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LEGEND

### STATE OF NORTH CAROLINA

### DEPARTMENT OF TRANSPORTATION

# **DIVISION OF HIGHWAYS**

GEOTECHNICAL UNIT

# **STRUCTURE** SUBSURFACE INVESTIGATION

STATE PROJECT 33433.1.1 I.D. NO. <b>B-4070</b>
F.A. PROJECT <b>BRZ-1347(2)</b>
COUNTY CHEROKEE
PROJECT DESCRIPTION BRIDGE NO. 112
ON SR-1347 OVER HANGING
DOG CREEK
SITE DESCRIPTION

STATE PROJECT REFERENCE NO. 33433.1.1 (B-4070) 1 9 STATE PROJ.NO. F.A.PROJ.NO.

### **CAUTION NOTICE**

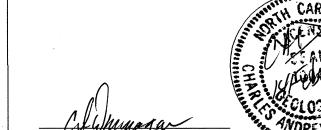
THE SUBSURFACE INFORMATION AND THE SUBSURFACE INVESTIGATION ON WHICH IT IS BASED WAS MADE FOR THE PURPOSE OF STUDY, PLANNING AND DESIGN, AND NOT FOR CONSTRUCTION OR PAY PURPOSES. THE VARIOUS FIELD BORING LOGS, ROCK CORES, AND SOIL TEST DATA AVAILABLE MAY BE REVIEWED OR INSPECTED IN RALEIGH BY CONTACTING THE N.C. DEPARTMENT OF TRANSPORTATION, GEOTECHNICAL UNIT @ (919) 250-4088. NEITHER THE SUBSURFACE PLANS AND REPORTS, NOR THE FIELD BORING LOGS, ROCK CORES, OR SOIL TEST DATA IS PART OF THE CONTRACT.

GENERAL SOIL AND ROCK STRATA DESCRIPTIONS AND INDICATED BOUNDARIES ARE BASED ON A GEOTECHNICAL INTERPRETATION OF ALL AVAILABLE SUBSURFACE DATA AND MAY NOT NECESSARLY REFLECT THE ACTUAL SUBSURFACE CONDITIONS BETWEEN BORINGS OR BETWEEN SAMPLED STRATA WITHIN THE BOREHOLE, THE LABORATORY SAMPLE DATA AND THE IN SITU (IN-PLACE) TEST DATA CAN BE RELIED ON ONLY TO THE DEGREE OF RELIABILITY INHERENT IN THE STANDARD TEST METHOD. THE OBSERVED WATER LEVELS OR SOIL MOISTURE CONDITIONS INDICATED IN THE SUBSURFACE INVESTIGATIONS ARE AS RECORDED AT THE TIME OF THE INVESTIGATION, THESE WATER LEVELS OR SOIL MOISTURE CONDITIONS MAY VARY CONSIDERABLY WITH TIME ACCORDING TO CLIMATIC CONDITIONS INCLUDING TEMPERATURES, PRECIPITATION AND WIND, AS WELL AS OTHER NON-CLIMATIC FACTORS.

THE BIDDER OR CONTRACTOR IS CAUTIONED THAT DETAILS SHOWN ON THE SUBSURFACE PLANS ARE PRELIMINARY ONLY AND IN MANY CASES THE FINAL DESIGN DETAILS ARE DIFFERENT. FOR BIDDING AND CONSTRUCTION PURPOSES, REFER TO THE CONSTRUCTION PLANS AND DOCUMENTS FOR FINAL DESIGN INFORMATION ON THIS PROJECT. THE DEPARTMENT DOES NOT WARRANT OR GUARANTEE THE SUFFICIENCY OR ACCURACY OF THE INVESTIGATION MADE, NOR THE INTERPRETATIONS MADE OR OPINION OF THE DEPARTMENT AS TO THE TYPE OF MATERIALS AND CONDITIONS TO BE ENCOUNTERED. THE BIDDER OR CONTRACTOR IS CAUTIONED TO MAKE SUCH INDEPENDENT SUBSURFACE INVESTIGATIONS AS HE DEEMS NECESSARY TO SATISFY HIMSELF AS TO CONDITIONS TO BE ENCOUNTERED ON THIS PROJECT. THE CONTRACTOR SHALL HAVE NO CLAIM FOR ADDITIONAL COMPENSATION OR FOR AN EXTENSION OF TIME FOR ANY REASON RESULTING FROM THE ACTUAL CONDITIONS ENCOUNTERED AT THE SITE DIFFERING FROM THOSE INDICATED IN THE SUBSURFACE INFORMATION.

INVESTIGATED BY C A DUNNAGAN PERSONNEL T B DANIEL CHECKED BY W D FRYE, Jr C J COFFEY R D CHILDERS SUBMITTED BY W D FRYE, Jr

DATE FEBRUARY 2006



NOTE - THE INFORMATION CONTAINED HEREIN IS NOT IMPLIED OR GUARANTEED BY THE N. C. DEPARTMENT OF TRANSPORTATION AS BEING ACCURATE NOR IT IS CONSIDERED TO BE PART OF THE PLANS, SPECIFICATIONS, OR CONTRACT FOR THE PROJECT.

NOTE - BY HAVING REQUESTED THIS INFORMATION THE CONTRACTOR SPECIFICALLY WAIVES ANY CLAIMS FOR INCREASED COMPENSATION OR EXTENSION OF TIME BASED ON DIFFERENCES BETWEEN THE CONDITIONS INDICATED HEREIN AND THE ACTUAL CONDITIONS AT THE PROJECT SITE.

DRAWN BY: C A DUNNAGAN

### NORTH CAROLINA DEPARTMENT OF TRANSPORTATION

#### DIVISION OF HIGHWAYS

GEOTECHNICAL ENGINEERING UNIT

#### PROJECT REFERENCE NO. 33433.I.I (B-4070) SHEET NO.

0F 9

### SUBSURFACE INVESTIGATION

SOIL AND ROCK LEGEND, TERMS, SYMBOLS, AND ABBREVIATIONS											
SOIL DESCRIPTION	GRADATION	ROCK DESCRIPTION	TERMS AND DEFINITIONS								
SOIL IS CONSIDERED TO BE THE UNCONSOLIDATED, SEMI-CONSOLIDATED, OR WEATHERED EARTH MATERIALS THAT CAN BE PENETRATED WITH A CONTINUOUS FLIGHT POWER AUGER, AND YIELD LESS THAN 180 BLOWS PER FOOT ACCORDING TO STANDARD PENETRATION TEST (AASHTO TZOR, ASTM D-1566, SOIL CLASSIFICATION IS ACCORDING TO STANDARD PENETRATION TEST (AASHTO TZOR, ASTM D-1566, SOIL CLASSIFICATION IS ACCORDING TO STANDARD PENETRATION TEST (AASHTO TZOR, ASTM D-1566, SOIL CLASSIFICATION IS ACCORDING TO STANDARD PENETRATION TEST (AASHTO TZOR, ASTM D-1566, SOIL CLASSIFICATION IS ACCORDING TO STANDARD PENETRATION TO STANDARD PENETRATION TEST (AASHTO TZOR, ASTM D-1566, SOIL CLASSIFICATION TO STANDARD PENETRATION TEST (AASHTO TZOR, ASTM D-1566, SOIL CLASSIFICATION TO STANDARD PENETRATION TEST (AASHTO TZOR, ASTM D-1566, SOIL CLASSIFICATION TO STANDARD PENETRATION TEST (AASHTO TZOR, ASTM D-1566, SOIL CLASSIFICATION TO STANDARD PENETRATION TEST (AASHTO TZOR, ASTM D-1566, SOIL CLASSIFICATION TO STANDARD PENETRATION TEST (AASHTO TZOR, ASTM D-1566, SOIL CLASSIFICATION TO STANDARD PENETRATION TEST (AASHTO TZOR, ASTM D-1566, SOIL CLASSIFICATION TEST (AASHTO TZOR, ASTM D-1566, SOIL CLASSIFICA	WELL GRADED - INDICATES A GOOD REPRESENTATION OF PARTICLE SIZES FROM FINE TO COARSE.  UNIFORM - INDICATES THAT SOIL PARTICLES ARE ALL APPROXIMATELY THE SAME SIZE. (ALSO POORLY GRADED)  GAP-GRADED - INDICATES A MIXTURE OF UNIFORM PARTICLES OF TWO OR MORE SIZES.	HARD ROCK IS NON-COASTAL PLAIN MATERIAL THAT IF TESTED, WOULD YIELD SPT REFUSAL AN INFERRED ROCK LINE INDICATES THE LEVEL AT WHICH NON-COASTAL PLAIN MATERIAL WOULD YIELD SPT REFUSAL. SPT REFUSAL IS PENETRATION BY A SPLIT SPOON SAMPLER EQUAL TO OR LESS THAN 0.1 FOOT PER 60 BLOWS. IN NON-COASTAL PLAIN MATERIAL, THE TRANSITION BETWEEN SOIL AND ROCK IS OFTEN REPRESENTED BY A ZONE	ALLUVIUM (ALLUV.) - SOILS THAT HAVE BEEN TRANSPORTED BY WATER.  AQUIFER - A WATER BEARING FORMATION OR STRATA.  ARENACEOUS - APPLIED TO ROCKS THAT HAVE BEEN DERIVED FROM SAND OR THAT CONTAIN SAND.								
CLASSIFICATION IS BASED ON THE AASHTO SYSTEM. BASIC DESCRIPTIONS GENERALLY SHALL INCLUDE: CONSISTENCY, COLOR, TEXTURE, MOISTURE, AASHTO CLASSIFICATION, AND OTHER PERTINENT FACTORS SUCH	ANGULARITY OF GRAINS  THE ANGULARITY OR ROUNDNESS OF SOIL GRAINS IS DESIGNATED BY THE TERMS; ANGULAR,	OF WEATHERED ROCK. ROCK MATERIALS ARE TYPICALLY DIVIDED AS FOLLOWS:	ARGILLACEOUS - APPLIED TO ALL ROCKS OR SUBSTANCES COMPOSED OF CLAY MINERALS,								
AS MINERALOGICAL COMPOSITION, ANGULARITY, STRUCTURE, PLASTICITY, ETC. EXAMPLE:  VERY STIFF, GRAY, SUTY CLAY, MOST WITH INTERGEDOED FINE SAND LIVERS, HISHLY, PLASTIC, 4-7-6	SUBANGULAR, SUBROUNDED, OR ROUNDED.	WEATHERED NON-COASTAL PLAIN MATERIAL THAT WOULD YIELD SPT N VALUES > 100 ROCK (WR)	OR HAVING A NOTABLE PROPORTION OF CLAY IN THEIR COMPOSITION, AS SHALE, SLATE, ETC.								
SOIL LEGEND AND AASHTO CLASSIFICATION	MINERALOGICAL COMPOSITION	CRYCTALLINE FINE TO COARSE GRAIN IGNEOUS AND METAMORPHIC ROCK THAT	ARTESIAN - GROUND WATER THAT IS UNDER SUFFICIENT PRESSURE TO RISE ABOVE THE LEVEL AT WHICH IT IS ENCOUNTERED, BUT WHICH DOES NOT NECESSARILY RISE TO DR ABOVE THE								
GENERAL GRANULAR MATERIALS SILT-CLAY MATERIALS ORGANIC MATERIALS CLASS. (≤ 35% PASSING *200) (> 35% PASSING *200) ORGANIC MATERIALS	MINERAL NAMES SUCH AS QUARTZ, FELDSPAR, MICA, TALC, KAOLIN, ETC. ARE USED IN DESCRIPTIONS WHENEVER THEY ARE CONSIDERED OF SIGNIFICANCE.	ROCK (CR) WOULD YIELD SPT REFUSAL IF TESTED. ROCK TYPE INCLUDES GRANITE, GNEISS, GABBRO, SCHIST, ETC.	GROUND SURFACE.  CALCAREOUS (CALC.) - SOILS THAT CONTAIN APPRECIABLE AMOUNTS OF CALCIUM CARBONATE.								
GROUP A-1 A-3 A-2 A-4 A-5 A-6 A-7 CLASS. A-1-a A-1-b A-2-4 A-2-5 A-2-6 A-2-7 A-4 A-5 A-6 A-7 A-3 A-6 A-7	COMPRESSIBILITY SLIGHTLY COMPRESSIBLE LIQUID LIMIT LESS THAN 31	NON-CRYSTALLINE FINE TO COARSE GRAIN METAMORPHIC AND NON-COASTAL PLAIN SEDIMENTARY ROCK THAT WOULD YELLD SPT REFUSAL IF TESTED, ROCK TYPE INCLUDES PHYLLITE, SLATE, SANDSTONE, ETC.	COLLUVIUM - ROCK FRAGMENTS MIXED WITH SOIL DEPOSITED BY GRAVITY ON SLOPE OR AT BOTTOM OF SLOPE.								
SYMBOL 000000000000000000000000000000000000	MODERATELY COMPRESSIBLE LIQUID LIMIT EQUAL TO 31-50 HIGHLY COMPRESSIBLE LIQUID LIMIT GREATER THAN 50	COASTAL PLAIN COASTAL PLAIN SEDIMENTS CEMENTED INTO ROCK, BUT MAY NOT YIELD SEDIMENTARY ROCK  SET REFUSAL, ROCK TYPE INCLUDES LIMESTONE, SANDSTONE, CEMENTED SHELL BEDG ETC.	CORE RECOVERY (REC.) - TOTAL LENGTH OF ALL MATERIAL RECOVERED IN THE CORE BARREL DIVIDED BY TOTAL LENGTH OF CORE RUN AND EXPRESSED AS A PERCENTAGE.								
Z PASSING SILT- SILT- MUCK,	PERCENTAGE OF MATERIAL  GRANUAR SILT - CLAY	(CP) SHELL BEDS, ETC. WEATHERING	DIKE - A TABULAR BODY OF IGNEOUS ROCK THAT CUTS ACROSS THE STRUCTURE OF ADJACENT								
4 0 38 MX 58 MX 51 MN	ORGANIC MATERIAL         SOILS         OTHER MATERIAL           TRACE OF ORGANIC MATTER         2 - 3%         3 - 5%         TRACE         1 - 10%           LITTLE ORGANIC MATTER         3 - 5%         5 - 12%         LITTLE         10 - 20%	FRESH ROCK FRESH, CRYSTALS BRIGHT, FEW JOINTS MAY SHOW SLIGHT STAINING. ROCK RINGS UNDER HAMMER IF CRYSTALLINE.	ROCKS OR CUTS MASSIVE ROCK.  DIP - THE ANGLE AT WHICH A STRATUM OR ANY PLANAR FEATURE IS INCLINED FROM THE HORIZONTAL.								
LIDUID LINDT PLASTIC INDEX 6 MX NP 18 MX 18 MX 19 MX 19 MX 18 MX 18 MX 19 MX 18 MX 19 MX 1	MODERATELY ORGANIC         5 - 10%         12 - 20%         SOME         20 - 35%           HIGHLY ORGANIC         >10%         >20%         HIGHLY         35% AND ABOVE	VERY SLIGHT ROCK GENERALLY FRESH, JOINTS STAINED, SOME JOINTS MAY SHOW THIN CLAY COATINGS IF OPEN, (CYSTALS ON A BROKEN SPECIMEN FACE SHINE BRIGHTLY, ROCK RINGS UNDER HAMMER BLOWS IF OF A CRYSTALLINE NATURE.	DIP DIRECTION (DIP AZIMUTH) - THE DIRECTION OR BEARING OF THE HORIZONTAL TRACE OF THE LINE OF DIP, MEASURED CLOCKWISE FROM NORTH.								
USUAL TYPES STONE FRAGS. FINE SILTY OR CLAYEY SILTY CLAYEY ORGANIC	GROUND WATER  WATER LEVEL IN BORE HOLE IMMEDIATELY AFTER DRILLING	SLIGHT ROCK GENERALLY FRESH, JOINTS STAINED AND DISCOLORATION EXTENDS INTO ROCK UP TO  (SLIJ) 1 INCH. OPEN JOINTS MAY CONTAIN CLAY. IN GRANITOID ROCKS SOWNE CCASSIONAL FELDSPAR CRYSTALS ARE DULL AND DISCOLORED. CRYSTALLINE ROCKS RING UNDER HAMMER BLOWS.	FAULT - A FRACTURE OR FRACTURE ZONE ALONG WHICH THERE HAS BEEN DISPLACEMENT OF THE SIDES RELATIVE TO ONE ANOTHER PARALLEL TO THE FRACTURE.  FISSILE - A PROPERTY OF SPLITTING ALONG CLOSELY SPACED PARALLEL PLANES.								
MATERIALS SAND SAND GRAVEL AND SAND SUILS SUILS	STATIC WATER LEVEL AFTER 24 HOURS	MODERATE SIGNIFICANT PORTIONS OF ROCK SHOW DISCOLORATION AND WEATHERING EFFECTS. IN	FLOAT - ROCK FRAGMENTS ON SURFACE NEAR THEIR ORIGINAL POSITION AND DISLODGED FROM								
AS A EXCELLENT TO GOOD FAIR TO POOR POOR UNSUITABL	PERCHED WATER SATURATED ZONE, OR WATER BEARING STRATA  SPRING OR SEEP	(MOD.) GRANITOID ROCKS, MOST FELOSPARS ARE DULL AND DISCOLORED, SOME SHOW CLAY. ROCK HAS DULL SOUND UNDER HAMMER BLOWS AND SHOWS SIGNIFICANT LOSS OF STRENGTH AS COMPARED WITH FRESH ROCK.	PARENT MATERIAL.  FLOOD PLAIN (FP) - LAND BORDERING A STREAM, BUILT OF SEDIMENTS DEPOSITED BY THE STREAM.								
PI OF A-7-5 SUBGROUP IS ≤ LL - 30; PI OF A-7-6 SUBGROUP IS > LL - 30  CONSISTENCY OR DENSENESS	MISCELLANEOUS SYMBOLS	MODERATELY ALL ROCK EXCEPT QUARTZ DISCOLORED OR STAINED. IN GRANITOID ROCKS, ALL FELDSPARS DULL SEVERE AND DISCOLORED AND A MAJORITY SHOW KAOLINIZATION. ROCK SHOWS SEVERE LOSS OF STRENGTH	FORMATION (FM.) - A MAPPABLE GEOLOGIC UNIT THAT CAN BE RECOGNIZED AND TRACED IN								
PRIMARY SOIL TYPE COMPACTNESS OR CONSISTENCY RANGE OF STANDARD RANGE OF UNCONFINED COMPRESSIVE STRENGTH (TONS/FT2')	ROADWAY EMBANKMENT (RE) WITH SOIL DESCRIPTION  ST TEST BORING DESIGNATIONS	(MOD. SEV.) AND CAN BE EXCAVATED WITH A GEOLOGIST'S PICK, ROCK GIVES "CLUNK" SOUND WHEN STRUCK,  IF TESTED, WOULD YIELD SPT REFUSAL	THE FIELD.  JOINT - FRACTURE IN ROCK ALONG WHICH NO APPRECIABLE MOVEMENT HAS OCCURRED.								
GENERALLY VERY LOOSE <4	S - BULK SAMPLE  AUGER BORING	SEVERE ALL ROCK EXCEPT QUARTZ DISCOLORED OR STAINED, ROCK FABRIC CLEAR AND EVIDENT BUT REDUCED (SEV.) IN STRENGTH TO STRONG SQLL, IN GRANITOID ROCKS ALL FELDSPARS ARE KAOLINIZED TO SOME	LEDGE - A SHELF-LIKE RIDGE OR PROJECTION OF ROCK WHOSE THICKNESS IS SMALL COMPARED TO ITS LATERAL EXTENT.								
MATERIAL MEDIUM DENSE 10 TO 30 N/A	SS - SPLIT SPOON SAMPLE  THAN BRADUAY FARANYMENT - CORE BORING	EXTENT. SOME FRAGMENTS OF STRONG ROCK USUALLY REMAIN.  IF TESTED, YIELDS SPT N VALUES > 100 BPF	LENS - A BODY OF SOIL OR ROCK THAT THINS OUT IN ONE OR MORE DIRECTIONS.								
(NON-COHESIVE) DENSE 30 TO 50 VERY DENSE >50	THAN ROADWAY EMBANKMENT - CORE BURING ST - SHELBY TUBE  INFERRED SOIL BOUNDARY SAMPLE	VERY SEVERE ALL ROCK EXCEPT QUARTZ DISCOLORED OR STAINED. ROCK FABRIC ELEMENTS ARE DISCERNIBLE BUT (Y SEV.) THE MASS IS EFFECTIVELY REDUCED TO SOIL STATUS, WITH ONLY FRAGMENTS OF STRONG ROCK	MOTTLED (MOT.)- IRREGULARLY MARKED WITH SPOTS OF DIFFERENT COLORS. MOTTLING IN SOILS USUALLY INDICATES POOR AERATION AND LACK OF GOOD DRAINAGE.								
VERY SOFT         <2         <0.25           GENERALLY         SOFT         2 TO 4         0.25 TO 0.50	MONITORING WELL DM - DECT TENT MODULLIS	REMAINING. SAPROLITE IS AN EXAMPLE OF ROCK WEATHERED TO A DEGREE SUCH THAT ONLY MINOR VESTIGES OF THE ORIGINAL ROCK FABRIC REMAIN. IF TESTED, YIELDS SPT N VALUES < 100 BPF	PERCHED WATER - WATER MAINTAINED ABOVE THE NORMAL GROUND WATER LEVEL BY THE PRESENCE OF AN INTERVENING IMPERVIOUS STRATUM.								
SILT-CLAY MEDIUM STIFF 4 TO 8 0.5 TO 1.0 MATERIAL STIFF 8 TO 15 1 TO 2	PIEZOMETER SAMPLE  INSTALLATION	COMPLETE ROCK REDUCED TO SOIL, ROCK FABRIC NOT DISCERNIBLE, OR DISCERNIBLE ONLY IN SMALL AND	RESIDUAL (RES.) SOIL - SOIL FORMED IN PLACE BY THE WEATHERING OF ROCK.								
(COHESIVE) VERY STIFF 15 TO 30 2 TO 4 HARD >30 >4	RS - ROCK SAMPLE  SLOPE INDICATOR	SCATTERED CONCENTRATIONS. QUARTZ MAY BE PRESENT AS DIKES OR STRINGERS. SAPROLITE IS ALSO AN EXAMPLE.	ROCK QUALITY DESIGNATION (ROD) - A MEASURE OF ROCK QUALITY DESCRIBED BY TOTAL LENGTH OF ROCK SEGMENTS EQUAL TO OR GREATER THAN 4 INCHES DIVIDED BY THE TOTAL LENGTH OF CORE RUN AND								
TEXTURE OR GRAIN SIZE	25/025 DIP & DIP DIRECTION OF INSTALLATION RT - RECOMPACTED TRIAXI SAMPLE  SPT N-VALUE	ROCK HARDNESS	EXPRESSED AS A PERCENTAGE.								
U.S. STD. SIEVE SIZE 4 10 40 60 200 270	SOUNDING ROD  SOUNDING ROD  SOUNDING ROD  SOUNDING ROD  REF)— SPT REFUSAL  RATIO SAMPLE  CBR - CALIFORNIA BEARING  REF)— SPT REFUSAL  RATIO SAMPLE	VERY HARD CANNOT BE SCRATCHED BY KNIFE OR SHARP PICK, BREAKING OF HAND SPECIMENS REQUIRES SEVERAL HARD BLOWS OF THE GEOLOGIST'S PICK,	SAPROLITE (SAP.) - RESIDUAL SOIL THAT RETAINS THE RELIC STRUCTURE OR FABRIC OF THE PARENT ROCK.								
OPENING (MM) 4.76 2.00 0.42 0.25 0.075 0.053	ABBREVIATIONS  AR - AUGER REFUSAL HI HIGHLY # - MOISTURE CONTENT	HARD CAN BE SCRATCHED BY KNIFE OR PICK ONLY WITH DIFFICULTY. HARD HAMMER BLOWS REQUIRED TO DETACH HAND SPECIMEN.	SILL - AN INTRUSIVE BODY OF IGNEOUS ROCK OF APPROXIMATELY UNIFORM THICKNESS AND RELATIVELY THIN COMPARED WITH ITS LATERAL EXTENT, THAT HAS BEEN EMPLACED PARALLEL TO THE BEDDING OR SCHISTOSITY OF THE INTRUDED ROCKS.								
GRAIN MM 305 75 2.0 0.25 0.05 0.005	BT - BORING TERMINATED MED MEDIUM V - VERY CL CLAY MICA MICACEOUS VST - VANE SHEAR TEST	MODERATELY CAN BE \$CRATCHED BY KNIFE OR PICK, COUGES OR GROOVES TO 0.25 INCHES DEEP CAN BE HARD EXCAVATED BY HARD BLOW OF A GEOLOGIST'S PICK, HAND SPECIMENS CAN BE DETACHED BY MODERATE BLOWS.	SLICKENSIDE - POLISHED AND STRIATED SURFACE THAT RESULTS FROM FRICTION ALONG A FAULT OR SLIP PLANE.								
SIZE IN 12 3  SOIL MOISTURE - CORRELATION OF TERMS	CPT - CONE PENETRATION TEST MOD MODERATELY WEA WEATHERED  CSE COARSE NP - NON PLASTIC 7- UNIT WEIGHT  DMT - DILATOMETER TEST ORG ORGANIC 7_d - DRY UNIT WEIGHT	MEDIUM CAN BE GROOVED OR GOUGED 0.05 INCHES DEEP BY FIRM PRESSURE OF KNIFE OR PICK POINT. HARD CAN BE EXCAVATED IN SMALL CHIPS TO PEICES I INCH MAXIMUM SIZE BY HARD BLOWS OF THE	STANDARD PENETRATION TEST (PENETRATION RESISTANCE) (SPT) - NUMBER OF BLOWS (N OR BPF) OF A 140 LB, HAMMER FALLING 30 INCHES REQUIRED TO PRODUCE A PENETRATION OF 1 FOOT INTO SOIL WITH A 2 INCH OUTSIDE DIAMETER SPLIT SPOON SAMPLER. SPT REFUSAL IS PENETRATION EQUAL TO OR LESS								
SOIL MOISTURE SCALE FIELD MOISTURE GUIDE FOR FIELD MOISTURE DESCRIPTION  GUIDE FOR FIELD MOISTURE DESCRIPTION	DPT - DYNAMIC PENETRATION TEST PMT - PRESSUREMETER TEST e - VOID RATIO SAP SAPROLITIC F - FINE SD SAND, SANDY	POINT OF A GEOLOGIST'S PICK.  SOFT CAN BE GROVED OR GOUGED READILY BY KNIFE OR PICK. CAN BE EXCAVATED IN FRAGMENTS FROM CHIPS TO SEVERAL INCHES IN SIZE BY MODERATE BLOWS OF A PICK POINT. SMALL, THIN	THAN 0.1 FOOT PER 60 BLOWS.  STRATA CORE RECOVERY (SREC.) - TOTAL LENGTH OF STRATA MATERIAL RECOVERED DIVIDED BY TOTAL LENGTH OF STRATUM AND EXPRESSED AS A PERCENTAGE.								
- SATURATED - USUALLY LIQUID: VERY WET, USUALLY LIQUID: VERY WET, USUALLY LIQUID WET TABLE LL LIQUID LIMIT (SAT.)	FOSS FOSSILIFEROUS SL SILT, SILTY FRAC FRACTURED, FRACTURES SLI SLIGHTLY FRAGS FRACMENTS TOR - TRICONE REFUSAL	PIECES CAN BE BROKEN BY FINGER PRESSURE.  VERY  CAN BE CARVED WITH KNIFE. CAN BE EXCAVATED READILY WITH POINT OF PICK. PIECES 1 INCH  SOFT  OR MORE IN THICKNESS CAN BE BROKEN BY FINGER PRESSURE. CAN BE SCRATCHED READILY BY	STRATA ROCK QUALITY DESIGNATION (SROD) - A MEASURE OF ROCK QUALITY DESCRIBED BY TOTAL LENGTH OF ROCK SEGMENTS WITHIN A STRATUM EQUAL TO OR GREATER THAN 4 INCHES DIVIDED								
PLASTIC SEMISOLID; REQUIRES DRYING TO	EQUIDMENT HOLD ON OUR TEST PROJECT	FINGERNAIL.	BY THE TOTAL LENGTH OF STRATA AND EXPRESSED AS A PERCENTAGE.  TOPSOIL (TS.) - SURFACE SOILS USUALLY CONTAINING ORGANIC MATTER.								
(PI) PLASTIC LIMIT ATTAIN OPTIMUM MOISTURE	EQUIPMENT USED ON SUBJECT PROJECT	FRACTURE SPACING BEDDING  IERM SPACING IERM IHICKNESS	BENCH MARK: #1 - RR SPIKE IN 18* PINE TREE RIGHT OF APPROXIMATE								
OM OPTIMUM MOISTURE - MOIST - (M) SOLID; AT OR NEAR OPTIMUM MOISTURE	DRILL UNITS:  ADVANCING TOOLS:  HAMMER TYPE:  X AUTOMATIC MANUAL  MANUAL	VERY WIDE MORE THAN 10 FEET VERY THICKLY BEDDED > 4 FEET	-L- STATION 5+00								
OM OPTIMUM MOISTURE - MUIST - (M) SOCIUE HT ON NEHR OPTIMUM MUISTURE SL SHRINKAGE LIMIT - MUIST - (M) SOCIUE HT ON NEHR OPTIMUM MUISTURE	MOBILE B CLAY BITS	MIDE 3 10 10 FEE1 THINLY BEDDED 0.16 - 1.5 FEET WEDY THINLY DEDDED 0.2 - 0.16 FEET	ASSUMED ELEVATION: 100.00 FT.								
- DRY - (D) REQUIRES ADDITIONAL WATER TO ATTAIN OPTIMUM MOISTURE	6° CONTINUOUS FLIGHT AUGER CORE SIZE:  BK-51 X 8° HOLLOW AUGERS 7-B	VERY CLOSE 0.06 TO THEET THICKLY LAMINATED 0.008 - 0.03 FEET	NOTES:								
PLASTICITY	A SHOCK ADDRES	THINLY LAMINATED < 0.008 FEET  INDURATION	-								
PLASTICITY INDEX (PI) DRY STRENGTH	THE CAPPINE MICEOTE	FOR SEDIMENTARY ROCKS, INDURATION IS THE HARDENING OF THE MATERIAL BY CEMENTING, HEAT, PRESSURE, ETC.	]								
NONPLASTIC 0-5 VERY LOW LOW PLASTICITY 6-15 SLIGHT	X CME-550	FRIABLE RUBBING WITH FINGER FREES NUMEROUS GRAINS; GENTLE BLOW BY HAMMER DISINTEGRATES SAMPLE,									
MED. PLASTICITY 16-25 MEDIUM	PORTABLE HOIST TRICONE STEEL TEETH POST HOLE DIGGER	MODERATELY INDURATED GRAINS CAN BE SEPARATED FROM SAMPLE WITH STEEL PROBE;									
HIGH PLASTICITY 26 OR MORE HIGH  COLOR	TRICONF TUNG-CARR HAND AUGER	BREAKS EASILY WHEN HIT WITH HAMMER.									
DESCRIPTIONS MAY INCLUDE COLOR OR COLOR COMBINATIONS (TAN, RED, YELLOW-BROWN, BLUE-GRAY).	CORE BIT SOUNDING ROD	INDURATED GRAINS ARE DIFFICULT TO SEPARATE WITH STEEL PROBE; DIFFICULT TO BREAK WITH HAMMER.	·								
MODIFIERS SUCH AS LIGHT, DARK, STREAKED, ETC. ARE USED TO DESCRIBE APPEARANCE.	OTHER OTHER OTHER	EXTREMELY INDURATED SHARP HAMMER BLOWS REQUIRED TO BREAK SAMPLE; SAMPLE BREAKS ACROSS GRAINS.									
		- State States Infood States	REVISED 03/07/05								



# STATE OF NORTH CAROLINA DEPARTMENT OF TRANSPORTATION

MICHAEL F. EASLEY
GOVERNOR

LYNDO TIPPETT SECRETARY

February 9, 2006

STATE PROJECT:

33433.1.1 (B-4070)

F. A. PROJECT:

BRZ-1347(2)

COUNTY:

Cherokee

**DESCRIPTION:** 

Bridge No. 112 on SR-1347 over Hanging Dog Creek

SUBJECT:

Geotechnical Report – Foundation Investigation

#### Introduction

This project is located in central Cherokee County, approximately 3.0 miles north of the town of Murphy. The proposed construction will replace the existing 3-span bridge with a single-span bridge. The span length will be 118.0 feet and the skew will be 90 degrees. The alignment will be shifted 40.0 feet downstream. The subsurface investigation was conducted using a CME-550 drill machine and 8-inch hollow stem augers. Standard Penetration Tests were performed at intervals of five feet. Soil samples were collected and submitted for testing for quality. The sample results were not available at the time of this writing.

#### **Geology and Rock Characteristics**

Rock "core" was obtained from the hollow stem augers from three of the four borings. This rock is a mica schist. Below a thin  $(\pm 1.0 \text{ foot})$  layer of weathered rock, the schist recovered is hard and fresh. The alluvial soils encountered in this project were deposited directly upon the weathered rock. In some cases, it is difficult to discern while drilling between the basal gravel/boulder layers and the weathered rock. Therefore, it is possible that the contact between alluvium and weathered rock may be as much as 1.0 foot higher in elevation as is indicated in the boring logs.

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WEBSITE: WWW.DOH.DOT.STATE.NC.US

LOCATION: CENTURY CENTER COMPLEX BUILDING B 1020 BIRCH RIDGE DRIVE RALEIGH NC 27610

#### **Foundation Materials**

#### End Bent One

Alluvium is present from the surface at End Bent One. It is comprised of 5.0 to 6.0 feet of silty sand underlain by 1.0 to 4.0 feet of gravel and boulders. In the boring for EB1-A, weathered rock was encountered at 9.5 feet (elevation 62.02). Hollow auger refusal on rock occurred at 10.0 feet (elevation 61.52). At EB1-B, weathered rock was noted at 7.0 feet (elevation 66.37). Refusal on rock was at 10.4 feet (elevation 62.97). Groundwater was measured in both borings immediately after drilling. In EB1-A, it was measured at 5.6 feet (elevation 65.92); in EB1-B, it was at 7.6 feet (elevation 65.77).

#### End Bent Two

The boring for EB2-A encountered fill material at the surface. Emplaced for a private access road, it consists of 6.5 feet of silty sand with boulders. Alluvium is present below this fill, and from the surface at EB2-B. The alluvial horizon is 7.5 to 11.0 feet of sandy silt. The basal gravel layer was not noted in EB2-A. In EB2-B, the gravel layer is approximately 2.0 feet thick. Weathered rock is directly below the alluvium. In the boring for EB2-A, it is at 14.0 feet (elevation 63.91). Hollow auger refusal on rock occurred in EB2-A at 15.1 feet (elevation 62.81). At the EB2-B site, weathered rock was recorded at 13.5 feet (elevation 62.47); refusal on rock occurred at 14.1 feet (elevation 61.87). Groundwater was not recorded from either boring.

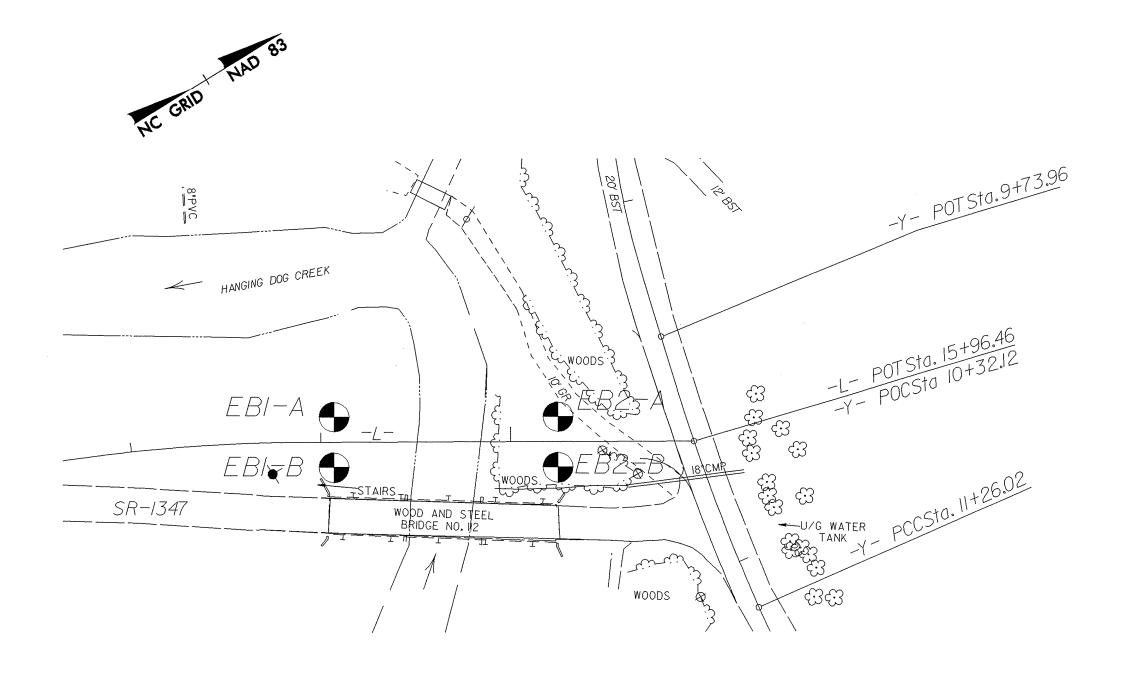
#### **Comments**

If steel H-piles are used at this site, pile tip protection should be used. This is because of the gravel/boulder layer encountered.

Respectfully Submitted,

Charles A. Dunnagan, L.G. Project Engineering Geologist

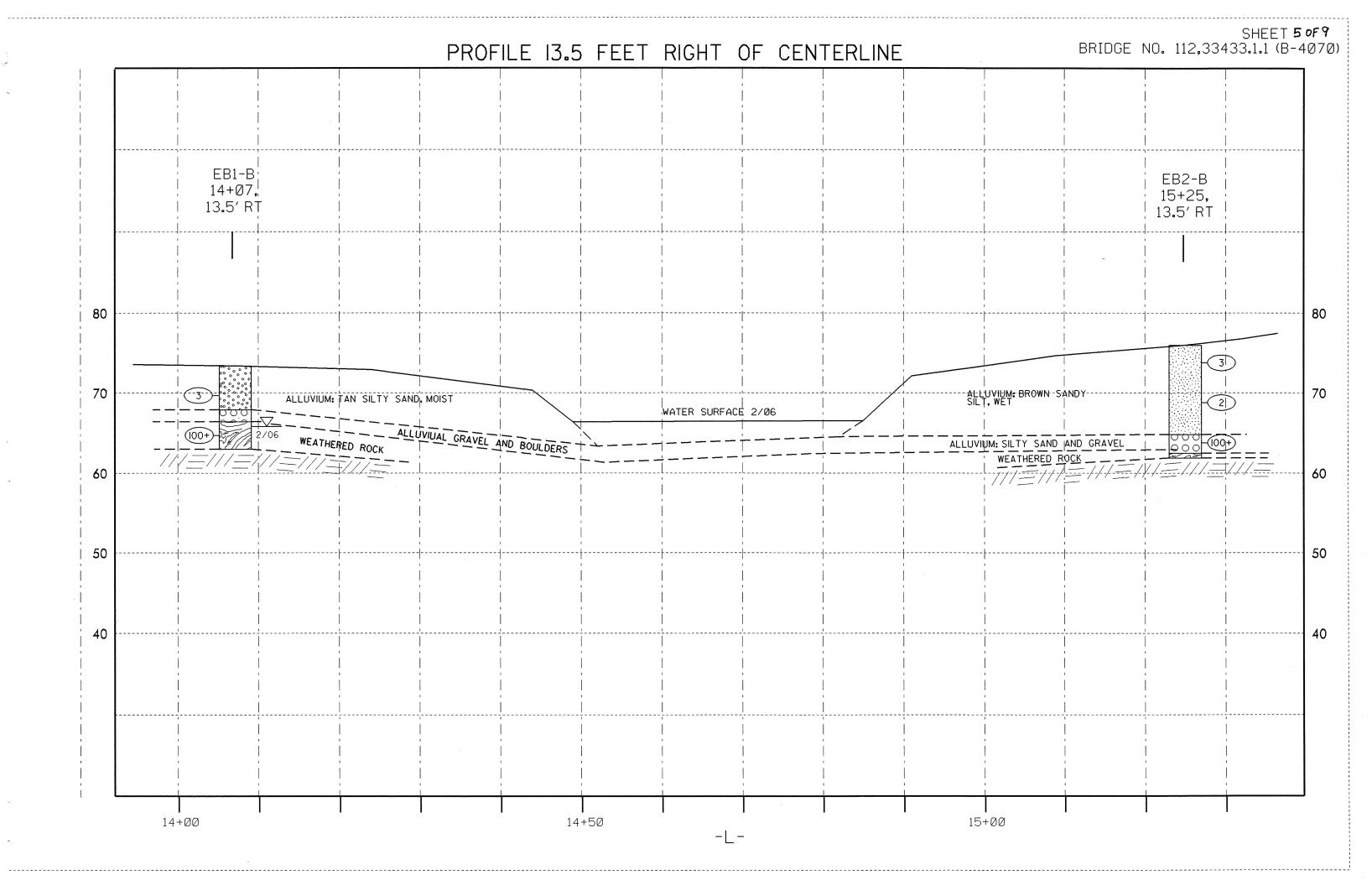
## BRIDGE NO.112 ON SR-1347 OVER HANGING DOG CREEK

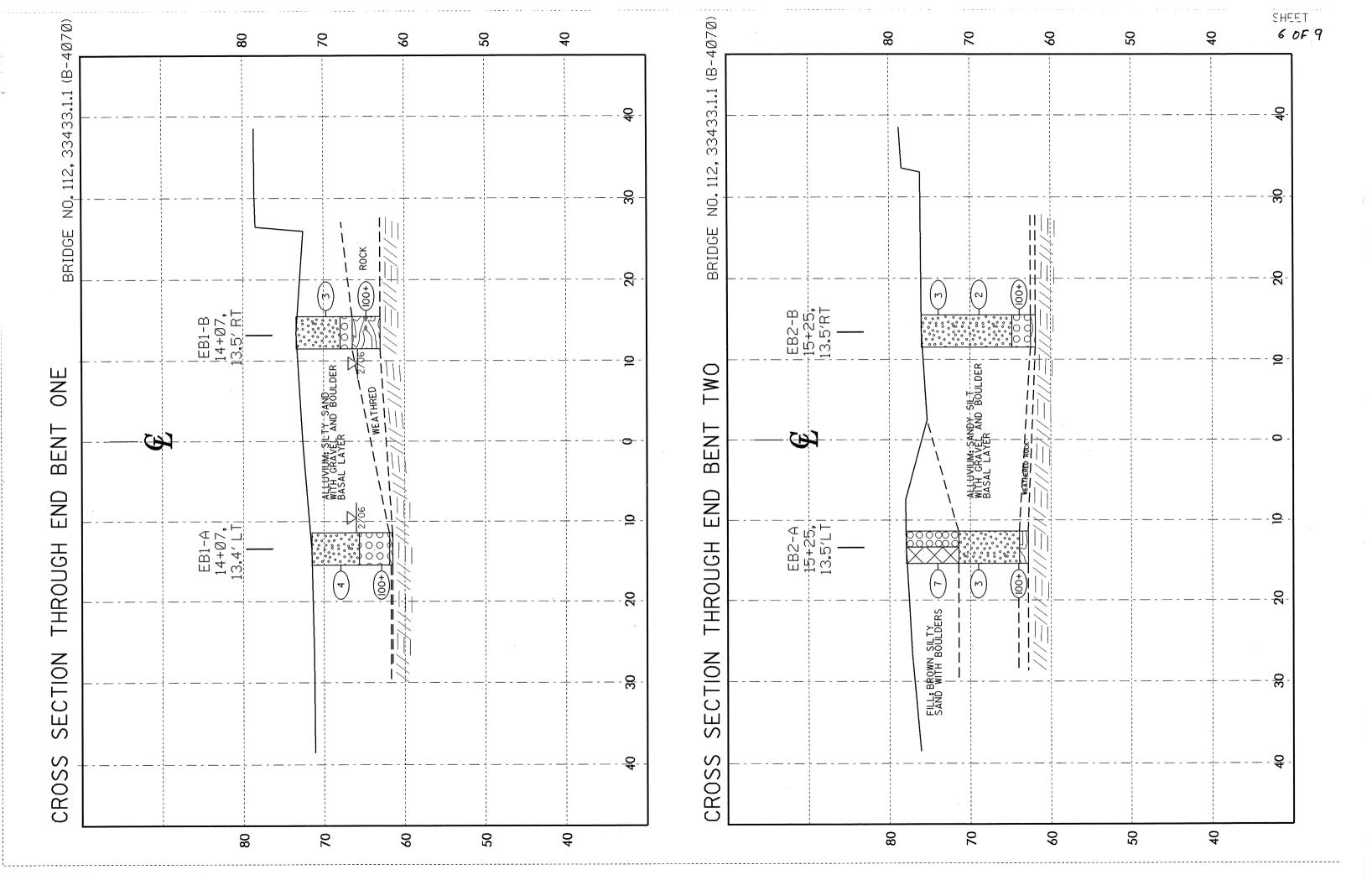


13+00

14+00

15+00





### NORTH CAROLINA DEPARTMENT OF TRANSPORTATION GEOTECHNICAL UNIT BORING LOG

			1 13				<del>, , , , , , , , , , , , , , , , , , , </del>			BORING					
	T NO 3343				ID B-4	<del></del>			IEROKEE		GEO	LOC	GIS	ST TB DANIEL	
	SCRIPTION		DGE !					HANGING	3 DOG CF	··T			_	+*	GND WATER
	NO EB1-A	<u> </u>				HING 0.00			· · · · · · · · · · · · · · · · · · ·	EASTIN				. ,	0 HR 5.60ft
ALIGNME						NG LOCAT			·		Γ 13.50ft	<u>t LT</u>			24 HR N/A
	RELEV 71.				ГОТАЈ	L DEPTH				ATE 2/02/	./06		_	COMPLETION DA	
	1ACHINE C								D H.S. AL	JGERS	***************************************			HAMMER TYPE	
SURFACE	E WATER			<del></del>	TOEN			TO ROC		1 A MOI		<del>7</del>		Log EB1-A, Page 1 of 1	
ELEV	DEPTH	ı	LOW C		PEN (ft)		BLOWS F			SAMPL			1		ND ROCK
· · · · · · · · · · · · · · · · · · ·		6in	biii,	6in	(ft)	11	25	+	<del>-</del>	00 NO	MOI	1 G	4	DESUR	RIPTION
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### NORTH CAROLINA DEPARTMENT OF TRANSPORTATION GEOTECHNICAL UNIT BORING LOG

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PR	OJECT	NO 3343	3.1.1		T	ID B-4				EROKEE		<del>,                                     </del>	.og	IST T B DANIEL	
				DGE 1	NO. 1	112 ON	SR-1347			G DOG CF	REEK	J.			GND WATER
во	RING N	NO EB1-B				NORTI	HING 0.00	)			EASTING	0.00		•	0 HR 7.60ft
AL	IGNME	ENT -L-			]	BORIN	G LOCAT	ION 14	+07.000		OFFSET	13.50ft	RT		24 HR N/A
CO	LLAR	ELEV 73.	37ft			TOTAI	DEPTH	10.40ft		START DA	TE 2/02/0	6		COMPLETION	DATE 02/02/06
DR	ILL MA	ACHINE C	OME 5	50				DRILL	METHO	D H.S. AL	IGERS			HAMMER TYPI	E AUTOMATIC
SU.	RFACE	WATER	DEPT	H				DEPTH	TO RO	CK N/A				Log EB1-B, Page 1 of	1
ш	LEV	DEPTH	BL	.OW (	CT	PEN	E	BLOWS F	PER FO	TC	SAMPLĘ	Y/	디		ND ROCK
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## NORTH CAROLINA DEPARTMENT OF TRANSPORTATION GEOTECHNICAL UNIT BORING LOG

	1									ORING	LOG			Section 1
PROJECT					D B-4				ROKEE		GEO	LOG	IST T B DANIEL	
SITE DES	CRIPTION	BRI	DGE	<u>NO. 1</u>	12 ON	I SR-1347	OVER H	HANGING	DOG CF	REEK			-c	GND WATER
BORING I	NO EB2-A	<u> </u>			ORT	HING 0.0	0	·		EASTING	0.00			0 HR N/A
ALIGNMI	ENT -L-			I	ORIN	G LOCAT	TION 15	+25.000		OFFSET	13.50ft	LT		24 HR N/A
COLLAR	ELEV 77.	91ft			OTAI	_ DEPTH	15.10ft		TART DA	ATE 2/01/0	)6		COMPLETION D.	ATE 02/01/06
DRILL MA	ACHINE (	CME 5	550				DRILL	METHO	H.S. AL	JGERS			HAMMER TYPE	AUTOMATIC
SURFACE	WATER				<del>,</del>			TO ROC					Log EB2-A, Page 1 of 1	
ELEV	DEPTH	l	-OW		PEN			PER FOO		SAMPLE	MOI	9		ID ROCK
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### NORTH CAROLINA DEPARTMENT OF TRANSPORTATION GEOTECHNICAL LINIT BORING LOG

8 OF 9

GEOTECHNICAL UNIT BORING LOG																
	FNO 3343		-	I	D B-4						IST TB DANIEL					
								VER HANGING DOG CREEK						GND WATE	R	
	NO EB2-B					IING 0.00		EA			ASTING 0.00				0 HR N/A	
ALIGNM	ENT -L-			F	ORIN	G LOCAT	<del></del>			OF	FSET 1:	3.50ft	RT		24 HR N/A	
COLLAR	ELEV 75.	97ft		7	OTAI	DEPTH	14.10ft	S	TART DA	TE	2/01/06	<del></del>		COMPLETION DA		3
DRILL M	ACHINE (	ME 5	50		DRILL METHOD H.S. AUGERS HAMMER TY					HAMMER TYPE	AUTOMATIC					
SURFACI	E WATER I						DEPTH	TO ROC	K N/A			•		Log EB2-B, Page 1 of 1		
ELEV	DEPTH		OW (		PEN			ER FOO		_1	MPLĘ	<b>Y</b> /	F	SOIL AN	ID ROCK	
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### FIELD SCOUR REPORT

PROJECT:	33433.1.1	ID:	B-4070	COUNTY: Cherokee	
DESCRIPTION(1): B	ridge No.112 o	n SR-134	7 over Hanging [	Oog Creek	

	EXISTING BRIDGE
Information from:	Field Inspection X Microfilm (reel pos: ) Other (explain)
Bridge No.: Foundation Type:	112 Length: 120 Total Bents: 4 Bents in Channel: 2 Bents in Floodplain: 2 Footings
EVIDENCE OF S Abutments or E	End Bent Slopes: None noted.
Interior Bents:	Minor amounts immediately downstream of concrete footingss.
Channel Bed:	None noted.
Channel Bank:	
	JR PROTECTION None
Extent(4):	
Effectiveness(5):	
Obstructions(6):	

#### **INSTRUCTIONS**

- 1 Describe the specific site's location, including route number and body of water crossed.
- 2 Note scour evidence at existing end bents or abutments (e.g. undermining, sloughing, degradations).
- 3 Note existing scour protection (e.g. rip rap).
- 4 Describe extent of existing scour protection.
- 5 Describe whether or not the scour protection appears to be working.
- 6 Note obstructions such as dams, fallen trees, debris at bents, etc.
- 7 Describe the channel bed material based on observation and/or samples. Include any lab results with report.
- 8 Describe the channel bank material based on observation and/or samples. Include any lab results with report.
- **9** Describe the material covering the banks (e.g. grass, trees, rip rap, none).
- 10 Determine the approximate floodplain width from field observation or a topographic map.
- 11 Describe the material covering the floodplain (e.g. grass, trees, crops).
- 12 Use professional judgement to specify if the stream is degrading, aggrading, or static.
- 13 Describe potential and direction of the stream to migrate laterally during the bridge's life (approx. 100 years).
- Give the geotechnically adjusted scour elevation (GASE) expected over the life of the bridge (approx. 100 years). This elevation can be given as a range across the site, or for each bent. Discuss the relationship between the Hydraulics Unit theoritical scour and the GASE. If the GASE is dependent on scour counter measures, explain (e.g. rip rap armoring on slopes). The GASE is based on the erodability of materials, giving consideration to the influence of joints, foliation, bedding characteristics, % core recovery, % RQD, differential weathering, shear strength, observations at existing structures, other tests deemed appropriate, and overall geologic conditions at the site.

1 D <sub>1</sub> 1	<u>DESIGN INFORMATION</u>	
Channel Bed Material(7)	): Sand, gravel, cobbles and boulders.	
•	·	
Channel Bank Material(8)	): Silty sand.	
( ·	: Silty sand.	
Channel Bank Cover(9)	): EB1: grass. EB2: trees and underbrush.	
	, .	
Floodplain Width(10)	): <u>EB1 &gt; 100ft.</u> <u>EB2 +/- 50ft.</u>	<u></u>
Floodplain Cover(11)	): <u>EB1 &gt; 100ft.</u> EB2 +/- 50ft.	
Stream is(12)	): Aggrading Degrading X Static	
hannel Migration Tendency(13	): North	
Observations and Other Com	ments:	
CECTECUNICALLY AD ILIC	TED COOLD ELEVATIONO(44)	
GEOTECHNICALLY ADJUS	TED SCOUR ELEVATIONS(14) Feet Meters	
BENT	<u>s</u>	
B1	B2 B3 B4	
SB Lanes, Lt		
SB Lanes, Rt		
NB Lanes, Lt		_
NB Lanes, Rt		_
·		_
· .		
Comparison of GASE to Hydr	raulics Unit theoretical scour:	
COIL ANALVEIC DECLILTE		
Bed or Bank	FROM CHANNEL BED AND BANK MATERIAL	
Sample No.		
Retained #4		
Passed #10		
Passed #40		
Passed #200		
Coarse Sand		
Fine Sand		
Silt		
Clay		
LL L		
PI I		
AASHTO		
Station		
Offset		
Depth		

Reported by:		Date:	2/8/2006
	C A Dunnagan	_	