

PROJECT: 33715.1.1 ID: B-4466

STATE OF NORTH CAROLINA
DEPARTMENT OF TRANSPORTATION
DIVISION OF HIGHWAYS
GEOTECHNICAL ENGINEERING UNIT

STATE	STATE PROJECT REFERENCE NO.	SHEET NO.	TOTAL SHEETS
N.C.	33715.1.1 (B-4466)	1	20

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STRUCTURE
SUBSURFACE INVESTIGATION

PROJ. REFERENCE NO. 33715.1.1 (B-4466) F.A. PROJ. BRZ-1104(II)

COUNTY CLAY

PROJECT DESCRIPTION BRIDGE NO. 4 ON SR-1104
OVER BRASSTOWN CREEK

SITE DESCRIPTION

CAUTION NOTICE

THE SUBSURFACE INFORMATION AND THE SUBSURFACE INVESTIGATION ON WHICH IT IS BASED WERE MADE FOR THE PURPOSE OF STUDY, PLANNING, AND DESIGN, AND NOT FOR CONSTRUCTION OR PAY PURPOSES. THE VARIOUS FIELD BORING LOGS, ROCK CORES, AND SOIL TEST DATA AVAILABLE MAY BE REVIEWED OR INSPECTED IN RALEIGH BY CONTACTING THE N. C. DEPARTMENT OF TRANSPORTATION, GEOTECHNICAL ENGINEERING UNIT AT (919) 250-4088. NEITHER THE SUBSURFACE PLANS AND REPORTS, NOR THE FIELD BORING LOGS, ROCK CORES, OR SOIL TEST DATA ARE PART OF THE CONTRACT.

GENERAL SOIL AND ROCK STRATA DESCRIPTIONS AND INDICATED BOUNDARIES ARE BASED ON A GEOTECHNICAL INTERPRETATION OF ALL AVAILABLE SUBSURFACE DATA AND MAY NOT NECESSARILY REFLECT THE ACTUAL SUBSURFACE CONDITIONS BETWEEN BORINGS OR BETWEEN SAMPLED STRATA WITHIN THE BOREHOLE. THE LABORATORY SAMPLE DATA AND THE IN SITU (IN-PLACE) TEST DATA CAN BE RELIED ON ONLY TO THE DEGREE OF RELIABILITY INHERENT IN THE STANDARD TEST METHOD. THE OBSERVED WATER LEVELS OR SOIL MOISTURE CONDITIONS INDICATED IN THE SUBSURFACE INVESTIGATIONS ARE AS RECORDED AT THE TIME OF THE INVESTIGATION. THESE WATER LEVELS OR SOIL MOISTURE CONDITIONS MAY VARY CONSIDERABLY WITH TIME ACCORDING TO CLIMATIC CONDITIONS INCLUDING TEMPERATURES, PRECIPITATION, AND WIND, AS WELL AS OTHER NON-CLIMATIC FACTORS.

THE BIDDER OR CONTRACTOR IS CAUTIONED THAT DETAILS SHOWN ON THE SUBSURFACE PLANS ARE PRELIMINARY ONLY AND IN MANY CASES THE FINAL DESIGN DETAILS ARE DIFFERENT. FOR BIDDING AND CONSTRUCTION PURPOSES, REFER TO THE CONSTRUCTION PLANS AND DOCUMENTS FOR FINAL DESIGN INFORMATION ON THIS PROJECT. THE DEPARTMENT DOES NOT WARRANT OR GUARANTEE THE SUFFICIENCY OR ACCURACY OF THE INVESTIGATION MADE, NOR THE INTERPRETATIONS MADE, OR OPINION OF THE DEPARTMENT AS TO THE TYPE OF MATERIALS AND CONDITIONS TO BE ENCOUNTERED. THE BIDDER OR CONTRACTOR IS CAUTIONED TO MAKE SUCH INDEPENDENT SUBSURFACE INVESTIGATIONS AS HE DEEMS NECESSARY TO SATISFY HIMSELF AS TO CONDITIONS TO BE ENCOUNTERED ON THIS PROJECT. THE CONTRACTOR SHALL HAVE NO CLAIM FOR ADDITIONAL COMPENSATION OR FOR AN EXTENSION OF TIME FOR ANY REASON RESULTING FROM THE ACTUAL CONDITIONS ENCOUNTERED AT THE SITE DIFFERING FROM THOSE INDICATED IN THE SUBSURFACE INFORMATION.

PERSONNEL

T B DANIEL

C J COFFEY

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INVESTIGATED BY C A DUNNAGAN

CHECKED BY W D FRYE, Jr

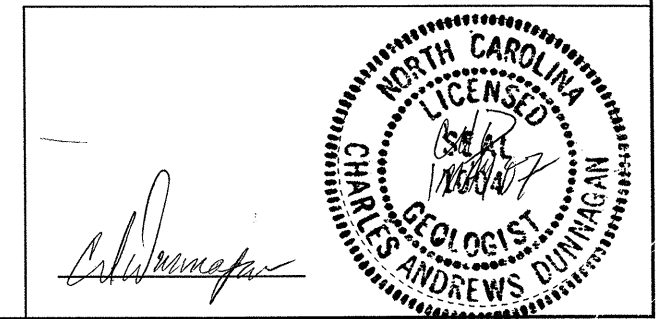
SUBMITTED BY W D FRYE, Jr

DATE MAY 2007

DRAWN BY: C A DUNNAGAN

NOTE - THE INFORMATION CONTAINED HEREIN IS NOT IMPLIED OR GUARANTEED BY THE N. C. DEPARTMENT OF TRANSPORTATION AS BEING ACCURATE NOR IT IS CONSIDERED TO BE PART OF THE PLANS, SPECIFICATIONS, OR CONTRACT FOR THE PROJECT.

NOTE - BY HAVING REQUESTED THIS INFORMATION THE CONTRACTOR SPECIFICALLY WAIVES ANY CLAIMS FOR INCREASED COMPENSATION OR EXTENSION OF TIME BASED ON DIFFERENCES BETWEEN THE CONDITIONS INDICATED HEREIN AND THE ACTUAL CONDITIONS AT THE PROJECT SITE.





NORTH CAROLINA DEPARTMENT OF TRANSPORTATION
DIVISION OF HIGHWAYS
GEOTECHNICAL ENGINEERING UNIT

PROJECT REFERENCE NO. 33715.1.1 (B-4466) SHEET NO. 2/20

SUBSURFACE INVESTIGATION

SOIL AND ROCK LEGEND, TERMS, SYMBOLS, AND ABBREVIATIONS

SOIL DESCRIPTION										GRADATION										ROCK DESCRIPTION										TERMS AND DEFINITIONS																													
SOIL IS CONSIDERED TO BE THE UNCONSOLIDATED, SEMI-CONSOLIDATED, OR WEATHERED EARTH MATERIALS THAT CAN BE PENETRATED WITH A CONTINUOUS FLIGHT POWER AUGER, AND YIELD LESS THAN 100 BLOWS PER FOOT ACCORDING TO STANDARD PENETRATION TEST (AASHTO T206, ASTM D-1586). SOIL CLASSIFICATION IS BASED ON THE AASHTO SYSTEM. BASIC DESCRIPTIONS GENERALLY SHALL INCLUDE: CONSISTENCY, COLOR, TEXTURE, MOISTURE, AASHTO CLASSIFICATION, AND OTHER PERTINENT FACTORS SUCH AS MINERALOGICAL COMPOSITION, ANGULARITY, STRUCTURE, PLASTICITY, ETC. EXAMPLE: <i>VERY STIFF, GRAY-SILTY CLAY, MOIST WITH INTERBEDDED FINE SAND LAYERS, HIGH PLASTIC, A-7-6</i>										WELL GRADED - INDICATES A GOOD REPRESENTATION OF PARTICLE SIZES FROM FINE TO COARSE. UNIFORM - INDICATES THAT SOIL PARTICLES ARE ALL APPROXIMATELY THE SAME SIZE. (ALSO POORLY GRADED) GAP-GRADED - INDICATES A MIXTURE OF UNIFORM PARTICLES OF TWO OR MORE SIZES. THE ANGULARITY OR ROUNDNESS OF SOIL GRAINS IS DESIGNATED BY THE TERMS: ANGULAR, SUBANGULAR, SUBROUNDED, OR ROUNDED.										HARD ROCK IS NON-COASTAL PLAIN MATERIAL THAT IF TESTED, WOULD YIELD SPT REFUSAL. AN INFERRED ROCK LINE INDICATES THE LEVEL AT WHICH NON-COASTAL PLAIN MATERIAL WOULD YIELD SPT REFUSAL. SPT REFUSAL IS PENETRATION BY A SPLIT SPOON SAMPLER EQUAL TO OR LESS THAN 0.1 FOOT PER 60 BLOWS. IN NON-COASTAL PLAIN MATERIAL, THE TRANSITION BETWEEN SOIL AND ROCK IS OFTEN REPRESENTED BY A ZONE OF WEATHERED ROCK. ROCK MATERIALS ARE TYPICALLY DIVIDED AS FOLLOWS: WEATHERED ROCK (WR)  NON-COASTAL PLAIN MATERIAL THAT WOULD YIELD SPT N VALUES > 100 BLOWS PER FOOT IF TESTED. CRYSTALLINE ROCK (CR)  FINE TO COARSE GRAIN IGNEOUS AND METAMORPHIC ROCK THAT WOULD YIELD SPT REFUSAL IF TESTED. ROCK TYPE INCLUDES GRANITE, GNEISS, GABBRO, SCHIST, ETC. NON-CRYSTALLINE ROCK (NCR)  FINE TO COARSE GRAIN METAMORPHIC AND NON-COASTAL PLAIN SEDIMENTARY ROCK THAT WOULD YIELD SPT REFUSAL IF TESTED. ROCK TYPE INCLUDES PHYLLITE, SLATE, SANDSTONE, ETC. COASTAL PLAIN SEDIMENTARY ROCK (CP)  COASTAL PLAIN SEDIMENTS CEMENTED INTO ROCK, BUT MAY NOT YIELD SPT REFUSAL. ROCK TYPE INCLUDES LIMESTONE, SANDSTONE, CEMENTED SHELL BEDS, ETC.										ALLUVIUM (ALLUV.) - SOILS THAT HAVE BEEN TRANSPORTED BY WATER. AQUIFER - A WATER BEARING FORMATION OR STRATA. ARENACEOUS - APPLIED TO ROCKS THAT HAVE BEEN DERIVED FROM SAND OR THAT CONTAIN SAND. ARGILLACEOUS - APPLIED TO ALL ROCKS OR SUBSTANCES COMPOSED OF CLAY MINERALS, OR HAVING A NOTABLE PROPORTION OF CLAY IN THEIR COMPOSITION, AS SHALE, SLATE, ETC. ARTESIAN - GROUND WATER THAT IS UNDER SUFFICIENT PRESSURE TO RISE ABOVE THE LEVEL AT WHICH IT IS ENCOUNTERED, BUT WHICH DOES NOT NECESSARILY RISE TO OR ABOVE THE GROUND SURFACE. CALCAREOUS (CALC.) - SOILS THAT CONTAIN APPRECIABLE AMOUNTS OF CALCIUM CARBONATE. COLLUVIUM - ROCK FRAGMENTS MIXED WITH SOIL DEPOSITED BY GRAVITY ON SLOPE OR AT BOTTOM OF SLOPE. CORE RECOVERY (REC.) - TOTAL LENGTH OF ALL MATERIAL RECOVERED IN THE CORE BARREL DIVIDED BY TOTAL LENGTH OF CORE RUN AND EXPRESSED AS A PERCENTAGE. DIKE - A TABULAR BODY OF IGNEOUS ROCK THAT CUTS ACROSS THE STRUCTURE OF ADJACENT ROCKS OR CUTS MASSIVE ROCK. DIP - THE ANGLE AT WHICH A STRATUM OR ANY PLANAR FEATURE IS INCLINED FROM THE HORIZONTAL. DIP DIRECTION (DIP AZIMUTH) - THE DIRECTION OR BEARING OF THE HORIZONTAL TRACE OF THE LINE OF DIP, MEASURED CLOCKWISE FROM NORTH. FAULT - A FRACTURE OR FRACTURE ZONE ALONG WHICH THERE HAS BEEN DISPLACEMENT OF THE SIDES RELATIVE TO ONE ANOTHER PARALLEL TO THE FRACTURE. FISSILE - A PROPERTY OF SPLITTING ALONG CLOSELY SPACED PARALLEL PLANES. FLOAT - ROCK FRAGMENTS ON SURFACE NEAR THEIR ORIGINAL POSITION AND DISLODGED FROM PARENT MATERIAL. FLOOD PLAIN (FP) - LAND BORDERING A STREAM, BUILT OF SEDIMENTS DEPOSITED BY THE STREAM. FORMATION (FM) - A MAPPABLE GEOLOGIC UNIT THAT CAN BE RECOGNIZED AND TRACED IN THE FIELD. JOINT - FRACTURE IN ROCK ALONG WHICH NO APPRECIABLE MOVEMENT HAS OCCURRED. LEDGE - A SHELF-LIKE RIDGE OR PROJECTION OF ROCK WHOSE THICKNESS IS SMALL COMPARED TO ITS LATERAL EXTENT. LENS - A BODY OF SOIL OR ROCK THAT THINS OUT IN ONE OR MORE DIRECTIONS. MOTTLED (MOT.) - IRREGULARLY MARKED WITH SPOTS OF DIFFERENT COLORS. MOTTLING IN SOILS USUALLY INDICATES POOR AERATION AND LACK OF GOOD DRAINAGE. PERCHED WATER - WATER MAINTAINED ABOVE THE NORMAL GROUND WATER LEVEL BY THE PRESENCE OF AN INTERVENING IMPERVIOUS STRATUM. RESIDUAL (RES.) SOIL - SOIL FORMED IN PLACE BY THE WEATHERING OF ROCK. ROCK QUALITY DESIGNATION (RQD) - A MEASURE OF ROCK QUALITY DESCRIBED BY TOTAL LENGTH OF ROCK SEGMENTS EQUAL TO OR GREATER THAN 4 INCHES DIVIDED BY THE TOTAL LENGTH OF CORE RUN AND EXPRESSED AS A PERCENTAGE. SAPROLITE (SAP.) - RESIDUAL SOIL THAT RETAINS THE RELIC STRUCTURE OR FABRIC OF THE PARENT ROCK. SILL - AN INTRUSIVE BODY OF IGNEOUS ROCK OF APPROXIMATELY UNIFORM THICKNESS AND RELATIVELY THIN COMPARED WITH ITS LATERAL EXTENT, THAT HAS BEEN EMPLACED PARALLEL TO THE BEDDING OR SCHISTOSITY OF THE INTRUDED ROCKS. SLICKENSIDE - POLISHED AND STRIATED SURFACE THAT RESULTS FROM FRICTION ALONG A FAULT OR SLIP PLANE. STANDARD PENETRATION TEST (PENETRATION RESISTANCE) (SPT) - NUMBER OF BLOWS (IN OR BPF) OF A 140 LB. HAMMER FALLING 30 INCHES REQUIRED TO PRODUCE A PENETRATION OF 1 FOOT INTO SOIL WITH A 2 INCH OUTSIDE DIAMETER SPLIT SPOON SAMPLER. SPT REFUSAL IS PENETRATION EQUAL TO OR LESS THAN 0.1 FOOT PER 60 BLOWS. STRATA CORE RECOVERY (SCRC) - TOTAL LENGTH OF STRATA MATERIAL RECOVERED DIVIDED BY TOTAL LENGTH OF STRATUM AND EXPRESSED AS A PERCENTAGE. STRATA ROCK QUALITY DESIGNATION (SRQD) - A MEASURE OF ROCK QUALITY DESCRIBED BY TOTAL LENGTH OF ROCK SEGMENTS WITHIN A STRATUM EQUAL TO OR GREATER THAN 4 INCHES DIVIDED BY THE TOTAL LENGTH OF STRATA AND EXPRESSED AS A PERCENTAGE. TOPSOIL (TS) - SURFACE SOILS USUALLY CONTAINING ORGANIC MATTER.																													
SOIL LEGEND AND AASHTO CLASSIFICATION										MINERALOGICAL COMPOSITION										WEATHERING										MISCELLANEOUS SYMBOLS																													
GENERAL CLASS. GRANULAR MATERIALS (<= 35% PASSING #200) SILT-CLAY MATERIALS (> 35% PASSING #200) ORGANIC MATERIALS										MINERAL NAMES SUCH AS QUARTZ, FELDSPAR, MICA, TALC, KAOLIN, ETC. ARE USED IN DESCRIPTIONS WHENEVER THEY ARE CONSIDERED OF SIGNIFICANCE.										FRESH ROCK GENERALLY FRESH, FEW JOINTS MAY SHOW SLIGHT STAINING. ROCK RINGS UNDER HAMMER IF CRYSTALLINE. VERY SLIGHT (V SLI.) ROCK GENERALLY FRESH, JOINTS STAINED, SOME JOINTS MAY SHOW THIN CLAY COATINGS IF OPEN. CRYSTALS ON A BROKEN SPECIMEN FACE SHINE BRIGHTLY. ROCK RINGS UNDER HAMMER BLOWS IF OF A CRYSTALLINE NATURE. SLIGHT (SLI.) ROCK GENERALLY FRESH, JOINTS STAINED AND DISCOLORATION EXTENDS INTO ROCK UP TO 1 INCH. OPEN JOINTS MAY CONTAIN CLAY. IN GRANITOID ROCKS SOME OCCASIONAL FELDSPAR CRYSTALS ARE DULL AND DISCOLORED. CRYSTALLINE ROCKS RING UNDER HAMMER BLOWS. MODERATE (MOD.) SIGNIFICANT PORTIONS OF ROCK SHOW DISCOLORATION AND WEATHERING EFFECTS. IN GRANITOID ROCKS, MOST FELDSPARS ARE DULL AND DISCOLORED, SOME SHOW CLAY. ROCK HAS DULL SOUND UNDER HAMMER BLOWS AND SHOWS SIGNIFICANT LOSS OF STRENGTH AS COMPARED WITH FRESH ROCK. MODERATELY SEVERE (MOD. SEV.) ALL ROCK EXCEPT QUARTZ DISCOLORED OR STAINED. IN GRANITOID ROCKS, ALL FELDSPARS DULL AND DISCOLORED AND A MAJORITY SHOW KAOLINIZATION. ROCK SHOWS SEVERE LOSS OF STRENGTH AND CAN BE EXCAVATED WITH A GEOLOGIST'S PICK. ROCK GIVES "CLUNK" SOUND WHEN STRUCK. <i>IF TESTED, WOULD YIELD SPT REFUSAL</i> SEVERE (SEV.) ALL ROCK EXCEPT QUARTZ DISCOLORED OR STAINED. ROCK FABRIC CLEAR AND EVIDENT BUT REDUCED IN STRENGTH TO STRONG SOIL. IN GRANITOID ROCKS ALL FELDSPARS ARE KAOLINIZED TO SOME EXTENT. SOME FRAGMENTS OF STRONG ROCK USUALLY REMAIN. <i>IF TESTED, YIELDS SPT N VALUES > 100 BPF</i> VERY SEVERE (V SEV.) ALL ROCK EXCEPT QUARTZ DISCOLORED OR STAINED. ROCK FABRIC ELEMENTS ARE DISCERNIBLE BUT THE MASS IS EFFECTIVELY REDUCED TO SOIL STATUS, WITH ONLY FRAGMENTS OF STRONG ROCK REMAINING. SAPROLITE IS AN EXAMPLE OF ROCK WEATHERED TO A DEGREE SUCH THAT ONLY MINOR VESTIGES OF THE ORIGINAL ROCK FABRIC REMAIN. <i>IF TESTED, YIELDS SPT N VALUES < 100 BPF</i> COMPLETE ROCK REDUCED TO SOIL. ROCK FABRIC NOT DISCERNIBLE, OR DISCERNIBLE ONLY IN SMALL AND SCATTERED CONCENTRATIONS. QUARTZ MAY BE PRESENT AS DIKES OR STRINGERS. SAPROLITE IS ALSO AN EXAMPLE.										SLIGHTLY COMPRESSIBLE LIQUID LIMIT LESS THAN 31 MODERATELY COMPRESSIBLE LIQUID LIMIT EQUAL TO 31-50 HIGHLY COMPRESSIBLE LIQUID LIMIT GREATER THAN 50										ROCK QUALITY DESIGNATION (RQD) - A MEASURE OF ROCK QUALITY DESCRIBED BY TOTAL LENGTH OF ROCK SEGMENTS EQUAL TO OR GREATER THAN 4 INCHES DIVIDED BY THE TOTAL LENGTH OF CORE RUN AND EXPRESSED AS A PERCENTAGE.										ROADWAY EMBANKMENT (RE) WITH SOIL DESCRIPTION SOIL SYMBOL ARTIFICIAL FILL (AF) OTHER THAN ROADWAY EMBANKMENT INFERRED SOIL BOUNDARY INFERRED ROCK LINE ALLUVIAL SOIL BOUNDARY DIP & DIP DIRECTION OF ROCK STRUCTURES SOUNDING ROD									
GROUP CLASS. A-1, A-1-b, A-2, A-2-4, A-2-5, A-2-6, A-2-7, A-4, A-5, A-6, A-7, A-1, A-2, A-3, A-4, A-5, A-6, A-7										TRACE OF ORGANIC MATTER 2 - 3% LITTLE ORGANIC MATTER 3 - 5% MODERATELY ORGANIC 5 - 10% HIGHLY ORGANIC > 10%										FRESH VERY SLIGHT (V SLI.) SLIGHT (SLI.) MODERATE (MOD.) MODERATELY SEVERE (MOD. SEV.) SEVERE (SEV.) VERY SEVERE (V SEV.) COMPLETE										SPT TEST BORING DESIGNATIONS S - BULK SAMPLE SS - SPLIT SPOON SAMPLE ST - SHELBY TUBE SAMPLE RS - ROCK SAMPLE RT - RECOMPACTED TRIAXIAL SAMPLE CBR - CALIFORNIA BEARING RATIO SAMPLE SPT N-VALUE SPT REFUSAL																													
SYMBOL										ORGANIC MATERIAL GRANULAR SOILS SILT-CLAY SOILS OTHER MATERIAL										WEATHERING										MISCELLANEOUS SYMBOLS																													
% PASSING # 10 # 40 # 200										PERCENTAGE OF MATERIAL										WEATHERING										MISCELLANEOUS SYMBOLS																													
LIQUID LIMIT PLASTIC INDEX										GROUND WATER										WEATHERING										MISCELLANEOUS SYMBOLS																													
GROUP INDEX										GROUND WATER										WEATHERING										MISCELLANEOUS SYMBOLS																													
USUAL TYPES OF MAJOR MATERIALS										GROUND WATER										WEATHERING										MISCELLANEOUS SYMBOLS																													
GEN. RATINGS AS A SUBGRADE										GROUND WATER										WEATHERING										MISCELLANEOUS SYMBOLS																													
PI OF A-7-5 SUBGROUP IS <= LL - 30; PI OF A-7-6 SUBGROUP IS > LL - 30										GROUND WATER										WEATHERING										MISCELLANEOUS SYMBOLS																													
CONSISTENCY OR DENSITY										TEXTURE OR GRAIN SIZE										ROCK HARDNESS										ABBREVIATIONS																													
PRIMARY SOIL TYPE COMPACTNESS OR CONSISTENCY RANGE OF STANDARD PENETRATION RESISTANCE (N-VALUE) RANGE OF UNCONFINED COMPRESSIVE STRENGTH (TONS/FT ²)										U.S. STD. SIEVE SIZE OPENING (MM) 4 10 40 60 200 270 4.76 2.00 0.42 0.25 0.075 0.053										VERY HARD CANNOT BE SCRATCHED BY KNIFE OR SHARP PICK. BREAKING OF HAND SPECIMENS REQUIRES SEVERAL HARD BLOWS OF THE GEOLOGIST'S PICK. HARD CAN BE SCRATCHED BY KNIFE OR PICK ONLY WITH DIFFICULTY. HARD HAMMER BLOWS REQUIRED TO DETACH HAND SPECIMEN. MODERATELY HARD CAN BE SCRATCHED BY KNIFE OR PICK. GOUGES OR GROOVES TO 0.25 INCHES DEEP CAN BE EXCAVATED BY HARD BLOW OF A GEOLOGIST'S PICK. HAND SPECIMENS CAN BE DETACHED BY MODERATE BLOWS. MEDIUM HARD CAN BE GROOVED OR GOUGED 0.05 INCHES DEEP BY FIRM PRESSURE OF KNIFE OR PICK POINT. CAN BE EXCAVATED IN SMALL CHIPS TO PIECES 1 INCH MAXIMUM SIZE BY HARD BLOWS OF THE POINT OF A GEOLOGIST'S PICK. SOFT CAN BE GROVED OR GOUGED READILY BY KNIFE OR PICK. CAN BE EXCAVATED IN FRAGMENTS FROM CHIPS TO SEVERAL INCHES IN SIZE BY MODERATE BLOWS OF A PICK POINT. SMALL, THIN PIECES CAN BE BROKEN BY FINGER PRESSURE. VERY SOFT CAN BE CARVED WITH KNIFE. CAN BE EXCAVATED READILY WITH POINT OF PICK. PIECES 1 INCH OR MORE IN THICKNESS CAN BE BROKEN BY FINGER PRESSURE. CAN BE SCRATCHED READILY BY FINGER NAIL.										AR - AUGER REFUSAL BT - BORING TERMINATED CL - CLAY CPT - CONE PENETRATION TEST CSE - COARSE DMT - DILATOMETER TEST DPT - DYNAMIC PENETRATION TEST e - VOID RATIO FOSS. - FOSSILIFEROUS FRAC. - FRACTURED, FRACTURES FRAGS. - FRAGMENTS HI. - HIGHLY MED. - MEDIUM MICA. - MICACEOUS MOD. - MODERATELY NP - NON PLASTIC ORG. - ORGANIC PMT - PRESSUREMETER TEST SAP. - SAPROLITIC SD. - SAND, SANDY SL. - SILT, SILTY SLI. - SLIGHTLY TCR - TRICONE REFUSAL w - MOISTURE CONTENT v - VERY VST - VANE SHEAR TEST WEA. - WEATHERED % - UNIT WEIGHT % _d - DRY UNIT WEIGHT FIAD - FILLED IMMEDIATELY AFTER DRILLING																													
GENERALY GRANULAR MATERIAL (NON-COHESIVE) VERY LOOSE 4 TO 10 MEDIUM DENSE 10 TO 30 DENSE 30 TO 50 VERY DENSE > 50										BOULDER (BLDR.) COBBLE (COB.) GRAVEL (GR.) COARSE SAND (CSE. SD.) FINE SAND (F. SD.) SILT (SL.) CLAY (CL.)										VERY HARD HARD MODERATELY HARD MEDIUM HARD SOFT VERY SOFT										AR - AUGER REFUSAL BT - BORING TERMINATED CL - CLAY CPT - CONE PENETRATION TEST CSE - COARSE DMT - DILATOMETER TEST DPT - DYNAMIC PENETRATION TEST e - VOID RATIO FOSS. - FOSSILIFEROUS FRAC. - FRACTURED, FRACTURES FRAGS. - FRAGMENTS HI. - HIGHLY MED. - MEDIUM MICA. - MICACEOUS MOD. - MODERATELY NP - NON PLASTIC ORG. - ORGANIC PMT - PRESSUREMETER TEST SAP. - SAPROLITIC SD. - SAND, SANDY SL. - SILT, SILTY SLI. - SLIGHTLY TCR - TRICONE REFUSAL w - MOISTURE CONTENT v - VERY VST - VANE SHEAR TEST WEA. - WEATHERED % - UNIT WEIGHT % _d - DRY UNIT WEIGHT FIAD - FILLED IMMEDIATELY AFTER DRILLING																													
GENERALY SILT-CLAY MATERIAL (COHESIVE) VERY SOFT 2 TO 4 SOFT 4 TO 8 MEDIUM STIFF 8 TO 15 STIFF 15 TO 30 HARD > 30										GRAIN MM 305 75 2.0 0.25 0.05 0.005 IN. 12 3										VERY HARD HARD MODERATELY HARD MEDIUM HARD SOFT VERY SOFT										AR - AUGER REFUSAL BT - BORING TERMINATED CL - CLAY CPT - CONE PENETRATION TEST CSE - COARSE DMT - DILATOMETER TEST DPT - DYNAMIC PENETRATION TEST e - VOID RATIO FOSS. - FOSSILIFEROUS FRAC. - FRACTURED, FRACTURES FRAGS. - FRAGMENTS HI. - HIGHLY MED. - MEDIUM MICA. - MICACEOUS MOD. - MODERATELY NP - NON PLASTIC ORG. - ORGANIC PMT - PRESSUREMETER TEST SAP. - SAPROLITIC SD. - SAND, SANDY SL. - SILT, SILTY SLI. - SLIGHTLY TCR - TRICONE REFUSAL w - MOISTURE CONTENT v - VERY VST - VANE SHEAR TEST WEA. - WEATHERED % - UNIT WEIGHT % _d - DRY UNIT WEIGHT FIAD - FILLED IMMEDIATELY AFTER DRILLING																													
SOIL MOISTURE - CORRELATION OF TERMS										EQUIPMENT USED ON SUBJECT PROJECT										FRACTURE SPACING										BEDDING																													
SOIL MOISTURE SCALE (ATTERBERG LIMITS) FIELD MOISTURE DESCRIPTION GUIDE FOR FIELD MOISTURE DESCRIPTION										DRILL UNITS: MOBILE B- BK-51 CME-45C CME-550 PORTABLE HOIST										TERM SPACING VERY WIDE MORE THAN 10 FEET WIDE 3 TO 10 FEET MODERATELY CLOSE 1 TO 3 FEET CLOSE 0.16 TO 1 FEET VERY CLOSE LESS THAN 0.16 FEET										TERM THICKNESS VERY THICKLY BEDDED > 4 FEET THICKLY BEDDED 1.5 - 4 FEET THINLY BEDDED 0.16 - 1.5 FEET VERY THINLY BEDDED 0.03 - 0.16 FEET THICKLY LAMINATED 0.008 - 0.03 FEET THINLY LAMINATED < 0.008 FEET																													
LL LIQUID LIMIT - SATURATED - (SAT.) USUALLY LIQUID; VERY WET, USUALLY FROM BELOW THE GROUND WATER TABLE										CLAY BITS 6" CONTINUOUS FLIGHT AUGER 8" HOLLOW AUGERS HARD FACED FINGER BITS TUNG-CARBIDE INSERTS TRICONE *STEEL TEETH TRICONE *TUNG-CARB. CORE BIT										FRIABLE RUBBING WITH FINGER FREES NUMEROUS GRAINS; GENTLE BLOW BY HAMMER DISINTEGRATES SAMPLE.										BENCH MARK: BM*2: 8-INCH SPIKE IN 12' PINE 40' RT OF -BL- STA 15+14 ELEVATION: 1688.89 FT.																													
PL PLASTIC LIMIT - WET - (W) SEMISOLID; REQUIRES DRYING TO ATTAIN OPTIMUM MOISTURE										ADVANCING TOOLS: 8" HOLLOW AUGERS HARD FACED FINGER BITS TUNG-CARBIDE INSERTS TRICONE *STEEL TEETH TRICONE *TUNG-CARB. CORE BIT										MODERATELY INDURATED GRAINS CAN BE SEPARATED FROM SAMPLE WITH STEEL PROBE; BREAKS EASILY WHEN HIT WITH HAMMER.										NOTES:																													
OM OPTIMUM MOISTURE - MOIST - (M) SOLID; AT OR NEAR OPTIMUM MOISTURE										HAMMER TYPE: AUTOMATIC MANUAL CORE SIZE: B-N-XL-H HAND TOOLS: POST HOLE DIGGER HAND AUGER SOUNDING ROD VANE SHEAR TEST										INDURATED GRAINS ARE DIFFICULT TO SEPARATE WITH STEEL PROBE; DIFFICULT TO BREAK WITH HAMMER.										NOTES:																													
SL SHRINKAGE LIMIT - DRY - (D) REQUIRES ADDITIONAL WATER TO ATTAIN OPTIMUM MOISTURE										EQUIPMENT USED ON SUBJECT PROJECT										EXTREMELY INDURATED SHARP HAMMER BLOWS REQUIRED TO BREAK SAMPLE; SAMPLE BREAKS ACROSS GRAINS.										NOTES:																													
PLASTICITY										INDURATION										INDURATION										INDURATION																													
NONPLASTIC 0-5 VERY LOW LOW PLASTICITY 6-15 SLIGHT MED. PLASTICITY 16-25 MEDIUM HIGH PLASTICITY 26 OR MORE HIGH										FOR SEDIMENTARY ROCKS, INDURATION IS THE HARDENING OF THE MATERIAL BY CEMENTING, HEAT, PRESSURE, ETC.										FRIABLE RUBBING WITH FINGER FREES NUMEROUS GRAINS; GENTLE BLOW BY HAMMER DISINTEGRATES SAMPLE.										FRIABLE RUBBING WITH FINGER FREES NUMEROUS GRAINS; GENTLE BLOW BY HAMMER DISINTEGRATES SAMPLE.																													
COLOR										INDURATION										INDURATION										INDURATION																													
DESCRIPTIONS MAY INCLUDE COLOR OR COLOR COMBINATIONS (TAN, RED, YELLOW-BROWN, BLUE-GRAY). MODIFIERS SUCH AS LIGHT, DARK, STREAKED, ETC. ARE USED TO DESCRIBE APPEARANCE.										INDURATION										INDURATED GRAINS ARE DIFFICULT TO SEPARATE WITH STEEL PROBE; DIFFICULT TO BREAK WITH HAMMER.										INDURATED GRAINS ARE DIFFICULT TO SEPARATE WITH STEEL PROBE; DIFFICULT TO BREAK WITH HAMMER.																													



STATE OF NORTH CAROLINA
DEPARTMENT OF TRANSPORTATION

MICHAEL F. EASLEY
GOVERNOR

LYNDO TIPPETT
SECRETARY

May 1, 2007

STATE PROJECT: 33715.1.1 (B-4466)
F. A. PROJECT: BRZ-1104 (11)
COUNTY: Clay
DESCRIPTION: Bridge No. 4 on SR-1104 over Brasstown Creek
SUBJECT: Geotechnical Report – Foundation Investigation

This project is located in southwest Clay County, approximately 1.5 miles from the border with Georgia. The proposed structure is a double-span, each of 81.0 feet. The skew is 90 degrees. The subsurface investigation was conducted using a CME-550 drill machine with an automatic drive hammer. The borings were advanced using both eight-inch hollow stem augers, and NW casing with advancer. Standard Penetration Tests were performed at intervals of 5.0 feet. Soil samples were collected and submitted for testing of quality. Rock core was retrieved using NXWL equipment. One (1) rock core sample was submitted for testing for Unit Weight, Compressive Strength (Qu), Young's Modulus (E) and Split Tensile Strength.

Physiography and Geology

The project site is located in moderately rolling countryside. Except for EB1-B, the bridge lies within a broad, flat floodplain. Adjacent to EB1-B is moderate to steep hillside. Brasstown Creek is incised approximately 10 feet into the floodplain. The geology of the area is the Brevard Fault Zone.

Roadway embankment was not encountered in the borings for Interior Bent-One. In the borings for the End Bents, embankment ranged from 1.0 to 5.0 feet thick and consisted of sandy gravels, silty sands, sandy silts and silty clays. Alluvium was encountered in each of the borings. It consists generally of loose silty sand with some soft sandy silts. A basal gravel layer is common.

Saprolite was not found above weathered rock (of schist) in the borings for End Bent One. Elsewhere, it was encountered between the elevations of 1647.8 and 1653.0. Weathered rock (of schist) was encountered in each of the borings. The elevations to the top of weathered rock (of schist) are 1651.8 to 1653.4. At the EB1-B site, a saprolite layer is present within the weathered rock (of schist) between the elevations 1649.9 and 1650.7.

Crystalline rock (schist) was encountered in each of the borings at this site. The elevation of the top of crystalline rock (schist) ranges across the site from 1643.0 (B1-A) to 1652.3 (EB1-A).

Rock Properties

The rock underlying this project is a member of the Dean Formation, labeled "Zd" on the Geologic Map of North Carolina (1985). The Dean Formation is mostly a light gray, sericite schist with abundant cross biotite porphyroblasts. The recovered core was predominately slightly weathered to fresh, and hard. Trace amounts of pyrite and chlorite were noted. The few naturally occurring discontinuities occurred as partings along schistosity at 50° and joints at 45°. The rock also tends to part readily along the schistosity during the drilling operation. The Recoveries across the site ranged from 81 to 100 percent (95 percent average). The RQD's ranged from 0 to 100 percent (89 percent average).

Groundwater

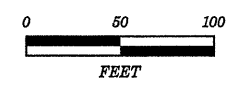
Static groundwater was measured in 4 of the 6 borings, using non-extraordinary means. The groundwater elevations ranged from 1654.3 to 1655.7.

Comments

This geotechnical foundation investigation was based on the Preliminary General Drawing received January 19, 2007. Additional information was gleaned from the Hydraulic Design Report dated January 10, 2007. If any significant changes are made in the design or location of the proposed structure, the subsurface information will have to be reviewed and modified as necessary.

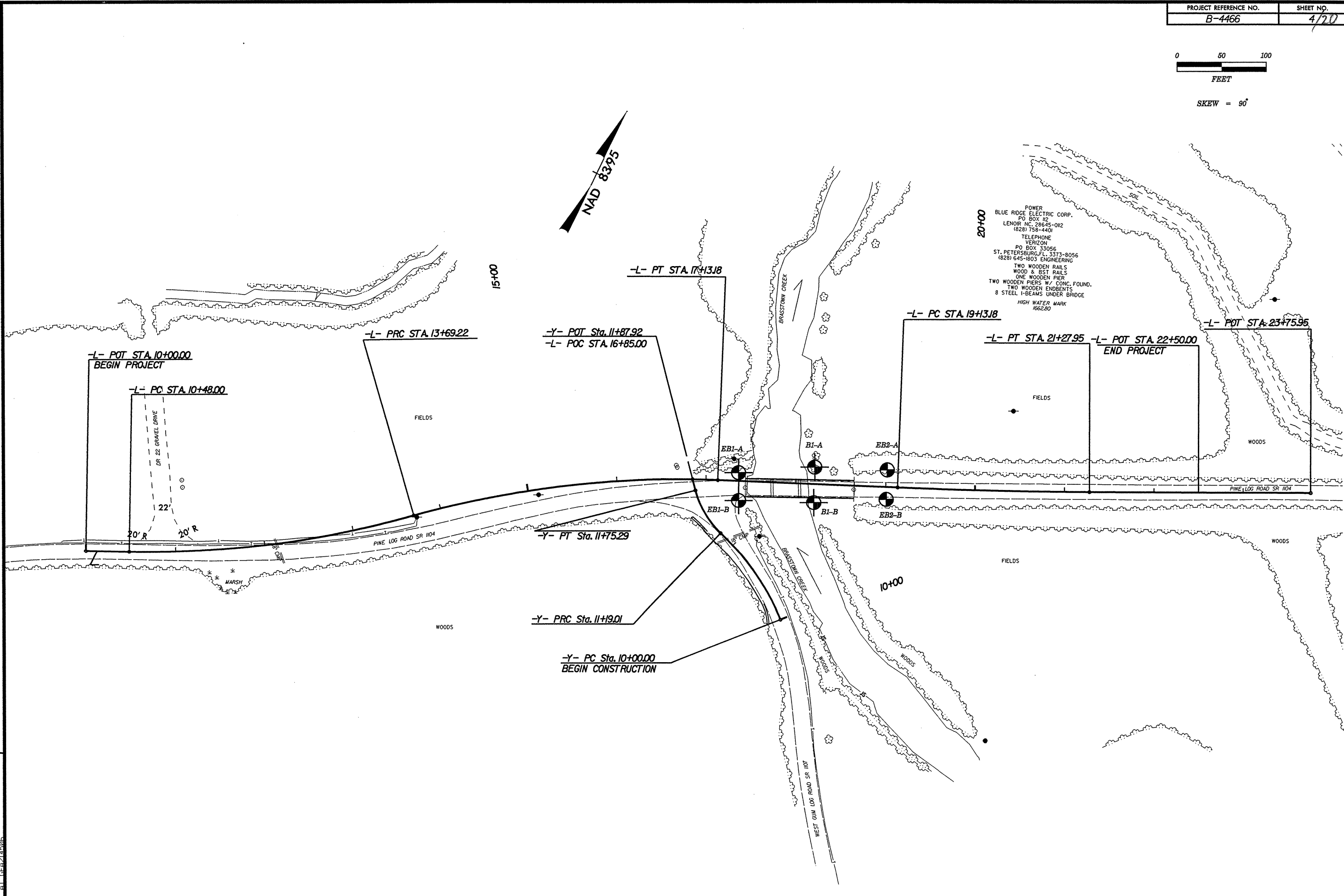
Respectfully Submitted,

Charles A. Dunnagan, L.G.
Project Geological Engineer



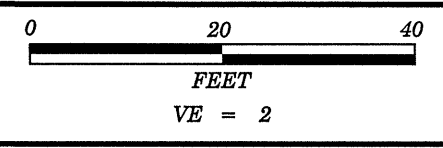
SKEW = 90°

8/17/99
 REVISIONS
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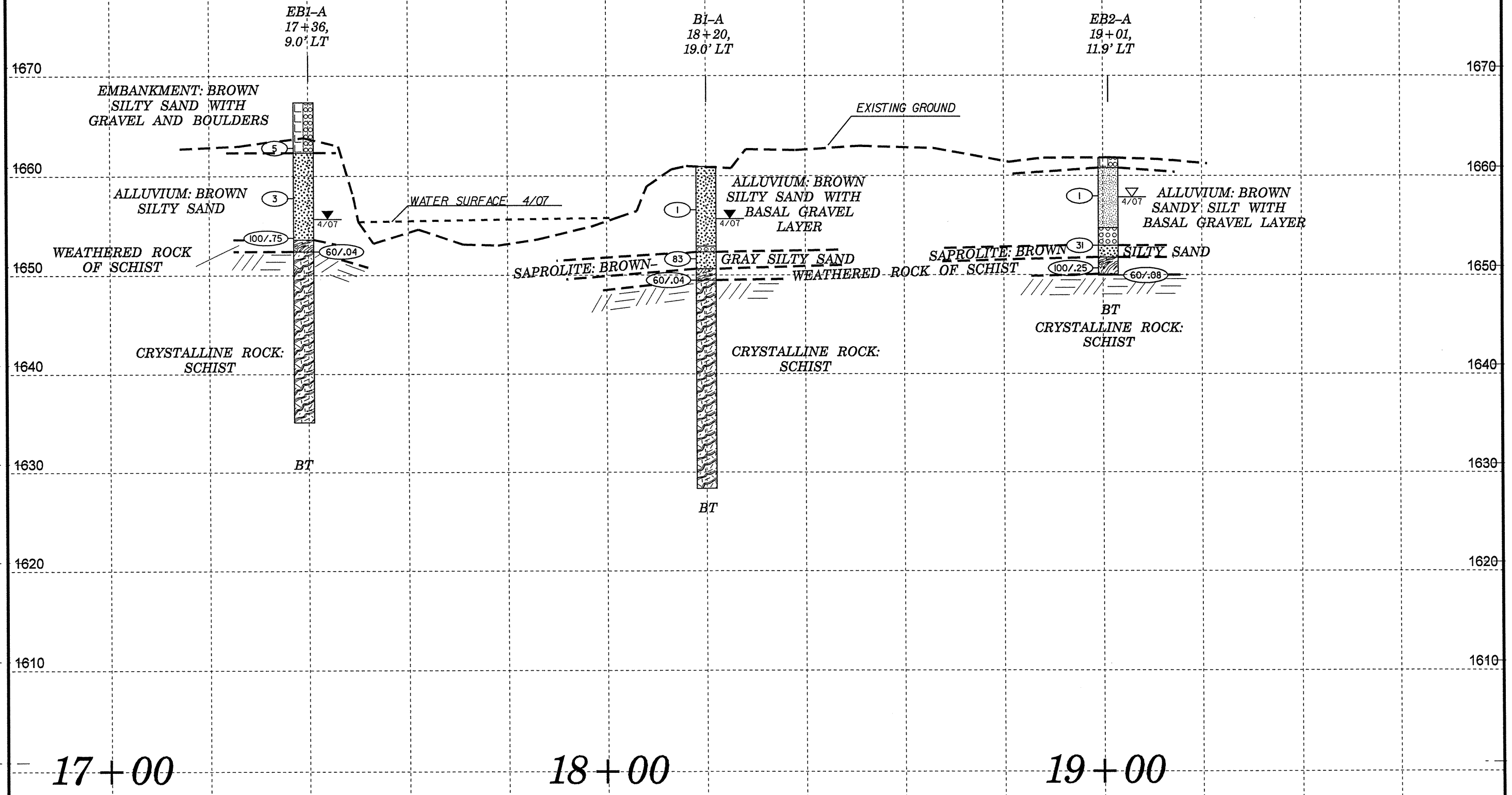


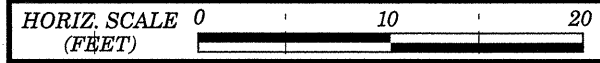
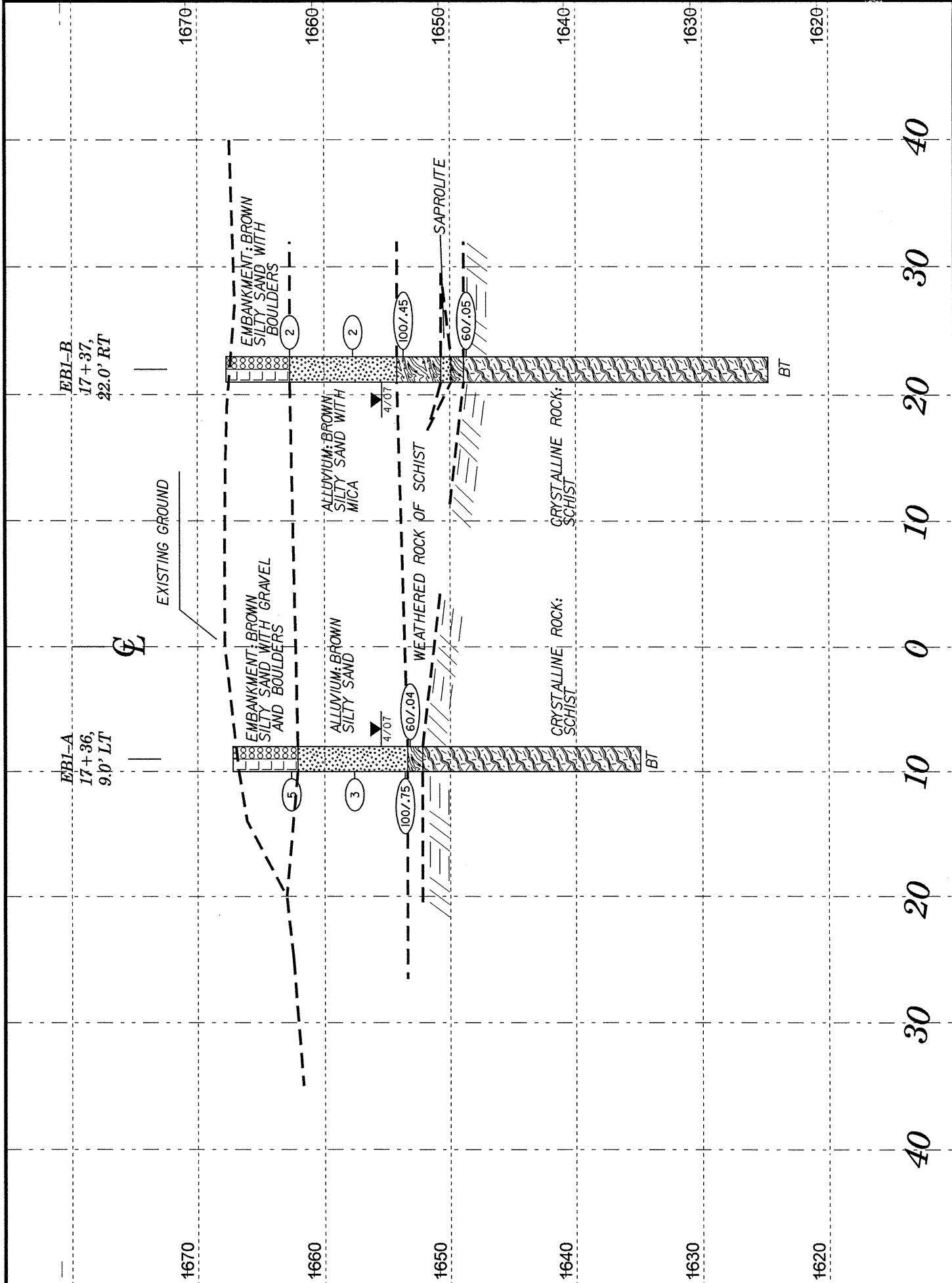
16+00 17+00 18+00 19+00

SEE SHEET 5 FOR -L- & -Y- PROFILES
 SEE SHEET S-1THUR S- FOR STRUCTURE PLANS



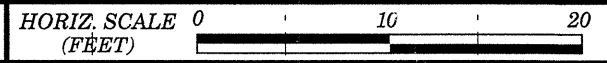
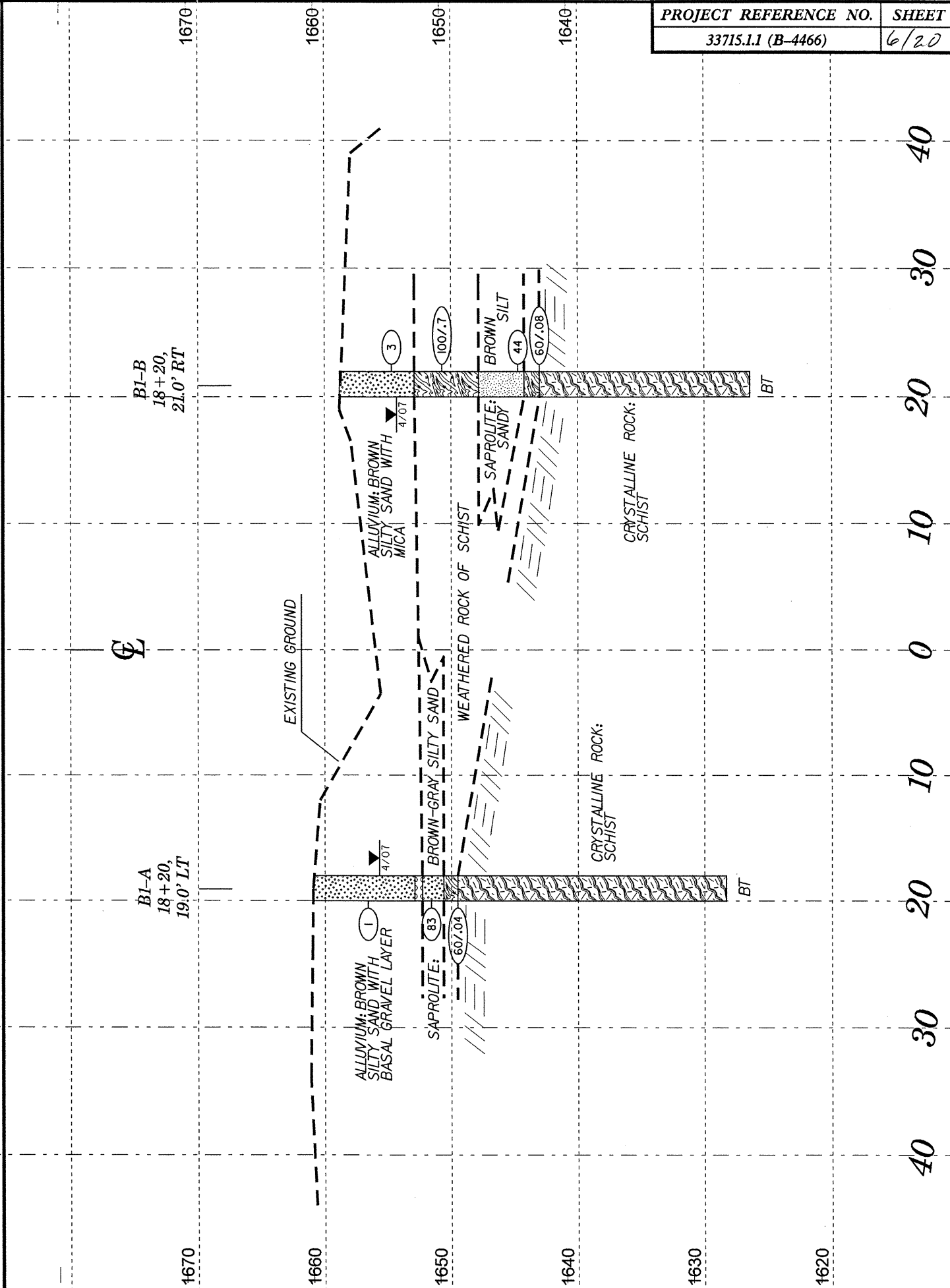
PROJECT REFERENCE NO.	SHEET
33715.1.1 (B-4466)	5/20
PROFILE 19.0' LEFT OF CENTERLINE -L-	





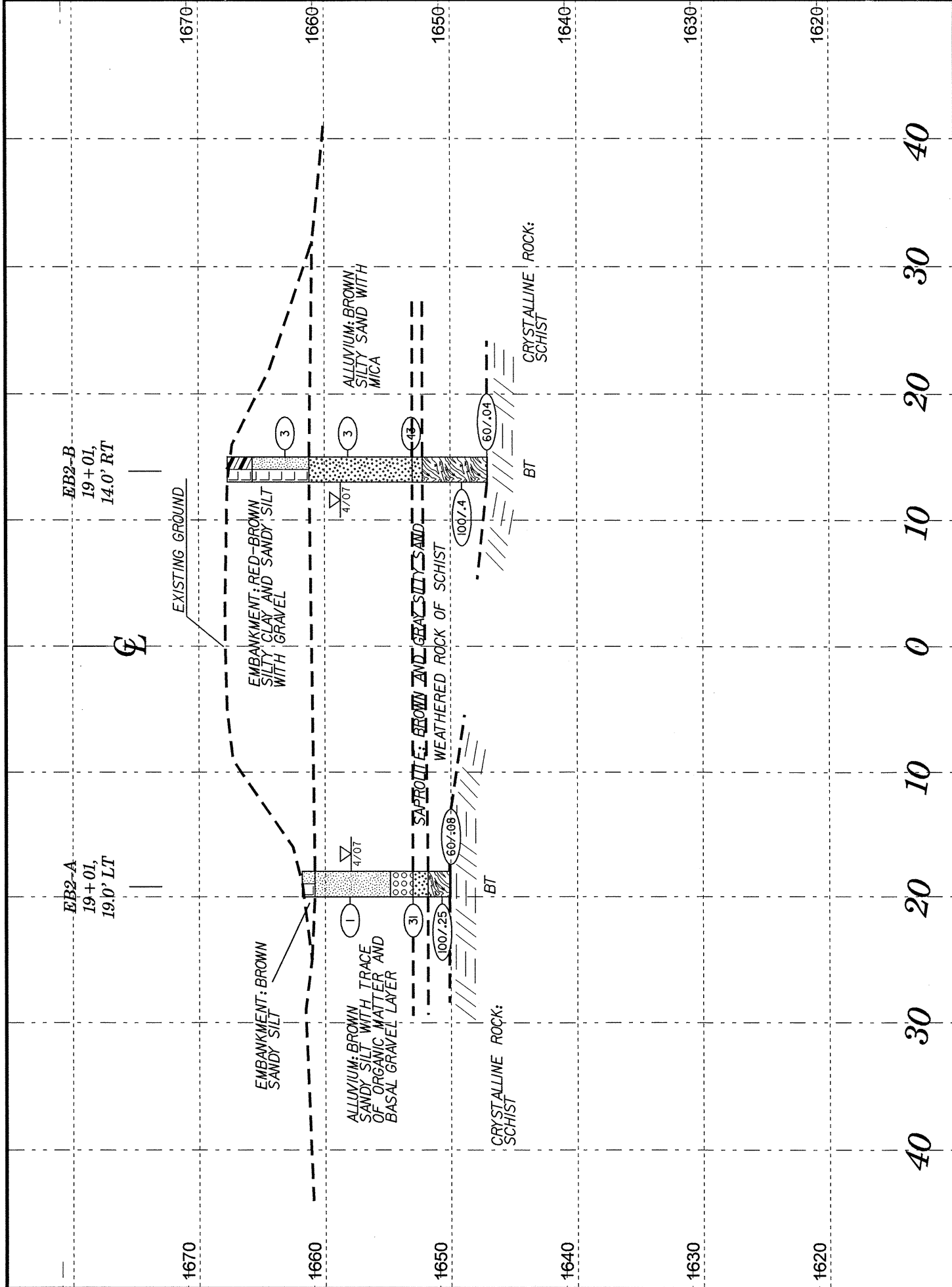
VE = 1

CROSS SECTION THROUGH END BENT ONE



VE = 1

CROSS SECTION THROUGH INTERIOR BENT ONE



HORIZ. SCALE 0 10 20 (FEET)

VE = 1

CROSS SECTION THROUGH END BENT TWO

PROJECT NO. 33715.1.1	ID. B-4466	COUNTY Clay	GEOLOGIST Daniel, T. B.
SITE DESCRIPTION Bridge No. 4 on SR-1104 over Brasstown Creek			GROUND WTR (ft)
BORING NO. EB1-A	STATION 17+36	OFFSET 9ft LT	ALIGNMENT -L-
COLLAR ELEV. 1,667.2 ft	TOTAL DEPTH 32.3 ft	NORTHING 491,369	EASTING 525,030
DRILL MACHINE CME-550X	DRILL METHOD NW Casing w/ SPT Core	HAMMER TYPE Automatic	
START DATE 04/10/07	COMP. DATE 04/11/07	SURFACE WATER DEPTH N/A	DEPTH TO ROCK 14.9 ft

ELEV (ft)	DEPTH (ft)	BLOW COUNT			BLOWS PER FOOT					SAMP. NO.	LOG	SOIL AND ROCK DESCRIPTION	DEPTH (ft)
		0.5ft	0.5ft	0.5ft	0	25	50	75	100				
1670												GROUND SURFACE	0.0
1,663.6	3.6	2	2	3							M	ROADWAY EMBANKMENT Brown silty sand with gravel and boulders.	5.1
1,658.6	8.6	0	1	2							W	ALLUVIAL Brown silty sand.	
1,653.6	13.6										Sat.	WEATHERED ROCK Weathered rock of schist.	13.8
1,652.3	14.9	49	51/25						100/75 60/04		CRYSTALLINE ROCK Gray sericite schist.	14.9	

Boring Terminated at Elevation 1,634.9 ft in Crystalline Rock (schist)

CORE BORING REPORT

PROJECT: 33715.1.1 I. D. NO: B-4466 BORING NO: EB1-A GEOLOGIST: C A Dunnagan

DESCRIPTION: Bridge No. 4 on SR-1104 over Brasstown Creek

COUNTY: Clay COLLAR ELEVATION: 1667.2 FT. TOTAL DEPTH: 32.3 FT.

ELEV. (FEET)	DEPTH (FEET)	DRILL RATE (MIN./FT.)	RUN (FEET)	REC. FEET %	RQD. FEET %	SAMP. #	FIELD CLASSIFICATION AND REMARKS
1652.2	15.0		2.3	2.2	0.0		Gray and brown sericite schist. Moderately severe to moderately weathered. Soft to hard. a) Parts along schistosity @ 50°. b) Joints @ 45°.
				96	0		
1649.9	17.3						17.6ft
1649.9	17.3		5.0	4.9	4.3		
				98	86		
1644.9	22.3						Gray sericite schist. Fresh, hard with moderately weathered zone from 18.2ft to 18.5ft. Trace of pyrite; trace of chlorite.
1644.9	22.3		5.0	4.9	4.9		
				98	98		
1639.9	27.3						
1639.9	27.3		5.0	4.6	4.6		
				92	92		
1634.9	32.3						

CORING TERMINATED AT ELEVATION 1634.9 FT.

DRILLER: C J Coffey CORE SIZE: NXWL EQUIPMENT: CME-550x



PROJECT NO. 33715.1.1	ID. B-4466	COUNTY Clay	GEOLOGIST Daniel, T. B.
SITE DESCRIPTION Bridge No. 4 on SR-1104 over Brasstown Creek			GROUND WTR (ft)
BORING NO. B1-B	STATION 18+20	OFFSET 21ft RT	ALIGNMENT -L-
COLLAR ELEV. 1,658.8 ft	TOTAL DEPTH 32.4 ft	NORTHING 491,377	EASTING 525,119
DRILL MACHINE CME-550X	DRILL METHOD NW Casing w/ SPT Core	HAMMER TYPE Automatic	
START DATE 04/09/07	COMP. DATE 04/10/07	SURFACE WATER DEPTH N/A	DEPTH TO ROCK 15.8 ft

SHEET 1 OF 1

DATE 13-Apr-07

CORE BORING REPORT

PROJECT: 33714.1.1 I. D. NO: B-4466 BORING NO: B1-B GEOLOGIST: C A Dunnagan

DESCRIPTION: Bridge No. 4 on SR-1104 over Brasstown Creek

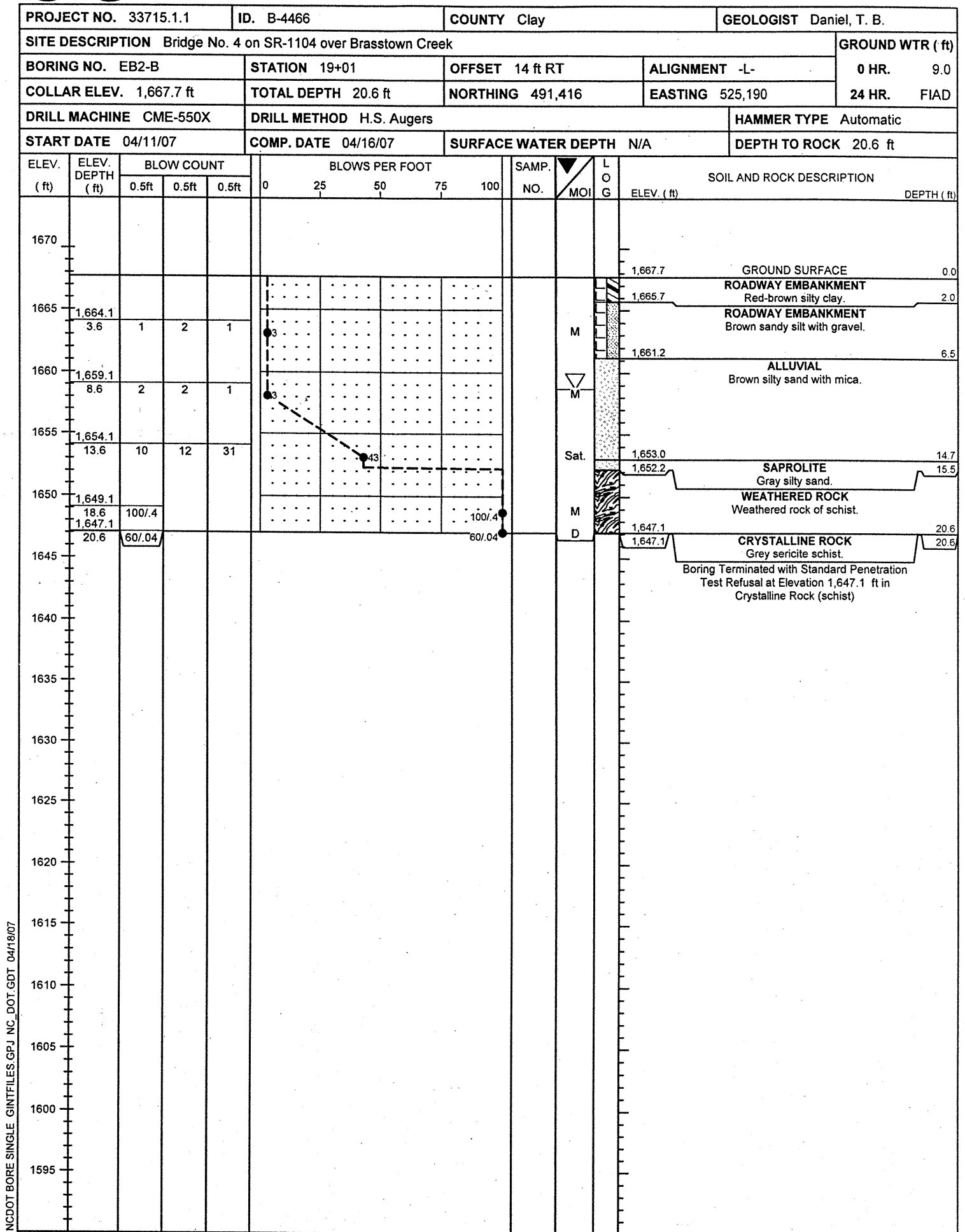
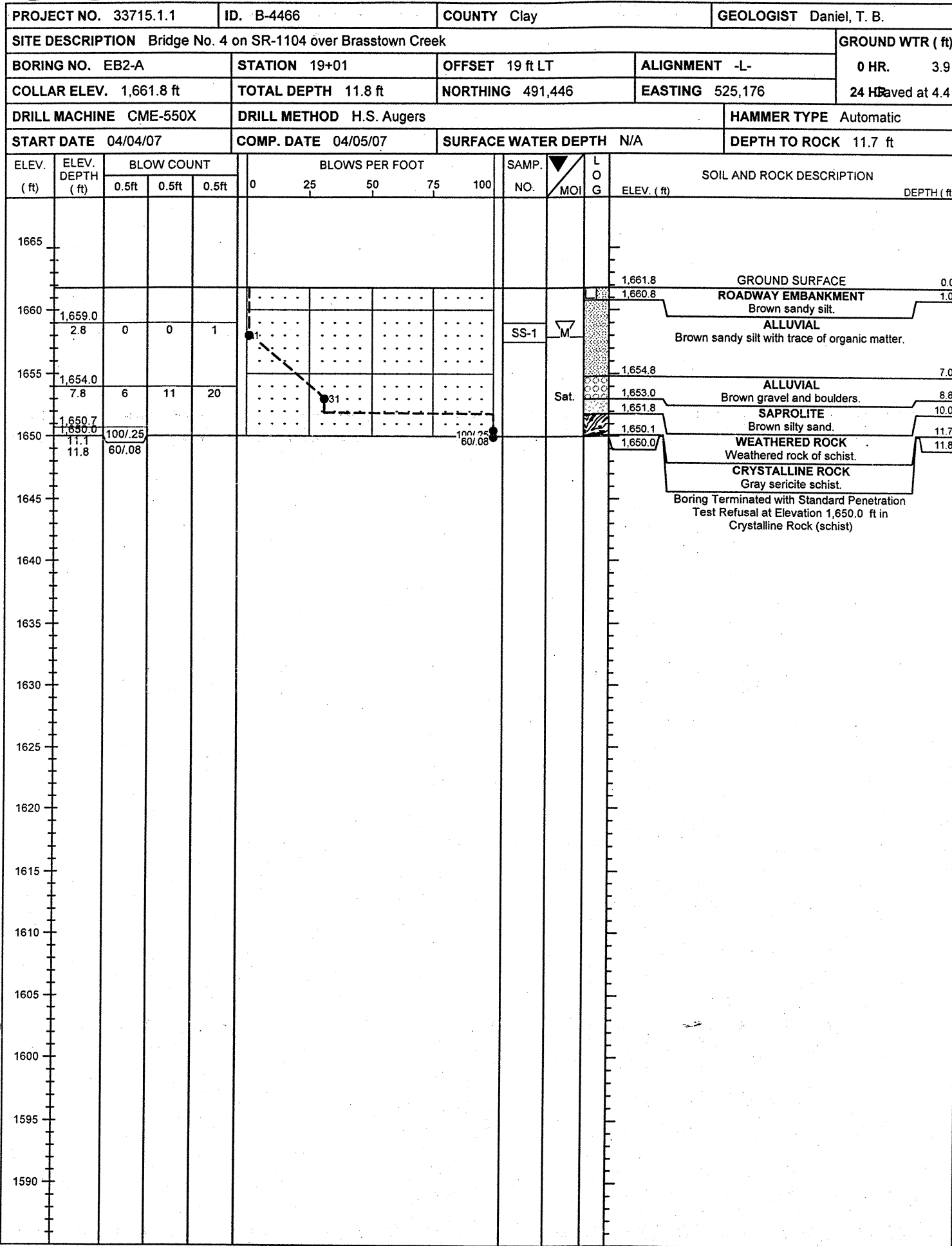
COUNTY: Clay COLLAR ELEVATION: 1658.8 FT. TOTAL DEPTH: 32.4 FT.

ELEV (ft)	DEPTH (ft)	BLOW COUNT			BLOWS PER FOOT					SAMP. NO.	LOG	SOIL AND ROCK DESCRIPTION	DEPTH (ft)
		0.5ft	0.5ft	0.5ft	0	25	50	75	100				
1660												GROUND SURFACE	0.0
1655.7	3.1	1	2	1								ALLUVIAL Brown silty sand with mica.	5.9
1650.7	8.1	21	71	29/2								WEATHERED ROCK Weathered rock of schist.	11.0
1645.7	13.1	8	9	35								SAPROLITE Brown sandy silt.	14.6
1643.0	15.8	60/08										WEATHERED ROCK Weathered rock of schist.	15.8
												CRYSTALLINE ROCK Gray sericite schist.	32.4
												Boring Terminated at Elevation 1,626.4 ft in Crystalline Rock (schist)	

ELEV. (FEET)	DEPTH (FEET)	DRILL RATE MIN./FT.	RUN (FEET)	REC. FEET %	RQD. FEET %	SAMP. #	FIELD CLASSIFICATION AND REMARKS
1642.9	15.9		1.5	1.1	1.1		
1641.4	17.4			73	73		
1641.4	17.4		5.0	5.0	5.0	RS-1	
1636.4	22.4			100	100		Light gray sericite schist with cross-biotite. Fresh, hard.
1636.4	22.4			5.0	5.0		Trace of pyrite; trace of chlorite.
1631.4	27.4			100	100		
1631.4	27.4		5.0	5.0	5.0		
1626.4	32.4			100	100		

CORING TERMINATED AT ELEVATION 1626.4 FT.

DRILLER: C J Coffey CORE SIZE: NXWL EQUIPMENT: CME-550x



JCS

**NORTH CAROLINA DEPARTMENT OF TRANSPORTATION
DIVISION OF HIGHWAYS-MATERIALS AND TESTS UNIT
SOILS TEST REPORT-SOILS LABORATORY**

T.I.P. ID #: B-4466

REPORT ON SAMPLES OF: Soils for Quality

PROJECT:	33715.1.1	COUNTY:	Clay	Owner:	NCDOT
DATE SAMPLED:	4.4.07	DATE RECEIVED:	4.13.07	DATE REPORTED:	4.19.07
SAMPLED FROM:	Bridge	SAMPLED BY:	C. A. Dunnagan		
SUBMITTED BY:	W. D. Frye	2002	STANDARD SPECIFICATION		
LABORATORY:	Asheville				

TEST RESULTS

Project Sample No.	SS-1	SS-2	SS-3	SS-4	SS-5			
Lab Sample No. A	154935	154936	154937	154938	154939			
HiCAMS Sample #	--	--	--	--	--			
Retained #4 Sieve %	0.0	0.0	0.0	0.0	0.0			
Passing #10 Sieve %	92	100	95	78	93			
Passing #40 Sieve %	90	100	81	70	90			
Passing #200 Sieve %	48	33	36	37	61			

MINUS #10 FRACTION

Soil Mortar - 100%								
Coarse Sand -Ret. #60	8	14	26	18	6			
Fine Sand - Ret. #270	49	58	50	41	37			
Silt 0.05-0.005 mm %	31	18	20	19	29			
Clay < 0.005 mm %	12	10	4	22	28			
Passing # 40 Sieve %	--	--	--	--	--			
Passing # 200 Sieve %	--	--	--	--	--			

Liquid Limit	26	20	29	26	36			
Plastic Index	NP	NP	NP	NP	NP			
AASHTO Classification	A-4 (3)	A-2-4 (0)	A-4 (0)	A-4 (0)	A-4 (5)			
Quantity								
Texture								
Station	19+01	18+20	18+20	17+37	17+37			
Hole No.								
Depth (ft) From:	3.3	3.8	13.6	4.5	9.5			
To:	4.3	4.8	14.6	5.5	10.5			

Remarks:

A-154935 - 154939

CC:

C. A. Dunnagan	
File	

SOILS ENGINEER:



**FIELD
 SCOUR REPORT**

WBS: 33715.1.1 TIP: B-4466 COUNTY: Clay

DESCRIPTION(1): Bridge No.4 on SR-1104 over Brasstown Creek

EXISTING BRIDGE

Information from: Field Inspection Microfilm _____ (reel _____ pos: _____)
 Other (explain) _____

Bridge No.: 4 Length: 120 Total Bents: 5 Bents in Channel: 2 Bents in Floodplain: 3
 Foundation Type: Footings (on piles?) _____

EVIDENCE OF SCOUR(2)

Abutments or End Bent Slopes: None noted.

Interior Bents: Minor amounts on the upstream (Right) sides of Interior Bents 1 and 2.

Channel Bed: None noted.

Channel Bank: Minor amount at existing EB1-A.

EXISTING SCOUR PROTECTION

Type(3): Wooden end-bent walls.

Extent(4): Five feet beyond width of bridge, either side.

Effectiveness(5): Good.

Obstructions(6): Rock outcrops in stream bed, approx. 50 feet downstream of existing bridge.

INSTRUCTIONS

- 1 Describe the specific site's location, including route number and body of water crossed.
- 2 Note scour evidence at existing end bents or abutments (e.g. undermining, sloughing, degradations).
- 3 Note existing scour protection (e.g. rip rap).
- 4 Describe extent of existing scour protection.
- 5 Describe whether or not the scour protection appears to be working.
- 6 Note obstructions such as dams, fallen trees, debris at bents, etc.
- 7 Describe the channel bed material based on observation and/or samples. Include any lab results with report.
- 8 Describe the channel bank material based on observation and/or samples. Include any lab results with report.
- 9 Describe the material covering the banks (e.g. grass, trees, rip rap, none).
- 10 Determine the approximate floodplain width from field observation or a topographic map.
- 11 Describe the material covering the floodplain (e.g. grass, trees, crops).
- 12 Use professional judgement to specify if the stream is degrading, aggrading, or static.
- 13 Describe potential and direction of the stream to migrate laterally during the bridge's life (approx. 100 years).
- 14 Give the design scour elevation (DSE) expected over the life of the bridge (approx. 100 years). This elevation can be given as a range across the site, or for each bent. Discuss the relationship between the Hydraulics Unit theoretical scour and the DSE. If the DSE is dependent on scour counter measures, explain (e.g. rip rap armoring on slopes). The DSE is based on the erodability of materials, giving consideration to the influence of joints, foliation, bedding characteristics, % core recovery, % RQD, differential weathering, shear strength, observations at existing structures, other tests deemed appropriate, and overall geologic conditions at the site.

DESIGN INFORMATION

Channel Bed Material(7): Silty sand with cobbles.

Channel Bank Material(8): Silty sand.

Channel Bank Cover(9): Cane and trees.

Floodplain Width(10): EB1-B = 0ft. All others greater than 100 feet.

Floodplain Cover(11): Grass.

Stream is(12): Aggrading _____ Degrading Static _____

Channel Migration Tendency(13): South-southwest

Observations and Other Comments: Existing End-Bent One marks the edge of the creek.

DESIGN SCOUR ELEVATIONS(14)

Feet Meters _____

BENTS

EB1	B1									
1656.8	1652.2									

Comparison of DSE to Hydraulics Unit theoretical scour:
 EB1 is about the same; B1 is approx 4.5 feet above theoretical scour.

SOIL ANALYSIS RESULTS FROM CHANNEL BED AND BANK MATERIAL

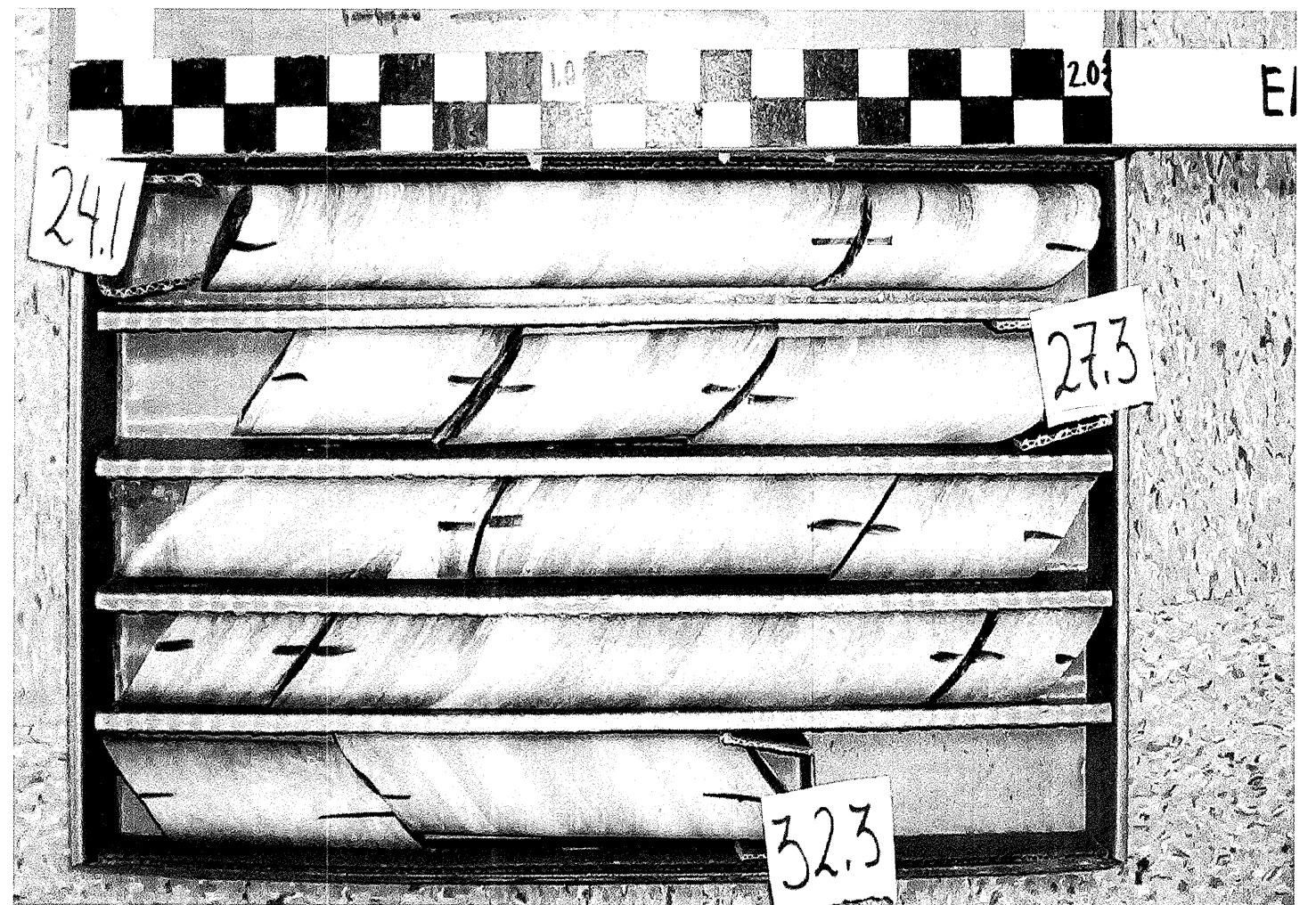
Bed or Bank							
Sample No.							
Retained #4							
Passed #10							
Passed #40							
Passed #200							
Coarse Sand							
Fine Sand							
Silt							
Clay							
LL							
PI							
AASHTO							
Station							
Offset							
Depth							

Reported by: *C. A. Dunnagan*
 C. A. Dunnagan

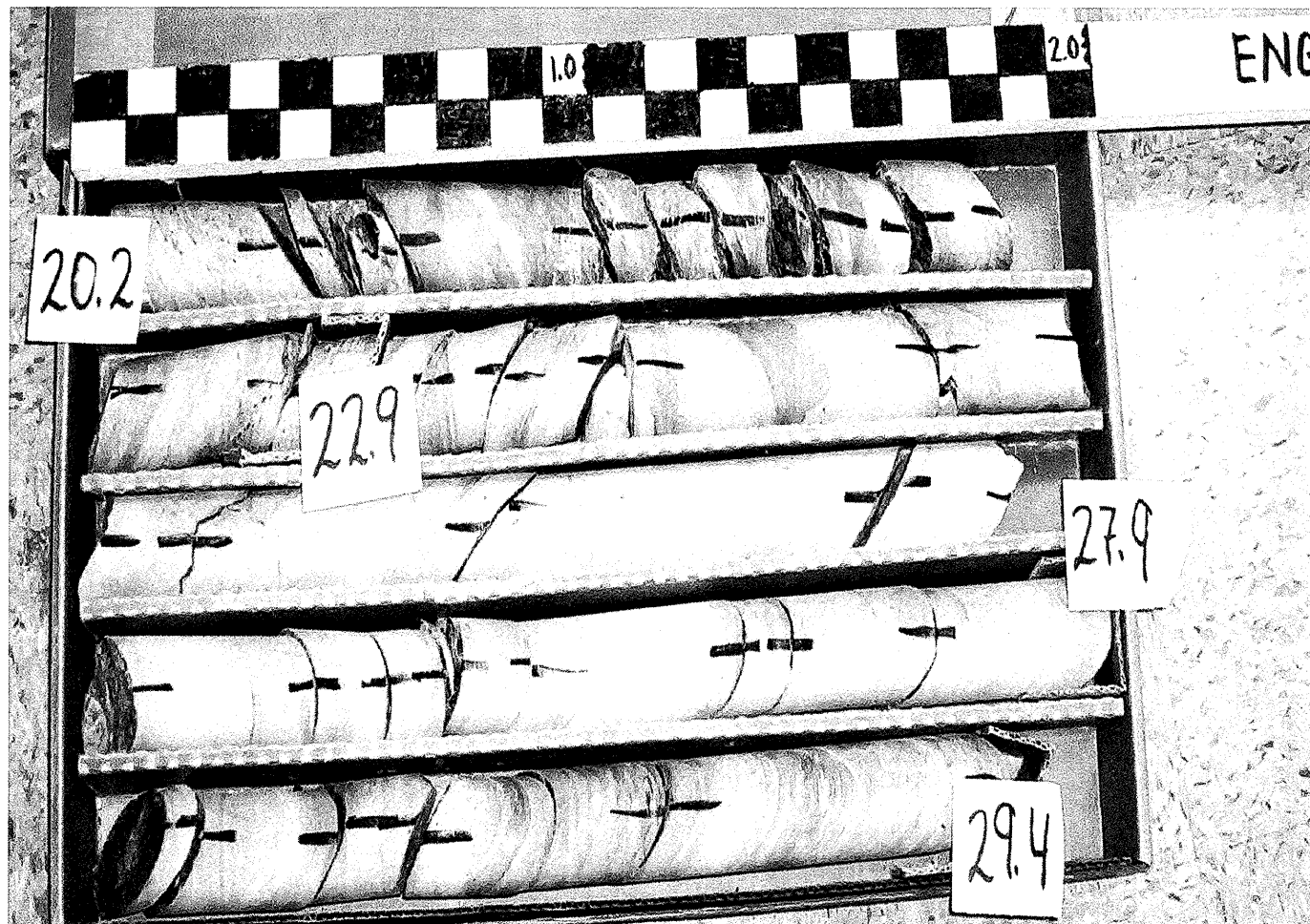
Date: 4/20/2007



33715.1.1 B-4466
 Bridge No. 4 on SR-1104
 Over Brasstown Creek.
 EB1-A
 Box 1 of 2



33715.1.1 B-4466
 Bridge No. 4 on SR-1104
 Over Brasstown Creek.
 EB1-A
 Box 2 of 2



33715.1.1 B-4466
 Bridge No. 4 on SR-1104
 Over Brasstown Creek.
 EB1-B
 Box 1 of 3



33715.1.1 B-4466
 Bridge No. 4 on SR-1104
 Over Brasstown Creek.
 EB1-B
 Box 2 of 3



33715.1.1 B-4466
Bridge No. 4 on SR-1104
Over Brasstown Creek.
EB1-B
Box 3 of 3



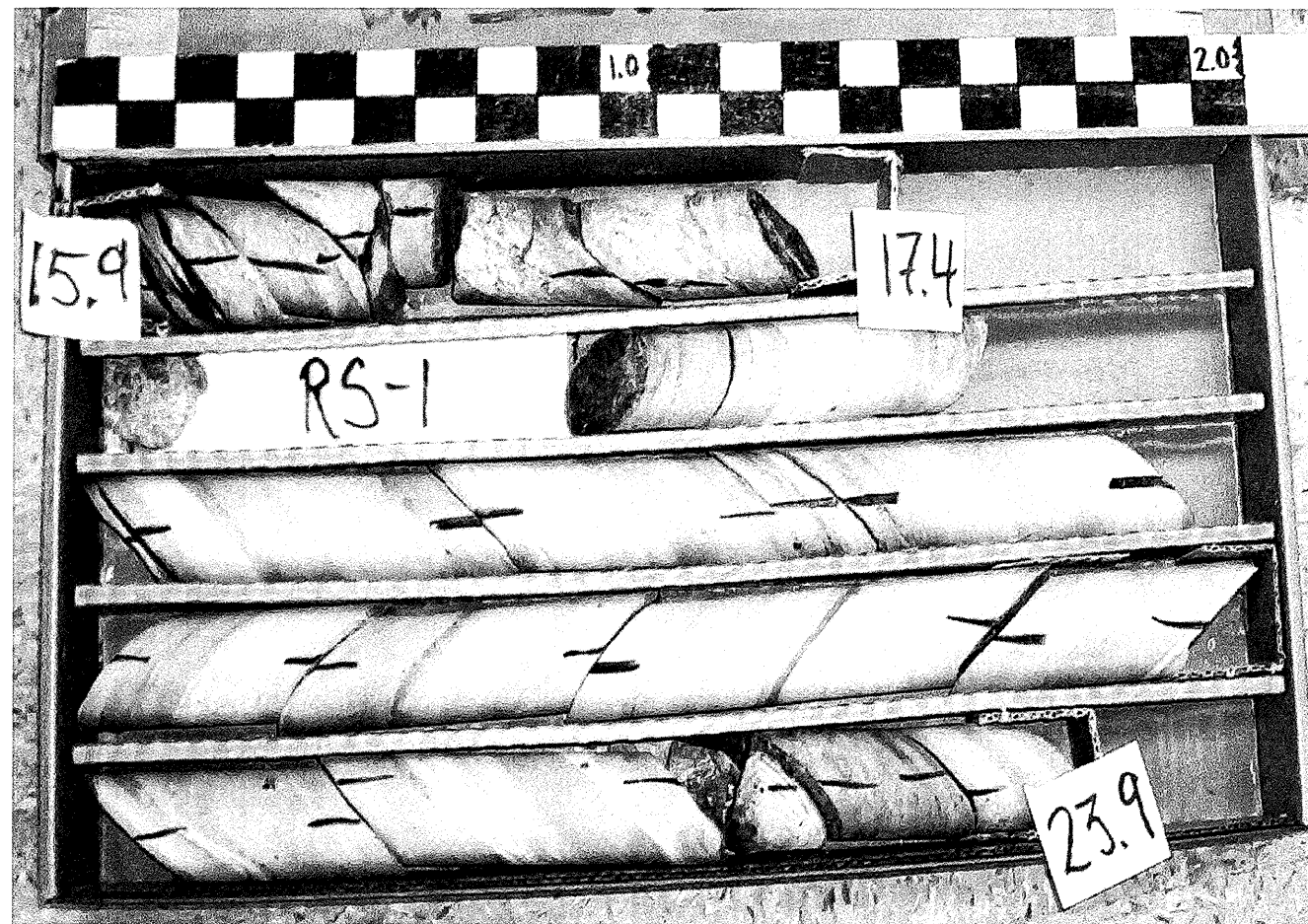
33715.1.1 B-4466
 Bridge No. 4 on SR-1104
 Over Brasstown Creek.
 B1-A
 Box 1 of 3



33715.1.1 B-4466
 Bridge No. 4 on SR-1104
 Over Brasstown Creek.
 B1-A
 Box 2 of 3



33715.1.1 B-4466
Bridge No. 4 on SR-1104
Over Brasstown Creek.
B1-A
Box 3 of 3



33715.1.1 B-4466
 Bridge No. 4 on SR-1104
 Over Brasstown Creek.
 B1-B
 Box 1 of 2



33715.1.1 B-4466
 Bridge No. 4 on SR-1104
 Over Brasstown Creek.
 B1-B
 Box 2 of 2