

								•	TOT	AL BI	LL	OF MA	ΓERIAL	***************************************							
	CONST., MAINT., & REMOVAL OF TEMP. ACCESS	REMOVAL OF EXISTING STRUCTURE	PDA TESTING	PDA ASSISTANCE	UNCLASSIFIED STRUCTURE EXCAVATION	CLASS A CONCRETE	BRIDGE APPROACH SLABS	REINFORCING STEEL	HP STE	12 X 53 EL PILES	GAL	24 X 0.5 VANIZED EL PILES	PILE REDRIVES	2-BAR METAL RAIL	1'-2'' × 2'-11 ¹ / ₄ '' CONCRETE PARAPET	RIP RAP CLASS II (2'-0"THICK)	FILTER FABRIC FOR DRAINAGE	ELASTOMERIC BEARINGS	PRE	0" x 1'-9" STRESSED RETE CORED SLABS	PREDRILLING OF PILES
	LUMP SUM	LUMP SUM	EACH	EACH	CU.YDS.	CU.YDS.	LUMP SUM	LBS.	NO.	LIN. FT.	NO.	LIN. FT.	EACH	LIN. FT.	LIN.FT.	TONS	SQ. YARDS	LUMP SUM	NO.	LIN. FT.	EACH
SUPERSTRUCTURE							LUMP SUM							862.63	877.63			LUMP SUM	117	5691.56	·
END BENT 1					1985	15.1		2273	8	520						230	256				
BENTS 1 THRU 8						140.0	·	23,016			56	3360									·
END BENT 2					165	15.1		2318	9	405						161	201				
												***************************************				· · · · · · · · · · · · · · · · · · ·					
TOTAL	LUMP SUM	LUMP SUM	2	2	2150	170.2	LUMP SUM	27,607	17	925	56	3360	35	862.63	877.63	391	457	LUMP SUM	117	5691.56	28

THIS BRIDGE HAS BEEN DESIGNED BY THE STRENGTH DESIGN METHOD AS SPECIFIED IN AASHTO STANDARD SPECIFICATIONS.

ASSUMED LIVE LOAD = HS 20 OR ALTERNATE LOADING EXCEPT THAT CORED SLAB UNITS HAVE BEEN DESIGNED FOR HS 25.

FOR OTHER DESIGN DATA AND GENERAL NOTES, SEE SHEET SN.

FOR EROSION CONTROL MEASURES SEE EROSION CONTROL PLANS.

THIS STRUCTURE HAS BEEN DESIGNED IN ACCORDANCE WITH HEC 18, "EVALUATING SCOUR AT BRIDGES", MAY, 2001.

THIS BRIDGE HAS BEEN DESIGNED IN ACCORDANCE WITH THE REQUIREMENTS OF THE STANDARD SPECIFICATIONS FOR SEISMIC DESIGN OF HIGHWAY BRIDGES FOR SEISMIC PERFORMANCE CATEGORY A.

THE CONTRACTOR SHALL PROVIDE INDEPENDENT ASSURANCE SAMPLES OF REINFORCING STEEL AS FOLLOWS: FOR PROJECTS REQUIRING UP TO 400 TONS OF REINFORCING STEEL, ONE 30 INCH SAMPLE OF EACH SIZE BAR USED, AND FOR PROJECTS REQUIRING OVER 400 TONS OF REINFORCING STEEL, TWO 30 INCH SAMPLES OF EACH SIZE BAR USED. THE BARS FROM WHICH THE SAMPLES ARE TAKEN MUST THEN BE SPLICED WITH REPLACEMENT BARS OF THE SIZE AND LENGTH OF THE SAMPLE, PLUS A MINIMUM LAP SPLICE OF THIRTY BAR DIAMETERS.

DRIVE PILES AT BENT 1 THROUGH BENT 8 TO A REQUIRED BEARING CAPACITY OF 180 TONS PER PILE. THE REQUIRED BEARING CAPACITY IS EQUAL TO THE ALLOWABLE BEARING CAPACITY WITH A MINIMUM FACTOR OF SAFETY OF TWO PLUS ADDITIONAL CAPACITY TO ACCOUNT FOR DOWN DRAG OR NEGATIVE SKIN FRICTION AND SCOUR.

DRIVE PILES AT END BENT 1 AND END BENT 2 TO A REQUIRED BEARING CAPACITY OF 100 TONS PER PILE. THE REQUIRED BEARING CAPACTY IS EQUAL TO THE ALLOWABLE BEARING CAPACITY WITH A MINIMUM FACTOR OF SAFETY OF TWO PLUS ADDITIONAL CAPACITY FOR DOWN DRAG OR NEGATIVE SKIN FRICTION AND SCOUR.

THE ALLOWABLE BEARING CAPACITY AT END BENT 1 AND END BENT 2 IS 50 TONS PER PILE.

THE ALLOWABLE BEARING CAPACITY AT BENT 1 THROUGH BENT 8 IS 85 TONS PER PILE.

DRIVE PILES AT BENT 1 THROUGH BENT 3 TO A TIP ELEVATION NO HIGHER THAN -28 FT.

DRIVE PILES AT BENT 4 TO A TIP ELEVATION NO HIGHER THAN -35 FT.

DRIVE PILES AT BENT 5 TO A TIP ELEVATION NO HIGHER THAN -42 FT.

DRIVE PILES AT BENT 6 TO A TIP ELEVATION NO HIGHER THAN -45 FT.

DRIVE PILES AT BENT 7 TO A TIP ELEVATION NO HIGHER THAN -38 FT.

DRIVE PILES AT BENT 8 TO A TIP ELEVATION NO HIGHER THAN -24 FT.

THE SCOUR CRITICAL ELEVATION FOR BENT 1 THROUGH BENT 8 IS -12 FT., -12 FT., -16 FT., -24 FT., -27 FT., -21 FT., AND -8 FT. RESPECTIVELY. THE SCOUR CRITICAL ELEVATIONS ARE FOR USE ONLY BY MAINTENANCE FORCES TO MONITOR POSIBLE SCOUR PROBLEMS DURING THE LIFE OF STRUCTURE.

TESTING THE FIRST PRODUCTION PILE WITH THE PILE DRIVING ANALYZER (PDA) DURING DRIVING, RE-STRIKING, OR RE-DRIVING IS REQUIRED AT THE INTERIOR BENTS. SEE PILE DRIVING ANALYZER SPECIAL PROVISIONS

PIPE PILE PLATES ARE NOT REQUIRED FOR PIPE PILES AT BENT 1 THROUGH 8.

IT HAS BEEN ESTIMATED THAT A HAMMER EQUIVALENT ENERGY RANGE OF 45,000 FT-LB. PER BLOW UP TO 80,000 FT-LB. PER BLOW WILL BE REQUIRED TO DRIVE PILES AT BENT 1 THROUGH BENT 8. THIS ESTIMATED ENERGY RANGE DOES NOT RELEASE THE CONTRACTOR FROM ARTICLE 450-5 OF THE STANDARD SPECIFICATIONS.

PREDRILLING PILES TO ELEVATION -28 FT. MAY BE UTILIZED TO INSTALL PILES AT BENT 1 THROUGH BENT 3 SEE PREDRILLING OF PILE SPECIAL PROVISIONS.

PREDRILLING PILES TO ELEVATION -29 FT. MAY BE UTILIZED TO INSTALL PILES AT BENT 4, SEE PREDRILLING OF PILE SPECIAL PROVISIONS.

PREDRILLING PILES TO ELEVATION -32 FT. MAY BE UTILIZED TO INSTALL PILES AT BENT 5, SEE PREDRILLING OF PILE SPECIAL PROVISIONS.

PREDRILLING PILES TO ELEVATION -34 FT. MAY BE UTILIZED TO INSTALL PILES AT BENT 6, SEE PREDRILLING OF PILE SPECIAL PROVISIONS.

PREDRILLING PILES TO ELEVATION -31 FT. MAY BE UTILIZED TO INSTALL PILES AT BENT 7. SEE PREDRILLING OF PILE SPECIAL PROVISIONS.

PREDRILLING PILES TO ELEVATION -24 FT. MAY BE UTILIZED TO INSTALL PILES AT BENT 8, SEE PREDRILLING OF PILE SPECIAL PROVISIONS.

THE EXISTING STRUCTURE CONSISTING OF 1 SPAN @ 67.5 FT. AND 6 SPANS @ 40.0' WITH 28'-0" CLEAR ROADWAY WIDTH AND STEEL PLANK FLOOR ON STEEL I BEAMS; END BENTS AND INTERIOR BENTS: TIMBER CAPS ON TIMBER PILES, AND LOCATED AT PROPOSED STRUCTURE SHALL SHALL BE REMOVED. THE EXISTING BRIDGE IS PRESENTLY POSTED BELOW THE LEGAL LOAD LIMIT. SHOULD THE STRUCTURAL INTEGRITY OF THE BRIDGE FURTHER DETERIORATE, THIS LOAD LIMITATION MAY BE REDUCED AS FOUND NECESSARY DURING THE LIFE OF PROJECT.

THE SUBSTRUCTURE OF THE EXISTING BRIDGE INDICATED ON THE PLANS IS FROM THE BEST INFORMATION AVAILABLE. SINCE THIS INFORMATION IS SHOWN FOR THE CONVENIENCE OF THE CONTRACTOR, THE CONTRACTOR SHALL HAVE NO CLAIM WHATSOEVER AGAINST THE DEPARTMENT OF TRANSPORTATION FOR ANY DELAYS OR ADDITIONAL COST INCURRED BASED ON DIFFERENCES BETWEEN THE EXISTING BRIDGE SUBSTRUCTURE SHOWN ON THE PLANS AND THE ACTUAL CONDITIONS AT THE PROJECT SITE.

REMOVAL OF THE EXISTING BRIDGE SHALL BE PERFORMED SO AS NOT TO ALLOW DEBRIS TO FALL INTO THE WATER. THE CONTRACTOR SHALL REMOVE THE BRIDGE AND SUBMIT PLANS FOR DEMOLITION IN ACCORDANCE WITH ARTICLE 402-2 OF THE STANDARD SPECIFICATIONS.

IN AS MUCH AS THE PAINT SYSTEM ON THE EXISTING STRUCTURAL STEEL CONTAINS LEAD, THE CONTRACTOR'S ATTENTION IS DIRECTED TO ARTICLE 107-1, OF THE STANDARD SPECIFICATIONS. ANY COSTS RESULTING FROM COMPLIANCE WITH APPLICABLE STATE OR FEDERAL REGULATIONS PERTAINING TO HANDLING OF MATERIALS CONTAINING LEAD BASED PAINT SHALL BE INCLUDED IN THE BID PRICE FOR "REMOVAL OF EXISTING STRUCTURE".

FOR SUBMITTAL OF WORKING DRAWINGS, SEE SPECIAL PROVISIONS.

FOR FALSEWORK AND FORMWORK, SEE SPECIAL PROVISIONS.

THE MATERIAL SHOWN IN THE CROSS-HATCHED AREA ON SHEET S-1 SHALL BE EXCAVATED FOR A DISTANCE OF 25 FT. LEFT SIDE AND 30 FT. RIGHT SIDE AT END BENT 1 AND END BENT 2 AS DIRECTED BY ENGINEER. THIS WORK WILL BE MEASURED AND PAID FOR AS

FOR CRANE SAFETY, SEE SPECIAL PROVISIONS.

UNCLASSIFIED STRUCTURE EXCAVATION.

WHEN DRIVING PILES, THE MAXIMUM BLOW COUNT SHALL NOT BE EXCEEDED.

FOR GROUT, SEE SPECIAL PROVISION "GROUT FOR STRUCTURES".

FOR CONSTRUCTION, MAINTENANCE AND REMOVAL OF TEMPORARY ACCESS AT STATION 22+00 -L-, SEE SPECIAL PROVISIONS.

FOR PRESTRESSED CONCRETE MEMBERS, SEE SPECIAL PROVISIONS.

OBSERVE A SEVENTY FIVE CALENDER DAY WAITING PERIOD AFTER CONSTRUCTING THE EMBANKMENT TO THE BOTTOM OF CAP ELEVATION BEFORE BEGINNING END BENT OR APPROACH SLAB CONSTRUCTION AT END BENT NO.1.

PROJECT NO. B-4020
BEAUFORT / PITT COUNTY
STATION: 22+00.00 -L-

SHEET 3 OF 3

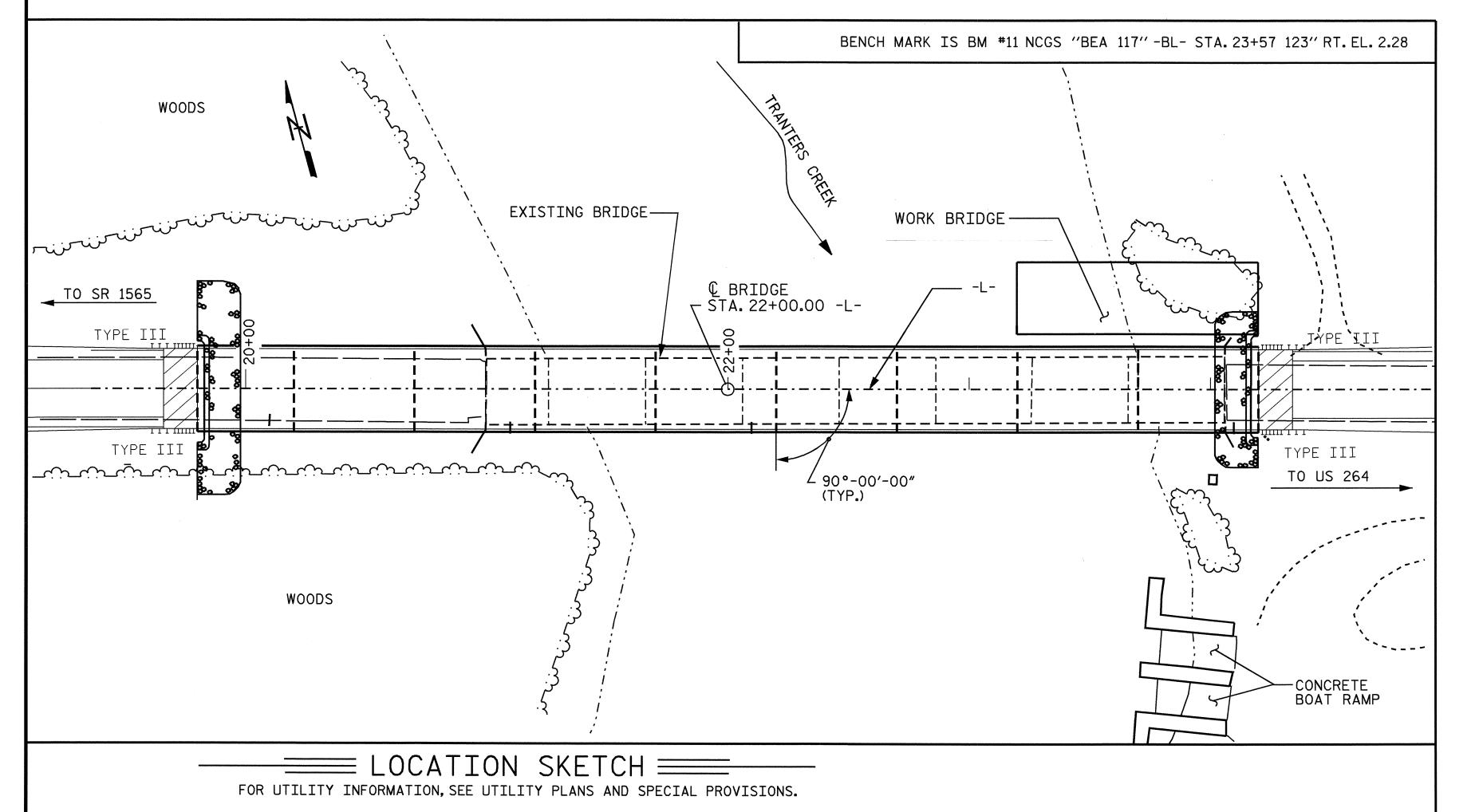
2 1545

DEPARTMENT OF TRANSPORTATION
RALEIGH

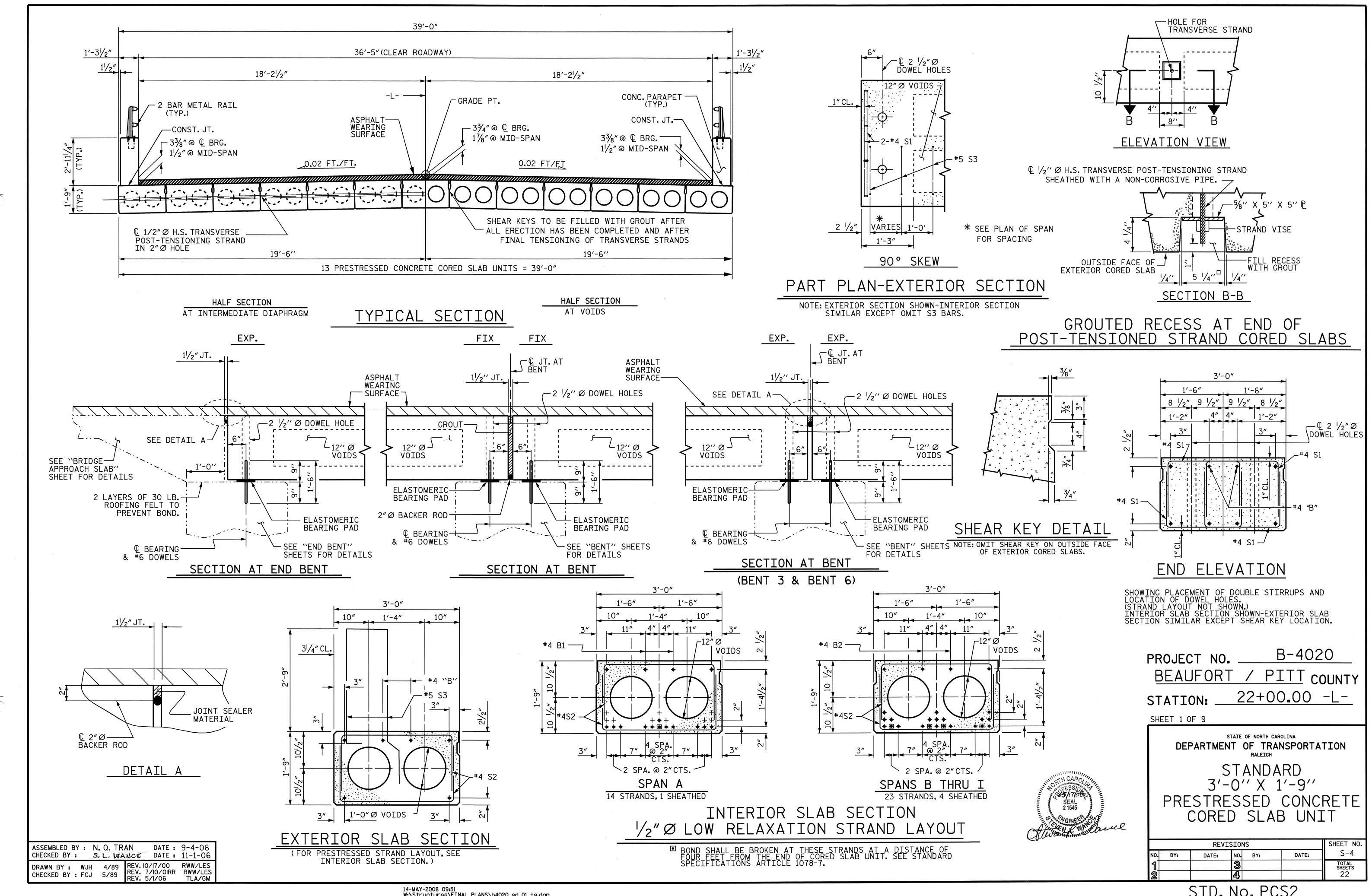
GENERAL DRAWING

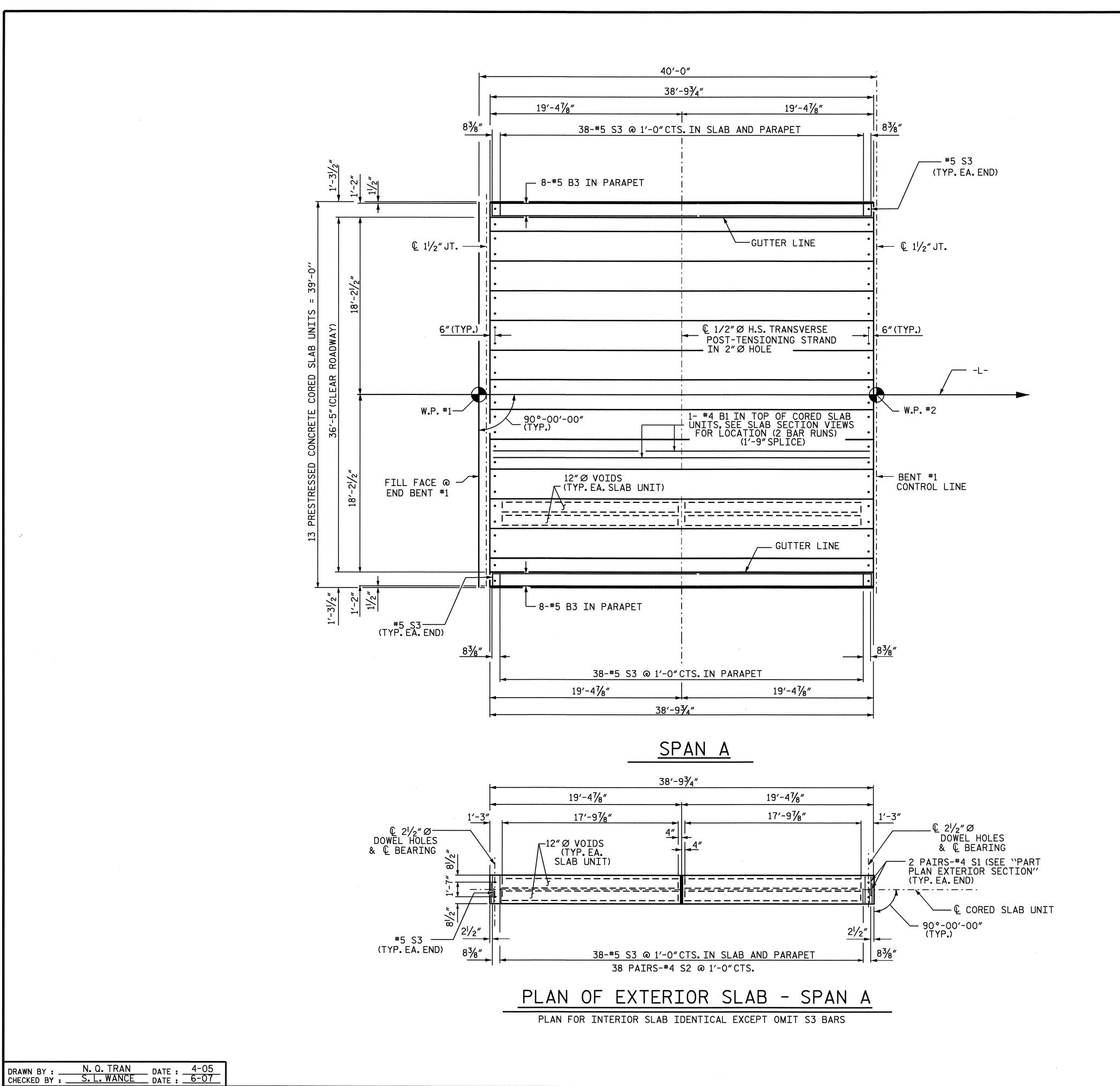
BRIDGE ON SR 1403 OVER TRANTERS CREEK BETWEEN SR 1565 AND US 264

	SHEET NO.					
NO.	BY:	DATE:	NO.	BY:	DATE:	S-3
1			3			TOTAL SHEETS
2	,		4			22



DRAWN BY: N.Q. TRAN DATE: 5-9-07
CHECKED BY: 5-L-WANCE DATE: 7-9-07





PROJECT NO. B-4020
BEAUFORT / PITT COUNTY
STATION: 22+00.00-L-

SHEET 2 OF 9

STATE OF NORTH CAROLINA

DEPARTMENT OF TRANSPORTATION

RALEIGH

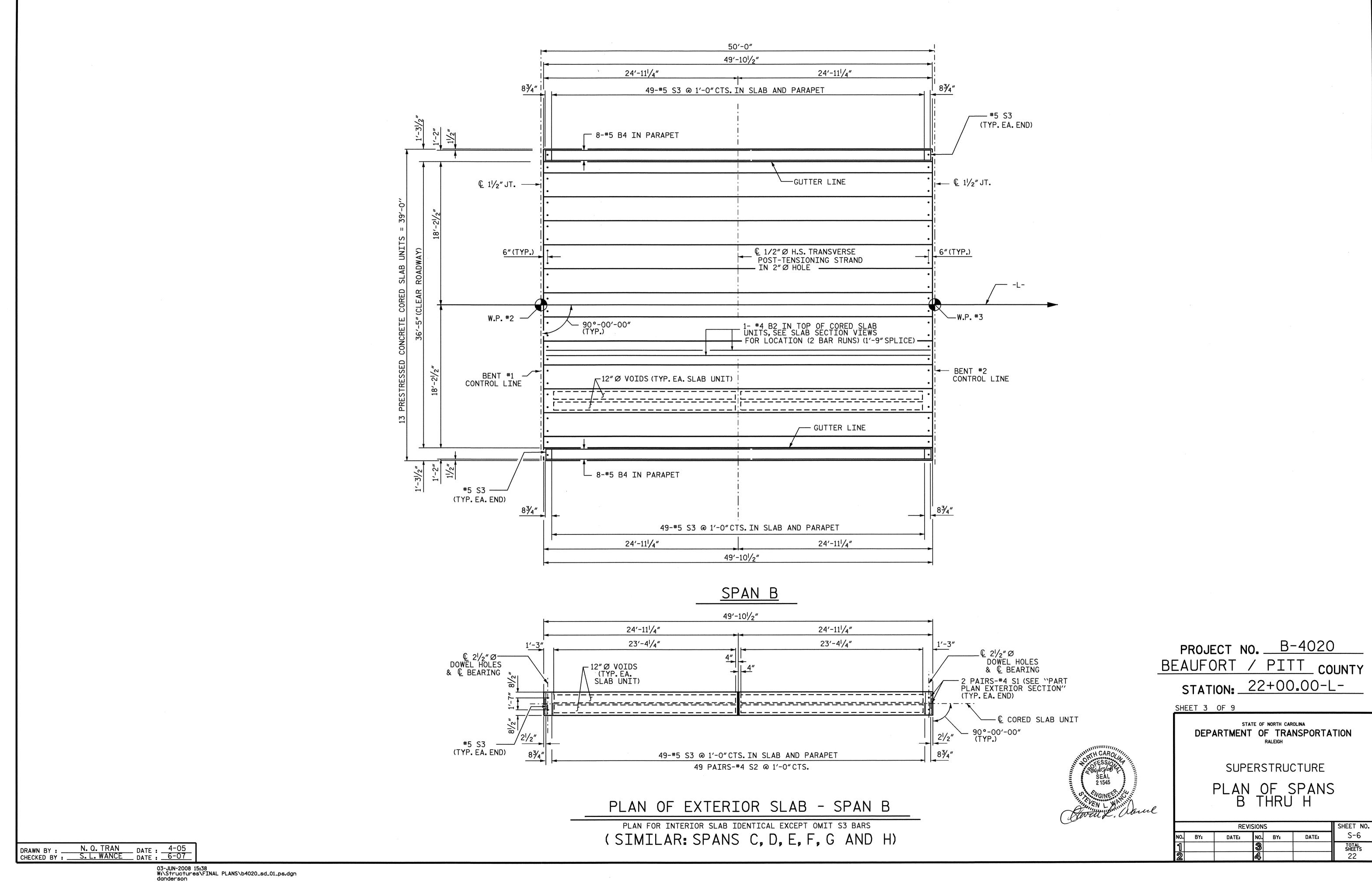
SUPERSTRUCTURE
PLAN OF SPAN A

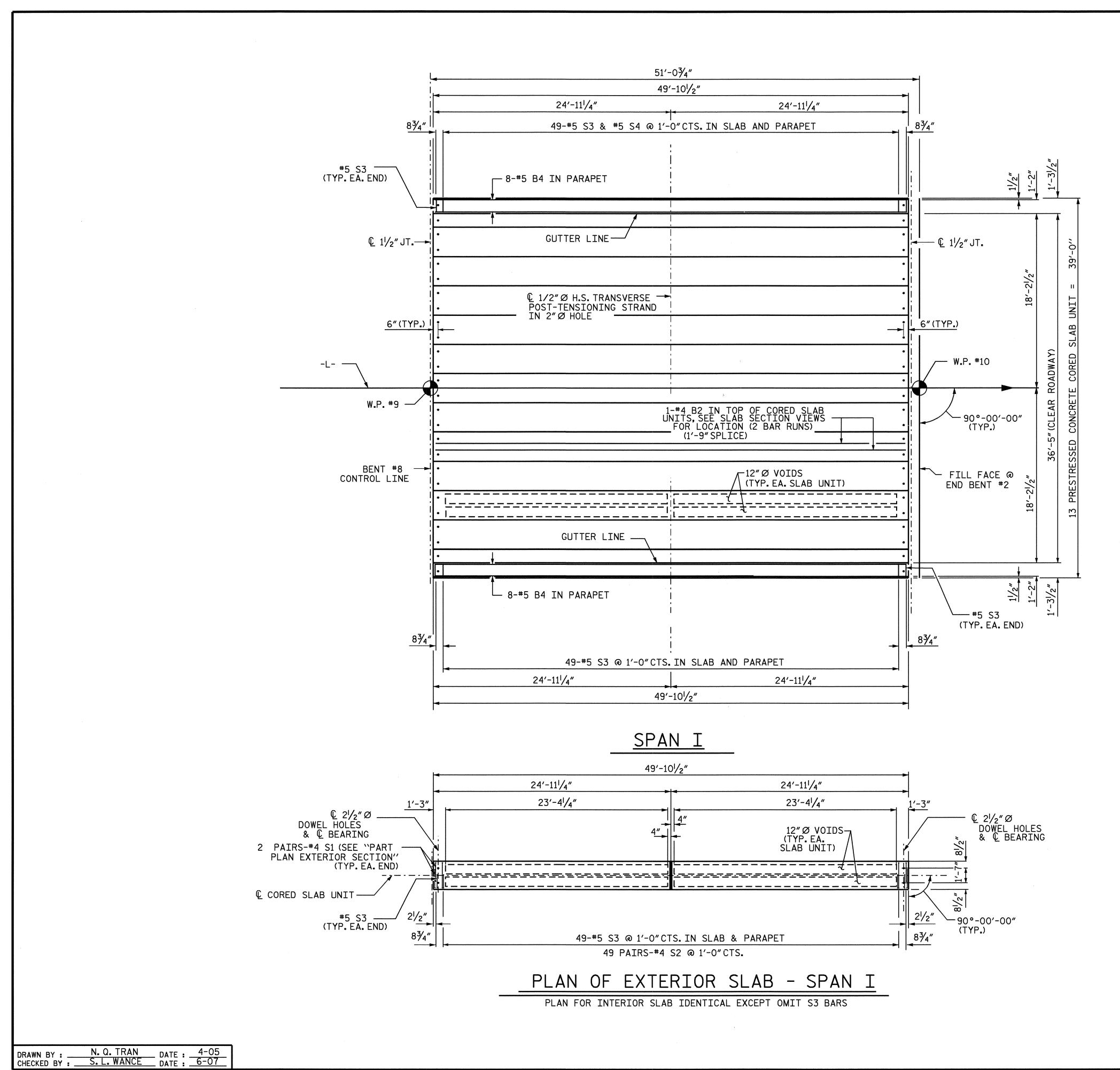
REVISIONS

No. BY: DATE: No. BY: DATE: S-5

1 3 TOTAL SHEETS
2 22

03-JUN-2008 15:21 W:\Structures\FINAL PLANS\b4020_sd_01_ps.dgn danderson





PROJECT NO. B-4020
BEAUFORT / PITT county
STATION: 22+00.00-L-

SHEET 4 OF 9

STATE OF NORTH CAROLINA

DEPARTMENT OF TRANSPORTATION
RALEIGH

SUPERSTRUCTURE

PLAN OF SPAN I

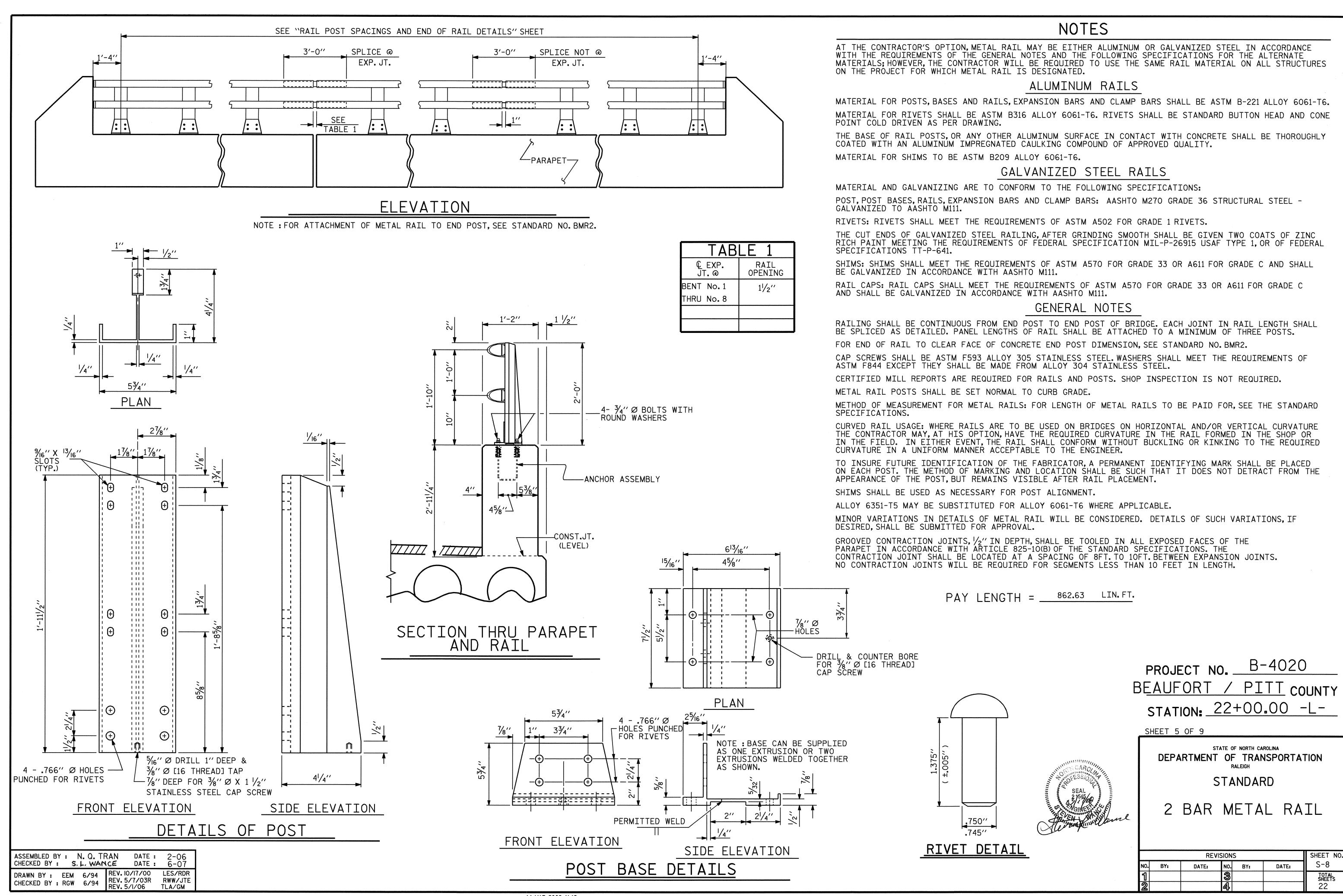
 REVISIONS
 SHEET NO.

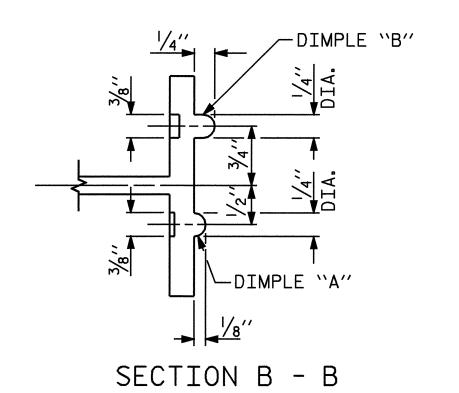
 NO.
 BY:
 DATE:
 S-7

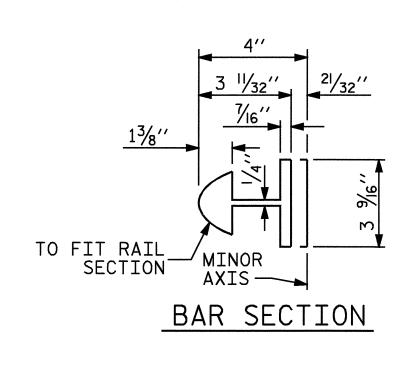
 1
 3
 TOTAL SHEETS

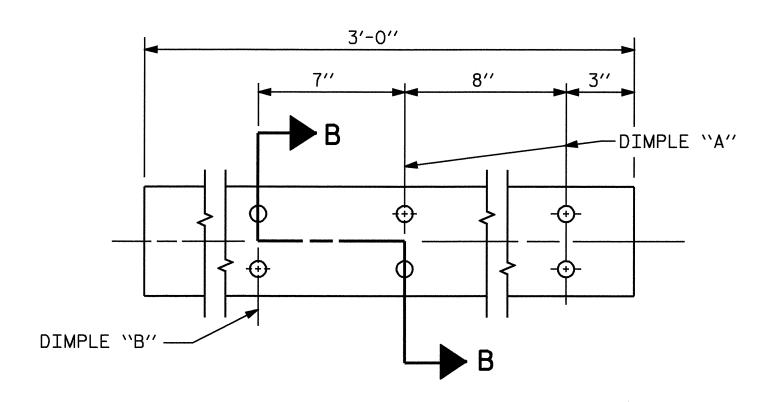
 2
 4
 22

03-JUN-2008 15:38
W:\Structures\FINAL PLANS\b4020_sd_01_ps.dgn
danderson

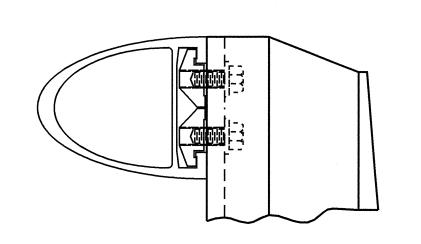








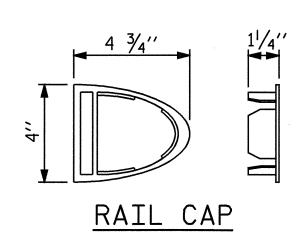
EXPANSION BAR DETAILS

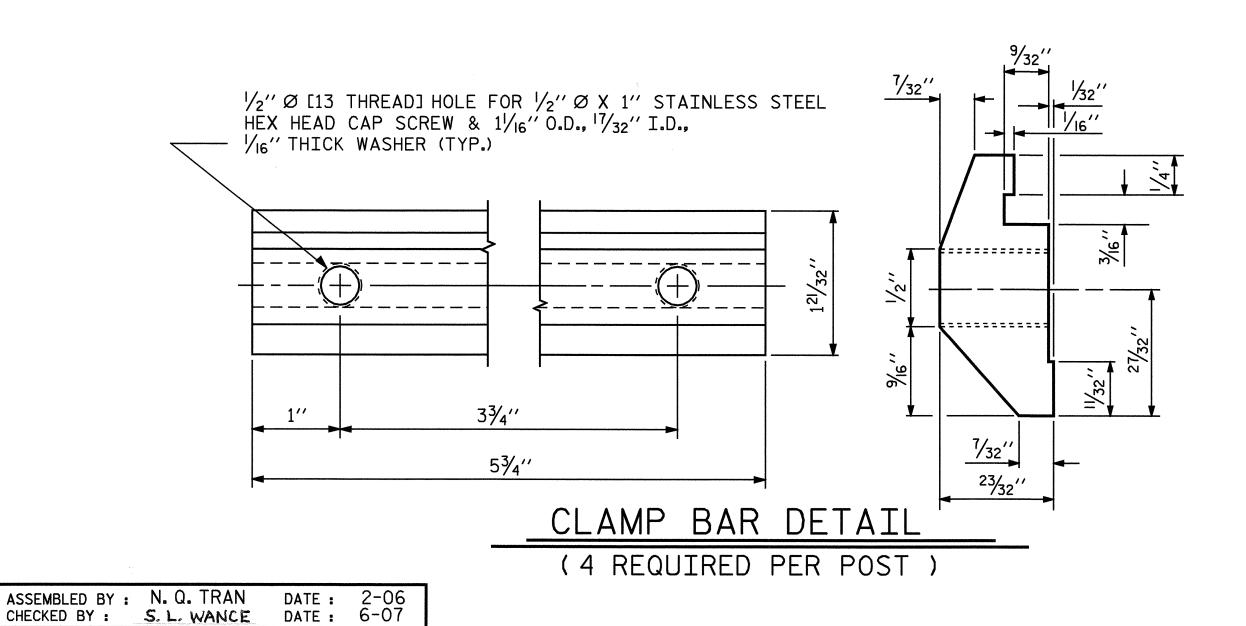


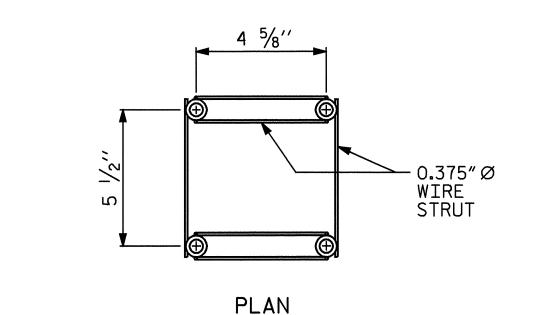
CLAMP ASSEMBLY

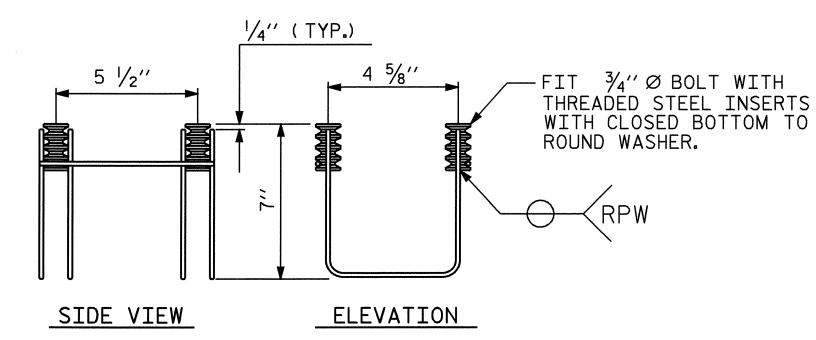
EEM/RGW MAB/LES

DRAWN BY: EEM 6/94 REV. 2/6/97 REV. 8/16/99 REV. 5/1/06





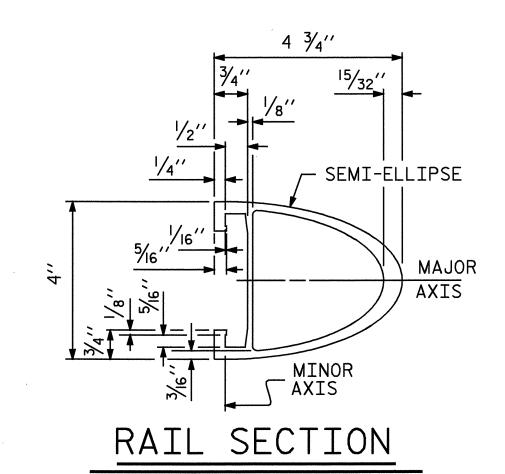




MINIMUM LENGTH OF THREADS IN INSERT (FERRULE): 13/4"

4-BOLT METAL RAIL ANCHOR ASSEMBLY

(140 ASSEMBLIES REQUIRED)



© 7%" Ø HOLES PERMITTED CUTLINE) FRONT PLATE PLATE PLATE PLATE PLATE REAR PLATE

SHIM DETAILS

NOTE:
SHIMS MAY BE CUT ALONG PERMITTED CUTLINE OR
SLOTTED TO EDGE OF PLATE TO FACILITATE PLACEMENT.

STRUCTURAL CONCRETE ANCHOR ASSEMBLY

THE STRUCTURAL CONCRETE ANCHOR ASSEMBLY SHALL CONSIST OF THE FOLLOWING COMPONENTS:

- A. FERRULES SHALL BE MADE FROM STEEL MEETING THE REQUIREMENTS OF AASHTO M169, GRADE 12L14 AND SHALL HAVE A MINIMUM LENGTH OF THREADS OF 2" FOR 3/4" FERRULES.
- B. 4 3/4" Ø X 21/2" BOLTS WITH WASHERS.BOLTS SHALL CONFORM TO THE REQUIREMENTS OF ASTM A307. BOLTS AND WASHERS SHALL BE GALVANIZED. AT THE CONTRACTOR'S OPTION, STAINLESS STEEL BOLTS AND WASHERS MAY BE USED AS AN ALTERNATE FOR THE 3/4" Ø X 21/2" GALVANIZED BOLTS AND WASHERS. THEY SHALL CONFORM TO OR EXCEED THE MECHANICAL REQUIREMENTS OF ASTM A307. THE USE OF THIS ALTERNATE SHALL BE APPROVED BY THE ENGINEER.
- C. WIRE STRUT SHOWN IN THE CONCRETE ANCHOR ASSEMBLY DETAIL IS THE MINIMUM ALLOWABLE SIZE AND SHALL HAVE A MINIMUM TENSILE STRENGTH OF 100,000 PSI. AS AN OPTION, A 7_6 " Ø WIRE STRUT WITH A MINIMUM TENSILE STRENGTH OF 90,000 PSI IS ACCEPTABLE.
- D. THE METAL RAIL ANCHOR ASSEMBLIES TO BE HOT DIPPED GALVANIZED TO CONFORM TO REQUIREMENTS OF AASHTO M111.
- E. THE COST OF THE METAL RAIL ANCHOR ASSEMBLY WITH BOLTS AND WASHERS COMPLETE IN PLACE SHALL BE INCLUDED IN THE PRICE BID FOR LINEAR FEET OF METAL RAIL.
- F. BOLTS TO BE TIGHTENED ONE-HALF TURN WITH A WRENCH FROM A FINGER-TIGHT POSITION.

THE CONTRACTOR MAY USE ADHESIVELY ANCHORED ANCHOR BOLTS IN PLACE OF THE METAL RAIL ANCHOR ASSEMBLY. LEVEL ONE FIELD TESTING IS REQUIRED, AND THE YIELD LOAD OF THE 3/4" Ø BOLT IS 10 KIPS. FOR ADHESIVELY ANCHORED ANCHOR BOLTS OR DOWELS, SEE SPECIAL PROVISIONS.

WHEN ADHESIVELY ANCHORED ANCHOR BOLTS ARE USED, BOLTS SHALL MEET THE REQUIREMENTS OF ASTM F593 ALLOY 304 STAINLESS STEEL WITH MINIMUM 75,000 PSI ULTIMATE STRENGTH. NUTS SHALL MEET THE REQUIREMENTS OF ASTM F594 ALLOY 304 STAINLESS STEEL AND WASHERS SHALL MEET THE REQUIREMENTS OF ASTM F844 EXCEPT THEY SHALL BE MADE FROM ALLOY 304 STAINLESS STEEL.

PROJECT NO. B-4020
BEAUFORT / PITT COUNTY
STATION: 22+00.00 -L-

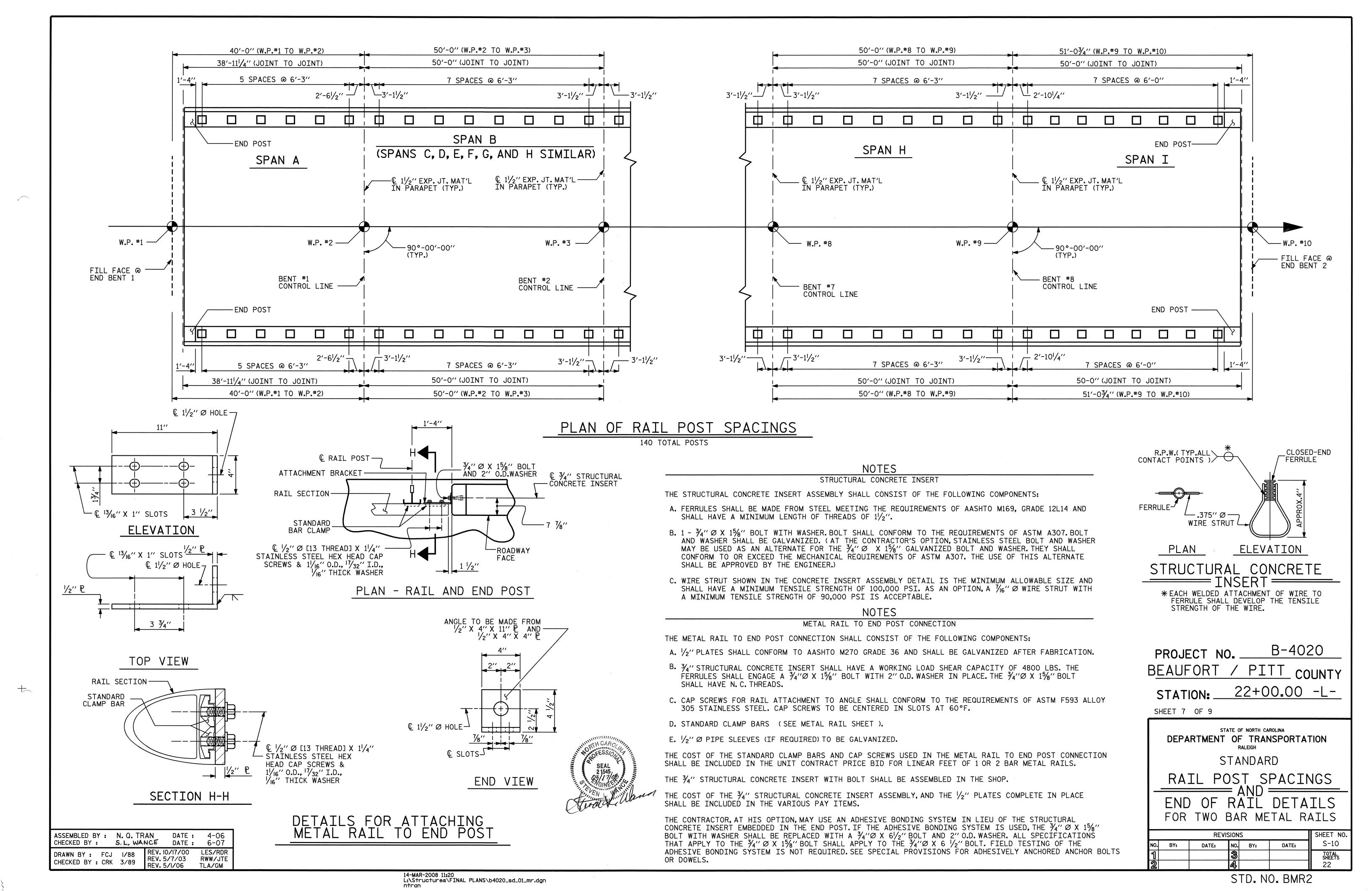
SHEET 6 OF 9

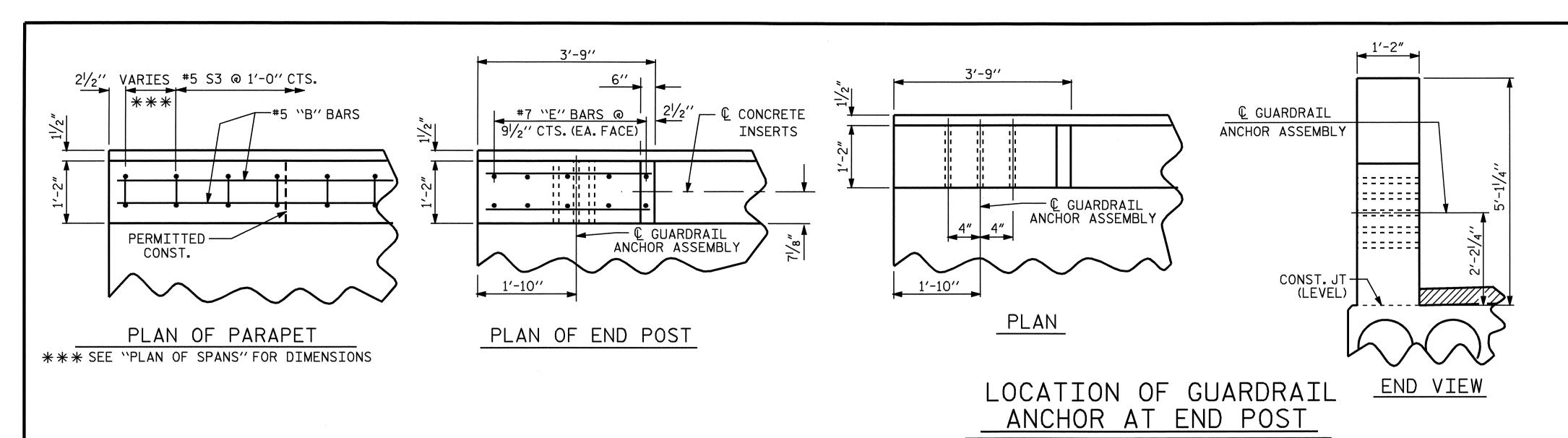
DEPARTMENT OF TRANSPORTATION
RALEIGH
STANDARD

2 BAR METAL RAIL

		SHEET NO.				
).	BY:	DATE:	NO.	BY:	DATE:	S-9
			3			TOTAL SHEETS
)			4			22

14-MAR-2008 11:19 L:\Structures\FINAL PLANS\b4020_sd_01_mr.dgn





NOTES

THE GUARDRAIL ANCHOR ASSEMBLY SHALL CONSIST OF A $\frac{1}{4}$ " HOLD DOWN PLATE AND 7 - $\frac{7}{8}$ " Ø BOLTS WITH NUTS AND WASHERS.

THE HOLD-DOWN PLATE SHALL CONFORM TO AASHTO M270 GRADE 36. AFTER FABRICATION, THE HOLD-DOWN PLATE SHALL BE HOT-DIP GALVANIZED IN ACCORDANCE WITH AASHTO M111.

BOLTS SHALL CONFORM TO THE REQUIREMENTS OF ASTM A307 AND NUTS SHALL CONFORM TO THE REQUIREMENTS OF AASHTO M291. BOLTS, NUTS AND WASHERS SHALL BE GALVANIZED. AT THE CONTRACTOR'S OPTION, STAINLESS STEEL BOLTS, NUTS AND WASHERS MAY BE USED AS AN ALTERNATE FOR THE 1/8" Ø GALVANIZED BOLTS, NUTS AND WASHERS. THEY SHALL CONFORM TO OR EXCEED THE MECHANICAL REQUIREMENTS OF ASTM A307. THE USE OF THIS ALTERNATE SHALL BE APPROVED BY THE ENGINEER.

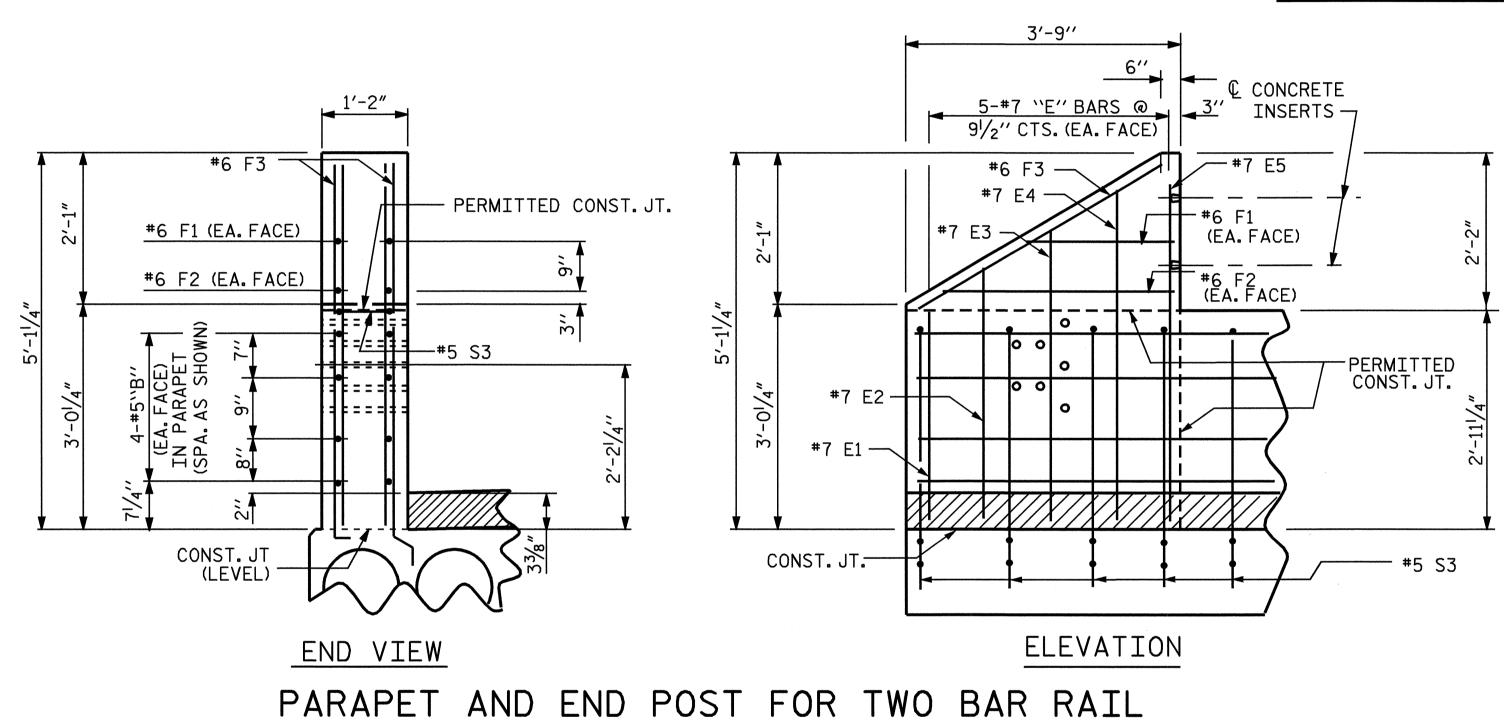
AFTER INSTALLATION, THE EXPOSED THREAD OF THE BOLT SHALL BE BURRED WITH A SHARP POINTED TOOL

THE COST OF THE GUARDRAIL ANCHOR ASSEMBLIES WITH BOLTS, NUTS AND WASHERS COMPLETE IN PLACE, SHALL BE INCLUDED IN THE VARIOUS PAY ITEMS.

THE VERTICAL REINFORCING BARS MAY BE SHIFTED SLIGHTLY IN THE END POST TO CLEAR ASSEMBLY BOLTS.

THE 1 $\frac{1}{4}$ $^{\prime\prime}$ \varnothing HOLES SHALL BE FORMED OR DRILLED WITH A CORE BIT. IMPACT TOOLS WILL NOT BE PERMITTED. ANY CONCRETE DAMAGED BY THIS WORK SHALL BE REPAIRED TO THE SATISFACTION OF THE ENGINEER.

ALL REINFORCING STEEL IN PARAPET AND END POSTS SHALL BE EPOXY COATED.



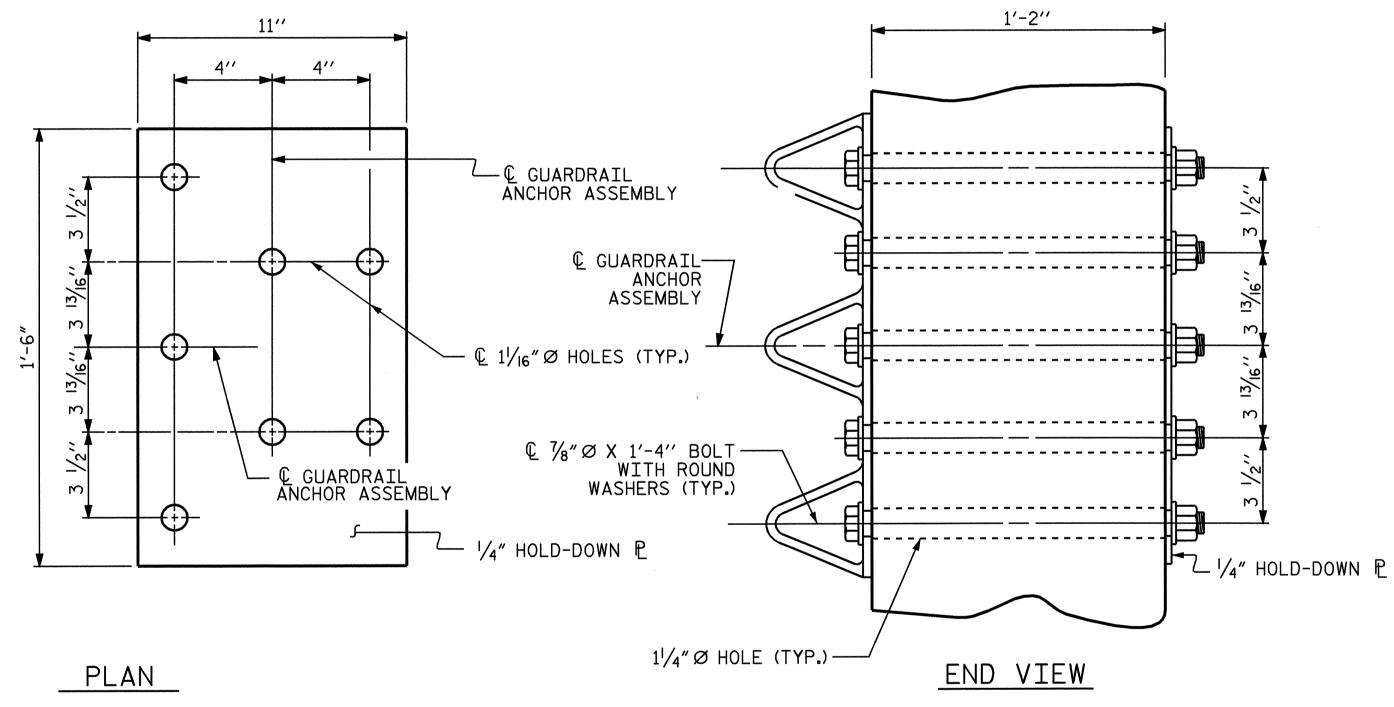
END BENT :		
**		**
	-L-	
**		**
		`
SKETO	CH SHOWING POINTS OF	ATTACHMENT
	**LOCATION OF GUARDRAIL A	TTACHMENT

ASSEMBLED BY: N.Q. TRAN DATE: 2-06 CHECKED BY: S.L. WANCE DATE: 6-07

DRAWN BY: EEM 6/94 CHECKED BY: RGW 6/94

RWW/LES RWW/JTE

BAR	NO.	SIZE	TYPE	LENGTH	WEIGHT			
		5						
 ★ B3	16	# 5	STR.	38′-5″	641			
 ₩ B4	128	# 5	STR.	49'-4"	6586			
* E1	8	# 7	STR.	2′-9″	45			
* E2	8	# 7	STR.	3'-3"	53			
 ★ E3	8	# 7	STR.	3'-9"	61			
∗ E4	8	# 7	STR.	4'-3"	69			
★ E5	8	# 7	STR.	4'-7"	75			
* F1	8	# 6	STR.	1′-10″	22			
* F2	<u>8</u> 8	# 6	STR.	3'-0"	22 36			
 ¥ F3	8	# 6	STR.	3′-8″	44			
* EPOXY COATED REINFORCING STEEL LBS. = 7632								
CLASS AA CONCRETE (IN END POSTS & PARAPET) CU.YDS. = 115.3								



GUARDRAIL ANCHOR ASSEMBLY DETAILS

PROJECT NO. B-4020 BEAUFORT / PITT COUNTY STATION: 22+00.00 -L-

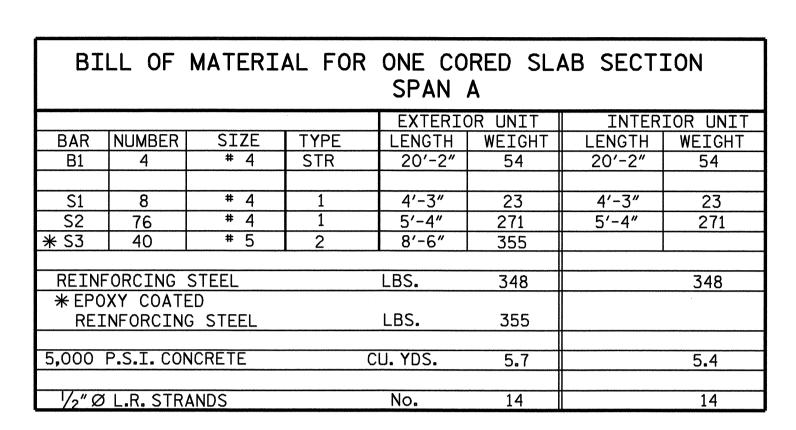
SHEET 8 OF 9

STATE OF NORTH CAROLINA DEPARTMENT OF TRANSPORTATION STANDARD GUARDRAIL ANCHORAGE DETAILS FOR METAL RAILS

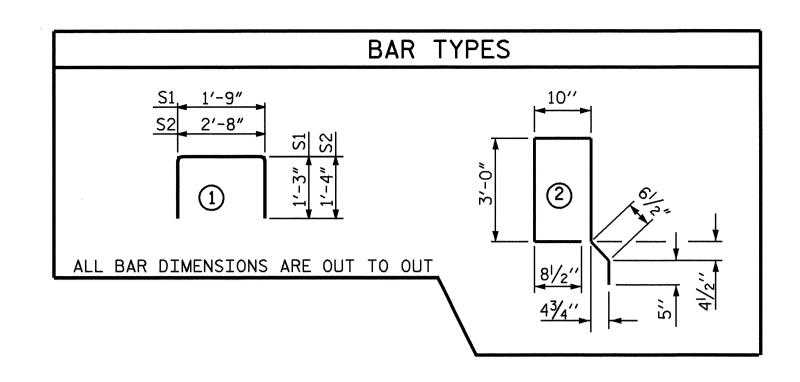
	SHEET NO.				
BY:	DATE:	NO.	BY:	DATE:	S-11
		3			TOTAL SHEETS
		4			22
	BY:		BY: DATE: NO.	3	BY: DATE: NO. BY: DATE:

14-MAY-2008 09:27 W:\Structures\FINAL PLANS\b4020_sd_01_mr.dgn

STD. NO. BMR8



BIL	BILL OF MATERIAL FOR ONE CORED SLAB SECTION SPANS B, C, D, E, F, G, H AND I								
		INTER	IOR UNIT						
BAR	NUMBER	SIZE	TYPE	LENGTH	WEIGHT	LENGTH	WEIGHT		
B2	4	# 4	STR	25′-8″	69	25′-8″	69		
S1	8	# 4	1	4'-3"	23	4′-3″	23		
S2	98	# 4	1	5'-4"	349	5′-4″	349		
* S3	51	# 5	2	8′-6″	452				
REINF	ORCING	STEEL		LBS.	441		441		
	XY COATE			LBS.	452				
5,000	P.S.I. CON	NCRETE	С	U. YDS.	6.9		6.9		
√2″ Ø	L.R. STR	ANDS		No.	23		23		



GRADE 270 S	TRANDS		
	1/2"Ø L.R.		
AREA (SQUARE INCHES)	0.153		
ULTIMATE STRENGTH (LBS.PER STRAND)	41,300		
APPLIED PRESTRESS (LBS.PER STRAND)	30,980		

DEAD LOAD DEFLECTION AN	ND CAMBER
	3'-0"× 1'-9"
	½″Ø L.R. STRAND
CAMBER (SLAB ALONE IN PLACE)	¹³ / ₁₆ ''
DEFLECTION DUE TO SUPERIMPOSED DEAD LOAD**	1/8′′
FINAL CAMBER	11/16"
** INCLUDES FUTURE WEARING SURFACE	

DEAD LOAD DEFLECTION A SPANS B, C, D, E, F, G,	
	3'-0"× 1'-9"
	1/2″Ø L.R. STRAND
CAMBER (SLAB ALONE IN PLACE)	21/16"
DEFLECTION DUE TO SUPERIMPOSED DEAD LOAD**	1/4"
FINAL CAMBER	1 ¹³ / ₁₆ "
** INCLUDES FUTURE WEARING SURFACE	

CORED SLABS REQUIRED							
		SPAN A	SPANS B, C, D, E, F, G, H & I				
	NUMBER PER SPAN	LENGTH	LENGTH	TOTAL LENGTH			
EXTERIOR C.S.	2	38′-9¾″	49'-101/2"	875'-71/2"			
INTERIOR C.S.	11	38′-9 ³ ⁄ ₄ ″	49′-101/2″	4815′-111/4″			
TOTAL				5691′-6 ³ ⁄4″			

NOTES

ALL PRESTRESSING STRANDS SHALL BE 7-WIRE LOW RELAXATION GRADE 270 STRANDS AND SHALL CONFORM TO AASHTO M203 EXCEPT FOR SAMPLING REQUIREMENTS WHICH SHALL BE IN ACCORDANCE WITH THE STANDARD SPECIFICATIONS.

ALL REINFORCING STEEL CAST WITH THE CORED SLAB SECTIONS SHALL BE GRADE 60 AND SHALL BE INCLUDED IN THE UNIT PRICE BID FOR PRESTRESSED CONCRETE CORED SLABS.

RECESSES FOR TRANSVERSE STRANDS SHALL BE GROUTED AFTER THE TENSIONING OF THE STRANDS.

THE $2\frac{1}{2}$ "Ø DOWEL HOLES AT FIXED ENDS OF SLAB SECTIONS SHALL BE FILLED WITH GROUT. THE $2\frac{1}{2}$ "Ø DOWEL HOLES AT EXPANSION ENDS OF SLAB SECTIONS SHALL BE FILLED WITH JOINT SEALER MATERIAL TO $1\frac{1}{2}$ " ABOVE THE TOP OF DOWELS AND THEN FILLED WITH GROUT.

THE JOINT SEALER MATERIAL SHALL CONFORM TO THE REQUIREMENTS OF TYPE SL LOW MODULUS SILICONE SEALANT. THE 2"Ø BACKER ROD SHALL CONFORM TO THE REQUIREMENTS OF TYPE M BOND BREAKER. SEE SECTION 1028 OF THE STANDARD SPECIFICATIONS.

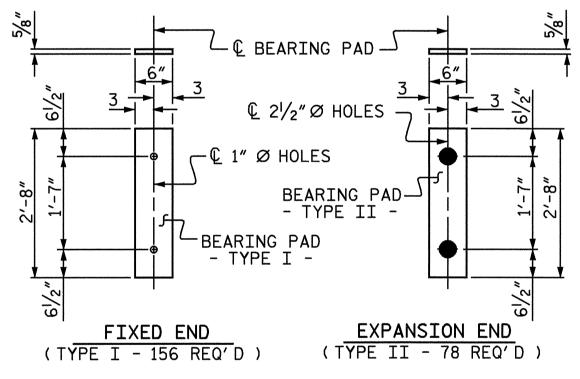
WHEN CORED SLABS ARE CAST, A POSITIVE HOLD-DOWN SYSTEM SHALL BE EMPLOYED TO PREVENT VOIDS FROM RISING OR MOVING SIDEWAYS. THIS SYSTEM SHALL BE DESIGNED TO BE LEFT IN PLACE UNTIL THE CONCRETE HAS REACHED RELEASE STRENGTH. AT LEAST THREE WEEKS PRIOR TO CASTING CORED SLABS, THE CONTRACTOR SHALL SUBMIT TO THE ENGINEER FOR REVIEW AND COMMENT, DETAILED DRAWINGS OF THE PROPOSED HOLD-DOWN SYSTEM. IN ADDITION TO STRUCTURAL DETAILS, LOCATION AND SPACING OF THE HOLD-DOWNS SHALL BE INDICATED.

THE TRANSFER OF LOAD FROM THE ANCHORAGES TO THE CORED SLAB UNIT SHALL BE DONE WHEN THE CONCRETE HAS REACHED A COMPRESSIVE STRENGTH OF NOT LESS THAN 4000 PSI.

PRESTRESSING STRANDS SHALL BE CUT FLUSH WITH THE CORED SLAB UNIT ENDS.

APPLY EPOXY PROTECTIVE COATING TO CORED SLAB UNIT ENDS.





ELASTOMERIC BEARING DETAILS

PROJECT NO. B-4020
BEAUFORT / PITT COUNTY
STATION: 22+00.00-L-

STATION: ____

SHEET 9 OF 9

DEF

3'-0" X 1'-9"
PRESTRESSED
CONCRETE CORED
SLAB UNIT

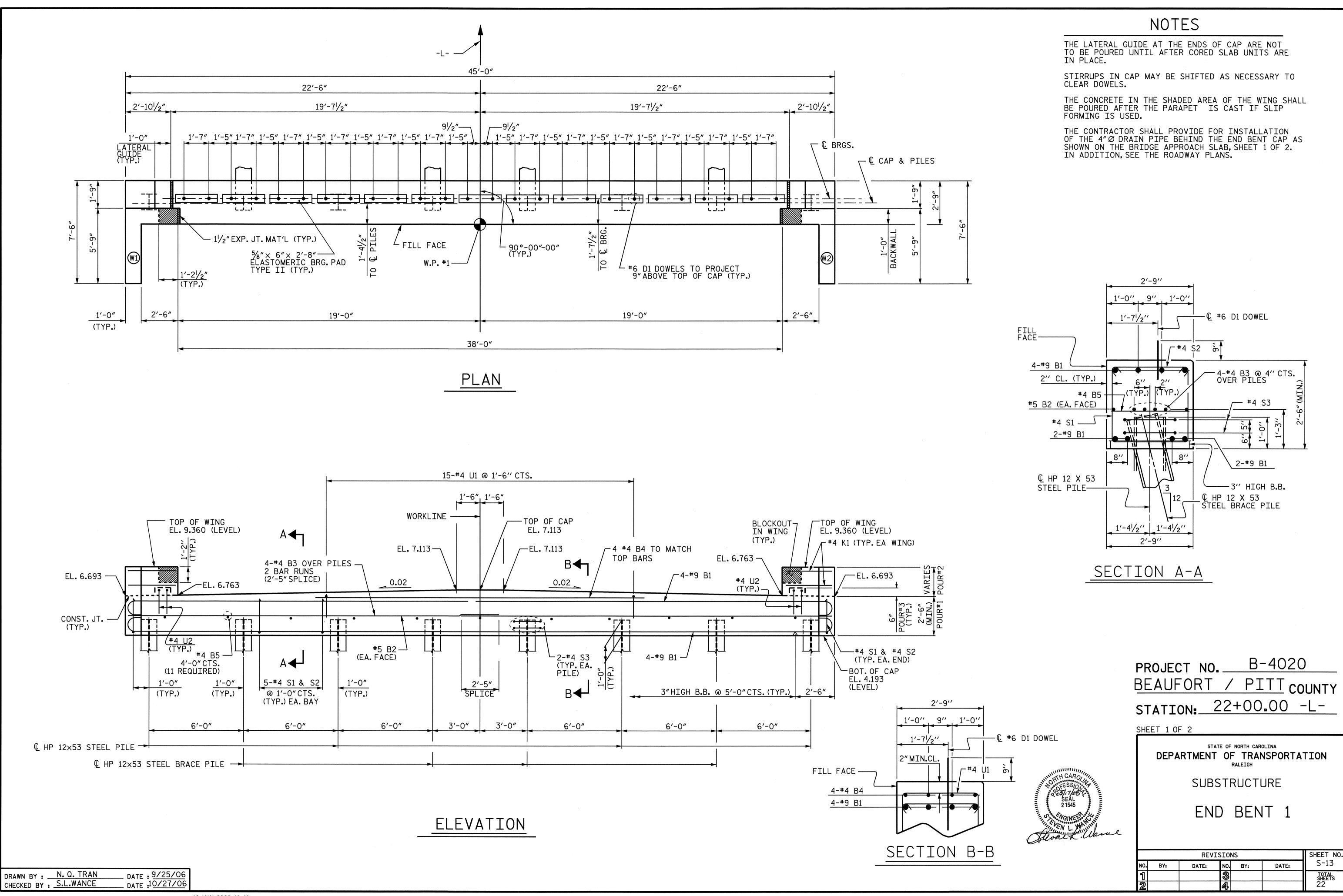
STATE OF NORTH CAROLINA

REVISIONS

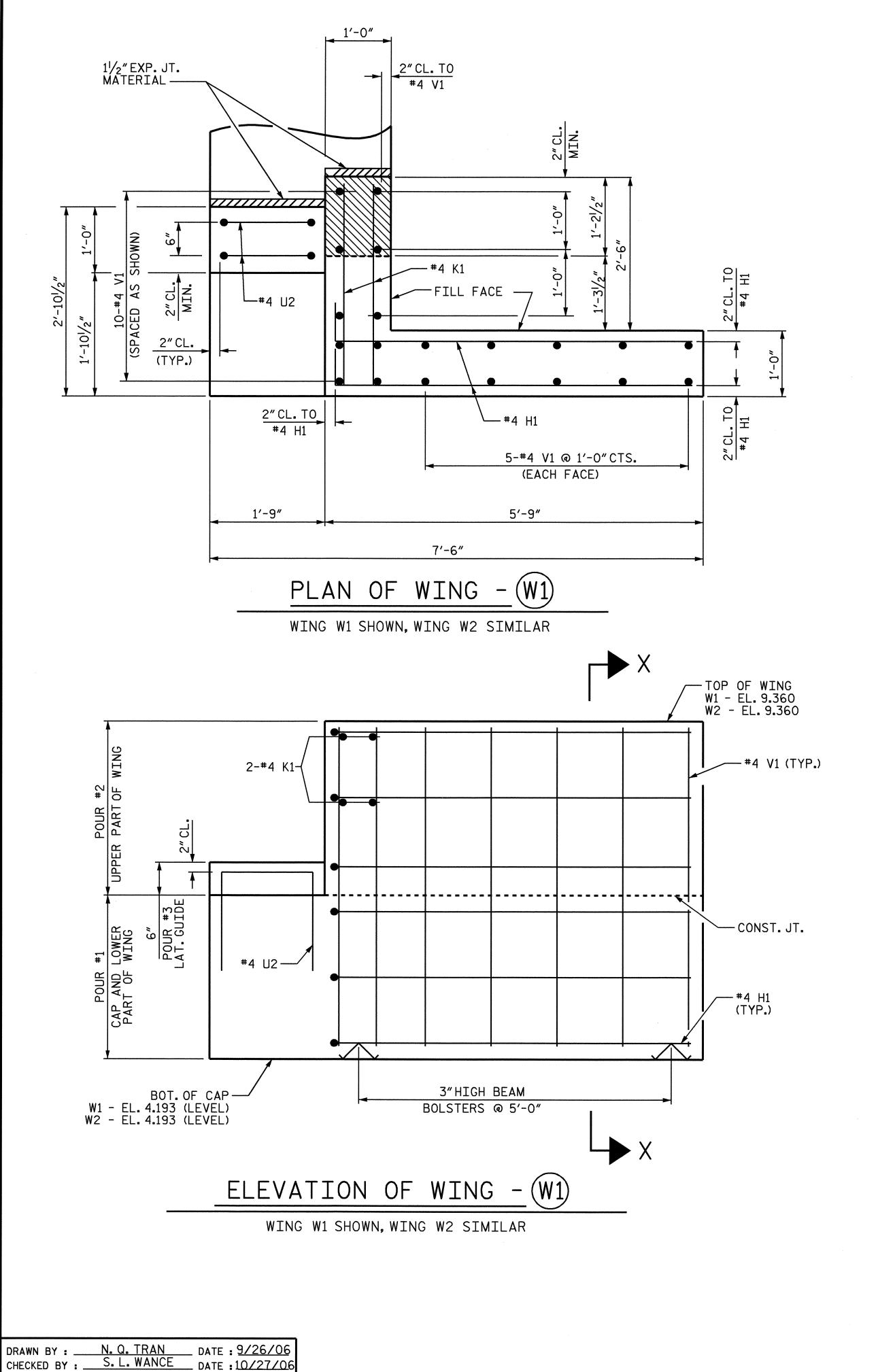
DATE: NO. BY: DATE: S-12

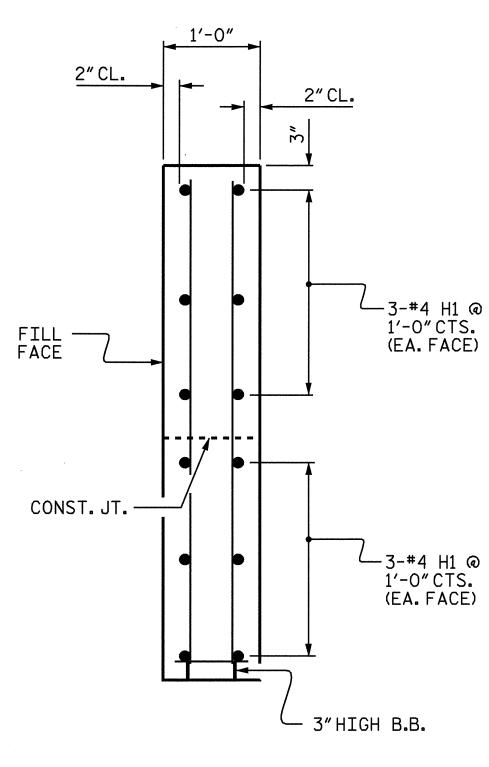
TOTAL SHEETS
22

ASSEMBLED BY: N.Q. TRAN DATE: 4-05 CHECKED BY: S.L. WANCE DATE: 6-07

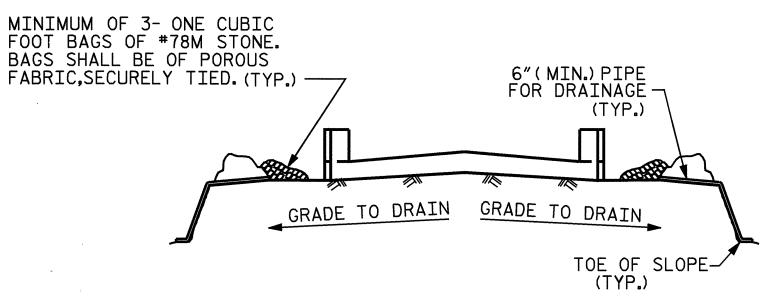


16-MAY-2008 10:40
r:\structures\finalplans\b4020_sd_01_eb.dgn
jtillman





SECTION X-X

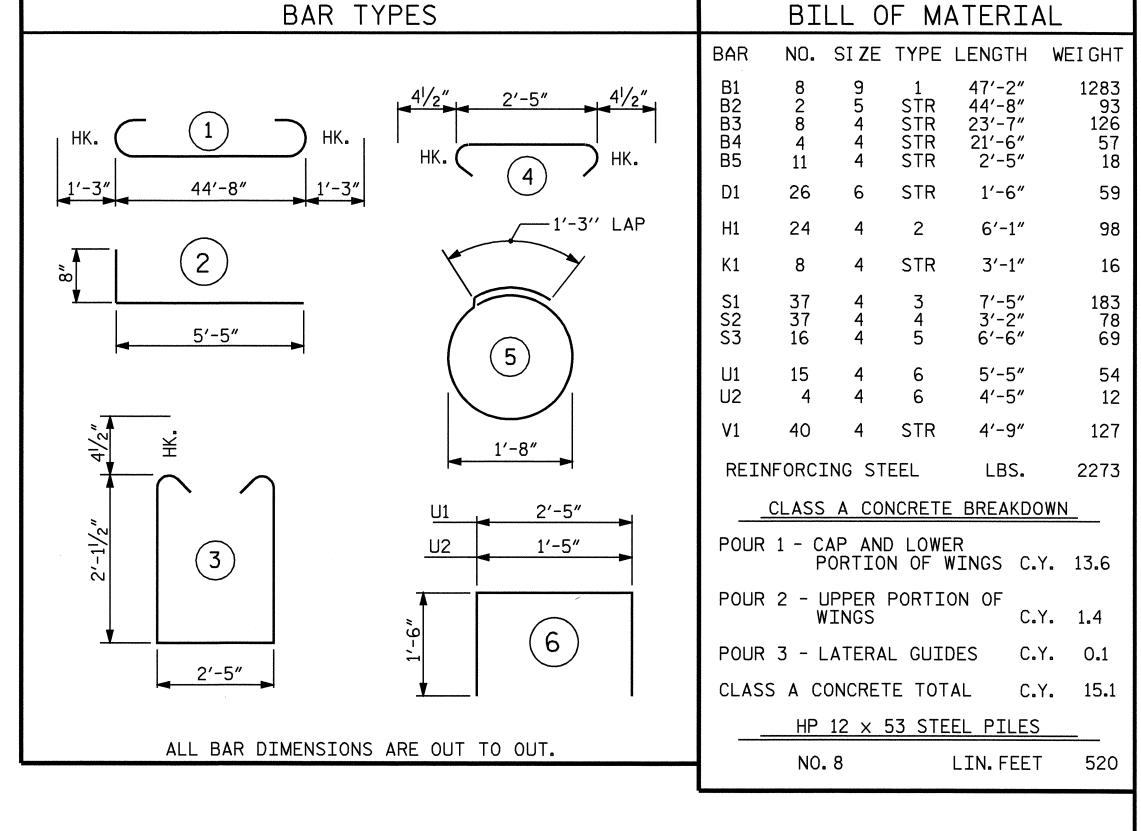


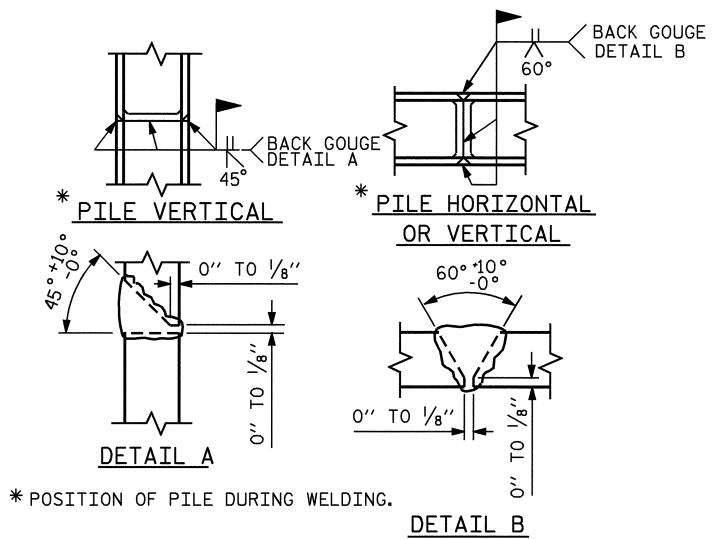
BAGGED STONE AND PIPE SHALL BE PLACED IMMEDIATELY AFTER COMPLETION OF END BENT EXCAVATION. PIPE MAY BE EITHER CONCRETE, CORRUGATED STEEL, CORRUGATED ALUMINUM ALLOY, OR CORRUGATED PLASTIC. PERFORATED PIPE WILL NOT BE ALLOWED.

BAGGED STONE SHALL REMAIN IN PLACE UNTIL THE ENGINEER DIRECTS THAT IT BE REMOVED. THE CONTRACTOR SHALL REMOVE AND DISPOSE OF SILT ACCUMULATIONS AT BAGGED STONE WHEN SO DIRECTED BY THE ENGINEER. BAGS SHALL BE REMOVED AND REPLACED WHENEVER THE ENGINEER DETERMINES THAT THEY HAVE DETERIORATED AND LOST THEIR EFFECTIVENESS.

NO SEPARATE PAYMENT WILL BE MADE FOR THIS WORK AND THE ENTIRE COST OF THIS WORK SHALL BE INCLUDED IN THE UNIT CONTRACT PRICE BID FOR THE SEVERAL PAY ITEMS.

TEMPORARY DRAINAGE AT END BENT





PILE SPLICE DETAILS

PROJECT NO. B-4020

BEAUFORT / PITT COUNTY

STATION: 22+00.00-L-

SHEET 2 OF 2

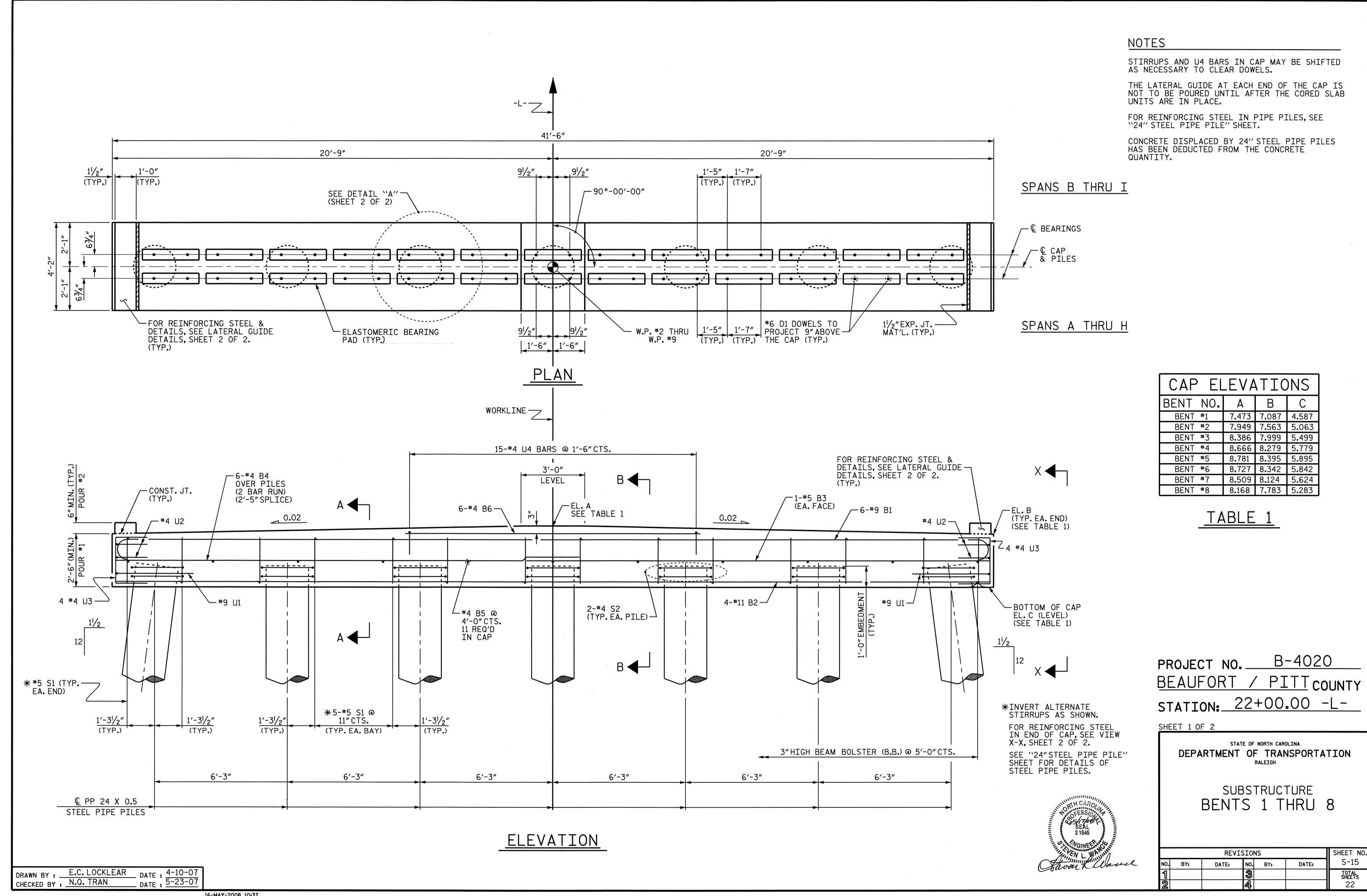
SEAL 2 1545 DEPARTMENT OF TRANSPORTATION
RALEIGH

SUBSTRUCTURE

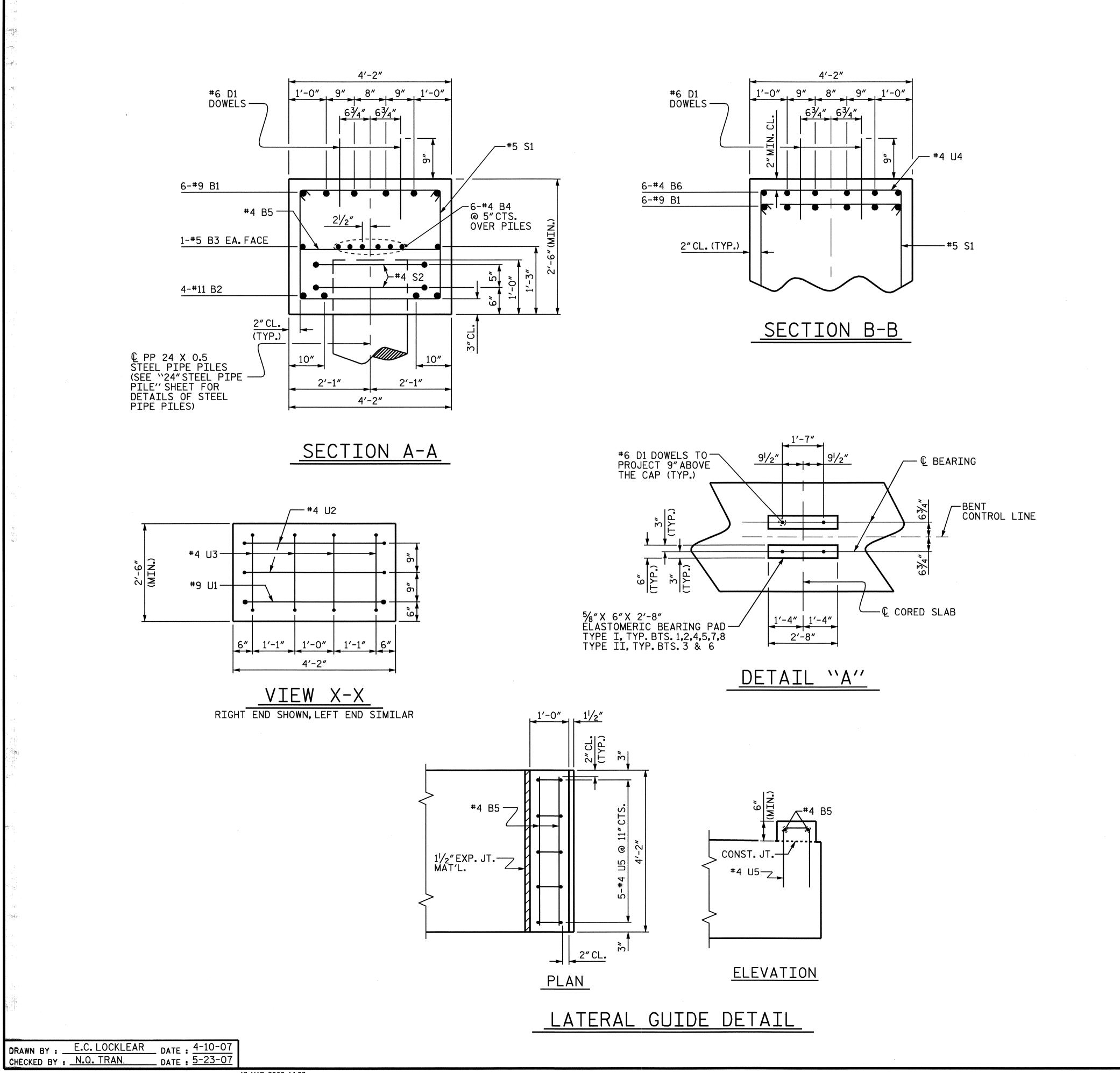
END BENT 1

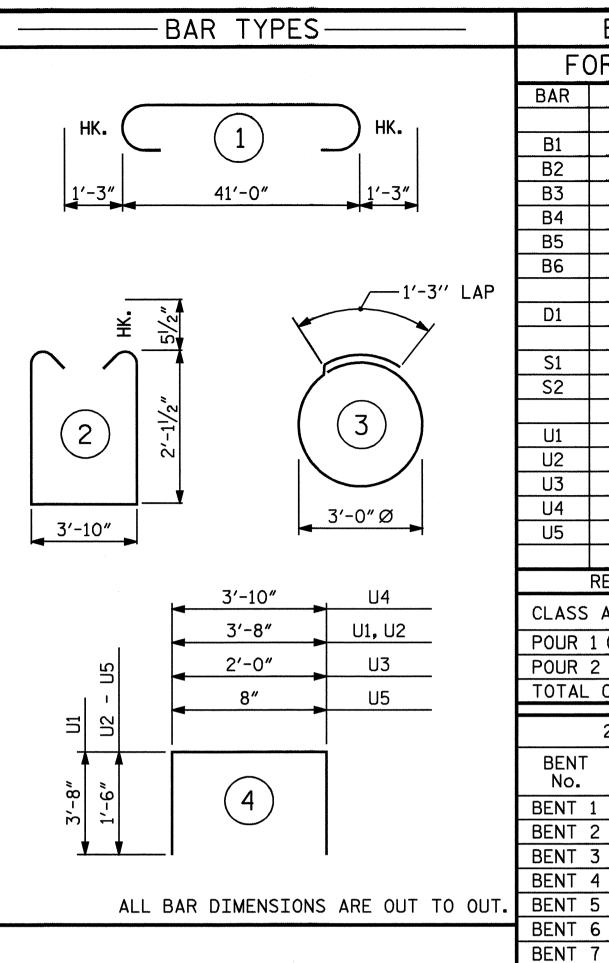
		REVIS	IO	NS		SHEET NO.
10.	BY:	DATE:	NO.	BY:	DATE:	S-14
1			3			TOTAL SHEETS
2			4			22

16-MAY-2008 10:40
r:\structures\final plans\b4020_sd_01_eb.dgn



16-MAY-2008 10:37
r:\structures\final plans\b4020_sd_01_bt.dgn
jtillman





	B5		15	#4	STR	3′-10′	"	38		
	В6		6	#4	STR	21'-6'	"	86		
۱P										
	D1		52	#6	STR	1′-6″		117		
i										
	S1		32	#5	2	9′-0″		300		
	S2		14	#4	3	10'-9'	"	101		
	U1		2	#9	4	11'-0'	′	75		
	U2		4	#4	4	6′-8″	'	18		
	U3		8	#4	4	5′-0″	'	27		
	U4		15	#4	4	6′-10°	"	68		
	U5		10	#4	4	3′-8″		24		
	REINFORCING STEEL = LBS 2877							2877		
	CLASS AA CONCRETE BREAKDOWN									
	POUR	1 ((CAP)	CAP) C.Y. 17.3 LATERAL GUIDES) C.Y. 0.2 ASS A_ CONCRETE C.Y. 17.5						
	POUR	2 (LATE							
	TOTAL	CI	ASS							
		2	4 X (0.5 ST	EEL F	PIPE PIL	.ES			
	BENT No.	NUMBER OF TOTAL LENGTH PIPE PILES (IN LIN. FT.)								
	BENT	1								
	BENT	2								
		3	7			385				
	BENT	4		7			55			
UT.		5		7			155			
		6		7		455				
		7		7		385				

BILL OF MATERIAL

FOR ONE BENT (8 REQ'D)

#5 STR

NO. | SIZE | TYPE | LENGTH | WEIGHT

#11 STR 41′-2″

#4 STR 21'-10"

43'-6"

41'-2"

280

887

86

175



B-4020 PROJECT NO. ____ BEAUFORT / PITT COUNTY STATION: 22+00.00 -L-

SHEET 2 OF 2

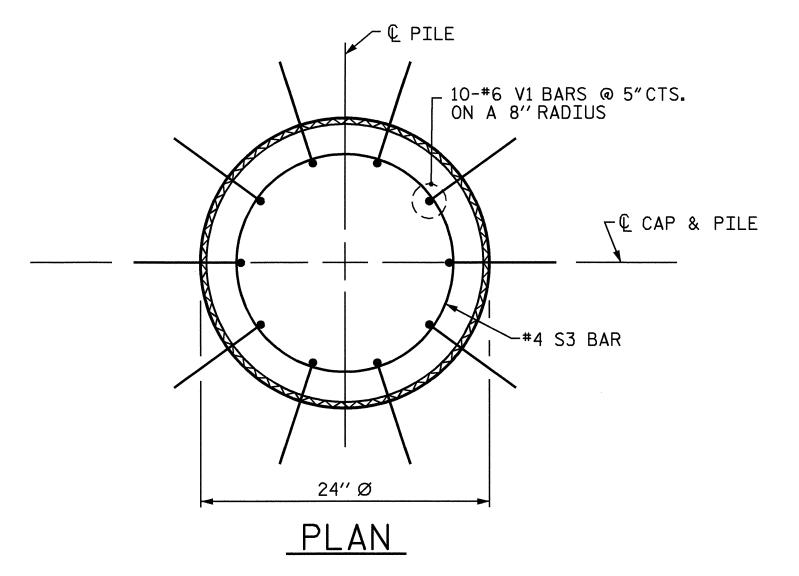
BENT 8

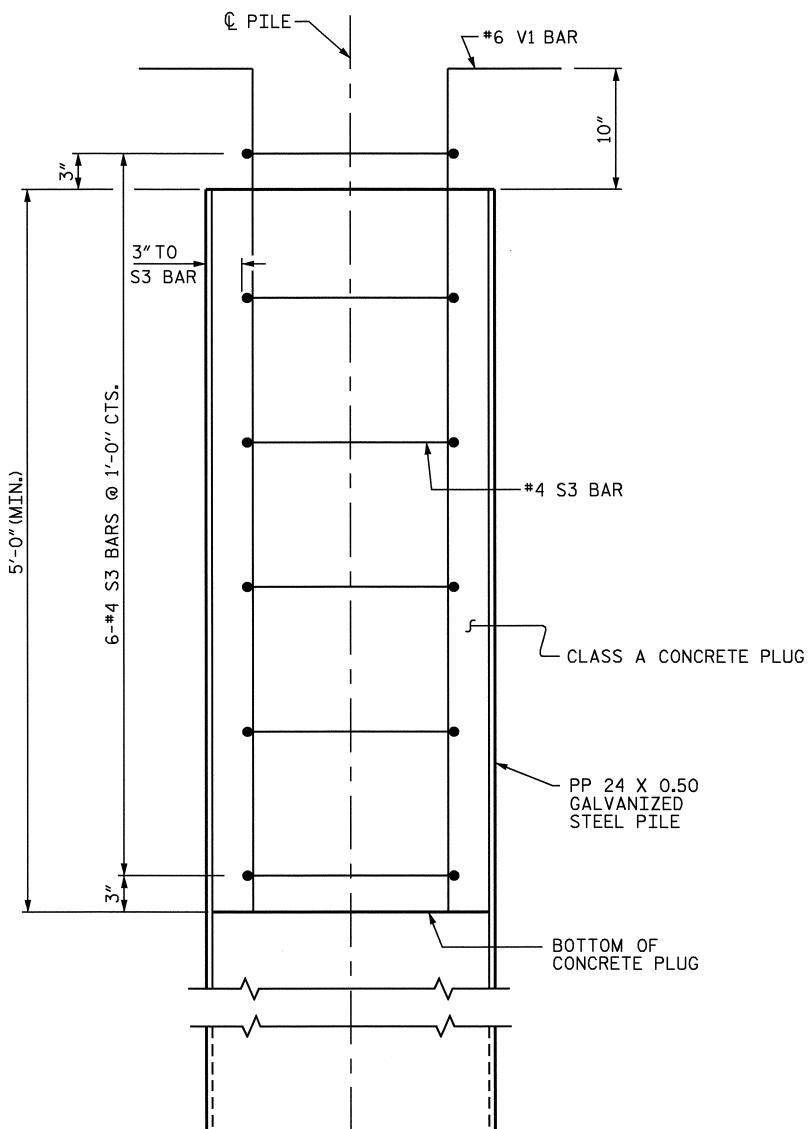
STATE OF NORTH CAROLINA DEPARTMENT OF TRANSPORTATION
RALEIGH

> SUBSTRUCTURE BENTS 1 THRU 8

	REVISIONS						
NO.	BY:	DATE:	NO.	BY:	DATE:	S-16	
1			3		•	TOTAL SHEETS	
2			4			22	

13-MAR-2008 14:23
W:\Structures\FINAL PLANS\b4020_sd_01_bt.dgn
DANDERSON





ELEVATION

PP 24 X 0.50 GALVANIZED STEEL PILE

(OPEN END)

AR DATE: 4-10-07

ASSEMBLED BY : E.C. LOCKLEAR DATE : 4-10-07 CHECKED BY : N.Q. TRAN DATE : 5-23-07

DRAWN BY : TLA 8/05 CHECKED BY : GM 9/05

ADDED IO/I/05 REV. 5/I/06R MAA/KMM

NOTES

PIPE PILES SHALL BE IN ACCORDANCE WITH SECTION 1084 OF THE STANDARD SPECIFICATIONS.

GALVANIZE STEEL PIPE PILES IN ACCORDANCE WITH SECTION 1076 OF THE STANDARD SPECIFICATIONS.

REMOVE AND REPLACE OR REPAIR TO THE SATISFACTION OF THE ENGINEER PILES THAT ARE DAMAGED, DEFORMED OR COLLAPSED DURING INSTALLATION OR DRIVING.

PILE SPLICES SHALL BE IN ACCORDANCE WITH THE STANDARD SPECIFICATIONS AND AWS D1.1.

FOR OPEN END PIPE PILES, REMOVE ENOUGH SOIL AND WATER FROM INSIDE THE PILES TO CONSTRUCT THE CONCRETE PLUG WITHOUT FOULING THE CONCRETE.

FORM THE CONCRETE PLUG SUCH THAT THE REINFORCING STEEL OR CONCRETE DOES NOT MOVE AND THE CLEARANCE FROM THE REINFORCING STEEL TO THE INSIDE OF THE PILE IS MAINTAINED AFTER CONCRETE PLACEMENT. DO NOT PLACE CONCRETE IN THE BENT CAP UNTIL THE CONCRETE PLUG HAS ATTAINED A MINIMUM COMPRESSIVE STRENGTH OF 1500 PSI.

THE REINFORCING STEEL, CLASS A CONCRETE, AND GALVANIZING ARE CONSIDERED INCIDENTAL TO THE CONTRACT UNIT PRICE BID PER LINEAR FOOT FOR PP 24 X 0.50 GALVANIZED STEEL PILES.

BILL OF MATERIAL FOR ONE PP 24 X 0.50 STEEL PILE								
BAR	NO.	SIZE	TYPE	LENGTH	WEIGHT			
S3	6	#4	1	6′-0′′	24			
V1	10	#6	2	6′-8′′	100			

CLASS A CONCRETE

5'-0" MINIMUM PLUG

REINFORCING STEEL =

0.5 CY

lbs

124

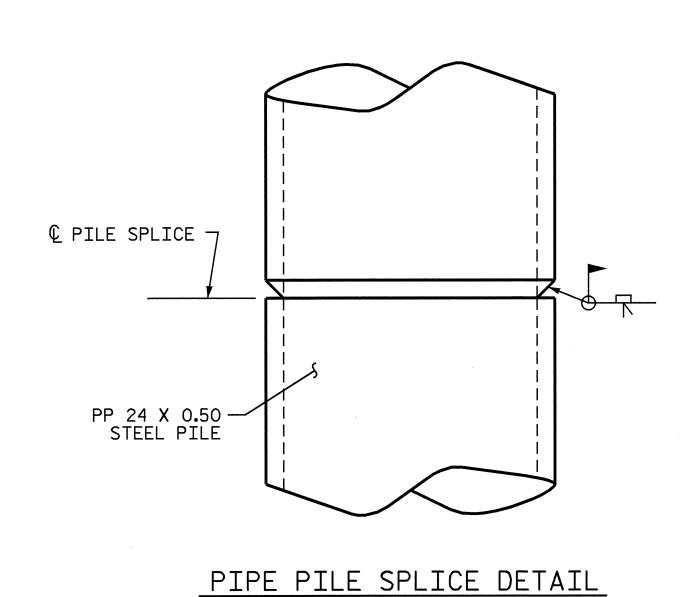
BAR TYPES

1'-3" LAP

2

1'-6"

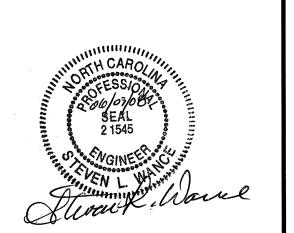
ALL BAR DIMENSIONS ARE OUT TO OUT.



PROJECT NO. B-4020

BEAUFORT / PITT COUNTY

STATION: 22+00.00 -L-



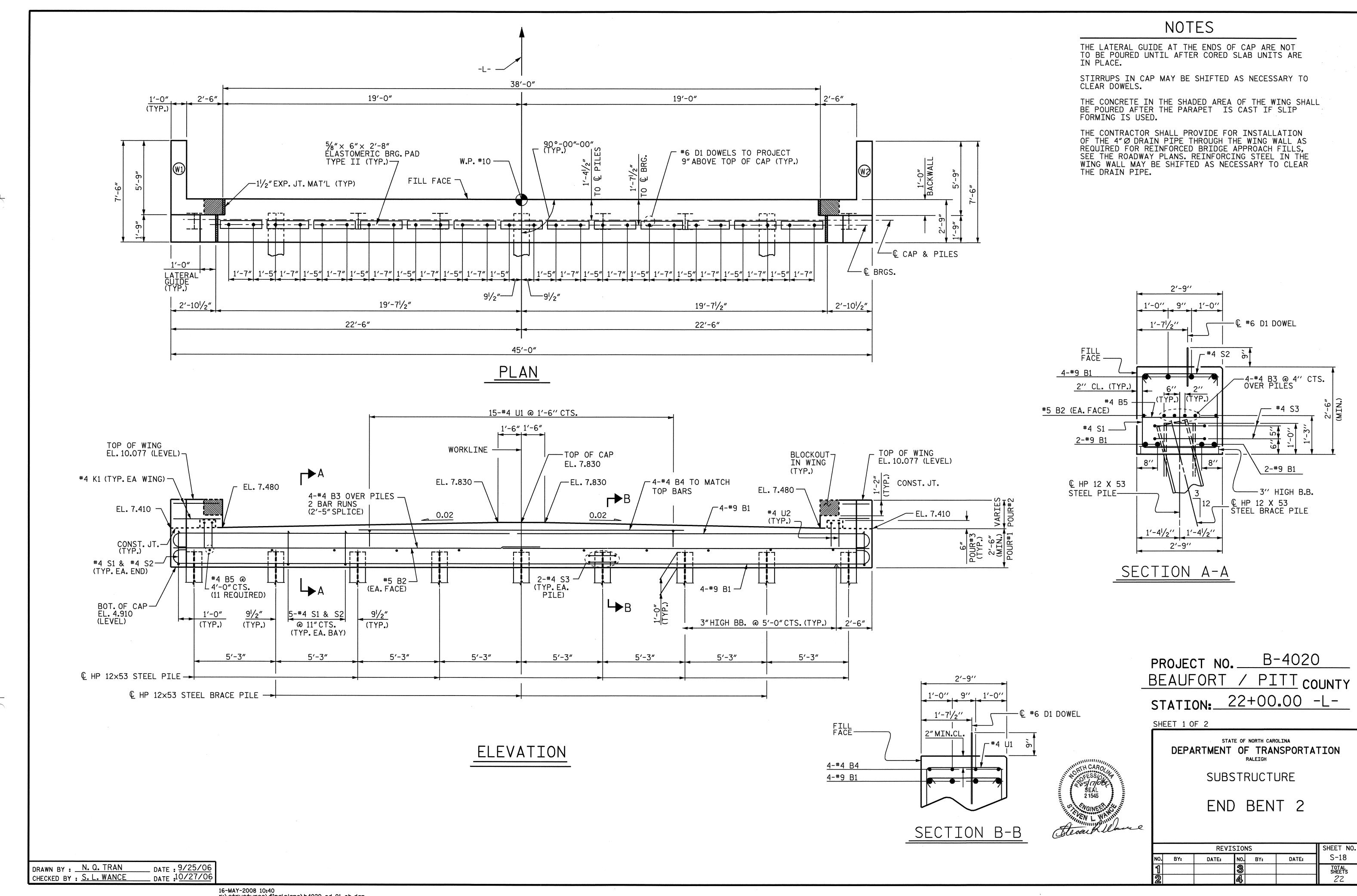
STATE OF NORTH CAROLINA

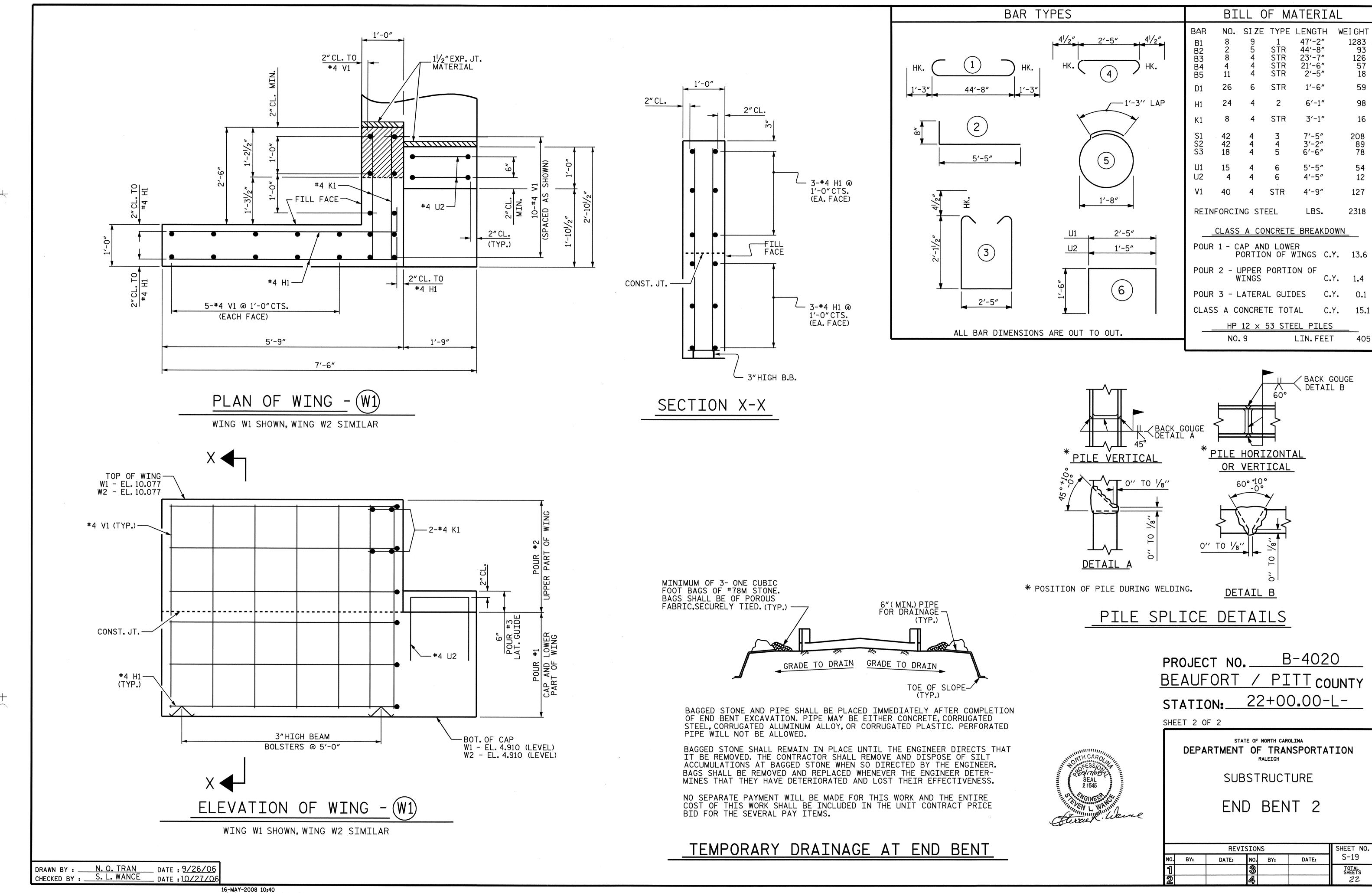
DEPARTMENT OF TRANSPORTATION
RALEIGH

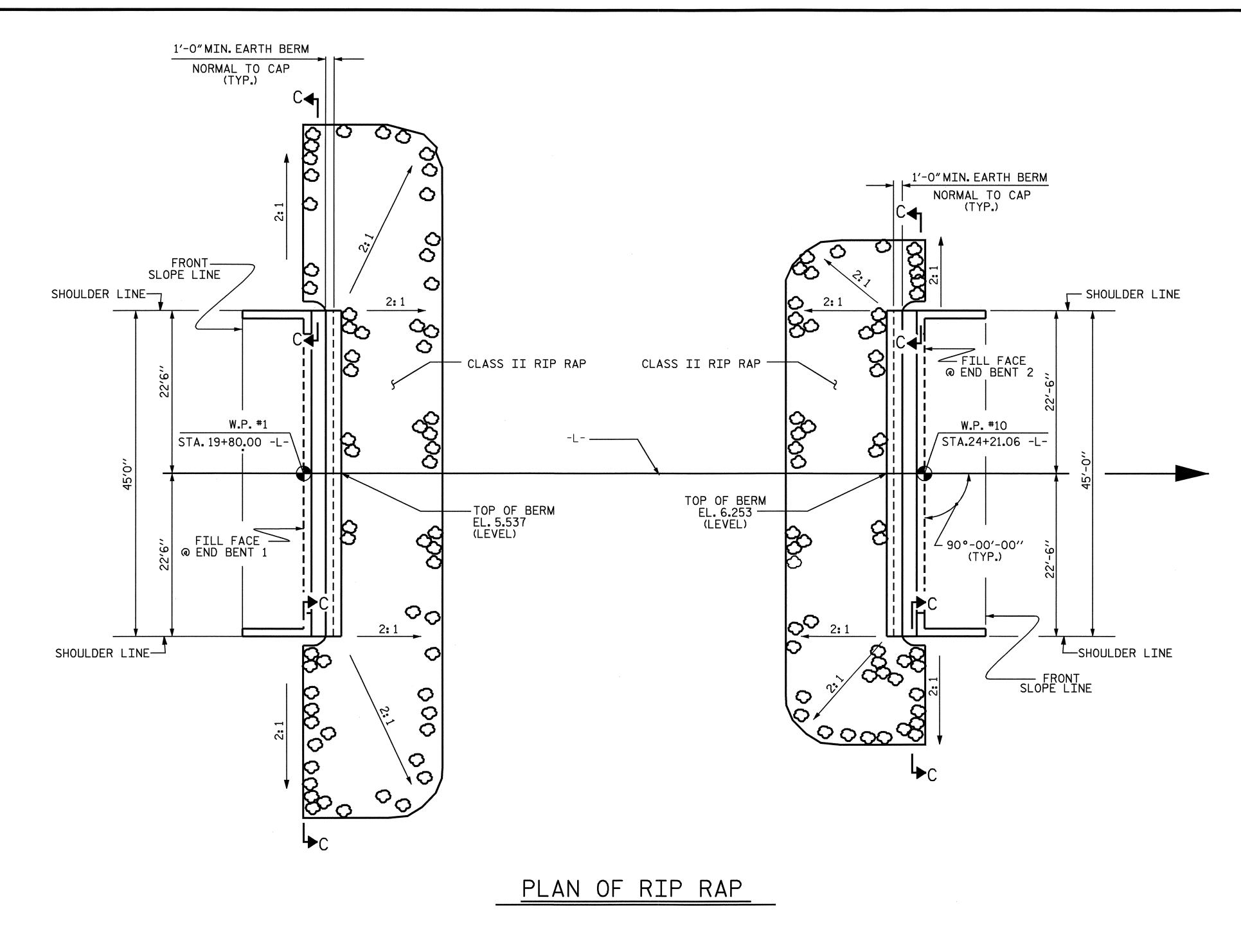
STANDARD

24" STEEL PIPE PILE

	SHEET NO.					
BY:	BY: DATE: NO. BY: DATE:					
		3			TOTAL SHEETS	
		4			22	

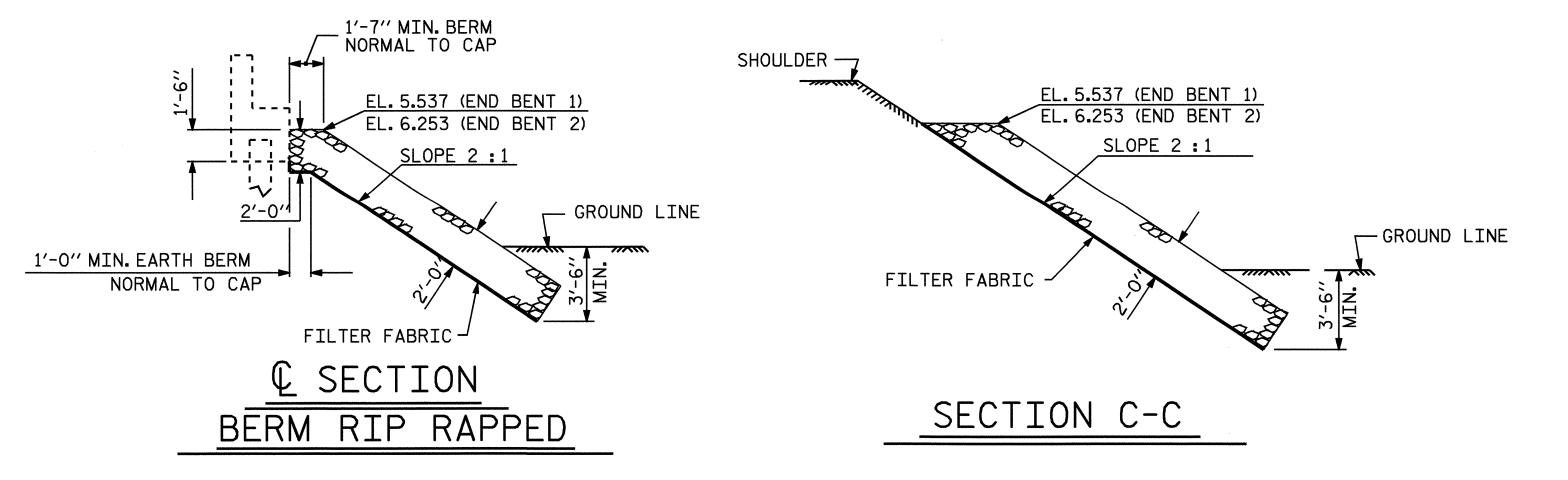






NOTES: FILTER FABRIC SHALL BE PLACED UNDER ENTIRE AREA OF RIP RAP.

ESTIMATED QUANTITIES							
BRIDGE @ STA. 22+00.00 -L-	RIP RAP CLASS II	FILTER FABRIC FOR DRAINAGE					
·	TONS	SQUARE YARDS					
END BENT 1	230	256					
END BENT 2	161	201					



PROJECT NO. B-4020 BEAUFORT / PITT COUNTY STATION: 22+00.00 -L-



STATE OF NORTH CAROLINA DEPARTMENT OF TRANSPORTATION RALEIGH

STANDARD

---RIP RAP DETAILS-

		REV	ISION	S		SHEET NO.
NO.	BY:	DATE:	NO.	BY:	DATE:	S-20
1			3			TOTAL SHEETS
2			4			22
SKE	w 90°	NO. RR2				

13-MAR-2008 14:26 W:\Structures\FINAL PLANS\b4020_sd_01_rr.dgn DANDERSON

DATE:2/15/06 DATE:6/1/06

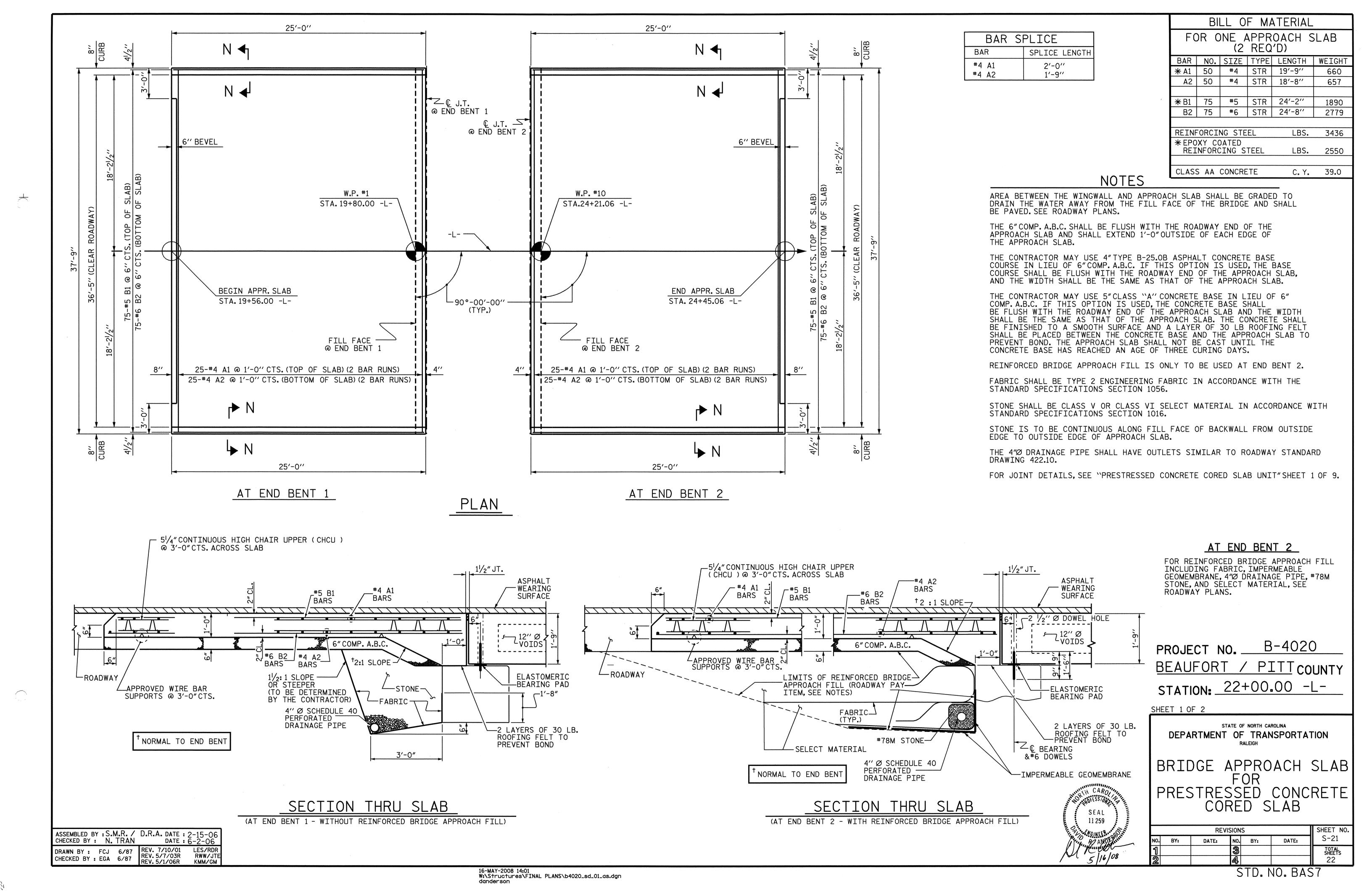
REV. 7/17/98 REV. 8/16/99 REV. 10/17/00

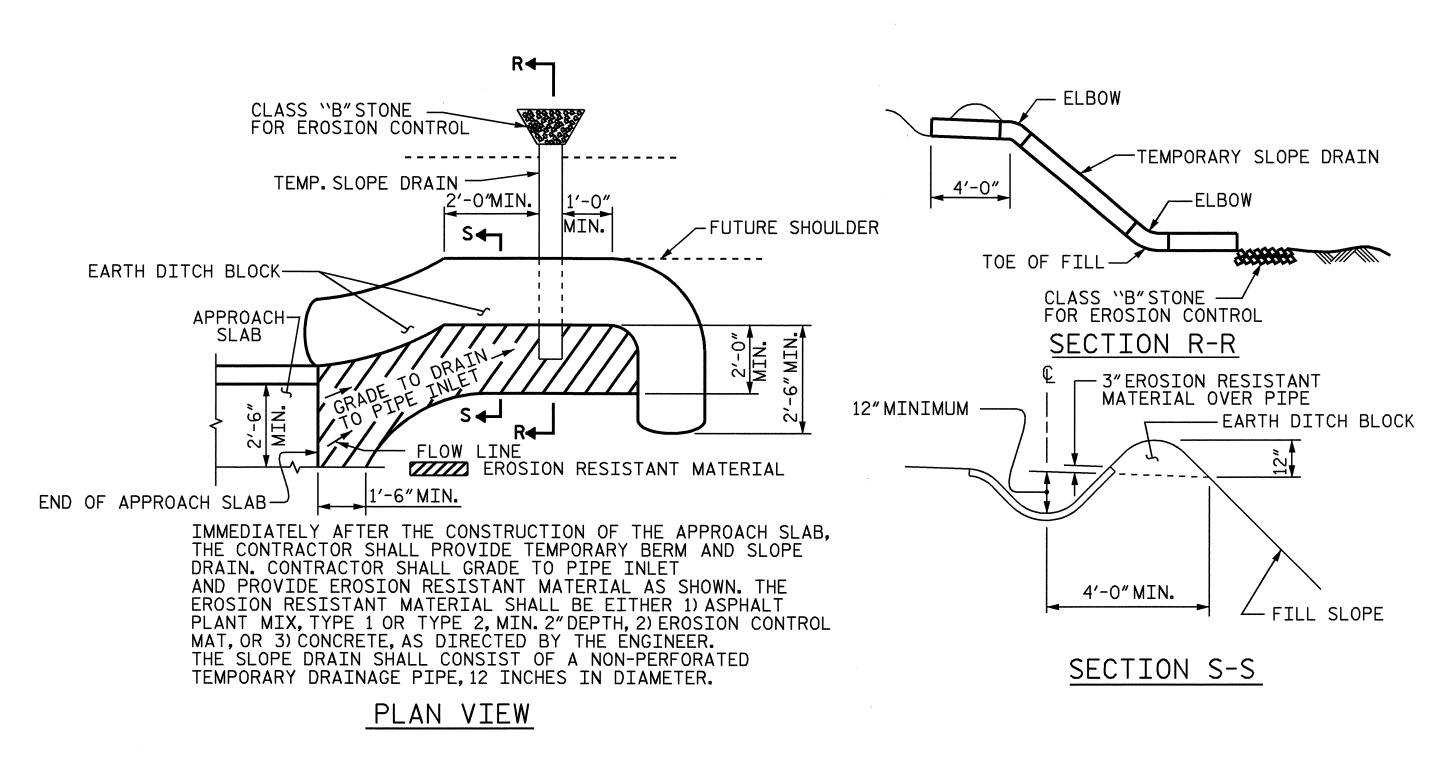
REK/RWW RWW/LES RWW/LES

ASSEMBLED BY : S. M. RASHIDI CHECKED BY :N.Q.TRAN

DRAWN BY: FCJ 2/88 CHECKED BY: ARB 8/88

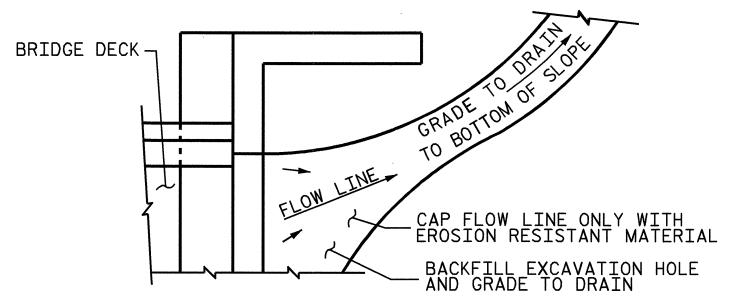
STD. NO. RR2





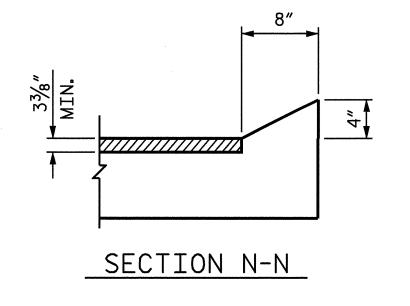
TEMPORARY BERM AND SLOPE DRAIN DETAILS

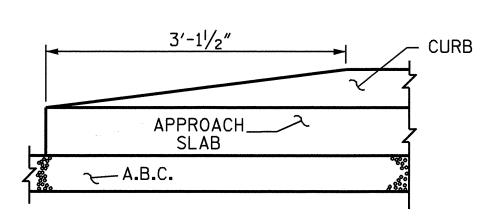
(TO BE USED WHEN SHOULDER BERM GUTTER IS REQUIRED)



NOTE: IF THE APPROACH SLAB IS NOT CONSTRUCTED IMMEDIATELY AFTER THE BACKFILLING OF THE END BENT EXCAVATION, GRADE TO DRAIN TO THE BOTTOM OF THE SLOPE AND PROVIDE EROSION RESISTANT MATERIAL, SUCH AS FIBERGLASS ROVING OR AS DIRECTED BY THE ENGINEER TO PREVENT SOIL EROSION AND TO PROTECT THE AREA ADJACENT TO THE STRUCTURE. THE CONTRACTOR WILL BE REQUIRED TO REMOVE THESE MATERIALS PRIOR TO CONSTRUCTION OF THE APPROACH SLAB.

TEMPORARY DRAINAGE DETAIL



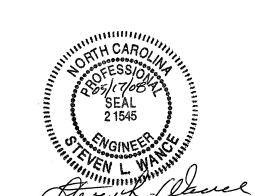


END OF CURB WITHOUT SHOULDER BERM GUTTER

CURB DETAILS

PROJECT NO. B-4020
BEAUFORT / PITT COUNTY
STATTON: 22+00.00 -L-

SHEET 2 OF 2



STATE OF NORTH CAROLINA

DEPARTMENT OF TRANSPORTATION
RALEIGH

STANDARD

BRIDGE APPROACH SLAB DETAILS

	SHEET NO.				
BY:	DATE:	NO.	BY:	DATE:	S-22
		3			TOTAL SHEETS
		4			22

ASSEMBLED BY: S. M. RASHIDI DATE:2/15/06 CHECKED BY: N. Q. TRAN DATE:6/2/06

DRAWN BY: FCJ II/88 REV.8/16/99 REV.10/17/00 RWW/LES REV.5/7/03 RWW/JTE

14-MAY-2008 10:44
W:\Structures\FINAL PLANS\b4020_sd_01_as.dgn

STD. NO. BAS10

STANDARD NOTES

DESIGN DATA:

---- A.A.S.H.T.O. (CURRENT) SPECIFICATIONS LIVE LOAD SEE A.A.S.H.T.O. IMPACT ALLOWANCE STRESS IN EXTREME FIBER OF 20,000 LBS. PER SQ. IN. STRUCTURAL STEEL - AASHTO M270 GRADE 36 - AASHTO M270 GRADE 50W - 27,000 LBS. PER SQ. IN. - AASHTO M270 GRADE 50 - 27,000 LBS. PER SQ. IN. REINFORCING STEEL IN TENSION GRADE 60 - - 24,000 LBS. PER SQ. IN. ---- 1.200 LBS. PER SQ. IN. CONCRETE IN COMPRESSION ---- SEE A.A.S.H.T.O. CONCRETE IN SHEAR STRUCTURAL TIMBER - TREATED OR ---- 1.800 LBS. PER SQ. IN. UNTREATED - EXTREME FIBER STRESS

MATERIAL AND WORKMANSHIP:

COMPRESSION PERPENDICULAR TO GRAIN

EQUIVALENT FLUID PRESSURE OF EARTH

EXCEPT AS MAY OTHERWISE BE SPECIFIED ON PLANS OR IN THE SPECIAL PROVISIONS, ALL MATERIAL AND WORKMANSHIP SHALL BE IN ACCORDANCE WITH THE 2002 STANDARD SPECIFICATIONS "FOR ROADS AND STRUCTURES" OF THE N. C. DEPARTMENT OF TRANSPORTATION.

OF TIMBER

_ _ _ _

375 LBS. PER SQ. IN.

30 LBS. PER CU. FT.

(MINIMUM)

STEEL SHEET PILING FOR PERMANENT OR TEMPORARY APPLICATIONS SHALL BE HOT ROLLED.

CONCRETE:

UNLESS OTHERWISE REQUIRED ON PLANS, CLASS A CONCRETE SHALL BE USED FOR ALL PORTIONS OF ALL STRUCTURES WITH THE EXCEPTION THAT: CLASS AA CONCRETE SHALL BE USED IN BRIDGE SUPERSTRUCTURES, ABUTMENT BACKWALLS, AND APPROACH SLABS; CLASS B CONCRETE SHALL BE USED FOR SLOPE PROTECTION AND RIP RAP; AND CLASS S SHALL BE USED FOR UNDERWATER FOOTING SEALS.

CONCRETE CHAMFERS:

UNLESS OTHERWISE NOTED ON THE PLANS, ALL EXPOSED CORNERS ON STRUCTURES SHALL BE CHAMFERED 3/4" WITH THE FOLLOWING EXCEPTIONS: TOP CORNERS OF CURBS MAY BE ROUNDED TO 1-1/2" RADIUS WHICH IS BUILT INTO CURB FORMS; CORNERS OF TRANSVERSE FLOOR EXPANSION JOINTS SHALL BE ROUNDED WITH A 1/4"FINISHING TOOL UNLESS OTHERWISE REQUIRED ON PLANS; AND CORNERS OF EXPANSION JOINTS IN THE ROADWAY FACES AND TOPS OF CURBS AND SIDEWALKS SHALL BE ROUNDED TO A 1/4" RADIUS WITH A FINISHING STONE OR TOOL UNLESS OTHERWISE REQUIRED ON PLANS.

DOWELS:

DOWELS WHEN INDICATED ON PLANS AS FOR CULVERT EXTENSIONS, SHALL BE EMBEDDED AT LEAST 12" INTO THE OLD CONCRETE AND GROUTED INTO PLACE WITH 1:2 CEMENT MORTAR.

ALLOWANCE FOR DEAD LOAD DEFLECTION, SETTLEMENT, ETC. IN CASTING SUPERSTRUCTURES:

BRIDGES SHALL BE BUILT ON THE GRADE OR VERTICAL CURVE SHOWN ON PLANS. SLABS, CURBS AND PARAPETS SHALL CONFORM TO THE GRADE OR CURVE.

ALL DIMENSIONS WHICH ARE GIVEN IN SECTION AND ARE AFFECTED BY DEAD LOAD DEFLECTIONS ARE DIMENSIONS AT CENTER LINE OF BEARING UNLESS OTHERWISE NOTED ON PLANS. IN SETTING FORMS FOR STEEL BEAM BRIDGES AND PRESTRESSED CONCRETE GIRDER BRIDGES, ADJUSTMENTS SHALL BE MADE DUE TO THE DEAD LOAD DEFLECTIONS FOR THE ELEVATIONS SHOWN. WHERE BLOCKS ARE SHOWN OVER BEAMS FOR BUILDING UP TO THE SLAB, THE VERTICAL DIMENSIONS OF THE BLOCKS SHALL BE ADJUSTED BETWEEN BEARINGS TO COMPENSATE FOR DEAD LOAD DEFLECTIONS, VERTICAL CURVE ORDINATE, AND ACTUAL BEAM CAMBER. WHERE BOTTOM OF SLAB IS IN LINE WITH BOTTOM OF TOP FLANGES, DEPTH OF SLAB BETWEEN BEARINGS SHALL BE ADJUSTED TO COMPENSATE FOR DEAD LOAD DEFLECTION, VERTICAL CURVE ORDINATE, AND ACTUAL BEAM CAMBER.

IN SETTING FALSEWORK AND FORMS FOR REINFORCED CONCRETE SPANS, AN ALLOWANCE SHALL BE MADE FOR DEAD LOAD DEFLECTIONS, SETTLEMENT OF FALSEWORK, AND PERMANENT CAMBER WHICH SHALL BE PROVIDED FOR IN ADDITION TO THE ELEVATIONS SHOWN. AFTER REMOVAL OF THE FALSEWORK, THE FINISHED STRUCTURES SHALL CONFORM TO THE PROFILE AND ELEVATIONS SHOWN ON THE PLANS AND CONSTRUCTION ELEVATIONS FURNISHED BY THE ENGINEER.

DETAILED DRAWINGS FOR FALSEWORK OR FORMS FOR BRIDGE SUPERSTRUCTURE AND ANY STRUCTURE OR PARTS OF A STRUCTURE AS NOTED ON THE PLANS SHALL BE SUBMITTED TO THE ENGINEER FOR APPROVAL BEFORE CONSTRUCTION OF THE FALSEWORK OR FORMS IS STARTED.

REINFORCING STEEL:

ALL REINFORCING STEEL SHALL BE DEFORMED WITH THE EXCEPTION OF #2
BARS WHICH MAY BE FABRICATED FROM COLD DRAWN STEEL WIRE. DIMENSIONS
RELATIVE TO PLACEMENT OF REINFORCING ARE TO CENTERS OF BARS UNLESS OTHERWISE
INDICATED IN THE PLANS. DIMENSIONS ON BAR DETAILS ARE TO CENTERS OF BARS
OR ARE OUT TO OUT AS INDICATED ON PLANS.

WIRE BAR SUPPORTS SHALL BE PROVIDED FOR REINFORCING STEEL WHERE INDICATED ON THE PLANS. WHEN BAR SUPPORT PIECES ARE PLACED IN CONTINUOUS LINES, THEY SHALL BE SO PLACED THAT THE ENDS OF THE SUPPORTING WIRES SHALL BE LAPPED TO LOCK LEGS ON ADJOINING PIECES.

STRUCTURAL STEEL:

AT THE CONTRACTOR'S OPTION, HE MAY SUBSTITUTE 7/8" Ø SHEAR STUDS FOR THE 3/4" Ø STUDS SPECIFIED ON THE PLANS. THIS SUBSTITUTION SHALL BE MADE AT THE RATE OF 3 - 7/8" Ø STUDS FOR 4 - 3/4" Ø STUDS, AND STUD SPACING CHANGES SHALL BE MADE AS NECESSARY TO PROVIDE THE SAME EQUIVALENT NUMBER OF 7/8" Ø STUDS ALONG THE BEAM AS SHOWN FOR 3/4" Ø STUDS BASED ON THE RATIO OF 3 - 7/8" Ø STUDS FOR 4 - 3/4" Ø STUDS. STUDS OF THE LENGTH SPECIFIED ON THE PLANS MUST BE PROVIDED. THE MAXIMUM SPACING SHALL BE 2'-0".

EXCEPT AT THE INTERIOR SUPPORTS OF CONTINUOUS BEAMS WHERE THE COVER PLATE IS IN CONTACT WITH BEARING PLATE, THE CONTRACTOR MAY, AT HIS OPTION, SUBSTITUTE FOR THE COVER PLATES DESIGNATED ON THE PLANS COVER PLATES OF THE EQUIVALENT AREA PROVIDED THESE PLATES ARE AT LEAST 5/16"IN THICKNESS AND DO NOT EXCEED A WIDTH EQUAL TO THE FLANGE WIDTH LESS 2"OR A THICKNESS EQUAL TO 2 TIMES THE FLANGE THICKNESS. THE SIZE OF FILLET WELDS SHALL CONFORM TO THE REQUIREMENTS OF THE CURRENT ANSI/AASHTO/AWS "BRIDGE WELDING CODE". ELECTROSLAG WELDING WILL NOT BE PERMITTED.

PLACEMENT OF BEAM OR GIRDER MEMBERS ON TRUCKS FOR HAULING SHALL BE DONE IN COMPLIANCE WITH LIMITS SHOWN ON SKETCHES PROVIDED TO THE MATERIALS AND TEST UNIT APPROVED BY THE STRUCTURE DESIGN UNIT DATED MAY 8,1991. THESE SKETCHES PRIMARILY LIMIT THE UNSUPPORTED CANTILEVER LENGTH OF MEMBERS. WHEN THE CONTRACTOR WISHES TO PLACE MEMBERS ON TRUCKS NOT IN ACCORDANCE WITH THESE LIMITS, TO SHIP BY RAIL, TO ATTACH SHIPPING RESTRAINTS TO THE MEMBERS OR TO INVERT MEMBERS, HE SHALL SUBMIT A SKETCH FOR APPROVAL PRIOR TO SHIPPING. SEE ALSO ARTICLE 1072-11.

WITH THE SOLE EXCEPTION OF EDGES AT SURFACES WHICH BEAR ON OTHER SURFACES, ALL SHARP EDGES AND ENDS OF SHAPES AND PLATES SHALL BE SLIGHTLY ROUNDED BY SUITABLE MEANS TO A RADIUS OF APPROXIMATELY 1/16 INCH OR EQUIVALENT FLAT SURFACE AT A SUITABLE ANGLE PRIOR TO PAINTING, GALVANIZING, OR METALLIZING.

HANDRAILS AND POSTS:

METAL STANDARDS AND FACES OF THE CONCRETE END POSTS FOR THE METAL RAIL SHALL BE SET NORMAL TO THE GRADE OF THE CURB, UNLESS OTHERWISE SHOWN ON PLANS. THE METAL RAIL AND TOPS OF CONCRETE POSTS USED WITH THE ALUMINUM RAIL SHALL BE BUILT PARALLEL TO THE GRADE OF THE CURB.

METAL HANDRAILS SHALL BE IN ACCORDANCE WITH THE PLANS. RAILS SHALL BE AS MANUFACTURED FOR BRIDGE RAILING. CASTINGS SHALL BE OF A UNIFORM APPEARANCE. FINS AND OTHER DEFORMATIONS RESULTING FROM CASTING OR OTHERWISE SHALL BE REMOVED IN A MANNER SO THAT A UNIFORM COLORING OF THE COMPLETED CASTING SHALL BE OBTAINED. CASTINGS WITH DISCOLORATIONS OR OF NON-UNIFORM COLORING WILL NOT BE ACCEPTED. CERTIFIED MILL REPORTS ARE REQUIRED FOR METAL RAILS AND POSTS.

SPECIAL NOTES:

GENERALLY, IN CASE OF DISCREPANCY, THIS STANDARD SHEET OF NOTES SHALL GOVERN OVER THE SPECIFICATIONS, BUT THE REMAINDER OF THE PLANS SHALL GOVERN OVER NOTES HEREON, AND SPECIAL PROVISIONS SHALL GOVERN OVER ALL. SEE SPECIFICATIONS ARTICLE 105-4.

ENGLISH