B-366I

CONTENTS: II+50 - I9+85 -L-

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DRAWN BY: ___

J.W. MANN

STATE OF NORTH CAROLINA

DEPARTMENT OF TRANSPORTATION

DIVISION OF HIGHWAYS

GEOTECHNICAL UNIT

SUBSURFACE INVESTIGATION

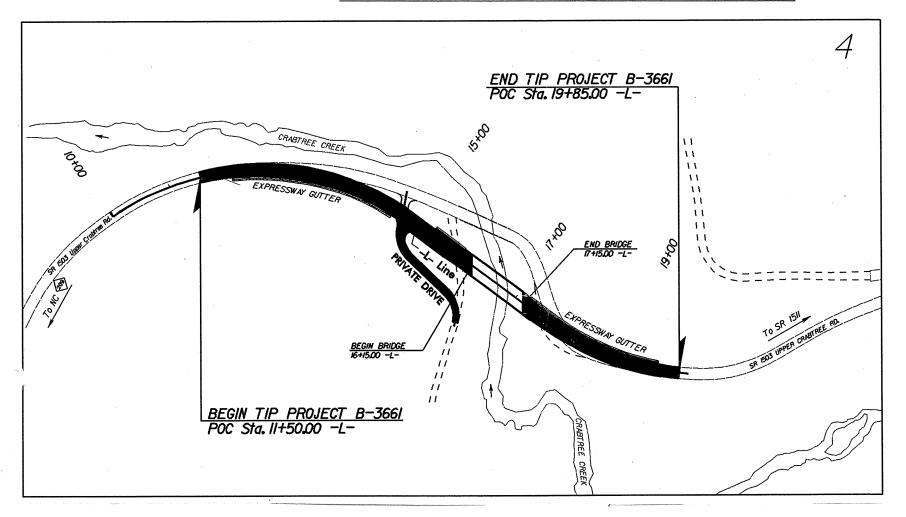
STATE PROJECT 8.2942001 I.D. NO. B-3661

F.A. PROJECT BRZ-I503(4)

COUNTY HAYWOOD

DESCRIPTION APPROACHES TO BRIDGE NO. 36

ON SR-I503 OVER CRABTREE CR.



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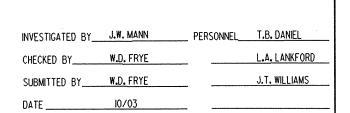
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N.C.	B-	1	16		
STATE PROLI	90.	P. A. PROL NO.	DESCRIPT	TON	1
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33206.2	.1	BRZ-1503 (4)	RW &	UTIL	
33206.3	.1	BRZ-1503 (4)	CONS	TR	

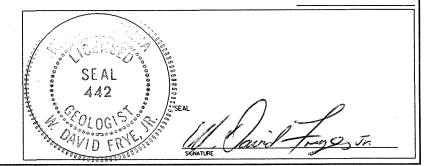
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THE SUBSURFACE INFORMATION AND THE SUBSURFACE INVESTIGATION ON WHICH IT IS BASED WAS MADE FOR THE PURPOSE OF STUDY, PLANNING AND DESIGN, AND NOT FOR CONSTRUCTION OR PAY PURPOSES. THE VARIOUS FIELD BORING LOGS, ROCK CORES, AND SOIL TEST DATA AVAILABLE MAY BE REVIEWED OR INSPECTED IN RALEIGH BY CONTACTING THE N.C. DEPARTMENT OF TRANSPORTATION, GEOTECHNICAL UNIT @ 1919 250-4088. NEITHER THE SUBSURFACE PLANS AND REPORTS, NOR THE FIELD BORING LOGS, ROCK CORES, OR SOIL TEST DATA IS PART OF THE CONTRACT.

GENERAL SOIL AND ROCK STRATA DESCRIPTIONS AND INDICATED BOUNDARIES ARE BASED ON A GEOTECHNICAL INTERPRETATION OF ALL AVALABLE SUBSURFACE DATA AND MAY ONT INCESSARILY REFLECT THE ACTUAL SUBSURFACE CONDITIONS BETWEEN BORINGS OR BETWEEN SAMPLED STRATA WITHIN THE BOREHOLE. THE LABORATORY SAMPLE DATA AND THE IN SITU (IN-PLACE) TEST DATA CAN BE RELIED ON ONLY TO THE DEGREE OF RELIABILITY INHERENT IN THE STANDARD TEST METHOD. THE OBSERVED WATER LEVELS OR SOIL MOISTURE CONDITIONS INDICATED IN THE SUBSURFACE INVESTIGATIONS ARE AS RECORDED AT THE TIME OF THE INVESTIGATION. THESE WATER LEVELS OR SOIL MOISTURE CONDITIONS WAY VARY CONSIDERABLY WITH TIME ACCORDING TO CLIMATIC CONDITIONS INCLUDING TEMPERATURES, PRECIPITATION AND WIND, AS WELL AS OTHER NON-CLIMATIC FACTORS.

THE BIDDER OR CONTRACTOR IS CAUTIONED THAT DETAILS SHOWN ON THE SUBSURFACE PLANS ARE PRELIMINARY ONLY AND IN MANY CASES THE FINAL DESIGN DETAILS ARE DIFFERENT. FOR BIDDING AND CONSTRUCTION PURPOSES, REFER TO THE CONSTRUCTION PLANS AND DOCUMENTS FOR FINAL DESIGN INFORMATION ON THIS PROJECT. THE DEPARTMENT DOES NOT WARRANT OR GUARANTEE THE SUFFICIENCY OR ACCURACY OF THE INVESTIGATION MADE, NOR THE INTERPRETATIONS MADE OR OPINION OF THE DEPARTMENT AS TO THE TYPE OF MATERIALS AND CONDITIONS TO BE ENCOUNTERED. THE BIDDER OR CONTRACTOR IS CAUTIONED TO MAKE SUCH INDEPENDENT SUBSURFACE INVESTIGATIONS AS HE DEEMS NECESSARY TO SATISFY HIMSELF AS TO CONDITIONS TO BE ENCOUNTERED ON THIS PROJECT. THE CONTRACTOR SHALL HAVE NO CLAIM FOR ADDITIONAL COMPENSATION OR FOR AN EXTENSION OF TIME FOR ANY REASON RESULTING FROM THE ACTUAL CONDITIONS ENCOUNTERED AT THE SITE DIFFERING FROM THOSE MOLICATED IN THE SUBSURFACE INFORMATION.





NORTH CAROLINA DEPARTMENT OF TRANSPORTATION

DIVISION OF HIGHWAYS

GEOTECHNICAL UNIT

SUBSURFACE INVESTIGATION

						SOIL AND ROO	CK LEGEND, TERMS	S, SYMBOLS,	AND ABBR	EVIATIONS			
		SOIL DESC	CRIPTION			GRADATION				OCK DESCRIPTION		TERMS AND DEFINITIONS	
SOIL IS CON	NSIDERED TO BE THE UNCON	NSOLIDATED, SEMI-C	CONSOLIDATED OR WEATH	HERED EARTH MATERIALS	WELL GRADED- INDICATES A GOI UNIFORM- INDICATES THAT SOIL	OD REPRESENTATION OF PARTICLE SIZES FR PARTICLES ARE ALL APPROXIMATELY THE S	OM FINE TO COARSE SAME SIZE.(ALSO			IAL THAT WHEN TESTED, WOULD YIEL! H NON-COASTAL PLAIN MATERIAL WO		ALLUVIUM (ALLUV.) - SDILS WHICH HAVE BEEN TRANSPORTED BY WATER.	
WHICH CAN BE PENETRATED WITH A CONTINUOUS FLIGHT POWER AUGER, AND WHICH YIELDS LESS THAN 100 BLOWS PER FOOT ACCORDING TO STANDARD PENETRATION TEST (AASHTO 1206, ASTM D-1586), SOIL			POORLY GRADED)			SPT REFUSAL IS PE	NETRATION BY A SPLIT	SPOON SAMPLER EQUAL TO OR LESS	S THAN 0.1 FOOT PER 60 BLOWS.				
CLASSIFICATION IS BASED ON THE AASHTO SYSTEM AND BASIC DESCRIPTIONS GENERALLY SHALL INCLUDE:			GAP-GRADED- INDICATES A MIXTURE OF UNIFORM PARTICLES OF TWO OR MORE SIZES. ANGULARITY OF GRAINS			OF WEATHERED ROCK	ζ.	RANSITION BETWEEN SOIL AND ROCK	IS OFTEN REPRESENTED BY A ZONE	ARENACEOUS - APPLIED TO ROCKS THAT HAVE BEEN DERIVED FROM SAND OR THAT CONTAIN SAND.			
	Y, COLOR, TEXTURE, MOISTURE DGICAL COMPOSITION, ANGUL				THE ANGULARITY OR ROUNDNESS	S OF SOIL GRAINS ARE DESIGNATED BY THE	TERMS; ANGULAR,	ROCK MATERIALS AF	E TYPICALLY DIVIDED	AS FOLOWS:		ARGILLACEOUS - APPLIED TO ALL ROCKS OR SUBSTANCES COMPOSED OF CLAY MINERALS,	
			DED FINE SAND LAYERS, HIGHLY PL		SUBANGULAR, SUBROUNDED, OR F	ROUNDED.	* acceptance and a property	WEATHERED ROCK (WR)	NON-COA	ASTAL PLAIN MATERIAL THAT YIELDS	SPT N VALUES > 100 BLOWS	OR HAVING A NOTABLE PROPORTION OF CLAY IN THEIR COMPOSITION, AS SHALE, SLATE, ETC. ARTESIAN - GROUND WATER THAT IS UNDER SUFFICIENT PRESSURE TO RISE ABOVE THE LEVEL	
SOIL LEGEND AND AASHTO CLASSIFICATION			MINERALOGICAL COMPOSITIO	N		111100	COARSE GRAIN IGNEOUS AND METAM	INRPHIC BOCK THAT	AT WHICH IS IS ENCOUNTERED, BUT WHICH DOES NOT NECESSARILY RISE TO OR ABOVE THE				
GENERAL	GRANULAR MATER	IALS SI	ILT-CLAY MATERIALS	ORGANIC MATERIALS		Z, FELDSPAR, MICA, TALC, KAOLIN, ETC. ARE U	SED IN DESCRIPTIONS	CRYSTALLINE ROCK (CR)	would Y السر	YIELD SPT REFUSAL IF TESTED. ROCI		GROUND SURFACE.	
CLASS.	(≤5% PASSING *20		>85% PASSING *200)		WHENEVER THEY ARE CONSIDERE	WHENEVER THEY ARE CONSIDERED OF SIGNIFICANCE.				GABBRO, SCHIST, ETC. COARSE GRAIN METAMORPHIC AND N	ON-COASTAL PLAIN	CALCAREOUS (CALC.) - SOILS WHICH CONTAIN APPRECIABLE AMOUNTS OF CALCIUM CARBONATE.	
GROUP	A-1 A-3		9-4 A-5 A-6 A-7	A-1, A-2 A-4, A-5 A-3 A-6, A-7		COMPRESSIBILITY		NON-CRYSTALLINE ROCK (NCR)	SEDIMEN	TARY ROCK THAT WOULD YEILD SPT		COLLUVIUM - ROCK FRAGMENTS MIXED WITH SOIL DEPOSITED BY GRAVITY ON SLOPE OR AT BOTTOM OF SLOPE.	
	A-1-a A-1-b A-2-4 A-	-2-5 A-2-6 A-2-7	A-7-6	H-3 H 3, H /	SLIGHTLY COMPRESSI MODERATELY COMPRES		LESS THAN 30 31-50	COASTAL PLAIN		S PHYLLITE, SLATE, SANDSTONE, ETC. PLAIN SEDIMENTS CEMENTED INTO I	ROCK, BUT MAY NOT YIELD	CORE RECOVERY (REC.) - TOTAL LENGTH OF ALL MATERIAL RECOVERED IN THE CORE BARREL DIVIDED BY TOTAL	
SYMBOL	0000000000				HIGHLY COMPRESSIBL		GREATER THAN 50	SEDIMENTARY ROCK	SPT REF	USAL. ROCK TYPE INCLUDES LIMESTO	ONE, SANDSTONE, CEMENTED	LENGTH OF CORE RUN AND EXPRESSED AS A PERCENTAGE.	
% PASSING	EQ MY			GRANULAR SILT- MUCK,		PERCENTAGE OF MATERIAL			J J JILLE D	WEATHERING		DIKE - A TABULAR BODY OF IGNEOUS ROCK THAT CUTS ACROSS THE STRUCTURE OF ADJACENT	
	30 MX50 MX51 MN			GRANULAR CLAY PEAT SOILS	ORGANIC MATERIAL	GRANULAR SILT- CLAY SOILS SOILS	OTHER MATERIAL				ATAITAIC DOCK DIAICC LINDED	ROCKS OR CUTS MASSIVE ROCK.	
* 200	15 MX 25 MX 10 MX 35 MX 35	5 MX35 MX35 MX36	5 MN 36 MN 36 MN 36 MN	SULS		2 - 3% 3 - 5% TRA			IF CRYSTALLINE.	, FEW JUINIS MAT SHOW SCIONI STA	AINING. RUCK RINGS UNDER	DIP - THE ANGLE AT WHICH A STRATUM OR ANY PLANAR FEATURE IS INCLINED FROM THE HORIZONTAL.	
.19UID LIMIT	40 MX41	1 MN 40 MX41 MN 40	8 MX 41 MN 40 MX 41 MN	SOILS WITH	LITTLE ORGANIC MATTER MODERATELY ORGANIC	3 - 5% 5 - 12% LIT 5 - 10% 12 - 20% SOM				S STAINED, SOME JOINTS MAY SHOW		DIP DIRECTION (DIP AZIMUTH) - THE DIRECTION OR BEARING OF THE HORIZONTAL TRACE OF	
PLASTIC INDEX	6 MX N.P. 10 MX 10	3 MX 11 MN 11 MN 10	MX 10 MX 11 MN 11 MN	LITTLE OR HIGHLY	HIGHLY ORGANIC	>10% >20% HIGH			S ON A BROKEN SPECI RYSTALLINE NATURE.	MEN FACE SHINE BRIGHTLY, ROCK RI	NGS UNDER HAMMER BLOWS IF	THE LINE OF DIP, MEASURED CLOCKWISE FROM NORTH.	
GROUP INDEX	0 0 0	4 MX 8	MX 12 MX 16 MX No MX	MODERATE ORGANIC		GROUND WATER				S STAINED AND DISCOLORATION EXTE	ENDS INTO ROCK UP TO	FAULT - A FRACTURE OR FRACTURE ZONE ALONG WHICH THERE HAS BEEN DISPLACEMENT OF THE	
	STONE FRAGS. FINE SILTY	Y OR CLAYEY	SILTY CLAYEY	ORGANIC	✓ WATER LE	EVEL IN BORE HOLE IMMEDIATELY AFTER	DRILLING.	(SLI.) 1 INCH.	OPEN JOINTS MAY CON	TAIN CLAY. IN GRANITOID ROCKS SON	ME OCCASIONAL FELDSPAR	SIDES RELATIVE TO ONE ANOTHER PARALLEL TO THE FRACTURE.	
OF MAJOR MATERIALS		EL AND SAND	SOILS SOILS	MATTER	STATIC WA	ATER LEVEL AFTER 24 HOURS.				OLORED. CRYSTALLINE ROCKS RING L		FISSILE - A PROPERTY OF SPLITTING ALONG CLOSELY SPACED PARALLEL PLANES.	
GEN. RATING			l	FAIR TO DOOR		WATER SATURATED TONE OR MATER BEAD!	INC STRATA			K SHOW DISCOLORATION AND WEATHE PARS ARE DULL AND DISCOLORED, SO		FLOAT - ROCK FRAGMENTS ON SURFACE NEAR THEIR ORIGINAL POSITION AND DISLODGED FROM PARENT MATERIAL.	
AS A SUBGRADE	EXCELLENT TO G	00D	FAIR TO POOR	POOR POOR UNSUITABLE				DULL SOUND UNDER HAMMER BLOWS AND SHOWS SIGNIFICANT LOSS OF STRENGTH AS COMPARED WITH FRESH ROCK.				FLOOD PLAIN (F.P.) - LAND BORDERING A STREAM, BUILT OF SEDIMENTS DEPOSITED BY	
OODOIGIDE	P.I. OF A-7-5	5 ≤ L.L 30 : F	P.I. OF A-7-6 > L.L.	30	OM SPRING OR	SEEPAGE				COLORED OR STAINED, IN GRANITOID	BUCKS OF EET DEDVIDE UNIT	THE STREAM.	
	CON	ISISTENCY C	OR DENSENESS	······································		MISCELLANEOUS SYMBOLS		SEVERE AND DIS	COLORED AND A MAJOR	TITY SHOW KADLINIZATION. ROCK SHO	WS SEVERE LOSS OF STRENGTH	FORMATION (FM.) - A MAPPABLE GEOLOGIC UNIT THAT CAN BE RECOGNIZED AND TRACED IN	
DDIMADY	SOIL TYPE COMPACT		RANGE OF STANDARD ETRATION RESISTENCE	RANGE OF UNCONFINED COMPRESSIVE STRENGTH	ROADWAY EMBANKM	IENT SPT CPT DPT DMT TEST BORIN PTION ST PMT	IG SAMPLE		I BE EXCAVATED WITH ED, WOULD YIELD SPT I	A GEOLOGIST'S PICK. ROCK GIVES *C	LUNK'SOUND WHEN STRUCK.	THE FIELD.	
FRIMANT	CONSIS	STENCY FEIN	(N-VALUE)	(TONS/FT ²)	WITH SOIL DESCRI	PTION VST PMT	DESIGNATIONS	I		ISCOLORED OR STAINED. ROCK FABRIC	CLEAR AND EVIDENT BUT BEDUCE	JOINT - FRACTURE IN ROCK ALONG WHICH NO APPRECIABLE MOVEMENT HAS OCCURRED.	
GENER	RALLY VERY L		<4		SOIL SYMBOL	AUGER BORING	S- BULK SAMPLE	(SEV.) IN STRE	NGTH TO STRONG SOIL.	. IN GRANITOID ROCKS ALL FELDSPA		LEDGE - A SHELF-LIKE RIDGE OR PROJECTION OF ROCK WHOSE THICKNESS IS SMALL COMPARED TO ITS LATERAL EXTENT.	
GRANU	ILAR LUUS	SE 1 DENSE	4 TO 10 10 TO 30	N/A	ARTIFICIAL FILL O	OTHER THAN			. SOME FRAGMENTS OF TED.YIELDS SPT N VAL:	STRONG ROCK USUALLY REMAIN.		LENS - A BODY OF SOIL OR ROCK THAT THINS OUT IN ONE OR MORE DIRECTIONS.	
MATER (NON-	COHESIVE) DENS	SE	30 TO 50		ROADWAY EMBANKM		SS- SPLIT SPOON SAMPLE			COLORED OR STAINED, ROCK FABRIC	FLEMENTS ARE DISCERNIBLE BUT	MOTTLED (MOT.) - IRREGULARLY MARKED WITH SPOTS OF DIFFERENT COLORS. MOTTLING IN	
	VERY U		>50		INFERRED SOIL BO	DUNDARIES 3	ST- SHELBY TUBE	(V. SEV.) THE MA	SS IS EFFECTIVELY RE	DUCED TO SOIL STATUS, WITH ONLY I	FRAGMENTS OF STRONG ROCK	SOILS USUALLY INDICATES POOR AERATION AND LACK OF GOOD DRAINAGE.	
GENER	VERY S		<2 2 TO 4	<0.25 0.25 TO 0.5		` MONITORING WEL				EXAMPLE OF ROCK WEATHERED TO A OCK FABRIC REMAIN. <i>IF TESTED, YIE</i>		PERCHED WATER - WATER MAINTAINED ABOVE THE NORMAL GROUND WATER LEVEL BY THE PRESENCE OF AN INTERVENING IMPERVIOUS STRATUM.	
SILT-	CLAY MEDIUM	1 STIFF	4 TO 8	0.5 TO 1	到底河 INFERRED ROCK LI	∧ PIEZUMETER	RS- ROCK SAMPLE	i		FABRIC NOT DISCERNIBLE, OR DISCER		RESIDUAL SOIL - SDIL FORMED IN PLACE BY THE WEATHERING OF ROCK.	
MATER (COHE			8 TO 15 15 TO 30	1 TO 2 2 TO 4	TTTTT ALLUVIAL SOIL BO	SUNDARY INSTALLATION SLOPE INDICATO	RT- RECOMPACTED R TRIAXIAL SAMPLE	SCATTER	RED CONCENTRATIONS. C	DUARTZ MAY BE PRESENT AS DIKES		ROCK QUALITY DESIGNATION (R.O.D.) - A MEASURE OF ROCK QUALITY DESCRIBED BY: TOTAL LENGTH OF	
100112	HARD		>30	>4	25/025 DIP/DIP DIRECTION	OF SINSTALLATION	CBR - CBR SAMPLE	ALSU AF	I EXAMPLE.	DOCK HAPPNECC		ROCK SEGMENTS EQUAL TO OR GREATER THAN 4 INCHES DIVIDED BY THE TOTAL LENGTH OF CORE RUN AND EXPRESSED AS A PERCENTAGE.	
	T	EXTURE OR	GRAIN SIZE		ROCK STRUCTURES	SPT N-VALUE				ROCK HARDNESS		SAPROLITE (SAP.) - RESIDUAL SOIL WHICH RETAINS THE RELIC STRUCTURE OR FABRIC OF THE	
HS STD SI	IEVE SIZE	4 10	40 60 200	270	- SOUNDING ROD	(REF)— SPT REFUSAL			I BE SCRATCHED BY KN AL HARD BLOWS OF THE	NIFE OR SHARP PICK, BREAKING OF H	HAND SPECIMENS REQUIRES	PARENT ROCK.	
U.S. STD. SIEVE SIZE 4 10 40 60 200 270 OPENING (MM) 4.76 2.0 0.42 0.25 0.075 0.053					HARD CAN BE SCRATCHED BY KNIFE OR PICK ONLY WITH DIFFICULTY, HARD HAMMER BLOWS REQUIRED				SILL - AN INTRUSIVE BODY OF IGNEOUS ROCK OF APPROXIMATELY UNIFORM THICKNESS AND				
BOULDE	ER COBBLE C	GRAVEL (COARSE FINE			ABBREVIATIONS			TACH HAND SPECIMEN.			RELATIVELY THIN COMPARED WITH ITS LATERAL EXTENT, WHICH HAS BEEN EMPLACED PARALLEL TO THE BEDDING OR SCHISTOSITY OF THE INTRUDED ROCKS	
·(BLDR		con :	SAND SAND	U (61.) (71.)	AR - AUGER REFUS BT - BORING TERM					OR PICK. GOUGES OR GROOVES TO		SLICKENSIDE - POLISHED AND STRIATED SURFACE THAT RESULTS FROM FRICTION ALONG A FAULT OR	
GRAIN !	MM 305 75	2.0	0.25	0.05 0.005	CL CLAY	SL SILT, SIL	.TY		ATED BY HARD BLOW OI DERATE BLOWS.	F A GEOLOGISTS PICK. HAND SPECIM	ENS CAN BE DETACHED	SLIP PLANE.	
SIZE I		2.0			CPT - CONE PENET	TRATION TEST SLI SLIGHTL TCR - TRICONE		ł		0.05 INCHES DEEP BY FIRM PRESSU	RE OF KNIFE OR PICK POINT.	STANDARD PENETRATION TEST (PENETRATION RESISTANCE) (SPT) - NUMBER OF BLOWS (N OR B.P.F.) OF A 140 LB. HAMMER FALLING 30 INCHES REQUIRED TO PRODUCE A PENETRATION OF 1 FOOT INTO SOIL WITH	
	SOIL MOIS	TURE - COR	RELATION OF	TERMS	DMT - DILATOMETER TEST				E EXCAVATED IN SMALL OF A GEOLOGISTS PICE	CHIPS TO PEICES 1 INCH MAXIMUM	SIZE BY HARD BLOWS OF THE	A 2 INCH OUTSIDE DIAMETER SPLIT SPOON SAMPLER. SPT REFUSAL IS LESS THAN 0.1 FOOT PENETRATION	
	MOISTURE SCALE	FIELD MOIST		FIELD MOISTURE DESCRIPTION	DPT - DYNAMIC PE → - VOID RATIO	NETRATION TEST /d - DRY UN		1		k. READILY BY KNIFE OR PICK. CAN BE	EXCAVATED IN FRAGMENTS	WITH 60 BLOWS.	
(ATTE	ERBERG LIMITS)	DESCRIPTIO	IN JOSE FOR		F FINE	W - MOISTURE	CONTENT	FROM	CHIPS TO SEVERAL INC	CHES IN SIZE BY MODERATE BLOWS O		STRATA CORE RECOVERY (SREC.) - TOTAL LENGTH OF STRATA MATERIAL RECOVERED DIVIDED BY TOTAL LENGTH OF STRATUM AND EXPRESSED AS A PERCENTAGE.	
		- SATURATE		.10UID; VERY WET, USUALLY	FOSS FOSSILIFE		HEAR TEST	ł	CAN BE BROKEN BY F			STRATA ROCK QUALITY DESIGNATION (S.R.O.D.) - A MEASURE OF ROCK QUALITY DESCRIBED BY:	
LL_	LIQUID LIMIT	(SAT.)	FROM BELO	OW THE GROUND WATER TABLE	FRAGS FRAGMEN		nerus (CO)			CAN BE EXCAVATED READILY WITH F BE BROKEN BY FINGER PRESSURE. CA		TOTAL LENGTH OF ROCK SEGMENTS WITHIN A STRATUM EQUAL TO OR GREATER THAN 4 INCHES DIVIDED BY THE	
PLASTIC	T		SEMISOL ID	REQUIRES DRYING TO	MED MEDIUM			FINGER	NAIL.			TOTAL LENGTH OF STRATA AND EXPRESSED AS A PERCENTAGE.	
RANGE <	Di some i suss	- WET - 0		TIMUM MOISTURE	EQUI	PMENT USED ON SUBJECT P	PROJECT	FRACTU	RE SPACING		EDDING	TOPSOIL (T.S.) - SURFACE SOILS USUALLY CONTAINING DRGANIC MATTER.	
PLL	+ PLASTIC LIMIT				DRILL UNITS:	ADVANCING TOOLS:	HAMMER TYPE:	TERM	SPACING	TERM VERY TUTCKLY REPR	THICKNESS ED > 4 FEET	BENCH MARK:	
OM	OPTIMUM MOISTURE	- MOIST -	(M) SOLID; AT	OR NEAR OPTIMUM MOISTURE		CLAY BITS	AUTOMATIC MANUAL	VERY WIDE WIDE	MORE THAN 10 FE 3 TO 10 FEET	EET VERY THICKLY BEDD THICKLY BEDDED	1.5 - 4 FEET		
SL	T				MOBILE B	hamad .		MODERATELY CLOS	E 1 TO 3 FEET	THINLY BEDDED VERY THINLY BEDDE	0.16 - 1.5 FEET D 0.03 - 0.16 FEET	ELEVATION:	
		~ DRY - (D		ADDITIONAL WATER TO		6 CONTINUOUS FLIGHT AUGER	CORE SIZE:	CLOSE VERY CLOSE	0.16 TO 1 FEET LESS THAN 0.16	FFFT THICKLY LAMINATED	0.008 - 0.03 FEET	NOTES:	
		טולו י לב	ATTAIN OP	PTIMUM MOISTURE	BK-51	X 8 HOLLOW AUGERS	□-в	VERT CLUSE	FEOD THEM ATTE	THINLY LAMINATED	< 0.008 FEET	_	
		PLAST	ICITY \		CME-45	HARD FACED FINGER BITS	X-NWL			INDURATION		-	
		PLASTICITY I	INDEX (PI)	DRY STRENGTH		TUNGCARBIDE INSERTS		FOR SEDIMENTARY ROO		HARDENING OF THE MATERIAL BY C			
NONPLASTI		0-5		VERY LOW	X CME-550	X CASING X W/ ADVANCER	н	FRIABLE		RUBBING WITH FINGER FREES NUMER GENTLE BLOW BY HAMMER DISINTEG			
MED. PLAST		6-15 16-25		SLIGHT MEDIUM			HAND TOOLS:						
HIGH PLAS		26 OR	MORE	HIGH	PORTABLE HOIST	TRICONESTEEL TEETH	POST HOLE DIGGER	MODERATEL		GRAINS CAN BE SEPARATED FROM SA BREAKS EASILY WHEN HIT WITH HAM			
		COL	LOR		OTHER CME-45C TRACK	TRICONE TUNGCARB.	HAND AUGER	THOUGATES		GRAINS ARE DIFFICULT TO SEPARAT			
DESCR	IPTIONS MAY INCLUDE CO	OLOR OR COLOR O	COMBINATIONS (TAN. RE	ED, YEL-BRN, BLUE-GRAY)		CORE BIT	SOUNDING ROD	INDURATED		DIFFICULT TO BREAK WITH HAMMER.			
	IERS SUCH AS LIGHT, DAF				OTHER	OTHER	VANE SHEAR TEST	EXTREMELY		SHARP HAMMER BLOWS REQUIRED TO	BREAK SAMPLE:		
							OTHER	L		SAMPLE BREAKS ACROSS GRAINS.			
												REVISED 09/15/00	

 ID
 STATE PROJECT NO.
 SHEET NO.
 TOTAL SHEETS

 B-3661
 8.2942001
 2
 16



STATE OF NORTH CAROLINA DEPARTMENT OF TRANSPORTATION

MICHAEL F. EASLEY
GOVERNOR

LYNDO TIPPETT SECRETARY

November 2003

STATE PROJECT:

8.2942001 (B-3661)

F. A. PROJECT:

BRZ-1503(4)

COUNTY:

Haywood

DESCRIPTION:

Approaches to Bridge No. 36 on SR-1503 over Crabtree Creek

SUBJECT:

Geotechnical Report – Inventory

Site Description

This project is located in east-central Haywood County near the community of Crabtree. The site area is rural with scattered residences. Mountainous slopes characterize the surrounding topography. The replacement structure for Bridge No. 36 is to be located approximately 100 feet upstream, south of the existing bridge. Cut excavations will dominate the project construction due to the relocation of the existing alignment.

The subsurface investigation was completed using a CME-550 drill unit equipped with NW wireline rock coring apparatus, and eight-inch hollow stem augers. Standard Penetration Tests (SPT) were performed at selected sites using an automatic drop hammer. Both rock and soil samples were retrieved from borings and analysis.

Stations 11+50 - 19+85 of Base Line -L- were investigated.

Areas of Special Geotechnical Interest

(1) Groundwater within 6 feet of ground surface.

Station 16+20

MAILING ADDRESS:
NC DEPARTMENT OF TRANSPORTATION
GEOTECHNICAL UNIT
1589 MAIL SERVICE CENTER
RALEIGH NC 27699-1589

TELEPHONE: 919-250-4088 FAX: 919-250-4237

WEBSITE: WWW.DOH.DOT.STATE.NC.US

LOCATION: CENTURY CENTER COMPLEX BUILDING B 1020 BIRCH RIDGE DRIVE RALEIGH NC 27610 (2) Cut Sections containing hard rock at or above grade.

Stations 12+00 - 12+50 Stations 14+50 - 15+00 Station 18+50

Rock Characteristics

The project area is located in the Blue Ridge Belt of the Mountain Physiographic Province. Rock exposed in existing cuts is a biotite gneiss unit that is gray, medium to coarse-grained, and well foliated. Observed discontinuities are primarily along foliation along with a prevalent joint set resulting in several dislodged blocks in the existing cuts.

Rock structure data collected at the site is presented below.

Structure	Strike & Dip	Dip/Dip Direction
Foliation	N 30° E, 42-45° SE	45/120°
Joint	N 48° W, 75° NE	75/42°

Geotechnical Description of the Project

(1) Stations 11+50-16+20

Cuts to the right of the alignment are proposed through this interval. These excavations will be primarily constructed in stiff to very dense saprolitic silty sand with intermittent ledges of weathered rock. Borings revealed hard rock lying sporadically in the cuts. The rock is dipping at approximately 45 degrees, and is interlayered with saprolite, most likely the result of differential weathering.

A. Retaining Wall: Stations 12+00 – 13+50

A retaining wall is proposed to the left of the alignment along these stations. It will be founded upon existing embankment, alluvium, saprolite, and both weathered and hard rock.

2) Stations 16+20 – 17+10

This interval consists of the proposed bridge crossing. A boring advanced at Station 16+20 revealed 3.5 feet of alluvial sand and gravel deposited over hard rock.

(3) <u>Stations 17+10 – 19+85</u>

Primary construction will be cuts located to the left of centerline. Slopes will be constructed in mostly loose sandy saprolite and sparse hard rock.

Respectfully Submitted,

J. W. Mann, TEG-III

COMPUTED BY: RAW CHECKED BY: KFH

DATE: Jul-07 DATE: Jul-07

EARTHWORK BALANCE SHEET

Volumes in Cubic Yards

WILLIAMSR PROJECT B-3661

COUNTY Haywood

DATE

<u>July-07</u>

OF <u>1</u>

SHEETS

		STATION	EXCAVATION				El	MBANKMEN	NT			WASTE				
ROADWAY	STATION		TOTAL UNCLASS.	ROCK	UNDERCUT		SUITABLE UNCLASS.	TOTAL	ROCK	EARTH	EMBANK. +15%	BORROW	ROCK	SUITABLE	UNSUIT.	TOTAL
	Summary No	. 1														
L	12+00.00	16+20.58	21,004	3,698			17,306	302	242		242		3,456	17,306		20,762
То	 tal Summary N	0.1	21,004	3,698			17,306	302	242		242		3,456	17,306		20,762
	Summary No	. 2														
L	17+09.04	19+80.00	785				785	248		248	285	-	ulaw - managaran	500		500
Т	otal Summary	No. 2	785				785	248		248	285			500		500
P	 Project Sub-Tot 	al .	21,789	3,698			18,091	550	242	248	527	,	3,456	17,806		21,262
Est. Loss Du	e to Clearing &	Grubbing	-1,980				-1,980							-1,980		-1,980
Rock Waste Adjust for Ro	to Replace Eartl	1				,,,,,,,,,,,,,,,,,,,,,,,,,,,,,,,,,,,,,,,			198	-248	-87		-198 1,425	285		87 1,425
<u> </u>	Project Tota		19,809	3,698			16,111	550	440		440		4,683	16,111		20,794
	Say		20,000													21,000
		·														
																<u> </u>

Est. Undercut Contingency Item = 500 Cu. Yd.

Est. Undercut Grade Point = 70 Cu. Yd.

= 500 Cu. Yd. Est. Select Granular Material

Est. Fabric for Soil Stabilization Contingency = 500 Sq. Yd.

Est. CL. IV Subgrade Stabilization Contingency = 500 Tons

Est. DDE = 20 Cu. Yd.

EARTHWORK QUANTITIES ARE CALCULATED BY THE ROADWAY DESIGN UNIT. THESE EARTHWORK QUANTITIES ARE BASED IN PART ON THE SUBSURFACE DATA PROVIDED BY THE GEOTECHNICAL ENGINEERING UNIT.

APPROACHES TO BRIDGE NO. 36 ON SR-1503 OVER CRABTREE CREEK

