D: B-4466

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OJECT:33715.1.1

STATE OF NORTH CAROLINA

DEPARTMENT OF TRANSPORTATION
DIVISION OF HIGHWAYS
GEOTECHNICAL ENGINEERING UNIT

STRUCTURE SUBSURFACE INVESTIGATION

PROJ.REFERENCE NO	33715.1.1 (B-4466)	F.A. PROJ	BRZ-IIO4(II)
COUNTYCLAY			
	BRIDGE NO. 4 ON SR-II	D4	
	OVER BRASSTOWN CRE		
SITE DESCRIPTION			

STATE	STATE PROJECT REPERENCE NO.	SHEET NO.	TOTAL SHEETS
N.C.	33715.1.1 (B-4466)	1	20

CAUTION NOTICE

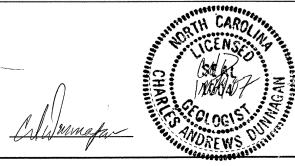
THE SUBSURFACE INFORMATION AND THE SUBSURFACE INVESTIGATION ON WHICH IT IS BASED WERE MADE FOR THE PURPOSE OF STUDY, PLANNING, AND DESIGN, AND NOT FOR CONSTRUCTION OR PAY PURPOSES. THE VARIOUS FIELD BORING LOGS, ROCK CORES, AND SOIL TEST DATA AVAILABLE MAY BE REVIEWED OR INSPECTED IN RALEIGH BY CONTACTING THE N.C. DEPARTMENT OF TRANSPORTATION, GEOTECHNICAL ENDINEERING UNIT AT (999) 250-4088. NEITHER THE SUBSURFACE PLANS AND REPORTS, NOR THE FIELD BORING LOGS, ROCK CORES, OR SOIL TEST DATA ARE PART OF THE CONTRACT.

GENERAL SOIL AND ROCK STRATA DESCRIPTIONS AND INDICATED BOUNDARIES ARE BASED ON A GEOTECHNICAL INTERPRETATION OF ALL AVAILABLE SUBSURFACE DATA AND MAY NOT NECESSARILY REFLECT THE ACTUAL SUBSURFACE CONDITIONS BETWEEN BORNOS OR BETWEEN SAMPLED STRATA WITHIN THE BOREHOLE. THE LABORATORY SAMPLE DATA AND THE IN STU UNIVIDENCE TEST DATA CAN BE RELED ON ONLY TO THE DEGREE OF RELIABILITY INHERENT IN THE STANDARD TEST METHOD. THE DISSERVED WATER LEVELS OR SOIL MOISTURE CONDITIONS INDICATED IN THE SUBSURFACE INVESTIGATIONS ARE AS RECORDED AT THE TIME OF THE INVESTIGATION. THESE WATER LEVELS OR SOIL MOISTURE CONDITIONS MAY VARY CONSIDERREALLY WITH TIME ACCORDING TO CLIMATIC CONDITIONS INCLUDING TEMPERATURES, PRECIPITATION, AND WIND. AS WELL AS OTHER NON-CLIMATIC FACTORS.

THE BIDDER OR CONTRACTOR IS CAUTIONED THAT DETAILS SHOWN ON THE SUBSURFACE PLANS ARE PRELIMINARY ONLY AND IN MANY CASES THE FINAL DESIGN DETAILS ARE DIFFERENT, FOR BIDDING AND CONSTRUCTION PURPOSES, REFER TO THE CONSTRUCTION PLANS AND DOCUMENTS FOR FINAL DESIGN NOFORMATION ON THIS PROJECT, THE DEFERTMENT DOES NOT WARRANT OR GUARANTEE THE SUFFICIENCY OR ACCURACY OF THE INVESTIGATION MADE, NOR THE INTERPRETATIONS MADE, OR OPINION OF THE DEPARTMENT AS TO THE TYPE OF MATERIALS AND CONDITIONS TO BE ENCOUNTERED. THE BIDDER OR CONTRACTOR IS CAUTIONED TO MAKE SHOULD HISDER DEFENS SUCCESSARY TO SATISFY HIMSELF AS TO CONDITIONS TO BE ENCOUNTERED ON THIS PROJECT. THE CONTRACTOR SHALL HAVE NO CLAIM FOR ADDITIONAL COMPENSATION OR FOR AN EXTENSION OF TIME FOR ANY REASON RESULTING FROM THE ACTUAL CONDITIONS ENCOUNTERED AT THE SITE DIFFERING FROM THOSE INDICATED IN THE SUBSURFACE INFORMATION.

	T B DANIEL
	C J COFFEY
	R D CHILDERS
INVESTIGATED BY	C A DUNNAGAN
CHECKED BY	₩ D FRYE,Jr
SUBMITTED BY_	₩ D FRYE, Jr
DATE	MAY 2007

PERSONNEL



NORTH CAROLINA DEPARTMENT OF TRANSPORTATION

PROJECT REFERENCE NO. 33715.1.1 (B-4466)

DIVISION OF HIGHWAYS GEOTECHNICAL ENGINEERING UNIT

SUBSURFACE INVESTIGATION

	SOIL AND ROCK LEGEND, TERM	S, SYMBOLS, AND ABBREVIATIONS	
SOIL DESCRIPTION	GRADATION	ROCK DESCRIPTION	TERMS AND DEFINITIONS
SOIL IS CONSIDERED TO BE THE UNCONSOLIDATED, SEMI-CONSOLIDATED, OR WEATHERED EARTH MATERIALS	WELL GRADED - INDICATES A GOOD REPRESENTATION OF PARTICLE SIZES FROM FINE TO COARSE. UNIFORM - INDICATES THAT SOIL PARTICLES ARE ALL APPROXIMATELY THE SAME SIZE. (ALSO	HARD ROCK IS NON-COASTAL PLAIN MATERIAL THAT IF TESTED, WOULD YIELD SPT REFUSAL. AN INFERRED ROCK LINE INDICATES THE LEVEL AT WHICH NON-COASTAL PLAIN MATERIAL WOULD YIELD SPT REFUSAL.	ALLUVIUM (ALLUV.) - SOILS THAT HAVE BEEN TRANSPORTED BY WATER.
THAT CAN BE PENETRATED WITH A CONTINUOUS FLIGHT POWER AUGER, AND YIELD LESS THAN 100 BLOWS PER FOOT ACCORDING TO STANDARD PENETRATION TEST (AASHTO 1206, ASTM D-1586). SOIL	POORLY GRADED) GAP-GRADED - INDICATES A MIXTURE OF UNIFORM PARTICLES OF TWO OR MORE SIZES.	SPT REFUSAL IS PENETRATION BY A SPLIT SPOON SAMPLER EQUAL TO OR LESS THAN 0.1 FOOT PER 60 BLOWS. IN NON-COASTAL PLAIN MATERIAL. THE TRANSITION BETWEEN SOIL AND ROCK IS OFTEN REPRESENTED BY A ZONE	ACUIFER - A WATER BEARING FORMATION OR STRATA.
CLASSIFICATION IS BASED ON THE AASHTO SYSTEM, BASIC DESCRIPTIONS GENERALLY SHALL INCLUDE: CONSISTENCY, COLOR, TEXTURE, MOISTURE, AASHTO CLASSIFICATION, AND OTHER PERTINENT FACTORS SUCH	ANGULARITY OF GRAINS	OF WEATHERED ROCK. ROCK MATERIALS ARE TYPICALLY DIVIDED AS FOLLOWS:	ARGILLACEOUS - APPLIED TO ROCKS THAT HAVE BEEN DERIVED FROM SAND OR THAT CONTAIN SAND. ARGILLACEOUS - APPLIED TO ALL ROCKS OR SUBSTANCES COMPOSED OF CLAY MINERALS,
AS MINERALOGICAL COMPOSITION, ANGULARITY, STRUCTURE, PLASTICITY, ETC. EXAMPLE:	THE ANGULARITY OR ROUNDNESS OF SOIL GRAINS IS DESIGNATED BY THE TERMS: ANGULAR, SUBROUNDED, OR ROUNDED.	WEATHERED NON-COASTAL PLAIN MATERIAL THAT WOULD YIELD SPT N VALUES > 100	OR HAVING A NOTABLE PROPORTION OF CLAY IN THEIR COMPOSITION, AS SHALE, SLATE, ETC.
VERY STIFF, GRAY, SUTY CLAL MOST WITH INTERECOOD FINE SHID LARESHIFMUP PLASTIC, A-7-6 SOIL LEGEND AND AASHTO CLASSIFICATION	MINERALOGICAL COMPOSITION	ROCK (WR) BLOWS PER FOOT IF TESTED.	ARTESIAN - GROUND WATER THAT IS UNDER SUFFICIENT PRESSURE TO RISE ABOVE THE LEVEL AT WHICH IT IS ENCOUNTERED, BUT WHICH DOES NOT NECESSARILY RISE TO OR ABOVE THE
COLT CLAY MATERIALS	MINERAL NAMES SUCH AS QUARTZ, FELDSPAR, MICA, TALC, KAOLIN, ETC. ARE USED IN DESCRIPTIONS	CRYSTALLINE ROCK (CR) FINE TO COARSE GRAIN IGNEOUS AND METAMORPHIC ROCK THAT WOULD YIELD SPT REFUSAL IF TESTED. ROCK TYPE INCLUDES GRANITE,	GROUND SURFACE.
CLASS. (≤ 35% PASSING *200) (> 35% PASSING *200) ORGANIC MATERIALS	WHENEVER THEY ARE CONSIDERED OF SIGNIFICANCE.	GNEISS, GABBRO, SCHIST, ETC.	CALCAREOUS (CALC.) - SOILS THAT CONTAIN APPRECIABLE AMOUNTS OF CALCIUM CARBONATE.
GROUP A-1 A-3 A-2 A-4 A-5 A-6 A-7 A-1, A-2 A-4, A-5 CLASS. A-1-6 A-1-6 A-1-6 A-1-6 A-2-4 A-2-5 A-2-6 A-2-7 A-2-7 A-3 A-3 A-6, A-7	COMPRESSIBILITY SLIGHTLY COMPRESSIBLE LIQUID LIMIT LESS THAN 31	NON-CRYSTALLINE ROCK (NCR) SEDIMENTARY ROCK THAT WOULD YELLD SYT REFUSAL IF TESTED. ROCK TYPE INCLUDES PHYLLITE, SLATE, SANDSTONE, ETC.	COLLUVIUM - ROCK FRAGMENTS MIXED WITH SOIL DEPOSITED BY GRAVITY ON SLOPE OR AT BOTTOM OF SLOPE.
SYMBOL 000000000000000000000000000000000000	MODERATELY COMPRESSIBLE LIQUID LIMIT EQUAL TO 31-50 HIGHLY COMPRESSIBLE LIQUID LIMIT GREATER THAN 50	COASTAL PLAIN COASTAL PLAIN SEDIMENTS CEMENTED INTO BOCK, BUT MAY NOT YIELD	CORE RECOVERY (REC.) - TOTAL LENGTH OF ALL MATERIAL RECOVERED IN THE CORE BARREL DIVIDED BY TOTAL
y Pacsaic	PERCENTAGE OF MATERIAL	(CP) SHELL BEDS. ETC.	LENGTH OF CORE RUN AND EXPRESSED AS A PERCENTAGE. DIKE - A TABULAR BODY OF IGNEOUS ROCK THAT CUTS ACROSS THE STRUCTURE OF ADJACENT
* 10 56 MX GRANULAR CLAY MUCK.	OPCONIC MATERIAL GRANULAR SILT - CLAY	WEATHERING	ROCKS OR CUTS MASSIVE ROCK.
# 40 38 MX 58 MX 51 MN PEAT SOILS PEAT SOILS PEAT 200 15 MX 25 MX 18 MX 35 MX 35 MX 35 MX 36 MN 36 MN 36 MN 36 MN 36 MN	TRACE OF ORGANIC MATTER 2 - 3% 3 - 5% TRACE 1 - 10%	FRESH ROCK FRESH, CRYSTALS BRIGHT, FEW JOINTS MAY SHOW SLIGHT STAINING, ROCK RINGS UNDER HAMMER IF CRYSTALLINE.	<u>DIP</u> - THE ANGLE AT WHICH A STRATUM OR ANY PLANAR FEATURE IS INCLINED FROM THE HORIZONTAL.
LIQUID LIMIT 48 MX 41 MN 48 MX 41 MN 48 MX 41 MN 48 MX 41 MN SOILS WITH	LITTLE ORGANIC MATTER 3 - 5% 5 - 12% LITTLE 10 - 20% MODERATELY ORGANIC 5 - 10% 12 - 20% SOME 20 - 35%	VERY SLIGHT ROCK GENERALLY FRESH, JOINTS STAINED, SOME JOINTS MAY SHOW THIN CLAY COATINGS IF OPEN,	DIP DIRECTION (DIP AZIMUTH) - THE DIRECTION OR BEARING OF THE HORIZONTAL TRACE OF
PLASTIC INDEX 6 MX NP 10 MX 10 MX 11 MN 111 MN 10 MX 10 MX 111 MN 111 MN LITTLE OR HIGHLY	HIGHLY ORGANIC >10% >20% HIGHLY 35% AND ABOVE	(V SLI,) CRYSTALS ON A BROKEN SPECIMEN FACE SHINE BRIGHTLY. ROCK RINGS UNDER HAMMER BLOWS IF OF A CRYSTALLINE NATURE.	THE LINE OF DIP, MEASURED CLOCKWISE FROM NORTH.
GROUP INDEX 0 0 0 4 MX 8 MX 12 MX 16 MX No MX MODERATE AMOUNTS OF SOILS		SLIGHT ROCK GENERALLY FRESH, JOINTS STAINED AND DISCOLORATION EXTENDS INTO ROCK UP TO	FAULT - A FRACTURE OR FRACTURE ZONE ALONG WHICH THERE HAS BEEN DISPLACEMENT OF THE SIDES RELATIVE TO ONE ANOTHER PARALLEL TO THE FRACTURE.
OF MAJOR GRAVEL, AND GRAVEL, AND GRAVEL AND SAND SOILS SOILS MATTER	WATER LEVEL IN BORE HOLE IMMEDIATELY AFTER DRILLING	(SLI.) 1 INCH. OPEN JOINTS MAY CONTAIN CLAY. IN GRANITOID ROCKS SOME OCCASIONAL FELDSPAR CRYSTALS ARE DULL AND DISCOLORED. CRYSTALLINE ROCKS RING UNDER HAMMER BLOWS.	FISSILE - A PROPERTY OF SPLITTING ALONG CLOSELY SPACED PARALLEL PLANES.
MATERIALS SAND SHIPD STATE STA	STATIC WATER LEVEL AFTER 24 HOURS	MODERATE SIGNIFICANT PORTIONS OF ROCK SHOW DISCOLORATION AND WEATHERING EFFECTS. IN	FLOAT - ROCK FRAGMENTS ON SURFACE NEAR THEIR ORIGINAL POSITION AND DISLODGED FROM
AS A EXCELLENT TO GOOD FAIR TO POOR POOR POOR UNSUITABLE	E PERCHED WATER, SATURATED ZONE, OR WATER BEARING STRATA	(MOD.) GRANITOID ROCKS, MOST FELDSPARS ARE DULL AND DISCOLORED, SOME SHOW CLAY, ROCK HAS DULL SOUND UNDER HAMMER BLOWS AND SHOWS SIGNIFICANT LOSS OF STRENGTH AS COMPARED	PARENT MATERIAL.
SUBGRADE PI OF A-7-5 SUBGROUP IS ≤ LL - 30 ; PI OF A-7-6 SUBGROUP IS > LL - 30	SPRING OR SEEP	WITH FRESH ROCK. MODERATELY ALL ROCK EXCEPT QUARTZ DISCOLORED OR STAINED. IN GRANITOID ROCKS, ALL FELDSPARS DULL	FLOOD PLAIN (FP) - LAND BORDERING A STREAM, BUILT OF SEDIMENTS DEPOSITED BY THE STREAM.
CONSISTENCY OR DENSENESS	MISCELLANEOUS SYMBOLS	SEVERE AND DISCOLORED AND A MAJORITY SHOW KAOLINIZATION. ROCK SHOWS SEVERE LOSS OF STRENGTH	FORMATION (FM.) - A MAPPABLE GEOLOGIC UNIT THAT CAN BE RECOGNIZED AND TRACED IN
PRIMARY SOIL TYPE COMPACTNESS OR RANGE OF STANDARD RANGE OF UNCONFINED PENETRATION RESISTENCE COMPRESSIVE STRENGTH	ROADWAY EMBANKMENT (RE) WITH SOIL DESCRIPTION SPT CPT SAMPLE DESIGNATIONS	(MOD. SEV.) AND CAN BE EXCAVATED WITH A GEOLOGIST'S PICK. ROCK GIVES "CLUNK" SOUND WHEN STRUCK. IF TESTED, WOULD YIELD SPT REFUSAL	THE FIELD. JOINT - FRACTURE IN ROCK ALONG WHICH NO APPRECIABLE MOVEMENT HAS OCCURRED.
CUNSISTENCY (N-VALUE) (TONS/FT ²)	→ S - BULK SAMPLE	SEVERE ALL ROCK EXCEPT QUARTZ DISCOLORED OR STAINED, ROCK FABRIC CLEAR AND EVIDENT BUT REDUCED	LEDGE - A SHELF-LIKE RIDGE OR PROJECTION OF ROCK WHOSE THICKNESS IS SMALL COMPARED TO
GENERALLY VERY LOOSE 4 TO 10	SOIL SYMBOL AUGER BORING SS - SPLIT SPOON	IN STRENGTH TO STRONG SOIL. IN GRANITOID ROCKS ALL FELDSPARS ARE KAOLINIZED TO SOME EXTENT. SOME FRAGMENTS OF STRONG ROCK USUALLY REMAIN.	ITS LATERAL EXTENT.
MATERIAL MEDIUM DENSE 10 TO 30 N/A	ARTIFICIAL FILL (AF) OTHER SAMPLE THAN ROADWAY EMBANKMENT CORE BORING	IF TESTED, YIELDS SPT N VALUES > 100 BPF	LENS - A BODY OF SOIL OR ROCK THAT THINS OUT IN ONE OR MORE DIRECTIONS. MOTTLED (MOT.) - IRREGULARLY MARKED WITH SPOTS OF DIFFERENT COLORS, MOTTLING IN
(NON-COHESIVE) VERY DENSE >50	ST - SHELBY TUBE	VERY SEVERE ALL ROCK EXCEPT QUARTZ DISCOLORED OR STAINED. ROCK FABRIC ELEMENTS ARE DISCERNIBLE BUT (V SEV.) THE MASS IS EFFECTIVELY REDUCED TO SOIL STATUS, WITH ONLY FRAGMENTS OF STRONG ROCK	SOILS USUALLY INDICATES POOR AERATION AND LACK OF GOOD DRAINAGE.
VERY SOFT (2 (0.25 GENERALLY SOFT 2 TO 4 0.25 TO 0.50	MONITORING WELL RS - BOCK SAMPLE	REMAINING. SAPROLITE IS AN EXAMPLE OF ROCK WEATHERED TO A DEGREE SUCH THAT ONLY MINOR VESTIGES OF THE ORIGINAL ROCK FABRIC REMAIN. IF TESTED YIELDS SPT N VALUES < 100 BPF	PERCHED WATER - WATER MAINTAINED ABOVE THE NORMAL GROUND WATER LEVEL BY THE PRESENCE OF AN INTERVENING IMPERVIOUS STRATUM.
SILT-CLAY MEDIUM STIFF 4 TO 8 0.5 TO 1.0	INFERRED ROCK LINE PIEZOMETER INSTALLATION RT - RECOMPACTED TRIAXIAL TOTAL ACCUMPANTAL CONTROL TO THE PROPERTY OF THE PROPE	COMPLETE ROCK REDUCED TO SOIL, ROCK FABRIC NOT DISCERNIBLE, OR DISCERNIBLE ONLY IN SMALL AND	RESIDUAL (RES.) SOIL - SOIL FORMED IN PLACE BY THE WEATHERING OF ROCK.
MATERIAL STIFF 8 TO 15 1 TO 2 (COHESIVE) VERY STIFF 15 TO 30 2 TO 4	SAMPLE SLOPE INDICATOR	SCATTERED CONCENTRATIONS. QUARTZ MAY BE PRESENT AS DIKES OR STRINGERS. SAPROLITE IS ALSO AN EXAMPLE.	ROCK QUALITY DESIGNATION (ROD) - A MEASURE OF ROCK QUALITY DESCRIBED BY TOTAL LENGTH OF
HARD >30 >4	25/825 DIP & DIP DIRECTION OF INSTALLATION CBR - CALIFORNIA BEARING RATIO SAMPLE RATIO SAMPLE	ROCK HARDNESS	ROCK SEGMENTS EQUAL TO OR GREATER THAN 4 INCHES DIVIDED BY THE TOTAL LENGTH OF CORE RUN AND EXPRESSED AS A PERCENTAGE.
TEXTURE OR GRAIN SIZE	- SPT N-VALUE	VERY HARD CANNOT BE SCRATCHED BY KNIFE OR SHARP PICK, BREAKING OF HAND SPECIMENS REQUIRES	SAPROLITE (SAP.) - RESIDUAL SOIL THAT RETAINS THE RELIC STRUCTURE OR FABRIC OF THE
U.S. STD. SIEVE SIZE 4 10 40 60 200 270 OPENING (MM) 4.76 2.00 0.42 0.25 0.075 0.053	SOUNDING ROD REF SPT REFUSAL	SEVERAL HARD BLOWS OF THE GEOLOGIST'S PICK.	PARENT ROCK. SILL - AN INTRUSIVE BODY OF IGNEOUS ROCK OF APPROXIMATELY UNIFORM THICKNESS AND
COARCE EINE	ABBREVIATIONS	HARD CAN BE SCRATCHED BY KNIFE OR PICK ONLY WITH DIFFICULTY. HARD HAMMER BLOWS REQUIRED TO DETACH HAND SPECIMEN.	RELATIVELY THIN COMPARED WITH ITS LATERAL EXTENT, THAT HAS BEEN EMPLACED PARALLEL TO THE BEDDING OR SCHISTOSITY OF THE INTRUDED ROCKS.
BOULDER	AR - AUGER REFUSAL HI HIGHLY # - MOISTURE CONTENT BT - BORING TERMINATED MED MEDIUM V - VERY	MODERATELY CAN BE SCRATCHED BY KNIFE OR PICK, GOUGES OR GROOVES TO 0.25 INCHES DEEP CAN BE	SLICKENSIDE - POLISHED AND STRIATED SURFACE THAT RESULTS FROM FRICTION ALONG A FAULT OR
GRAIN MM 305 75 2.0 0.25 0.05 0.005	CL CLAY MICA MICACEOUS VST - VANE SHEAR TEST	HARD EXCAVATED BY HARD BLOW OF A GEOLOGIST'S PICK. HAND SPECIMENS CAN BE DETACHED BY MODERATE BLOWS.	SLIP PLANE.
SIZE IN. 12 3	CPT - CONE PENETRATION TEST MOD MODERATELY WEA WEATHERED CSE COARSE NP - NON PLASTIC 7 - UNIT WEIGHT	MEDIUM CAN BE GROOVED OR GOUGED 0.05 INCHES DEEP BY FIRM PRESSURE OF KNIFE OR PICK POINT.	STANDARD PENETRATION TEST (PENETRATION RESISTANCE) (SPT) - NUMBER OF BLOWS (N OR BPF) OF A 140 LB. HAMMER FALLING 30 INCHES REQUIRED TO PRODUCE A PENETRATION OF 1 FOOT INTO SOIL WITH
SOIL MOISTURE - CORRELATION OF TERMS	DMT - DILATOMETER TEST ORG ORGANIC $\gamma_{ m d}$ - DRY UNIT WEIGHT DPT - DYNAMIC PENETRATION TEST PMT - PRESSUREMETER TEST	HARD CAN BE EXCAVATED IN SMALL CHIPS TO PEICES 1 INCH MAXIMUM SIZE BY HARD BLOWS OF THE POINT OF A GEOLOGIST'S PICK.	A 2 INCH OUTSIDE DIAMETER SPLIT SPOON SAMPLER. SPT REFUSAL IS PENETRATION EQUAL TO OR LESS
SOIL MOISTURE SCALE FIELD MOISTURE GUIDE FOR FIELD MOISTURE DESCRIPTION GUIDE FOR FIELD MOISTURE DESCRIPTION	e - VOID RATIO SAP SAPROLITIC FIAD - FILLED IMMEDIATELY AFTER DRILLING	SOFT CAN BE GROVED OR GOUGED READILY BY KNIFE OR PICK. CAN BE EXCAVATED IN FRAGMENTS FROM CHIPS TO SEVERAL INCHES IN SIZE BY MODERATE BLOWS OF A PICK POINT. SMALL, THIN	THAN 0.1 FOOT PER 60 BLOWS. STRATA CORE RECOVERY (SREC.) - TOTAL LENGTH OF STRATA MATERIAL RECOVERED DIVIDED BY TOTAL LENGTH
- SATURATED - USUALLY LIQUID; VERY WET, USUALLY	F - FINE SD SAND, SANDY FOSS FOSSILIFEROUS SL SILT, SILTY	PIECES CAN BE BROKEN BY FINGER PRESSURE.	OF STRATUM AND EXPRESSED AS A PERCENTAGE.
(SAT,) FROM BELOW THE GROUND WATER TABLE	FRAC FRACTURED, FRACTURES SLI SLIGHTLY FRAGS FRAGMENTS TCR - TRICONE REFUSAL	VERY CAN BE CARVED WITH KNIFE. CAN BE EXCAVATED READILY WITH POINT OF PICK, PIECES 1 INCH SOFT OR MORE IN THICKNESS CAN BE BROKEN BY FINGER PRESSURE. CAN BE SCRATCHED READILY BY	STRATA ROCK QUALITY DESIGNATION (SROD) - A MEASURE OF ROCK QUALITY DESCRIBED BY TOTAL LENGTH OF ROCK SEGMENTS WITHIN A STRATUM EQUAL TO OR GREATER THAN 4 INCHES DIVIDED BY THE
PLASTIC SEMISOLIDA PEDILIPES DRYING TO		FINGERNAIL.	TOTAL LENGTH OF STRATA AND EXPRESSED AS A PERCENTAGE. 10PSOIL (TS.) - SURFACE SOILS USUALLY CONTAINING ORGANIC MATTER.
RANGE - WET - (W) SCHISCILLY MOISTURE - WET - (W) ATTAIN OPTIMUM MOISTURE	EQUIPMENT USED ON SUBJECT PROJECT	FRACTURE SPACING BEDDING TERM SPACING TERM THICKNESS	
PLC - FLASTIC CIPIL	DRILL UNITS: ADVANCING TOOLS: HAMMER TYPE:	VERY THICKLY BEDDED > 4 FEET	BENCH MARK: BM*2: 8-INCH SPIKE IN 12* PINE 40' RT OF -BL- STA 15+14
OM OPTIMUM MOISTURE - MOIST - (M) SOLID; AT OR NEAR OPTIMUM MOISTURE	MOBILE B CLAY BITS	WIDE 3 TO 10 FEET THICKLY BEDDED 1.5 - 4 FEET	ELEVATION: 1688.89 FT.
SL SHRINKAGE LIMIT	6° CONTINUOUS FLIGHT AUGER CORE \$17F:	MODERATELY CLOSE	NOTES:
- DRY - (D) ATTAIN OPTIMUM MOISTURE	BK-51 8* HOLLOW AUGERS	VERY CLOSE LESS THAN 0.16 FEET THINLY LAMINATED < 0.008 FEET	
PLASTICITY	CME-45C HARD FACED FINGER BITS N-N XWL	INDURATION	,
PLASTICITY INDEX (PI) DRY STRENGTH	TUNGCARRIDE INSERTS	FOR SEDIMENTARY ROCKS, INDURATION IS THE HARDENING OF THE MATERIAL BY CEMENTING, HEAT, PRESSURE, ETC.	
NONPLASTIC Ø-5 VERY LOW LOW PLASTICITY 6-15 SLIGHT	CME-550	FRIABLE RUBBING WITH FINGER FREES NUMEROUS GRAINS; GENTLE BLOW BY HAMMER DISINTEGRATES SAMPLE.	
MED. PLASTICITY 16-25 MEDIUM	PORTABLE HOIST TRICONE STEEL TEETH POST HOLE DIGGER	MODERATELY INDURATED GRAINS CAN BE SEPARATED FROM SAMPLE WITH STEEL PROBE:	
HIGH PLASTICITY 26 OR MORE HIGH COLOR	TRICONE *TUNG,-CARB. HAND AUGER	BREAKS EASILY WHEN HIT WITH HAMMER.	
	CORE BIT SOUNDING ROD	INDURATED GRAINS ARE DIFFICULT TO SEPARATE WITH STEEL PROBE; DIFFICULT TO BREAK WITH HAMMER.	
DESCRIPTIONS MAY INCLUDE COLOR OR COLOR COMBINATIONS (TAN, RED, YELLOW-BROWN, BLUE-GRAY). MODIFIERS SUCH AS LIGHT, DARK, STREAKED, ETC. ARE USED TO DESCRIBE APPEARANCE.	VANE SHEAR TEST	EXTREMELY INDURATED SHARP HAMMER BLOWS REQUIRED TO BREAK SAMPLE;	
The state of the s		SAMPLE BREAKS ACROSS GRAINS.	
			REVISED 02/23/06



STATE OF NORTH CAROLINA DEPARTMENT OF TRANSPORTATION

MICHAEL F. EASLEY **GOVERNOR**

LYNDO TIPPETT SECRETARY

May 1, 2007

STATE PROJECT: 33715.1.1 (B-4466)

F. A. PROJECT:

BRZ-1104 (11)

COUNTY:

Clay

DESCRIPTION:

Bridge No. 4 on SR-1104 over Brasstown Creek

SUBJECT:

Geotechnical Report – Foundation Investigation

This project is located in southwest Clay County, approximately 1.5 miles from the border with Georgia. The proposed structure is a double-span, each of 81.0 feet. The skew is 90 degrees. The subsurface investigation was conducted using a CME-550 drill machine with an automatic drive hammer. The borings were advanced using both eight-inch hollow stem augers, and NW casing with advancer. Standard Penetration Tests were performed at intervals of 5.0 feet. Soil samples were collected and submitted for testing of quality. Rock core was retrieved using NXWL equipment. One (1) rock core sample was submitted for testing for Unit Weight, Compressive Strength (Qu), Young's Modulus (E) and Split Tensile Strength.

Physiography and Geology

The project site is located in moderately rolling countryside. Except for EB1-B, the bridge lies within a broad, flat floodplain. Adjacent to EB1-B is moderate to steep hillside. Brasstown Creek is incised approximately 10 feet into the floodplain. The geology of the area is the Brevard Fault Zone.

Roadway embankment was not encountered in the borings for Interior Bent-One. In the borings for the End Bents, embankment ranged from 1.0 to 5.0 feet thick and consisted of sandy gravels, silty sands, sandy silts and silty clays. Alluvium was encountered in each of the borings. It consists generally of loose silty sand with some soft sandy silts. A basal gravel layer is common.

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LOCATION: **CENTURY CENTER COMPLEX** 1020 BIRCH RIDGE DRIVE RALEIGH NC 27610

Saprolite was not found above weathered rock (of schist) in the borings for End Bent One. Elsewhere, it was encountered between the elevations of 1647.8 and 1653.0. Weathered rock (of schist) was encountered in each of the borings. The elevations to the top of weathered rock (of schist) are 1651.8 to 1653.4. At the EB1-B site, a saprolite layer is present within the weathered rock (of schist) between the elevations 1649.9 and 1650.7.

Crystalline rock (schist) was encountered in each of the borings at this site. The elevation of the top of crystalline rock (schist) ranges across the site from 1643.0 (B1-A) to 1652.3 (EB1-A).

Rock Properties

The rock underlying this project is a member of the Dean Formation, labeled "Zd" on the Geologic Map of North Carolina (1985). The Dean Formation is mostly a light gray, sericite schist with abundant cross biotite porphyroblasts. The recovered core was predominately slightly weathered to fresh, and hard. Trace amounts of pyrite and chlorite were noted. The few naturally occurring discontinuities occurred as partings along schistocity at 50° and joints at 45°. The rock also tends to part readily along the schistocity during the drilling operation. The Recoveries across the site ranged from 81 to 100 percent (95 percent average). The RQD's ranged from 0 to 100 percent (89 percent average).

Groundwater

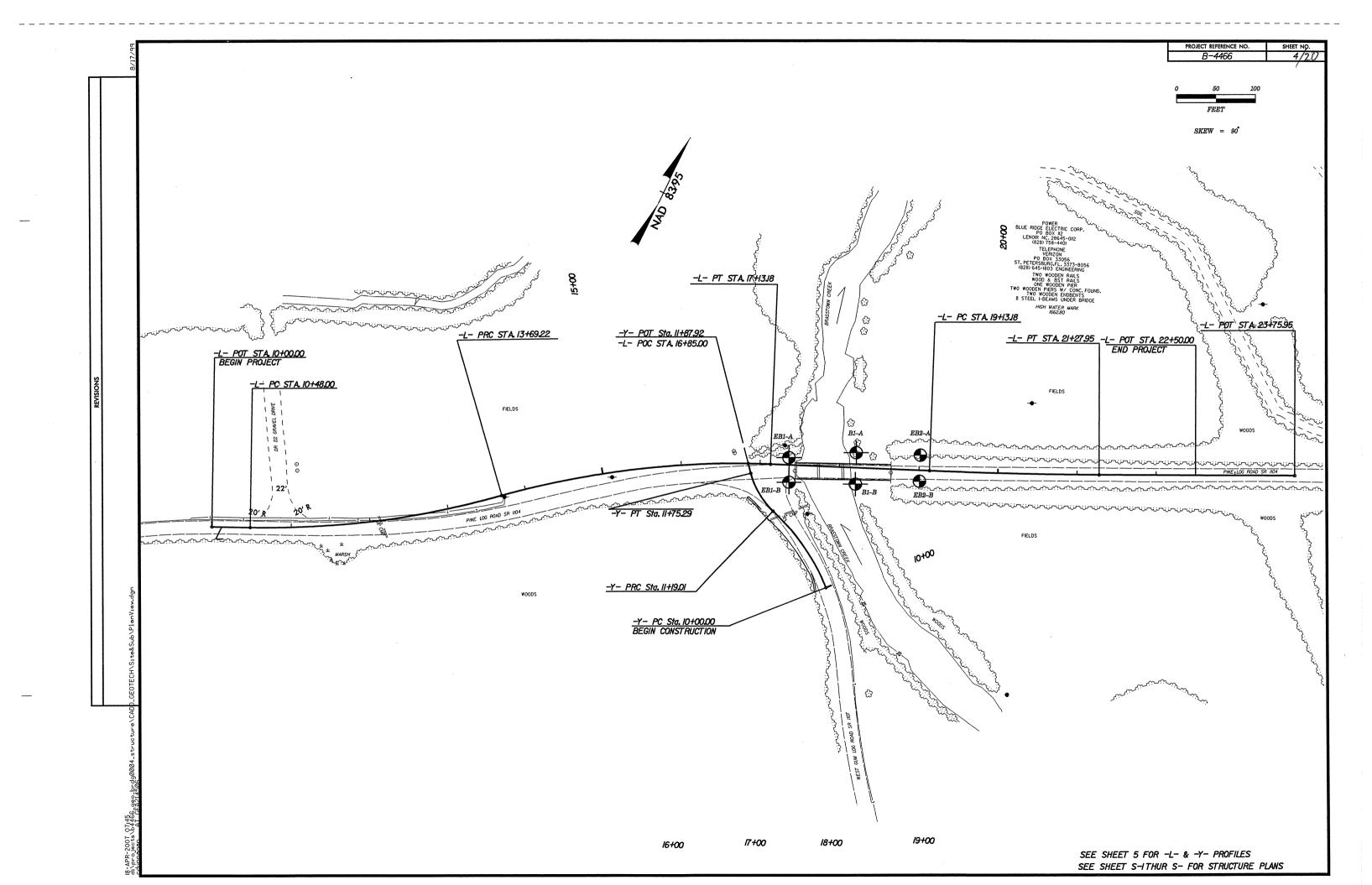
Static groundwater was measured in 4 of the 6 borings, using non-extraordinary means. The groundwater elevations ranged from 1654.3 to 1655.7.

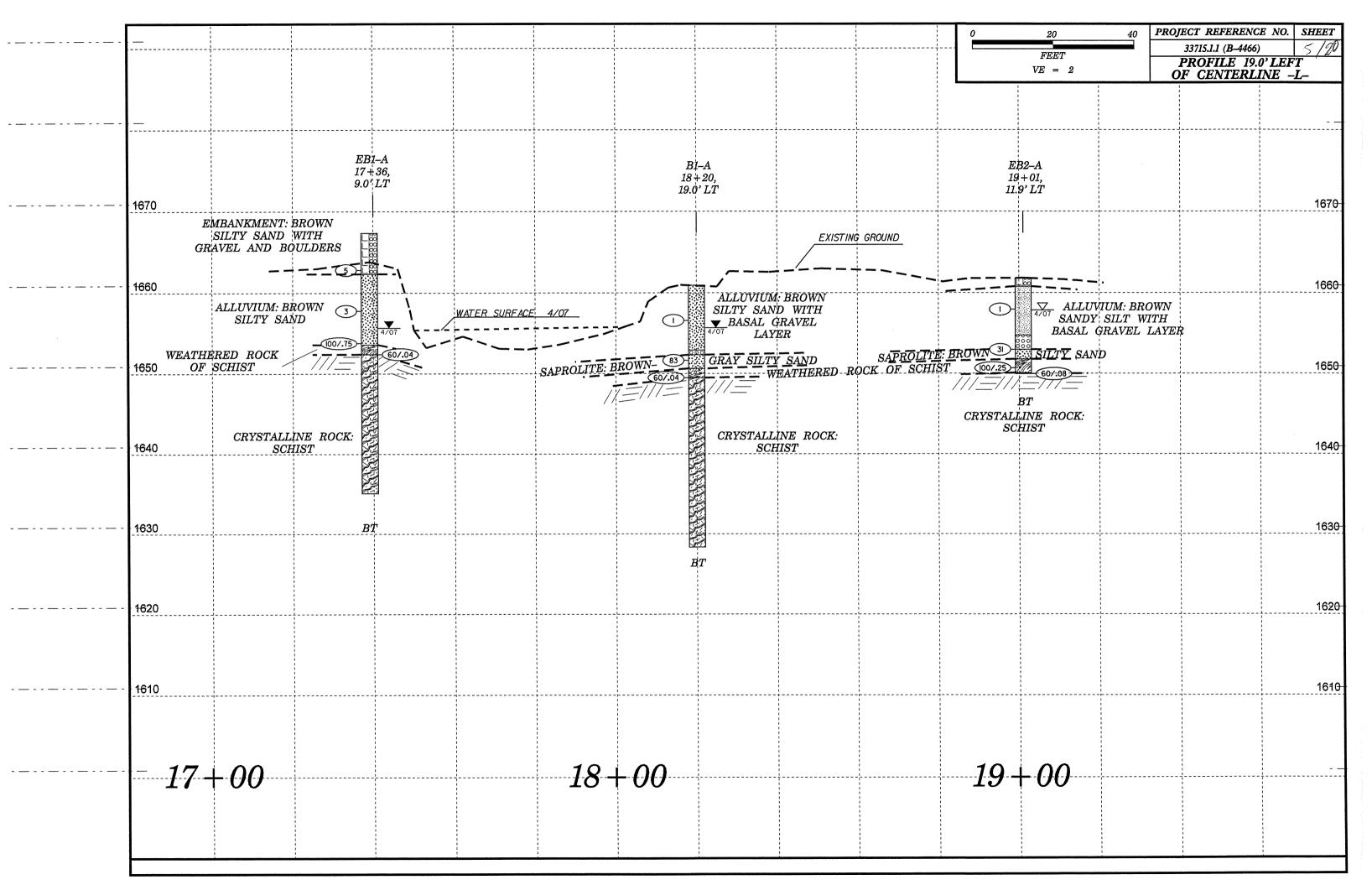
Comments

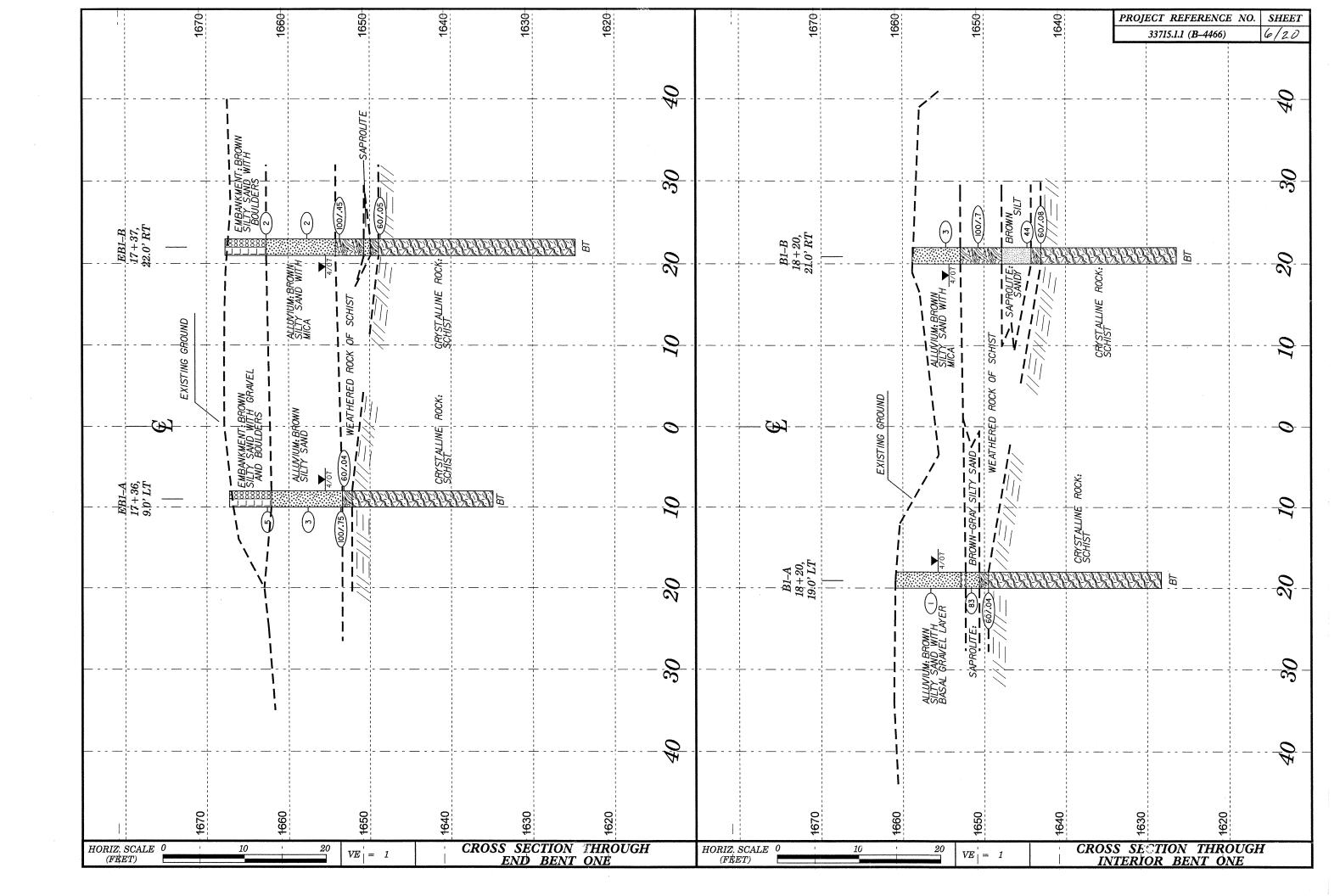
This geotechnical foundation investigation was based on the Preliminary General Drawing received January 19, 2007. Additional information was gleaned from the Hydraulic Design Report dated January 10, 2007. If any significant changes are made in the design or location of the proposed structure, the subsurface information will have to be reviewed and modified as necessary.

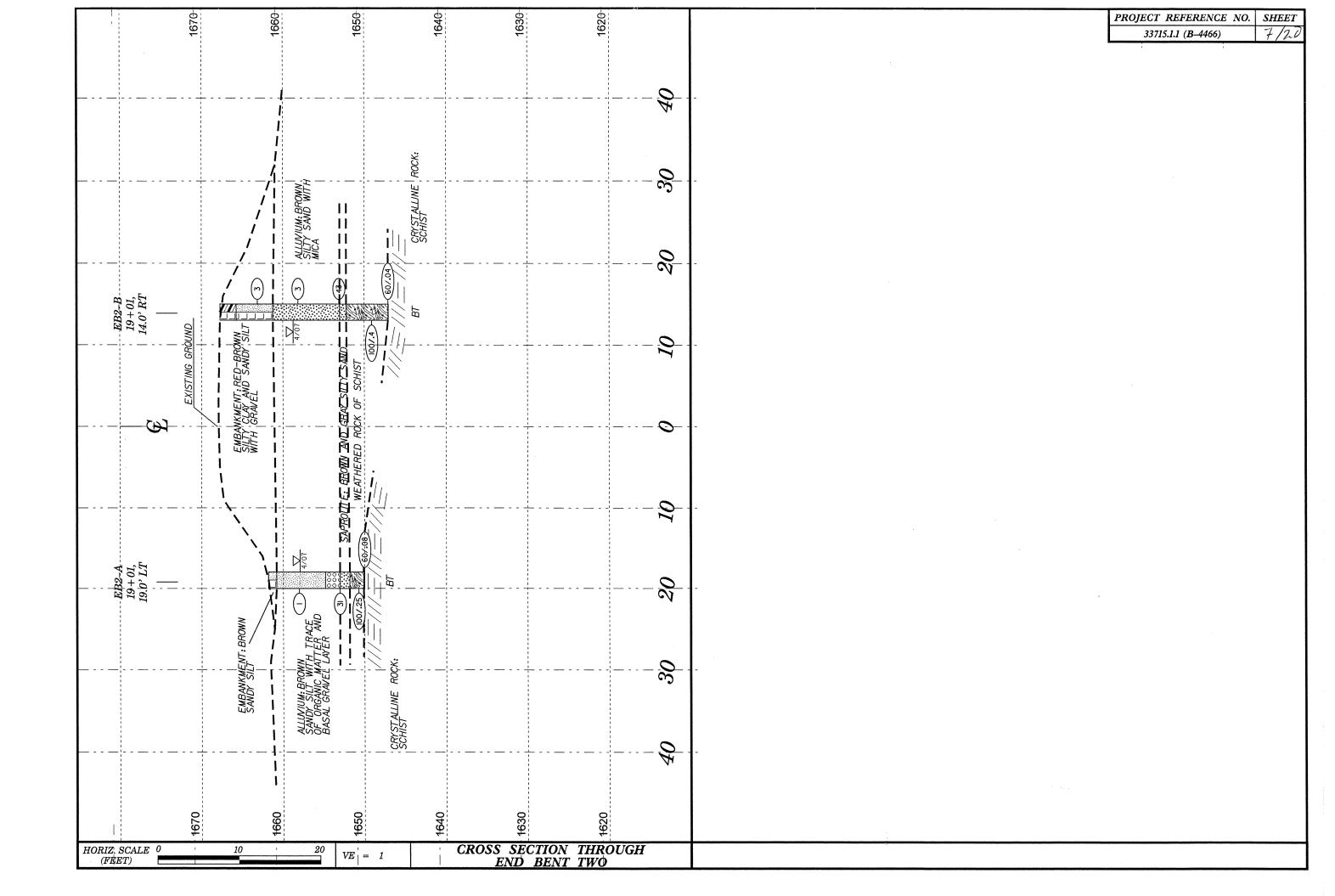
Respectfully Submitted,

Charles A. Dunnagan, L.G. Project Geological Engineer









PROJE	CT NO.	3371	5.1.1		D.	B-446	6				СО	UNTY	Clay	··········			To	GEOLOGIST	Γ Daniel, T. B.	
						SR-110		er Bras	stowi	n Cre							I_			O WTR (ft)
	G NO.				_	TATION			· · · · · · · · · · · · · · · · · · ·			FSET	9ft LT			ALIC	SNMENT	· -L-	0 HR.	
	R ELEV		7.2 ft		┞	OTAL D			3 ft		-		IG 491				TING 5		24 HR.	
	MACHIN			·	⊢	RILL M				sina w	┸								TYPE Automati	·····
	DATE			`	┢	OMP. D				,g			E WATE	R DE	PTH	N/A		 	ROCK 14.9 ft	
		·	ow cor	INT	۲	J		LOWS		OOT	100	10.70	SAMP.	V/	7 7			100, 11110	710011 14.511	
ELEV (ft)	DEPTH (ft)	0.5ft	0.5ft	0.5ft	1	0	25		50		' 5	100	NO.	МО	0 1 G	ELEV. (ft)		IL AND ROCK	DESCRIPTION	DEPTH (f
1670 _							!		-1		L) MIC		ELLV. (II)				DEF TITE
•						-										- 1,667.2		GROUND		0.
-							$\exists \mid \exists$		T : :	• •	T : :				E co				WBANKMENT gravel and boulders	S.
1,663.6-	- 3.6				_	1	. .		1		١				F	-		•		
-	<u> </u>	2	2	3		• 5	: :	: : :	::		: :			М	<u> </u>	1,662.1		ALLU	VIAI	5.
- -						<u> </u>	<u> </u>		 ` `							-		Brown si		
,658.6	- 8.6 -	0	1	2	+	3 : :	: :	: : :	::	• • •	: :	::		w		- -				
	•					T° : :	: :	: : :	::	• • •	: :	::		V		-				
 1,653.6-	13.6					1	: :				١			·		 - 1,653.4		:	•	13.
1,652.3	14.9	49 60/.04	51/.25				\Box	777			[· ·]	00/.75 60/.04	3	Sat.		1,652.3		WEATHER Weathered re		14.
-	t	007.04				<u> </u>	<u> </u>		• •	• •	<u> · · ·</u>	501.54					L	CRYSTALL	INE ROCK	
-						: : :	: :		::		: :	::	1			_		Gray serio	ite schist.	
-							: :		::	• •	: :	::		·		- -				
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					+	<u> </u>	<u> </u>		<u> </u>	<u> </u>				ļ		1,634.9	Boring 1	Terminated at	Elevation 1,634.9 ft	.32.
-					l											-	Doming	Crystalline F	Rock (schist)	""
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SHEET _1_ OE1_

							DATE 42 A 07
					CC	RE	BORING REPORT
PROJE	CT: _	33715	<u>5.1.1</u>	I. D. NO:	B-4	466	BORING NO: <u>EB1-A</u> GEOLOGIST: <u>C A Dunnagan</u>
DESCR	IPTION:	Photocolonia	Bridge	: No. 4 on	SR-1104	over Br	rasstown Creek
COUNT	Y:	Clay			COLLA	AR ELEV	VATION: <u>1667.2</u> FT. TOTAL DEPTH: <u>32.3</u> FT.
ELEV. (FEET)	1 1		RUN	REC. FEET %	RQD. FEET %	SAMP.	FIELD CLASSIFICATION AND REMARKS
1652.2		IVII) 4.71 1.	(1 km km 1	2.2	0.0	#	Gray and brown sericite schist. Moderately severe to moderately
	- s		2.3	-96	0		weathered. Soft to hard. a) Parts along schistocity @ 50°. b) Joints @ 45°.
1649.9 1649.9				4.9	4.3		17.6ft
			5.0	4.9 98	86		
1644.9 1644.9	1		_			<u> </u>	Gray sericite schist. Fresh, hard with moderately weathered zone
	-		5.0	4.9 98	4.9 98	·	from 18.2ft to 18.5ft. Trace of pyrite; trace of chlorite.
1639.9 1639.9	1			90	30		
			5.0	4.6	4.6		
1634.9	32.3			92	92		
				·			
					(25)		
						ν	CORING TERMINATED AT ELEVATION 1634.9 FT.
Di	RILLER:	C J Co	offey	- 		CORE	E SIZE: NXWL EQUIPMENT: CME-550x

NCDOT GEOTECHNICAL ENGINEERING UNIT BORELOG REPORT

PROJE	CT NO.	3371	5.1.1	IE).	B-4466			COUNTY	Clay				GEOLOGIST Dan	iel, T. B.	
SITE D	ESCRIP	TION	Bridge	No. 4	on	SR-1104	over Bras	stown Cre	ek						GROUND \	WTR (ft)
BORIN	G NO.	EB1-B			S	TATION 1	7+37		OFFSET	22ft R	Т		ALIGNMEN	T -L-	0 HR.	12.0
COLLA	RELEV	. 1,66	7.7 ft		T	OTAL DEP	TH 42.9	ft	NORTHIN	IG 491	1,341		EASTING	525,044	24 HR.	12.3
DRILL	MACHIN	IE CM	1E-550)	<u> </u>	DI	RILL METH	HOD NV	V Casing w	/ SPT Core	9				HAMMER TYPE	Automatic	
START	DATE	, <u>.</u>			C	OMP. DATI	E 04/10/	/07	SURFAC			A	N/A	DEPTH TO ROCI	K 18.8 ft	
ELEV (ft)	DEPTH (ft)	0.5ft	0.5ft	0.5ft		0 25		PER FOOT 50 7	5 100	SAMP. NO.		L 0 1 G	SC ELEV. (ft)	OIL AND ROCK DESCR		DEPTH (ft)
1670 _	-												•	•		
1070	-											-		0001110 0110		
1	-			<u> </u>	Ħ						<u> </u>		1,667.7	ROADWAY EMBANKI	MENT	0.0
1,663.7	- - 4.0							<u> </u>				<u>FI</u>	•	Brown sandy silt	•	ŀ
1,003.72	- 4.0	1 .	1	1		P 2 · · ·				SS-4	м	LI.	1,662.7	41110041	·	5.0
]	_					::::					1	F	i c	ALLUVIAL Dark brown sandy silt wi	th mica.	
1,658.7	9.0	0	1	1							١.,	F				
]			•	'		Q 2				SS-5	W	F		:		
1,653.7	- 14.0					<u> </u>							1,654.2		· · · · · · · · · · · · · · · · · · ·	13.5
	_	100/.45				: : : :			100/.45		М			WEATHERED ROO Weathered rock of so		
	-					• • • •						4	1,650.7 1,649.9	SAPROLITE	·	17.0 17.8
1,648.7	<u>- 19.0</u> -	60/.05			Ш	:			60/.05				1 648.9	Silty sand.		18.8
1 1	-					: : : :								WEATHERED ROC Weathered rock of so	hist.]
	-				$\ \cdot\ $		·						•	Gray sericite schis		
	-				$\ $											
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						::::			::::							İ
					片			,				-	1,624.8 Boring	Terminated at Elevation	1,624.8 ft in	42.9
	:													Crystalline Rock (sch	ist)	
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						יחר	DATE 13-Apr-07
					CO	/KΕ I	BORING REPORT
PROJEC	OT: _	33715	<u>5.1.1</u>	I. D. NO:	B-44	<u> 466 </u>	BORING NO: <u>EB1-B</u> GEOLOGIST: <u>C A Dunnagan</u>
DESCR	IPTION:		Bridge	No.4 on	SR-1104	over Bra	asstown Creek
COUNT	Y:	Clay		-	COLLA	√R ELE\	VATION: <u>1667.7</u> FT. TOTAL DEPTH: <u>42.9</u> FT.
ELEV. (FEET)	DEPTH (FEET)	i .	RUN (FEET)	REC. FEET %	RQD. FEET %	SAMP.	FIELD CLASSIFICATION AND REMARKS
1647.5							
			2.7	2.2	2.2 ،		
1644.8				81	81		
1644.8	22.9		50	5.0	5.0		
1639.8	27.9		5.0	100	100	:	
1639.8				5.0	5.0		Medium dark gray to gray sericite schist with cross-biotite. Fresh, hard. Trace of biotite; trace of chlorite.
4004.0	22.0	·	5.0	100	100		
1634.8 1634.8				5.0	5.0		
	,		5.0	100	100		
1629.8 1629.8							
			5.0	4.8	4.8		
1624.8	42.9			96	96		
	,						
					Ş.		
			<i>z</i> .				CORING TERMINATED AT ELEVATION 1624.8 FT.
DF	RILLER:	C J Co	offey	<u>-</u> , ' .		CORI	E SIZE: NXWL EQUIPMENT: CME-550x

PROJE	ECT NO.	3371	5.1.1	. 10).	B-4466			COUNTY	Clay				GEOLOGIST Dan	iel, T. B.	
							over Bra	sstown Cre						<u> </u>	GROUND	WTR (ft)
	IG NO.					TATION 1			OFFSET	19ft L7	Т		ALIGNME	NT -L-	0 HR.	N/A
	AR ELEV		0.9 ft			TAL DEP		6 ft	NORTHI				EASTING	525,103	24 HR.	5.2
	MACHIN							N Casing w	<u></u>		·			HAMMER TYPE	Automatic	
	DATE					MP. DAT			SURFAC		R DE	PTH	N/A	DEPTH TO ROCI		
ELEV	DEPTH		OW COL	I JNT	П			PER FOOT	1	SAMP.	V /	1-1		· ·		
(ft)	(ft)	0.5ft	0.5ft	0.5ft	11	0 2			,5 100	NO.	MOI	0 G	ELEV. (ft)	SOIL AND ROCK DESCR	RIPTION	DEPTH (ft)
					\prod	· · · · · · · · · · · · · · · · · · ·							- Control of the Cont			
1665													•			
1005	‡															
	‡													GROUND SURFAC	SE	
-	<u> </u>		ļ		Ħ					 	-		1,660.9	ALLUVIAL		0.0
1,657.6	3.3					::::		: : : :					, ,	Brown silty sand	• .	
	‡	0	0	1	1	1 : : :		::::		SS-2	Sat.					
-	<u> </u>				ŀ			 				1 :	-			
1,652.6	8.3	22	40	43	┦┞		-:-:-:	L::::			м	000	1,652.9 1,652.3	ALLUVIAL		8.0 8.6
1,649.5	111	24	70	"		• • • •			. ●83		I M	777	1,650.6 -1,649.5	Brown sand and gra	ivel	10.3
1,043.3	1	60/.04							60/.04				1,043.0	Brown-gray silty sa		
	Ī				Ш		,							WEATHERED ROO Weathered rock of so	chist.	
-	Ŧ				1								-	CRYSTALLINE RO Gray sericite schis		
	Ŧ															
	Ŧ										1		· · · · · · · · · · · · · · · · · · ·	* * * * * * * * * * * * * * * * * * *		-
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-	‡	,	-		Ш								1,628.3			32.6
	‡				l							F	Borir	ng Terminated at Elevatio Crystalline Rock (scl		·
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PROJE	OT: _	33715	.1.1	I. D. NO:	B-44		
DESCR	PTION:	-	Bridge	No. 4 on	SR-1104	over Br	asstown Creek
COUNT	Y:	Clay			COLLA	R ELEV	/ATION:1660.9 FT.
ELEV. (FEET)	DEPTH (FEET)	DRILL RATE MIN./FT.	RUN (FEET)	REC. FEET %	RQD. FEET %	SAMP.	FIELD CLASSIFICATION AND REMARKS
1649.4	11.5		4.4	0.9	, 0.9		
1648.3	12.6		1.1	82	82		
1648.3	12.6		5.0	4.9	4.9		
1643.3	17.6		3.0	98	98		
1643.3	17.6		5.0	5.0	5.0		Light gray sericite schist with cross-biotite. Slightly weathered to fresh; hard. Trace of pyrite.
1638.3	22.6		3.0	100	100		
1638.3	22.6		5.0	5.0	5.0		
1633.3	27.6		5.0	100	100		
1633.3	27.6			5.0	5.0		
1628.3	32.6		5.0	100	100		
·	•						
				, , , ,	let Termina		
							CORING TERMINATED AT ELEVATION 1628.3 FT.
DI	RILLER:	C J Co	offey			CORI	E SIZE: NXWL EQUIPMENT: CME-550x

	3371			D. B-4466	COUNTY Clay	GEOLOGIST Dar	·
TE DESCRIF	TION	Bridge	No. 4	on SR-1104 over Brasstown Cree	,	ya	GROUND WTR (f
ORING NO.	B1-B			STATION 18+20	OFFSET 21ft RT	ALIGNMENT -L-	0 HR. N//
OLLAR ELEV	/. 1,65	8.8 ft		TOTAL DEPTH 32.4 ft	NORTHING 491,377	EASTING 525,119	24 HR. 4.
RILL MACHI	NE CM	IE-550)	<	DRILL METHOD NW Casing w	/ SPT Core	HAMMER TYPE	Automatic
TART DATE	04/09/	07		COMP. DATE 04/10/07	SURFACE WATER DEPTH N//	A DEPTH TO ROC	K 15.8 ft
LEV DEPTH (ft)	0.5ft	0.5ft	O.5ft	BLOWS PER FOOT 0 25 50 7	5 100 SAMP. V L O MOI G ELI	SOIL AND ROCK DESCR EV. (ft)	RIPTION DEPTH
660						658.8 GROUND SURFA ALLUVIAL Brown silty sand with	
650.7 8.1	21	71	29/.2	-	1,6:	WEATHERED RO Weathered rock of s	
645.7 13.1 643.0 15.8	8	9	35	44	SS-3 W 1,6.	SAPROLITE Brown sandy silt 44.2 43.0 WEATHERED RO	
+ + + + + + + + + + + + + + + + + + + +	60/.08				RS-1	Weathered rock of se CRYSTALLINE RO Gray sericite schie	chist.
						26.4	
1	·	-				Boring Terminated at Elevatio Crystalline Rock (sci	n 1,626.4 ft in
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					CC	RE	BORING REPORT	13-Apr-07
PROJE	ст: _	33714	.1.1	I. D. NO:	B-44	466	BORING NO: <u>B1-B</u> GEOLOGIST: <u>C A</u>	Dunnagan
DESCR	IPTION:	***************************************	Bridge	No. 4 on	SR-1104	over B	asstown Creek	
COUNT	Y:	Clay		-	COLLA	R ELE	/ATION: <u>1658.8</u> FT. TOTAL DEPTH: <u>32.4</u>	FT.
ELEV. (FEET)	DEPTH (FEET)	1	RUN (FEET)	REC. FEET %	RQD. FEET %	SAMP.	FIELD CLASSIFICATION AND REMARKS	
1642.9			1.5	1.1	٠1.1			
1641.4			1.0	73	73			
1641.4	17.4		5.0	5.0	5.0	RS-1		
1636.4 1636.4				100	100		Light gray sericite schist with cross-biotite. Fresh, hard. Trace of pyrite; trace of chlorite.	
			5.0	5.0 100	5.0 100			
1631.4 1631.4				100	100			. :
	2		5.0	5.0	5.0			
1626.4	32.4			100	100			
÷								
						·	CORING TERMINATED AT ELEVATION 1626.4 FT.	
Di	RILLER:	C J Co	offey	_	*	COR	E SIZE: <u>NXWL</u> EQUIPMENT: <u>CME</u> -	550x

SHEET

PROJE	ECT NO.	3371	5.1.1	11	D. B-4466			COUNTY	Clay					GEOLOGIST	Daniel, T. B.	
SITE D	ESCRIP	TION	Bridge	No. 4	on SR-110	4 over Bras	sstown Cre	ek							GROUND	WTR (
BORIN	IG NO.	EB2-A			STATION	19+01		OFFSET	19 ft L	Т.		ALIG	NMENT	L-	0 HR.	3
COLLA	AR ELEV	1,66	1.8 ft		TOTAL DE	PTH 11.8	8 ft	NORTHI	NG 491	1,446		EAS	TING 5	525,176	24 HDave	ed at 4
ORILL	MACHIN	NE CN	E-550	X	DRILL ME	THOD H.	S. Augers							HAMMER TY	PE Automatic	
	DATE	<u></u>			COMP. DA			SURFAC	F WATE	FR DE	PTH	N/A		 	OCK 11.7 ft	
LEV.	ELEV.		OW COL	JNT	11		PER FOOT	100111710	SAMP.	·	1	T		1 111 10 11		
(ft)	DEPTH (ft)	0.5ft	0.5ft	0.5ft	┨,			5 100	NO.		0 G	ELEV. (ft)		IL AND ROCK DE	SCRIPTION	DEPTH
																DEFII
1665 _						•		·				_ 				
	<u> </u>				 	T	T		╂	╂	L	_ 1,661.8 _ 1,660.8		GROUND SUP ROADWAY EMBA		
1660 -	1,659.0					 	 					F		Brown sand	y silt.	\int
	2.8	0	0	1	1.				SS-1	_MZ			Brown s	ALLUVIA andy silt with trace		•
1655 -	1,654.0						 		1		000	1,654.8		ALLUVIA	.1	
	7.8	6	11	20		31 · ·				Sat.	200	1,653.0 1,651.8		Brown gravel and	boulders.	
	1,650.7	100/.25	4.7,								10	1,650.1		SAPROLI Brown silty s		\int
1650 -	11.1	60/.08						100/ 25	1			1,650.0		WEATHERED Weathered rock	ROCK .	
	ļ.											- 1	······································	CRYSTALLINE	ROCK	1
- - 1645	<u> </u>									`		<u> </u>	Boring T	Gray sericite : erminated with Sta	andard Penetration	٦
:	‡									l		_	Test	Refusal at Elevation Crystalline Rock	on 1.650.0 ft in	•
•	ŀ								1			_		· ·	(SCHSI)	
640 -	<u> </u>											<u>_</u>	•			
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- 1630	_											_				
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1625 –	_															
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	ECT NO.				D. B-44					VTNUC	Clay	,			GEOLOGIST Dar		
	DESCRIP			No. 4				stown C								GROUND W	
	IG NO.				STATIO					FSET				ALIGNME		0 HR.	9.0
	AR ELEV						TH 20.6			RTHIN	IG 49	1,416		EASTING	525,190	24 HR.	FIAD
	MACHIN			΄				S. Auger	s						HAMMER TYPE	Automatic	
	TDATE	,			COMP.		04/16			IRFAC		ER DE	_	N/A	DEPTH TO ROC	K 20.6 ft	
LEV.	ELEV. DEPTH		OW COL		- ,			PER FOO		400	SAME		[]		SOIL AND ROCK DESCI	RIPTION	
(ft)	(ft)	0.5ft	0.5ft	0.5ft	╫	25		50	75 	100	NO.	MO	I G	ELEV. (ft)			EPTH (
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•	Ī					•								1,667.7	GROUND SURFA	CE	
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665 -	T 1,664.1				-			<u> </u>	 					1,665.7	Red-brown silty cl		2
	3.6	1	2	1	3 .					: : :		м		-	Brown sandy silt with	gravel.	
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660 -	1,659.1 8.6	2	2	1	┧╠╌		• • • •	 	: :					-	Brown silty sand with	mica.	
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655 -	† †1,654.1				1			<u> </u>	<u> </u>]				-			
	13.6	10	12	31	11::	::	4:	3	: :	: : :		Sat.		1,653.0			14
	‡		ŀ		::	::	.	†	:+-				113	1,652.2	SAPROLITE Gray silty sand		<u></u>
650 -	1,649.1				 	$\overline{\cdot}$		-				1			WEATHERED RO	CK	J
	18.6 1,647.1	100/.4			1 1			<u> </u>		100/.4		M		- 1,647.1	Weathered rock of s	CHIST.	20
645 -	20.6	60/.04	1							~60/.04		<u> </u>	1 "1	1,647.1	CRYSTALLINE RO		20
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JCS NORTH CAROLINA DEPARTMENT OF TRANSPORTATION DIVISION OF HIGHWAYS-MATERIALS AND TESTS UNIT SOILS TEST REPORT-SOILS LABORATORY

T.I.P. ID #: B-	-4466							
REPORT ON SAMPI	ES OF: Soil	ls for Quality						,
PROJECT:	33715.1.1	CO	DUNTY: C	lay		Owner:	NCDOT	
DATE SAMPLED:	4.4.07	DATE	RECEIVED:	4.13.07	D	ATE REPO	ORTED:	4.19.07
SAMPLED FROM:	Bridge			PLED BY:	C. A. Dun			
SUBMITTED BY:	W. D. Frye				2002		OARD SPE	CIFICATION
LABORATORY:	Asheville		·····					
	L							
			TEST RI	COTIL TO				
D 1 (G 1 N	SS-1	SS-2			1 60.5		<u> </u>	
Project Sample No.	154935	154936	SS-3 154937	SS-4 154938	SS-5 154939			
Lab Sample No. A HiCAMS Sample #	134933	134930	134937	134936	134939			
Retained #4 Sieve %	0.0	0.0	0.0	0.0	0.0			
Passing #10 Sieve %	92	100	95	78	93			
Passing #40 Sieve %	90	100	81	70	90			
Passing #200 Sieve %	48	33	36	37	61			
				L		·		
		M	INUS #10]	ED A CTIC	NNT .			
[G 1134	T	. 141	INUS #10 I	RACIIC	<u> </u>			
Soil Mortar - 100%		1.4	26	10		<u> </u>		
Coarse Sand -Ret. #60	8 49	14 58	26 50	18	37			
Fine Sand - Ret. #270 Silt 0.05-0.005 mm %	31	18	20	19	$\frac{37}{29}$			
Clay < 0.005 mm %	12	10	4	22	28			
Passing # 40 Sieve %								
Passing # 200 Sieve %								
		L	<u> </u>	<u> </u>	······································			<u></u>
Liquid Limit	26	20	29	26	36	<u> </u>		
Plastic Index	NP	NP	NP	NP	NP NP			
AASHTO Classification	A-4 (3)	A-2-4 (0)	A-4 (0)	A-4 (0)	A-4 (5)			
Quantity					1			
Texture								
Station	19+01	18+20	18+20	17+37	17+37			
Hole No.					·			
Depth (ft) From:	3.3	3.8	13.6	4.5	9.5			
То:	4.3	4.8	14.6	5.5	10.5			
Remarks:								
A-154935 - 154939								
CC:		-						
C. A. Dunnagan								

G:/Everyone.../M&T Forms/Regional Lab Statesville/Soils Test Report M&T 503E

File

SOILS ENGINEER:

8-19-2000



FIELD SCOUR REPORT

WBS: 33715.1.1 TIP:

B-4466

COUNTY: Clay

DESCRIPTION(1): Bridge No.4 on SR-1104 over Brasstown Creek

	EX	ISTING BR	IDGE		
Information from:	Field Inspection X Other (explain)				_)
Bridge No.: 4 Foundation Type: Footi	Length: 120 Total ngs (on piles?)	Bents: 5	Bents in Chann	el: 2 Bents i	n Floodplain: 3
EVIDENCE OF SCOU Abutments or End Bo	R(2) ent Slopes: None noted.				
Interior Bents: Minor	r amounts on the upstream	(Right) sides o	of Interior Bents	1 and 2.	
Channel Bed: None	noted.				
Channel Bank: Mino	r amount at existing EB1-A.				
EXISTING SCOUR PF Type(3): Wood	ROTECTION den end-bent walls.	q			
Extent(4): Five	feet beyond width of bridge,	either side.		AND THE RESIDENCE AND THE RESI	
Effectiveness(5): Good	e la maio. El la completa la la mandrata mandrata de la completa de mandrata en mandrata de mandrata esta esta	FO 5c t	4	violina buidan	
Obstructions(b): Rock	outcrops in stream bed, ap	prox. 50 feet o	ownstream of e	existing bridge.	

INSTRUCTIONS

- 1 Describe the specific site's location, including route number and body of water crossed.
- 2 Note scour evidence at existing end bents or abutments (e.g. undermining, sloughing, degradations).
- 3 Note existing scour protection (e.g. rip rap).
- 4 Describe extent of existing scour protection.
- 5 Describe whether or not the scour protection appears to be working.
- 6 Note obstructions such as dams, fallen trees, debris at bents, etc.
- 7 Describe the channel bed material based on observation and/or samples. Include any lab results with report.
- 8 Describe the channel bank material based on observation and/or samples. Include any lab results with report.
- **9** Describe the material covering the banks (e.g. grass, trees, rip rap, none).
- 10 Determine the approximate floodplain width from field observation or a topographic map.
- 11 Describe the material covering the floodplain (e.g. grass, trees, crops).
- 12 Use professional judgement to specify if the stream is degrading, aggrading, or static.
- 13 Describe potential and direction of the stream to migrate laterally during the bridge's life (approx. 100 years).
- Give the design scour elevation (DSE) expected over the life of the bridge (approx. 100 years). This elevation can be given as a range across the site, or for each bent. Discuss the relationship between the Hydraulics Unit theoritical scour and the DSE. If the DSE is dependent on scour counter measures, explain (e.g. rip rap armoring on slopes). The DSE is based on the erodability of materials, giving consideration to the influence of joints, foliation, bedding characteristics, % core recovery, % RQD, differential weathering, shear strength, observations at existing structures, other tests deemed appropriate, and overall geologic conditions at the site.

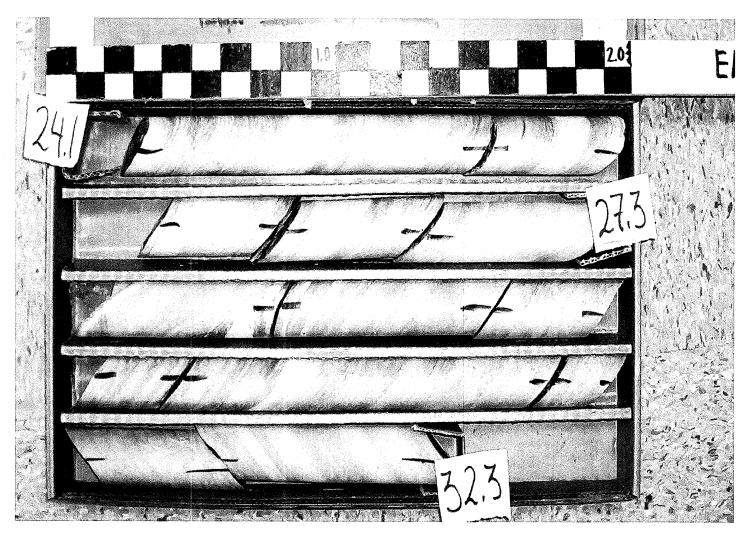
Channel Bank Material(8): Silty sand. Channel Bank Material(8): Silty sand. Channel Bank Cover(9): Cane and trees. Floodplain Width(10): EB1-B = Oft. All others greater than 100 feet. Floodplain Cover(11): Grass. Stream is(12): Aggrading Degrading X Static mel Migration Tendency(13): South-southwest bservations and Other Comments: Existing End-Bent One marks the edge of the creek. ESIGN SCOUR ELEVATIONS(14) Feet X Meters BENTS EB1 B1 1656.8 1652.2 Meters B1 is about the same; B1 is approx 4.5 feet above theoretical scour. OIL ANALYSIS RESULTS FROM CHANNEL BED AND BANK MATERIAL Send or Bank Sample No. Retained #4 Peassed #10 Peassed #40 assed #200 coarse Sand Fine Sand Sitt Clay LLL P AASHTO Station Offset Depth	Channel										
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AASHTO Station Offset	OIL ANALYS Bed or Bank Sample No. Retained #4 Passed #10 Passed #200 oarse Sand Fine Sand Silt	f DSE to H he same; E	lydraulics Uni B1 is approx 4	it theoretical s 4.5 feet above	scour: e theoret	ical scour.	TERIAL				
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Template Revised 02/07/06

Reported by: Colomogra

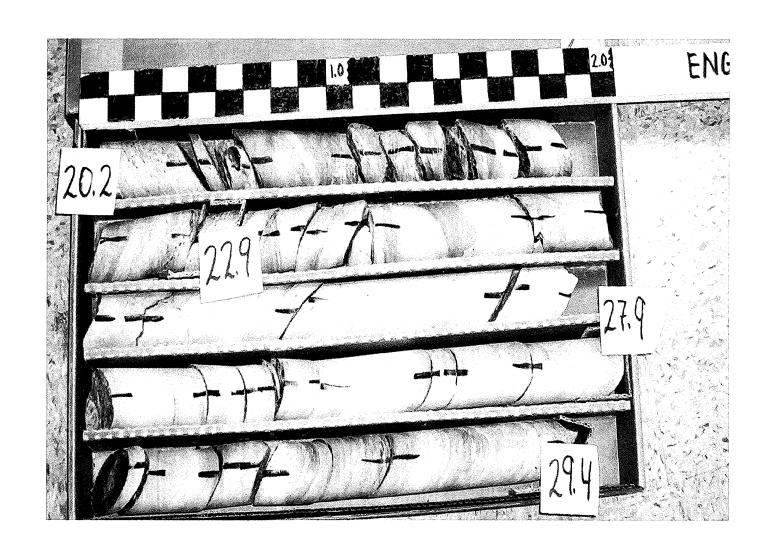
Date: 4/20/2007

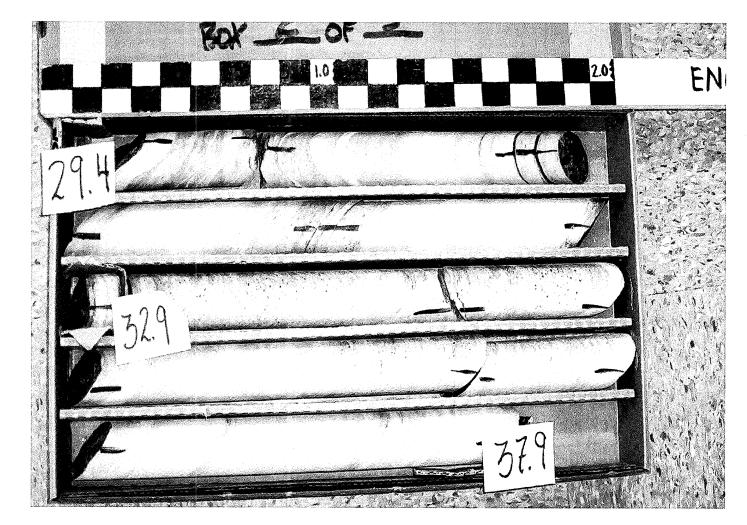




33715.1.1 B-4466
Bridge No. 4 on SR-1104
Over Brasstown Creek.
EB1-A
Box 1 of 2

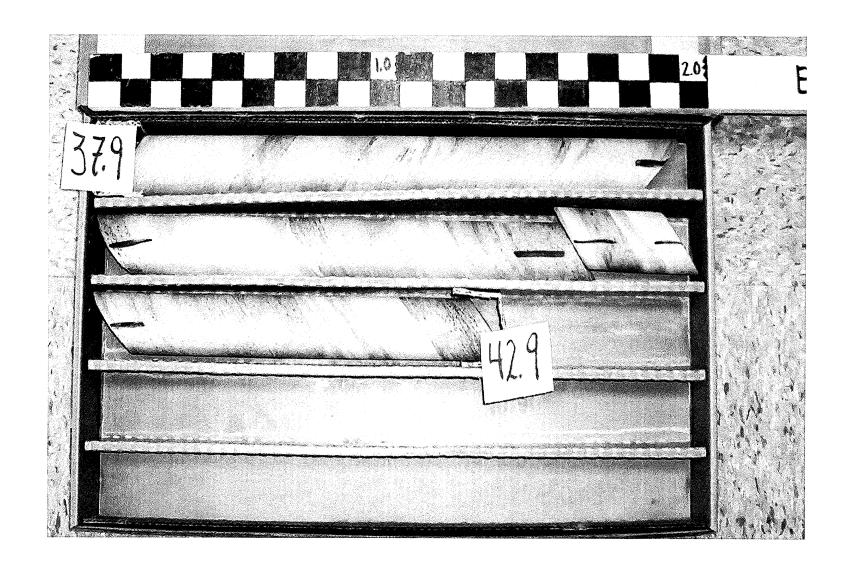
33715.1.1 B-4466
Bridge No. 4 on SR-1104
Over Brasstown Creek.
EB1-A
Box 2 of 2





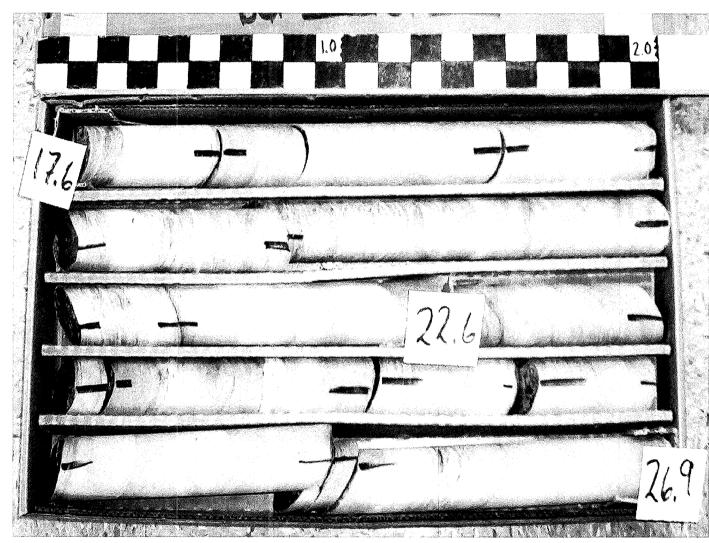
33715.1.1 B-4466
Bridge No. 4 on SR-1104
Over Brasstown Creek.
EB1-B
Box 1 of 3

33715.1.1 B-4466
Bridge No. 4 on SR-1104
Over Brasstown Creek.
EB1-B
Box 2 of 3



33715.1.1 B-4466
Bridge No. 4 on SR-1104
Over Brasstown Creek.
EB1-B
Box 3 of 3



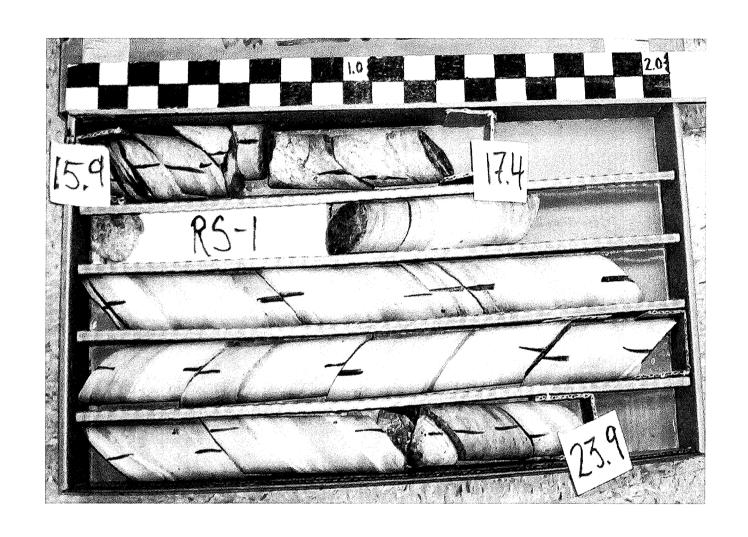


33715.1.1 B-4466
Bridge No. 4 on SR-1104
Over Brasstown Creek.
B1-A
Box 1 of 3

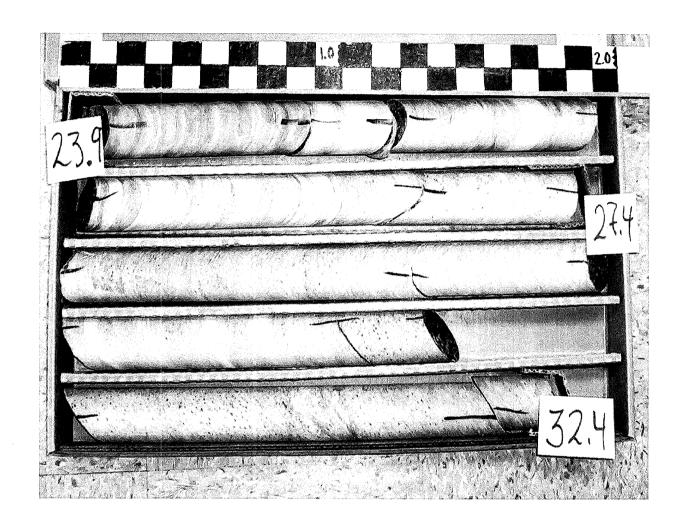
33715.1.1 B-4466
Bridge No. 4 on SR-1104
Over Brasstown Creek.
B1-A
Box 2 of 3



33715.1.1 B-4466
Bridge No. 4 on SR-1104
Over Brasstown Creek.
B1-A
Box 3 of 3



33715.1.1 B-4466
Bridge No. 4 on SR-1104
Over Brasstown Creek.
B1-B
Box 1 of 2



33715.1.1 B-4466
Bridge No. 4 on SR-1104
Over Brasstown Creek.
B1-B
Box 2 of 2