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PROFILE

STATE OF NORTH CAROLINA

DEPARTMENT OF TRANSPORTATION

DIVISION OF HIGHWAYS

GEOTECHNICAL UNIT

STRUCTURE SUBSURFACE INVESTIGATION

STATE PROJECT 335	99.1.1 I.D. NO. B-4161
F.A. PROJECT BRZ-1 1	32 (6)
COUNTY JACKSON	A Transfer of the Control of the Con
PROJECT DESCRIPTION	BRIDGE NO. 211
	TUCKASEGEE RIVE
ON SR-1132	· · · · · · · · · · · · · · · · · · ·
SITE DESCRIPTION	

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STATE	PROJ. NO.	F.A.PROJ.NO.	DESCRIP	TION
N.C.	33509	.1.1 (B-4161)	1	11
STATE	STATE PI	OJECT REFERENCE NO.	SHEET NO.	SHEET

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THE BIDDER OR CONTRACTOR IS CAUTIONED THAT DETAILS SHOWN ON THE SUBSURFACE PLANS ARE PRELIMINARY ONLY AND IN MANY CASES THE FINAL DESIGN DETAILS ARE DIFFERENT. FOR BIDDING AND CONSTRUCTION PURPOSES, REFER TO THE CONSTRUCTION PLANS AND DOCUMENTS FOR FINAL DESIGN AND CONSTRUCTION PURPOSES, REFER TO THE CONSTRUCTION PLANS AND DOCUMENTS FOR FINAL DESIGN INFORMATION ON THIS PROJECT. THE DEPARTMENT DOES NOT WARRANT OR GUARANTEE THE SUFFICIENCY OR ACCURACY OF THE INVESTIGATION MADE, NOR THE INTERPRETATIONS MADE OR OPINION OF THE DEPARTMENT AS TO THE TYPE OF MATERIALS AND CONDITIONS TO BE ENCOUNTERED. THE BIDDER OR CONTRACTOR IS CAUTIONED TO MAKE SUCH INDEPENDENT SUBSURFACE INVESTIGATIONS AS HE DEEMS NECESSARY TO SATISFY HIMSELF AS TO CONDITIONS TO BE ENCOUNTERED ON THIS PROJECT. THE CONTRACTOR SHALL HAVE NO CLAIM FOR ADDITIONAL COMPENSATION OR FOR AN EXTENSION OF THE FOR ANY REASON RESULTING FROM THE ACTUAL CONDITIONS ENCOUNTERED AT THE SITE DIFFERING FROM THOSE INDICATED IN THE SUBSURFACE INFORMATION.

INVESTIGATED BY J.W. MANN PERSONNEL T.B. DANIEL W.D. FRYE C.J. COFFEY CHECKED BY_ W.D. FRYE R.D. CHILDERS SUBMITTED BY 02/01/06

NOTE - THE INFORMATION CONTAINED HEREIN IS NOT IMPLIED OR GUARANTEED BY THE N. C. DEPARTMENT OF TRANSPORTATION AS BEING ACCURATE NOR IT IS CONSIDERED TO BE PART OF THE PLANS, SPECIFICATIONS. OR CONTRACT FOR THE PROJECT.

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DRAWN BY: J.W. MANN

NORTH CAROLINA DEPARTMENT OF TRANSPORTATION

DIVISION OF HIGHWAYS

GEOTECHNICAL UNIT

SUBSURFACE INVESTIGATION

	SOIL AND ROCK LEGEND, TE	RMS, SYMBOLS, AND ABBREVIATIONS	
SOIL DESCRIPTION	GRADATION	ROCK DESCRIPTION	TERMS AND DEFINITIONS
SOIL IS CONSIDERED TO BE THE UNCONSOLIDATED, SEMI-CONSOLIDATED OR WEATHERED EARTH MATERIALS	WELL GRADED- INDICATES A GOOD REPRESENTATION OF PARTICLE SIZES FROM FINE TO COARSE	HARD ROCK IS NON-COASTAL PLAIN MATERIAL THAT WHEN TESTED, WOULD YIELD SPT REFUSAL, AN INFERRED	ALLUVIUM (ALLUV.) - SOILS WHICH HAVE BEEN TRANSPORTED BY WATER.
WHICH CAN BE PENETRATED WITH A CONTINUOUS FLIGHT POWER AUGER, AND WHICH YIELDS LESS THAN	UNIFORM- INDICATES THAT SOIL PARTICLES ARE ALL APPROXIMATELY THE SAME SIZE. (ALSO POORLY GRADED)	ROCK LINE INDICATES THE LEVEL AT WHICH NON-COASTAL PLAIN MATERIAL WOULD YIELD SPT REFUSAL. SPT REFUSAL IS PENETRATION BY A SPLIT SPOON SAMPLER EQUAL TO OR LESS THAN 0.1 FOOT PER 60 BLOWS.	ADUIFER - A WATER BEARING FORMATION OR STRATA.
100 BLOWS PER FODT ACCORDING TO STANDARD PENETRATION TEST (AASHTO T206, ASTM D-1586), SOIL CLASSIFICATION IS BASED ON THE AASHTO SYSTEM AND BASIC DESCRIPTIONS GENERALLY SHALL INCLUDE:	GAP-GRADED- INDICATES A MIXTURE OF UNIFORM PARTICLES OF TWO OR MORE SIZES. ANGULARITY OF GRAINS	IN NON-COASTAL PLAIN MATERIAL, THE TRANSITION BETWEEN SOIL AND ROCK IS OFTEN REPRESENTED BY A ZONE OF WEATHERED ROCK.	ARENACEOUS - APPLIED TO ROCKS THAT HAVE BEEN DERIVED FROM SAND OR THAT CONTAIN SAND.
CONSISTENCY, COLOR, TEXTURE, MOISTURE, AASHTO CLASSIFICATION, AND OTHER PERTINENT FACTORS SUCH AS MINERALOGICAL COMPOSITION, ANGULARITY, STRUCTURE, PLASTICITY, ETC. EXAMPLE:	THE ANGULARITY OR ROUNDNESS OF SOIL GRAINS ARE DESIGNATED BY THE TERMS; ANGULAR.	ROCK MATERIALS ARE TYPICALLY DIVIDED AS FOLOWS:	ARGILLACEOUS - APPLIED TO ALL ROCKS OR SUBSTANCES COMPOSED OF CLAY MINERALS,
VERY STIFF, GRAY SULTY CLAY, WOST WITH INTERBEDOED FINE SAND LIVERS, HIGHLY PLASTIC, A-7-6	SUBANGULAR, SUBROUNDED, OR ROUNDED.	WEATHERED NON-COASTAL PLAIN MATERIAL THAT YIELDS SPT N VALUES > 1000 BLOWS	OR HAVING A NOTABLE PROPORTION OF CLAY IN THEIR COMPOSITION, AS SHALE, SLATE, ETC.
SOIL LEGEND AND AASHTO CLASSIFICATION	MINERALOGICAL COMPOSITION		ARTESIAN - GROUND WATER THAT IS UNDER SUFFICIENT PRESSURE TO RISE ABOVE THE LEVEL AT WHICH IS IS ENCOUNTERED, BUT WHICH DOES NOT NECESSARILY RISE TO OR ABOVE THE
GENERAL GRANULAR MATERIALS SILT-CLAY MATERIALS OPERANC MATERIALS	MINERAL NAMES SUCH AS QUARTZ, FELDSPAR, MICA, TALC, KAOLIN, ETC. ARE USED IN DESCRIPTIONS	CRYSTALLINE ROCK (CR) FINE TO COARSE GRAIN IGNEOUS AND METAMORPHIC ROCK THAT WOULD YIELD SPT REFUSAL IF TESTED. ROCK TYPE INCLUDES GRANITE,	GROUND SURFACE.
CLASS. (35% PASSING *200) (%5% PASSING *200)	WHENEVER THEY ARE CONSIDERED OF SIGNIFICANCE.	GNEISS, GABBRO, SCHIST, ETC.	CALCAREOUS (CALC.) - SOILS WHICH CONTAIN APPRECIABLE AMOUNTS OF CALCIUM CARBONATE,
GROUP A-1 A-3 A-2 A-4 A-5 A-6 A-7 A-1, A-2 A-4, A-5 CLASS. A-1-a A-1-b A-2-4 A-2-5 A-2-7 A-3 A-6, A-7	COMPRESSIBILITY	NUN-CATSTRILLINE SEDIMENTARY ROCK THAT WOULD YEILD SPT REFUSAL IF TESTED, ROCK TYPE	COLLUVIUM - ROCK FRAGMENTS MIXED WITH SOIL DEPOSITED BY GRAVITY ON SLOPE OR AT BOTTOM
0000000000	SLIGHTLY COMPRESSIBLE LIQUID LIMIT LESS THAN 30 MODERATELY COMPRESSIBLE LIQUID LIMIT 31-50	COASTAL PLAIN COASTAL PLAIN SEDIMENTS CEMENTED INTO ROCK, BUT MAY NOT YIELD	OF SLOPE. CORE RECOVERY (REC.) - TOTAL LENGTH OF ALL MATERIAL RECOVERED IN THE CORE BARREL DIVIDED BY TOTAL
SYMBOL BOOOGOOGO	HIGHLY COMPRESSIBLE LIQUID LIMIT GREATER THAN 50	SEDIMENTARY ROCK SPT REFUSAL ROCK TYPE INCLUDES LIMESTONE, SANDSTONE, CEMENTED SHELL BEDS, ETC.	LENGTH OF CORE RUN AND EXPRESSED AS A PERCENTAGE.
7. PASSING SILT- GRANULAR SILT- MUCK,	PERCENTAGE OF MATERIAL GRANULAR SILT- CLAY	WEATHERING	DIKE - A TABULAR BODY OF IGNEOUS ROCK THAT CUTS ACROSS THE STRUCTURE OF ADJACENT
40 30 MX50 MX51 MN S03LS CLAY PEAT	SOILS SOILS OTHER MATERIAL	FRESH ROCK FRESH, CRYSTALS BRIGHT, FEW JOINTS MAY SHOW SLIGHT STAINING, ROCK RINGS UNDER	ROCKS OR CUTS MASSIVE ROCK.
■ 200 15 MX 25 MX10 MX 35 MX35 MX35 MX35 MX36 MN36 MN36 MN36 MN	TRACE OF ORGANIC MATTER 2 - 3% 3 - 5% TRACE 1 - 10%	HAMMER IF CRYSTALLINE.	DIP - THE ANGLE AT WHICH A STRATUM OR ANY PLANAR FEATURE IS INCLINED FROM THE HORIZONTAL.
LIGUID LIMIT 40 MX41 MN 40 MX41 MN 40 MX41 MN 40 MX41 MN SOILS WITH	MODERATELY ORGANIC 5 - 10% 12 - 20% SOME 20 - 35%	VERY SLIGHT ROCK GENERALLY FRESH, JOINTS STAINED, SOME JOINTS MAY SHOW THIN CLAY COATINGS IF OPEN,	DIP DIRECTION (DIP AZIMUTH) - THE DIRECTION OR BEARING OF THE HORIZONTAL TRACE OF
PLASTIC INDEX 6 MX N.P. 10 MX 10 MX 11 MN 11 MN 10 MX 11 MN 11 MN LITTLE OR HIGHLY	HIGHLY ORGANIC >10% >20% HIGHLY 35% AND ABOVE	(V.SLI.) CRYSTALS ON A BROKEN SPECIMEN FACE SHINE BRIGHTLY. ROCK RINGS UNDER HAMMER BLOWS IF OF A CRYSTALLINE NATURE.	THE LINE OF DIP, MEASURED CLOCKWISE FROM NORTH.
AMOUNTS OF	GROUND WATER	SLIGHT ROCK GENERALLY FRESH, JOINTS STAINED AND DISCOLORATION EXTENDS INTO ROCK UP TO	FAULT - A FRACTURE OR FRACTURE ZONE ALONG WHICH THERE HAS BEEN DISPLACEMENT OF THE SIDES RELATIVE TO ONE ANOTHER PARALLEL TO THE FRACTURE.
OF HE TIPE STUTE THIS FINE SILTY OR CLAYEY SILTY CLAYEY ORGANIC	WATER LEVEL IN BORE HOLE IMMEDIATELY AFTER DRILLING.	(SLI,) 1 INCH, OPEN JOINTS MAY CONTAIN CLAY, IN GRANITOID ROCKS SOME OCCASIONAL FELDSPAR CRYSTALS ARE DULL AND DISCOLORED, CRYSTALLINE ROCKS RING UNDER HAMMER BLOWS.	FISSILE - A PROPERTY OF SPLITTING ALONG CLOSELY SPACED PARALLEL PLANES.
MATERIALS SAND SAND GRAVEL AND SAND SULS SULS	STATIC WATER LEVEL AFTER 24 HOURS.	MODERATE SIGNIFICANT PORTIONS OF ROCK SHOW DISCOLORATION AND WEATHERING EFFECTS. IN	FLOAT - ROCK FRAGMENTS ON SURFACE NEAR THEIR ORIGINAL POSITION AND DISLODGED FROM
GEN. RATING AS A EXCELLENT TO GOOD FAIR TO POOR POOR POOR POOR POOR POOR POOR	PERCHED WATER, SATURATED ZONE OR WATER BEARING STRATA	(MDD.) GRANITOID ROCKS, MOST FELDSPARS ARE DULL AND DISCOLORED, SOME SHOW CLAY, ROCK HAS	PARENT MATERIAL.
SUBGRADE POOR POOR WHOLEHAM	SPRING OR SEEPAGE	DULL SOUND UNDER HAMMER BLOWS AND SHOWS SIGNIFICANT LOSS OF STRENGTH AS COMPARED WITH FRESH ROCK.	FLOOD PLAIN (F.P.) - LAND BORDERING A STREAM, BUILT OF SEDIMENTS DEPOSITED BY
P.I. OF A-7-5 ≤ L.L 30 : P.I. OF A-7-6 > L.L 30		MODERATELY ALL ROCK EXCEPT QUARTZ DISCOLORED OR STAINED. IN GRANITOID ROCKS, ALL FELDSPARS DULL	THE STREAM.
CONSISTENCY OR DENSENESS RANGE OF STANDARD RANGE OF UNCONFINED	MISCELLANEOUS SYMBOLS	SEVERE AND DISCOLORED AND A MAJORITY SHOW KAOLINIZATION. ROCK SHOWS SEVERE LOSS OF STRENGTH (MOD. SEV.) AND CAN BE EXCAVATED WITH A GEOLOGIST'S PICK, ROCK GIVES "CLUNK" SOUND WHEN STRUCK.	FORMATION (FM.) - A MAPPABLE GEOLOGIC UNIT THAT CAN BE RECOGNIZED AND TRACED IN THE FIELD.
PRIMARY SOIL TYPE COMPACTNESS OR PENETRATION RESISTENCE COMPRESSIVE STRENGTH	ROADWAY EMBANKMENT WITH SOIL DESCRIPTION ROADWAY EMBANKMENT WITH SOIL DESCRIPTION ROADWAY EMBANKMENT WITH SOIL DESCRIPTION ROADWAY EMBANKMENT OFFICIAL DESCRIPTION ROADWAY EMBANKMENT OFFICIAL DESCRIPTION ROADWAY EMBANKMENT OFFICIAL DESCRIPTION OFFICIAL DESCRIPTION OFFICIAL DESCRIPTION	IF TESTED, WOULD YIELD SPT REFUSAL	JOINT - FRACTURE IN ROCK ALONG WHICH NO APPRECIABLE MOVEMENT HAS OCCURRED.
(MARIDE) (LONO) 1-)	TESTONALIONS	SEVERE ALL ROCKS EXCEPT QUARTZ DISCOLORED OR STAINED, ROCK FABRIC CLEAR AND EVIDENT BUT REDUCED	LEDGE - A SHELF-LIKE RIDGE OR PROJECTION OF ROCK WHOSE THICKNESS IS SMALL COMPARED TO
GENERALLY VERY LOOSE	SOIL SYMBOL AUGER BORING S- BULK SAMPLE	(SEV.) IN STRENGTH TO STRONG SOIL, IN GRANITOID ROCKS ALL FELDSPARS ARE KAOLINIZED TO SOME EXTENT, SOME FRAGMENTS OF STRONG ROCK USUALLY REMAIN.	ITS LATERAL EXTENT.
MATERIAL MEDIUM DENSE 10 TO 30 N/A	ARTIFICIAL FILL OTHER THAN ROADWAY FARANYMENTS CORE BORING SS- SPLIT SPOON COMPLE	IF TESTED, YIELDS SPT N VALUES > 100 BPF	LENS - A BODY OF SOIL OR ROCK THAT THINS OUT IN ONE OR MORE DIRECTIONS.
(NON-COHESIVE) DENSE 30 TO 50 VERY DENSE >50	SHIPLE	VERY SEVERE ALL ROCK EXCEPT QUARTZ DISCOLORED OR STAINED. ROCK FABRIC ELEMENTS ARE DISCERNIBLE BUT	MOTTLED (MOT.) - IRREGULARLY MARKED WITH SPOTS OF DIFFERENT COLORS. MOTTLING IN SOILS UBUALLY INDICATES POOR AERATION AND LACK OF GOOD DRAINAGE.
VERY SOFT <2 <0.25	ST- SHELBY TUBE SAMPLE MONITORING WELL SAMPLE	(V, SEV.) THE MASS IS EFFECTIVELY REDUCED TO SOIL STATUS, WITH ONLY FRAGMENTS OF STRONG ROCK REMAINING. SAPROLITE IS AN EXAMPLE OF ROCK WEATHERED TO A DEGREE SUCH THAT ONLY MINOR	PERCHED WATER - WATER MAINTAINED ABOVE THE NORMAL GROUND WATER LEVEL BY THE PRESENCE OF AN
GENERALLY SOFT 2 TO 4 0.25 TO 0.5	INFERRED ROCK LINE A PIEZOMETER RS- ROCK SAMPLE	VESTIGES OF THE ORIGINAL ROCK FABRIC REMAIN. IF TESTED, YIELDS SPT N VALUES < 100 BPF	INTERVENING IMPERVIOUS STRATUM.
SILT-CLAY MEDIUM STIFF 4 TO 8 0.5 TO 1	ALLUVIAL SOIL BOUNDARY ALLUVIAL SOIL BOUNDARY ALLUVIAL SOIL BOUNDARY	COMPLETE ROCK REDUCED TO SOIL, ROCK FABRIC NOT DISCERNIBLE, OR DISCERNIBLE ONLY IN SMALL AND SCATTERED CONCENTRATIONS, QUARTZ MAY BE PRESENT AS DIKES OR STRINGERS, SAPROLITE IS	RESIDUAL SOIL - SOIL FORMED IN PLACE BY THE WEATHERING OF ROCK.
(COHESIVE) VERY STIFF 15 TO 30 2 TO 4	SLOPE INDICATOR TRIAXIAL SAMPLE	ALSO AN EXAMPLE.	ROCK QUALITY DESIGNATION (R.O.D.) - A MEASURE OF ROCK QUALITY DESCRIBED BY: TOTAL LENGTH OF ROCK SEGMENTS EQUAL TO OR GREATER THAN 4 INCHES DIVIDED BY THE TOTAL LENGTH OF CORE RUN AND
	25/025 DIP/DIP DIRECTION OF INSTALLATION CBR - CBR SAMPLE ROCK STRUCTURES	ROCK HARDNESS	EXPRESSED AS A PERCENTAGE.
TEXTURE OR GRAIN SIZE	SPT N-VALUE	VERY HARD CANNOT BE SCRATCHED BY KNIFE OR SHARP PICK, BREAKING OF HAND SPECIMENS REQUIRES	SAPROLITE (SAP.) - RESIDUAL SOIL WHICH RETAINS THE RELIC STRUCTURE OR FABRIC OF THE
U.S. STD. SIEVE SIZE 4 10 40 60 200 270	● - SOUNDING ROD REF SPT REFUSAL	SEVERAL HARD BLOWS OF THE GEOLOGISTS PICK.	PARENT ROCK.
OPENING (MM) 4.76 2.0 0.42 0.25 0.075 0.053	ABBREVIATIONS	HARD CAN BE SCRATCHED BY KNIFE OR PICK ONLY WITH DIFFICULTY. HARD HAMMER BLOWS REQUIRED	SILL - AN INTRUSIVE BODY OF IGNEOUS ROCK OF APPROXIMATELY UNIFORM THICKNESS AND RELATIVELY THIN COMPARED WITH ITS LATERAL EXTENT, WHICH HAS BEEN EMPLACED PARALLEL
BOULDER COBBLE GRAVEL COARSE FINE SILT CLAY	AR - AUGER REFUSAL PMT - PRESSUREMETER TEST	TO DETACH HAND SPECIMEN.	TO THE BEDDING OR SCHISTOSITY OF THE INTRUDED ROCKS
(BLDR.) (COB.) (GR.) (CSE. SD.) (F. SD.) (SL.) (CL.)	BT - BORING TERMINATED SD SAND, SANDY	MODERATELY CAN BE SCRATCHED BY KNIFE OR PICK. GOUGES OR GROOVES TO 0.25 INCHES DEEP CAN BE HARD EXCAVATED BY HARD BLOW OF A GEOLOGISTS PICK, HAND SPECIMENS CAN BE DETACHED	SLICKENSIDE - POLISHED AND STRIATED SURFACE THAT RESULTS FROM FRICTION ALONG A FAULT OR SLIP PLANE.
GRAIN MM 305 75 2.0 0.25 0.05 0.005	CL CLAY SL SILT, SILTY CPT - CONE PENETRATION TEST SLI SLIGHTLY	BY MODERATE BLOWS.	STANDARD PENETRATION TEST (PENETRATION RESISTANCE) (SPT) - NUMBER OF BLOWS (N OR B.P.F.) OF
SIZE IN. 12' 3'	CSE COARSE TCR - TRICONE REFUSAL	MEDIUM CAN BE GROOVED OR GOUGED 0.05 INCHES DEEP BY FIRM PRESSURE OF KNIFE OR PICK POINT. HARD CAN BE EXCAVATED IN SMALL CHIPS TO PEICES 1 INCH MAXIMUM SIZE BY HARD BLOWS OF THE	A 140 LB. HAMMER FALLING 30 INCHES REQUIRED TO PRODUCE A PENETRATION OF 1 FOOT INTO SOIL WITH
SOIL MOISTURE - CORRELATION OF TERMS SOIL MOISTURE SCALE FIELD MOISTURE COURSE FOR FIELD MOISTURE DESCRIPTION OF TERMS	DMT - DILATOMETER TEST DPT - DYNAMIC PENETRATION TEST ONLY THE PROPERTY OF T	POINT OF A GEOLOGISTS PICK.	A 2 INCH DUTSIDE DIAMETER SPLIT SPOON SAMPLER, SPT REFUSAL IS LESS THAN 0.1 FOOT PENETRATION WITH 60 BLOWS.
(ATTERBERG LIMITS) SOIL MOISTORE STALE FIELD MOISTORE GUIDE FOR FIELD MOISTURE DESCRIPTION	e - VOID RATIO /d - DRY ONLY WEIGHT	SOFT CAN BE GROVED OR GOUGED READILY BY KNIFE OR PICK, CAN BE EXCAVATED IN FRAGMENTS	STRATA CORE RECOVERY (SREC.) - TOTAL LENGTH OF STRATA MATERIAL RECOVERED DIVIDED BY TOTAL LENGTH
- SATURATED - USUALLY LIQUID; VERY WET, USUALLY	F FINE W - MOISTURE CONTENT FOSS FOSSILIFEROUS V VERY	FROM CHIPS TO SEVERAL INCHES IN SIZE BY MODERATE BLOWS OF A PICK POINT. SMALL, THIN PIECES CAN BE BROKEN BY FINGER PRESSURE.	OF STRATUM AND EXPRESSED AS A PERCENTAGE.
(SAT.) FROM BELOW THE GROUND WATER TABLE	FRAC FRACTURED VST - VANE SHEAR TEST	VERY CAN BE CARVED WITH KNIFE. CAN BE EXCAVATED READILY WITH POINT OF PICK, PIECES 1 INCH	STRATA ROCK QUALITY DESIGNATION (S.R.O.D.) - A MEASURE OF ROCK QUALITY DESCRIBED BY:
PLASTIC LIQUID LIMIT	FRAGS FRAGMENTS MED MEDIUM	SOFT OR MORE IN THICKNESS CAN BE BROKEN BY FINGER PRESSURE, CAN BE SCRATCHED READILY BY FINGERNAIL.	TOTAL LENGTH OF ROCK SEGMENTS WITHIN A STRATUM EDUAL TO OR GREATER THAN 10 CENTIMETERS DIVIDED BY THE TOTAL LENGTH OF STRATA AND EXPRESSED AS A PERCENTAGE.
RANGE SEMISOLID; REDUIRES DRYING TO ATTAIN OPTIMUM MOISTURE	EQUIPMENT USED ON SUBJECT PROJECT	FRACTURE SPACING BEDDING	TOPSOIL (T.S.) - SURFACE SOILS USUALLY CONTAINING ORGANIC MATTER.
(PI) PL PLASTIC LIMIT	THE TYPE	TERM SPACING TERM THICKNESS	BENCH MARK: B.M. #1: RR SPIKE IN UTILITY POLE
ON OPTIMUM MOISTURE - MOIST - (M) SOLID; AT OR NEAR OPTIMUM MOISTURE	DRILL UNITS: ADVANCING TOOLS: HAMMER TYPE: X AUTOMATIC MANU	VERY THICKLY PENDED > 4 FEET	126.42' LEFT OF STA. 16+28.03 -L-
OM OPTIMUM MOISTURE - MOIST - (M) SOLID; AT OR NEAR OPTIMUM MOISTURE SL SHRINKAGE LIMIT	MOBILE B- CLAY BITS	WIDE STOTE FEET THAT Y DEDOCT ON A SECURIT	ELEVATION: 2160.38
REQUIRES ADDITIONAL WATER TO	6° CONTINUOUS FLIGHT AUGER CORE SIZE:	CLOSE 016 TO 1 EFET VERY THINLY BEDDED 0.03 - 0.16 FEET	
- DRY - (D) ATTAIN OPTIMUM MOISTURE	BK-51 8* HOLLOW AUGERS -8	VERY CLOSE LESS THAN 0.16 FEET THICKLY LAMINATED 0.008 - 0.03 FEET THINLY LAMINATED < 0.008 FEET	NOTES:
PLASTICITY	-	INDURATION	1
PLASTICITY INDEX (PI) DRY STRENGTH		FOR SEDIMENTARY ROCKS, INDURATION IS THE HARDENING OF THE MATERIAL BY CEMENTING, HEAT, PRESSURE, ETC.	1
NONPLASTIC 0-5 VERY LOW	X CME-5500 TUNGCARBIDE INSERTS	FRIABLE RUBBING WITH FINGER FREES NUMEROUS GRAINS:	
LOW PLASTICITY 6-15 SLIGHT	X CASING X W/ ADVANCER HAND TOOLS:	GENTLE BLOW BY HAMMER DISINTEGRATES SAMPLE.	
MED. PLASTICITY 16-25 MEDIUM HIGH PLASTICITY 26 OR MORE HIGH	PORTABLE HOIST TRICONE STEEL TEETH POST HOLE DIGGER	MODERATELY INDURATED GRAINS CAN BE SEPARATED FROM SAMPLE WITH STEEL PROBE;	
COLOR	OTHER TRICONE TUNGCARB. HAND AUGER	BREAKS EASILY WHEN HIT WITH HAMMER.	
	OTHER SOUNDING ROD	INDURATED GRAINS ARE DIFFICULT TO SEPARATE WITH STEEL PROBE; DIFFICULT TO BREAK WITH HAMMER.	
DESCRIPTIONS MAY INCLUDE COLOR OR COLOR COMBINATIONS (TAN, RED, YEL-BRN, BLUE-GRAY) MODIFIERS SUCH AS LIGHT, DARK, STREAKED, ETC. ARE USED TO DESCRIBE APPEARANCE.	OTHER OTHER VANE SHEAR TEST		
	OTHER	EXTREMELY INDURATED SHARP HAMMER BLOWS REQUIRED TO BREAK SAMPLE; SAMPLE BREAKS ACROSS GRAINS.	

ID S B-4161

STATE PROJECT NO. SHEET NO. TOTAL SHEETS

33509.1.1



STATE OF NORTH CAROLINA DEPARTMENT OF TRANSPORTATION

MICHAEL F. EASLEY
GOVERNOR

LYNDO TIPPETT SECRETARY

February 7, 2006

STATE PROJECT:

33509.1.1 (B-4161)

F. A. PROJECT:

BRZ-1132(6)

COUNTY:

Jackson

DESCRIPTION:

Bridge No. 211 on SR-1132 over West Fork Tuckasegee River

SUBJECT:

Geotechnical Report – Foundation Investigation

Project Description

This project is located in central Jackson County, approximately 1.5 miles south of the Tuckasegee community. The replacement structure is to be located 25± feet south of the existing crossing. Proposed is a 124-foot single-span steel girder bridge erected on a 90-degree skew. The recommended roadway width is 22 feet.

The subsurface investigation was conducted during December 2005, utilizing a CME-550 ORV drill unit equipped with an automatic drive hammer for Standard Penetration Testing (SPT). The tests were performed through NX casing. NXWL rock coring apparatus was used to retrieve rock specimens at Boring EB2-A. Representative soil samples were collected and submitted to the Materials and Tests Unit for quality analysis.

Physiography and Geology

This project is located in the Blue Ridge Belt of the Mountain Physiographic Province. The area is underlain by a Late Proterozoic Era muscovite biotite gneiss unit of the Ashe Metamorphic Suite and Tallulah Falls Formation. This unit has been intruded by a Devonian Age quartz diorite pluton. The contact with this intrusion was evidenced in Boring EB2-A.

MAILING ADDRESS: NC DEPARTMENT OF TRANSPORTATION GEOTECHNICAL ENGINEERING UNIT 1589 MAIL SERVICE CENTER RALEIGH NC 27699-1589 TELEPHONE: 919-250-4088 FAX: 919-250-4237

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LOCATION: CENTURY CENTER COMPLEX BUILDING B 1020 BIRCH RIDGE DRIVE Topography in the area is steep, however local relief at the bridge site is approximately 40 feet. Land use is residential and commercial.

The West Fork Tuckasegee River is part of the Little Tennessee River Basin. Its confluence with the Tuckasegee River is approximately 1 mile north of the project area. Locally, the river is fast flowing and has developed a floodplain of ±400 feet. The channel bed is composed of silt, sand, gravel, and cobbles.

Foundation Materials

End Bent One

Boring EB1-B revealed ±9 feet of alluvial loose silty sand deposited over saprolite. The saprolite horizon is composed of medium to very dense silty sand and is approximately 16 feet thick. Saprolite grades into weathered rock at Elevation 2131± feet. Ten feet of weathered rock was penetrated before the contact with hard rock at Elevation 2121± feet.

End Bent Two

Embankment composed of 10± feet of soft to medium stiff sandy clayey silt has been placed over alluvium at Boring EB2-A. Alluvium occurs as surficial soil at location EB2-B. This deposit consists of soft silt and loose sand with gravel and cobbles. Saprolitic sand lies beneath the alluvium at EB2-A and laterally grades into weathered rock at EB2-B. Both of these layers are less than 4 feet thick, with the top of weathered rock lying at Elevation 2146±. Hard rock elevations across the bent varies from ±2138 to ±2142 feet. Retrieved rock core from Boring EB2-A yielded average Recovery and Rock Quality Designation (RQD) values of 86% and 57% respectively.

Groundwater

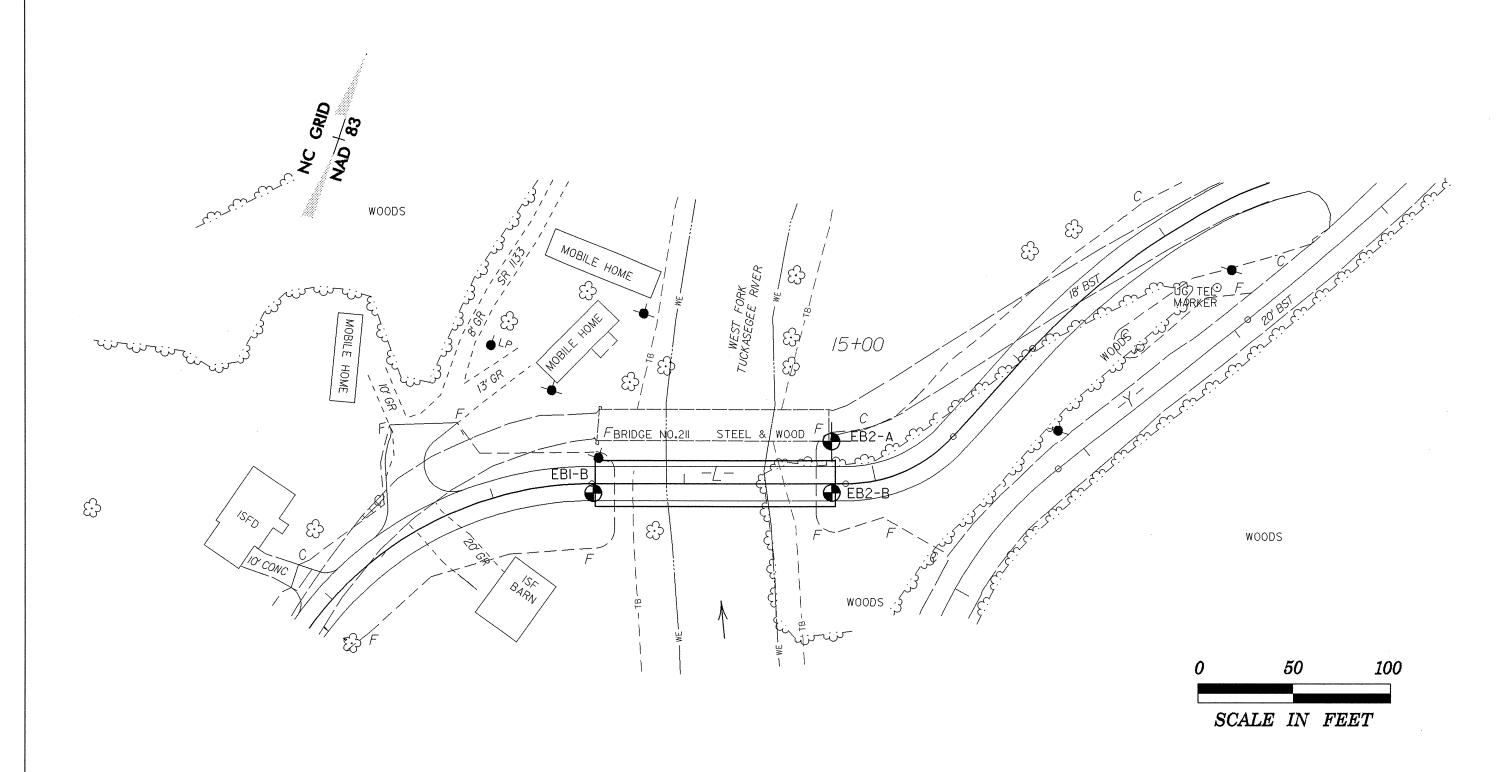
Groundwater elevations were measured in all borings during a period of normal rainfall. Elevations ranged from 2147± to 2151± feet. All measurements were recorded in the alluvial horizon.

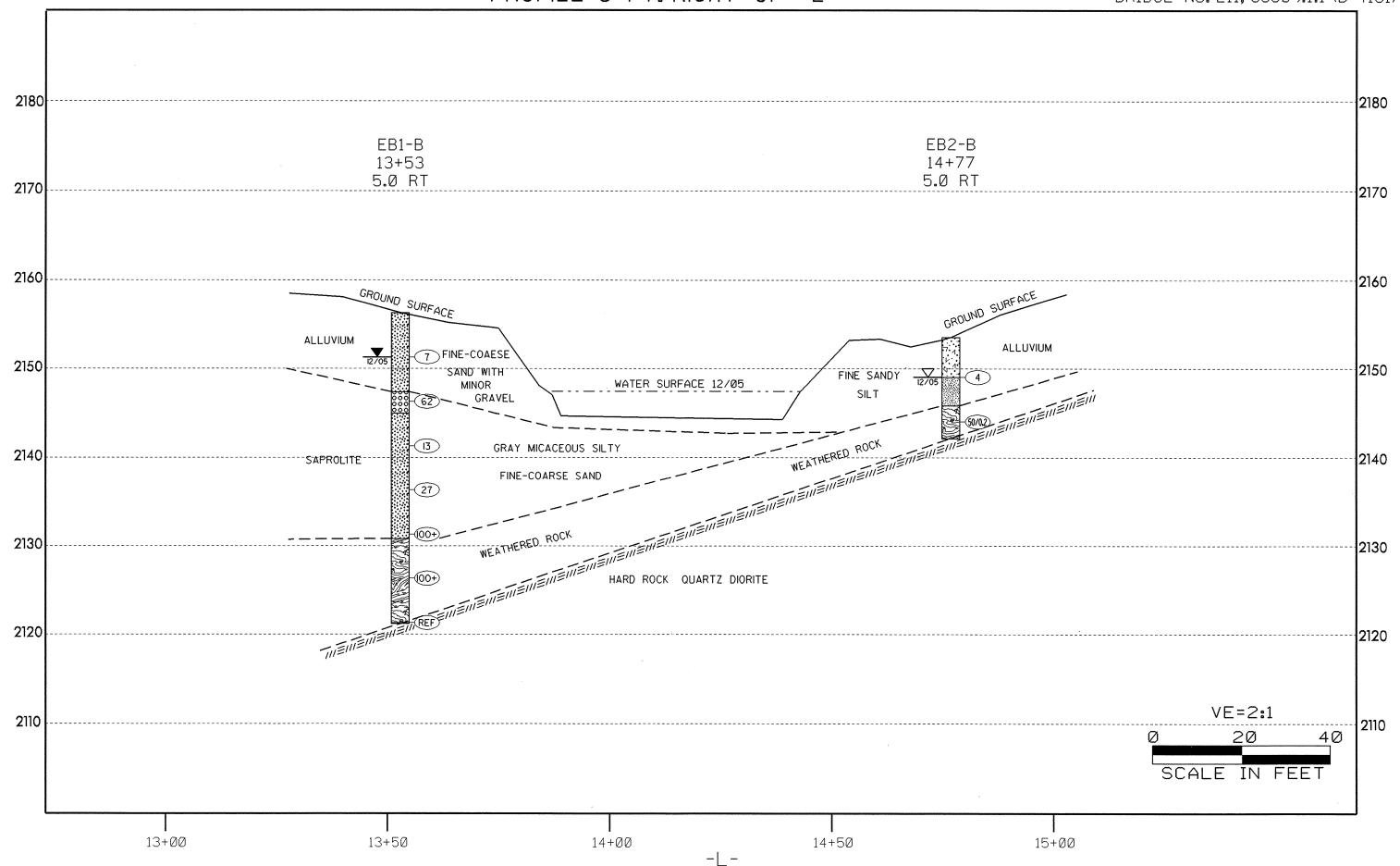
Respectfully Submitted,

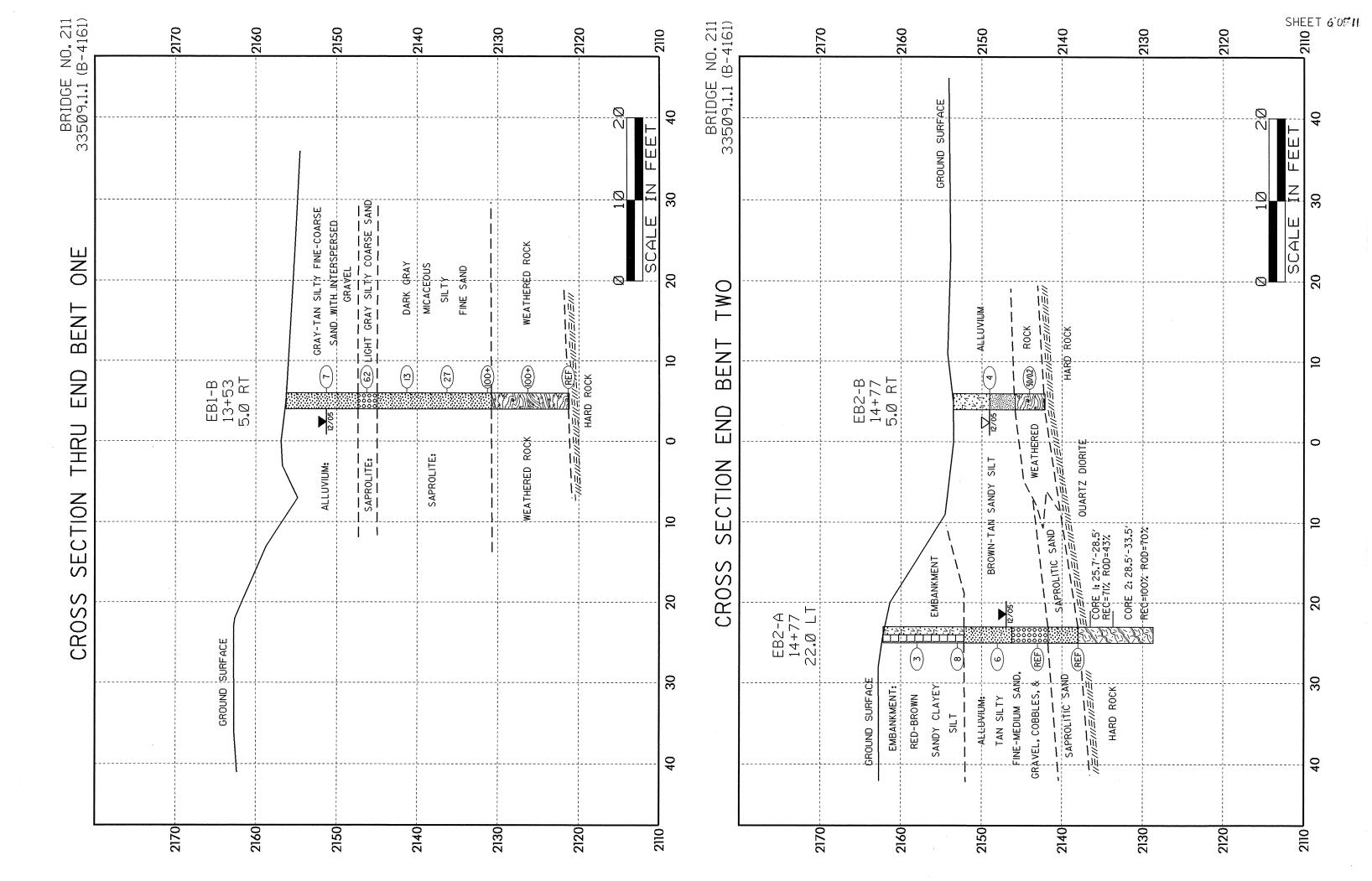
John W. Mann, LG

Project Engineering Geologist

BRIDGE NO. 211 SITE PLAN







NORTH CAROLINA DEPARTMENT OF TRANSPORTATION GEOTECHNICAL UNIT BORING LOG

	GEOTECHNICAL UNIT BORING LOG							ICAL L	JNIT B	ORING	LOG			
PROJECT	NO 3350	9.1.1			ID B-4	161	COU	NTY JAC	CKSON		GEO	LOC	GIST T.B. DANIEL	
SITE DES	CRIPTION	N BRI	DGE	NO. 2	211 ON	I SR-1132	OVER V	N. FORK	TUCKAS	EGEE RIV	ER		ut.	GND WATER
BORING 1	NO EB1-E	}		1	NORT	HING 0.0	0			EASTING	0.00			0 HR N/A
ALIGNMI	ENT -L-				BORIN	G LOCAT	TION 13	+53.000		OFFSET	5.00ft F	24 HR 5.00ft		
COLLAR	ELEV 21	6.29	t		TOTAI	L DEPTH	35.10ft		START DA	ATE 12/21	/05		COMPLETION D.	ATE 12/21/05
DRILL MA	ACHINE (CME-	550				DRILL	METHO	SPT CO	ORE BORIN	٧G		HAMMER TYPE	AUTOMATIC
SURFACE	E WATER								K 35.10f				Log EB1-B, Page 1 of 1	
ELEV	DEPTH		OW (PEN			PER FOC		SAMPLE	\mathbf{Y}	L		D ROCK
		6in	6in	6in	(ft)	0 2	25 (50	75 10	NO NO	MOI	Ō G	DESCR	RIPTION
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-	F													
2156.29	0.00	2	2	2	1.0		-Ground	Surface						
						X4							ALLUVIUM: GR	AY-TAN SILTY
	E ·				200	-t====		122222					FINE TO COAR	SE SAND WITH
	5.00	5	4	3	1.0	-1-7		1-2			▼		INTERSPERS	SED GRAVEL
2150.00						-X				SS-4				
-	F						-						-	
-	10.00	32	28	34	1.0			62		SS-5		0000	SAPROLITE: LIG	
	_										1.		COARS	
-	15.00	2	5	9	1.0								SAPROLITE:	
2140.00_	- 10.00	_		ľ	'."	X				SS-6			MICACEOUS SI	LIT FINE SAND
	_													
	20.00	6	13	14	1.0		27	<u> </u>						
	_						X			SS-7			*	
-										+1.				
2130.00_	25.00	19	42	58	0.9				100			2 =		
-	_								2				WEATHER	RED ROCK
_	30.00	35	65		0.4		,		100-					
	- 00.00			Švi i	"				<u> </u>	1				
-														
2121.19	35.00	60			0.1				60			\$		
						BORING	TERMIN	ATED-A	FELEV.				\ HARD	ROCK /
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NORTH CAROLINA DEPARTMENT OF TRANSPORTATION GEOTECHNICAL UNIT BORING LOG

						GEOT	<u>ECHN</u>	ICAL (JNIT B	ORING	LOG			
PROJEC	T NO 3350	9.1.1		1	D B-4	161	COU	NTY JAC	CKSON		GEO	LOG	IST T.B. DANIEL	
SITE DES	SCRIPTION	I BRII	DGE I	NO. 2	11 ON	I SR-1132	OVER V	V. FORK	TUCKAS	EGÉE RIVI	ΞR			GND WATER
BORING	NO EB2-A			N	ORTI	HING 0.00)			EASTING	0.00			0 HR N/A
ALIGNM	ENT -L-			В	BORIN	G LOCAT	TION 14+	-77.000		OFFSET 2	22.00ft	LT		24 HR 15.30ft
COLLAR	ELEV 216	52.76f	t	T	OTAI	AL DEPTH 33.50ft START DATE 12/13/05 COMPLETION							COMPLETION D	ATE 12/14/05
DRILL M	IACHINE (CME-5	550			DRILL METHOD SPT CORE BORING HAMMER TYPE						AUTOMATIC		
	E WATER						DEPTH TO ROCK 24.20ft			t			Log EB2-A, Page 1 of 1	
ELEV	DEPTH	BL	OW (CT	PEN	E	BLOWS F	PER FOO)T	SAMPLE	Y/	الم	SOIL AN	ID ROCK
ELEV	DEFIN	6in	6in	6in	(ft)	0 2	25 5	50 1	75 10	NO 1	MOI	OG	DESCF	RIPTION
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2162.76				 	- 5			Sunace	 			-1-	EMBANKMENT	: RED-BROWN
2160.00_	<u> </u>											EI,		AYEY SILT
	<u>+</u> 4.20	2	1	2	1.0	3				SS-1				·
	+					X				33-1				
	Ŧ			_	ا ، ا									
	+ 9.20	3	3	5	1.0	1-7-8								
2150.00_	#					1-1								SILTY FINE TO
2150.00_	 14.20	2	2	4	1.0	1-1-6							MEDIO	M SAND
	± '	_	-	i i		*				SS-2	▼ -			
	+											0000	ALLUVIAL GRAV	'EL & BOULDERS
	干 19.20	60			0.0				60	ll .		0000		
	‡											0000	SAPROL	TIC CAND
2140.00_	‡												SAPROL	TIC SAND
	± 24.20	60			0.0				>	K		No.	LIARD BOCK, O	LIADTZ DIODITE
	#									CORE 1			RUN 1: 25.7-2	UARTZ DIORITE 8.5' REC=71%
	±									CORE 2			RQD=43% RU	N 2: 28.5-33.5'
	土									CORE 2			REC=100%	% RQD=70%
2139:28-	+		,						-			網		
2129:26	F					DODING	TCDM	ATED-A						
	T					21	29:3' IN F	ARDRO	OCK					
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SHEET _1_ OE1_

							DATE 0/0/0000
				•	CC	RE	BORING REPORT
PROJE	CT: _	33509	.1.1	1. D. NO:	B-41	161	BORING NO:EB2-A GEOLOGIST:J.W. MANN
DESCR	IPTION:		BRIDO	SE NO. 65	ON S.R.	1760 C	VER NORTH MUDDY CREEK
COUNT	Y:	JACKSO	N		COLLA	R ELEV	ATION: <u>2162.8</u> FT. TOTAL DEPTH: <u>33.5</u> FT.
ELEV. (FEET)		DRILL RATE MIN./FT.	RUN (FFFT)	REC. FEET %	RQD. FEET %	SAMP.	FIELD CLASSIFICATION AND REMARKS
2137.1	25.7	WIN.// 1.	2.8	2.0 71	1.2 43		25.7-28.7': LIGHT GRAY-WHITE , HARD, SLIGHTLY WEATHERED CLOSELY FRACTURED QUARTZ DIORITE STRINGER.
2134.3	28.5	``	5.0	5.0	3.5 70		28.7-33.5': GRAY SLIGHTLY WEATHERED, HARD, CLOSELY FRACTURED MUSCOVITE-BIOTITE GNEISS.
2129.3	·						
	,		· -				CORING TERMINATED AT ELEVATION 2129.3 FT.
D	RILLER:	R.D. CHI	LDERS	<u> </u>		COR	E SIZE: <u>NXWL</u> EQUIPMENT: <u>CME-550</u>

NORTH CAROLINA DEPARTMENT OF TRANSPORTATION GEOTECHNICAL UNIT BORING LOG

8 OF 11

	GEOTECHNICAL UNIT BORING LOG															
PROJECT	r NO 3350	9.1.1			ID B-4	161	COU	NTY JA	CKSON			GEOI	OG	IST T.B. DANIEL		
SITE DES	CRIPTION	BRII	DGE N	10.2	211 ON	SR-1132	OVER \	N. FORI	K TUCKAS	EGEE	RIV	ER "			GND WATER	
BORING	NO EB2-B				NORTH	IING 0.00)			EAST	ING	0.00			0 HR N/A	
ALIGNM	ENT -L-				BORIN	G LOCAT	ION 14	+77.000		OFFS	SET S	5.00ft F	RT_		24 HR 4.50ft	
COLLAR	ELEV 215	3.54f	t		TOTAL	DEPTH	1 11.40ft START DATE 12/14/05							COMPLETION DATE 12/14/05		
	ACHINE (DRILL	METHO	DD SPT C	ORE B	ORIN	1G		HAMMER TYPE	AUTOMATIC	
	E WATER						DEPTH	TO RO	CK 9.50ft					Log EB2-B, Page 1 of 1		
			OW C	CT	PEN	E		PER FO		SAM	IPLE	Y /	۲		D ROCK	
ELEV	DEPTH	6in	6in	6in	(ft)	0 2	5	50	75 10	N N	0 1	MOI	Ğ	DESCR	RIPTION	
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	‡															
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	1															
2153.54	±						-Ground	Surfac	e				·			
2100.01										S	-1		, 	ALLUVIUM: TA	N SANDY SILT	
2150.00_	<u>+</u>								· - :			Y	, , ;			
	4.50	1	2	2	1.0	4		-		ll s	S-3	—		ALLUVIUM BRO	WN SANDY SILT	
	Ŧ					件						į		ALLOVIOW. BIXO	VVIV O/ (IND) OIL!	
	士 9.50	50			0.1				50	U				WEATHER	RED ROCK	
2142.14	± 3.50	30			0.1					Ň						
2172.17						BORING	-TERMI	NATED	AF ELEV.							
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NORTH CAROLINA DEPARTMENT OF TRANSPORTATION DIVISION OF HIGHWAYS-MATERIALS AND TESTS UNIT SOILS TEST REPORT-SOILS LABORATORY

T.I.P. ID#:	B-4161								
	ı								
REPORT ON SA	MPLES OF:	Soils for Clas	ssification						
				,					
PROJECT:	33509.1.	.1	COUNTY:	Jackson			Owner:		
DATE SAMPLE	D: 12.21.05	DA'	TE RECEIVE	ED: 12.2	8.05		DATE REI	PORTED:	1.5.06
SAMPLED FRO			SA	MPLED B	Y :	T. Dani	el / J. W. M:	ann	
SUBMITTED BY		rye				2002	STAN	DARD SPE	CIFICATION
LABORATORY		K						-	

TEST RESULTS

Project Sample No.	S-1	SS-1	SS-2	SS-3	SS-4	SS-5	SS-6	SS-7
Lab Sample No. A	151390	151391	151392	151393	151394	151395	151396	151397
HiCAMS Sample #								
Retained #4 Sieve %								
Passing #10 Sieve %	96	90	98	99	91	57	98	99
Passing #40 Sieve %	88	70	84	92	57	41	85	93
Passing #200 Sieve %	63	39	14	36	23	10	12	· 16

MINUS #10 FRACTION

Soil Mortar - 100%								
Coarse Sand -Ret. #60	16	34	45	25	52	49	42	29
Fine Sand - Ret. #270	22	28	44	45	26	38	50	63
Silt 0.05-0.005 mm %	40	12	7	14	8	9	6	6
Clay < 0.005 mm %	22	26	4	16	14	4	2	2
Passing # 40 Sieve %								
Passing # 200 Sieve %							~	

Liquid Limit	68	42	27	33	29	22	42	43
Plastic Index	NP	NP	NP	NP	NP	NP	NP	NP
ASHTO Classification	A-5 (9)	A-5 (1)	A-2-4 (0)	A-4 (0)	A-2-4 (0)	A-1-b (0)	A-2-5 (0)	A-2-5 (0)
Quantity								
Texture								
Station	14+77	14+77	14+77	14+77	13+53	13+53	13+53	13+53
Hole No.								
Depth (ft) From:	0.0	4.2	14.2	4.5	5.0	10.0	15.0	20.0
To:	2.0	5.7	15.7	6.0	6.5	11.5	16.5	21.5

Remarks:

A-151390 - 151397

CC:

	· · · · · · · · · · · · · · · · · · ·
J. W. Mann	
File	

SOILS ENGINEER:

FIELD SCOUR REPORT

SHEET 1

PROJECT:	33509.1.1	ID:	B-4161	COUNTY: JACKSON	

DESCRIPTION(1): BRIDGE NO. 211 ON WEST FORK TUCKASEGEE RIVER

	EX	ISTING BRIDGE	-		
Information from:	Field Inspection X Other (explain) HYDR		(reel po	os:) ·	
Bridge No.: 211 Foundation Type:	Length: 123 Total	Bents: 4 Bents	s in Channel: 0	Bents in Floodplain:	4
EVIDENCE OF SCOUP Abutments or End Be	R(2) nt Slopes: NONE NOTED				
Interior Bents: NONE	NOTED				
Channel Bed: NONE	NOTED				
Channel Bank: MINO	R UNDERMINING UPSTR	EAM OF END BEN	T ONE		
EXISTING SCOUR PRO Type(3): ASPH					
Extent(4): PLAC	ED ON BOTH END BENT	SLOPES			
Effectiveness(5): MINIM	I AL				-
Obstructions(6): NONE					

INSTRUCTIONS

- 1 Describe the specific site's location, including route number and body of water crossed.
- 2 Note scour evidence at existing end bents or abutments (e.g. undermining, sloughing, degradations).
- 3 Note existing scour protection (e.g. rip rap).
- 4 Describe extent of existing scour protection.
- 5 Describe whether or not the scour protection appears to be working.
- 6 Note obstructions such as dams, fallen trees, debris at bents, etc.
- 7 Describe the channel bed material based on observation and/or samples. Include any lab results with report.
- 8 Describe the channel bank material based on observation and/or samples. Include any lab results with report.
- **9** Describe the material covering the banks (e.g. grass, trees, rip rap, none).
- 10 Determine the approximate floodplain width from field observation or a topographic map.
- 11 Describe the material covering the floodplain (e.g. grass, trees, crops).
- 12 Use professional judgement to specify if the stream is degrading, aggrading, or static.
- 13 Describe potential and direction of the stream to migrate laterally during the bridge's life (approx. 100 years).
- Give the geotechnically adjusted scour elevation (GASE) expected over the life of the bridge (approx. 100 years). This elevation can be given as a range across the site, or for each bent. Discuss the relationship between the Hydraulics Unit theoritical scour and the GASE. If the GASE is dependent on scour counter measures, explain (e.g. rip rap armoring on slopes). The GASE is based on the erodability of materials, giving consideration to the influence of joints, foliation, bedding characteristics, % core recovery, % RQD, differential weathering, shear strength, observations at existing structures, other tests deemed appropriate, and overall geologic conditions at the site.

SHEET 10 OF 11	
ION	
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Channal	Ded Meterial(7)	CAND C			CDDL EC		!				
Channe	Bed Material(7):	,	The second secon	AVEL, C							
Channel I	Bank Material(8):	ALLUVIA	AL SILT &	& GRAVI	ΞL	,			minima and a superior of		nort seeminkokokokoke ili alkida han
Channe	el Bank Cover(9):	TREES,	GRASS							is an an one of the	an market and a state of the st
Floor	dplain Width(10):	~400'				•					
Flood	dplain Cover(11):	GRASS,	RESIDE	ENCES			j.				
	Stream is(12):	. Ag	grading		Degra	ading _	X	(Static		٠
nnel Migratio	on Tendency(13):	: TOWAR	D END E	BENT ON	IE	nde gan a s'angger a solution de minerales (de la			A-14 - 100 S-1000 S-1000 S-1000 T-14 T-15	kannan kanan sa sa makan pengaban As	tiel die ein engele eksenderen onderhole des kiel der
	and Other Comn										
SEOTECHNIC	CALLY ADJUST	ED SCOL	JR ELEV	/ATIONS	6(14)	Fee	t	М	eters		
	BENTS B1	B2	В3	В4							
SB L	anes, Lt	T		D4			T	T	T	T	T
	anes, Rt										
	anes, Lt	 									ļ
NB La	anes, Rt		THE MENTS I STREET, IN A PRINCIPAL OF A P		· January some service reserv	* **********					
	e entre de la contra del contra de la contra del la con		PROFESSION STANDARDS TO	TO THE RESERVE AND ADDRESS OF THE PARTY OF T	LLATA MATE MINNAME TO LAMP A TATOL FOR THE P			w			ļ
/A: PROPO	of GASE to Hydra	PAN REPL	_ACEME	NT BRID)GE	BAATE	DIAI				
Bed or Bank	SIS RESULTS F	ROW CHA	ANNEL	BED AN	D BANK	WATE	KIAL	т-			
Retained #4											
Retained #4 Passed #10											
Retained #4 Passed #10 Passed #40											
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Retained #4 Passed #10 Passed #40 assed #200 oarse Sand											
Retained #4 Passed #10 Passed #40 assed #200 oarse Sand Fine Sand											
Retained #4 Passed #10 Passed #40 assed #200 oarse Sand Fine Sand Silt											
Retained #4 Passed #10 Passed #40 assed #200 oarse Sand Fine Sand Silt Clay											
Retained #4 Passed #10 Passed #40 assed #200 oarse Sand Fine Sand Silt Clay LL											
Silt Clay											
Retained #4 Passed #10 Passed #40 assed #200 coarse Sand Fine Sand Silt Clay LL PI											
Retained #4 Passed #10 Passed #40 assed #200 coarse Sand Fine Sand Clay LL PI AASHTO											

eported by:

Date: 1/18/2006

33509.1.1 B.4161

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JACK G.

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