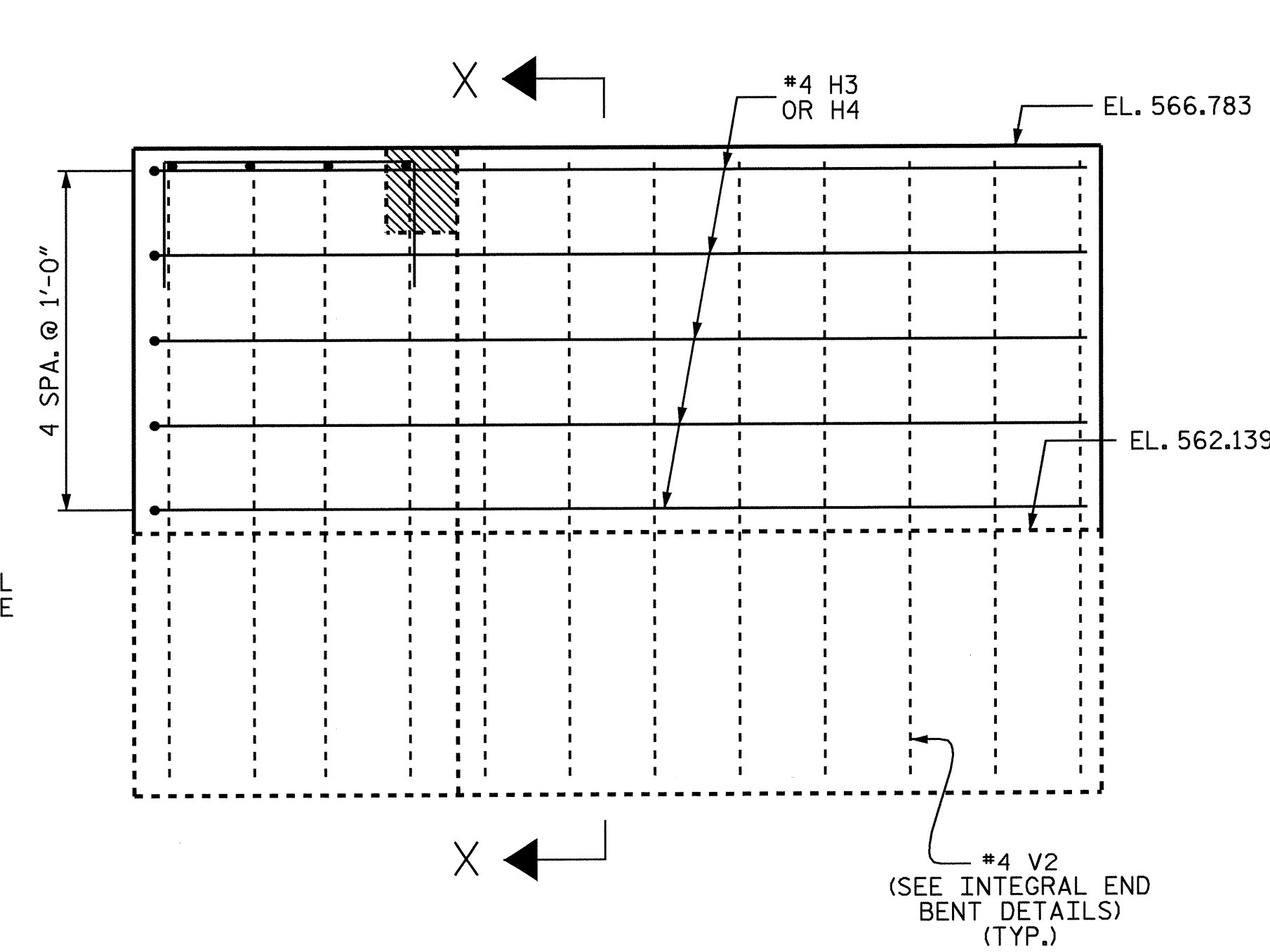
ELEVATION OF WING-W1
(STAGE 1)



SECTION X-X



SECTION Y-Y



ELEVATION OF WING-W2
(STAGE 2)

NOTE: LOWER PORTION OF WINGS
BUILT WITH INTEGRAL END BENTS.

PROJECT NO. B-4252
ROCKINGHAM COUNTY
STATION: 31+61.50 -L-

SHEET 2 OF 2

STATE OF NORTH CAROLINA
DEPARTMENT OF TRANSPORTATION
RALEIGH

SUPERSTRUCTURE
TOP OF WINGS
END BENT #2

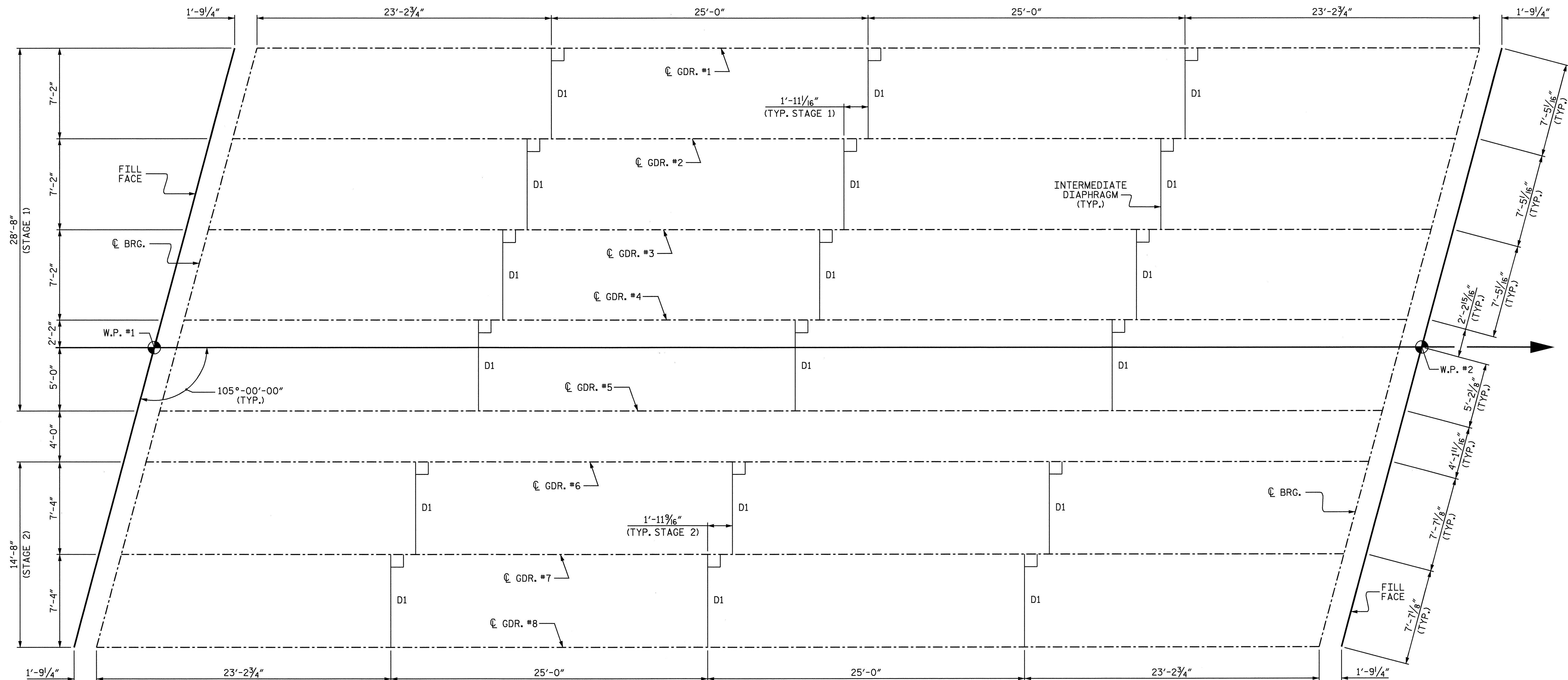


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REVISIONS						SHEET NO.
NO.	BY:	DATE:	NO.	BY:	DATE:	S-47
1			3			TOTAL SHEETS
2			4			70

STR.#2



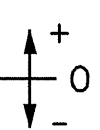
FRAMING PLAN

DEAD LOAD DEFLECTION TABLE FOR GIRDERS

TENTH POINTS	0	.1	.2	.3	.4	.5	.6	.7	.8	.9	0
DEFLECTION DUE TO WEIGHT OF GIRDER	0.000	-0.028	-0.052	-0.072	-0.084	-0.088	-0.084	-0.072	-0.052	-0.028	0.000
DEFLECTION DUE TO WEIGHT OF SLAB *	0.000	-0.107	-0.200	-0.273	-0.319	-0.335	-0.319	-0.273	-0.200	-0.107	0.000
DEFLECTION DUE TO WEIGHT OF BARRIER RAIL	0.000	-0.009	-0.018	-0.024	-0.029	-0.030	-0.029	-0.024	-0.018	-0.009	0.000
TOTAL DEAD LOAD DEFLECTION	0.000	-0.144	-0.270	-0.369	-0.432	-0.453	-0.432	-0.369	-0.270	-0.144	0.000
VERTICAL CURVE ORDINATE	0.000	0.000	0.000	0.000	0.000	0.000	0.000	0.000	0.000	0.000	0.000
REQUIRED CAMBER	0"	1 3/4"	3 1/4"	4 7/16"	5 3/16"	5 7/16"	5 3/16"	4 7/16"	3 1/4"	1 3/4"	0"

* INCLUDES SLAB, BUILDUPS & STAY-IN-PLACE FORMS.
 ALL VALUES ARE SHOWN IN FEET (DECIMAL FORM), EXCEPT "REQUIRED CAMBER", WHICH IS GIVEN IN INCHES (FRACTION FORM).

SIGN CONVENTION FOR DEAD LOAD DEFLECTION



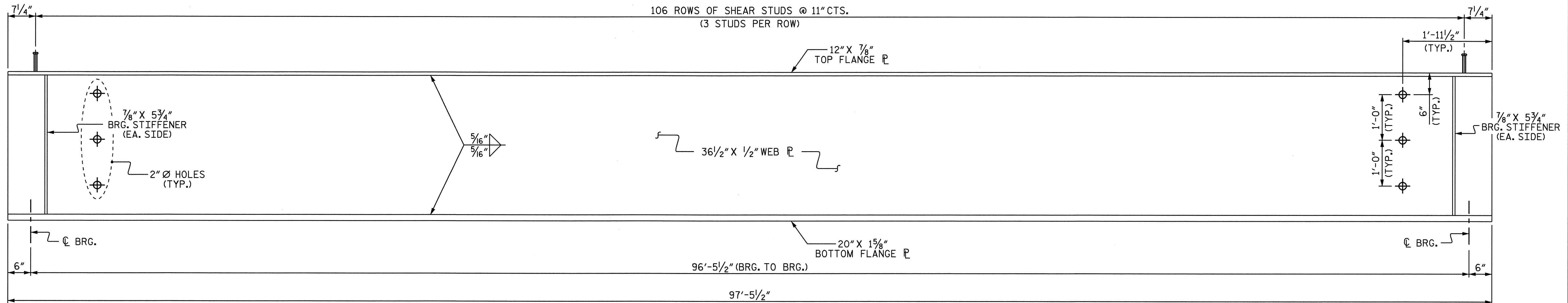
PROJECT NO. B-4252
ROCKINGHAM COUNTY
 STATION: 31+61.50 -L-

STATE OF NORTH CAROLINA
 DEPARTMENT OF TRANSPORTATION
 RALEIGH
 SUPERSTRUCTURE
 FRAMING PLAN
 AND
 DEAD LOAD DEFLECTIONS

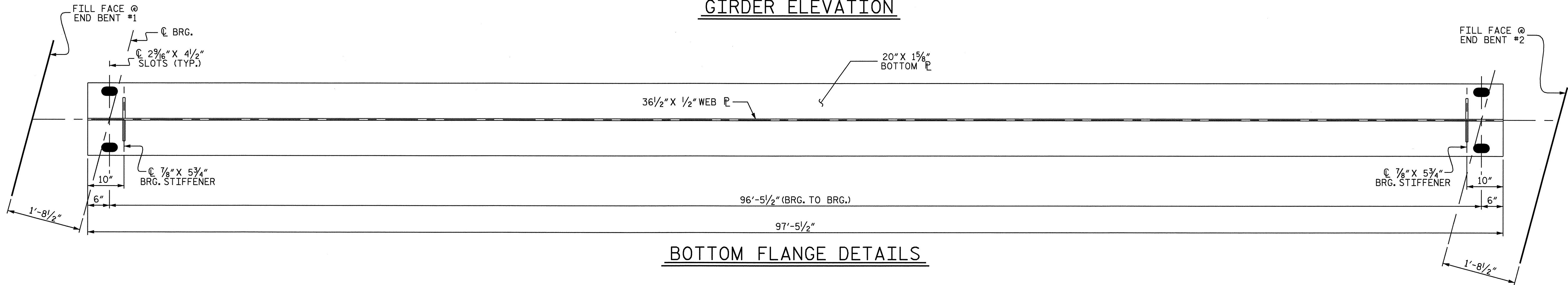


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NO.	BY:	DATE:	NO.	BY:	DATE:	S-48	
1			3			TOTAL SHEETS	
2			4			70	

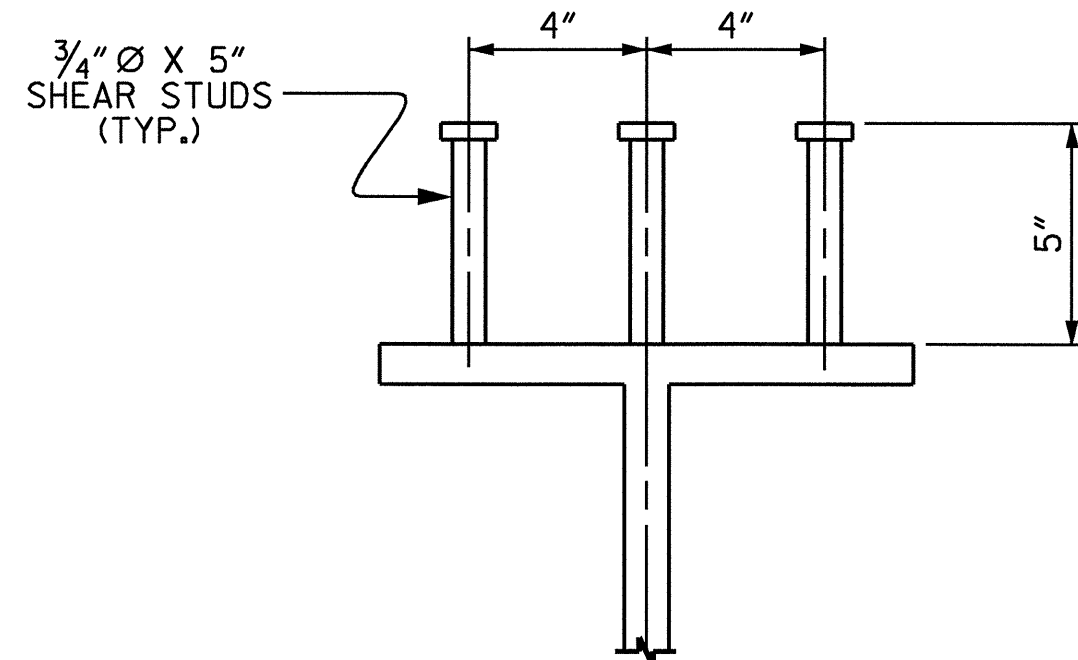
DRAWN BY: S. DOMBROWSKI DATE: 3/07
 CHECKED BY: M.K. BEARD DATE: 10/07



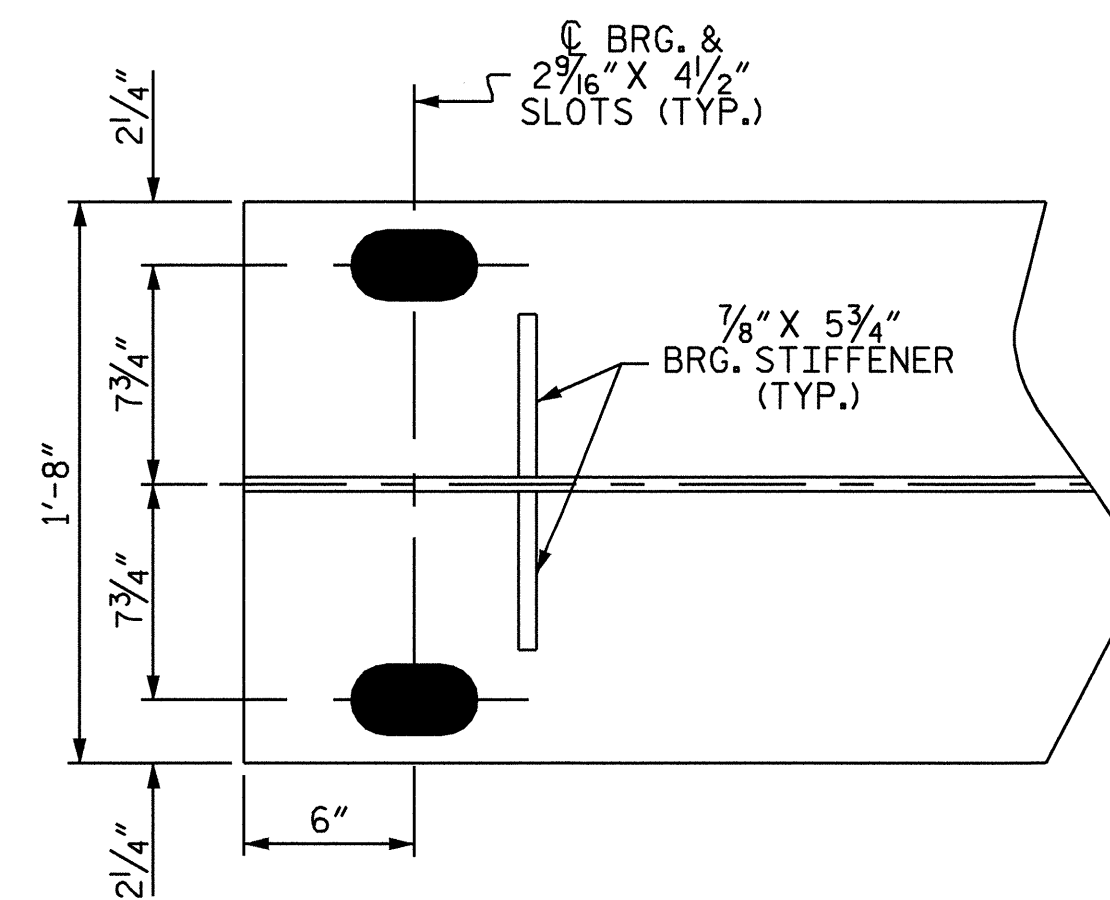
GIRDER ELEVATION



BOTTOM FLANGE DETAILS



SHEAR STUD DETAILS



PARTIAL BOTTOM FLANGE DETAIL

PROJECT NO. B-4252
ROCKINGHAM COUNTY
 STATION: 31+61.50 -L-

SHEET 1 OF 2

STATE OF NORTH CAROLINA
 DEPARTMENT OF TRANSPORTATION
 RALEIGH

SUPERSTRUCTURE
 STRUCTURAL STEEL

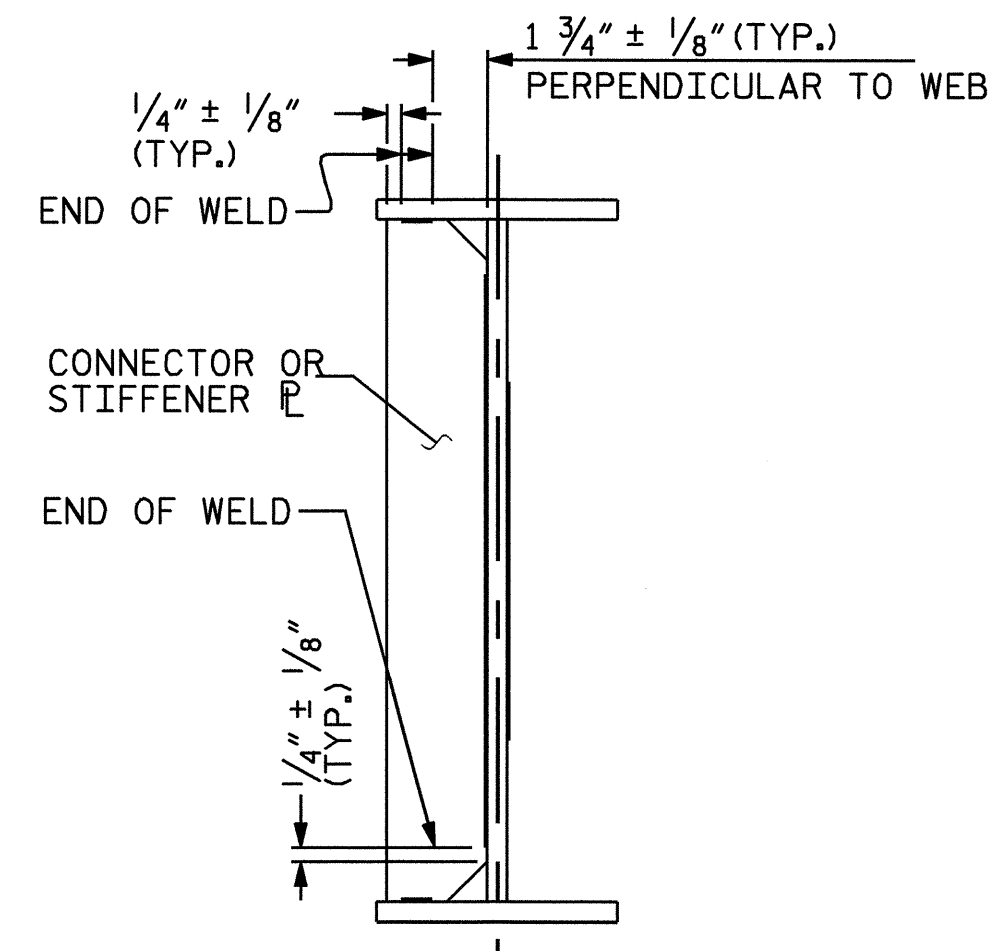


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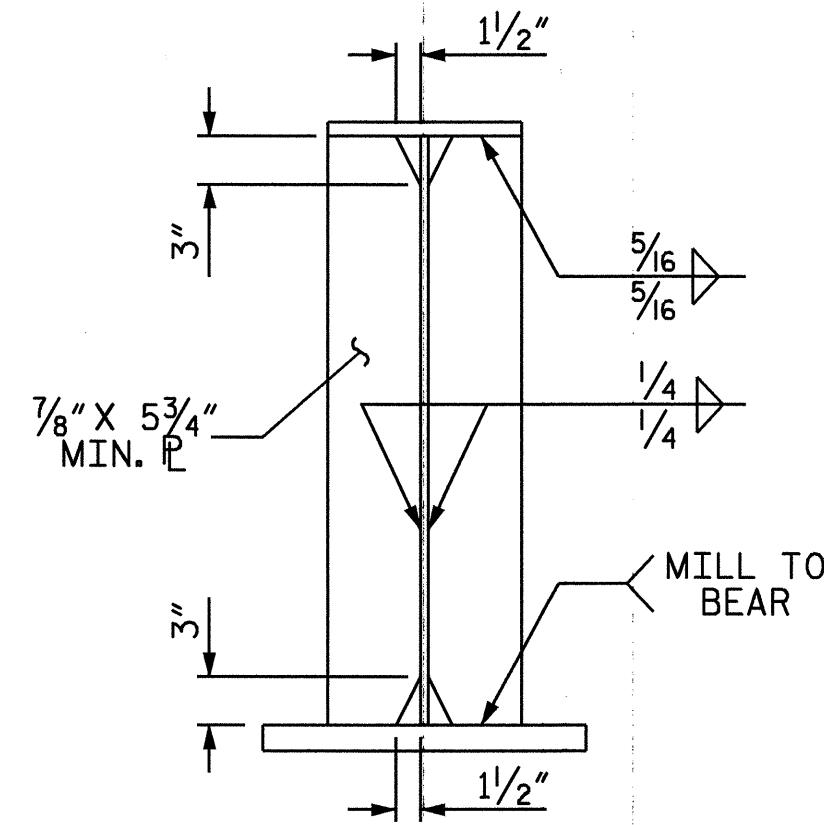
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REVISIONS						SHEET NO.
NO.	BY:	DATE:	NO.	BY:	DATE:	S-49
1			3			TOTAL SHEETS
2			4			70

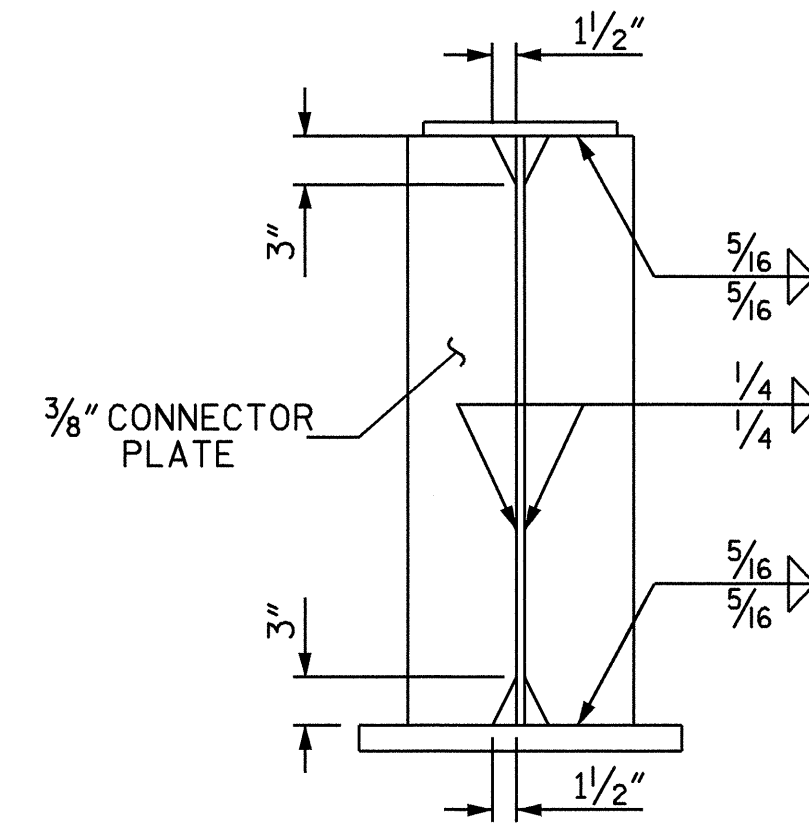
STR.#2



TYPICAL STIFFENER OR
CONNECTOR PLATE CONNECTIONS
WELD TERMINATION DETAILS



BEARING STIFFENER DETAIL
(AT END BENTS)



CONNECTOR PLATE DETAIL

NOTES

ALL STRUCTURAL STEEL SHALL BE AASHTO M270 GRADE 50W.

ALL DIMENSIONS SHOWN ARE HORIZONTAL OR VERTICAL, UNLESS OTHERWISE NOTED.

BEARING STIFFENERS ARE TO BE PLACED NORMAL TO THE WEB OF THE GIRDER AND SHALL BE PLUMB.

SHOP SPLICES ARE PERMITTED TO LIMIT THE MAXIMUM REQUIRED FLANGE PIECE LENGTHS TO 60 FEET AND WEB PIECE LENGTHS TO 45 FEET. PERMITTED FLANGE AND WEB SHOP SPLICES SHALL NOT BE LOCATED WITHIN 15 FEET OF MAXIMUM DEAD LOAD DEFLECTION. KEEP 2 FEET MINIMUM BETWEEN WEB AND FLANGE SHOP SPLICES. KEEP 6" MINIMUM BETWEEN CONNECTOR PLATE OR TRANSVERSE STIFFENER WELDS AND WEB OR FLANGE SHOP SPLICES.

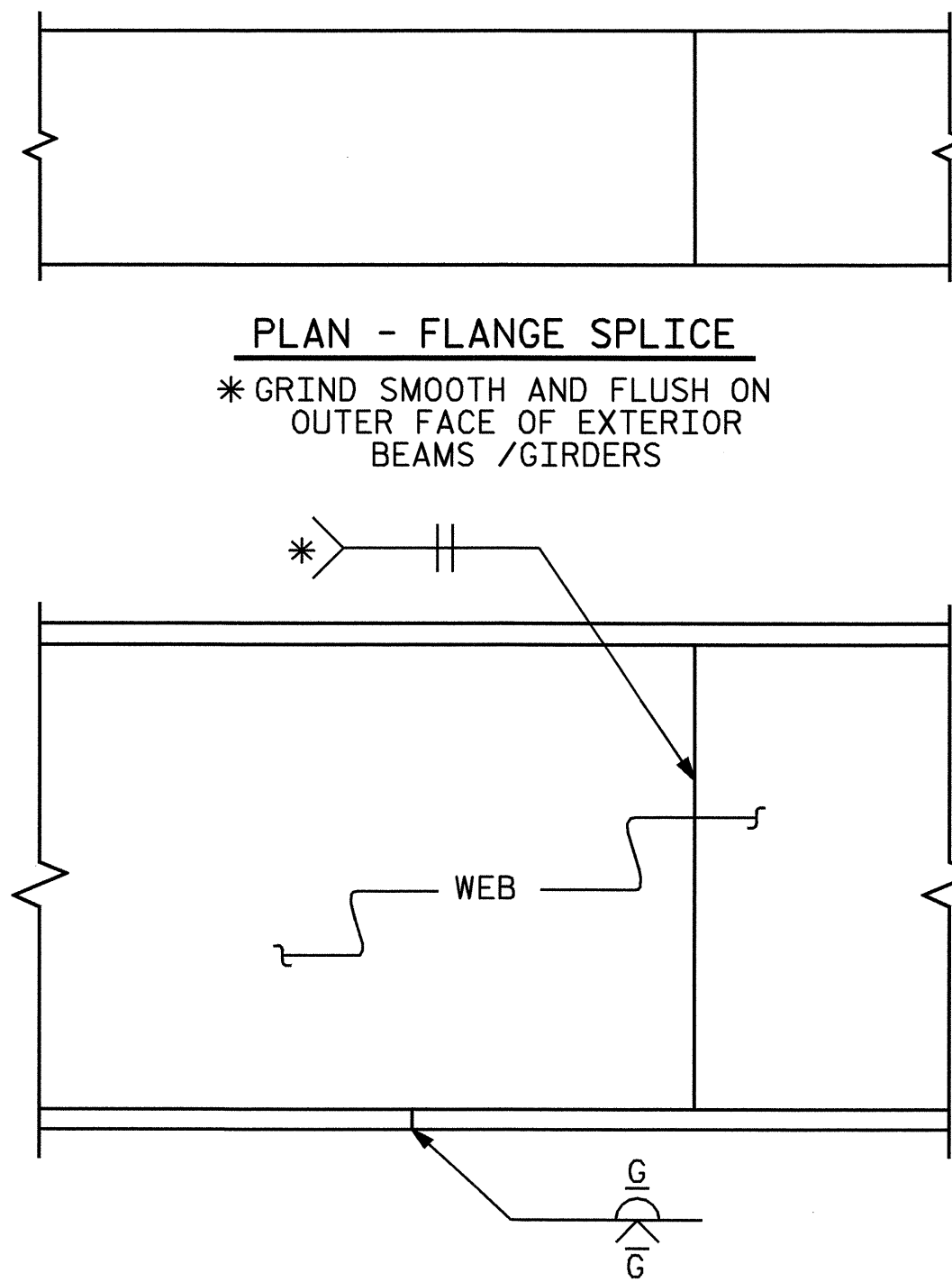
STUDS ON GIRDERS MAY BE SHIFTED UP TO 1" IF NECESSARY TO CLEAR FLANGE SPLICE WELD.

ALL SURFACES OF BEARING PLATES SHALL BE SMOOTH AND STRAIGHT.

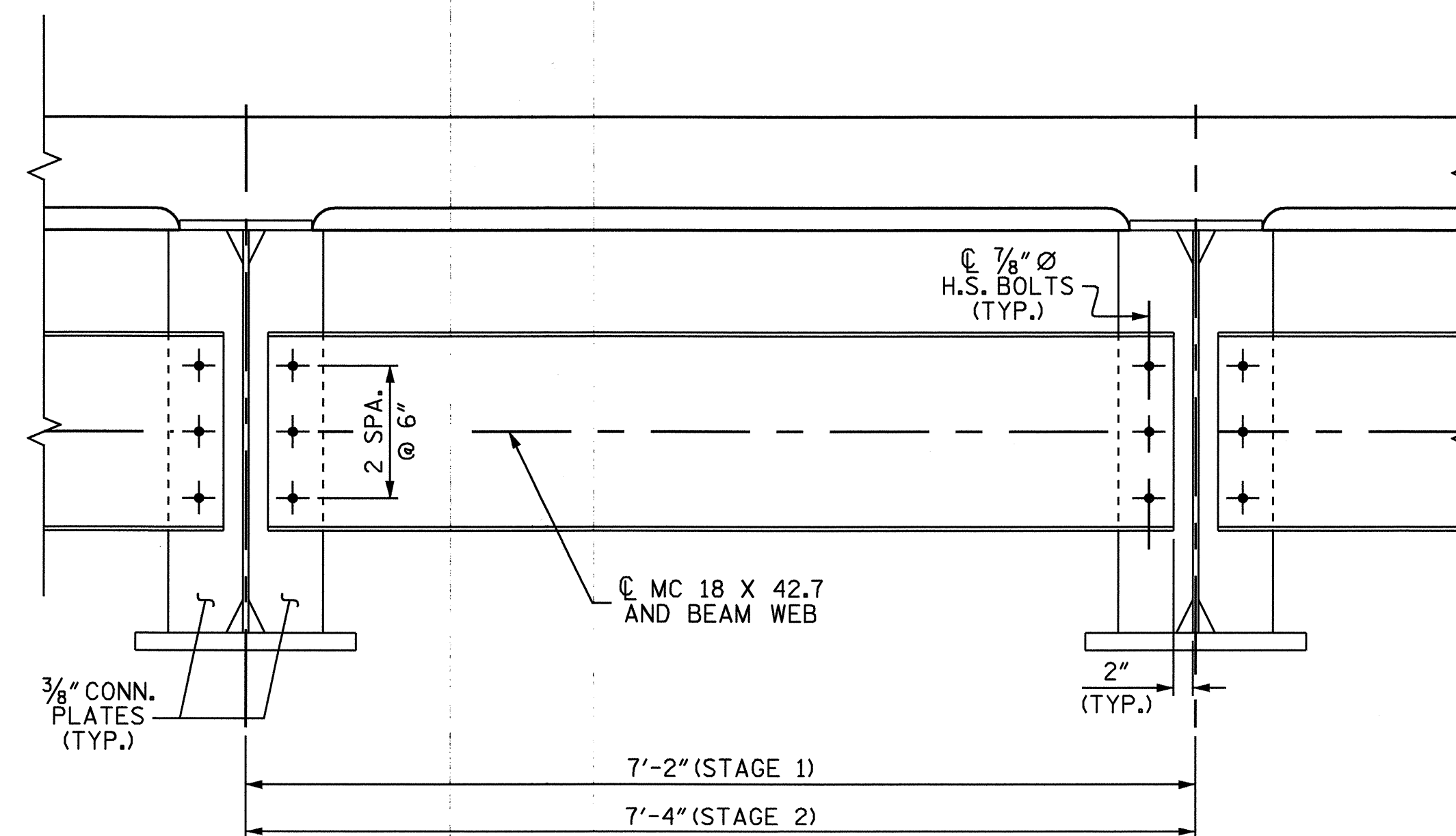
ENDS OF GIRDERS SHALL BE PLUMB.

ALL SURFACES OF BEARING PLATES SHALL BE SMOOTH AND STRAIGHT.

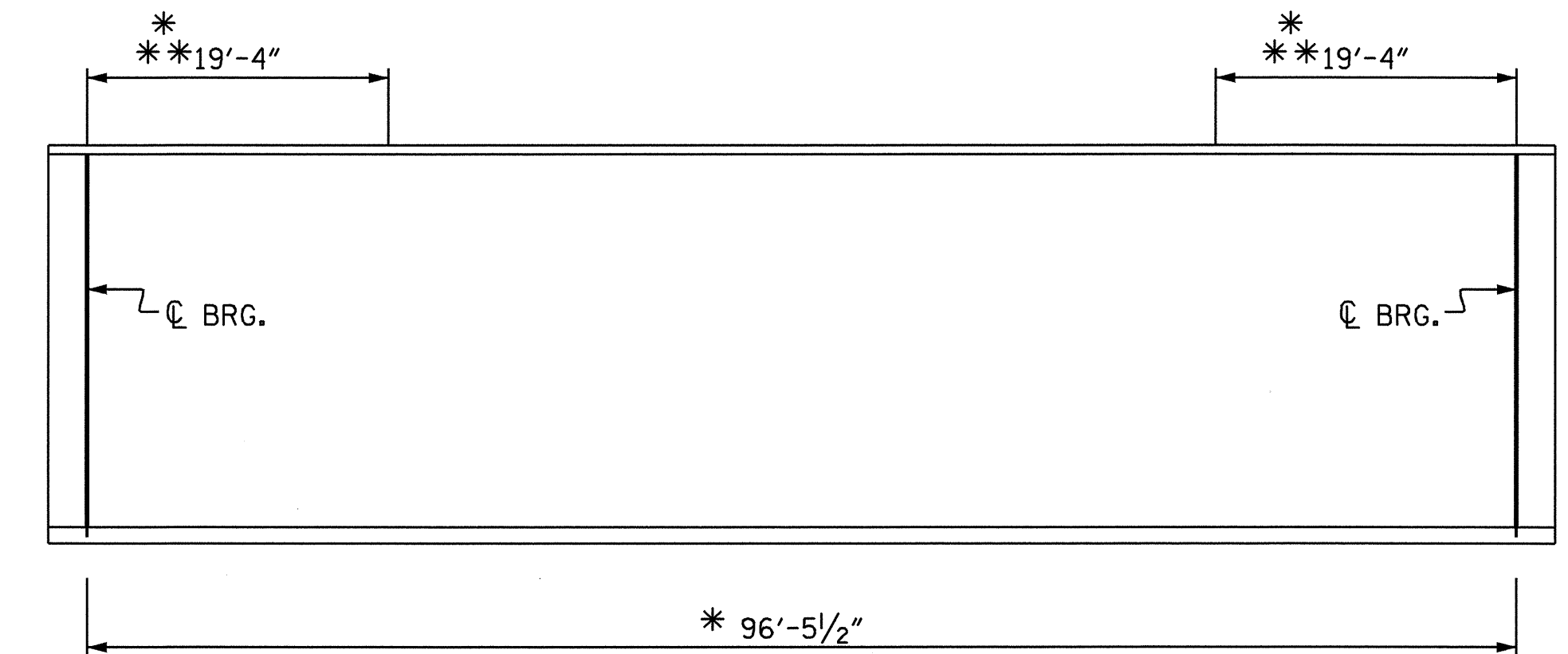
FOR "DEAD LOAD DEFLECTION TABLE FOR GIRDERS", SEE "SUPERSTRUCTURE BILL OF MATERIAL" SHEET



TYPICAL FLANGE AND WEB BUTT JOINT



INTERMEDIATE DIAPHRAGM (D1)



CHARPY V-NOTCH TEST FOR GIRDERS

* CHARPY V-NOTCH TESTS ARE REQUIRED FOR ALL TOP OR BOTTOM FLANGE PLATES WHICH FALL WITHIN THESE LIMITS, ALL WEB PLATES, AND ALL SPLICE PLATES. IF A PERMITTED SHOP FLANGE SPLICE IS NOT USED, CHARPY V-NOTCH TESTS WILL BE REQUIRED FOR THE ENTIRE FLANGE PLATE. FOR CHARPY V-NOTCH TESTS, SEE ARTICLE 1072-9 OF THE STANDARD SPECIFICATIONS.

** NO WELDING OF FORMS OR FALSEWORK TO THE TOP FLANGE WILL BE PERMITTED IN THIS REGION.

PROJECT NO. B-4252
ROCKINGHAM COUNTY
STATION: 31+61.50 -L-

SHEET 2 OF 2

STATE OF NORTH CAROLINA
DEPARTMENT OF TRANSPORTATION
RALEIGH

SUPERSTRUCTURE
STRUCTURAL STEEL



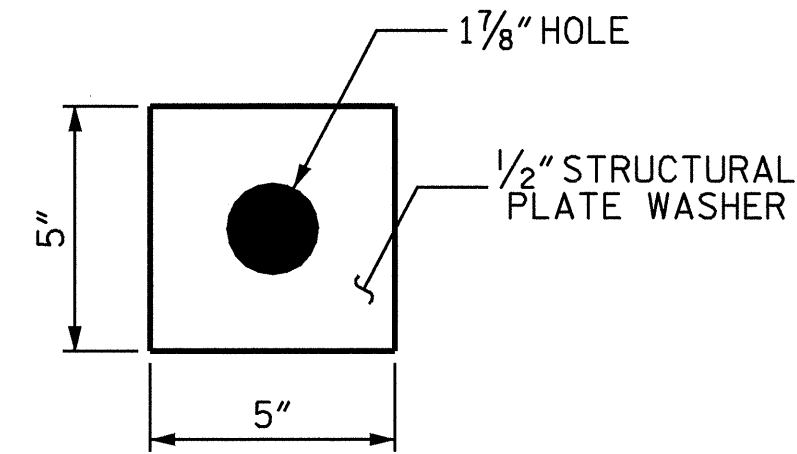
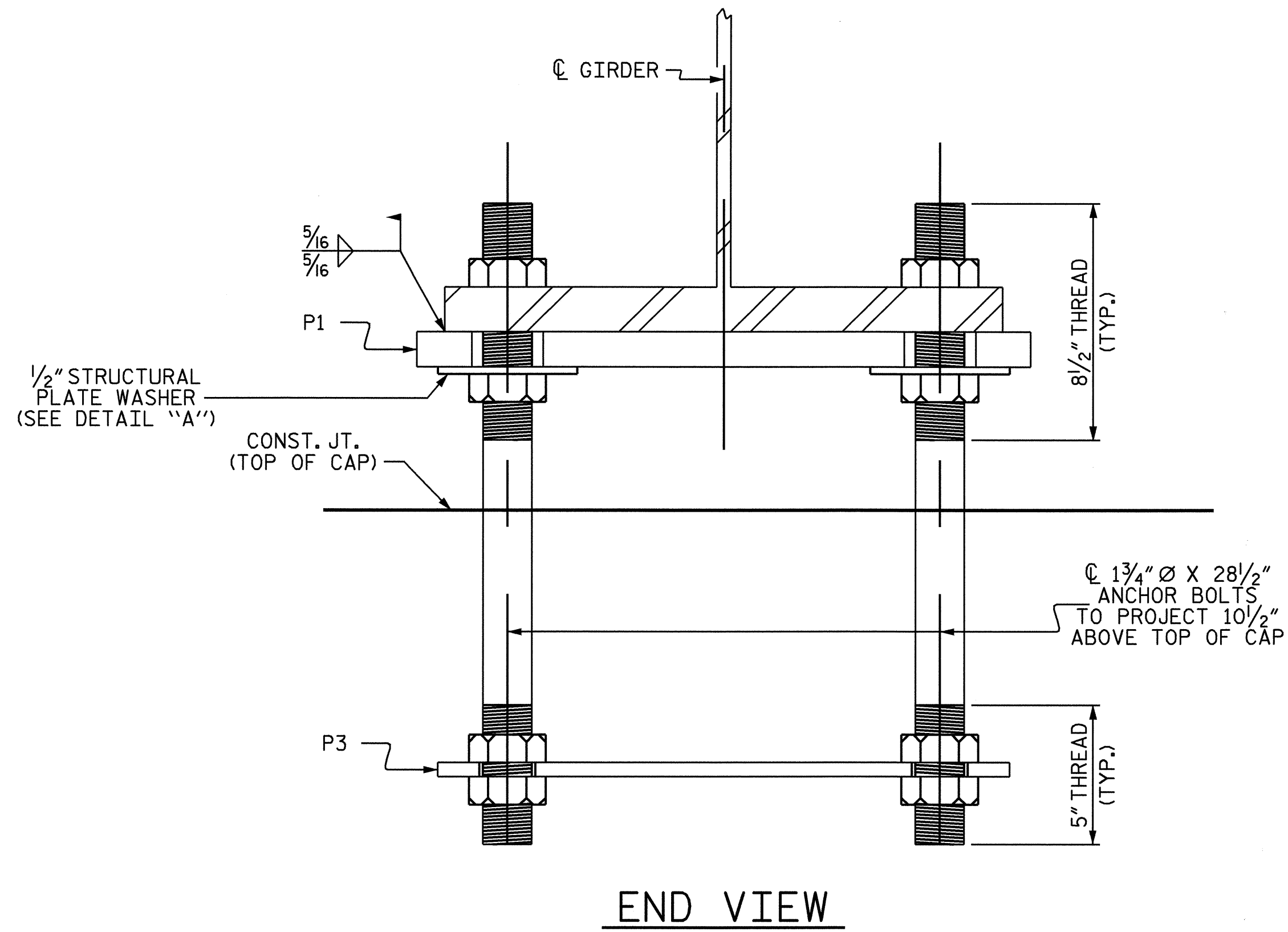
DRAWN BY : S. DOMBROWSKI DATE : 12/06
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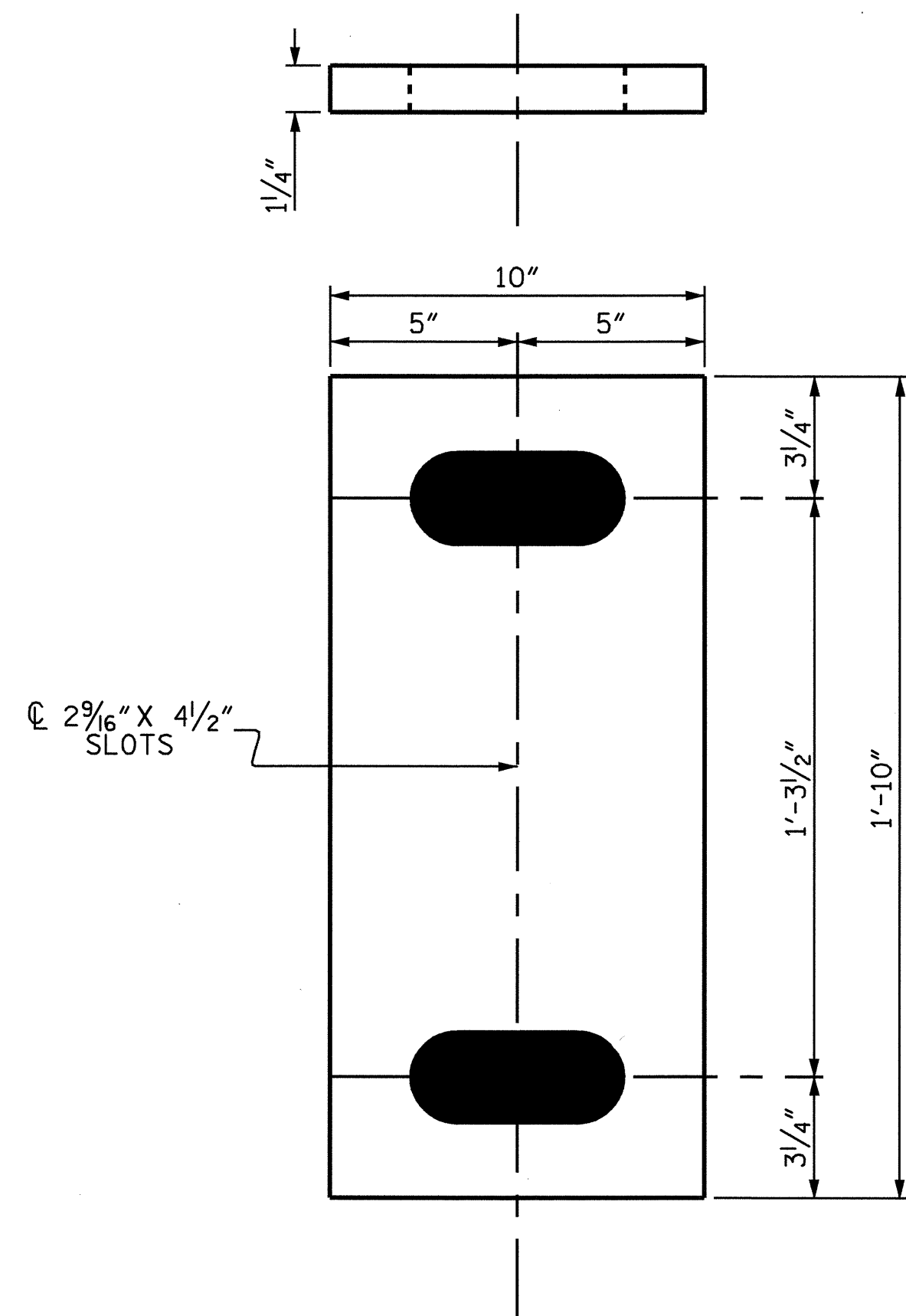
REVISIONS						SHEET NO.	
NO.	BY:	DATE:	NO.	BY:	DATE:	S-50	
1			3			TOTAL SHEETS	
2			4			70	

STR. #2

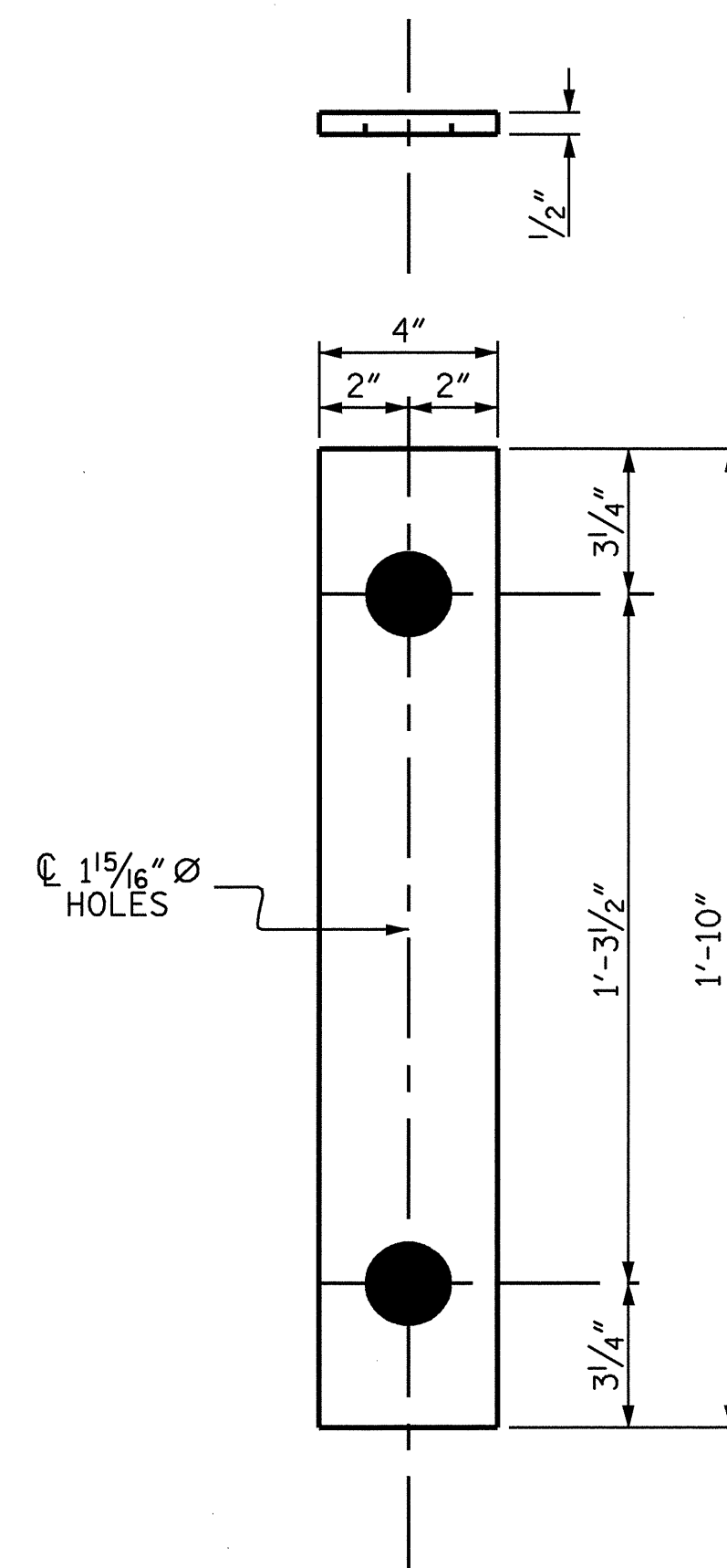
NC005



DETAIL "A"



LEVELING PLATE (P1)
(16 REQUIRED)



ANCHORAGE PLATE (P3)
(16 REQUIRED)

NOTES

STRUCTURAL PLATE WASHERS SHALL BE AASHTO M270 GRADE 50W.

FOR AASHTO M270 GRADE 50W STRUCTURAL STEEL, LEVELING PLATES AND ANCHORAGE PLATES SHALL BE AASHTO M270 GRADE 50W AND SHALL NOT BE GALVANIZED. ANCHOR BOLTS, NUTS AND WASHERS SHALL BE GALVANIZED IN ACCORDANCE WITH THE STANDARD SPECIFICATIONS.

ANCHOR BOLTS SHALL MEET THE REQUIREMENTS OF ASTM A449. NUTS SHALL MEET THE REQUIREMENTS OF AASHTO M291-DH OR AASHTO M292-2H. WASHERS SHALL MEET THE REQUIREMENTS OF AASHTO M293. SHOP DRAWINGS ARE NOT REQUIRED FOR ANCHOR BOLTS, NUTS AND WASHERS. SHOP INSPECTION IS REQUIRED.

ALL SURFACES OF LEVELING PLATES SHALL BE SMOOTH AND STRAIGHT.

AT ALL FIXED POINTS OF SUPPORT AT INTEGRAL END BENT 1 AND 2, TOP NUTS FOR ANCHOR BOLTS ARE TO BE TIGHTENED FINGER TIGHT AND THEN BACKED OFF 1/2 TURN. THE THREAD OF THE NUT AND BOLT SHALL THEN BE BURRED WITH A SHARP POINTED TOOL.

PROJECT NO. B-4252
ROCKINGHAM COUNTY
 STATION: 31+61.50 -L-

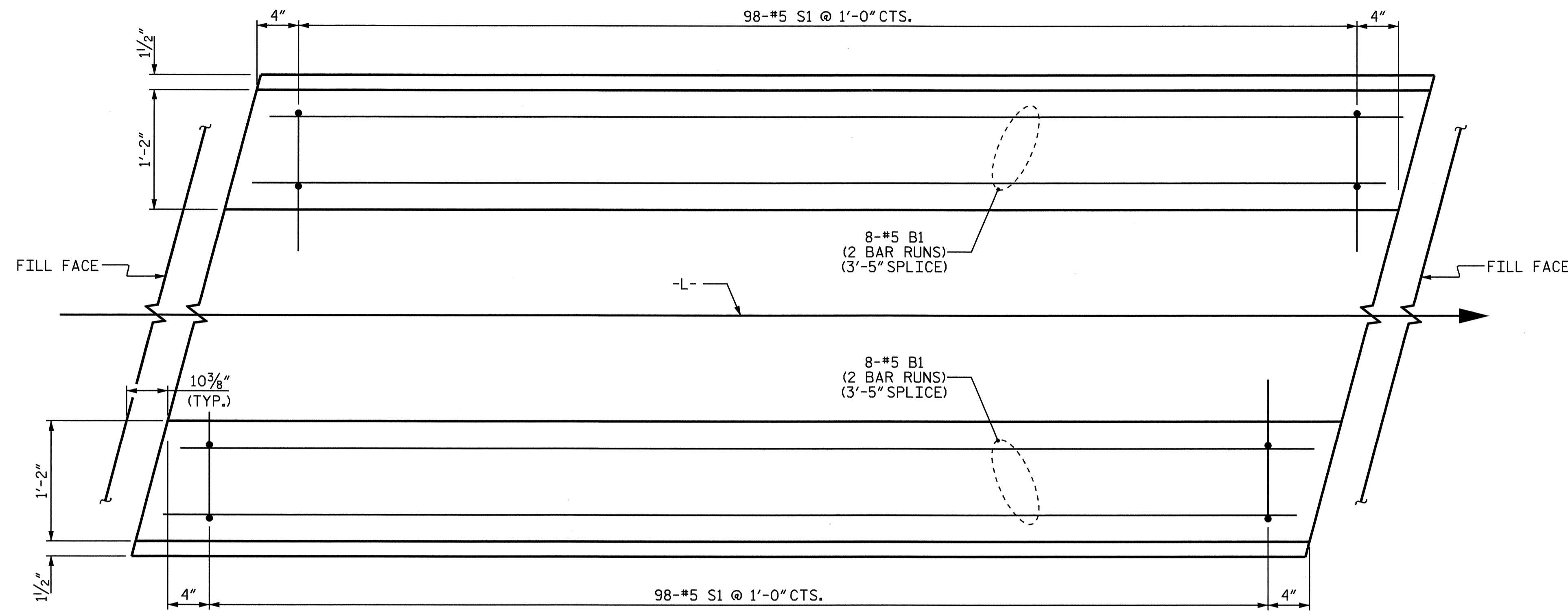
STATE OF NORTH CAROLINA
 DEPARTMENT OF TRANSPORTATION
 RALEIGH

SUPERSTRUCTURE
 LEVELING PLATE
 DETAILS



REVISIONS						SHEET NO.
NO.	BY:	DATE:	NO.	BY:	DATE:	S-51
1			3			TOTAL SHEETS
2			4			70

DRAWN BY: S. DOMBROWSKI DATE: 12/06
 CHECKED BY: M.K. BEARD DATE: 10/07



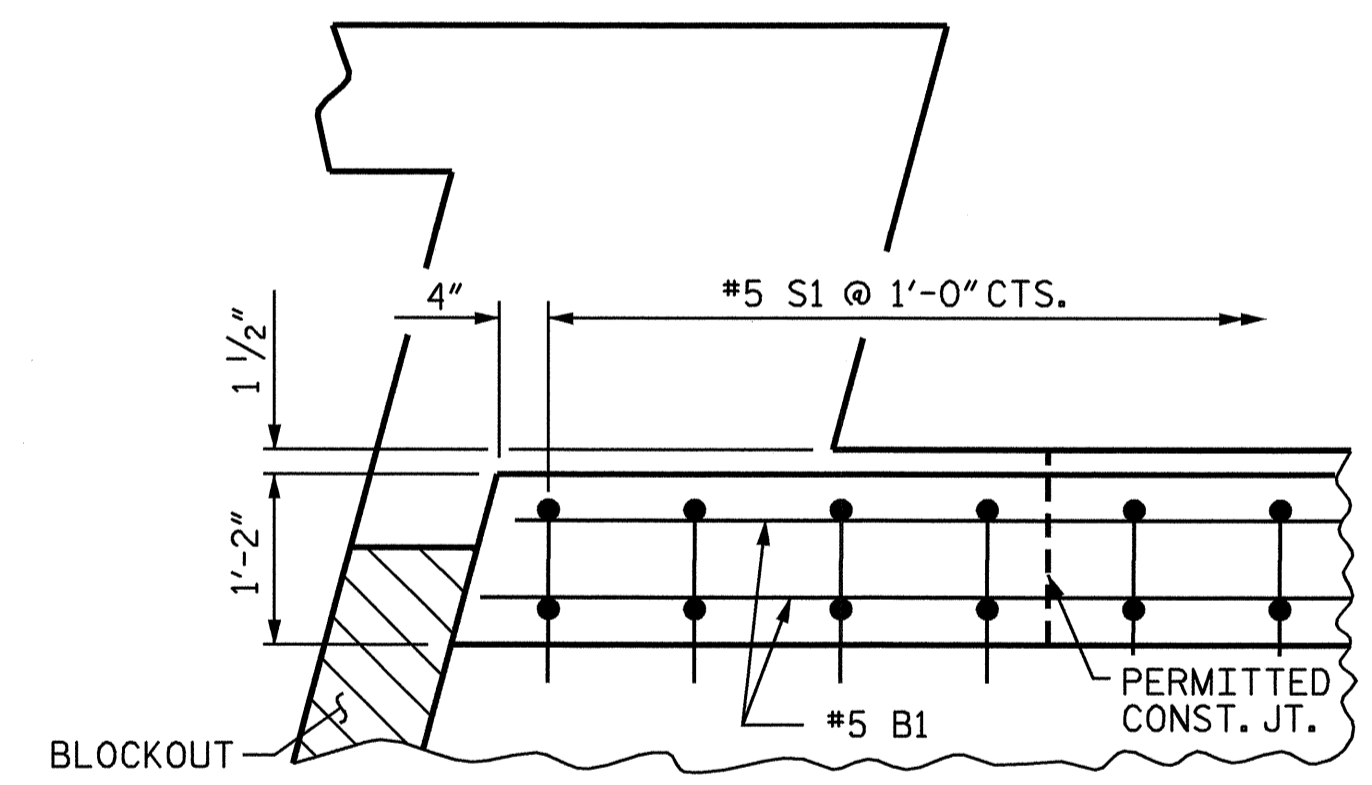
BAR TYPE		BILL OF MATERIAL					
		PARAPETS & FOUR END POSTS					
		BAR NO.	SIZE	TYPE	LENGTH	WEIGHT	
		*B1	32	#5	STR	50'-9"	1694
		*E1	8	#7	STR	2'-6"	41
		*E2	8	#7	STR	3'-0"	49
		*E3	8	#7	STR	3'-6"	57
*E4	8	#7	STR	4'-0"	65		
*E5	8	#7	STR	4'-4"	71		
*F1	8	#6	STR	2'-0"	24		
*F2	4	#6	STR	3'-6"	21		
*F3	4	#6	STR	3'-3"	20		
*F4	4	#6	STR	3'-11"	24		
*F5	4	#6	STR	3'-8"	22		
*S1	196	#5	1	7'-0"	1431		
*EPOXY COATED REINFORCING STEEL					= 3519 LBS		
CLASS "AA" CONCRETE					21.9 C.Y.		
1'-2" X 2'-6" CONCRETE PARAPET					196.55 LIN.FT.		

ALL BAR DIMENSIONS ARE OUT TO OUT

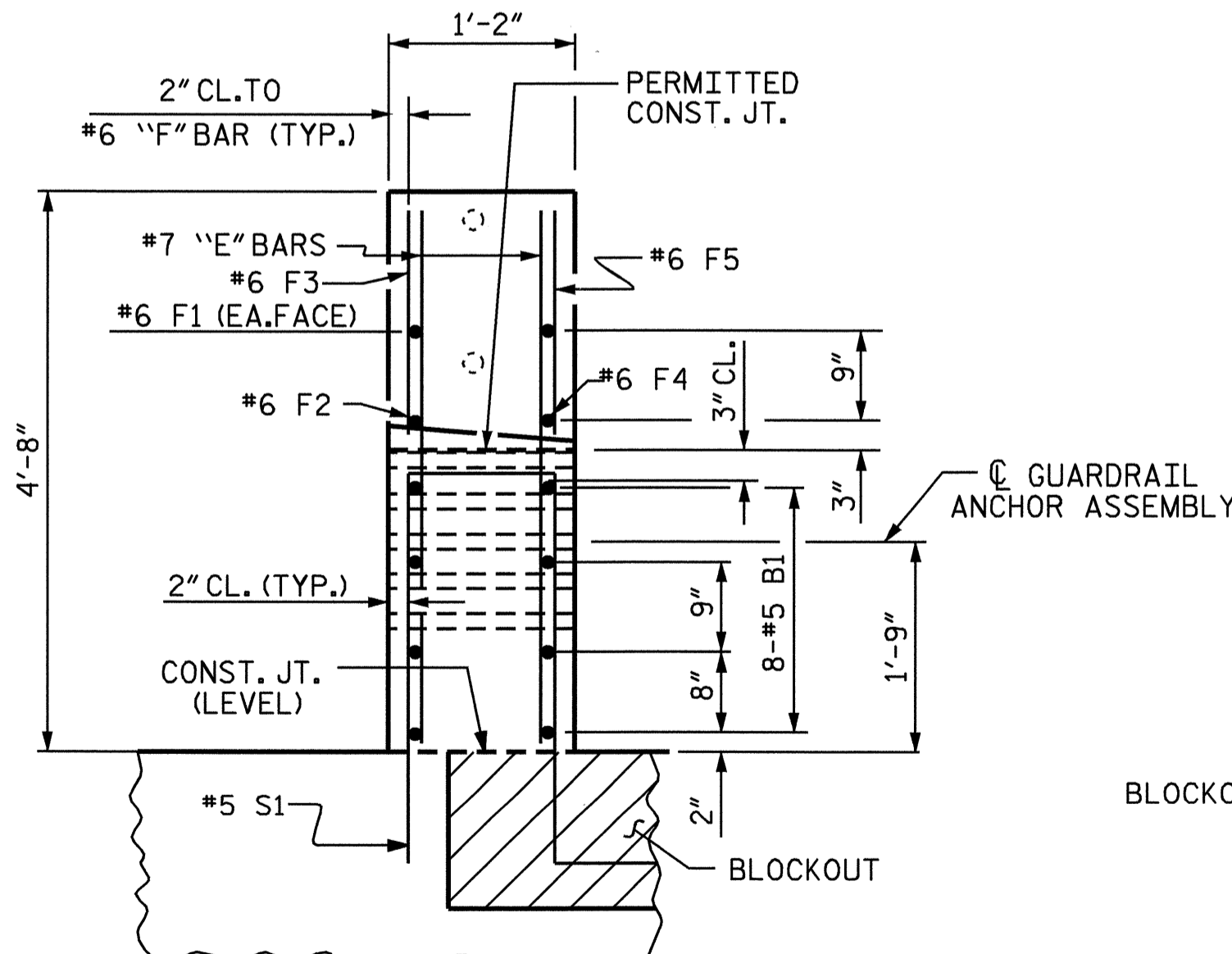
NOTES

ALL REINFORCING STEEL IN PARAPET SHALL BE EPOXY COATED.
 FOR DETAIL OF CONCRETE INSERT AND GUARDRAIL ANCHOR ASSEMBLY, SEE "RAIL POST SPACING AND END OF RAIL DETAILS" SHEET

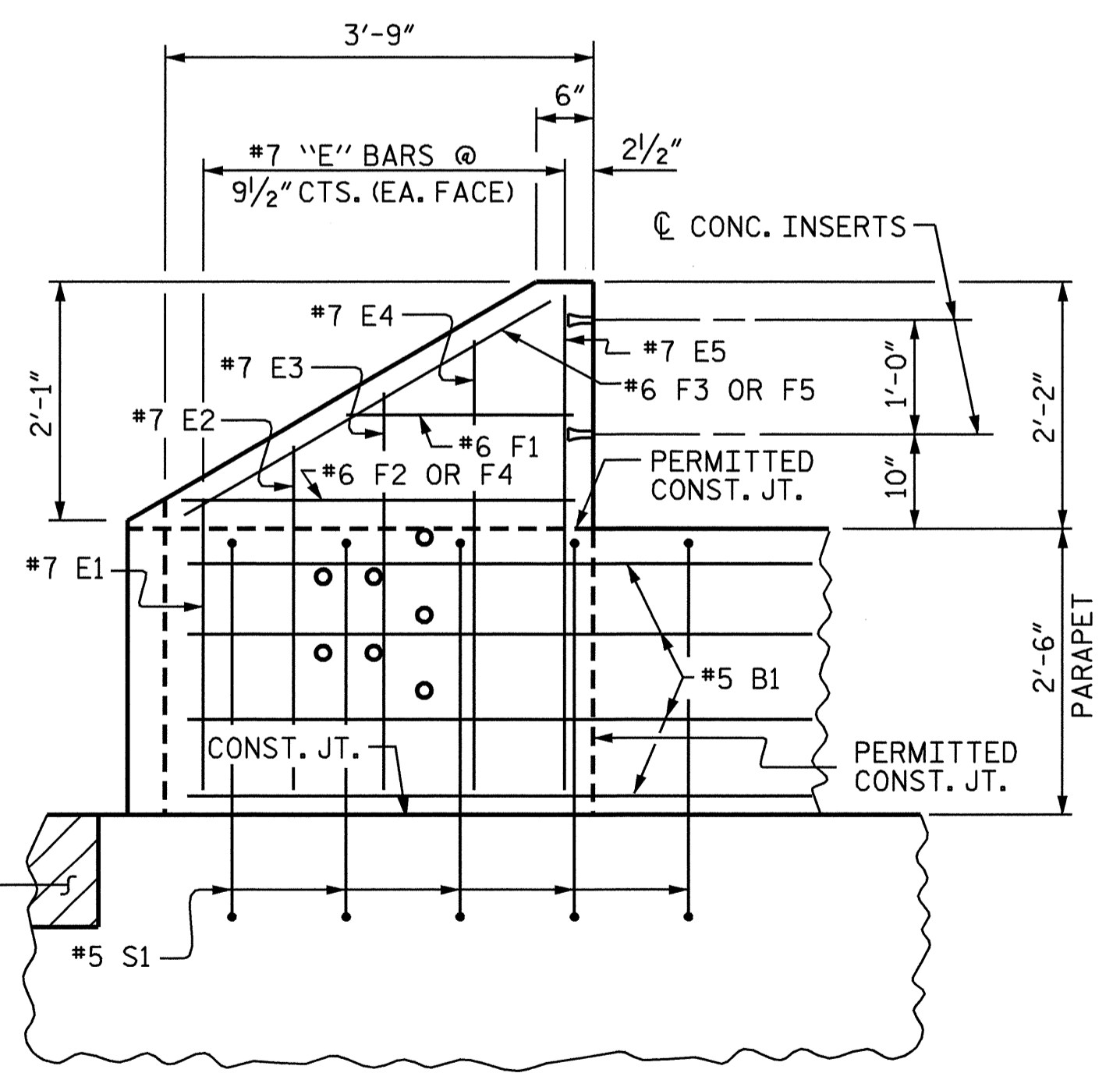
PLAN OF PARAPET



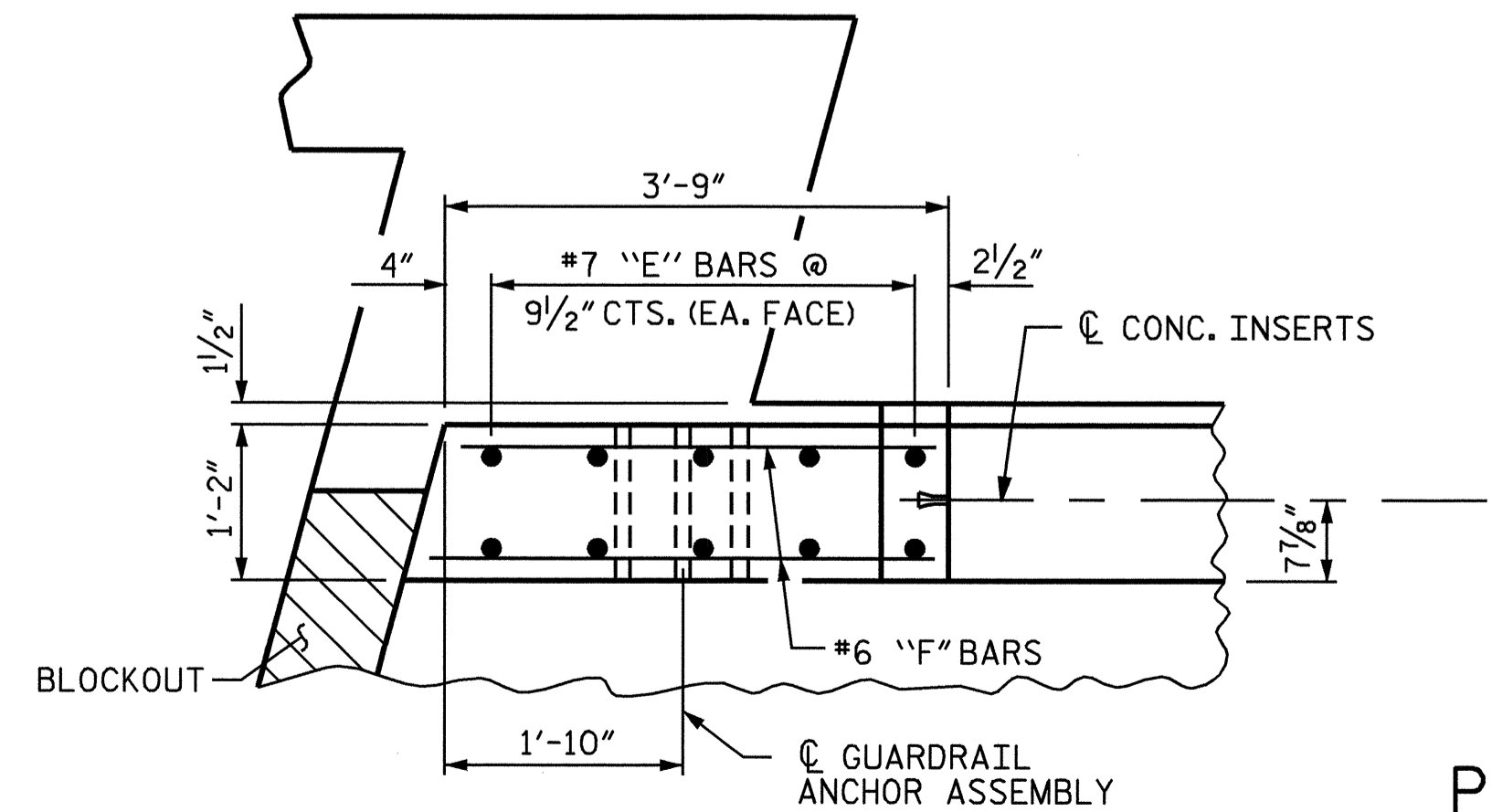
PLAN OF PARAPET



END VIEW



ELEVATION



PLAN OF END POST

PARAPET AND END POST FOR TWO BAR RAIL

PROJECT NO. B-4252
ROCKINGHAM COUNTY
 STATION: 31+61.50 -L-

SHEET 1 OF 5

STATE OF NORTH CAROLINA
 DEPARTMENT OF TRANSPORTATION
 RALEIGH

CONCRETE PARAPET AND END POST DETAILS
 FOR TWO BAR METAL RAILS

REVISIONS				SHEET NO.	
NO.	BY:	DATE:	NO.	BY:	DATE:
1			3		
2			4		

TOTAL SHEETS: 70



DRAWN BY: S. DOMBROWSKI DATE: 3/07
 CHECKED BY: M.K. BEARD DATE: 10/07

20-MAR-2008 15:35
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STR.#2

NOTES

AT THE CONTRACTOR'S OPTION, METAL RAIL MAY BE EITHER ALUMINUM OR GALVANIZED STEEL IN ACCORDANCE WITH THE REQUIREMENTS OF THE GENERAL NOTES AND THE FOLLOWING SPECIFICATIONS FOR THE ALTERNATE MATERIALS; HOWEVER, THE CONTRACTOR WILL BE REQUIRED TO USE THE SAME RAIL MATERIAL ON ALL STRUCTURES ON THE PROJECT FOR WHICH METAL RAIL IS DESIGNATED.

ALUMINUM RAILS

MATERIAL FOR POSTS, BASES AND RAILS, EXPANSION BARS AND CLAMP BARS SHALL BE ASTM B-221 ALLOY 6061-T6. MATERIAL FOR RIVETS SHALL BE ASTM B316 ALLOY 6061-T6. RIVETS SHALL BE STANDARD BUTTON HEAD AND CONE POINT COLD DRIVEN AS PER DRAWING.

THE BASE OF RAIL POSTS, OR ANY OTHER ALUMINUM SURFACE IN CONTACT WITH CONCRETE SHALL BE THOROUGHLY COATED WITH AN ALUMINUM IMPREGNATED CAULKING COMPOUND OF APPROVED QUALITY.

MATERIAL FOR SHIMS TO BE ASTM B209 ALLOY 6061-T6.

GALVANIZED STEEL RAILS

MATERIAL AND GALVANIZING ARE TO CONFORM TO THE FOLLOWING SPECIFICATIONS:

POST, POST BASES, RAILS, EXPANSION BARS AND CLAMP BARS: AASHTO M270 GRADE 36 STRUCTURAL STEEL - GALVANIZED TO AASHTO M111.

RIVETS: RIVETS SHALL MEET THE REQUIREMENTS OF ASTM A502 FOR GRADE 1 RIVETS.

THE CUT ENDS OF GALVANIZED STEEL RAILING, AFTER GRINDING SMOOTH SHALL BE GIVEN TWO COATS OF ZINC RICH PAINT MEETING THE REQUIREMENTS OF FEDERAL SPECIFICATION MIL-P-26915 USAF TYPE 1, OR OF FEDERAL SPECIFICATIONS TT-P-641.

SHIMS: SHIMS SHALL MEET THE REQUIREMENTS OF ASTM A570 FOR GRADE 33 OR A611 FOR GRADE C AND SHALL BE GALVANIZED IN ACCORDANCE WITH AASHTO M111.

RAIL CAPS: RAIL CAPS SHALL MEET THE REQUIREMENTS OF ASTM A570 FOR GRADE 33 OR A611 FOR GRADE C AND SHALL BE GALVANIZED IN ACCORDANCE WITH AASHTO M111.

GENERAL NOTES

RAILING SHALL BE CONTINUOUS FROM END POST TO END POST OF BRIDGE. EACH JOINT IN RAIL LENGTH SHALL BE SPLICED AS DETAILED. PANEL LENGTHS OF RAIL SHALL BE ATTACHED TO A MINIMUM OF THREE POSTS.

FOR END OF RAIL TO CLEAR FACE OF CONCRETE END POST DIMENSION, SEE STANDARD NO. BMR2.

CAP SCREWS SHALL BE ASTM F593 ALLOY 305 STAINLESS STEEL. WASHERS SHALL MEET THE REQUIREMENTS OF ASTM F844 EXCEPT THEY SHALL BE MADE FROM ALLOY 304 STAINLESS STEEL.

CERTIFIED MILL REPORTS ARE REQUIRED FOR RAILS AND POSTS. SHOP INSPECTION IS NOT REQUIRED.

METAL RAIL POSTS SHALL BE SET NORMAL TO CURB GRADE.

METHOD OF MEASUREMENT FOR METAL RAILS: FOR LENGTH OF METAL RAILS TO BE PAID FOR, SEE THE STANDARD SPECIFICATIONS.

CURVED RAIL USAGE: WHERE RAILS ARE TO BE USED ON BRIDGES ON HORIZONTAL AND/OR VERTICAL CURVATURE THE CONTRACTOR MAY, AT HIS OPTION, HAVE THE REQUIRED CURVATURE IN THE RAIL FORMED IN THE SHOP OR IN THE FIELD. IN EITHER EVENT, THE RAIL SHALL CONFORM WITHOUT BUCKLING OR KINKING TO THE REQUIRED CURVATURE IN A UNIFORM MANNER ACCEPTABLE TO THE ENGINEER.

TO INSURE FUTURE IDENTIFICATION OF THE FABRICATOR, A PERMANENT IDENTIFYING MARK SHALL BE PLACED ON EACH POST. THE METHOD OF MARKING AND LOCATION SHALL BE SUCH THAT IT DOES NOT DETRACT FROM THE APPEARANCE OF THE POST, BUT REMAINS VISIBLE AFTER RAIL PLACEMENT.

SHIMS SHALL BE USED AS NECESSARY FOR POST ALIGNMENT.

ALLOY 6351-T5 MAY BE SUBSTITUTED FOR ALLOY 6061-T6 WHERE APPLICABLE.

MINOR VARIATIONS IN DETAILS OF METAL RAIL WILL BE CONSIDERED. DETAILS OF SUCH VARIATIONS, IF DESIRED, SHALL BE SUBMITTED FOR APPROVAL.

GROOVED CONTRACTION JOINTS, 1/2" IN DEPTH, SHALL BE TOOLED IN ALL EXPOSED FACES OF THE PARAPET IN ACCORDANCE WITH ARTICLE 825-10(B) OF THE STANDARD SPECIFICATIONS. THE CONTRACTION JOINT SHALL BE LOCATED AT A SPACING OF 8FT. TO 10FT. BETWEEN EXPANSION JOINTS. NO CONTRACTION JOINTS WILL BE REQUIRED FOR SEGMENTS LESS THAN 10 FEET IN LENGTH.

PAY LENGTH = 180.92 LIN. FT.

PROJECT NO. B-4252
ROCKINGHAM COUNTY
 STATION: 31+61.50 -L-

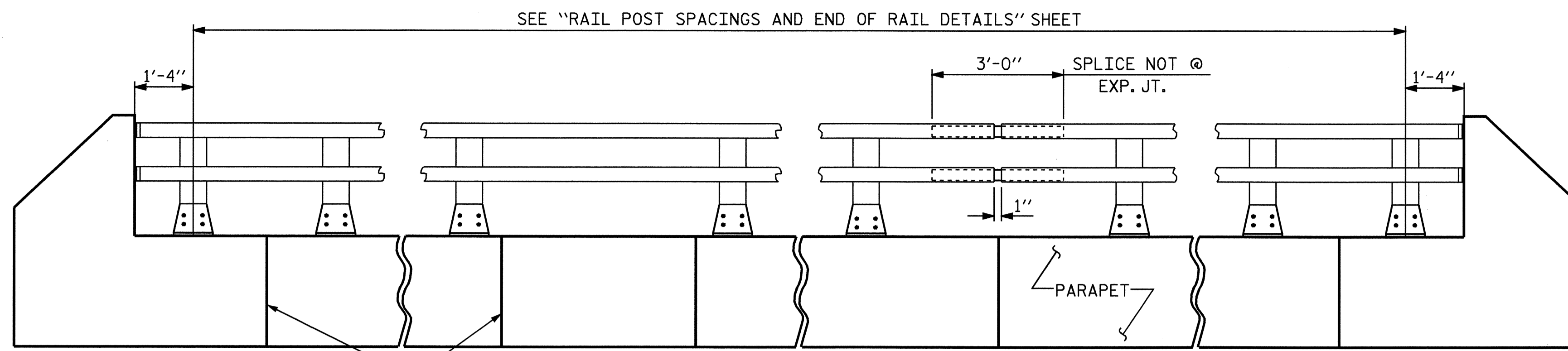
SHEET 2 OF 5

STATE OF NORTH CAROLINA
 DEPARTMENT OF TRANSPORTATION
 RALEIGH

STANDARD

2 BAR METAL RAIL

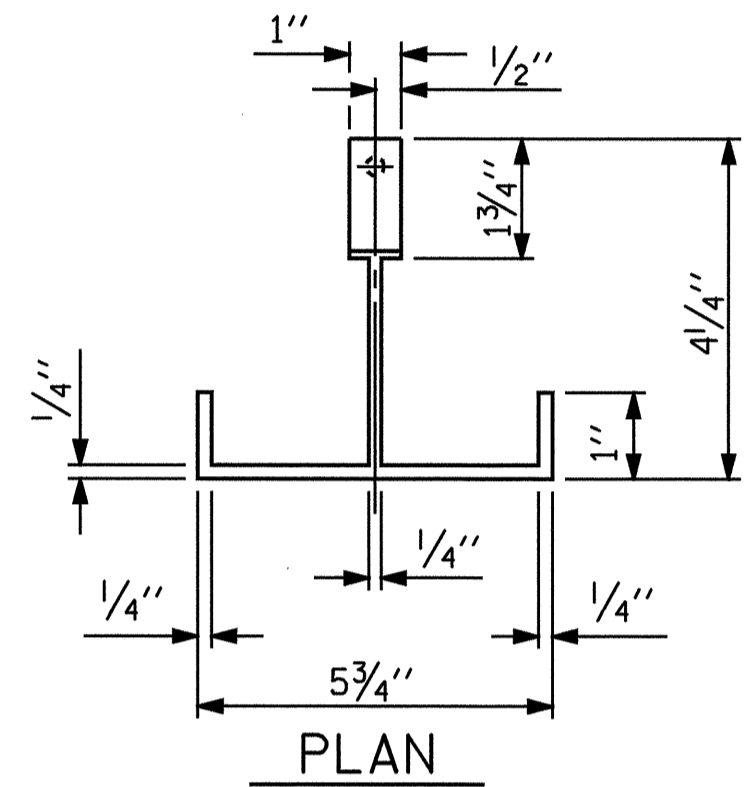
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NO.	BY:	DATE:	NO.	BY:	DATE:	S-53	
1			3			TOTAL SHEETS 70	
2			4				



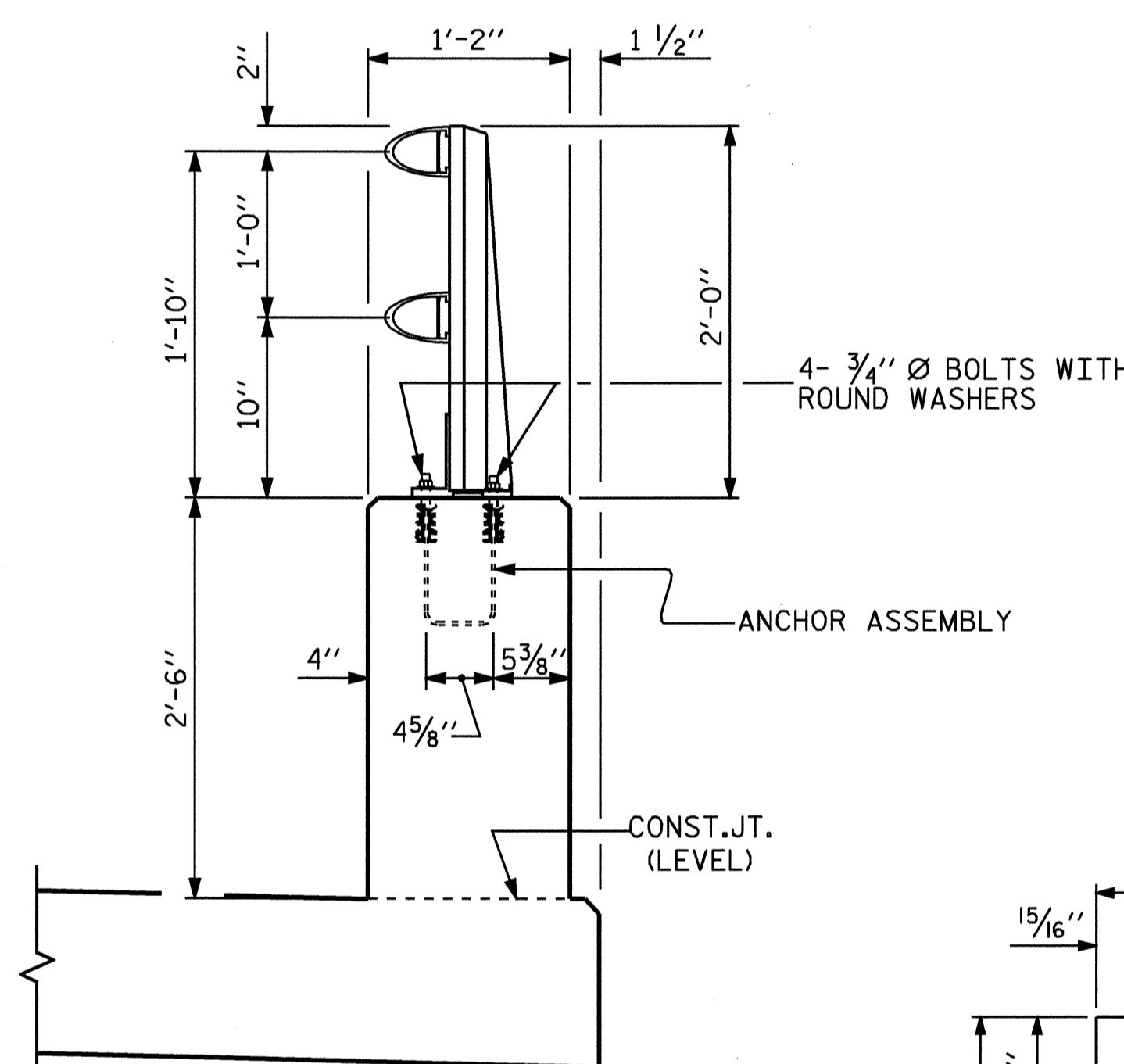
ELEVATION

TOOLED CONTRACTION JT. (SEE NOTES)

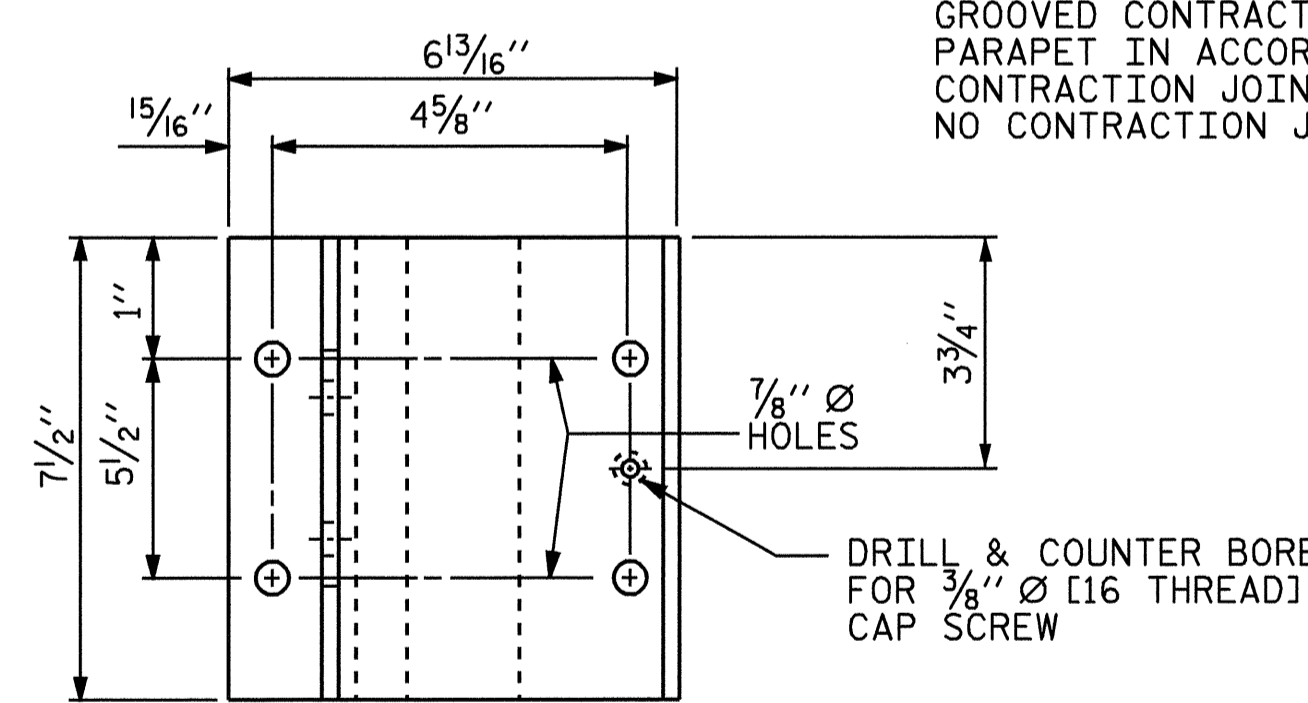
NOTE: FOR ATTACHMENT OF METAL RAIL TO END POST, SEE STANDARD NO. BMR2.



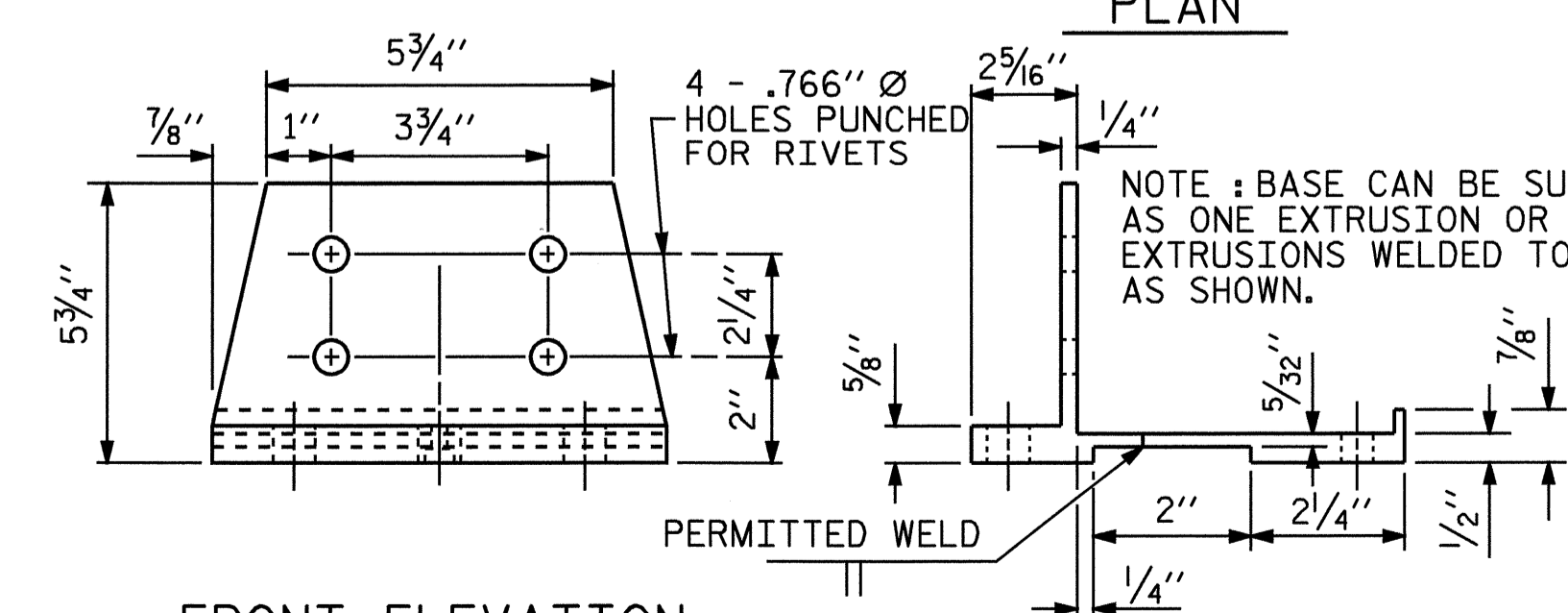
PLAN



SECTION THRU PARAPET AND RAIL



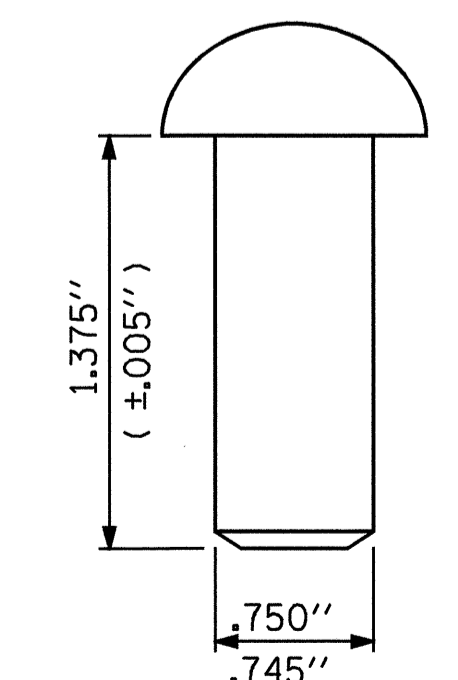
PLAN



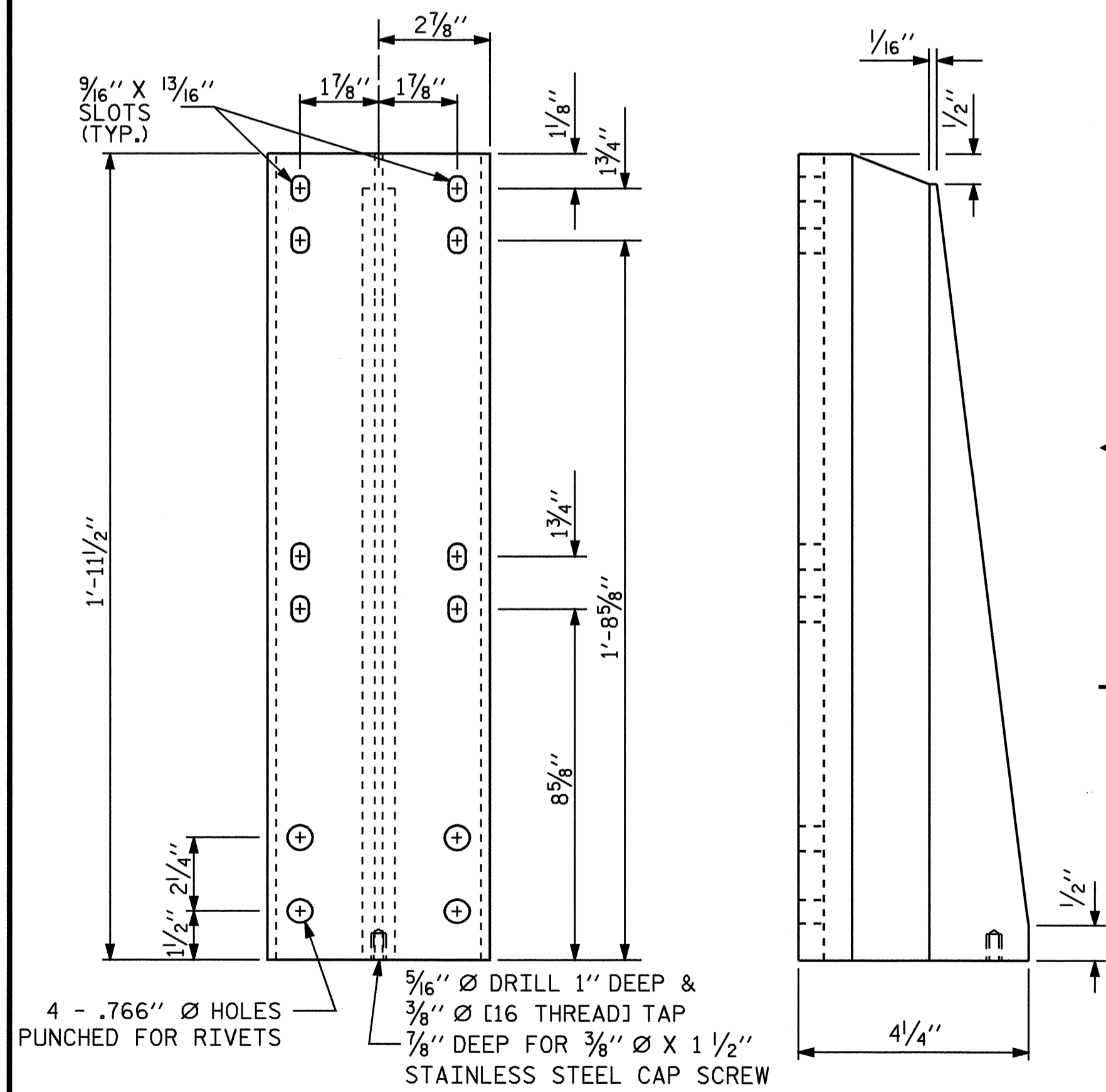
FRONT ELEVATION

SIDE ELEVATION

POST BASE DETAILS



RIVET DETAIL

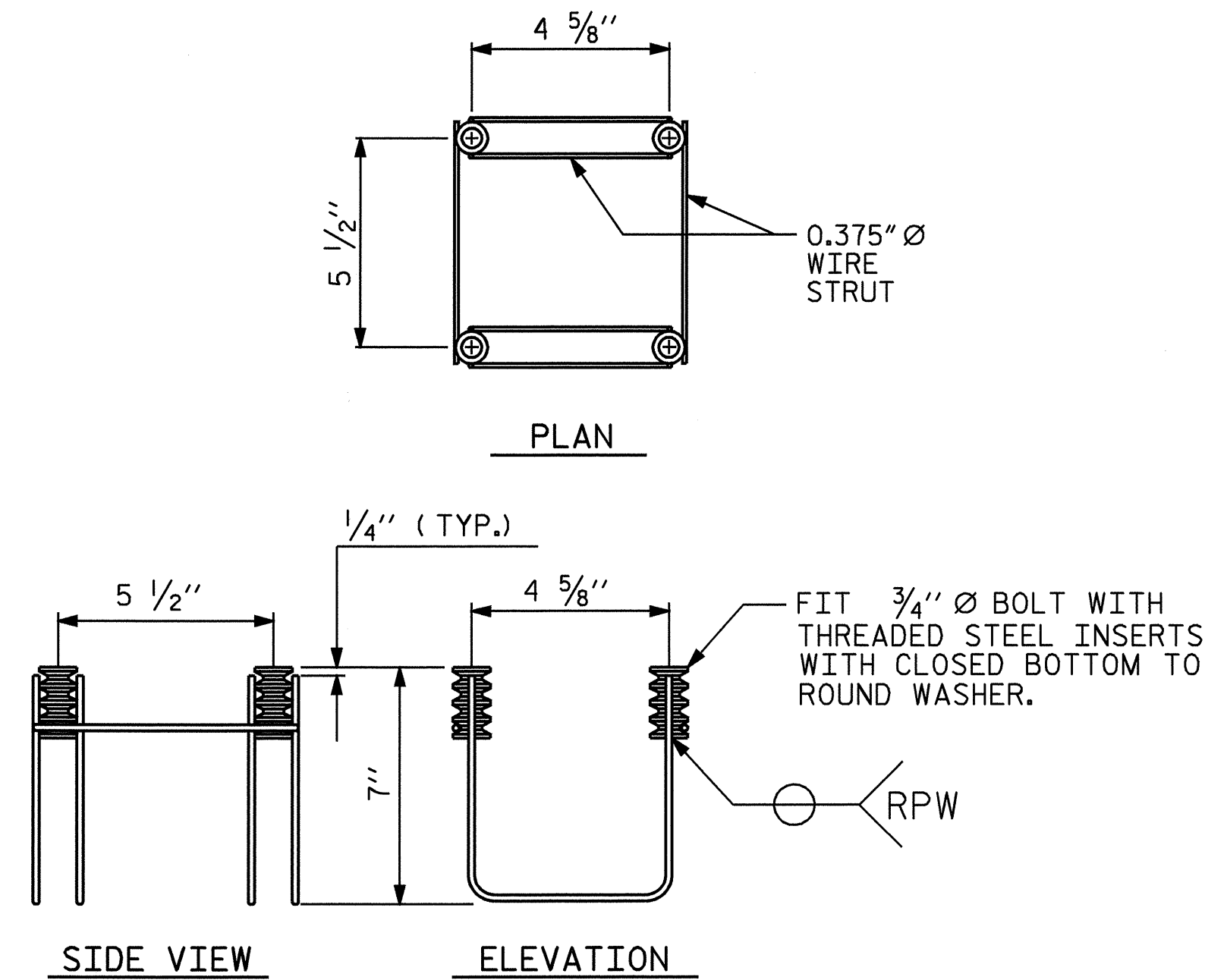
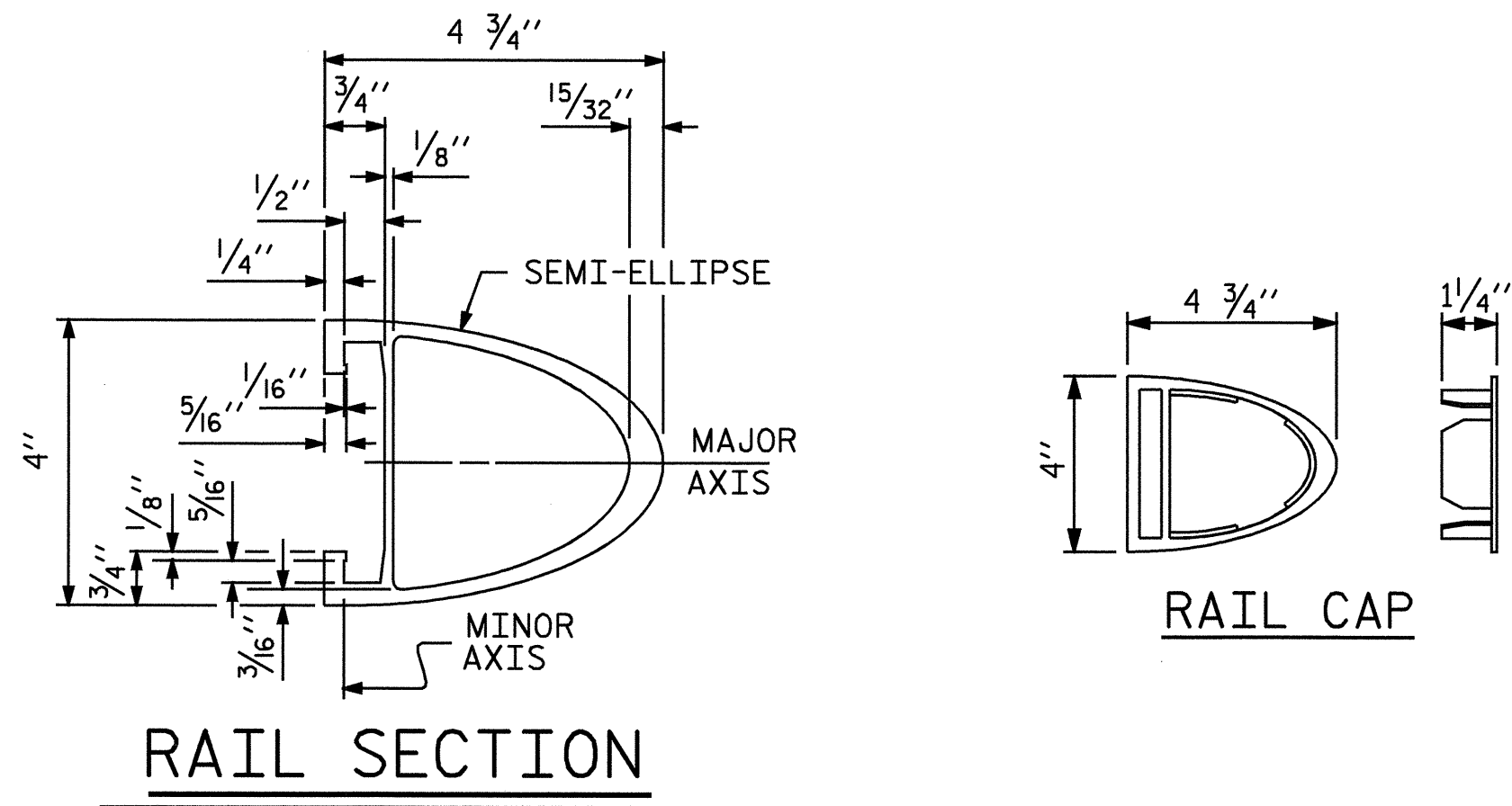


FRONT ELEVATION

SIDE ELEVATION

DETAILS OF POST

ASSEMBLED BY : S. DOMBROWSKI	DATE : 7/07
CHECKED BY : M.K. BEARD	DATE : 10/07
DRAWN BY : EEM 6/94	REV. 10/17/00 LES/RDR
CHECKED BY : RGW 6/94	REV. 5/7/03R RWW/JTE
	REV. 5/1/06 TLA/GM



MINIMUM LENGTH OF THREADS IN INSERT (FERRULE) : 1 3/4"

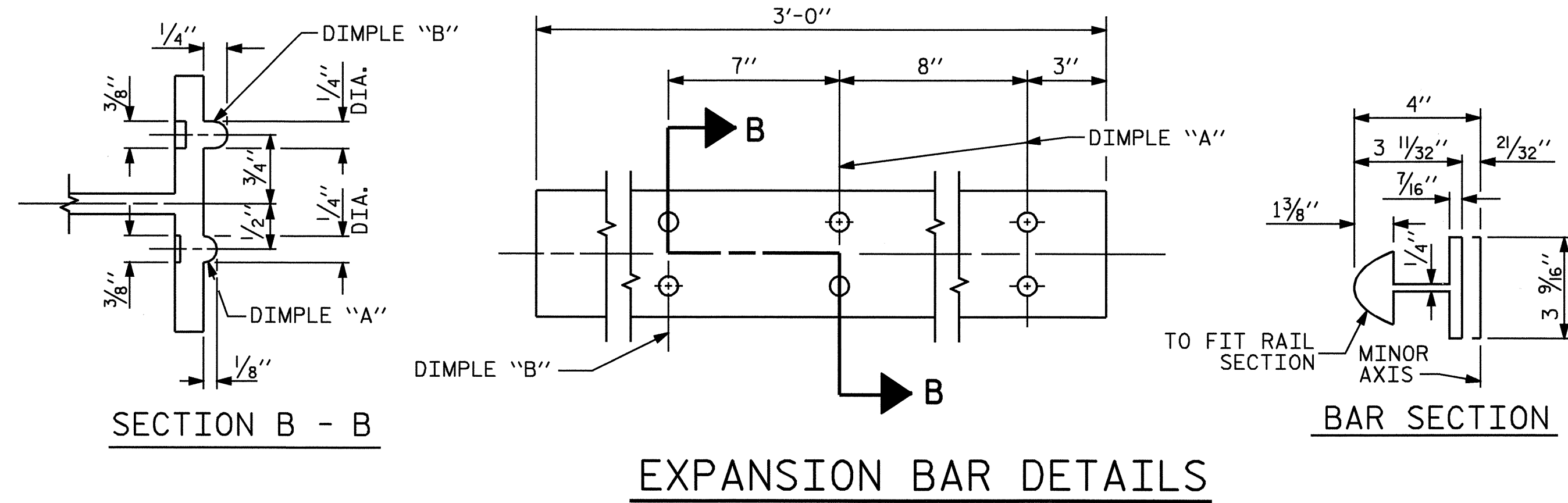
4-BOLT METAL RAIL ANCHOR ASSEMBLY

(32 ASSEMBLIES REQUIRED)

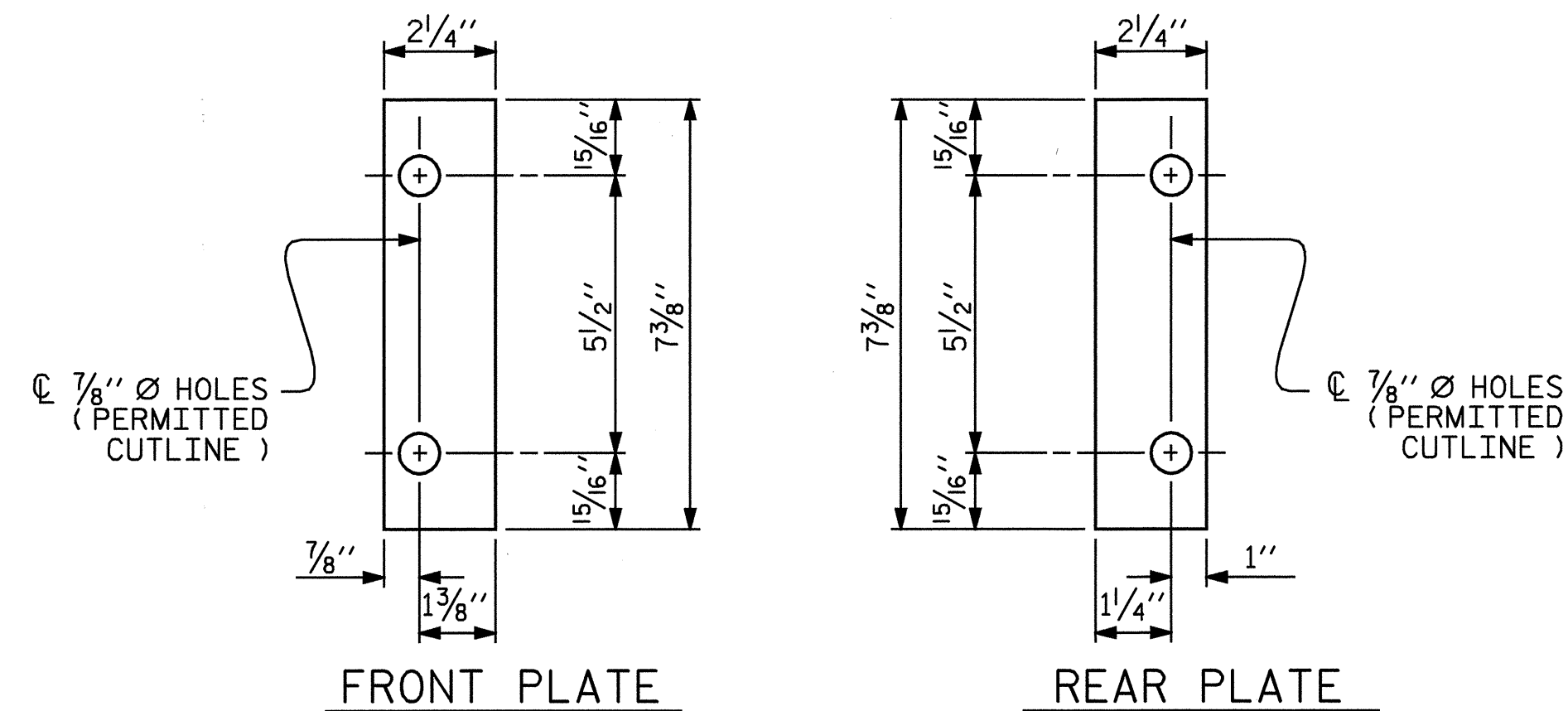
- NOTES**
STRUCTURAL CONCRETE ANCHOR ASSEMBLY
- THE STRUCTURAL CONCRETE ANCHOR ASSEMBLY SHALL CONSIST OF THE FOLLOWING COMPONENTS :
- FERRULES SHALL BE MADE FROM STEEL MEETING THE REQUIREMENTS OF AASHTO M169, GRADE 12L14 AND SHALL HAVE A MINIMUM LENGTH OF THREADS OF 2" FOR 3/4" FERRULES.
 - 4 - 3/4" Ø X 2 1/2" BOLTS WITH WASHERS. BOLTS SHALL CONFORM TO THE REQUIREMENTS OF ASTM A307. BOLTS AND WASHERS SHALL BE GALVANIZED. AT THE CONTRACTOR'S OPTION, STAINLESS STEEL BOLTS AND WASHERS MAY BE USED AS AN ALTERNATE FOR THE 3/4" Ø X 2 1/2" GALVANIZED BOLTS AND WASHERS. THEY SHALL CONFORM TO OR EXCEED THE MECHANICAL REQUIREMENTS OF ASTM A307. THE USE OF THIS ALTERNATE SHALL BE APPROVED BY THE ENGINEER.
 - WIRE STRUT SHOWN IN THE CONCRETE ANCHOR ASSEMBLY DETAIL IS THE MINIMUM ALLOWABLE SIZE AND SHALL HAVE A MINIMUM TENSILE STRENGTH OF 100,000 PSI. AS AN OPTION, A 7/16" Ø WIRE STRUT WITH A MINIMUM TENSILE STRENGTH OF 90,000 PSI IS ACCEPTABLE.
 - THE METAL RAIL ANCHOR ASSEMBLIES TO BE HOT DIPPED GALVANIZED TO CONFORM TO REQUIREMENTS OF AASHTO M111.
 - THE COST OF THE METAL RAIL ANCHOR ASSEMBLY WITH BOLTS AND WASHERS COMPLETE IN PLACE SHALL BE INCLUDED IN THE PRICE BID FOR LINEAR FEET OF METAL RAIL.
 - BOLTS TO BE TIGHTENED ONE-HALF TURN WITH A WRENCH FROM A FINGER-TIGHT POSITION.

THE CONTRACTOR MAY USE ADHESIVELY ANCHORED ANCHOR BOLTS IN PLACE OF THE METAL RAIL ANCHOR ASSEMBLY. LEVEL ONE FIELD TESTING IS REQUIRED, AND THE YIELD LOAD OF THE 3/4" Ø BOLT IS 10 KIPS. FOR ADHESIVELY ANCHORED ANCHOR BOLTS OR DOWELS, SEE SPECIAL PROVISIONS.

WHEN ADHESIVELY ANCHORED ANCHOR BOLTS ARE USED, BOLTS SHALL MEET THE REQUIREMENTS OF ASTM F593 ALLOY 304 STAINLESS STEEL WITH MINIMUM 75,000 PSI ULTIMATE STRENGTH. NUTS SHALL MEET THE REQUIREMENTS OF ASTM F594 ALLOY 304 STAINLESS STEEL AND WASHERS SHALL MEET THE REQUIREMENTS OF ASTM F844 EXCEPT THEY SHALL BE MADE FROM ALLOY 304 STAINLESS STEEL.

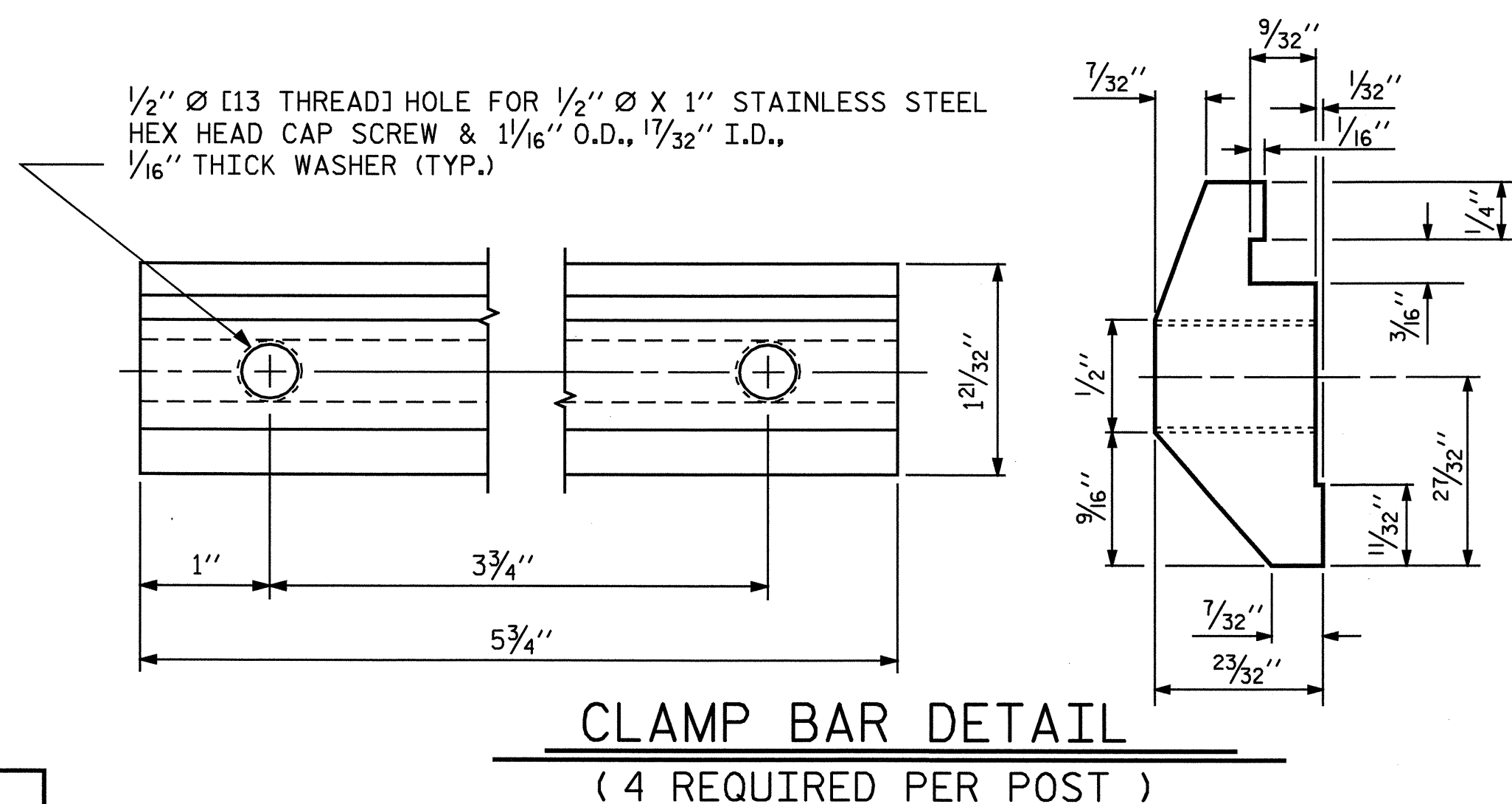


EXPANSION BAR DETAILS



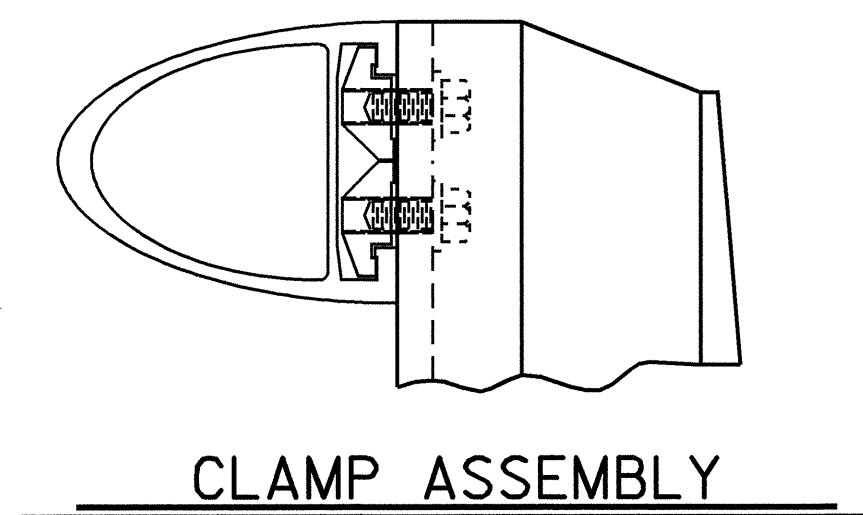
SHIM DETAILS

NOTE : SHIMS MAY BE CUT ALONG PERMITTED CUTLINE OR SLOTTED TO EDGE OF PLATE TO FACILITATE PLACEMENT.



CLAMP BAR DETAIL

(4 REQUIRED PER POST)



CLAMP ASSEMBLY

PROJECT NO. B-4252
ROCKINGHAM COUNTY
STATION: 31+61.50 -L-

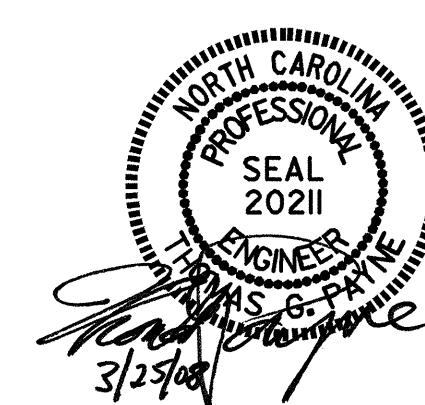
SHEET 3 OF 5

STATE OF NORTH CAROLINA
DEPARTMENT OF TRANSPORTATION
RALEIGH

STANDARD

2 BAR METAL RAIL

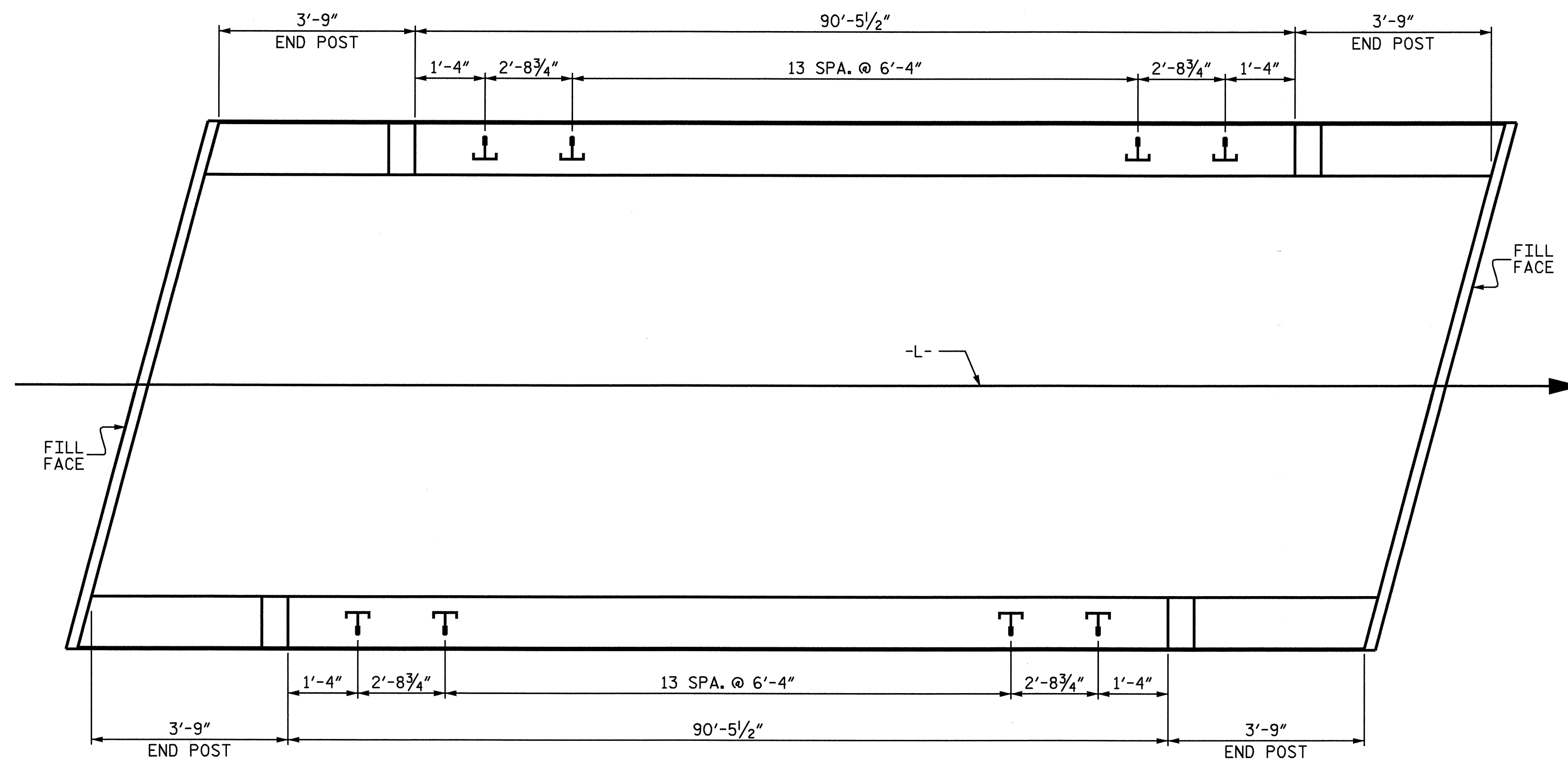
ASSEMBLED BY : S. DOMBROWSKI	DATE : 3/07
CHECKED BY : M.K. BEARD	DATE : 10/07
DRAWN BY : EEM 6/94	REV. 2/6/97 EEM/RGW
CHECKED BY : RGW 6/94	REV. 8/16/99 MAB/LES
	REV. 5/1/06R KMM/GM



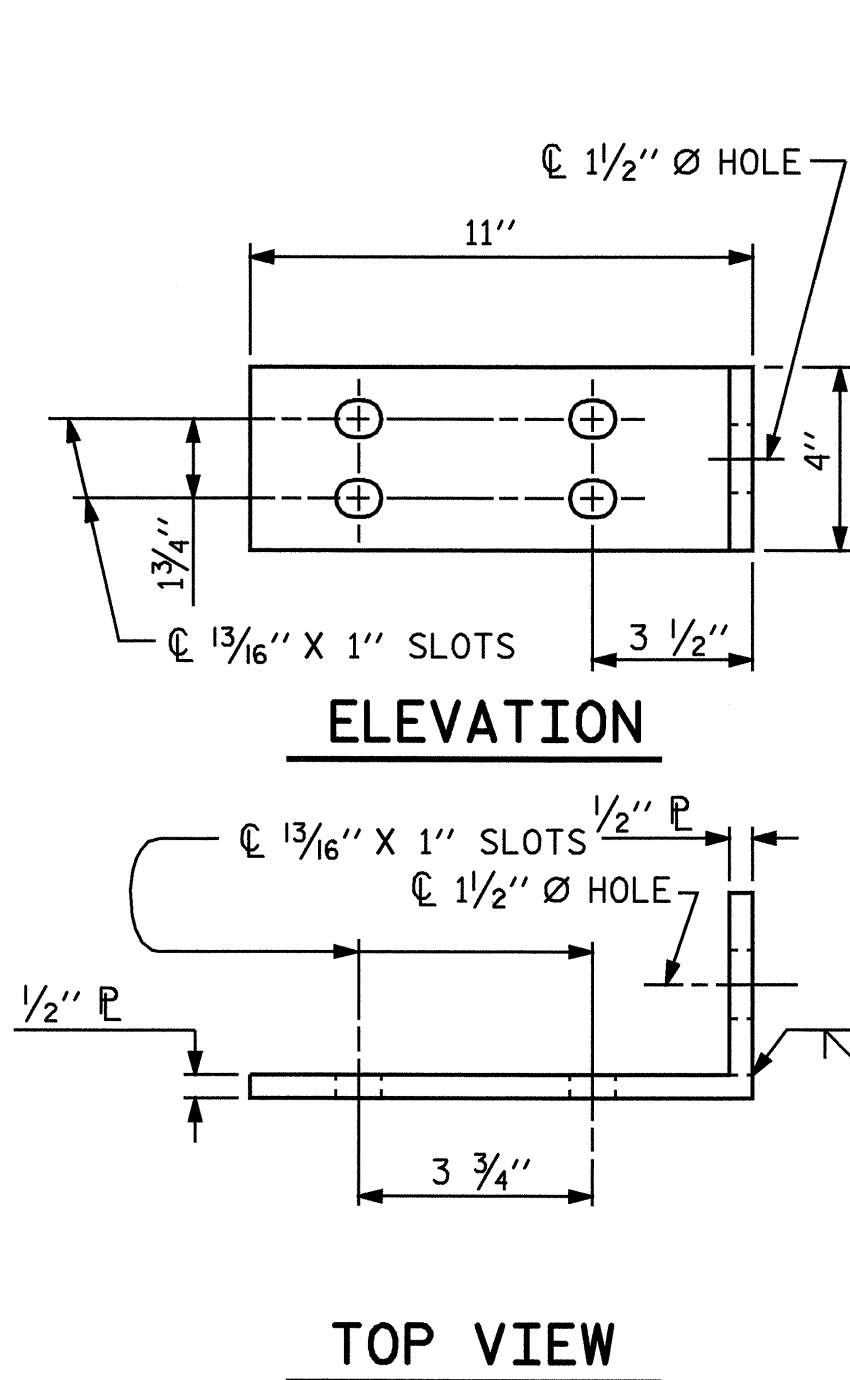
REVISIONS						SHEET NO.
NO.	BY:	DATE:	NO.	BY:	DATE:	S-54
1			3			TOTAL SHEETS
2			4			70

STR.#2

STD. NO. BMR4

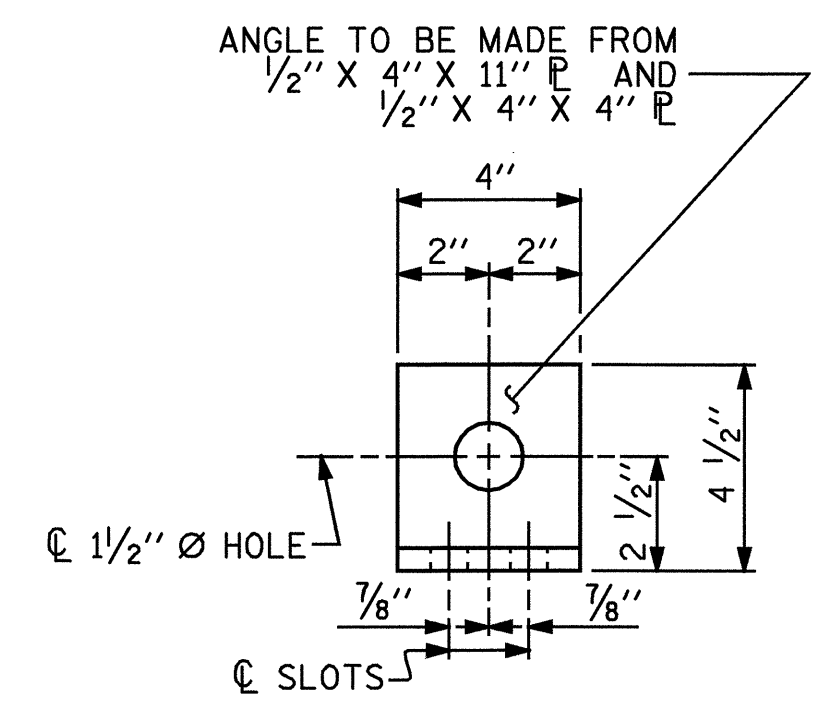


PLAN OF RAIL POST SPACINGS

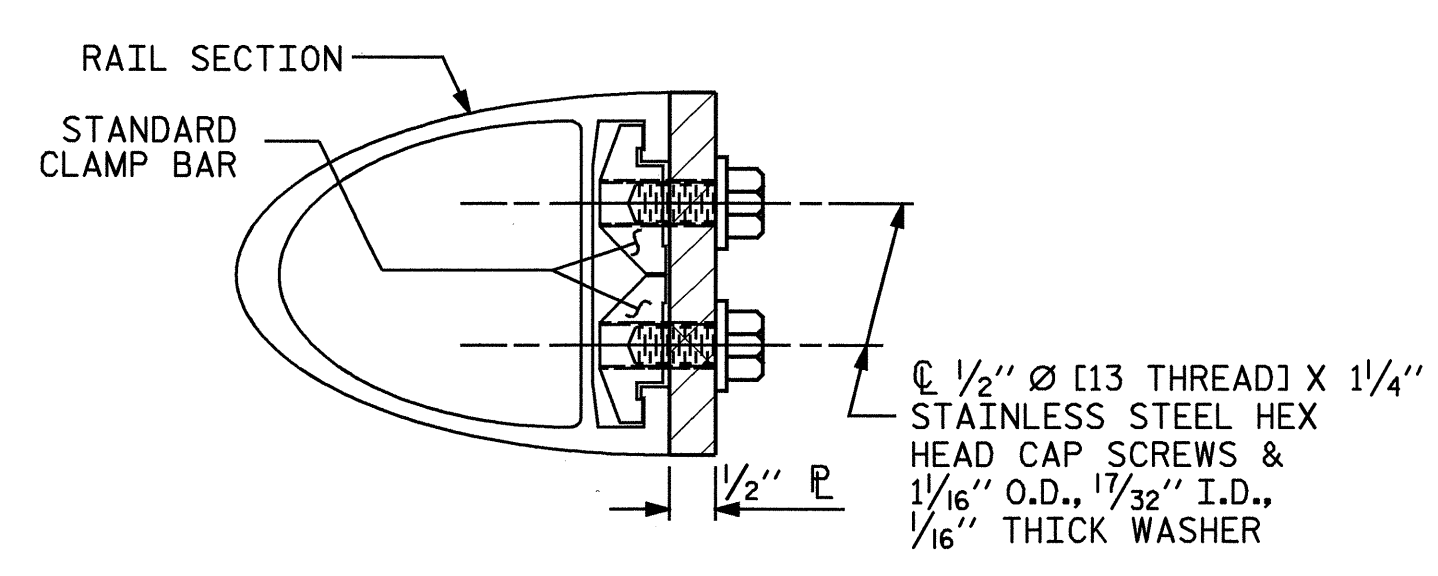


ELEVATION

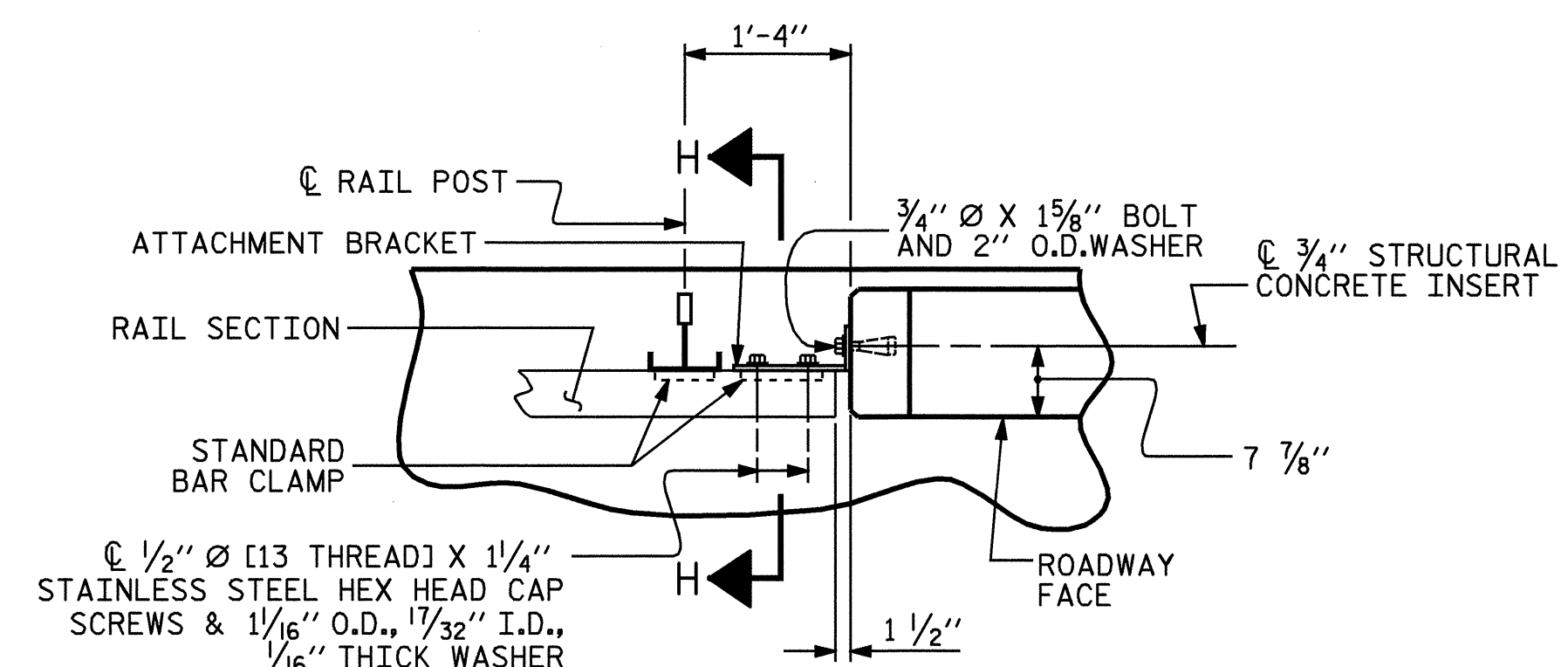
TOP VIEW



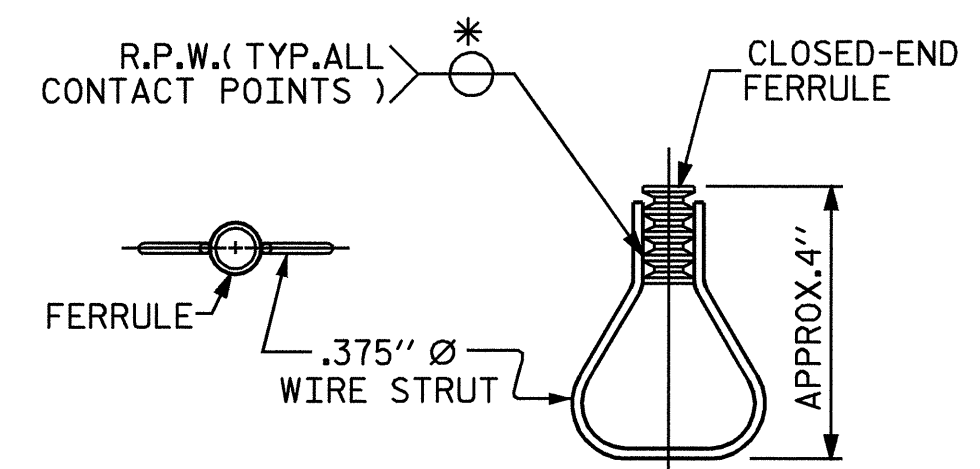
END VIEW (FIX AND EXP.)



SECTION H-H (FIX)



PLAN - RAIL AND END POST



PLAN ELEVATION STRUCTURAL CONCRETE INSERT

* EACH WELDED ATTACHMENT OF WIRE TO FERRULE SHALL DEVELOP THE TENSILE STRENGTH OF THE WIRE.

PROJECT NO. B-4252
ROCKINGHAM COUNTY
 STATION: 31+61.50 -L-

SHEET 4 OF 5

STATE OF NORTH CAROLINA
 DEPARTMENT OF TRANSPORTATION
 RALEIGH
 STANDARD
 RAIL POST SPACINGS
 AND
 END OF RAIL DETAILS
 FOR TWO BAR METAL RAILS



REVISIONS						SHEET NO.	
NO.	BY:	DATE:	NO.	BY:	DATE:	S-55	
1			3			TOTAL SHEETS 70	
2			4				

ASSEMBLED BY : S. DOMBROWSKI	DATE : 7/07
CHECKED BY : M.K. BEARD	DATE : 10/07
DRAWN BY : FCJ 1/88	REV. 10/17/00 LES/RDR
CHECKED BY : CRK 3/89	REV. 5/7/03 RWW/JTE
	REV. 5/1/06 TLA/GM

DETAILS FOR ATTACHING METAL RAIL TO END POST

NOTES

THE GUARDRAIL ANCHOR ASSEMBLY SHALL CONSIST OF A 1/4" HOLD DOWN PLATE AND 7 - 7/8" Ø BOLTS WITH NUTS AND WASHERS.

THE HOLD-DOWN PLATE SHALL CONFORM TO AASHTO M270 GRADE 36. AFTER FABRICATION, THE HOLD-DOWN PLATE SHALL BE HOT-DIP GALVANIZED IN ACCORDANCE WITH AASHTO M111.

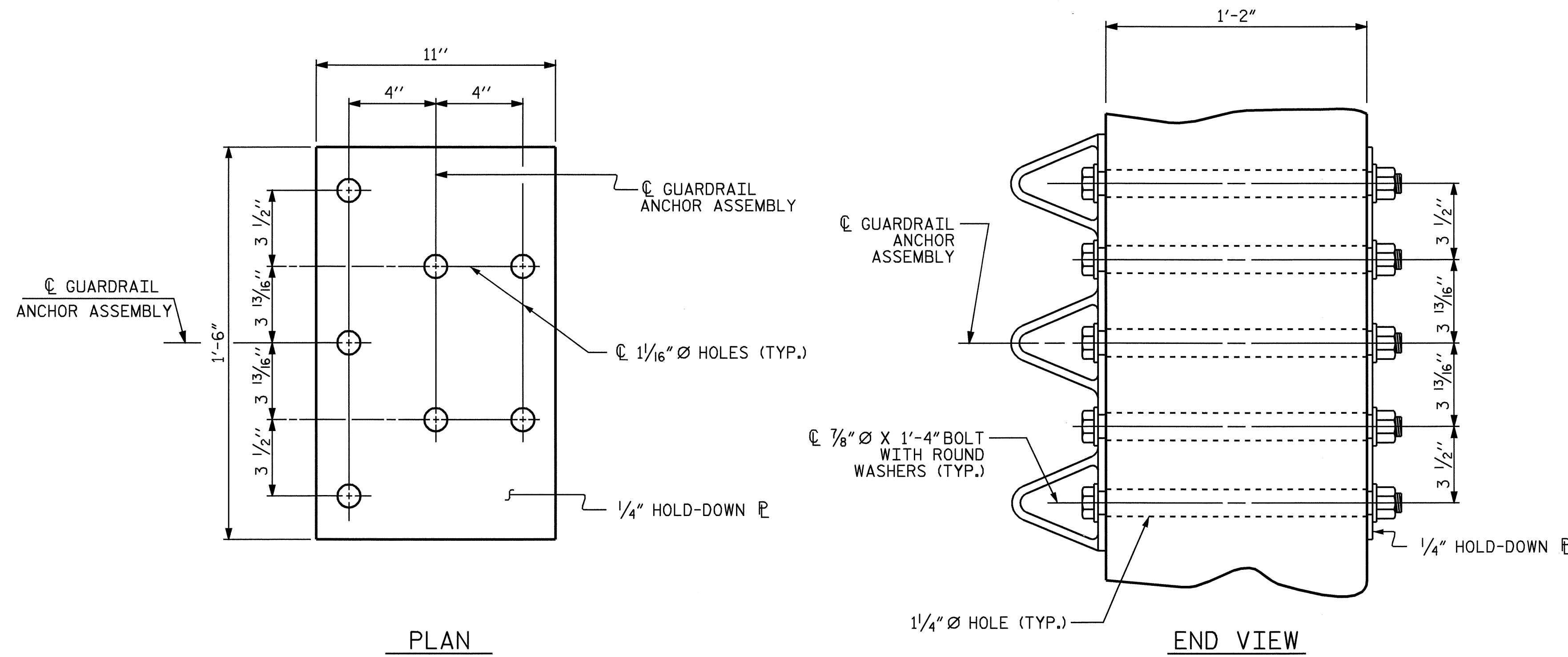
BOLTS SHALL CONFORM TO THE REQUIREMENTS OF ASTM A307 AND NUTS SHALL CONFORM TO THE REQUIREMENTS OF AASHTO M291. BOLTS, NUTS AND WASHERS SHALL BE GALVANIZED. AT THE CONTRACTOR'S OPTION, STAINLESS STEEL BOLTS, NUTS AND WASHERS MAY BE USED AS AN ALTERNATE FOR THE 7/8" Ø GALVANIZED BOLTS, NUTS AND WASHERS. THEY SHALL CONFORM TO OR EXCEED THE MECHANICAL REQUIREMENTS OF ASTM A307. THE USE OF THIS ALTERNATE SHALL BE APPROVED BY THE ENGINEER.

AFTER INSTALLATION, THE EXPOSED THREAD OF THE BOLT SHALL BE BURRED WITH A SHARP POINTED TOOL.

THE COST OF THE GUARDRAIL ANCHOR ASSEMBLIES WITH BOLTS, NUTS AND WASHERS COMPLETE IN PLACE, SHALL BE INCLUDED IN THE VARIOUS PAY ITEMS.

THE VERTICAL REINFORCING BARS MAY BE SHIFTED SLIGHTLY IN THE END POST TO CLEAR ASSEMBLY BOLTS.

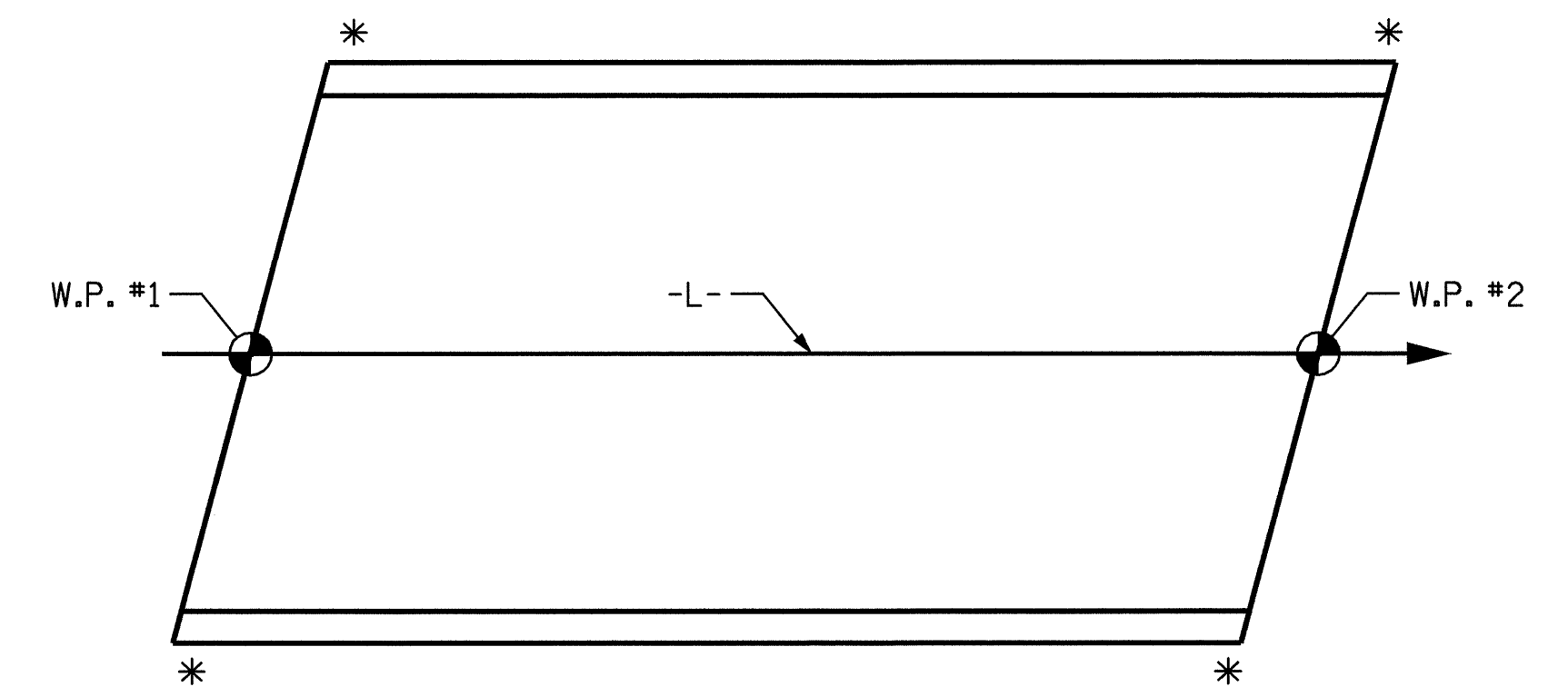
THE 1 1/4" Ø HOLES SHALL BE FORMED OR DRILLED WITH A CORE BIT. IMPACT TOOLS WILL NOT BE PERMITTED. ANY CONCRETE DAMAGED BY THIS WORK SHALL BE REPAIRED TO THE SATISFACTION OF THE ENGINEER.



PLAN

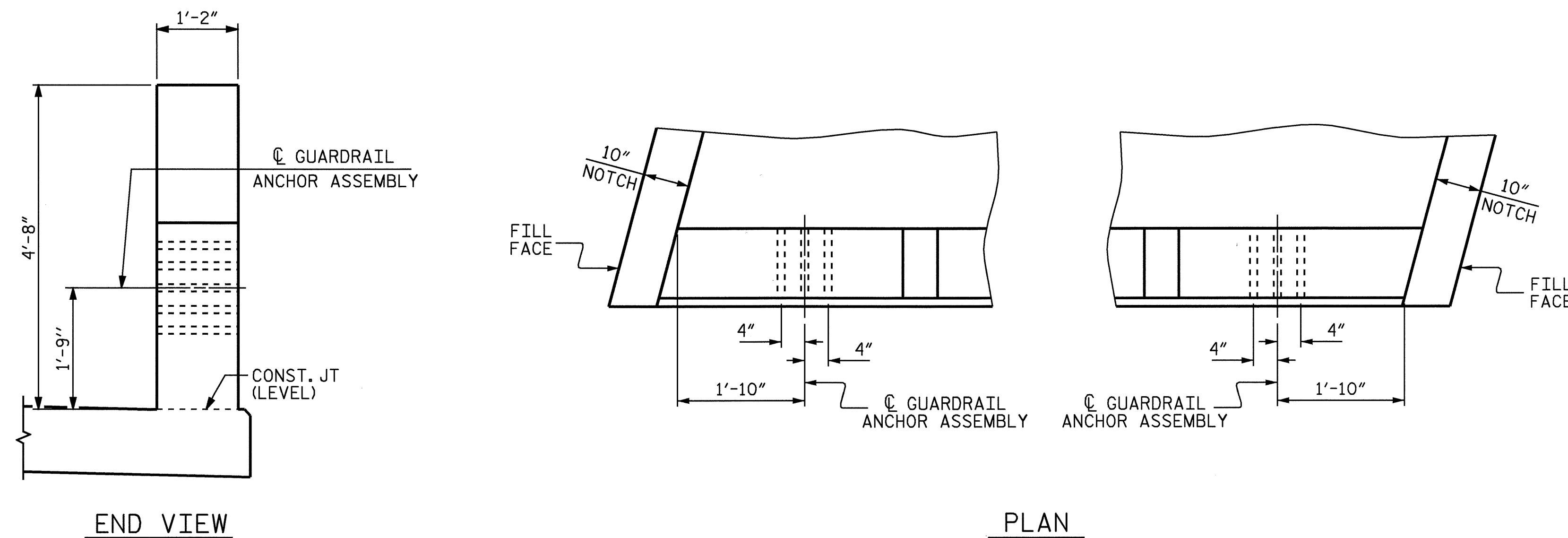
END VIEW

GUARDRAIL ANCHOR ASSEMBLY DETAILS



SKETCH SHOWING POINTS OF ATTACHMENT

* LOCATION OF GUARDRAIL ATTACHMENT



END VIEW

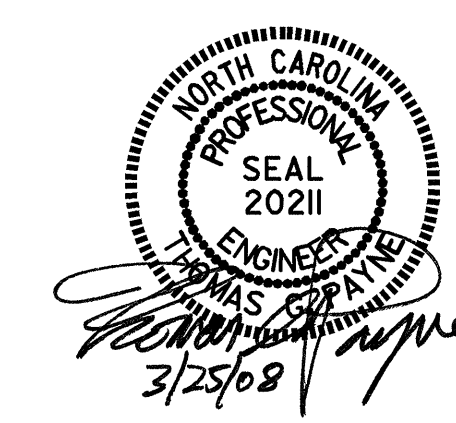
PLAN

LOCATION OF GUARDRAIL ANCHOR AT END POST

PROJECT NO. B-4252
ROCKINGHAM COUNTY
 STATION: 31+61.50 -L-

SHEET 5 OF 5

STATE OF NORTH CAROLINA DEPARTMENT OF TRANSPORTATION RALEIGH						SHEET NO. S-56
STANDARD GUARDRAIL ANCHORAGE DETAILS FOR METAL RAILS						TOTAL SHEETS 70
REVISIONS						
NO.	BY:	DATE:	NO.	BY:	DATE:	
1			3			
2			4			



ASSEMBLED BY : S. DOMBROWSKI	DATE : 7/07
CHECKED BY : M.K. BEARD	DATE : 10/07
DRAWN BY : EEM 6/94	REV. 10/17/00 RWW/LES
CHECKED BY : RGW 6/94	REV. 5/7/03 RWW/JTE
	REV. 5/1/06 TLA/GM

SUPERSTRUCTURE REINFORCING STEEL LENGTHS ARE BASED ON THE FOLLOWING MINIMUM SPLICE LENGTHS

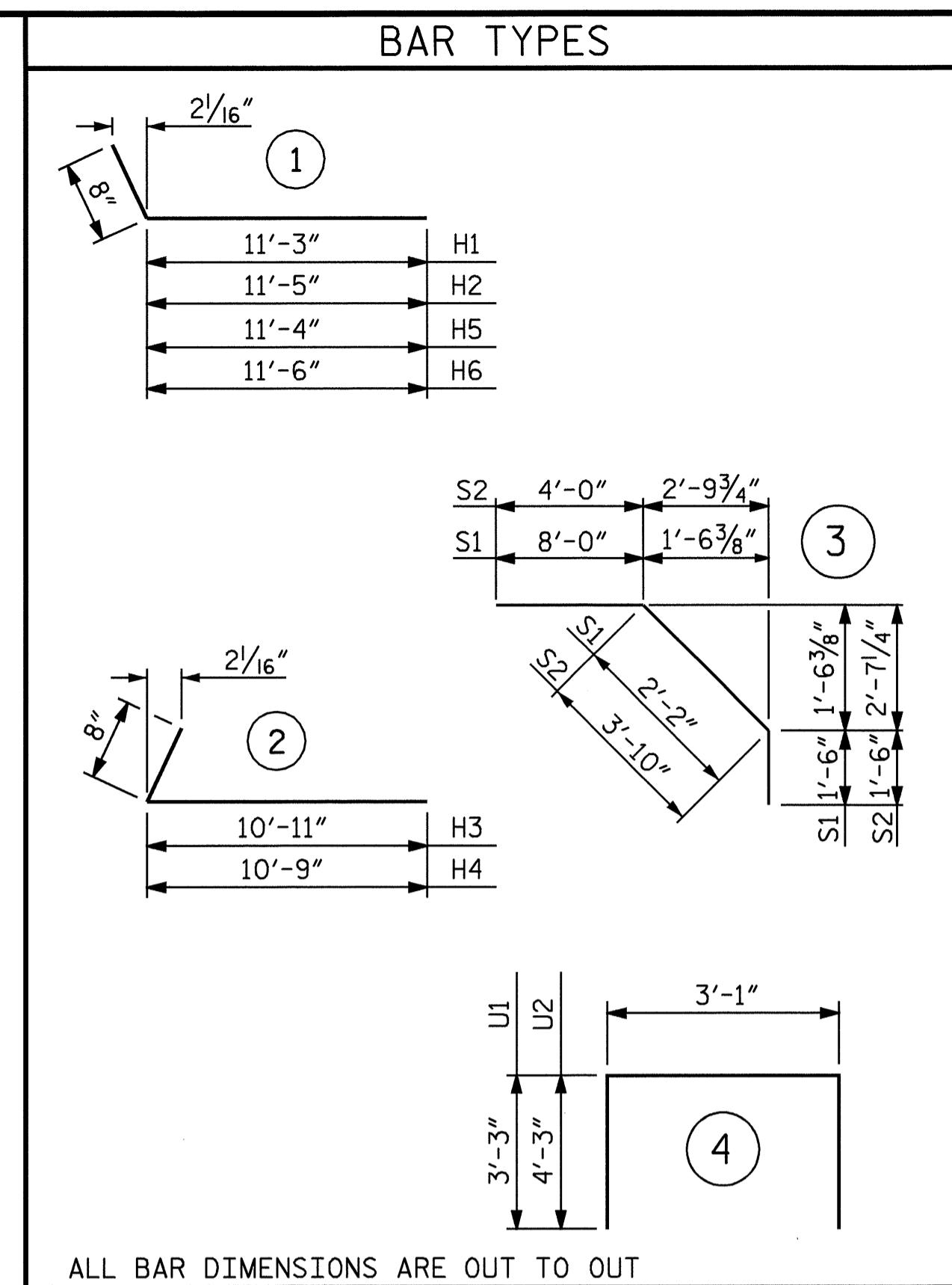
BAR SIZE	SUPERSTRUCTURE EXCEPT APPROACH SLABS, PARAPET, AND BARRIER RAIL		APPROACH SLABS		PARAPET AND BARRIER RAIL
	EPOXY COATED	UNCOATED	EPOXY COATED	UNCOATED	
#4	2'-0"	1'-9"	2'-0"	1'-9"	2'-9"
#5	2'-6"	2'-2"	2'-6"	2'-2"	3'-5"
#6	3'-0"	2'-7"	3'-10"	2'-7"	4'-4"
#7	5'-3"	3'-6"			
#8	6'-10"	4'-7"			

REINFORCING STEEL QUANTITIES			
	STAGE #1	STAGE #2	TOTAL
REINFORCING STEEL	10,845 LBS.	6,309 LBS.	17,154

EPOXY COATED REINFORCING STEEL QUANTITIES			
	STAGE #1	STAGE #2	TOTAL
EPOXY COATED REINFORCING STEEL	10,665 LBS.	6,701 LBS.	17,366
PARAPETS & END POSTS			3,519 LBS.
TOTAL			20,885 LBS.

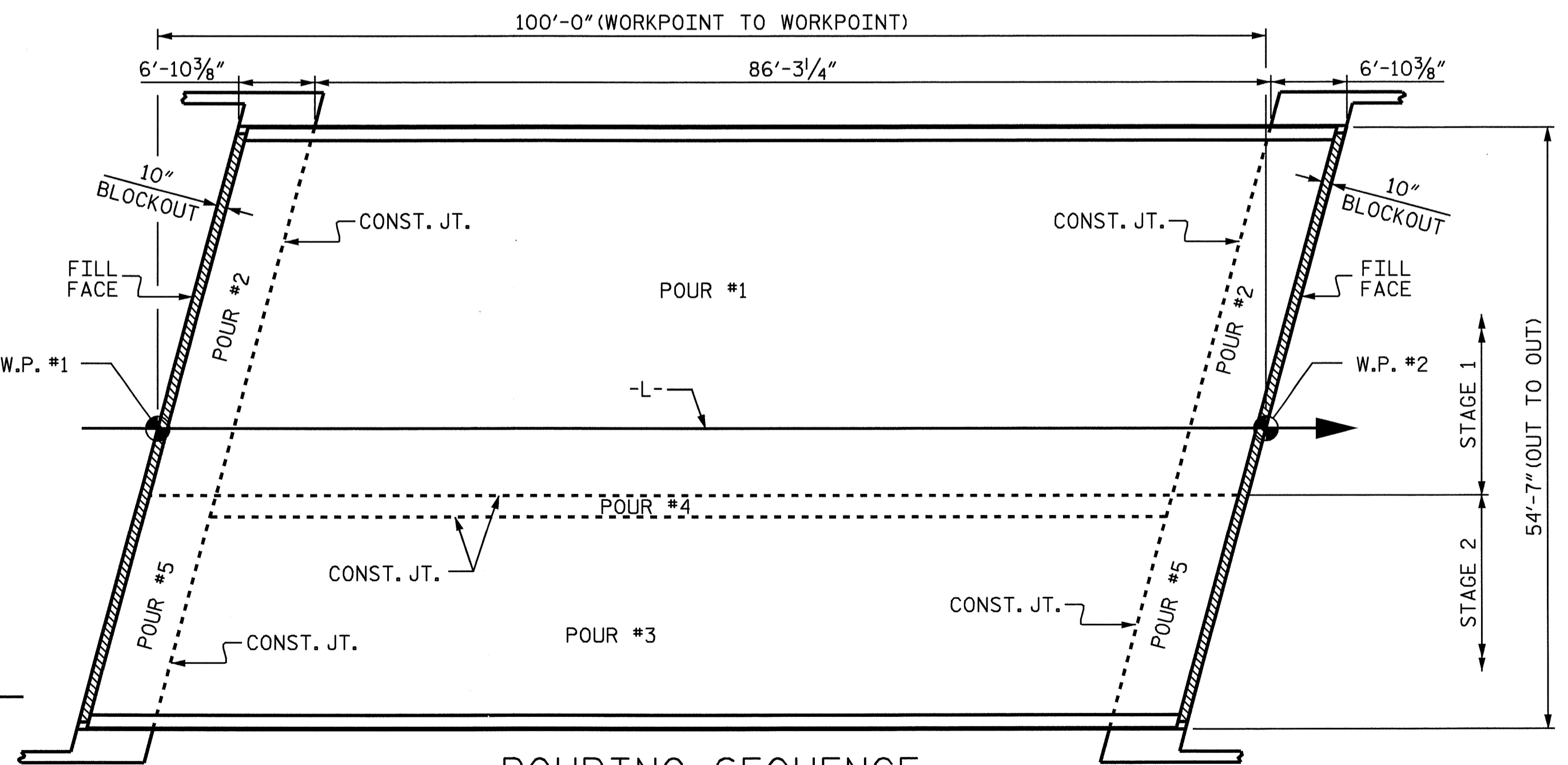
POUR SEQUENCE BREAKDOWN						
	POUR #1	POUR #2	POUR #3	POUR #4	POUR #5	TOTAL
STAGE 1	81.6	53.1				134.7
STAGE 2			47.1	5.0	31.9	84.0
TOTAL CONCRETE (CU.YD.)						218.7

NOTE: POUR #2 AND POUR #5 INCLUDE THE UPPER PORTION OF THE END BENT CAPS AND WINGS.



ALL BAR DIMENSIONS ARE OUT TO OUT

BILL OF MATERIAL STAGE 1						BILL OF MATERIAL STAGE 2					
BAR	NO.	SIZE	TYPE	LENGTH	WEIGHT	BAR	NO.	SIZE	TYPE	LENGTH	WEIGHT
* A1	143	#5	STR	32'-10"	4897	* A3	149	#5	STR	18'-10"	2927
A2	143	#5	STR	32'-10"	4897	A4	149	#5	STR	18'-10"	2927
* A101	4	#5	STR	28'-10"	120						
* A102	4	#5	STR	24'-2"	101						
* A103	4	#5	STR	19'-6"	81	* A111	4	#5	STR	14'-10"	62
* A104	4	#5	STR	14'-6"	60	* A112	4	#5	STR	10'-2"	42
* A105	4	#5	STR	9'-10"	41	* A113	4	#5	STR	5'-6"	23
* A106	4	#5	STR	5'-2"	22						
A201	4	#5	STR	28'-10"	120						
A202	4	#5	STR	24'-2"	101						
A203	4	#5	STR	19'-6"	81	A211	4	#5	STR	14'-10"	62
A204	4	#5	STR	14'-6"	60	A212	4	#5	STR	10'-2"	42
A205	4	#5	STR	9'-10"	41	A213	4	#5	STR	5'-6"	23
A206	4	#5	STR	5'-2"	22						
* B1	90	#6	STR	23'-0"	3109	* B1	58	#6	STR	23'-0"	2004
* B2	48	#4	STR	29'-0"	930	* B2	32	#4	STR	29'-0"	620
B3	74	#6	STR	23'-0"	2556	B3	42	#6	STR	23'-0"	1451
B4	37	#5	STR	56'-4"	2174	B4	21	#5	STR	56'-4"	1234
* D1	157	#5	STR	3'-4"	546	* D1	157	#5	STR	3'-4"	546
H3	5	#4	2	11'-7"	39	H1	5	#4	1	11'-11"	40
H4	5	#4	2	11'-5"	38	H2	5	#4	1	12'-1"	40
H5	5	#4	1	12'-0"	40	H3	5	#4	2	11'-7"	39
H6	5	#4	1	12'-2"	41	H4	5	#4	2	11'-5"	38
K1	16	#4	STR	21'-2"	226	K2	8	#4	STR	24'-8"	132
K3	8	#4	STR	3'-2"	17	K3	8	#4	STR	3'-2"	17
* S1	54	#4	3	11'-8"	421	* S1	34	#4	3	11'-8"	265
* S2	54	#4	3	9'-4"	337	* S2	34	#4	3	9'-4"	212
U1	54	#4	4	9'-7"	346	U1	34	#4	4	9'-7"	218
U2	6	#4	4	11'-7"	46	U2	6	#4	4	11'-7"	46
REINFORCING STEEL	= 10845 LBS					REINFORCING STEEL	= 6309 LBS				
* EPOXY COATED REINFORCING STEEL	= 10665 LBS					* EPOXY COATED REINFORCING STEEL	= 6701 LBS				



POURING SEQUENCE AND LAYOUT FOR COMPUTING AREA OF REINFORCED CONCRETE DECK SLAB (SQ. FT. = 5364)

GROOVING BRIDGE FLOORS	
BRIDGE DECK	4799 SQ.FT.
APPROACH SLAB	2235 SQ.FT.
TOTAL	7034 SQ.FT.

PROJECT NO. B-4252
ROCKINGHAM COUNTY
 STATION: 31+61.50 -L-

STATE OF NORTH CAROLINA
 DEPARTMENT OF TRANSPORTATION
 RALEIGH

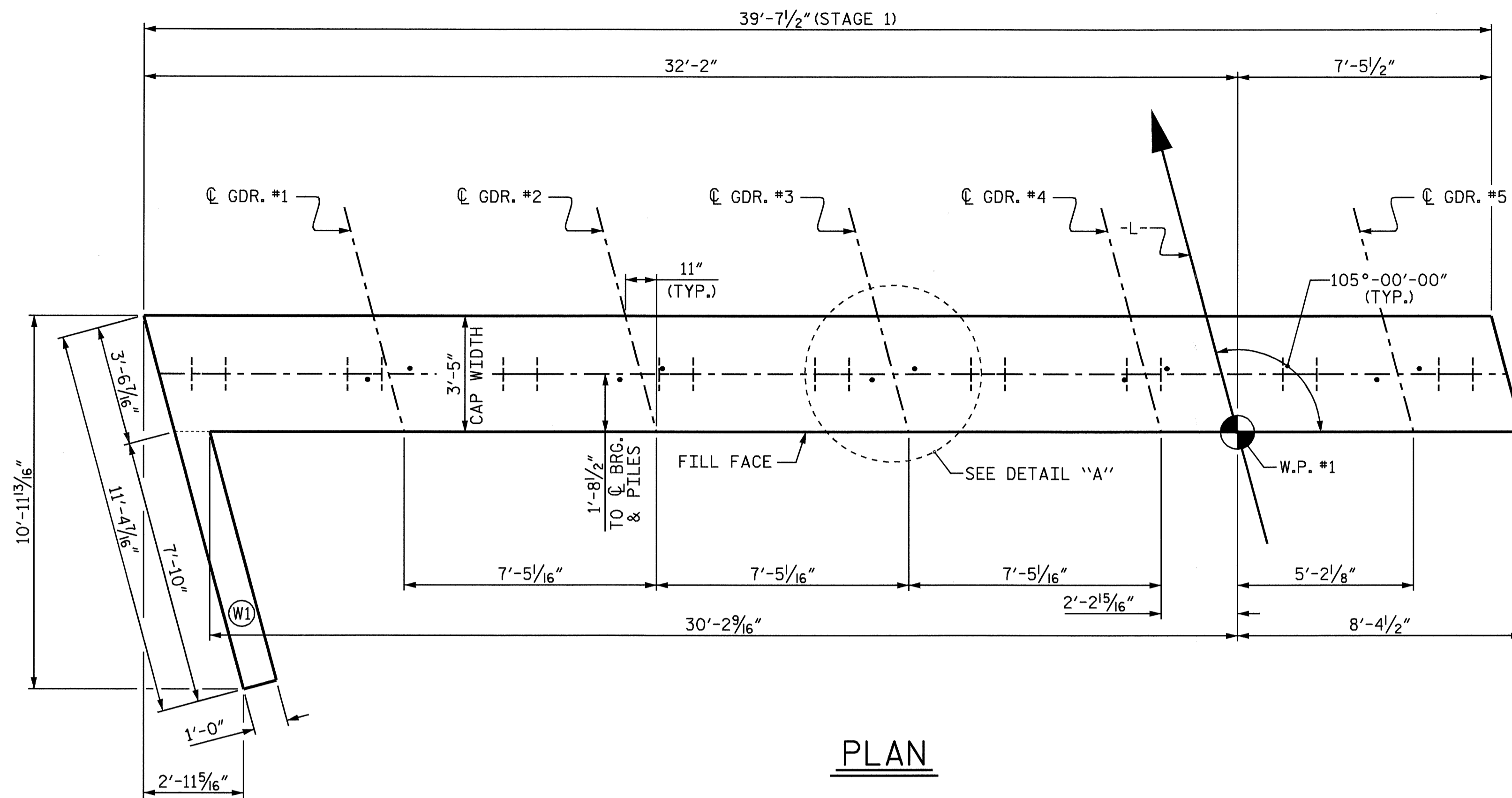
SUPERSTRUCTURE
 BILL OF MATERIALS



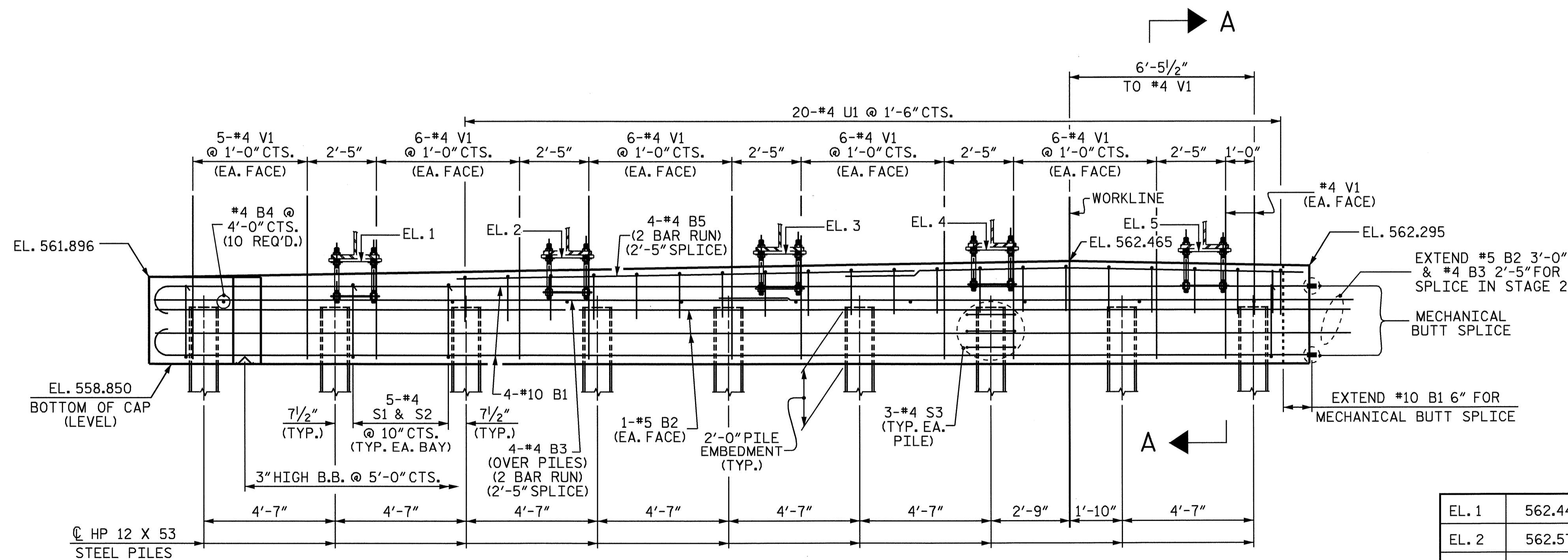
REVISIONS						SHEET NO.
NO.	BY:	DATE:	NO.	BY:	DATE:	S-57
1			3			TOTAL SHEETS 70
2			4			

DRAWN BY: S. DOMBROWSKI DATE: 3/07
 CHECKED BY: M.K. BEARD DATE: 10/07

STR.#2



PLAN



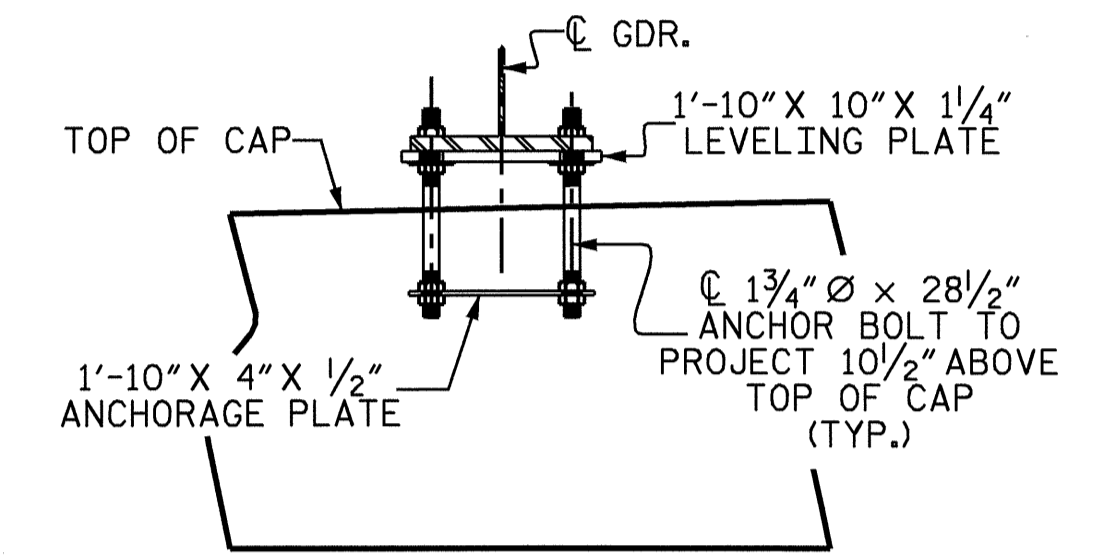
ELEVATION

EL. 1	562.440
EL. 2	562.578
EL. 3	562.715
EL. 4	562.853
EL. 5	562.790

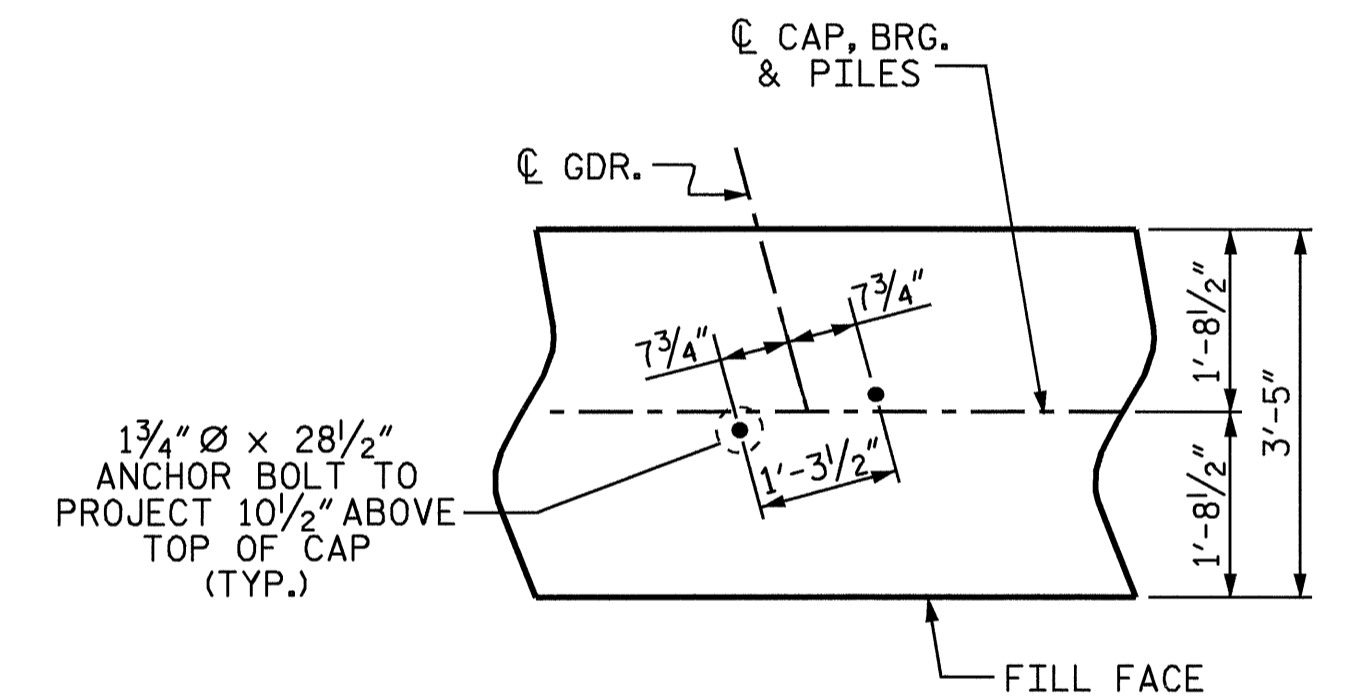
ELEVATIONS SHOWN ARE TO BOTTOM OF LEVELING PLATE

NOTES

STIRRUPS IN CAP MAY BE SHIFTED AS NECESSARY TO CLEAR ANCHOR BOLTS.
 SEE SUPERSTRUCTURE SHEETS FOR UPPER PART OF INTEGRAL END BENT DETAILS.
 THE CONTRACTOR SHALL PROVIDE FOR INSTALLATION OF THE 4" DIAMETER DRAIN PIPE THROUGH THE WING WALL AS REQUIRED FOR REINFORCED BRIDGE APPROACH FILLS, SEE ROADWAY PLANS. REINFORCING STEEL IN THE WING WALL MAY BE SHIFTED AS NECESSARY TO CLEAR THE DRAIN PIPES.



ELEVATION VIEW



PLAN VIEW

DETAIL "A"

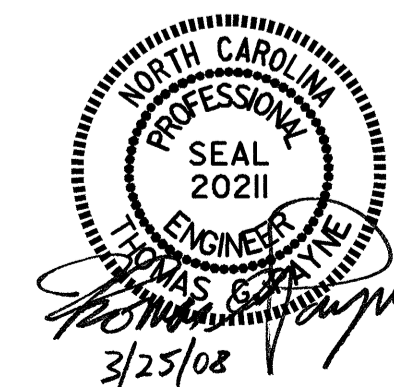
PROJECT NO. B-4252
ROCKINGHAM COUNTY
 STATION: 31+61.50 -L-

SHEET 1 OF 4

STATE OF NORTH CAROLINA
 DEPARTMENT OF TRANSPORTATION
 RALEIGH

SUBSTRUCTURE
 INTEGRAL
 END BENT #1
 (STAGE 1)

REVISIONS						TOTAL SHEETS
NO.	BY:	DATE:	NO.	BY:	DATE:	70
1			3			
2			4			



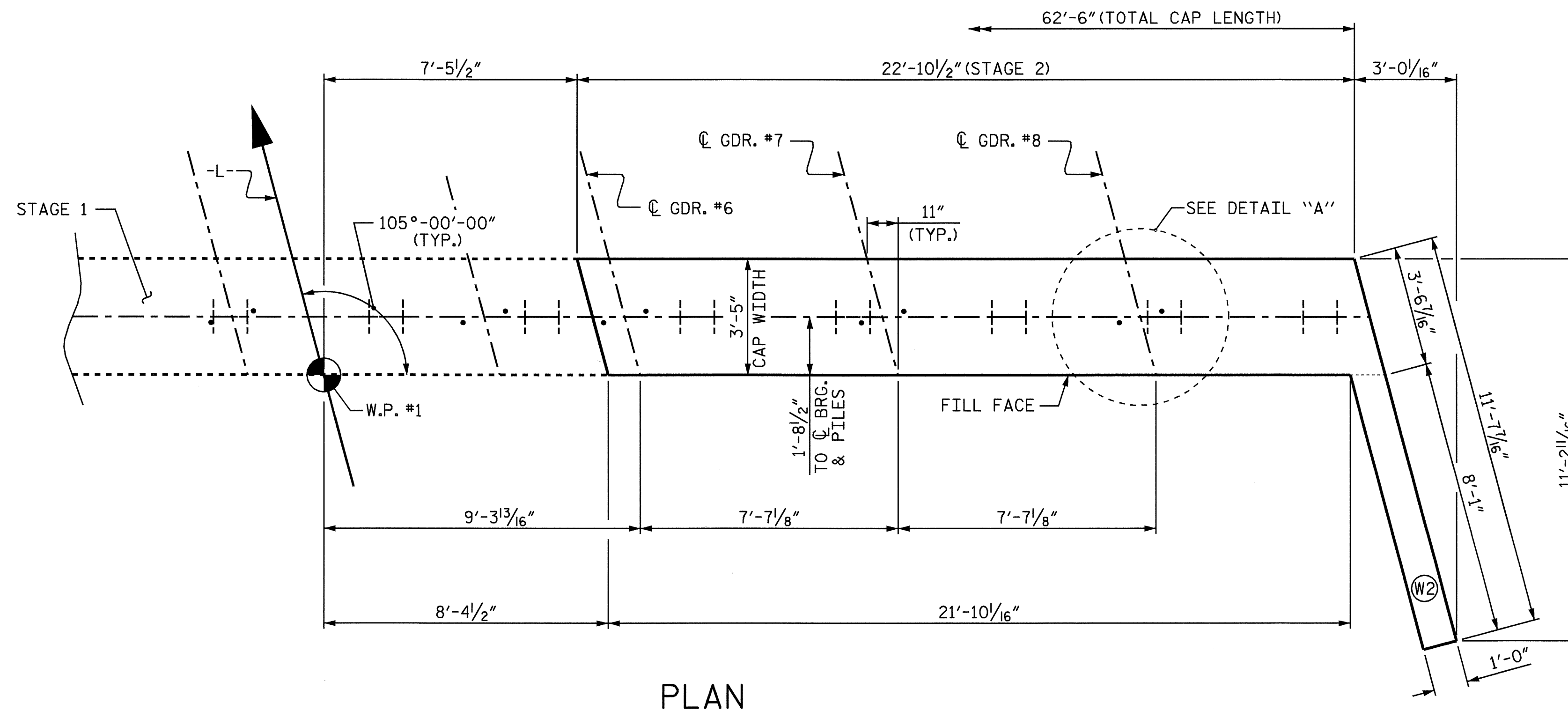
DRAWN BY : S. DOMBROWSKI DATE : 1/07
 CHECKED BY : M.K. BEARD DATE : 2/08

NOTES

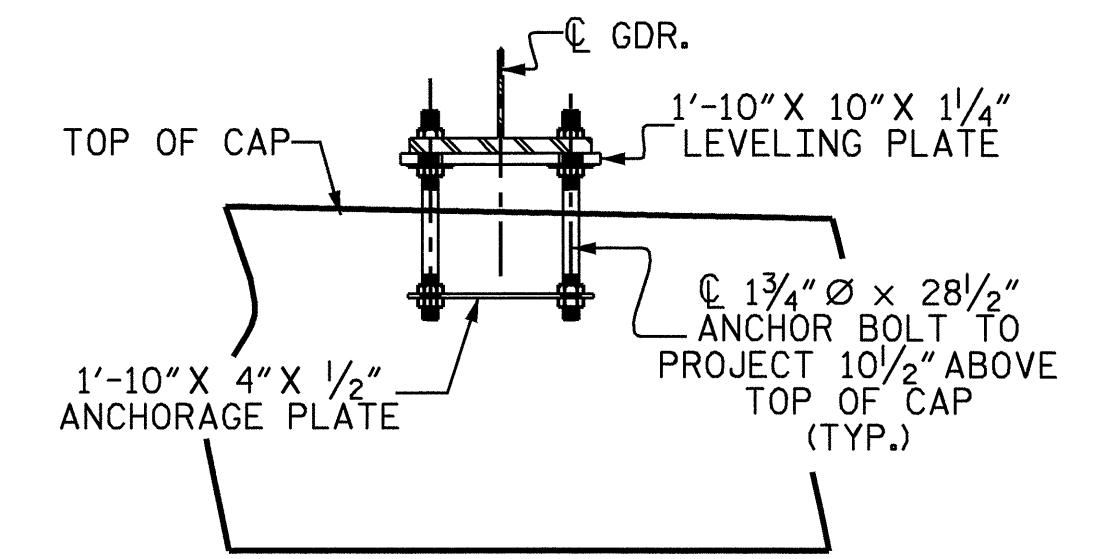
STIRRUPS IN CAP MAY BE SHIFTED AS NECESSARY TO CLEAR ANCHOR BOLTS.

SEE SUPERSTRUCTURE SHEETS FOR UPPER PART OF INTEGRAL END BENT DETAILS.

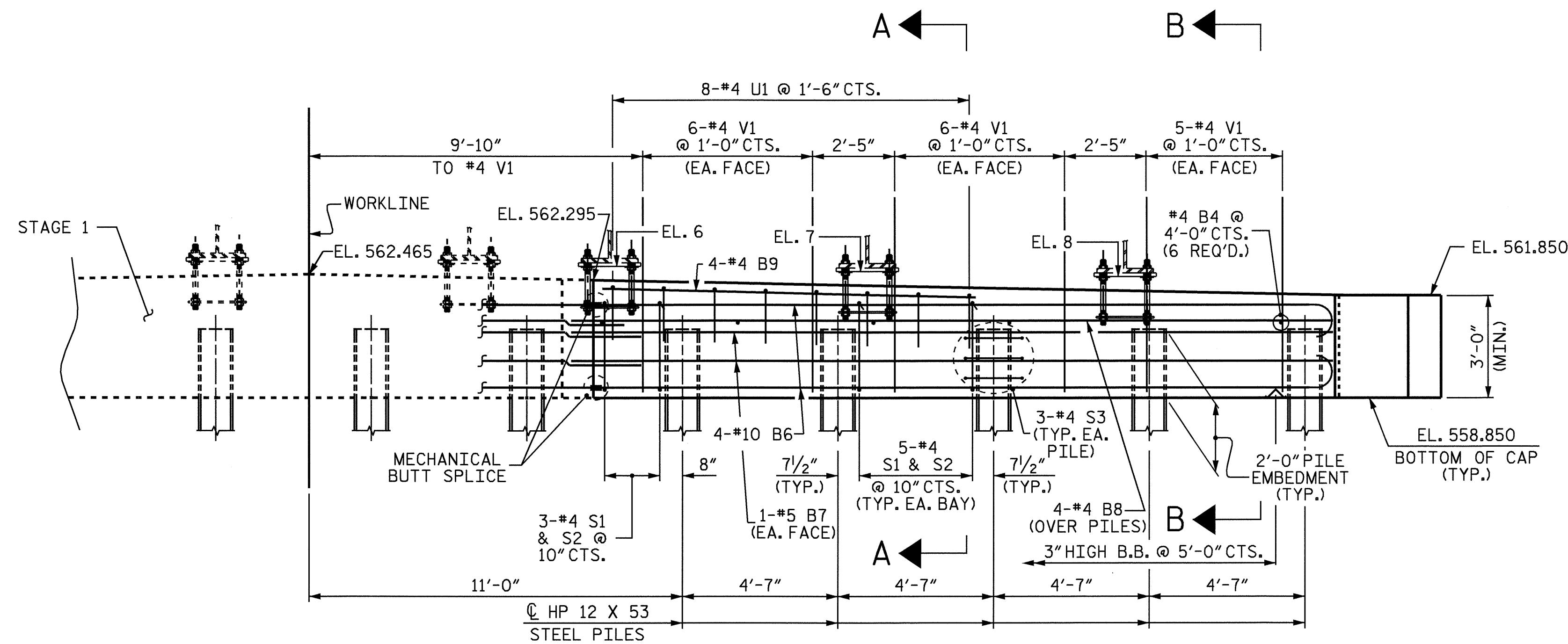
THE CONTRACTOR SHALL PROVIDE FOR INSTALLATION OF THE 4" DIAMETER DRAIN PIPE THROUGH THE WING WALL AS REQUIRED FOR REINFORCED BRIDGE APPROACH FILLS, SEE ROADWAY PLANS. REINFORCING STEEL IN THE WING WALL MAY BE SHIFTED AS NECESSARY TO CLEAR THE DRAIN PIPES.



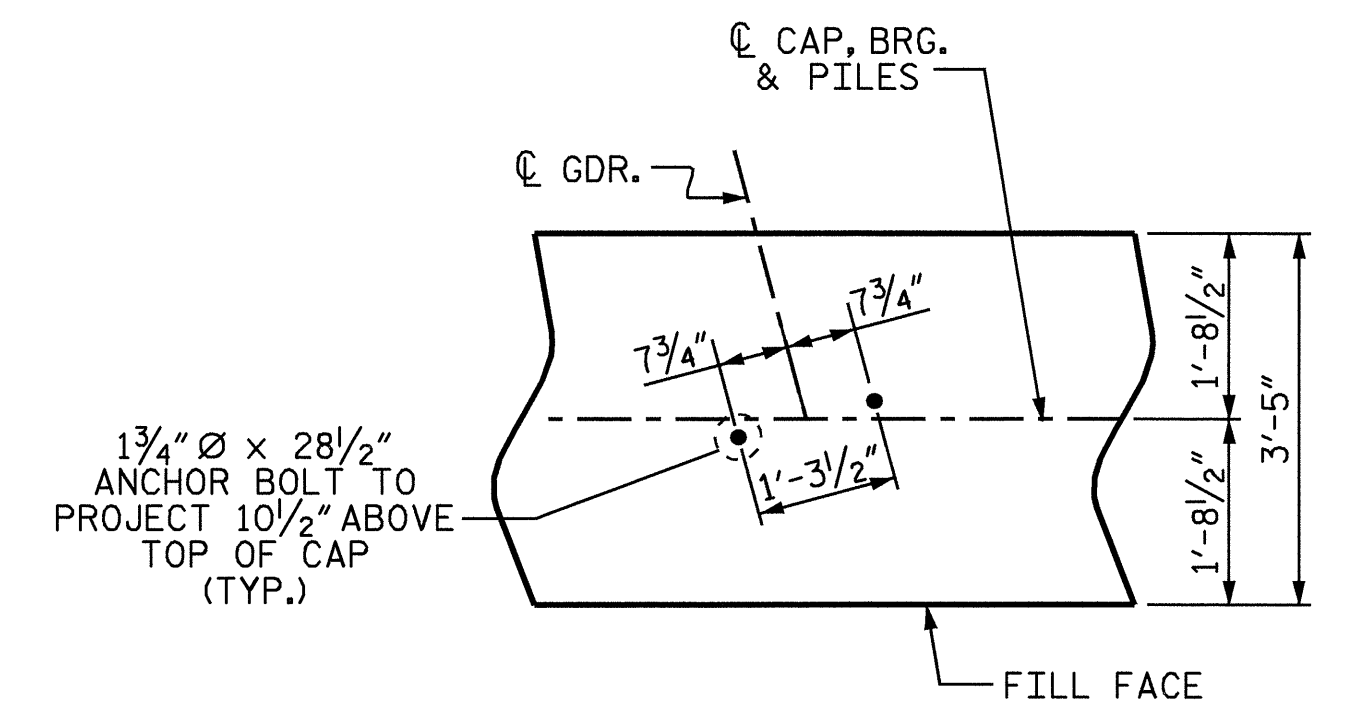
PLAN



ELEVATION VIEW



ELEVATION



PLAN VIEW

DETAIL "A"

EL. 6	562.707
EL. 7	562.555
EL. 8	562.402

ELEVATIONS SHOWN ARE TO BOTTOM OF LEVELING PLATE

PROJECT NO. B-4252
ROCKINGHAM COUNTY
 STATION: 31+61.50 -L-

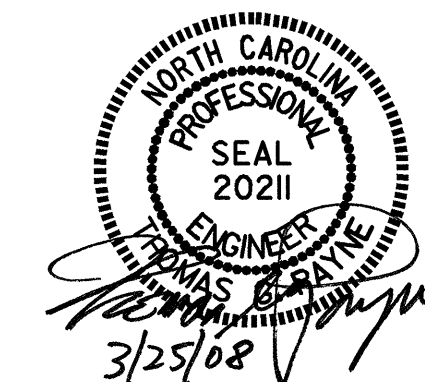
SHEET 2 OF 4

STATE OF NORTH CAROLINA
 DEPARTMENT OF TRANSPORTATION
 RALEIGH

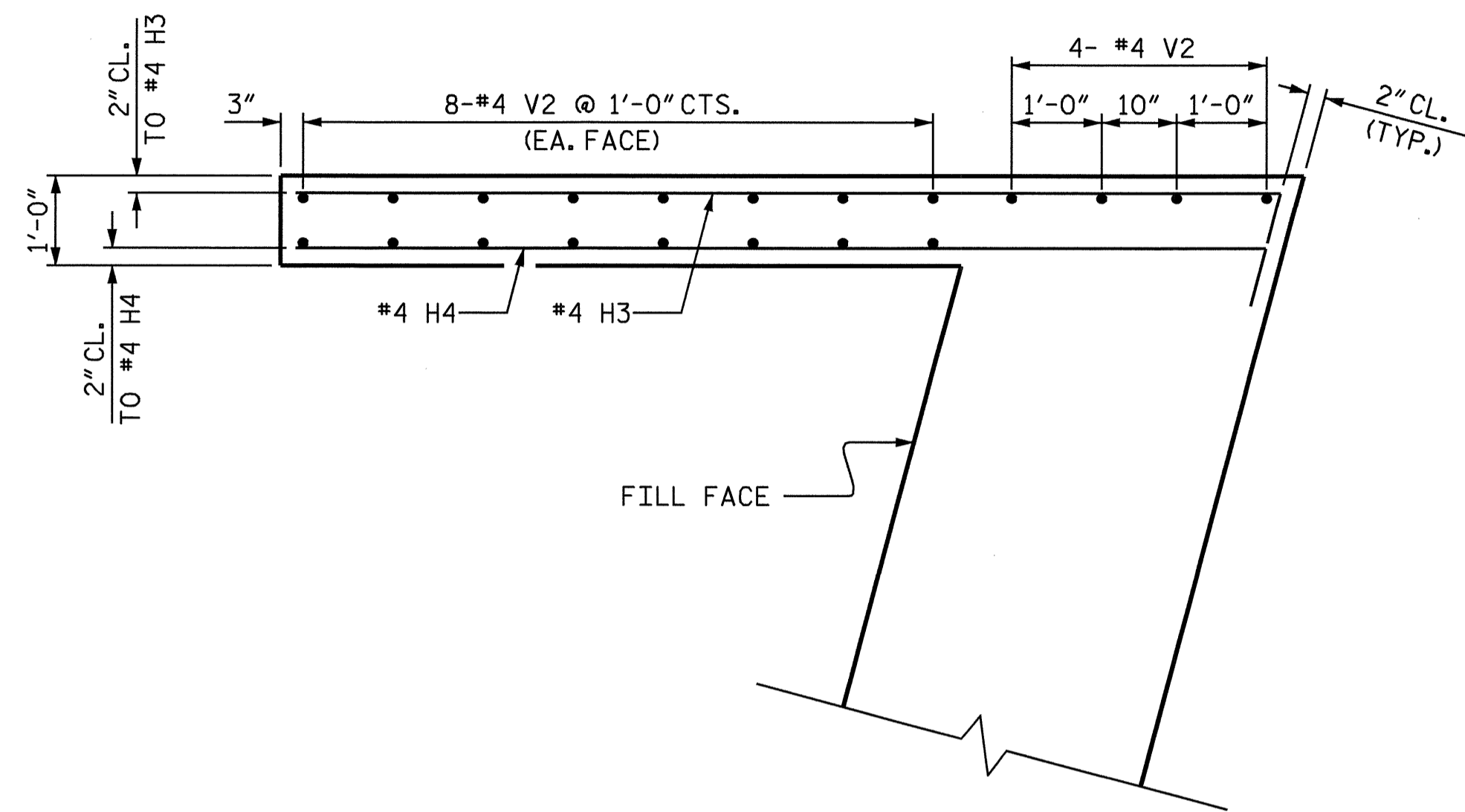
SUBSTRUCTURE
 INTEGRAL
 END BENT #1
 (STAGE 2)

REVISIONS						TOTAL SHEETS
NO.	BY:	DATE:	NO.	BY:	DATE:	70
1			3			
2			4			

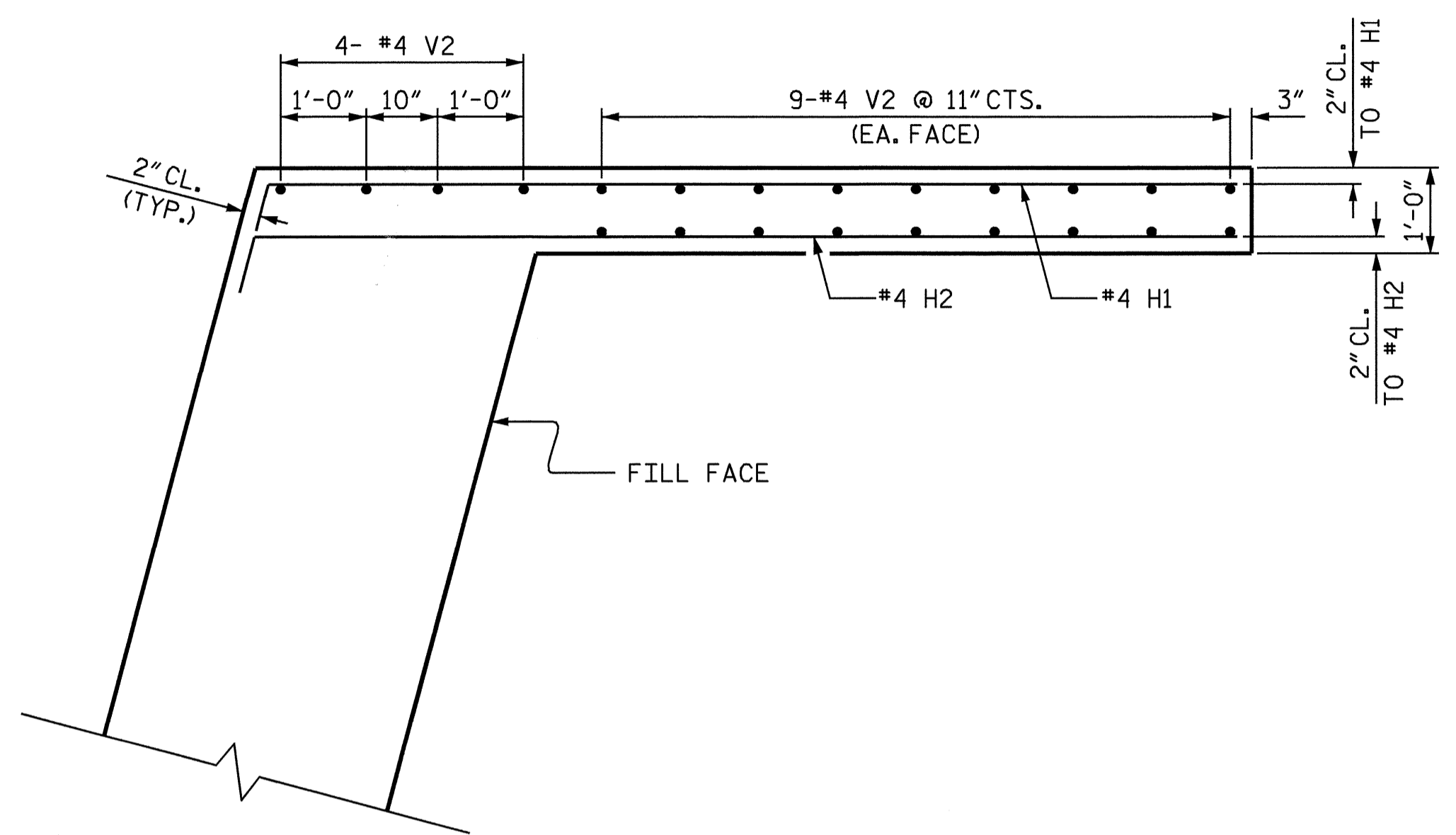
SHEET NO.
 S-59



DRAWN BY : S. DOMBROWSKI DATE : 1/07
 CHECKED BY : M.K. BEARD DATE : 2/08

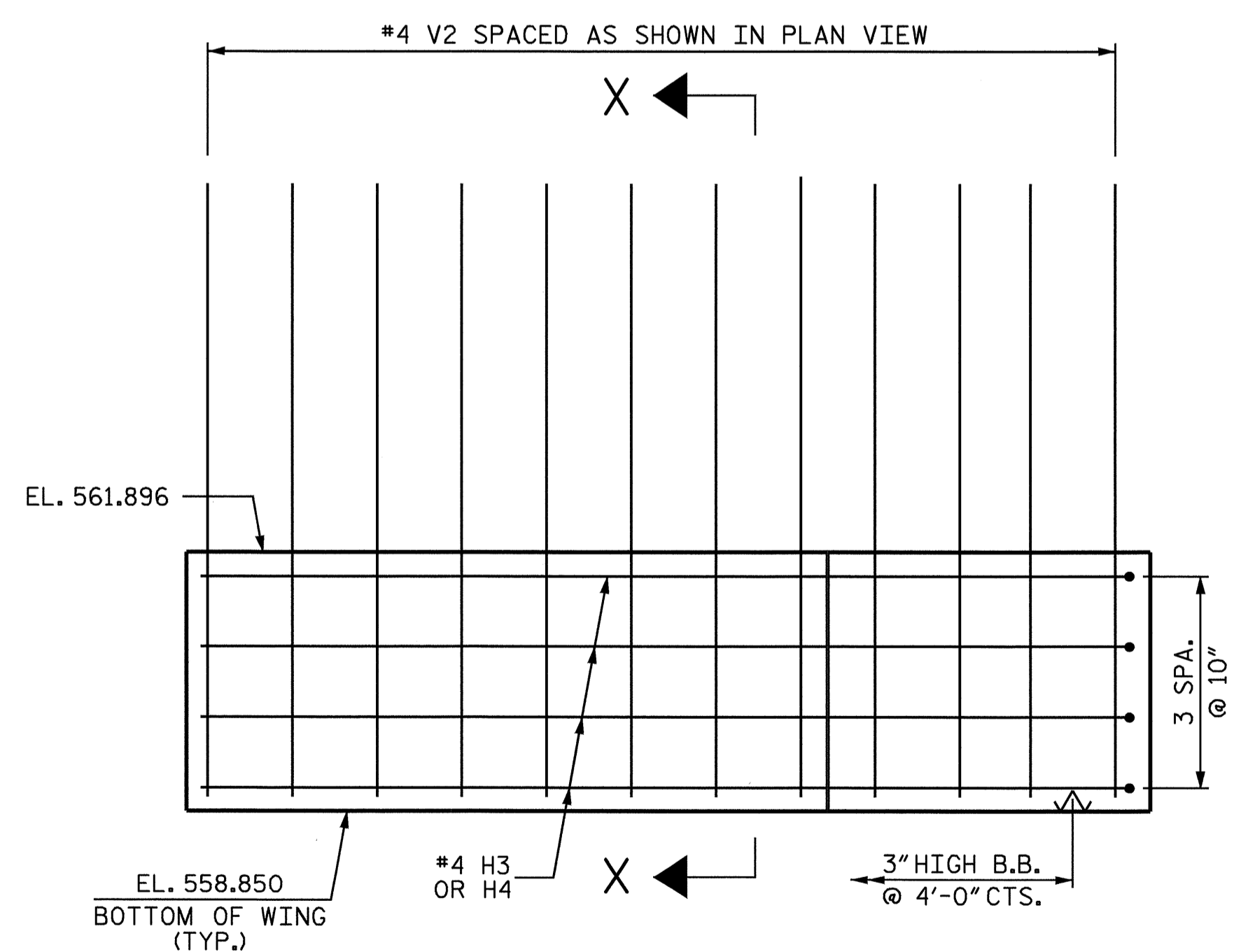


PLAN OF LEFT WING W1
(STAGE 1)

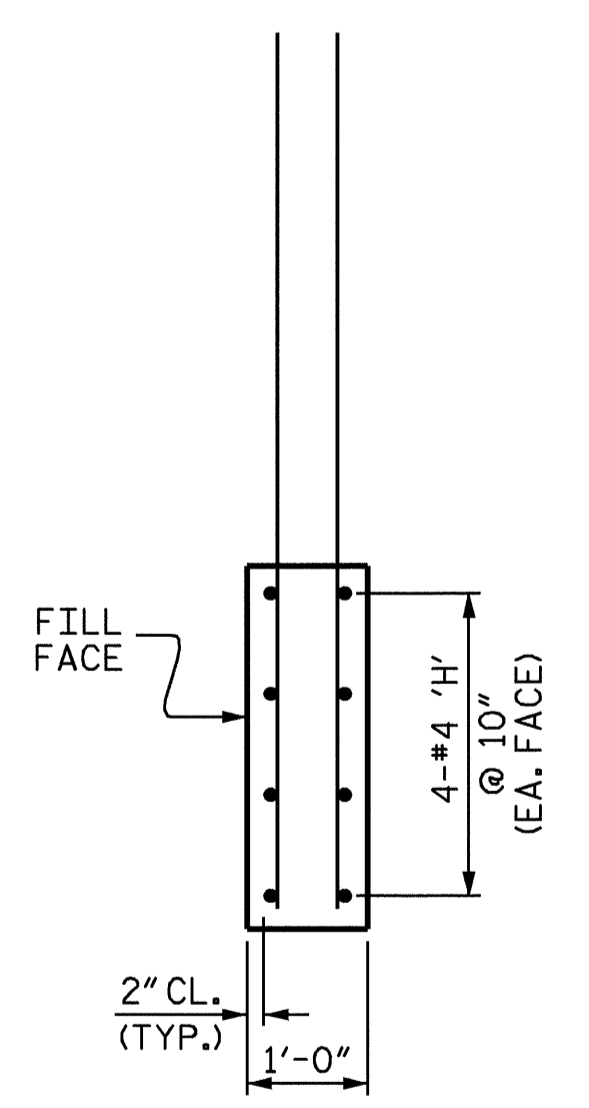


PLAN OF RIGHT WING W2
(STAGE 2)

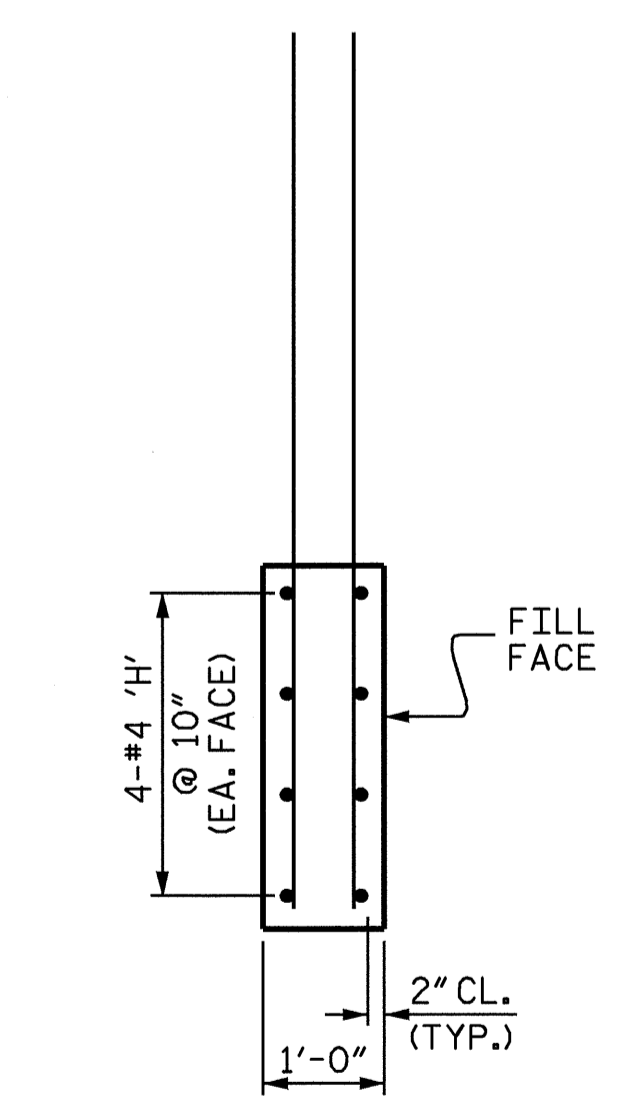
NOTE: THE UPPER PORTION OF THE WINGS SHALL BE POURED WITH THE SUPERSTRUCTURE. FOR DETAILS AND REINFORCING STEEL, SEE SUPERSTRUCTURE DETAILS.



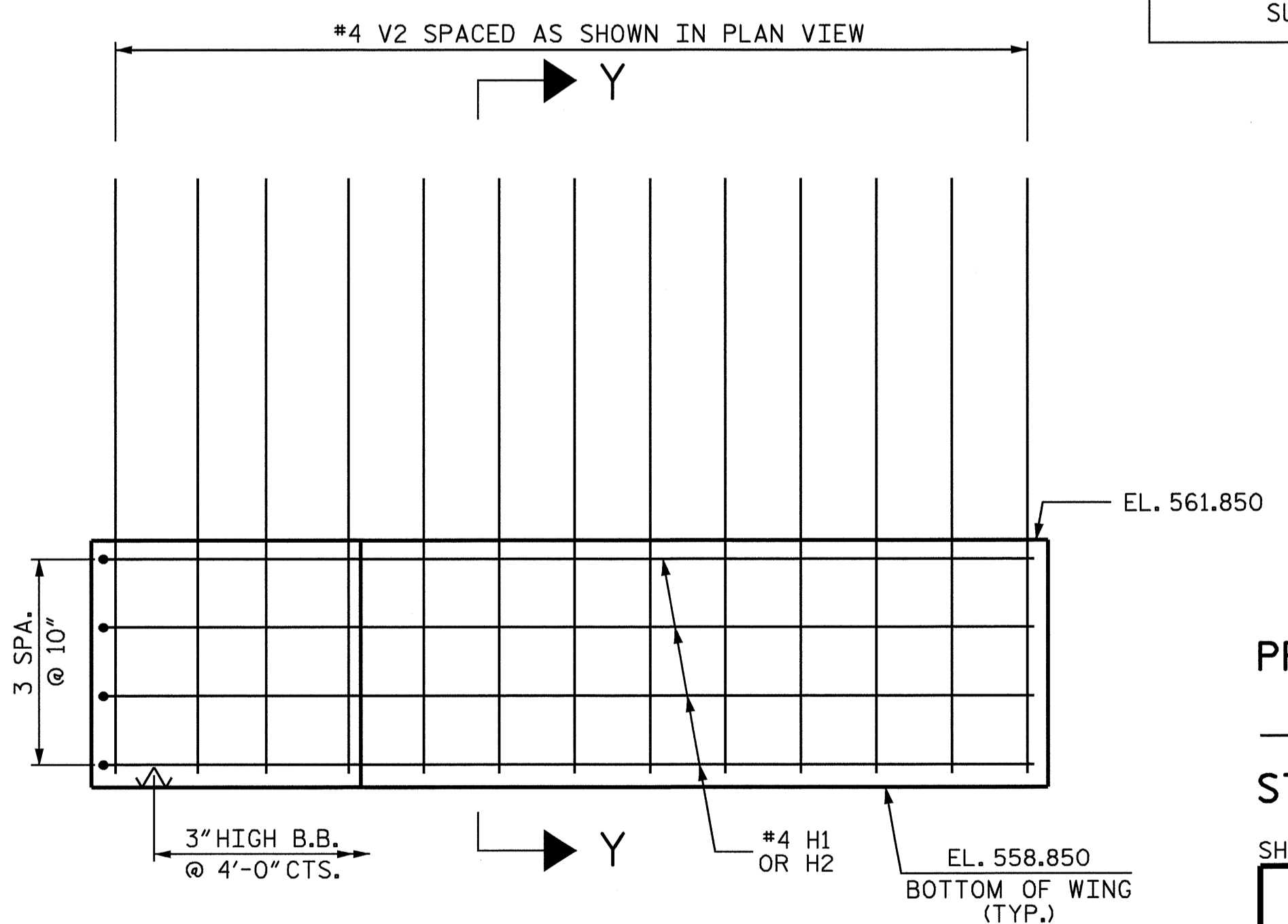
ELEVATION OF LEFT WING W1
(STAGE 1)



SECTION X-X



SECTION Y-Y



ELEVATION OF RIGHT WING W2
(STAGE 2)

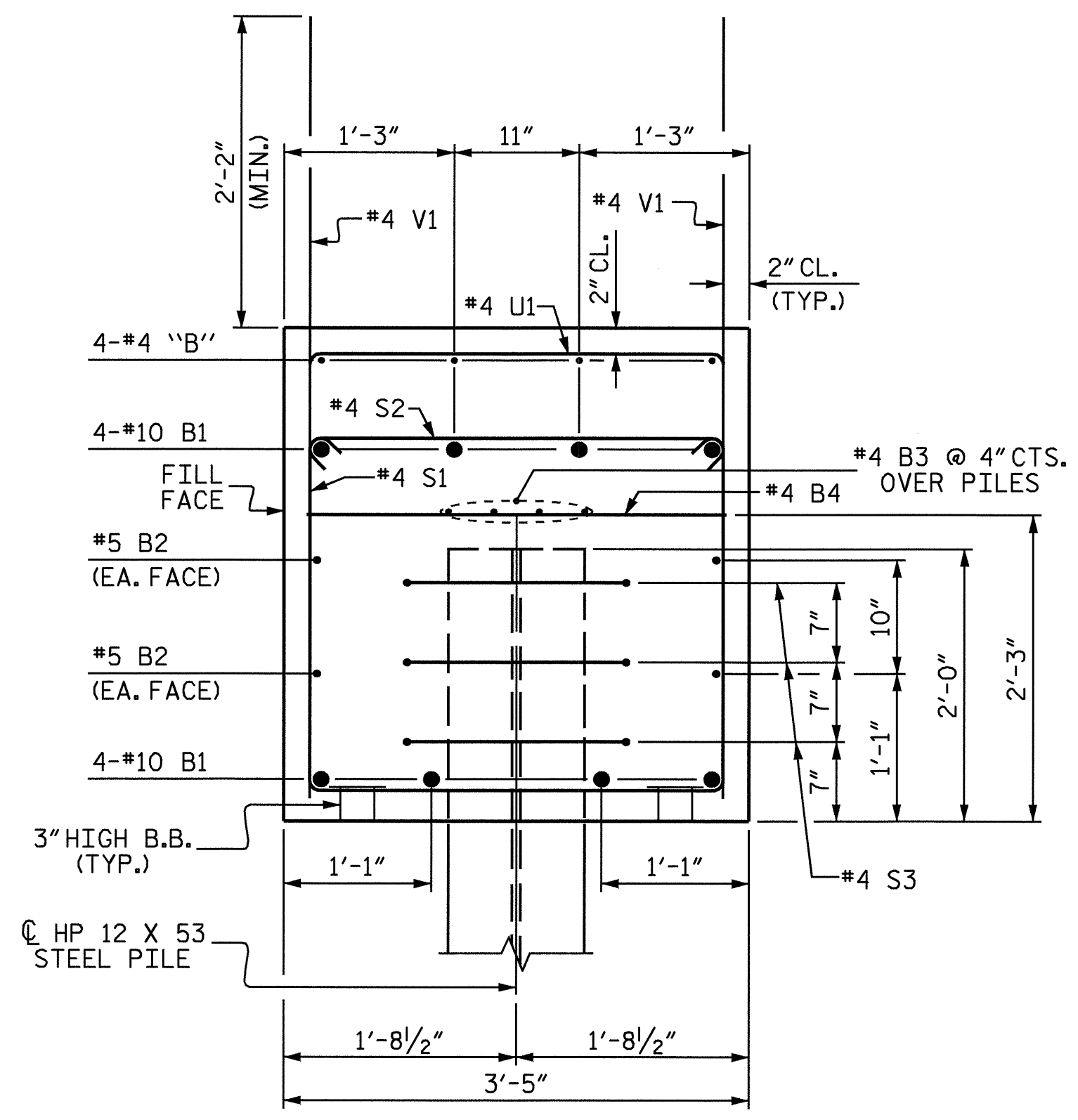
PROJECT NO. B-4252
ROCKINGHAM COUNTY
STATION: 31+61.50 -L-

SHEET 3 OF 4
STATE OF NORTH CAROLINA
DEPARTMENT OF TRANSPORTATION
RALEIGH
SUBSTRUCTURE
INTEGRAL
END BENT #1

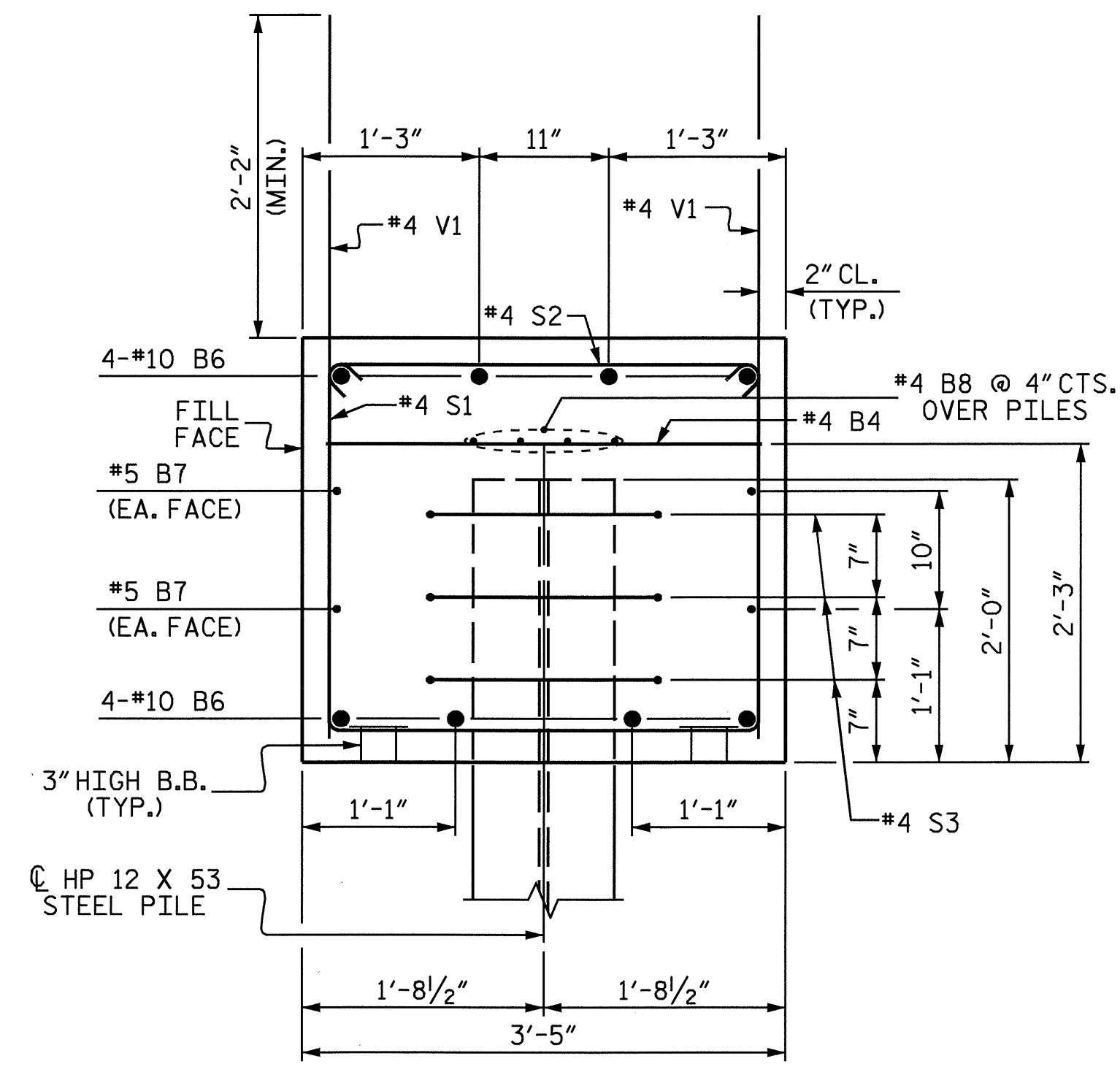


DRAWN BY : S. DOMBROWSKI DATE : 1/07
CHECKED BY : M.K. BEARD DATE : 2/08

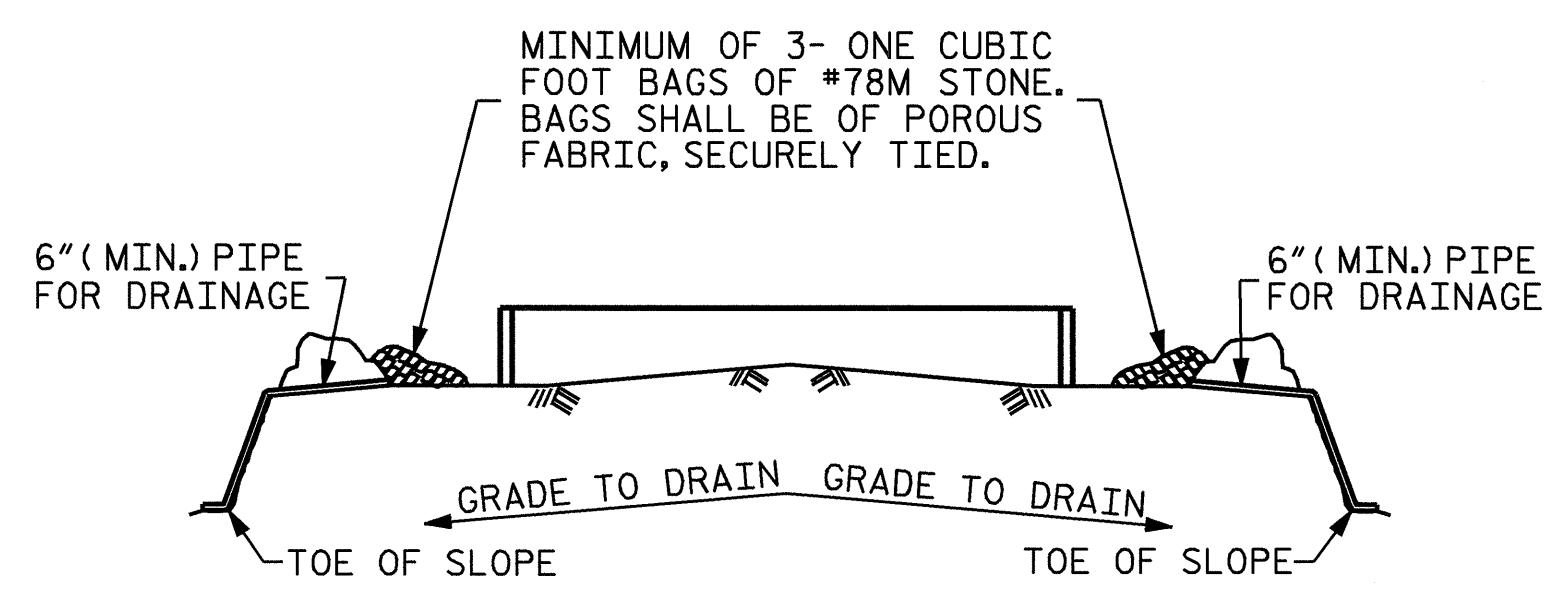
REVISIONS						SHEET NO.	
NO.	BY:	DATE:	NO.	BY:	DATE:	S-60	
1			3			TOTAL SHEETS 70	
2			4				



SECTION A-A



SECTION B-B

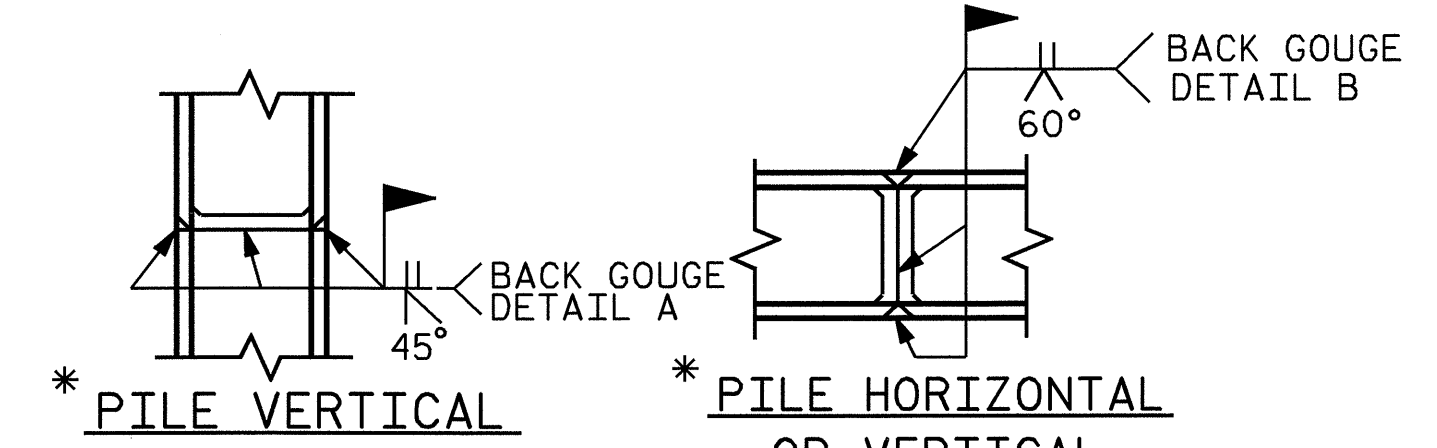


BAGGED STONE AND PIPE SHALL BE PLACED IMMEDIATELY AFTER COMPLETION OF END BENT EXCAVATION. PIPE MAY BE EITHER CONCRETE, CORRUGATED STEEL, CORRUGATED ALUMINUM ALLOY, OR CORRUGATED PLASTIC. PERFORATED PIPE WILL NOT BE ALLOWED.

BAGGED STONE SHALL REMAIN IN PLACE UNTIL THE ENGINEER DIRECTS THAT IT BE REMOVED. THE CONTRACTOR SHALL REMOVE AND DISPOSE OF SILT ACCUMULATIONS AT BAGGED STONE WHEN SO DIRECTED BY THE ENGINEER. BAGS SHALL BE REMOVED AND REPLACED WHENEVER THE ENGINEER DETERMINES THAT THEY HAVE DETERIORATED AND LOST THEIR EFFECTIVENESS.

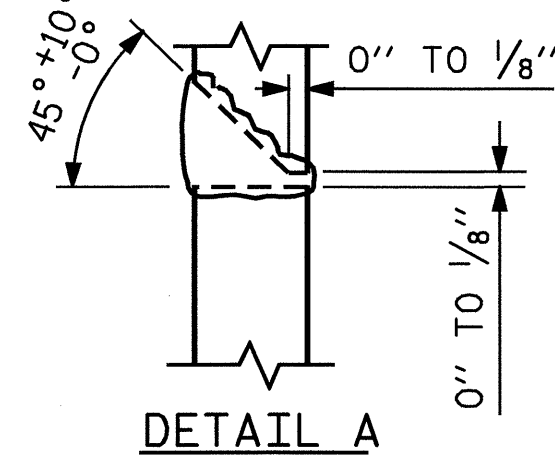
NO SEPARATE PAYMENT WILL BE MADE FOR THIS WORK AND THE ENTIRE COST OF THIS WORK SHALL BE INCLUDED IN THE UNIT CONTRACT PRICE BID FOR THE SEVERAL PAY ITEMS.

TEMPORARY DRAINAGE AT END BENT

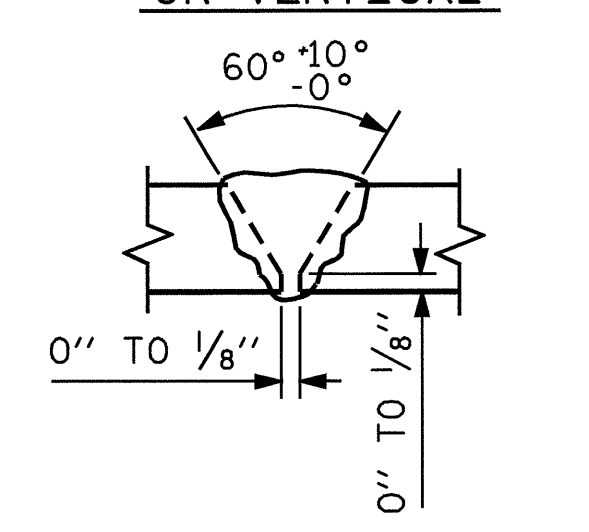


* PILE VERTICAL

* PILE HORIZONTAL OR VERTICAL



DETAIL A



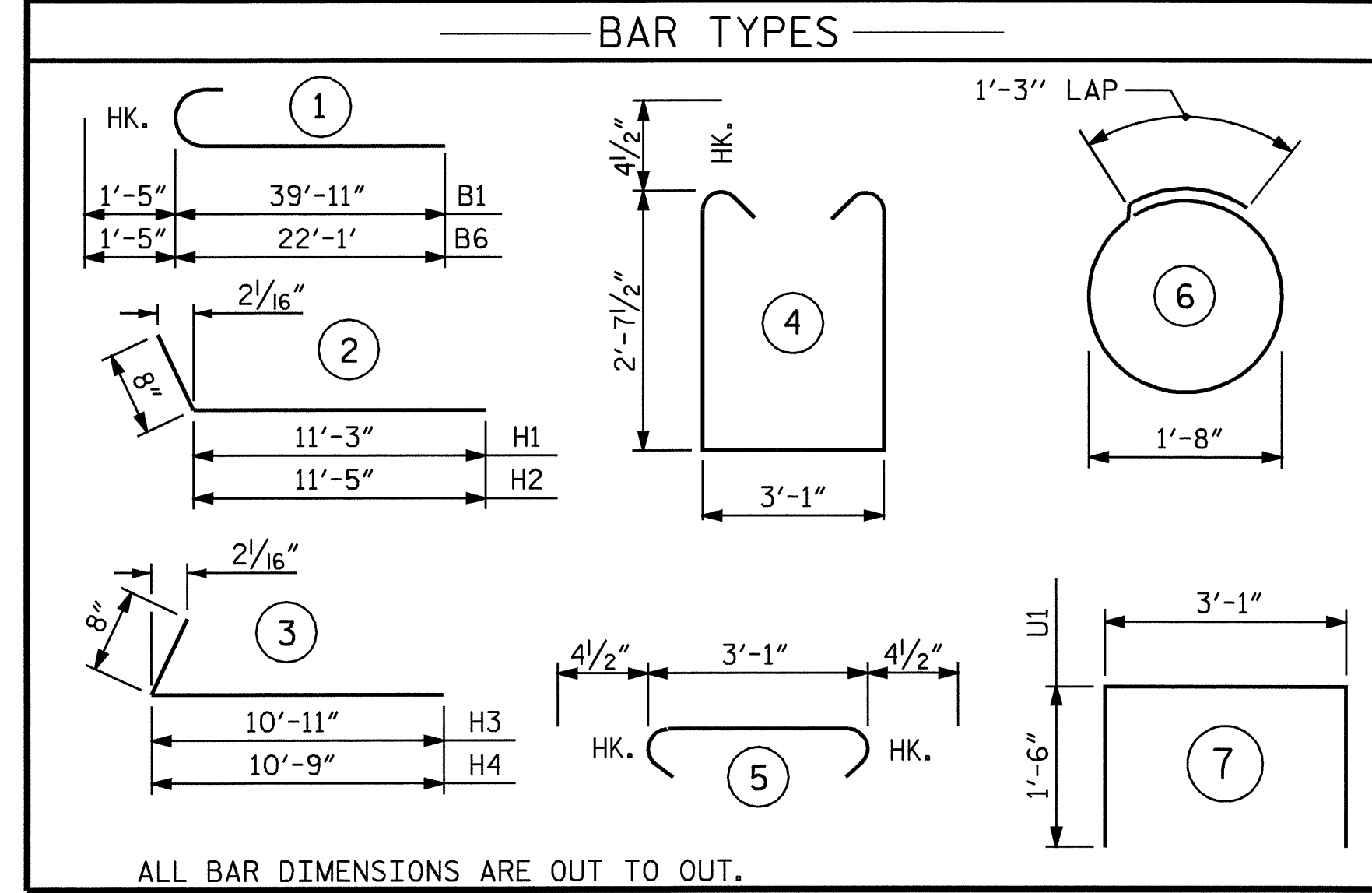
DETAIL B

* POSITION OF PILE DURING WELDING.

PILE SPLICE DETAILS

BILL OF MATERIAL																			
INTEGRAL END BENT #1																			
STAGE 1					STAGE 2														
BAR	NO.	SIZE	TYPE	LENGTH	WEIGHT	BAR	NO.	SIZE	TYPE	LENGTH	WEIGHT								
B1	8	10	1	41'-4"	1423	B4	6	4	STR	3'-1"	12								
B2	4	5	STR	42'-6"	177	B6	8	10	1	23'-6"	809								
B3	8	4	STR	22'-2"	118	B7	4	5	STR	22'-8"	95								
B4	10	4	STR	3'-1"	21	B8	4	4	STR	22'-8"	61								
B5	8	4	STR	15'-10"	85	B9	4	4	STR	12'-2"	33								
H3	4	4	2	11'-7"	31	H1	4	4	3	11'-11"	32								
H4	4	4	2	11'-5"	31	H2	4	4	3	12'-1"	32								
S1	40	4	4	9'-1"	243	S1	23	4	4	9'-1"	140								
S2	40	4	5	3'-10"	102	S2	23	4	5	3'-10"	59								
S3	27	4	6	6'-6"	117	S3	15	4	6	6'-6"	65								
U1	20	4	7	6'-1"	81	U1	8	4	7	6'-1"	33								
V1	62	4	STR	5'-9"	238	V1	34	4	STR	5'-9"	131								
V2	20	4	STR	7'-3"	97	V2	22	4	STR	7'-3"	107								
REINFORCING STEEL					LBS	2764	REINFORCING STEEL					LBS	1609						
CLASS 'A' CONCRETE					17.8 C.Y.					CLASS 'A' CONCRETE					10.3 C.Y.				
HP 12X53 STEEL PILES					NO. 9					HP 12X53 STEEL PILES					NO. 5				
					135 LIN.FT.										75 LIN.FT.				
PILE EXCAVATION IN SOIL					111 LIN.FT.					PILE EXCAVATION IN SOIL					61 LIN.FT.				
PILE EXCAV. NOT IN SOIL					24 LIN.FT.					PILE EXCAV. NOT IN SOIL					13 LIN.FT.				

TOTAL BILL OF MATERIAL															
REINFORCING STEEL					LBS	4373	HP 12X53 STEEL PILES					NO. 14	210 LIN.FT.		
CLASS 'A' CONCRETE					28.1 C.Y.					PILE EXCAVATION IN SOIL					172 LIN.FT.
										PILE EXCAV. NOT IN SOIL					37 LIN.FT.



PROJECT NO. B-4252
ROCKINGHAM COUNTY
 STATION: 31+61.50 -L-

SHEET 4 OF 4

STATE OF NORTH CAROLINA DEPARTMENT OF TRANSPORTATION RALEIGH					
SUBSTRUCTURE INTEGRAL END BENT #1					
REVISIONS					
NO.	BY:	DATE:	NO.	BY:	DATE:
1			3		
2			4		



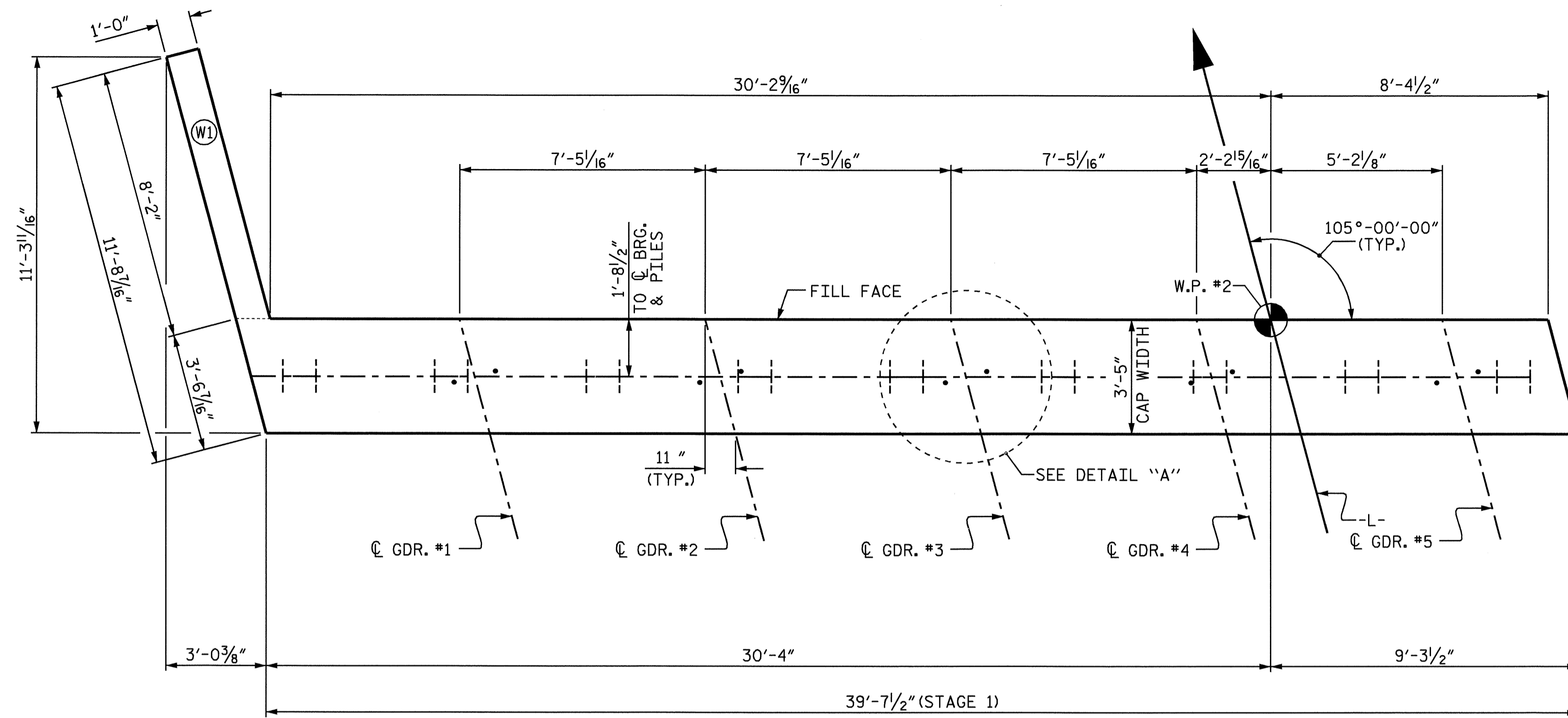
DRAWN BY : S. DOMBROWSKI DATE : 1/07
 CHECKED BY : M.K. BEARD DATE : 2/08

NOTES

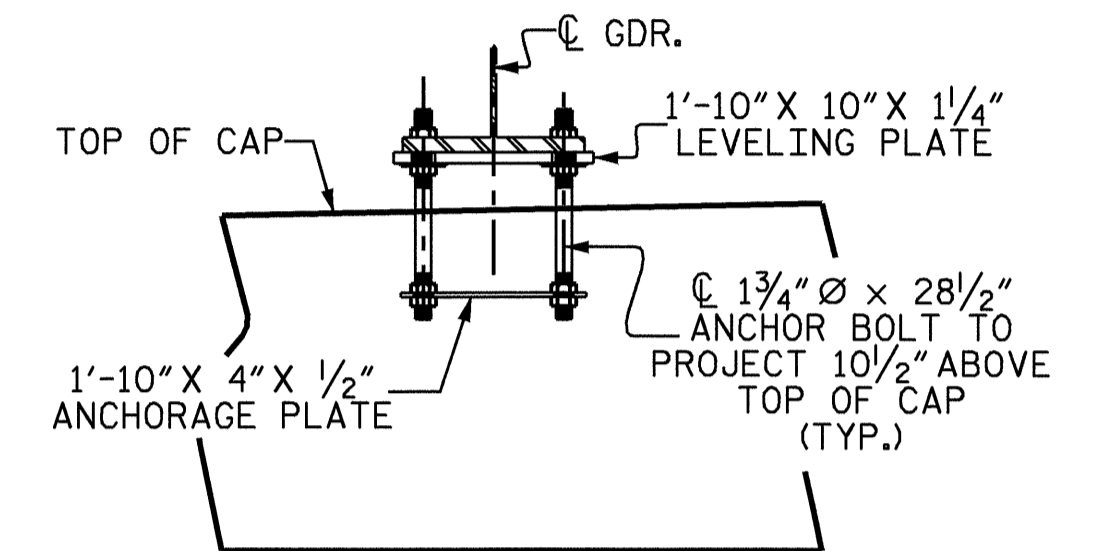
STIRRUPS IN CAP MAY BE SHIFTED AS NECESSARY TO CLEAR ANCHOR BOLTS.

SEE SUPERSTRUCTURE SHEETS FOR UPPER PART OF INTEGRAL END BENT DETAILS.

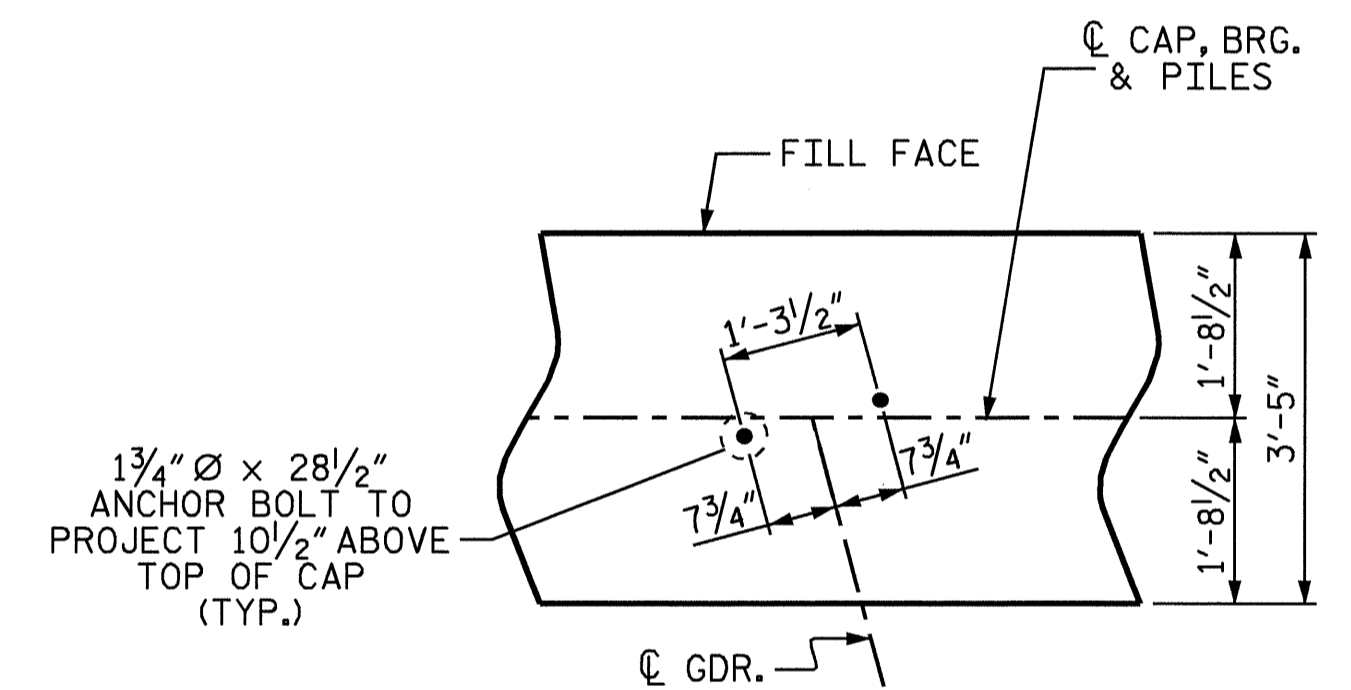
THE CONTRACTOR SHALL PROVIDE FOR INSTALLATION OF THE 4" DIAMETER DRAIN PIPE THROUGH THE WING WALL AS REQUIRED FOR REINFORCED BRIDGE APPROACH FILLS, SEE ROADWAY PLANS. REINFORCING STEEL IN THE WING WALL MAY BE SHIFTED AS NECESSARY TO CLEAR THE DRAIN PIPES.



PLAN

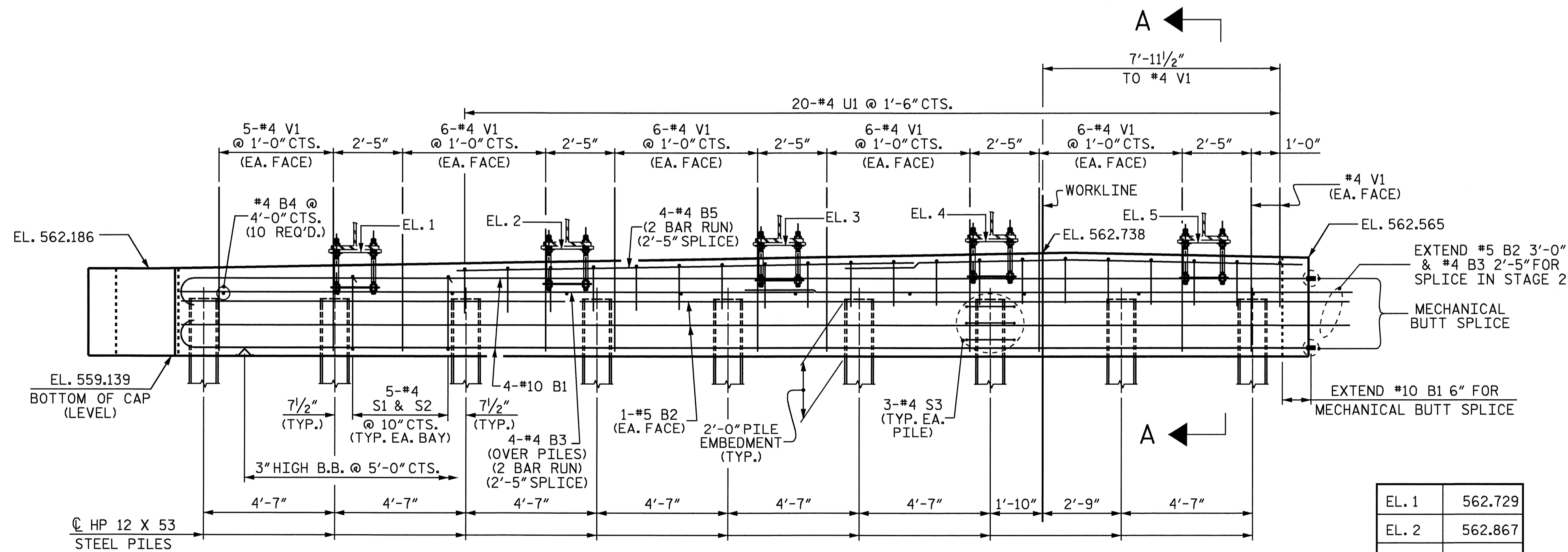


ELEVATION VIEW



PLAN VIEW

DETAIL "A"



ELEVATION

EL. 1	562.729
EL. 2	562.867
EL. 3	563.005
EL. 4	563.142
EL. 5	563.080

ELEVATIONS SHOWN ARE TO BOTTOM OF LEVELING PLATE



PROJECT NO. B-4252
ROCKINGHAM COUNTY
 STATION: 31+61.50 -L-

SHEET 1 OF 4

STATE OF NORTH CAROLINA
 DEPARTMENT OF TRANSPORTATION
 RALEIGH

SUBSTRUCTURE
 INTEGRAL
 END BENT #2
 (STAGE 1)

REVISIONS						SHEET NO.
NO.	BY:	DATE:	NO.	BY:	DATE:	S-62
1			3			TOTAL SHEETS
2			4			70

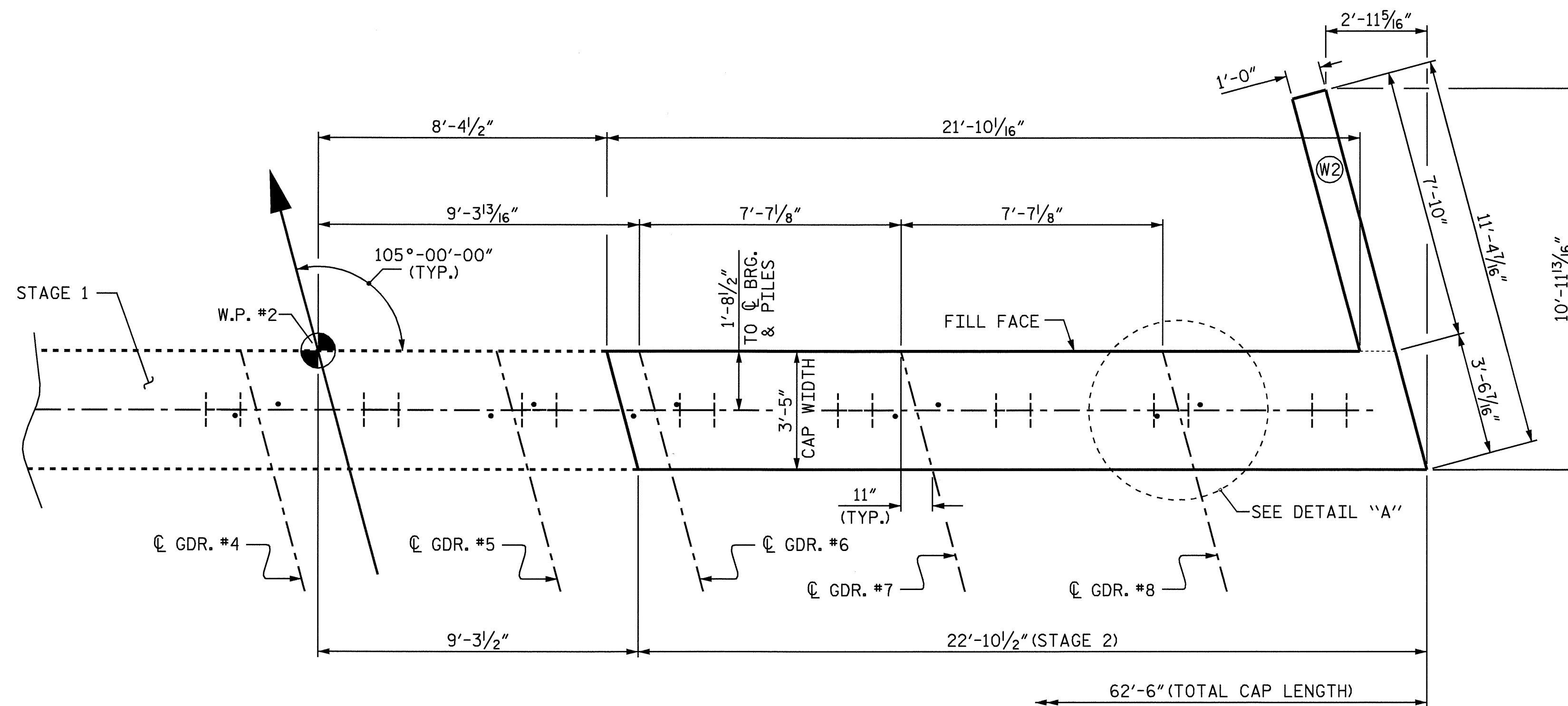
DRAWN BY : S. DOMBROWSKI DATE : 1/07
 CHECKED BY : M.K. BEARD DATE : 2/08

NOTES

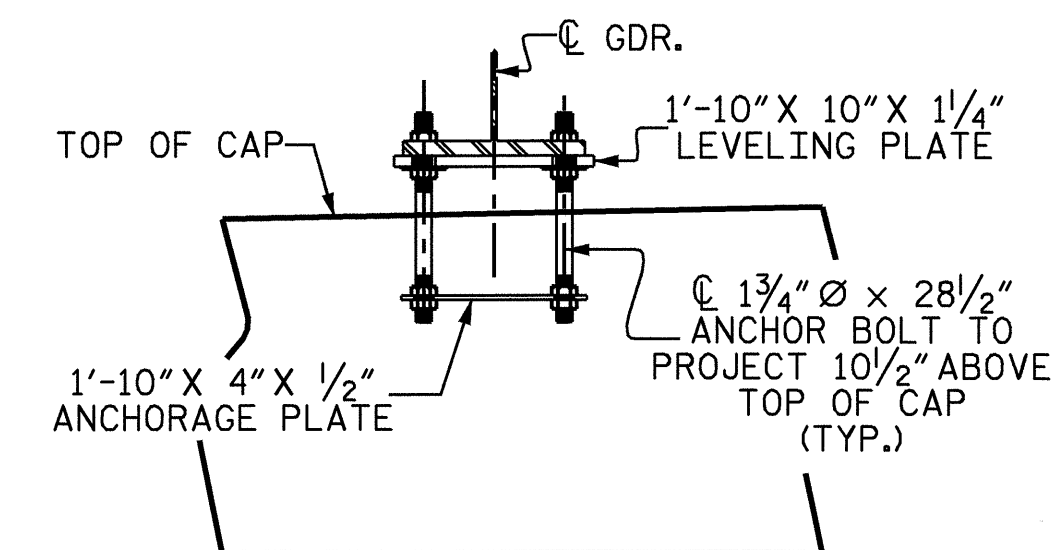
STIRRUPS IN CAP MAY BE SHIFTED AS NECESSARY TO CLEAR ANCHOR BOLTS.

SEE SUPERSTRUCTURE SHEETS FOR UPPER PART OF INTEGRAL END BENT DETAILS.

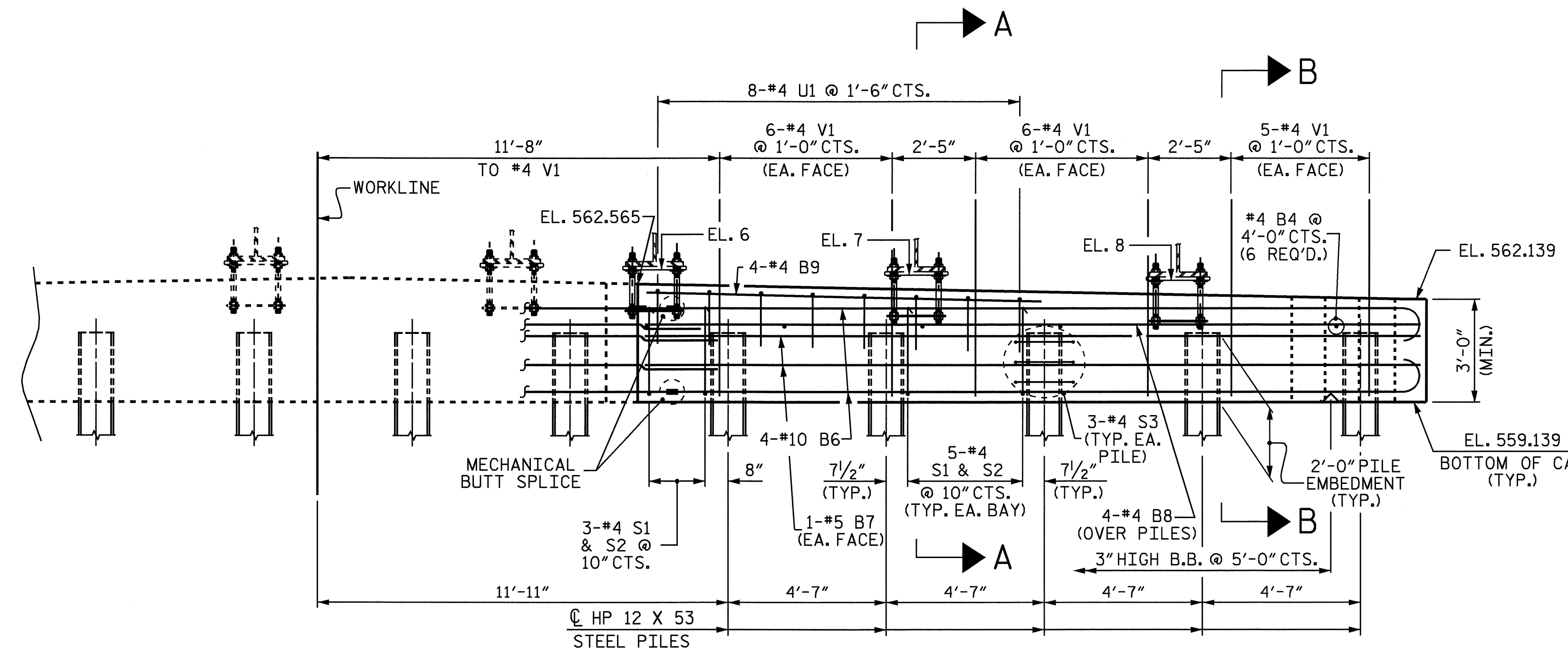
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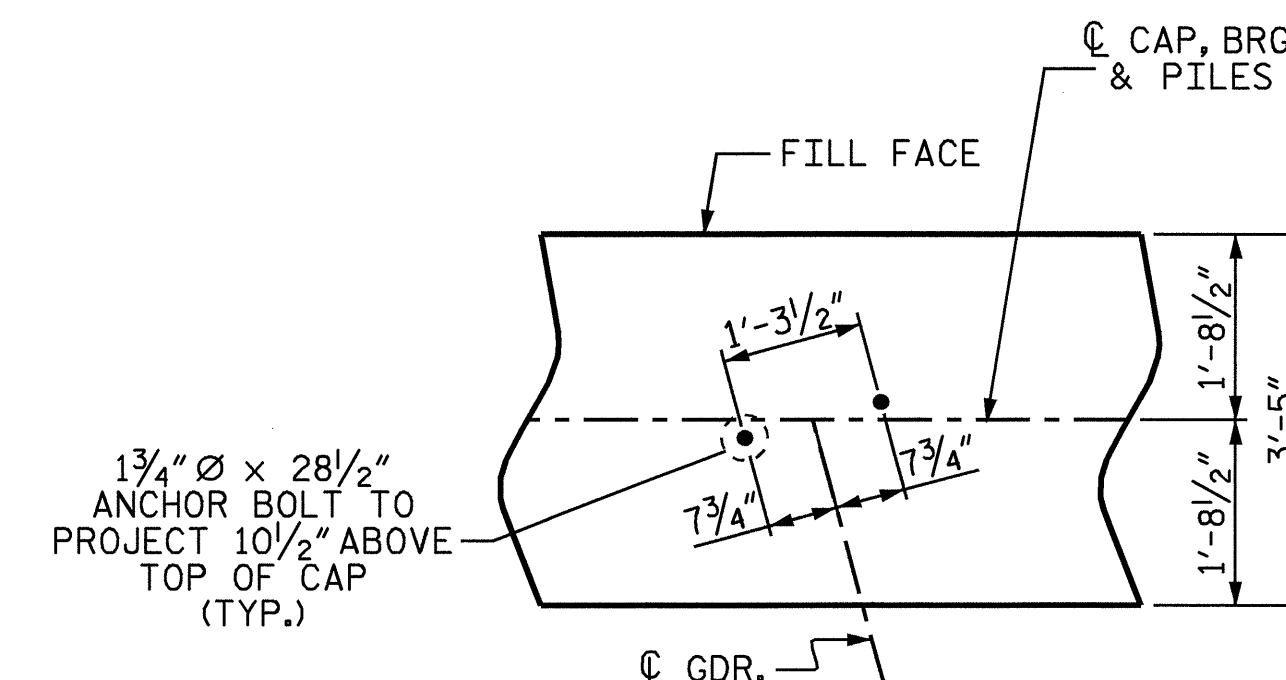
PLAN



ELEVATION VIEW



ELEVATION



PLAN VIEW

DETAIL "A"

EL. 6	562.997
EL. 7	562.844
EL. 8	562.691

ELEVATIONS SHOWN ARE TO BOTTOM OF LEVELING PLATE

PROJECT NO. B-4252
ROCKINGHAM COUNTY
 STATION: 31+61.50 -L-

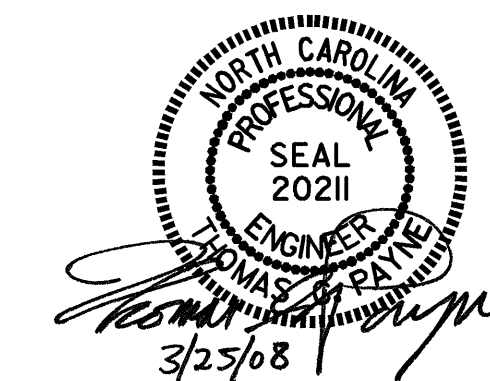
SHEET 2 OF 4

STATE OF NORTH CAROLINA
 DEPARTMENT OF TRANSPORTATION
 RALEIGH

SUBSTRUCTURE
 INTEGRAL
 END BENT #2
 (STAGE 2)

REVISIONS				SHEET NO.	
NO.	BY:	DATE:	NO.	BY:	DATE:
1			3		
2			4		

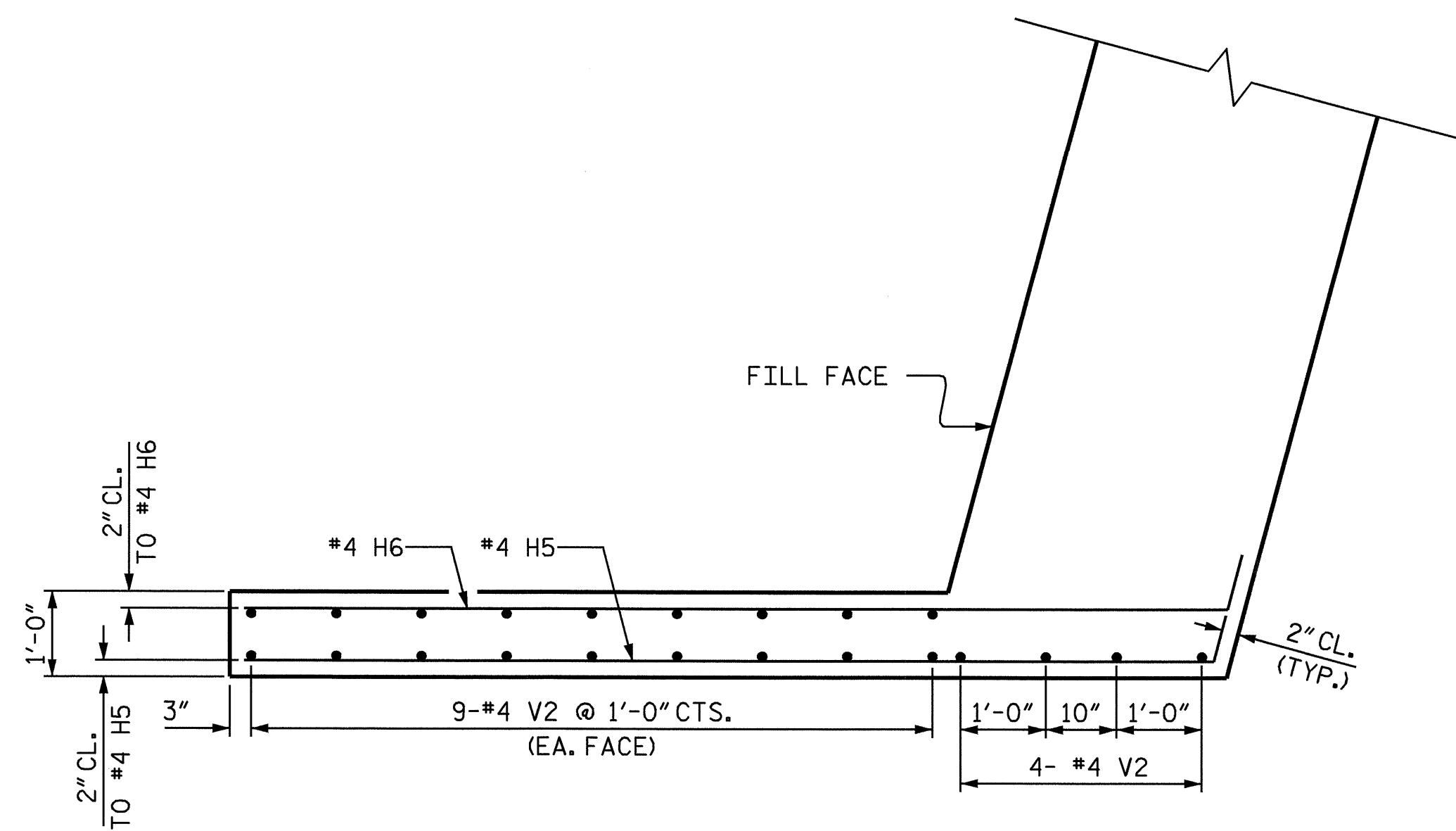
TOTAL SHEETS 70



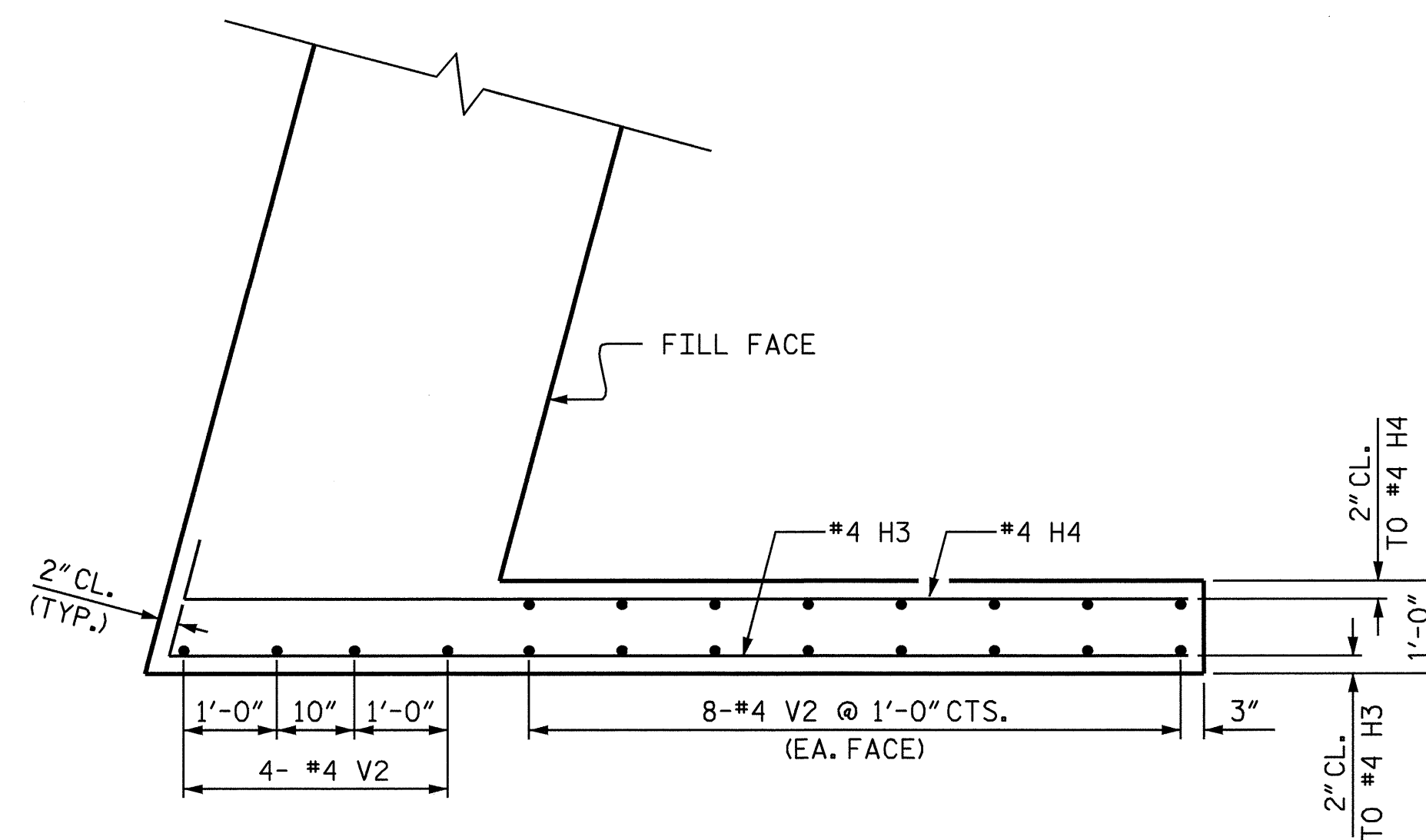
DRAWN BY: S. DOMBROWSKI DATE: 1/07
 CHECKED BY: M.K. BEARD DATE: 2/08

20-MAR-2008 15:34
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STR. #2

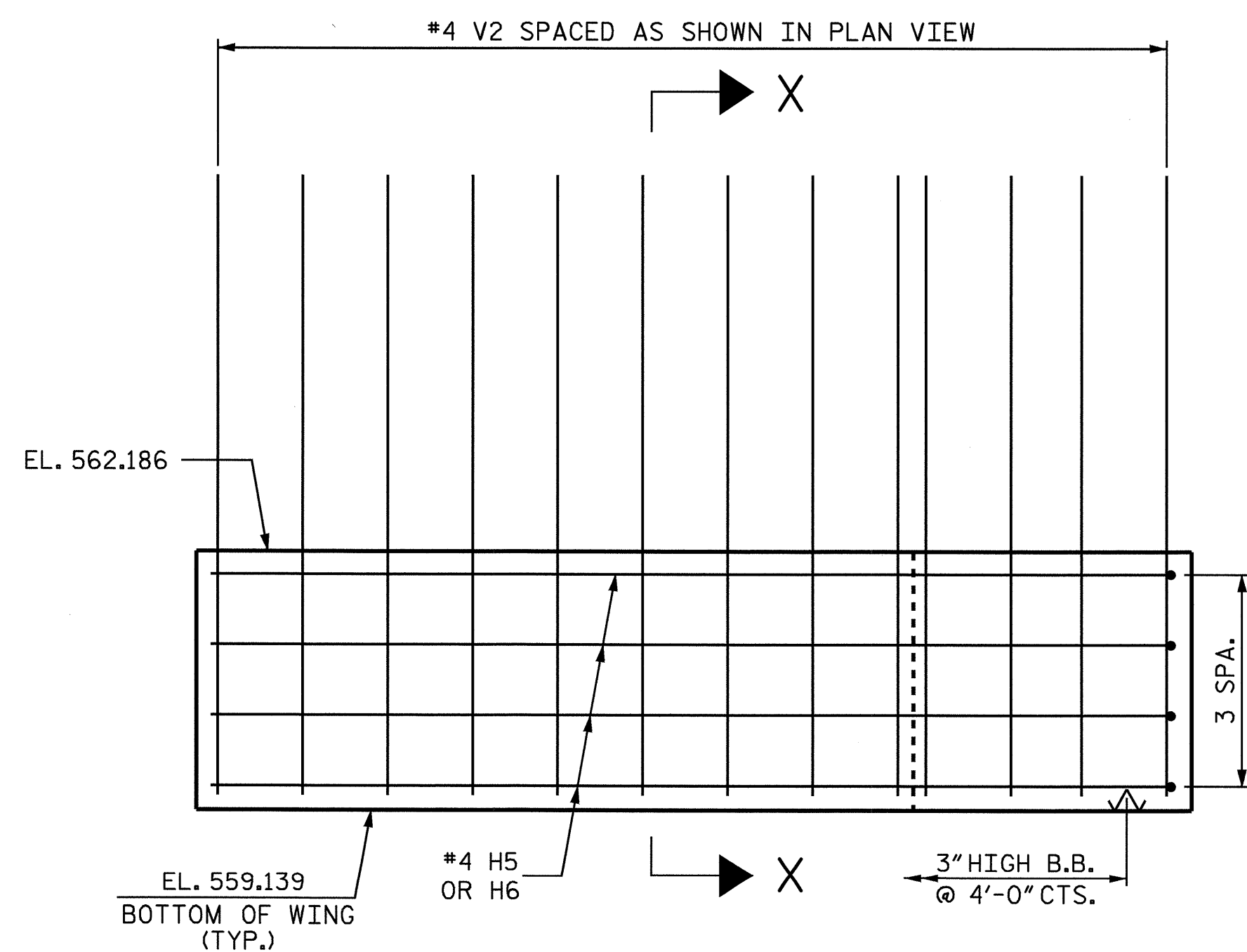


PLAN OF LEFT WING W1
(STAGE 1)

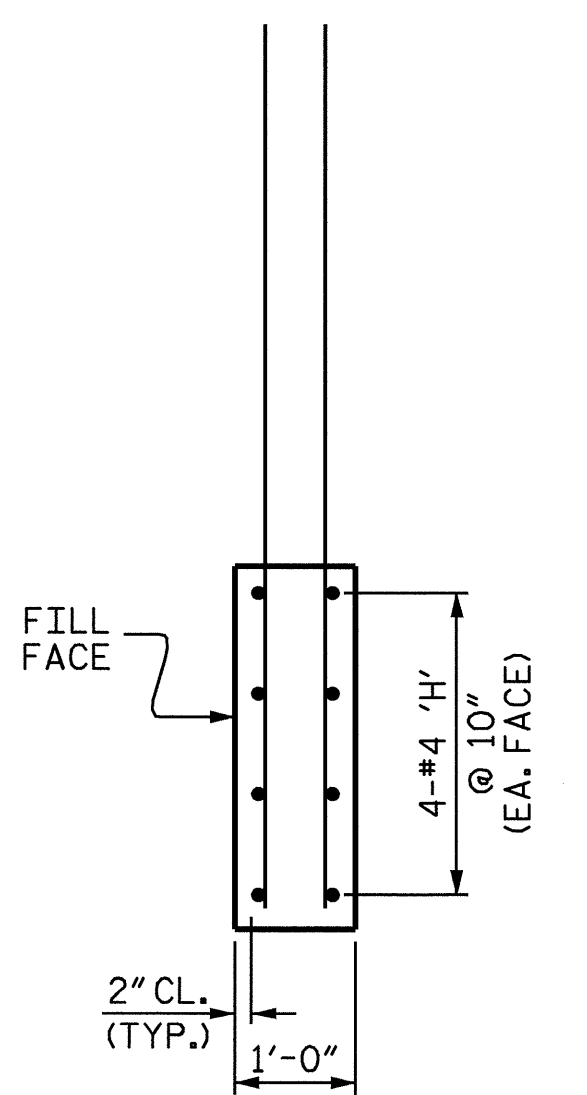


PLAN OF RIGHT WING W2
(STAGE 2)

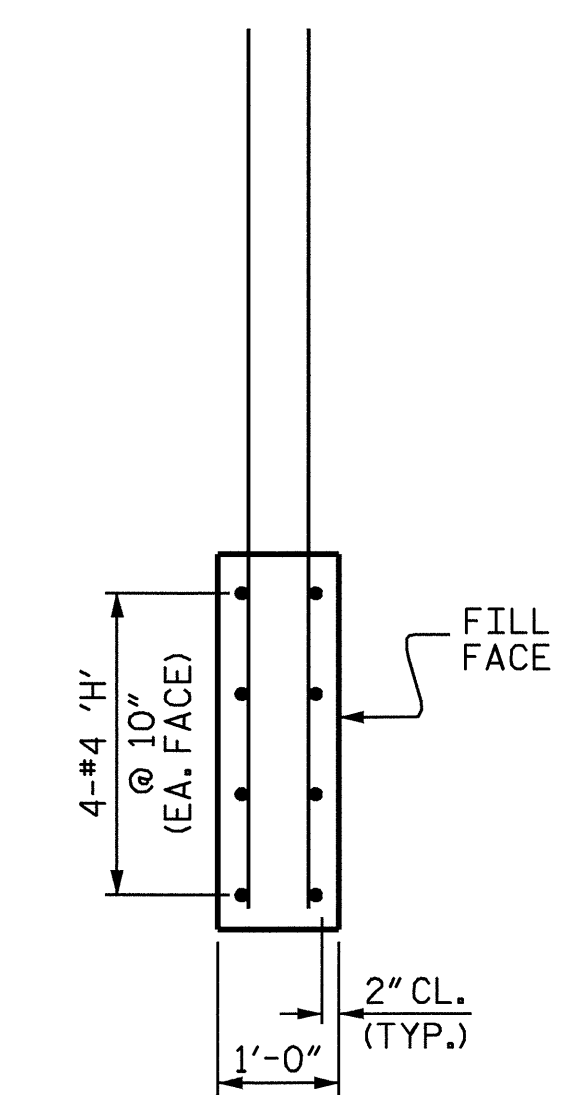
NOTE: THE UPPER PORTION OF THE WINGS SHALL BE POURED WITH THE SUPERSTRUCTURE. FOR DETAILS AND REINFORCING STEEL, SEE SUPERSTRUCTURE DETAILS.



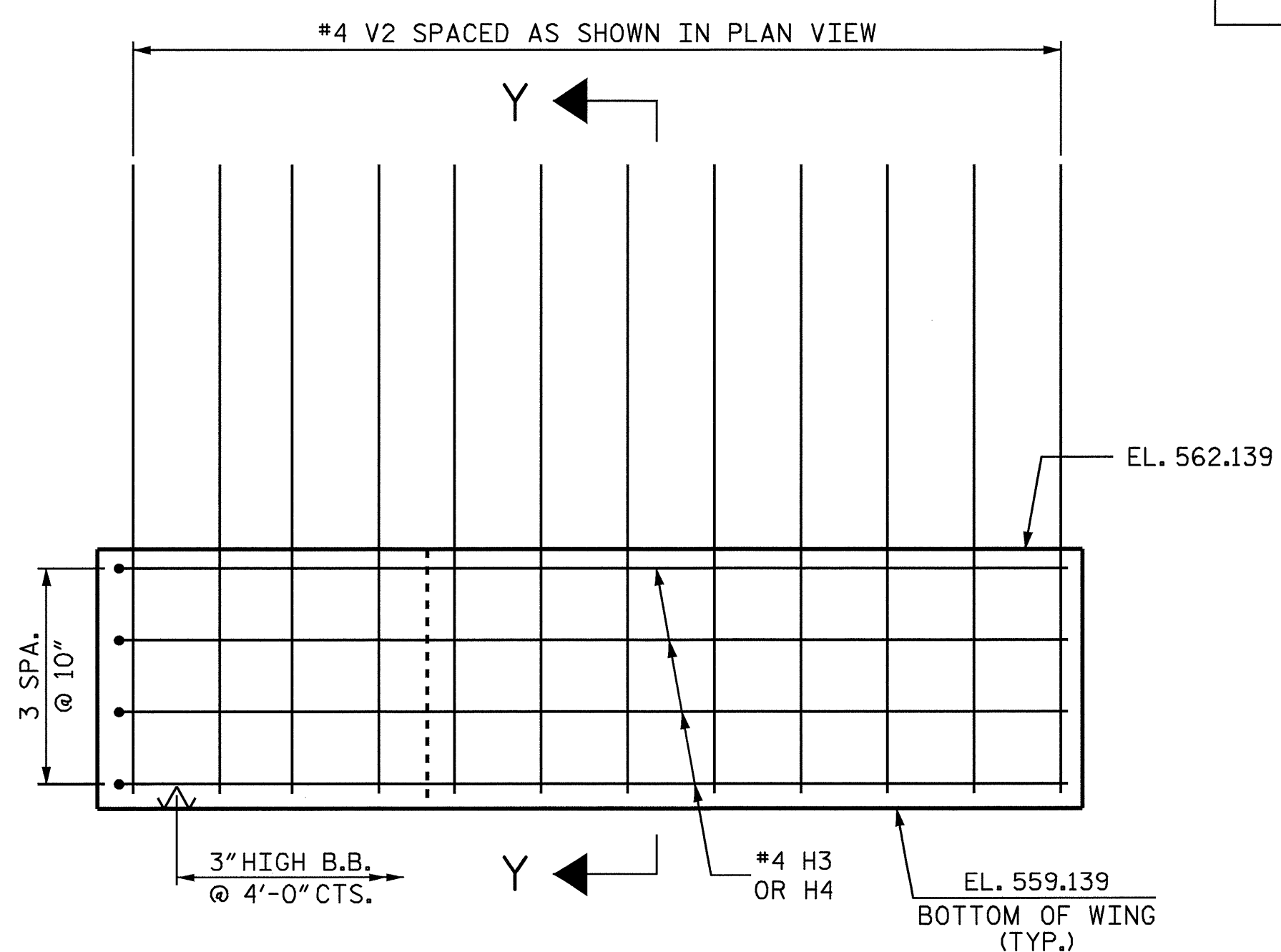
ELEVATION OF LEFT WING W1
(STAGE 1)



SECTION X-X



SECTION Y-Y



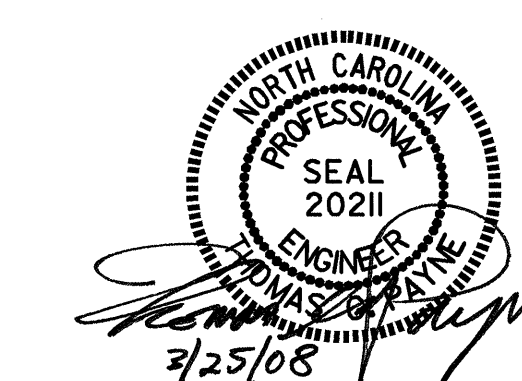
ELEVATION OF RIGHT WING W2
(STAGE 2)

PROJECT NO. B-4252
ROCKINGHAM COUNTY
STATION: 31+61.50 -L-

SHEET 3 OF 4

STATE OF NORTH CAROLINA
DEPARTMENT OF TRANSPORTATION
RALEIGH

**SUBSTRUCTURE
INTEGRAL
END BENT #2**

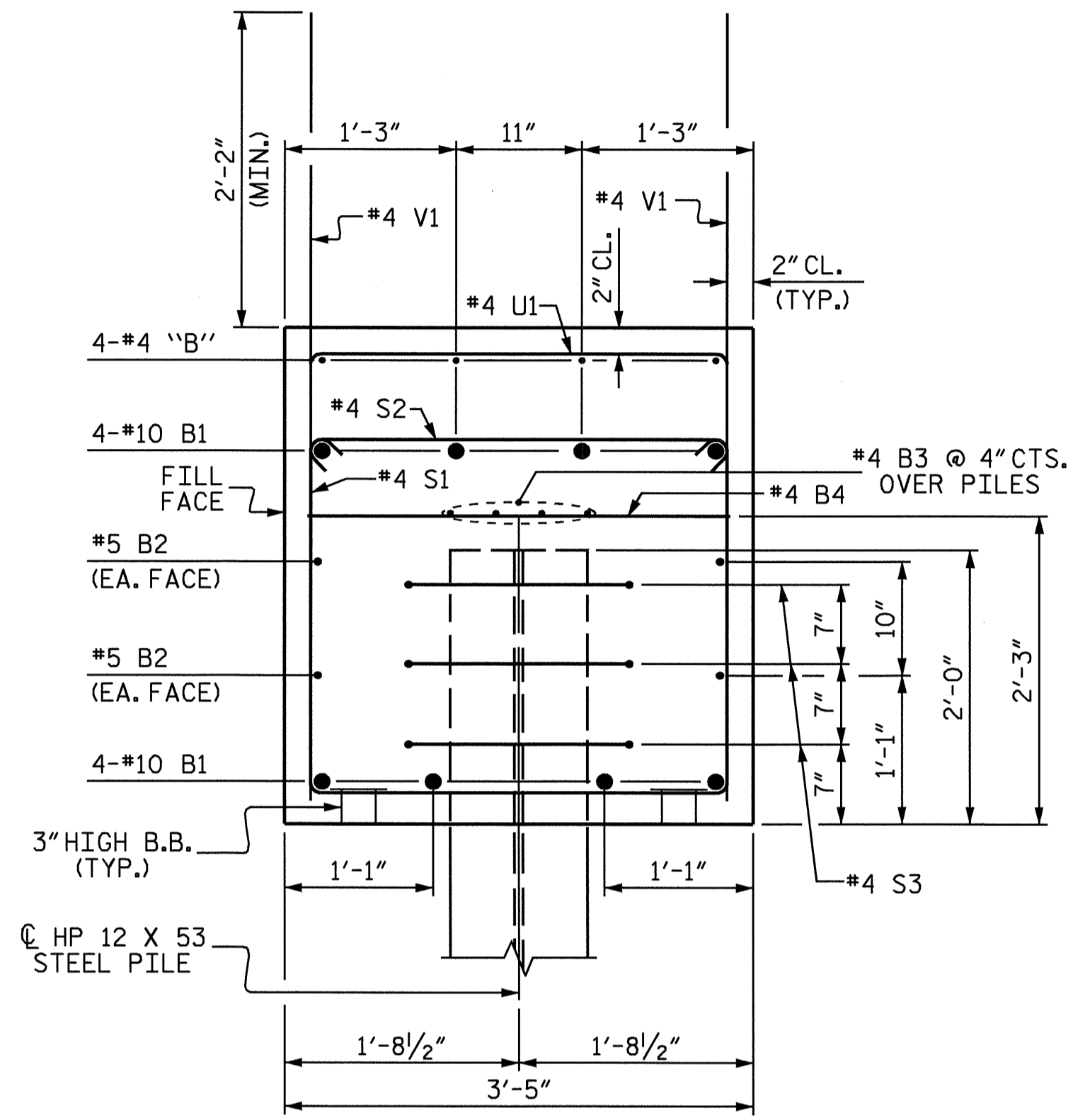


DRAWN BY : S. DOMBROWSKI DATE : 1/07
CHECKED BY : M.K. BEARD DATE : 2/08

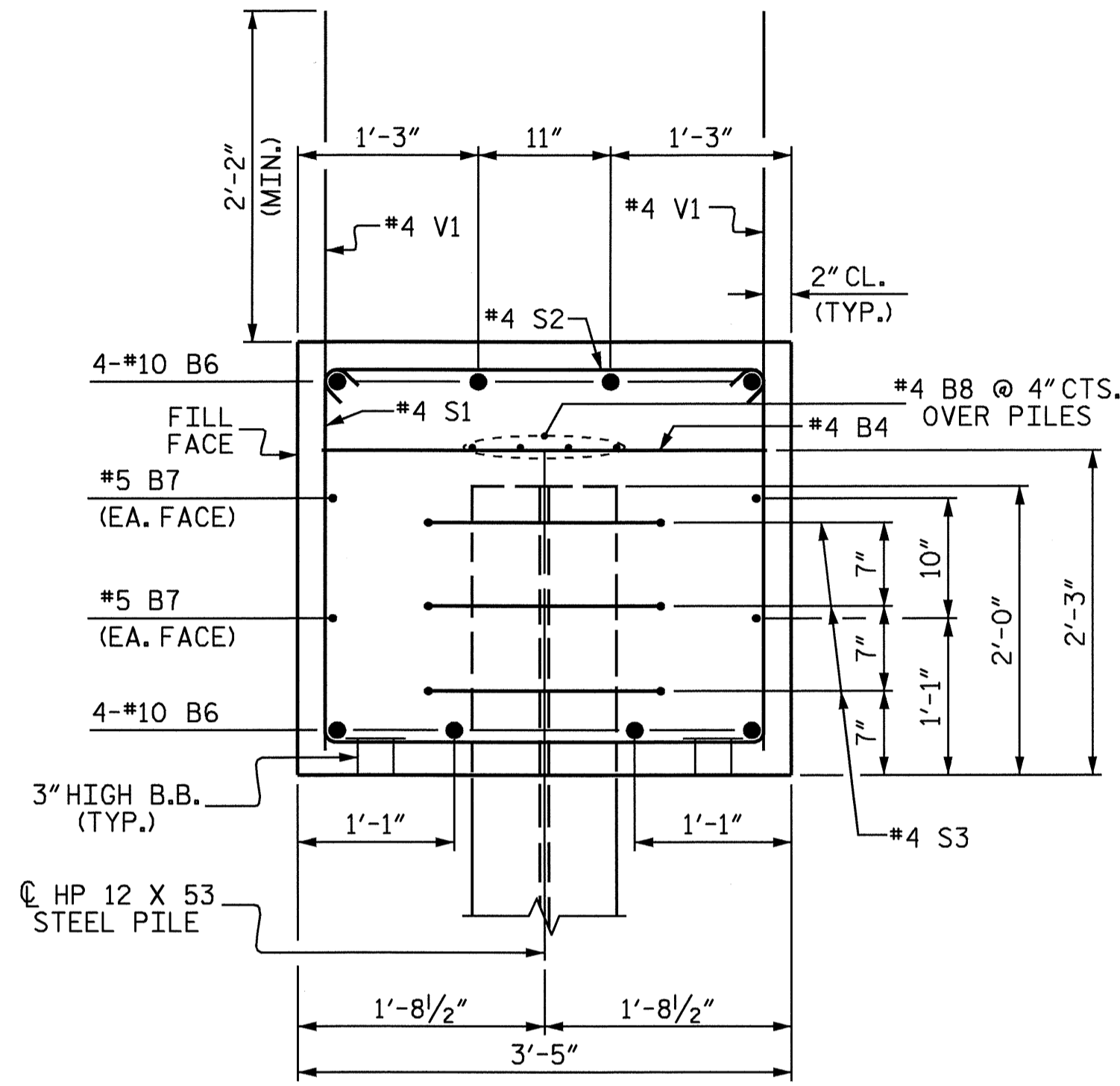
20-MAR-2008 15:34
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klayne

REVISIONS						SHEET NO.
NO.	BY:	DATE:	NO.	BY:	DATE:	S-64
1			3			TOTAL SHEETS
2			4			70

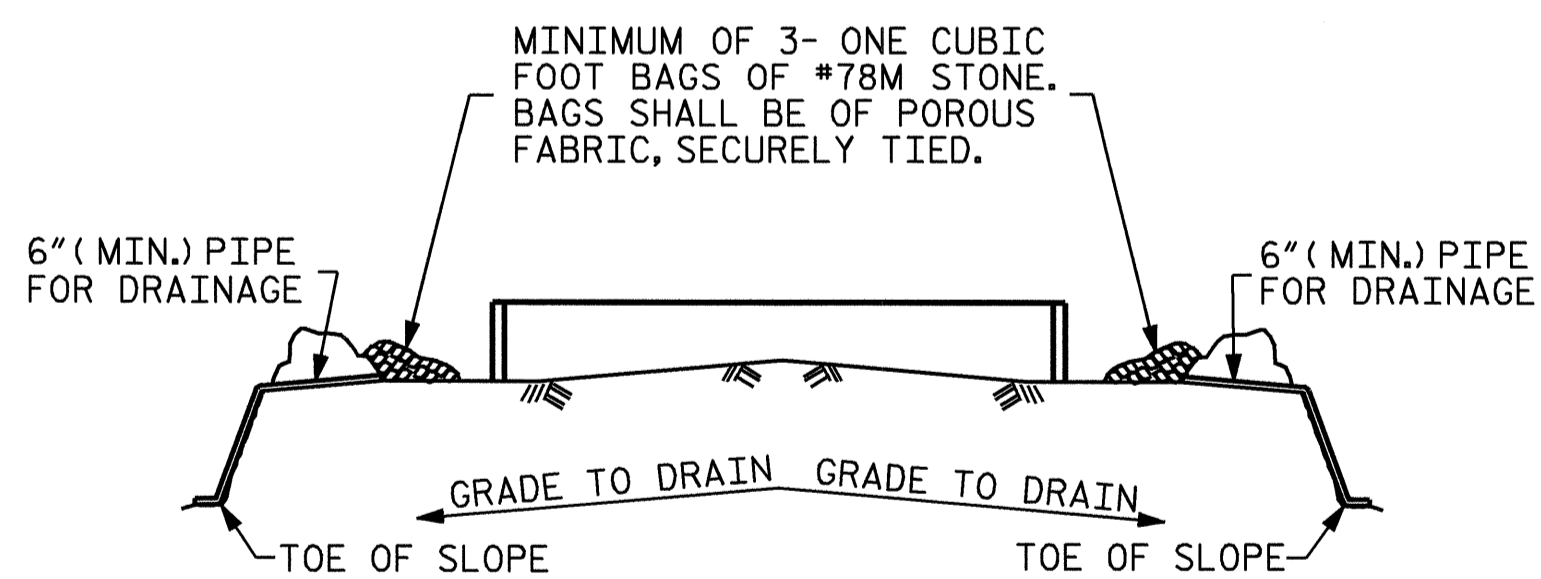
STR. #2



SECTION A-A



SECTION B-B

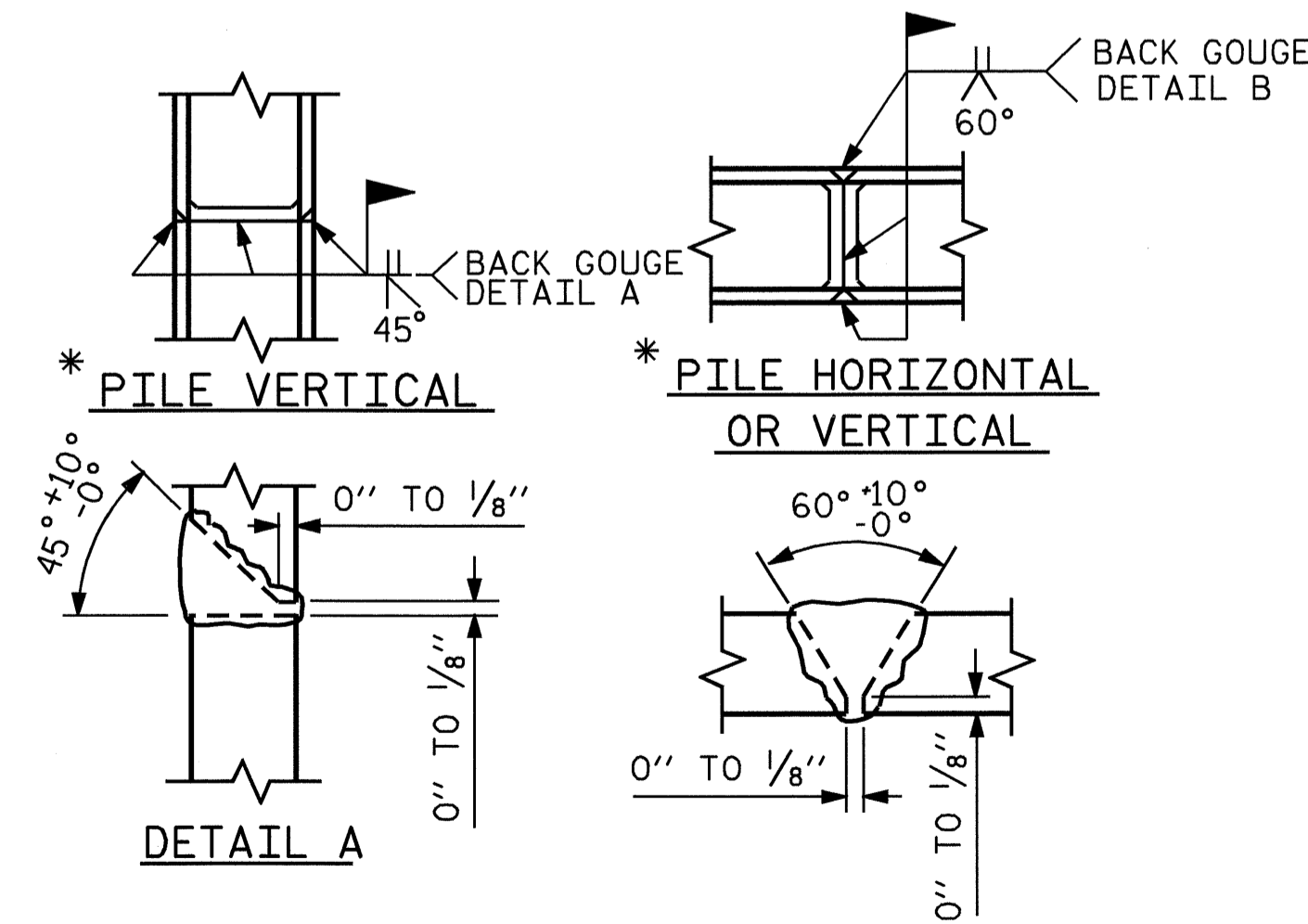


BAGGED STONE AND PIPE SHALL BE PLACED IMMEDIATELY AFTER COMPLETION OF END BENT EXCAVATION. PIPE MAY BE EITHER CONCRETE, CORRUGATED STEEL, CORRUGATED ALUMINUM ALLOY, OR CORRUGATED PLASTIC. PERFORATED PIPE WILL NOT BE ALLOWED.

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TEMPORARY DRAINAGE AT END BENT



* POSITION OF PILE DURING WELDING. DETAIL B

PILE SPLICE DETAILS

BILL OF MATERIAL

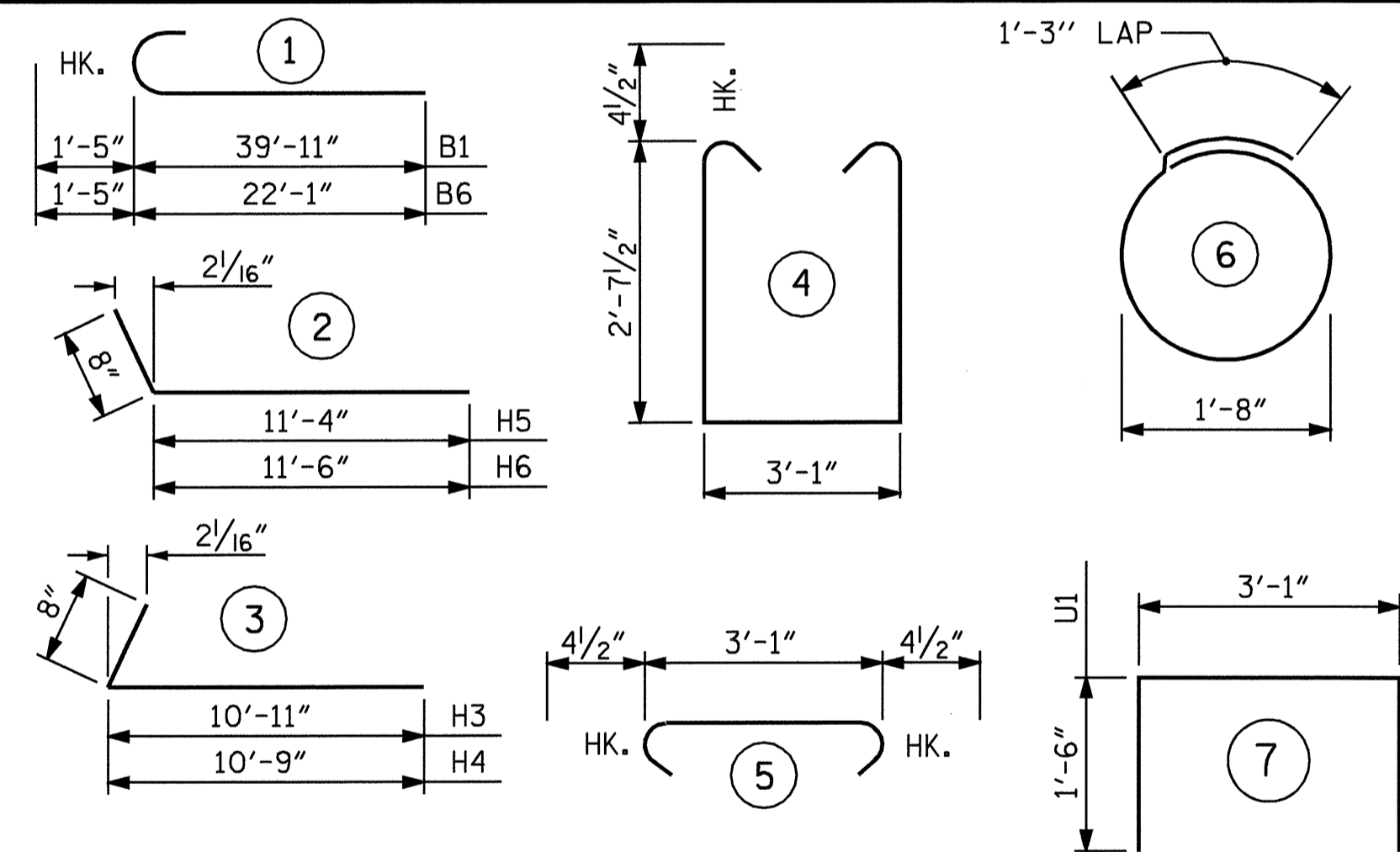
INTEGRAL END BENT #2

STAGE 1					STAGE 2								
BAR NO.	NO.	SIZE	TYPE	LENGTH	WEIGHT	BAR NO.	NO.	SIZE	TYPE	LENGTH	WEIGHT		
B1	8	10	1	41'-4"	1423	B4	6	4	STR	3'-1"	12		
B2	4	5	STR	42'-6"	177	B6	8	10	1	23'-6"	809		
B3	8	4	STR	22'-2"	118	B7	4	5	STR	22'-8"	95		
B4	10	4	STR	3'-1"	21	B8	4	4	STR	22'-8"	61		
B5	8	4	STR	15'-10"	85	B9	4	4	STR	12'-2"	33		
H5	4	4	2	12'-0"	32	H3	4	4	3	11'-7"	31		
H6	4	4	2	12'-2"	33	H4	4	4	3	11'-5"	31		
S1	40	4	4	9'-1"	243	S1	23	4	4	9'-1"	140		
S2	40	4	5	3'-10"	102	S2	23	4	5	3'-10"	59		
S3	27	4	6	6'-6"	117	S3	15	4	6	6'-6"	65		
U1	20	4	7	6'-1"	81	U1	8	4	7	6'-1"	33		
V1	62	4	STR	5'-9"	238	V1	34	4	STR	5'-9"	131		
V2	22	4	STR	7'-3"	107	V2	20	4	STR	7'-3"	97		
REINFORCING STEEL					LBS	2777	REINFORCING STEEL					LBS	1597
CLASS 'A' CONCRETE					17.7 C.Y.		CLASS 'A' CONCRETE					10.3 C.Y.	
HP 12X53 STEEL PILES					135 LIN.FT.		HP 12X53 STEEL PILES					75 LIN.FT.	
NO. 9							NO. 5						
PILE EXCAVATION IN SOIL					107 LIN.FT.		PILE EXCAVATION IN SOIL					60 LIN.FT.	
PILE EXCAV. NOT IN SOIL					28 LIN.FT.		PILE EXCAV. NOT IN SOIL					16 LIN.FT.	

TOTAL BILL OF MATERIAL

REINFORCING STEEL	LBS	4374	HP 12X53 STEEL PILES	NO. 14	210 LIN.FT.
CLASS 'A' CONCRETE	28.0 C.Y.		PILE EXCAVATION IN SOIL	167 LIN.FT.	
			PILE EXCAV. NOT IN SOIL	44 LIN.FT.	

BAR TYPES



ALL BAR DIMENSIONS ARE OUT TO OUT.

PROJECT NO. B-4252
 ROCKINGHAM COUNTY
 STATION: 31+61.50 -L-

SHEET 4 OF 4

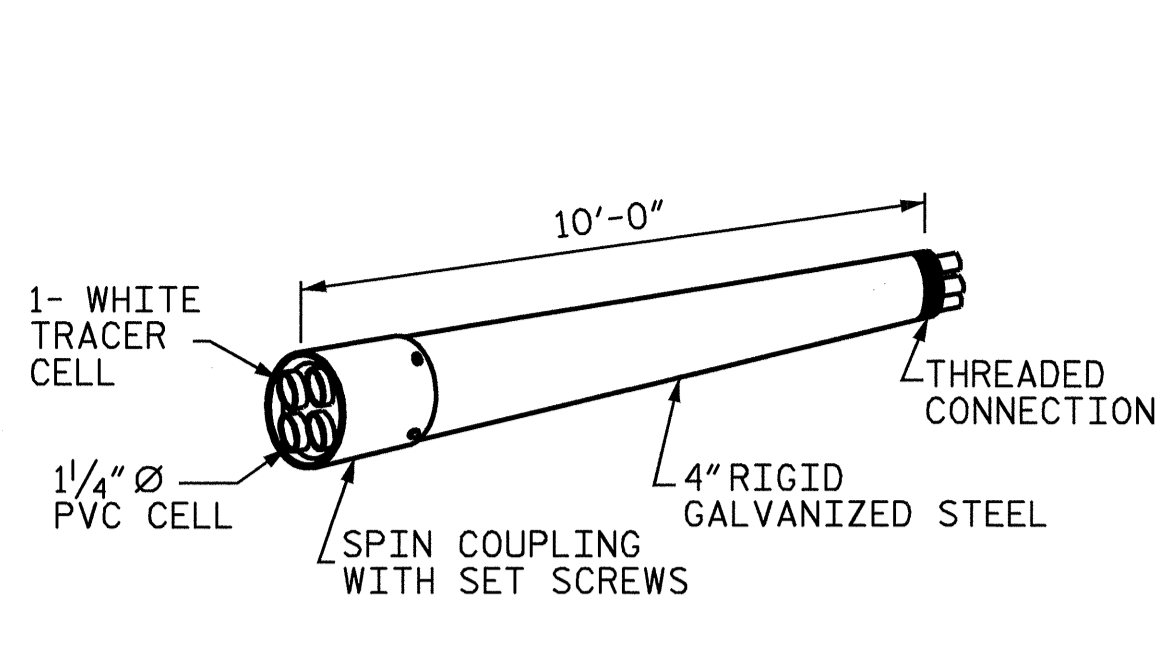
STATE OF NORTH CAROLINA
 DEPARTMENT OF TRANSPORTATION
 RALEIGH

SUBSTRUCTURE
 INTEGRAL END BENT #2

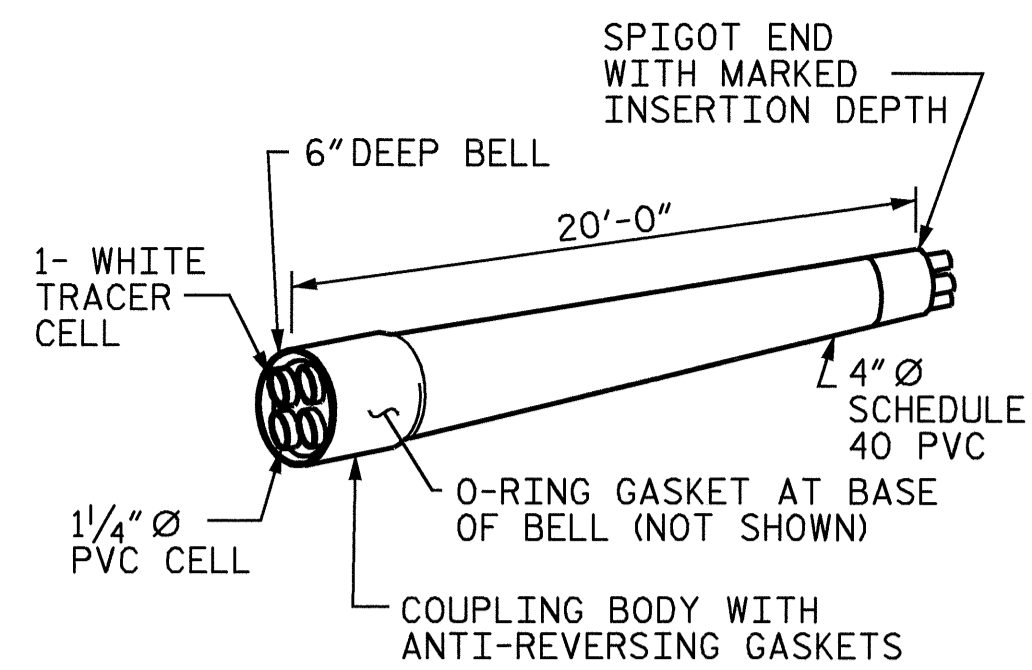
REVISIONS						SHEET NO.	
NO.	BY:	DATE:	NO.	BY:	DATE:	S-65	
1			3			TOTAL SHEETS 70	
2			4				



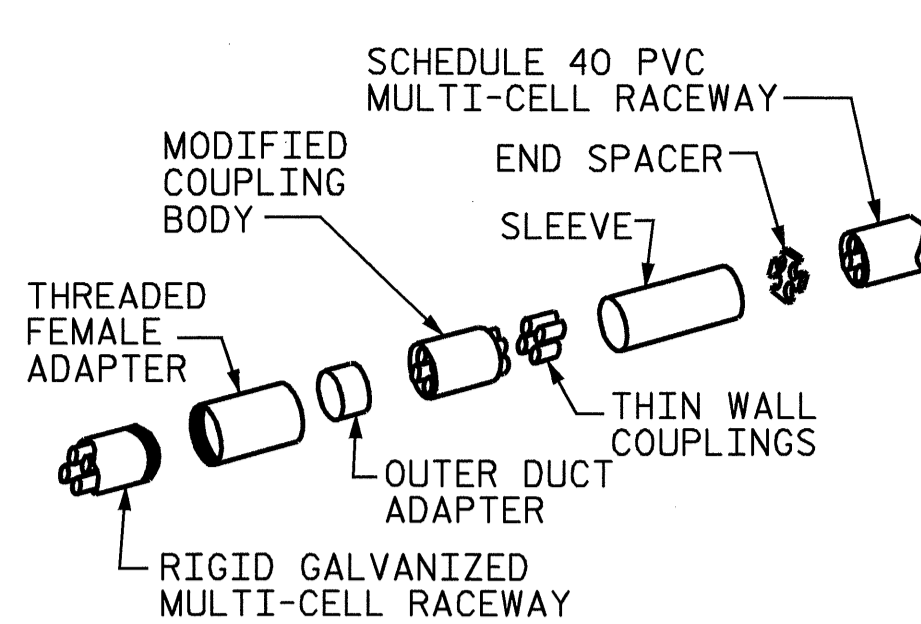
DRAWN BY: S. DOMBROWSKI DATE: 1/07
 CHECKED BY: M.K. BEARD DATE: 2/08



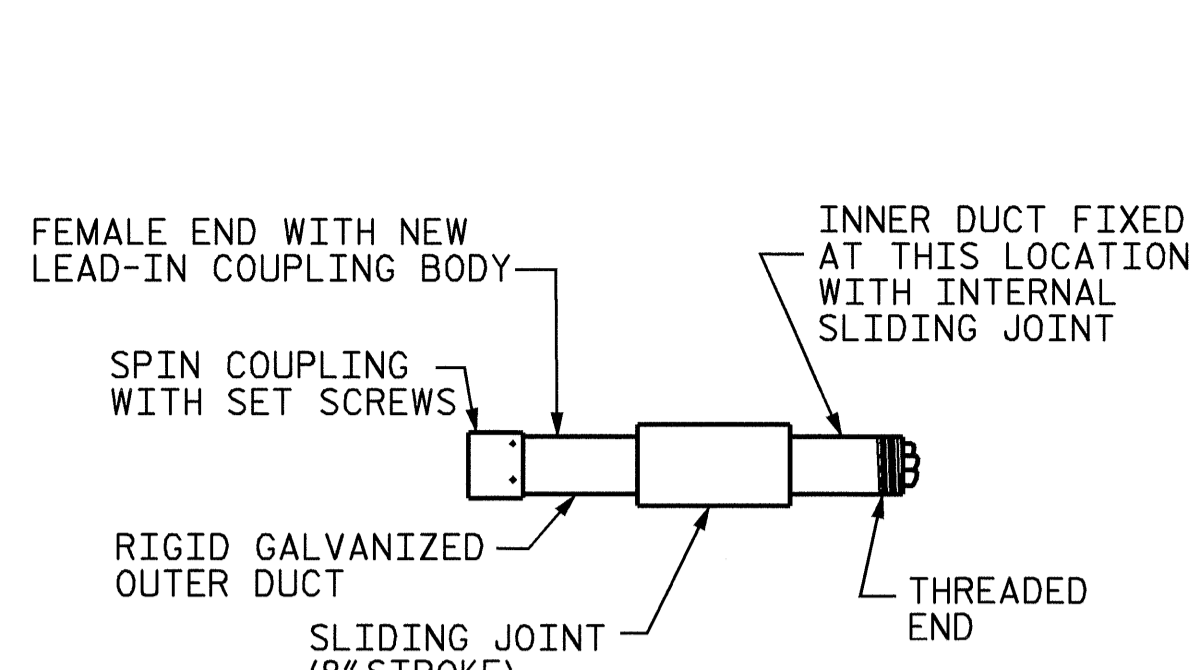
**RIGID GALVANIZED (RGC)
MULTI-CELL RACEWAY**



**SCHEDULE 40 PVC
MULTI-CELL RACEWAY**



TRANSITION ADAPTER



EXPANSION JOINT FITTING

NOTES

THE CONTRACTOR SHALL BE RESPONSIBLE FOR DETERMINING THE TOTAL QUANTITY OF CONDUIT NEEDED TO COMPLETE THE WORK AND THAT THE CONDUIT(S) ARE PLACED AT THE NOTED DIMENSION AND ABOVE THE BOTTOM OF THE GIRDER.

THE INSTALLATION OF THE CONDUIT SYSTEM SHALL BE PAID FOR AS LUMP SUM. THE PRICE SHALL INCLUDE ALL CONDUIT, HANGERS, STABILIZERS, EXPANSION JOINTS, CONCRETE INSERTS, PVC SLEEVES AND ALL NECESSARY HARDWARE TO COMPLETE THE WORK.

THE CONTRACTOR SHALL FIELD VERIFY THAT THE CONDUIT SYSTEM IS NOT IN CONFLICT WITH THE GUARDRAIL POSTS.

SEE DETAIL "C" FOR HANGER ASSEMBLY INSTALLATION.

INSTALL SLEEVES PARALLEL TO GIRDERS. SEE DETAIL "B" FOR SLEEVE INSTALLATION.

PROVIDE TRANSITION ADAPTOR (AND EXPANSION JOINT) FOR CONDUIT AT END BENT 1 AND END BENT 2.

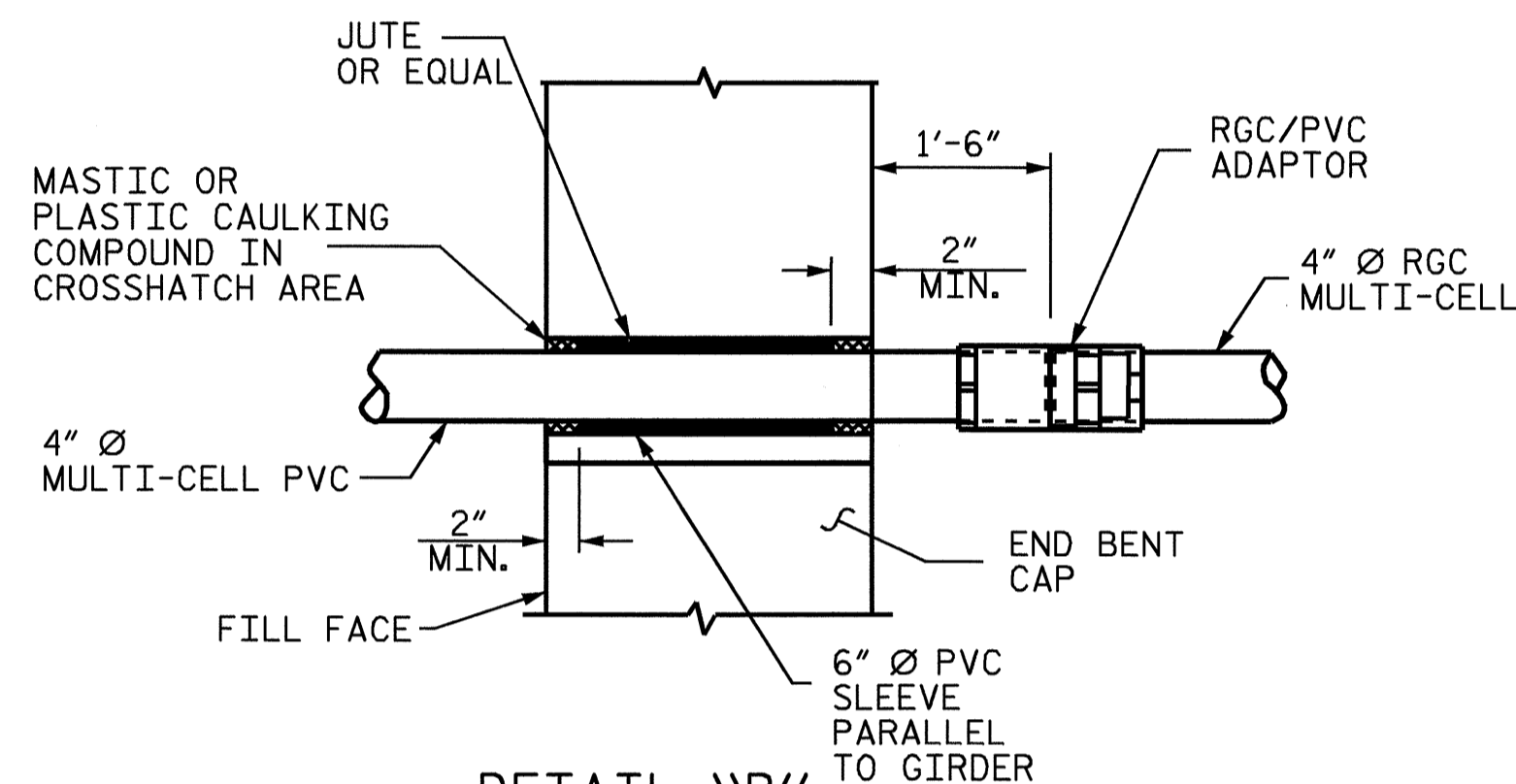
INSTALL STABILIZER AT OR NEAR MIDSPAN OF SPAN A. STABILIZER CAN NOT BE USED INSTEAD OF A HANGER ASSEMBLY.

THE CONCRETE SCREW INSERT SHALL HAVE A ROD SIZE OF 5/8" AND A PULL FORCE OF 1260 lbs.

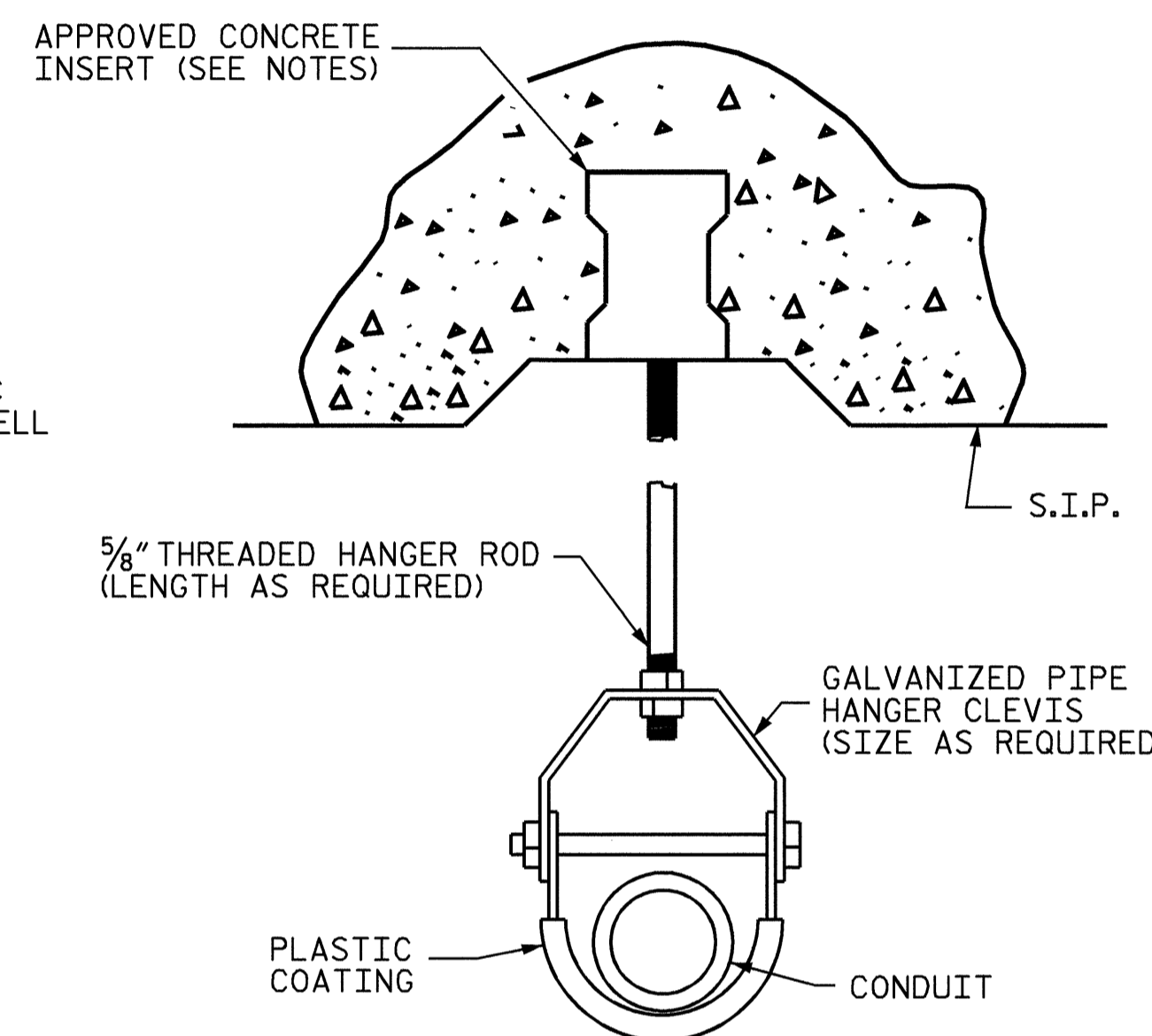
FOR ELECTRICAL CONDUIT SYSTEM, SEE SPECIAL PROVISIONS.

DETAIL "D"

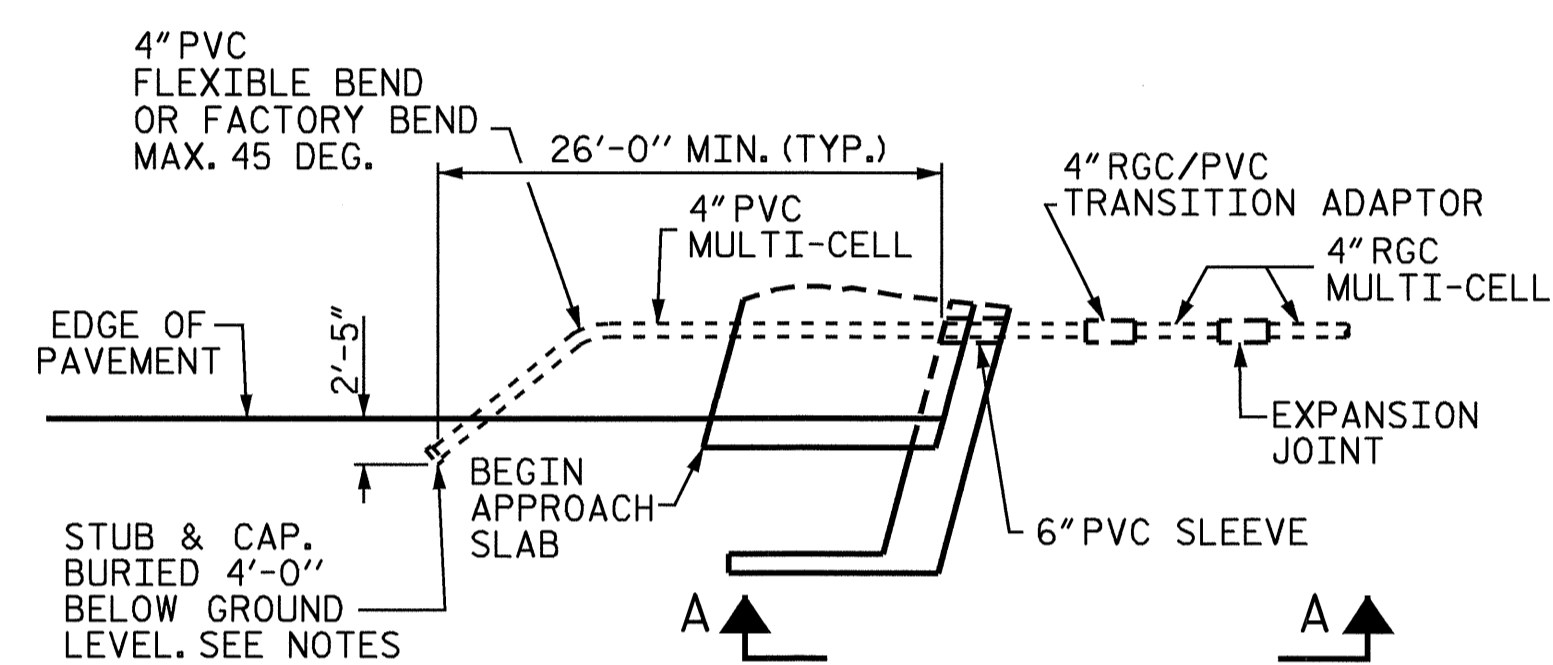
4" MULTI-CELL COMPONENTS



DETAIL "B"
PVC SLEEVE INSTALLATION & RGC/PVC ADAPTOR AT END BENT.

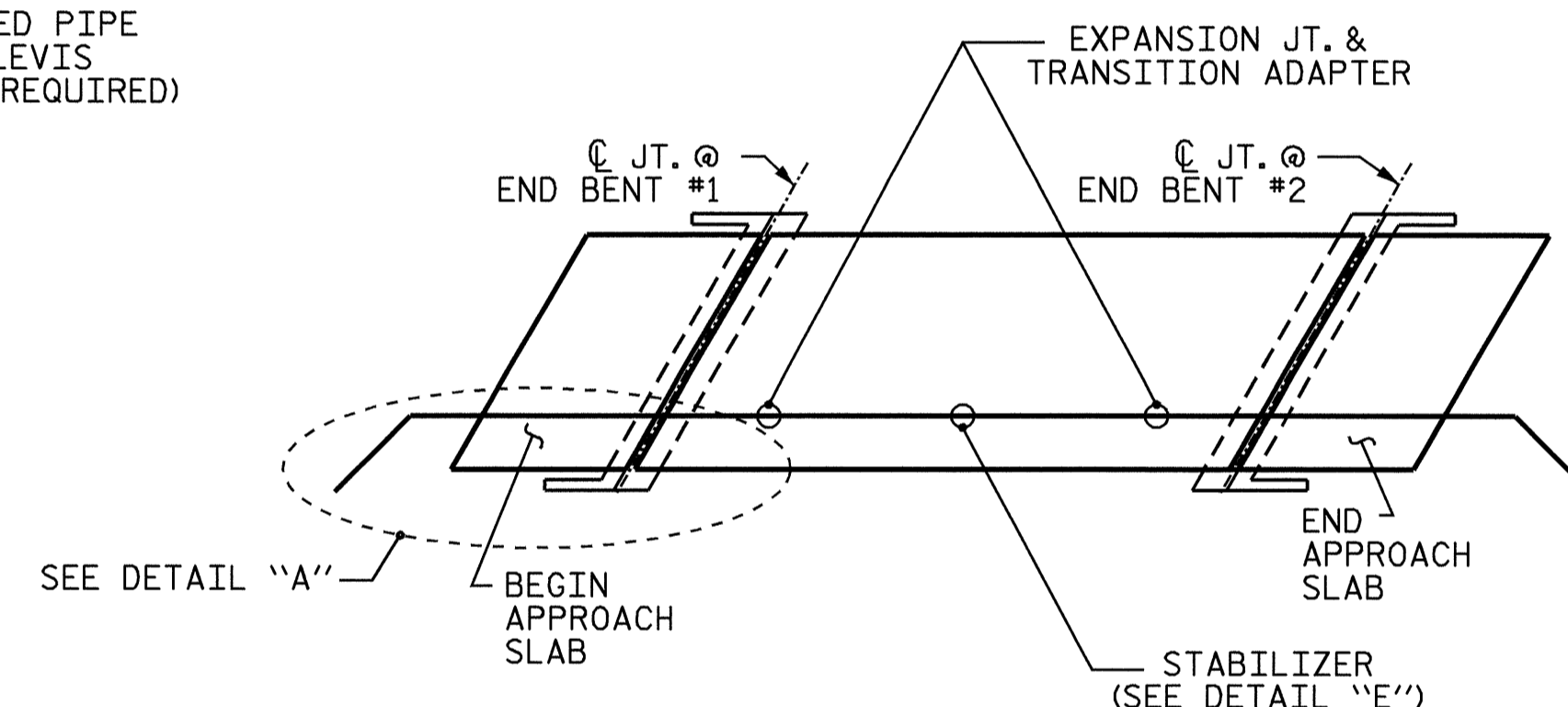


DETAIL "C"
HANGER ASSEMBLY

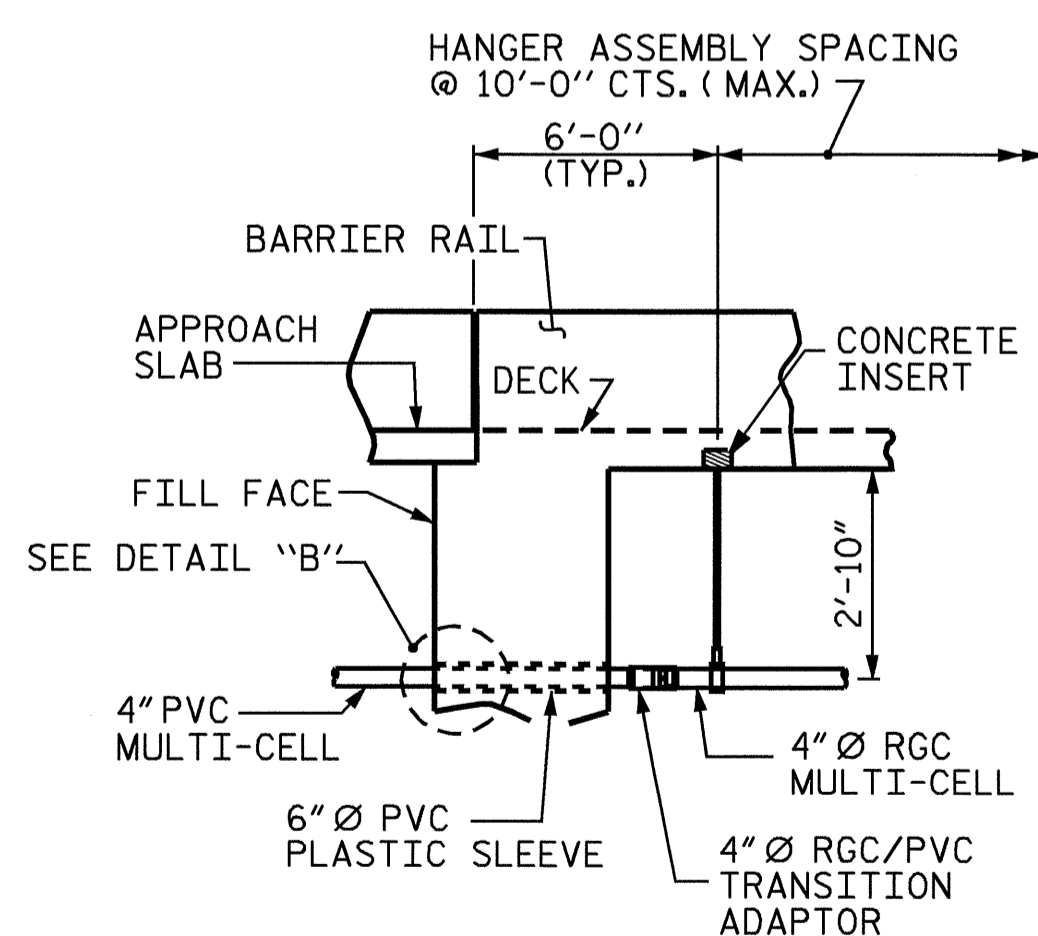


DETAIL "A"

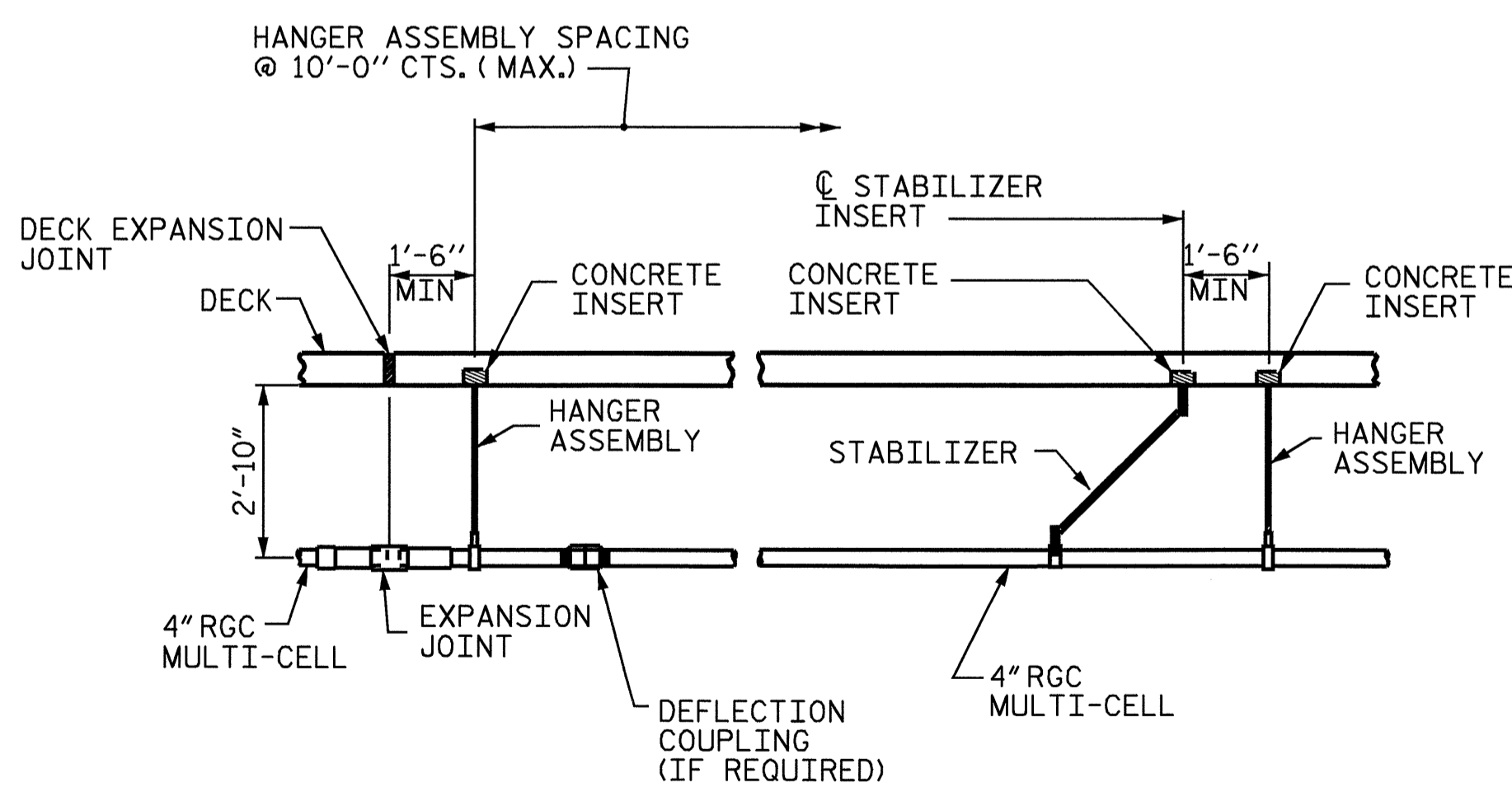
TERMINATION OF CONDUIT AT WING WALL



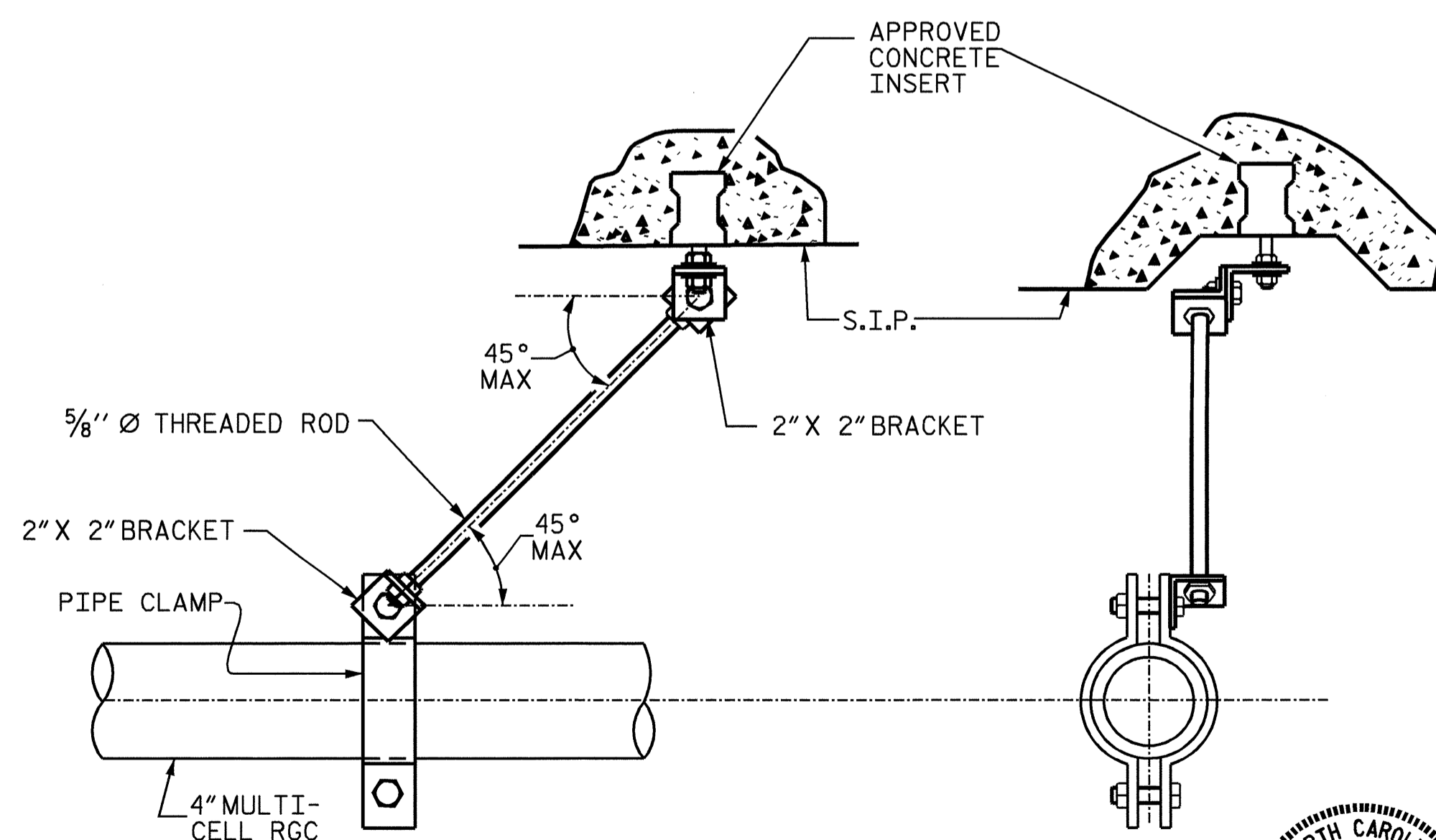
CONDUIT LAYOUT



VIEW A-A



VIEW B-B
STEEL OR CONCRETE GIRDERS



DETAIL "E"
STABILIZER

ELECTRIC CONDUIT DETAILS

ASSEMBLED BY : S. DOMBROWSKI	DATE : 12/07
CHECKED BY : M.K. BEARD	DATE : 1/08
DRAWN BY : RWW 2-4-03	REV. 5/1/06 TLA/GM
CHECKED BY : DBM 2-4-03	

02-JUN-2008 14:59
R:\Str\structures\plans\str2\B-4252.sd_UT_02.dgn
tpoyme

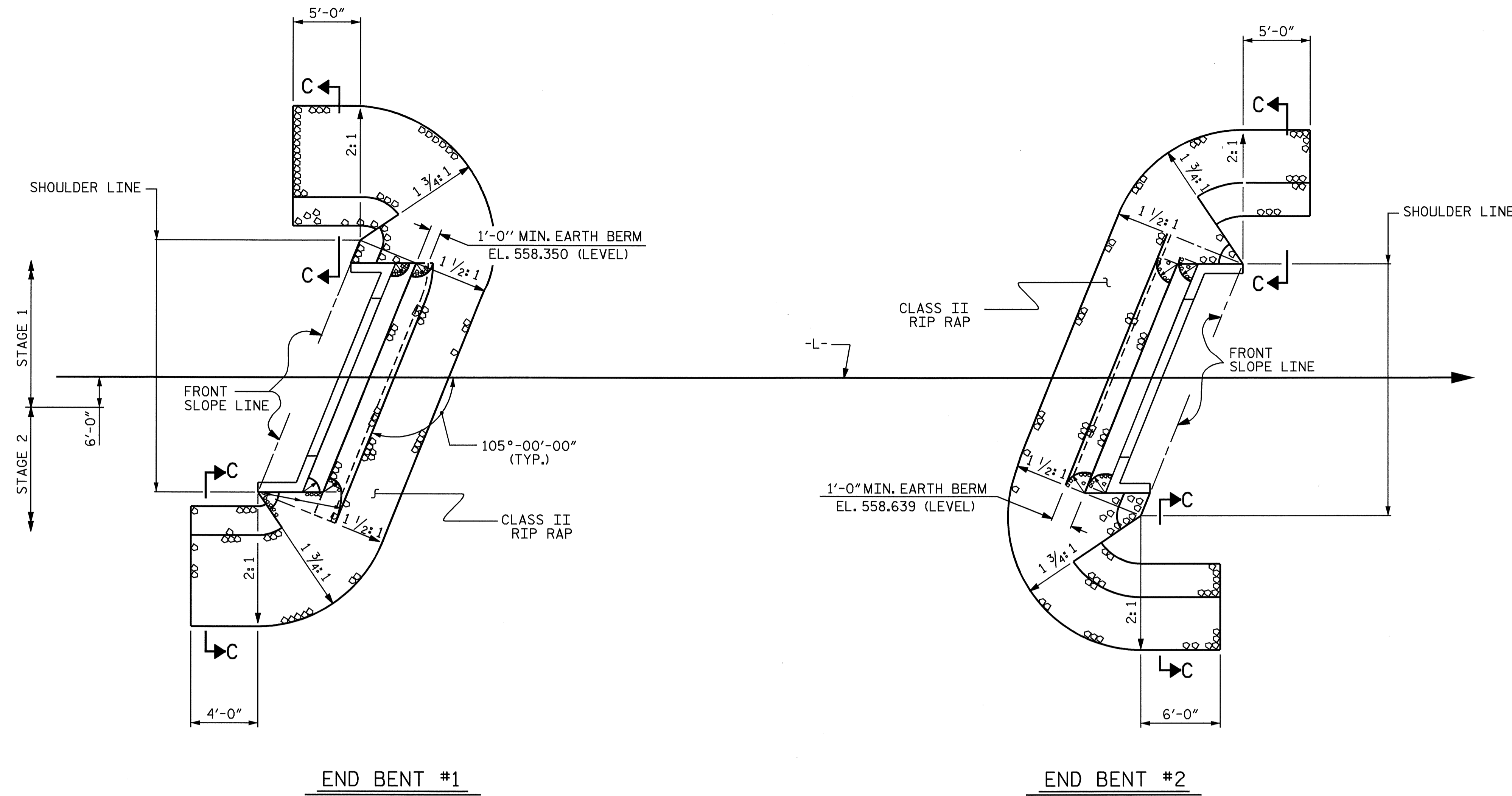
STR. #2

PROJECT NO. B-4252
ROCKINGHAM COUNTY
STATION: 31+61.50 -L-

STATE OF NORTH CAROLINA DEPARTMENT OF TRANSPORTATION RALEIGH					
STANDARD ELECTRICAL CONDUIT SYSTEM DETAILS					
REVISIONS					
NO.	BY:	DATE:	NO.	BY:	DATE:
1			3		
2			4		
					SHEET NO. S-66
					TOTAL SHEETS 70

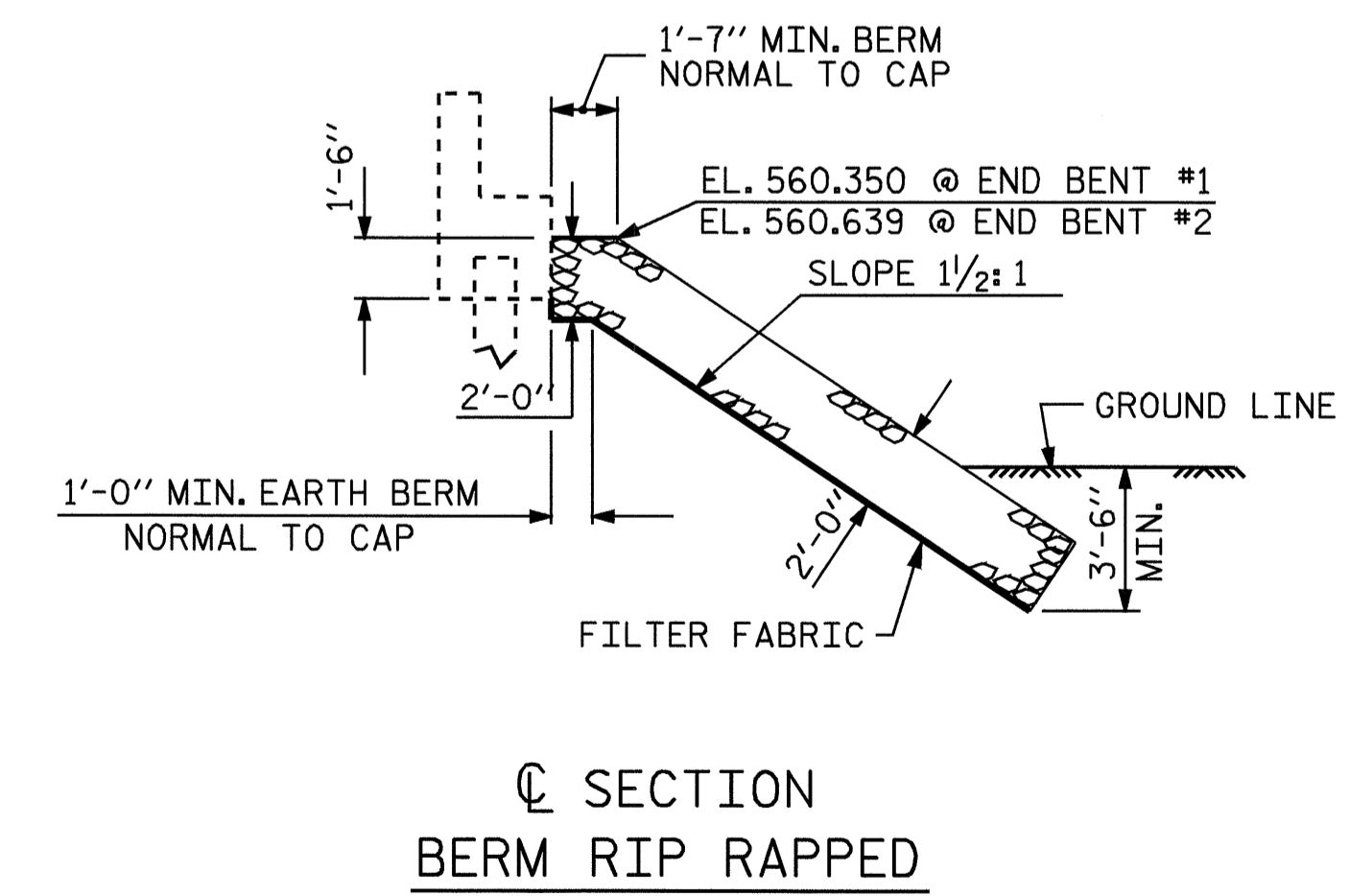


(SHT 3) STD. NO. ECS1

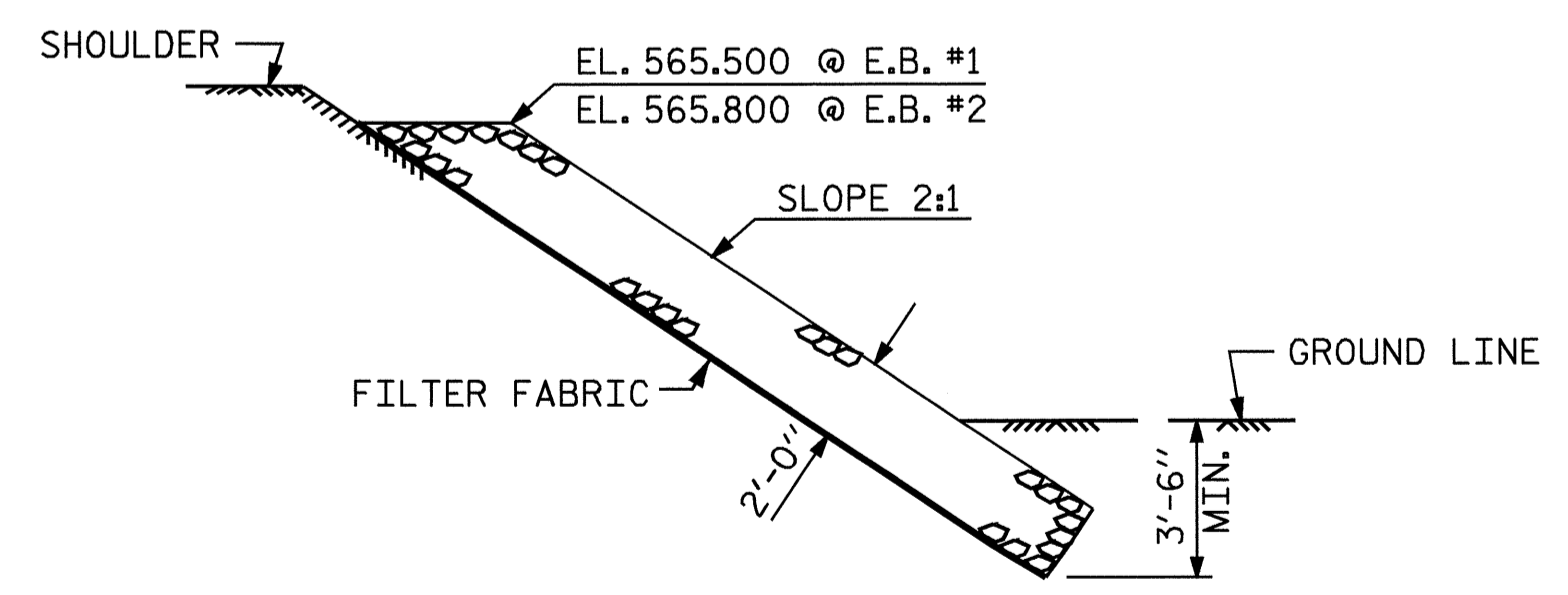


ESTIMATED QUANTITIES		
BRIDGE @ STA. 31+61.50 -L-	RIP RAP CLASS II (2'-0" THICK)	FILTER FABRIC FOR DRAINAGE
	TONS	SQUARE YARDS
END BENT #1	320	355
END BENT #2	260	285

PLAN OF RIP RAP



SECTION C-C
BERM RIP RAPPED



SECTION L-L

PROJECT NO. B-4252
ROCKINGHAM COUNTY
 STATION: 31+61.50 -L-

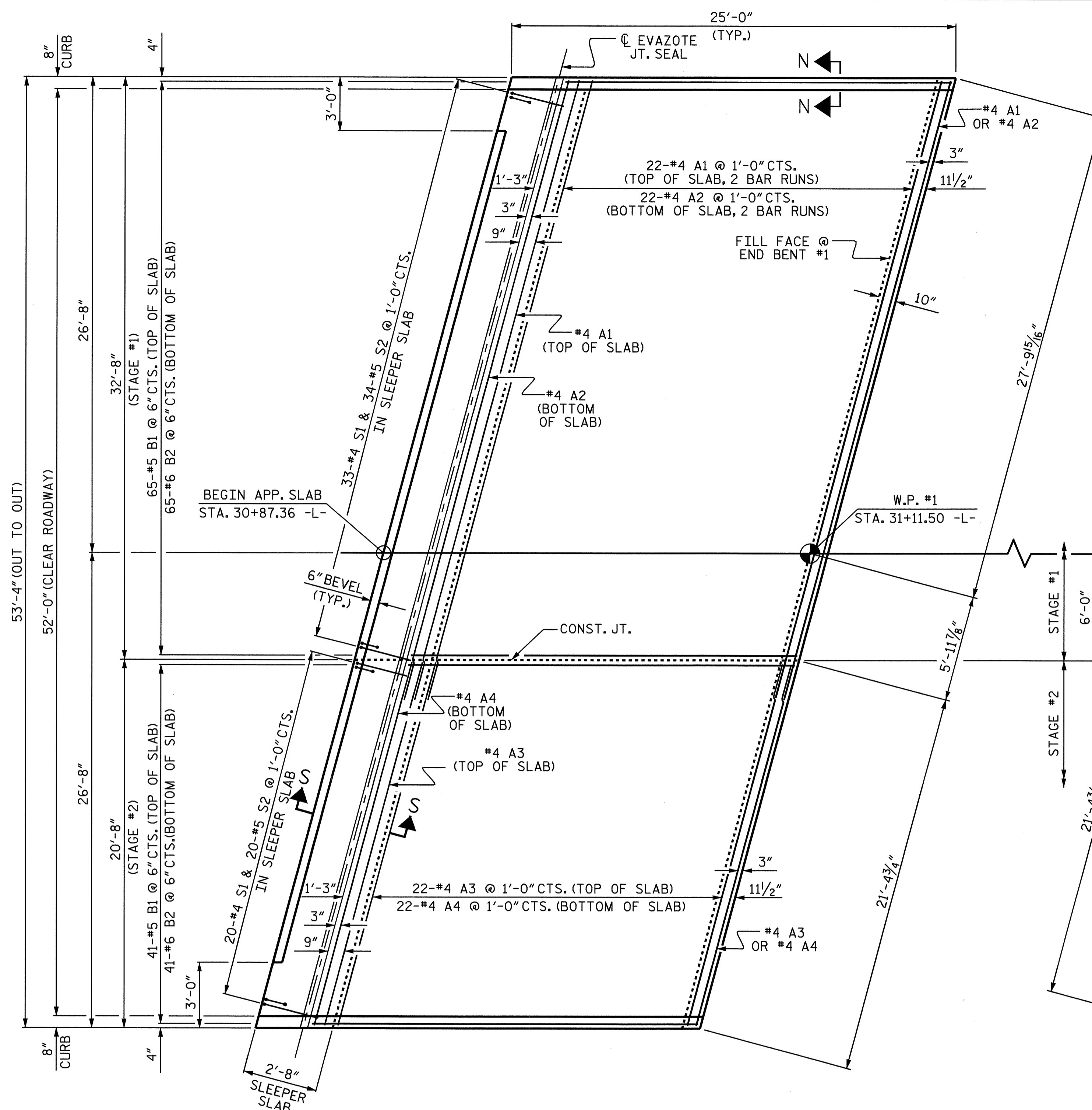
STATE OF NORTH CAROLINA
 DEPARTMENT OF TRANSPORTATION
 RALEIGH

STANDARD
 RIP RAP DETAILS

REVISIONS						SHEET NO.
NO.	BY:	DATE:	NO.	BY:	DATE:	S-67
1			3			TOTAL SHEETS
2			4			70

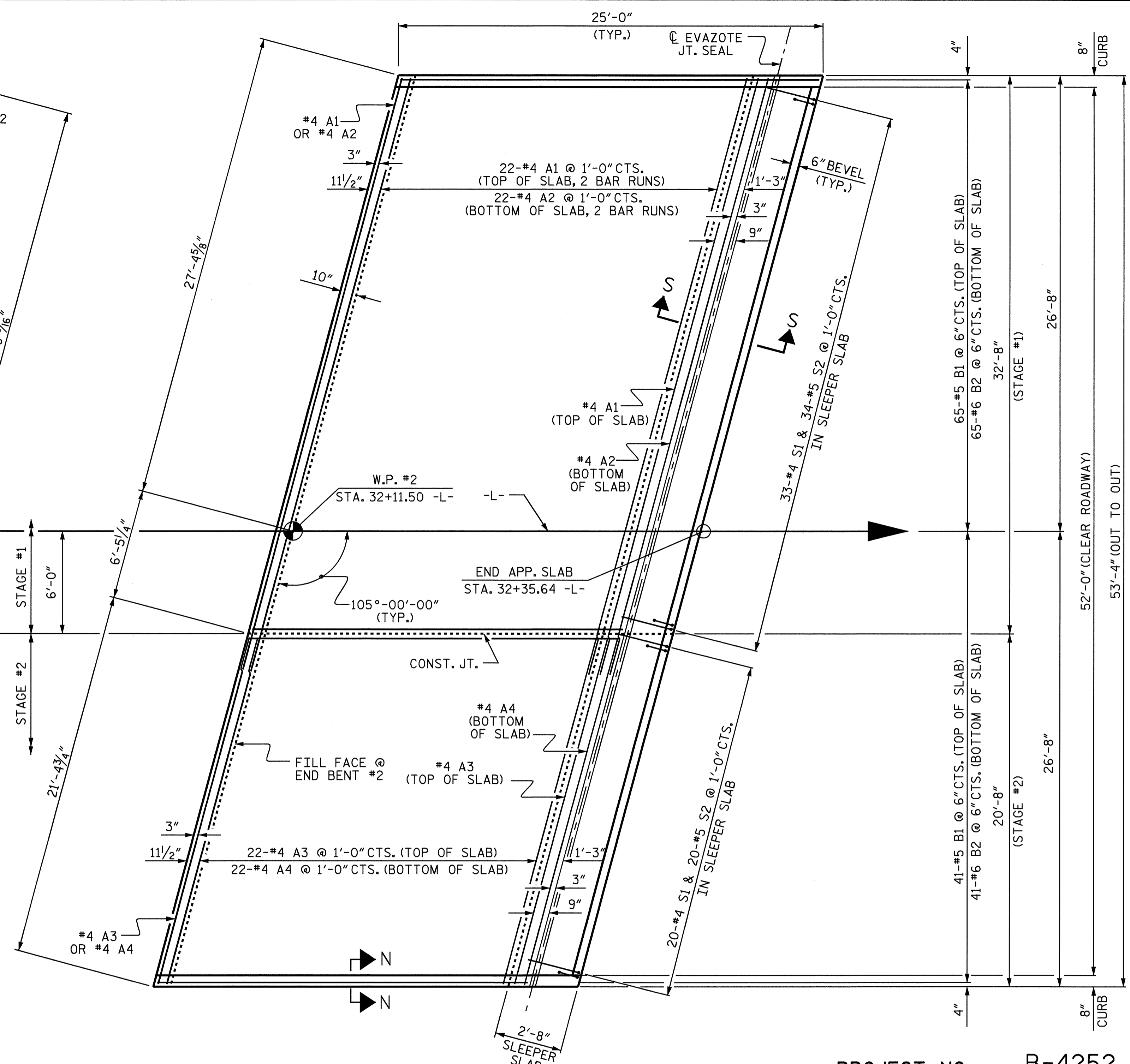


ASSEMBLED BY : R. G. EMERSON DATE : 01/07
 CHECKED BY : M. K. BEARD DATE : 07/07
 DRAWN BY : REK 1/84 REV. 8/16/99 RWW/LES
 CHECKED BY : RDU 1/84 REV. 10/17/00 RWW/LES
 REV. 5/1/06 TLA/GM

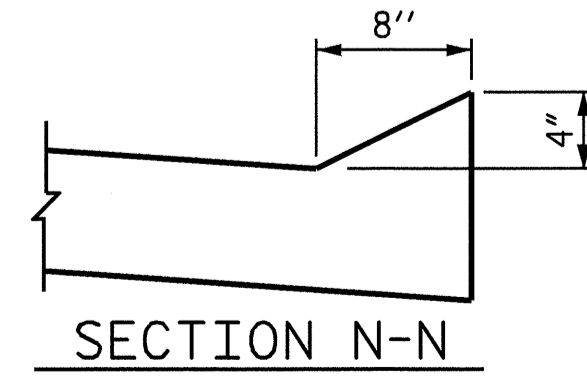


PLAN @ END BENT #1

#4 "A" BARS IN SLEEPER SLAB NOT SHOWN FOR CLARITY.



PLAN @ END BENT #2



EXTEND #4 A1 TOP BARS 2'-2" BEYOND CONST. JT. FOR 2'-0" SPLICE.
EXTEND #4 A2 BOTTOM BARS 1'-11" FOR 1'-9" SPLICE.

PROJECT NO. B-4252
ROCKINGHAM COUNTY
STATION: 31+61.50 -L-

SHEET 1 OF 3

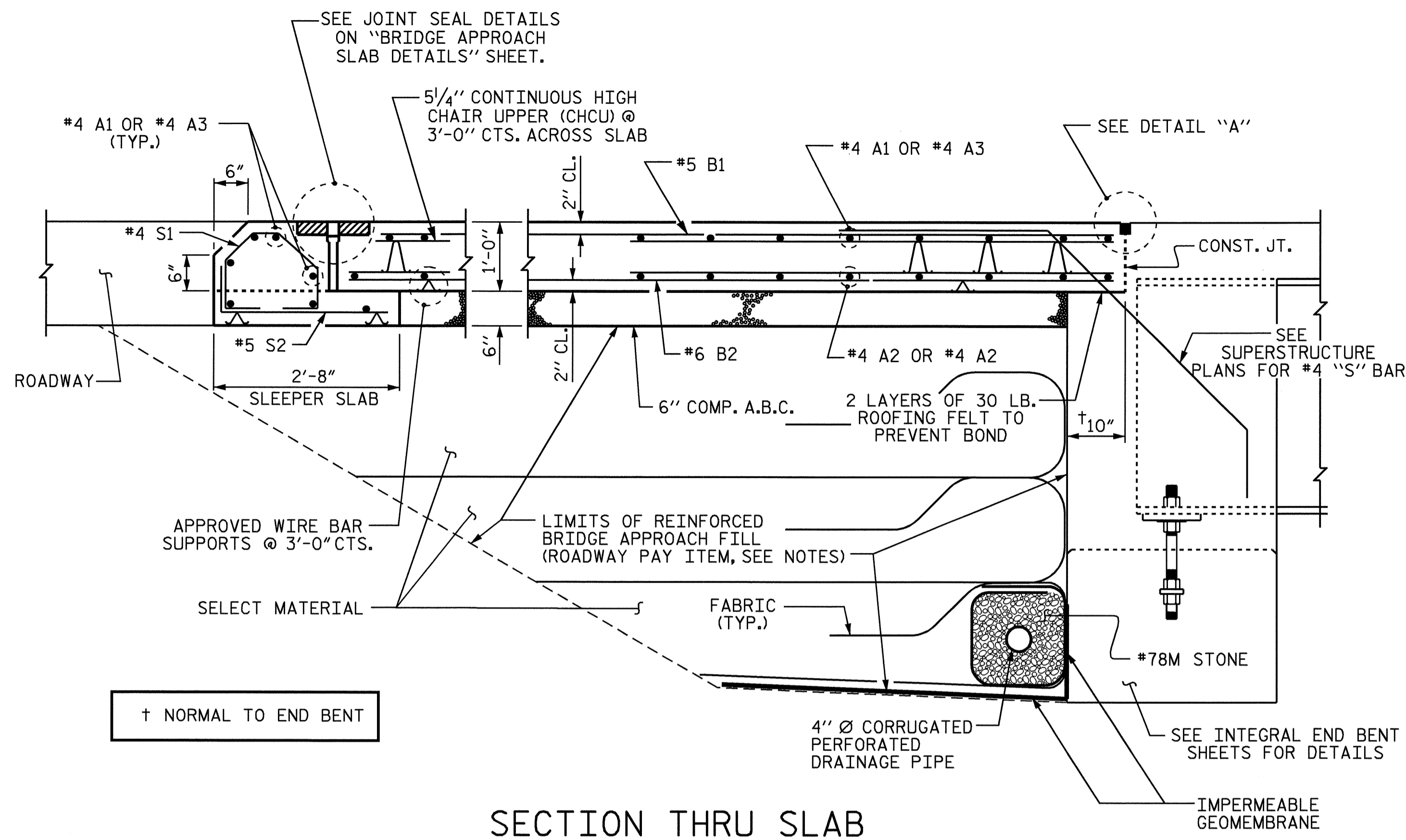
STATE OF NORTH CAROLINA
DEPARTMENT OF TRANSPORTATION
RALEIGH
STANDARD
BRIDGE APPROACH
SLAB FOR INTEGRAL
ABUTMENT
STAGE #1 & STAGE #2



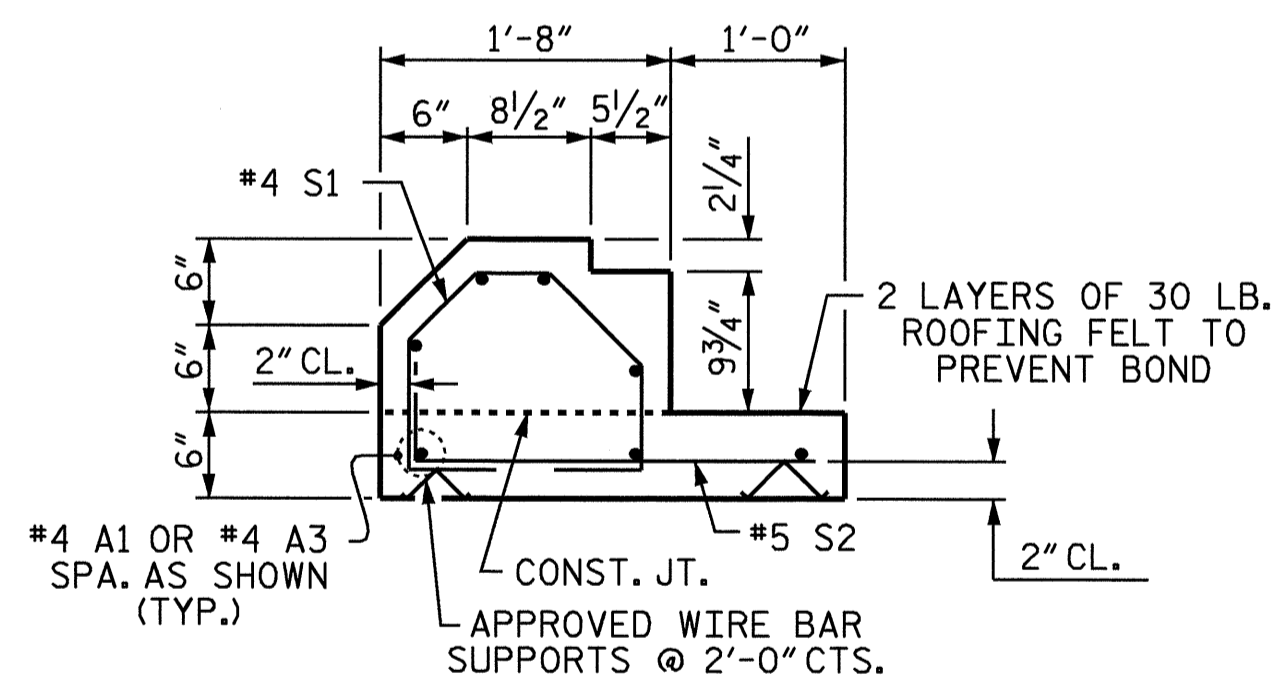
DRAWN BY: R. G. EMERSON DATE: 03/07
CHECKED BY: J. P. ADAMS DATE: 03/07

REVISIONS						SHEET NO.	
NO.	BY:	DATE:	NO.	BY:	DATE:	S-68	
1			3			TOTAL SHEETS	
2			4			70	

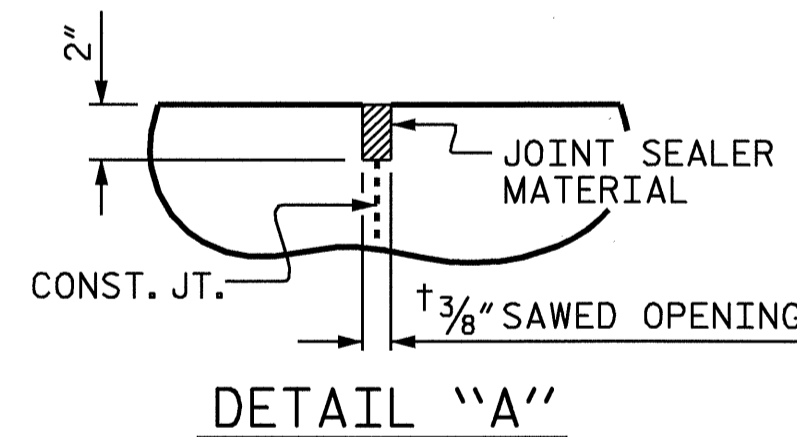
STR. #2



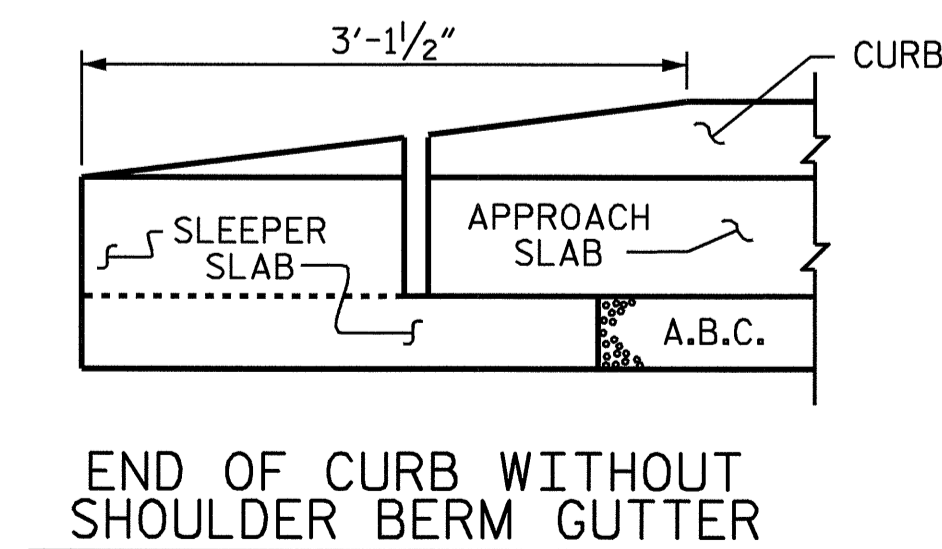
SECTION THRU SLAB



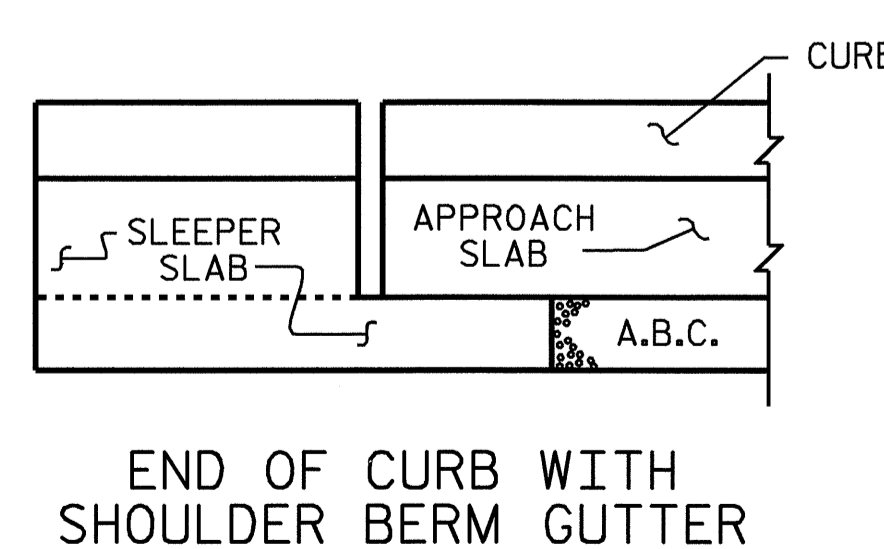
SECTION S-S
SHOWING SLEEPER SLAB



DETAIL "A"



END OF CURB WITHOUT SHOULDER BERM GUTTER



END OF CURB WITH SHOULDER BERM GUTTER

NOTES

APPROACH SLAB SHALL NOT BE CONSTRUCTED PRIOR TO COMPLETION OF THE BRIDGE DECK.

FOR REINFORCED BRIDGE APPROACH FILL INCLUDING FABRIC, IMPERMEABLE GEOMEMBRANE, 4" Ø DRAINAGE PIPE, #78M STONE, AND SELECT MATERIAL, SEE ROADWAY PLANS.

AREA BETWEEN THE WINGWALL AND APPROACH SLAB SHALL BE GRADED TO DRAIN THE WATER AWAY FROM THE FILL FACE OF THE BRIDGE AND SHALL BE PAVED. SEE ROADWAY PLANS.

THE 6" COMP. A.B.C. SHALL EXTEND 10'-0" BEYOND THE END OF THE APPROACH SLAB AND 1'-0" OUTSIDE OF EACH EDGE OF SLAB.

THE CONTRACTOR MAY USE 4" TYPE B-25.0B ASPHALT CONCRETE BASE COURSE IN LIEU OF 6" COMP. A.B.C. IF THIS OPTION IS USED, THE BASE COURSE SHALL EXTEND 1'-0" BEYOND THE END OF THE SLEEPER SLAB AND THE WIDTH SHALL BE THE SAME AS THAT OF THE APPROACH SLAB.

THE CONTRACTOR MAY USE 5" CLASS "A" CONCRETE BASE IN LIEU OF 6" COMP. A.B.C. IF THIS OPTION IS USED, THE CONCRETE BASE SHALL EXTEND 1'-0" BEYOND THE END OF THE SLEEPER SLAB AND THE WIDTH SHALL BE THE SAME AS THAT OF THE APPROACH SLAB. THE CONCRETE SHALL BE FINISHED TO A SMOOTH SURFACE AND A LAYER OF 30 LB ROOFING FELT SHALL BE PLACED BETWEEN THE CONCRETE BASE AND THE APPROACH SLAB TO PREVENT BOND. THE APPROACH SLAB SHALL NOT BE CAST UNTIL THE CONCRETE BASE HAS REACHED AN AGE OF THREE CURING DAYS.

THE VERTICAL JOINT ON THE RIGHT AND LEFT SIDE OF THE APPROACH SLAB AT THE ENDS OF THE EVAZOTE JOINT SHALL BE FILLED WITH SILICONE OR OTHER APPROVED MATERIAL IN ORDER TO PREVENT BACKFILL FROM ENTERING THE JOINT OPENING.

THE JOINT OPENING AT THE APPROACH SLAB/DECK INTERFACE SHALL BE SAWED NO MORE THAN 12 HOURS AFTER THE APPROACH SLAB IS CAST. THE JOINT SHALL BE CLEANED OF ALL DEBRIS BEFORE THE SEALANT IS APPLIED. THE JOINT SEALER MATERIAL SHALL CONFORM TO THE REQUIREMENTS OF TYPE SL LOW MODULUS SILICONE SEALANT.

WITH EVAZOTE JOINT SEAL

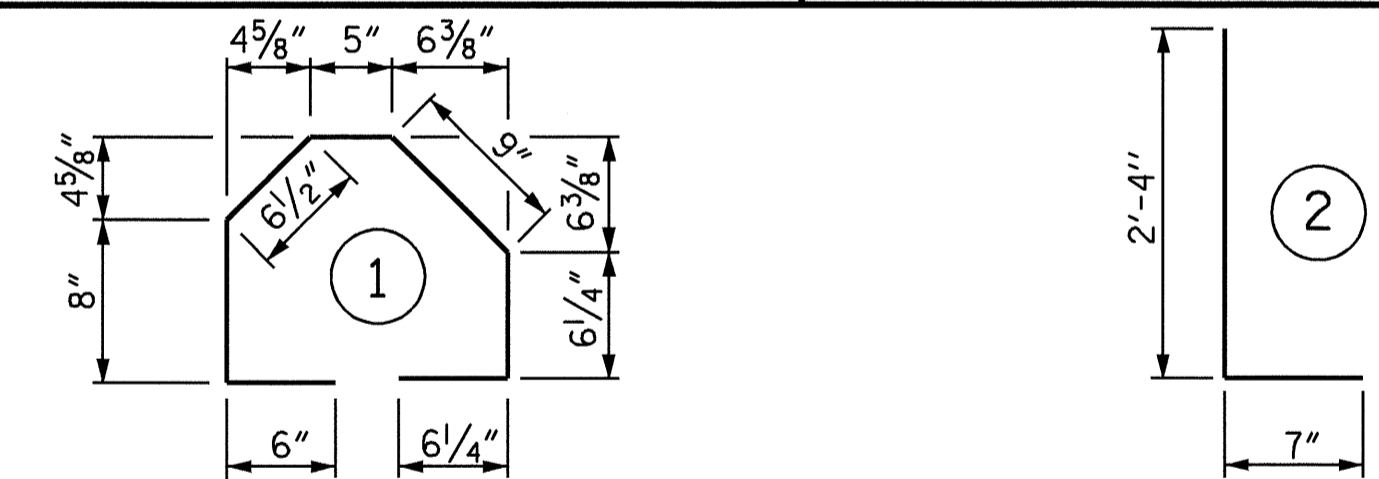
FOR EVAZOTE JOINT SEALS, SEE SPECIAL PROVISIONS.

THE NOMINAL UNCOMPRESSED SEAL WIDTH OF THE EVAZOTE JOINT SEAL SHALL BE 2 3/16".

FOR ELASTOMERIC CONCRETE, SEE SPECIAL PROVISIONS.

BILL OF MATERIAL

APP. SLAB @ END BENT #1 STAGE 1						APP. SLAB @ END BENT #2 STAGE 1					
BAR	NO.	SIZE	TYPE	LENGTH	WEIGHT	BAR	NO.	SIZE	TYPE	LENGTH	WEIGHT
* A1	60	#4	STR	18'-11"	758	* A1	60	#4	STR	18'-11"	758
A2	48	#4	STR	18'-11"	599	A2	48	#4	STR	18'-11"	599
* B1	65	#5	STR	22'-4"	1514	* B1	65	#5	STR	22'-4"	1514
B2	65	#6	STR	22'-9"	2221	B2	65	#6	STR	22'-9"	2221
* S1	33	#4	1	3'-11"	86	* S1	33	#4	1	3'-11"	86
S2	33	#5	2	2'-9"	100	S2	33	#5	2	2'-9"	100
REINFORCING STEEL						REINFORCING STEEL					
LBS. 2920						LBS. 2920					
* EPOXY COATED REINFORCING STEEL						* EPOXY COATED REINFORCING STEEL					
LBS. 2358						LBS. 2358					
CLASS AA CONCRETE						CLASS AA CONCRETE					
POUR #1 - SLAB & CURB						POUR #1 - SLAB & CURB					
C. Y. 31.9						C. Y. 31.9					
POUR #2 - SLEEPER SLAB						POUR #2 - SLEEPER SLAB					
C. Y. 3.5						C. Y. 3.5					
TOTAL						TOTAL					
C. Y. 35.4						C. Y. 35.4					
APP. SLAB @ END BENT #1 STAGE 2						APP. SLAB @ END BENT #2 STAGE 2					
BAR	NO.	SIZE	TYPE	LENGTH	WEIGHT	BAR	NO.	SIZE	TYPE	LENGTH	WEIGHT
* A3	30	#4	STR	21'-1"	423	* A3	30	#4	STR	21'-1"	423
A4	24	#4	STR	21'-1"	338	A4	24	#4	STR	21'-1"	338
* B1	41	#5	STR	22'-4"	955	* B1	41	#5	STR	22'-4"	955
B2	41	#6	STR	22'-9"	1401	B2	41	#6	STR	22'-9"	1401
* S1	20	#4	1	3'-11"	52	* S1	20	#4	1	3'-11"	52
S2	20	#5	2	2'-9"	61	S2	20	#5	2	2'-9"	61
REINFORCING STEEL						REINFORCING STEEL					
LBS. 1800						LBS. 1800					
* EPOXY COATED REINFORCING STEEL						* EPOXY COATED REINFORCING STEEL					
LBS. 1430						LBS. 1430					
CLASS AA CONCRETE						CLASS AA CONCRETE					
POUR #1 - SLAB & CURB						POUR #1 - SLAB & CURB					
C. Y. 21.2						C. Y. 21.2					
POUR #2 - SLEEPER SLAB						POUR #2 - SLEEPER SLAB					
C. Y. 2.2						C. Y. 2.2					
TOTAL						TOTAL					
C. Y. 23.4						C. Y. 23.4					



ALL BAR DIMENSIONS ARE OUT TO OUT

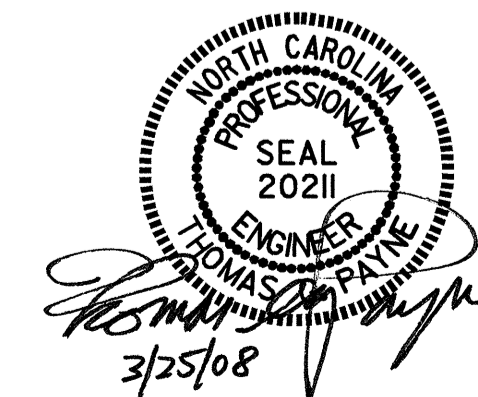
SPLICE CHART

#4 A1	2'-0"
#4 A2	1'-9"
#4 A3	2'-0"
#4 A4	1'-9"

PROJECT NO. B-4252
ROCKINGHAM COUNTY
 STATION: 31+61.50 -L-

SHEET 2 OF 3

STATE OF NORTH CAROLINA
 DEPARTMENT OF TRANSPORTATION
 RALEIGH
 STANDARD
 BRIDGE
 APPROACH SLAB FOR
 INTEGRAL ABUTMENT

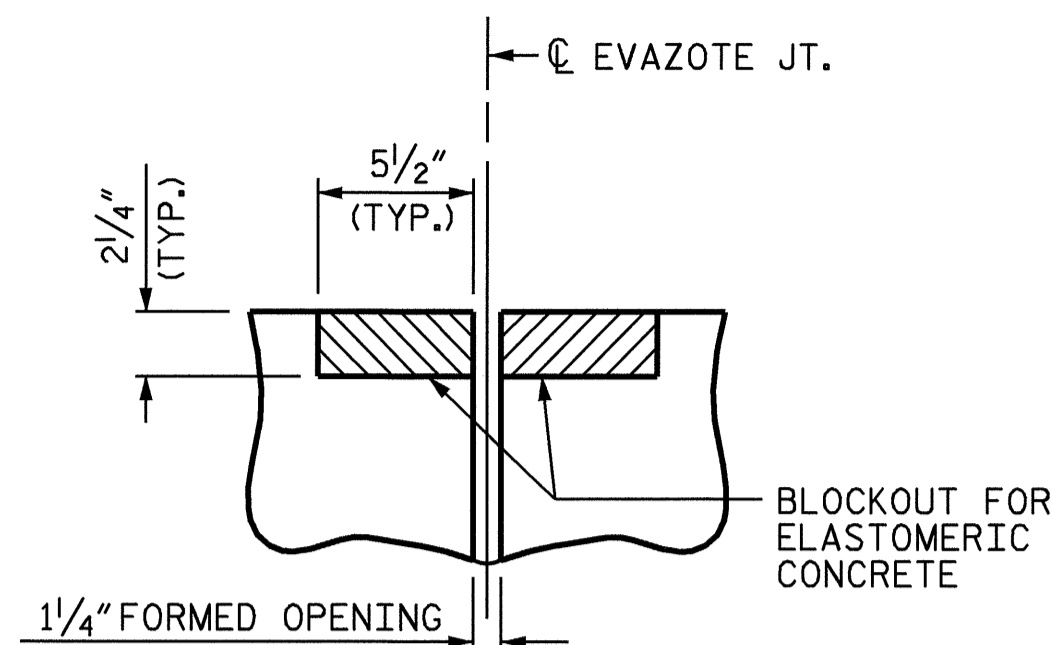


REVISIONS						SHEET NO.	
NO.	BY:	DATE:	NO.	BY:	DATE:	S-69	
1			3			TOTAL SHEETS 70	
2			4				

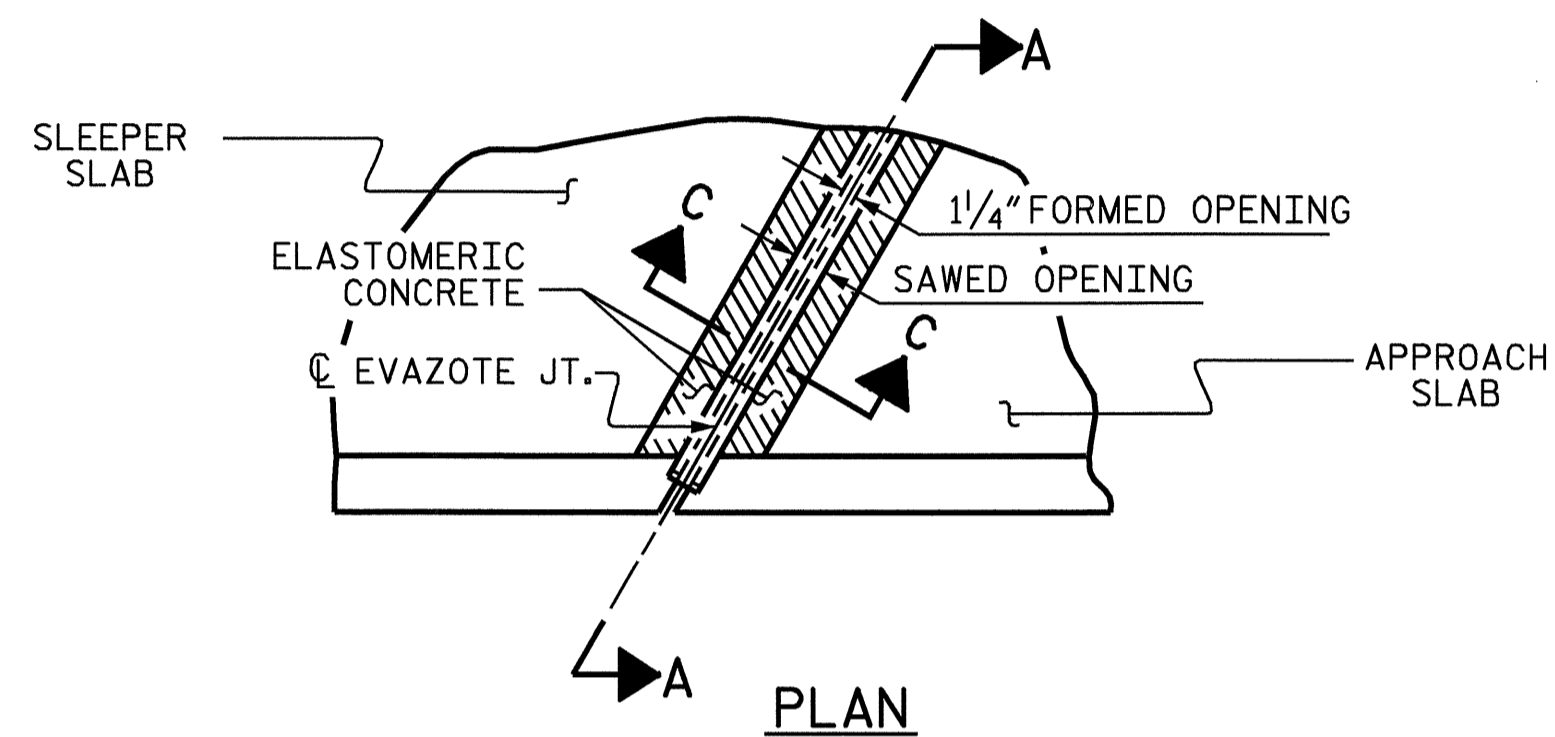
ASSEMBLED BY : R. G. EMERSON DATE : 03/07
 CHECKED BY : J. P. ADAMS DATE : 03/07
 DRAWN BY : TLA 10/05
 CHECKED BY : GM 5/06
 ADDED 5/1/06R KMM/GM

STR. #2

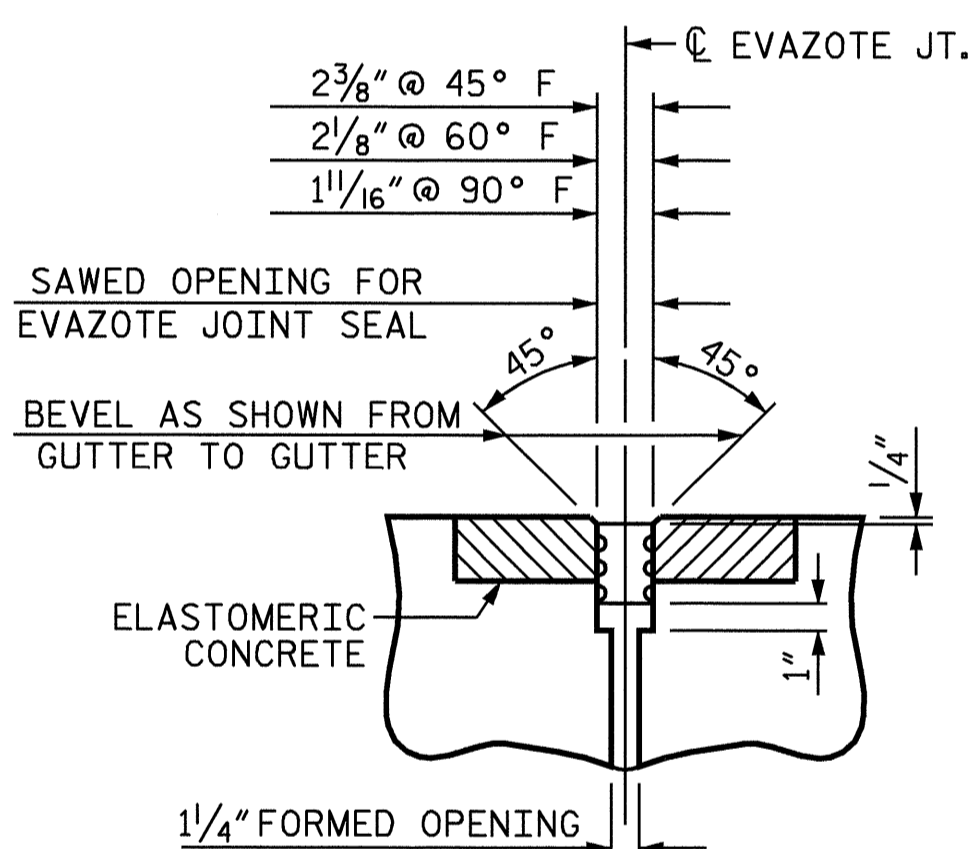
STD. NO. BAS11 (SHT 11)



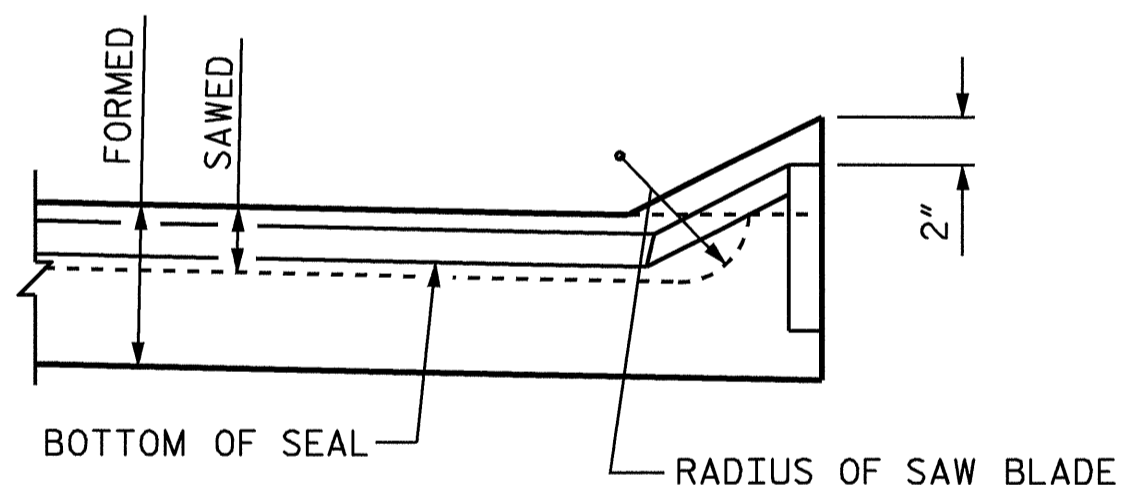
SECTION C-C
EVAZOTE JOINT SEAL
(PRE-SAWED ELASTOMERIC
CONCRETE DIMENSIONS)



PLAN



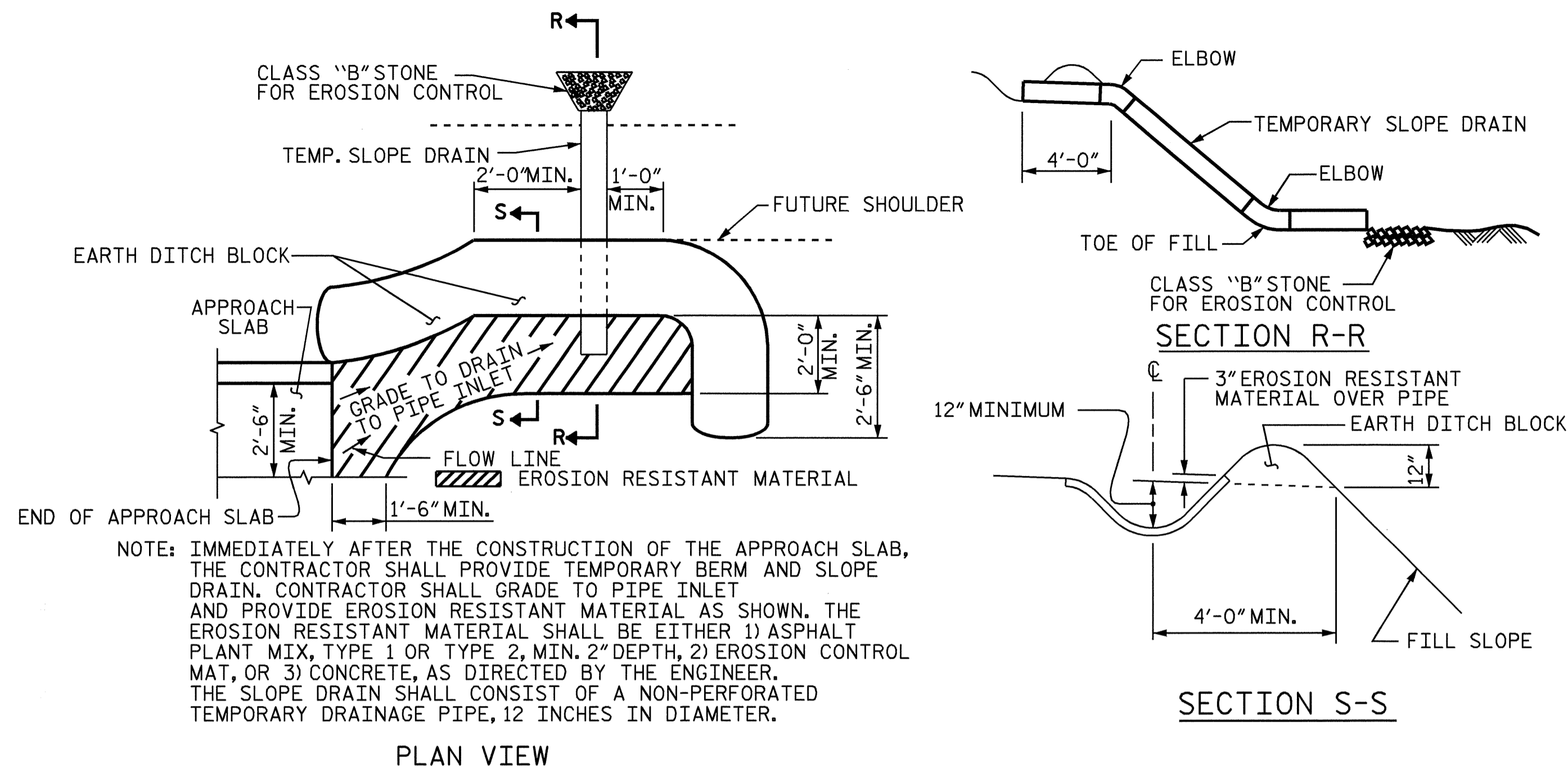
SECTION C-C
EVAZOTE JOINT SEAL



SECTION A-A

ELASTOMERIC CONCRETE			
END BENT NO.	STAGE #1 (CU. FT.)	STAGE #2 (CU. FT.)	TOTAL ELASTOMERIC CONCRETE * (CU. FT.)
1	5.8	3.7	9.5
2	5.8	3.7	9.5
TOTAL	11.6	7.4	19.0

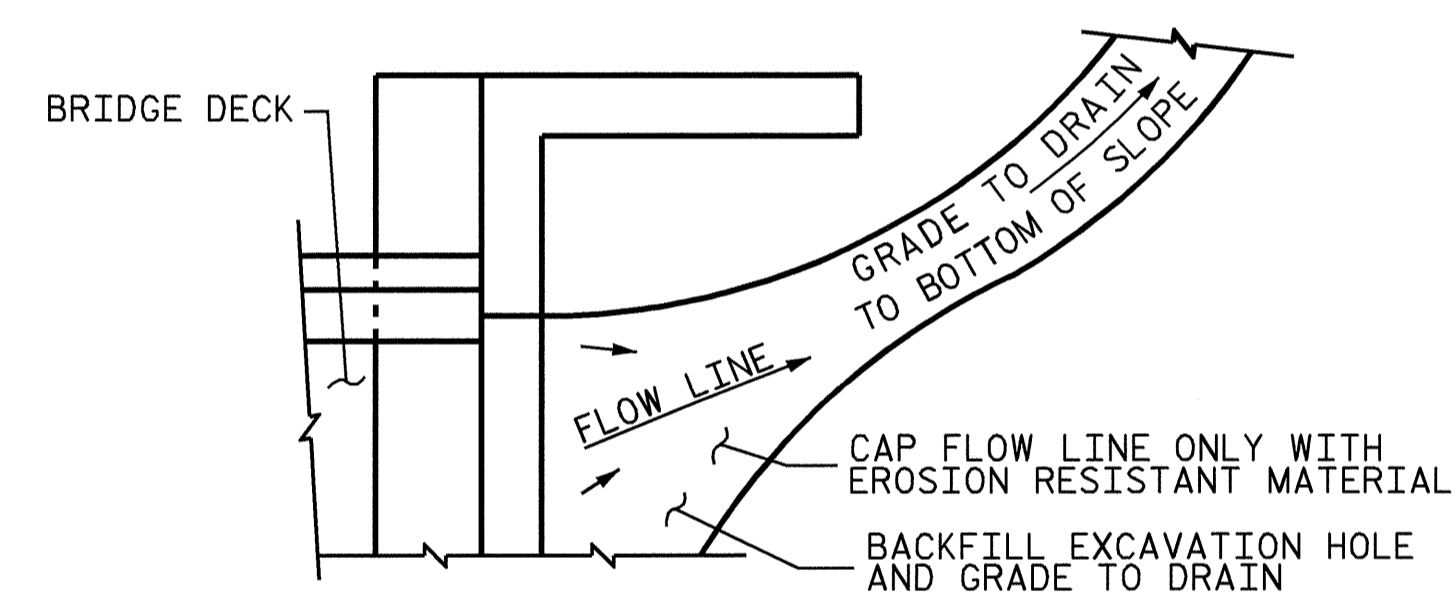
* BASED ON THE MINIMUM BLOCKOUT SHOWN.



PLAN VIEW

TEMPORARY BERM AND SLOPE DRAIN DETAILS

(TO BE USED WHEN SHOULDER BERM GUTTER IS REQUIRED)



NOTE: IF THE APPROACH SLAB IS NOT CONSTRUCTED IMMEDIATELY AFTER THE BACKFILLING OF THE END BENT EXCAVATION, GRADE TO DRAIN TO THE BOTTOM OF THE SLOPE AND PROVIDE EROSION RESISTANT MATERIAL, SUCH AS FIBERGLASS ROVING OR AS DIRECTED BY THE ENGINEER TO PREVENT SOIL EROSION AND TO PROTECT THE AREA ADJACENT TO THE STRUCTURE. THE CONTRACTOR WILL BE REQUIRED TO REMOVE THESE MATERIALS PRIOR TO CONSTRUCTION OF THE APPROACH SLAB.

TEMPORARY DRAINAGE DETAIL

PROJECT NO. B-4252
ROCKINGHAM COUNTY
 STATION: 31+61.50 -L-

SHEET 3 OF 3

STATE OF NORTH CAROLINA
 DEPARTMENT OF TRANSPORTATION
 RALEIGH
 STANDARD
 BRIDGE APPROACH
 SLAB DETAILS



ASSEMBLED BY : R. G. EMERSON DATE : 03/07
 CHECKED BY : J. P. ADAMS DATE : 03/07
 DRAWN BY : FCJ 11/88 REV. 10/17/00 RWW/LES
 CHECKED BY : ARB 11/88 REV. 5/7/03 RWW/JTE
 REV. 5/1/06 TLA/GM

REVISIONS						1988
NO.	BY:	DATE:	NO.	BY:	DATE:	SHEET NO.
1			3			S-70
2			4			TOTAL SHEETS 70

STR. #2

STD. NO. BAS10 (SHT 7)

STANDARD NOTES

DESIGN DATA:

SPECIFICATIONS	-----	A.A.S.H.T.O. (CURRENT)
LIVE LOAD	-----	SEE PLANS
IMPACT ALLOWANCE	-----	SEE A.A.S.H.T.O.
STRESS IN EXTREME FIBER OF		
STRUCTURAL STEEL - AASHTO M270 GRADE 36	-	20,000 LBS. PER SQ. IN.
- AASHTO M270 GRADE 50W	-	27,000 LBS. PER SQ. IN.
- AASHTO M270 GRADE 50	-	27,000 LBS. PER SQ. IN.
REINFORCING STEEL IN TENSION		
GRADE 60	--	24,000 LBS. PER SQ. IN.
CONCRETE IN COMPRESSION	-----	1,200 LBS. PER SQ. IN.
CONCRETE IN SHEAR	-----	SEE A.A.S.H.T.O.
STRUCTURAL TIMBER - TREATED OR		
UNTREATED - EXTREME FIBER STRESS	-----	1,800 LBS. PER SQ. IN.
COMPRESSION PERPENDICULAR TO GRAIN OF TIMBER	-----	375 LBS. PER SQ. IN.
EQUIVALENT FLUID PRESSURE OF EARTH	-----	30 LBS. PER CU. FT. (MINIMUM)

MATERIAL AND WORKMANSHIP:

EXCEPT AS MAY OTHERWISE BE SPECIFIED ON PLANS OR IN THE SPECIAL PROVISIONS, ALL MATERIAL AND WORKMANSHIP SHALL BE IN ACCORDANCE WITH THE 2002 STANDARD SPECIFICATIONS "FOR ROADS AND STRUCTURES" OF THE N. C. DEPARTMENT OF TRANSPORTATION.

STEEL SHEET PILING FOR PERMANENT OR TEMPORARY APPLICATIONS SHALL BE HOT ROLLED.

CONCRETE:

UNLESS OTHERWISE REQUIRED ON PLANS, CLASS A CONCRETE SHALL BE USED FOR ALL PORTIONS OF ALL STRUCTURES WITH THE EXCEPTION THAT: CLASS AA CONCRETE SHALL BE USED IN BRIDGE SUPERSTRUCTURES, ABUTMENT BACKWALLS, AND APPROACH SLABS; CLASS B CONCRETE SHALL BE USED FOR SLOPE PROTECTION AND RIP RAP; AND CLASS S SHALL BE USED FOR UNDERWATER FOOTING SEALS.

CONCRETE CHAMFERS:

UNLESS OTHERWISE NOTED ON THE PLANS, ALL EXPOSED CORNERS ON STRUCTURES SHALL BE CHAMFERED 3/4" WITH THE FOLLOWING EXCEPTIONS: TOP CORNERS OF CURBS MAY BE ROUNDED TO 1-1/2" RADIUS WHICH IS BUILT INTO CURB FORMS; CORNERS OF TRANSVERSE FLOOR EXPANSION JOINTS SHALL BE ROUNDED WITH A 1/4" FINISHING TOOL UNLESS OTHERWISE REQUIRED ON PLANS; AND CORNERS OF EXPANSION JOINTS IN THE ROADWAY FACES AND TOPS OF CURBS AND SIDEWALKS SHALL BE ROUNDED TO A 1/4" RADIUS WITH A FINISHING STONE OR TOOL UNLESS OTHERWISE REQUIRED ON PLANS.

DOWELS:

DOWELS WHEN INDICATED ON PLANS AS FOR CULVERT EXTENSIONS, SHALL BE EMBEDDED AT LEAST 12" INTO THE OLD CONCRETE AND GROUTED INTO PLACE WITH 1:2 CEMENT MORTAR.

ALLOWANCE FOR DEAD LOAD DEFLECTION, SETTLEMENT, ETC. IN CASTING SUPERSTRUCTURES:

BRIDGES SHALL BE BUILT ON THE GRADE OR VERTICAL CURVE SHOWN ON PLANS. SLABS, CURBS AND PARAPETS SHALL CONFORM TO THE GRADE OR CURVE. ALL DIMENSIONS WHICH ARE GIVEN IN SECTION AND ARE AFFECTED BY DEAD LOAD DEFLECTIONS ARE DIMENSIONS AT CENTER LINE OF BEARING UNLESS OTHERWISE NOTED ON PLANS. IN SETTING FORMS FOR STEEL BEAM BRIDGES AND PRESTRESSED CONCRETE GIRDER BRIDGES, ADJUSTMENTS SHALL BE MADE DUE TO THE DEAD LOAD DEFLECTIONS FOR THE ELEVATIONS SHOWN. WHERE BLOCKS ARE SHOWN OVER BEAMS FOR BUILDING UP TO THE SLAB, THE VERTICAL DIMENSIONS OF THE BLOCKS SHALL BE ADJUSTED BETWEEN BEARINGS TO COMPENSATE FOR DEAD LOAD DEFLECTIONS, VERTICAL CURVE ORDINATE, AND ACTUAL BEAM CAMBER. WHERE BOTTOM OF SLAB IS IN LINE WITH BOTTOM OF TOP FLANGES, DEPTH OF SLAB BETWEEN BEARINGS SHALL BE ADJUSTED TO COMPENSATE FOR DEAD LOAD DEFLECTION, VERTICAL CURVE ORDINATE, AND ACTUAL BEAM CAMBER.

IN SETTING FALSEWORK AND FORMS FOR REINFORCED CONCRETE SPANS, AN ALLOWANCE SHALL BE MADE FOR DEAD LOAD DEFLECTIONS, SETTLEMENT OF FALSEWORK, AND PERMANENT CAMBER WHICH SHALL BE PROVIDED FOR IN ADDITION TO THE ELEVATIONS SHOWN. AFTER REMOVAL OF THE FALSEWORK, THE FINISHED STRUCTURES SHALL CONFORM TO THE PROFILE AND ELEVATIONS SHOWN ON THE PLANS AND CONSTRUCTION ELEVATIONS FURNISHED BY THE ENGINEER.

DETAILED DRAWINGS FOR FALSEWORK OR FORMS FOR BRIDGE SUPERSTRUCTURE AND ANY STRUCTURE OR PARTS OF A STRUCTURE AS NOTED ON THE PLANS SHALL BE SUBMITTED TO THE ENGINEER FOR APPROVAL BEFORE CONSTRUCTION OF THE FALSEWORK OR FORMS IS STARTED.

REINFORCING STEEL:

ALL REINFORCING STEEL SHALL BE DEFORMED WITH THE EXCEPTION OF #2 BARS WHICH MAY BE FABRICATED FROM COLD DRAWN STEEL WIRE. DIMENSIONS RELATIVE TO PLACEMENT OF REINFORCING ARE TO CENTERS OF BARS UNLESS OTHERWISE INDICATED IN THE PLANS. DIMENSIONS ON BAR DETAILS ARE TO CENTERS OF BARS OR ARE OUT TO OUT AS INDICATED ON PLANS.

WIRE BAR SUPPORTS SHALL BE PROVIDED FOR REINFORCING STEEL WHERE INDICATED ON THE PLANS. WHEN BAR SUPPORT PIECES ARE PLACED IN CONTINUOUS LINES, THEY SHALL BE SO PLACED THAT THE ENDS OF THE SUPPORTING WIRES SHALL BE LAPPED TO LOCK LEGS ON ADJOINING PIECES.

STRUCTURAL STEEL:

AT THE CONTRACTOR'S OPTION, HE MAY SUBSTITUTE 7/8" Ø SHEAR STUDS FOR THE 3/4" Ø STUDS SPECIFIED ON THE PLANS. THIS SUBSTITUTION SHALL BE MADE AT THE RATE OF 3 - 7/8" Ø STUDS FOR 4 - 3/4" Ø STUDS, AND STUD SPACING CHANGES SHALL BE MADE AS NECESSARY TO PROVIDE THE SAME EQUIVALENT NUMBER OF 7/8" Ø STUDS ALONG THE BEAM AS SHOWN FOR 3/4" Ø STUDS BASED ON THE RATIO OF 3 - 7/8" Ø STUDS FOR 4 - 3/4" Ø STUDS. STUDS OF THE LENGTH SPECIFIED ON THE PLANS MUST BE PROVIDED. THE MAXIMUM SPACING SHALL BE 2'-0".

EXCEPT AT THE INTERIOR SUPPORTS OF CONTINUOUS BEAMS WHERE THE COVER PLATE IS IN CONTACT WITH BEARING PLATE, THE CONTRACTOR MAY, AT HIS OPTION, SUBSTITUTE FOR THE COVER PLATES DESIGNATED ON THE PLANS COVER PLATES OF THE EQUIVALENT AREA PROVIDED THESE PLATES ARE AT LEAST 5/16" IN THICKNESS AND DO NOT EXCEED A WIDTH EQUAL TO THE FLANGE WIDTH LESS 2" OR A THICKNESS EQUAL TO 2 TIMES THE FLANGE THICKNESS. THE SIZE OF FILLET WELDS SHALL CONFORM TO THE REQUIREMENTS OF THE CURRENT ANSI/AASHTO/AWS "BRIDGE WELDING CODE". ELECTROSLAG WELDING WILL NOT BE PERMITTED.

PLACEMENT OF BEAM OR GIRDER MEMBERS ON TRUCKS FOR HAULING SHALL BE DONE IN COMPLIANCE WITH LIMITS SHOWN ON SKETCHES PROVIDED TO THE MATERIALS AND TEST UNIT APPROVED BY THE STRUCTURE DESIGN UNIT DATED MAY 8, 1991. THESE SKETCHES PRIMARILY LIMIT THE UNSUPPORTED CANTILEVER LENGTH OF MEMBERS. WHEN THE CONTRACTOR WISHES TO PLACE MEMBERS ON TRUCKS NOT IN ACCORDANCE WITH THESE LIMITS, TO SHIP BY RAIL, TO ATTACH SHIPPING RESTRAINTS TO THE MEMBERS OR TO INVERT MEMBERS, HE SHALL SUBMIT A SKETCH FOR APPROVAL PRIOR TO SHIPPING. SEE ALSO ARTICLE 1072-11.

WITH THE SOLE EXCEPTION OF EDGES AT SURFACES WHICH BEAR ON OTHER SURFACES, ALL SHARP EDGES AND ENDS OF SHAPES AND PLATES SHALL BE SLIGHTLY ROUNDED BY SUITABLE MEANS TO A RADIUS OF APPROXIMATELY 1/16 INCH OR EQUIVALENT FLAT SURFACE AT A SUITABLE ANGLE PRIOR TO PAINTING, GALVANIZING, OR METALLIZING.

HANDRAILS AND POSTS:

METAL STANDARDS AND FACES OF THE CONCRETE END POSTS FOR THE METAL RAIL SHALL BE SET NORMAL TO THE GRADE OF THE CURB, UNLESS OTHERWISE SHOWN ON PLANS. THE METAL RAIL AND TOPS OF CONCRETE POSTS USED WITH THE ALUMINUM RAIL SHALL BE BUILT PARALLEL TO THE GRADE OF THE CURB.

METAL HANDRAILS SHALL BE IN ACCORDANCE WITH THE PLANS. RAILS SHALL BE AS MANUFACTURED FOR BRIDGE RAILING. CASTINGS SHALL BE OF A UNIFORM APPEARANCE. FINIS AND OTHER DEFORMATIONS RESULTING FROM CASTING OR OTHERWISE SHALL BE REMOVED IN A MANNER SO THAT A UNIFORM COLORING OF THE COMPLETED CASTING SHALL BE OBTAINED. CASTINGS WITH DISCOLORATIONS OR OF NON-UNIFORM COLORING WILL NOT BE ACCEPTED. CERTIFIED MILL REPORTS ARE REQUIRED FOR METAL RAILS AND POSTS.

SPECIAL NOTES:

GENERALLY, IN CASE OF DISCREPANCY, THIS STANDARD SHEET OF NOTES SHALL GOVERN OVER THE SPECIFICATIONS, BUT THE REMAINDER OF THE PLANS SHALL GOVERN OVER NOTES HEREON, AND SPECIAL PROVISIONS SHALL GOVERN OVER ALL. SEE SPECIFICATIONS ARTICLE 105-4.

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