

FOUNDATION LAYOUT

END BENT BRACE PILES ARE BATTERED AT 3:12 DIMENSIONS LOCATING END BENT PILES ARE SHOWN TO CENTERLINE OF PILES

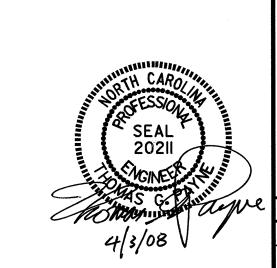
END BENT #1

END BENT #2

#### FOUNDATION NOTES

DRIVE PILES AT END BENT #1 AND #2 TO A REQUIRED BEARING CAPACITY OF 120 TONS PER PILE. THE REQUIRED BEARING CAPACITY IS EQUAL TO THE ALLOWABLE BEARING CAPACITY WITH A MINIMUM FACTOR OF SAFETY OF TWO. THE ALLOWABLE BEARING CAPACITY FOR PILES AT END BENT #1 AND #2 IS 60 TONS PER PILE.

STEEL PILE POINTS WITH TEETH ARE REQUIRED FOR STEEL PILES AT END BENT #1 AND #2. SEE SECTION 450 OF THE STANDARD SPECIFICATIONS.



PROJECT NO. B-3661

HAYWOOD COUNTY

STATION: 16+65.00 -L-

SHEET 2 OF 3

STATE OF NORTH CAROLINA

DEPARTMENT OF TRANSPORTATION

RALEIGH

GENERAL DRAWING

FOR BRIDGE OVER CRABTREE CREEK ON SR 1503 BETWEEN NC 209 AND SR 1502

REVISIONS

SHEET NO.

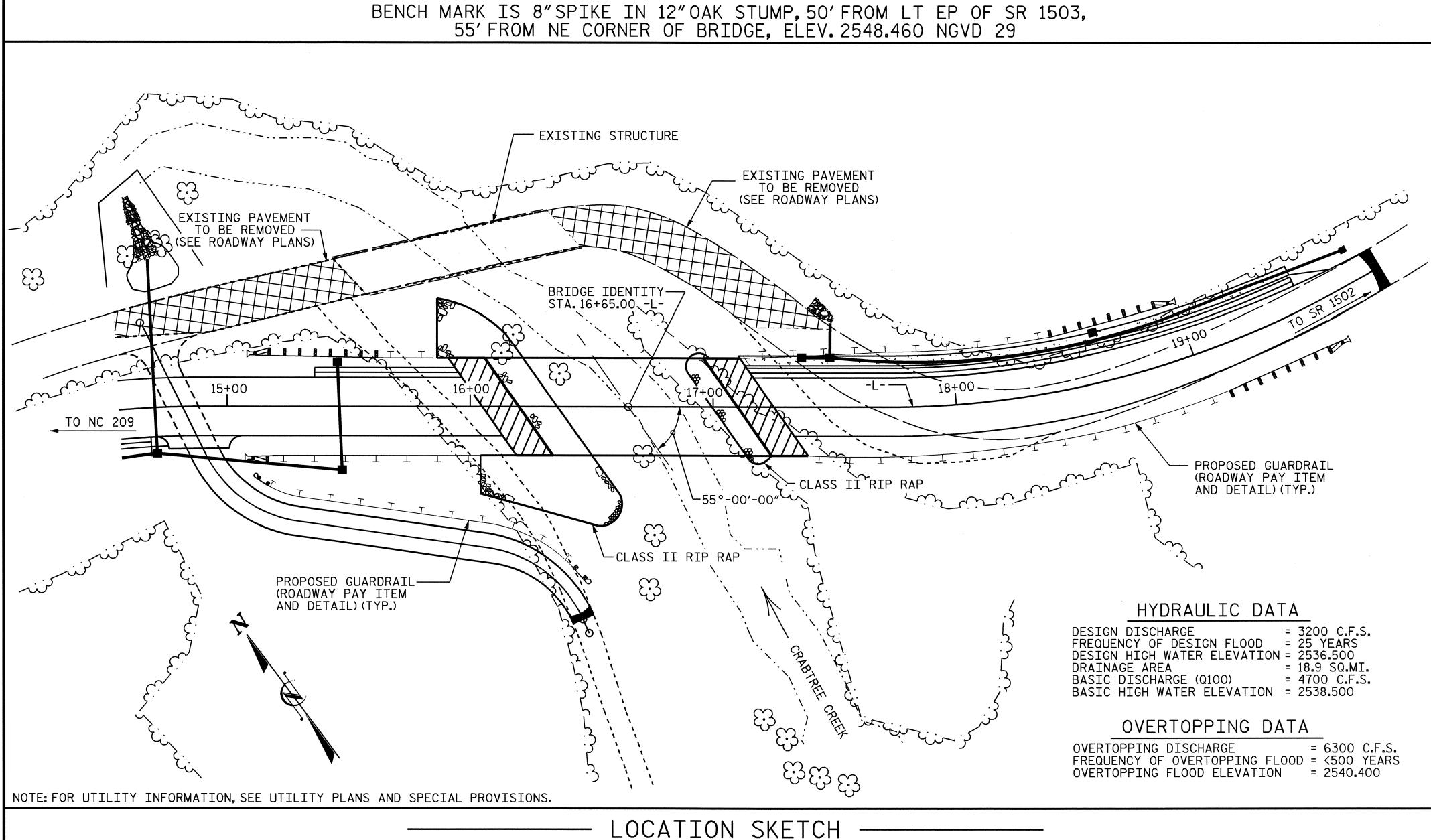
S-2

TOTAL SHEETS

22

DRAWN BY: J.P. ADAMS

CHECKED BY: S.H. SOCKWELL/T. G. P. DATE: 12/12/07



NOTES

ASSUMED LIVE LOAD = HS 20 OR ALTERNATE LOADING, EXCEPT THAT THE GIRDERS HAVE BEEN DESIGNED FOR HS25.

FOR OTHER DESIGN DATA AND GENERAL NOTES, SEE SHEET SN.

FOR EROSION CONTROL MEASURES SEE EROSION CONTROL PLANS.

THIS BRIDGE HAS BEEN DESIGNED BY THE STRENGTH DESIGN METHOD AS SPECIFIED IN AASHTO STANDARD SPECIFICATIONS.

ALL STRUCTURAL STEEL SHALL BE AASHTO M270 GRADE 50W AND PAINTED IN ACCORDANCE WITH SYSTEM 4 OF ARTICLE 442-7 OF THE STANDARD SPECIFICATIONS UNLESS OTHERWISE NOTED ON THE PLANS.

AFTER SERVING AS A TEMPORARY STRUCTURE, THE EXISTING STRUCTURE CONSISTING OF 3 SPANS (1 @ 30'-3", 1 @ 30'-1", AND 1 @ 30'-3") OF 8 LINES OF 14" I-BEAMS AND A STEEL PLANK FLOOR AND ASPHALT WEARING SURFACE, WITH A CLEAR ROADWAY WIDTH OF 19'-2" AND SUPPORTED ON TIMBER CAPS AND TIMBER PILES WITH REINFORCED CONCRETE SILLS AT INTERIOR BENTS AND LOCATED DOWNSTREAM FROM THE PROPOSED STRUCTURE SHALL BE REMOVED. THE EXISTING BRIDGE IS PRESENTLY POSTED BELOW THE LEGAL LOAD LIMIT. SHOULD THE STRUCTURAL INTEGRITY OF THE BRIDGE FURTHER DETERIORATE, THIS LOAD LIMITATION MAY BE REDUCED AS FOUND NECESSARY DURING THE LIFE OF THE PROJECT.

REMOVAL OF THE EXISTING BRIDGE SHALL BE PERFORMED SO AS NOT TO ALLOW DEBRIS TO FALL INTO THE WATER. THE CONTRACTOR SHALL REMOVE THE BRIDGE AND SUBMIT PLANS FOR DEMOLITION IN ACCORDANCE WITH ARTICLE 402-2 OF THE STANDARD SPECIFICATIONS.

THE MATERIAL SHOWN IN THE CROSS-HATCHED AREA ON "GENERAL DRAWING" SHEET 1 OF 3 SHALL BE EXCAVATED FOR A DISTANCE OF 26 FT. EACH SIDE OF CENTERLINE ROADWAY AS DIRECTED BY THE ENGINEER. THIS WORK WILL BE PAID FOR AT THE CONTRACT LUMP SUM PRICE FOR UNCLASSIFIED STRUCTURE EXCAVATION. SEE SECTION 412 OF THE STANDARD SPECIFICATIONS.

THE CLASS AA CONCRETE IN THE BRIDGE DECK SHALL CONTAIN FLY ASH OR GROUND GRANULATED BLAST FURNACE SLAG AT THE SUBSTITUTION RATE SPECIFIED IN ARTICLE 1024-1 AND IN ACCORDANCE WITH ARTICLES 1024-5 AND 1024-6 OF THE STANDARD SPECIFICATIONS. NO PAYMENT WILL BE MADE FOR THIS SUBSTITUTION AS IT IS CONSIDERED INCIDENTAL TO THE COST OF THE REINFORCED CONCRETE DECK SLAB.

THIS STRUCTURE HAS BEEN DESIGNED IN ACCORDANCE WITH HEC 18, "EVALUATING SCOUR AT BRIDGES", MAY, 2001.

THIS BRIDGE HAS BEEN DESIGNED IN ACCORDANCE WITH THE REQUIREMENTS OF THE AASHTO STANDARD SPECIFICATIONS FOR SEISMIC DESIGN OF HIGHWAY BRIDGES FOR SEISMIC PERFORMANCE CATEGORY B.

THE CONTRACTOR SHALL PROVIDE INDEPENDENT ASSURANCE SAMPLES OF REINFORCING STEEL AS FOLLOWS: FOR PROJECTS REQUIRING UP TO 400 TONS OF REINFORCING STEEL, ONE 30 INCH SAMPLE OF EACH SIZE BAR USED, AND FOR PROJECTS REQUIRING OVER 400 TONS OF REINFORCING STEEL, TWO 30 INCH SAMPLES OF EACH SIZE BAR USED. THE BARS FROM WHICH THE SAMPLES ARE TAKEN MUST THEN BE SPLICED WITH REPLACEMENT BARS OF THE SIZE AND LENGTH OF THE SAMPLE, PLUS A MINIMUM LAP SPLICE OF THIRTY BAR DIAMETERS.

INASMUCH AS THE PAINT SYSTEM ON THE EXISTING STRUCTURAL STEEL CONTAINS LEAD, THE CONTRACTOR'S ATTENTION IS DIRECTED TO ARTICLE 107-1 OF THE STANDARD SPECIFICATIONS. ANY COSTS RESULTING FROM COMPLIANCE WITH APPLICABLE STATE OR FEDERAL REGULATIONS PERTAINING TO HANDLING OF MATERIALS CONTAINING LEAD BASED PAINT SHALL BE INCLUDED IN THE BID PRICE FOR "REMOVAL OF EXISTING STRUCTURE."

NEEDLE BEAMS WILL NOT BE ALLOWED UNLESS OTHERWISE CALLED FOR ON THE PLANS OR APPROVED BY THE ENGINEER.

FOR SUBMITTAL OF WORKING DRAWINGS, SEE SPECIAL PROVISIONS.

FOR FALSEWORK AND FORMWORK, SEE SPECIAL PROVISIONS.

FOR CRANE SAFETY, SEE SPECIAL PROVISIONS.

FOR SHIPPING STEEL STRUCTURAL MEMBERS, SEE SPECIAL PROVISIONS.

REMOVABLE FORMS MAY BE USED IN LIEU OF METAL STAY-IN-PLACE FORMS IN ACCORDANCE WITH ARTICLE 420-3 OF THE STANDARD SPECIFICATIONS.

FOR GROUT FOR STRUCTURES, SEE SPECIAL PROVISIONS.

					— тс	TAL B	ILL OF	MATER	IAl							
	REMOVAL OF EXISTING STRUCTURE	UNCLASSIFIED STRUCTURE EXCAVATION	REINFORCED CONCRETE DECK SLAB	GROOVING BRIDGE FLOORS	CLASS A CONCRETE	BRIDGE APPROACH SLABS	REINFORCING STEEL	STRUCTURAL STEEL (APPROX.)	HP :	l2 X 53 EL PILES	STEEL PILE POINTS	CONCRETE BARRIER RAIL	RIP RAP CLASS II (2'-0'' THICK)	FILTER FABRIC FOR DRAINAGE	ELASTOMERIC BEARINGS	EVAZOTE JOINT SEALS
	LUMP SUM	LUMP SUM	SQ.FT.	SQ.FT.	CU. YDS.	LUMP SUM	LBS.	LBS.	NO.	LIN.FT.	EACH	LIN.FT.	TONS	SQ. YDS.	LUMP SUM	LUMP SUM
SUPERSTRUCTURE	LUMP SUM		3046	3067		LUMP SUM		97900				194.92			LUMP SUM	LUMP SUM
END BENT #1					29.2		4408		7	70	7		182	202		
END BENT #2		LUMP SUM			30.3		4786		7	125	7		20	22		
TOTAL	LUMP SUM	LUMP SUM	3046	3067	59 <b>.</b> 5	LUMP SUM	9194	97900	14	195	14	194.92	202	224	LUMP SUM	LUMP SUM

PROJECT NO. B-3661

HAYWOOD COUNTY

STATION: 16+65.00 -L-

SHEET 3 OF 3

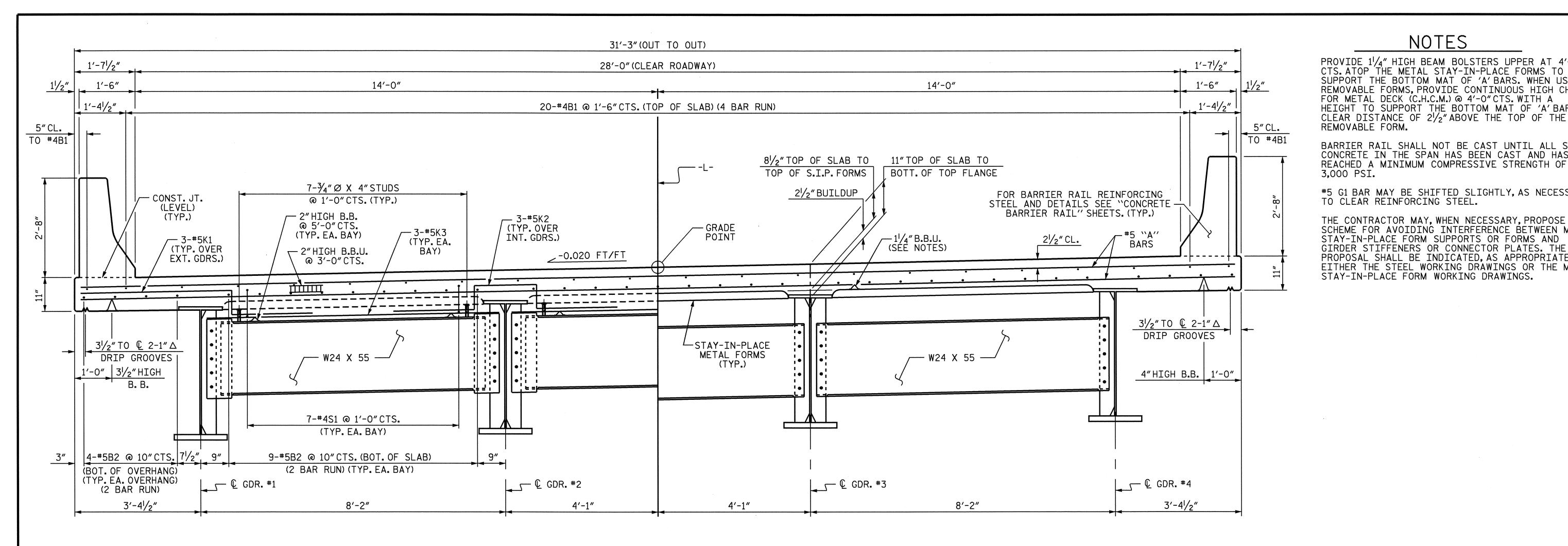
DEPARTMENT OF TRANSPORTATION
RALEIGH

GENERAL DRAWING

FOR BRIDGE OVER CRABTREE CREEK ON SR 1503 BETWEEN NC 209 AND SR 1502

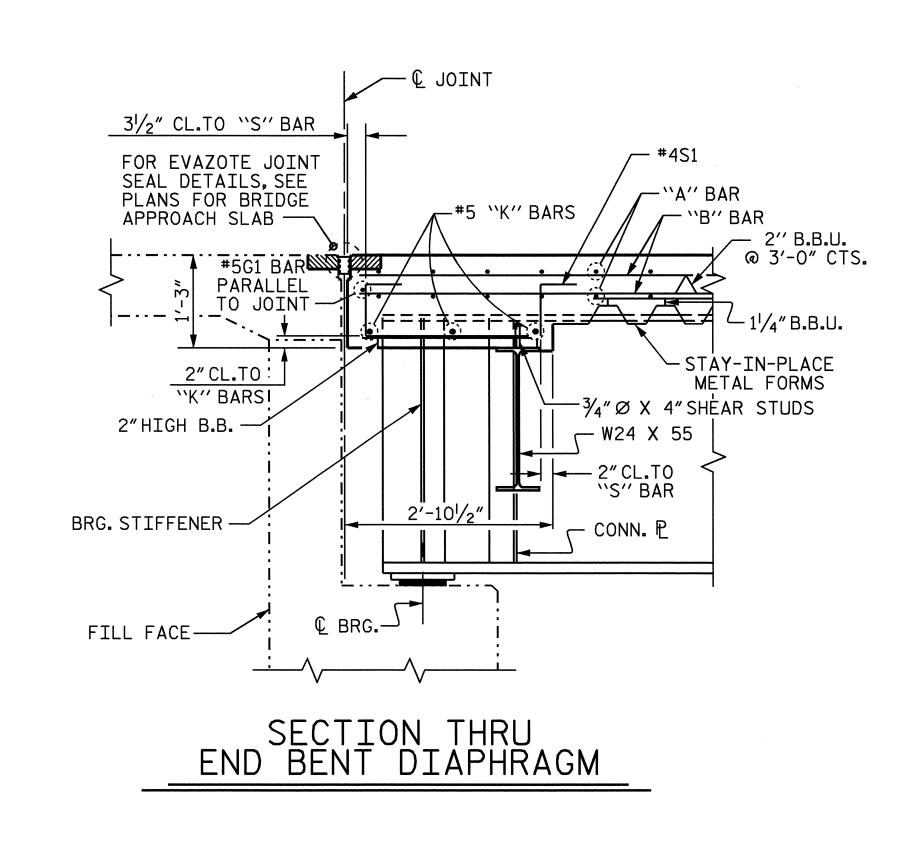
		REV:	ISION	S		SHEET NO.	
ο.	BY:	DATE:	NO.	BY:	DATE:	S-3	
			3			TOTAL SHEETS	
2			4			22	

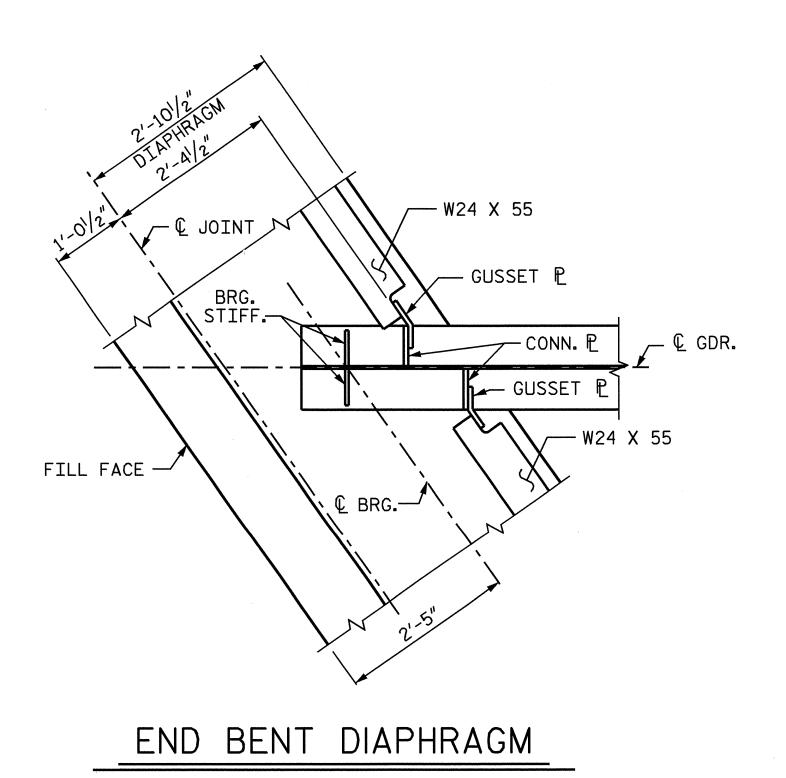
DRAWN BY: J.P. ADAMS/R. G. E. DATE: 09/07
CHECKED BY: S.H. SOCKWELL/T. G. P. DATE: 12/12/07



TYPICAL HALF SECTION @ END BENT DIAPHRAGM

TYPICAL HALF SECTION @ INTERMEDIATE DIAPHRAGM





PROJECT NO. B-3661 HAYWOOD COUNTY STATION: 16+65.00 -L-

NOTES

FOR METAL DECK (C.H.C.M.) @ 4'-0"CTS. WITH A

PROVIDE 11/4" HIGH BEAM BOLSTERS UPPER AT 4'-0" CTS. ATOP THE METAL STAY-IN-PLACE FORMS TO

SUPPORT THE BOTTOM MAT OF 'A' BARS. WHEN USING

REMOVABLE FORMS, PROVIDE CONTINUOUS HIGH CHAIRS

HEIGHT TO SUPPORT THE BOTTOM MAT OF 'A' BARS A

BARRIER RAIL SHALL NOT BE CAST UNTIL ALL SLAB CONCRETE IN THE SPAN HAS BEEN CAST AND HAS

#5 G1 BAR MAY BE SHIFTED SLIGHTLY, AS NECESSARY,

THE CONTRACTOR MAY, WHEN NECESSARY, PROPOSE A SCHEME FOR AVOIDING INTERFERENCE BETWEEN METAL

PROPOSAL SHALL BE INDICATED, AS APPROPRIATE, ON

EITHER THE STEEL WORKING DRAWINGS OR THE METAL

STAY-IN-PLACE FORM SUPPORTS OR FORMS AND

STAY-IN-PLACE FORM WORKING DRAWINGS.

GIRDER STIFFENERS OR CONNECTOR PLATES. THE

REACHED A MINIMUM COMPRESSIVE STRENGTH OF

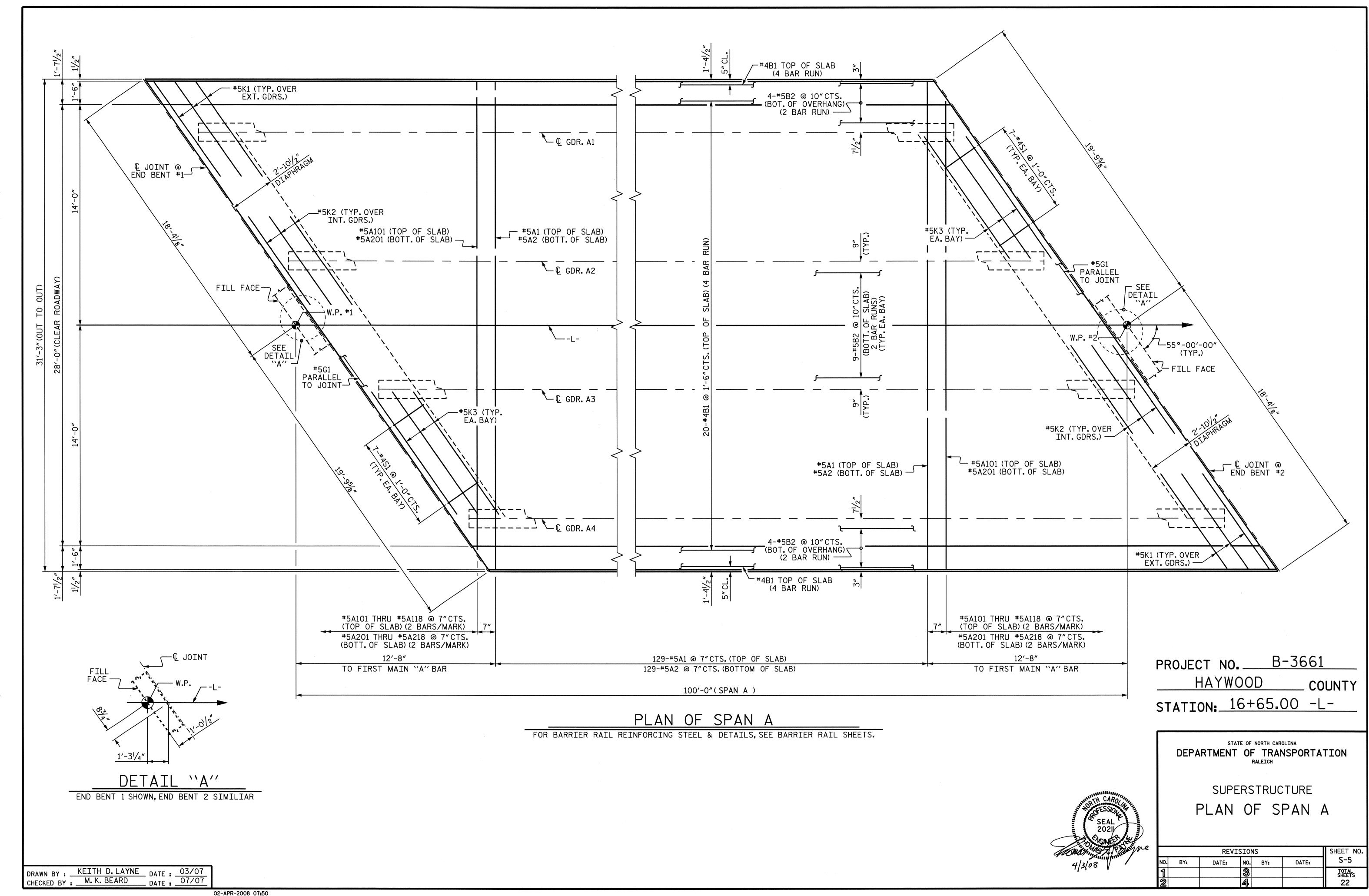
TO CLEAR REINFORCING STEEL.

3,000 PSI.

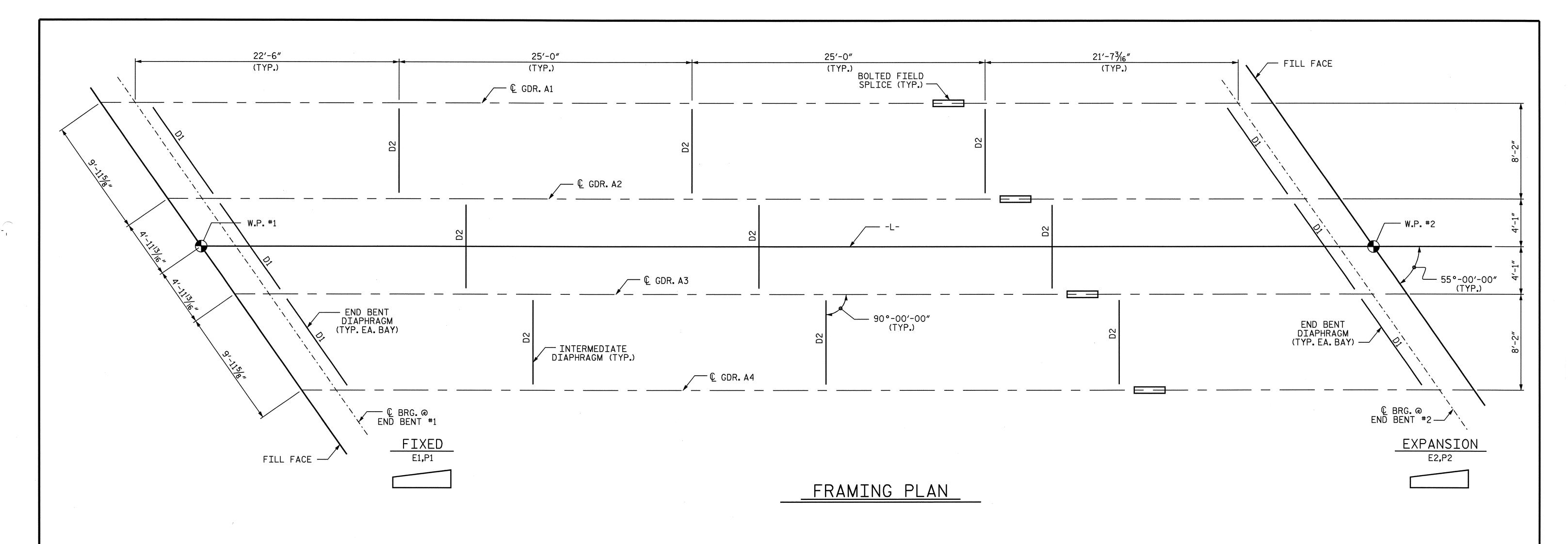
STATE OF NORTH CAROLINA DEPARTMENT OF TRANSPORTATION SUPERSTRUCTURE TYPICAL SECTION

SHEET NO. REVISIONS S-4 BY: DATE: BY: DATE: TOTAL SHEETS 22

DRAWN BY: KEITH D. LAYNE DATE: 03/07
CHECKED BY: M. K. BEARD DATE: 07/07



02-APR-2008 07:50 F:\STRUCTØI\Final\B-3661\_sd\_PS.dgn klayne



DEAD LOAD DEF	FLEC	TION	ITA	3LE	FOR	GIRE	ERS				
				EXTE	RIOR	GIRDE	RS A1	& A4			
TENTH POINTS	0	.1	.2	<b>.</b> 3	.4	<b>.</b> 5	<b>.</b> 6	.7	.8	.9	0
DEFLECTION DUE TO WEIGHT OF BEAM	0	-0.021	-0.040	-0.054	-0.064	-0.067	-0.064	-0.054	-0.040	-0.021	0
*DEFLECTION DUE TO WEIGHT OF SLAB	0	-0.066	-0.134	-0.188	-0.222	-0.233	-0.222	-0.188	-0.134	-0.066	0
DEFLECTION DUE TO WEIGHT OF BARRIER RAIL	0	-0.008	-0.015	-0.021	-0.025	-0.026	-0.025	-0.021	-0.015	-0.008	0
TOTAL DEAD LOAD DEFLECTION	0	-0.095	-0.189	-0.263	-0.310	-0.326	-0.310	-0.263	-0.189	-0.095	0
VERTICAL CURVE ORDINATE											
ORDINATE DUE TO SUPERELEVATION											
REQUIRED CAMBER	0	11/8	21/4	31/8	311/16	3 <sup>15</sup> / <sub>16</sub>	311/16	31/8	21/4	11/8	0

——— DEAD LOAD DEF	FLEC	TION	I TAE	3LE I	FOR	GIRE	ERS				
				INTE	RIOR	GIRDE	RS A2	2 & A3	3		
TENTH POINTS	0	.1	.2	.3	.4	<b>.</b> 5	.6	.7	.8	.9	0
DEFLECTION DUE TO WEIGHT OF BEAM	0	-0.021	-0.040	-0.054	-0.064	-0.067	-0.064	-0.054	-0.040	-0.021	0
*DEFLECTION DUE TO WEIGHT OF SLAB	0	-0.061	-0.125	-0.175	-0.207	-0.217	-0.207	-0.175	-0.125	-0.061	0
DEFLECTION DUE TO WEIGHT OF BARRIER RAIL	0	-0.008	-0.015	-0.021	-0.024	-0.025	-0.024	-0.021	-0.015	-0.008	0
TOTAL DEAD LOAD DEFLECTION	0	-0.090	-0.180	-0.250	-0.294	-0.309	-0.294	-0.250	-0.180	-0.090	0
VERTICAL CURVE ORDINATE				·							
ORDINATE DUE TO SUPERELEVATION											
REQUIRED CAMBER	0	11/16	21/8	3	31/2	311/16	31/2	3	21/8	11/16	0

\* INCLUDES SLAB, BUILDUPS & STAY-IN-PLACE FORMS.
ALL VALUES ARE SHOWN IN FEET (DECIMAL FORM), EXCEPT "REQUIRED CAMBER", WHICH IS GIVEN IN INCHES (FRACTION FORM).

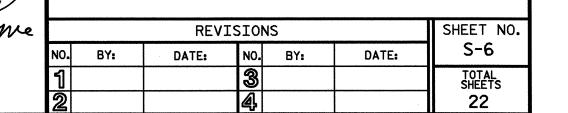
DRAWN BY: KEITH D. LAYNE DATE: 03/07
CHECKED BY: M. K. BEARD DATE: 07/07

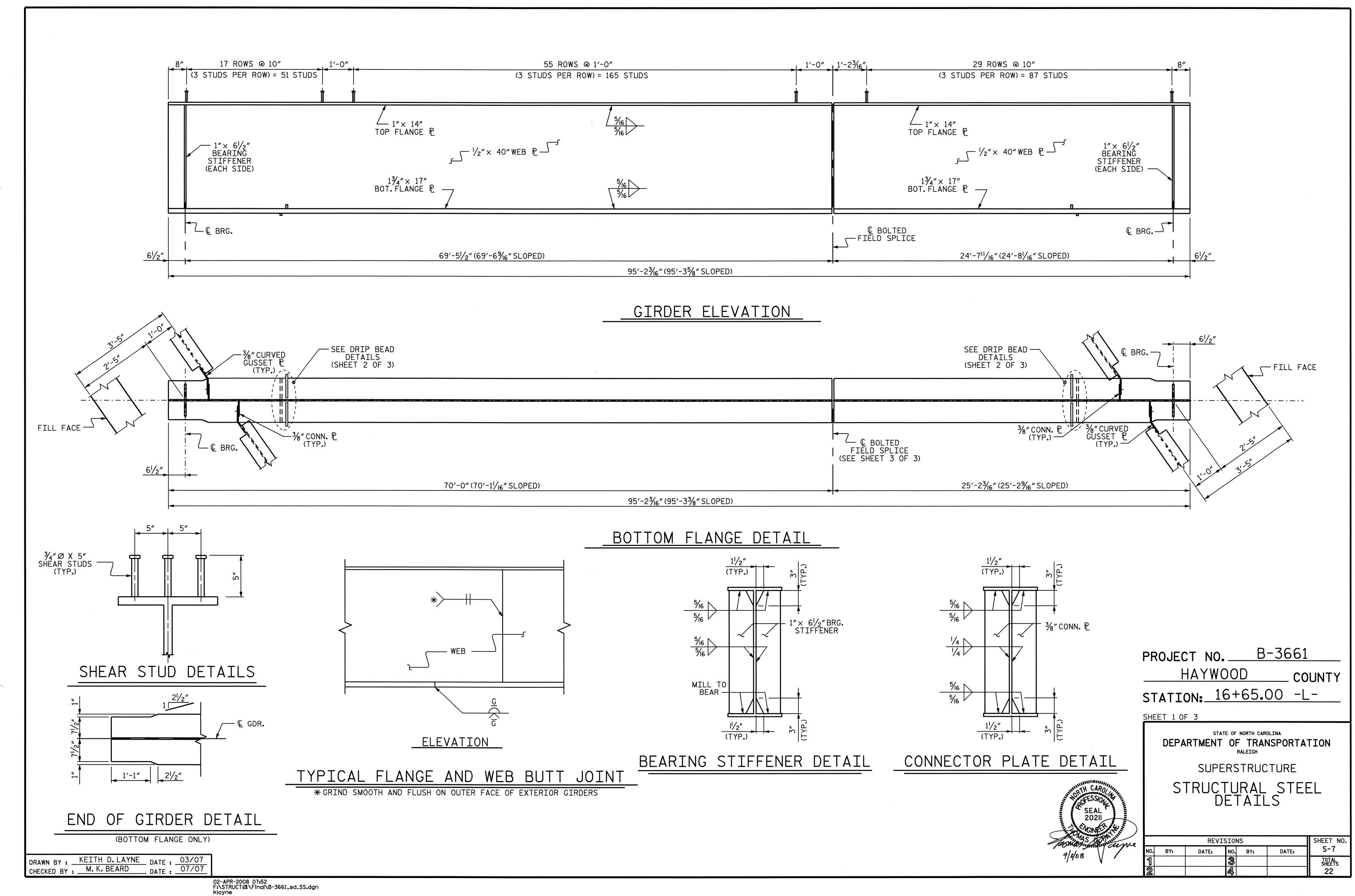
SIGN CONVENTION FOR DEAD LOAD DEFLECTION  $-\frac{1}{2}$ 

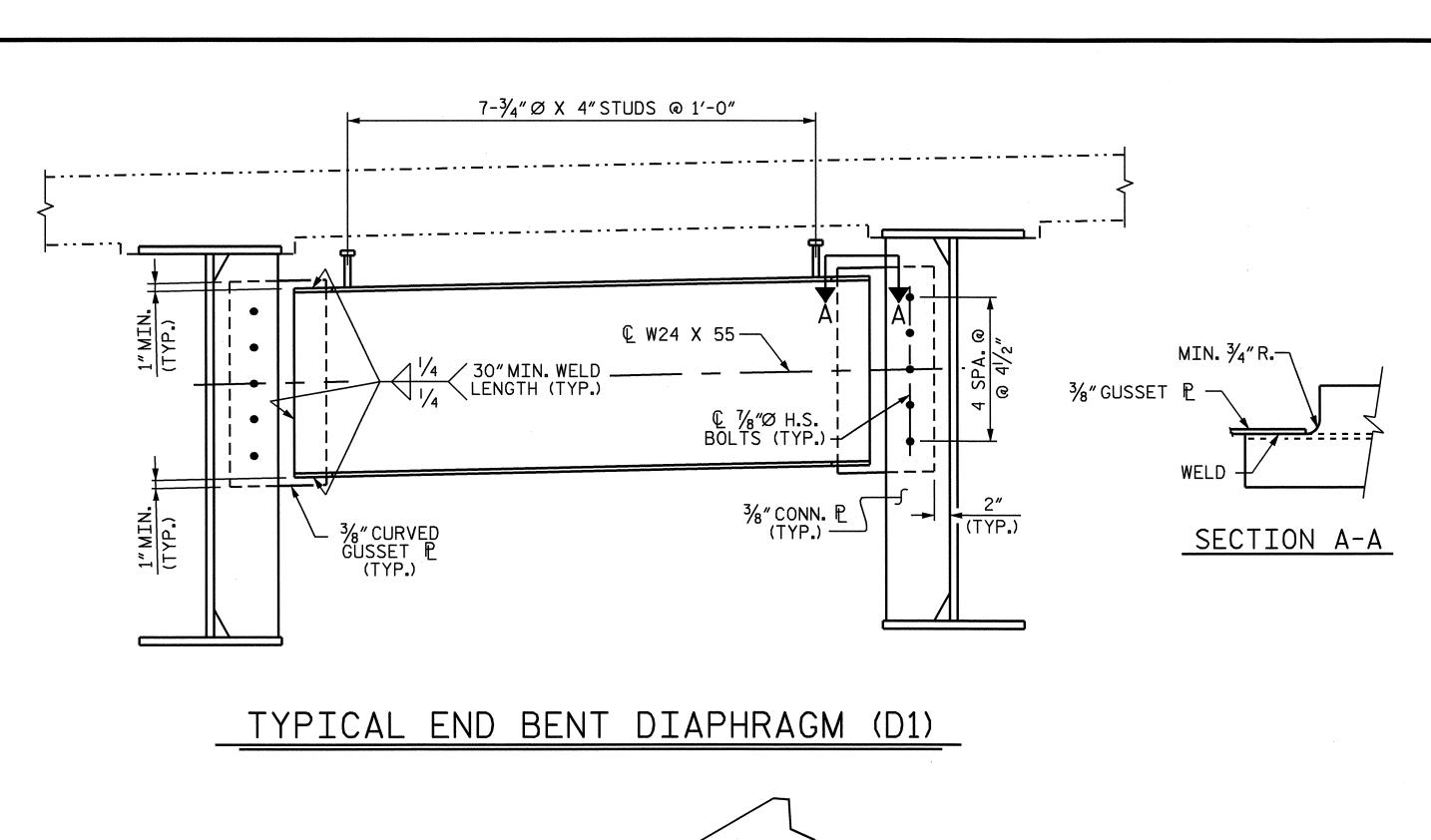
PROJECT NO. B-3661 HAYWOOD \_\_\_\_ COUNTY STATION: 16+65.00 -L-

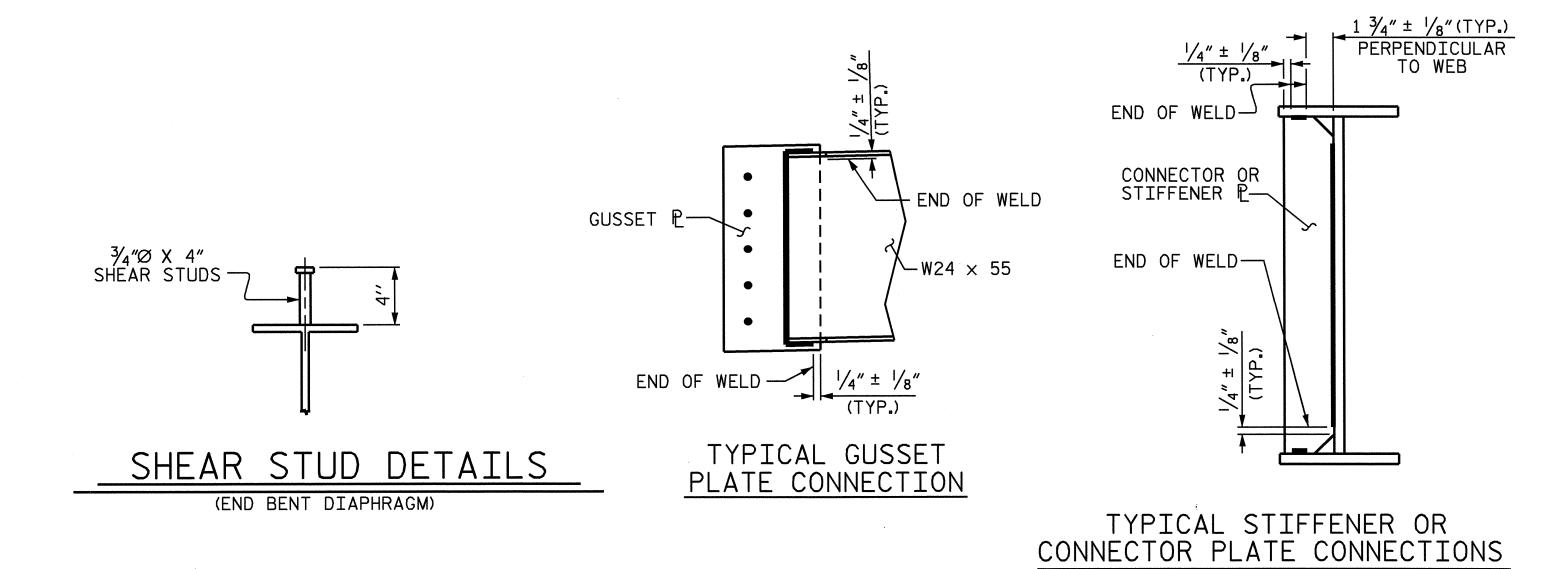
STATE OF NORTH CAROLINA DEPARTMENT OF TRANSPORTATION RALEIGH

> SUPERSTRUCTURE FRAMING PLAN

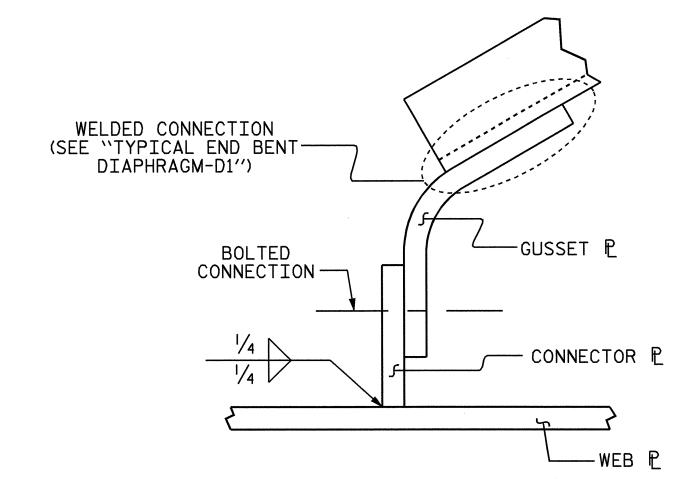




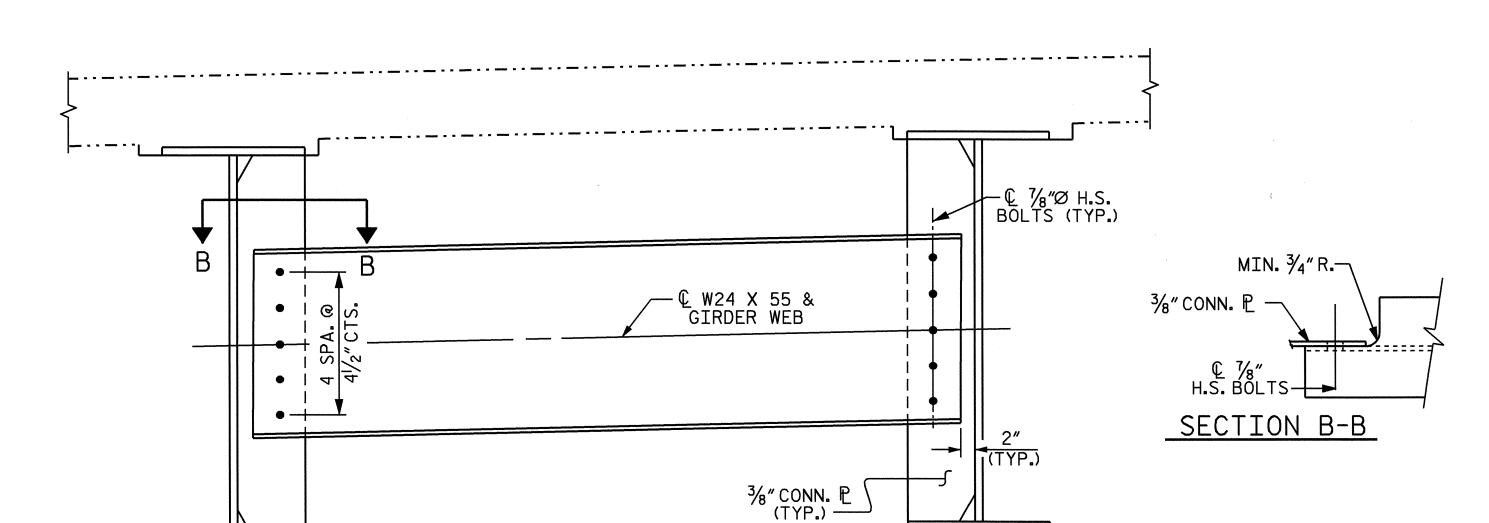




WELD TERMINATION DETAILS

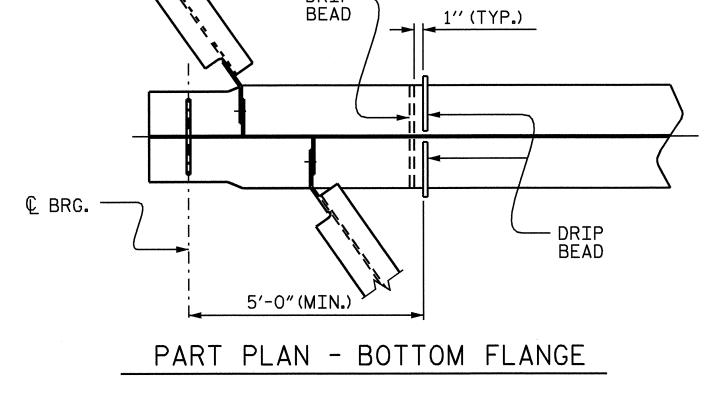


## WELD DETAIL FOR CURVED GUSSET P

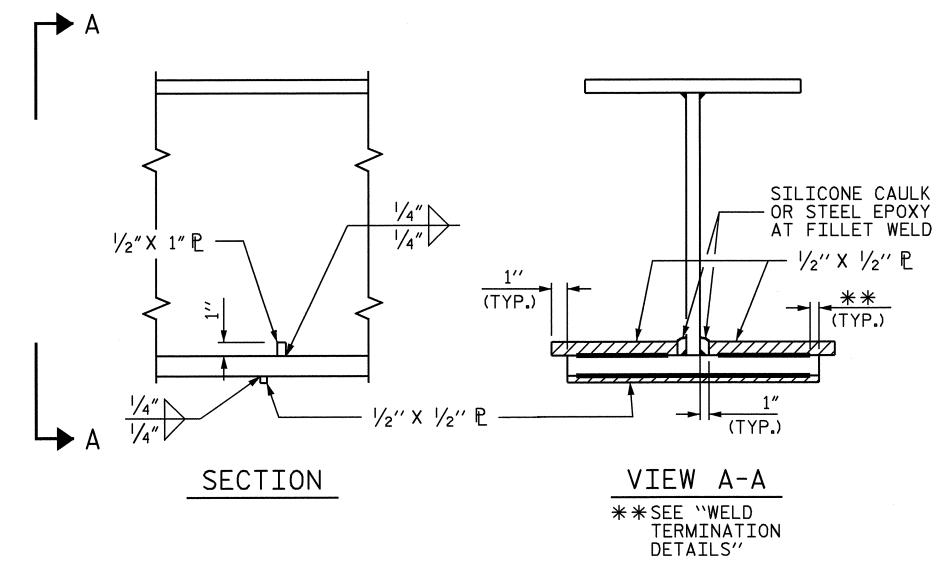


TYPICAL INTERMEDIATE DIAPHRAGM (D2)

DRAWN BY: KEITH D. LAYNE DATE: 03/07
CHECKED BY: M. K. BEARD DATE: 07/07



DRIP — BEAD



DRIP BEAD DETAILS

TYPICAL FOR ALL GHIRDERS

PROJECT NO. B-3661 HAYWOOD \_ COUNTY STATION: 16+65.00 -L-

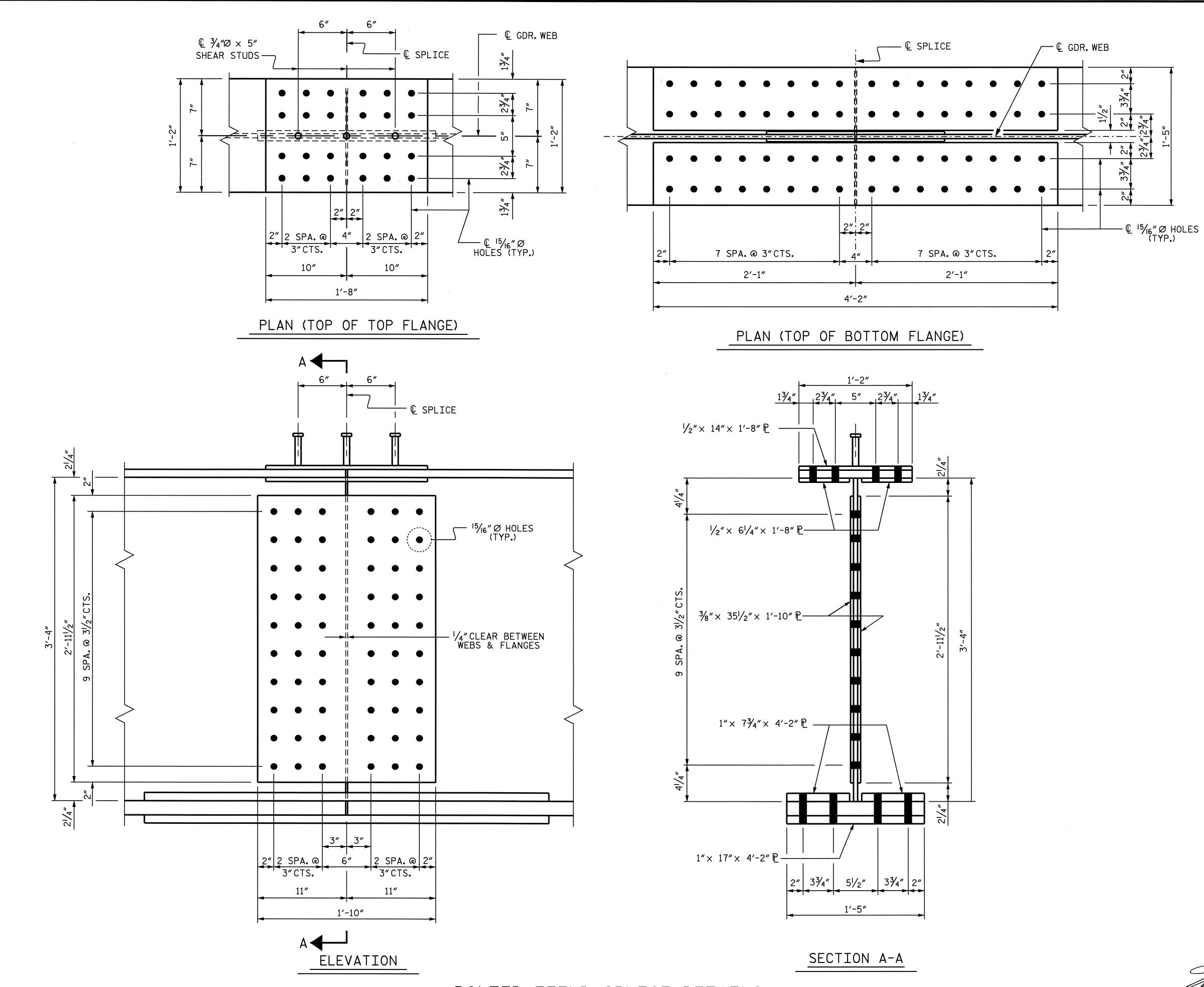
SHEET 2 OF 3 STATE OF NORTH CAROLINA

DEPARTMENT OF TRANSPORTATION SUPERSTRUCTURE

STRUCTURAL STEEL DETAILS

REVISIONS SHEET NO. S-8 BY: DATE: TOTAL SHEETS

02-APR-2008 07:52 F:\STRUCTØ\\Final\B-3661\_sd\_SS.dgn



**NOTES** 

ALL STRUCTURAL STEEL SHALL BE AASHTO M270 GRADE 50W AND PAINTED IN ACCORDANCE WITH SYSTEM 4 OF ARTICLE 442-7 OF THE STANDARD SPECIFICATIONS UNLESS OTHERWISE NOTED ON THE PLANS.

ALL DIMENSIONS SHOWN ARE HORIZONTAL OR VERTICAL. UNLESS OTHERWISE NOTED.

ALL FIELD CONNECTIONS TO BE 1/8" DIA. HIGH STRENGTH BOLTS UNLESS OTHERWISE NOTED.

BEARING STIFFENERS ARE TO BE PLACED NORMAL TO THE WEB OF THE GIRDER AND SHALL BE PLUMB.

A CHARPY V-NOTCH TEST IS REQUIRED FOR WEB PLATES, BOTTOM FLANGE PLATES, BOTTOM FLANGE SPLICE PLATES AND WEB SPLICE PLATES (IF USED) FOR ALL GIRDERS AND IN ACCORDANCE WITH ARTICLE 1072-9 OF THE STANDARD SPECIFICATIONS.

STUDS ON GIRDERS MAY BE SHIFTED UP TO 1"IF NECESSARY TO CLEAR FLANGE SPLICE WELD.

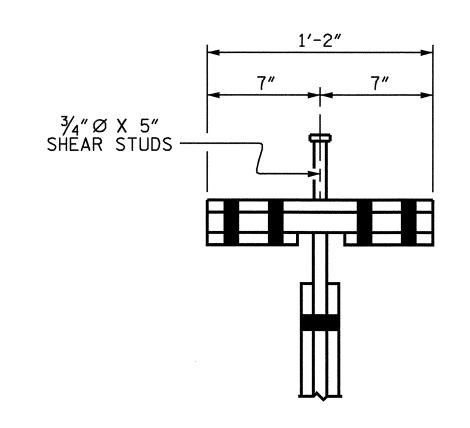
STIFFENERS ARE NOT REQUIRED ON THE OUTSIDE OF EXTERIOR GIRDERS.

SHOP SPLICES ARE PERMITTED TO LIMIT THE MAXIMUM REQUIRED FLANGE PIECE LENGTHS TO 60 FEET AND WEB PIECE LENGTHS TO 45 FEET. PERMITTED FLANGE AND WEB SHOP SPLICES SHALL NOT BE LOCATED WITHIN 15 FEET OF MAXIMUM DEAD LOAD DEFLECTION (NOR WITHIN 15 FEET OF INTERMEDIATE BEARINGS OF CONTINUOUS UNITS). KEEP 2 FEET MINIMUM BETWEEN WEB AND FLANGE SHOP SPLICES. KEEP 6"MINIMUM BETWEEN CONNECTOR PLATE OR TRANSVERSE STIFFENER WELDS AND WEB OR FLANGE SHOP SPLICES.

END OF GIRDERS SHALL BE PLUMB.

TENSION ON THE AASHTO M164 BOLTS SHALL BE CALIBRATED USING DIRECT TENSION INDICATOR WASHERS IN ACCORDANCE WITH ARTICLE 440-8 OF THE STANDARD SPECIFICATIONS.

FOR HIGH STRENGTH BOLTS, SEE SPECIAL PROVISIONS.



SHEAR STUD DETAIL FOR TOP FLANGE SPLICE PLATE

PROJECT NO. B-3661 \_ COUNTY

HAYWOOD

STATION: 16+65.00 -L-

SHEET 3 OF 3

STATE OF NORTH CAROLINA DEPARTMENT OF TRANSPORTATION

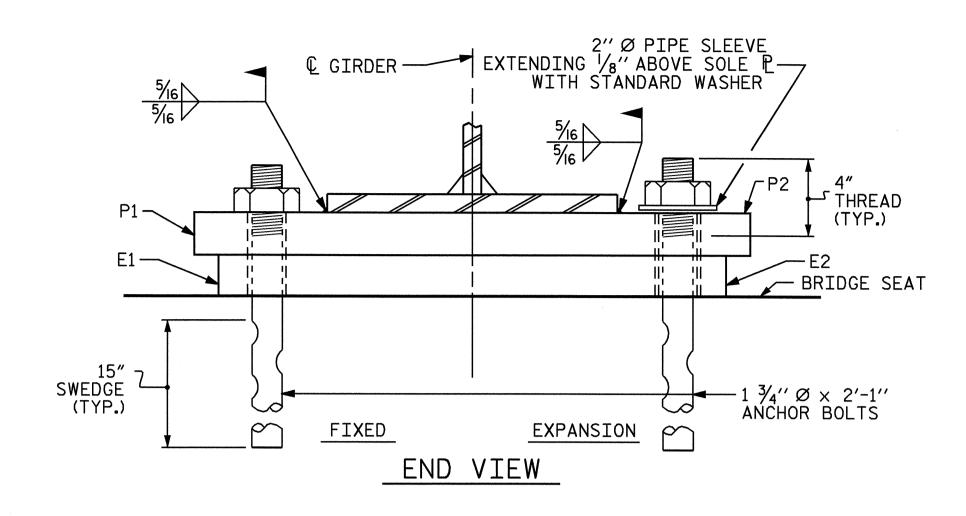
SUPERSTRUCTURE

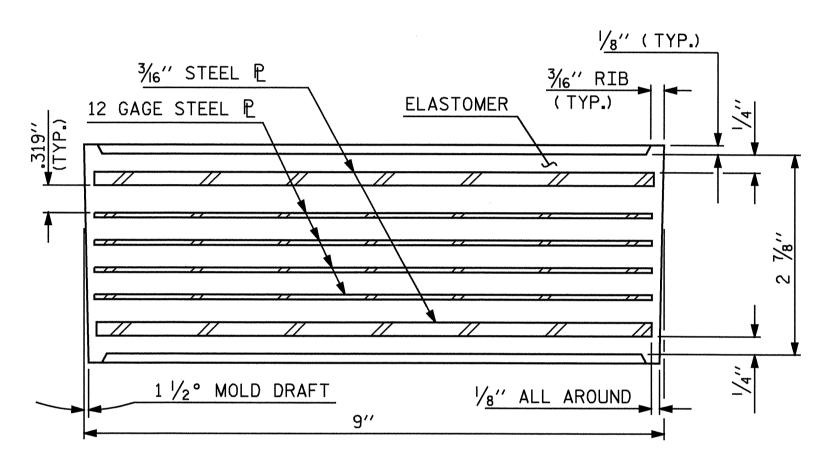
STRUCTURAL STEEL DETAILS

**REVISIONS** SHEET NO. S-9 NO. BY: DATE: DATE: TOTAL SHEETS 22

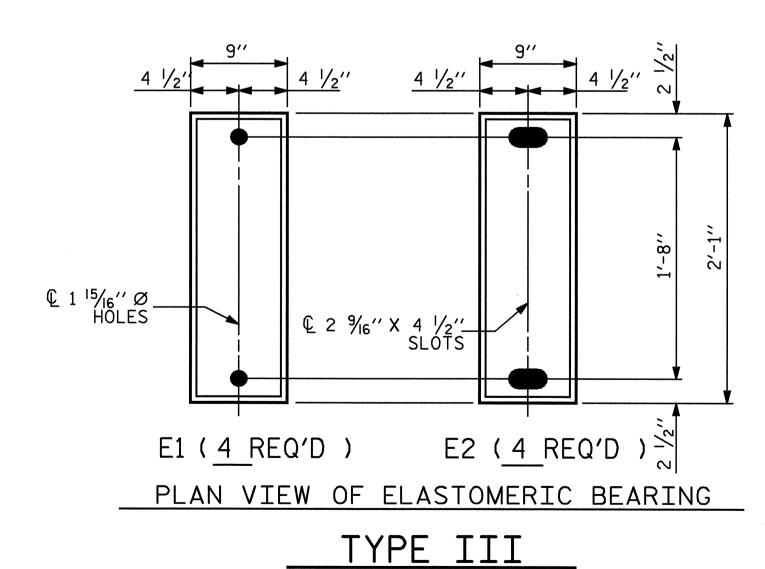
BOLTED FIELD SPLICE DETAILS

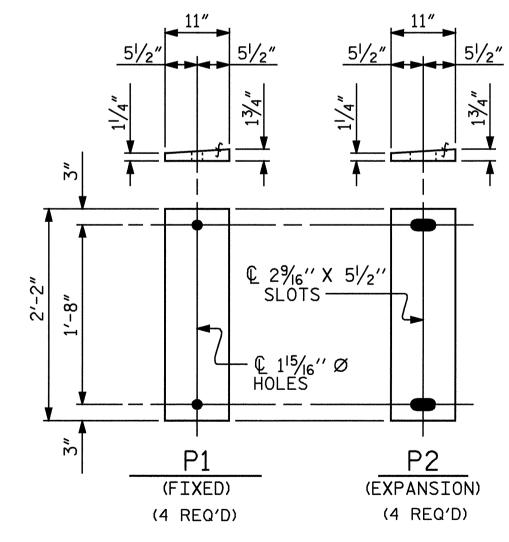
DRAWN BY: KEITH D. LAYNE DATE: 03/07
CHECKED BY: M. K. BEARD DATE: 07/07





TYPICAL SECTION OF ELASTOMERIC BEARINGS





SOLE PLATE DETAILS ( "P")

#### NOTES

AT ALL FIXED POINTS OF SUPPORT, NUTS FOR ANCHOR BOLTS ARE TO BE TIGHTENED FINGER TIGHT AND THEN BACKED OFF 1/2 TURN. THE THREAD OF THE NUT AND BOLT SHALL THEN BE BURRED WITH A SHARP POINTED TOOL.

THE 2" Ø PIPE SLEEVE SHALL BE CUT FROM SCHEDULE 40 PVC PLASTIC PIPE. THE PVC PLASTIC PIPE SHALL MEET THE REQUIREMENTS OF ASTM D1785.

THE PAYMENT FOR THE PIPE SLEEVES SHALL BE INCLUDED IN THE SEVERAL PAY ITEMS.

FOR AASHTO M270 GRADE 50W STRUCTURAL STEEL, SOLE PLATE SHALL BE AASHTO M270 GRADE 50W AND SHALL NOT BE GALVANIZED. ANCHOR BOLTS, NUTS AND WASHERS SHALL BE GALVANIZED IN ACCORDANCE WITH THE STANDARD SPECIFICATIONS.

ANCHOR BOLTS SHALL MEET THE REQUIREMENTS OF ASTM A449. NUTS SHALL MEET THE REQUIREMENTS OF AASHTO M291-DH OR AASHTO M292-2H. WASHERS SHALL MEET THE REQUIREMENTS OF AASHTO M293. SHOP DRAWINGS ARE NOT REQUIRED FOR ANCHOR BOLTS, NUTS AND WASHERS. SHOP INSPECTION IS REQUIRED.

WHEN FIELD WELDING THE SOLE PLATE TO THE GIRDER FLANGE, USE TEMPERATURE INDICATING WAX PENS, OR OTHER SUITABLE MEANS, TO ENSURE THAT THE TEMPERATURE OF THE SOLE PLATE DOES NOT EXCEED 300°F. TEMPERATURES ABOVE THIS MAY DAMAGE THE ELASTOMER.

ALL SURFACES OF BEARING PLATES SHALL BE SMOOTH AND STRAIGHT.

-LOAD RA	ATINGS-
	MAX.D.L.+L.L.
TYPE III	144 K

PROJECT NO. B-3661

HAYWOOD COUNTY

STATION: 16+65.00 -L-

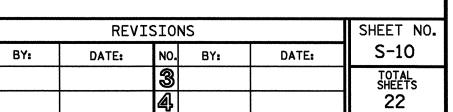
DEPARTMENT OF TRANSPORTATION
RALEIGH

STANDARD

ELASTOMERIC BEARING

DETAILS

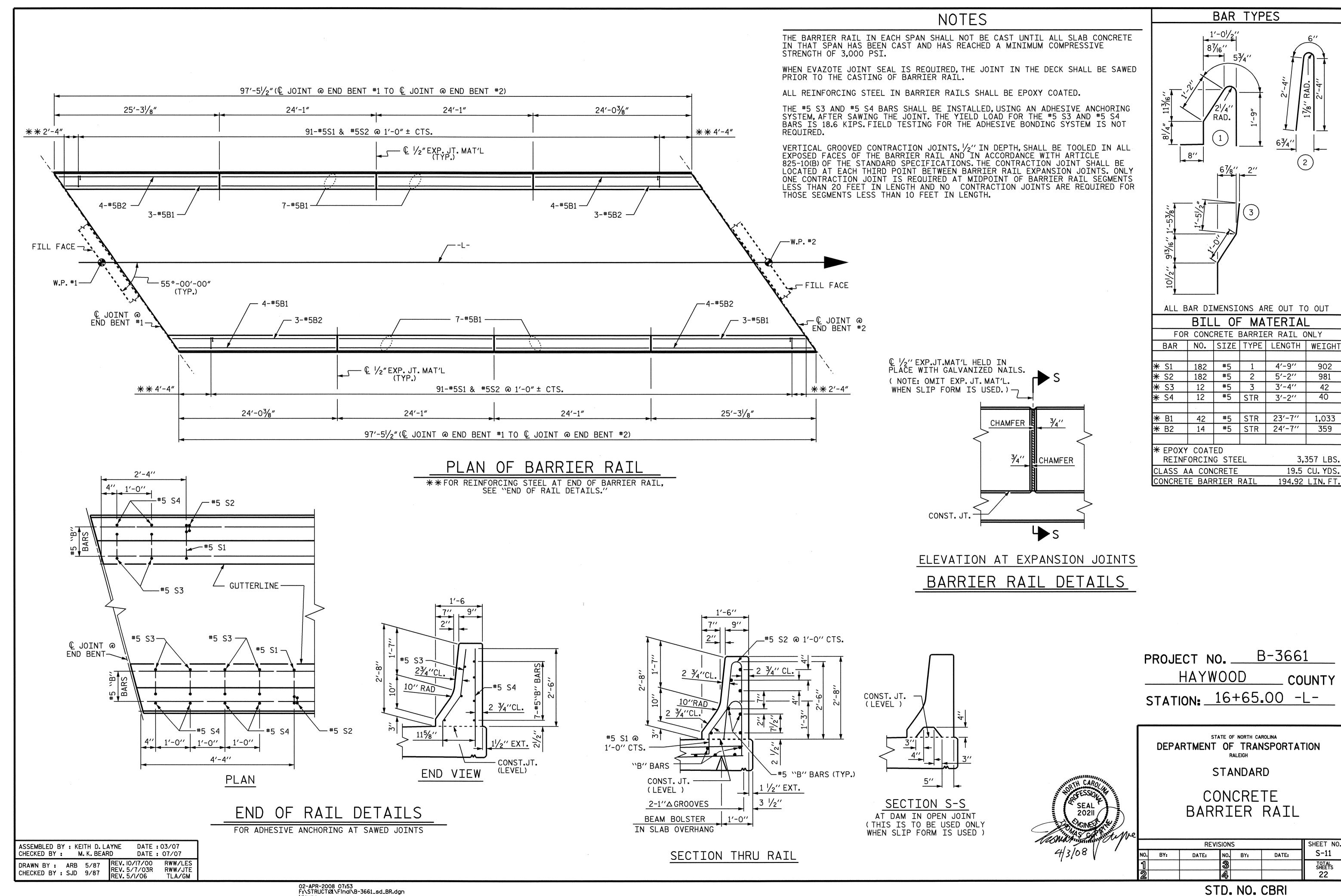
(STEEL SUPERSTRUCTURE)

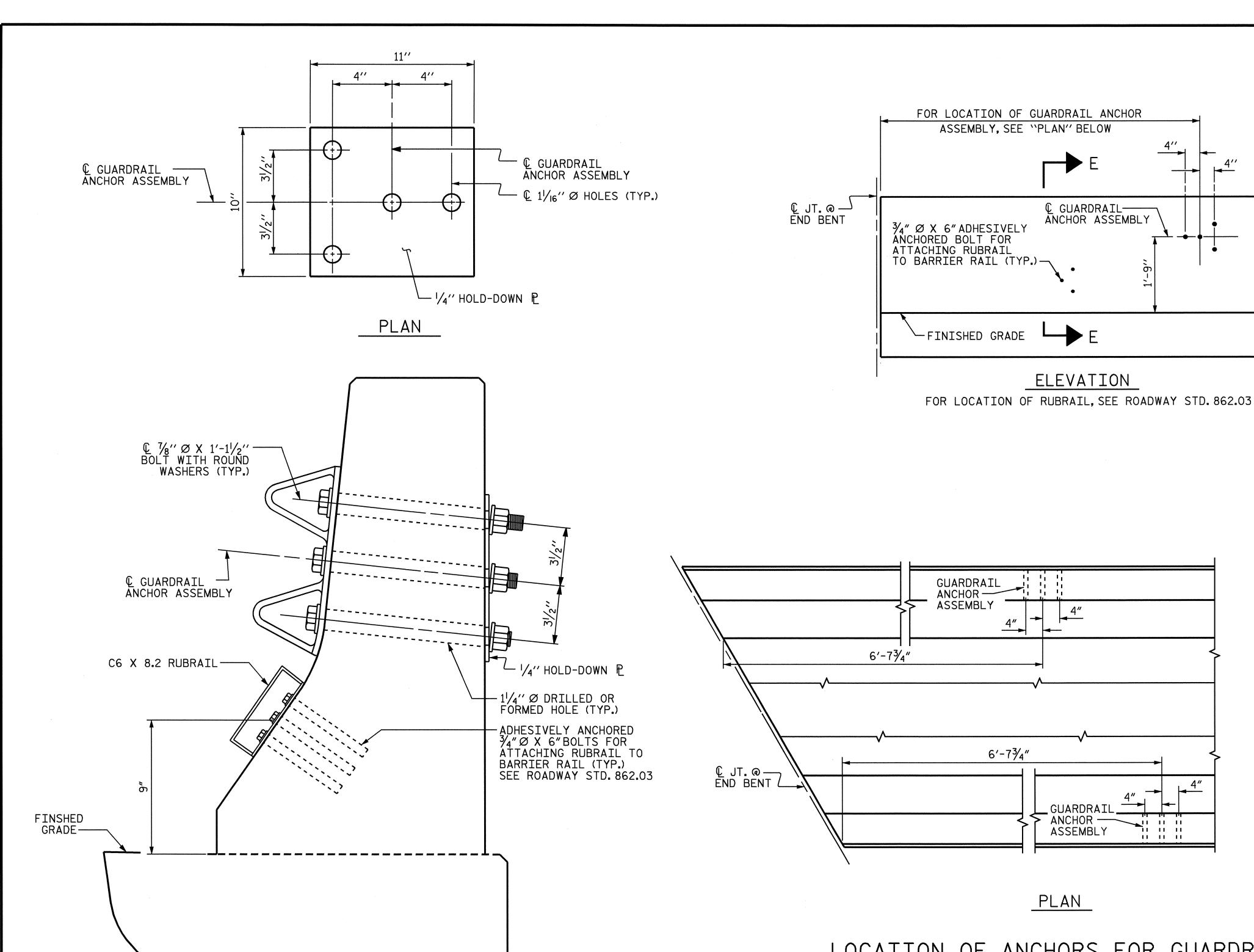


ASSEMBLED BY: KEITH D. LAYNE DATE: 3/07 CHECKED BY: M. K. BEARD DATE: 07/07

DRAWN BY: JMB II/87 REV. 7/17/98 RWW/LES REV. 8/16/99 MAB/LES REV. 10/17/00 RWW/LES

02-APR-2008 07:59 F:\STRUCTØI\Final\B-3661\_sd\_BG.dgn klayne





LOCATION OF ANCHORS FOR GUARDRAIL

END BENT #1 SHOWN, END BENT #2 SIMILAR.

#### NOTES

THE GUARDRAIL ANCHOR ASSEMBLY SHALL CONSIST OF A  $\frac{1}{4}$ " HOLD DOWN PLATE AND 4 -  $\frac{7}{8}$ " Ø BOLTS WITH NUTS AND WASHERS, RUBRAIL, AND ADHESIVELY ANCHORED BOLTS.

THE HOLD-DOWN PLATE SHALL CONFORM TO AASHTO M270 GRADE 36. AFTER FABRICATION, THE HOLD-DOWN PLATE SHALL BE HOT-DIP GALVANIZED IN ACCORDANCE WITH AASHTO M111.

BOLTS SHALL CONFORM TO THE REQUIREMENTS OF ASTM A307 AND NUTS SHALL CONFORM TO THE REQUIREMENTS OF AASHTO M291. BOLTS, NUTS AND WASHERS SHALL BE GALVANIZED. (AT THE CONTRACTOR'S OPTION, STAINLESS STEEL BOLTS, NUTS AND WASHERS MAY BE USED AS AN ALTERNATE FOR THE 1/8" Ø GALVANIZED BOLTS, NUTS AND WASHERS. THEY SHALL CONFORM TO OR EXCEED THE MECHANICAL REQUIREMENTS OF ASTM A307. THE USE OF THIS ALTERNATE SHALL BE APPROVED BY THE ENGINEER.)

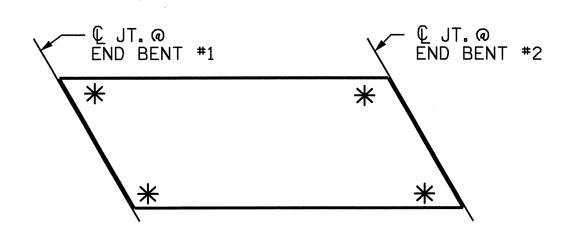
THE GUARDRAIL ANCHOR ASSEMBLY IS REQUIRED AT ALL POINTS WHERE APPROACH GUARDRAIL IS TO BE ATTACHED TO THE END OF BARRIER RAIL. FOR POINTS OF ATTACHMENT, SEE SKETCH.

AFTER INSTALLATION, THE EXPOSED THREAD OF THE BOLT SHALL BE BURRED WITH A SHARP POINTED TOOL.

THE COST OF THE GUARDRAIL ANCHOR ASSEMBLY SHALL BE INCLUDED IN THE UNIT CONTRACT PRICE BID FOR CONCRETE BARRIER RAIL.

THE 1  $\frac{1}{4}$ " Ø HOLES SHALL BE FORMED OR DRILLED WITH A CORE BIT. IMPACT TOOLS WILL NOT BE PERMITTED. ANY CONCRETE DAMAGED BY THIS WORK SHALL BE REPAIRED TO THE SATISFACTION OF THE ENGINEER.

THE C6 X 8.2 RUBRAIL IS TO BE ADHESIVELY ANCHORED TO THE RAIL USING THREE 3/4" Ø X 6"BOLTS WITH WASHERS. LEVEL ONE TESTING IS REQUIRED, AND THE YIELD LOAD OF THE 3/4"BOLT IS 12 KIPS, FOR ADHESIVELY ANCHORED ANCHOR BOLTS OR DOWELS, SEE SPECIAL PROVISIONS. SEE ROADWAY STANDARD 862.03 FOR DETAILS AND LOCATION OF THE RUBRAIL.



SKETCH SHOWING POINTS OF ATTACHMENTS

\* DENOTES GUARDRAIL ANCHOR ASSEMBLY

PROJECT NO. <u>B-3661</u> HAYWOOD \_ COUNTY STATION: 16+65.00 -L-

> STATE OF NORTH CAROLINA DEPARTMENT OF TRANSPORTATION

> > STANDARD

GUARDRAIL ANCHORAGE



FOR BARRIER RAIL

SHEET NO. REVISIONS S-12 BY:

ASSEMBLED BY: KEITH D. LAYNE DATE: 03/07

ADDED 5/I/06 KMM/GM

CHECKED BY : M. K. BEARD

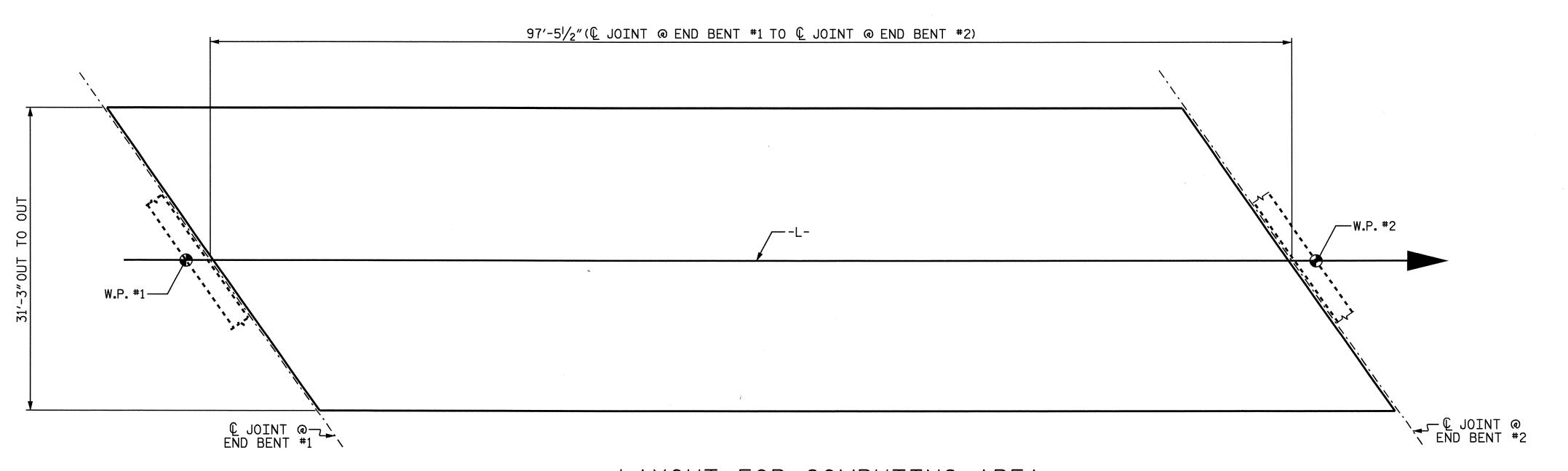
DRAWN BY: TLA 5/06 CHECKED BY : GM 5/06 SECTION E-E

GUARDRAIL ANCHOR ASSEMBLY DETAILS

	ENGTH	S ARE	BASED	ON TH	S STEEL E LENGTHS
BAR SIZE	SUPERSTE EXCEPT A SLABS, P AND BARR	APPROACH ARAPET,	APPROAC	H SLABS	PARAPET AND BARRIER
	EPOXY COATED	UNCOATED	EPOXY COATED	UNCOATED	RAIL
#4	2′-0″	1'-9"	2′-0″	1'-9"	2′-9″
#5	2′-6″	2′-2″	2′-6″	2′-2″	3′-5″
#6	3′-0″	2′-7″	3′-10″	2′-7″	4'-4"
#7	5′-3″	3′-6″			
#8	6′-10″	4′-7″			

GROOVING	BRIDGE FLOORS
APPROACH SLABS	671 SQ. FT.
BRIDGE DECK	2396 SQ. FT.
TOTAL	3067_SQ.FT.

				— BIL	L OF N	ИАТЕ	RIA						ВА	R TYPES-	
BAR	NO.	SIZE	TYPE	LENGTH	WEIGHT	BAR	NO.	SIZE	TYPE	LENGTH	WEIGHT				
													· ·	4'-10"	
<b>∗</b> A1	129	#5	STR	30′-11″	4,160	A211	4	#5	STR	12'-11"	54				
						A212	4	#5	STR	11'-3"	47		<u> </u>		***************************************
<b>*</b> A101	4	#5	STR	29′-7″	123	A213	4	#5	STR	9'-7"	40	0.1	THIS LEG		_
* A102	4	#5	STR	27′-11″	116	A214	4	#5	STR	7′-11″	33	U	/ER GIRDER	J ( 1	) »õ
* A103	4	#5	STR	26′-3″	110	A215	4	#5	STR	6'-3"	26				
* A104	4	#5	STR	24′-7″	103	A216	4	#5	STR	4'-7"	19				
* A105	4	#5	STR	22'-11"	96	A217	4	#5	STR	2'-11"	12		1'-	-10"	2'-6"
* A106	4	#5	STR	21′-3″	89	A218	4	#5	STR	1′-3″	5				
* A107	4	#5	STR	19'-7"	82							<del> </del>			
<b>≭</b> A108	4	#5	STR	<u> 17'-11"</u>	75	* B1	88	#4	STR	25′-9″	1,514	<b>"</b> 6			
* A109	4	#5	STR	<u> 16'-3"</u>	68	B2	70	#5	STR	49′-7″	3,620	6		·/	
* A110	4	#5	STR	14'-7"	61	-						<u> </u>			
* A111	4	#5	STR	12'-11"	54	<b>★</b> G1	2	#5	STR	37′-9″	79	2	·′-6″	2'-6"	
* A112	4	#5	STR	11'-3"	47							-			
* A113	4	#5	STR	9′-7″	40	* K1	12	#5	1	8'-1"	101		. 6′′ .	CII	
* A114	4	#5	STR	7'-11"	33	* K2	12	#5	2	8'-4"	104		<b> </b>	<del>-6" </del>	
* A115	4	#5	STR	6'-3"	26	<b></b> ★ K3	18	#5	STR	9'-5"	177			<u> </u>	
* A116	4	#5	STR	4'-7"	19					4. 4. 4.					
* A117	4	#5	STR	2'-11"	12	* S1	42	#4	3	4'-10"	136		1 (3	)   "6	
* A118	4	#5	STR	1′-3″	5		NEOD (	NTNO C			070 1 00			´   ↓	
40	100	4.5	CTD	70/ 11//	4.460	KFT	NF OR	CING S	<u> IEEL</u>	8	,939 LBS				
A2	129	#5	STR	30′-11″	4,160	<u>ا</u> ا	DOVV	~^ A T E !					2'-4	"	
4001	4	#-	CTD	00/ 7//	107			COATE		7	470 LDC				
A201	4	#5 #5	STR STR	29'-7" 27'-11"	123 116	KE	TINL OF	RCING	SIEEL	· · · · · · · · · · · · · · · · · · ·	,430 LBS		ALL BAR DIMEN	SIONS ARE OUT	TO OUT
A202 A203	4	#5	STR	26'-3"	110										
A203	7	#5	STR	26-3 24'-7"	103							SUPER	2 I KUC I UK	F RILL OF	MATERIAL —
A204	4	#5	STR	22'-11"	96					What the second			CLASS AA	REINFORCING	EPOXY COATED
A205	4	#5	STR	21'-3"	89								CLASS AA CONCRETE	STEEL	REINFORCING
A207	4	#5	STR	19'-7"	82										STEEL
A208	4	#5	STR	17'-11"	75		<del></del>						( CU.YDS.)	( LBS.)	(LBS.)
A209	4	#5	STR	16'-3"	68							SPAN "A"	96.4	8,939	7,430
A210	4	#5	STR	14'-7"	61										
,,,,,	•		- 111	- · · ·							***************************************	TOTALS**	96.4	8,939	7,430
												**QUANTIT	IES FOR BARRI	ER RAIL ARE NOT	INCLUDED



LAYOUT FOR COMPUTING AREA
OF REINFORCED CONCRETE DECK SLAB
(SQ.FT. = 3046)

PROJECT NO. B-3661 HAYWOOD \_\_\_ COUNTY STATION: 16+65.00 -L-

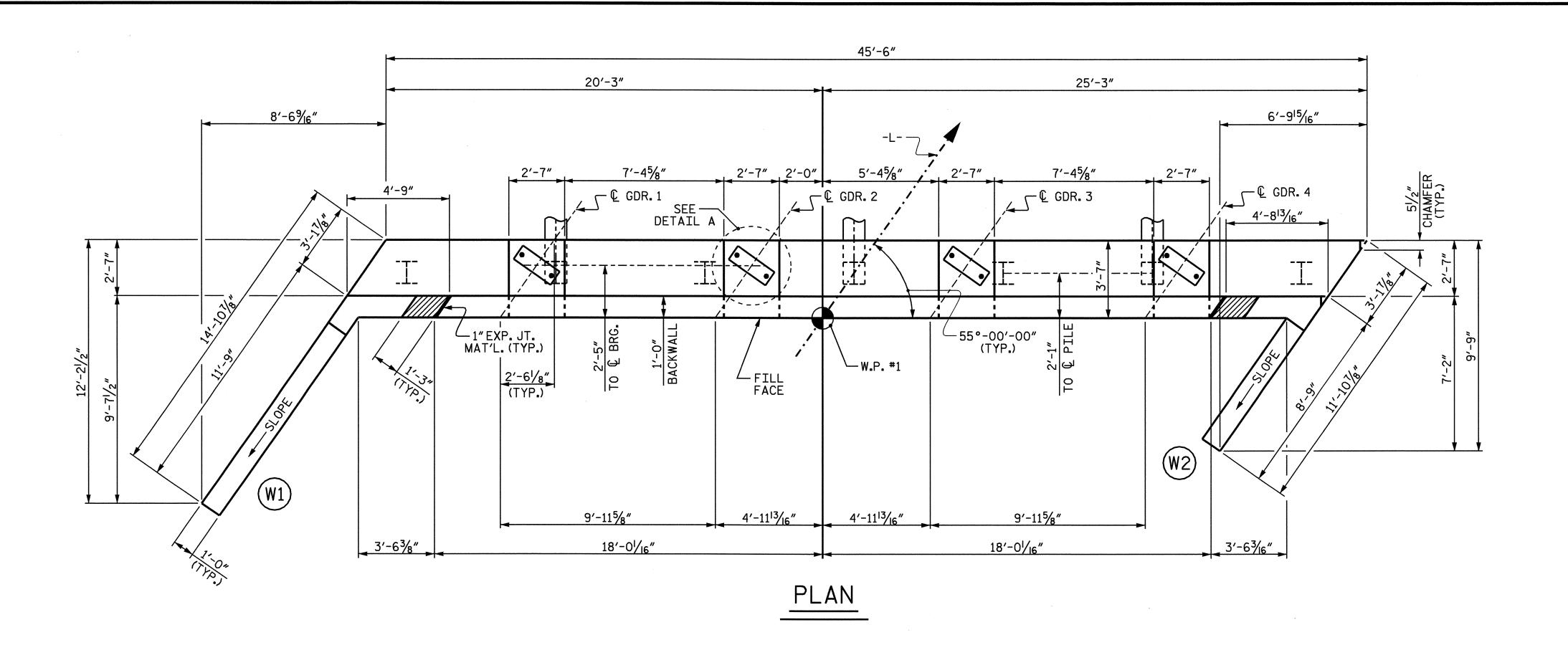
STATE OF NORTH CAROLINA DEPARTMENT OF TRANSPORTATION RALEIGH

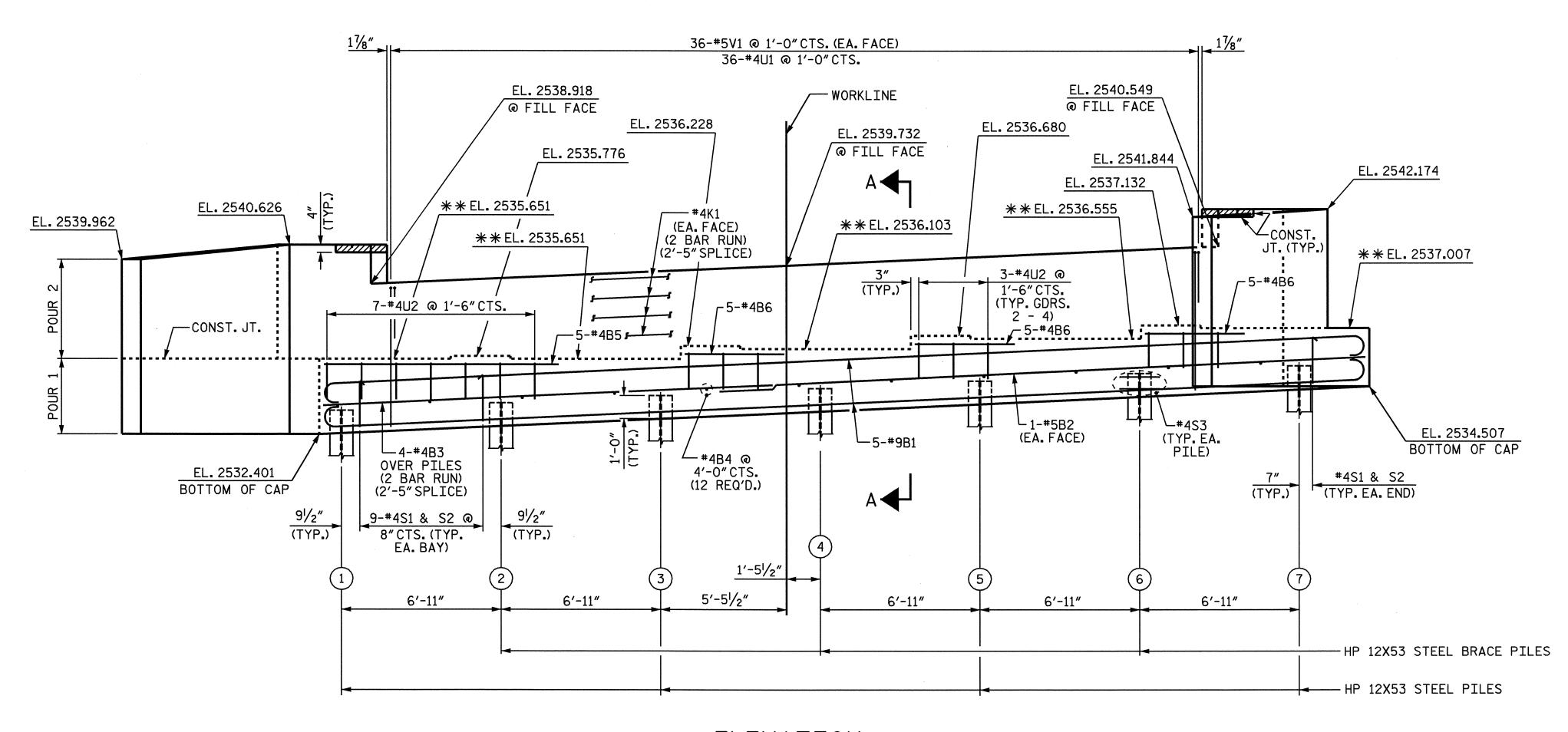
SUPERSTRUCTURE BILL OF MATERIAL

		REV	ISION	S		SHEET NO.
NO.	BY:	DATE:	NO.	BY:	DATE:	S-13
1			3			TOTAL SHEETS
2			4			22

DRAWN BY: KEITH D. LAYNE DATE: 03/07
CHECKED BY: M. K. BEARD DATE: 07/07

02-APR-2008 07:54 F:\STRUCT@\\Final\B-3661\_sd\_BM.dgn klayne





#### NOTES

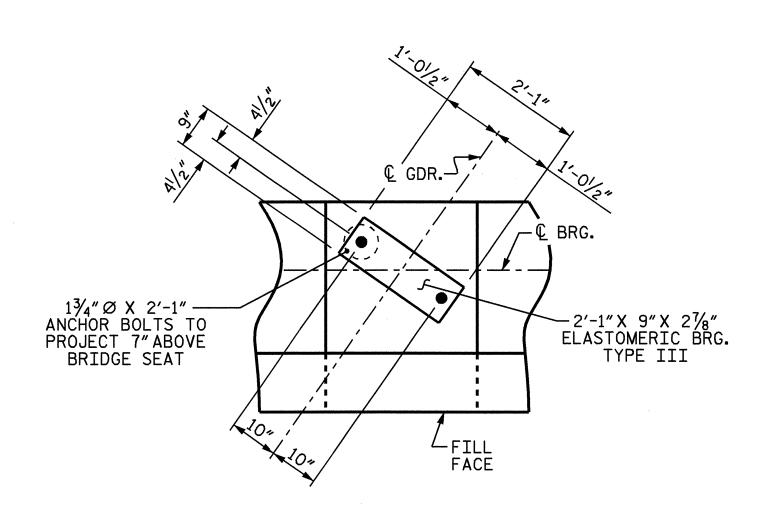
STIRRUPS IN CAP MAY BE SHIFTED AS NECESSARY TO CLEAR ANCHOR BOLTS.

BACKWALL SHALL BE PLACED BEFORE APPLYING THE EPOXY PROTECTIVE COATING.

THE TOP SURFACE OF THE CAP EXCEPT THE BRIDGE SEAT BUILDUPS SHALL BE SLOPED TRANSVERSELY FROM THE FILL FACE TO THE BACK FACE AT THE RATE OF 2%.

THE CONCRETE IN THE SHADED AREA OF THE WING SHALL BE POURED AFTER THE JOINT BETWEEN THE DECK AND THE APPROACH SLAB HAS BEEN SAWED AND THE BARRIER RAIL IS CAST IF SLIP FORMING IS USED.

THE CONTRACTOR SHALL PROVIDE FOR INSTALLATION OF THE 4"DIAMETER DRAIN PIPE THROUGH THE WING WALL AS REQUIRED FOR REINFORCED BRIDGE APPROACH FILLS, SEE THE ROADWAY PLANS. REINFORCING STEEL IN THE WING WALL MAY BE SHIFTED AS NECESSARY TO CLEAR THE DRAIN PIPE



DETAIL A

(TYP.EACH GIRDER)

PILE No.	TOP OF PILE EL.
1	2533.494
2	2533 <b>.</b> 814
3	2534.134
4	2534.454
5	2534.775
6	2535.095
7	2535.415

PROJECT NO. B-3661

HAYWOOD COUNTY

STATION: 16+65.00 -L-

SHEET 1 OF 3

STATE OF NORTH CAROLINA

DEPARTMENT OF TRANSPORTATION

RALEIGH

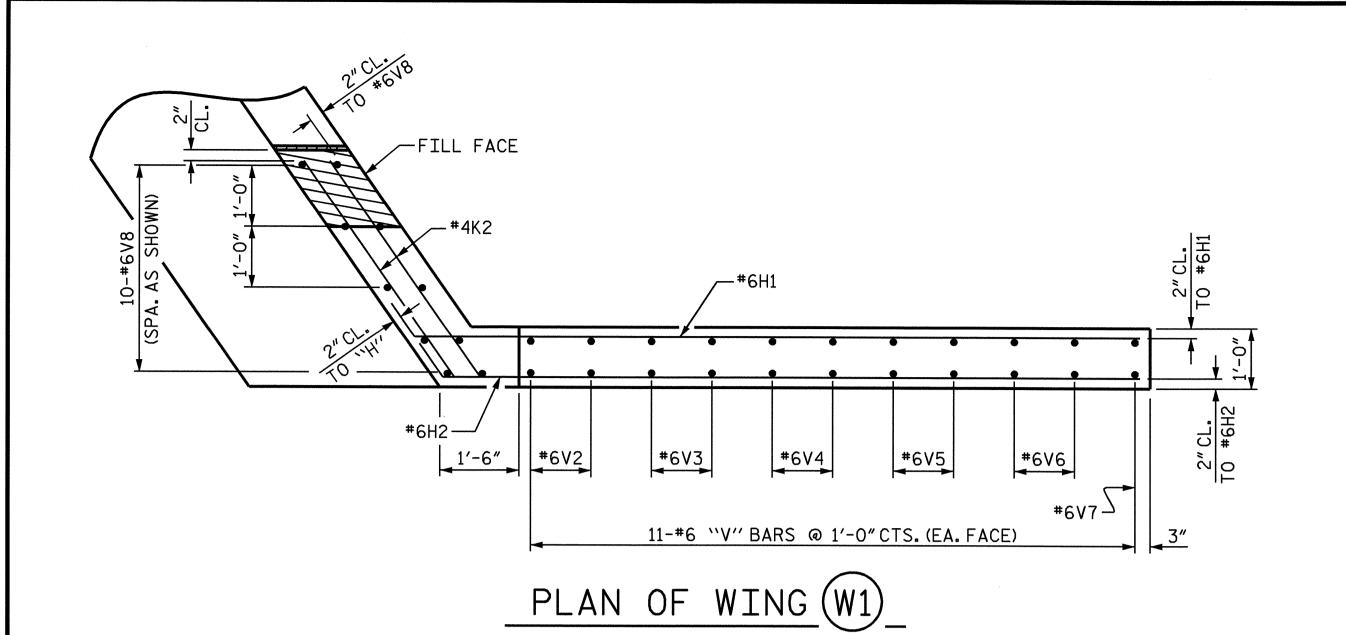
SUBSTRUCTURE

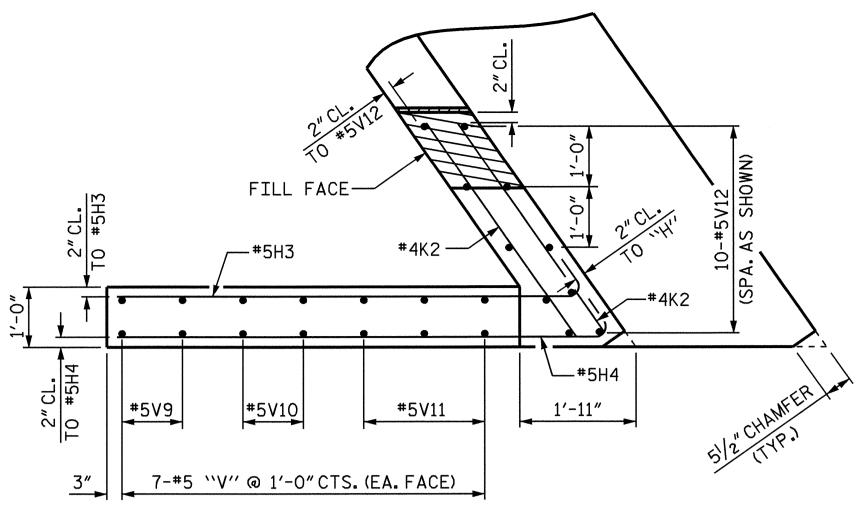
END BENT #1

		REV:	ISION	S		SHEET NO.
NO.	BY:	DATE:	NO.	BY:	DATE:	<b>∏</b> S−14
1			3			TOTAL SHEETS
2			4			22

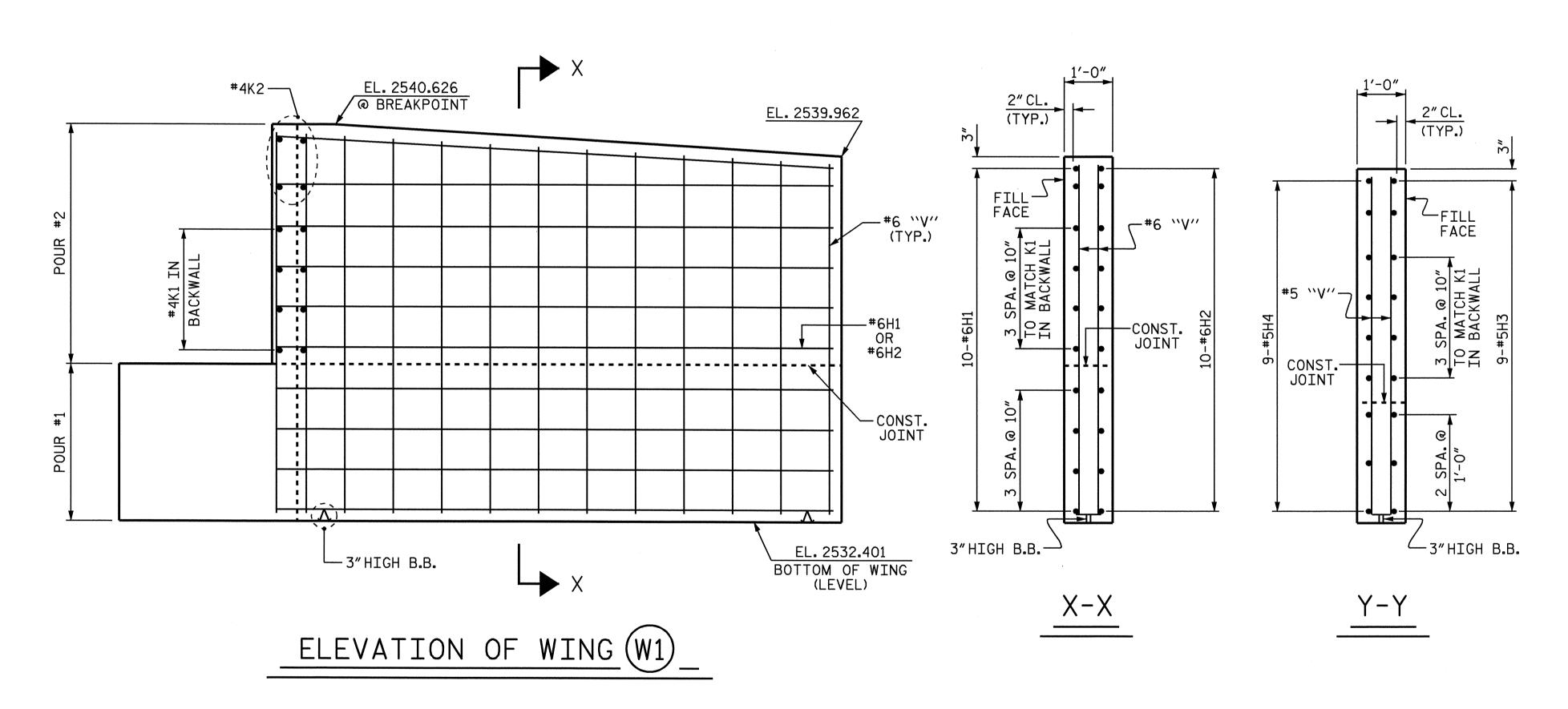
ELEVATION

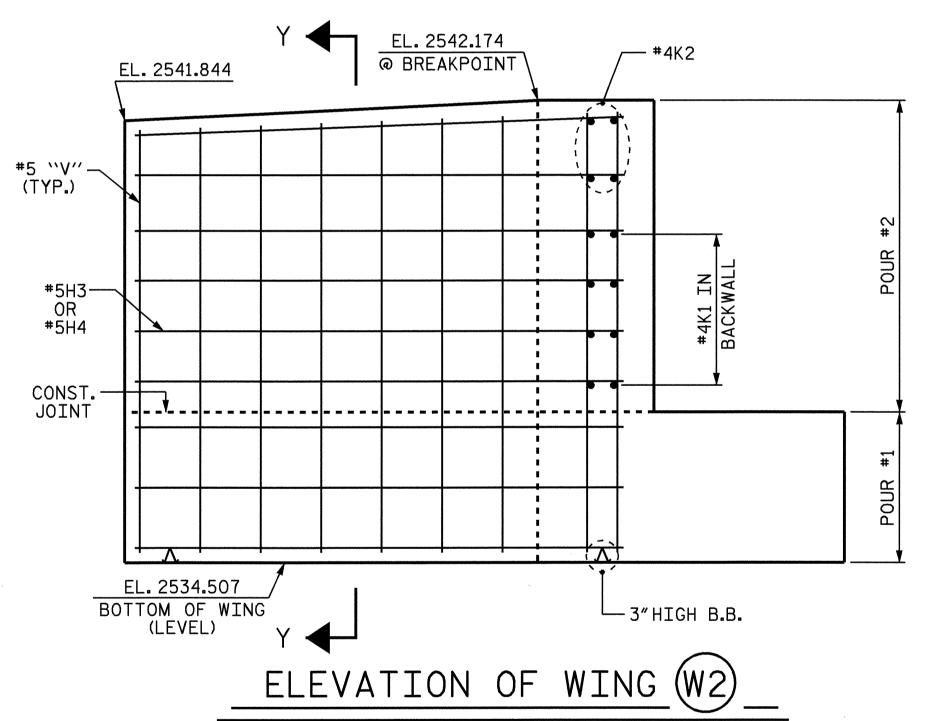
DRAWN BY: M.K. BEARD DATE: 07/31/07
CHECKED BY: K.D. LAYNE DATE: 10/07





# PLAN OF WING (W2)\_





PROJECT NO. B-3661

HAYWOOD COUNTY

STATION: 16+65.00 -L-

SHEET 2 OF 3

SEAL 20211
20211
20211
20211
20211
20211
20211

STATE OF NORTH CAROLINA

DEPARTMENT OF TRANSPORTATION

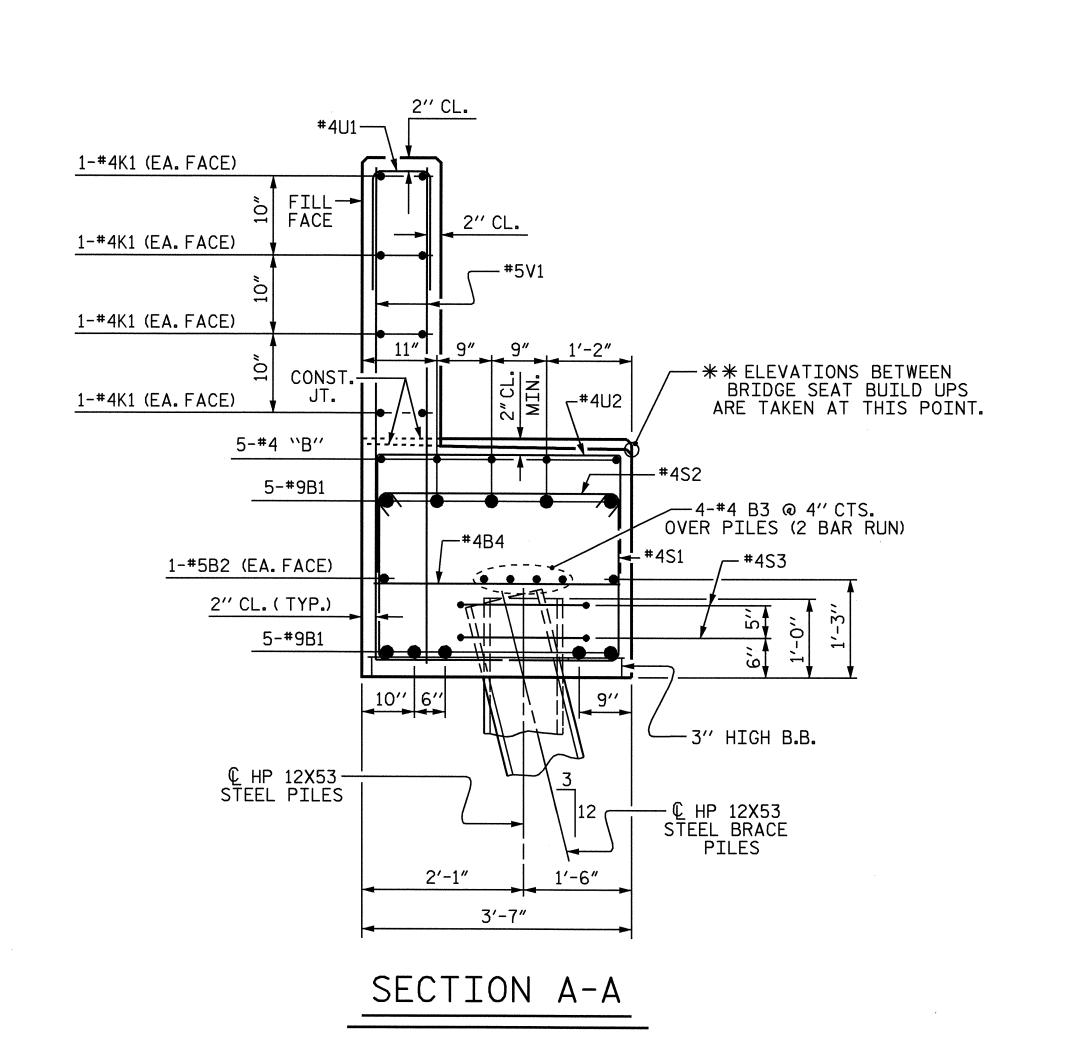
RALEIGH

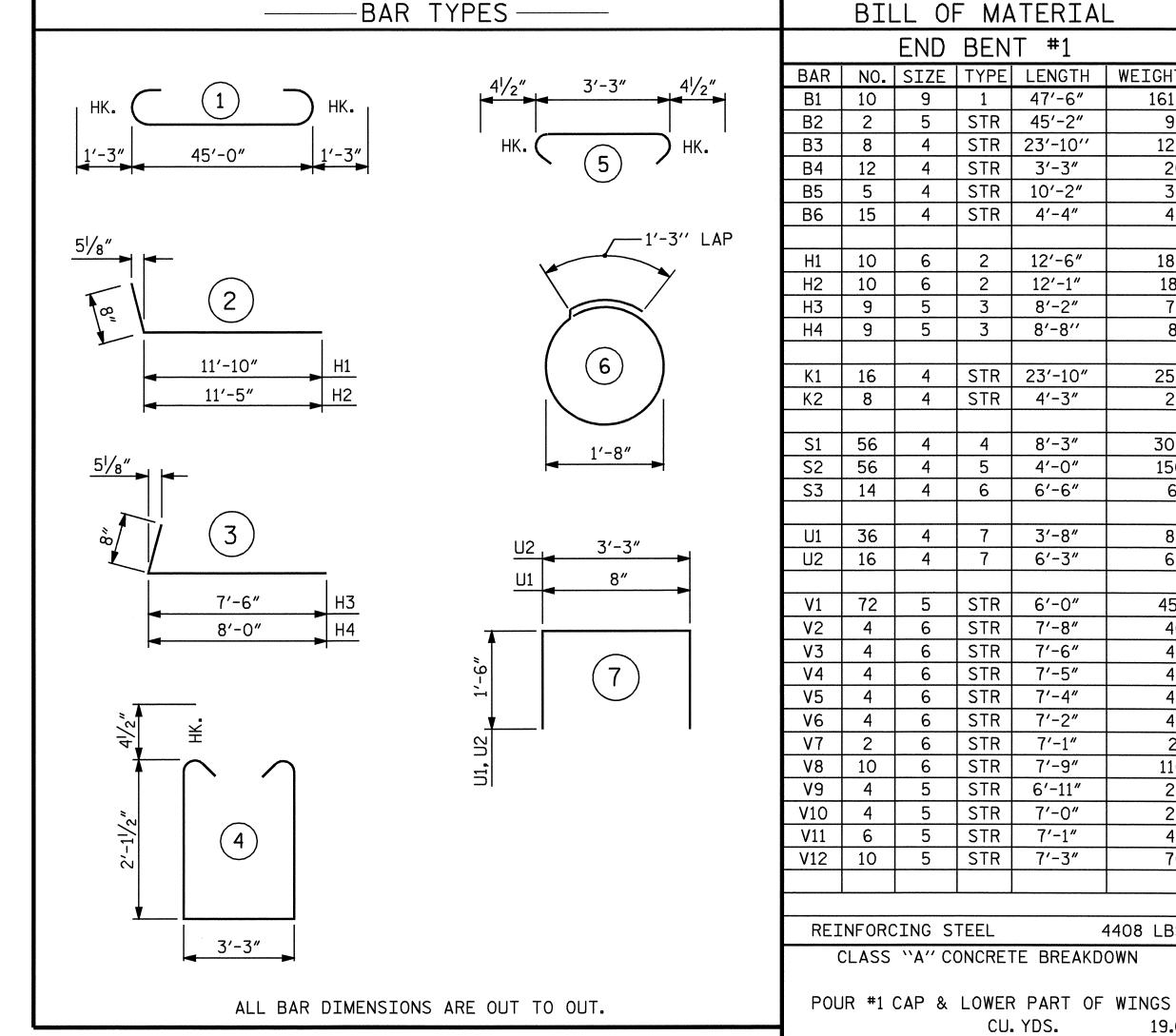
SUBSTRUCTURE

END BENT #1

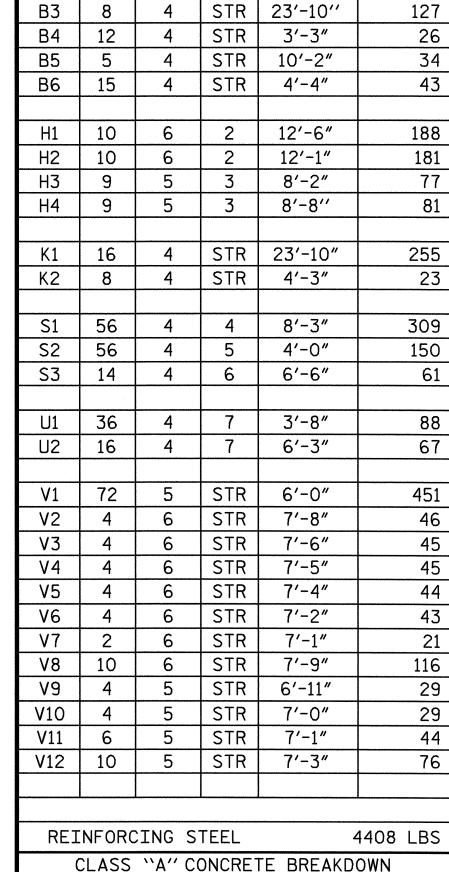
		REV	ISION	S		SHEET NO.
NO.	BY:	DATE:	NO.	BY:	DATE:	∏ S-15
1			3			TOTAL SHEETS
2			4			22

DRAWN BY: M.K. BEARD DATE: 08/02/07
CHECKED BY: K.D. LAYNE DATE: 10/07





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CU. YDS.

CU. YDS.

CU. YDS.

POUR #2 UPPER WINGS & BACKWALL

CLASS "A" CONCRETE TOTAL

HP 12X53 STEEL PILES

No. 7

STEEL PILE POINTS

19.0

29.2

70 LIN. FT.

EACH 7

kbeard

OF MATERIAL

NO. | SIZE | TYPE | LENGTH | WEIGHT

47'-6"

45'-2"

1615

94

END BENT #1

5 STR

kbeard 14-FEB-2008 15:35

BACK GOUGE DETAIL B ≺BACK GOUGE DETAIL A \*PILE <u>VERTICAL</u> PILE HORIZONTAL OR VERTICAL +10° 60°-0° √ TO 1/8" 0" TO 1/8" DETAIL A DETAIL B POSITION OF PILE DURING WELDING. PILE SPLICE DETAILS

\_ DATE : <u>08/03/07</u>

M.K. BEARD

DRAWN BY :

CHECKED BY : K.D. LAYNE

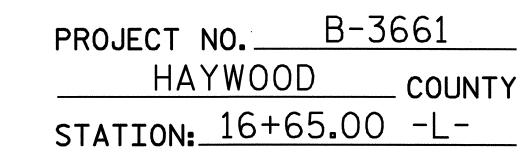
MINIMUM OF 3- ONE CUBIC FOOT BAGS OF #78M STONE. -BAGS SHALL BE OF POROUS FABRIC, SECURELY TIED. 6"(MIN.)PIPE-FOR DRAINAGE ..---------GRADE TO DRAIN - TOE OF SLOPE

BAGGED STONE AND PIPE SHALL BE PLACED IMMEDIATELY AFTER COMPLETION OF END BENT EXCAVATION. PIPE MAY BE EITHER CONCRETE, CORRUGATED STEEL, CORRUGATED ALUMINUM ALLOY, OR CORRUGATED PLASTIC. PERFORATED PIPE WILL NOT BE ALLOWED.

BAGGED STONE SHALL REMAIN IN PLACE UNTIL THE ENGINEER DIRECTS THAT IT BE REMOVED. THE CONTRACTOR SHALL REMOVE AND DISPOSE OF SILT ACCUMULATIONS AT BAGGED STONE WHEN SO DIRECTED BY THE ENGINEER. BAGS SHALL BE REMOVED AND REPLACED WHENEVER THE ENGINEER DETER-MINES THAT THEY HAVE DETERIORATED AND LOST THEIR EFFECTIVENESS.

NO SEPARATE PAYMENT WILL BE MADE FOR THIS WORK AND THE ENTIRE COST OF THIS WORK SHALL BE INCLUDED IN THE UNIT CONTRACT PRICE BID FOR THE SEVERAL PAY ITEMS.

TEMPORARY DRAINAGE AT END BENT



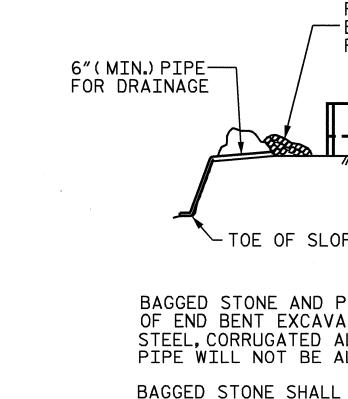
SHEET 3 OF 3

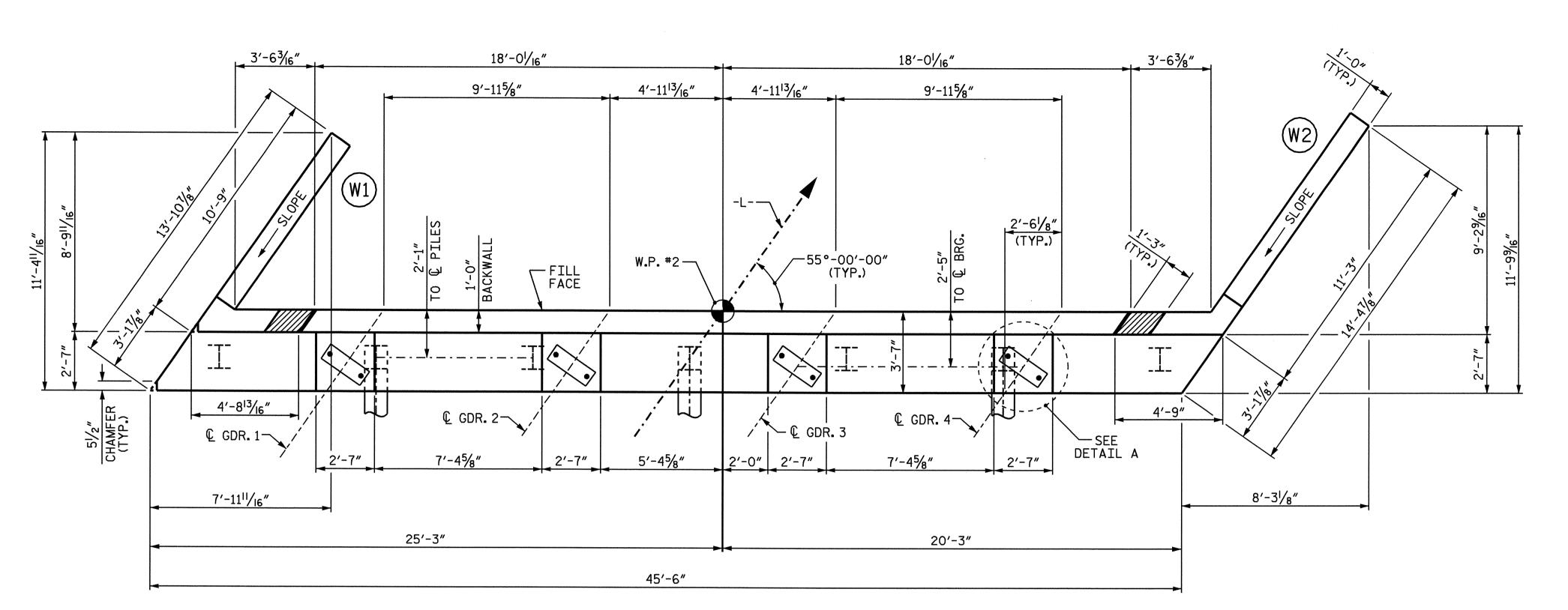
STATE OF NORTH CAROLINA DEPARTMENT OF TRANSPORTATION

SUBSTRUCTURE

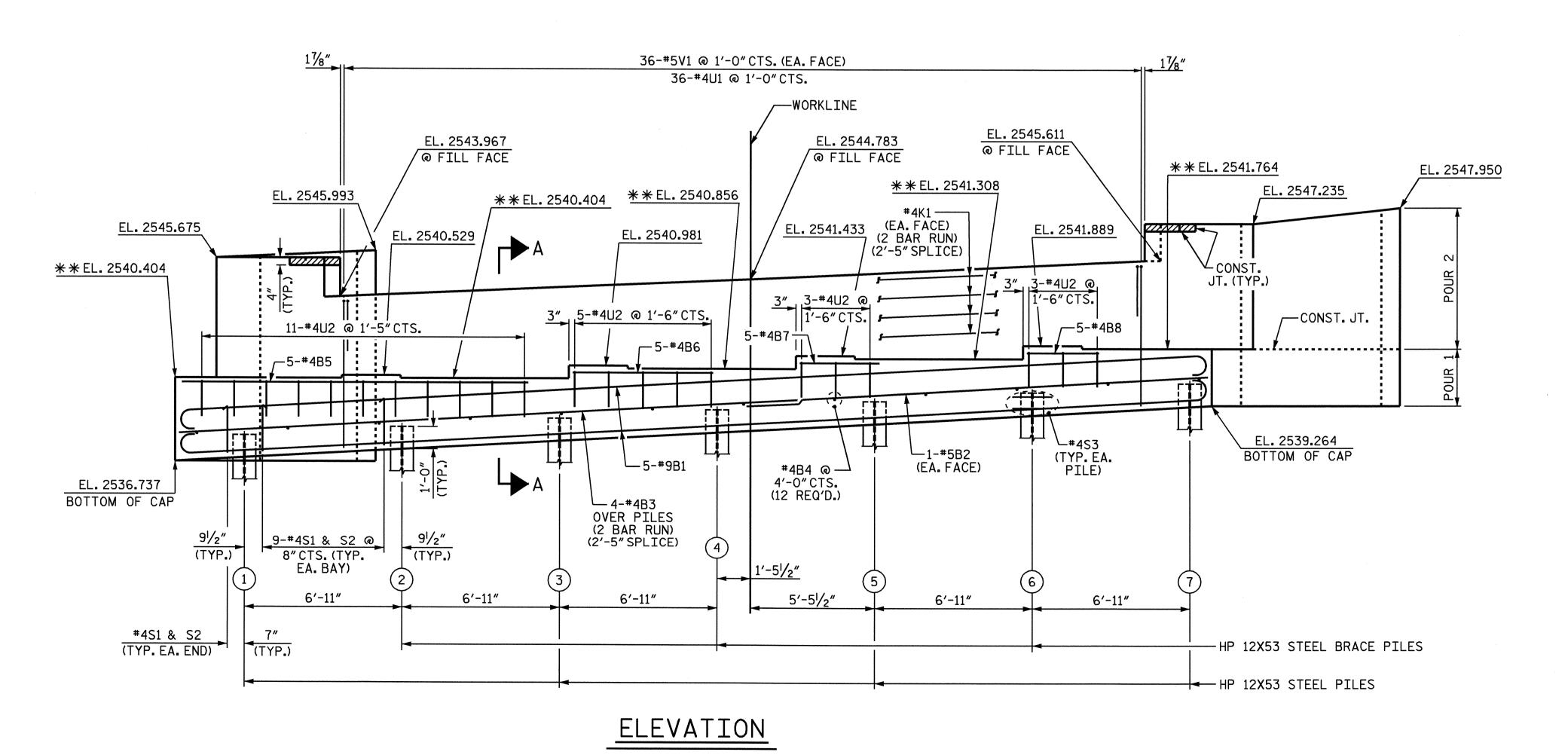
END BENT #1

		SHEET NO.				
NO.	BY:	DATE:	NO.	BY:	DATE:	S-16
1			3			TOTAL SHEETS
2			4			22





#### <u>PLAN</u>



#### NOTES

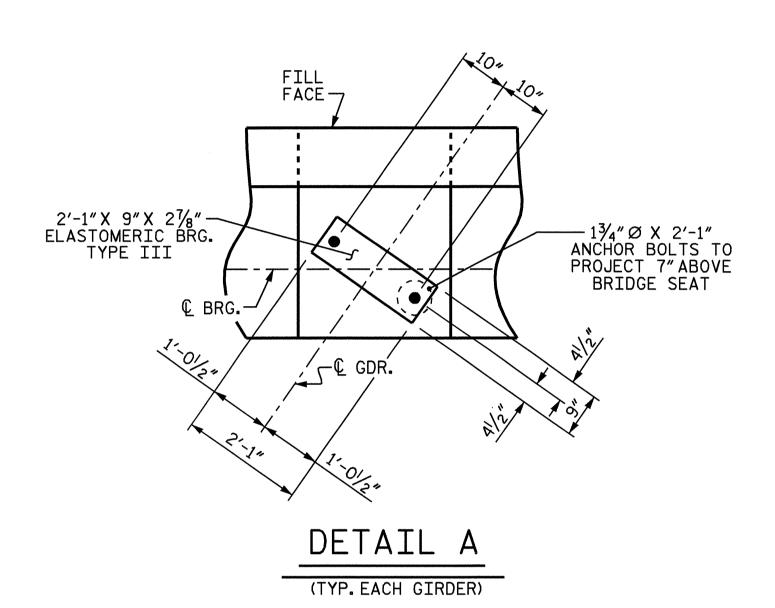
STIRRUPS IN CAP MAY BE SHIFTED AS NECESSARY TO CLEAR ANCHOR BOLTS.

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PILE No.	TOP OF PILE EL.
1	2537.847
2	2538.232
3	2538.616
4	2539.000
5	2539.384
6	2539.768
7	2540.152

PROJECT NO. B-3661

HAYWOOD COUNTY

STATION: 16+65.00 -L-

SHEET 1 OF 3

STATE OF NORTH CAROLINA
DEPARTMENT OF TRANSPORTATION

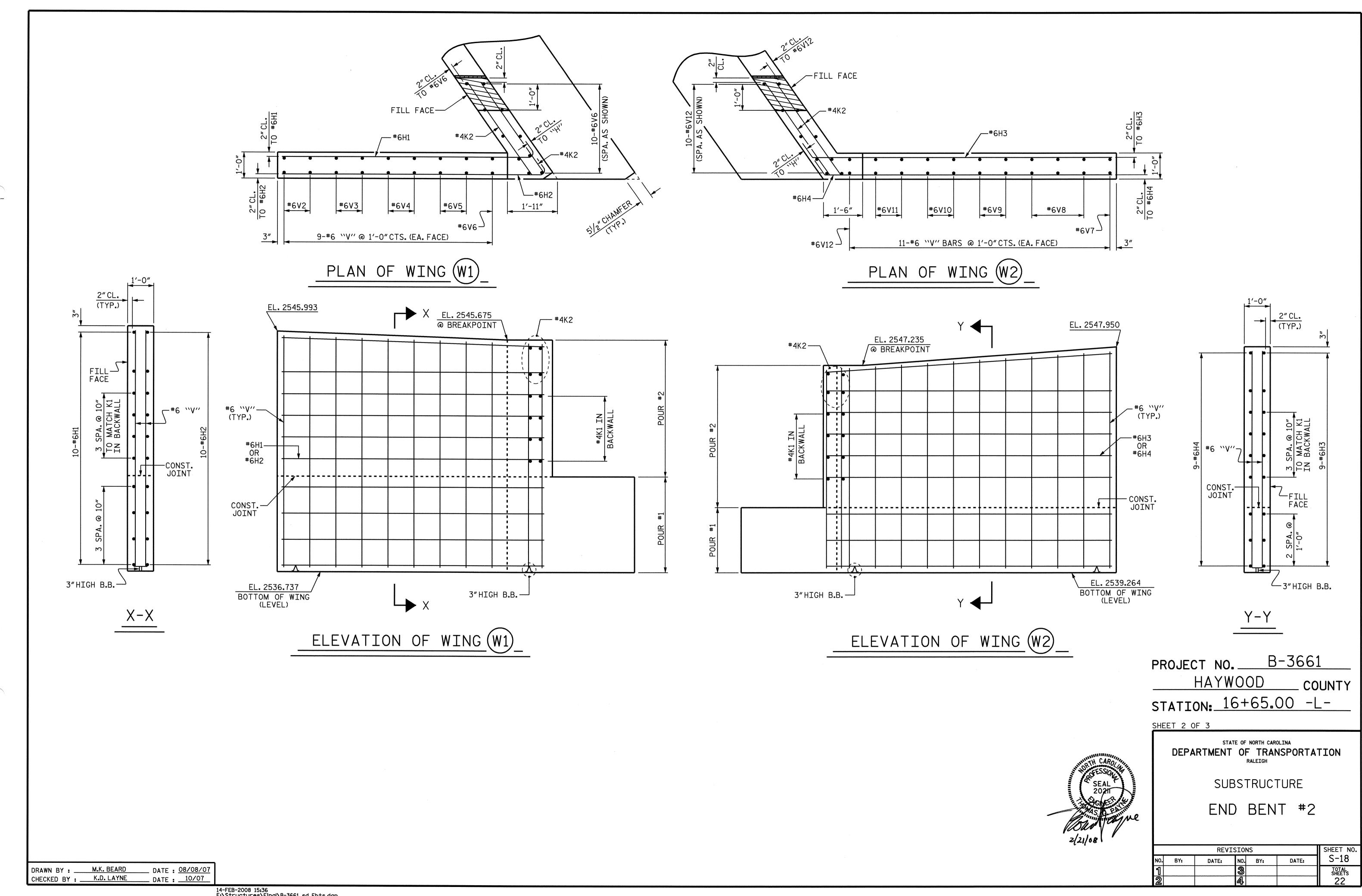
SUBSTRUCTURE

END BENT #2

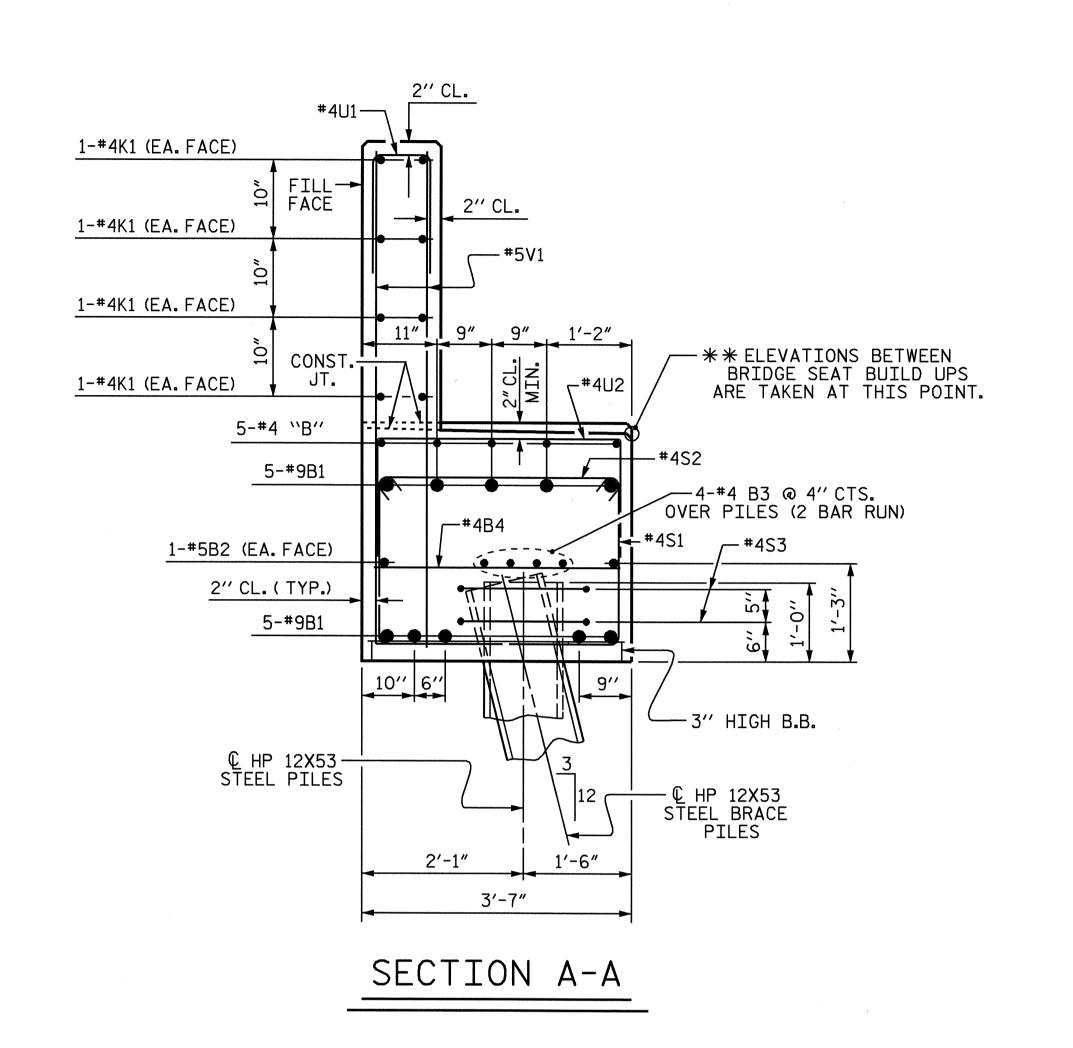
		SHEET NO.				
NO.	BY:	DATE:	NO.	BY:	DATE:	S-17
1			3			TOTAL SHEETS
2			4			22

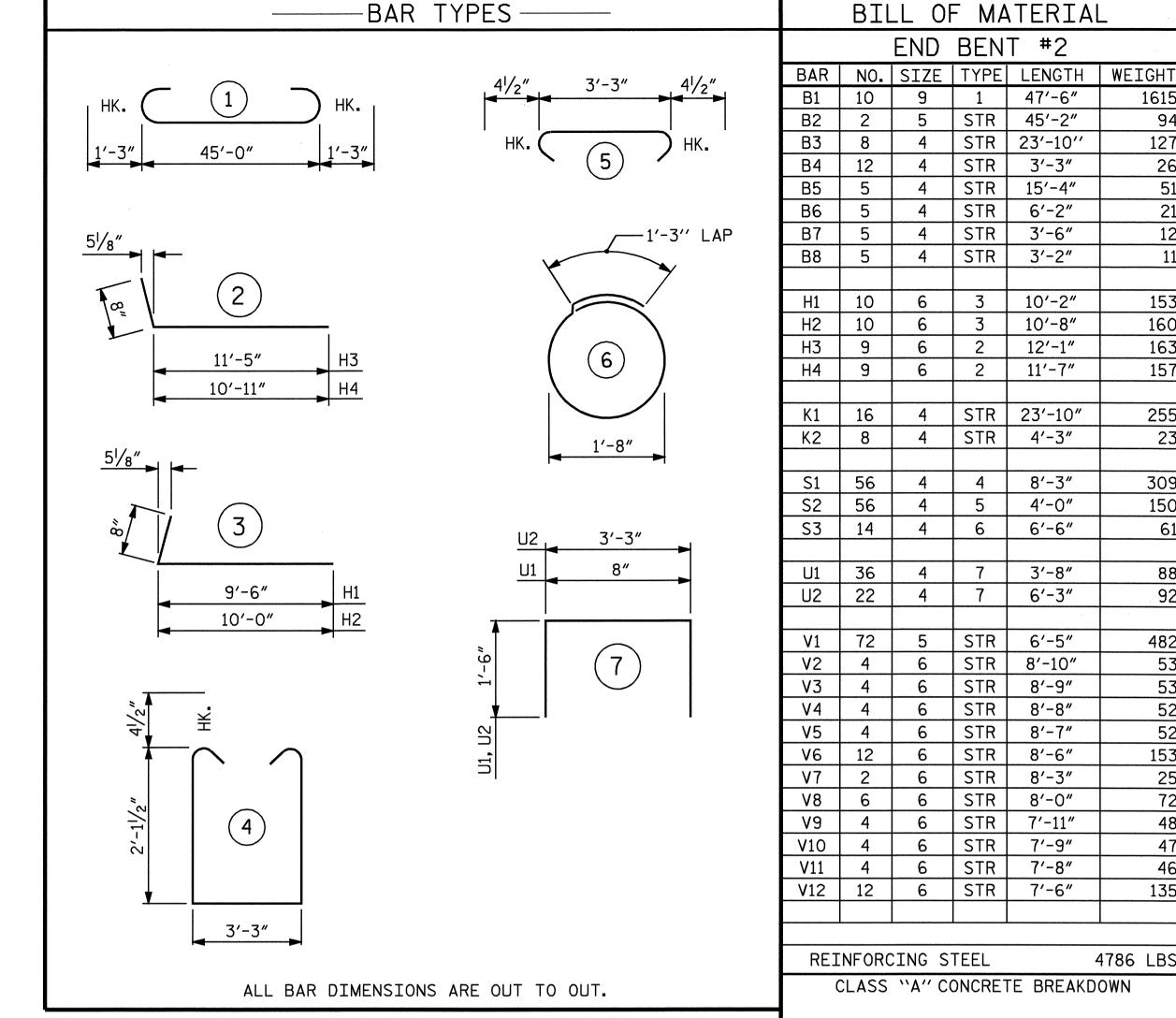
DRAWN BY: M.K. BEARD DATE: 08/07/07
CHECKED BY: K.D. LAYNE DATE: 10/07

14-FEB-2008 15:36
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kbeard

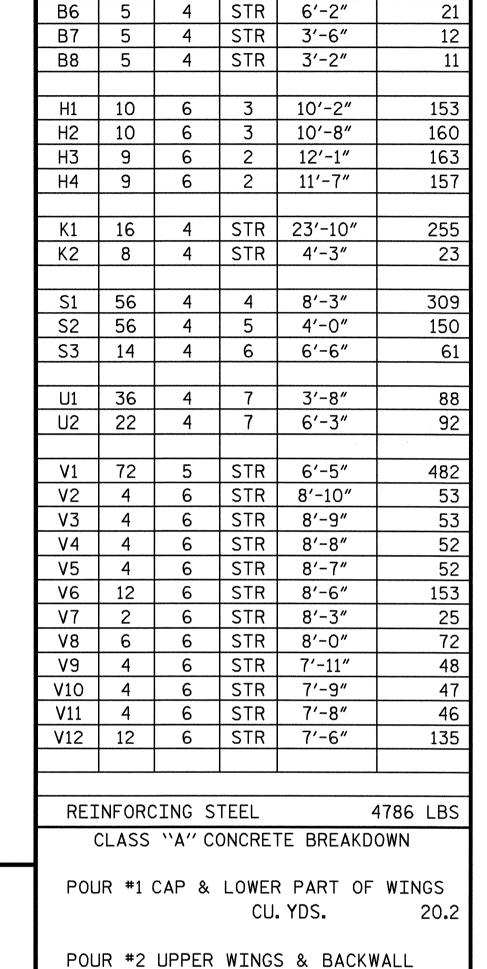


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CU. YDS.

CU. YDS.

30.3

125 LIN. FT.

EACH 7

kbeard

47′-6″

45'-2"

3′-3″

15'-4"

1615

94

127

26

OF MATERIAL

END BENT #2

4 | STR | 23'-10''

5 STR

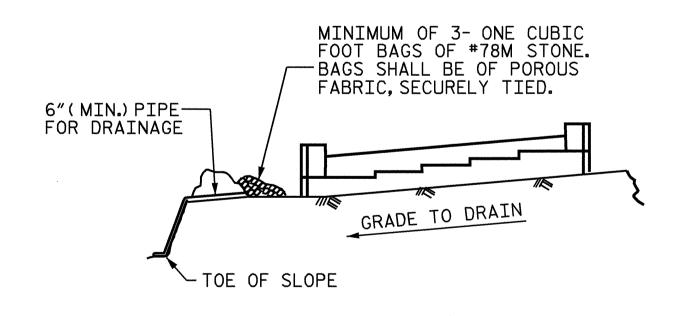
4 STR

4 STR

kbeard 14-FEB-2008 15:36

BACK GOUGE DETAIL B ≺BACK GOUGE DETAIL A PILE VERT<u>ical</u> PILE HORIZONTAL OR VERTICAL 0" TO 1/8" 60° -0° O'' TO 1/8" DETAIL A DETAIL B POSITION OF PILE DURING WELDING.

PILE SPLICE DETAILS



BAGGED STONE AND PIPE SHALL BE PLACED IMMEDIATELY AFTER COMPLETION OF END BENT EXCAVATION. PIPE MAY BE EITHER CONCRETE, CORRUGATED STEEL, CORRUGATED ALUMINUM ALLOY, OR CORRUGATED PLASTIC. PERFORATED PIPE WILL NOT BE ALLOWED.

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NO SEPARATE PAYMENT WILL BE MADE FOR THIS WORK AND THE ENTIRE COST OF THIS WORK SHALL BE INCLUDED IN THE UNIT CONTRACT PRICE BID FOR THE SEVERAL PAY ITEMS.

TEMPORARY DRAINAGE AT END BENT



PROJECT NO. B-366	51
HAYWOOD C	OUNT
STATION: 16+65.00 -	••••

CLASS "A" CONCRETE TOTAL

HP 12X53 STEEL PILES

STEEL PILE POINTS

No. 7

SHEET 3 OF 3

STATE OF NORTH CAROLINA DEPARTMENT OF TRANSPORTATION

SUBSTRUCTURE

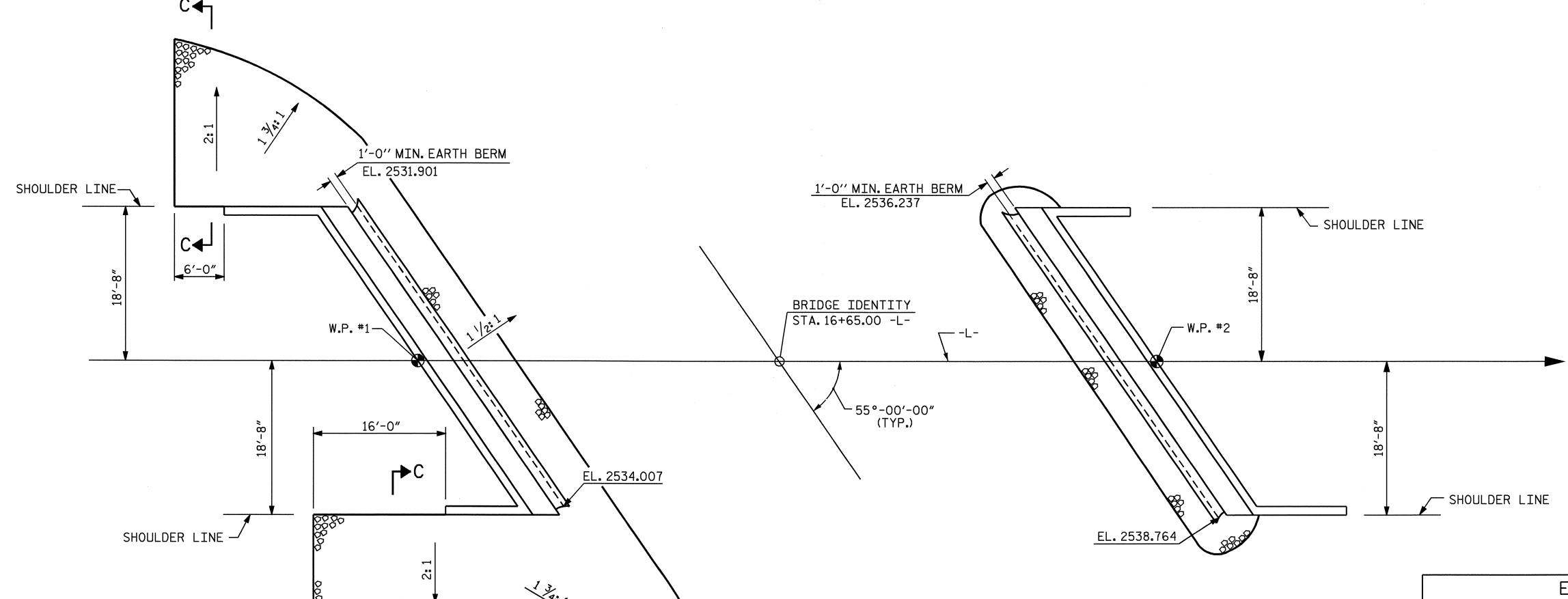
END BENT #2

		SHEET NO.				
NO.	BY:	DATE:	NO.	BY:	DATE:	S-19
1			3			TOTAL SHEETS
2			4	·		22

\_ DATE : 08/08/07 M.K. BEARD DRAWN BY : \_\_\_ \_ DATE : 10/07 CHECKED BY : K.D. LAYNE

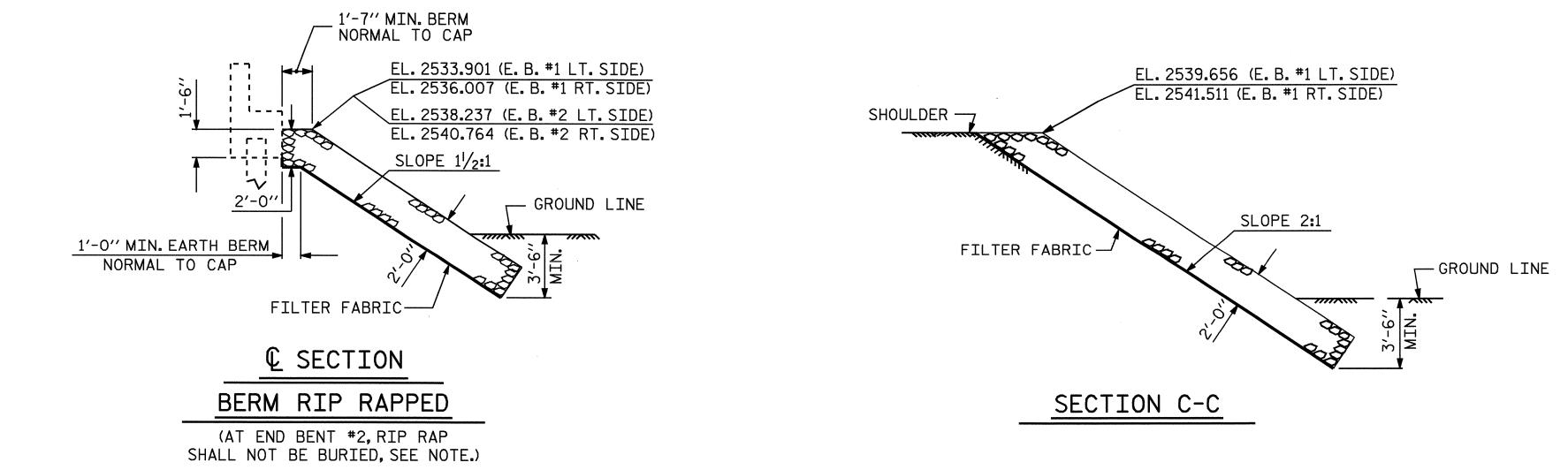


CONTRACTOR SHALL MINIMIZE DISTURBANCE OF THE EXISTING SLOPE AT END BENT #2. THE DISTURBED AREA SHALL BE RIP RAPPED AS DIRECTED BY THE ENGINEER.



ESTIMATED QUANTITIES				
BRIDGE @ STA.16+65.00 -L-	RIP RAP CLASS II (2'-0" THICK)	FILTER FABRIC FOR DRAINAGE		
	TONS	SQUARE YARDS		
END BENT #1	182	202		
END BENT: #2	20	22		

### PLAN OF RIP RAP



PROJECT NO. B-3661 HAYWOOD \_ COUNTY

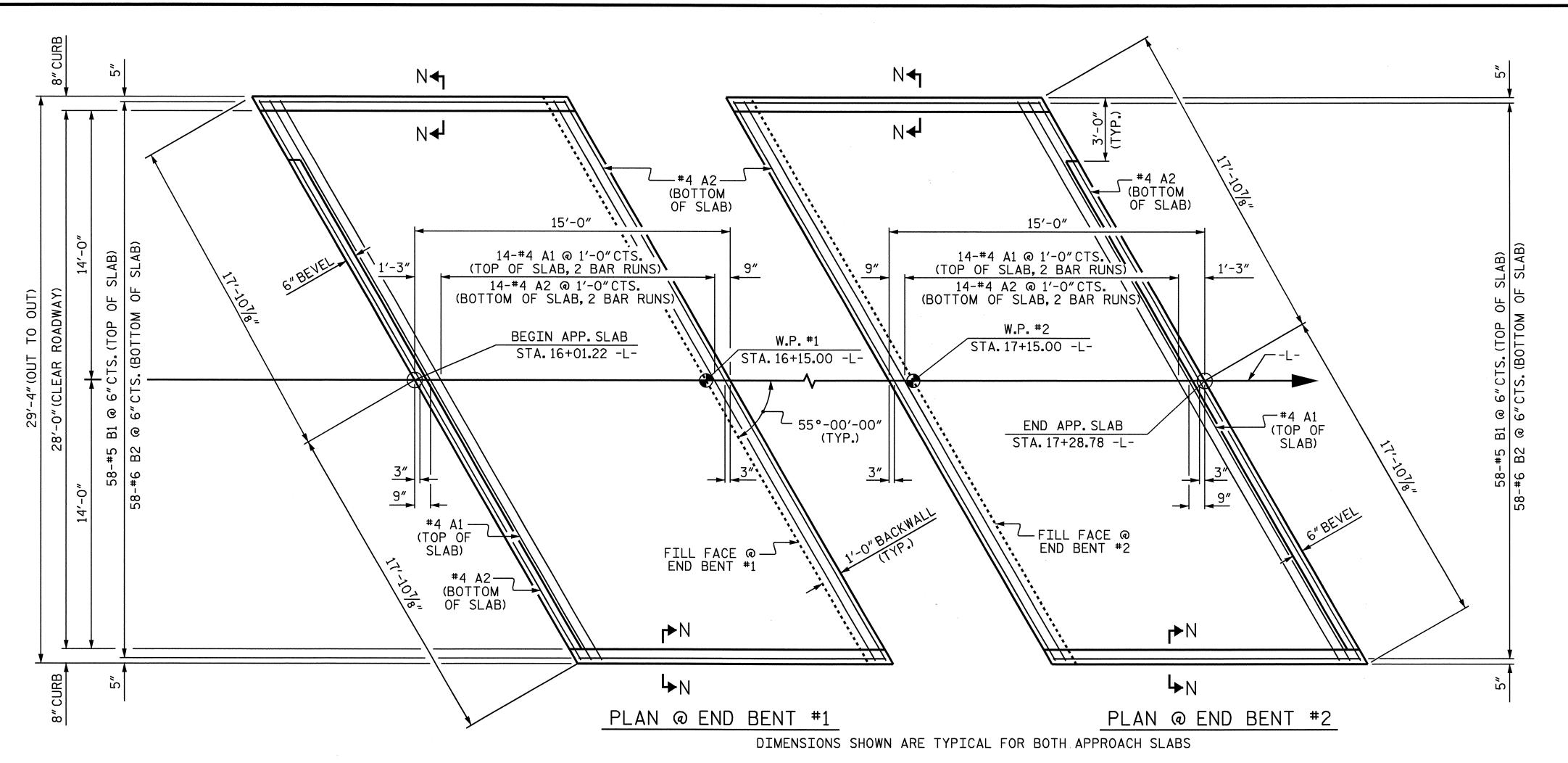
16+65.00-L-

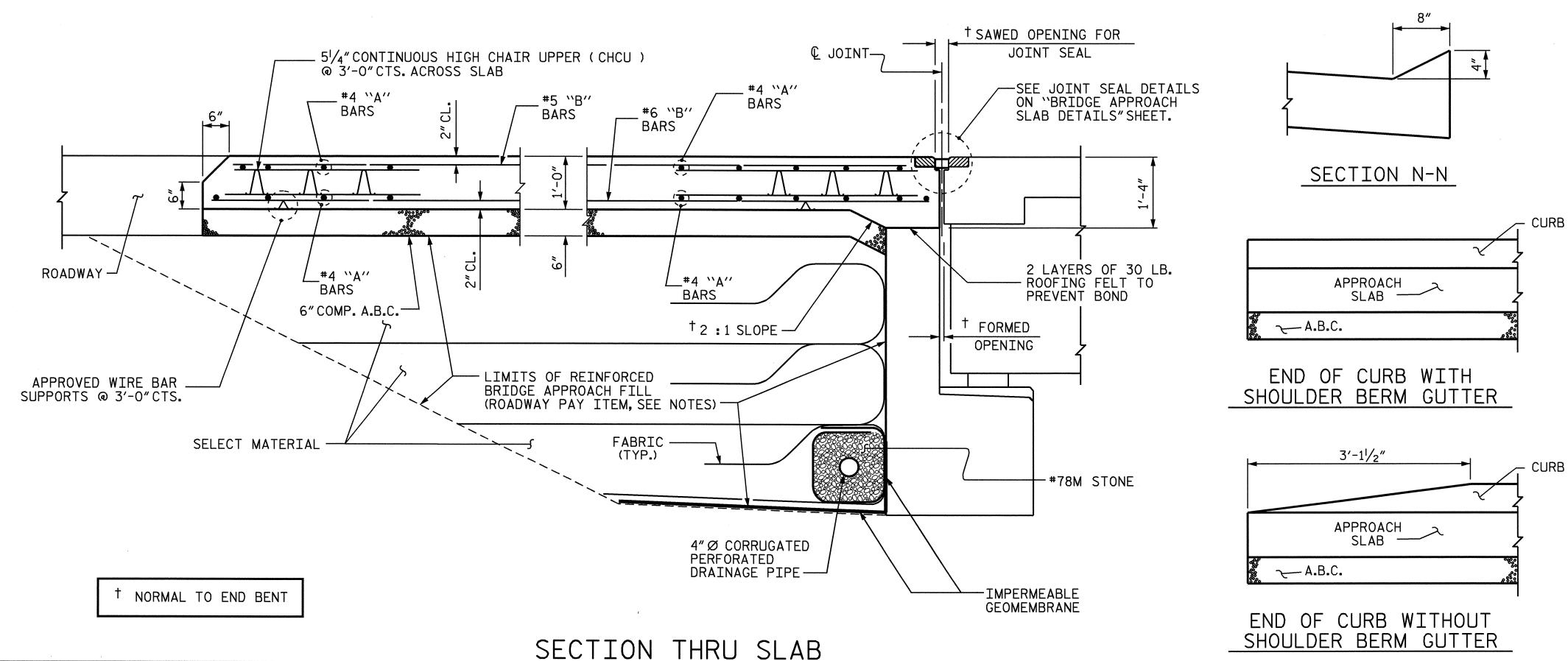
STATE OF NORTH CAROLINA DEPARTMENT OF TRANSPORTATION RALEIGH

-RIP RAP DETAILS-

		SHEET NO.				
0.	BY:	DATE:	NO.	BY:	DATE:	S-20
			3			TOTAL SHEETS
2			4			22

ASSEMBLED BY: R.G. EMERSON DATE: 04/07 CHECKED BY: T.G. PAYNE DATE: 09/07





03-APR-2008 09:32 R:\Structures\Final\B-3661\_sd\_AS.dgn

SHOWING SECTION WITHOUT CONCRETE WEARING SURFACE

ASSEMBLED BY: R. G. EMERSON DATE: 04/07 CHECKED BY: K. D. LAYNE DATE: 07/07

DRAWN BY: EEM 3/95 REV. 7/10/01 LES/RDR REV. 5/7/03R RWW/JTE REV. 5/1/06R KMM/GM

	BI	LL O	F MA	ATERIAL	,
FC	OR C	OACH S IRED)	SLAB		
BAR	NO.	SIZE	TYPE	LENGTH	WEIGHT
<b>*</b> A1	30	#4	STR	18'-9"	376
A2	32	#4	STR	18'-7"	397
<b>₩</b> B1	58	#5	STR	13′-6″	817
B2	58	#6	STR	14'-8"	1278
REIN	FORCI	NG STE	LBS.	1675	
	XY CO NFORC	DATED CING S	LBS.	1193	
CLASS AA CONCRETE				C. Y.	16.9

SPLICE	LENGTHS
BAR	LENGTH
#4 A1	2′-0″
#4 A2	1'-9"

#### NOTES

APPROACH SLAB SHALL NOT BE CONSTRUCTED PRIOR TO COMPLETION OF THE BRIDGE DECK.

FOR REINFORCED BRIDGE APPROACH FILL INCLUDING FABRIC, IMPERMEABLE GEOMEMBRANE, 4" Ø DRAINAGE PIPE, #78M STONE, AND SELECT MATERIAL, SEE ROADWAY PLANS.

AREA BETWEEN THE WINGWALL AND APPROACH SLAB SHALL BE GRADED TO DRAIN THE WATER AWAY FROM THE FILL FACE OF THE BRIDGE AND SHALL BE PAVED. SEE ROADWAY PLANS.

THE 6"COMP. A.B.C. SHALL BE FLUSH WITH THE ROADWAY END OF THE APPROACH SLAB AND SHALL EXTEND 1'-0"OUTSIDE EACH EDGE OF THE APPROACH SLAB.

THE CONTRACTOR MAY USE 4"TYPE B-25.0B ASPHALT CONCRETE BASE COURSE IN LIEU OF 6"COMP. A.B.C. IF THIS OPTION IS USED, THE BASE COURSE SHALL BE FLUSH WITH THE ROADWAY END OF THE APPROACH SLAB, AND THE WIDTH SHALL BE THE SAME AS THAT OF THE APPROACH SLAB.

THE CONTRACTOR MAY USE 5"CLASS "A" CONCRETE BASE IN LIEU OF 6" COMP. A.B.C. IF THIS OPTION IS USED, THE CONCRETE BASE SHALL BE FLUSH WITH THE ROADWAY END OF THE APPROACH SLAB, AND THE WIDTH SHALL BE THE SAME AS THAT OF THE APPROACH SLAB. THE CONCRETE SHALL BE FINISHED TO A SMOOTH SURFACE AND A LAYER OF 30 LB ROOFING FELT SHALL BE PLACED BETWEEN THE CONCRETE BASE AND THE APPROACH SLAB TO PREVENT BOND. THE APPROACH SLAB SHALL NOT BE CAST UNTIL THE CONCRETE BASE HAS REACHED AN AGE OF THREE CURING DAYS.

THE JOINT SHALL BE SAWED PRIOR TO THE CASTING OF THE BARRIER RAIL.

#### WITH EVAZOTE JOINT SEAL

FOR EVAZOTE JOINT SEALS, SEE SPECIAL PROVISIONS.

THE NOMINAL UNCOMPRESSED SEAL WIDTH OF THE EVAZOTE JOINT SEAL SHALL BE  $2^{1}/2^{\prime\prime}$ .

FOR ELASTOMERIC CONCRETE, SEE SPECIAL PROVISIONS.

PROJECT NO. B-3661

HAYWOOD COUNTY

STATION: 16+65.00 -L-

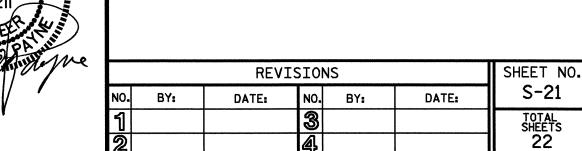
SHEET 1 OF 2

STATE OF NORTH CAROLINA

DEPARTMENT OF TRANSPORTATION
RALEIGH

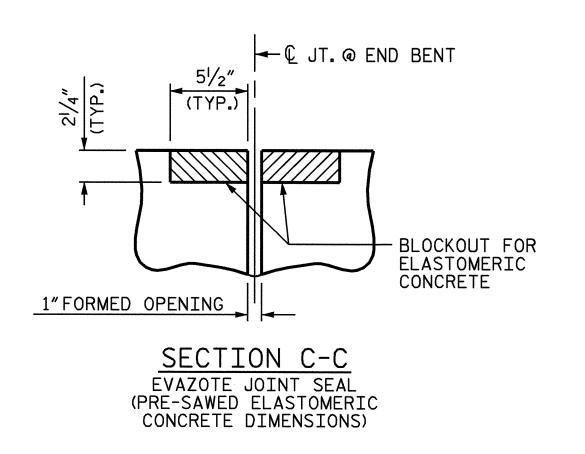
STANDARD

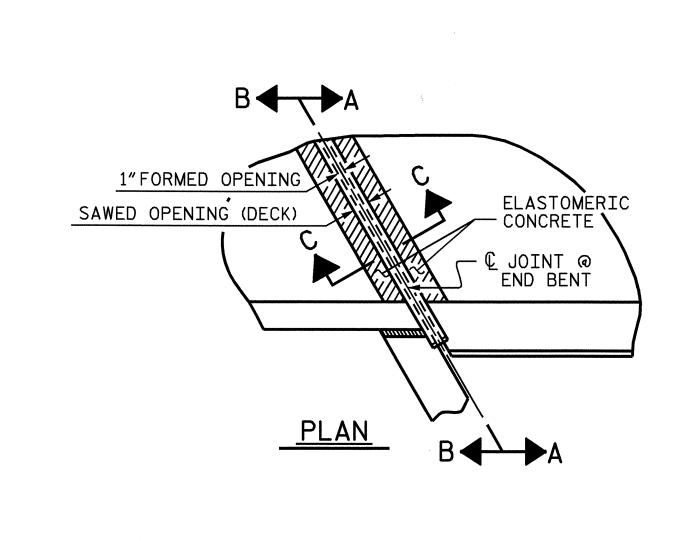
BRIDGE APPROACH SLAB FOR FLEXIBLE PAVEMENT

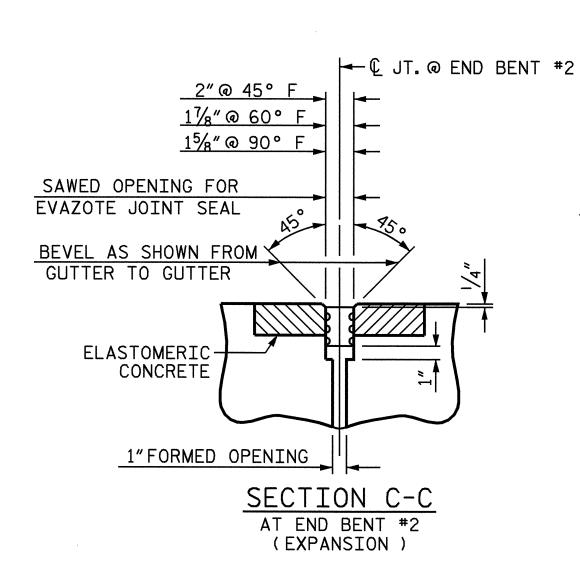


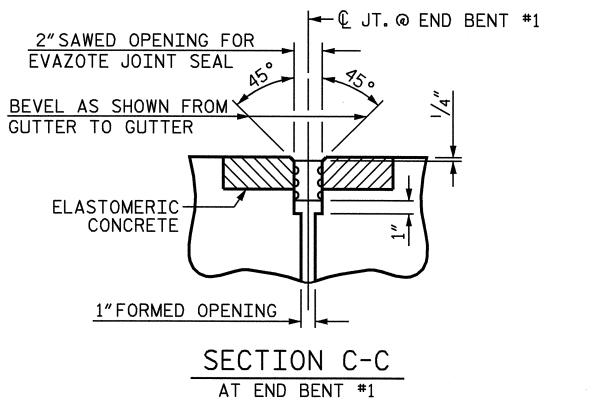
SEAL 20211
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CURB DETAILS

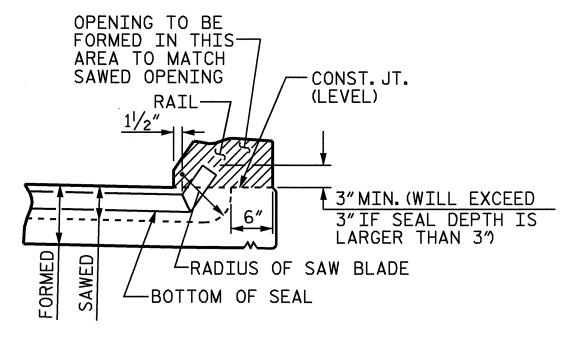


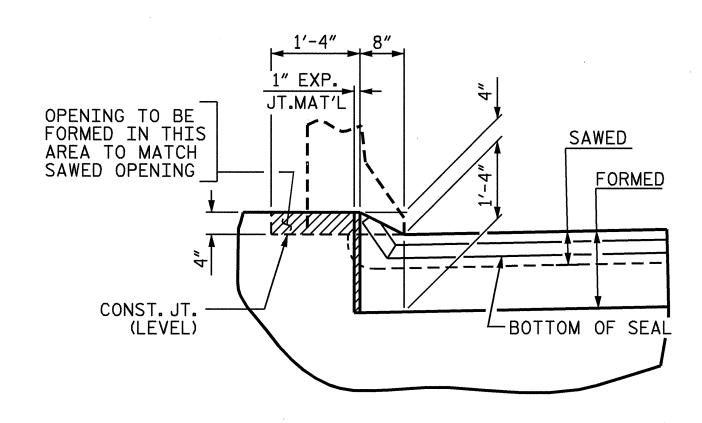






(FIXED)





SECTION A-A

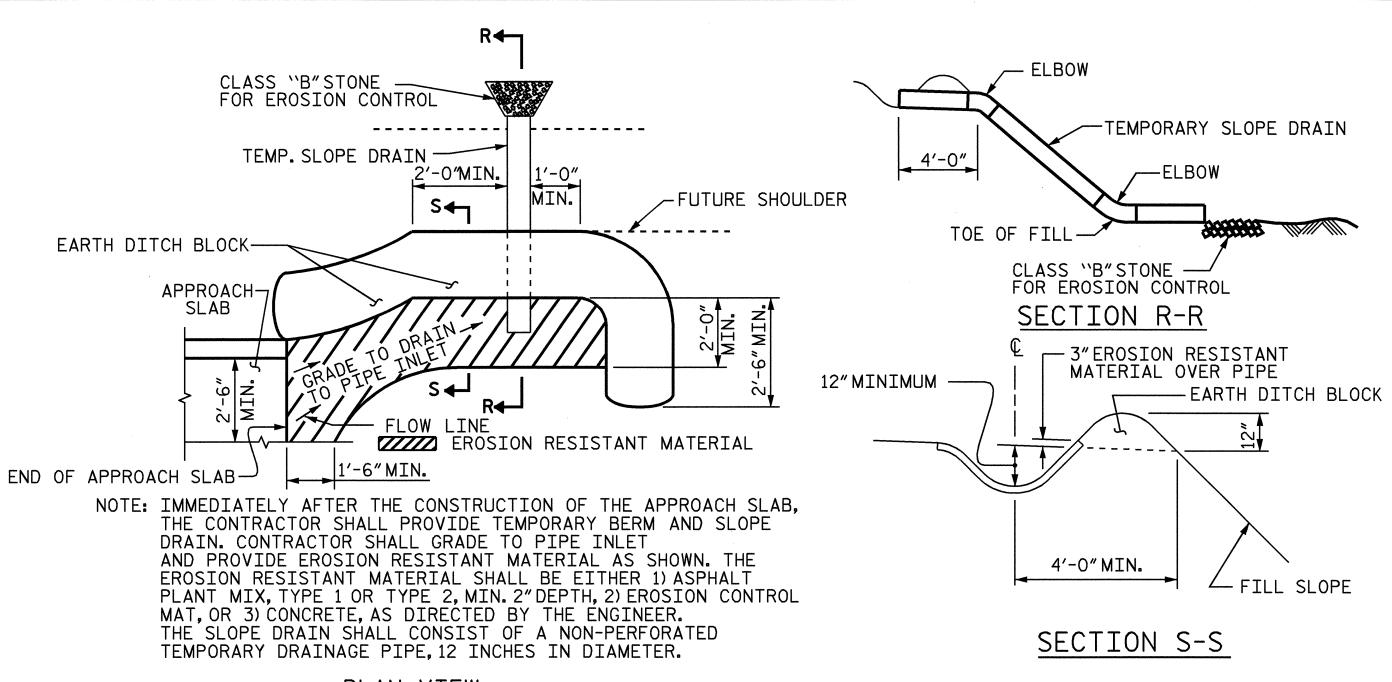
# ELASTOMERIC CONCRETE END BENT NO. 1 5.9 2 5.9 TOTAL 11.8

\* BASED ON THE MINIMUM BLOCKOUT SHOWN.

JOINT SEAL DETAILS @ END BENT

SECTION B-B

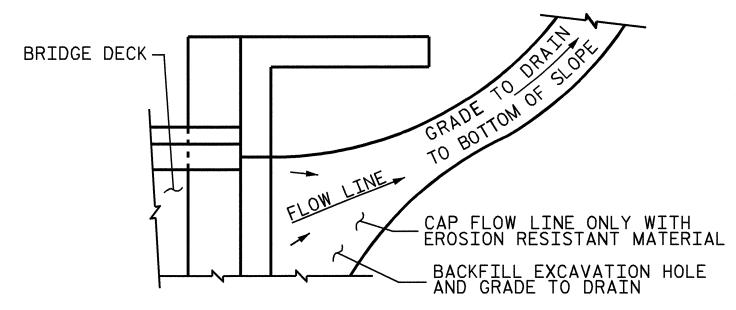
EVAZOTE JOINT SEAL TO BE CUT, HEAT WELDED AND TURNED UP PARALLEL TO SLOPED FACE OF THE BARRIER RAIL.



PLAN VIEW

#### TEMPORARY BERM AND SLOPE DRAIN DETAILS

(TO BE USED WHEN SHOULDER BERM GUTTER IS REQUIRED)



NOTE: IF THE APPROACH SLAB IS NOT CONSTRUCTED IMMEDIATELY AFTER THE BACKFILLING OF THE END BENT EXCAVATION, GRADE TO DRAIN TO THE BOTTOM OF THE SLOPE AND PROVIDE EROSION RESISTANT MATERIAL, SUCH AS FIBERGLASS ROVING OR AS DIRECTED BY THE ENGINEER TO PREVENT SOIL EROSION AND TO PROTECT THE AREA ADJACENT TO THE STRUCTURE. THE CONTRACTOR WILL BE REQUIRED TO REMOVE THESE MATERIALS PRIOR TO CONSTRUCTION OF THE APPROACH SLAB.

TEMPORARY DRAINAGE DETAIL

PROJECT NO. B-3661

HAYWOOD COUNTY

STATION: 16+65.00 -L-

SHEET 2 OF 2

DEPARTMENT OF TRANSPORTATION
RALEIGH
STANDARD

BRIDGE APPROACH
SLAB DETAILS

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ASSEMBLED BY : R. G. EMERSON DATE : 04/07
CHECKED BY : K. D. LAYNE DATE : 07/07

DRAWN BY : FCJ II/88
CHECKED BY : ARB II/88
REV. IO/I7/00 RWW/LES
REV. 5/7/03 RWW/JTE
TLA/GM

#### STANDARD NOTES

#### DESIGN DATA:

STRUCTURAL STEEL - AASHTO M270 GRADE 36 - 20,000 LBS. PER SQ. IN.

- AASHTO M270 GRADE 50 - 27,000 LBS. PER SQ. IN.

- AASHTO M270 GRADE 50 - 27,000 LBS. PER SQ. IN.

- AASHTO M270 GRADE 50 - 27,000 LBS.PER SQ. IN.

REINFORCING STEEL IN TENSION

GRADE 60 - - 24,000 LBS. PER SQ. IN.

CONCRETE IN COMPRESSION ----- 1,200 LBS. PER SQ. IN.

CONCRETE IN SHEAR ------ SEE A.A.S.H.T.O.

STRUCTURAL TIMBER - TREATED OR

UNTREATED - EXTREME FIBER STRESS ---- 1,800 LBS. PER SQ. IN.

COMPRESSION PERPENDICULAR TO GRAIN
OF TIMBER ----

.

EQUIVALENT FLUID PRESSURE OF EARTH

375 LBS. PER SQ. IN.
30 LBS. PER CU. FT.

(MINIMUM)

#### MATERIAL AND WORKMANSHIP:

EXCEPT AS MAY OTHERWISE BE SPECIFIED ON PLANS OR IN THE SPECIAL PROVISIONS, ALL MATERIAL AND WORKMANSHIP SHALL BE IN ACCORDANCE WITH THE 2002 STANDARD SPECIFICATIONS "FOR ROADS AND STRUCTURES" OF THE N. C. DEPARTMENT OF TRANSPORTATION.

STEEL SHEET PILING FOR PERMANENT OR TEMPORARY APPLICATIONS SHALL BE HOT ROLLED.

#### CONCRETE:

UNLESS OTHERWISE REQUIRED ON PLANS, CLASS A CONCRETE SHALL BE USED FOR ALL PORTIONS OF ALL STRUCTURES WITH THE EXCEPTION THAT: CLASS AA CONCRETE SHALL BE USED IN BRIDGE SUPERSTRUCTURES, ABUTMENT BACKWALLS, AND APPROACH SLABS; CLASS B CONCRETE SHALL BE USED FOR SLOPE PROTECTION AND RIP RAP; AND CLASS S SHALL BE USED FOR UNDERWATER FOOTING SEALS.

#### CONCRETE CHAMFERS:

UNLESS OTHERWISE NOTED ON THE PLANS, ALL EXPOSED CORNERS ON STRUCTURES SHALL BE CHAMFERED 3/4" WITH THE FOLLOWING EXCEPTIONS: TOP CORNERS OF CURBS MAY BE ROUNDED TO 1-1/2" RADIUS WHICH IS BUILT INTO CURB FORMS; CORNERS OF TRANSVERSE FLOOR EXPANSION JOINTS SHALL BE ROUNDED WITH A 1/4" FINISHING TOOL UNLESS OTHERWISE REQUIRED ON PLANS; AND CORNERS OF EXPANSION JOINTS IN THE ROADWAY FACES AND TOPS OF CURBS AND SIDEWALKS SHALL BE ROUNDED TO A 1/4" RADIUS WITH A FINISHING STONE OR TOOL UNLESS OTHERWISE REQUIRED ON PLANS.

#### DOWELS:

DOWELS WHEN INDICATED ON PLANS AS FOR CULVERT EXTENSIONS, SHALL BE EMBEDDED AT LEAST 12" INTO THE OLD CONCRETE AND GROUTED INTO PLACE WITH 1:2 CEMENT MORTAR.

# ALLOWANCE FOR DEAD LOAD DEFLECTION, SETTLEMENT, ETC. IN CASTING SUPERSTRUCTURES:

BRIDGES SHALL BE BUILT ON THE GRADE OR VERTICAL CURVE SHOWN ON PLANS. SLABS, CURBS AND PARAPETS SHALL CONFORM TO THE GRADE OR CURVE.

ALL DIMENSIONS WHICH ARE GIVEN IN SECTION AND ARE AFFECTED BY DEAD LOAD DEFLECTIONS ARE DIMENSIONS AT CENTER LINE OF BEARING UNLESS OTHERWISE NOTED ON PLANS. IN SETTING FORMS FOR STEEL BEAM BRIDGES AND PRESTRESSED CONCRETE GIRDER BRIDGES, ADJUSTMENTS SHALL BE MADE DUE TO THE DEAD LOAD DEFLECTIONS FOR THE ELEVATIONS SHOWN. WHERE BLOCKS ARE SHOWN OVER BEAMS FOR BUILDING UP TO THE SLAB, THE VERTICAL DIMENSIONS OF THE BLOCKS SHALL BE ADJUSTED BETWEEN BEARINGS TO COMPENSATE FOR DEAD LOAD DEFLECTIONS, VERTICAL CURVE ORDINATE, AND ACTUAL BEAM CAMBER. WHERE BOTTOM OF SLAB IS IN LINE WITH BOTTOM OF TOP FLANGES, DEPTH OF SLAB BETWEEN BEARINGS SHALL BE ADJUSTED TO COMPENSATE FOR DEAD LOAD DEFLECTION, VERTICAL CURVE ORDINATE, AND ACTUAL BEAM CAMBER.

IN SETTING FALSEWORK AND FORMS FOR REINFORCED CONCRETE SPANS, AN ALLOWANCE SHALL BE MADE FOR DEAD LOAD DEFLECTIONS, SETTLEMENT OF FALSEWORK, AND PERMANENT CAMBER WHICH SHALL BE PROVIDED FOR IN ADDITION TO THE ELEVATIONS SHOWN. AFTER REMOVAL OF THE FALSEWORK, THE FINISHED STRUCTURES SHALL CONFORM TO THE PROFILE AND ELEVATIONS SHOWN ON THE PLANS AND

CONSTRUCTION ELEVATIONS FURNISHED BY THE ENGINEER.

DETAILED DRAWINGS FOR FALSEWORK OR FORMS FOR BRIDGE SUPERSTRUCTURE
AND ANY STRUCTURE OR PARTS OF A STRUCTURE AS NOTED ON THE PLANS SHALL
BE SUBMITTED TO THE ENGINEER FOR APPROVAL BEFORE CONSTRUCTION OF THE
FALSEWORK OR FORMS IS STARTED.

#### REINFORCING STEEL:

ALL REINFORCING STEEL SHALL BE DEFORMED WITH THE EXCEPTION OF #2
BARS WHICH MAY BE FABRICATED FROM COLD DRAWN STEEL WIRE. DIMENSIONS
RELATIVE TO PLACEMENT OF REINFORCING ARE TO CENTERS OF BARS UNLESS OTHERWISE
INDICATED IN THE PLANS. DIMENSIONS ON BAR DETAILS ARE TO CENTERS OF BARS
OR ARE OUT TO OUT AS INDICATED ON PLANS.

WIRE BAR SUPPORTS SHALL BE PROVIDED FOR REINFORCING STEEL WHERE INDICATED ON THE PLANS. WHEN BAR SUPPORT PIECES ARE PLACED IN CONTINUOUS LINES, THEY SHALL BE SO PLACED THAT THE ENDS OF THE SUPPORTING WIRES SHALL BE LAPPED TO LOCK LEGS ON ADJOINING PIECES.

#### STRUCTURAL STEEL:

AT THE CONTRACTOR'S OPTION, HE MAY SUBSTITUTE 7/8" Ø SHEAR STUDS FOR THE 3/4" Ø STUDS SPECIFIED ON THE PLANS. THIS SUBSTITUTION SHALL BE MADE AT THE RATE OF 3 - 7/8" Ø STUDS FOR 4 - 3/4" Ø STUDS, AND STUD SPACING CHANGES SHALL BE MADE AS NECESSARY TO PROVIDE THE SAME EQUIVALENT NUMBER OF 7/8" Ø STUDS ALONG THE BEAM AS SHOWN FOR 3/4" Ø STUDS BASED ON THE RATIO OF 3 - 7/8" Ø STUDS FOR 4 - 3/4" Ø STUDS. STUDS OF THE LENGTH SPECIFIED ON THE PLANS MUST BE PROVIDED. THE MAXIMUM SPACING SHALL BE 2'-0".

EXCEPT AT THE INTERIOR SUPPORTS OF CONTINUOUS BEAMS WHERE THE COVER PLATE IS IN CONTACT WITH BEARING PLATE, THE CONTRACTOR MAY, AT HIS OPTION, SUBSTITUTE FOR THE COVER PLATES DESIGNATED ON THE PLANS COVER PLATES OF THE EQUIVALENT AREA PROVIDED THESE PLATES ARE AT LEAST 5/16" IN THICKNESS AND DO NOT EXCEED A WIDTH EQUAL TO THE FLANGE WIDTH LESS 2"OR A THICKNESS EQUAL TO 2 TIMES THE FLANGE THICKNESS. THE SIZE OF FILLET WELDS SHALL CONFORM TO THE REQUIREMENTS OF THE CURRENT ANSI/AASHTO/AWS "BRIDGE WELDING CODE". ELECTROSLAG WELDING WILL NOT BE PERMITTED.

PLACEMENT OF BEAM OR GIRDER MEMBERS ON TRUCKS FOR HAULING SHALL
BE DONE IN COMPLIANCE WITH LIMITS SHOWN ON SKETCHES PROVIDED TO THE MATERIALS
AND TEST UNIT APPROVED BY THE STRUCTURE DESIGN UNIT DATED MAY 8, 1991.
THESE SKETCHES PRIMARILY LIMIT THE UNSUPPORTED CANTILEVER LENGTH OF MEMBERS.
WHEN THE CONTRACTOR WISHES TO PLACE MEMBERS ON TRUCKS NOT IN ACCORDANCE
WITH THESE LIMITS, TO SHIP BY RAIL, TO ATTACH SHIPPING RESTRAINTS TO THE
MEMBERS OR TO INVERT MEMBERS, HE SHALL SUBMIT A SKETCH FOR APPROVAL
PRIOR TO SHIPPING. SEE ALSO ARTICLE 1072-11.

WITH THE SOLE EXCEPTION OF EDGES AT SURFACES WHICH BEAR ON OTHER SURFACES, ALL SHARP EDGES AND ENDS OF SHAPES AND PLATES SHALL BE SLIGHTLY ROUNDED BY SUITABLE MEANS TO A RADIUS OF APPROXIMATELY 1/16 INCH OR EQUIVALENT FLAT SURFACE AT A SUITABLE ANGLE PRIOR TO PAINTING, GALVANIZING, OR METALLIZING.

#### HANDRAILS AND POSTS:

METAL STANDARDS AND FACES OF THE CONCRETE END POSTS FOR THE METAL RAIL SHALL BE SET NORMAL TO THE GRADE OF THE CURB, UNLESS OTHERWISE SHOWN ON PLANS. THE METAL RAIL AND TOPS OF CONCRETE POSTS USED WITH THE ALUMINUM RAIL SHALL BE BUILT PARALLEL TO THE GRADE OF THE CURB.

METAL HANDRAILS SHALL BE IN ACCORDANCE WITH THE PLANS. RAILS SHALL BE AS MANUFACTURED FOR BRIDGE RAILING. CASTINGS SHALL BE OF A UNIFORM APPEARANCE. FINS AND OTHER DEFORMATIONS RESULTING FROM CASTING OR OTHERWISE SHALL BE REMOVED IN A MANNER SO THAT A UNIFORM COLORING OF THE COMPLETED CASTING SHALL BE OBTAINED. CASTINGS WITH DISCOLORATIONS OR OF NON-UNIFORM COLORING WILL NOT BE ACCEPTED. CERTIFIED MILL REPORTS ARE REQUIRED FOR METAL RAILS AND POSTS.

#### SPECIAL NOTES:

GENERALLY, IN CASE OF DISCREPANCY, THIS STANDARD SHEET OF NOTES SHALL GOVERN OVER THE SPECIFICATIONS, BUT THE REMAINDER OF THE PLANS SHALL GOVERN OVER NOTES HEREON, AND SPECIAL PROVISIONS SHALL GOVERN OVER ALL. SEE SPECIFICATIONS ARTICLE 105-4.

ENGLISH

STD. NO. SN