VICINITY MAP (NOT TO SCALE)

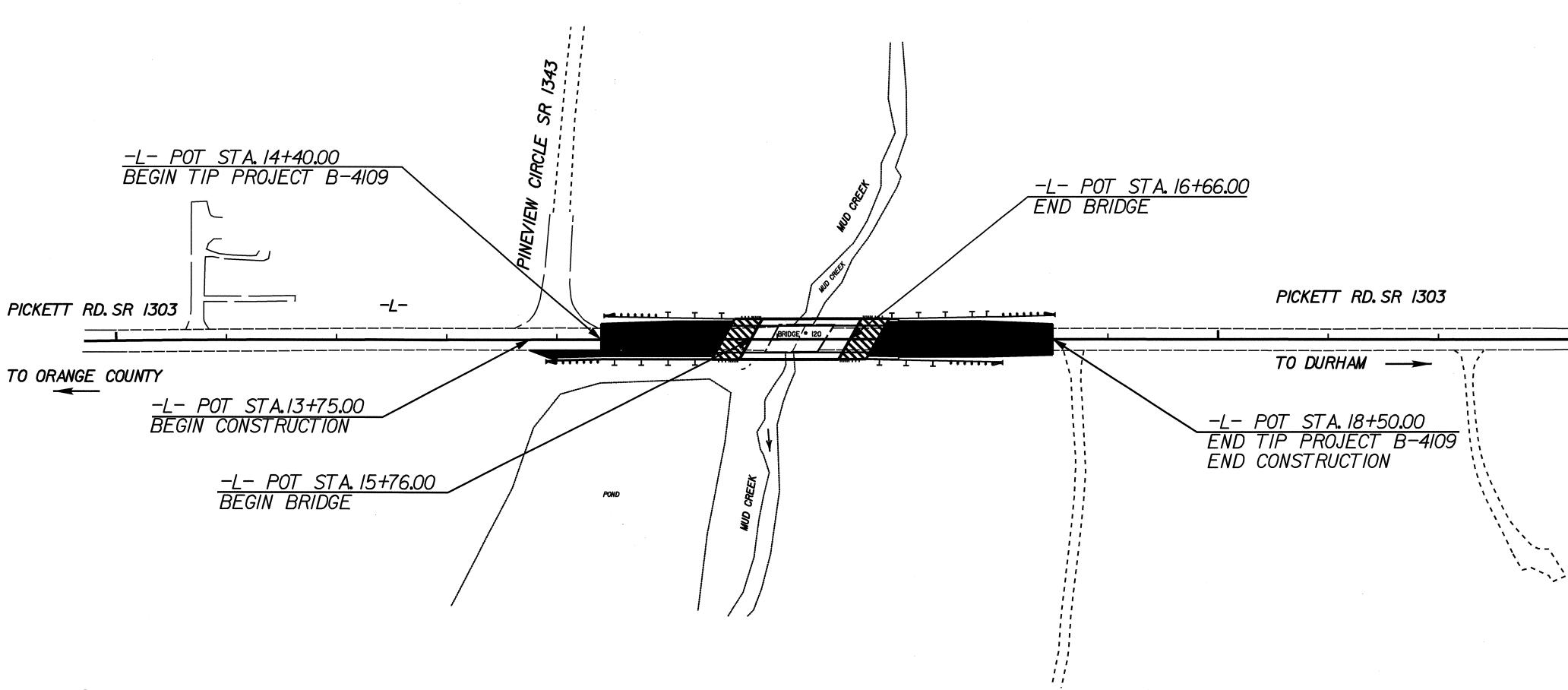
STATE OF NORTH CAROLINA DIVISION OF HIGHWAYS

DURHAM COUNTY

LOCATION: BRIDGE NO. 120 OVER MUD CREEK ON SR 1303 IN DURHAM

TYPE OF WORK: PAVING, GRADING, DRAINAGE, AND STRUCTURE

STATE	B STATE PROJECT REFERENCE NO.			TOTAL SHEETS		
N.C.		B-4109				
WBS	NO.	F. A. PROJ. NO.	DESCRIP	MON		
3346	4.1.1	BRZ-1303(3)	P.E	•		
3346	4.2.1	BRZ-1303(3)	R∕W, U	R/W, UTIL		
3346	4.3.1	BRZ-1303(3)	CON!	CONSTR.		
				-		
'						



STRUCTURE

DESIGN DATA ADT 2008 = 6,100ADT 2030 = 12,400DHV = 13 %D = 60 %= 3 % *

V = 40 MPHFUNCTION. = URBAN CLASS. LOCAL * (TTST 1% + DUALS 2%) PROJECT LENGTH

LENGTH ROADWAY TIP PROJECT B-4109 = 0.061 MI LENGTH STRUCTURE TIP PROJECT B-4109 = 0.017 MI TOTAL LENGTH TIP PROJECT B-4109 = 0.078 MI

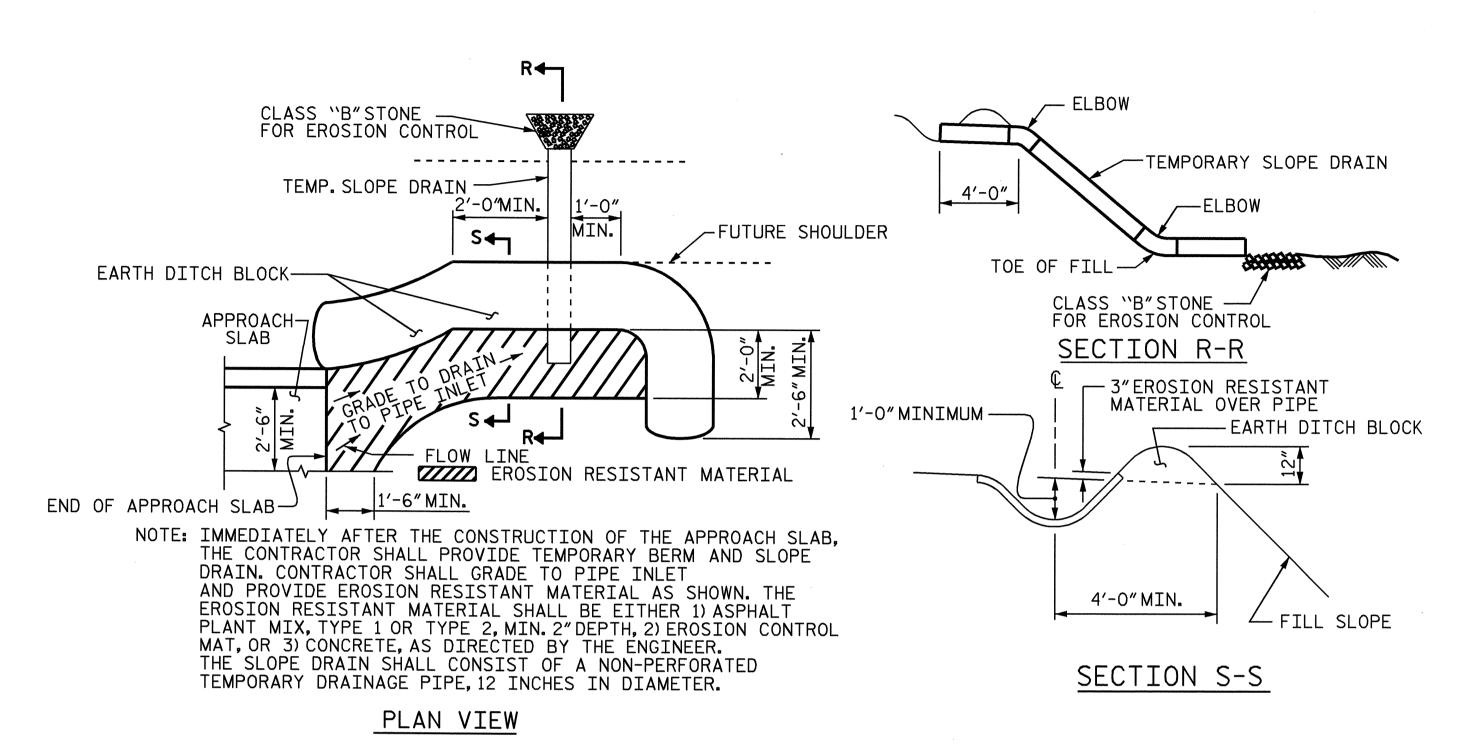
Prepared in the Office of: **DIVISION OF HIGHWAYS** 2006 STANDARD SPECIFICATIONS

LETTING DATE: MAY 20, 2008

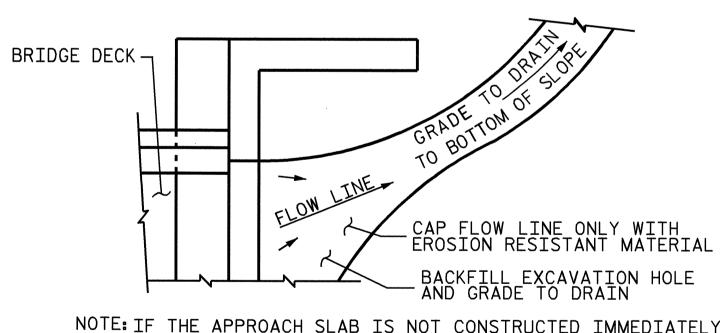
B. C. Hunt, PE PROJECT ENGINEER V. A. Patel, PE PROJECT DESIGN ENGINEER STRUCTURE DESIGN UNIT

1000 BIRCH RIDGE DR., RALEIGH, NC 27610

DIVISION OF HIGHWAYS STATE OF NORTH CAROLINA



TEMPORARY BERM AND SLOPE DRAIN DETAILS



NOTE: IF THE APPROACH SLAB IS NOT CONSTRUCTED IMMEDIATELY AFTER THE BACKFILLING OF THE END BENT EXCAVATION, GRADE TO DRAIN TO THE BOTTOM OF THE SLOPE AND PROVIDE EROSION RESISTANT MATERIAL, SUCH AS FIBERGLASS ROVING OR AS DIRECTED BY THE ENGINEER TO PREVENT SOIL EROSION AND TO PROTECT THE AREA ADJACENT TO THE STRUCTURE. THE CONTRACTOR WILL BE REQUIRED TO REMOVE THESE MATERIALS PRIOR TO CONSTRUCTION OF THE APPROACH SLAB.

TEMPORARY DRAINAGE DETAIL

PROJECT NO. B-4109 DURHAM STATION: 16+21.00 -L-

SHEET 2 OF 2

STATE OF NORTH CAROLINA DEPARTMENT OF TRANSPORTATION RALEIGH

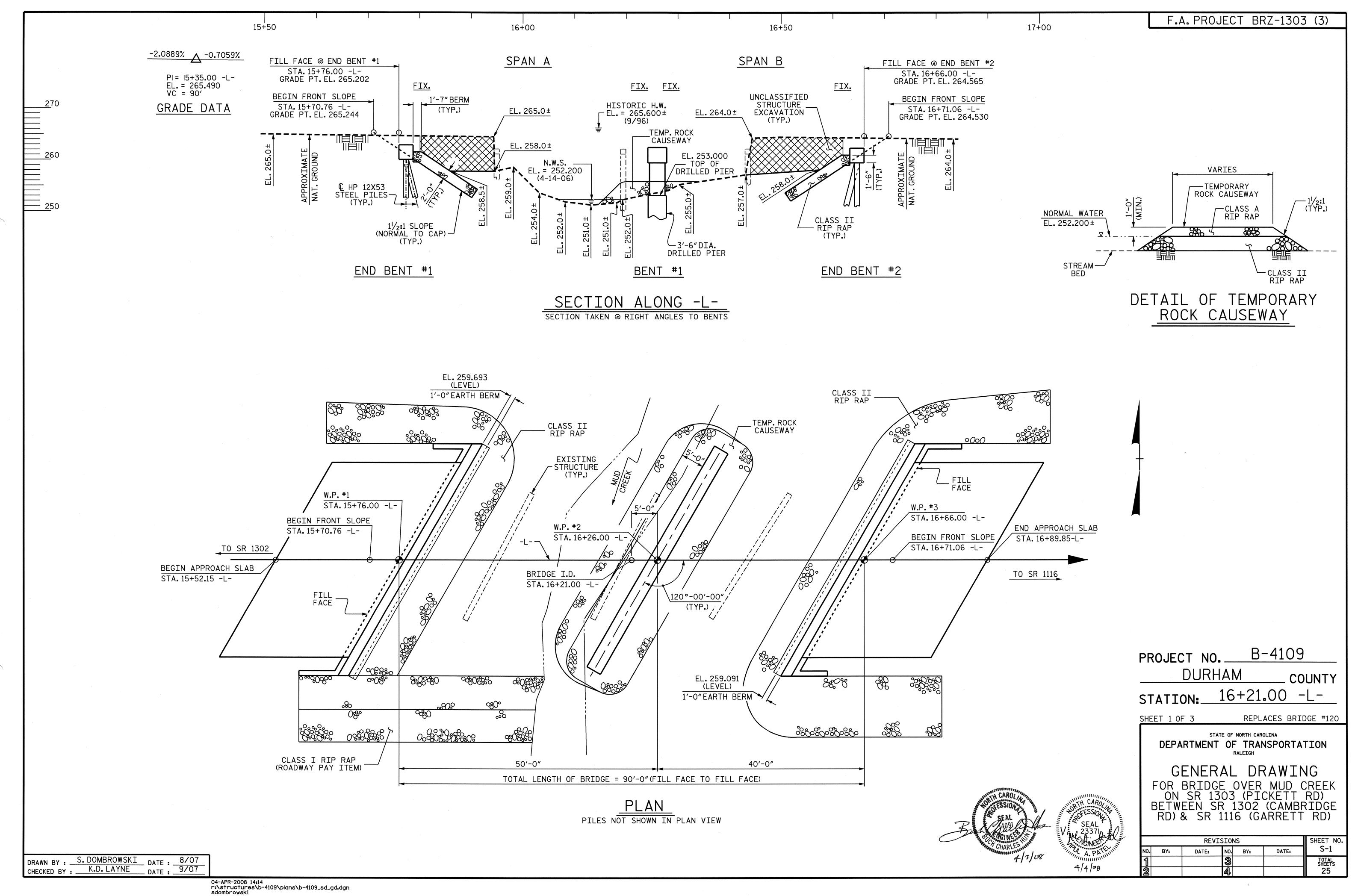
BRIDGE APPROACH SLAB DETAILS

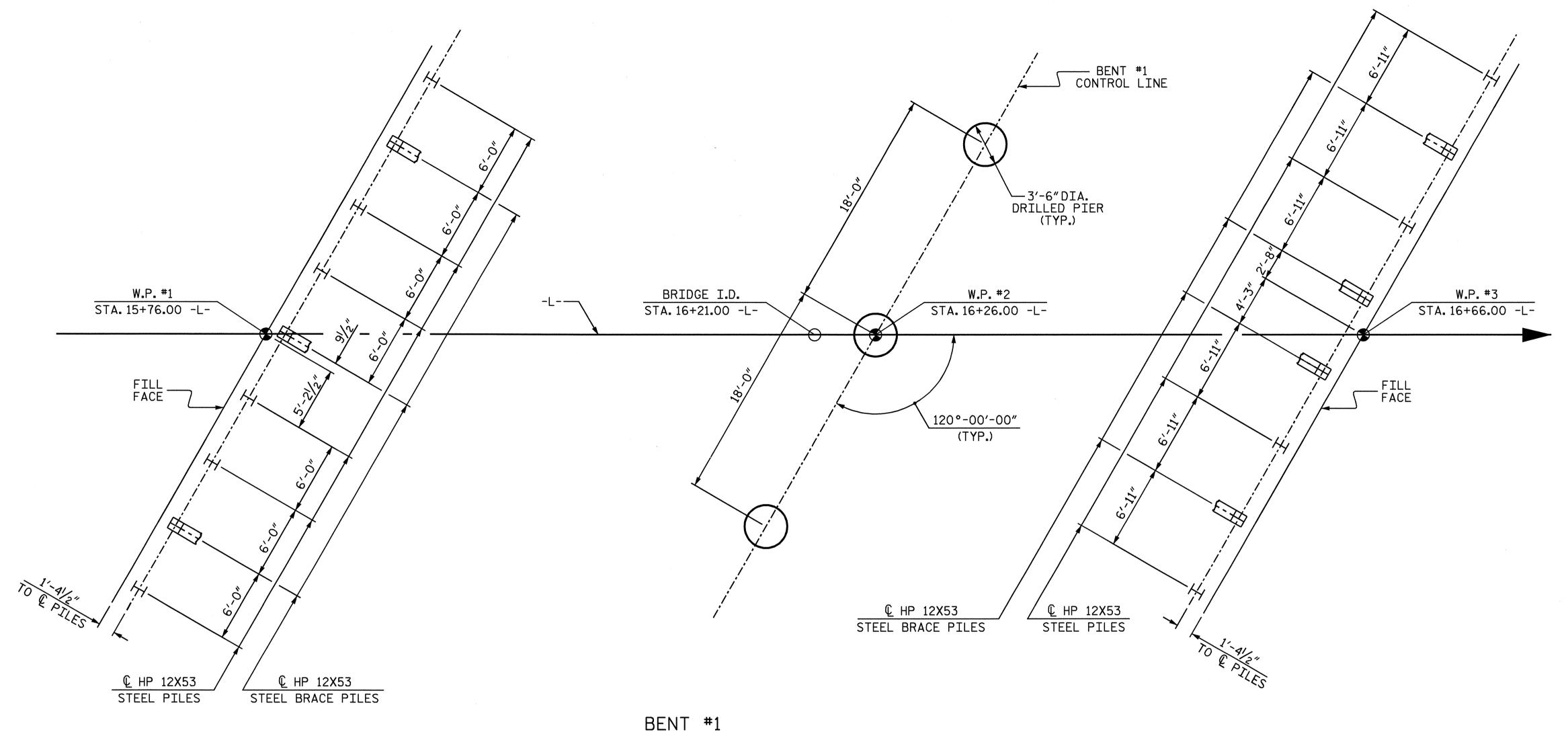
					SHEET NO.					
	REVISIONS									
BY:	DATE:	NO.	BY:	DATE:	S-24					
	3									
		4			TOTAL SHEETS 24					
STD. NO	. BAS10									

CHECKED BY : S. DOMBROWSKI DATE : 08/07 REV. 8/16/99 MAB/LES REV. 10/17/00 RWW/LES REV. 5/7/03 RWW/JTE DRAWN BY: FCJ 11/88 CHECKED BY : ARB 11/88

DATE: 08/07

ASSEMBLED BY : R. G. EMERSON





END BENT #1

END BENT #2

FOUNDATION LAYOUT

DIMENSIONS LOCATING DRILLED PIERS ARE TO DRILLED PIER CENTER. ALL PILES ARE HP 12X53. END BENT BRACE PILES ARE BATTERED 3:12. DIMENSIONS LOCATING PILES ARE TO THE CENTERLINE OF PILE.

NOTES

DRILLED PIERS AT BENT #1 ARE DESIGNED FOR BOTH SKIN FRICTION AND END BEARING. CHECK FIELD CONDITIONS FOR THE REQUIRED END BEARING CAPACITY OF 45 TSF.

DRILLED PIERS AT BENT #1 ARE DESIGN FOR AN APPLIED LOAD OF 185 TONS EACH AT THE TOP OF THE COLUMN.

DRILLED PIERS AT BENT #1 SHALL EXTEND TO AN ELEVATION NO HIGHER THAN 225,000 AND SATISFY THE REQUIRED END BEARING CAPACITY.

PERMANENT STEEL CASING IS REQUIRED FOR DRILLED PIERS AT BENT #1. DO NOT EXTEND THE CASING BELOW ELEVATION 235.000 WITHOUT PRIOR APPROVAL FROM THE ENGINEER. SEE DRILLED PIERS SPECIAL PROVISION.

DO NOT DEWATER THE DRILLED PIER EXCAVATIONS AT BENT #1. CLEAN THE BOTTOM OF THE EXCAVATIONS WITH A SUBMERSIBLE PUMP OR AN AIRLIFT. WET PLACEMENT OF CONCRETE IS REQUIRED. SEE DRILLED PIERS SPECIAL PROVISION.

DRILLED PIER EXCAVATIONS AT BENT #1 WILL EXTEND INTO MATERIAL THAT DETERIORATES WHEN EXPOSED TO THE ELEMENTS. CHECK FIELD CONDITIONS FOR THE REQUIRED END BEARING CAPACITY AND PLACE CONCRETE IMMEDIATELY AFTER THE EXCAVATION IS COMPLETED.

THE SCOUR CRITICAL ELEVATIONS FOR BENT #1 IS 231,000. SCOUR CRITICAL ELEVATIONS ARE USED TO MONITOR POSSIBLE SCOUR PROBLEMS DURING THE LIFE OF THE STRUCTURE.

SPT TESTING IS NOT REQUIRED TO DETERMINE THE END BEARING CAPACITY OF THE DRILLED PIERS AT BENT #1.

SID INSPECTIONS MAY BE REQUIRED TO INSPECT THE BOTTOM CLEANLINESS OF THE DRILLED PIERS. THE ENGINEER WILL DETERMINE THE NEED FOR SID INSPECTIONS. SEE DRILLED PIERS SPECIAL PROVISION.

CSL TUBES ARE REQUIRED AND CSL TESTING MAY BE REQUIRED FOR THE DRILLED PIERS. THE ENGINEER WILL DETERMINE THE NEED FOR CSL TESTING. SEE CROSSHOLE SONIC LOGGING SPECIAL PROVISION.

FOR DRILLED PIERS, SEE DRILLED PIERS SPECIAL PROVISION.

DRIVE PILES AT END BENT #1 AND END BENT #2 TO A REQUIRED BEARING CAPACITY OF 100 TONS PER PILE. THE REQUIRED BEARING CAPACITY IS EQUAL TO THE ALLOWABLE BEARING CAPACITY WITH A MINIMUM FACTOR OF SAFETY OF TWO.

THE ALLOWABLE BEARING CAPACITY FOR PILES AT END BENT #1 AND END BENT #2 IS 50 TONS PER PILE.

B-4109 PROJECT NO. __ DURHAM COUNTY <u>16+21.00</u> -L-STATION:_

SHEET 2 OF 3

OFESSION I

SEAL P. E. SEAL P. L. SEAL P. L.

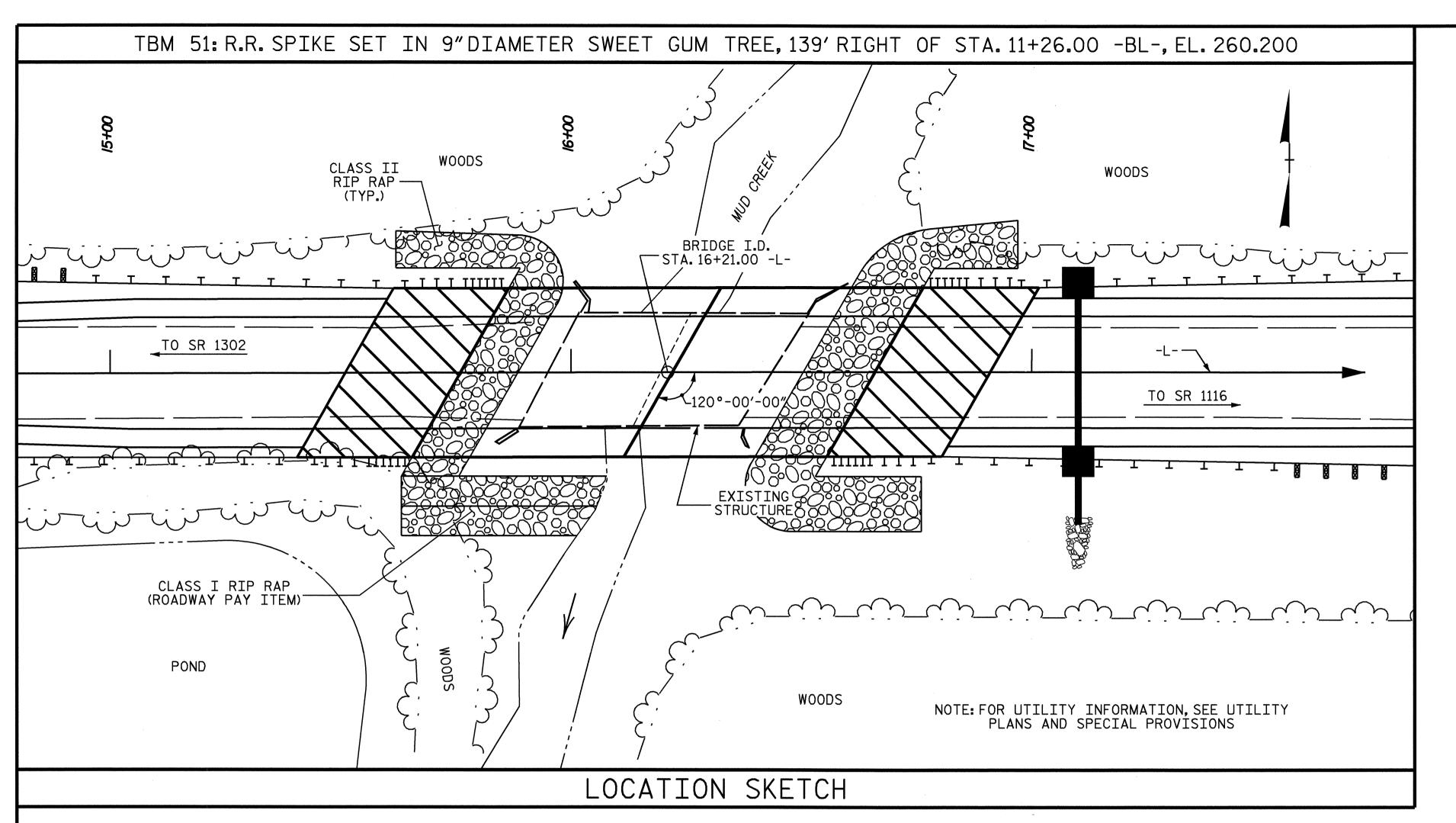
STATE OF NORTH CAROLINA DEPARTMENT OF TRANSPORTATION RALEIGH

GENERAL DRAWING FOR BRIDGE OVER MUD CREEK ON SR 1303 (PICKETT RD) BETWEEN SR 1302 (CAMBRIDGE RD) & SR 1116 (GARRETT RD)

SHEET NO. **REVISIONS** S-2 DATE: NO. BY: DATE: BY: TOTAL SHEETS 25

DRAWN BY: S. DOMBROWSKI DATE: 8/07
CHECKED BY: K.D. LAYNE DATE: 9/07

04-APR-2008 14:14 r:\structures\b-4109\plans\b-4109_sd_gd.dgn



NOTES

ASSUMED LIVE LOAD = HS20 OR ALTERNATE LOADING, EXCEPT THAT CORED SLAB UNITS HAVE BEEN DESIGNED FOR HS25.

FOR OTHER DESIGN DATA AND GENERAL NOTES, SEE SHEET

FOR EROSION CONTROL MEASURES SEE EROSION CONTROL PLANS.

THIS BRIDGE HAS BEEN DESIGNED BY THE STRENGTH DESIGN METHOD AS SPECIFIED IN AASHTO STANDARD SPECIFICATIONS.

THE EXISTING STRUCTURE CONSISTING OF 2 SPANS @ 25'-0" AND CLEAR ROADWAY WIDTH OF 23'-3" AND HAVING A TIMBER DECK ON 15 LINES OF 61/2" X 12" STEEL BEAMS SUPPORTED BY STEEL CAPS AND TIMBER PILES AND LOCATED AT THE PROPOSED STRUCTURE SITE SHALL BE REMOVED. THE EXISTING BRIDGE IS PRESENTLY POSTED BELOW THE LEGAL LOAD LIMIT. SHOULD THE STRUCTURAL INTEGRITY OF THE BRIDGE FURTHER DETERIORATE, THIS LOAD LIMITATION MAY BE REDUCED AS FOUND NECESSARY DURING THE LIFE OF THE PROJECT.

REMOVAL OF THE EXISTING BRIDGE SHALL BE PERFORMED SO AS NOT TO ALLOW DEBRIS TO FALL INTO THE WATER. THE CONTRACTOR SHALL REMOVE THE BRIDGE AND SUBMIT PLANS FOR DEMOLITION IN ACCORDANCE WITH ARTICLE 402-2 OF THE STANDARD SPECIFICATIONS.

THE MATERIAL SHOWN IN THE CROSS-HATCHED AREA ON SHEET S-1 SHALL BE EXCAVATED FOR A DISTANCE OF 30 FT. EACH SIDE OF CENTERLINE ROADWAY AS DIRECTED BY THE ENGINEER. THIS WORK WILL BE PAID FOR AT THE CONTRACT LUMP SUM PRICE FOR UNCLASSIFIED STRUCTURE EXCAVATION.

THE SUBSTRUCTURE OF THE EXISTING BRIDGE INDICATED ON THE PLANS IS FROM THE BEST INFORMATION AVAILABLE. SINCE THIS INFORMATION IS SHOWN FOR THE CONVENIENCE OF THE CONTRACTOR, THE CONTRACTOR SHALL HAVE NO CLAIM WHATSOEVER AGAINST THE DEPARTMENT OF TRANSPORTATION FOR ANY DELAYS OR ADDITIONAL COST INCURRED BASED ON DIFFERENCES BETWEEN THE EXISTING BRIDGE SUBSTRUCTURE SHOWN ON THE PLANS AND THE ACTUAL CONDITIONS AT THE PROJECT SITE.

ASPHALT WEARING SURFACE IS INCLUDED IN ROADWAY QUANTITY ON ROADWAY PLANS.

THIS STRUCTURE HAS BEEN DESIGNED IN ACCORDANCE WITH HEC 18, "EVALUATING SCOUR AT BRIDGES", MAY 2001.

THIS BRIDGE HAS BEEN DESIGNED IN ACCORDANCE WITH THE REQUIREMENTS OF THE AASHTO STANDARD SPECIFICATIONS FOR SEISMIC DESIGN OF HIGHWAY BRIDGES FOR SEISMIC PERFORMANCE CATEGORY A.

THE CONTRACTOR SHALL PROVIDE INDEPENDENT ASSURANCE SAMPLES OF REINFORCING STEEL AS FOLLOWS: FOR PROJECTS REQUIRING UP TO 400 TONS OF REINFORCING STEEL, ONE 30 INCH SAMPLE OF EACH SIZE BAR USED. AND FOR PROJECTS REQUIRING OVER 400 TONS OF REINFORCING STEEL, TWO 30 INCH SAMPLES OF EACH SIZE BAR USED. THE BARS FROM WHICH THE SAMPLES ARE TAKEN MUST THEN BE SPLICED WITH REPLACEMENT BARS OF THE SIZE AND LENGTH OF THE SAMPLE, PLUS A MINIMUM LAP SPLICE OF THIRTY BAR DIAMETERS.

INASMUCH AS THE PAINT SYSTEM ON THE EXISTING STRUCTURAL STEEL CONTAINS LEAD, THE CONTRACTOR'S ATTENTION IS DIRECTED TO ARTICLE 107-1 OF THE STANDARD SPECIFICATIONS. ANY COSTS RESULTING FROM COMPLIANCE WITH APPLICABLE STATE OR FEDERAL REGULATIONS PERTAINING TO HANDLING OF MATERIALS CONTAINING LEAD BASED PAINT SHALL BE INCLUDED IN THE BID PRICE FOR "REMOVAL OF EXISTING STRUCTURE AT STATION 16+21.00 -L-."

AT THE CONTRACTOR'S OPTION, AND UPON REMOVAL OF THE CAUSEWAY, THE CLASS II RIP RAP USED IN THE CAUSEWAY MAY BE PLACED AS RIP RAP SLOPE PROTECTION. SEE SPECIAL PROVISIONS FOR CONSTRUCTION, MAINTENANCE AND REMOVAL OF TEMPORARY ACCESS AT STATION 16+21.00 -L-.

FOR SUBMITTAL OF WORKING DRAWINGS, SEE SPECIAL PROVISIONS.

FOR CRANE SAFETY. SEE SPECIAL PROVISIONS.

FOR GROUT FOR STRUCTURES, SEE SPECIAL PROVISIONS.

FOR FALSEWORK & FORMWORK, SEE SPECIAL PROVISIONS.

FOR PRESTRESSED CONCRETE MEMBERS, SEE SPECIAL PROVISIONS.

	TOTAL BILL OF MATERIAL											
	CONSTRUCTION, MAINTENANCE & REMOVAL OF TEMPORARY ACCESS	REMOVAL OF EXISTING STRUCTURE	3'-6"DIA. DRILLED PIERS IN SOIL	3'-6"DIA. DRILLED PIERS NOT IN SOIL	PERMANENT STEEL CASING FOR 3'-6"DIA. DRILLED PIER	SID INSPECTION	CROSSHOLE SONIC LOGGING	UNCLASSIFIED STRUCTURE EXCAVATION	CLASS A CONCRETE			
	LUMP SUM	LUMP SUM	LIN.FT.	LIN.FT.	LIN.FT.	EACH	EACH	LUMP SUM	CU.YDS.			
SUPERSTRUCTURE								LUMP SUM				
END BENT #1									17 . 8			
BENT #1			59.0	25.0	54 . 0	1	1		26.2			
END BENT #2									17.8			
TOTAL	LUMP SUM	LUMP SUM	59.0	25.0	54 . 0	1	1	LUMP SUM	61.8			

	——————————————————————————————————————											
	BRIDGE APPROACH SLABS	REINFORCING STEEL	SPIRAL COLUMN REINFORCING STEEL	HP 12X53 STEEL PILES		TWO BAR METAL RAIL	1'-2" X 2'-9" CONCRETE PARAPET	RIP RAP CLASS II (2'-0"THICK)	FILTER FABRIC FOR DRAINAGE	ELASTOMERIC BEARINGS	PRE:	0"X 1'-9" STRESSED NCRETE ED SLABS
	LUMP SUM	LBS.	LBS.	NO.	LIN.FT.	LIN.FT.	LIN.FT.	TONS	SQ.YDS.	LUMP SUM	NO.	LIN.FT.
SUPERSTRUCTURE	LUMP SUM					158.46	174.80			LUMP SUM	26	1,134.38
END BENT #1		2,459		9	225			225	250			
BENT #1		7,574	2203									
END BENT #2		2,460		8	200			300	335			
TOTAL	LUMP SUM	12,493	2203	17	425	158.46	174.80	525	585	LUMP SUM	26	1,134.38

HYDRAULIC DATA

= 3140 C.F.S. DESIGN DISCHARGE FREQUENCY OF DESIGN FLOOD = 25 YR. = 263.800 DESIGN HIGH WATER ELEVATION DRAINAGE AREA = 5.37 SQ.MI.BASIC DISCHARGE (Q100) = 4398 C.F.S. BASIC HIGH WATER ELEVATION = 264.800

OVERTOPPING FLOOD DATA

= 2498 C.F.S. OVERTOPPING DISCHARGE FREQUENCY OF OVERTOPPING FLOOD = < 10 YR. * OVERTOPPING FLOOD ELEVATION

* OVERTOPPING OCCURS AT ROADWAY STA. 19+25.00 -L-AT ROADWAY CENTERLINE.

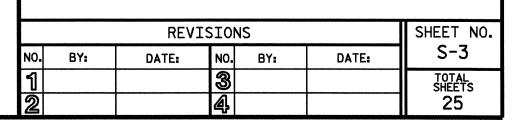
B-4109 PROJECT NO. _ DURHAM COUNTY 16+21.00 -L-STATION:

SHEET 3 OF 3

STATE OF NORTH CAROLINA DEPARTMENT OF TRANSPORTATION RALEIGH

GENERAL DRAWING

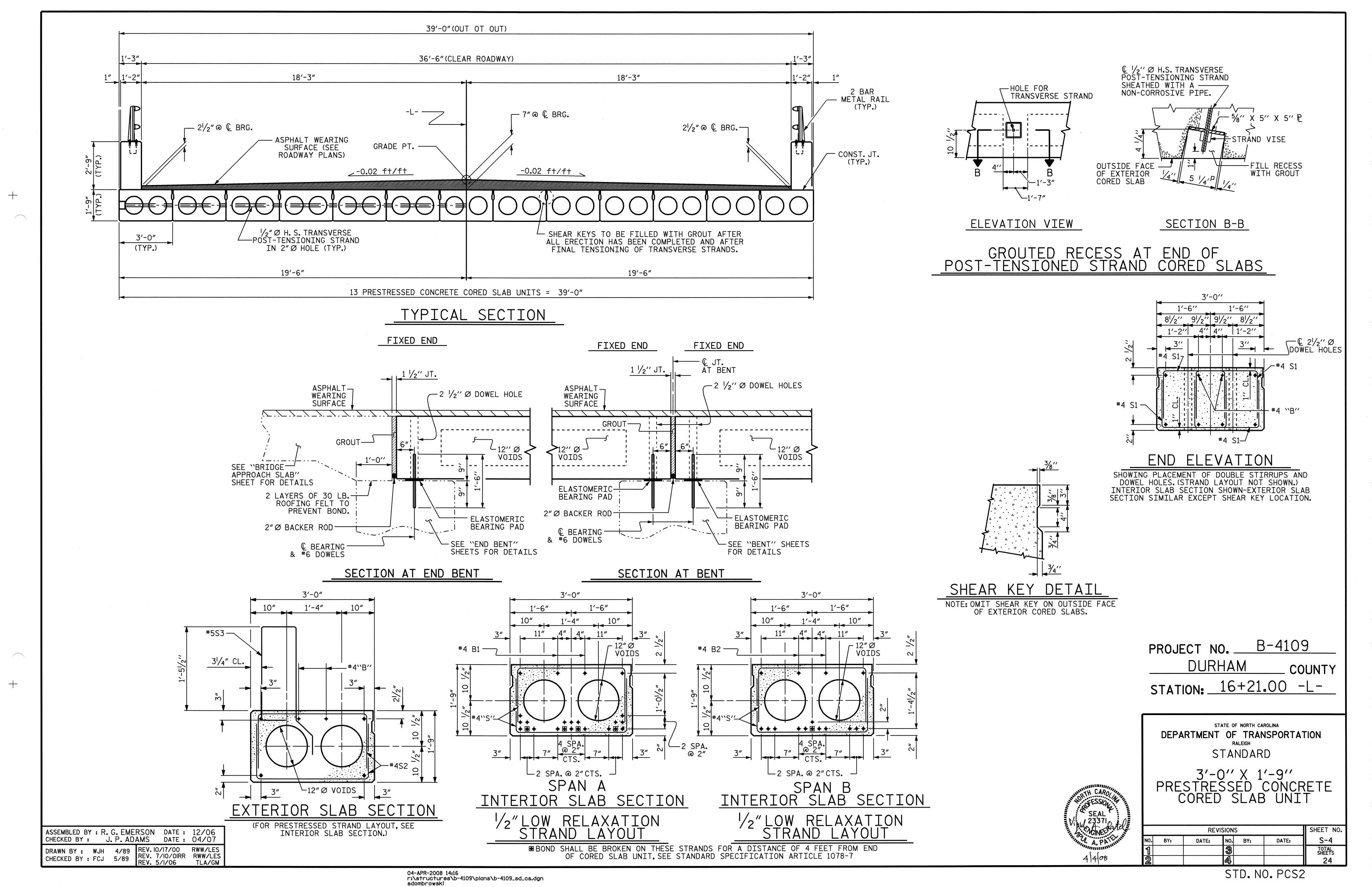
FOR BRIDGE OVER MUD CREEK ON SR 1303 (PICKETT RD) BETWEEN SR 1302 (CAMBRIDGE RD) & SR 1116 (GARRETT RD)

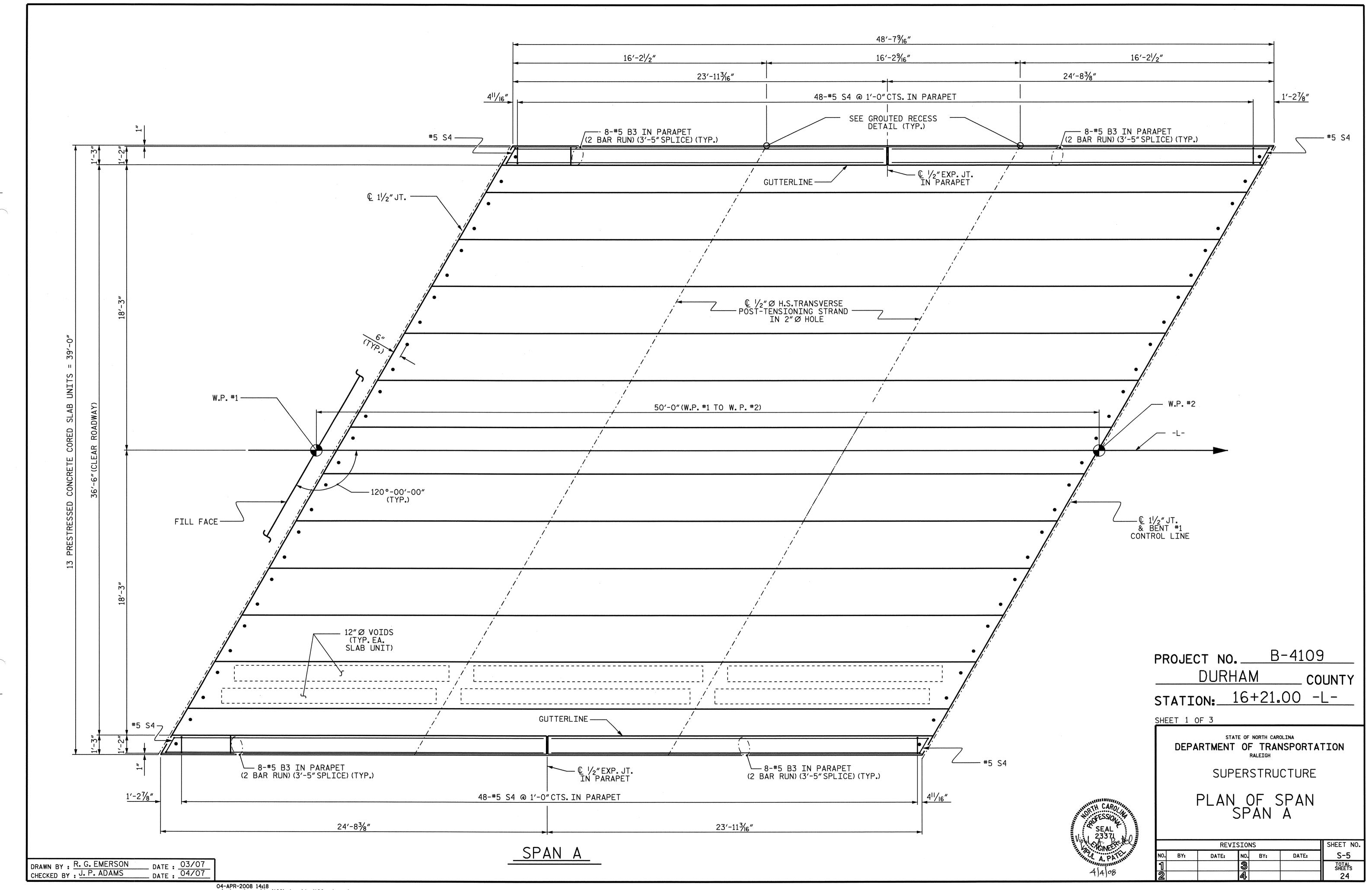


S. DOMBROWSKI _ DATE : K.D. LAYNE CHECKED BY : _ DATE : _

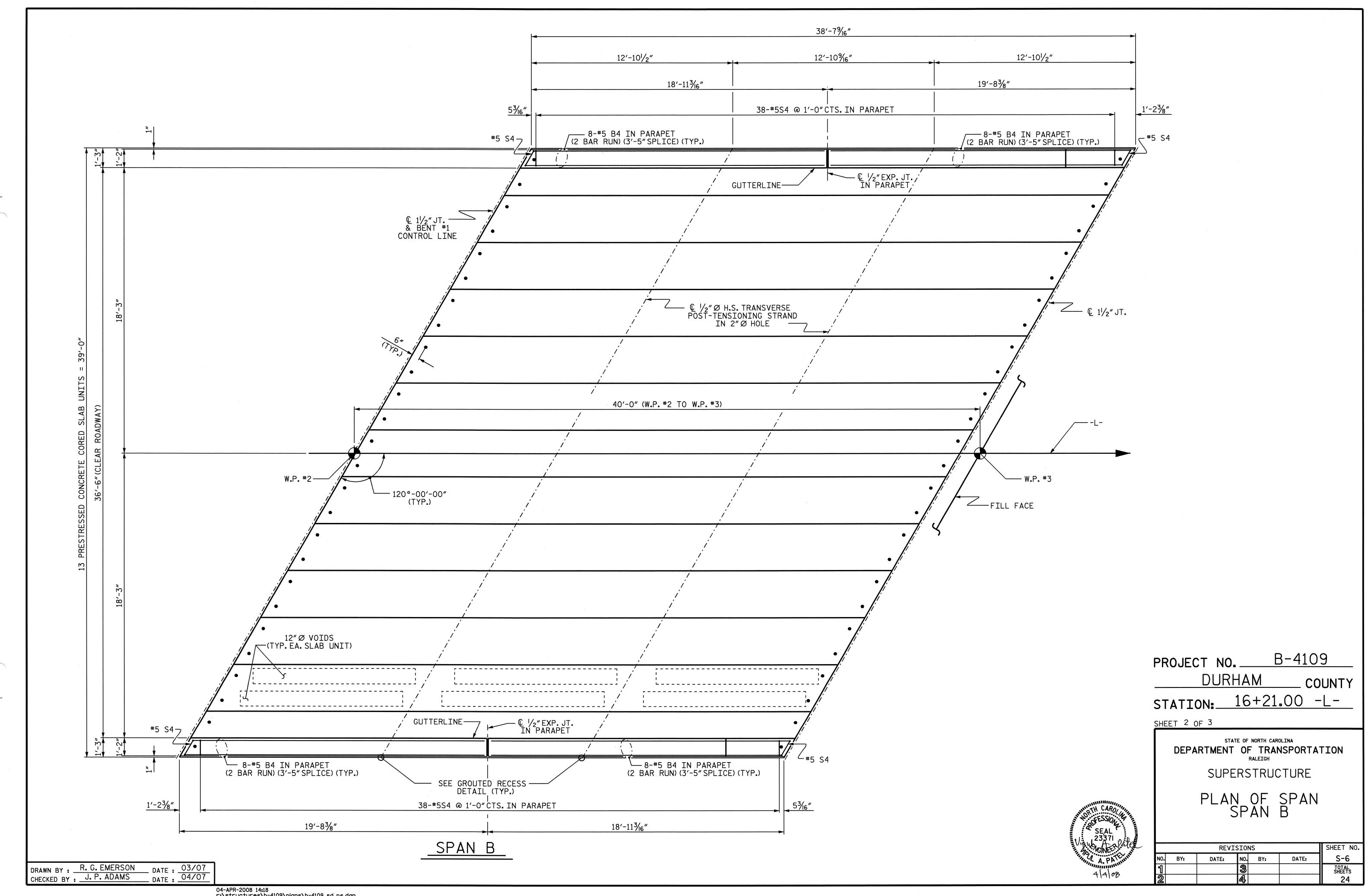
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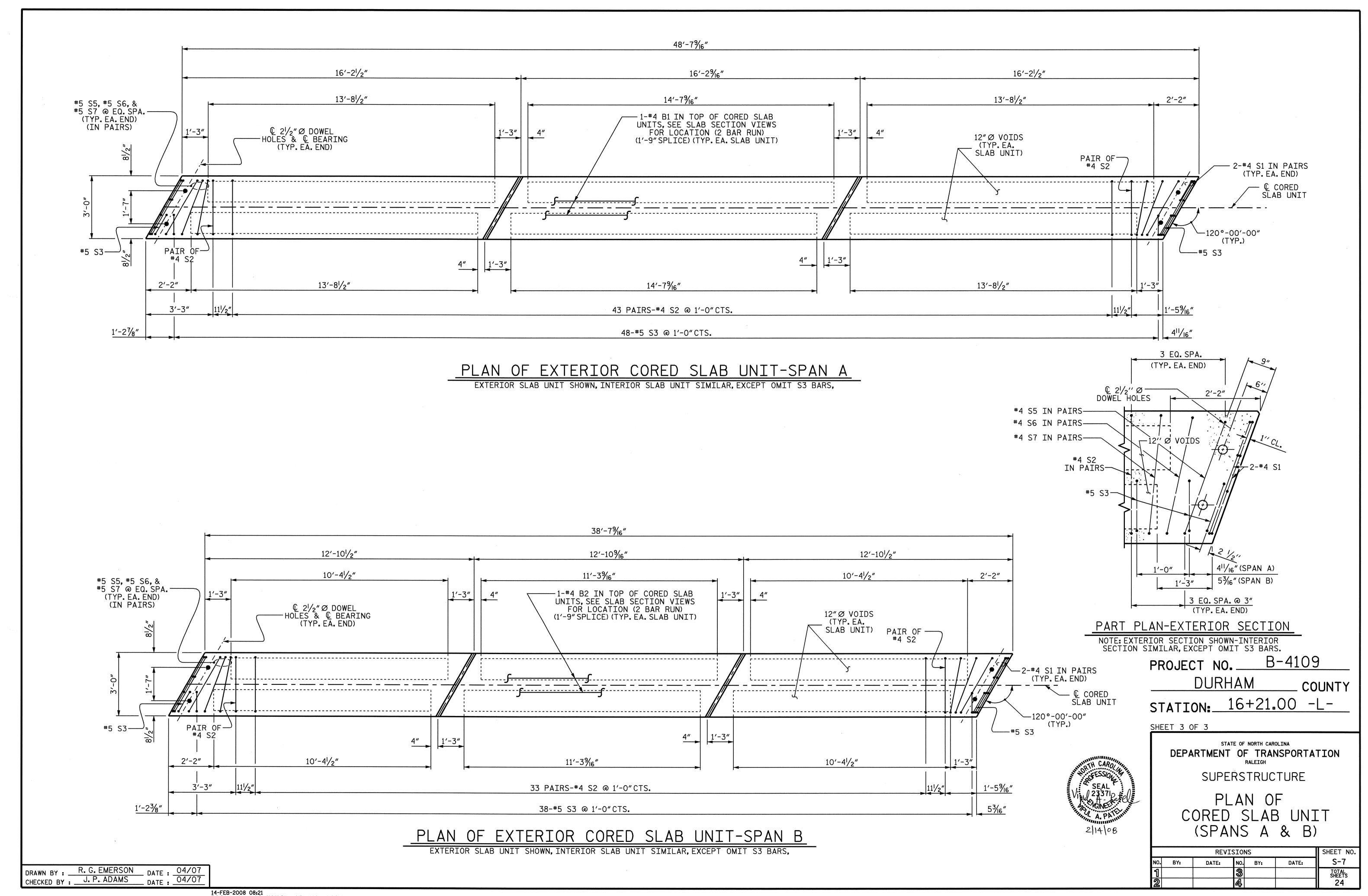




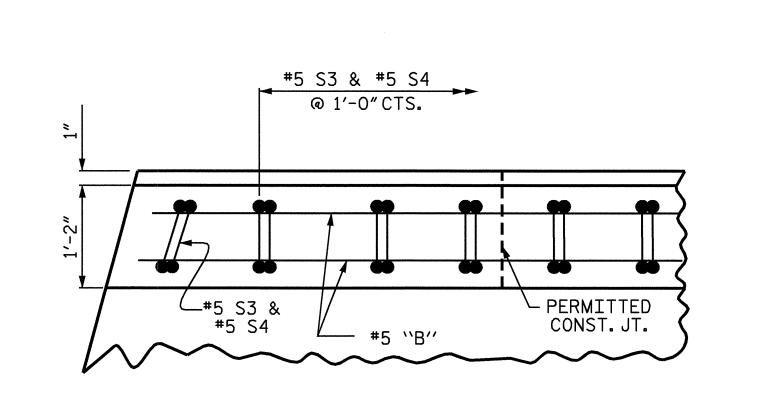
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sdombrowski



14-FEB-2008 08:21 R:\Structures\B-4109\Plans\B-4109_sd_PS.dgn



PLAN OF PARAPET

3'-9"

5-#7 "E" BARS @ 21/2"

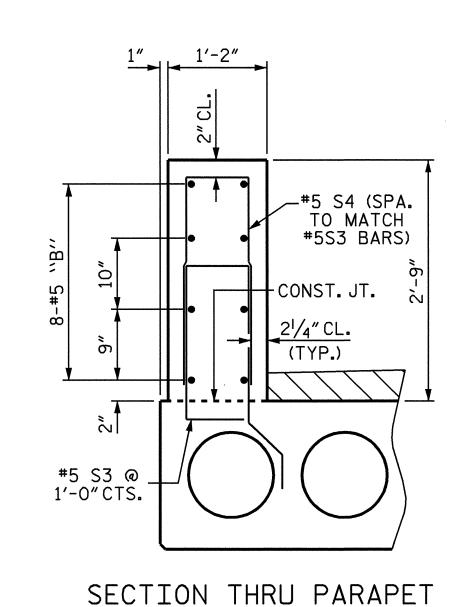
91/2" CTS. (EA. FACE)

#6 "F"BARS

1'-10"

Q GUARDRAIL

ANCHOR ASSEMBLY

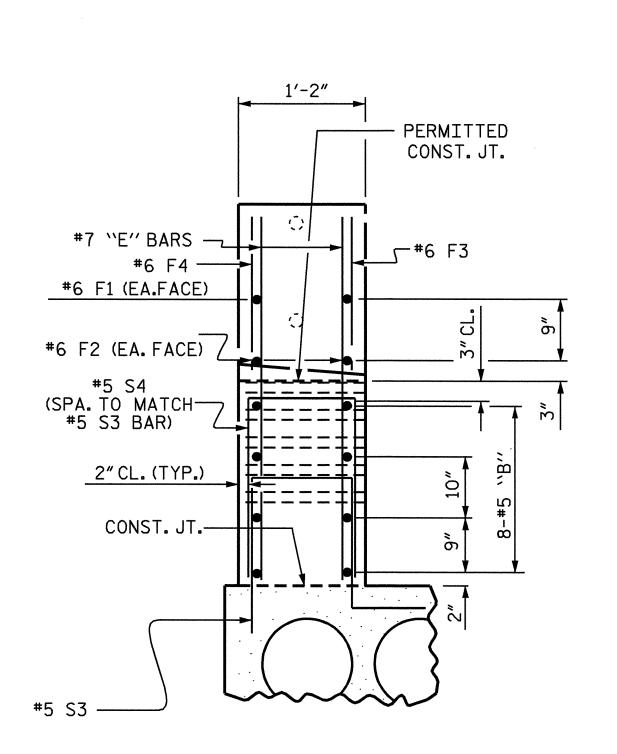


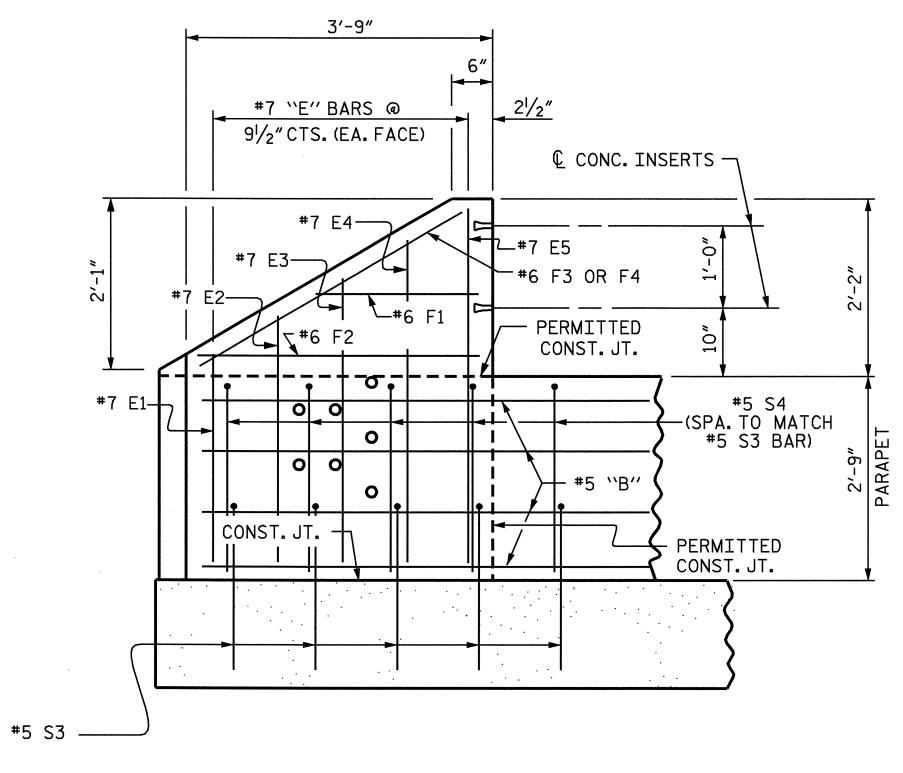
NOTES:

ALL REINFORCING STEEL IN THE PARAPET SHALL BE EPOXY COATED.

FOR DETAIL OF CONCRETE INSERT AND GUARDRAIL ANCHOR ASSEMBLY, SEE "RAIL POST SPACING AND END OF RAIL DETAILS" SHEET.







© OPEN JT. PLACE WITH GALVANIZED NAILS.
(NOTE: OMIT EXP. JT. MAT'L.
WHEN SLIP FORMING IS USED)

CHAMFER

3/4"

CHAMFER

3/4"

CHAMFER

CHAMFER

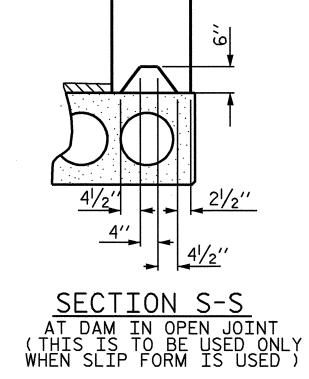
CHAMFER

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PARAPET DETAILS

PROJECT NO. B-4109

DURHAM COUNTY

STATION: 16+21.00 -L-

DEPARTMENT OF TRANSPORTATION
RALEIGH

END POSTS AND PARAPET DETAILS

FOR TWO BAR METAL RAIL

 REVISIONS
 SHEET NO.

 NO.
 BY:
 DATE:
 S-8

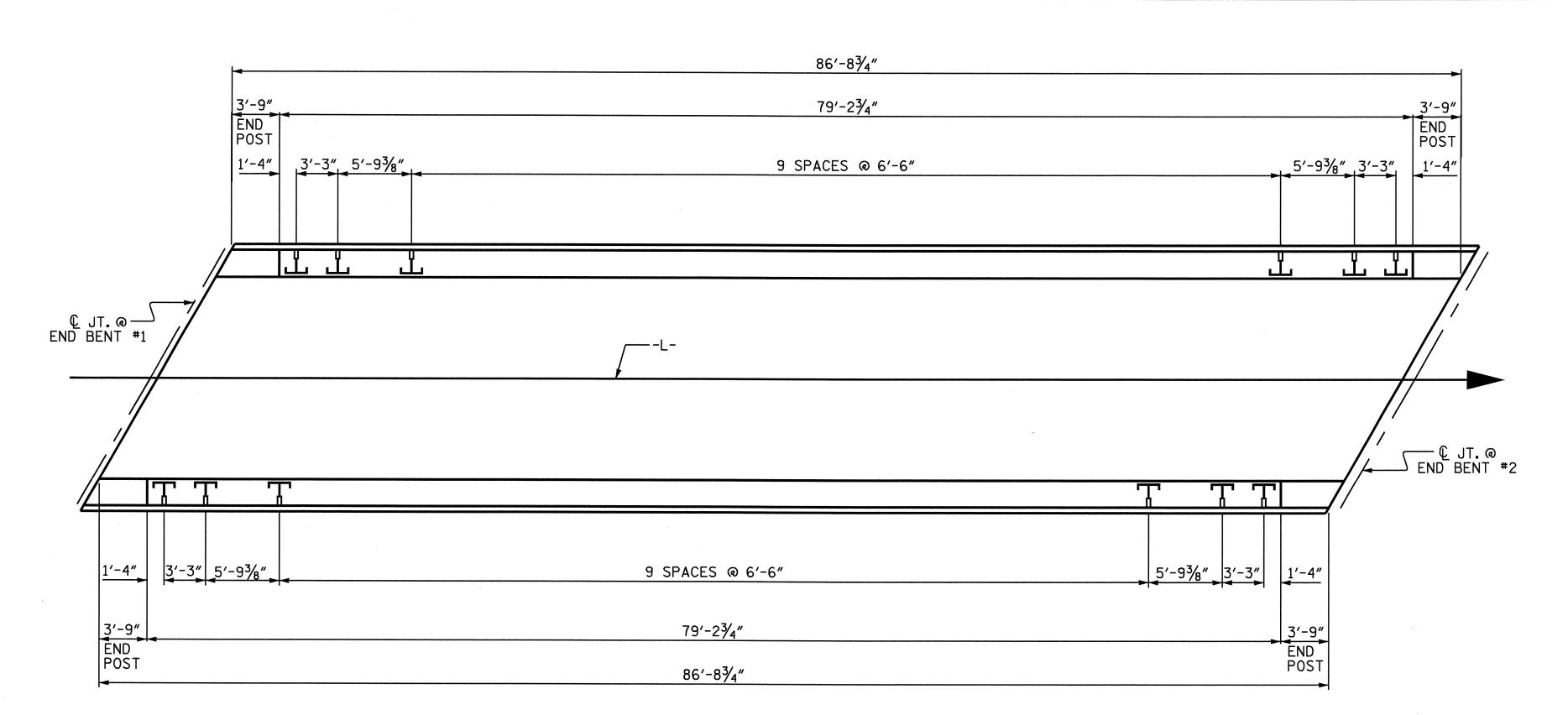
 1
 3
 TOTAL SHEETS

 2
 4
 24

END VIEW ELEVATION

PARAPET AND END POST FOR TWO BAR RAIL

DRAWN BY: R.G. EMERSON DATE: 12/06
CHECKED BY: J. P. ADAMS DATE: 04/07



PLAN OF RAIL POST SPACINGS

NOTES

STRUCTURAL CONCRETE INSERT

THE STRUCTURAL CONCRETE INSERT ASSEMBLY SHALL CONSIST OF THE FOLLOWING COMPONENTS:

- A. FERRULES SHALL BE MADE FROM STEEL MEETING THE REQUIREMENTS OF AASHTO M169, GRADE 12L14 AND SHALL HAVE A MINIMUM LENGTH OF THREADS OF $1\frac{1}{2}$.
- B. $1-\frac{3}{4}$ " Ø X $1\frac{5}{8}$ " BOLT WITH WASHER.BOLT SHALL CONFORM TO THE REQUIREMENTS OF ASTM A307.BOLT AND WASHER SHALL BE GALVANIZED. (AT THE CONTRACTOR'S OPTION, STAINLESS STEEL BOLT AND WASHER MAY BE USED AS AN ALTERNATE FOR THE $\frac{3}{4}$ " Ø X $1\frac{5}{8}$ " GALVANIZED BOLT AND WASHER.THEY SHALL CONFORM TO OR EXCEED THE MECHANICAL REQUIREMENTS OF ASTM A307. THE USE OF THIS ALTERNATE SHALL BE APPROVED BY THE ENGINEER.)
- C. WIRE STRUT SHOWN IN THE CONCRETE INSERT ASSEMBLY DETAIL IS THE MINIMUM ALLOWABLE SIZE AND SHALL HAVE A MINIMUM TENSILE STRENGTH OF 100,000 PSI. AS AN OPTION, A 7_{16} " Ø WIRE STRUT WITH A MINIMUM TENSILE STRENGTH OF 90,000 PSI IS ACCEPTABLE.

NOTES

METAL RAIL TO END POST CONNECTION

THE METAL RAIL TO END POST CONNECTION SHALL CONSIST OF THE FOLLOWING COMPONENTS:

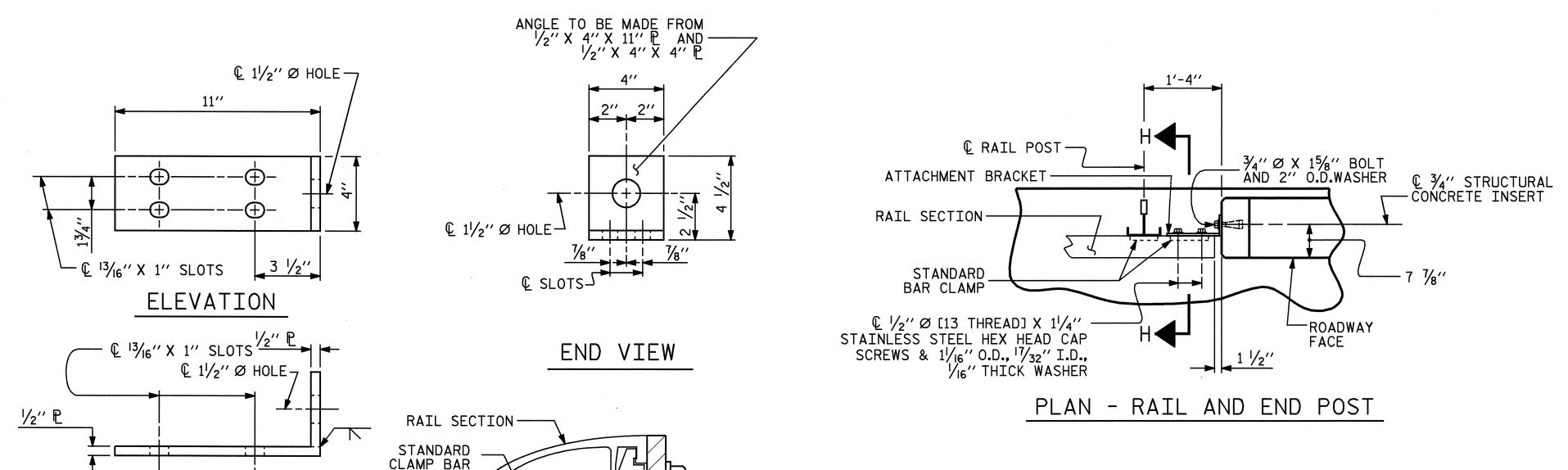
- A. 1/2" PLATES SHALL CONFORM TO AASHTO M270 GRADE 36 AND SHALL BE GALVANIZED AFTER FABRICATION.
- B. $\frac{3}{4}$ " STRUCTURAL CONCRETE INSERT SHALL HAVE A WORKING LOAD SHEAR CAPACITY OF 4800 LBS. THE FERRULES SHALL ENGAGE A $\frac{3}{4}$ " Ø X $1\frac{5}{8}$ " BOLT WITH 2" O.D. WASHER IN PLACE. THE $\frac{3}{4}$ " Ø X $1\frac{5}{8}$ " BOLT SHALL HAVE N. C. THREADS.
- C. CAP SCREWS FOR RAIL ATTACHMENT TO ANGLE SHALL CONFORM TO THE REQUIREMENTS OF ASTM F593 ALLOY 305 STAINLESS STEEL. CAP SCREWS TO BE CENTERED IN SLOTS AT 60°F.
- D. STANDARD CLAMP BARS (SEE METAL RAIL SHEET).
- E. $\frac{1}{2}$ " Ø PIPE SLEEVES (IF REQUIRED) TO BE GALVANIZED.

THE COST OF THE STANDARD CLAMP BARS AND CAP SCREWS USED IN THE METAL RAIL TO END POST CONNECTION SHALL BE INCLUDED IN THE UNIT CONTRACT PRICE BID FOR LINEAR FEET OF 1 OR 2 BAR METAL RAILS.

THE $\frac{3}{4}$ " STRUCTURAL CONCRETE INSERT WITH BOLT SHALL BE ASSEMBLED IN THE SHOP.

THE COST OF THE $\frac{3}{4}$ " STRUCTURAL CONCRETE INSERT ASSEMBLY, AND THE $\frac{1}{2}$ " PLATES COMPLETE IN PLACE SHALL BE INCLUDED IN THE VARIOUS PAY ITEMS.

THE CONTRACTOR, AT HIS OPTION, MAY USE AN ADHESIVE BONDING SYSTEM IN LIEU OF THE STRUCTURAL CONCRETE INSERT EMBEDDED IN THE END POST. IF THE ADHESIVE BONDING SYSTEM IS USED, THE $\frac{3}{4}$ " $\frac{3}{4}$ " $\frac{6}{2}$ " BOLT WITH WASHER SHALL BE REPLACED WITH A $\frac{3}{4}$ " $\frac{6}{2}$ " BOLT AND 2" O.D. WASHER. ALL SPECIFICATIONS THAT APPLY TO THE $\frac{3}{4}$ " $\frac{6}{2}$ " BOLT. FIELD TESTING OF THE ADHESIVE BONDING SYSTEM IS NOT REQUIRED.



 $\mathbb{Q}^{1/2}$ " Ø [13 THREAD] X $1^{1/4}$ " STAINLESS STEEL HEX

HEAD CAP SCREWS & 1/16" O.D., 17/32" I.D., 1/16" THICK WASHER

PLAN ELEVATION

STRUCTURAL CONCRETE

TNSFRT

R.P.W.(TYP.ALL\

CONTACT POINTS)/

FERRULE-

*EACH WELDED ATTACHMENT OF WIRE TO FERRULE SHALL DEVELOP THE TENSILE STRENGTH OF THE WIRE.

CLOSED-END

FERRULE

PROJECT NO. B-4109

DURHAM COUNTY

STATION: 16+21.00 -L-

DEPARTMENT OF TRANSPORTATION
RALEIGH
STANDARD

REVISIONS

BY: DATE: NO. BY: DATE: S-9

TOTAL SHEETS
24

DETAILS FOR ATTACHING METAL RAIL TO END POST

SECTION H-H

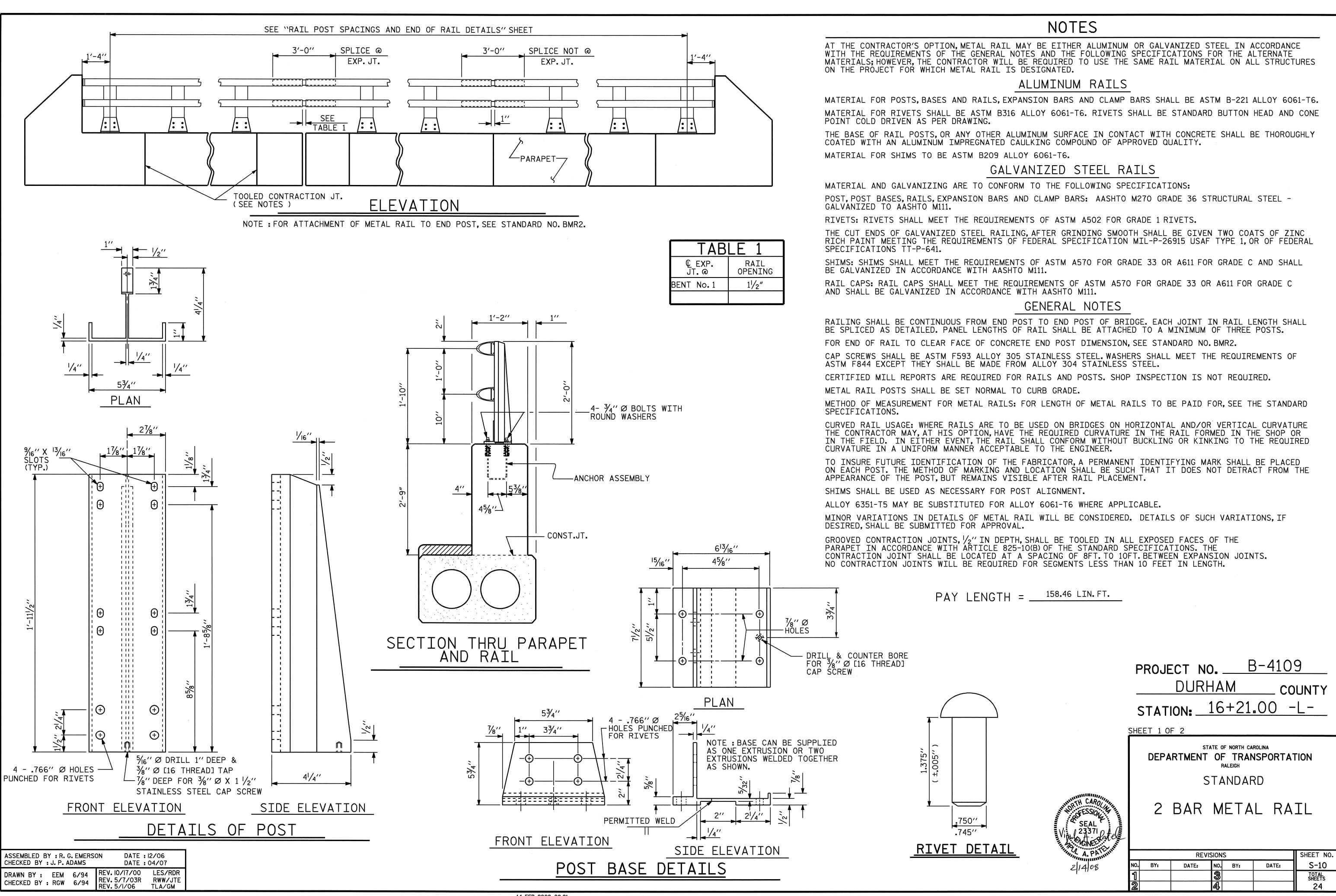
ASSEMBLED BY : R. G. EMERSON DATE : 12/06
CHECKED BY : J. P. ADAMS DATE :03/07

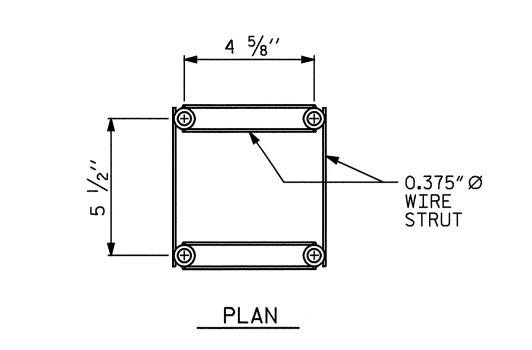
DRAWN BY : FCJ I/88
CHECKED BY : CRK 3/89

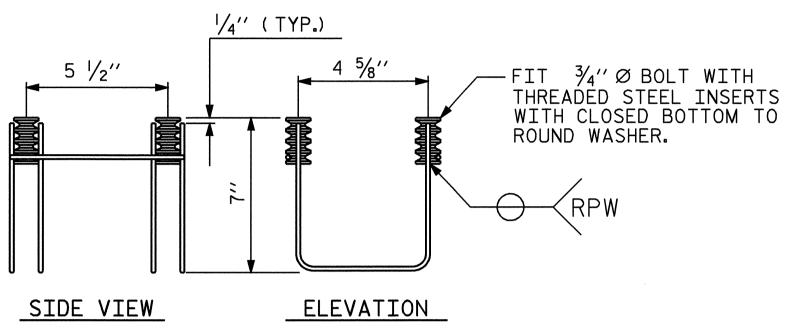
REV. 5/7/03
RWW/JTE
REV. 5/1/06
TLA/GM

3 3/4"

TOP VIEW







MINIMUM LENGTH OF THREADS IN INSERT (FERRULE): 13/4"

4-BOLT METAL RAIL ANCHOR ASSEMBLY

(28 ASSEMBLIES REQUIRED)

NOTES

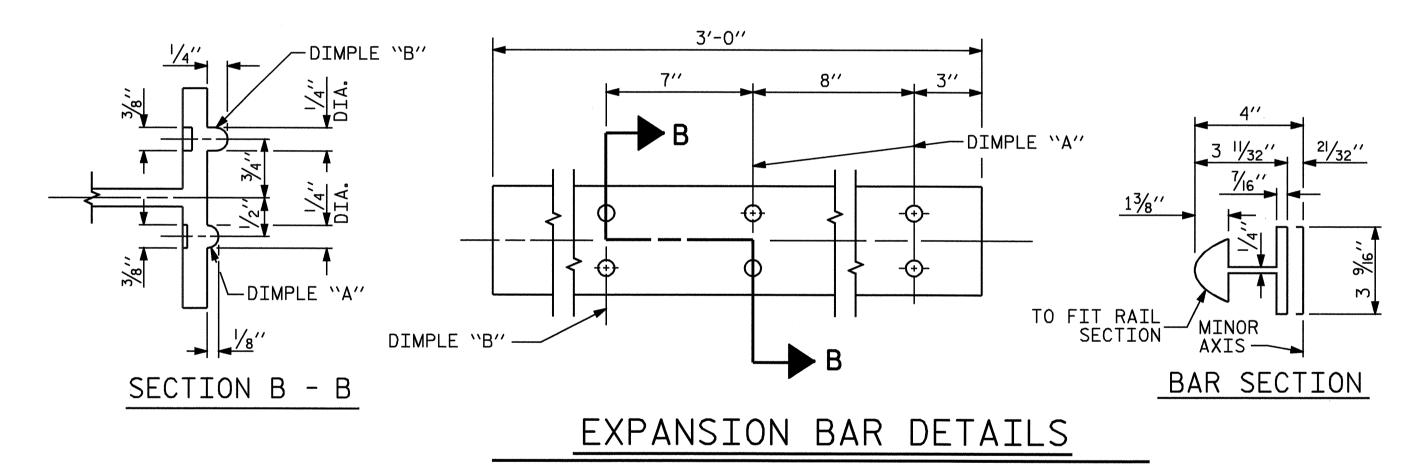
STRUCTURAL CONCRETE ANCHOR ASSEMBLY

THE STRUCTURAL CONCRETE ANCHOR ASSEMBLY SHALL CONSIST OF THE FOLLOWING COMPONENTS:

- A. FERRULES SHALL BE MADE FROM STEEL MEETING THE REQUIREMENTS OF AASHTO M169. GRADE 12L14 AND SHALL HAVE A MINIMUM LENGTH OF THREADS OF 2" FOR 3/4" FERRULES.
- B. 4 3/4" Ø X 21/2" BOLTS WITH WASHERS. BOLTS SHALL CONFORM TO THE REQUIREMENTS OF ASTM A307. BOLTS AND WASHERS SHALL BE GALVANIZED. AT THE CONTRACTOR'S OPTION, STAINLESS STEEL BOLTS AND WASHERS MAY BE USED AS AN ALTERNATE FOR THE $\sqrt[3]{4}$ " \varnothing X $2^{1}/2$ " GALVANIZED BOLTS AND WASHERS. THEY SHALL CONFORM TO OR EXCEED THE MECHANICAL REQUIREMENTS OF ASTM A307. THE USE OF THIS ALTERNATE SHALL BE APPROVED BY THE ENGINEER.
- C. WIRE STRUT SHOWN IN THE CONCRETE ANCHOR ASSEMBLY DETAIL IS THE MINIMUM ALLOWABLE SIZE AND SHALL HAVE A MINIMUM TENSILE STRENGTH OF 100,000 PSI. AS AN OPTION, A $7_{16}^{\prime\prime}$ Ø WIRE STRUT WITH A MINIMUM TENSILE STRENGTH OF 90,000 PSI IS ACCEPTABLE.
- D. THE METAL RAIL ANCHOR ASSEMBLIES TO BE HOT DIPPED GALVANIZED TO CONFORM TO REQUIREMENTS OF AASHTO M111.
- E. THE COST OF THE METAL RAIL ANCHOR ASSEMBLY WITH BOLTS AND WASHERS COMPLETE IN PLACE SHALL BE INCLUDED IN THE PRICE BID FOR LINEAR FEET OF METAL RAIL.
- F. BOLTS TO BE TIGHTENED ONE-HALF TURN WITH A WRENCH FROM A FINGER-TIGHT POSITION.

THE CONTRACTOR MAY USE ADHESIVELY ANCHORED ANCHOR BOLTS IN PLACE OF THE METAL RAIL ANCHOR ASSEMBLY. LEVEL ONE FIELD TESTING IS REQUIRED, AND THE YIELD LOAD OF THE 34% BOLT IS 10 KIPS. FOR ADHESIVELY ANCHORED ANCHOR BOLTS OR DOWELS, SEE SPECIAL PROVISIONS.

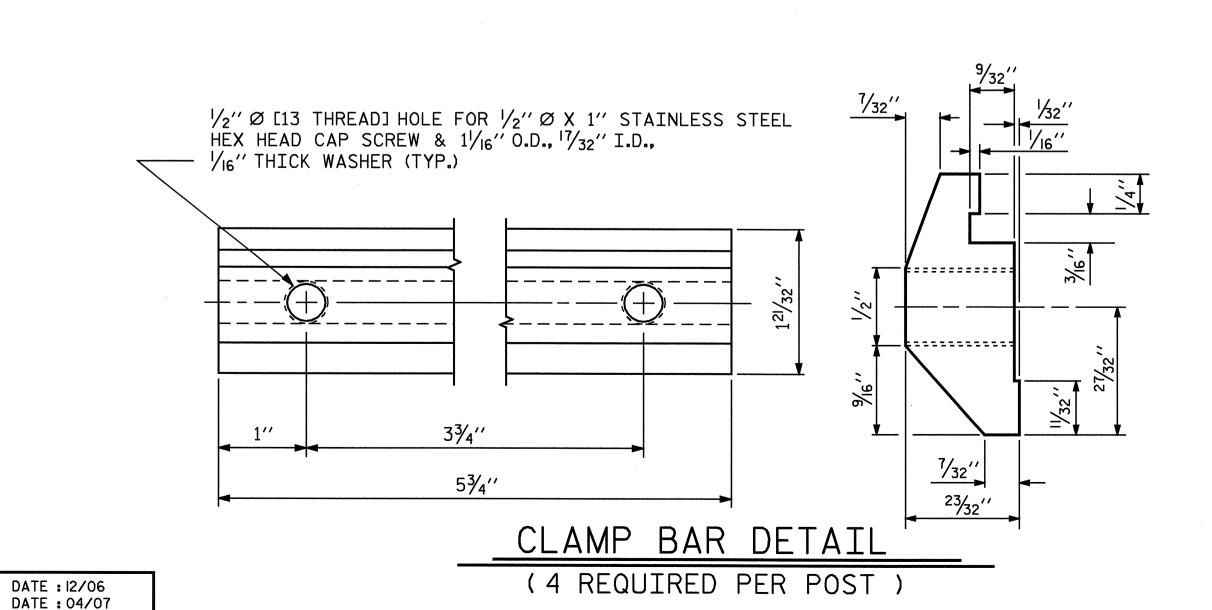
WHEN ADHESIVELY ANCHORED ANCHOR BOLTS ARE USED, BOLTS SHALL MEET THE REQUIREMENTS OF ASTM F593 ALLOY 304 STAINLESS STEEL WITH MINIMUM 75,000 PSI ULTIMATE STRENGTH. NUTS SHALL MEET THE REQUIREMENTS OF ASTM F594 ALLOY 304 STAINLESS STEEL AND WASHERS SHALL MEET THE REQUIREMENTS OF ASTM F844 EXCEPT THEY SHALL BE MADE FROM ALLOY 304 STAINLESS STEEL.

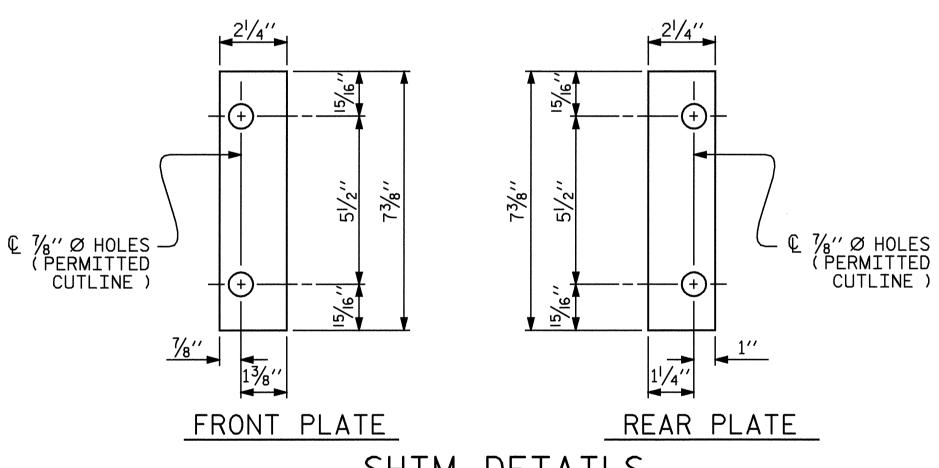


ASSEMBLED BY : R. G. EMERSON

DRAWN BY: EEM 6/94 REV. 2/6/97 EEM/RGW REV. 8/16/99 MAB/LES REV. 5/1/06R KMM/GM

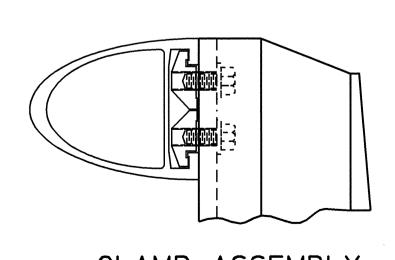
CHECKED BY : J. P. ADAMS



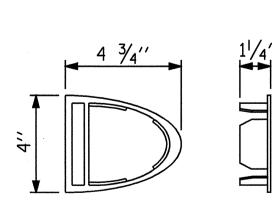


SHIM DETAILS

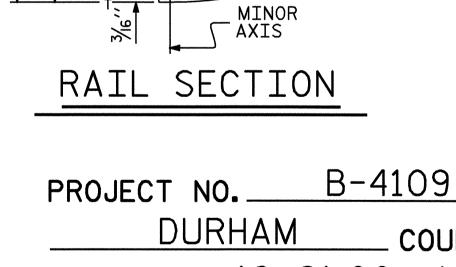
NOTE: SHIMS MAY BE CUT ALONG PERMITTED CUTLINE OR SLOTTED TO EDGE OF PLATE TO FACILITATE PLACEMENT.



CLAMP ASSEMBLY



RAIL CAP



COUNTY 16+21.00 -L-STATION:_

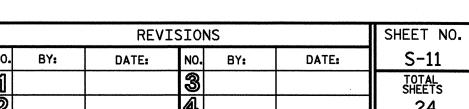
/- SEMI-ELLIPSE

MAJOR

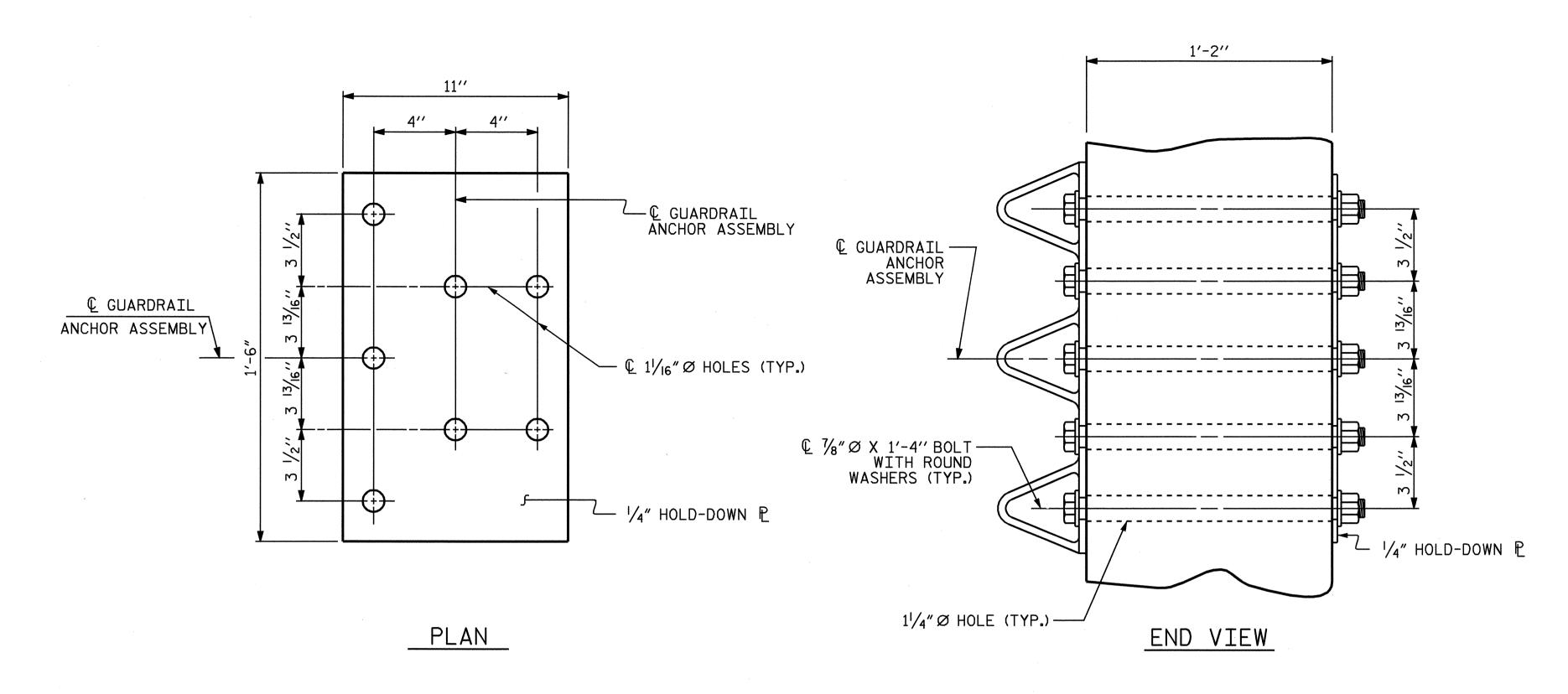
SHEET 2 OF 2

STATE OF NORTH CAROLINA DEPARTMENT OF TRANSPORTATION STANDARD

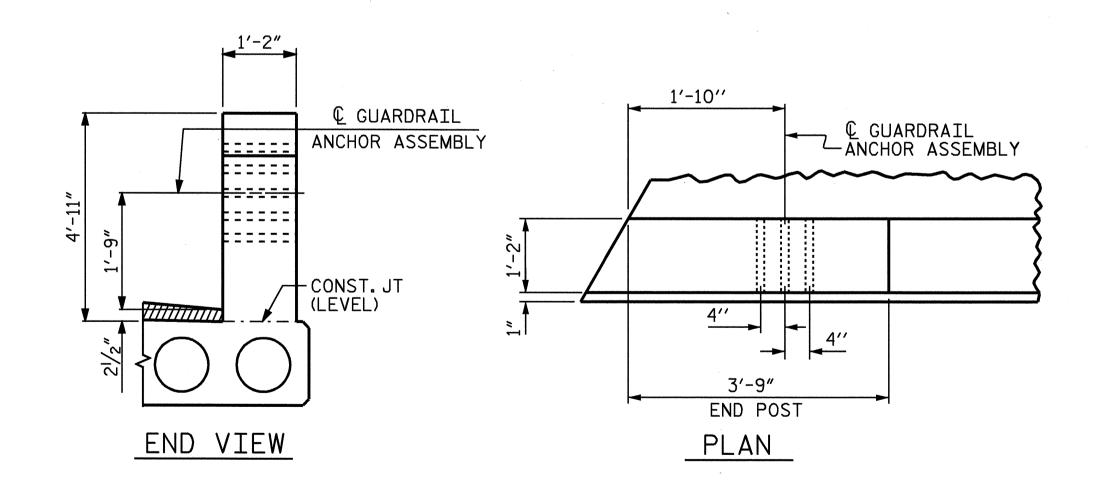
2 BAR METAL RAIL



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sdombrowski



GUARDRAIL ANCHOR ASSEMBLY DETAILS



LOCATION OF GUARDRAIL ANCHOR AT END POST

ASSEMBLED BY: R.G. EMERSON DATE: 12/06
CHECKED BY: J.P. ADAMS DATE: 03/07

DRAWN BY: EEM 6/94
CHECKED BY: RGW 6/94
REV. 5/7/03
REV. 5/1/06
REV. 5/1/06

NOTES

THE GUARDRAIL ANCHOR ASSEMBLY SHALL CONSIST OF A $1/4^{\prime\prime}$ HOLD DOWN PLATE AND 7 - $1/8^{\prime\prime}$ Ø BOLTS WITH NUTS AND WASHERS.

THE HOLD-DOWN PLATE SHALL CONFORM TO AASHTO M270 GRADE 36.AFTER FABRICATION, THE HOLD-DOWN PLATE SHALL BE HOT-DIP GALVANIZED IN ACCORDANCE WITH AASHTO M111.

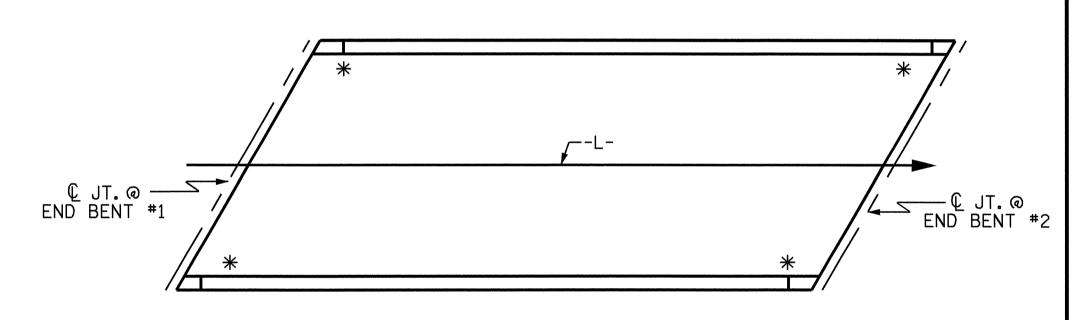
BOLTS SHALL CONFORM TO THE REQUIREMENTS OF ASTM A307 AND NUTS SHALL CONFORM TO THE REQUIREMENTS OF AASHTO M291. BOLTS, NUTS AND WASHERS SHALL BE GALVANIZED. AT THE CONTRACTOR'S OPTION, STAINLESS STEEL BOLTS, NUTS AND WASHERS MAY BE USED AS AN ALTERNATE FOR THE 1/8" Ø GALVANIZED BOLTS, NUTS AND WASHERS. THEY SHALL CONFORM TO OR EXCEED THE MECHANICAL REQUIREMENTS OF ASTM A307. THE USE OF THIS ALTERNATE SHALL BE APPROVED BY THE ENGINEER.

AFTER INSTALLATION, THE EXPOSED THREAD OF THE BOLT SHALL BE BURRED WITH A SHARP POINTED TOOL.

THE COST OF THE GUARDRAIL ANCHOR ASSEMBLIES WITH BOLTS, NUTS AND WASHERS COMPLETE IN PLACE, SHALL BE INCLUDED IN THE VARIOUS PAY ITEMS.

THE VERTICAL REINFORCING BARS MAY BE SHIFTED SLIGHTLY IN THE END POST TO CLEAR ASSEMBLY BOLTS.

THE 1 $\frac{1}{4}$ " Ø HOLES SHALL BE FORMED OR DRILLED WITH A CORE BIT. IMPACT TOOLS WILL NOT BE PERMITTED. ANY CONCRETE DAMAGED BY THIS WORK SHALL BE REPAIRED TO THE SATISFACTION OF THE ENGINEER.



*LOCATION OF GUARDRAIL ATTACHMENT

PROJECT NO. B-4109

DURHAM COUNTY

STATION: 16+21.00 -L-

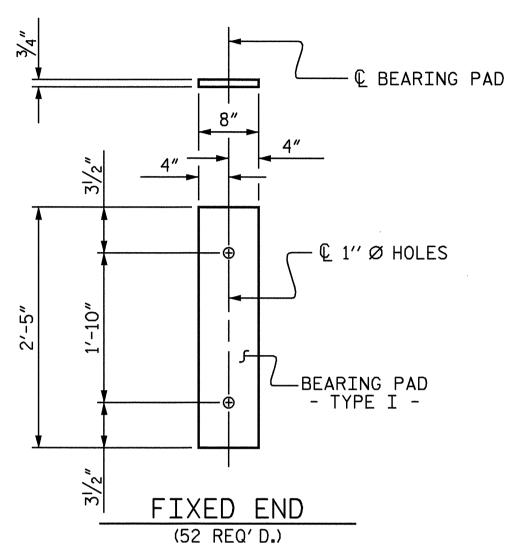
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DEPARTMENT OF TRANSPORTATION
RALEIGH
STANDARD
GUARDRAIL ANCHORAGE
DETAILS
FOR METAL RAILS

		SHEET NO.				
0.	BY:	DATE:	NO.	BY:	DATE:	S-12
0			જી			TOTAL SHEETS
2						24

DEAD LOAD DEFLECTION AND CAMBER								
	SPAN A	SPAN B						
	½″Ø L.R. STRAND	¹/₂″Ø L.R. STRAND						
CAMBER (SLAB ALONE IN PLACE)	17⁄8″ ♠	13/ ₁₆ " 🛉						
DEFLECTION DUE TO SUPERIMPOSED DEAD LOAD	5/16″ ₩	1/8″ ₩						
FINAL CAMBER	19⁄ ₁₆ ″ ♦	/16″ ∱						

** INCLUDES FUTURE WEARING SURFACE

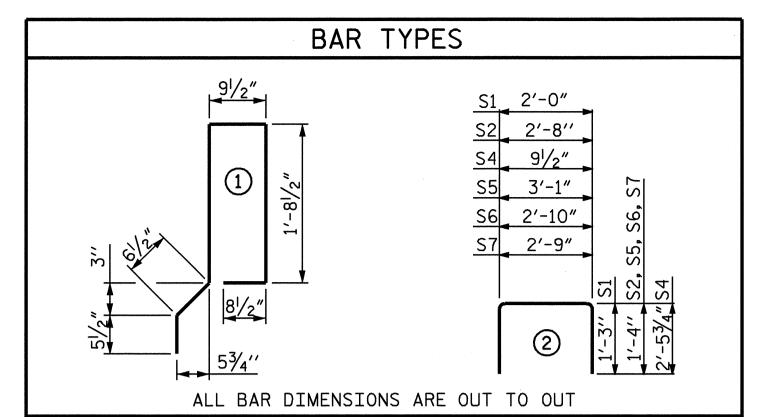


ELASTOMERIC BEARING DETAILS

CORED SLABS REQUIRED								
SPAN A	NUMBER	LENGTH	TOTAL LENGTH					
EXTERIOR C.S.	2	48'-7 ⁹ / ₁₆ "	97.26					
INTERIOR C.S.	11	48′-79 ₁₆ "	534.93					
TOTAL	13	48′-79 ₁₆ "	632.19					
SPAN B	NUMBER	LENGTH	TOTAL LENGTH					
EXTERIOR C.S.	2	38′-7 ⁹ / ₁₆ "	77.26					
INTERIOR C.S.	11	38′-79⁄ ₁₆ ″	424.93					
TOTAL	38'-7 ⁹ / ₁₆ "	502.19						
TOTAL LENGTH (SPAN A & B) 1134.38								

GRADE 270 STRANDS						
	1/₂″Ø L.R.					
AREA (SQUARE INCHES)	0.153					
ULTIMATE STRENGTH (LBS.PER STRAND)	41,300					
APPLIED PRESTRESS (LBS.PER STRAND)	30,980					

BILL OF MATERIAL FOR PARAPET AND END POST									
BAR	NO.	SIZE	TYPE	LENGTH	WEIGHT				
 ₩ B3	64	#5	STR.	13′-11″	929				
 ₩84	64	#5	STR.	11′-5″	762				
₩E1	8	#7	STR.	2′-10″	46				
 ₩E2	8	#7	STR.	3′-4″	55				
 ₩E3	8	#7	STR.	3′-10″	63				
 ₩E4	8	#7	STR.	4'-4"	71				
₩E5	8	#7	STR.	4′-8″	76				
米 F1	8	#6	STR.	2′-0″	24				
米 F2	8	#6	STR.	3′-5″	41				
∗ F3	4	#6	STR.	4′-6″	27				
 ₩F4	4	#6	STR.	4'-0"	24				
* S4	180	#5	2	5′-9″	1080				
* EPOXY COATED REINFORCING STEEL 3198 LBS.									
CLASS AA CONCRETE 21.4 CU. YDS.									
1'-2" CONC	X 2'- RETE	-9" : PAR	APET	174.80	LIN.FT.				



BILL OF MATERIAL FOR ONE CORED SLAB UNIT-SPAN A

EXTERIOR UNIT | INTERIOR UNIT

BAR	NUMBER	SIZE	TYPE	LENGTH	WEIGHT	LENGTH	WEIGHT
B1	4	#4	STR	25′-1″	67	25′-1″	67
S1	8	#4	2	4'-6"	24	4'-6"	24
S2	90	#4	2	5′-4″	321	5′-4″	321
* S3	50	#5	1	5′-11 ″	309		
S5	4	#5	2	5'-9"	24	5'-9"	24
S6	4	#5	2	5′-6″	23	5′-6″	23
S7	4	#5	2	5′-5″	23	5′-5″	23
REINFO	ORCING S	STEEL			482 LBS.		482 LBS.
★ EP0X	Y COATE	D REIN	FORCING	STEEL	309 LBS.		
5,000	P.S.I.CC	7.0	CU. YDS.				
1/2″Ø L	.R. STRA	NDS		No.	22		22

BILL OF MATERIAL FOR ONE CORED SLAB UNIT-SPAN B

				FX I FKT	OK UNTI	TNIERT	OK ONTI
BAR	NUMBER	SIZE	TYPE	LENGTH	WEIGHT	LENGTH	WEIGHT
B2	4	#4	STR	20′-1″	54	20′-1″	54
S1	8	#4	2	4′-6″	24	4'-6"	24
S2	70	#4	2	5′-4″	249	5′-4″	249
* S3	40	#5	1	5′-11″	247		
S5	4	#5	2	5′-9″	24	5′-9″	24
S6	4	#5	2	5′-6″	23	5'-6 "	23
S7	4	#5	2	5′-5 ″	23	5'-5 "	23
REINFO	ORCING S	STEEL			397 LBS.		397 LBS.
⋆ EPOX	Y COATE	D REIN	FORCING	STEEL	247		
5,000	P.S.I. CC	NCRETE		5.7	CU. YDS.	5.7	CU. YDS.
1/2" Ø L	R. STRA	NDS		No.	14		14

NOTES

ALL PRESTRESSING STRANDS SHALL BE 7-WIRE LOW RELAXATION GRADE 270 STRANDS AND SHALL CONFORM TO AASHTO M203 EXCEPT FOR SAMPLING REQUIREMENTS WHICH SHALL BE IN ACCORDANCE WITH THE STANDARD SPECIFICATIONS.

ALL REINFORCING STEEL CAST WITH THE CORED SLAB SECTIONS SHALL BE GRADE 60 AND SHALL BE INCLUDED IN THE UNIT PRICE BID FOR PRESTRESSED CONCRETE CORED SLABS.

RECESSES FOR TRANSVERSE STRANDS SHALL BE GROUTED AFTER THE TENSIONING OF THE STRANDS.

THE $2\frac{1}{2}$ Ø DOWEL HOLES AT FIXED ENDS OF SLAB SECTIONS SHALL BE FILLED WITH GROUT.

THE 2"Ø BACKER ROD SHALL CONFORM TO THE REQUIREMENTS OF TYPE M BOND BREAKER. SEE SECTION 1028 OF THE STANDARD SPECIFICATIONS.

WHEN CORED SLABS ARE CAST, A POSITIVE HOLD-DOWN SYSTEM SHALL BE EMPLOYED TO PREVENT VOIDS FROM RISING OR MOVING SIDEWAYS. THIS SYSTEM SHALL BE DESIGNED TO BE LEFT IN PLACE UNTIL THE CONCRETE HAS REACHED RELEASE STRENGTH. AT LEAST THREE WEEKS PRIOR TO CASTING CORED SLABS, THE CONTRACTOR SHALL SUBMIT TO THE ENGINEER FOR REVIEW AND COMMENT, DETAILED DRAWINGS OF THE PROPOSED HOLD-DOWN SYSTEM. IN ADDITION TO STRUCTURAL DETAILS, LOCATION AND SPACING OF THE HOLD-DOWNS SHALL BE INDICATED.

THE TRANSFER OF LOAD FROM THE ANCHORAGES TO THE CORED SLAB UNIT SHALL BE DONE WHEN THE CONCRETE HAS REACHED A COMPRESSIVE STRENGTH OF NOT LESS THAN 4,000 PSI.

ALL REINFORCING STEEL IN PARAPET AND END POST SHALL BE EPOXY COATED.

PRESTRESSING STRANDS SHALL BE CUT FLUSH WITH THE CORED SLAB UNIT FNDS.

APPLY EPOXY PROTECTIVE COATING TO CORED SLAB UNIT ENDS.

VERTICAL GROOVED CONTRACTION JOINTS, ½" IN DEPTH, SHALL BE TOOLED IN ALL EXPOSED FACES OF THE PARAPET AND IN ACCORDANCE WITH ARTICLE 825-10(B) OF THE STANDARD SPECIFICATIONS. A VERTICAL CONTRACTION JOINT SHALL BE LOCATED AT EACH THIRD POINT BETWEEN PARAPET EXPANSION JOINTS. ONLY ONE CONTRACTION JOINT IS REQUIRED AT MIDPOINT OF PARAPET SEGMENTS LESS THAN 20 FEET IN LENGTH AND NO CONTRACTION JOINTS ARE REQUIRED FOR THOSE SEGMENTS LESS THAN 10 FEET IN LENGTH.

FOR GROUT FOR STRUCTURES, SEE SPECIAL PROVISIONS.

FOR PRESTRESSED CONCRETE MEMBERS, SEE SPECIAL PROVISIONS.

PROJECT NO. B-4109

DURHAM COUNTY

STATION: 16+21.00 -L-

DEPARTMENT OF TRANSPORTATION

RALEIGH

STANDARD

3'-0" X 1'-9"

PRESTRESSED CONCRETE

CORED SLAB UNIT

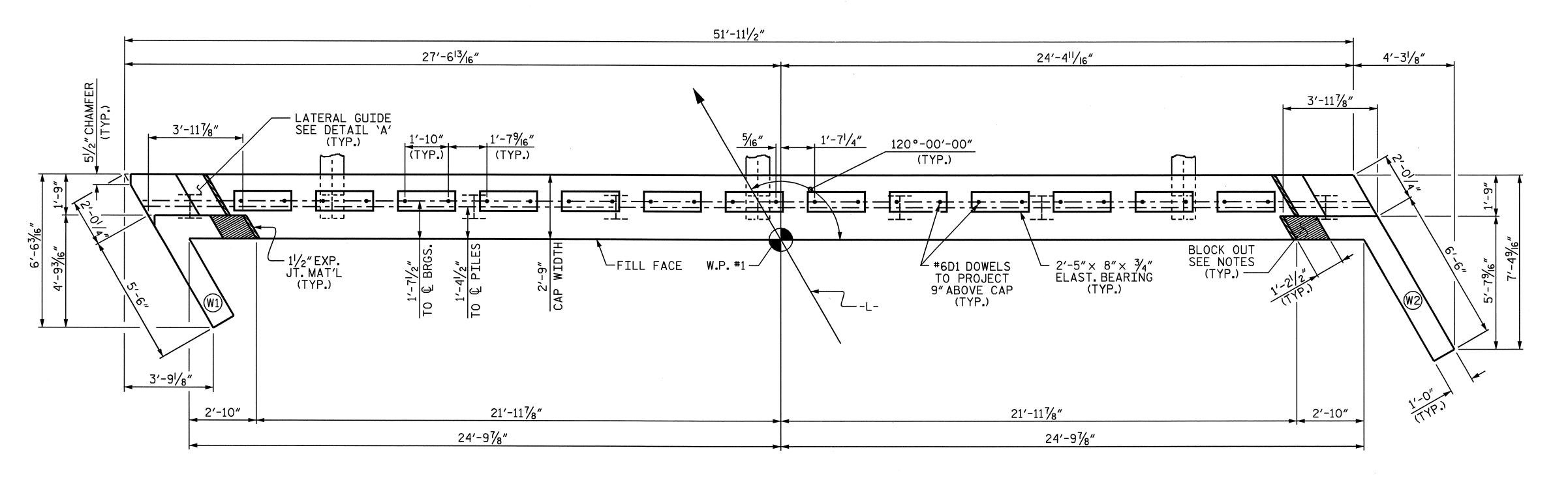


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NO.	BY:	DATE:	NO.	BY:	DATE:	S-13
1			3			TOTAL SHEETS
2			4			24

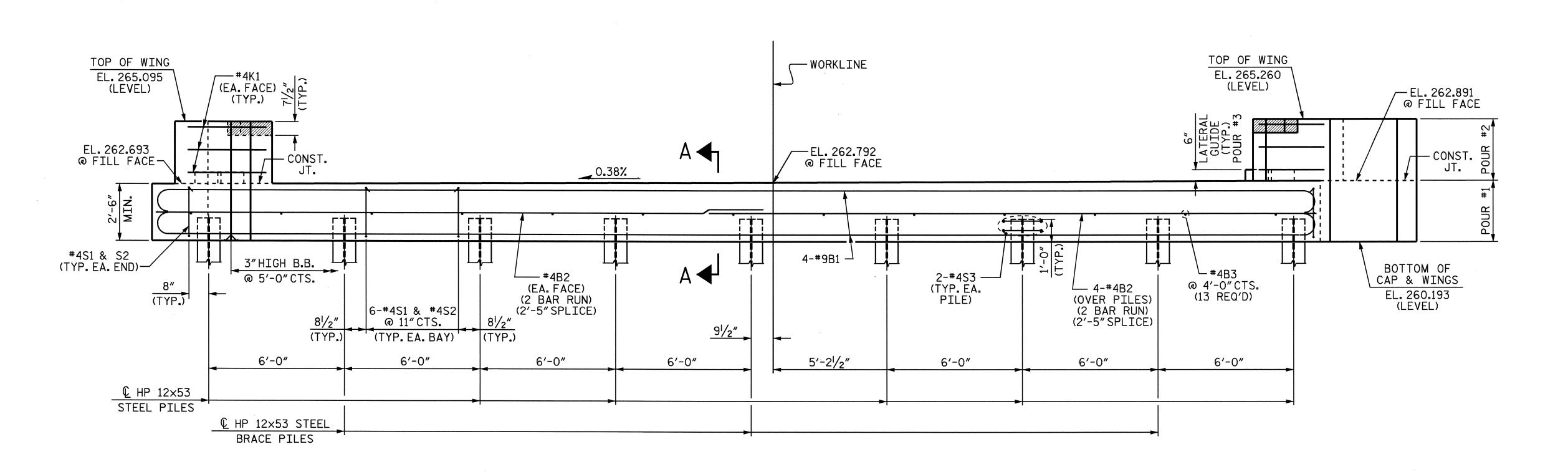
ASSEMBLED BY : R. G. EMERSON DATE : 12/06
CHECKED BY : J. P. ADAMS DATE : 04/07

DRAWN BY : WJH 4/89
CHECKED BY : FCJ 5/89

REV. 7/10/01 RWW/LES
REV. 5/7/03RRR RWW/JTE
REV. 5/1/06 TLA/GM



<u>PLAN</u>



<u>ELEVATION</u>

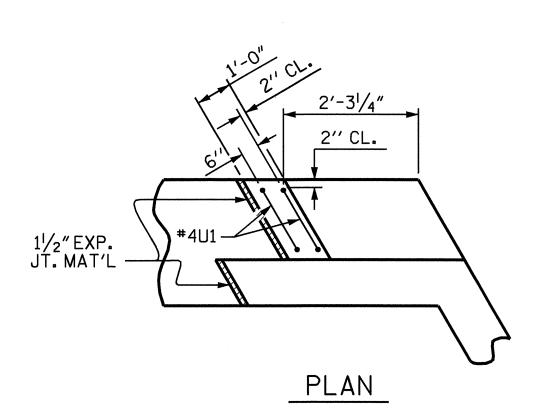
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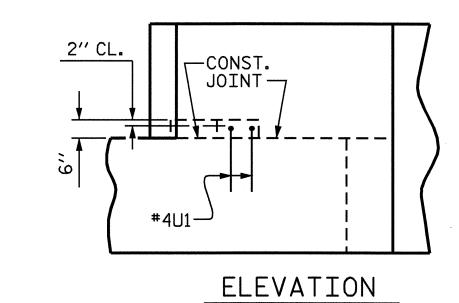
STIRRUPS IN CAP MAY BE SHIFTED AS NECESSARY TO CLEAR #6D1 DOWELS.

THE CONCRETE IN THE SHADED AREA OF THE WING SHALL BE POURED AFTER THE PARAPET AND END POST ARE CAST IF SLIP FORMING IS USED.

THE CONTRACTOR SHALL PROVIDE FOR INSTALLATION OF THE 4"DIAMETER DRAIN PIPE THROUGH THE WING WALL AS REQUIRED FOR REINFORCED BRIDGE APPROACH FILLS, SEE THE ROADWAY PLANS. REINFORCING STEEL IN THE WING WALL MAY BE SHIFTED AS NECESSARY TO CLEAR THE DRAIN PIPE.

THE LATERAL GUIDE AT EACH END OF THE CAP IS NOT TO BE POURED UNTIL AFTER CORED SLAB UNITS ARE IN PLACE.





DETAIL 'A'

PROJECT NO. B-4109

DURHAM COUNTY

STATION: 16+21.00 -L-

SHEET 1 OF 3

DEPARTMENT OF TRANSPORTATION
RALEIGH

SUBSTRUCTURE END BENT #1

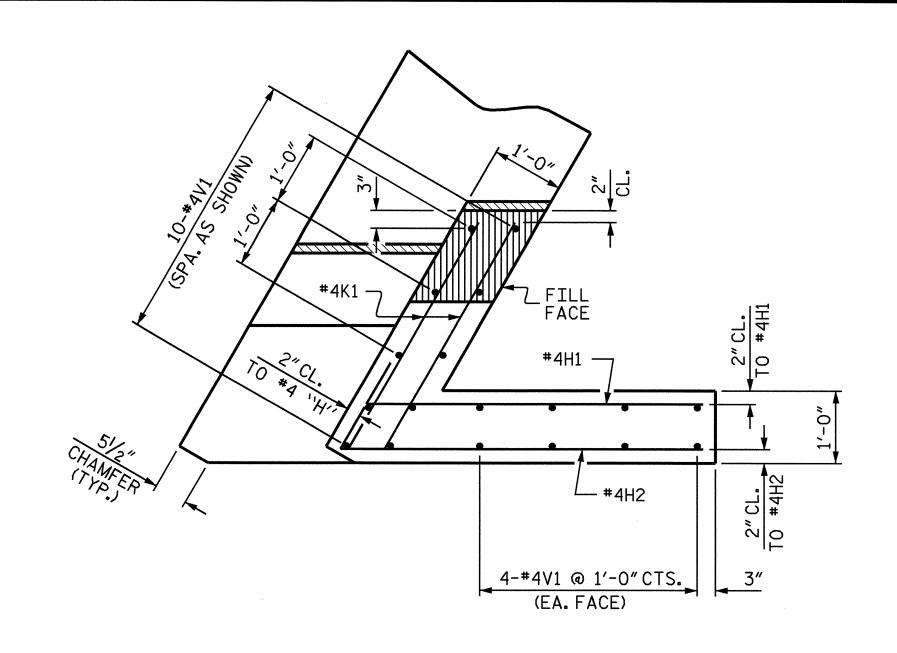
REVISIONS

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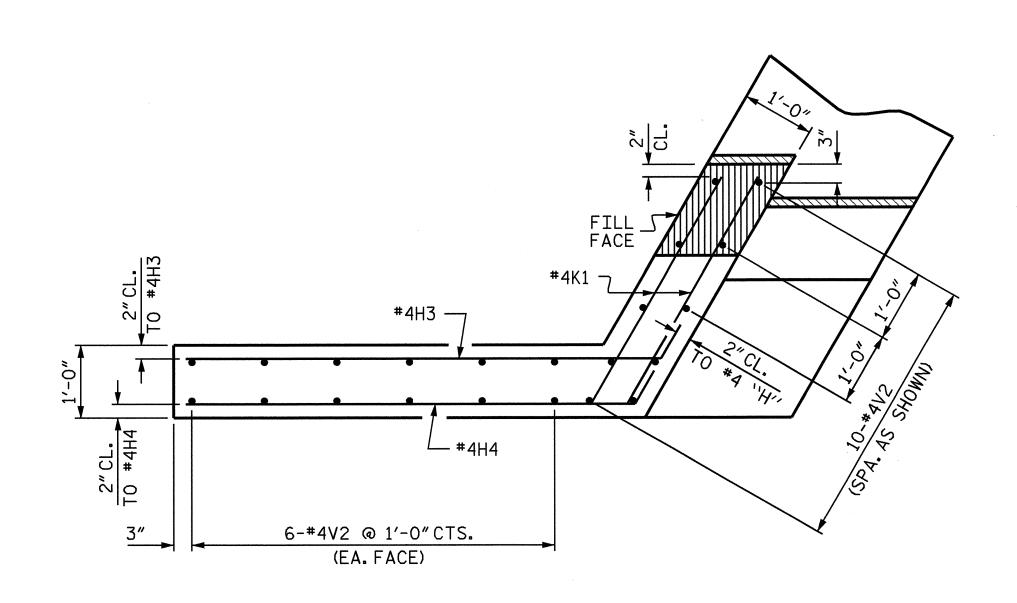
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2 4 24

DRAWN BY: Keith D. Layne DATE: 7/07
CHECKED BY: T.R. PETERSON DATE: 08/07

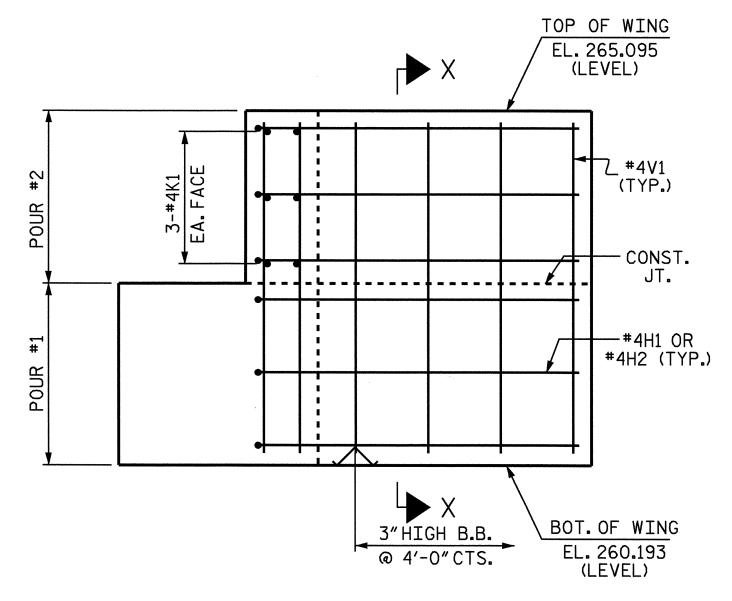
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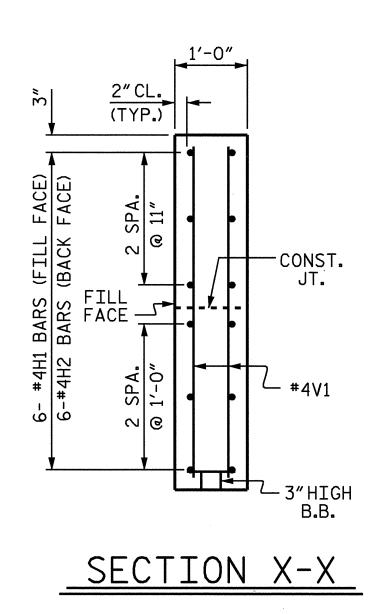
PLAN OF WING - W1



PLAN OF WING - W2



ELEVATION OF WING - W1



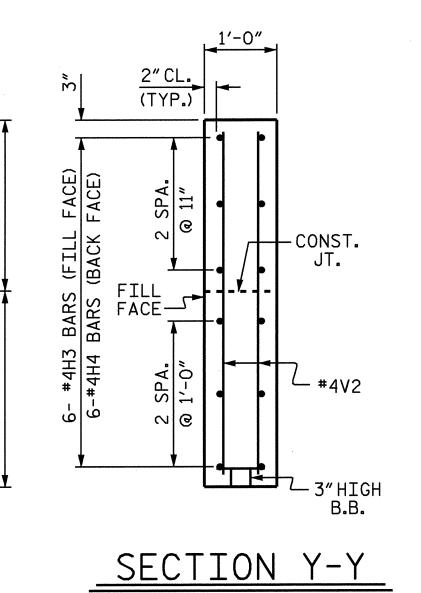
#4V2 (TYP.) THE PROPERTY OF WING EL. 260.193 (LEVEL)

#4H3 OR #4H4 (TYP.)

BOT. OF WING EL. 260.193 (LEVEL)

#4H3 OR #4H4 (TYP.)

<u>ELEVATION OF WING - W2</u>



PROJECT NO. B-4109

DURHAM COUNTY

STATION: 16+21.00 -L-

SHEET 2 OF 3

STATE OF NORTH CAROLINA

DEPARTMENT OF TRANSPORTATION

RALEIGH

SUBSTRUCTURE
END BENT #1



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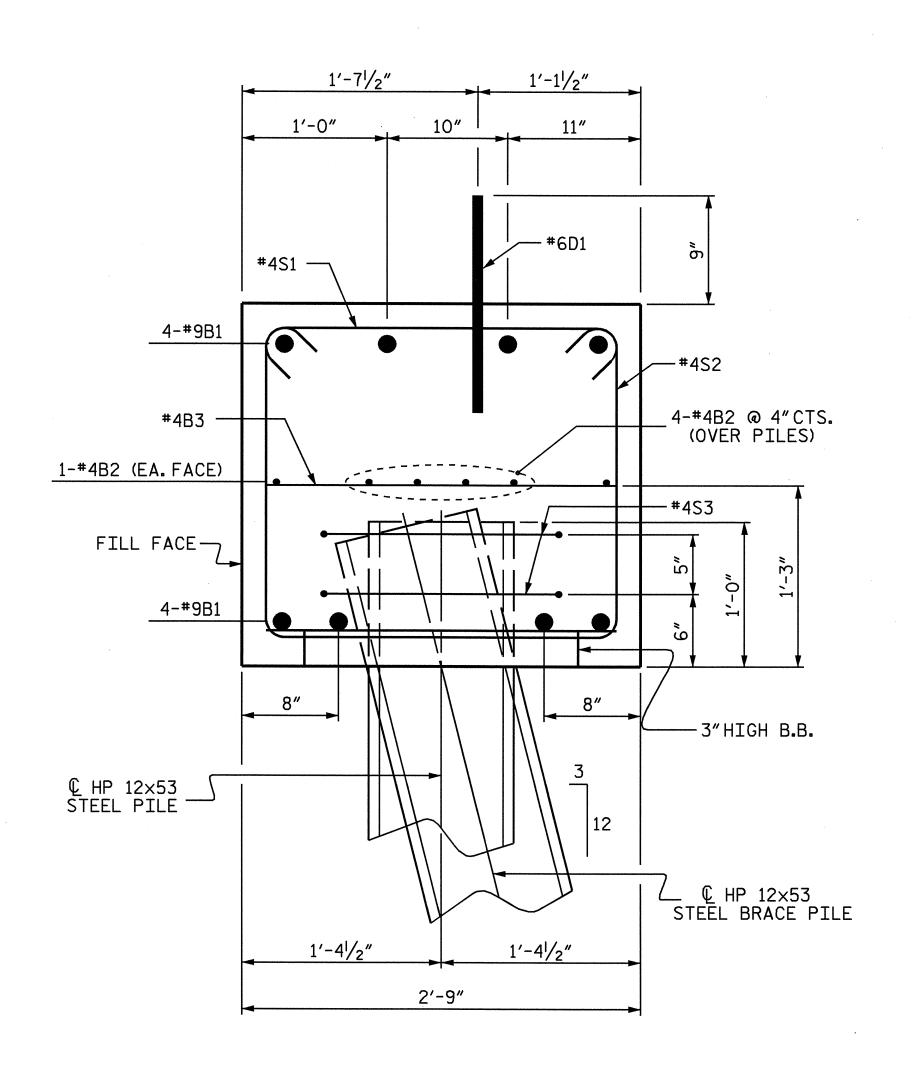
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D. BY: DATE: NO. BY: DATE:

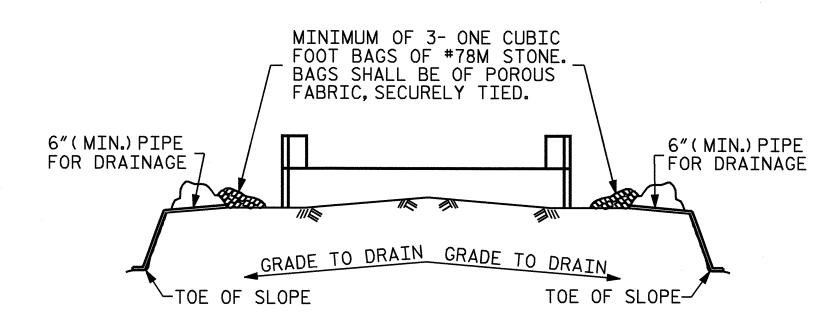
S-15

TOTAL SHEETS
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DRAWN BY: KEITH D. LAYNE DATE: 07/07 CHECKED BY: T.R. PETERSON DATE: 08/07



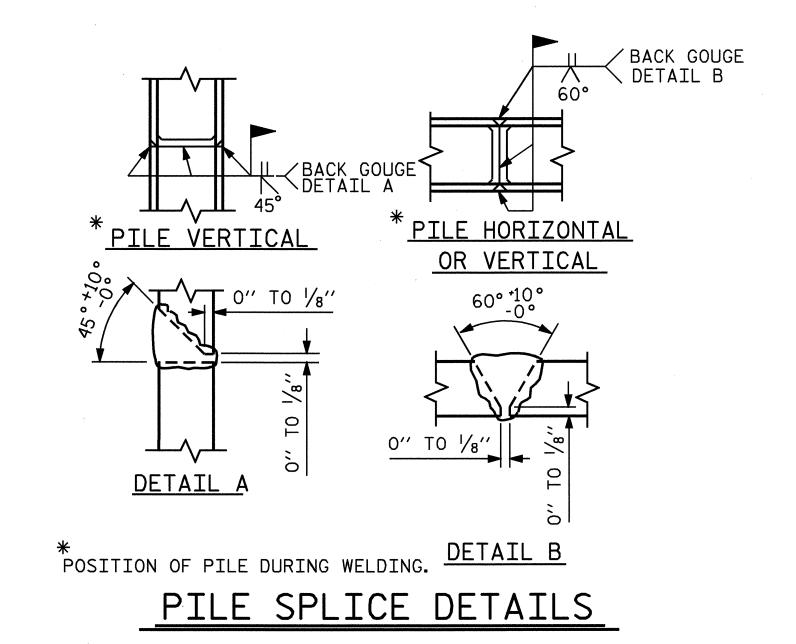
SECTION A-A

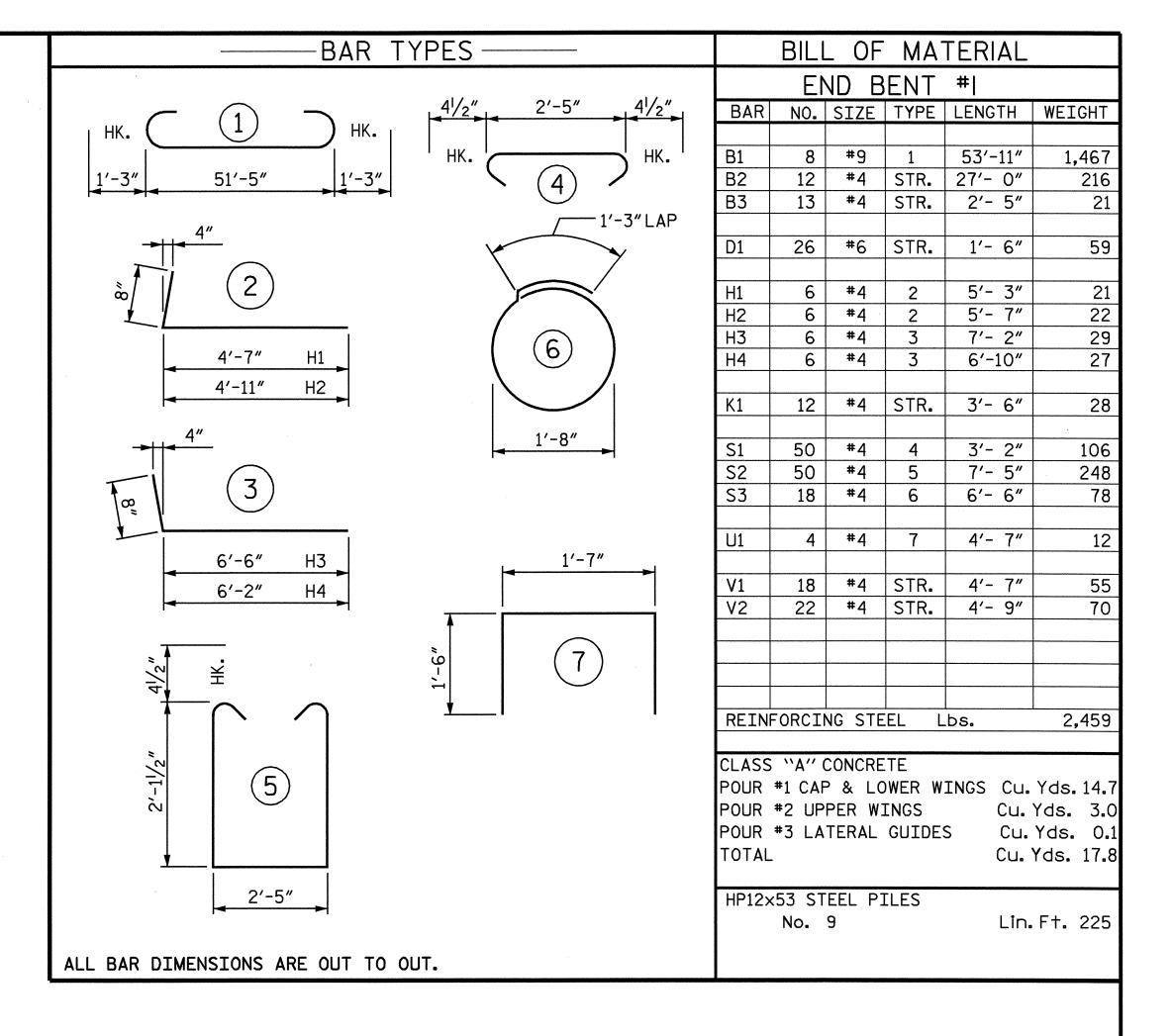


BAGGED STONE AND PIPE SHALL BE PLACED IMMEDIATELY AFTER COMPLETION OF END BENT EXCAVATION. PIPE MAY BE EITHER CONCRETE, CORRUGATED STEEL, CORRUGATED ALUMINUM ALLOY, OR CORRUGATED PLASTIC. PERFORATED PIPE WILL NOT BE ALLOWED.

BAGGED STONE SHALL REMAIN IN PLACE UNTIL THE ENGINEER DIRECTS THAT IT BE REMOVED. THE CONTRACTOR SHALL REMOVE AND DISPOSE OF SILT ACCUMULATIONS AT BAGGED STONE WHEN SO DIRECTED BY THE ENGINEER. BAGS SHALL BE REMOVED AND REPLACED WHENEVER THE ENGINEER DETER-MINES THAT THEY HAVE DETERIORATED AND LOST THEIR EFFECTIVENESS.

NO SEPARATE PAYMENT WILL BE MADE FOR THIS WORK AND THE ENTIRE COST OF THIS WORK SHALL BE INCLUDED IN THE UNIT CONTRACT PRICE BID FOR THE SEVERAL PAY ITEMS.





B-4109 PROJECT NO. ___ DURHAM COUNTY STATION: 16+21.00 -L-

SHEET 3 OF 3

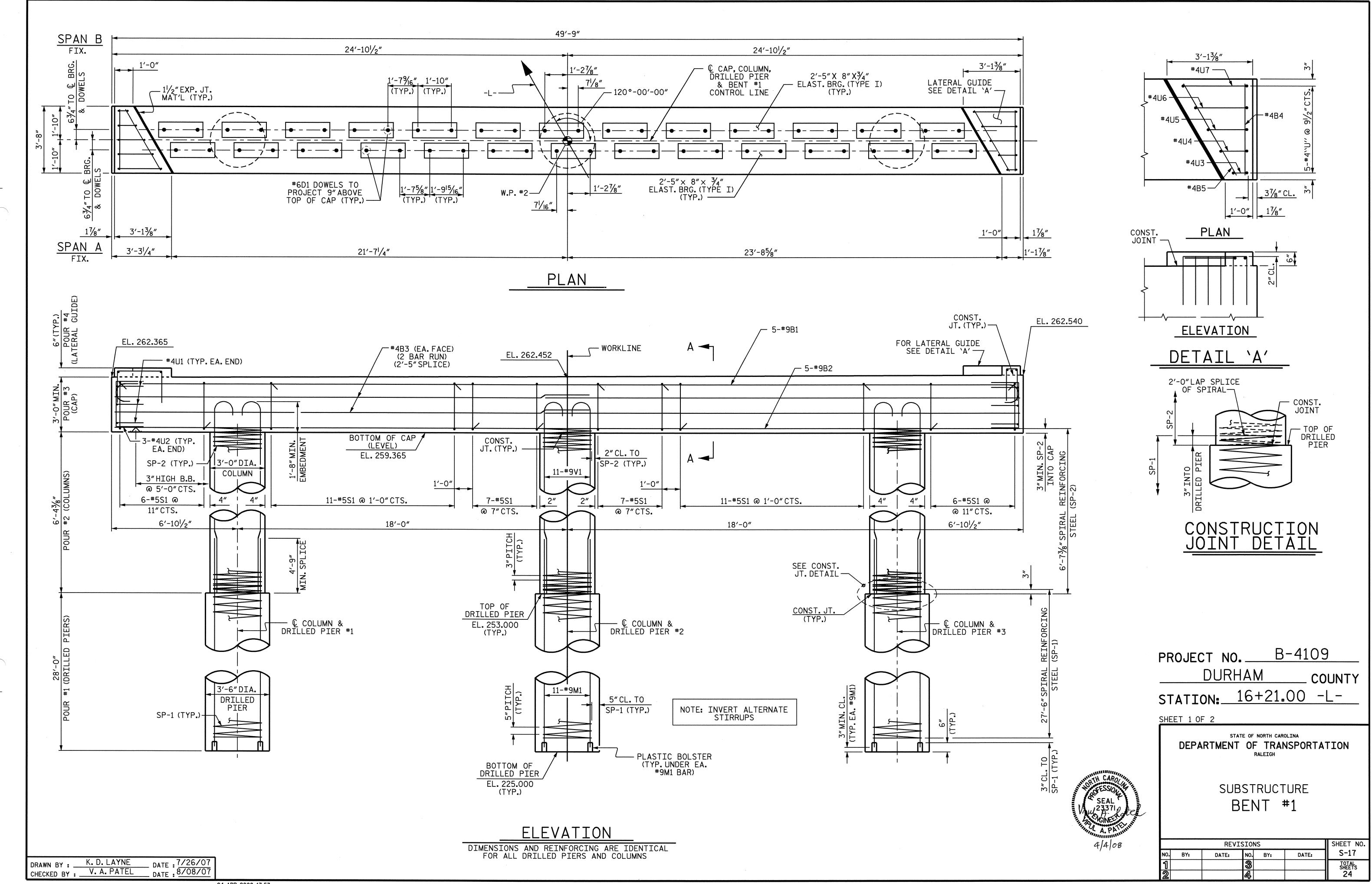
STATE OF NORTH CAROLINA DEPARTMENT OF TRANSPORTATION

> SUBSTRUCTURE END BENT #1

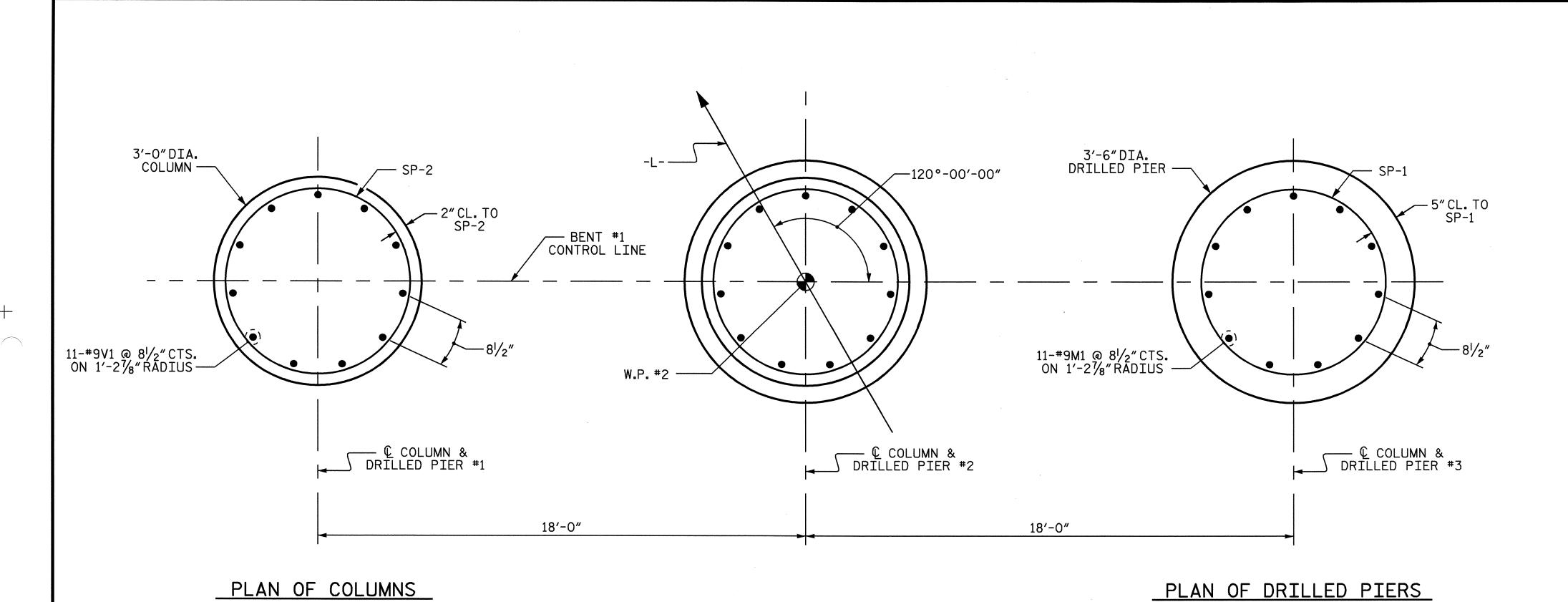
SHEET NO. REVISIONS S-16 DATE: NO. BY: TOTAL SHEETS 24

TEMPORARY DRAINAGE AT END BENT

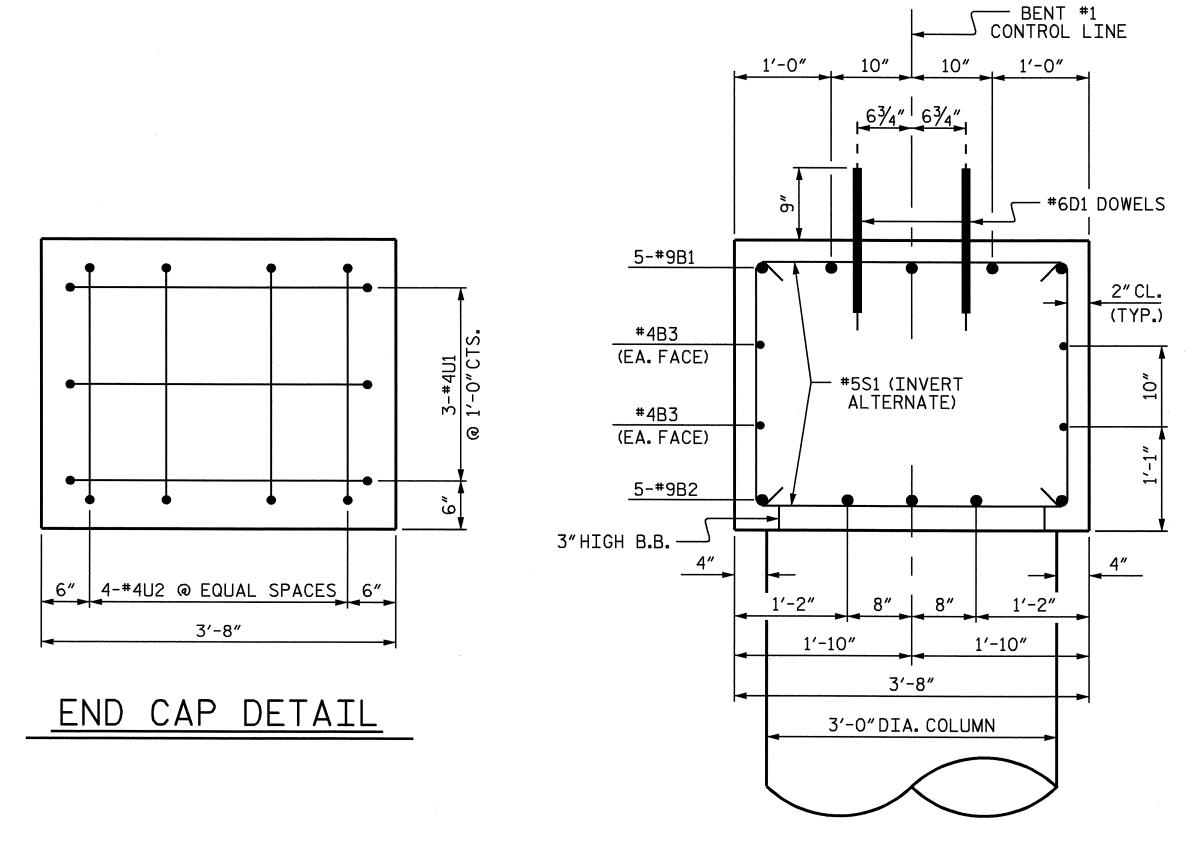
DRAWN BY: KEITH D. LAYNE DATE: 07/07 CHECKED BY: T.R. PETERSON DATE: 08/07



04-APR-2008 13:53 R:\Structures\B-4109\Plans\B-4109_sd_Bt1.dgn klayne



PLAN OF COLUMNS AND DRILLED PIERS



_ DATE : 7/26/07 _ DATE : 8/08/07

K. D. LAYNE

V. A. PATEL

DRAWN BY :

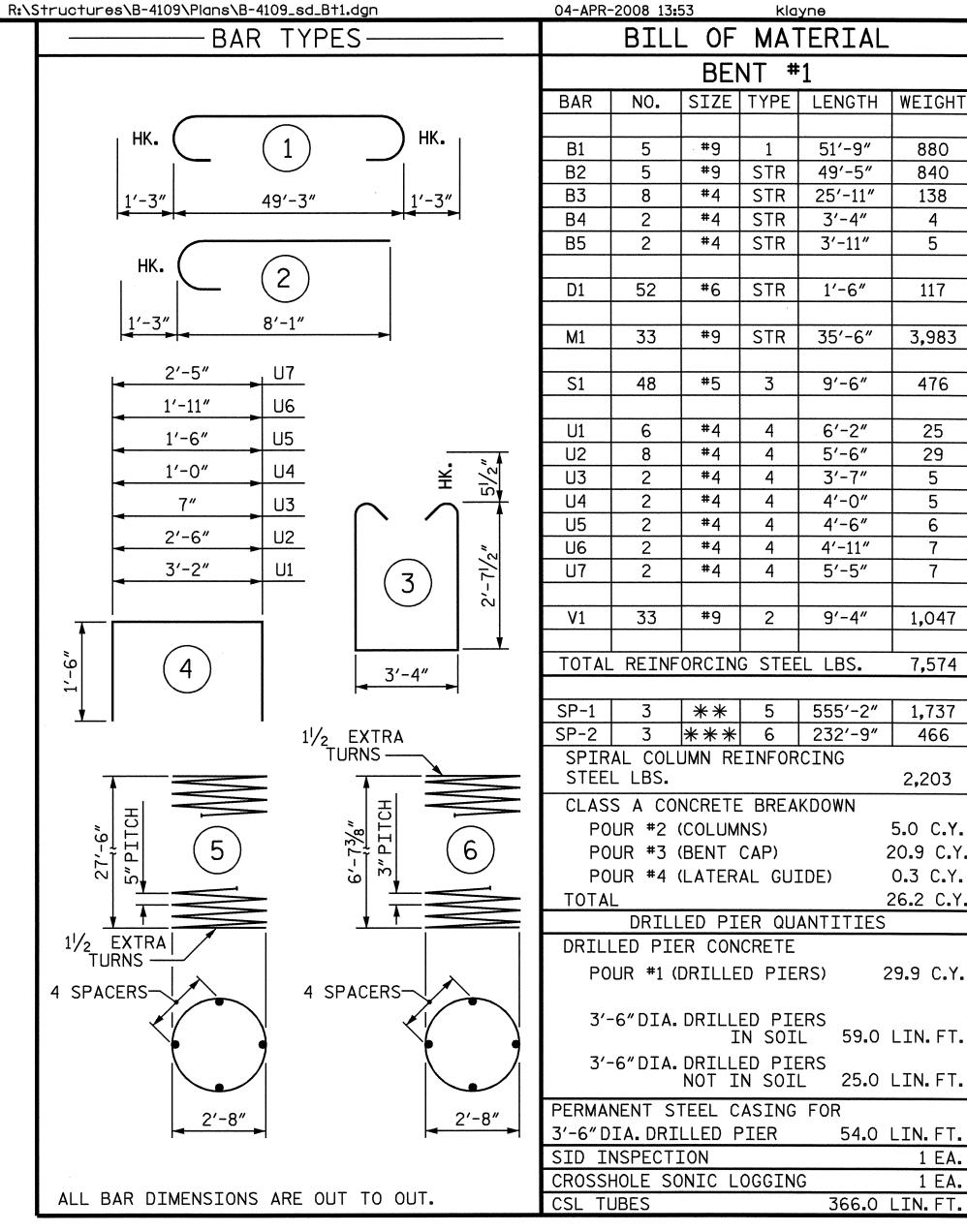
CHECKED BY : _

NOTES

STIRRUPS IN CAP MAY BE SHIFTED AS NECESSARY TO CLEAR DOWELS. HOOKS ON "V"BARS MAY BE TURNED AS NECESSARY FOR PLACING REINFORCING STEEL.

ALL STEEL IN THE DRILLED PIERS IS INCLUDED IN THE PAY ITEMS FOR "REINFORCING STEEL" AND "SPIRAL COLUMN REINFORCING STEEL".

THE CONTRACTOR'S ATTENTION IS CALLED TO THE FACT THAT THE LONGITUDINAL REINFORCEMENT FOR THE DRILLED PIERS IS DETAILED WITH 3 FEET OF EXTRA LENGTH.



- ** THE SP-1 SPIRAL REINFORCING STEEL SHALL BE W31 OR D-31 COLD DRAWN WIRE OR #5 PLAIN OR DEFORMED BAR.
- *** THE SP-2 SPIRAL REINFORCING STEEL SHALL BE W20 OR D-20 COLD DRAWN WIRE OR #4 PLAIN OR DEFORMED BAR.

PROJECT NO. B-4109 DURHAM _ COUNTY 16+21.00 -L-STATION:_

SHEET 2 OF 2

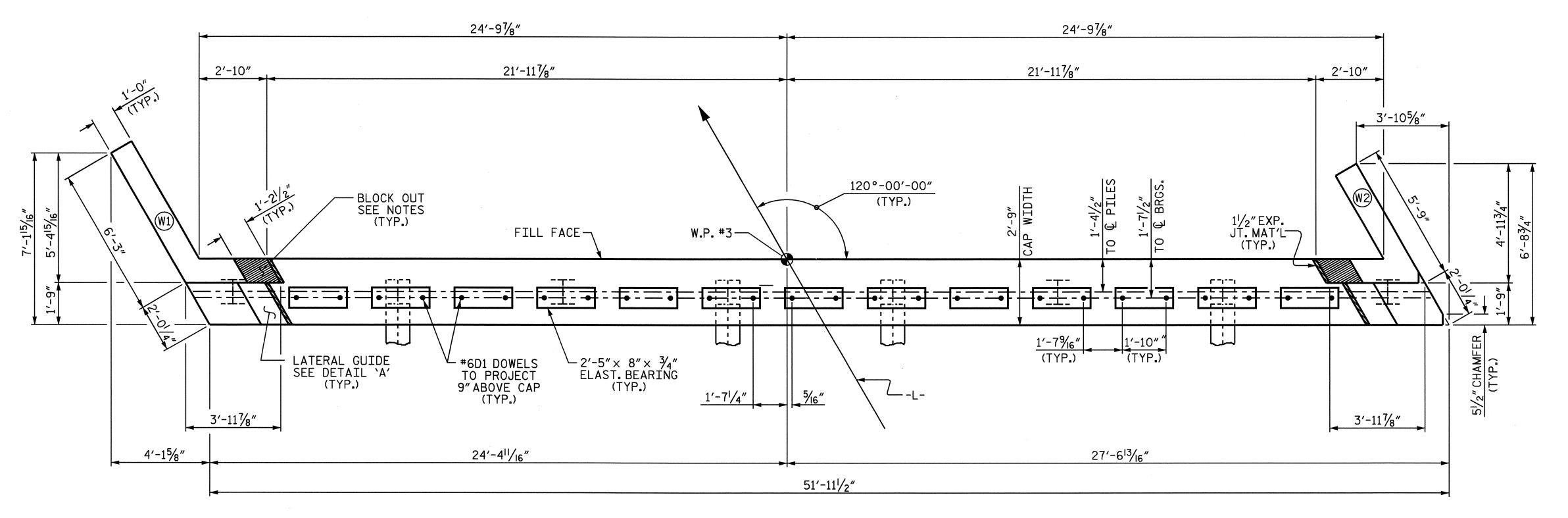
STATE OF NORTH CAROLINA DEPARTMENT OF TRANSPORTATION RALEIGH

> SUBSTRUCTURE BENT #1

REVISIONS SHEET NO. S-18 NO. BY: DATE: DATE: BY: TOTAL SHEETS 24

FOR DRILLED PIERS, SEE DRILLED PIERS SPECIAL PROVISION.

SECTION A-A



<u>PLAN</u>

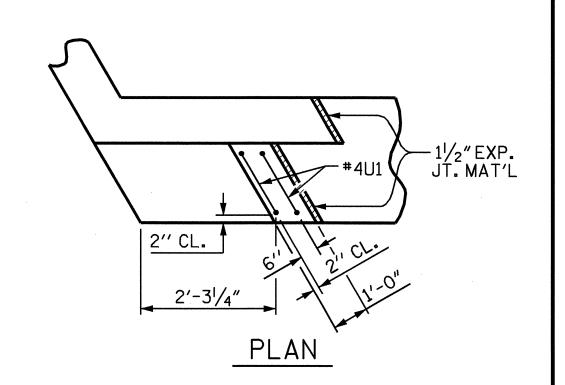


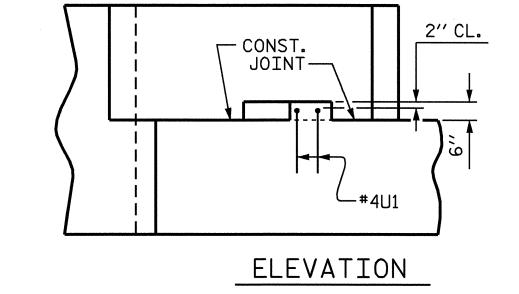
STIRRUPS IN CAP MAY BE SHIFTED AS NECESSARY TO CLEAR #6D1 DOWELS.

THE CONCRETE IN THE SHADED AREA OF THE WING SHALL BE POURED AFTER THE PARAPET AND END POST ARE CAST IF SLIP FORMING IS USED.

THE CONTRACTOR SHALL PROVIDE FOR INSTALLATION OF THE 4"DIAMETER DRAIN PIPE THROUGH THE WING WALL AS REQUIRED FOR REINFORCED BRIDGE APPROACH FILLS, SEE THE ROADWAY PLANS. REINFORCING STEEL IN THE WING WALL MAY BE SHIFTED AS NECESSARY TO CLEAR THE DRAIN PIPE.

THE LATERAL GUIDE AT EACH END OF THE CAP IS NOT TO BE POURED UNTIL AFTER CORED SLAB UNITS ARE IN PLACE.





DETAIL 'A'

PROJECT NO. B-4109

DURHAM COUNTY

STATION: 16+21.00 -L-

SHEET 1 OF 3

STATE OF NORTH CAROLINA

DEPARTMENT OF TRANSPORTATION
RALEIGH

SUBSTRUCTURE

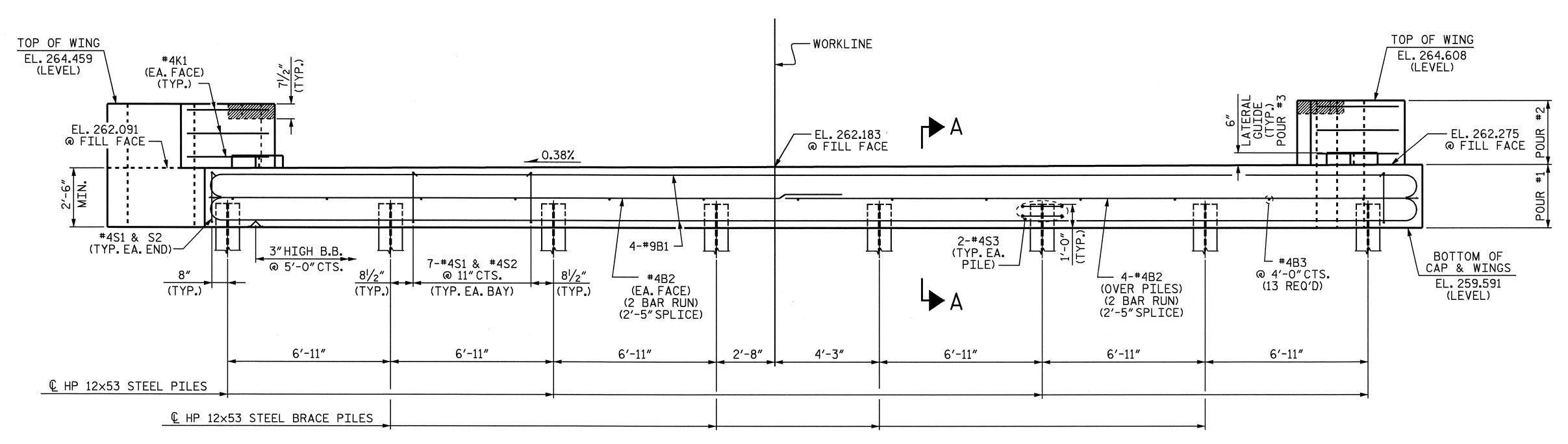
END BENT #2

REVISIONS

NO. BY: DATE: NO. BY: DATE:

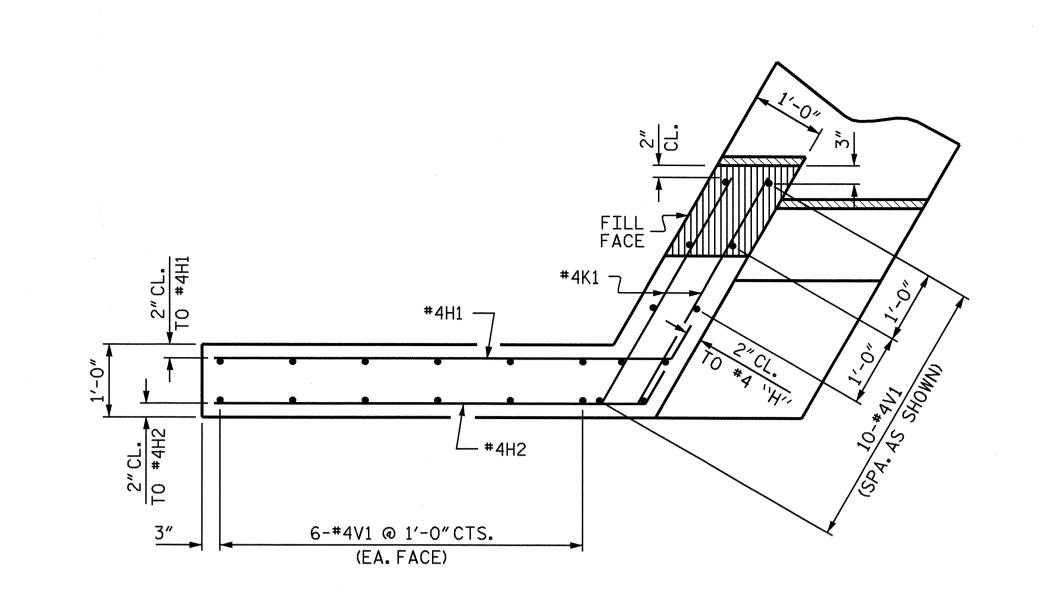
SHEET NO. S-19

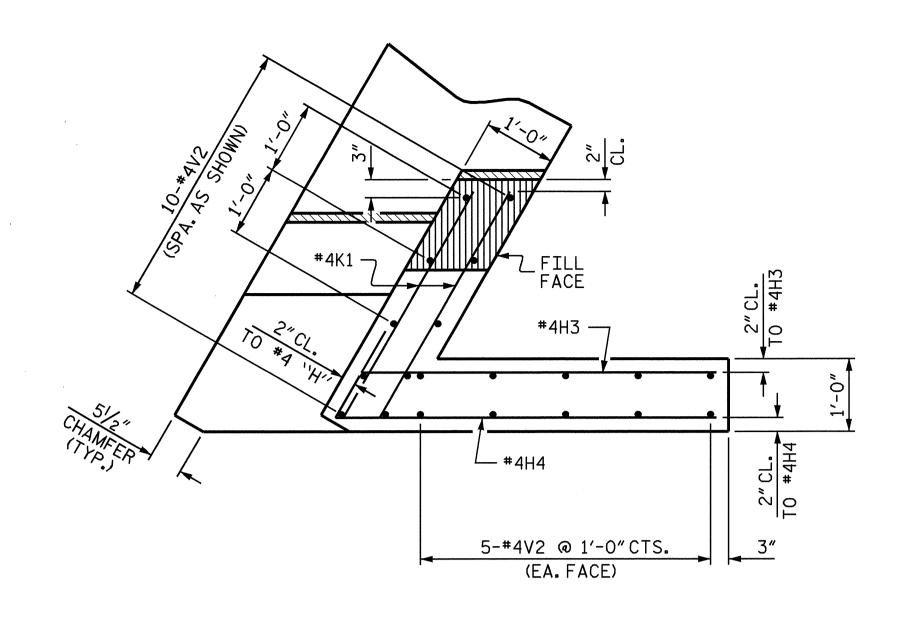
TOTAL SHEETS
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ELEVATION

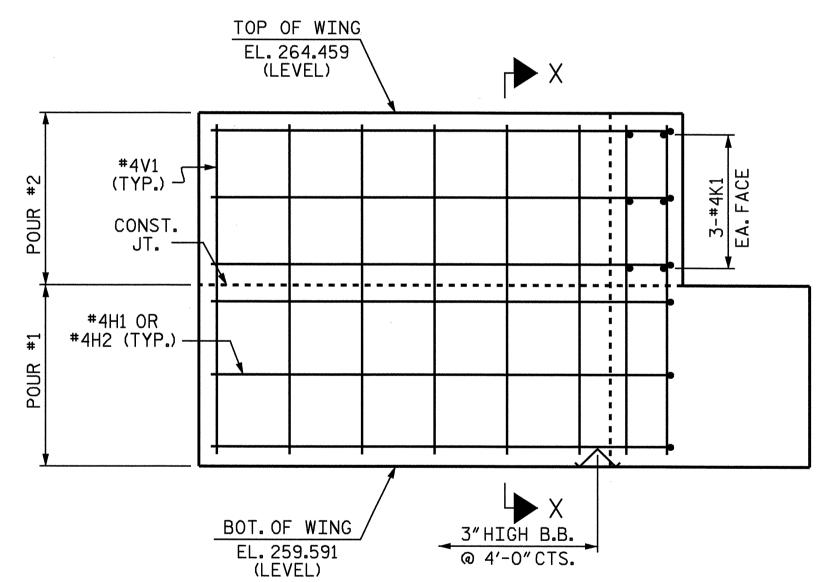
DRAWN BY: KEITH D. LAYNE DATE: 07/07 CHECKED BY: T. R. PETERSON DATE: 08/07

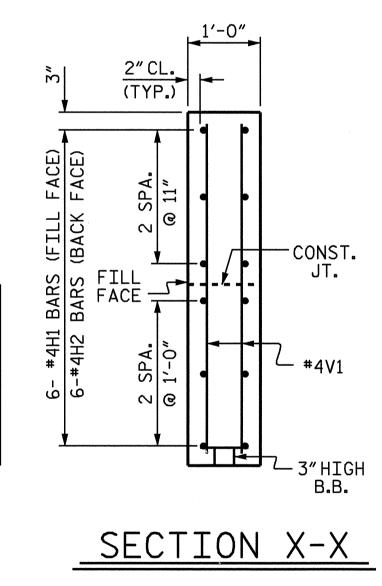


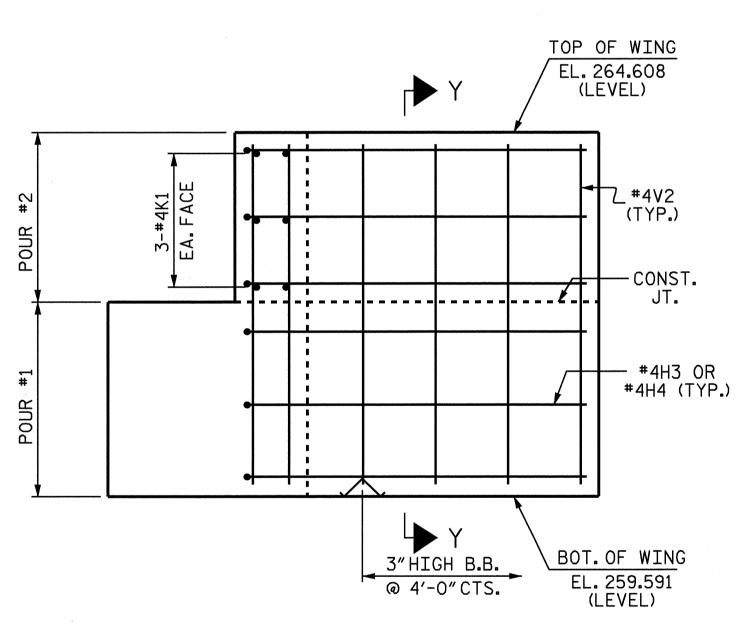


PLAN OF WING - W1

PLAN OF WING - W2







6- #4H3 BARS (FILL FACE)

6- #4H4 BARS (BACK FACE)

7 SPA. 2 SPA. 4 SPA. 4 SPA. 4 SPA. 4 SPA. 4 SPA. 4 SPA. 5 SPA. 4 SPA. 5 SPA.

ELEVATION OF WING - W1

ELEVATION OF WING - W2

PROJECT NO. B-4109

DURHAM COUNTY

STATION: 16+21.00 -L-

SHEET 2 OF 3

BY:

STATE OF NORTH CAROLINA

DEPARTMENT OF TRANSPORTATION

RALEIGH

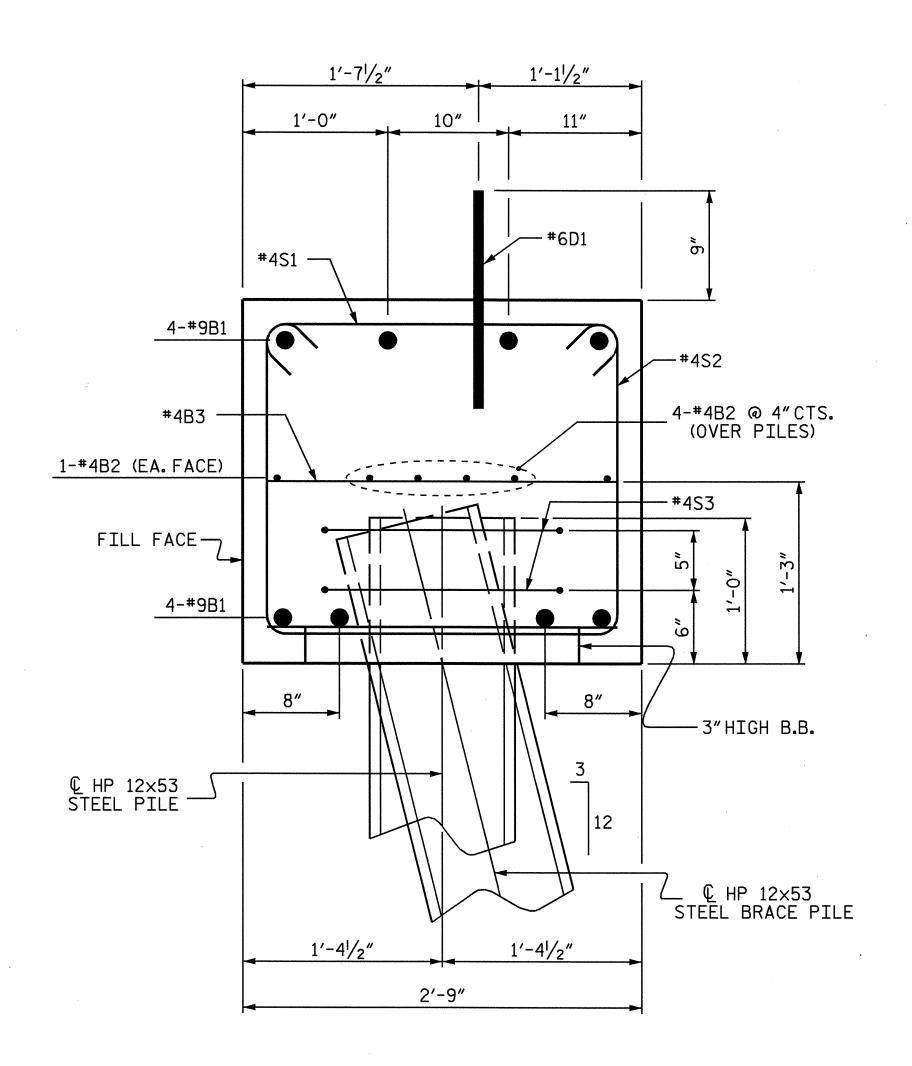
SUBSTRUCTURE
END BENT #2

SEAL 23371

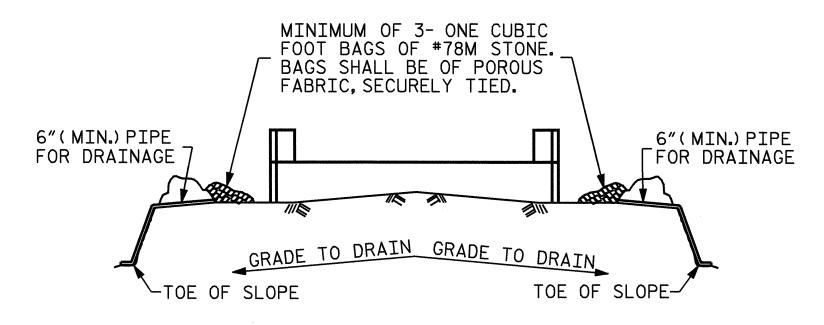
A. PATELLINIA A. P

REV	SHEET			
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DRAWN BY: KEITH D. LAYNE DATE: 07/07 CHECKED BY: T. R. PETERSON DATE: 08/07



SECTION A-A

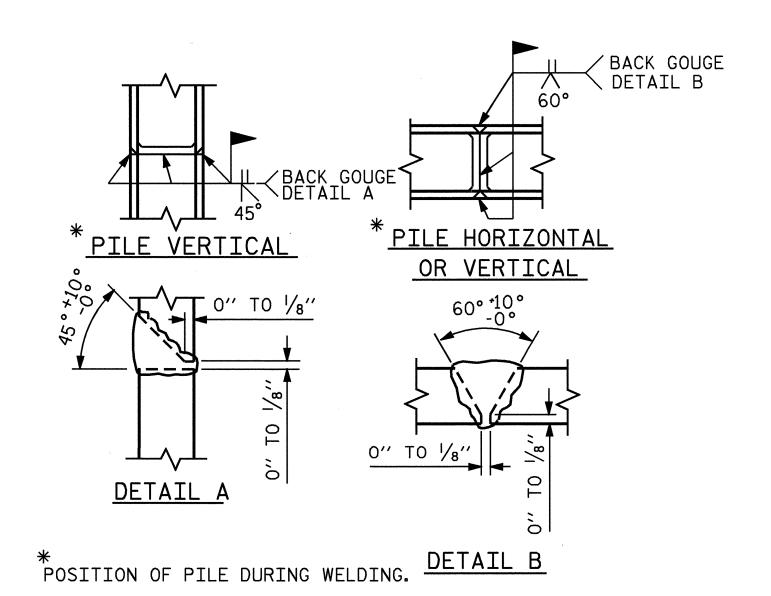


BAGGED STONE AND PIPE SHALL BE PLACED IMMEDIATELY AFTER COMPLETION OF END BENT EXCAVATION. PIPE MAY BE EITHER CONCRETE, CORRUGATED STEEL, CORRUGATED ALUMINUM ALLOY, OR CORRUGATED PLASTIC. PERFORATED PIPE WILL NOT BE ALLOWED.

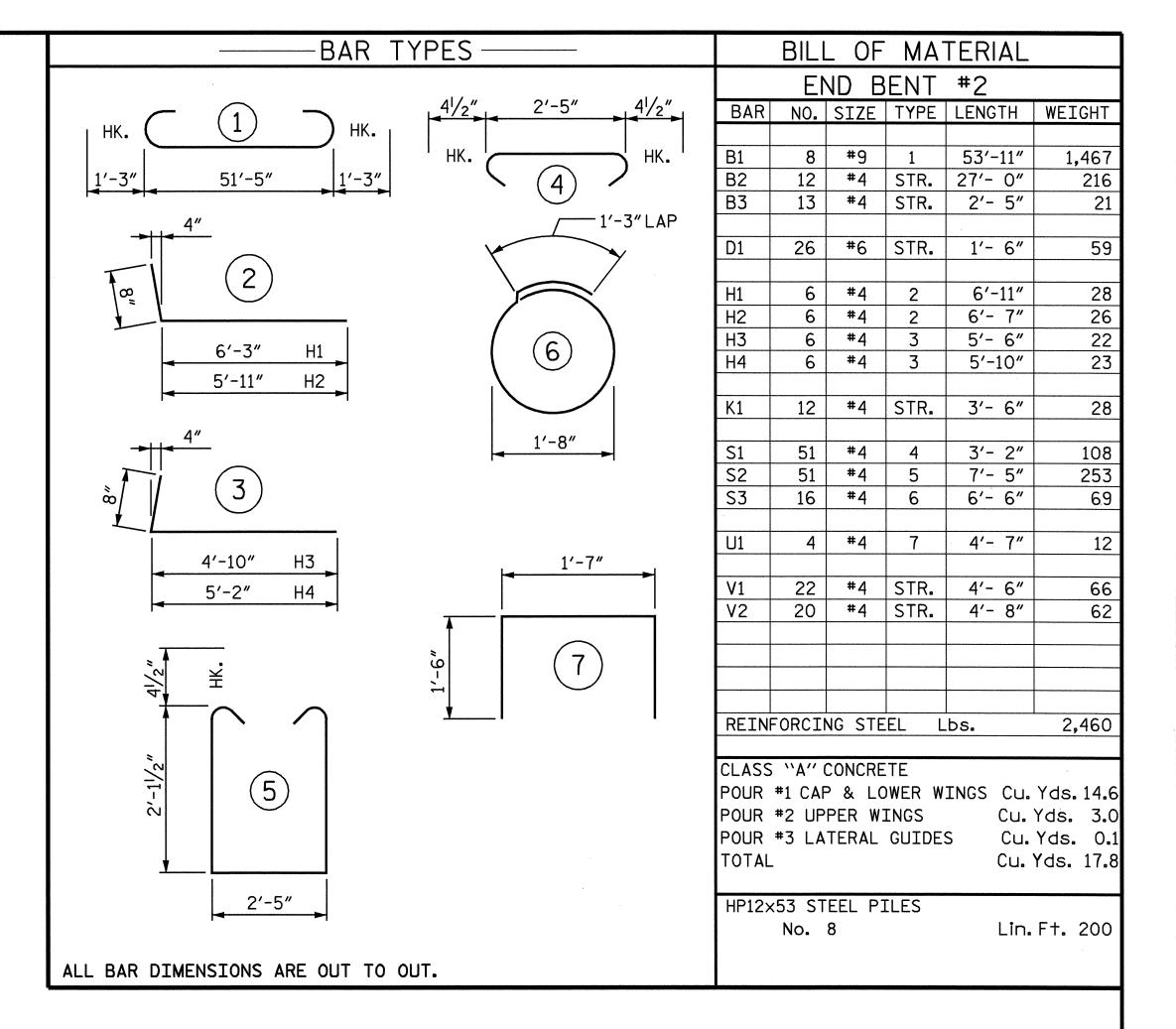
BAGGED STONE SHALL REMAIN IN PLACE UNTIL THE ENGINEER DIRECTS THAT IT BE REMOVED. THE CONTRACTOR SHALL REMOVE AND DISPOSE OF SILT ACCUMULATIONS AT BAGGED STONE WHEN SO DIRECTED BY THE ENGINEER. BAGS SHALL BE REMOVED AND REPLACED WHENEVER THE ENGINEER DETERMINES THAT THEY HAVE DETERIORATED AND LOST THEIR EFFECTIVENESS.

NO SEPARATE PAYMENT WILL BE MADE FOR THIS WORK AND THE ENTIRE COST OF THIS WORK SHALL BE INCLUDED IN THE UNIT CONTRACT PRICE BID FOR THE SEVERAL PAY ITEMS.

TEMPORARY DRAINAGE AT END BENT



PILE SPLICE DETAILS



PROJECT NO. B-4109

DURHAM COUNTY

STATION: 16+21.00 -L-

SHEET 3 OF 3

STATE OF NORTH CAROLINA

DEPARTMENT OF TRANSPORTATION

RALEIGH

SUBSTRUCTURE END BENT #2

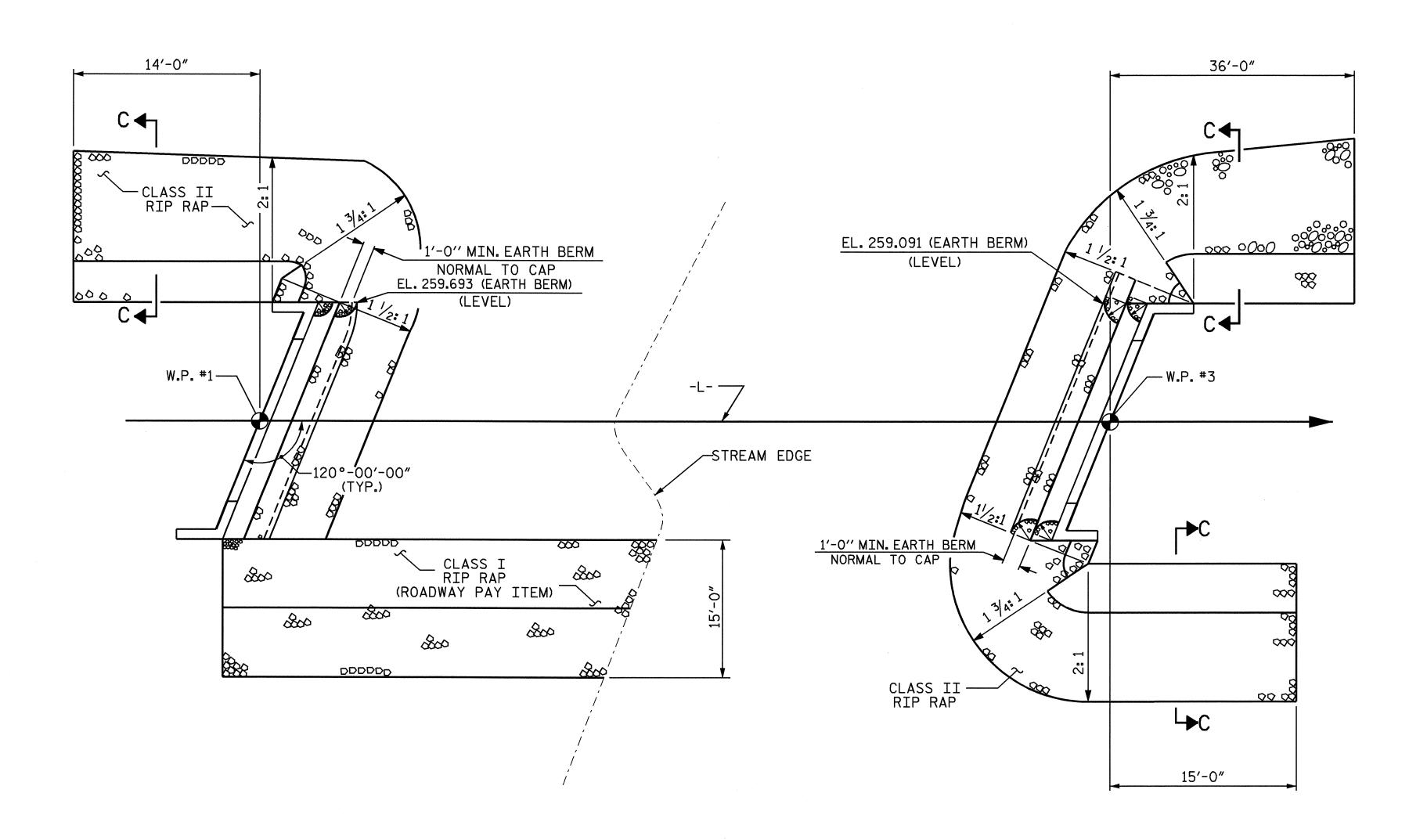
REVISIONS

NO. BY: DATE: NO. BY: DATE:

1 3 TOTAL SHEETS
24 24

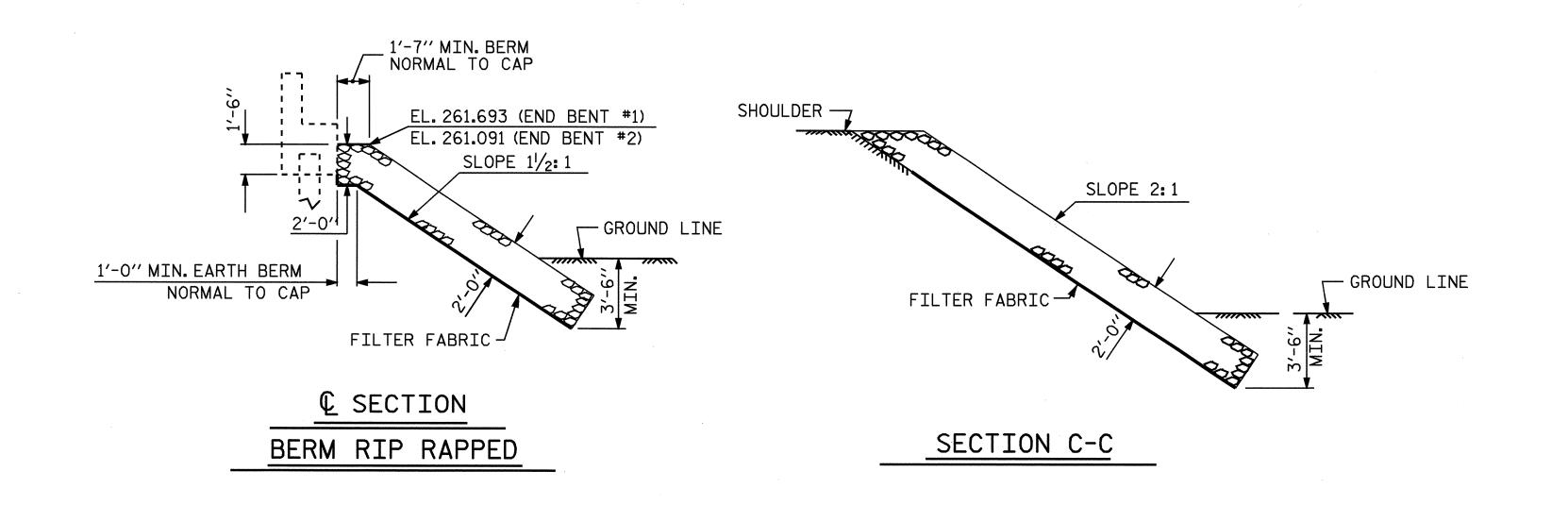
DRAWN BY: KEITH D. LAYNE DATE: 07/07 CHECKED BY: T.R. PETERSON DATE: 08/07

04-APR-2008 13:52 R:\Structures\B-4109\Plans\B-4109_sd_Ebts.dgn



ESTIMATED QUANTITIES					
BRIDGE @ STA.16+21.00 -L-	RIP RAP CLASS II	FILTER FABRIC FOR DRAINAGE			
	TONS	SQUARE YARDS			
END BENT #1	225	250			
END BENT #2	300	335			

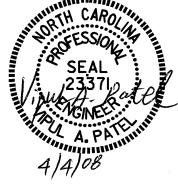
PLAN OF RIP RAP



PROJECT NO. B-4109 DURHAM ___ COUNTY STATION: 16+21.00 -L-

STATE OF NORTH CAROLINA DEPARTMENT OF TRANSPORTATION RALEIGH STANDARD

-RIP RAP DETAILS-



	SEAL	
î /	SEAL 233710	
	MANTE A. PATENTINE	
•	1/4/08	

REVISIONS				SHEET NO.		
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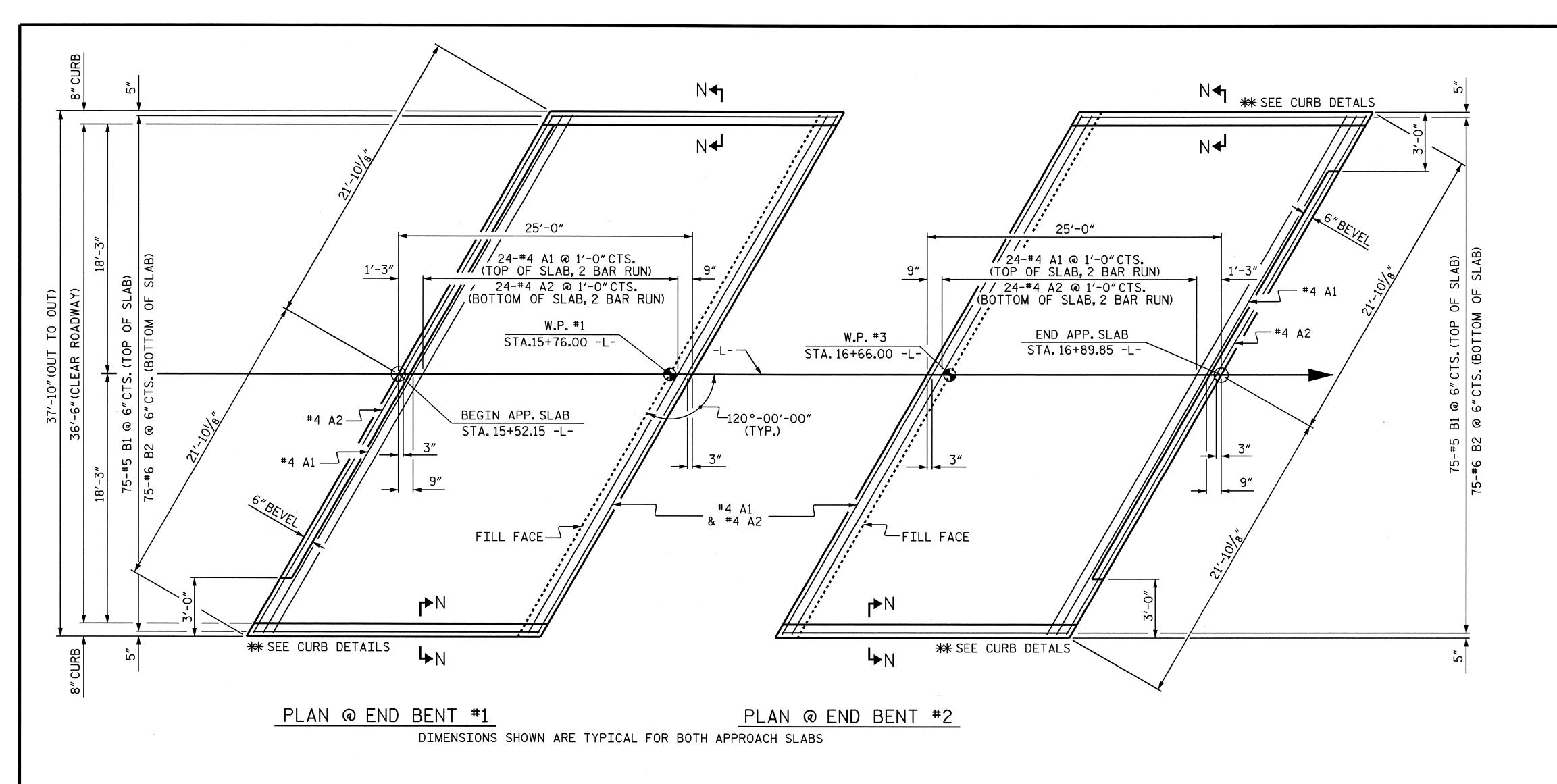
ASSEMBLED BY : R. G. EMERSON CHECKED BY : G. A. THOMPSON

DRAWN BY: REK 1/84 CHECKED BY: RDU 1/84

DATE : 01/07 DATE : 01/07

RWW/LES RWW/LES TLA/GM

REV. 8/16/99 REV. 10/17/00 REV. 5/1/06



#4 A1 2'-0"
#4 A2 1'-9"

FOR ONE APPROACH SLAB (2 REQ'D.) BAR NO. SIZE TYPE LENGTH WEIGHT * A1 52 #4 | STR | 22'-8" 787 A2 52 #4 STR 22'-7" 784 *B1 | 75 | #5 | STR | 24'-1" 1884 B2 | 75 | #6 | STR | 24'-7" 2769 3553 REINFORCING STEEL LBS. ***** EPOXY COATED REINFORCING STEEL LBS. 2671 CLASS AA CONCRETE C. Y. 37.4

BILL OF MATERIAL

NOTES

FOR REINFORCED BRIDGE APPROACH FILL INCLUDING FABRIC, IMPERMEABLE GEOMEMBRANE, 4" Ø DRAINAGE PIPE, #78M STONE, AND SELECT MATERIAL, SEE ROADWAY PLANS.

AREA BETWEEN THE WINGWALL AND APPROACH SLAB SHALL BE GRADED TO DRAIN THE WATER AWAY FROM THE FILL FACE OF THE BRIDGE AND SHALL BE PAVED. SEE ROADWAY PLANS.

THE 6"COMP. A.B.C. SHALL BE FLUSH WITH THE ROADWAY END OF THE APPROACH SLAB AND SHALL EXTEND 1'-0"OUTSIDE OF EACH EDGE OF THE APPROACH SLAB.

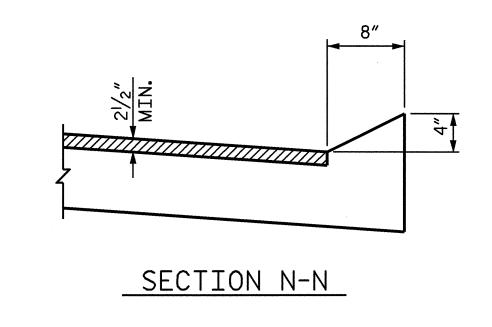
THE CONTRACTOR MAY USE 4"TYPE B-25.0B ASPHALT CONCRETE BASE COURSE IN LIEU OF 6"COMP. A.B.C. IF THIS OPTION IS USED, THE BASE COURSE SHALL BE FLUSH WITH THE ROADWAY END OF THE APPROACH SLAB, AND THE WIDTH SHALL BE THE SAME AS THAT OF THE APPROACH SLAB.

THE CONTRACTOR MAY USE 5"CLASS "A" CONCRETE BASE IN LIEU OF 6" COMP. A.B.C. IF THIS OPTION IS USED, THE CONCRETE BASE SHALL BE FLUSH WITH THE ROADWAY END OF THE APPROACH SLAB, AND THE WIDTH SHALL BE THE SAME AS THAT OF THE APPROACH SLAB. THE CONCRETE SHALL BE FINISHED TO A SMOOTH SURFACE AND A LAYER OF 30 LB ROOFING FELT SHALL BE PLACED BETWEEN THE CONCRETE BASE AND THE APPROACH SLAB TO PREVENT BOND. THE APPROACH SLAB SHALL NOT BE CAST UNTIL THE CONCRETE BASE HAS REACHED AN AGE OF THREE CURING DAYS.

FOR JOINT DETAILS, SEE "PRESTRESSED CONCRETE CORED SLAB UNIT" SHEETS.

THE JOINT AT THE END BENT SHALL BE GROUTED AS SOON AS PRACTICAL AFTER THE CONSTRUCTION OF THE APPROACH SLABS.

APPROACH SLAB GROOVING IS NOT REQUIRED.



PROJECT NO. B-4109

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STATION: 16+21.00 -L-

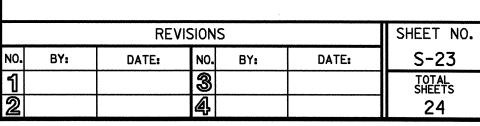
SHEET 1 OF 2

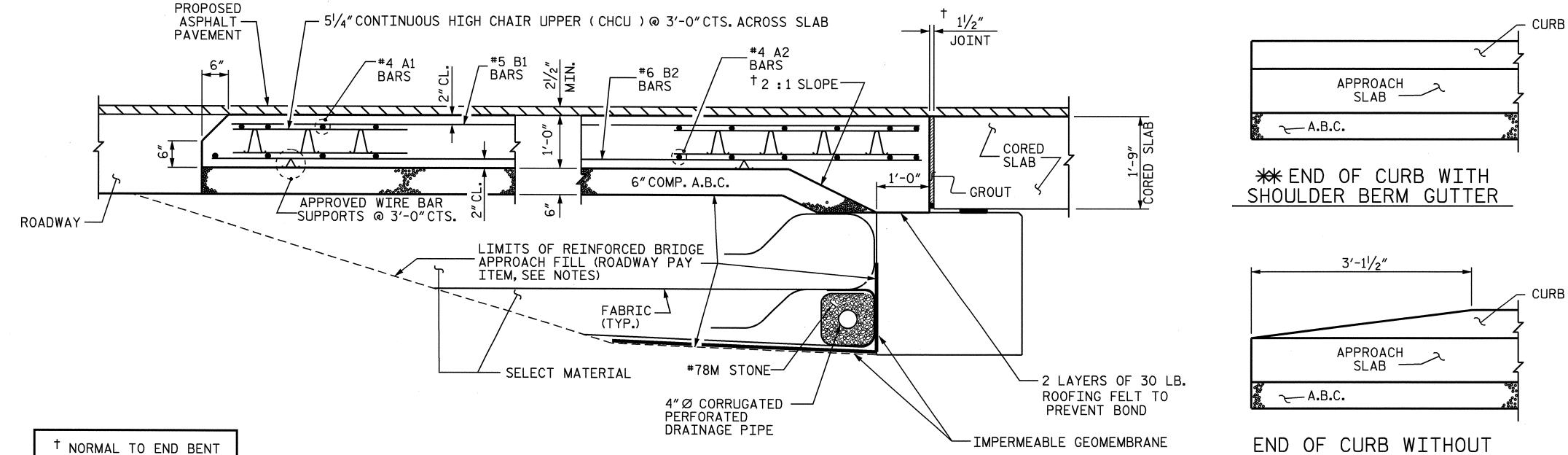
STATE OF NORTH CAROLINA

DEPARTMENT OF TRANSPORTATION
RALEIGH

STANDARD

BRIDGE APPROACH SLAB FOR PRESTRESSED CONCRETE CORED SLAB





END OF CURB WITHOUT SHOULDER BERM GUTTER

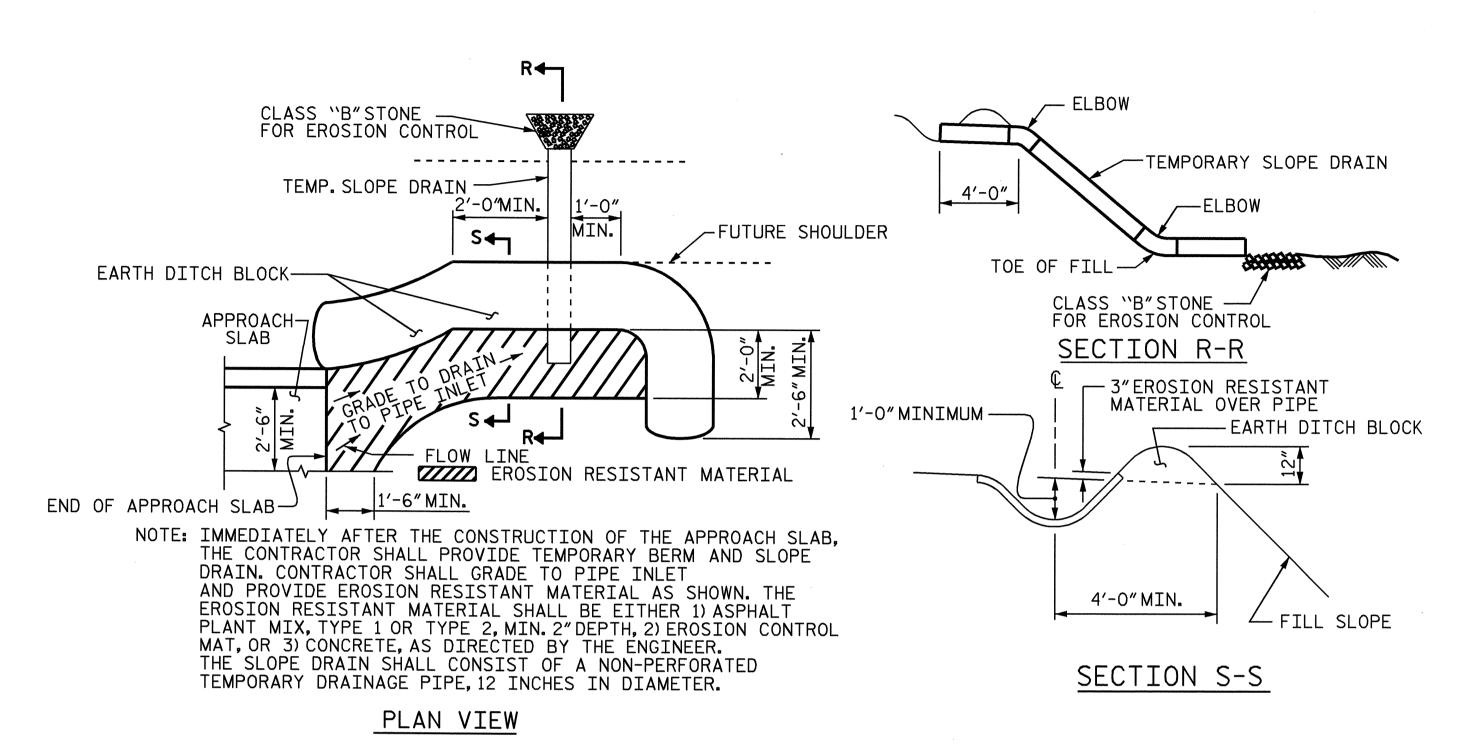
CURB DETAILS

SECTION THRU SLAB

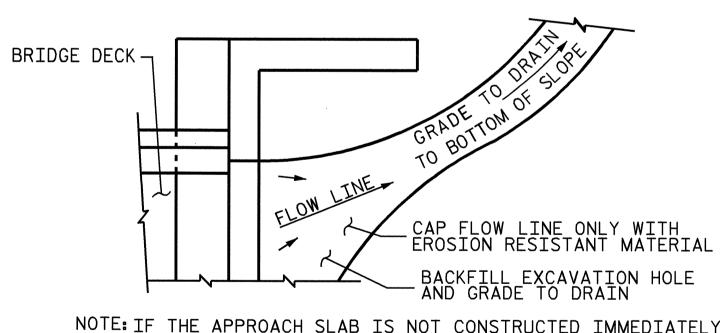
ASSEMBLED BY : R. G. EMERSON DATE : 08/07 CHECKED BY : S. DOMBROWSKI DATE : 08/07

LES/RDR RWW/JTE TLA/GM

DRAWN BY: FCJ 6/87 REV. 7/10/01 CHECKED BY: EGA 6/87 REV. 5/1/06



TEMPORARY BERM AND SLOPE DRAIN DETAILS



NOTE: IF THE APPROACH SLAB IS NOT CONSTRUCTED IMMEDIATELY AFTER THE BACKFILLING OF THE END BENT EXCAVATION, GRADE TO DRAIN TO THE BOTTOM OF THE SLOPE AND PROVIDE EROSION RESISTANT MATERIAL, SUCH AS FIBERGLASS ROVING OR AS DIRECTED BY THE ENGINEER TO PREVENT SOIL EROSION AND TO PROTECT THE AREA ADJACENT TO THE STRUCTURE. THE CONTRACTOR WILL BE REQUIRED TO REMOVE THESE MATERIALS PRIOR TO CONSTRUCTION OF THE APPROACH SLAB.

TEMPORARY DRAINAGE DETAIL

PROJECT NO. B-4109 DURHAM STATION: 16+21.00 -L-

SHEET 2 OF 2

STATE OF NORTH CAROLINA DEPARTMENT OF TRANSPORTATION RALEIGH

BRIDGE APPROACH SLAB DETAILS

REVISIONS					SHEET NO.
BY:	DATE:	NO.	BY:	DATE:	S-24
		3			TOTAL SHEETS
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STD. NO	. BAS10				

CHECKED BY : S. DOMBROWSKI DATE : 08/07 REV. 8/16/99 MAB/LES REV. 10/17/00 RWW/LES REV. 5/7/03 RWW/JTE DRAWN BY: FCJ 11/88 CHECKED BY : ARB 11/88

DATE: 08/07

ASSEMBLED BY : R. G. EMERSON

STANDARD NOTES

DESIGN DATA:

A.A.S.H.T.O. (CURRENT) SPECIFICATIONS SEE PLANS LIVE LOAD ---- SEE A.A.S.H.T.O. IMPACT ALLOWANCE STRESS IN EXTREME FIBER OF STRUCTURAL STEEL - AASHTO M270 GRADE 36 20,000 LBS. PER SQ. IN. - AASHTO M270 GRADE 50W - 27,000 LBS. PER SQ. IN. - AASHTO M270 GRADE 50 - 27,000 LBS. PER SQ. IN. REINFORCING STEEL IN TENSION -- 24,000 LBS. PER SQ. IN. 1.200 LBS. PER SQ. IN. CONCRETE IN COMPRESSION ---- SEE A.A.S.H.T.O. CONCRETE IN SHEAR STRUCTURAL TIMBER - TREATED OR 1.800 LBS. PER SQ. IN. UNTREATED - EXTREME FIBER STRESS COMPRESSION PERPENDICULAR TO GRAIN 375 LBS. PER SQ. IN. OF TIMBER ----30 LBS. PER CU. FT. EQUIVALENT FLUID PRESSURE OF EARTH

MATERIAL AND WORKMANSHIP:

EXCEPT AS MAY OTHERWISE BE SPECIFIED ON PLANS OR IN THE SPECIAL PROVISIONS, ALL MATERIAL AND WORKMANSHIP SHALL BE IN ACCORDANCE WITH THE 2002 STANDARD SPECIFICATIONS "FOR ROADS AND STRUCTURES" OF THE N. C. DEPARTMENT OF TRANSPORTATION.

(MINIMUM)

STEEL SHEET PILING FOR PERMANENT OR TEMPORARY APPLICATIONS SHALL BE HOT ROLLED.

CONCRETE:

UNLESS OTHERWISE REQUIRED ON PLANS, CLASS A CONCRETE SHALL BE USED FOR ALL PORTIONS OF ALL STRUCTURES WITH THE EXCEPTION THAT: CLASS AA CONCRETE SHALL BE USED IN BRIDGE SUPERSTRUCTURES, ABUTMENT BACKWALLS, AND APPROACH SLABS; CLASS B CONCRETE SHALL BE USED FOR SLOPE PROTECTION AND RIP RAP; AND CLASS S SHALL BE USED FOR UNDERWATER FOOTING SEALS.

CONCRETE CHAMFERS:

UNLESS OTHERWISE NOTED ON THE PLANS, ALL EXPOSED CORNERS ON STRUCTURES SHALL BE CHAMFERED 3/4"WITH THE FOLLOWING EXCEPTIONS: TOP CORNERS OF CURBS MAY BE ROUNDED TO 1-1/2"RADIUS WHICH IS BUILT INTO CURB FORMS; CORNERS OF TRANSVERSE FLOOR EXPANSION JOINTS SHALL BE ROUNDED WITH A 1/4"FINISHING TOOL UNLESS OTHERWISE REQUIRED ON PLANS; AND CORNERS OF EXPANSION JOINTS IN THE ROADWAY FACES AND TOPS OF CURBS AND SIDEWALKS SHALL BE ROUNDED TO A 1/4"RADIUS WITH A FINISHING STONE OR TOOL UNLESS OTHERWISE REQUIRED ON PLANS.

DOWELS:

DOWELS WHEN INDICATED ON PLANS AS FOR CULVERT EXTENSIONS, SHALL BE EMBEDDED AT LEAST 12" INTO THE OLD CONCRETE AND GROUTED INTO PLACE WITH 1:2 CEMENT MORTAR.

ALLOWANCE FOR DEAD LOAD DEFLECTION, SETTLEMENT, ETC. IN CASTING SUPERSTRUCTURES:

BRIDGES SHALL BE BUILT ON THE GRADE OR VERTICAL CURVE SHOWN ON PLANS.
SLABS, CURBS AND PARAPETS SHALL CONFORM TO THE GRADE OR CURVE.

ALL DIMENSIONS WHICH ARE GIVEN IN SECTION AND ARE AFFECTED BY DEAD LOAD DEFLECTIONS ARE DIMENSIONS AT CENTER LINE OF BEARING UNLESS OTHERWISE NOTED ON PLANS. IN SETTING FORMS FOR STEEL BEAM BRIDGES AND PRESTRESSED CONCRETE GIRDER BRIDGES, ADJUSTMENTS SHALL BE MADE DUE TO THE DEAD LOAD DEFLECTIONS FOR THE ELEVATIONS SHOWN. WHERE BLOCKS ARE SHOWN OVER BEAMS FOR BUILDING UP TO THE SLAB, THE VERTICAL DIMENSIONS OF THE BLOCKS SHALL BE ADJUSTED BETWEEN BEARINGS TO COMPENSATE FOR DEAD LOAD DEFLECTIONS, VERTICAL CURVE ORDINATE, AND ACTUAL BEAM CAMBER. WHERE BOTTOM OF SLAB IS IN LINE WITH BOTTOM OF TOP FLANGES, DEPTH OF SLAB BETWEEN BEARINGS SHALL BE ADJUSTED TO COMPENSATE FOR DEAD LOAD DEFLECTION, VERTICAL CURVE ORDINATE, AND

IN SETTING FALSEWORK AND FORMS FOR REINFORCED CONCRETE SPANS, AN ALLOWANCE SHALL BE MADE FOR DEAD LOAD DEFLECTIONS, SETTLEMENT OF FALSEWORK AND PERMANENT CAMBER WHICH SHALL BE PROVIDED FOR IN ADDITION TO THE ELEVATIONS SHOWN. AFTER REMOVAL OF THE FALSEWORK, THE FINISHED STRUCTURES SHALL CONFORM TO THE PROFILE AND ELEVATIONS SHOWN ON THE PLANS AND CONSTRUCTION ELEVATIONS FURNISHED BY THE ENGINEER.

DETAILED DRAWINGS FOR FALSEWORK OR FORMS FOR BRIDGE SUPERSTRUCTURE AND ANY STRUCTURE OR PARTS OF A STRUCTURE AS NOTED ON THE PLANS SHALL BE SUBMITTED TO THE ENGINEER FOR APPROVAL BEFORE CONSTRUCTION OF THE FALSEWORK OR FORMS IS STARTED.

REINFORCING STEEL:

ALL REINFORCING STEEL SHALL BE DEFORMED WITH THE EXCEPTION OF #2
BARS WHICH MAY BE FABRICATED FROM COLD DRAWN STEEL WIRE. DIMENSIONS
RELATIVE TO PLACEMENT OF REINFORCING ARE TO CENTERS OF BARS UNLESS OTHERWISE
INDICATED IN THE PLANS. DIMENSIONS ON BAR DETAILS ARE TO CENTERS OF BARS
OR ARE OUT TO OUT AS INDICATED ON PLANS.

WIRE BAR SUPPORTS SHALL BE PROVIDED FOR REINFORCING STEEL WHERE INDICATED ON THE PLANS. WHEN BAR SUPPORT PIECES ARE PLACED IN CONTINUOUS LINES, THEY SHALL BE SO PLACED THAT THE ENDS OF THE SUPPORTING WIRES SHALL BE LAPPED TO LOCK LEGS ON ADJOINING PIECES.

STRUCTURAL STEEL:

AT THE CONTRACTOR'S OPTION, HE MAY SUBSTITUTE 7/8% SHEAR STUDS FOR THE 3/4% STUDS SPECIFIED ON THE PLANS. THIS SUBSTITUTION SHALL BE MADE AT THE RATE OF 3-7/8% STUDS FOR 4-3/4% STUDS, AND STUD SPACING CHANGES SHALL BE MADE AS NECESSARY TO PROVIDE THE SAME EQUIVALENT NUMBER OF 7/8% STUDS ALONG THE BEAM AS SHOWN FOR 3/4% STUDS BASED ON THE RATIO OF 3-7/8% STUDS FOR 4-3/4% STUDS. STUDS OF THE LENGTH SPECIFIED ON THE PLANS MUST BE PROVIDED. THE MAXIMUM SPACING SHALL BE 2'-0%.

EXCEPT AT THE INTERIOR SUPPORTS OF CONTINUOUS BEAMS WHERE THE COVER PLATE IS IN CONTACT WITH BEARING PLATE, THE CONTRACTOR MAY, AT HIS OPTION, SUBSTITUTE FOR THE COVER PLATES DESIGNATED ON THE PLANS COVER PLATES OF THE EQUIVALENT AREA PROVIDED THESE PLATES ARE AT LEAST 5/16"IN THICKNESS AND DO NOT EXCEED A WIDTH EQUAL TO THE FLANGE WIDTH LESS 2"OR A THICKNESS EQUAL TO 2 TIMES THE FLANGE THICKNESS. THE SIZE OF FILLET WELDS SHALL CONFORM TO THE REQUIREMENTS OF THE CURRENT ANSI/AASHTO/AWS "BRIDGE WELDING CODE". ELECTROSLAG WELDING WILL NOT BE PERMITTED.

PLACEMENT OF BEAM OR GIRDER MEMBERS ON TRUCKS FOR HAULING SHALL
BE DONE IN COMPLIANCE WITH LIMITS SHOWN ON SKETCHES PROVIDED TO THE MATERIALS
AND TEST UNIT APPROVED BY THE STRUCTURE DESIGN UNIT DATED MAY 8, 1991.
THESE SKETCHES PRIMARILY LIMIT THE UNSUPPORTED CANTILEVER LENGTH OF MEMBERS.
WHEN THE CONTRACTOR WISHES TO PLACE MEMBERS ON TRUCKS NOT IN ACCORDANCE
WITH THESE LIMITS, TO SHIP BY RAIL, TO ATTACH SHIPPING RESTRAINTS TO THE
MEMBERS OR TO INVERT MEMBERS, HE SHALL SUBMIT A SKETCH FOR APPROVAL
PRIOR TO SHIPPING. SEE ALSO ARTICLE 1072-11.

WITH THE SOLE EXCEPTION OF EDGES AT SURFACES WHICH BEAR ON OTHER SURFACES, ALL SHARP EDGES AND ENDS OF SHAPES AND PLATES SHALL BE SLIGHTLY ROUNDED BY SUITABLE MEANS TO A RADIUS OF APPROXIMATELY 1/16 INCH OR EQUIVALENT FLAT SURFACE AT A SUITABLE ANGLE PRIOR TO PAINTING, GALVANIZING, OR METALLIZING.

HANDRAILS AND POSTS:

METAL STANDARDS AND FACES OF THE CONCRETE END POSTS FOR THE METAL RAIL SHALL BE SET NORMAL TO THE GRADE OF THE CURB, UNLESS OTHERWISE SHOWN ON PLANS. THE METAL RAIL AND TOPS OF CONCRETE POSTS USED WITH THE ALUMINUM RAIL SHALL BE BUILT PARALLEL TO THE GRADE OF THE CURB.

METAL HANDRAILS SHALL BE IN ACCORDANCE WITH THE PLANS. RAILS SHALL BE AS MANUFACTURED FOR BRIDGE RAILING. CASTINGS SHALL BE OF A UNIFORM APPEARANCE. FINS AND OTHER DEFORMATIONS RESULTING FROM CASTING OR OTHERWISE SHALL BE REMOVED IN A MANNER SO THAT A UNIFORM COLORING OF THE COMPLETED CASTING SHALL BE OBTAINED. CASTINGS WITH DISCOLORATIONS OR OF NON-UNIFORM COLORING WILL NOT BE ACCEPTED. CERTIFIED MILL REPORTS ARE REQUIRED FOR METAL RAILS AND POSTS.

SPECIAL NOTES:

GENERALLY, IN CASE OF DISCREPANCY, THIS STANDARD SHEET OF NOTES SHALL GOVERN OVER THE SPECIFICATIONS, BUT THE REMAINDER OF THE PLANS SHALL GOVERN OVER NOTES HEREON, AND SPECIAL PROVISIONS SHALL GOVERN OVER ALL. SEE SPECIFICATIONS ARTICLE 105-4.

ENGLISH