NORTH CAROLINA DEPARTMENT OF TRANSPORTATION

DIVISION OF HIGHWAYS

GEOTECHNICAL UNIT

SUBSURFACE INVESTIGATION

		SOIL ANI	D ROCK LEGEND, TERM	is, symbols,	AND ABBREVI	ATIONS	
SOIL DESCRIPTION	,	GRADATION			ROCK I	DESCRIPTION	TERMS AND DEFINITIONS
SOIL IS CONSIDERED TO BE THE UNCONSOLIDATED, SEMJ-CONSOLIDATED OR WI WHICH CAN BE PENETRATED WITH A CONTINUOUS FLIGHT POWER AUGER, AND W 100 BLOWS PER FOOT ACCORDING TO STANDARD PENETRATION TEST (AGSHIO CLASSIFICATION IS BASED ON THE AASHTO SYSTEM AND BASIC DESCRIPTIONS CONSISTENCY, COLOR, TEXTURE, MOISTURE, AASHTO CLASSIFICATION, AND OTHER AS MINERALOGICAL COMPOSITION, ANGULARITY, STRUCTURE, PLASTICITY, ETC. EX	HICH YIELDS LESS THAN 1206, ASTM D-1586), SOIL GENERALLY SHALL INCLUDE: PERTINENT FACTORS SUCH AMPLE:	MELL GROCE: INDICATES A GOOD REPRESENTATION OF PARTICLE UNIFORM INDICATES THAT SOIL PARTICLES ARE ALL APPROXIMA POORLY GRADED: OAP-GRADED: INDICATES A MIXTURE OF UNIFORM PARTICLES OF ANGULARITY OF COMMENT OF THE ANGULARITY OF ROUNDIESS OF SOIL GRAINS ARE DESIGNATION SUBANGULAR, SUBROUNDED, OR ROUNDED.	TELY THE SAME SIZE, IALSO TWO OR MORE SIZES, GRAINS	SPT REFUSAL IS PE IN NON-COASTAL PL OF WEATHERED ROCK	ES THE LEVEL AT WHICH NON-CE METRATION BY A SPLIT SPOON AIN MATERIAL, THE TRANSITIO K. RE TYPICALLY DIVIDED AS FOLO NON-COASTAL PL	T WHEN TESTED, WOULD YIELD SPT REFUSAL, AN INFERRED COASTAL PLAIN MATERIAL WOULD YIELD SPT REFUSAL. SAMPLER EQUAL TO OR LESS THAN QUI FOOT PER 60 BLOWS ON BETWEEN SOIL AND ROCK IS OFTEN REPRESENTED BY A ZOUNG. LAIN MATERIAL THAT YIELDS SPT N VALUES > 100 BLOWS	ALLUYIUM (ALLUV.) - SOILS WHICH HAVE BEEN TRANSPORTED BY WATER. ADUIFER - A WATER BEARING FORMATION OR STRATA. ARENACEOUS - APPLIED TO ROCKS THAT HAVE BEEN DERIVED FROM SAND OR THAT CONTAIN SAND. ARGILLACEOUS - APPLIED TO ALL ROCKS OR SUBSTANCES COMPOSED OF CLAY MINERALS, OR HAVING A NOTABLE PROPORTION OF CLAY IN THEIR COMPOSITION, AS SHALE, SLATE, ETC.
SOIL LEGEND AND AASHTO CLASSIF		MINERALOGICAL COMP		CRYSTALLINE	FINE TO COARSE	GRAIN IGNEOUS AND METAMORPHIC ROCK THAT	ARTESIAN - GROUND WATER THAT IS UNDER SUFFICIENT PRESSURE TO RISE ABOVE THE LEVEL AT WHICH IS IS ENCOUNTERED, BUT WHICH DOES NOT NECESSARILY RISE TO OR ABOVE THE
GENERAL GRANULAR MATERIALS SILT-CLAY MATERIALS CLASS. (±35% PASSING *200) (*25% PASSING *200)	ORGANIC MATERIALS	MINERAL NAMES SUCH AS QUARTZ, FELDSPAR, MICA, TALC, KAOLIN, WHENEVER THEY ARE CONSIDERED OF SIGNIFICANCE.	ETC. ARE USED IN DESCRIPTIONS	ROCK (CR)	GNEISS, GABBRO,	SCHIST, ETC.	GROUND SURFACE. CALCAREOUS (CALC.) - SOILS WHICH CONTAIN APPRECIABLE AMOUNTS OF CALCIUM CARBONATE.
GROUP CLASS. A-1-a A-1-b A-2-4 A-2-5 A-2-6 A-2-7 A-4 A-5 A-6 A-6 A-6 A-1-a A-1-b A-2-4 A-2-5 A-2-6 A-2-7 A-4 A-2-6 A-2-7 A-2-6 A-2-6 A-2-7 A-2-6 A-2-7 A-2-6 A-2-6 A-2-7 A-2-6 A-2-7 A-2-6 A-2-6 A-2-6 A-2-7 A-2-6 A-2-6 A-2-7 A-2-6 A-2-6 A-2-7 A-2-6 A-2-6 A-2-7 A-2-6 A-2-6 A-2-6 A-2-7 A-2-6 A-2-6 A-2-6 A-2-7 A-2-6 A-2-6 A-2-6 A-2-7 A-2-6 A	77 A-1, A-2 A-4, A-5 7-6 A-3 A-6, A-7	COMPRESSIBIL SLIGHTLY COMPRESSIBLE	DUID LIMIT LESS THAN 30	NON-CRYSTALLINE ROCK (NCR)	SEDIMENTARY ROO	GRAIN METAMORPHIC AND NON-COASTAL PLAIN ICK THAT WOULD YEILD SPT REFUSAL IF TESTED, ROCK TYPI ITE, SLATE, SANDSTONE, ETC.	
SYMBOL BOOCOBOOOD BOOC			DUID LIMIT 31-50 DUID LIMIT GREATER THAN 50	COASTAL PLAIN SEDIMENTARY ROCK (CP)	COASTAL PLAIN S	SEDIMENTS CEMENTED INTO ROCK, BUT MAY NOT YIELD OCK TYPE INCLUDES LIMESTONE, SANDSTONE, CEMENTED	CORE RECOVERY (REC.) - TOTAL LENGTH OF ALL MATERIAL RECOVERED IN THE CORE BARREL DIVIDED BY TOTAL LENGTH OF CORE RUN AND EXPRESSED AS A PERCENTAGE.
% PASSING	GRANULAR SILT- MUCK,	PERCENTAGE OF MA		CF7	SHELL BEDS, ETC	ATHERING	DIKE - A TABULAR BODY OF IGNEOUS ROCK THAT CUTS ACROSS THE STRUCTURE OF ADJACENT
# 40 30 MX50 MX51 MN # 200 15 MX 25 MX30 MX 35 MX35 MX35 MX35 MX36 MN 36	I SOILS I com a I PEAI	ORGANIC MATERIAL SOILS SOILS TRACE OF ORGANIC MATTER 2 - 3% 3 - 5% LITTLE ORGANIC MATTER 3 - 5% 5 - 12%	OTHER MATERIAL TRACE 1 - 10% LITTLE 10 - 20%	FRESH ROCK FE	RESH, CRYSTALS BRIGHT, FEW JO IF CRYSTALLINE.	DINTS MAY SHOW SLIGHT STAINING ROCK RINGS UNDER	DIP - THE ANGLE AT WHICH A STRATUM OR ANY PLANAR FEATURE IS INCLINED FROM THE HORIZONTAL.
LIQUID LINT 48 MX41 MN 18 MX 10 MX 11 MN 11 MN 18 MX 10 MX 11 MN 11	MN LITTLE OR HIGHLY	MODERATELY ORGANIC 5 - 10% 12 - 20% HIGHLY ORGANIC >10% >20%	SOME 20 - 35% HIGHLY 35% AND ABOVE	(V. SLI.) CRYSTAL	ENERALLY FRESH, JOINTS STAINE LS ON A BROKEN SPECIMEN FACI RYSTALLINE NATURE.	ED, SOME JOINTS MAY SHOW THIN CLAY COATINGS IF OPEN, EE SHINE BRIGHTLY. ROCK RINGS UNDER HAMMER BLOWS IF	DIP DIRECTION (DIP AZIMUTH) - THE DIRECTION OR BEARING OF THE HORIZONTAL TRACE OF THE LINE OF DIP, MEASURED CLOCKWISE FROM NORTH.
GROUP INDEX 0 0 0 4 MX 8 MX 12 MX 16 MX No USUAL TYPES STONE FRACS. OF MAJOR GRAVEL AND FINE SILTY OR CLAYEY SILTY CLAYE	AMOUNTS OF SOILS	C GROUND WATER WATER LEVEL IN BORE HOLE IMMEDIATE	······································	SLIGHT ROCK GE (SLI.) 1 INCH.	ENERALLY FRESH, JOINTS STAINE OPEN JOINTS MAY CONTAIN CLA	ED AND DISCOLORATION EXTENDS INTO ROCK UP TO AY, IN GRANITOID ROCKS SOME OCCASIONAL FELDSPAR	FAULT - A FRACTURE OR FRACTURE ZONE ALONG WHICH THERE HAS BEEN DISPLACEMENT OF THE SIDES RELATIVE TO ONE ANOTHER PARALLEL TO THE FRACTURE.
MATERIALS SAND SAND GRAVEL AND SAND SOILS SOILS GEN. RATING	MATTER	STATIC WATER LEVEL AFTER 24 HOUR	RS.	CRYSTAL	LS ARE DULL AND DISCOLORED.	CRYSTALLINE ROCKS RING UNDER HAMMER BLOWS, DISCOLORATION AND WEATHERING EFFECTS, IN	FISSILE - A PROPERTY OF SPLITTING ALONG CLOSELY SPACED PARALLEL PLANES.
AS A EXCELLENT TO GOOD FAIR TO POOR SUBGRADE	FAIR TO POOR UNSUITABLE	PERCHED WATER, SATURATED ZONE OR WA	ATER BEARING STRATA	(MOD.) GRANITO	DID ROCKS, MOST FELDSPARS ARE	E DULL AND DISCOLORED, SOME SHOW CLAY, ROCK HAS D SHOWS SIGNIFICANT LOSS OF STRENGTH AS COMPARED	FLOAT - ROCK FRAGMENTS ON SURFACE NEAR THEIR ORIGINAL POSITION AND DISLODGED FROM PARENT MATERIAL.
P.I. OF A-7-5 ≤ L.L 3Ø : P.I. OF A-7-6 > 1		SPRING OR SEEPAGE		•	RESH ROCK. CK EXCEPT QUARTZ DISCOLORED	OR STAINED. IN GRANITOID ROCKS, ALL FELDSPARS DULL	FLOOD PLAIN (F.P.) - LAND BORDERING A STREAM, BUILT OF SEDIMENTS DEPOSITED BY THE STREAM.
CONSISTENCY OR DENSENES COMPACTNESS OR RANGE OF STANDARD COMPACTNESS OR RANGE OF STANDARD COMPACTNESS OR RANGE OF STANDARD	RANGE OF UNCONFINED	MISCELLANEOUS S'		SEVERE AND DIS	COLORED AND A MAJORITY SHOW	W KAOLINIZATION. ROCK SHOWS SEVERE LOSS OF STRENGTH GIST'S PICK. ROCK GIVES 'CLUNK' SOUND WHEN STRUCK,	FORMATION (FM.) - A MAPPABLE GEOLOGIC UNIT THAT CAN BE RECOGNIZED AND TRACED IN THE FIELD.
CONSISTENCY PENETRATION RESISTENCY (N-VALUE)	E COMPRESSIVE STRENGTH (TONS/FT ²)	ROADWAY EMBANKMENT SPT CPT ONT THE WITH SOIL DESCRIPTION	EST BORING SAMPLE DESIGNATIONS	l	<u>ED. WOULD YIELD SPT REFUSAL</u> CKS EXCEPT QUARTZ DISCOLORFO	D OR STAINED ROCK FABRIC CLEAR AND EVIDENT BUT REDU	JOINT - FRACTURE IN ROCK ALONG WHICH NO APPRECIABLE MOVEMENT HAS OCCURRED.
GENERALLY VERY LOOSE 4 GRANULAR LOOSE 4 TO 10	N/A		R BORING S- BULK SAMPLE	(SEV.) IN STRE	ENGTH TO STRONG SOIL. IN GRAP . SOME FRAGMENTS OF STRONG I	NITOID ROCKS ALL FELDSPARS ARE KAOLINIZED TO SOME ROCK USUALLY REMAIN.	LEDGE - A SHELF-LIKE RIDGE OR PROJECTION OF ROCK WHOSE THICKNESS IS SMALL COMPARED TO ITS LATERAL EXTENT.
MATERIAL MEDIUM DENSE 10 TO 30	N/A	ARTIFICIAL FILL OTHER THAN ROADWAY EMBANKMENTS CORE	BORING SS- SPLIT SPOON SAMPLE	IF TEST	TED. YIELDS SPT N VALUES > 10	<i>189 BPF</i> OR STAINED. ROCK FABRIC ELEMENTS ARE DISCERNIBLE BU	LENS - A BODY OF SOIL OR ROCK THAT THINS OUT IN ONE OR MORE DIRECTIONS. MOTTLED (MOT.) - IRREGULARLY MARKED WITH SPOTS OF DIFFERENT COLORS, MOTTLING IN
VERY SOFT <2	<0.25	- INFERRED SOIL BOUNDARIES MONIT	ST- SHELBY TUBE FORING WELL SAMPLE	(V. SEV.) THE MAS	SS IS EFFECTIVELY REDUCED TO NG. SAPROLITE IS AN EXAMPLE	O SOIL STATUS, WITH ONLY FRAGMENTS OF STRONG ROCK OF ROCK WEATHERED TO A DEGREE SUCH THAT ONLY MINDR	SOILS USUALLY INDICATES POOR AERATION AND LACK OF GOOD DRAINAGE.
GENERALLY SOFT 2 TO 4 SILT-CLAY MEDIUM STIFF 4 TO 8	0.25 TO 0.5 0.5 TO 1	SIISIIS INFERRED ROCK LINE	METER RS- ROCK SAMPLE	VESTIGE	S OF THE ORIGINAL ROCK FABR	RIC REMAIN. <u>IF TESTED, YIELDS SPT N VALUES < 100 BPF</u> NOT DISCERNIBLE, OR DISCERNIBLE ONLY IN SMALL AND	INTERVENING IMPERVIOUS STRATUM.
MATERIAL STIFF 8 TO 15 (COHESIVE) VERY STIFF 15 TO 30 HARD >30	1 TO 2 2 TO 4	SLOPE	ALLATION RT- RECOMPACTED I INDICATOR TRIAXIAL SAMPLE	SCATTER	RED CONCENTRATIONS. QUARTZ M	MAY BE PRESENT AS DIKES OR STRINGERS, SAPROLITE IS	RESIDUAL SOIL - SOIL FORMED IN PLACE BY THE WEATHERING OF ROCK. ROCK QUALITY DESIGNATION (R.O.D.) - A MEASURE OF ROCK QUALITY DESCRIBED BY TOTAL LENGTH OF
TEXTURE OR GRAIN SIZE	>4	ROCK STRUCTURES	LLATION CBR - CBR SAMPLE			HARDNESS	ROCK SEGMENTS EDUAL TO OR GREATER THAN 4 INCHES DIVIDED BY THE TOTAL LENGTH OF CORE RUN AND EXPRESSED AS A PERCENTAGE.
	270	→ SOUNDING ROD REED—— SPT R	N-VALUE REFUSAL	VERY HARD CANNOT SEVERA	T BE SCRATCHED BY KNIFE OR S AL HARD BLOWS OF THE GEOLOG	SHARP PICK. BREAKING OF HAND SPECIMENS REQUIRES	SAPROLITE (SAP.) - RESIDUAL SOIL WHICH RETAINS THE RELIC STRUCTURE OR FABRIC OF THE PARENT ROCK.
BOULDER COBBLE GRAVEL COARSE F.	075 0.053	ABBREVIATION AR - AUGER REFUSAL PMT			SCRATCHED BY KNIFE OR PICK FACH HAND SPECIMEN.	ONLY WITH DIFFICULTY. HARD HAMMER BLOWS REQUIRED	SILL - AN INTRUSIVE BODY OF IGNEOUS ROCK OF APPROXIMATELY UNIFORM THICKNESS AND RELATIVELY THIN COMPARED WITH ITS LATERAL EXTENT, WHICH HAS BEEN EMPLACED PARALLEL TO THE BEDDING OR SCHISTOSITY OF THE INTRUDED ROCKS
	AND (SL.) (CL.) 0.05 0.005	BT - BORING TERMINATED SD CL CLAY SL	- PRESSUREMETER TEST - SAND, SANDY - SILT, SILTY	HARD EXCAVA	E SCRATCHED BY KNIFE OR PICK ATED BY HARD BLOW OF A GEOL DERATE BLOWS.	C. GOUGES OR GROOVES TO 0.25 INCHES DEEP CAN BE LOGISTS PICK. HAND SPECIMENS CAN BE DETACHED	SLICKENSIDE - POLISHED AND STRIATED SURFACE THAT RESULTS FROM FRICTION ALONG A FAULT OR SLIP PLANE.
SOIL MOISTURE - CORRELATION OF	TERMS	CSE COARSE TCR	- SLIGHTLY - TRICONE REFUSAL - UNIT WEIGHT	MEDIUM CAN BE HARD CAN BE	GROOVED OR GOUGED 0.05 INC EXCAVATED IN SMALL CHIPS T	CHES DEEP BY FIRM PRESSURE OF KNIFE OR PICK POINT. TO PEICES 1 INCH MAXIMUM SIZE BY HARD BLOWS OF THE	STANDARD PENETRATION TEST (PENETRATION RESISTANCE) (SPT) - NUMBER OF BLOWS (N OR B.P.F.) OF A 140 LB. HAMMER FALLING 30 INCHES REQUIRED TO PRODUCE A PENETRATION OF 1 FOOT INTO SOLI WITH A 2 INCH OUTSIDE DIAMPETER SPLIT SPOON SAMPLER, SPT REFUSAL IS LESS THAN BL. FOOT PENETRATION
SOIL MOISTURE SCALE FIELD MOISTURE GUIDE FO	OR FIELD MOISTURE DESCRIPTION	e - VOID RATIO	- DRY UNIT WEIGHT	SOFT CAN BE		BY KNIFE OR PICK. CAN BE EXCAVATED IN FRAGMENTS	WITH 60 BLOWS.
- SATURATED - USUALLY	' LIQUID; VERY WET, USUALLY	FOSS FOSSILIFEROUS V	MOISTURE CONTENT VERY		CHIPS TO SEVERAL INCHES IN S CAN BE BROKEN BY FINGER PR	GIZE BY MODERATE BLOWS OF A PICK POINT. SMALL, THIN RESSURE.	STRATA CORE RECOVERY (SREC.) - TOTAL LENGTH OF STRATA MATERIAL RECOVERED DIVIDED BY TOTAL LENGTH OF STRATUM AND EXPRESSED AS A PERCENTAGE.
PLASTIC LIQUID LIMIT	ELOW THE GROUND WATER TABLE	FRAC FRACTURED VST FRAGS FRAGMENTS MED MEDIUM	- VANE SHEAR TEST	VERY CAN BE SOFT OR MOR FINGER	RE IN THICKNESS CAN BE BROKE	EXCAVATED READILY WITH POINT OF PICK. PIECES 1 INCH EN BY FINGER PRESSURE. CAN BE SCRATCHED READILY BY	STRATA ROCK QUALITY DESIGNATION (S.R.Q.D.) - A MEASURE OF ROCK QUALITY DESCRIBED BY: TOTAL LENGTH OF ROCK SEGMENTS WITHIN A STRATUM EQUAL TO OR GREATER THAN 10 CENTIMETERS DIVIDED BY THE TOTAL LENGTH OF STRATA AND EXPRESSED AS A PERCENTAGE.
	ID; REQUIRES DRYING TO OPTIMUM MOISTURE	EQUIPMENT USED ON SUB-	JECT PROJECT		RE SPACING	BEDDING	IDPSOIL (T.S.) - SURFACE SOILS USUALLY CONTAINING ORGANIC MATTER.
	AT OR NEAR OPTIMUM MOISTURE	DRILL UNITS: ADVANCING TOOLS:	HAMMER TYPE:	IERM VERY WIDE	SPACING MORE THAN 10 FEET	TERM THICKNESS VERY THICKLY BEDDED > 4 FEET	BENCH MARK: BL-6 STATION 19+71 18' LT
OM OPTIMUM MOISTURE - MOIST - (M) SOLID; SL SHRINKAGE LIMIT	HI ON NEAR OF THOM MOISTONE	MOBILE B- CLAY BITS	AUTOMATIC MANUAL	WIDE MODERATELY CLOSE	3 TO 10 FEET	THICKLY BEDDED 1.5 - 4 FEET THINLY BEDDED 0.16 - 1.5 FEET	ELEVATION: 9.52
	S ADDITIONAL WATER TO OPTIMUM MOISTURE	6° CONTINUOUS FLIGHT AL BK-51 8° HOLLOW AUGERS		CLOSE VERY CLOSE	0.16 TO 1 FEET LESS THAN 0.16 FEET	VERY THINLY BEDDED 0.03 - 0.16 FEET THICKLY LAMINATED 0.008 - 0.03 FEET	NOTES:
PLASTICITY		CME-45C HARD FACED FINGER BITS	s			THINLY LAMINATED < 0.008 FEET URATION	→ XX ASPHALT
PLASTICITY INDEX (PI) NONPLASTIC 0-5	DRY STRENGTH VERY LOW	TUNGCARBIDE INSERTS	 	FOR SEDIMENTARY ROCK		NG OF THE MATERIAL BY CEMENTING, HEAT, PRESSURE, ETC.	N.M.= NO MEASUREMENT
LOW PLASTICITY 6-15 MED. PLASTICITY 16-25	SLIGHT MEDIUM	CME-550 CASING W/ ADVANCE CASING OF MY ADVANCE CASI	CER HAND TOOLS.	FRIABLE		WITH FINGER FREES NUMEROUS GRAINS: BLOW BY HAMMER DISINTEGRATES SAMPLE.	
HIGH PLASTICITY 26 OR MORE	нібн	PORTABLE HOIST TRICONE 215/16 STEEL TRICONE TRICONE TRICONE		MODERATELY		AN BE SEPARATED FROM SAMPLE WITH STEEL PROBE; EASILY WHEN HIT WITH HAMMER.	
COLOR DESCRIPTIONS MAY INCLUDE COLOR OR COLOR COMBINATIONS (TAN.)	RED YEL-RRN RIVE-CRAVI	OTHER TRICONE TUNG.	-CARB. HAND AUGER SOUNDING ROD	INDURATED		ARE DIFFICULT TO SEPARATE WITH STEEL PROBE:	
MODIFIERS SUCH AS LIGHT, DARK, STREAKED, ETC. ARE USED TO DES		OTHER DRAG BIT	VANE SHEAR TEST OTHER	EXTRÉMELY	INDURATED SHARP HA	AMMER BLOWS REQUIRED TO BREAK SAMPLE:	
L			O THEN	<u> </u>	SAMPLE F	BREAKS ACROSS GRAINS.	1

ID B-4031

STATE PROJECT NO. SHEET NO. TOTAL SHEETS
33398.1.1 2 14



STATE OF NORTH CAROLINA DEPARTMENT OF TRANSPORTATION

MICHAEL F. EASLEY
GOVERNOR

P.O. BOX 25201, RALEIGH, N.C. 27611-5201

LYNDO TIPPETT SECRETARY

May 11, 2006

STATE PROJECT:

33398.1.1 B-4031

F.A. PROJECT:

BRSTP-0179 (2)

COUNTY:

Brunswick

DESCRIPTION:

Bridge No. 72 on NC 179 Over Jinnys Branch

SUBJECT:

Geotechnical Report - Bridge Foundation Investigation for

NC 179 over Jinnys Branch at -L- Sta. 25+70.00

Site Description

The proposed bridge site is located at the existing NC 179 bridge over Jinnys Branch, approximately 4 miles south of Shallotte. The replacement structure will be built along the existing alignment. Based on the proposed design, the new structure will have four spans having a total length of 300 feet. The bents will have a skew of 90 degrees.

One Standard Penetration Test (SPT) boring was made at or near each proposed bent location to provide subsurface information relative to foundation design. In addition to Standard Penetration Tests, rock core was obtained from one interior bent location. The SPT borings were made with ATV mounted CME-550 and CME-45C drill machines and advanced by rotary drill methods using bentonite drilling fluid. A HQ core barrel with H casing was used to obtain the rock core.

The bridge site is located in the Coastal Plain Physiographic Province and is underlain by recent tidal marsh deposits and Cretaceous marine sediments of the Pee Dee Formation. Jinnys Branch is a tidal marsh channel, $20\pm$ feet wide and 2 to 4 feet deep. Topography along the project is nearly flat to gently sloping. Elevations at the site range from -3± feet along the streambed to $11\pm$ feet along the existing NC 179 embankment.

Ground water elevations range from $2\pm$ feet to $5\pm$ feet, whereas the surface of Jinnys Branch ranges from $0\pm$ to -1 \pm feet.

Sheet 3

Soil Description

Surficial soils generally consist of $5\pm$ to $14\pm$ feet of very loose to medium dense alluvial sand and silty sand (A-3, A-2-4) with $2\pm$ to $5\pm$ feet of very soft muck. These soils are underlain by the marine sediments of the Cretaceous age Pee Dee Formation. The Pee Dee Formation consists of 43+ feet of soft to hard green/gray, calcareous clayey silt (A-4) and sandy clay (A-6) with thin limestone layers.

Based on the proposed design, the existing grade will be raised approximately $2\pm$ feet at the bridge site. The existing embankment material primarily consists of $4\pm$ feet to $7\pm$ feet of very loose to dense silty sand (A-2-4). The proposed end bent slopes will be mainly constructed within the existing embankment. Some additional fill will be required for construction of the end bent and side slopes. Borrow meeting Coastal Plain criteria is available in nearby areas.

This Geotechnical Foundation Report is based on the Bridge Survey and Hydraulic Design Report for Jinnys Branch dated December 8, 2005. If significant changes are made in the design or location of the proposed structure, the subsurface information should be reviewed and modified as necessary.

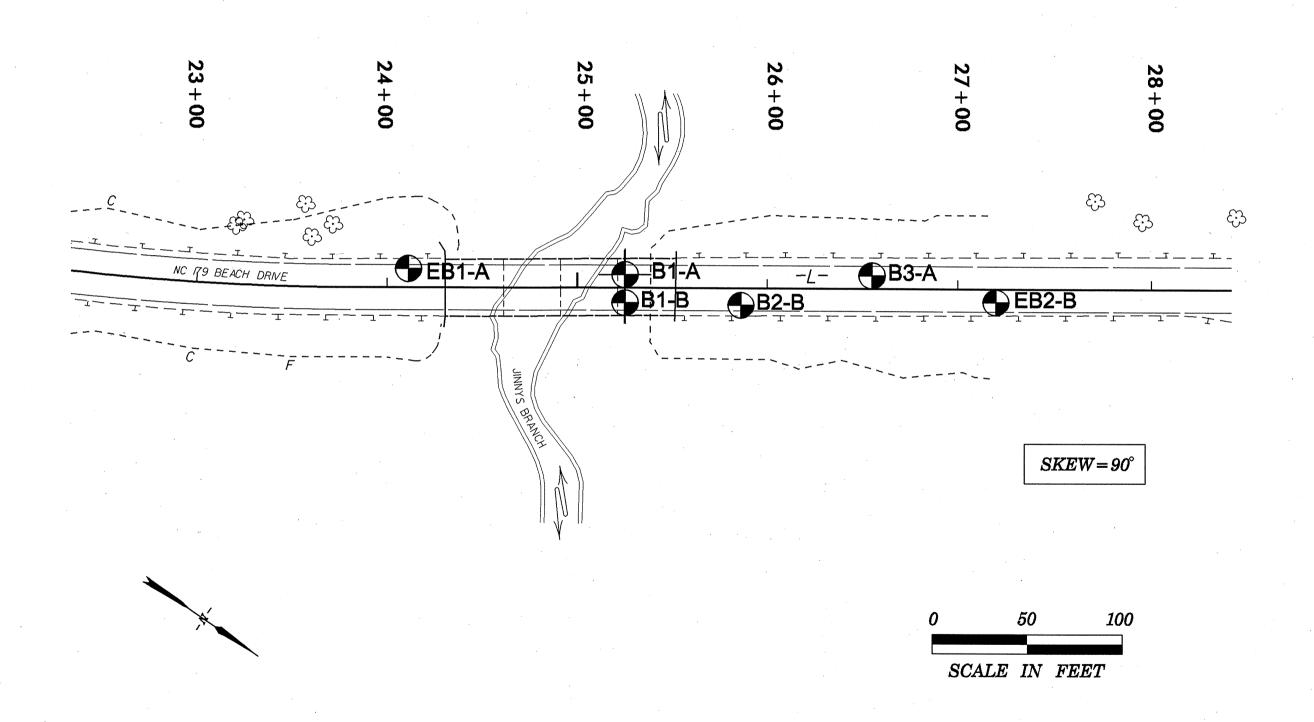
(fare)

Joseph L Stone, L.G. Engineering Geologist II

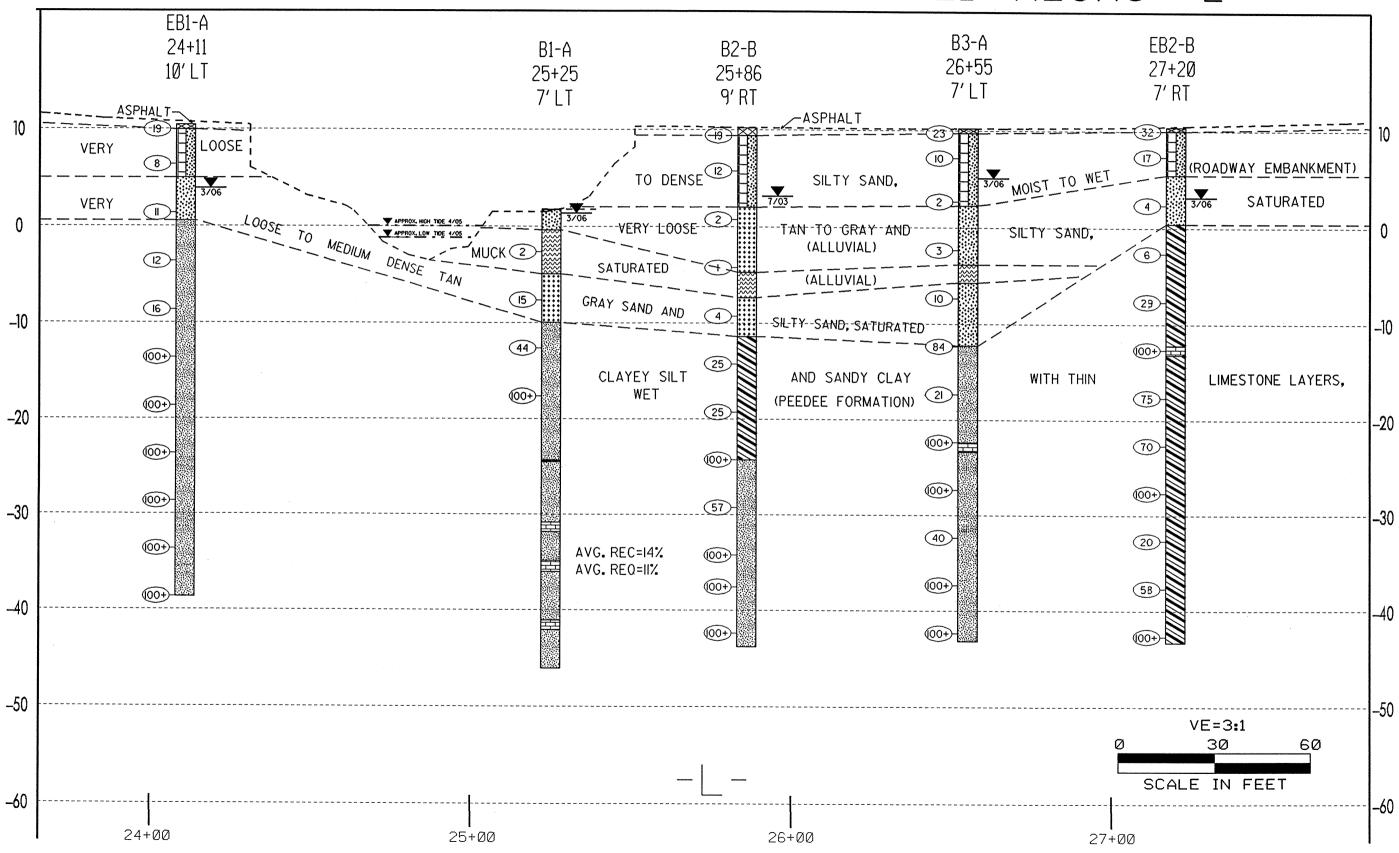
 PROJECT REFERENCE NO.
 SHEET

 33398.1.1 (B-4031)
 4

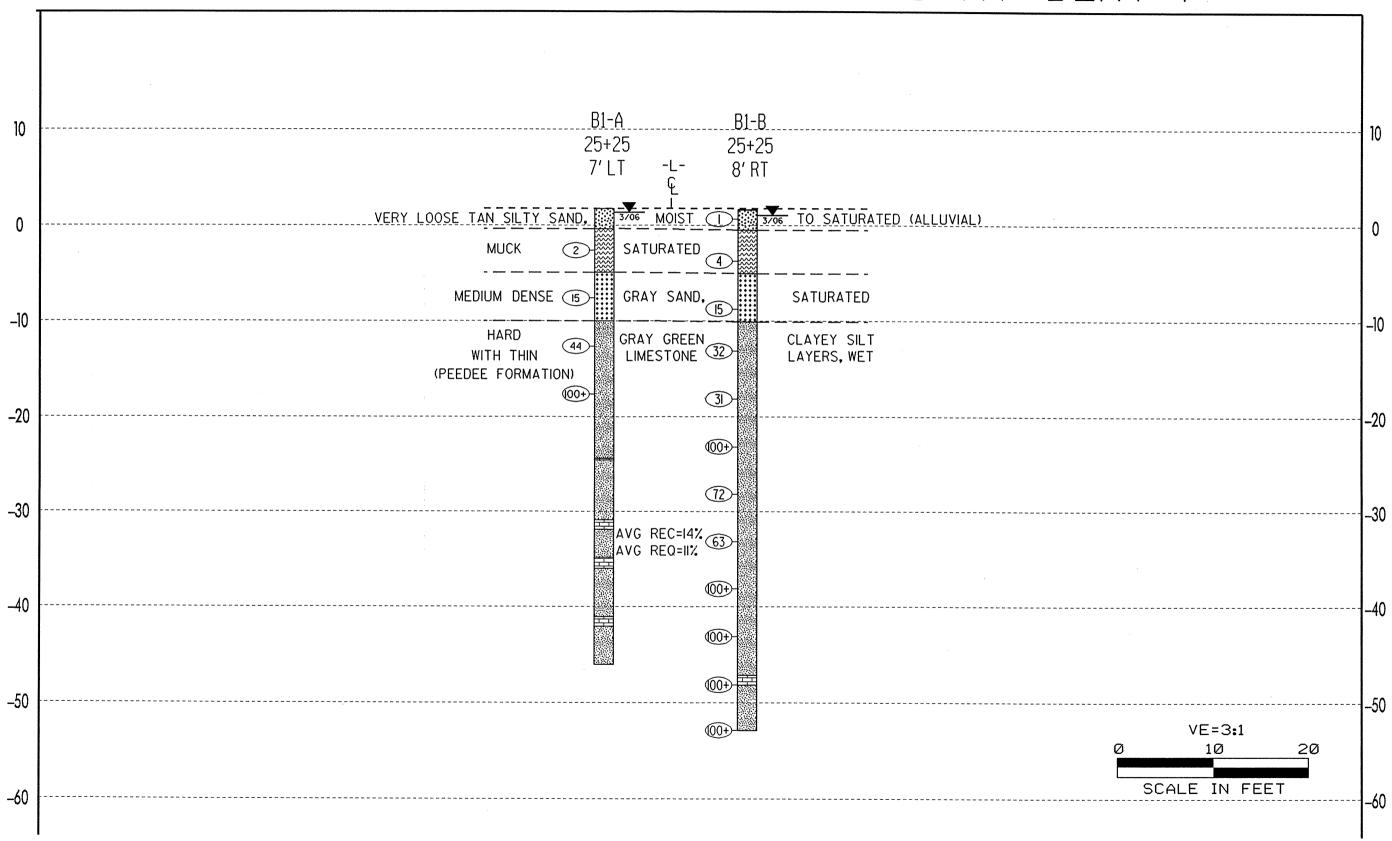
TEST SITE PLAN



PROFILE THROUGH BORINGS PROJECTED ALONG -L-



CROSS SECTION THROUGH BORINGS AT BENT I



North Carolina department of transportation of transportation north carolina department of transportation GEOTECHNICAL UNIT BORING LOG

GEOTECHNICAL UNIT BORING LOG

PROJECT NO. 3	33398.1.1	ID. B-4031 COUNTY BRUNS	VICK GEOI	OGIST J. L. STONE	PROJECT NO. 33398.I.I ID. B-403I COUNTY BRUNSWICK GEOLOGIST J. L. STONE
SITE DESCRIPTION		. 72 ON NC 179 OVER JINNYS E		GROUND WATER	Table Tabl
BORING NO. E	BI-A BORING	LOCATION 24+II OFFSET	IO'LT ALIG	NMENT -L- 0 HR. N.M.	BORING NO. BI-A BORING LOCATION 25+25 OFFSET 7'LT ALIGNMENT -L- OHR. N.M.
COLLAR ELEVATI		NORTHING 63052.2009		79413.7877 24 HR . 6.6	COLLAR ELEVATION 1.7' NORTHING 63157.6509 EASTING 2179370.3677 24 HR. 0.4
TOTAL DEPTH 4			HOD ROTARY W/		TOTAL DEPTH 47.8' DRILL MACHINE CM-550 DRILL METHOD ROTARY W/MUD HAMMER TYPE MANUAL
START DATE OF			RFACE WATER DEPTH	I N/A DEPTH TO ROCK N/A	START DATE 03/15/06 COMPLETION DATE 03/16/06 SURFACE WATER DEPTH N/A DEPTH TO ROCK N/A
1 F I F V 1	HBLOW COUNT 10.510.510.51		SAMPLE WOI.	SOIL AND ROCK DESCRIPTION	ELEV. DEPTH BLOW COUNT PEN. BLOWS PER FOOT SAMPLE OF SOIL AND ROCK NUMBER NUMBER MOI. G DESCRIPTION
1 1				·	
10.0 + 0.5	5 11 10 9	1.0		ASPHALT	0.0 GRAY SILTY SAND, SATURATED (ALLUVIAL)
$\frac{1}{5.0} = 3.6$	6 4 4 4	1.0	SS-13	TAN SILTY SAND, MOIST (ROADWAY EMBANKMENT)	4.4 1 1 1 1.0 X2
7.6	6 4 5 6	1.0	SS-14 T	GRAY SILTY SAND, MOIST TO SATURATED (ALLUVIAL)	1 -3.0 +
0.0 + 12.6	6 2 3 9	1.0		(ALLOVIAL)	1 -10.0 +
-5.0 + 12.0	0 2 3 7	1.0	SS-I5		
17.6	6 4 7 9	1.0			19.4 33 67 0.6
-10.0 + 22.6	6 5 95	0.8	(SS-16		-20.0 +
-15.0 +					GRAY GREEN CLAYEY SILT WITH -25.0 + THIN LIMESTONE LAYERS, WET (PEEDEE FORMATION)
	6 78 22	0.9		GRAY GREEN CLAYEY SILT WITH THIN LIMESTONE LAYERS, WET (PEEDEE FORMATION)	AVG. REC=14%
-20.0 + 32.6	6 8 30 70	0.8		(PEEDEE FORMATION)	-30.0
-25.0 +					35.0 +
-30.0	6 37 32 68	0.9 			
42.6	6 29 71	0.9	SS-17		
-35 . 0 + 47 . 6	6 26 65 35	0.9			-45.0
-40.0		BORING TERMINATED AT			
-45.0 +		- ELEVATION - 38,7 FEET - IN-HARD - CLAYEY - SILT-			
73.0 					
-50.0 +					-60.0 +
-55.0					-65.0
<u> </u>					
-60.0 +					
-65.0					
	<u> </u>		1	<u> </u>	

NORTH CAROLINA DEPARTMENT OF TRANSPORTATION DIVISION OF HIGHWAYS GEOTECHNICAL UNIT CORE BORING REPORT

	CT NO.: 3						DUNTY: Brunswick BORING NO.: B1-A
COLLA DRILL	ESCRIPT AR ELEV.: ER: Donal DEPTH:	: 1.7 ft. ld Rhodes		COR GEO	C 179 ov E SIZE: LOGIST AL RUN	: HQ Г: JLS	EQUIPMENT: CME-550 PERSONNEL: Brian Miller DATE: 3-15-06
ELEV. (FT.)	DEPTH (FT.)	DRILL RATE MN/FT	RUN (FT.)	RQD %	REC	SAMP NO.	FIELD CLASSIFICATION AND REMARKS
-15.5 -21.1	20.2	0.58 0.83 1.25	2.6	0	0		NO RECOVERY
-21.1	22.8 26.3	0.23 0.43 4.50	3.5	0	11		GREEN TO GRAY VERY SOFT LIMY MUDSTONE AND MICRITIC LIMESTONE
-24.6 -26.1	26.3 27.8	1.23	1.5	0	0		NO RECOVERY
-26.1 -28.6	27.8 30.3	2.63 2.50 4.66	2.5	0	20		GREEN TO GRAY VERY SOFT LIMY MUDSTONE
-28.6 -31.1	30.3	1.66 2.16 2.33	2.5	24	24		GREEN TO GRAY VERY SOFT LIMY MUDSTONE AND MICRITIC LIMESTONE
-31.1	32.8	1.90 3.20 3.16 2.33	4.0	0	20		GREEN TO GRAY VERY SOFT LIMY MUDSTONE AND MICRITIC LIMESTONE
-35.1 -36.1	36.8 37.8	2.85	1.0	50	60		GREEN TO GRAY VERY SOFT LIMY MUDSTONE AND MICRITIC LIMESTONE

Sheet 8 of 14

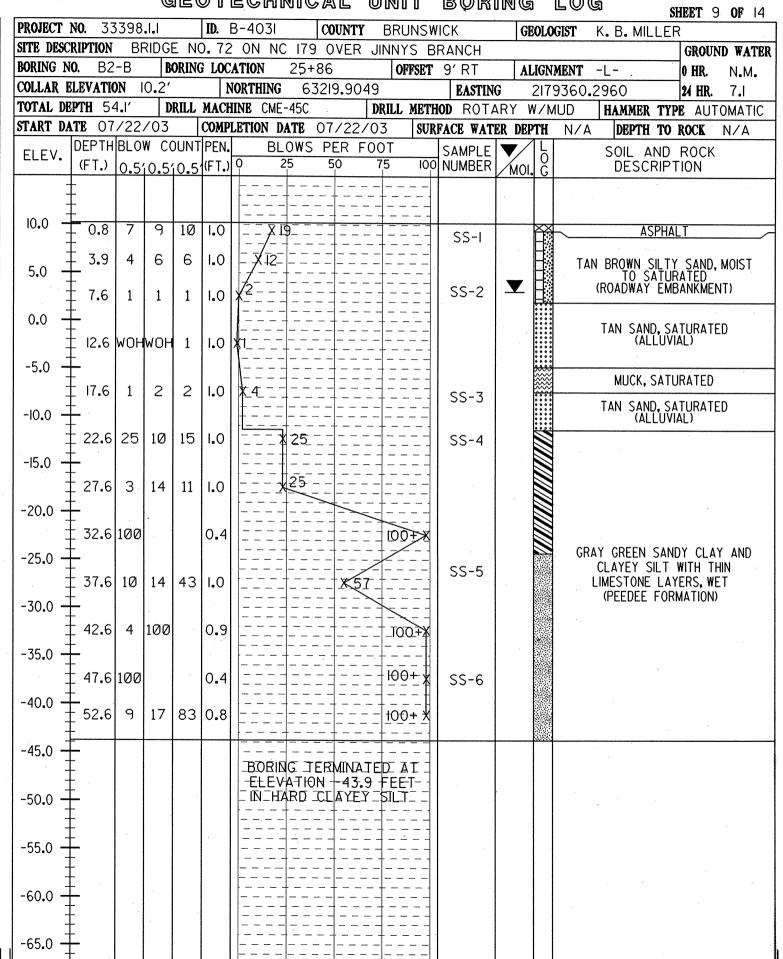
NORTH CAROLINA DEPARTMENT OF TRANSPORTATION DIVISION OF HIGHWAYS GEOTECHNICAL UNIT CORE BORING REPORT

COLLA DRILL	ESCRIPT AR ELEV. ER: Donal DEPTH:	: 1.7 ft. ld Rhodes 47.8		COR GEO	E SIZE	: HQ Г: JL ST	EQUIPMENT: CME-550 ONE PERSONNEL: Brian Miller DATE: 3-15-06
ELEV. (FT.)	DEPTH (FT.)	DRILL RATE MN/FT	RUN (FT.)	RQD %	REC	SAMP NO.	FIELD CLASSIFICATION AND REMARKS
-36.1 -41.1	37.8 42.8	3.00 1.75 0.85 1.38 1.70	5.0	20	30		GREEN TO GRAY VERY SOFT LIMY MUDSTONE AND MICRITIC LIMESTONE
-41.1 -41.1	42.8	1.70 1.65 1.27 1.25 1.17	5.0	20	20		GREEN TO GRAY VERY SOFT LIMY MUDSTONE AND MICRITIC LIMESTONE
-46.1	47.8	1.92					

north carolina department of transportation horth carolina department of transportation GEOTECHNICAL UNIT BORING LOG

PROJECT NO. 33398.L. ID. B-4031 COUNTY BRUNSWICK GEOLOGIST J. L. STONE SITE DESCRIPTION BRIDGE NO. 72 ON NC 179 OVER JINNYS BRANCH GROUND WATER BORING NO. BI-B 25+25 BORING LOCATION OFFSET 8' RT ALIGNMENT -L-O HR. N.M. COLLAR ELEVATION 1.6 NORTHING 63163,7250 **EASTING** 2179384.0829 24 HR. 0.6 TOTAL DEPTH 54.5' DRILL MACHINE CME-550 DRILL METHOD ROTARY W/MUD HAMMER TYPE MANUAL START DATE 03/14/06 COMPLETION DATE 03/15/06 SURFACE WATER DEPTH N/A DEPTH TO ROCK N/A DEPTHIBLOW COUNTIPEN BLOWS PER FOOT SAMPLE SOIL AND ROCK ELEV. 0.510.510.5(FT.) 50 75 100 NUMBER DESCRIPTION /MOL 0.0 WOHWOH 1 1.0 GRAY SILTY SAND (ALLUVIAL)
WET TO SATURATED SS-7 V 0.0 2 . 2 4.4 1.0 SS-8 37% MUCK, SATURATED -5.0 TAN SAND. SATURATED 7 8 1.0 SS-9 -10.0 19 | 13 | 1.0 13.8 8 SS-IO 12% -15.018.8 | 10 | 12 | 19 | 1.0 -20.0 23.8 100 +00F SS-II 0.4 -25.0 28.8 7 34 38 1.0 72 - --30.0 33.8 17 28 35 1.0 -35.0 38.8 55 45 0.9 HO0+ SS-I2 -40.0 43.8 64 36 0.9 t00+ -45.0 48.8 60 0.1 t00+ -50.0 53.8 6 94 100+ BORING TERMINATED AT -55.0 ELEVATION -52.9 IN -GLAYEY- SILT -60.0 -65.0 -70.0 -75.0

GEOTECHNICAL UNIT BORING LOG



North Carolina department of transportation of transportation of transportation GEOTECHNICAL UNIT BORING LOG

GEOTECHNICAL UNIT BORING LOG

PROJECT NO. 33398.I.I ID. B-4031 COUNTY BRUNSWICK GEOLOGIST J. L. STONE	PROJECT NO. 33398.1		
		RIDGE NO. 72 ON NC 179 OVER JINNYS BE	
BORING NO. B3-A BORING LOCATION 26+55 OFFSET 7' LT ALIGNMENT -L- 0 HR. COLLAR ELEVATION 10.2' NORTHING 63276.5 56 EASTING 21793 7.7257 24 HR.	N.M. BORING NO. EB2-B	BORING LOCATION 27+20 OFFSET	
COLLAR ELEVATION10.2'NORTHING63276.5156EASTING2179317.725724 HR.TOTAL DEPTH53.4'DRILL MACHINECME-550DRILL METHODROTARYW/MUDHAMMER TYPEMAN			EASTING 2179304.2055 24 HR. 7.3
START DATE 03/22/06 COMPLETION DATE 03/22/06 SURFACE WATER DEPTH N/A DEPTH TO ROCK		- Andrew Programmer Andrew Programmer Andrew Programmer Andrew Programmer Andrew Programmer Andrew Programmer	OD ROTARY W/MUD HAMMER TYPE MANUAL
S. S. DEPTHBLOW COUNTIPEN. BLOWS PER FOOT SAMPLE VIL SOIL AND BOCK	DEPTHIBLOW		SAMPLE SOIL AND ROCK
ELEV. (FT.) 0.5/10.5/(FT.) 0 25 50 75 100 NUMBER MOI. 6 DESCRIPTION	1 1 - 1 - 7 1 1		SAMPLE O SOIL AND ROCK NUMBER MOI. G DESCRIPTION
±	<u> </u>		INIOI. U
10.0 - 0.5 13 12 11 1.0 - 25 ASPHALT	10.0 + 0.4 23		SS-23 ASPHALT
5.0 + 3.1 4 5 5 1.0 - × +0	NU, 1 1	9 8 1.0 717	SS-23 ASPHALT TAN SILTY SAND, MOIST (ROADWAY EMBANKMENT)
	T)		
7.6 1 1 1 1.0 2 2 3 3 3 3 3 3 3 3 3 3 3 3 3 3 3 3 3		2 2 1.0	SS-24 GRAY SILTY SAND, MOIST TO SATURATED (ALLUVIAL)
0.0 + GRAY SILTY SAND, SATUR	TED 0.0 +		
12.6 1 1 2 1.0 x 3 SS-19 (ALLUVIAL)		2 4 1.0 46 -	SS-25
-5.0 + MUCK, SATURATED	-5.0 +		
	TED + 17.6 19	18 11 1.0 <u>X</u> 29	
-10.0 +	10.0 +		
22.6 21 56 28 1.0 X84 SS-20 18%	<u>+ 22.6 60 </u>	0.0 100+	
-15.0 	-15.0 ±		GRAY GREEN SANDY CLAY WITH THIN LIMESTONE LAYERS, WET (PEEDEE FORMATION)
	27.6 19	44 31 1.0 <u> </u>	SS-26 (PEEDEE FORMATION)
-20.0 +	-20.0 +		
	32.6 16	11 59 1.0 - - - 	
-25.0 + GRAY GREEN CLAYEY SILT			
-25.0 — GRAY GREEN CLAYEY SILT GRAY GREEN CLAYEY SILT THIN LIMESTONE LAYERS (PEEDEE FORMATION)	WEI	76 0.7	
-30.0 +	-30.0 +		
+42.6 8 20 20 1.0		8 12 1.0 × 20	SS-27
-35.0 +	-35.0 +		
47.6 76 24 0.9	1 +	15 43 1.0	
1-40.0 ± 10 1 1 1 1 1 1 1 1 1 1 1 1 1 1 1 1 1	1 -40.0 ±		
52.6 11 89 0.8	52.6 60	0.1	
-45.0 -45.0 -45.0		BORING TERMINATED AT	
	-45 . 0 ±		
+		Land Limestone	
-50.0 +	-50.0 +		
-55.0 +	-55.0 +		
-60.0 +	-60.0 +		
-65.0 	-65.0 +		<u>. </u>

B-4031 BRIDGE NO. 72 ON NC 179 OVER JINNYS BRANCH

HOLE#	SAMPLE#	PASS 10	PASS 40	PASS 200	CSESAND	FINESAND	SI	CL	LL	PI	CLASS	DEPTH	MOIST.	ORG.
B2-B	SS-1	100	95	15	12.4	74.2	3.8	9.6	15	NP	A-2-4(0)	1.0-2.3		
	SS-2	100	95	16	15.4	70.6	6.4	7.6		NP	A-2-4(0)	7.6-9.1		
	SS-3	99	68	8	48.0	45.0	1.4	5.6	22	NP	A-3(0)	17.6-19.1		
	SS-4	100	98	69	4.8	34.2	23.4	37.6	24	19	A-6(9)	22.0-24.1		
	SS-5	100	96	58	10.0	37.0	21.4	31.6		2	A-4(0)	37.6-39.1		
	SS-6	100	99	68	4.4	35.8	22.2	37.6	30	8	A-4(4)	47.6-49.1		
B1-B	SS-7	100	89	22	23.5	56.8	8.9	10.8	19	NP	A-2-4(0)	0.0-1.5		
	SS-8	100	98	11	7.8	83.5	5.9	2.8	26	NP	A-2-4(0)	4.4-5.9	36.6	26.6
	SS-9	98	79	6	34.5	60.6	4.1	8.0	23	NP	A-3(0)	9.4-10.9		
	SS-10	100	94	60	10.8	33.7	24.6	30.9	31	2	A-4(0)	13.8-15.3	11.5	
	SS-11	100	95	71	8.2	26.3	26.6	38.9	32	4	A-4(2)	23.8-24.2		
	SS-12	100	95	67	9.0	31.5	26.6	32.9	34	3	A-4(2)	38.8-39.7		
EB1-A	SS-13	100	97	14	9.4	79.6	4.1	6.8	19	NP	A-2-4(0)	0.5-2.0		
	SS-14	97	85	11	29.3	61.8	6.1	2.8	16	NP	A-2-4(0)	7.6-9.1		
	SS-15	100	100	56	7.0	38.7	21.4	32.9	29	4	A-4(1)	12.6-14.1		
	SS-16	100	95	57	10.8	37.1	19.2	32.9	35	6	A-4(2)	22.6-23.4		
	SS-17	100	89	64	15.0	26.5	22.0	36.5	33	4	A-4(2)	42.6-43.5		
B3-A	SS-18	100	95	25	11.0	68.0	10.5	10.4	18	NP	A-2-4(0)	3.1-4.6		
	SS-19	86	73	26	24.3	47.9	11.3	16.4	27	3	A-2-4(0)	12.6-14.1		
	SS-20	100	98	62	6.6	36.5	20.4	36.5	29	4	A-4(1)	22.6-24.1	17.7	
	SS-21	100	85	62	20.1	22.5	25.0	32.5		5	A-4(1)	37.6-38.0		
	SS-22	100	99	67	3.0	39.3	21.2	36.5	28	6	A-4(2)	52.6-53.4		
EB2-B	SS-23	100	97	25	7.0	72.6	9.9	10.4	18	NP	A-2-4(0)	0.4-1.9		
	SS-24	100	80	19	23.1	60.2	6.3	10.4	18	NP	A-2-4(0)	7.6-9.1		
	SS-25	100	99	58	8.8	34.7	13.9	42.5	32	12	A-6(5)	12.6-14.1		
	SS-26	93	89	66	6.8	26.9	35.8	30.5	30	11	A-6(5)	27.6-29.1		
	SS-27	100	97	67	5.8	35.5	22.2	36.5	30	12	A-6(6)	47.6-49.1		



FIELD SCOUR REPORT

WBS: 33398.1.1 TIP: B-4031 COUNTY: BRUNSWICK	
DESCRIPTION(1): BRIDGE NO. 72 ON NC 179 OVER JINNYS BRANCH	
EXISTING BRIDGE	
Information from: Field Inspection X Microfilm (reel pos:) Other (explain) PROJECT WISE DATA BASE	
Bridge No.: 72 Length: 125 Total Bents: 5 Bents in Channel: 3 Bents in Floodplain: 2 Foundation Type: TIMBER PILES	_
EVIDENCE OF SCOUR(2) Abutments or End Bent Slopes: SOME SLUMPING OF END SLOPES	_
Interior Bents: NONE NOTED	- -
Channel Bed: NONE NOTED	_
Channel Bank: SOME SLUMPING ALONG CHANNEL BANKS	- -
EXISTING SCOUR PROTECTION Type(3): 1) WOODED END WALLS 2) RIP RAP END SLOPES	
Extent(4): 1) 4 FEET OUTSIDE EDGE OF BRIDGE 2) RIP RAP AT BOTH END SLOPES	
Effectiveness(5): MODERATELY EFFECTIVE	
Obstructions(6): NONE NOTED	-

INSTRUCTIONS

- 1 Describe the specific site's location, including route number and body of water crossed.
- 2 Note scour evidence at existing end bents or abutments (e.g. undermining, sloughing, degradations).
- Note existing scour protection (e.g. rip rap).
- 4 Describe extent of existing scour protection.
- 5 Describe whether or not the scour protection appears to be working.
- 6 Note obstructions such as dams, fallen trees, debris at bents, etc.
- 7 Describe the channel bed material based on observation and/or samples. Include any lab results with report.
- 8 Describe the channel bank material based on observation and/or samples. Include any lab results with report.
- 9 Describe the material covering the banks (e.g. grass, trees, rip rap, none).
- 10 Determine the approximate floodplain width from field observation or a topographic map.
- 11 Describe the material covering the floodplain (e.g. grass, trees, crops).
- 12 Use professional judgement to specify if the stream is degrading, aggrading, or static.
- 13 Describe potential and direction of the stream to migrate laterally during the bridge's life (approx. 100 years).
- Give the design scour elevation (DSE) expected over the life of the bridge (approx. 100 years). This elevation can be given as a range across the site, or for each bent. Discuss the relationship between the Hydraulics Unit theoritical scour and the DSE. If the DSE is dependent on scour counter measures, explain (e.g. rip rap armoring on slopes). The DSE is based on the erodability of materials, giving consideration to the influence of joints, foliation, bedding characteristics, % core recovery, % RQD, differential weathering, shear strength, observations at existing structures, other tests deemed appropriate, and overall geologic conditions at the site.

Channel	Bed Ma	terial(7)	: SILT	Y SA	ND AI		K								
Channel E	Bank Ma	terial(8):	SILT	Y SA		S-7) AND									-
Channe	l Bank C	over(9)	MAR	SH G	RAS	3									
Flood	dplain Wi	idth(10):	APP	ROXI	MATE	LY 300 I	FEET				······································				
Flood	dplain Co	ver(11):	MAR	SH G	RASS	3									
Channel Migratio	n Tende	ncy(13):	MOE	ERA	TE TO	THE SO	OUTH E	AST							
Observations	and Othe	er Comn	nents:												
			-	······································				***************************************							Pullakoruskini
DESIGN SCO	UR ELE	VATIO	NS(14)				Fe	et X	_	Mete	ers	- AND THE COLUMN ASSESSMENT ASSES		
		BENTS	<u>.</u>												
		B1		2	B3										
SBI	anes, Lt	·			0		T	T			·			1	-
	•		-0.	~											
	anes, Rt		-				<u> </u>								
	anes, Lt														
NB La	anes, Rt														
	• •						<u> </u>								_
							 								-
		Ľ				J	J	<u> </u>				l			
0	(505)													•	
Comparison of															
GEOTECHNIC										L DES	SIGN S	COUF	₹ ELEV/	ATIONS	
AS OUTLINE	D BY TH	E BRID	GE SI	JRVE	Y AN	D HYDR.	AULIC F	REPOR	T.						
0011 411111															**************************************
SOIL ANALY Bed or Bank		ULTS F	ROM	CHA	NNEL	BED A	ND BAN	K MAT	ERIAL		·			· · · · · · · · · · · · · · · · · · ·	
Sample No.							1				-				
Retained #4							<u> </u>		7		_				
Passed #10															
Passed #40											<u> </u>				
Passed #200			9	See S	heet 1	11.									
Coarse Sand						esults".									
Fine Sand			1			: SS-7, S									
Silt			_ '	ui sai	inhies	. 33-1, S	0-0				-				
Clay									<u> </u>		1		-		ı
			-						ļ	****		***************************************			ı
LL			\dashv												ı
PI					,				ļ						ı
AASHTO									J ·						,
Station															ĺ
Offset		· .													
Depth									Y	***********					

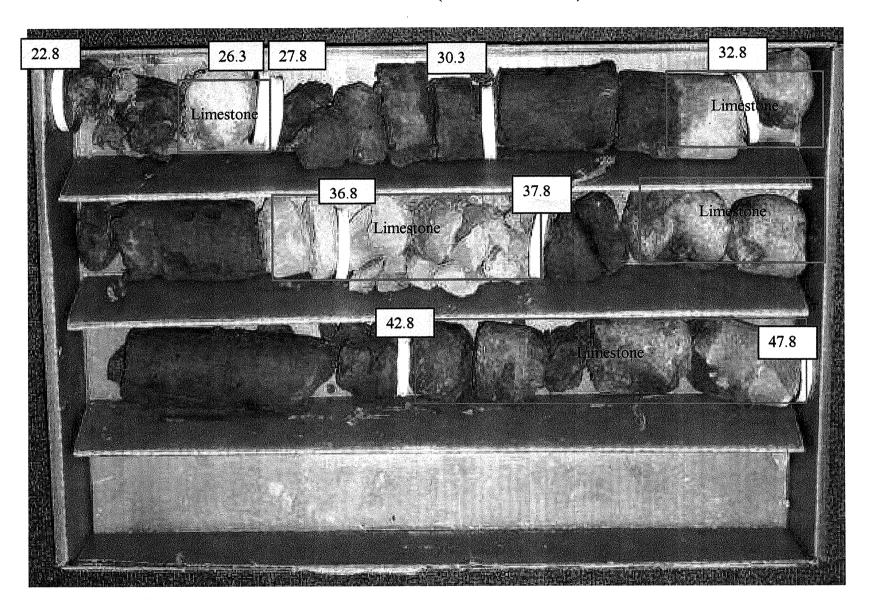
Reported by:

Template Revised 02/07/06

Date: 5/11/2006

33398.1.1 B-4031 Bridge No 72 on NC 179 over Jinnys Branch

Core Photograph B1-A Sta. 25+25 7 LT (20.2 feet to 47.8 feet)





CONTENTS:

11

DESCRIPTION

TITLE SHEET LEGEND

SITE PLAN

CROSS SECTION

SCOUR REPORT CORE PHOTOGRAPH

SOIL TEST RESULTS

SITE PHOTOGRAPH

PROFILE

STRUCTURE INVENTORY REPORT

BORE LOG & CORE REPORTS

STATE OF NORTH CAROLINA

DEPARTMENT OF TRANSPORTATION

DIVISION OF HIGHWAYS GEOTECHNICAL UNIT

STRUCTURE SUBSURFACE INVESTIGATION

STATE PROJECT 33398.1.1 I.D. NO. B-4031 F.A. PROJECT_*BRSTP-0179* (2) COUNTY____BRUNSWICK PROJECT DESCRIPTION BRIDGE NO. 72 ON NC 179 OVER JINNYS BRANCH AT -L- STATION 25 + 70.00

***************************************	BRS1F-41/7 (2)	CONS	
32396.1.1	BRSTP-4179 (2)	P.E.	
STATE PROJ.NO.	P.A.PROLNO.	DESCRIP	TION
N.C. 33398.1.1	(B-4031)	1	14
STATE STATE PROJECT	T REFERENCE NO.	SHEET NO.	TOTAL SHBBTS

CAUTION NOTICE

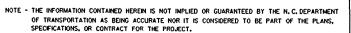
THE SUBSURFACE INFORMATION AND THE SUBSURFACE INVESTIGATION ON WHICH IT IS BASED WAS MADE FOR THE PURPOSE OF STUDY, PLANNING AND DESIGN, AND NOT FOR CONSTRUCTION OR PAY PURPOSES. THE VARIOUS FIELD BORING LOGS, ROCK CORES, AND SOIL TEST DATA AVAILABLE MAY BE REVIEWED OR INSPECTED IN RALEIGH BY CONTACTING THE N. C. DEPARTMENT OF TRANSPORTATION, GEOTECHNICAL UNIT @ (919) 250-4088. NEITHER THE SUBSURFACE PLANS AND REPORTS, NOR THE FIELD BORING LOGS. ROCK CORES, OR SOIL TEST DATA IS PART OF THE CONTRACT.

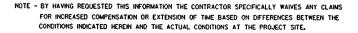
GENERAL SOIL AND ROCK STRATA DESCRIPTIONS AND INDICATED BOUNDARIES ARE RASED ON A GEOTECHNICAL INTERPRETATION OF ALL AVAILABLE SUBSURFACE DATA AND MAY NOT NECESSARILY REFLECT THE ACTUAL SUBSURFACE CONDITIONS BETWEEN BORINGS OR BETWEEN SAMPLED STRATA WITHIN THE BOREHOLE, THE LABORATORY SAMPLE DATA AND THE IN SITU (IN-PLACE) TEST DATA CAN BE RELIED ON ONLY TO THE DEGREE OF RELIABILITY INHERENT IN THE STANDARD TEST NETHOD. RELIED ON ONE TO THE DEVICE OF RELIABLITY WITHTEN HE THE STANDARD TEST RETITIVE.

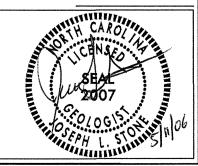
THE OBSERVED WATER LEVELS OR SOIL MOISTURE CONDITIONS MOICATED IN THE SUBSURFACE INVESTIGATIONS ARE AS RECORDED AT THE TIME OF THE INVESTIGATION. THESE WATER LEVELS OR SOIL MOISTURE CONDITIONS MAY VARY CONSIDERABLY WITH TIME ACCORDING TO CLIMATIC CONDITIONS INCLUDING TEMPERATURES, PRECIPITATION AND WIND, AS WELL AS OTHER NON-CLIMATIC FACTORS.

THE BIDDER OR CONTRACTOR IS CAUTIONED THAT DETAILS SHOWN ON THE SUBSURFACE PLANS ARE PRELIMINARY ONLY AND IN MANY CASES THE FINAL DESIGN DETAILS ARE DIFFERENT, FOR BIDDING AND CONSTRUCTION PURPOSES, REFER TO THE CONSTRUCTION PLANS AND DOCUMENTS FOR FINAL DESIGN INFORMATION ON THIS PROJECT. THE DEPARTMENT DOES NOT WARRANT OR GUARANTEE THE SUFFICIENCY OR ACCURACY OF THE INVESTIGATION MADE, NOR THE INTERPETATIONS MADE OR OPENIOD OF THE DEPARTMENT AS TO THE TYPE OF MATERIALS AND CONDITIONS TO BE ENCOUNTERED. THE BIDDER OR CONTRACTOR IS CAUTIONED TO MAKE SUCH INDEPENDENT SUBSURFACE INVESTIGATIONS AS HE DEEMS NECESSARY TO SATISFY HIMSELF AS TO CONDITIONS TO BE ENCOUNTERED ON THIS PROJECT. THE CONTRACTOR SHALL HAVE NO CLAIM FOR ADDITIONAL COMPENSATION OR FOR AN EXTENSION OF TIME FOR ANY REASON RESULTING FROM THE ACTUAL CONDITIONS ENCOUNTERED AT THE SITE DIFFERING FROM THOSE INDICATED IN THE SUBSURFACE INFORMATION.

INVESTIGATED BY J. L. STONE CHECKED BY D. N. ARGENBRIGHT SUBMITTED BY D. N. ARGENBRIGHT ELD MAY 2006 MACTEC







33398.1.1 B-4031 Brunswick Co.

Bridge No. 72 on NC 179 over Jinnys Branch



Looking Southeast Toward End Bent 1