

NOTE: SEE SHEET 1A FOR PLAN SHEET LAYOUT AT TIME OF INVESTIGATION

STATE OF NORTH CAROLINA
DEPARTMENT OF TRANSPORTATION
DIVISION OF HIGHWAYS
GEOTECHNICAL ENGINEERING UNIT

STATE	STATE PROJECT REFERENCE NO.	SHEET NO.	TOTAL SHEETS
N.C.	K-3807	1	89
STATE PROJ. NO.	F.A. PROJ. NO.	DESCRIPTION	
37331.2.1	IMS-73(12)	P.E., R/W	
37331.3.1	IMS-73(12)	CONSTR.	

CONTENTS

LINE	STATION	PLAN	PROFILE	XSECT
-L1-	10+00.00 to 39+00.00	4-6, 8	9-10	15-40
-L2-	10+21.17 to 20+04.02	5, 8	10	41-50
-L3-	10+00.00 to 59+50.00	5-8	11-12	51-88
-L4-	10+42.36 to 22+27.22	7	13	
-L5-	10+27.81 to 23+72.59	7	14	

SAMPLE SHEET (SHEET 89)

ROADWAY
SUBSURFACE INVESTIGATION

PROJ. REFERENCE NO. 37331.1.1 K-3807 F.A. PROJ. STP-1306(15)
COUNTY RANDOLPH
PROJECT DESCRIPTION I-73/74 AND U.S. 220 BYPASS NORTHBOUND AND SOUTHBOUND REST AREAS SOUTH OF SEAGROVE

INVENTORY

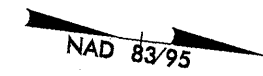
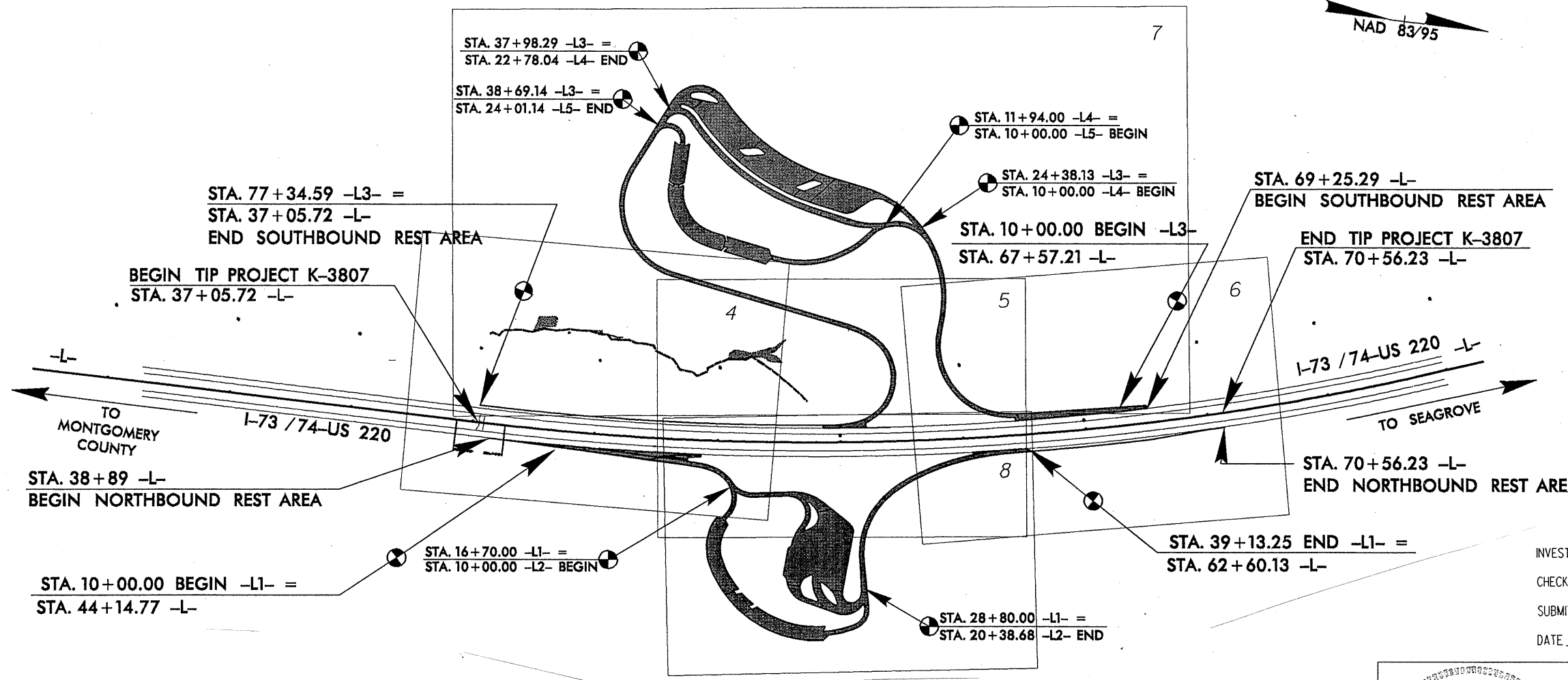
CAUTION NOTICE

THE SUBSURFACE INFORMATION AND THE SUBSURFACE INVESTIGATION ON WHICH IT IS BASED WERE MADE FOR THE PURPOSE OF STUDY, PLANNING, AND DESIGN, AND NOT FOR CONSTRUCTION OR PAY PURPOSES. THE VARIOUS FIELD BORING LOGS, ROCK CORES, AND SOIL TEST DATA AVAILABLE MAY BE REVIEWED OR INSPECTED IN RALEIGH BY CONTACTING THE N. C. DEPARTMENT OF TRANSPORTATION, GEOTECHNICAL ENGINEERING UNIT AT 1919 250-4088. NEITHER THE SUBSURFACE PLANS AND REPORTS, NOR THE FIELD BORING LOGS, ROCK CORES, OR SOIL TEST DATA ARE PART OF THE CONTRACT.

GENERAL SOIL AND ROCK STRATA DESCRIPTIONS AND INDICATED BOUNDARIES ARE BASED ON A GEOTECHNICAL INTERPRETATION OF ALL AVAILABLE SUBSURFACE DATA AND MAY NOT NECESSARILY REFLECT THE ACTUAL SUBSURFACE CONDITIONS BETWEEN BORINGS OR BETWEEN SAMPLED STRATA WITHIN THE BOREHOLE. THE LABORATORY SAMPLE DATA AND THE IN SITU (IN-PLACE) TEST DATA CAN BE RELIED ON ONLY TO THE DEGREE OF RELIABILITY INHERENT IN THE STANDARD TEST METHOD. THE OBSERVED WATER LEVELS OR SOIL MOISTURE CONDITIONS INDICATED IN THE SUBSURFACE INVESTIGATIONS ARE AS RECORDED AT THE TIME OF THE INVESTIGATION. THESE WATER LEVELS OR SOIL MOISTURE CONDITIONS MAY VARY CONSIDERABLY WITH TIME ACCORDING TO CLIMATIC CONDITIONS INCLUDING TEMPERATURES, PRECIPITATION, AND WIND, AS WELL AS OTHER NON-CLIMATIC FACTORS.

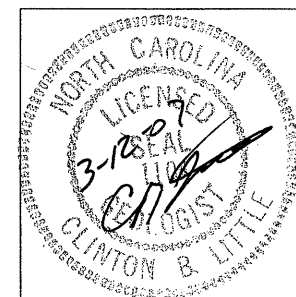
THE BIDDER OR CONTRACTOR IS CAUTIONED THAT DETAILS SHOWN ON THE SUBSURFACE PLANS ARE PRELIMINARY ONLY AND IN MANY CASES THE FINAL DESIGN DETAILS ARE DIFFERENT. FOR BIDDING AND CONSTRUCTION PURPOSES, REFER TO THE CONSTRUCTION PLANS AND DOCUMENTS FOR FINAL DESIGN INFORMATION ON THIS PROJECT. THE DEPARTMENT DOES NOT WARRANT OR GUARANTEE THE SUFFICIENCY OR ACCURACY OF THE INVESTIGATION MADE, NOR THE INTERPRETATIONS MADE, OR OPINION OF THE DEPARTMENT AS TO THE TYPE OF MATERIALS AND CONDITIONS TO BE ENCOUNTERED. THE BIDDER OR CONTRACTOR IS CAUTIONED TO MAKE SUCH INDEPENDENT SUBSURFACE INVESTIGATIONS AS HE DEEMS NECESSARY TO SATISFY HIMSELF AS TO CONDITIONS TO BE ENCOUNTERED ON THIS PROJECT. THE CONTRACTOR SHALL HAVE NO CLAIM FOR ADDITIONAL COMPENSATION OR FOR AN EXTENSION OF TIME FOR ANY REASON RESULTING FROM THE ACTUAL CONDITIONS ENCOUNTERED AT THE SITE DIFFERING FROM THOSE INDICATED IN THE SUBSURFACE INFORMATION.

CONTRACT: C201776 ID: K-3807



PERSONNEL
J.K. STICKNEY
C.L. SMITH
T.C. McCALL

INVESTIGATED BY J.E. BEVERLY
CHECKED BY C.B. LITTLE
SUBMITTED BY C.B. LITTLE
DATE _____



DRAWN BY: J.K. McCLURE

NOTE - THE INFORMATION CONTAINED HEREIN IS NOT IMPLIED OR GUARANTEED BY THE N. C. DEPARTMENT OF TRANSPORTATION AS BEING ACCURATE NOR IS IT CONSIDERED TO BE PART OF THE PLANS, SPECIFICATIONS, OR CONTRACT FOR THE PROJECT.

NOTE - BY HAVING REQUESTED THIS INFORMATION THE CONTRACTOR SPECIFICALLY WAIVES ANY CLAIMS FOR INCREASED COMPENSATION OR EXTENSION OF TIME BASED ON DIFFERENCES BETWEEN THE CONDITIONS INDICATED HEREIN AND THE ACTUAL CONDITIONS AT THE PROJECT SITE.

See Sheet 1-A For Index of Sheets
See Sheet 1-B For Conventional Symbols

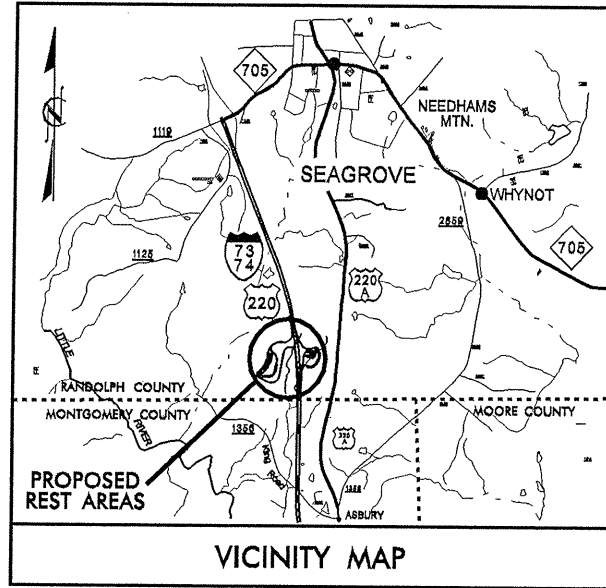
STATE OF NORTH CAROLINA
DIVISION OF HIGHWAYS

STATE	STATE PROJECT REFERENCE NO.	SHEET NO.	TOTAL SHEETS
N.C.	K-3807	1A	89
STATE PROJ. NO.	F.A. PROJ. NO.	DESCRIPTION	
34911.1.1	STP-1306(15)	P.E., RW	

RANDOLPH COUNTY

LOCATION: SOUTH OF SEAGROVE — I-73/74 AND U.S. 220 BYPASS NORTHBOUND AND SOUTHBOUND REST AREAS

TYPE OF WORK: GRADING, DRAINAGE, PAVING, TRAFFIC CONTROL AND SIGNING



STA. 37+98.29 -L3- =
STA. 22+78.04 -L4- END

STA. 38+69.14 -L3- =
STA. 24+01.14 -L5- END

STA. 11+94.00 -L4- =
STA. 10+00.00 -L5- BEGIN

STA. 24+38.13 -L3- =
STA. 10+00.00 -L4- BEGIN

STA. 10+00.00 -L3-
BEGIN SOUTHBOUND REST AREA
TIP PROJECT K-3807

STA. 60+18.53 -L3-
END SOUTHBOUND REST AREA
TIP PROJECT K-3807

STA. 10+00.00 -L1-
BEGIN NORTHBOUND REST AREA
TIP PROJECT K-3807

STA. 16+70.00 -L1- =
STA. 10+00.00 -L2- BEGIN

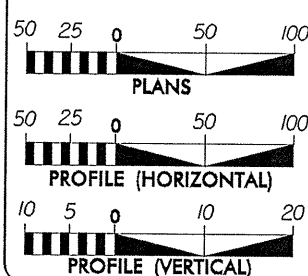
STA. 39+13.25 -L1-
END NORTHBOUND REST AREA
TIP PROJECT K-3807

STA. 28+80.00 -L1- =
STA. 20+38.68 -L2- END

CLEARING NOTE METHOD II.

INCOMPLETE PLANS
DO NOT USE FOR R/W ACQUISITION
PRELIMINARY PLANS
DO NOT USE FOR CONSTRUCTION

GRAPHIC SCALES



DESIGN DATA

ADT 2004 = n/a
ADT 2030 = 2,200
DHV = 11 %
D = 55 %
T = 35 % *
V = 30 MPH
* TTST 20% DUAL 15%

PROJECT LENGTH

LENGTH ROADWAY F.A. PROJECT STP-1306(8) = 4 MI
TOTAL LENGTH STATE PROJECT 34911JJ = 4 MI

Prepared In the Office of:
DIVISION OF HIGHWAYS
1000 Birch Ridge Dr., NC 27610

2006 STANDARD SPECIFICATIONS
RIGHT OF WAY DATE:
DECEMBER 16, 2005

LETTING DATE:
MAY 15, 2007

JIMMY GOODNIGHT
PROJECT ENGINEER

TIM GOINS
PROJECT DESIGN ENGINEER

HYDRAULICS ENGINEER

ROADWAY DESIGN
ENGINEER

APPROVED
DIVISION ADMINISTRATOR

DIVISION OF HIGHWAYS
STATE OF NORTH CAROLINA

DEPARTMENT OF TRANSPORTATION
FEDERAL HIGHWAY ADMINISTRATION

APPROVED
DIVISION ADMINISTRATOR

PROJECT: 37331.1.1 TIP PROJ: K-3807

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NORTH CAROLINA DEPARTMENT OF TRANSPORTATION
 DIVISION OF HIGHWAYS
 GEOTECHNICAL ENGINEERING UNIT

PROJECT REFERENCE NO.
 K-3807

SHEET NO
 2

SUBSURFACE INVESTIGATION

SOIL AND ROCK LEGEND, TERMS, SYMBOLS, AND ABBREVIATIONS

SOIL DESCRIPTION										GRADATION										ROCK DESCRIPTION										TERMS AND DEFINITIONS																																																																																																									
SOIL IS CONSIDERED TO BE THE UNCONSOLIDATED, SEMI-CONSOLIDATED, OR WEATHERED EARTH MATERIALS THAT CAN BE PENETRATED WITH A CONTINUOUS FLIGHT POWER AUGER, AND YIELD LESS THAN 100 BLOWS PER FOOT ACCORDING TO STANDARD PENETRATION TEST (AASHTO T206, ASTM D-1586). SOIL CLASSIFICATION IS BASED ON THE AASHTO SYSTEM. BASIC DESCRIPTIONS GENERALLY SHALL INCLUDE: CONSISTENCY, COLOR, TEXTURE, MOISTURE, AASHTO CLASSIFICATION, AND OTHER PERTINENT FACTORS SUCH AS MINERALOGICAL COMPOSITION, ANGULARITY, STRUCTURE, PLASTICITY, ETC. EXAMPLE: VERY STIFF, GRAY, SILTY CLAY, MOST WITH INTERBEDDED FINE SAND LAYERS, HIGHLY PLASTIC, A-7-6										WELL GRADED - INDICATES A GOOD REPRESENTATION OF PARTICLE SIZES FROM FINE TO COARSE. UNIFORM - INDICATES THAT SOIL PARTICLES ARE ALL APPROXIMATELY THE SAME SIZE. (ALSO POORLY GRADED) GAP-GRADED - INDICATES A MIXTURE OF UNIFORM PARTICLES OF TWO OR MORE SIZES. ANGULARITY OF GRAINS THE ANGULARITY OR ROUNDNESS OF SOIL GRAINS IS DESIGNATED BY THE TERMS: ANGULAR, SUBANGULAR, SUBROUNDED, OR ROUNDED.										HARD ROCK IS NON-COASTAL PLAIN MATERIAL THAT IF TESTED, WOULD YIELD SPT REFUSAL. AN INFERRED ROCK LINE INDICATES THE LEVEL AT WHICH NON-COASTAL PLAIN MATERIAL WOULD YIELD SPT REFUSAL. SPT REFUSAL IS PENETRATION BY A SPLIT SPOON SAMPLER EQUAL TO OR LESS THAN 0.1 FOOT PER 60 BLOWS, IN NON-COASTAL PLAIN MATERIAL. THE TRANSITION BETWEEN SOIL AND ROCK IS OFTEN REPRESENTED BY A ZONE OF WEATHERED ROCK. ROCK MATERIALS ARE TYPICALLY DIVIDED AS FOLLOWS: WEATHERED ROCK (WR) - NON-COASTAL PLAIN MATERIAL THAT WOULD YIELD SPT N VALUES > 100 BLOWS PER FOOT IF TESTED. CRYSTALLINE ROCK (CR) - FINE TO COARSE GRAIN IGNEOUS AND METAMORPHIC ROCK THAT WOULD YIELD SPT REFUSAL IF TESTED. ROCK TYPE INCLUDES GRANITE, GNEISS, GABBRO, SCHIST, ETC. NON-CRYSTALLINE ROCK (NCR) - FINE TO COARSE GRAIN METAMORPHIC AND NON-COASTAL PLAIN SEDIMENTARY ROCK THAT WOULD YIELD SPT REFUSAL IF TESTED. ROCK TYPE INCLUDES PHYLLITE, SLATE, SANDSTONE, ETC. COASTAL PLAIN SEDIMENTARY ROCK (CP) - COASTAL PLAIN SEDIMENTS CEMENTED INTO ROCK, BUT MAY NOT YIELD SPT REFUSAL. ROCK TYPE INCLUDES LIMESTONE, SANDSTONE, CEMENTED SHELL BEDS, ETC.										ALLUVIUM (ALLUV.) - SOILS THAT HAVE BEEN TRANSPORTED BY WATER. AQUIFER - A WATER BEARING FORMATION OR STRATA. ARENACEOUS - APPLIED TO ROCKS THAT HAVE BEEN DERIVED FROM SAND OR THAT CONTAIN SAND. ARGILLACEOUS - APPLIED TO ALL ROCKS OR SUBSTANCES COMPOSED OF CLAY MINERALS, OR HAVING A NOTABLE PROPORTION OF CLAY IN THEIR COMPOSITION, AS SHALE, SLATE, ETC. ARTESIAN - GROUND WATER THAT IS UNDER SUFFICIENT PRESSURE TO RISE ABOVE THE LEVEL AT WHICH IT IS ENCOUNTERED, BUT WHICH DOES NOT NECESSARILY RISE TO OR ABOVE THE GROUND SURFACE. CALCAREOUS (CALC.) - SOILS THAT CONTAIN APPRECIABLE AMOUNTS OF CALCIUM CARBONATE. COLLUVIUM - ROCK FRAGMENTS MIXED WITH SOIL DEPOSITED BY GRAVITY ON SLOPE OR AT BOTTOM OF SLOPE. CORE RECOVERY (REC) - TOTAL LENGTH OF ALL MATERIAL RECOVERED IN THE CORE BARREL DIVIDED BY TOTAL LENGTH OF CORE RUN AND EXPRESSED AS A PERCENTAGE. DIKE - A TABULAR BODY OF IGNEOUS ROCK THAT CUTS ACROSS THE STRUCTURE OF ADJACENT ROCKS OR CUTS MASSIVE ROCK. DIP - THE ANGLE AT WHICH A STRATUM OR ANY PLANAR FEATURE IS INCLINED FROM THE HORIZONTAL. DIP DIRECTION (DIP AZIMUTH) - THE DIRECTION OR BEARING OF THE HORIZONTAL TRACE OF THE LINE OF DIP, MEASURED CLOCKWISE FROM NORTH. FAULT - A FRACTURE OR FRACTURE ZONE ALONG WHICH THERE HAS BEEN DISPLACEMENT OF THE SIDES RELATIVE TO ONE ANOTHER PARALLEL TO THE FRACTURE. FISSILE - A PROPERTY OF SPLITTING ALONG CLOSELY SPACED PARALLEL PLANES. FLOAT - ROCK FRAGMENTS ON SURFACE NEAR THEIR ORIGINAL POSITION AND DISLOGGED FROM PARENT MATERIAL. FLOOD PLAIN (FP) - LAND BORDERING A STREAM, BUILT OF SEDIMENTS DEPOSITED BY THE STREAM. FORMATION (FM) - A MAPPABLE GEOLOGIC UNIT THAT CAN BE RECOGNIZED AND TRACED IN THE FIELD. JOINT - FRACTURE IN ROCK ALONG WHICH NO APPRECIABLE MOVEMENT HAS OCCURRED. LEDGE - A SHELF-LIKE RIDGE OR PROJECTION OF ROCK WHOSE THICKNESS IS SMALL COMPARED TO ITS LATERAL EXTENT. LENS - A BODY OF SOIL OR ROCK THAT THINS OUT IN ONE OR MORE DIRECTIONS. MOTTLED (MOT.) - IRREGULARLY MARKED WITH SPOTS OF DIFFERENT COLORS, MOTTLING IN SOILS USUALLY INDICATES POOR AERATION AND LACK OF GOOD DRAINAGE. PERCHED WATER - WATER MAINTAINED ABOVE THE NORMAL GROUND WATER LEVEL BY THE PRESENCE OF AN INTERVENING IMPERVIOUS STRATUM. RESIDUAL (RES.) SOIL - SOIL FORMED IN PLACE BY THE WEATHERING OF ROCK. ROCK QUALITY DESIGNATION (RQD) - A MEASURE OF ROCK QUALITY DESCRIBED BY TOTAL LENGTH OF ROCK SEGMENTS EQUAL TO OR GREATER THAN 4 INCHES DIVIDED BY THE TOTAL LENGTH OF CORE RUN & EXPRESSED AS A PERCENTAGE. SAPROLITE (SAP) - RESIDUAL SOIL THAT RETAINS THE RELIC STRUCTURE OR FABRIC OF THE PARENT ROCK. SILL - AN INTRUSIVE BODY OF IGNEOUS ROCK OF APPROXIMATELY UNIFORM THICKNESS AND RELATIVELY THIN COMPARED WITH ITS LATERAL EXTENT, THAT HAS BEEN EMPLACED PARALLEL TO THE BEDDING OR SCHISTOSITY OF THE INTRUDED ROCKS. SLICKENSIDE - POLISHED AND STRIATED SURFACE THAT RESULTS FROM FRICTION ALONG A FAULT OR SLIP PLANE. STANDARD PENETRATION TEST (PENETRATION RESISTANCE) (SPT) - NUMBER OF BLOWS (N OR BPF) OF A 140 LB. HAMMER FALLING 30 INCHES REQUIRED TO PRODUCE A PENETRATION OF 1 FOOT INTO SOIL WITH A 2 INCH OUTSIDE DIAMETER SPLIT SPOON SAMPLER. SPT REFUSAL IS PENETRATION EQUAL TO OR LESS THAN 0.1 FOOT PER 60 BLOWS. STRATA CORE RECOVERY (SCREC) - TOTAL LENGTH OF STRATA MATERIAL RECOVERED DIVIDED BY TOTAL LENGTH OF STRATUM AND EXPRESSED AS A PERCENTAGE. STRATA ROCK QUALITY DESIGNATION (SRQD) - A MEASURE OF ROCK QUALITY DESCRIBED BY TOTAL LENGTH OF ROCK SEGMENTS WITHIN A STRATUM EQUAL TO OR GREATER THAN 4 INCHES DIVIDED BY TOTAL LENGTH OF STRATA AND EXPRESSED AS A PERCENTAGE. TOPSOIL (TS) - SURFACE SOILS USUALLY CONTAINING ORGANIC MATTER.																																																																																																									
SOIL LEGEND AND AASHTO CLASSIFICATION										MINERALOGICAL COMPOSITION										WEATHERING																																																																																																																			
<table border="1"> <tr> <th>GENERAL CLASS.</th> <th colspan="5">GRANULAR MATERIALS (<= 35% PASSING #200)</th> <th colspan="5">SILT-CLAY MATERIALS (> 35% PASSING #200)</th> <th colspan="5">ORGANIC MATERIALS</th> </tr> <tr> <th>GROUP CLASS.</th> <th>A-1</th> <th>A-1-b</th> <th>A-3</th> <th>A-2-4</th> <th>A-2-5</th> <th>A-2-6</th> <th>A-2-7</th> <th>A-4</th> <th>A-5</th> <th>A-6</th> <th>A-7</th> <th>A-1, A-2</th> <th>A-3</th> <th>A-4, A-5</th> <th>A-6, A-7</th> </tr> <tr> <th>SYMBOL</th> <td></td> <td></td> <td></td> <td></td> <td></td> <td></td> <td></td> <td></td> <td></td> <td></td> <td></td> <td></td> <td></td> <td></td> <td></td> </tr> <tr> <th>% PASSING</th> <td colspan="5">50 MX</td> <td colspan="5">40 MX</td> <td colspan="5">30 MX</td> </tr> <tr> <td>40</td> <td colspan="5">15 MX</td> <td colspan="5">10 MX</td> <td colspan="5">5 MX</td> </tr> <tr> <td>200</td> <td colspan="5">15 MX</td> <td colspan="5">10 MX</td> <td colspan="5">5 MX</td> </tr> </table>										GENERAL CLASS.	GRANULAR MATERIALS (<= 35% PASSING #200)					SILT-CLAY MATERIALS (> 35% PASSING #200)					ORGANIC MATERIALS					GROUP CLASS.	A-1	A-1-b	A-3	A-2-4	A-2-5	A-2-6	A-2-7	A-4	A-5	A-6	A-7	A-1, A-2	A-3	A-4, A-5	A-6, A-7	SYMBOL																% PASSING	50 MX					40 MX					30 MX					40	15 MX					10 MX					5 MX					200	15 MX					10 MX					5 MX					MINERAL NAMES SUCH AS QUARTZ, FELDSPAR, MICA, TALC, KAOLIN, ETC. ARE USED IN DESCRIPTIONS WHENEVER THEY ARE CONSIDERED OF SIGNIFICANCE. COMPRESSIBILITY LIQUID LIMIT LESS THAN 31 LIQUID LIMIT EQUAL TO 31-50 LIQUID LIMIT GREATER THAN 50 PERCENTAGE OF MATERIAL ORGANIC MATERIAL GRANULAR SOILS SILT-CLAY SOILS OTHER MATERIAL TRACE OF ORGANIC MATTER 2 - 3% 3 - 5% TRACE 1 - 10% LITTLE ORGANIC MATTER 3 - 5% 5 - 12% LITTLE 10 - 20% MODERATELY ORGANIC 5 - 10% 12 - 20% SOME 20 - 35% HIGHLY ORGANIC >10% >20% HIGHLY 35% AND ABOVE										FRESH ROCK FRESH, CRYSTALS BRIGHT, FEW JOINTS MAY SHOW SLIGHT STAINING, ROCK RINGS UNDER HAMMER IF CRYSTALLINE. VERY SLIGHT ROCK GENERALLY FRESH, JOINTS STAINED, SOME JOINTS MAY SHOW THIN CLAY COATINGS IF OPEN, CRYSTALS ON A BROKEN SPECIMEN FACE SHINE BRIGHTLY, ROCK RINGS UNDER HAMMER BLOWS IF OF A CRYSTALLINE NATURE. SLIGHT ROCK GENERALLY FRESH, JOINTS STAINED AND DISCOLORATION EXTENDS INTO ROCK UP TO 1 INCH, OPEN JOINTS MAY CONTAIN CLAY. IN GRANITOID ROCKS SOME OCCASIONAL FELDSPAR CRYSTALS ARE DULL AND DISCOLORED, CRYSTALLINE ROCKS RING UNDER HAMMER BLOWS. MODERATE SIGNIFICANT PORTIONS OF ROCK SHOW DISCOLORATION AND WEATHERING EFFECTS. IN GRANITOID ROCKS, MOST FELDSPARS ARE DULL AND DISCOLORED, SOME SHOW CLAY. ROCK HAS DULL SOUND UNDER HAMMER BLOWS AND SHOWS SIGNIFICANT LOSS OF STRENGTH AS COMPARED WITH FRESH ROCK. MODERATELY SEVERE ALL ROCK EXCEPT QUARTZ DISCOLORED OR STAINED. IN GRANITOID ROCKS, ALL FELDSPARS DULL AND DISCOLORED AND A MAJORITY SHOW KAOLINIZATION, ROCK SHOWS SEVERE LOSS OF STRENGTH AND CAN BE EXCAVATED WITH A GEOLOGIST'S PICK. ROCK GIVES "CLUNK" SOUND WHEN STRUCK. IF TESTED, WOULD YIELD SPT REFUSAL. SEVERE ALL ROCK EXCEPT QUARTZ DISCOLORED OR STAINED, ROCK FABRIC CLEAR AND EVIDENT BUT REDUCED IN STRENGTH TO STRONG SOIL. IN GRANITOID ROCKS ALL FELDSPARS ARE KAOLINIZED TO SOME EXTENT, SOME FRAGMENTS OF STRONG ROCK USUALLY REMAIN. IF TESTED, YIELDS SPT N VALUES > 100 BPF. VERY SEVERE ALL ROCK EXCEPT QUARTZ DISCOLORED OR STAINED, ROCK FABRIC ELEMENTS ARE DISCERNIBLE BUT THE MASS IS EFFECTIVELY REDUCED TO SOIL STATUS, WITH ONLY FRAGMENTS OF STRONG ROCK REMAINING. SAPROLITE IS AN EXAMPLE OF ROCK WEATHERED TO A DEGREE SUCH THAT ONLY MINOR VESTIGES OF THE ORIGINAL ROCK FABRIC REMAIN. IF TESTED, YIELDS SPT N VALUES < 100 BPF. COMPLETE ROCK REDUCED TO SOIL, ROCK FABRIC NOT DISCERNIBLE, OR DISCERNIBLE ONLY IN SMALL AND SCATTERED CONCENTRATIONS. QUARTZ MAY BE PRESENT AS DIKES OR STRINGERS, SAPROLITE IS ALSO AN EXAMPLE.																			
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STATE OF NORTH CAROLINA
DEPARTMENT OF TRANSPORTATION

MICHAEL F. EASLEY
GOVERNOR

P.O. BOX 25201, RALEIGH, N.C. 27611-5201

LYNDO TIPPETT
SECRETARY

February 16, 2007

STATE PROJECT: 37331.1.1 (K-3807)
F.A. PROJECT: STP - 1306(15)
COUNTY: Randolph
DESCRIPTION: I-73/74 and US 220 Bypass Northbound and Southbound Rest Areas South of Seagrove

SUBJECT: Geotechnical Report - Inventory

This report presents the findings of the Geotechnical Investigation for the proposed northbound and southbound rest areas along I-73/74 and US 220 bypass. The location of the rest areas is just north of the Montgomery County line and just south of the city of Seagrove. The site of the rest area is heavily wooded and is higher in elevation than much of the surrounding area. The northbound rest area is defined by alignments -L1- and -L2- where as the southbound rest area is defined by alignments -L3-, -L4-, and -L5-.

The geotechnical field investigation for this project was conducted between the months of May and October of 2006. An ATV mounted CME 550X drill machine with automatic drop hammer was utilized to perform test borings along the proposed corridor. The following survey lines, building locations, and hazardous spill basins are addressed in this inventory report:

<u>Line</u>	<u>Station</u>
-L1-	10+00 - 39+13.25
-L2-	10+00 - 20+38.68
-L3-	10+00 - 60+18.53
-L4-	10+00 - 22+78.04
-L5-	10+00 - 24+01.14
NBL Rest Area Buildings	Left of -L1- 22+00 - 22+50
SBL Rest Area Buildings	Right of -L5- 19+90 - 20+20
NBL Hazardous Spill Basin #1	-L2- 12+35, 97' RT
NBL Hazardous Spill Basin #2	-L2- 14+30, 99' RT
SBL Hazardous Spill Basin #1	-L3- 52+41, 33' LT
SBL Hazardous Spill Basin #2	-L3- 54+57, 92' LT

Areas of Special Geotechnical Interest:

1. *Groundwater:*

Groundwater was encountered sporadically throughout the course of this investigation. The majority of holes drilled were dry after 24 hours. Where groundwater was encountered it was generally deep within residual soils. Following are known instances where groundwater was found at or above proposed grade.

<u>Alignment</u>	<u>Station Range</u>
-L1-	28+00 - 32+00
-L2-	13+25 - 14+25
-L2-	15+25 - 15+75
-L3-	21+00 - 22+50

2. *Crystalline Rock:*

During the course of this investigation there were many instances where crystalline rock was encountered. Rock locations were generally defined by auger refusal. Elevations and depths where rock occurs are quite erratic across the site. Rock outcrops were noted sporadically across the site. The erratic rock line depicted on profiles and cross sections is likely due to the uneven weathering of the parent rock types and/or the presence of large boulders. Following is a list of areas where rock is known to occur at or above proposed grade.

<u>Alignment</u>	<u>Station Range</u>
-L1-	29+50 - 30+25
-L1-	30+75 - 31+25
-L1-	32+25 - 39+00
-L2-	14+25 - 16+60
-L3-	16+00 - 21+50

3. *High PI Soils:* (PI's 26 or greater)

Several instances of high PI clays were noted during this investigation. Most samples returning high PI values were obtained in the upper 6 feet of residual soils. PI values for clay soils range from 11 to 59. Known occurrences of high PI clays are as follows:

<u>Station Location</u>	<u>Appx. Depth Interval (feet)</u>	<u>High PI Value/Range (26+)</u>
-L1- 21+00 - 24+00	0 - 6	32 - 39
-L2- 10+00 - 13+00	5 - 18	37
-L2- 17+00 - 18+00	18 - 20	27
-L3- 18+10 - 21+00	0 - 6	29
-L3- 31+00 - 36+00	0 - 6	30
-L3- 41+00 - 59+50	0 - 6	26 - 59
-L4- 16+50 - 19+00	12 - 16 & 28 - 31	26

4. Alluvial Soils / Wet Areas:

There were few areas containing alluvial soils along the project corridor. No permanent creeks or streams bisect the project. There are several wet weather drainage features incised into the slopes at both NBL and SBL rest area locations. None of the drainage features or alluvial areas should be of special concern.

5. Artificial Fill Soils:

A large area of artificial fill exists within the NBL rest area. The fill is the result of an abandoned borrow pit that was dumped in for years by local residents. According to locals, the pit was originally dug as a borrow source during the construction of US 220. Afterward, the property owner opened up the land to the local population as a dumpsite and the hole was subsequently filled back in with a mixture of soil and debris. Nobody knows the exact contents of the fill however our test borings showed wood, nails, roofing tiles, tin, plastic, etc. The fill encompasses an area of approximately 16,000 square yards essentially in-between alignments -L1- and -L2-. Fill thickness varies but is known to be close to 30 feet deep in places. Proposed construction features impacted by the fill are the entrance ramps and parking areas along -L2- stations 15+00 – 18+75, and -L1- stations 17+60 – 19+50. Picnic and recreation areas as well as the rest area buildings will also fall within the fill area. An estimate of the fill extent is delineated on plan view sheet #8.

Physiography/Geology:

The project area is located in southern Randolph County just north of the Montgomery County border. The city of Seagrove lies just to the north. The proposed sites are along the top of a local hill range resulting in steeply sloping topography in much of the area. Steep side slopes were cut into the hills during construction for 4 lane US 220. Approximate elevation range is 650 – 770 feet across the site.

Geologically this site is part of the Carolina Slate Belt and is underlain by Cenozoic age felsic metavolcanic rock types.

Soil Properties:

1. Residual Soils:

These soils are derived from in place weathering of parent materials. They occur in a variety of consistencies, classifications, and stratigraphic sequences. For this project residual soils are further subdivided into clays, and silts. A classification for sand was not returned in any of our sample data.

Clays were the predominant soil type. They consist primarily of red-tan-brown-yellow medium stiff to stiff sandy silty clay in the AASHTO classifications of A-7-5, A-7-6, and A-6. In most instances clay soils are found throughout the stratigraphic column and appear to be well drained. Based on sample data the plasticity index ranges from 11 to 59 with a corresponding liquid limit range between 34 and 90.

Silts consist primarily of white-tan-yellow medium stiff to stiff clayey sandy silt in the AASHTO classifications of A-4 and A-5. Silts were also found throughout the stratigraphic column and were also well drained.

2. Alluvial Soils:

Only wet weather drainage features are present on this project. Therefore only 2 instances of alluvium were noted during this investigation. A boring in a small drainage feature between -L3- stations 56+80 and 57+55 encountered 6 feet of gray medium stiff and highly plastic (PI=59) silty clay (A-7-6). The second area lies at the bottom of the artificial fill in a boring located at -L2- station 16+60. Alluvium was encountered at a depth of 28 feet and consists of tan-brown stiff highly plastic (PI=54) silty clay (A-7-5) with root mat. Alluvium at this location is believed to be the result of a buried streambed.

3. Fill Soils:

A large area of artificial fill was encountered within the proposed limits of the NBL rest area. This area is described in more detail above. The fill is predominantly soil intermixed with various types of debris such as wood, various metal objects, plastic, roofing tiles, rock fragments and boulders. Soil type was determined to be a very soft to very stiff sandy clayey silt (A-4, A-5) and / or medium stiff to very stiff sandy silty clay (A-7-6). Fill depth is close to 30 feet in some instances.

Rock Properties:

The presence of rock was determined primarily by power auger refusal for this investigation. There were many locations where borings encountered rock at both shallow and deep intervals. The type of rock underlying this region is a felsic metavolcanic variety.

Hazardous Spill Basins:

Four proposed hazardous spill basins were investigated at the request of the NCDOT Hydraulics Unit. Soil at all 4 locations is residual in nature and comprised of stiff sandy silty clay (A-7-5) or stiff sandy silt (A-4). Rock was not encountered at any of these locations. Following is boring data at each of the proposed basins:

<u>Location</u>	<u>Basin Target Elevation</u>	<u>Boring Elevation Achieved</u>	<u>Termination Notes</u>
NBL:			
-L2- 12+35, 97' RT	690'	687.5'	stiff clay (A-7-5)
-L2- 14+30, 99' LT	698'	691.5	stiff silt (A-4)
SBL:			
-L3- 52+41, 33' LT	698'	698.6'	stiff clay (A-7-5)
-L3- 54+57, 92' LT	692'	692.9'	stiff clay (A-7-5)

Rest Area Buildings (NBL):

A total of 4 borings were performed for the NBL building structures. The approximate center point for each of the 2 buildings is -L1- station 22+05, 118' LT and -L1- station 22+72, 115' LT. The 2 borings

on the eastern side of the buildings (-L1- 22+00, 100' LT, & 22+50, 105' LT) encountered residual red-tan stiff highly plastic sandy silty clay (A-7-5) and stiff clayey sandy silt (A-4). The 2 borings on the west end of the structures (-L1- 22+00, 135' LT, & 22+50, 140' LT) encountered 1 to 8 feet of artificial fill comprised of gray stiff clayey sandy silt (A-4) with rock fragments. Each of the latter 2 borings terminated by auger refusal on boulders. Termination elevations for each of the borings are as follows:

<u>-L1- Sta. Location</u>	<u>Termination Elevation (feet)</u>	<u>Notes</u>
22+00, 100' LT	699.4	Terminated in residual silt (A-4)
22+00, 135' LT	707.4	Auger refusal on boulder (fill)
22+50, 105' LT	700.2	Terminated in residual clay (A-7-5)
22+50, 140' LT	715.9	Auger refusal on boulder (fill)

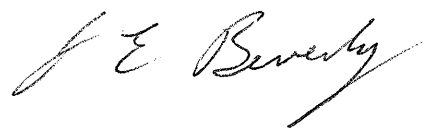
Rest Area Buildings (SBL):

Four borings were performed for the SBL building structures. The approximate center point for each of the 2 buildings is -L5- station 20+20, 118' RT and 19+90, 175' RT. Soil types encountered are residual red-tan-brown stiff sandy silty clay (A-7-5) and tan-brown-gray very stiff clayey sandy silt (A-4). Surficial boulders were noted in and around the rest area site. In one instance a hole was relocated due to an encounter with a boulder just below the ground surface. Boring locations and their termination depths are as follows:

<u>-L5- Sta. Location</u>	<u>Termination Elevation (feet)</u>	<u>Notes</u>
19+12, 160' RT	749.4	Terminated in residual silt (A-4)
19+50, 190' RT	768	Auger refusal on boulder
20+25, 115' RT	753.4	Terminated in residual clay (A-7-5)
20+80, 115' RT	754.2	Terminated in residual clay (A-7-5)

Respectfully Submitted,

J.E. Beverly, Project Geo-Engineer



EARTHWORK BALANCE SHEET

Volumes in Cubic Yards

PROJECT K-3807

COUNTY Randolph

DATE 17-Dec-07

COMPILED BY: TDG

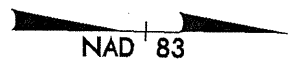
SHEET 3 OF 89 SHEETS

STATION	STATION	EXCAVATION					EMBANKMENT				BORROW	WASTE					
		TOTAL UNCLASS.	ROCK	UNDERCUT	UNSUIT. UNCLASS.	SUITABLE UNCLASS.	TOTAL	ROCK	EARTH	EMBANK. (+) 20 %		ROCK	SUITABLE	UNSUIT.	TOTAL		
L 37+50	57+50	2125				2125	5477		5477	6572	4447						
L1 10+00	30+00	35680	156	466	236	35288	15998	156	15842	19166			16278	702		16980	
L1 30+50	43+50	29249	9897			19352	10262	9897	365	10335			18914			18914	
L2 11+00	19+41.22	59908	4306	4136	11739	43863	4310	4306	4	4311			43858	15875		59733	
	SUBTOTAL	126962	14359	4602	11975	100628	36047	14359	21688	40384	4447		79050	16577		95627	
L3 10+00	30+00	35323				35323	14089		14089	16907			18416			18416	
L3 30+50	59+48.30	82334				82334	165574		165574	198689	116355						
	SUBTOTAL	117657				117657	179663		179663	215596	116355		18416			18416	
	TOTAL	244619	14359	4602	11975	218285	215710	14359	201351	255980	120802		97466	16577		114043	
LOSS DUE	TO C&G	-64000				-64000											
ADDITIONAL	UNDERCUT			13033			13033		13033	15640	64000						
EARTH WAS	TO REP. BOR										15640			13033		13033	
SHLDR.	MATERIAL						2169		2169	2603	2603		-22863		-22863		-22863
PROJECT	TOTALS	180619	14359	17635	11975	154285	230912	14359	216553	274223	180182		74603	29610		104213	
EST. FOR	REPLACE										9009						
TOPSOIL IN	BORROW																
PIT																	
GRAND	TOTALS	180619									189191						
SAY		181200									190000						

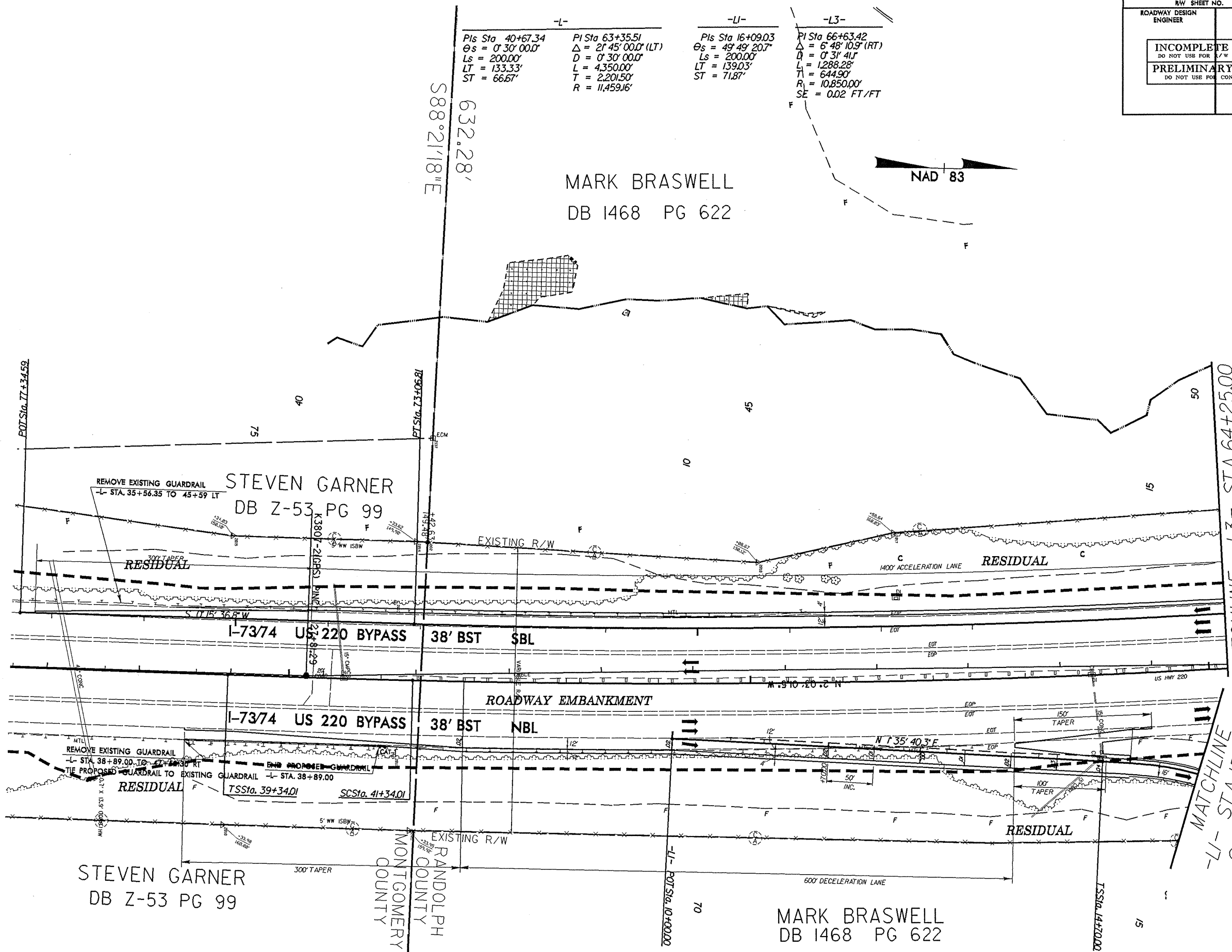
NOTE: EARTHWORK QUANTITIES ARE CALCULATED BY THE ROADWAY DESIGN UNIT. THESE EARTHWORK QUANTITIES ARE BASED IN PART ON SUBSURFACE DATA PROVIDED BY THE GEOTECHNICAL ENGINEERING UNIT.

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K-3807	4
R/W SHEET NO.	
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-L-	-L-	-L-	-L3-
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$\theta_s = 0^\circ 30' 00.0"$	$\Delta = 2^\circ 45' 00.0" (LT)$	$\theta_s = 49^\circ 49' 20.7"$	$\Delta = 6^\circ 48' 10.9" (RT)$
$L_s = 200.00'$	$D = 0^\circ 30' 00.0"$	$L_s = 200.00'$	$D = 0^\circ 31' 41.1"$
$LT = 133.33'$	$L = 4,350.00'$	$LT = 139.03'$	$L = 1,288.28'$
$ST = 66.67'$	$T = 2,201.50'$	$ST = 71.87'$	$T = 644.90'$
	$R = 11,459.16'$		$R_1 = 10,850.00'$
			$SE = 0.02 \text{ FT/FT}$



MARK BRASWELL
DB 1468 PG 622



STEVEN GARNER
DB Z-53 PG 99

MARK BRASWELL
DB 1468 PG 622

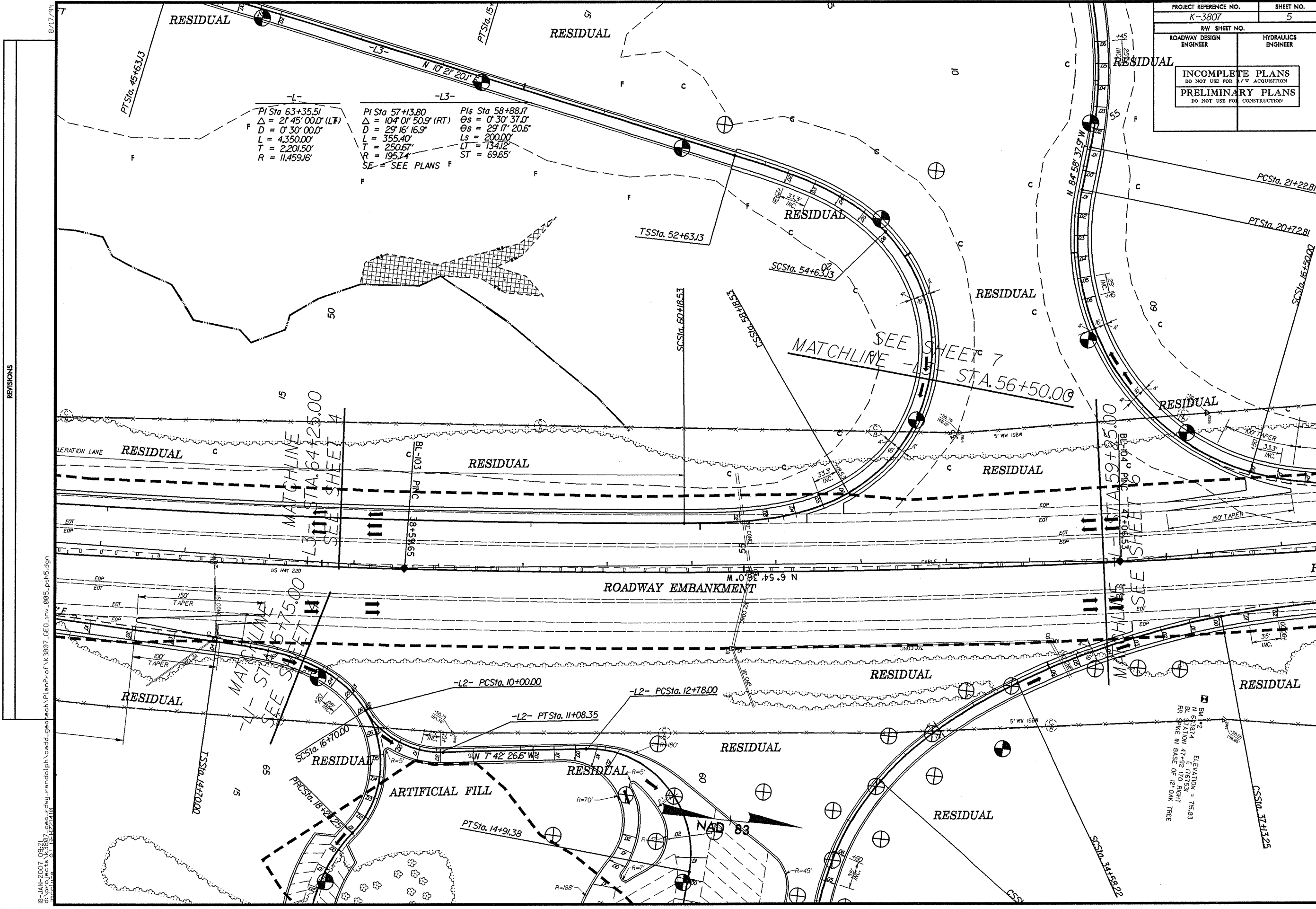
MATCHLINE -L3- STA.64+25.00
SEE SHEET 7

MATCHLINE -L1- STA.15+75.00
SEE SHEET 5

REVISIONS

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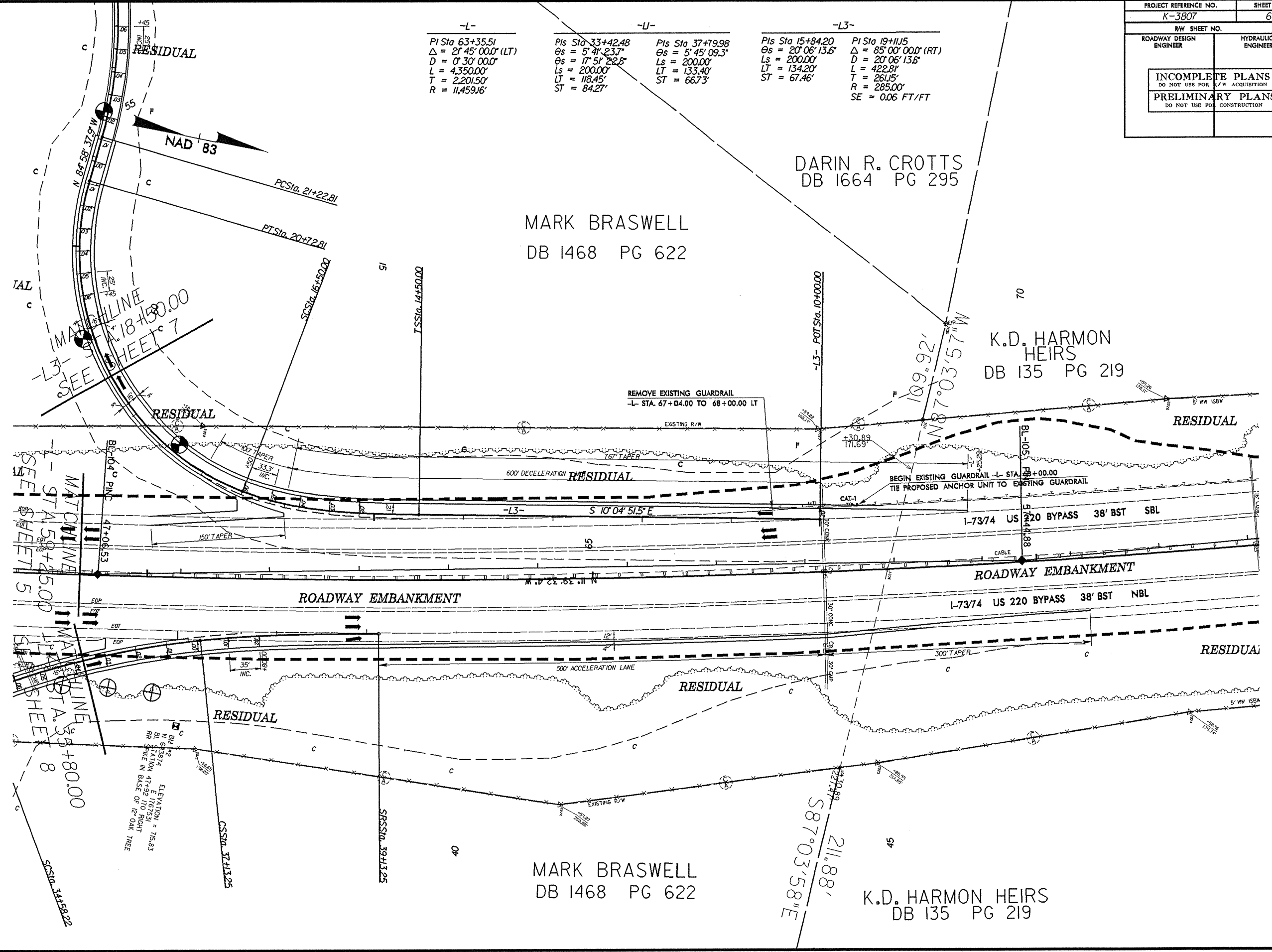


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PROJECT REFERENCE NO. K-3807		SHEET NO. 6	
RW SHEET NO.			
ROADWAY DESIGN ENGINEER	HYDRAULICS ENGINEER		
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-L-	-U-	-L3-
PI Sta 63+35.51 $\Delta = 2^\circ 45' 00.0"$ (LT) D = 0' 30' 00.0" L = 4,350.00' T = 2,201.50' R = 11,459.16'	PI Sta 33+42.48 $\theta_s = 5^\circ 41' 23.7"$ $\theta_s = 17^\circ 51' 22.8"$ Ls = 200.00' LT = 118.45' ST = 84.27'	PI Sta 37+79.98 $\theta_s = 5^\circ 45' 09.3"$ Ls = 200.00' LT = 133.40' ST = 66.73'
		PI Sta 15+84.20 $\theta_s = 20^\circ 06' 13.6"$ Ls = 200.00' LT = 134.20' ST = 67.46'
		PI Sta 19+11.15 $\Delta = 85^\circ 00' 00.0"$ (RT) D = 20' 06' 13.6" L = 422.81' T = 261.15' R = 285.00' SE = 0.06 FT/FT



DARIN R. CROTTS
DB 1664 PG 295

MARK BRASWELL
DB 1468 PG 622

K.D. HARMON HEIRS
DB 135 PG 219

MARK BRASWELL
DB 1468 PG 622

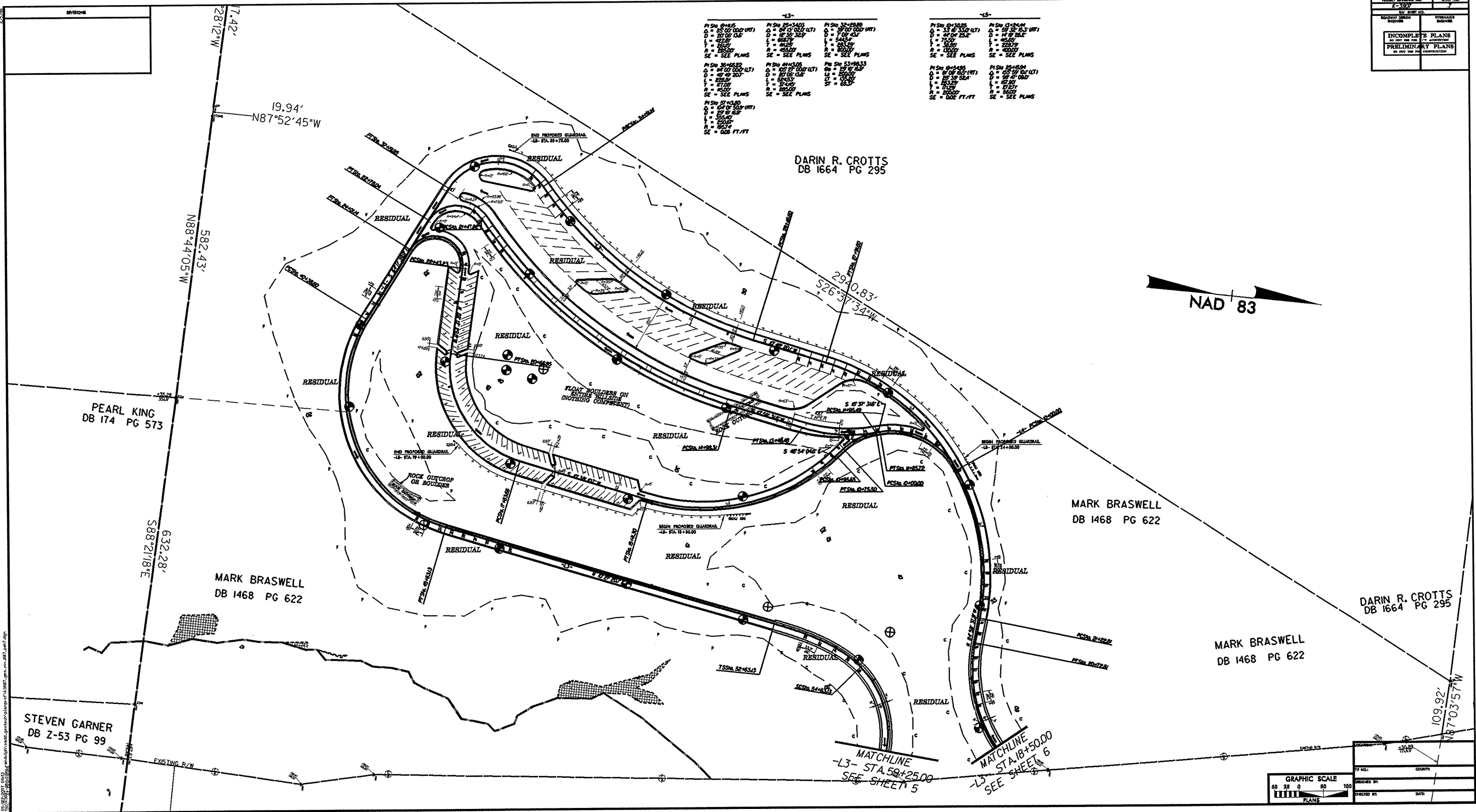
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PROJECT REFERENCE NO.	DB 1664
SHEET NO.	7
DATE	11/11/03
DESIGNED BY	DAVID L. HARRIS
CHECKED BY	DAVID L. HARRIS
INCOMPLETE PLANS	NO
PRELIMINARY PLANS	NO

-L3-			-L5-		
PI Stn 0+000.00	PI Stn 0+340.00	PI Stn 0+680.00	PI Stn 0+340.00	PI Stn 0+680.00	PI Stn 1+020.00
Δ = 90°00'00" (RT)	Δ = 87°00'00" (LT)	Δ = 90°00'00" (RT)	Δ = 33°16'33" (RT)	Δ = 33°16'33" (RT)	Δ = 33°16'33" (RT)
D = 200.00'	D = 200.00'	D = 200.00'	D = 47'00" (RT)	D = 47'00" (RT)	D = 47'00" (RT)
L = 220.00'	L = 220.00'	L = 220.00'	L = 75.00'	L = 75.00'	L = 75.00'
T = 141.42'	T = 141.42'	T = 141.42'	T = 38.57'	T = 38.57'	T = 38.57'
A = 100.00'	A = 100.00'	A = 100.00'	A = 100.00'	A = 100.00'	A = 100.00'
SE = SEE PLANS	SE = SEE PLANS	SE = SEE PLANS	SE = SEE PLANS	SE = SEE PLANS	SE = SEE PLANS
PI Stn 0+680.00	PI Stn 1+020.00	PI Stn 1+360.00	PI Stn 1+020.00	PI Stn 1+360.00	PI Stn 1+700.00
Δ = 90°00'00" (RT)	Δ = 90°00'00" (RT)	Δ = 90°00'00" (RT)	Δ = 33°16'33" (RT)	Δ = 33°16'33" (RT)	Δ = 33°16'33" (RT)
D = 200.00'	D = 200.00'	D = 200.00'	D = 47'00" (RT)	D = 47'00" (RT)	D = 47'00" (RT)
L = 220.00'	L = 220.00'	L = 220.00'	L = 75.00'	L = 75.00'	L = 75.00'
T = 141.42'	T = 141.42'	T = 141.42'	T = 38.57'	T = 38.57'	T = 38.57'
A = 100.00'	A = 100.00'	A = 100.00'	A = 100.00'	A = 100.00'	A = 100.00'
SE = SEE PLANS	SE = SEE PLANS	SE = SEE PLANS	SE = SEE PLANS	SE = SEE PLANS	SE = SEE PLANS
PI Stn 1+360.00	PI Stn 1+700.00	PI Stn 2+040.00	PI Stn 1+700.00	PI Stn 2+040.00	PI Stn 2+380.00
Δ = 90°00'00" (RT)	Δ = 90°00'00" (RT)	Δ = 90°00'00" (RT)	Δ = 33°16'33" (RT)	Δ = 33°16'33" (RT)	Δ = 33°16'33" (RT)
D = 200.00'	D = 200.00'	D = 200.00'	D = 47'00" (RT)	D = 47'00" (RT)	D = 47'00" (RT)
L = 220.00'	L = 220.00'	L = 220.00'	L = 75.00'	L = 75.00'	L = 75.00'
T = 141.42'	T = 141.42'	T = 141.42'	T = 38.57'	T = 38.57'	T = 38.57'
A = 100.00'	A = 100.00'	A = 100.00'	A = 100.00'	A = 100.00'	A = 100.00'
SE = SEE PLANS	SE = SEE PLANS	SE = SEE PLANS	SE = SEE PLANS	SE = SEE PLANS	SE = SEE PLANS



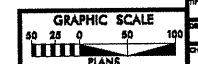
DARIN R. CROTTIS
DB 1664 PG 295

MARK BRASWELL
DB 1468 PG 622

DARIN R. CROTTIS
DB 1664 PG 295

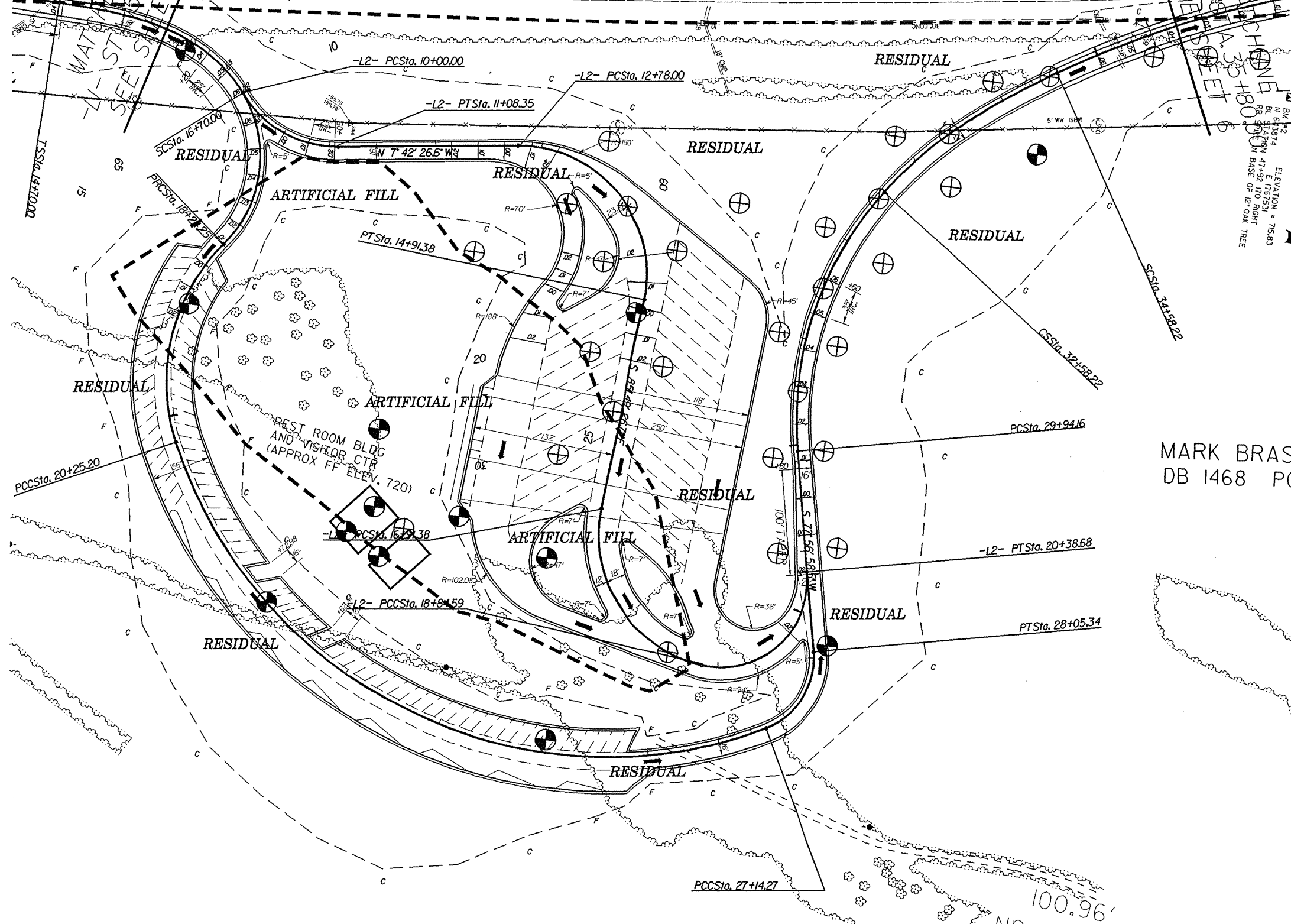
MARK BRASWELL
DB 1468 PG 622

STEVEN GARNER
DB Z-53 PG 99



PROJECT REFERENCE NO. K-3807	SHEET NO. 8
RW SHEET NO.	
ROADWAY DESIGN ENGINEER	HYDRAULICS ENGINEER
INCOMPLETE PLANS DO NOT USE FOR ACQUISITION	
PRELIMINARY PLANS DO NOT USE FOR CONSTRUCTION	

PI Sta 16+09.03 $\Delta = 49^{\circ} 20' 20.7''$ $D = 200.00'$ $L = 139.03'$ $T = 71.87'$ $R = 115.00'$ SE = SEE PLANS	PI Sta 17+61.27 $\Delta = 76^{\circ} 51' 09.5''$ (RT) $D = 49^{\circ} 49' 20.7''$ $L = 154.25'$ $T = 91.23'$ $R = 115.00'$ SE = SEE PLANS	PI Sta 19+36.65 $\Delta = 63^{\circ} 51' 42.8''$ (LT) $D = 31^{\circ} 49' 51.6''$ $L = 200.94'$ $T = 112.39'$ $R = 180.00'$ SE = SEE PLANS	PI Sta 24+69.05 $\Delta = 1^{\circ} 48' 58.6''$ (LT) $D = 13^{\circ} 19' 28.6''$ $L = 689.07'$ $T = 443.85'$ $R = 430.00'$ SE = SEE PLANS	PI Sta 27+67.54 $\Delta = 74^{\circ} 32' 10.2''$ (LT) $D = 8^{\circ} 51' 04.0''$ $L = 91.07'$ $T = 53.27'$ $R = 70.00'$ SE = 0.02 FT/FT	PI Sta 10+59.56 $\Delta = 59^{\circ} 07' 27.7''$ (LT) $D = 54^{\circ} 34' 02.7''$ $L = 108.35'$ $T = 59.56'$ $R = 105.00'$ SE = SEE PLANS	PI Sta 14+25.88 $\Delta = 107^{\circ} 52' 59.9''$ (RT) $D = 47^{\circ} 44' 47.3''$ $L = 213.38'$ $T = 147.88'$ $R = 120.00'$ SE = SEE PLANS
PI Sta 31+34.12 $\Delta = 46^{\circ} 59' 05.3''$ (RT) $D = 17^{\circ} 47' 37.4''$ $L = 264.05'$ $T = 139.96'$ $R = 322.00'$ SE = SEE PLANS	PI Sta 33+42.48 $\Delta = 5^{\circ} 41' 23.7''$ $D = 17^{\circ} 51' 22.8''$ $Ls = 200.00'$ $LT = 118.45'$ $ST = 84.27'$	PI Sta 35+86.43 $\Delta = 14^{\circ} 40' 16.1''$ (RT) $D = 5^{\circ} 45' 09.5''$ $L = 255.03'$ $T = 128.22'$ $R = 996.00'$ SE = SEE PLANS	PI Sta 37+79.98 $\Delta = 81^{\circ} 45' 09.3''$ M.O. 9E. 19.9 N $Ls = 200.00'$ $LT = 133.40'$ $ST = 66.73'$	PI Sta 18+13.28 $\Delta = 88^{\circ} 33' 38.4''$ (LT) $D = 45^{\circ} 50' 11.8''$ $L = 193.21'$ $T = 121.90'$ $R = 125.00'$ SE = 0.02 FT/FT	PI Sta 19+96.77 $\Delta = 107^{\circ} 39' 56.3''$ (LT) $D = 69^{\circ} 52' 22.4''$ $L = 154.09'$ $T = 112.17'$ $R = 82.00'$ SE = 0.02 FT/FT	

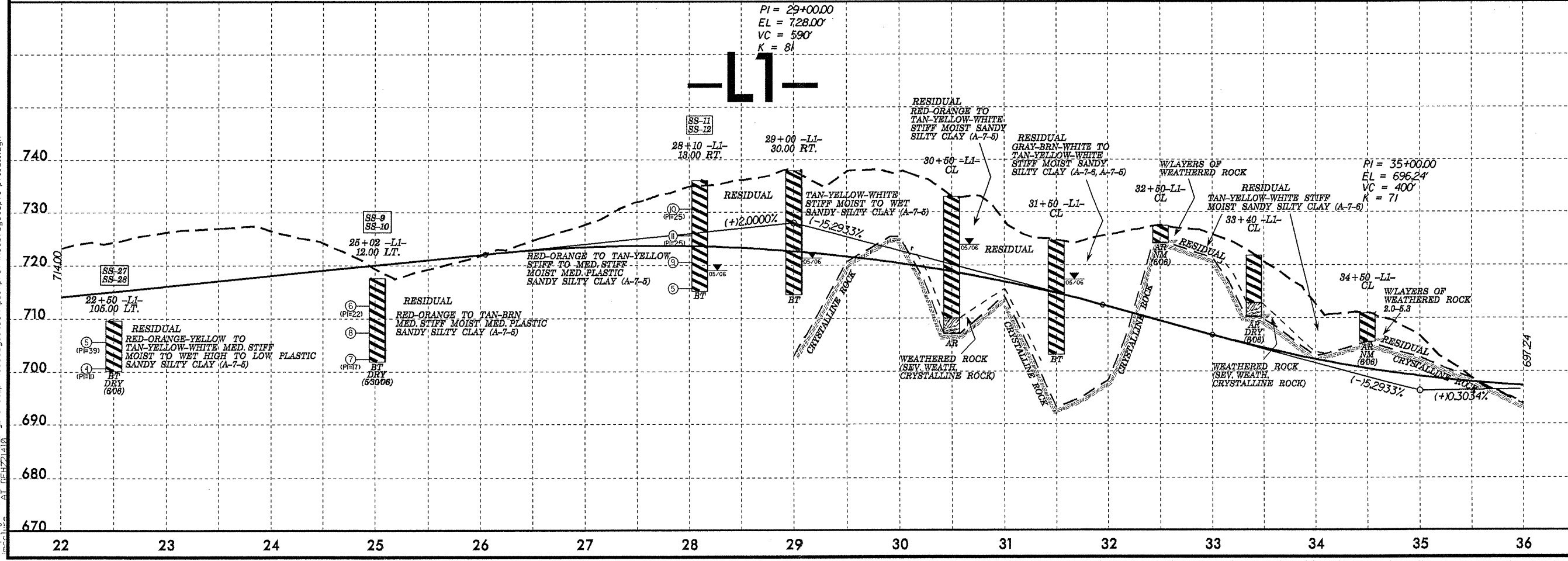
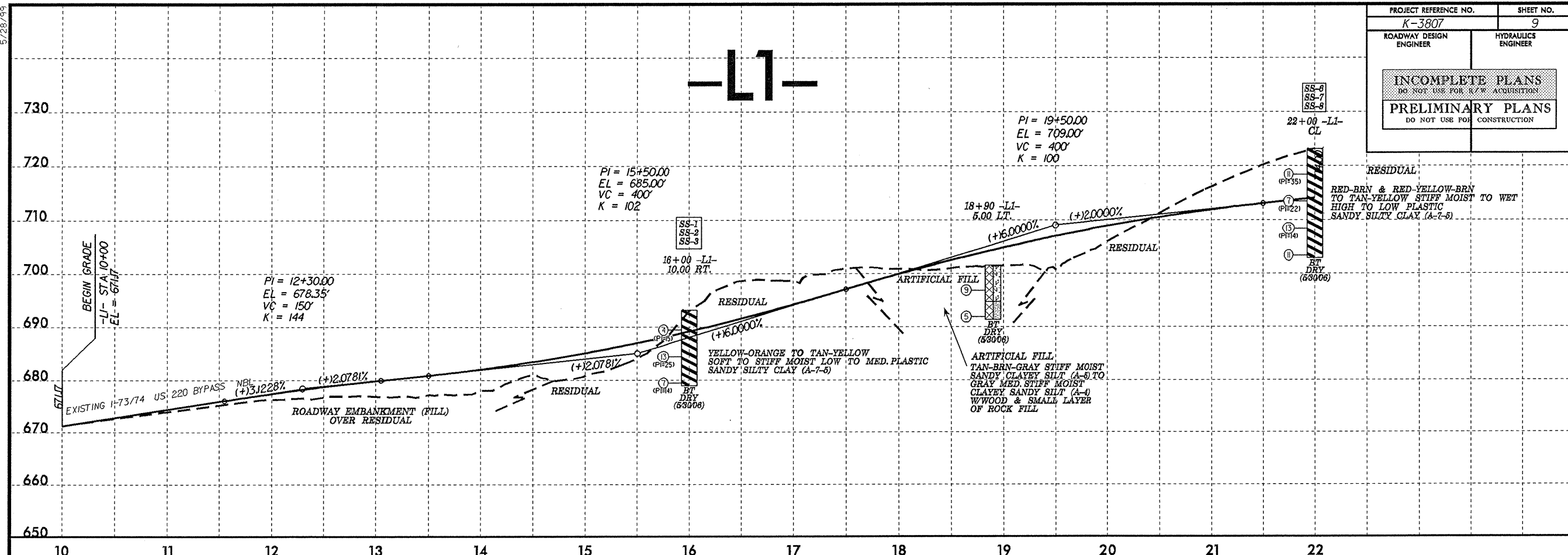


MARK BRASWELL
DB 1468 PG 622

449.93'
N83°41'30"W

8/17/99
 REVISIONS
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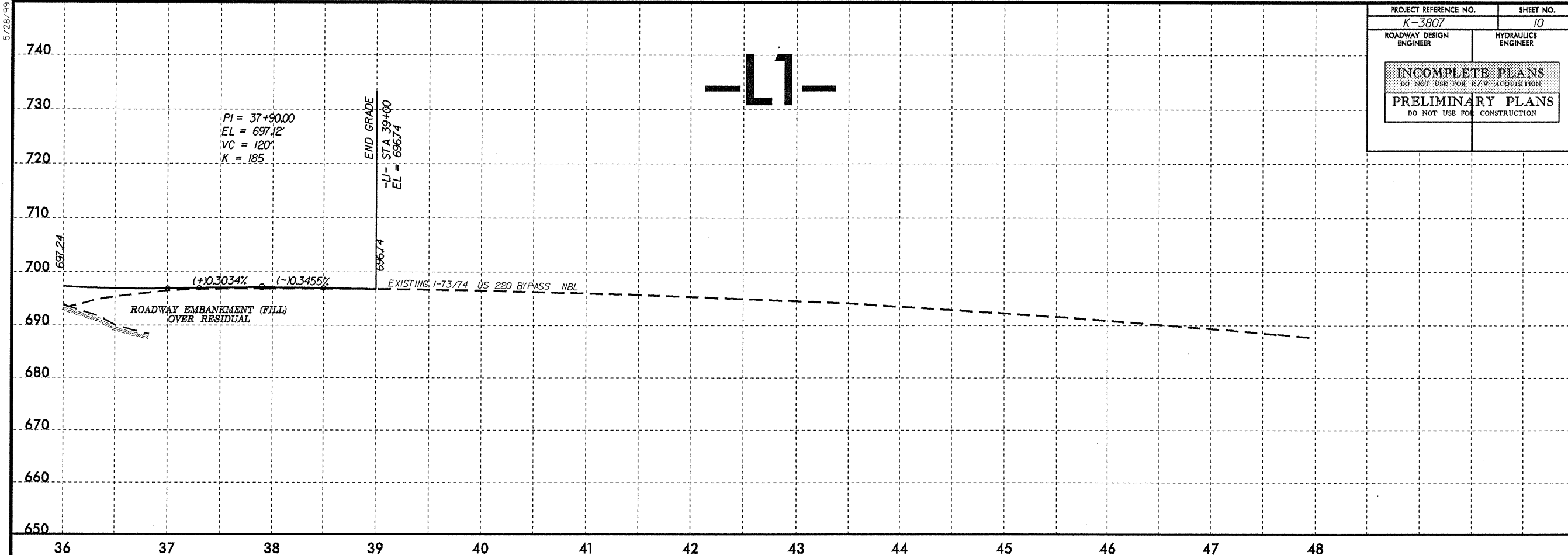
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INCOMPLETE PLANS DO NOT USE FOR R/W ACQUISITION	
PRELIMINARY PLANS DO NOT USE FOR CONSTRUCTION	



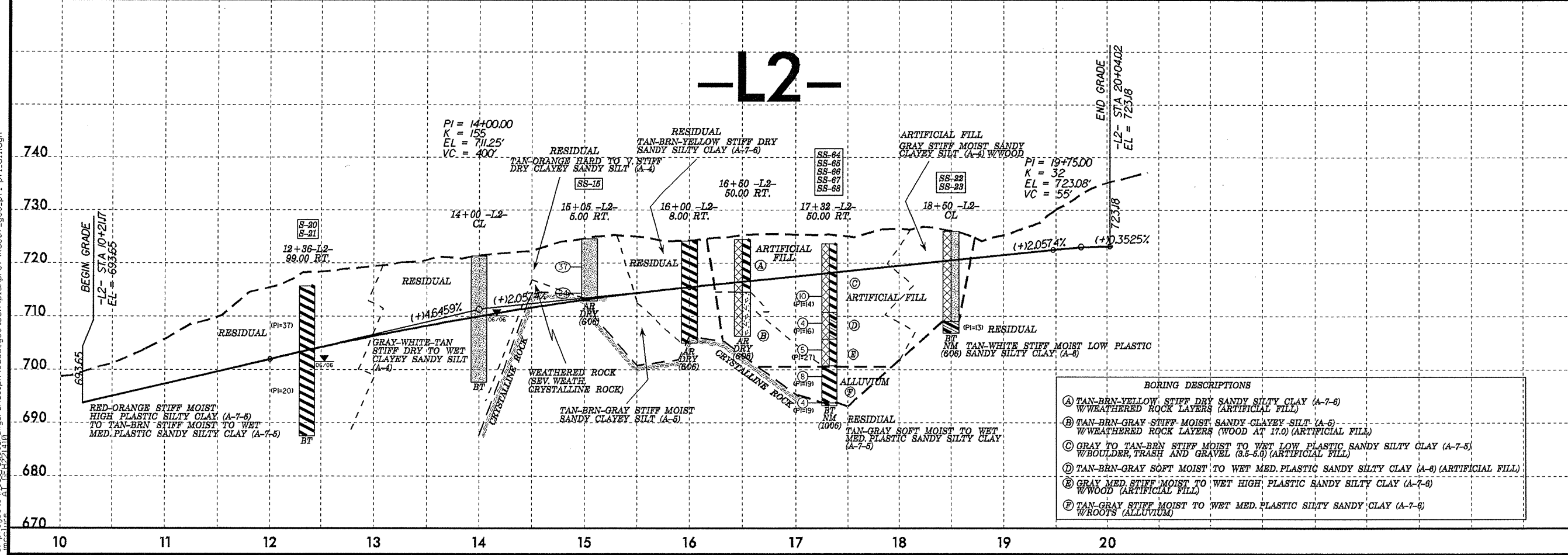
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PROJECT REFERENCE NO. K-3807	SHEET NO. 10
ROADWAY DESIGN ENGINEER	HYDRAULICS ENGINEER
INCOMPLETE PLANS DO NOT USE FOR R/W ACQUISITION	
PRELIMINARY PLANS DO NOT USE FOR CONSTRUCTION	

5/28/99



-L2-



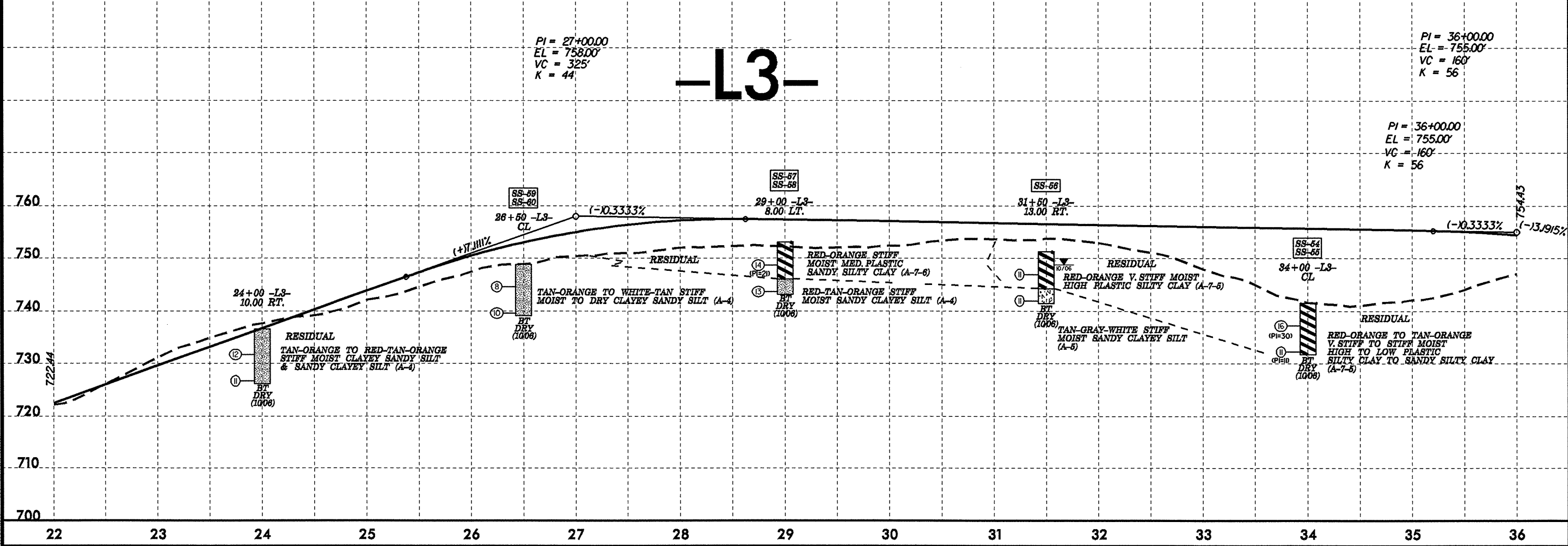
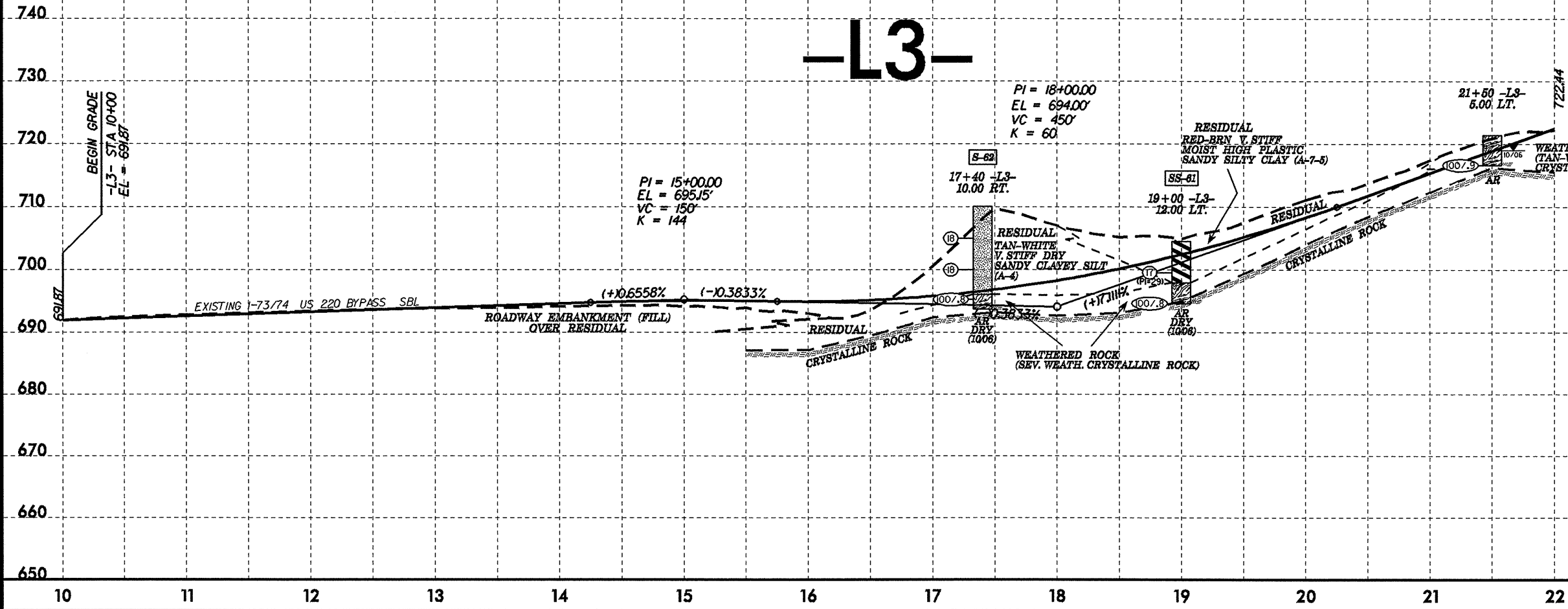
BORING DESCRIPTIONS

Ⓐ	TAN-BRN-YELLOW STIFF DRY SANDY SILTY CLAY (A-7-6) W/WEATHERED ROCK LAYERS (ARTIFICIAL FILL)
Ⓑ	TAN-BRN-GRAY STIFF-MOIST SANDY CLAYEY SILT (A-6) W/WEATHERED ROCK LAYERS (WOOD AT 17.0) (ARTIFICIAL FILL)
Ⓒ	GRAY TO TAN-BRN STIFF MOIST TO WET LOW PLASTIC SANDY SILTY CLAY (A-7-5) W/BOULDER, TRASH AND GRAVEL (3.5-6.0) (ARTIFICIAL FILL)
Ⓓ	TAN-BRN-GRAY SOFT MOIST TO WET MED. PLASTIC SANDY SILTY CLAY (A-8) (ARTIFICIAL FILL)
Ⓔ	GRAY MED. STIFF MOIST TO WET HIGH PLASTIC SANDY SILTY CLAY (A-7-6) W/WOOD (ARTIFICIAL FILL)
Ⓕ	TAN-GRAY STIFF MOIST TO WET MED. PLASTIC SILTY SANDY CLAY (A-7-6) W/ROOTS (ALLUVIUM)

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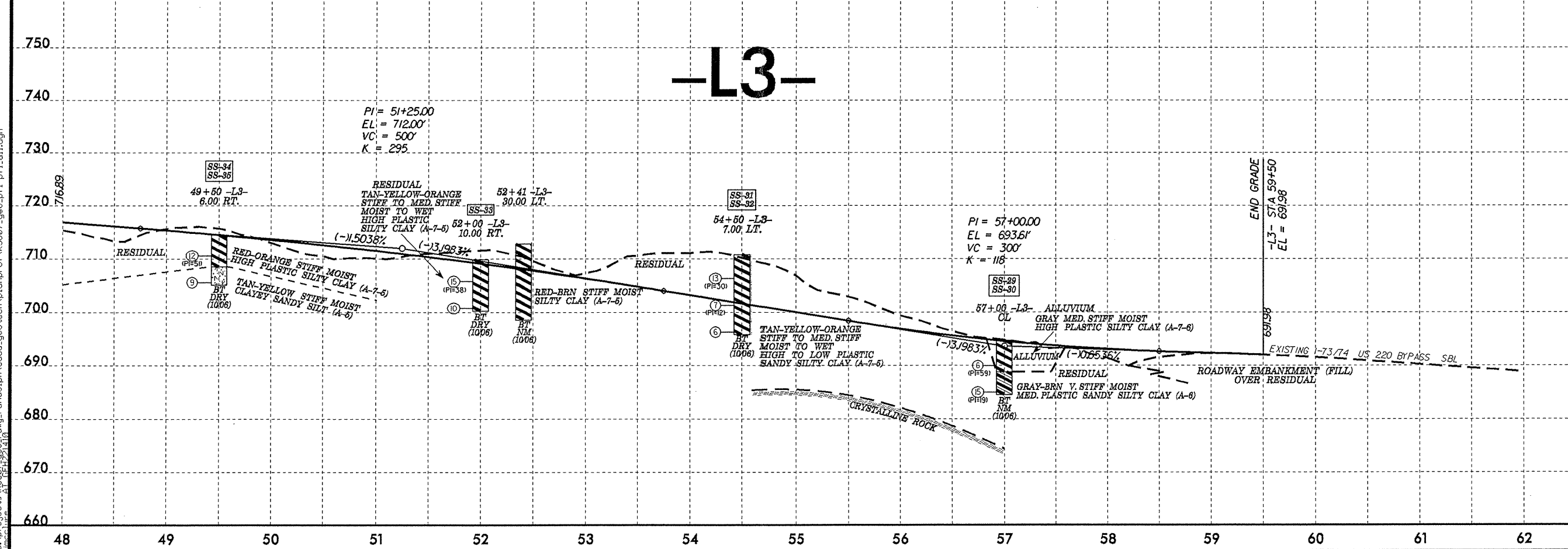
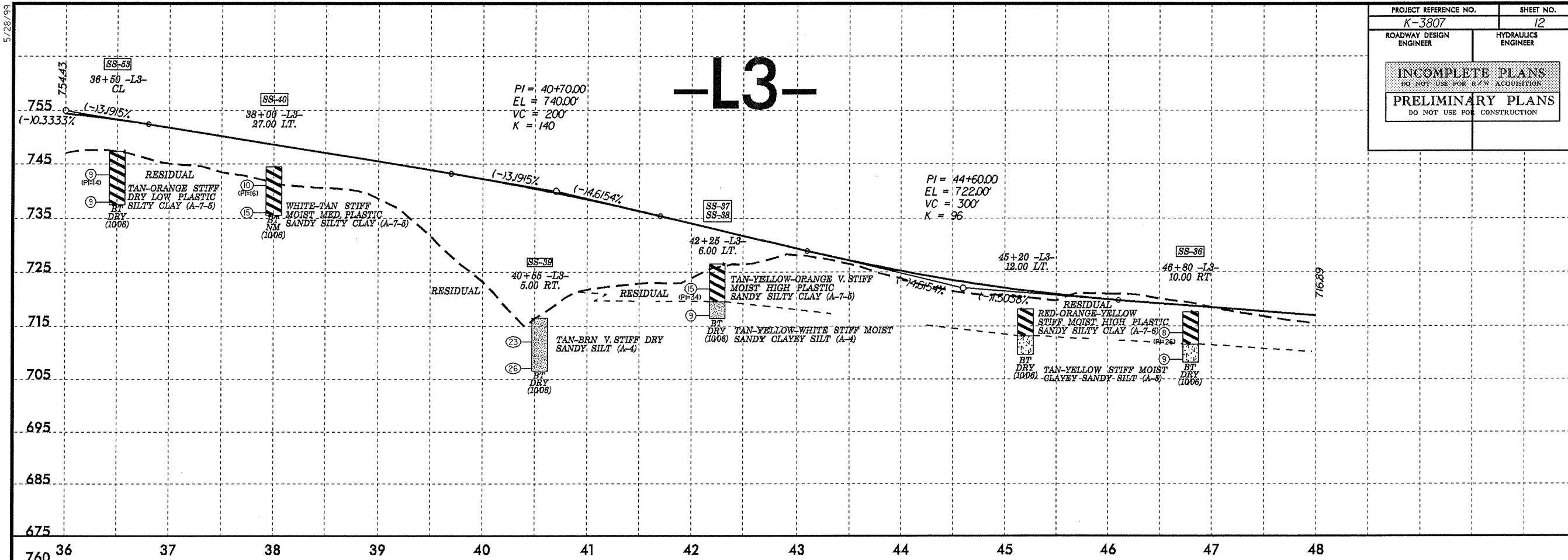
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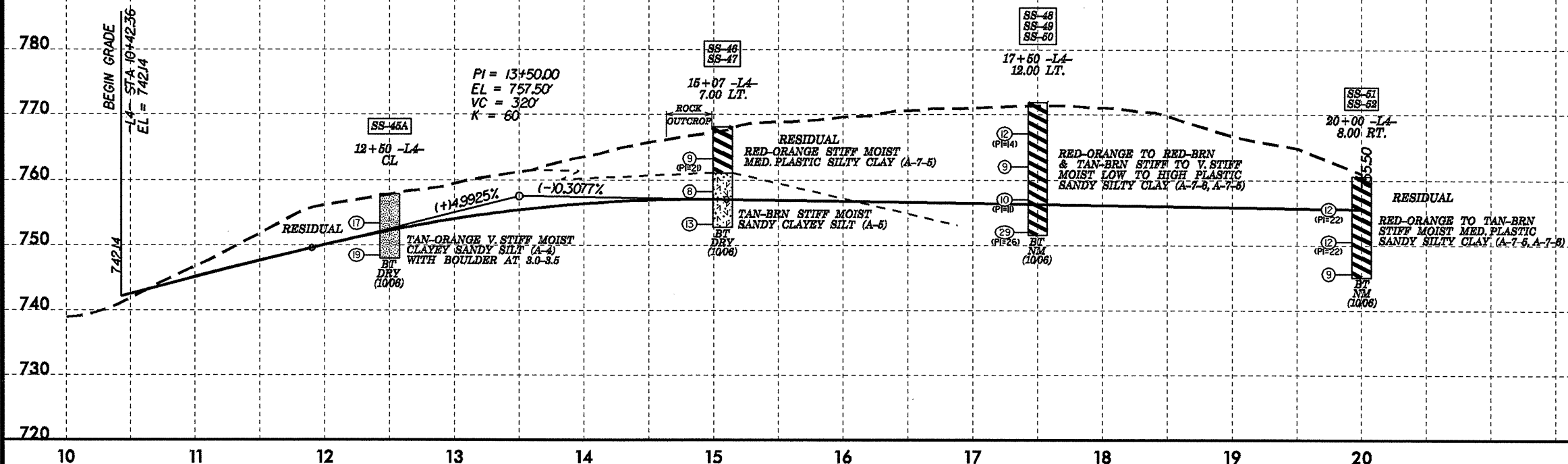
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ROADWAY DESIGN ENGINEER	HYDRAULICS ENGINEER
INCOMPLETE PLANS DO NOT USE FOR R/W ACQUISITION	
PRELIMINARY PLANS DO NOT USE FOR CONSTRUCTION	



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 5/28/99

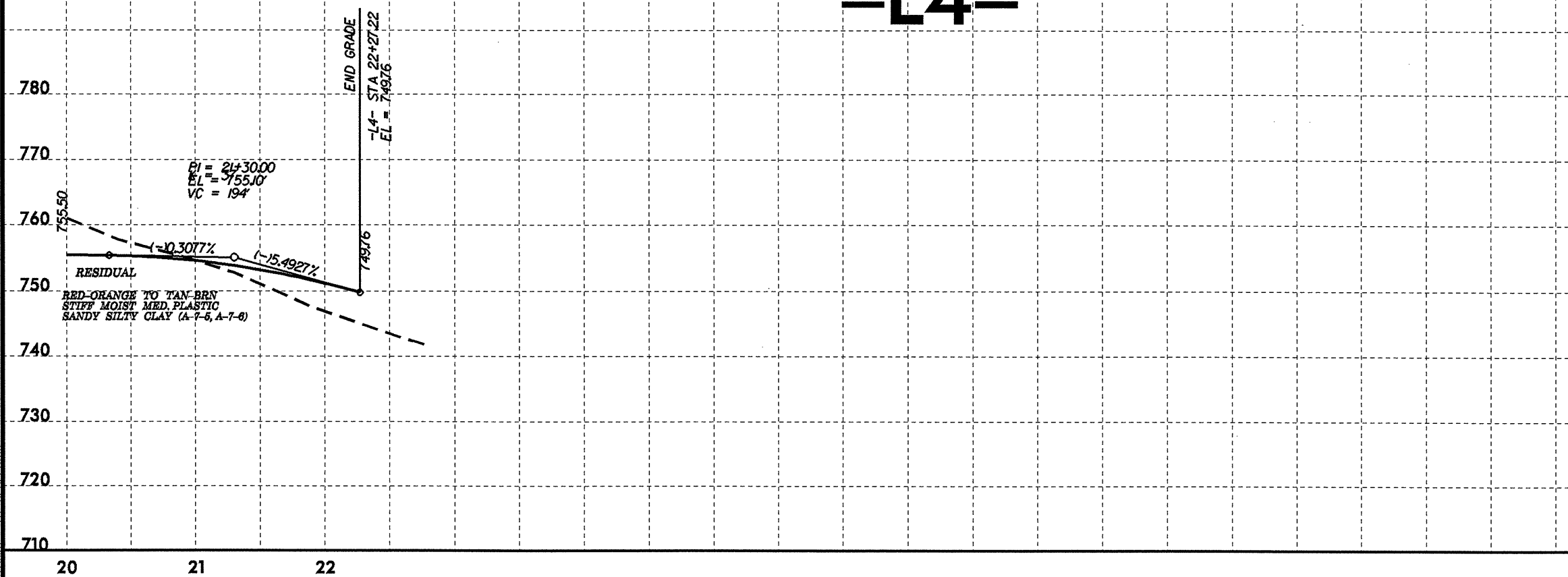
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-L4-

-L4-

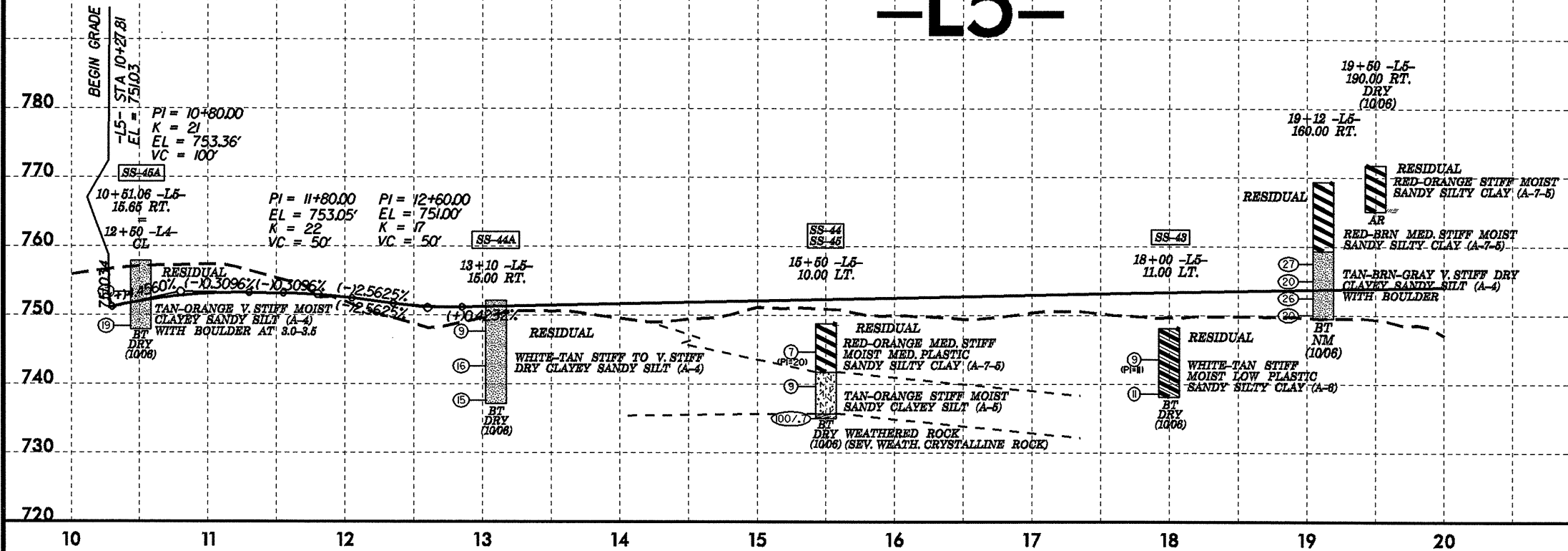


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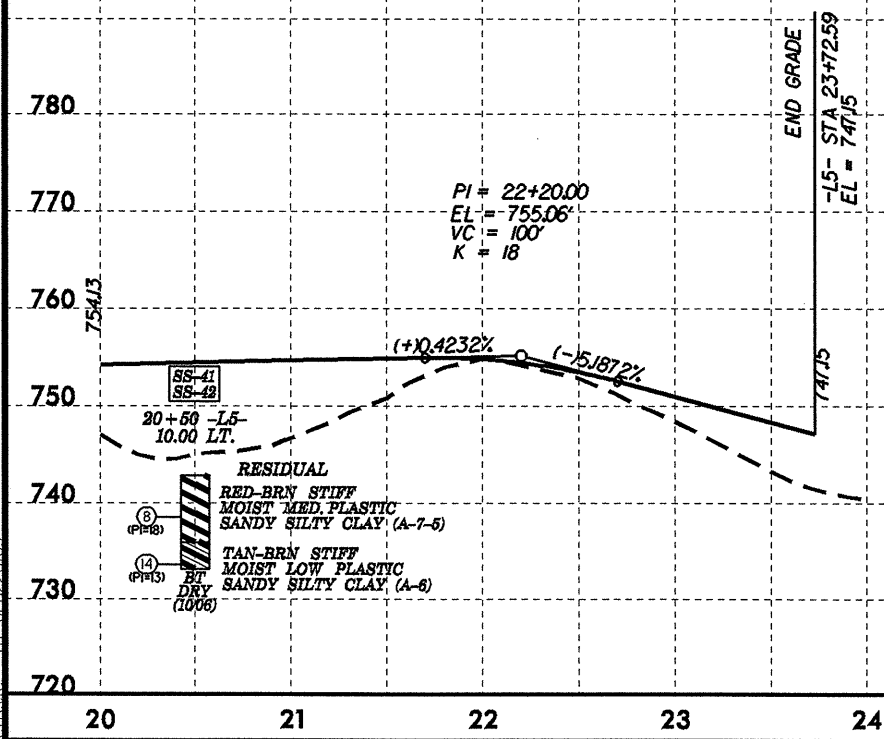
5/28/99

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-L5-



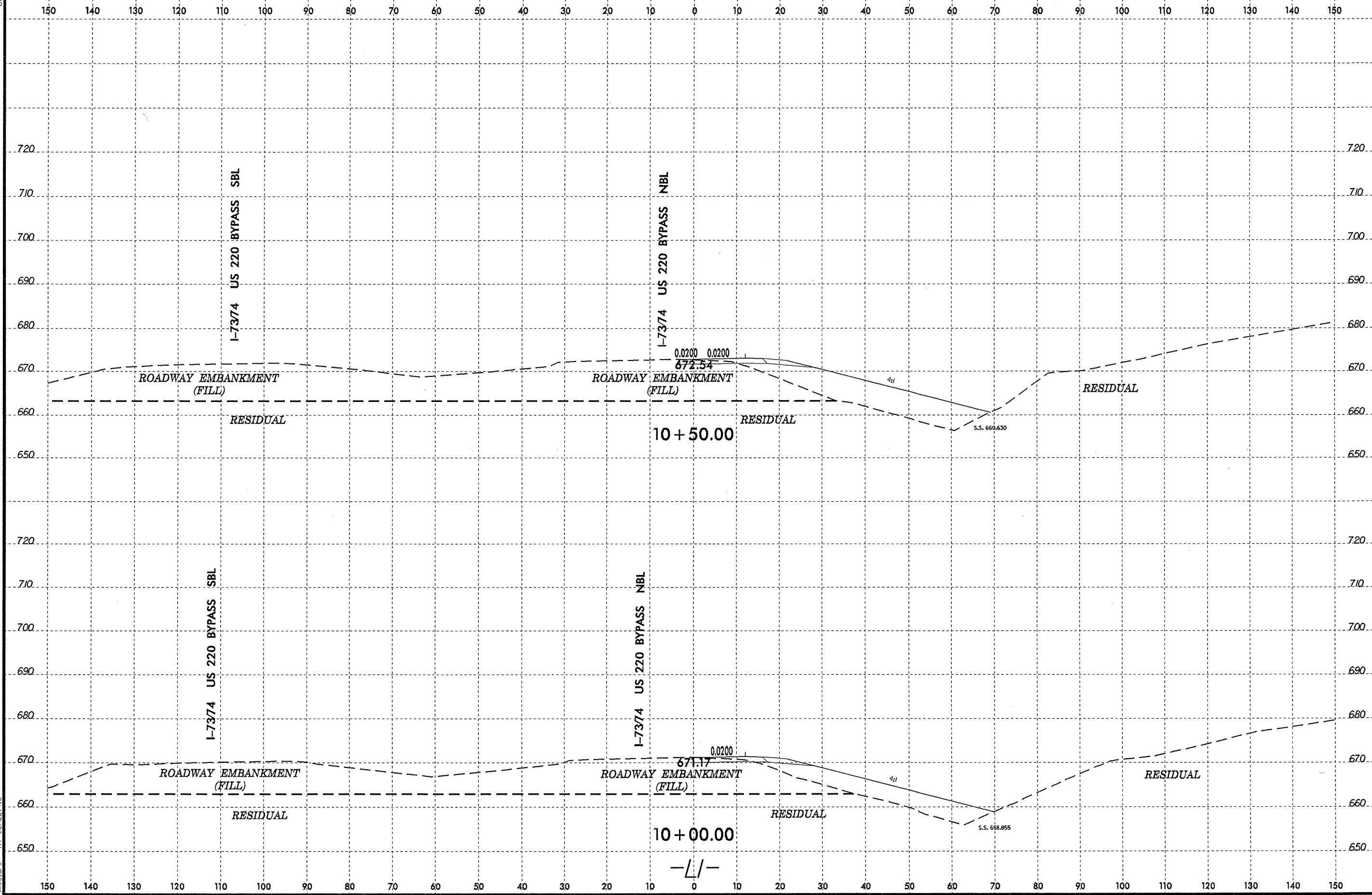
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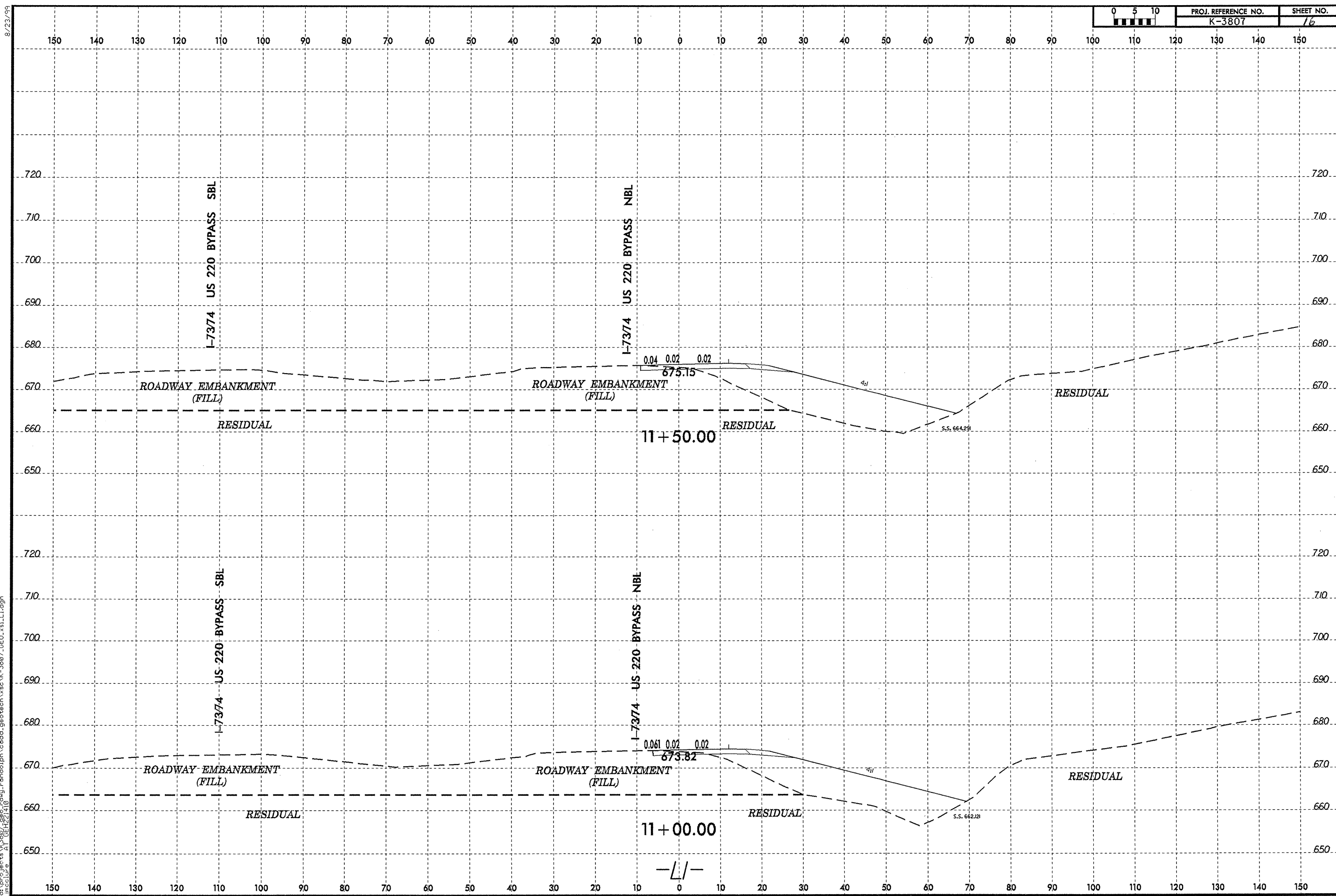
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 10+00.00

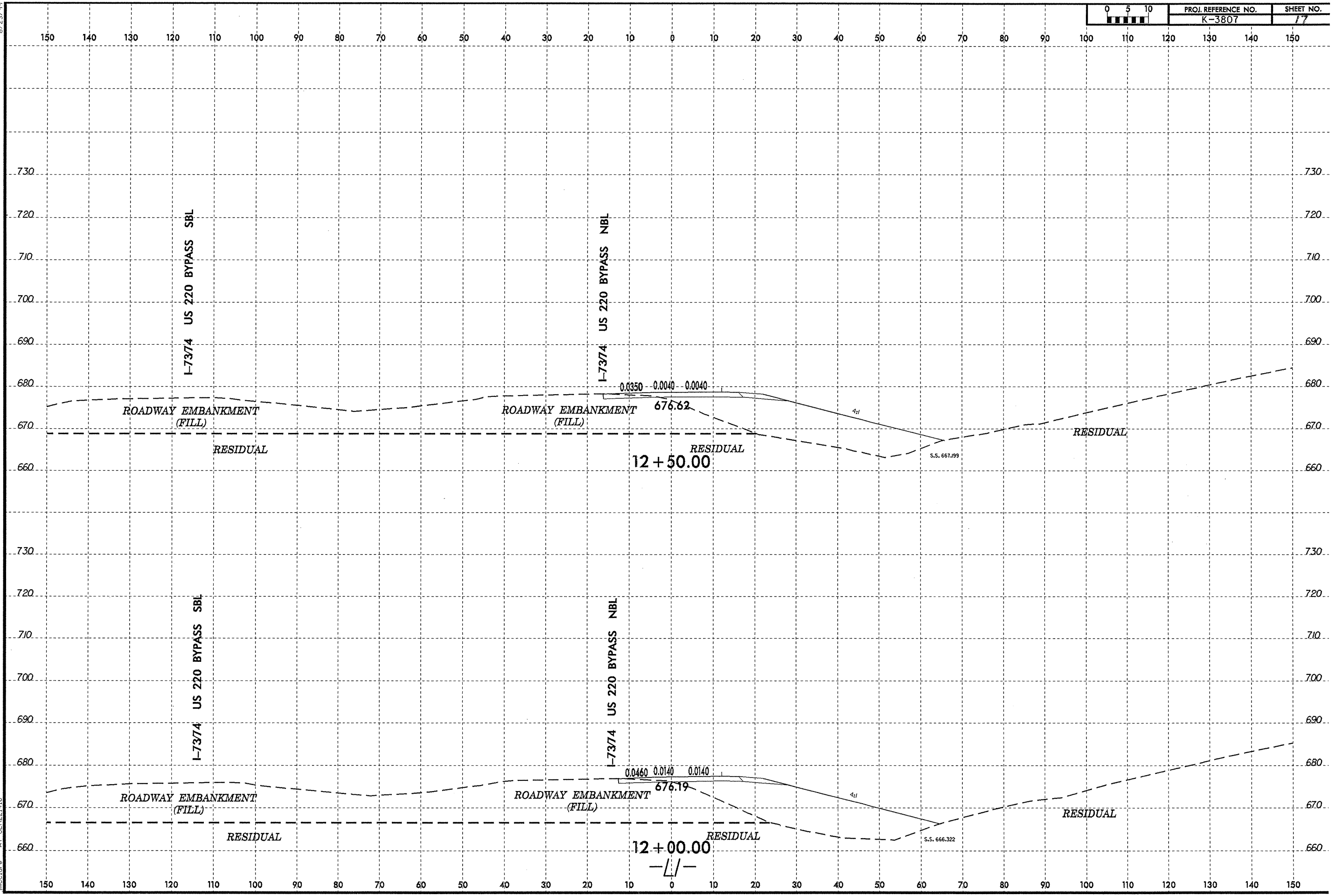
-L/-



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 11+00.00

8/23/99

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geotech\vsp\K-3807_650_xst.L1.dgn



ROADWAY EMBANKMENT (FILL)

RESIDUAL

I-7374 US 220 BYPASS SBL

ROADWAY EMBANKMENT (FILL)

RESIDUAL

I-7374 US 220 BYPASS NBL

0.0350 0.0040 0.0040

676.62

12 + 50.00

S.S. 667.099

ROADWAY EMBANKMENT (FILL)

RESIDUAL

I-7374 US 220 BYPASS SBL

ROADWAY EMBANKMENT (FILL)

RESIDUAL

I-7374 US 220 BYPASS NBL

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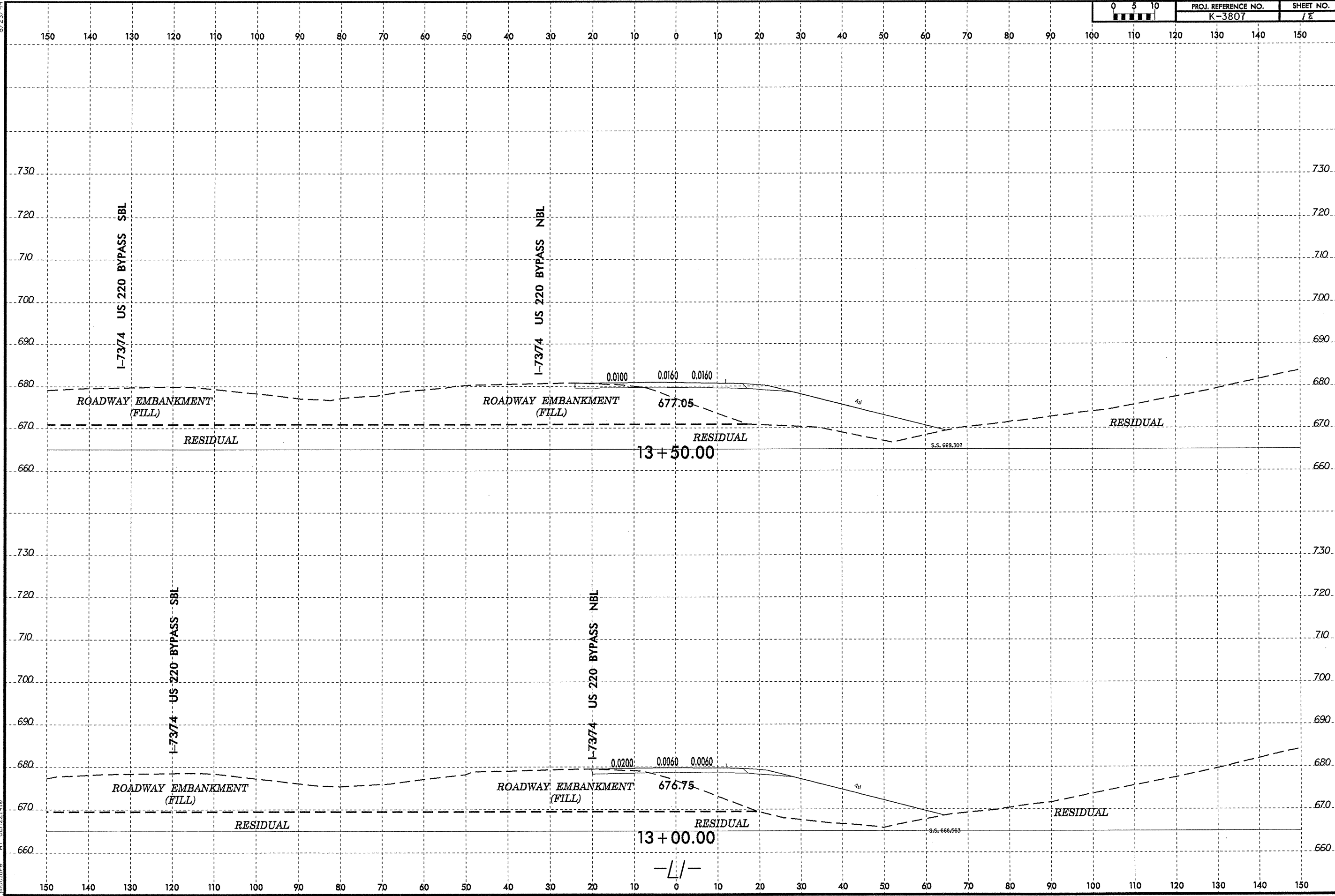
12 + 00.00

S.S. 666.322

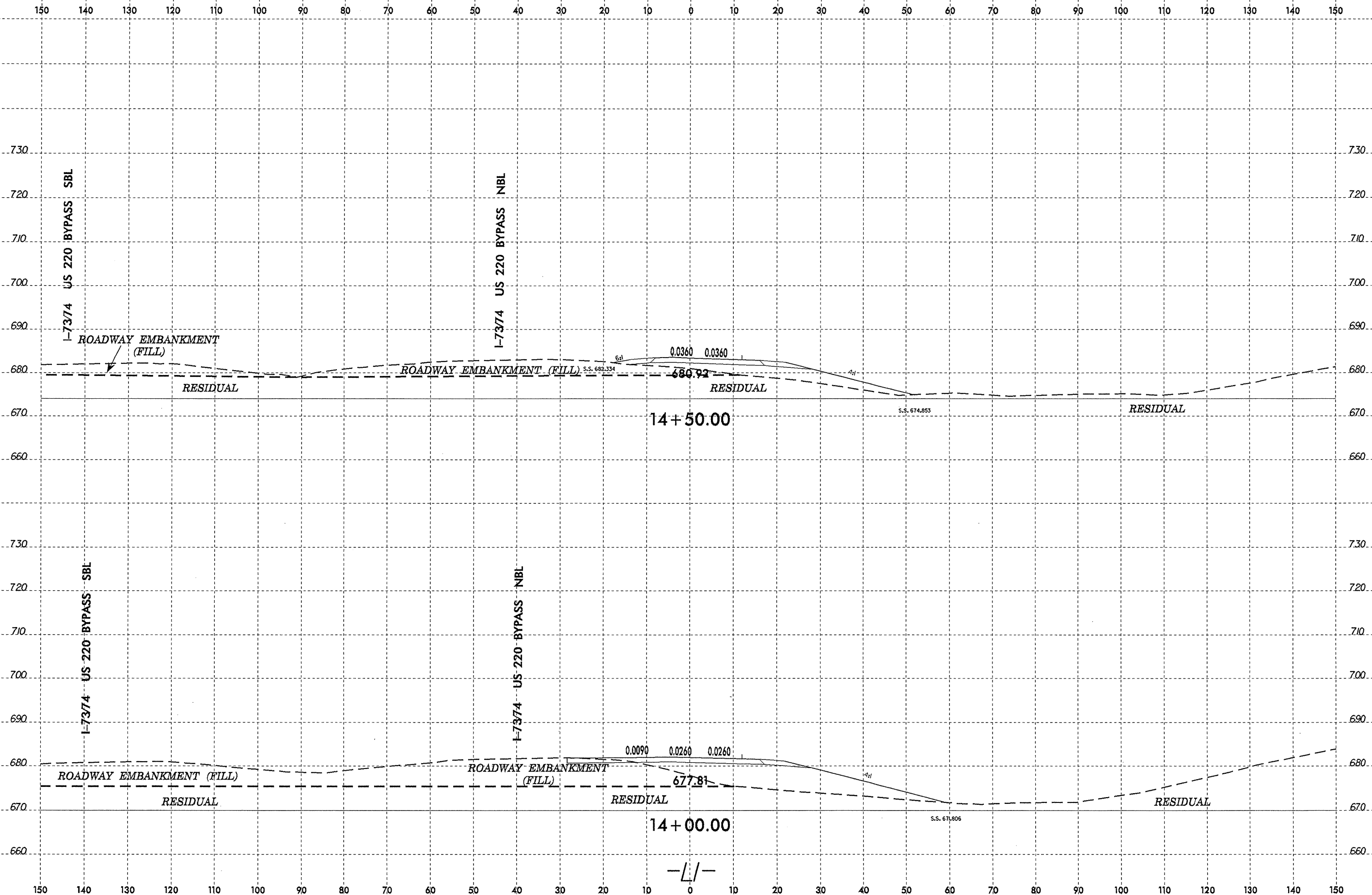
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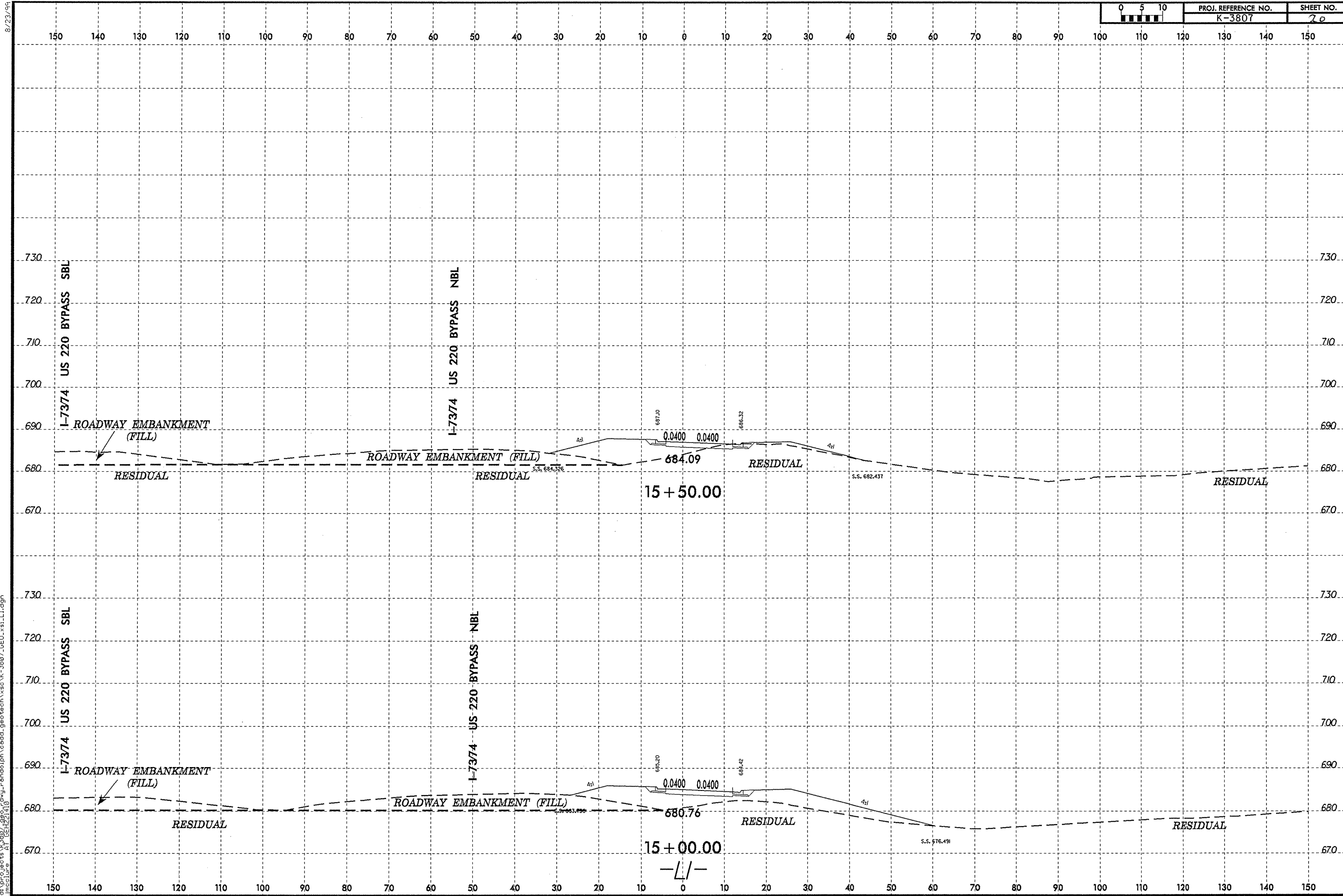
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-L/-



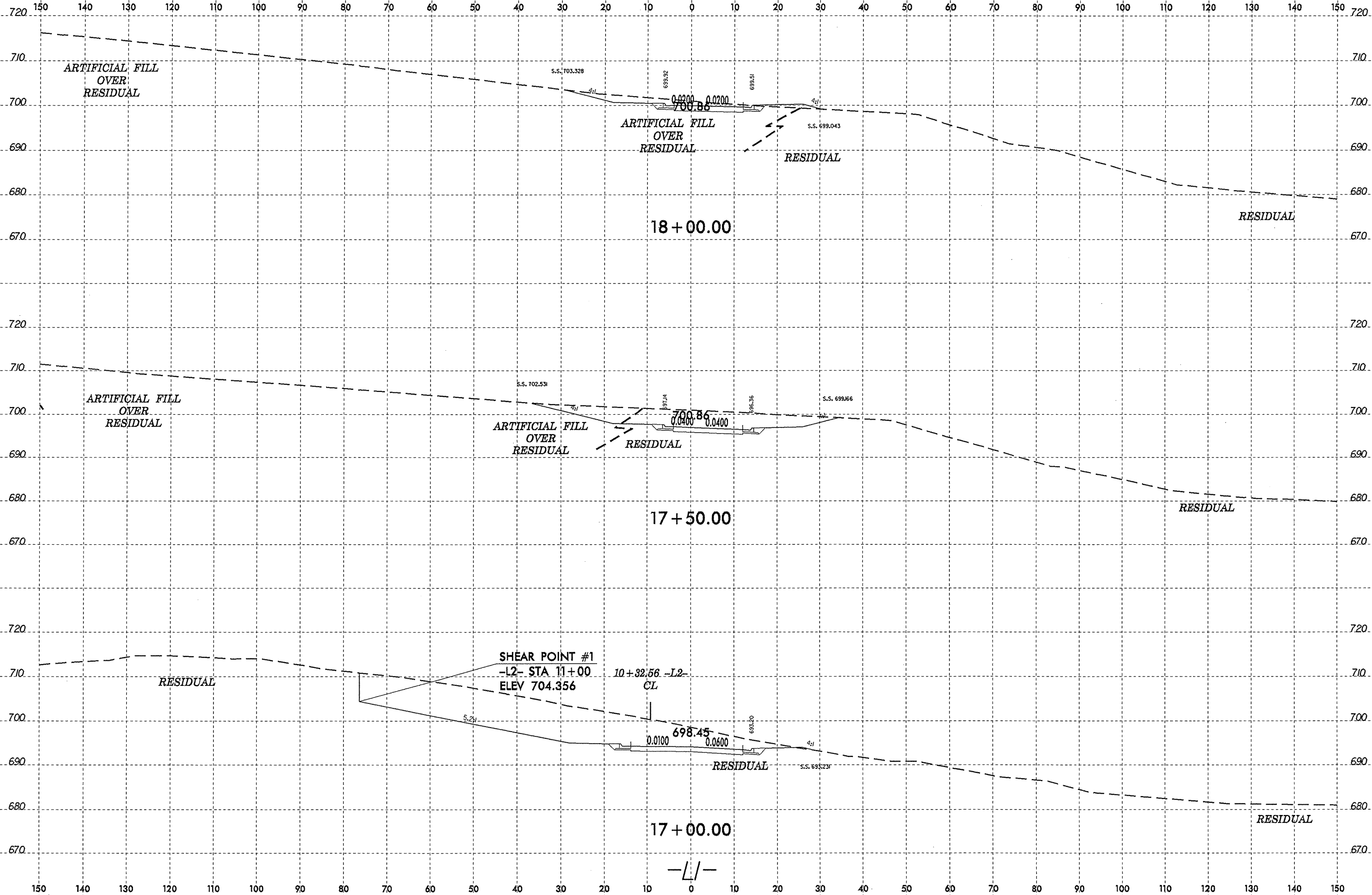
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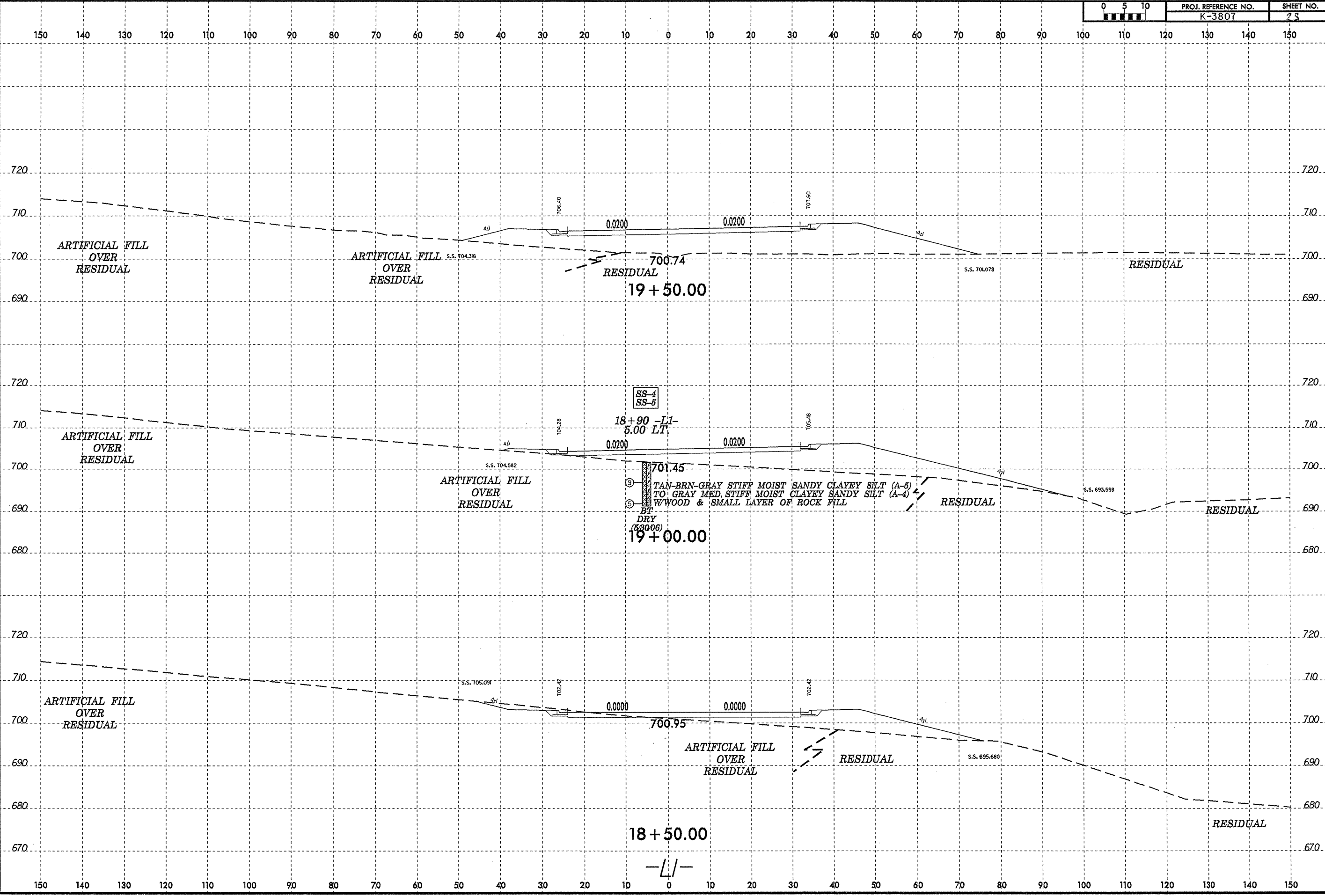
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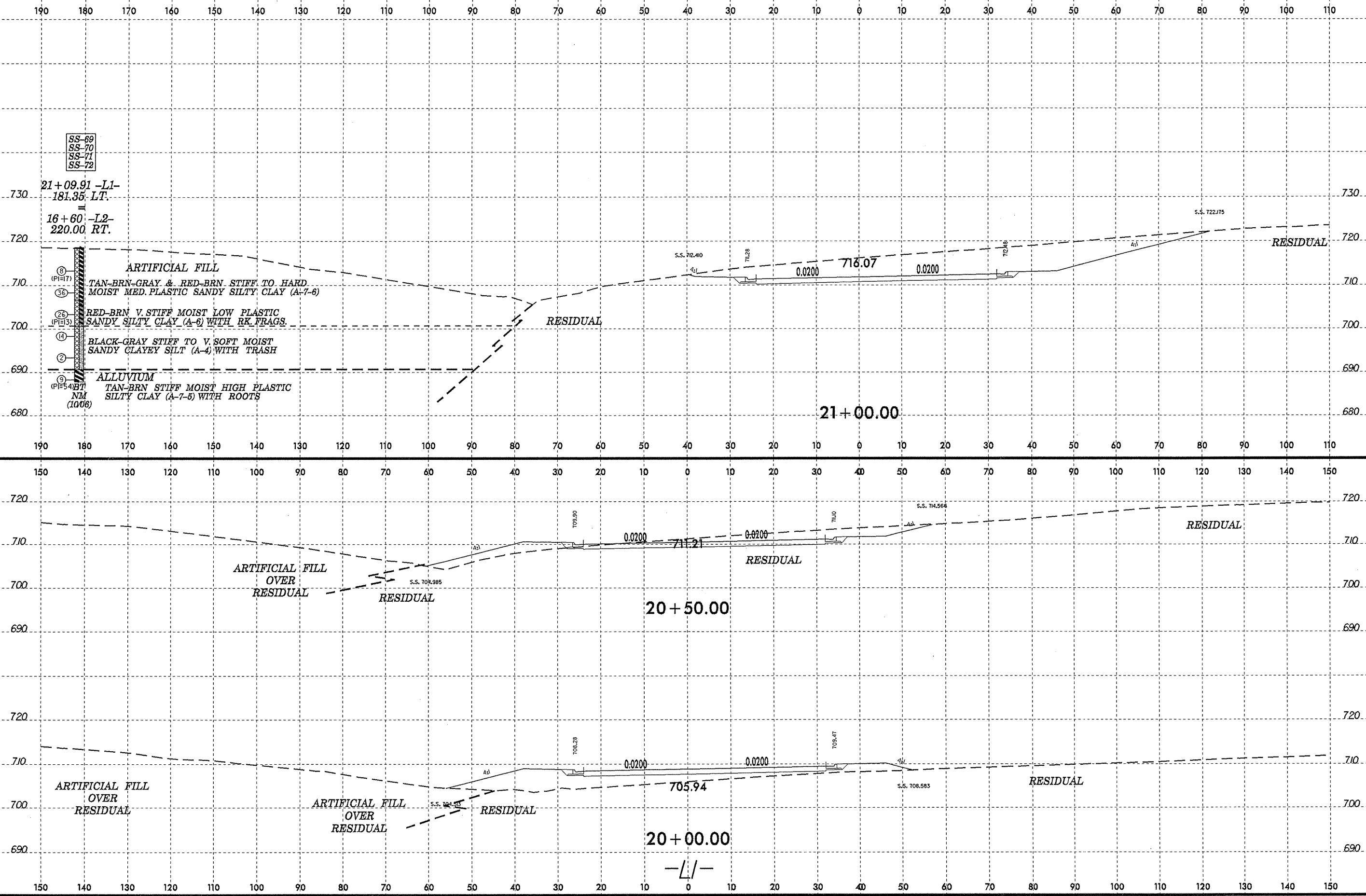


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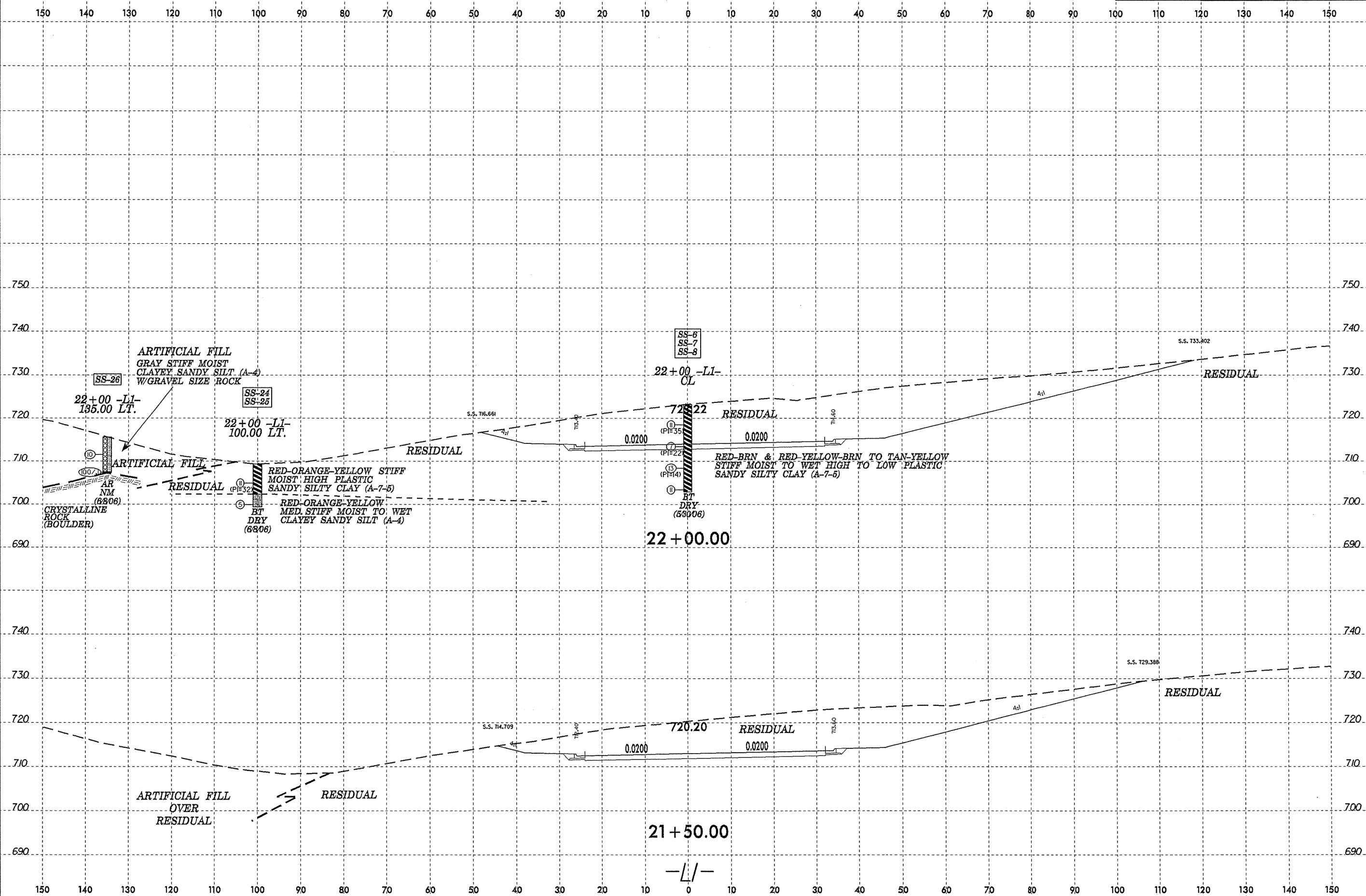


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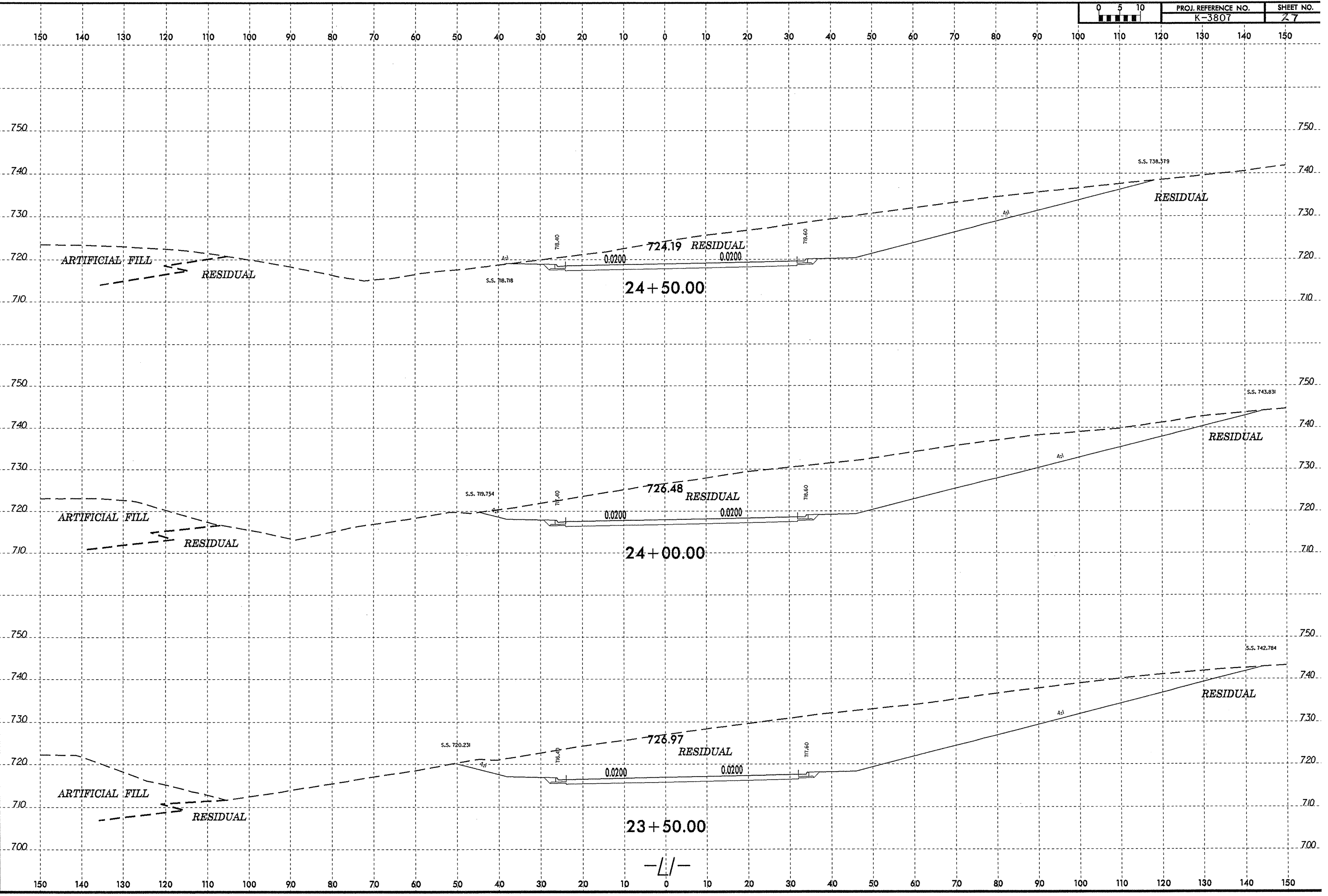
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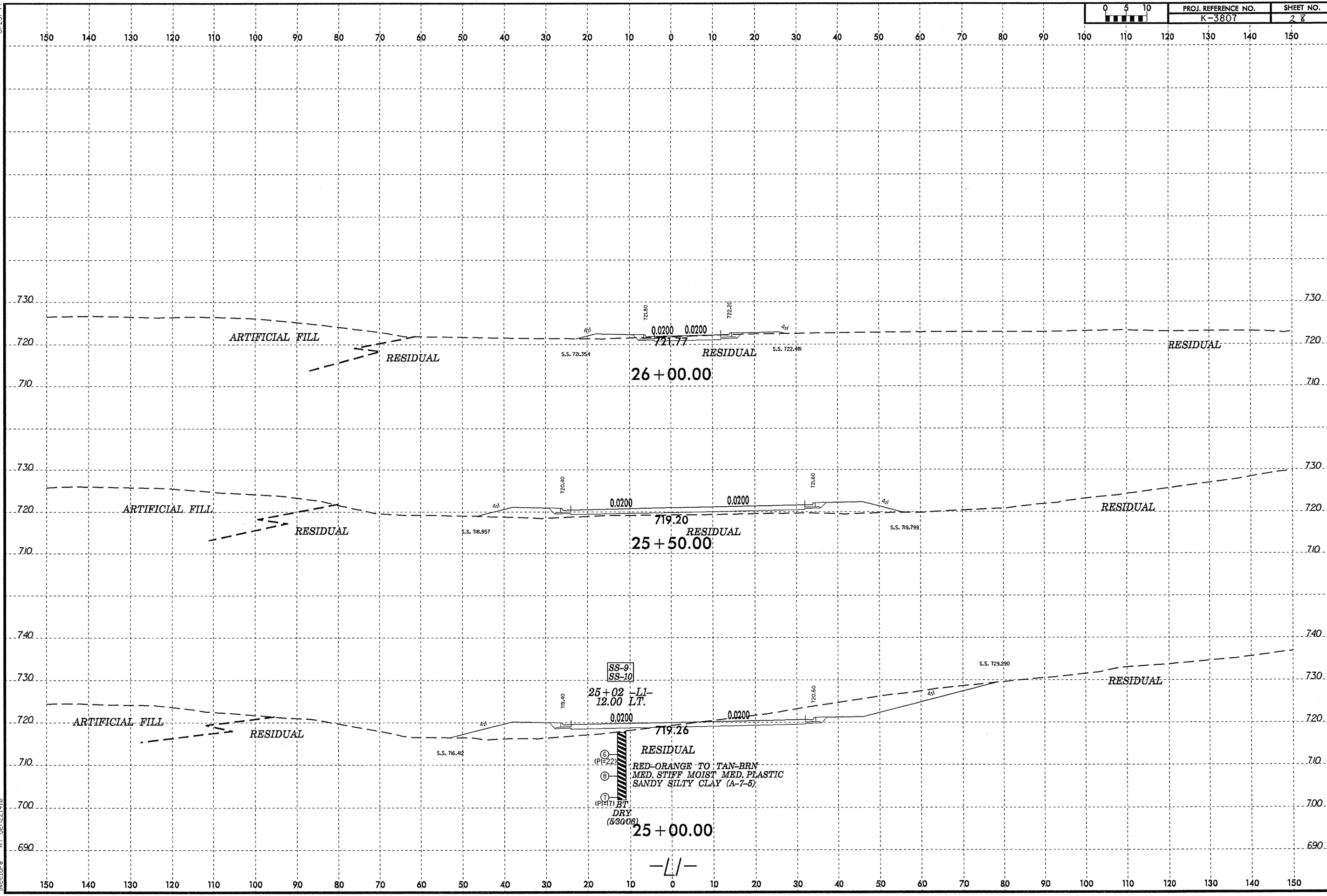
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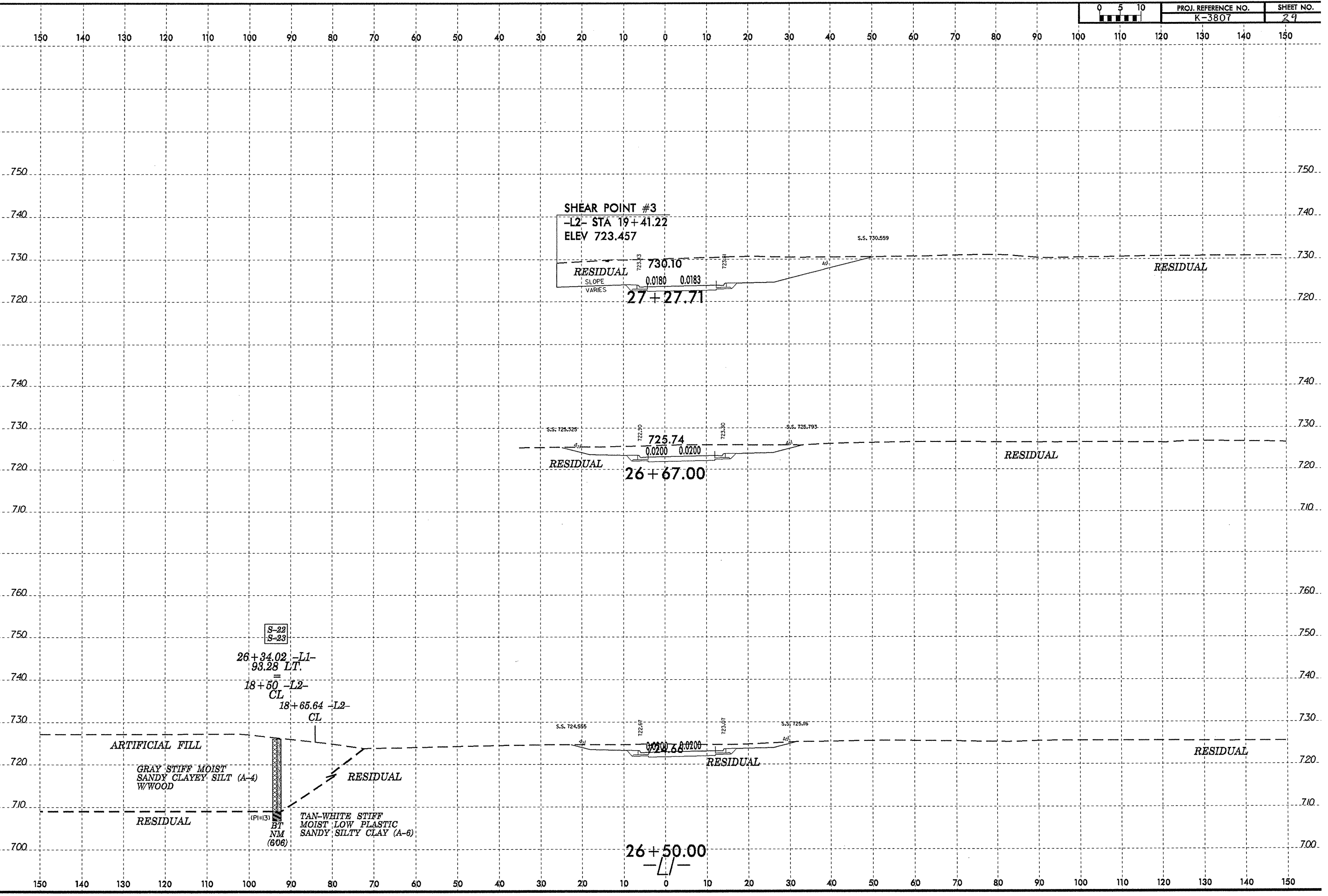


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8/23/99
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msculing



SHEAR POINT #3
 -L2- STA 19+41.22
 ELEV 723.457

RESIDUAL
 SLOPE
 VARIES
 730.10
 0.0180 0.0183
 27+27.71

S.S. 725.325
 725.74
 0.0200 0.0200
 RESIDUAL
 26+67.00

S-22
 S-23
 26+34.02 -L1-
 93.28 LT.
 18+50 -L2-
 CL
 18+65.64 -L2-
 CL

ARTIFICIAL FILL
 GRAY STIFF MOIST
 SANDY CLAYEY SILT (A-4)
 W/WOOD

RESIDUAL

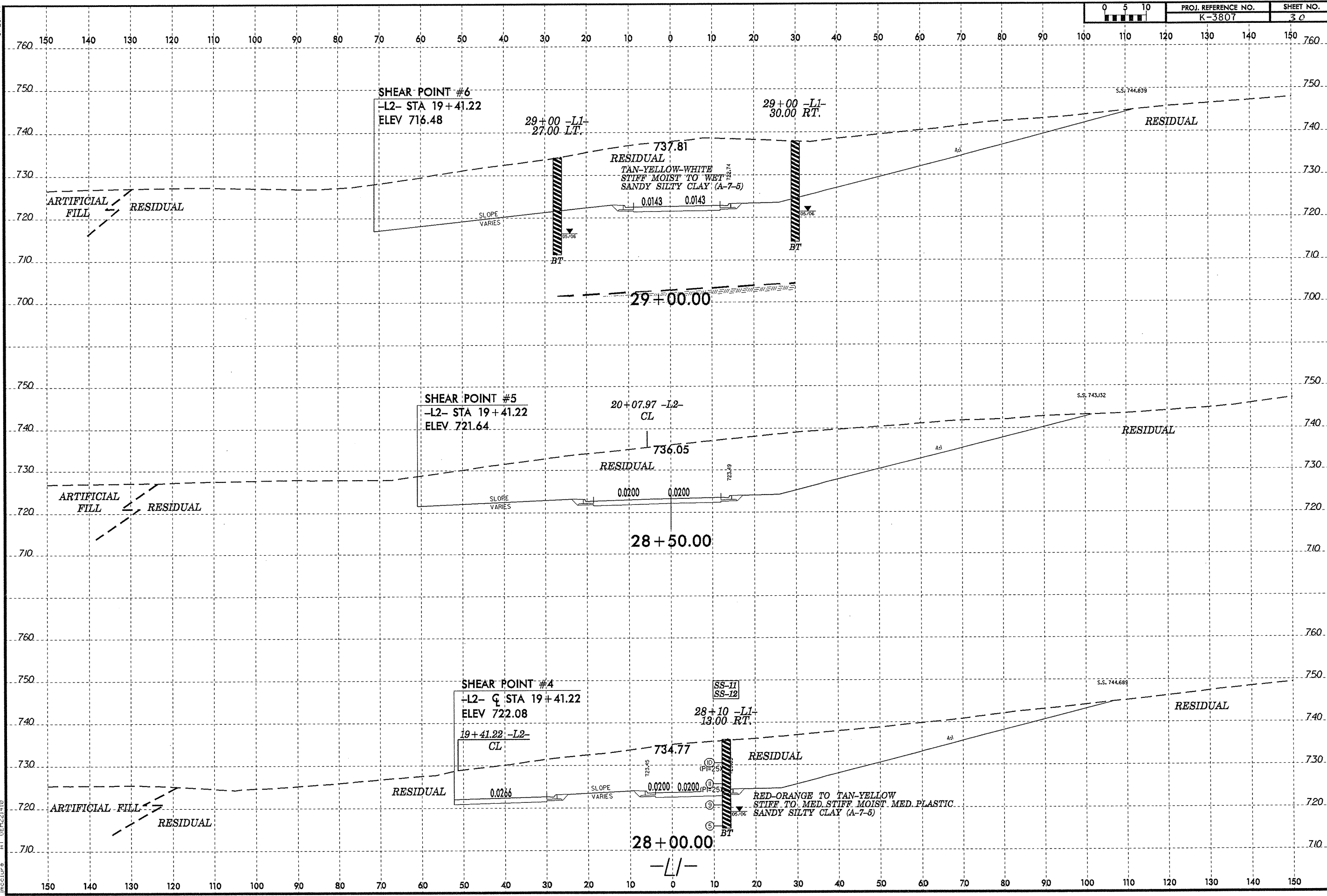
(PI=13)
 BT
 NM
 (606)
 TAN-WHITE STIFF
 MOIST LOW PLASTIC
 SANDY SILTY CLAY (A-6)

724.68
 0.0200 0.0200
 RESIDUAL
 26+50.00

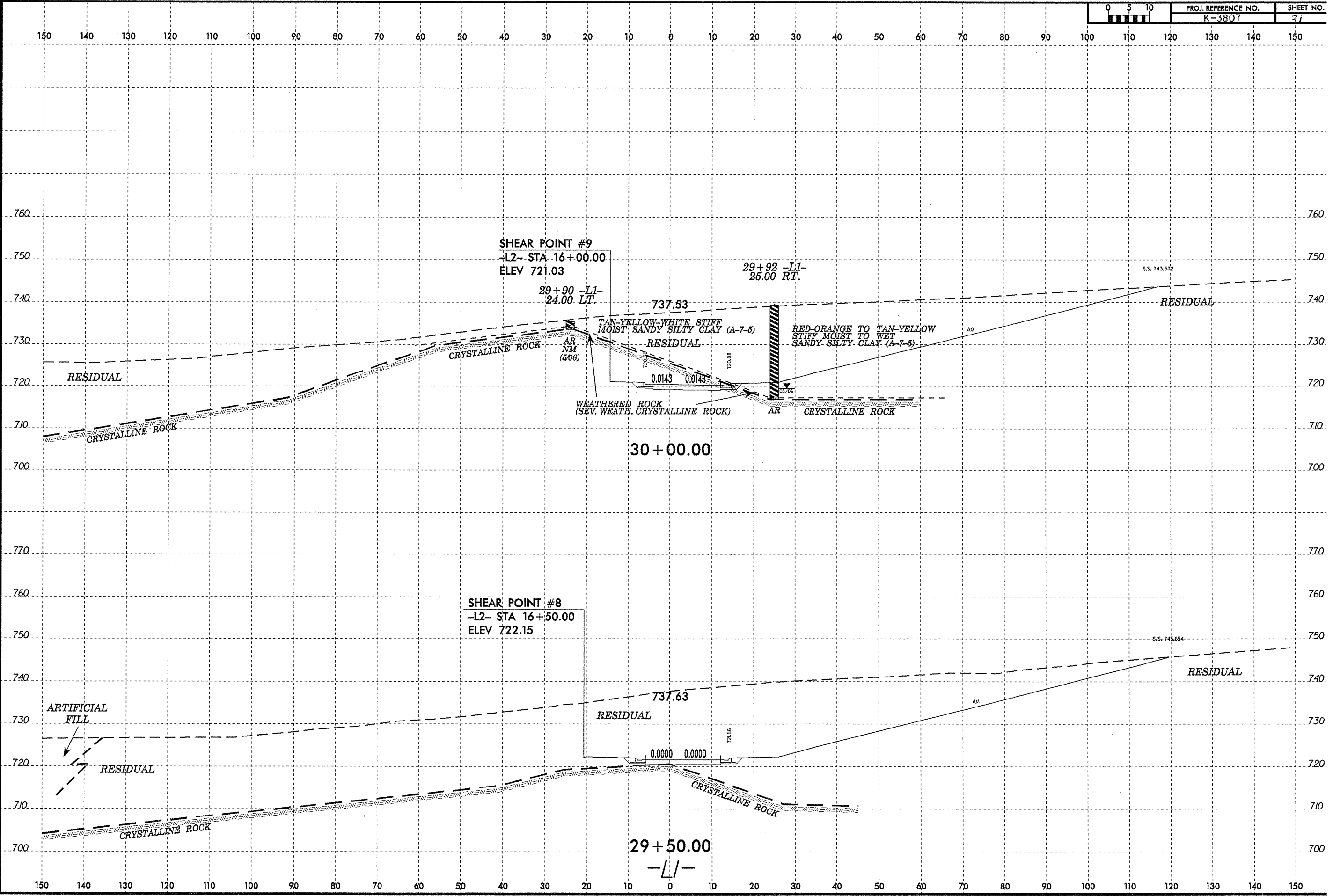
RESIDUAL

RESIDUAL

8/23/99
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meclure



SHEAR POINT #9
-L2- STA 16+00.00
ELEV 721.03

29+92 -L1-
25.00 RT.

29+90 -L1-
24.00 LT.

S.S. 743.572

737.53

TAN-YELLOW-WHITE STIFF
MOIST SANDY SILTY CLAY (A-7-5)

RED-ORANGE TO TAN-YELLOW
STIFF MOIST TO WET
SANDY SILTY CLAY (A-7-5)

CRYSTALLINE ROCK

RESIDUAL

CRYSTALLINE ROCK

WEATHERED ROCK
(SEV. WEATH. CRYSTALLINE ROCK)

30+00.00

SHEAR POINT #8
-L2- STA 16+50.00
ELEV 722.15

S.S. 745.654

737.63

RESIDUAL

0.0000 0.0000

29+50.00

-L1-

RESIDUAL

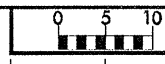
CRYSTALLINE ROCK

ARTIFICIAL
FILL

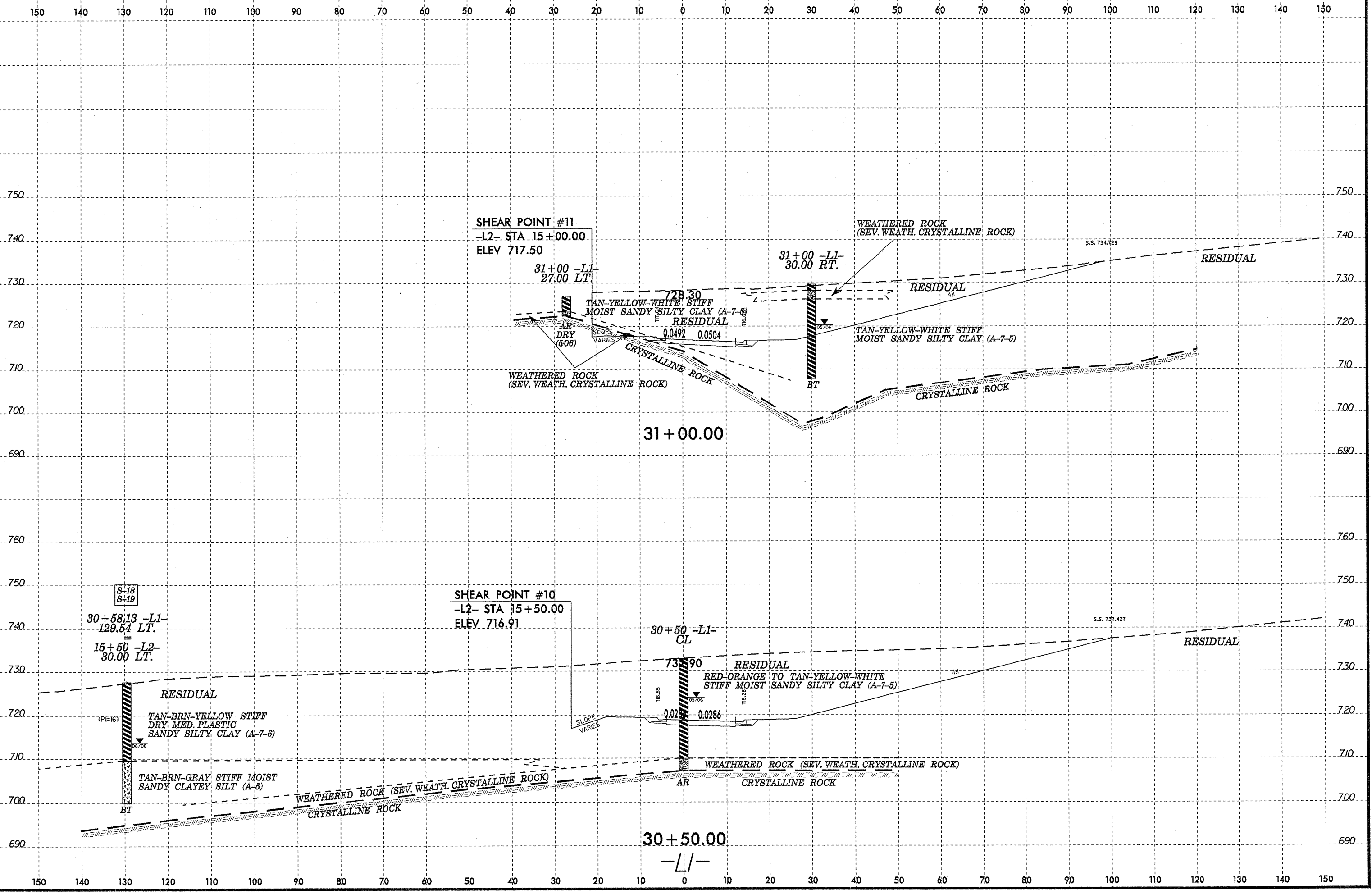
RESIDUAL

CRYSTALLINE ROCK

8/23/09

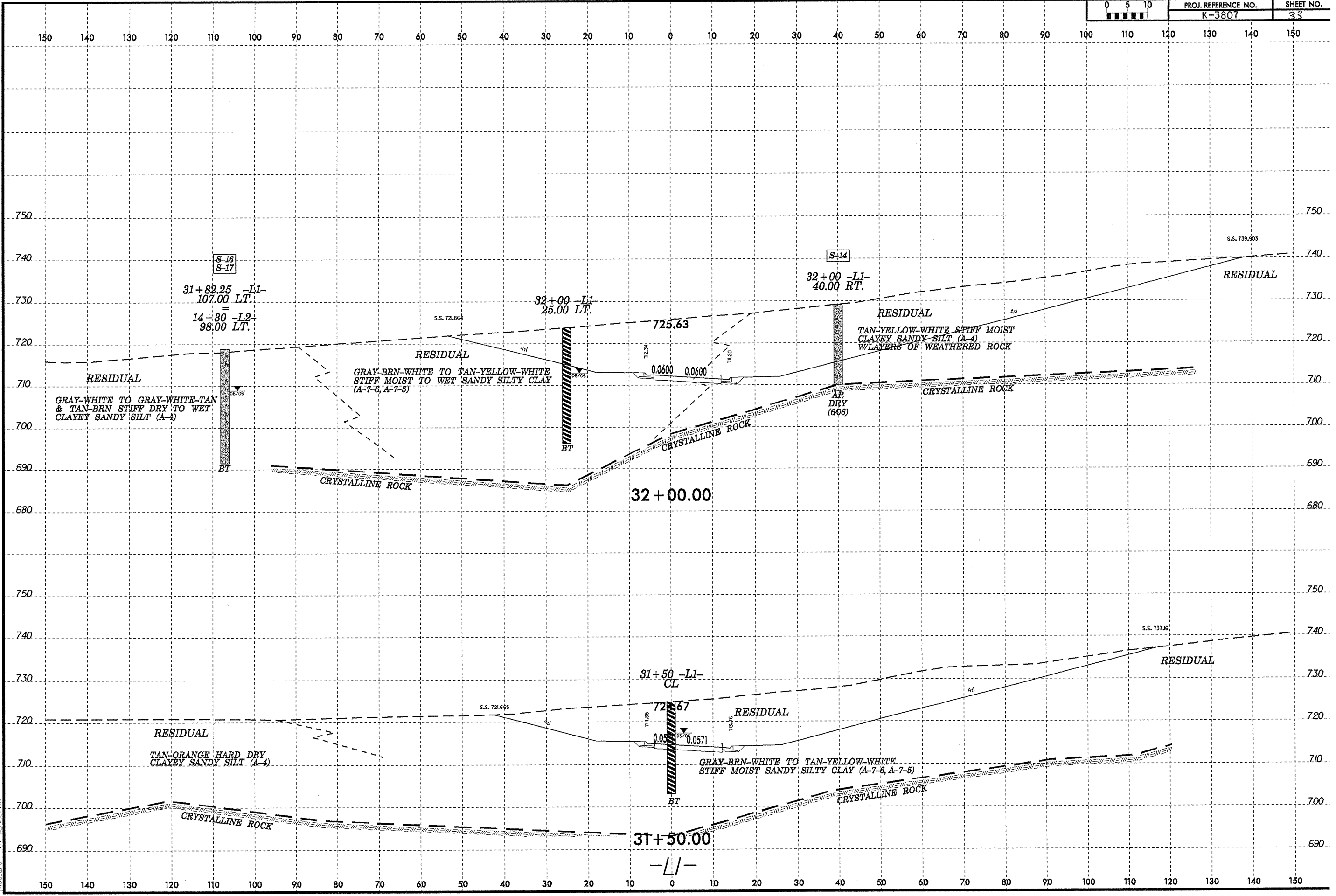


PROJ. REFERENCE NO. K-3807	SHEET NO. 32
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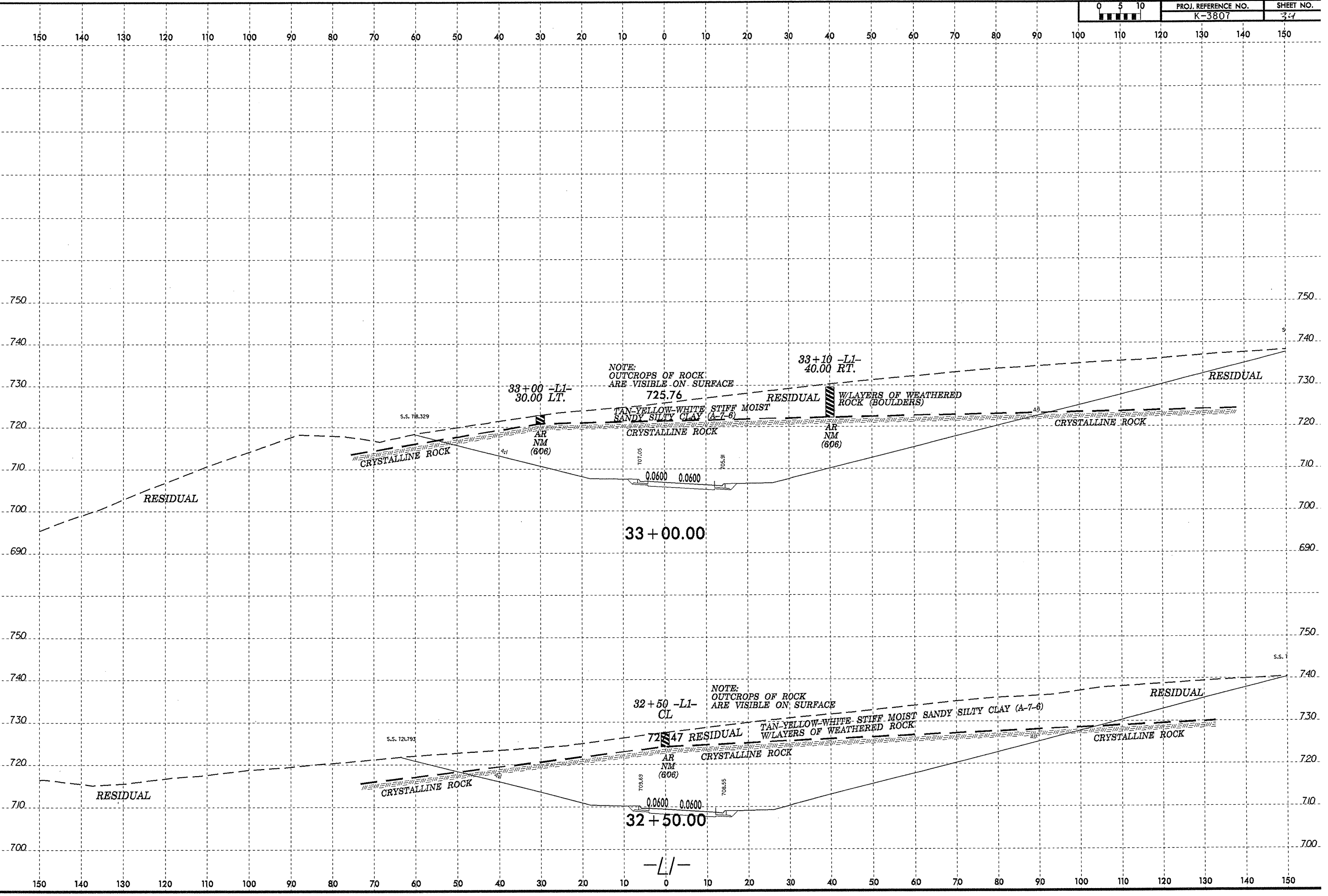
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8/23/09
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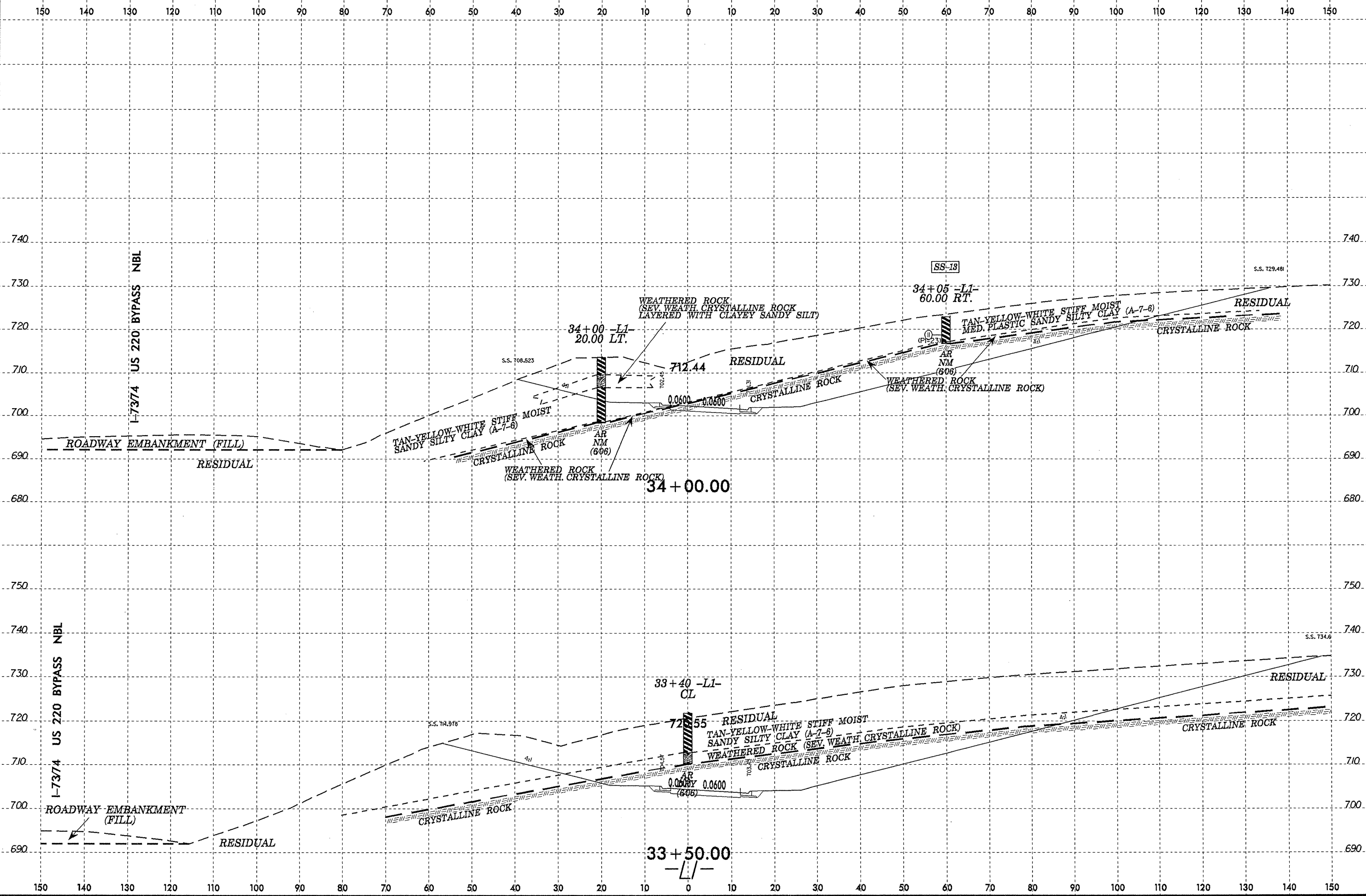


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	K-3807	34

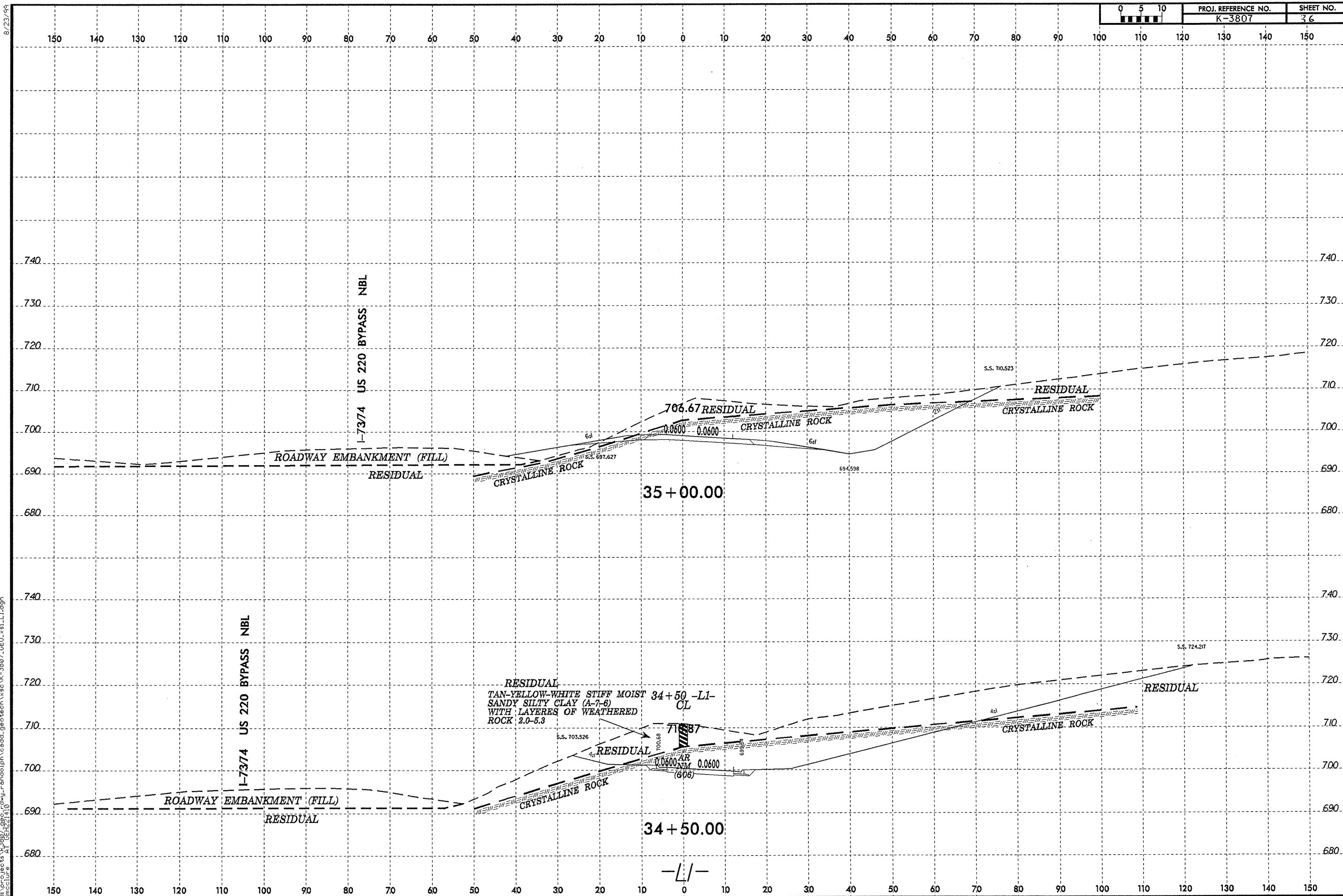
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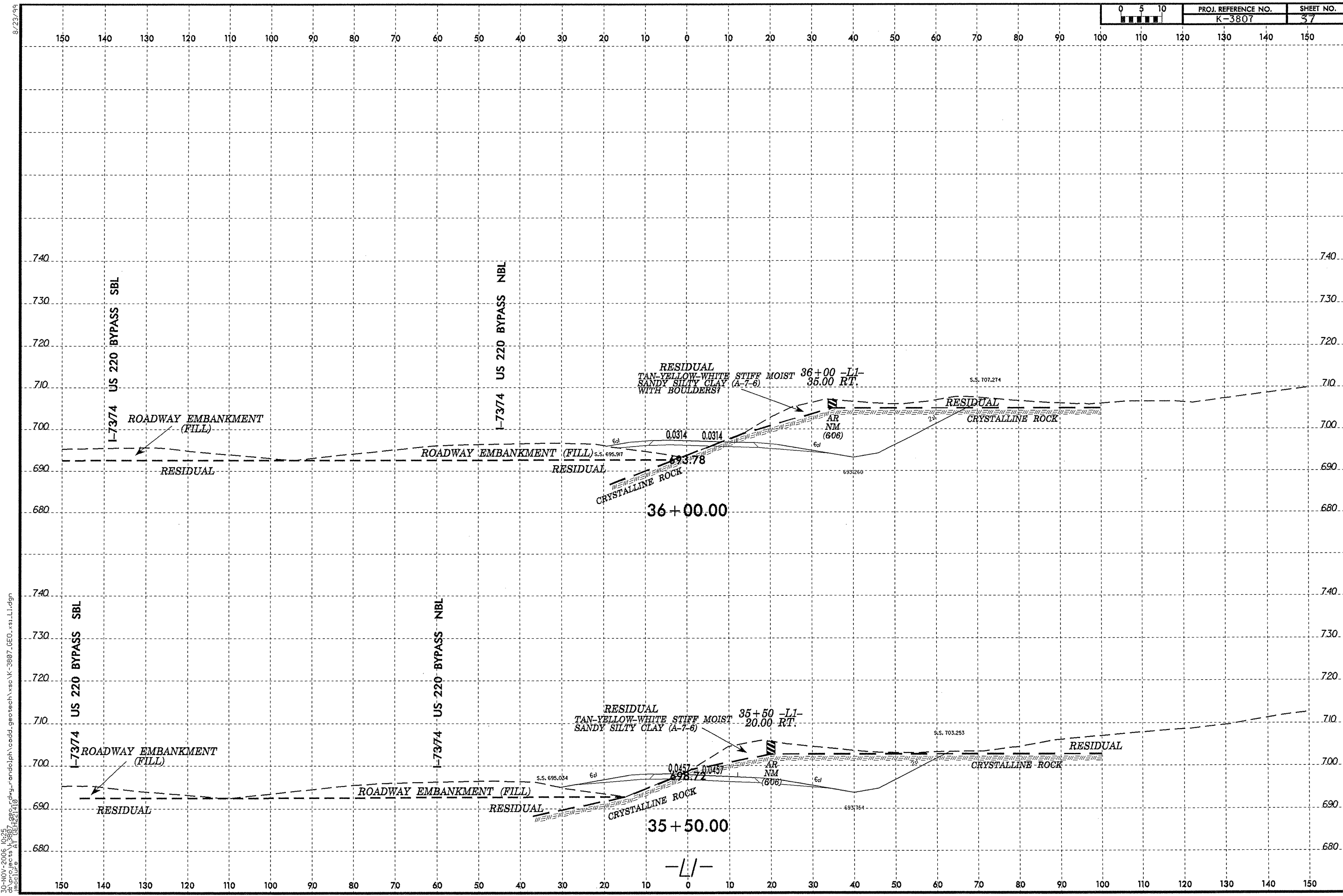
8/23/98



30-NOV-2006 10:24
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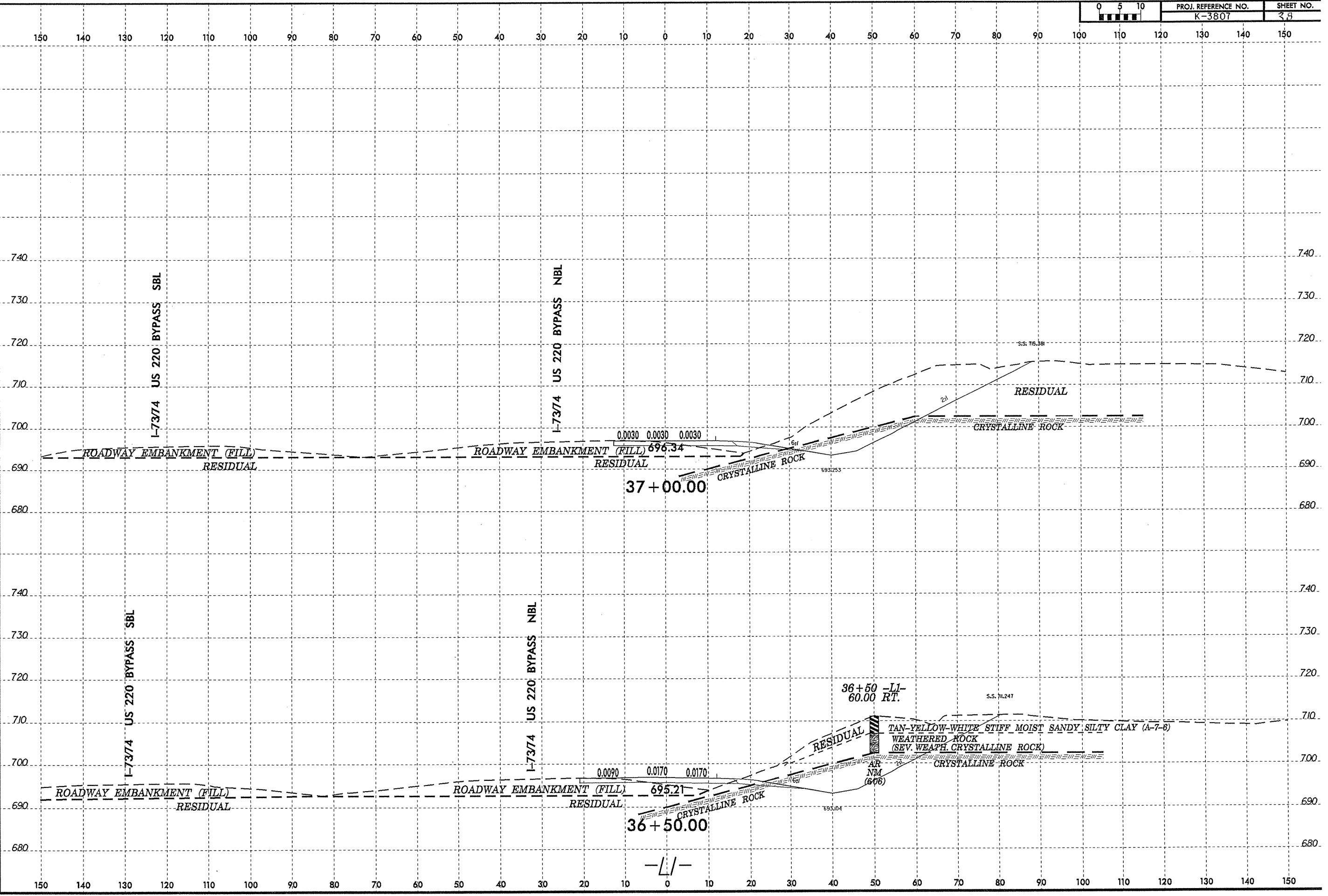
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 11-7374 US 220 BYPASS NBL



30-NOV-2006 10:55
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 10/27/06 10:55 AM
 10/27/06 10:55 AM

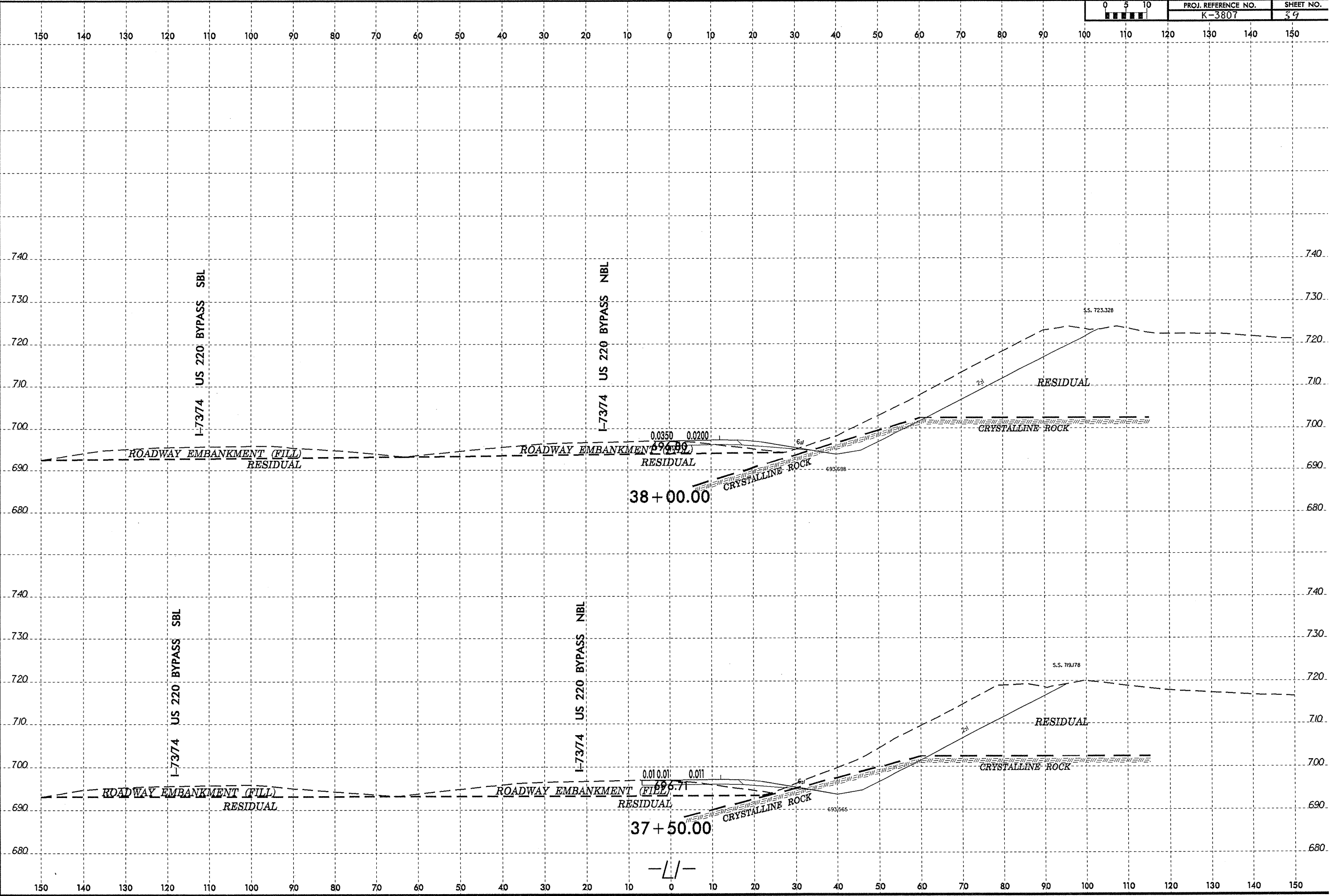
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30-NOV-2006 10:26
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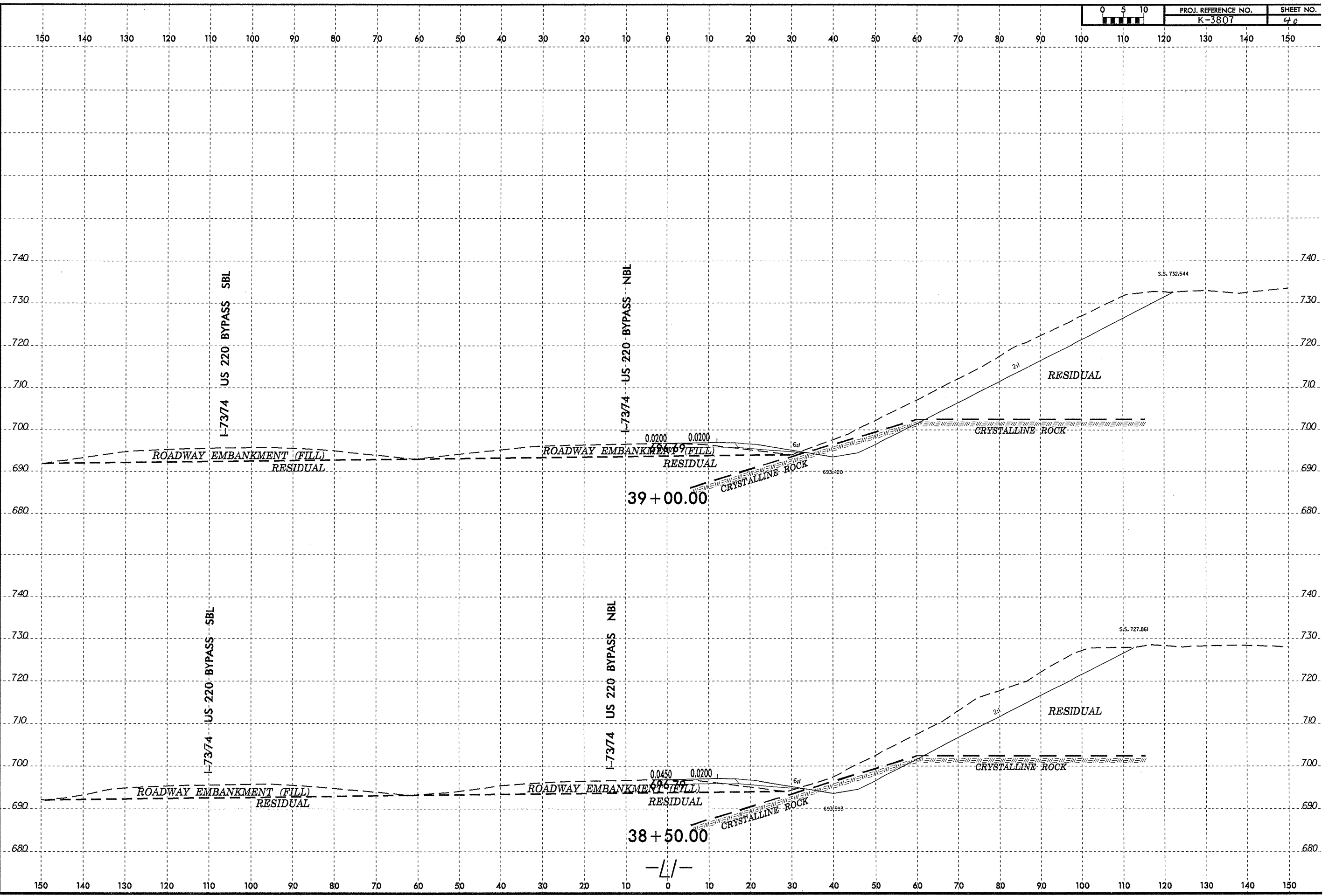
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8/23/99
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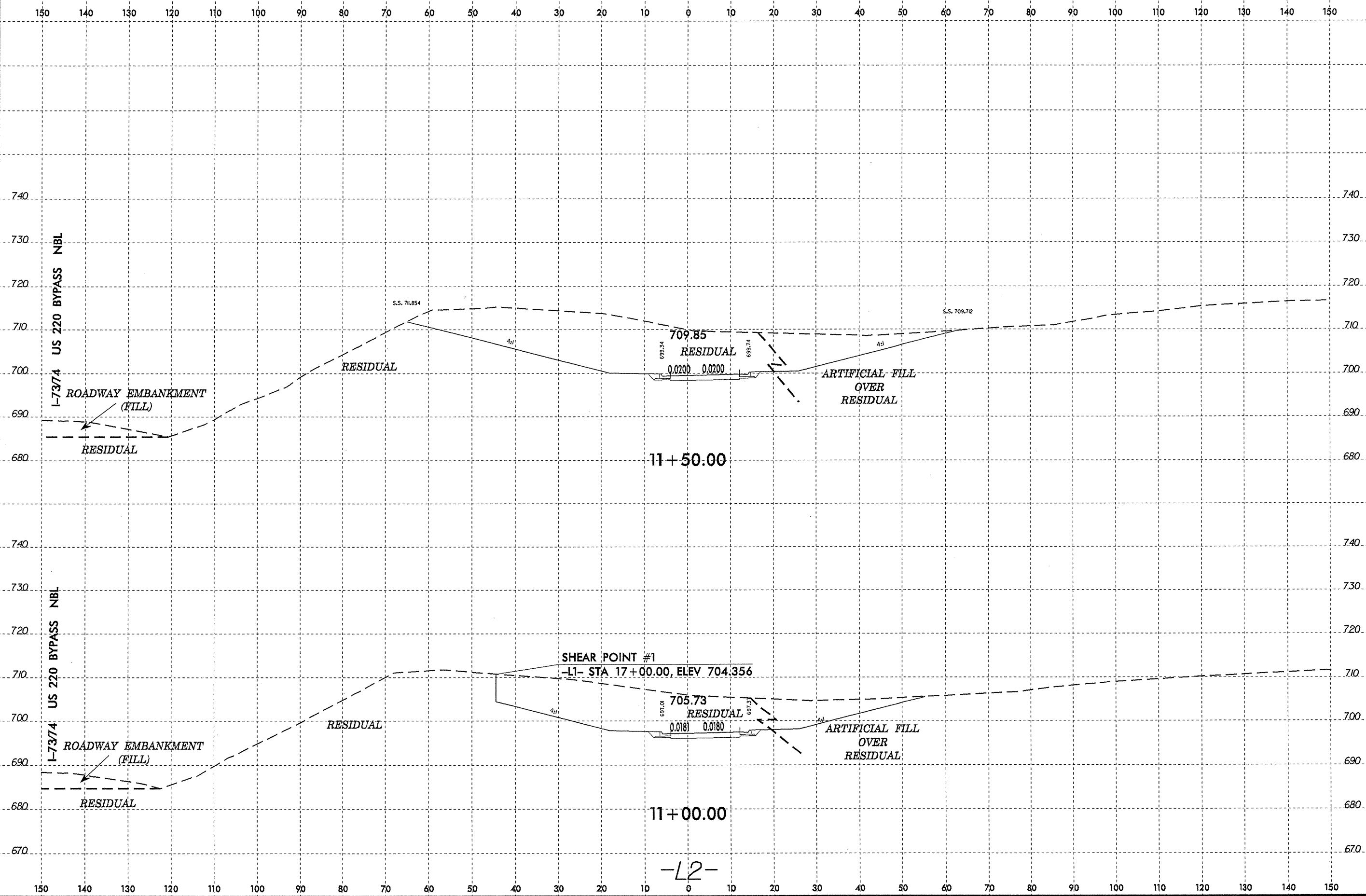
8/23/99

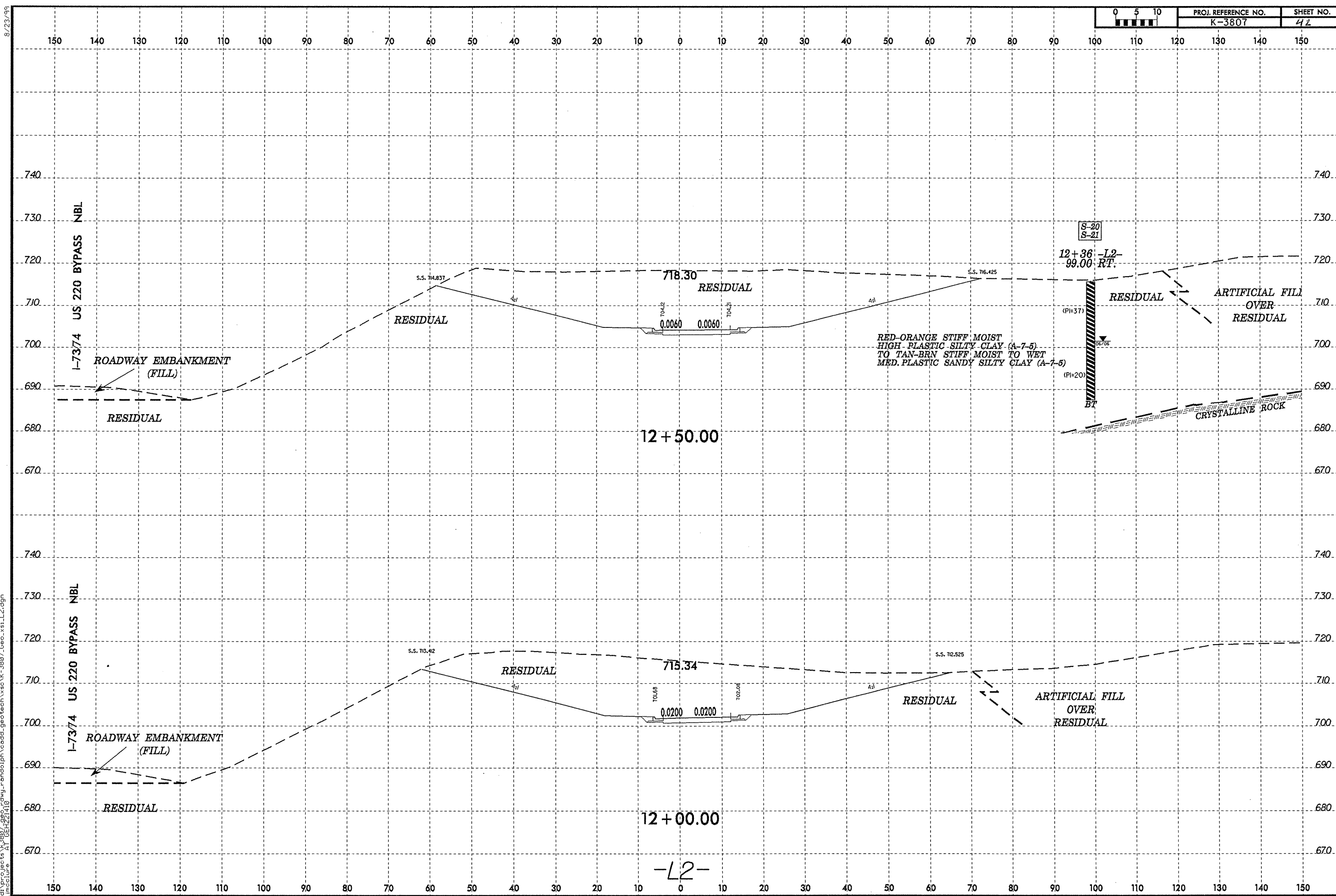
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8/23/99
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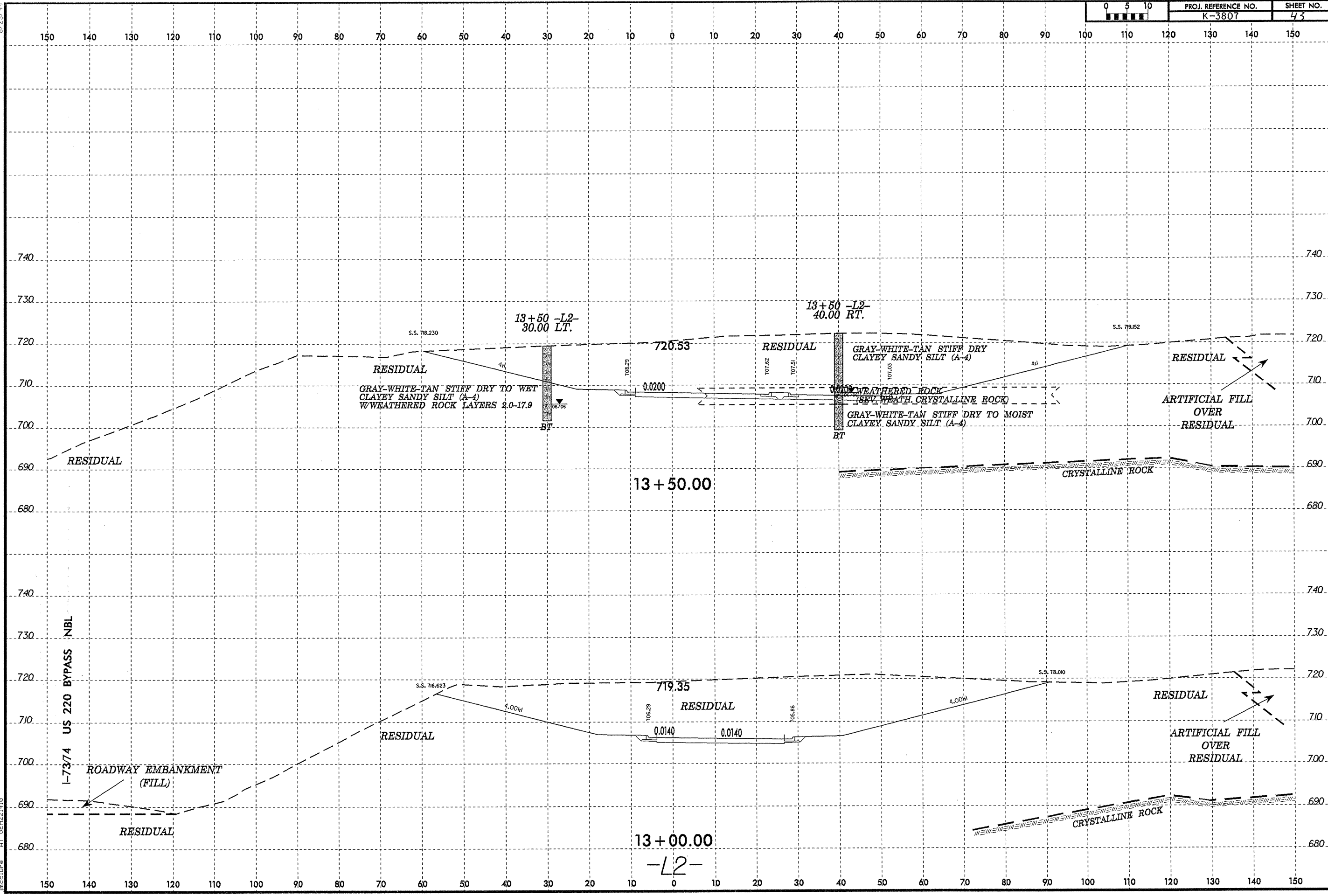
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30-Nov-2006 11:41 am
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 8/23/99

30-NOV-2006 11:42
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 8/23/99



I-73/74 US 220 BYPASS NBL

ROADWAY EMBANKMENT (FILL)

13+50 -L2- 30.00 LT.

13+50 -L2- 40.00 RT.

13+50.00

13+00.00
-L2-

RESIDUAL

RESIDUAL

RESIDUAL

ARTIFICIAL FILL OVER RESIDUAL

GRAY-WHITE-TAN STIFF DRY TO WET CLAYEY SANDY SILT (A-4)
WEATHERED ROCK LAYERS 2.0-17.9

GRAY-WHITE-TAN STIFF DRY CLAYEY SANDY SILT (A-4)

WEATHERED ROCK (SEV. WEATH. CRYSTALLINE ROCK)

GRAY-WHITE-TAN STIFF DRY TO MOIST CLAYEY SANDY SILT (A-4)

CRYSTALLINE ROCK

CRYSTALLINE ROCK

S.S. 718.230

S.S. 719.052

S.S. 716.623

S.S. 719.010

719.35

RESIDUAL

RESIDUAL

ARTIFICIAL FILL OVER RESIDUAL

0.0140

0.0140

4.00H

4.00H

720.53

0.0200

707.62

707.51

707.03

4H

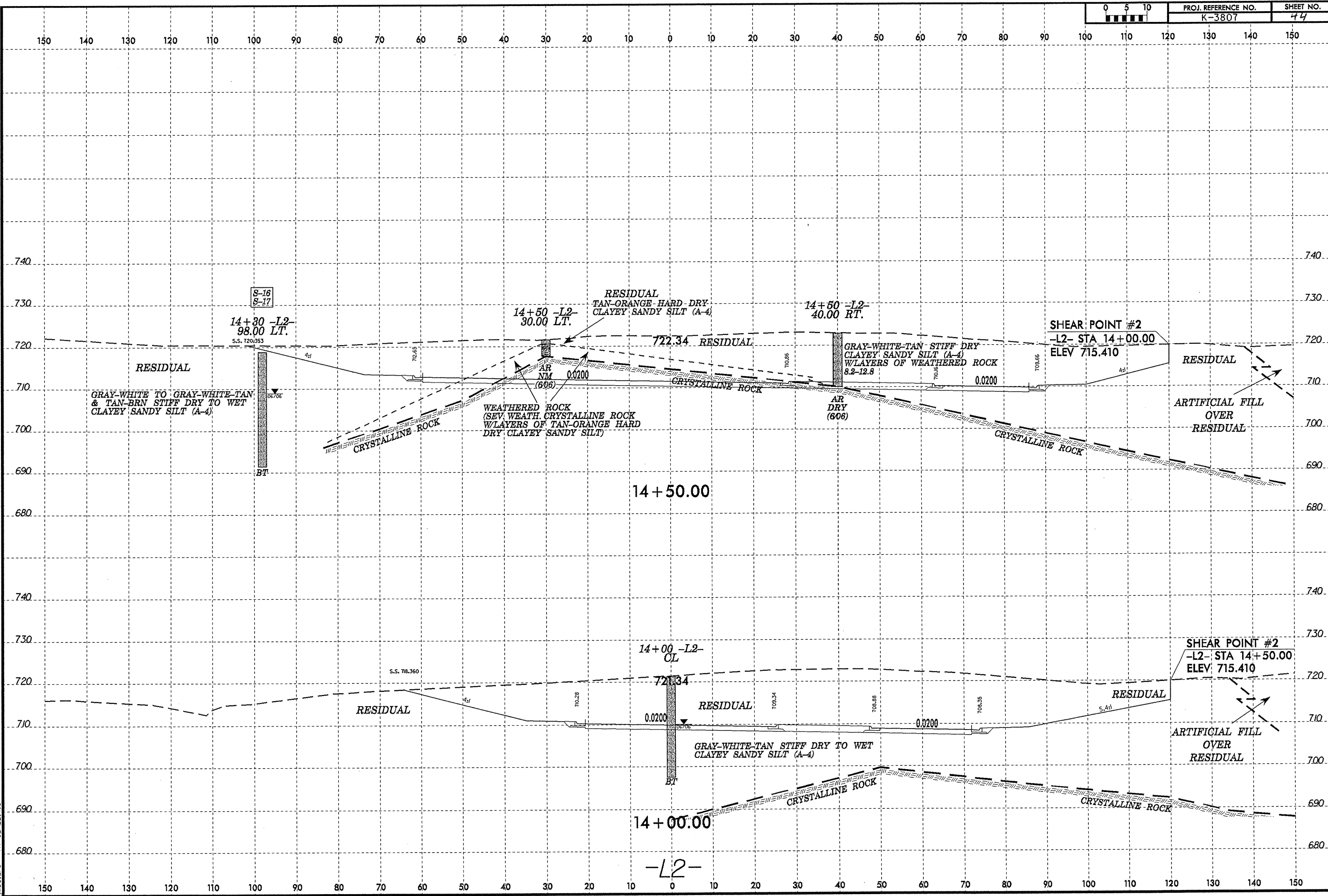
4H

BT

BT

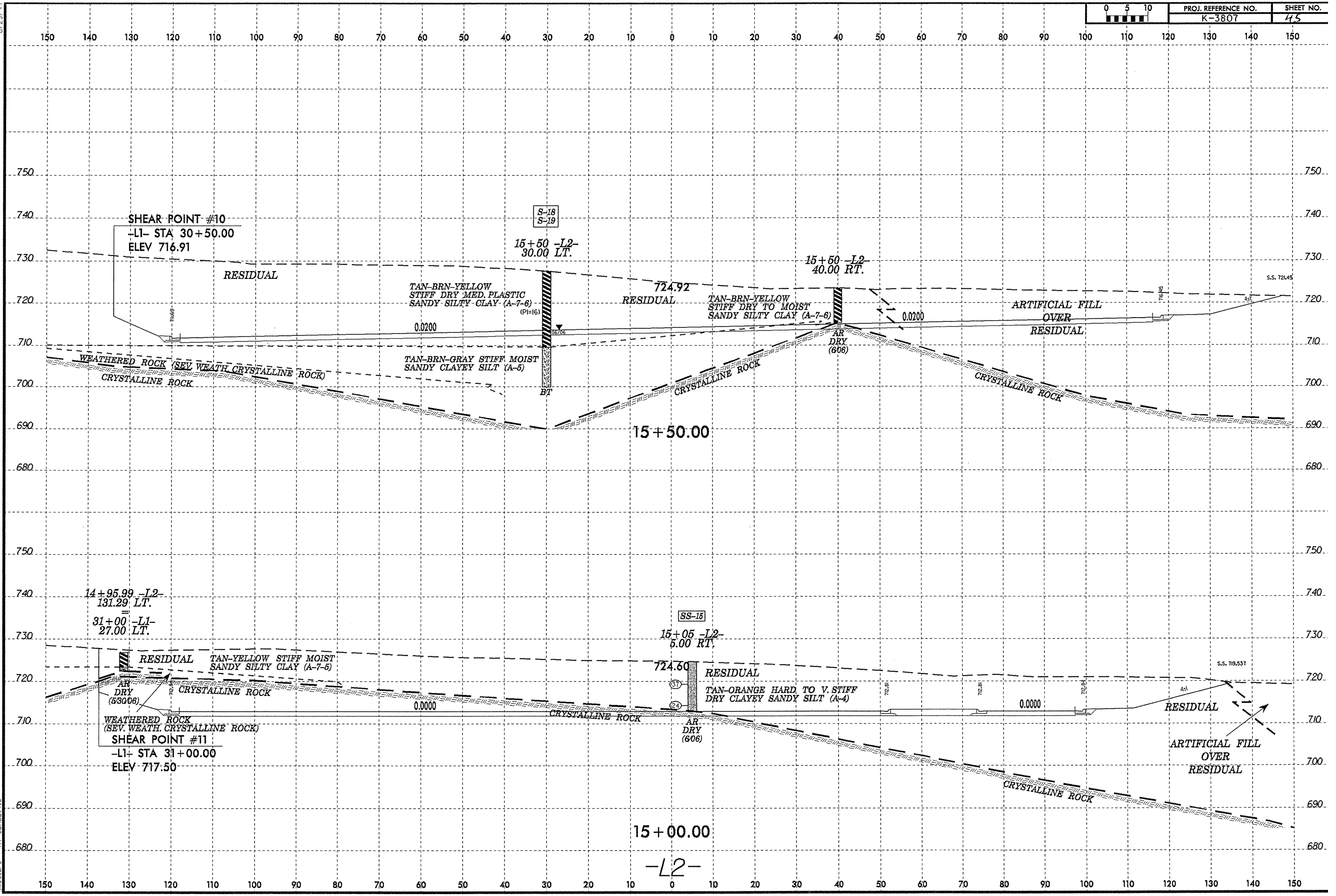
8/23/99

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13555.dwg
13555.dwg
13555.dwg



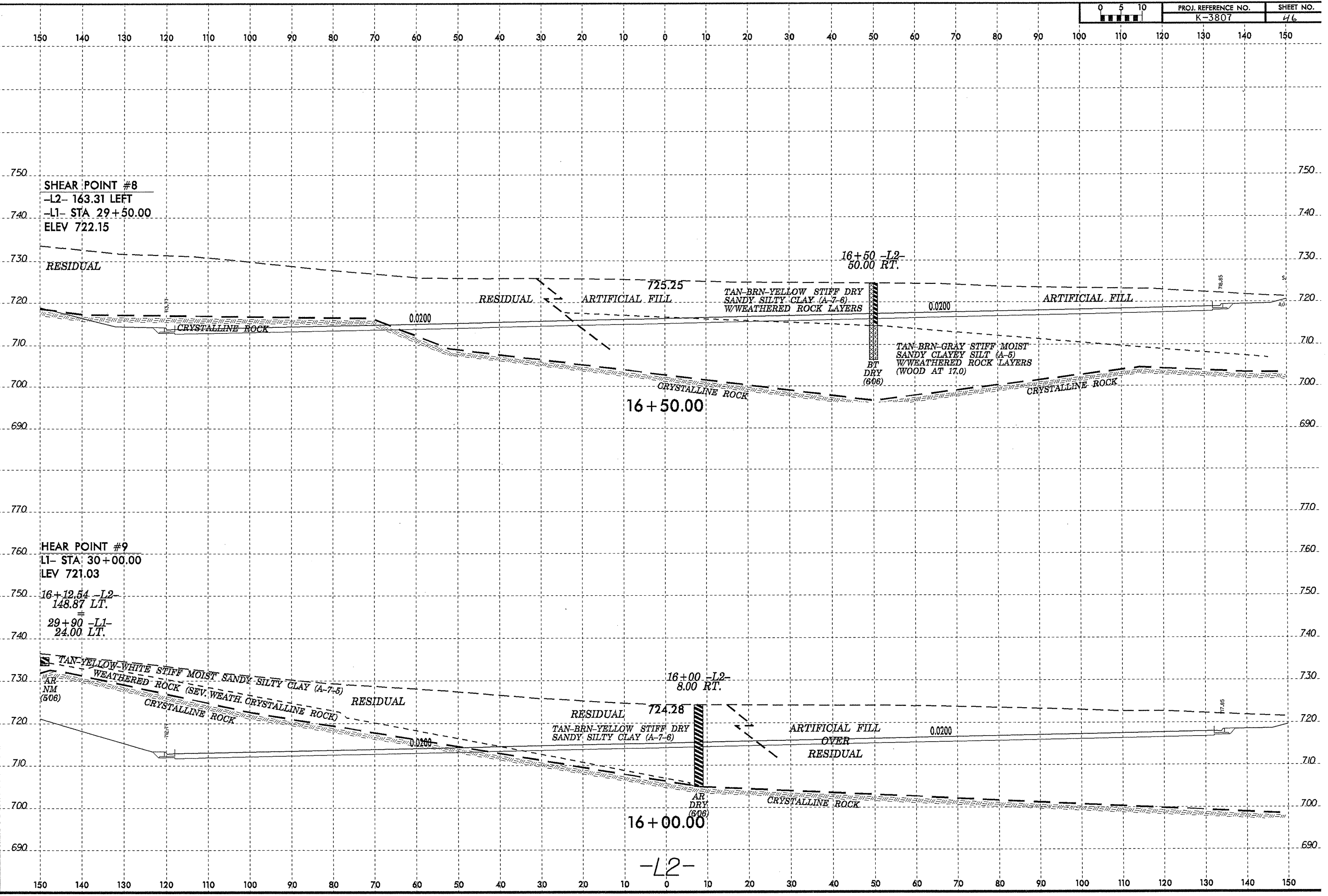
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8/23/99
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 11:45
 11:45

8/23/99
30-NOV-2006 11:43
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module: AI_061221.dwg
Geo_xsl.L2.dgn



SHEAR POINT #8
 -L2- 163.31 LEFT
 -L1- STA 29+50.00
 ELEV 722.15

16+50 -L2-
50.00 RT.

HEAR POINT #9
 L1- STA 30+00.00
 LEV 721.03

16+12.54 -L2-
148.87 LT.
=

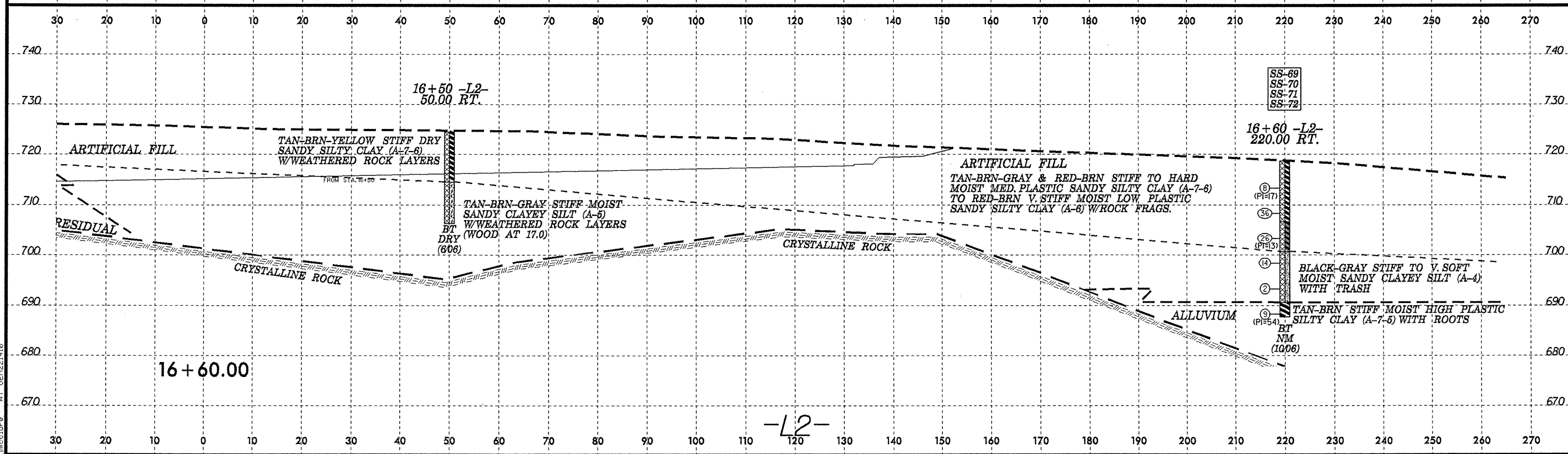
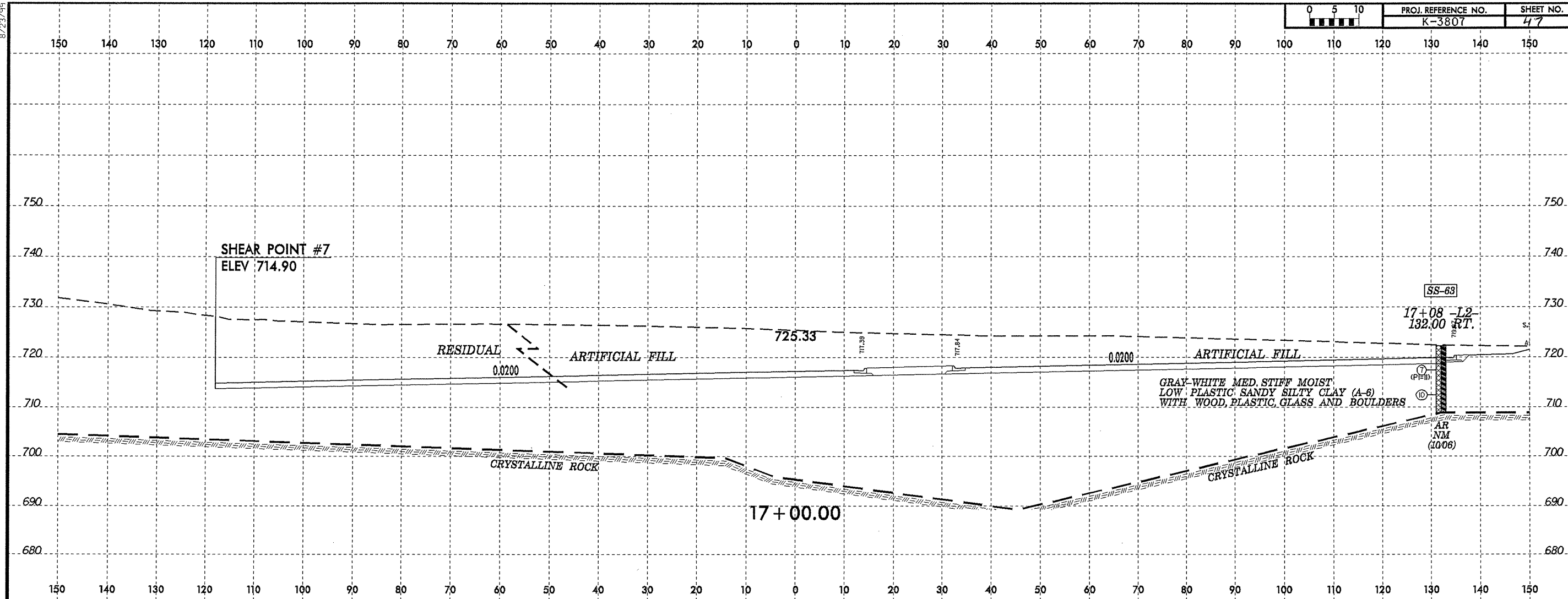
29+90 -L1-
24.00 LT.

16+50.00

16+00 -L2-
8.00 RT.

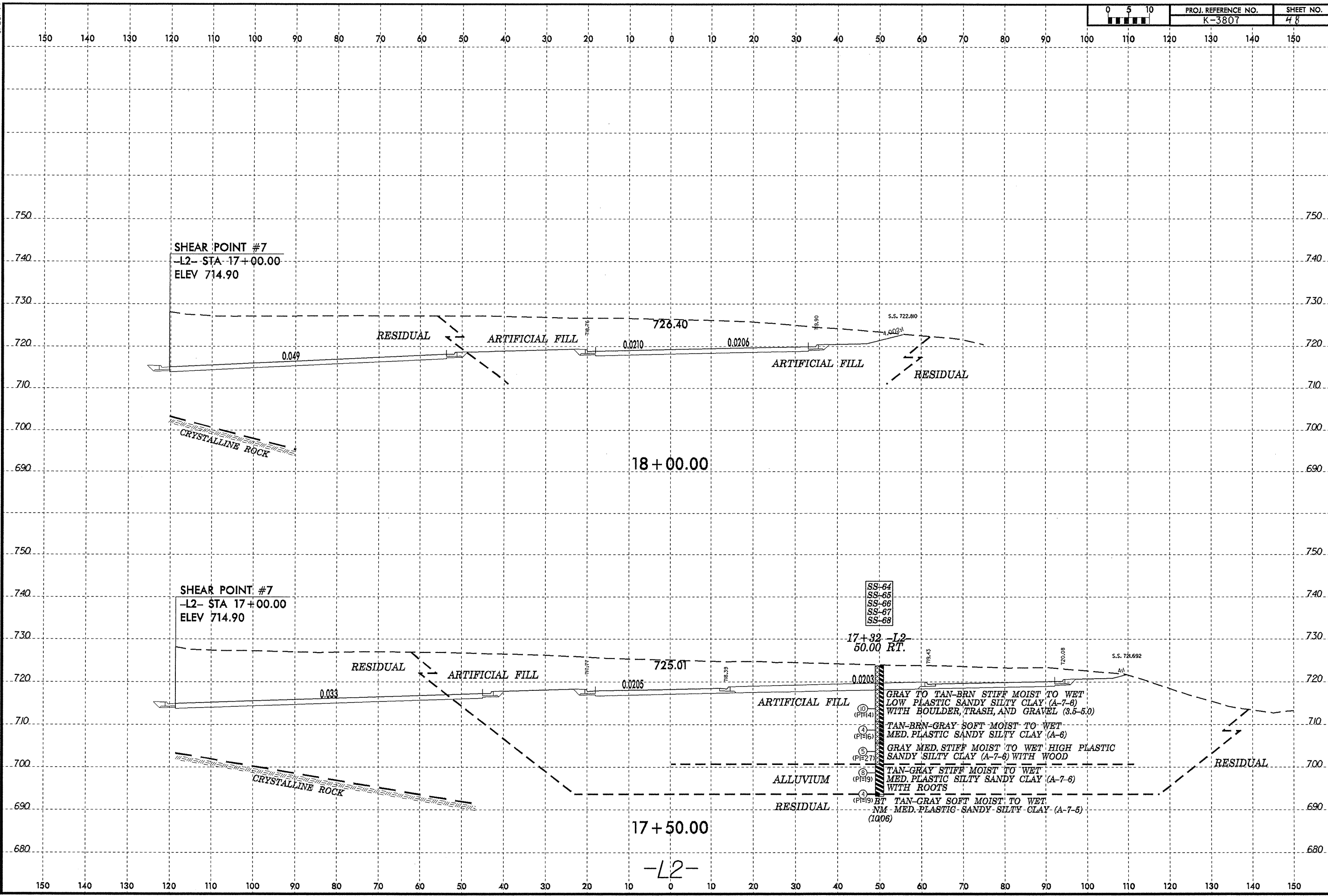
16+00.00

-L2-



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 10/27/06 10:07 AM
 8/23/06

0 5 10	PROJ. REFERENCE NO. K-3807	SHEET NO. 48
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SHEAR POINT #7
-L2- STA 17+00.00
ELEV 714.90

SHEAR POINT #7
-L2- STA 17+00.00
ELEV 714.90

SS-64
SS-65
SS-66
SS-67
SS-68

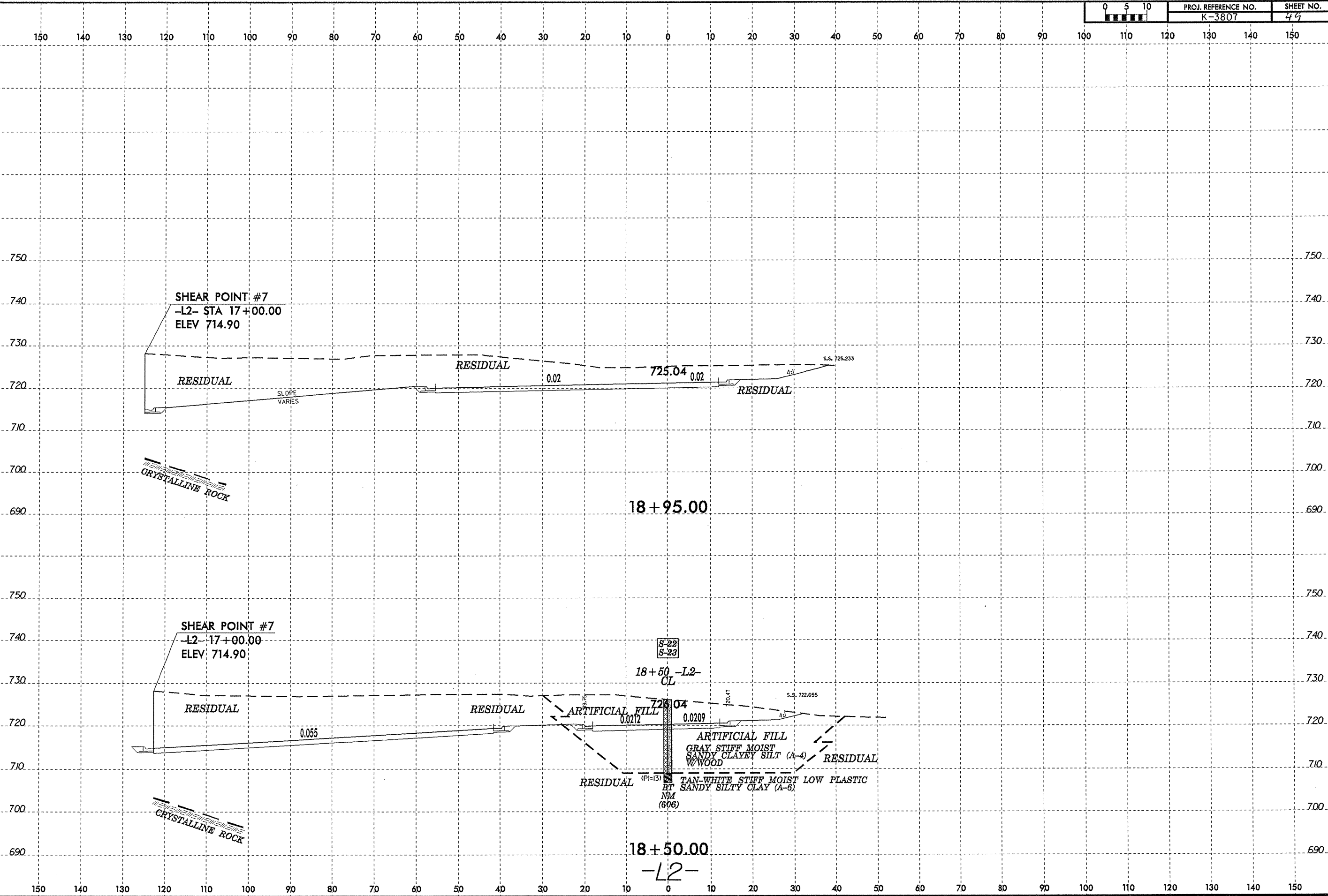
17+32 -L2
50.00 RT.

- (10) GRAY TO TAN-BRN STIFF MOIST TO WET LOW PLASTIC SANDY SILTY CLAY (A-7-8) WITH BOULDER, TRASH, AND GRAVEL (3.5-5.0)
- (9) TAN-BRN-GRAY SOFT MOIST TO WET MED. PLASTIC SANDY SILTY CLAY (A-8)
- (8) GRAY MED. STIFF MOIST TO WET HIGH PLASTIC SANDY SILTY CLAY (A-7-6) WITH WOOD
- (7) TAN-GRAY STIFF MOIST TO WET MED. PLASTIC SILTY SANDY CLAY (A-7-6) WITH ROOTS
- (6) TAN-GRAY SOFT MOIST TO WET MED. PLASTIC SANDY SILTY CLAY (A-7-5)
- (5) BT (1006)

8/23/99
 18-JAN-2007 08:45
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 include AT GEH221410

8/23/99

30-NOV-2006 11:45
d:\cadd\p\h\cadd\geotech\k-3807\Geo_xsi_L2.dgn



SHEAR POINT #7
 -L2- STA 17+00.00
 ELEV 714.90

RESIDUAL

SLOPE VARIES

RESIDUAL

0.02

725.04

0.02

RESIDUAL

S.S. 725.233

CRYSTALLINE ROCK

18+95.00

SHEAR POINT #7
 -L2- STA 17+00.00
 ELEV 714.90

RESIDUAL

0.055

RESIDUAL

ARTIFICIAL FILL

0.0212

728.04

0.0209

RESIDUAL

S.S. 722.655

ARTIFICIAL FILL
 GRAY STIFF MOIST
 SANDY CLAYEY SILT (A-4)
 W/ WOOD

RESIDUAL
 TAN-WHITE STIFF MOIST LOW PLASTIC
 SANDY SILTY CLAY (A-6)

(PI=13)

NM

(606)

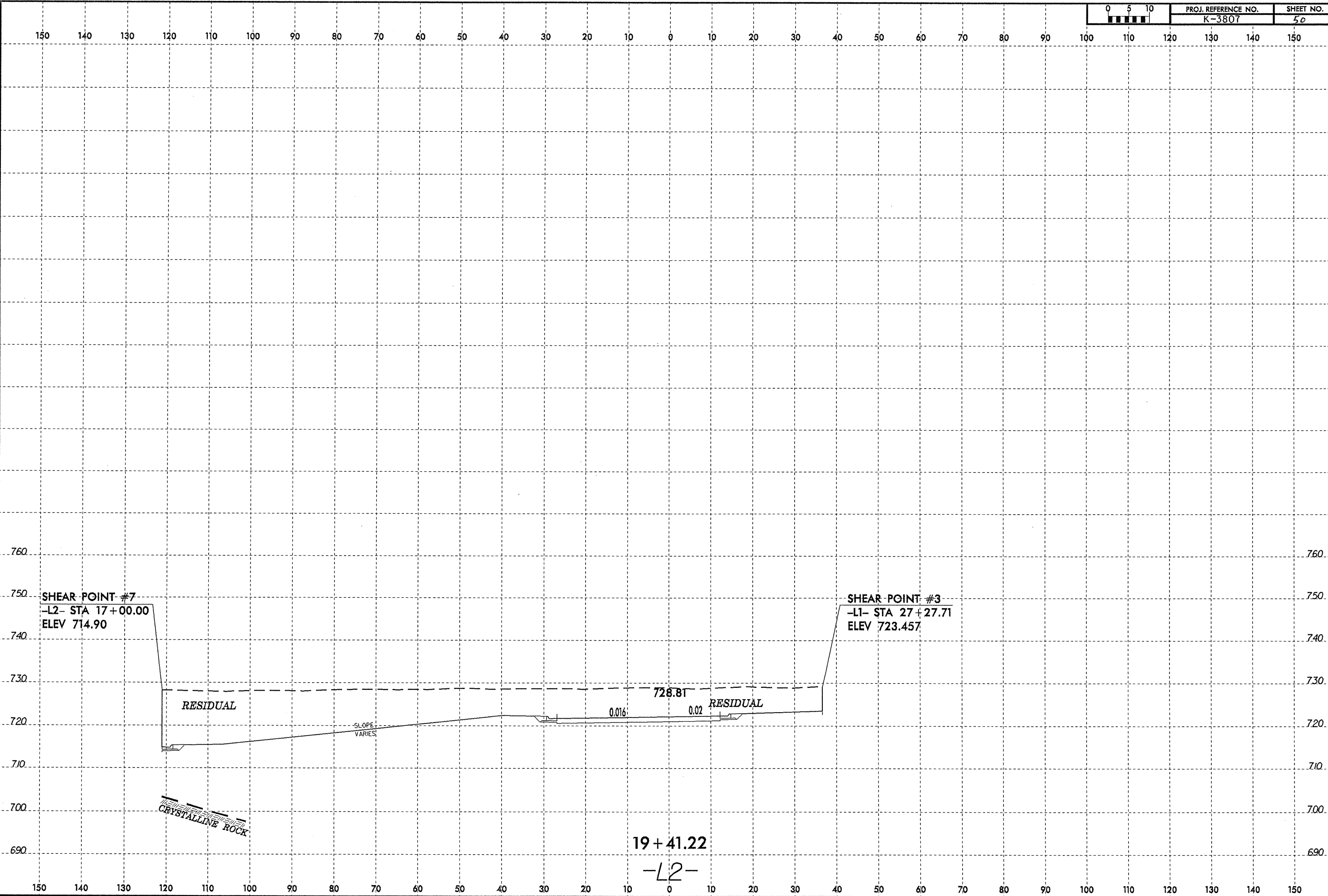
CRYSTALLINE ROCK

18+50.00

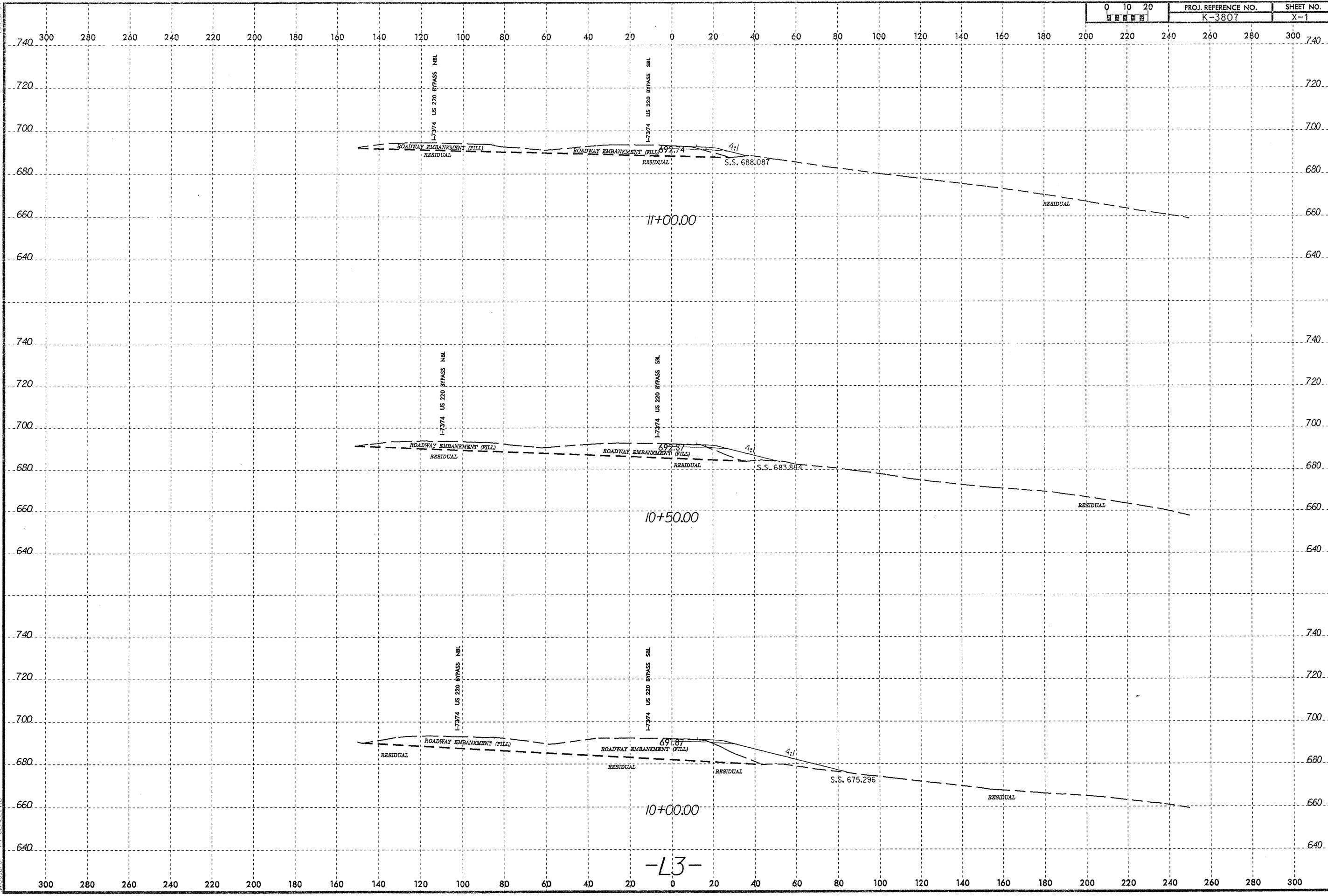
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8/23/19
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30-NOV-2006 11:45
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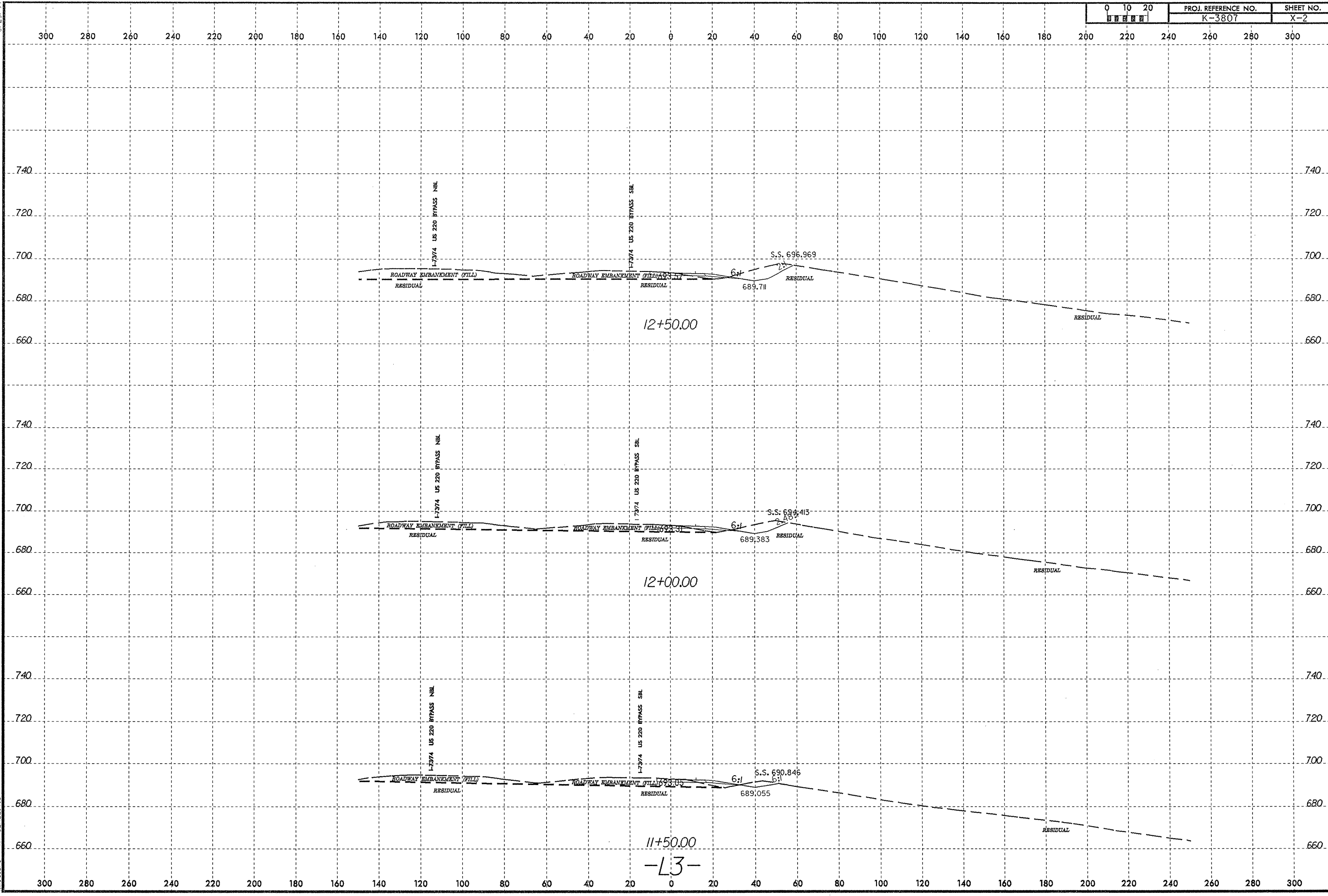
19+41.22
-L2-



-L3-

0 10 20	PROJ. REFERENCE NO.	SHEET NO.
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8/23/99
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 11/22/10
 mcdclure



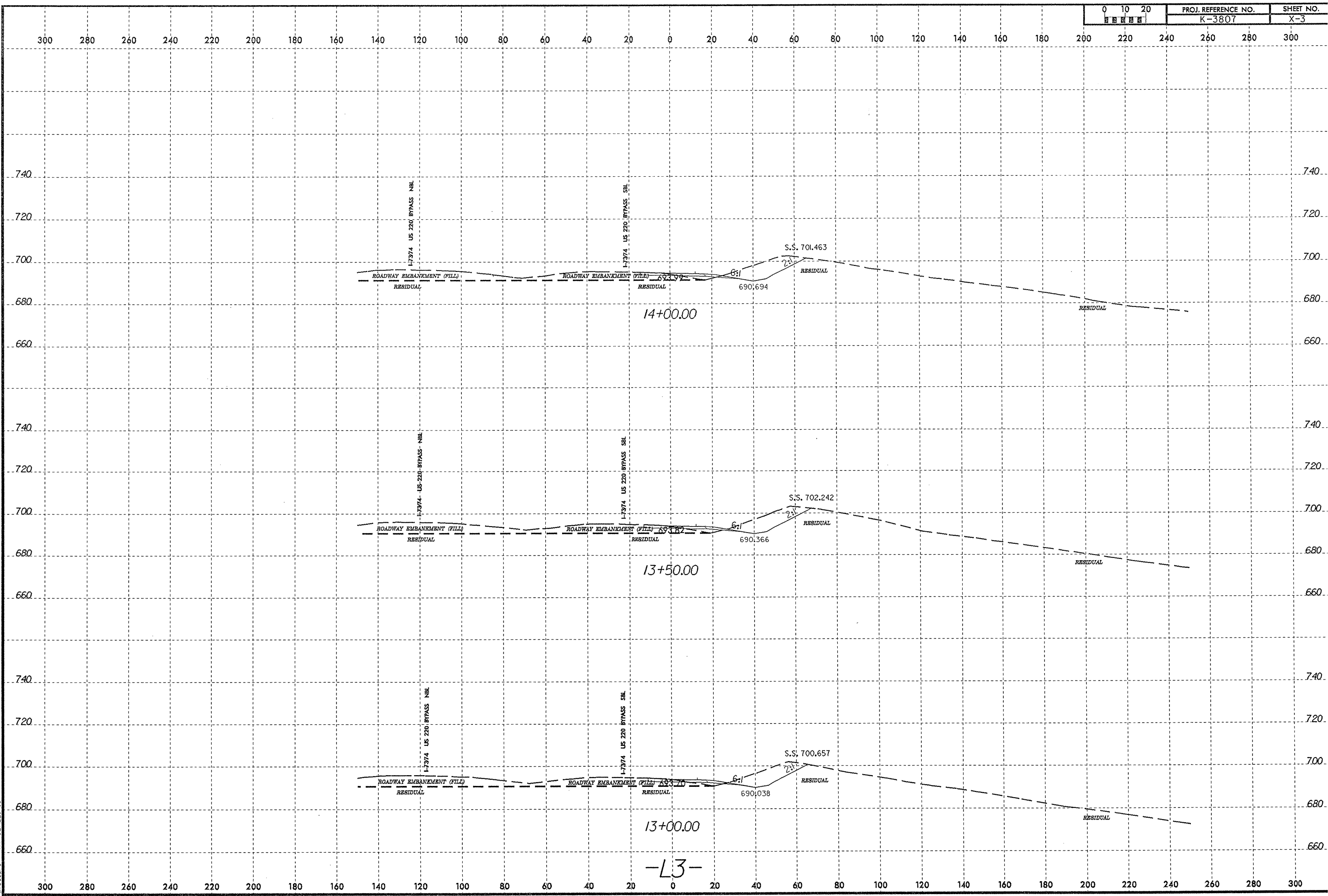
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12+00.00

11+50.00

-L3-

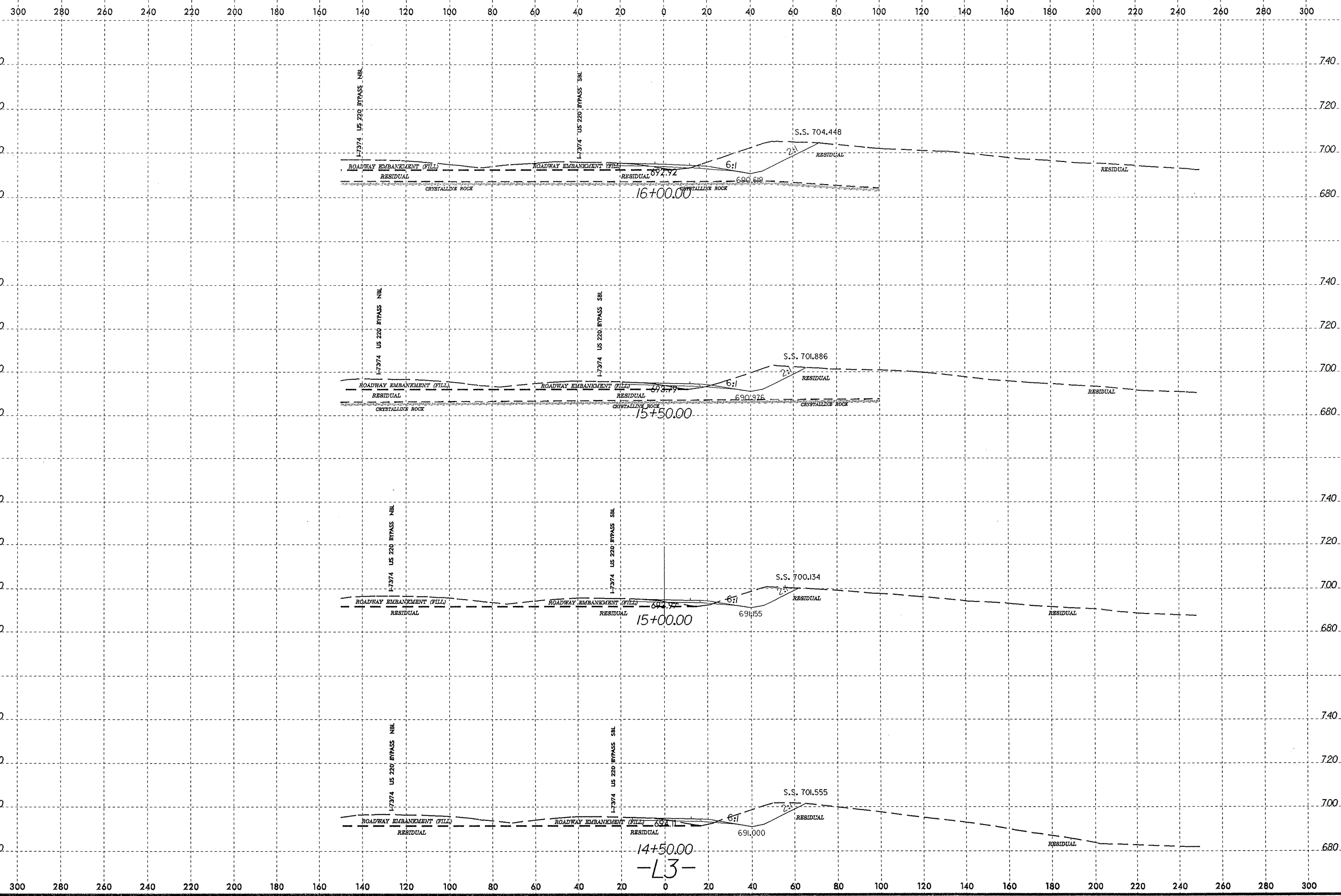
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	K-3807	X-3



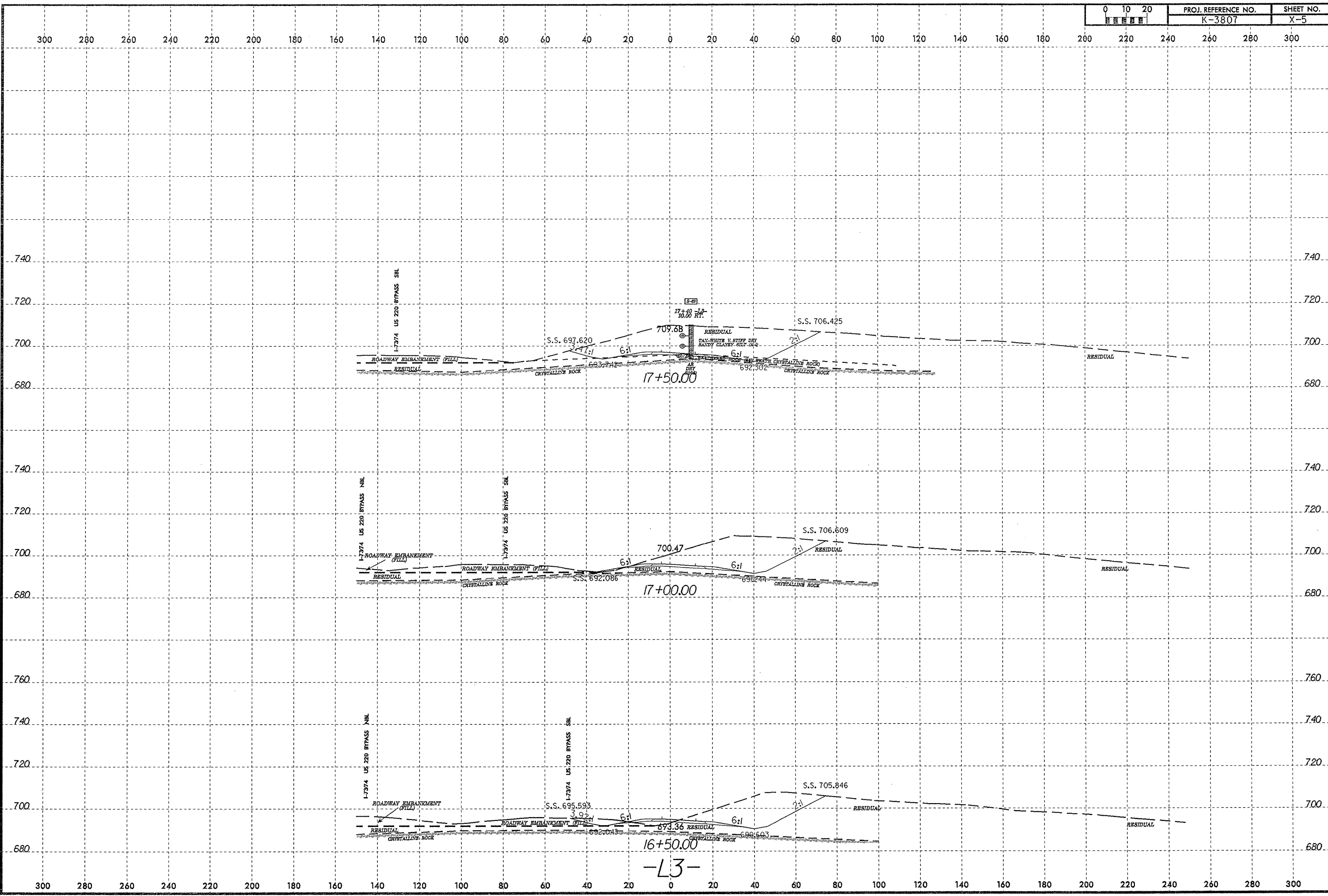
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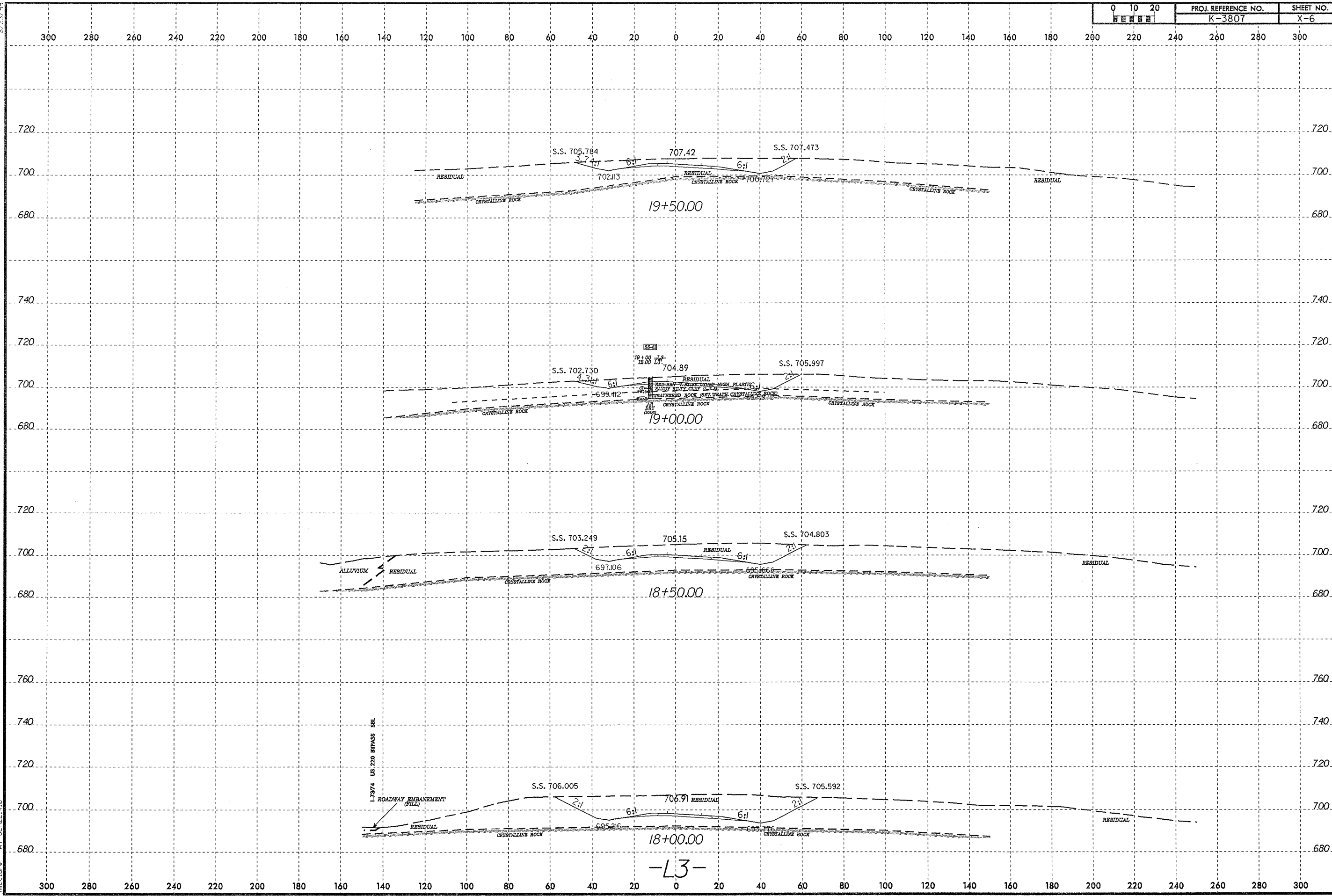
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-L3-



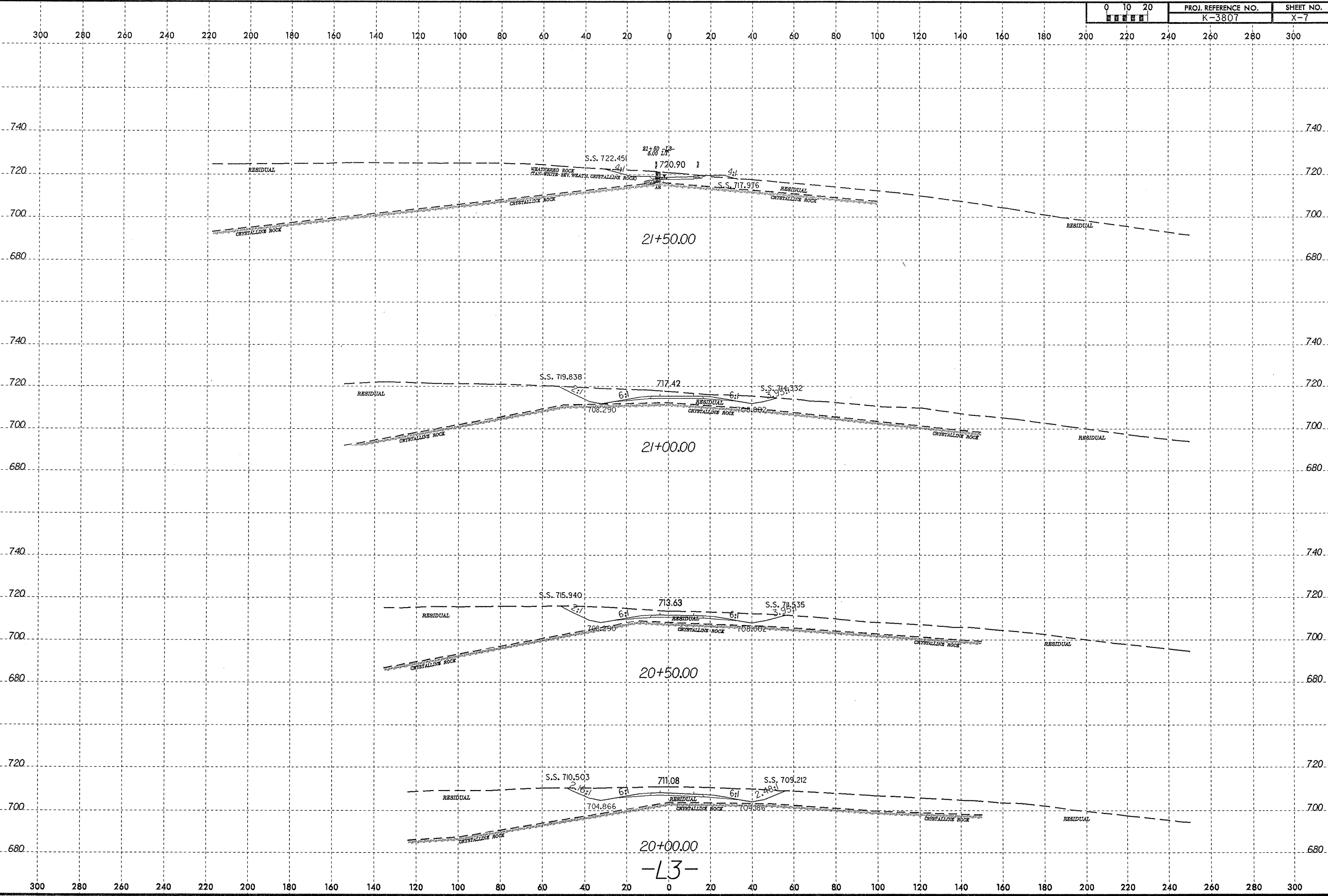
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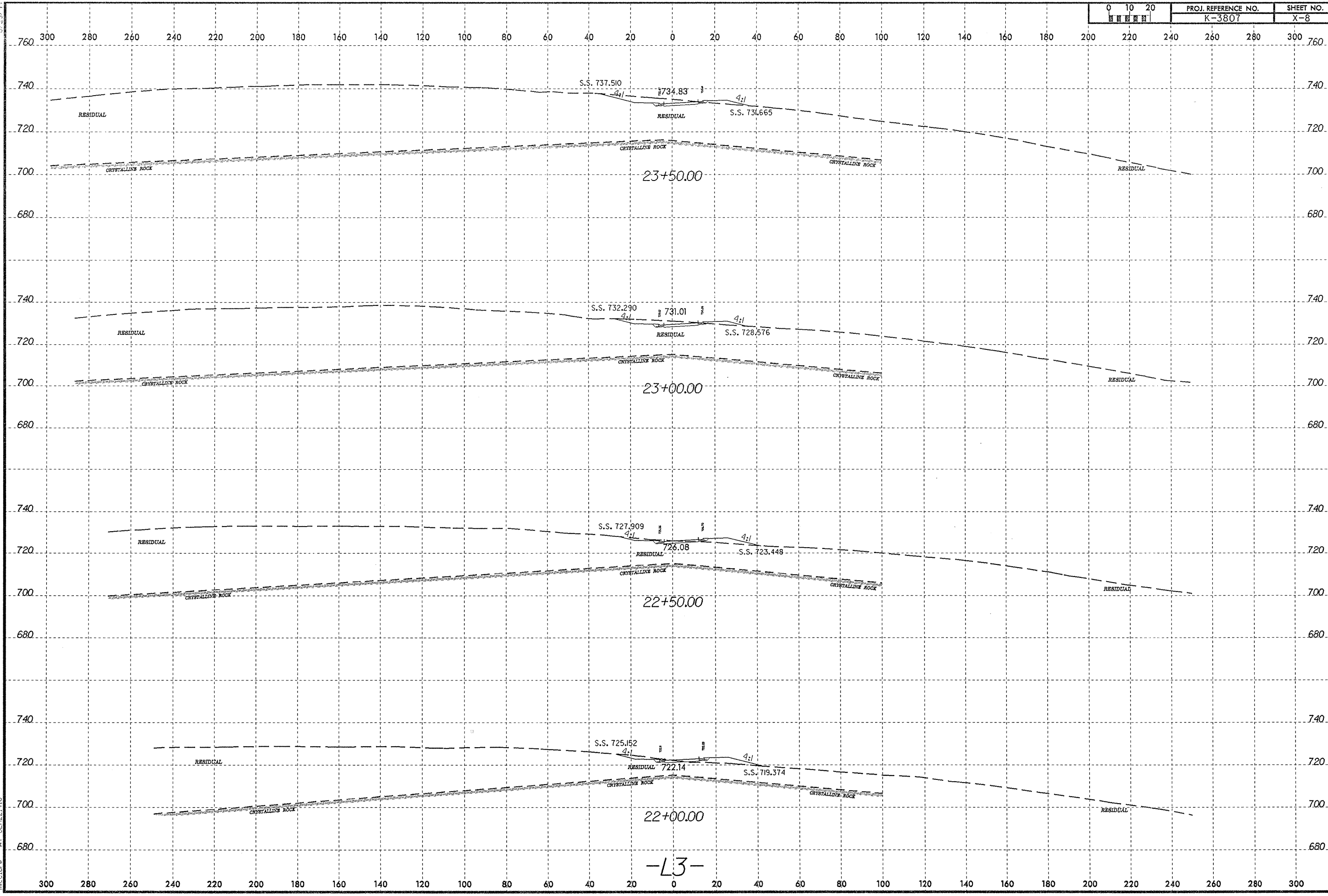
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8/23/99
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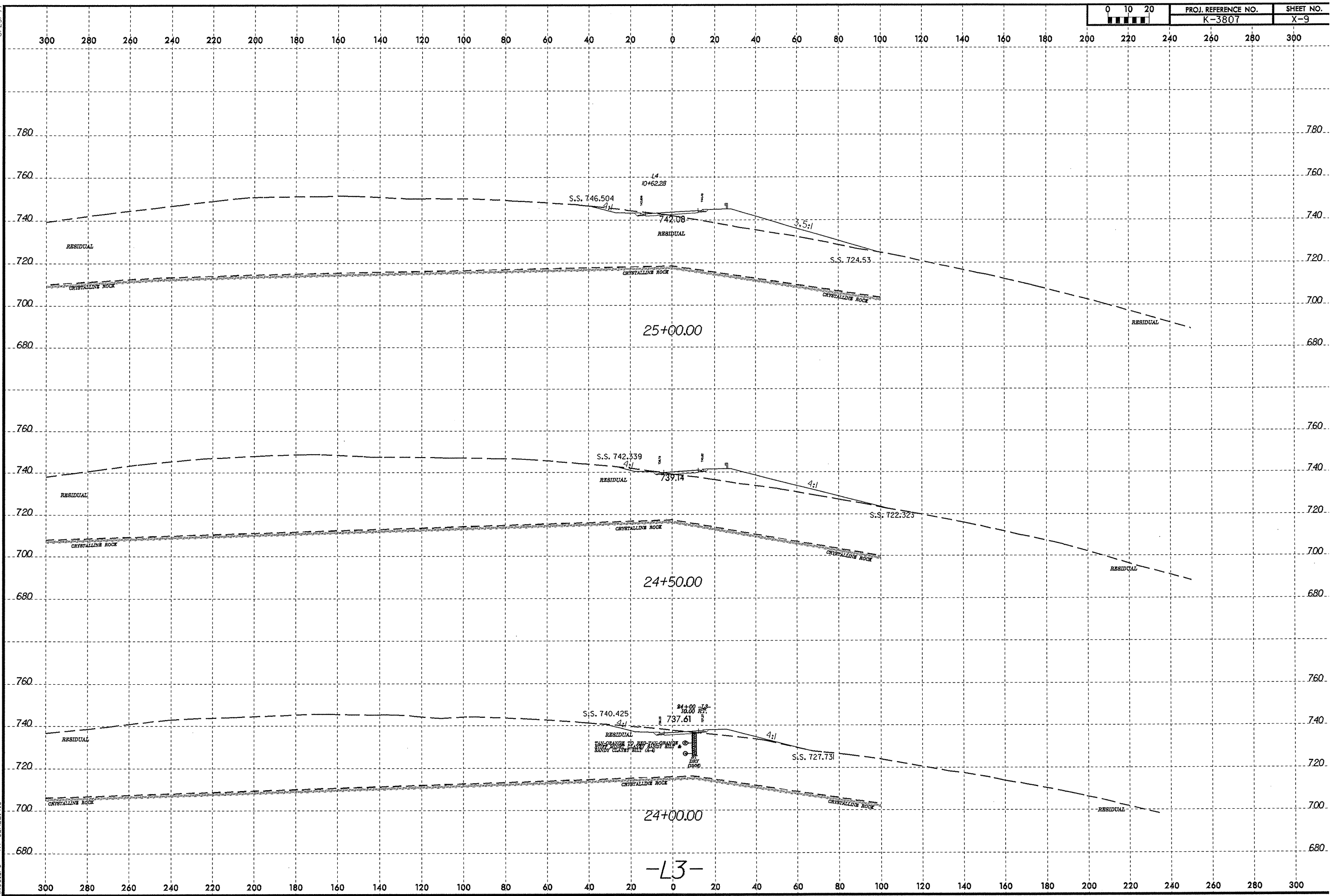


-L3-



-L3-

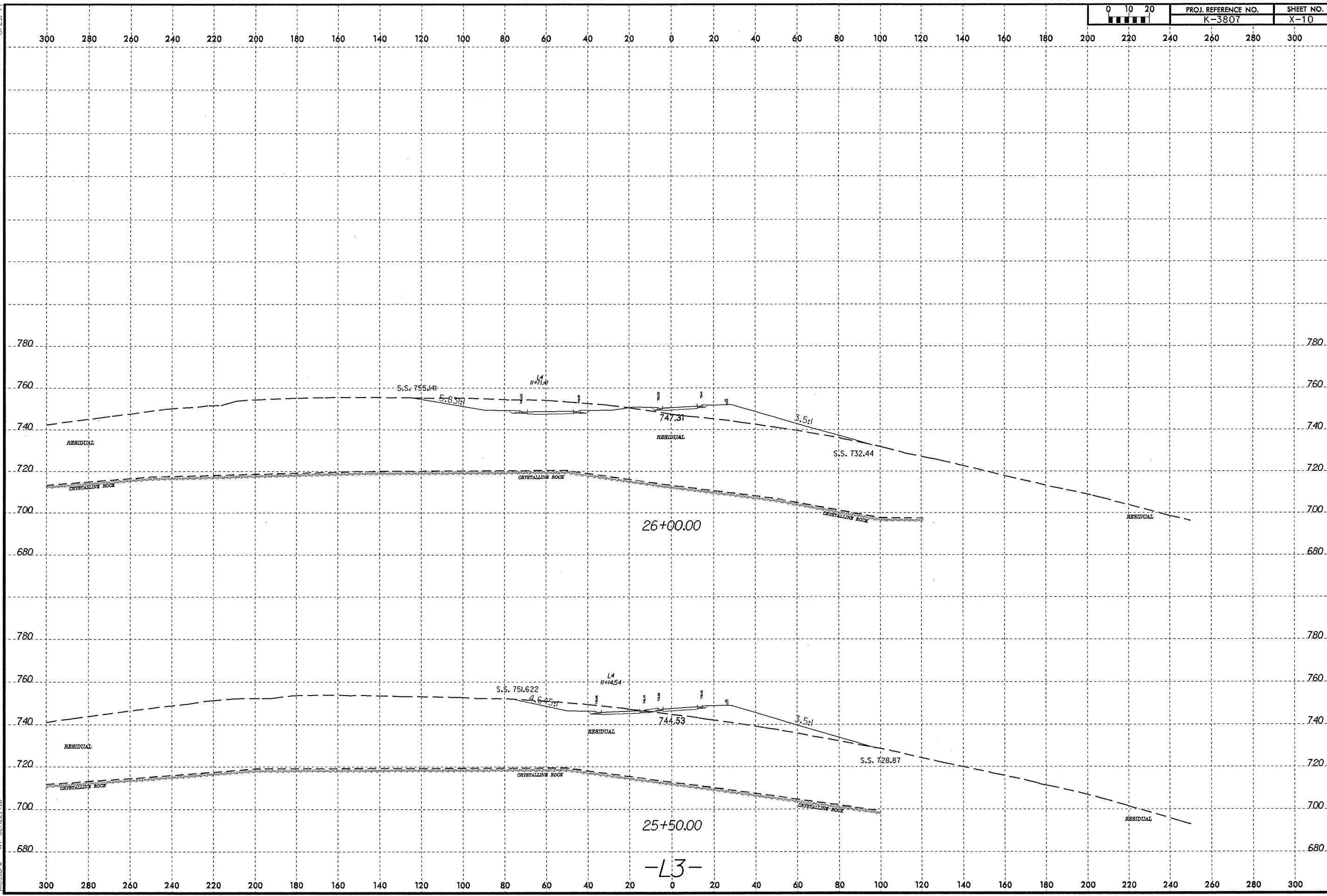
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 22-DEC-2006 09:20
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-L3-

09-JAN-2007 10:05
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0 10 20	PROJ. REFERENCE NO.	SHEET NO.
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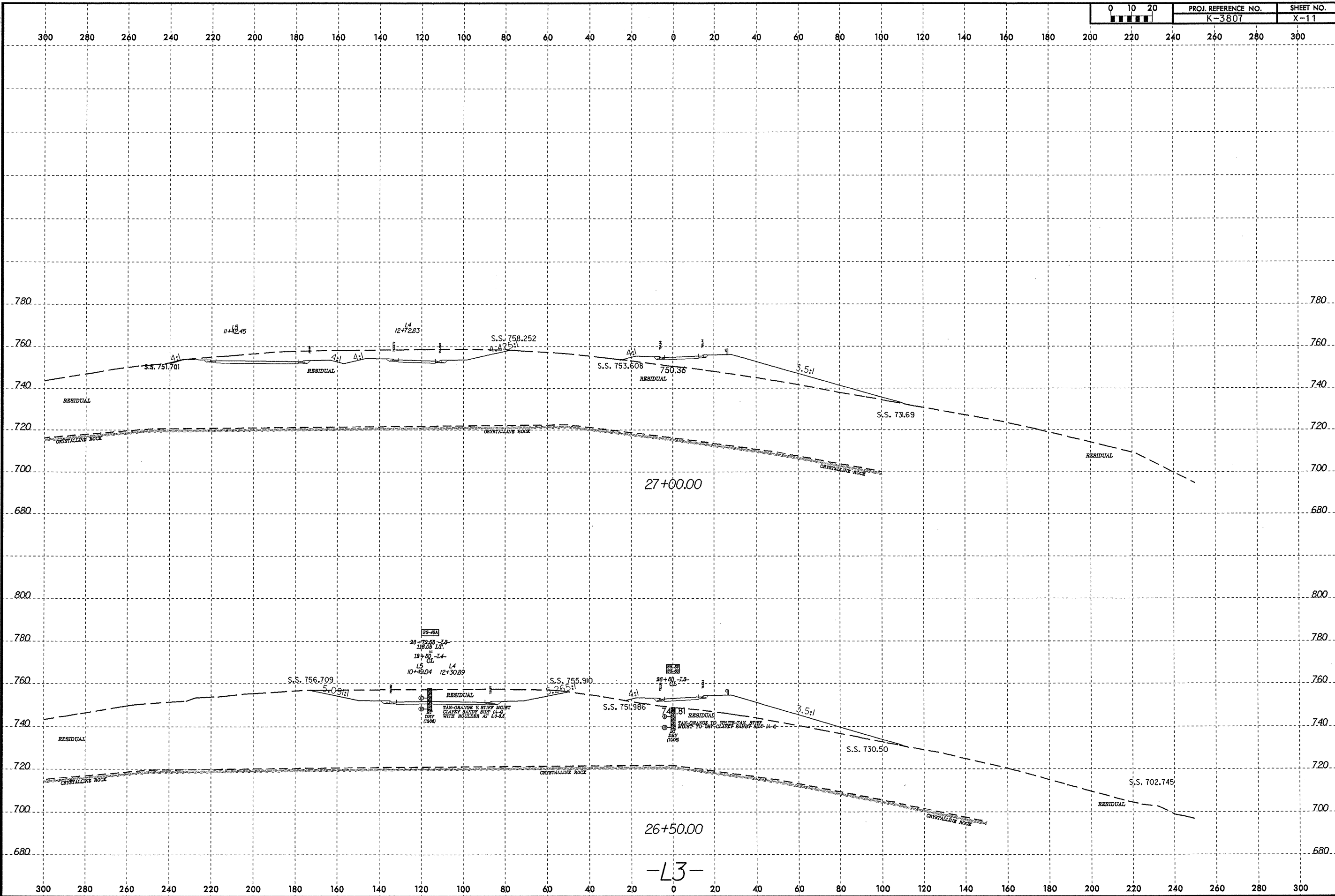
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25+50.00

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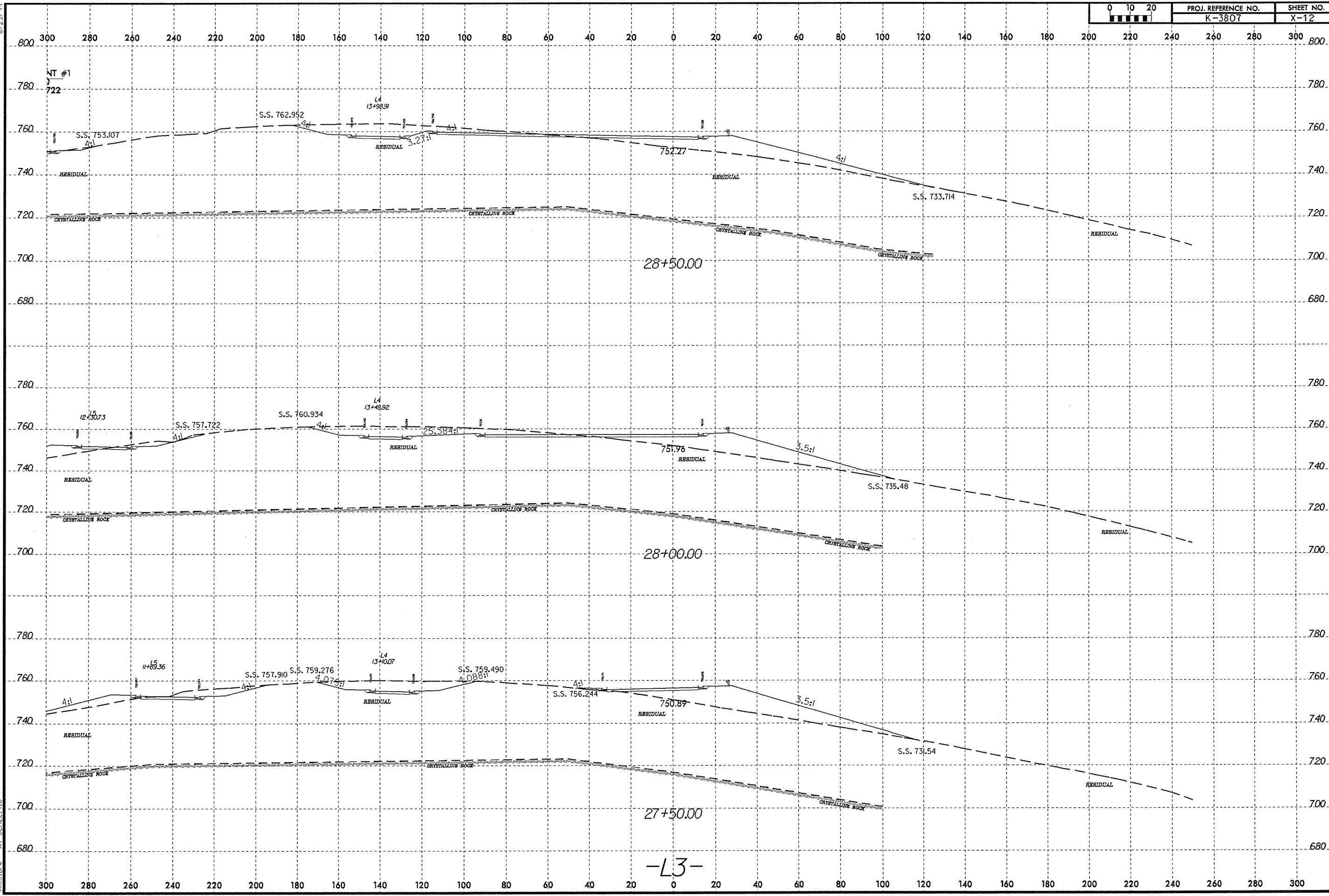
8/23/99



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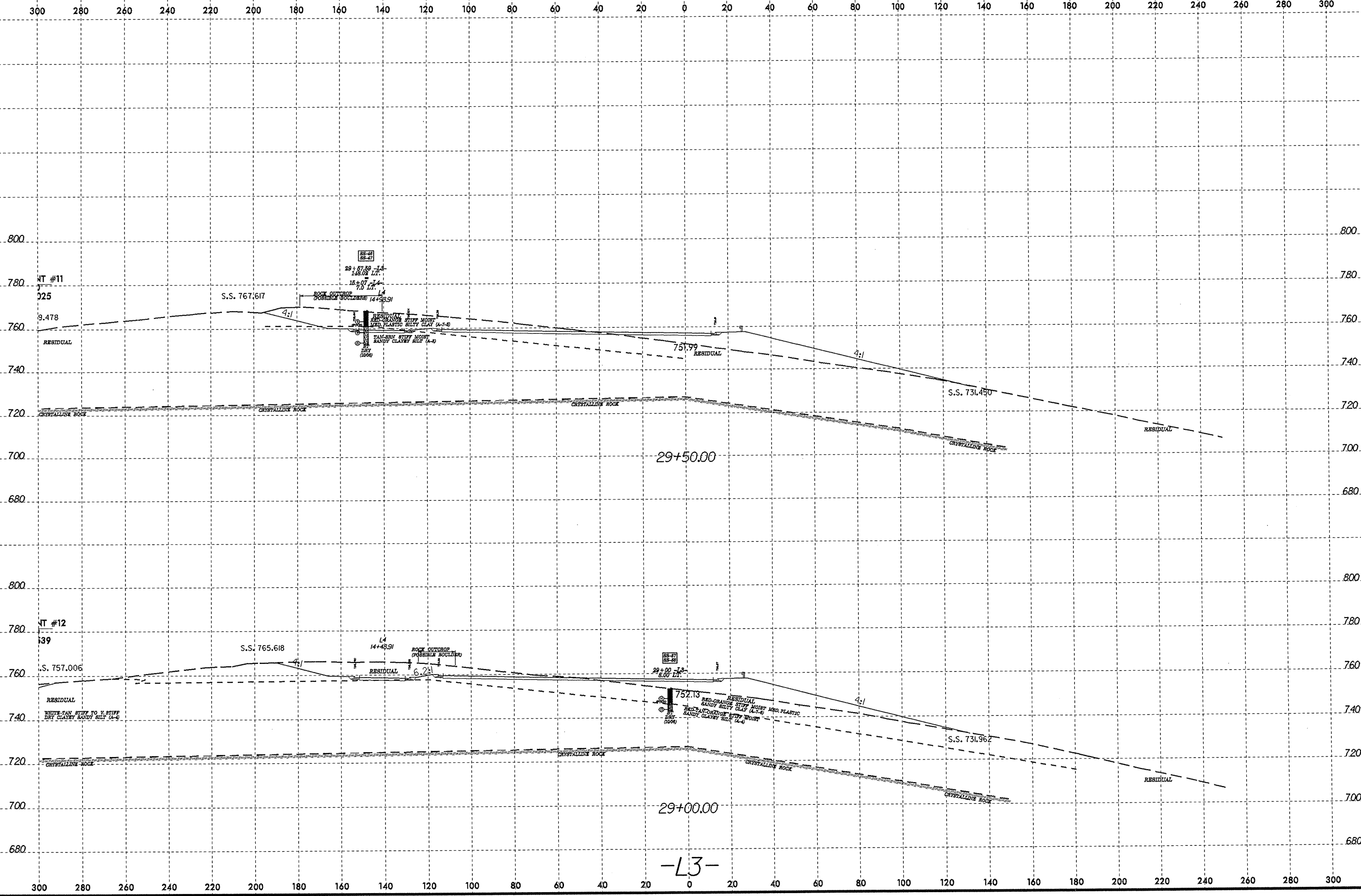
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8/23/99

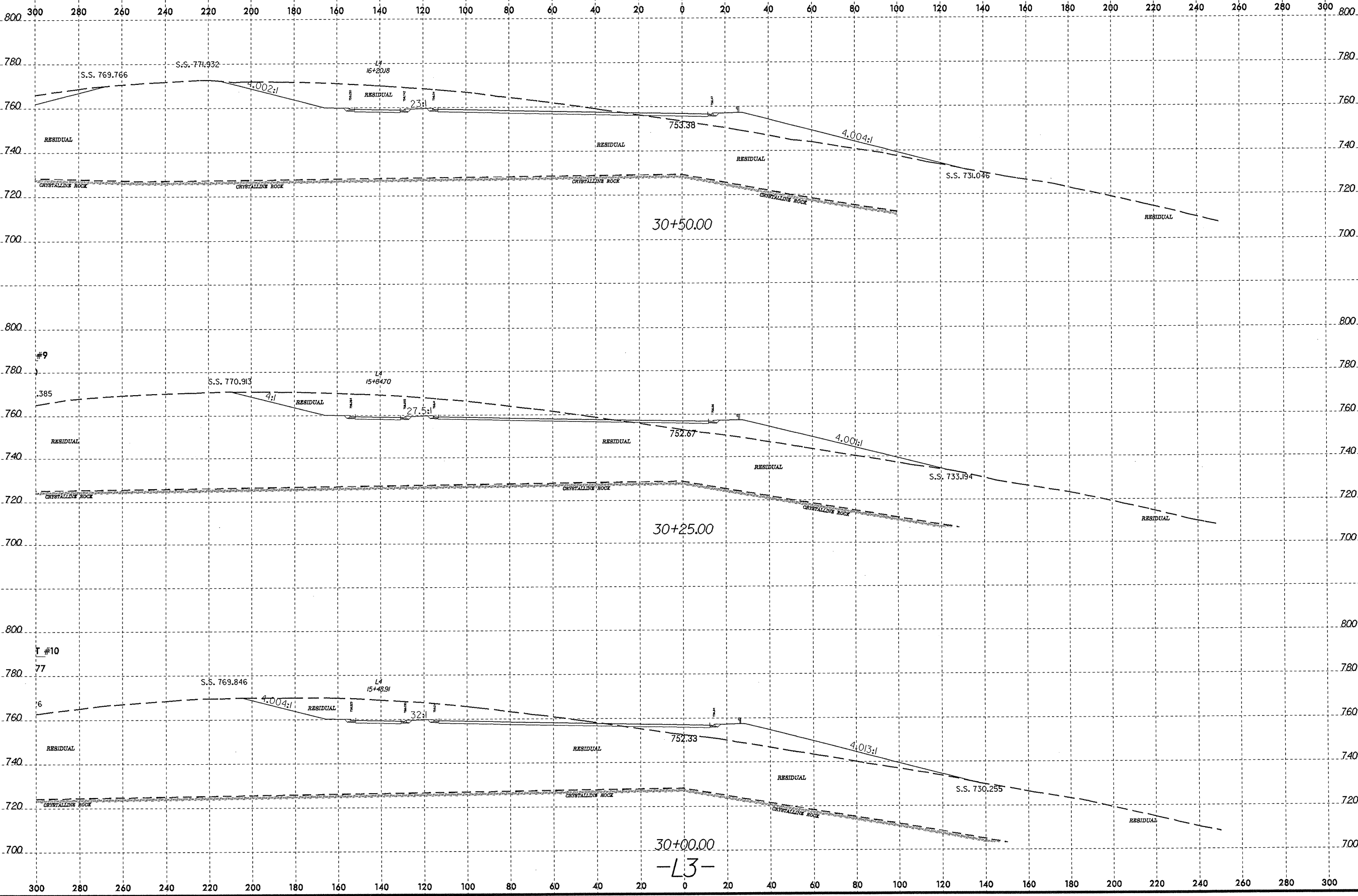
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-L3-

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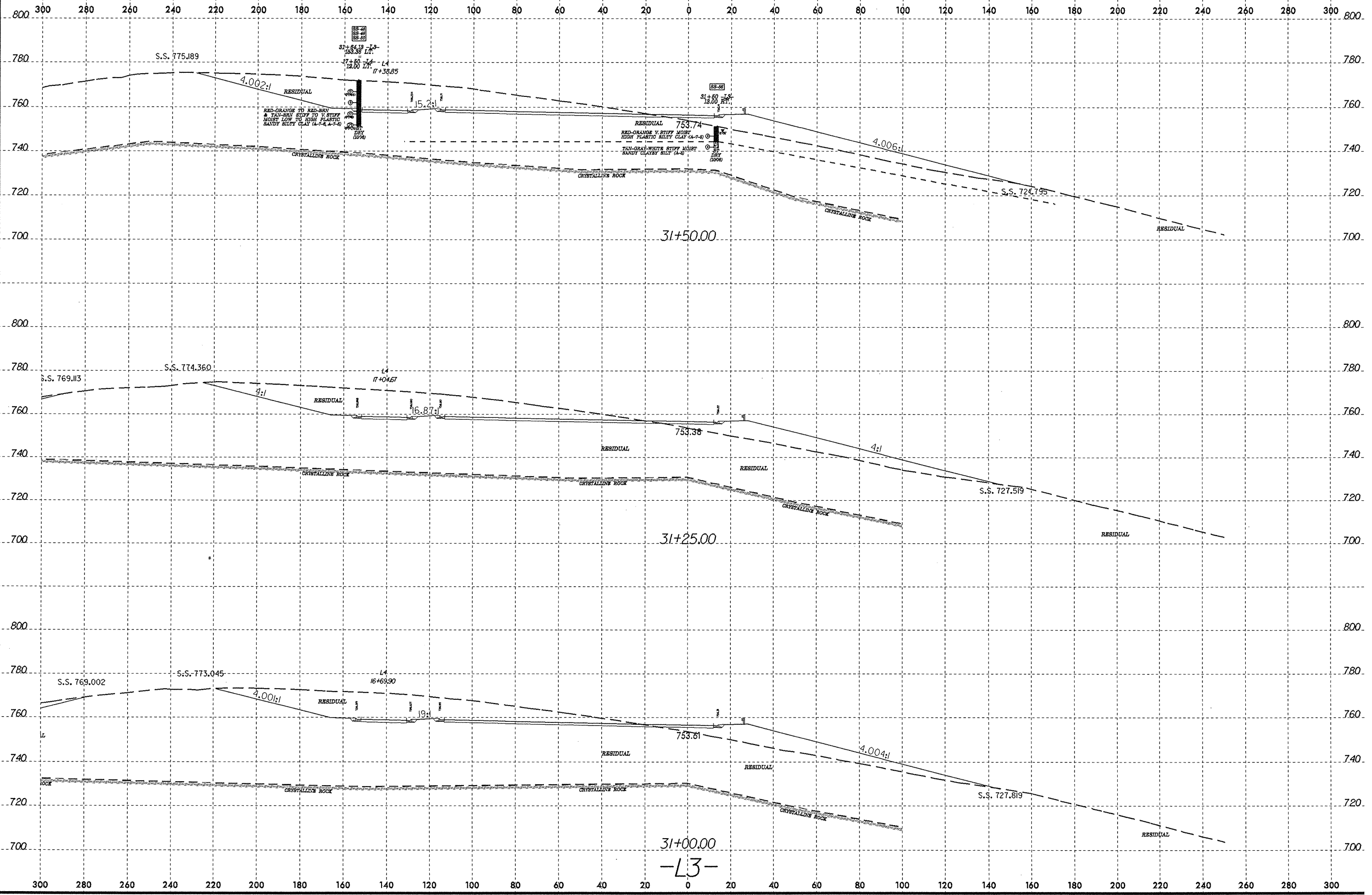




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09-JAN-2007 10:37
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8/23/95



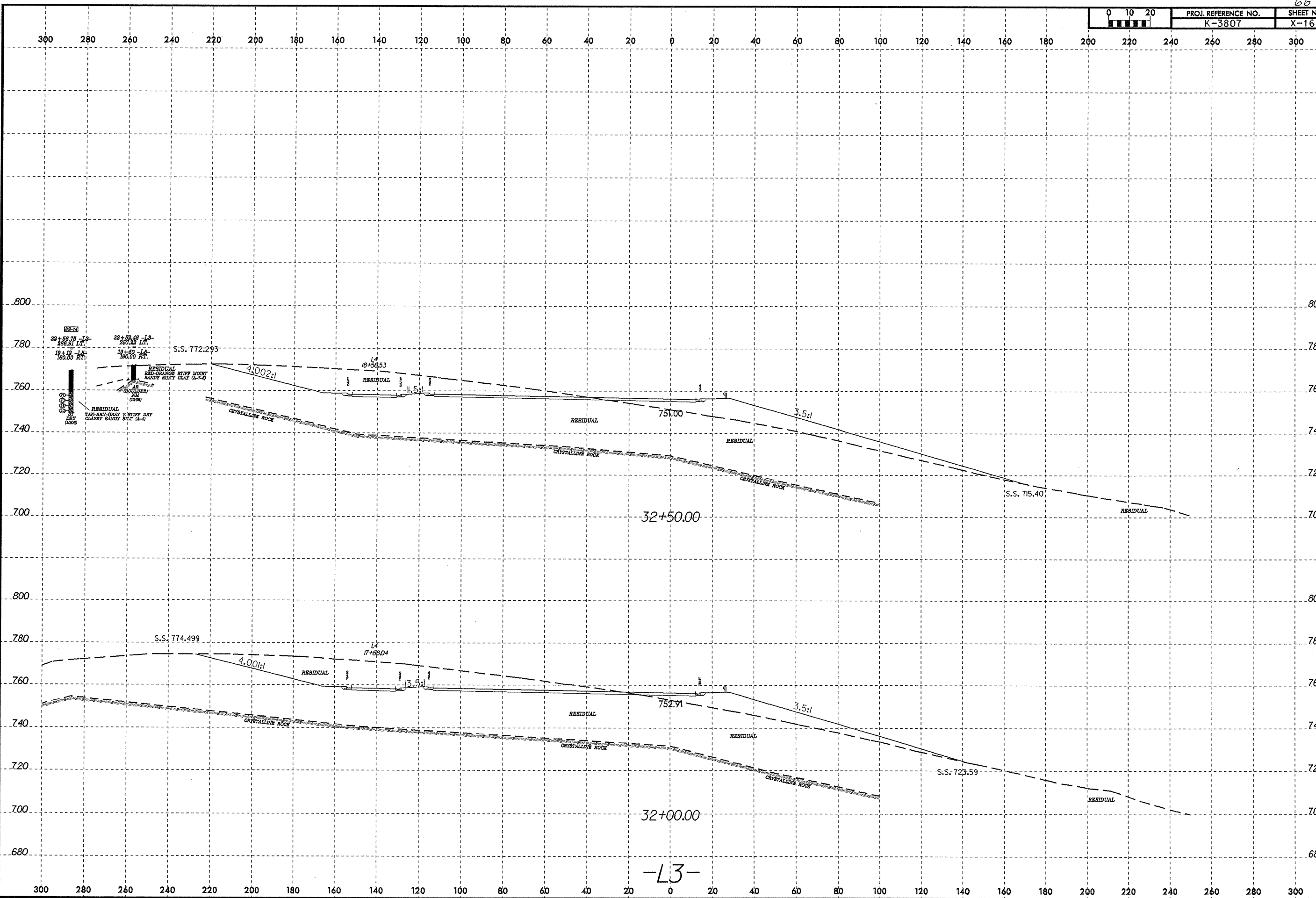
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08/23/95

8/23/99

66

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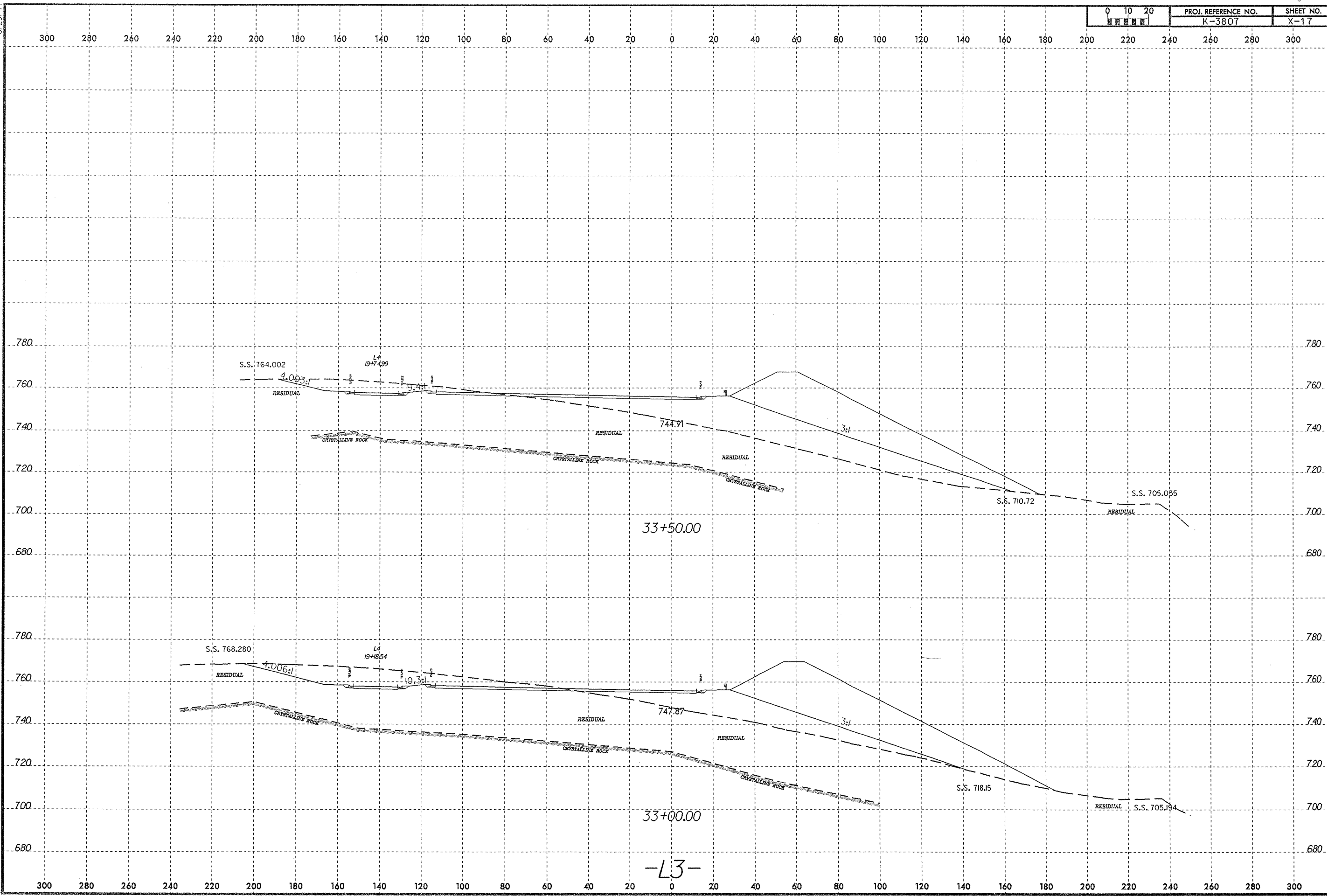


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01-MAR-2007 14:49
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8/23/94



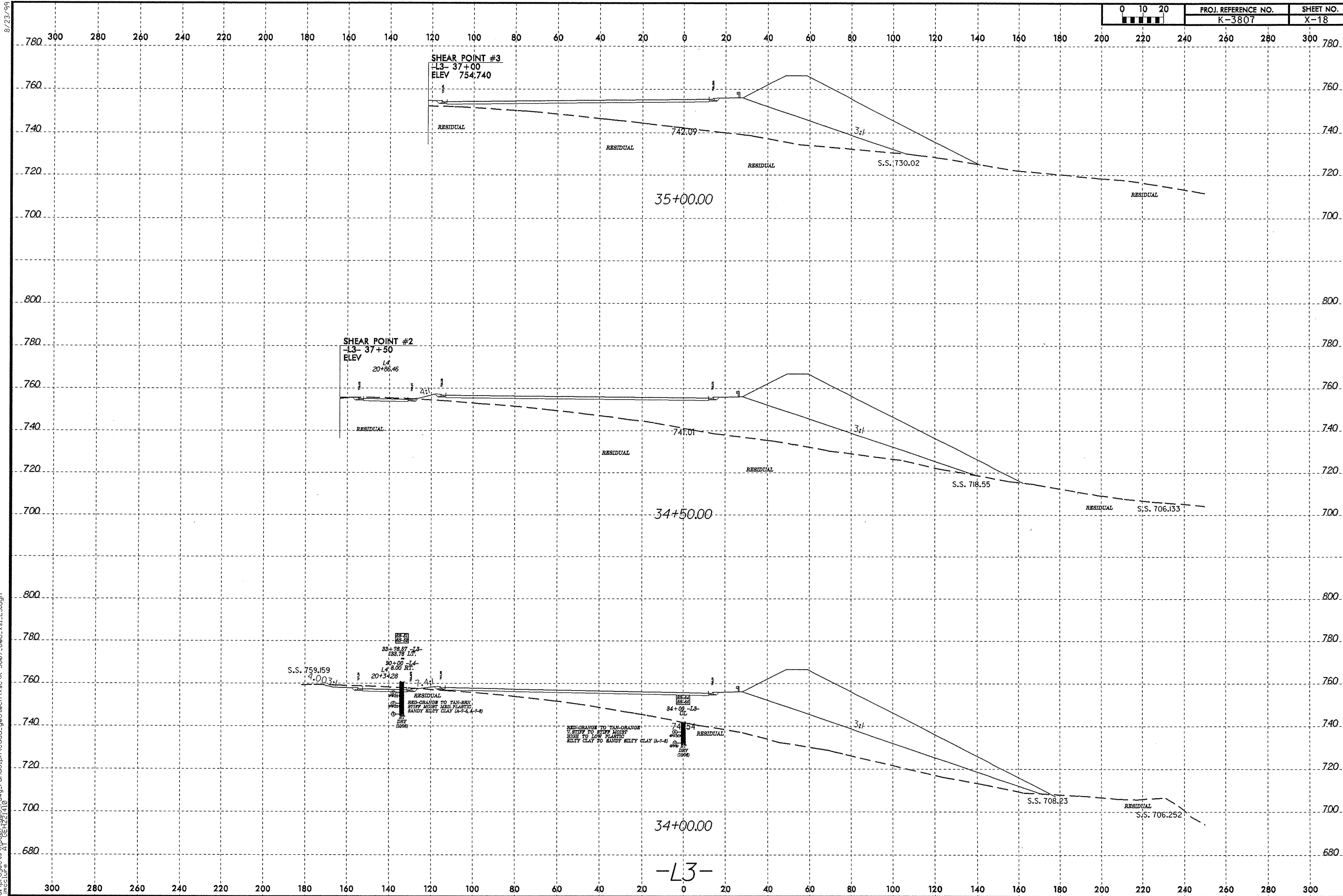
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33+00.00

-L3-

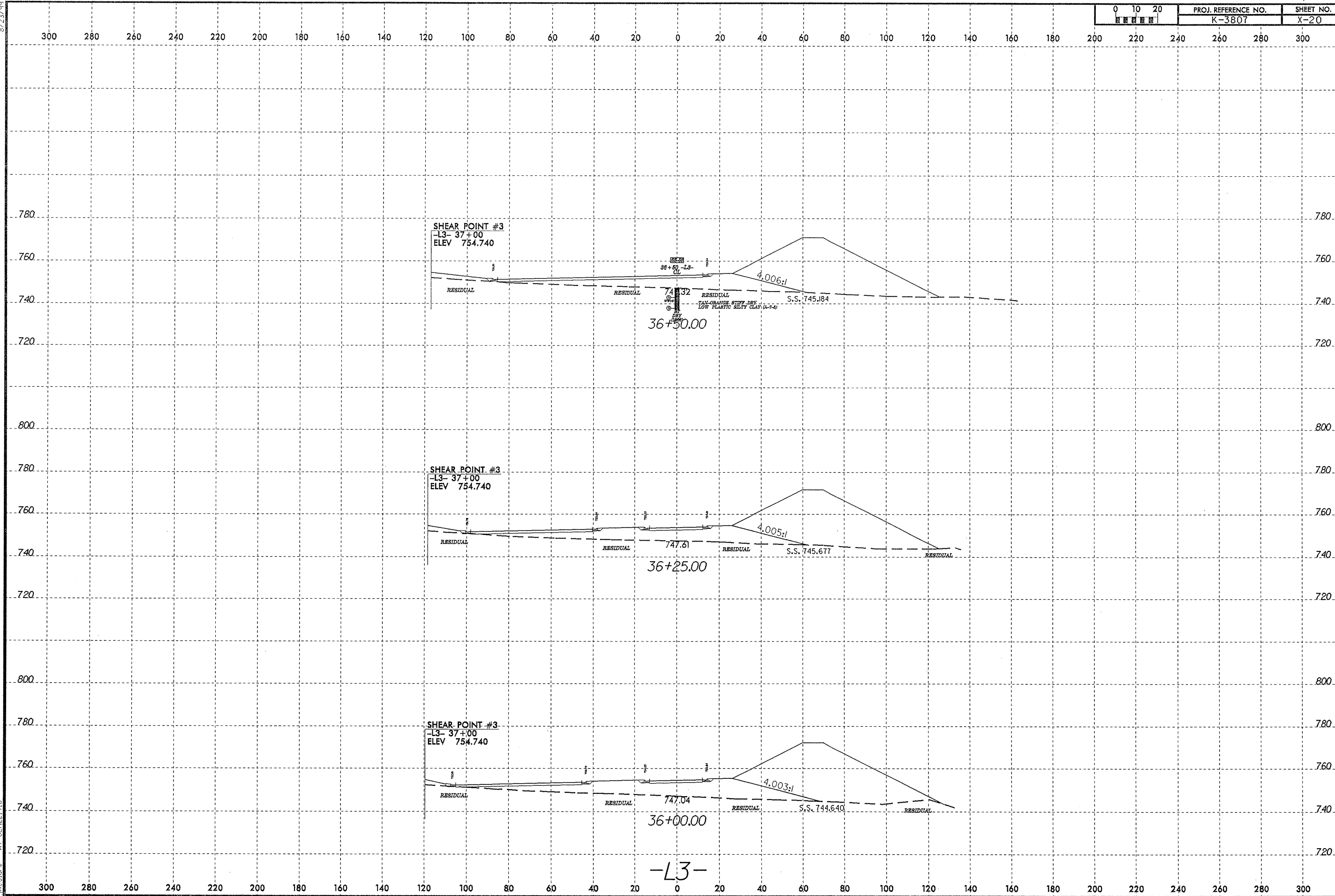
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8/23/99
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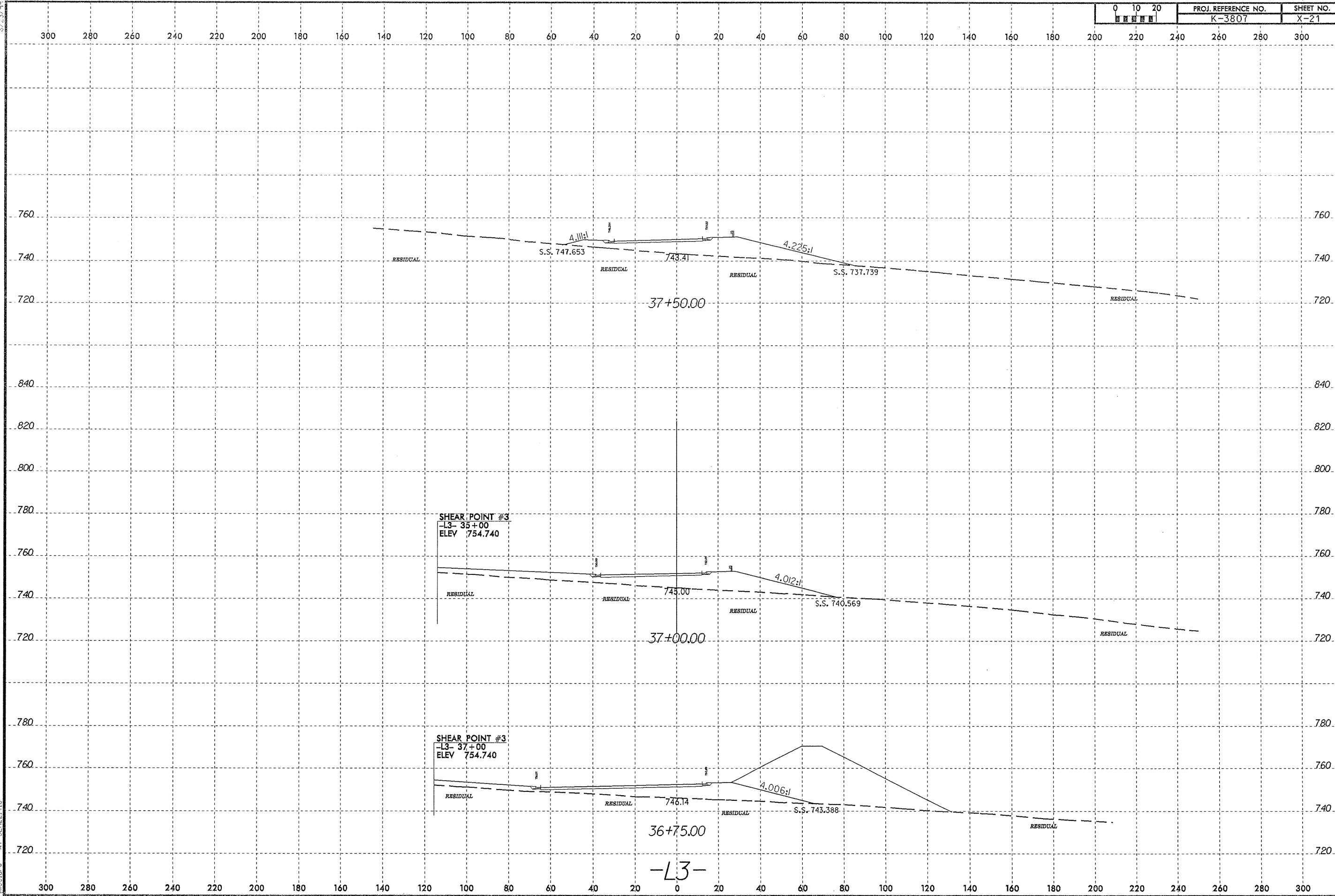
-L3-



-L3-

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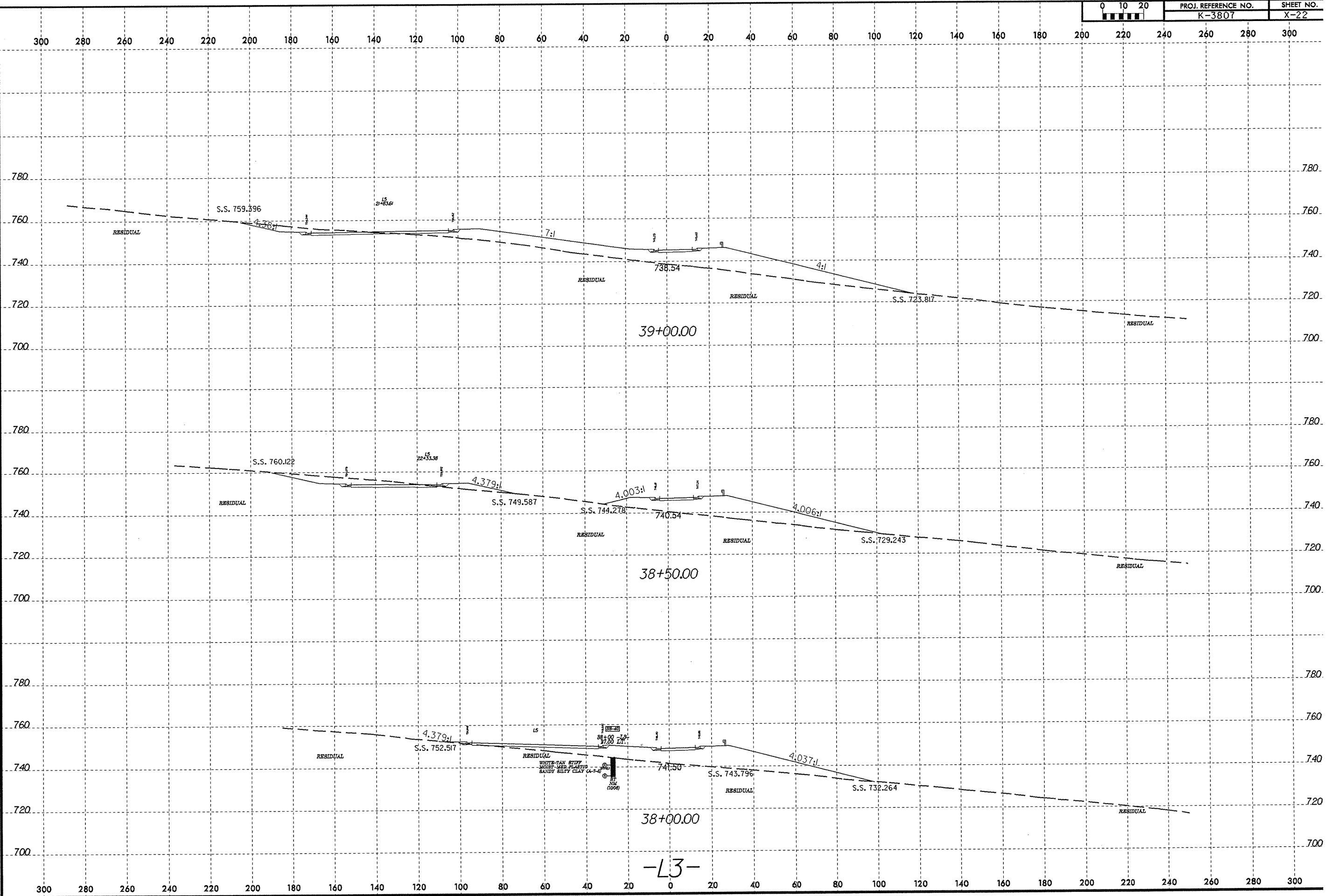
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22-DEC-2006 09:37
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 10/22/06

-L3-

09-JAN-2007 10:39
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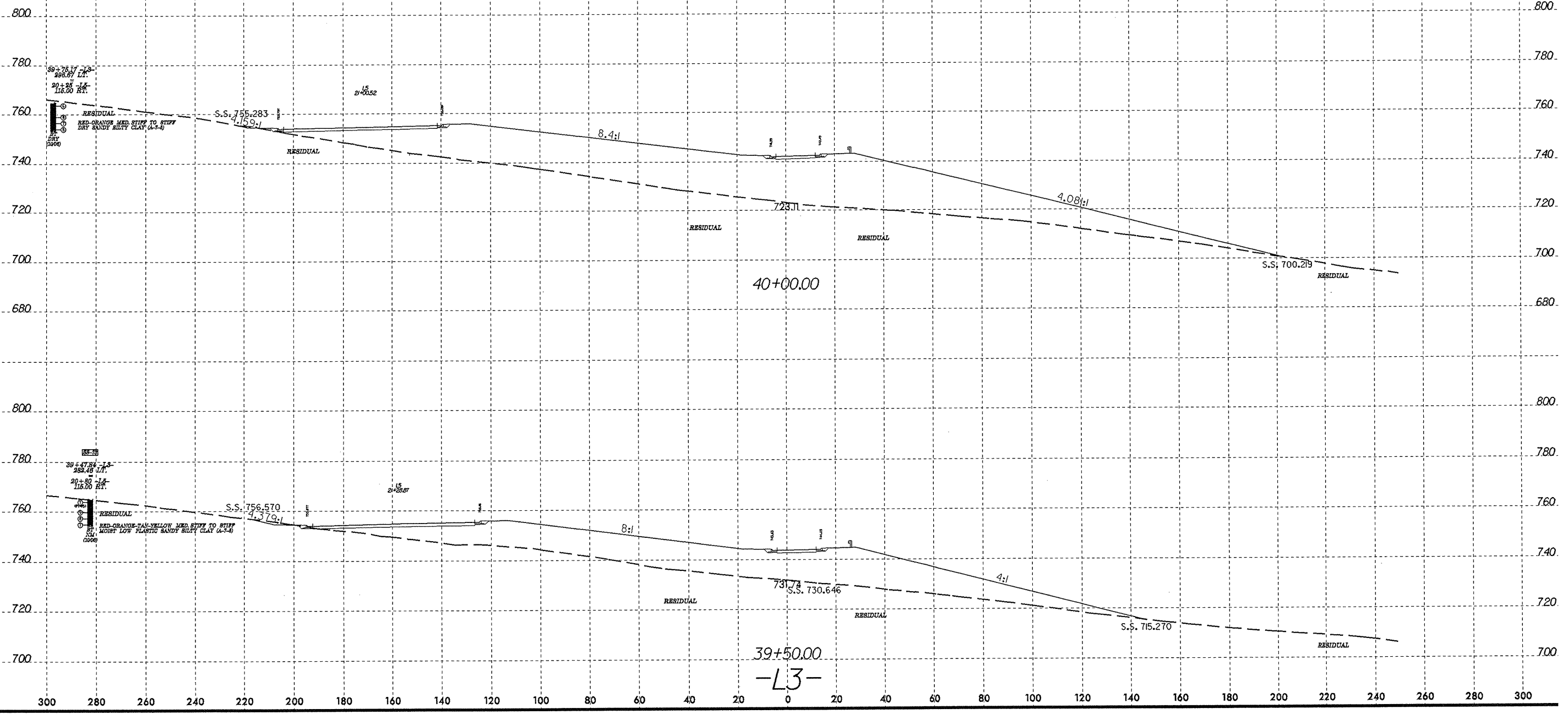


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8/23/99

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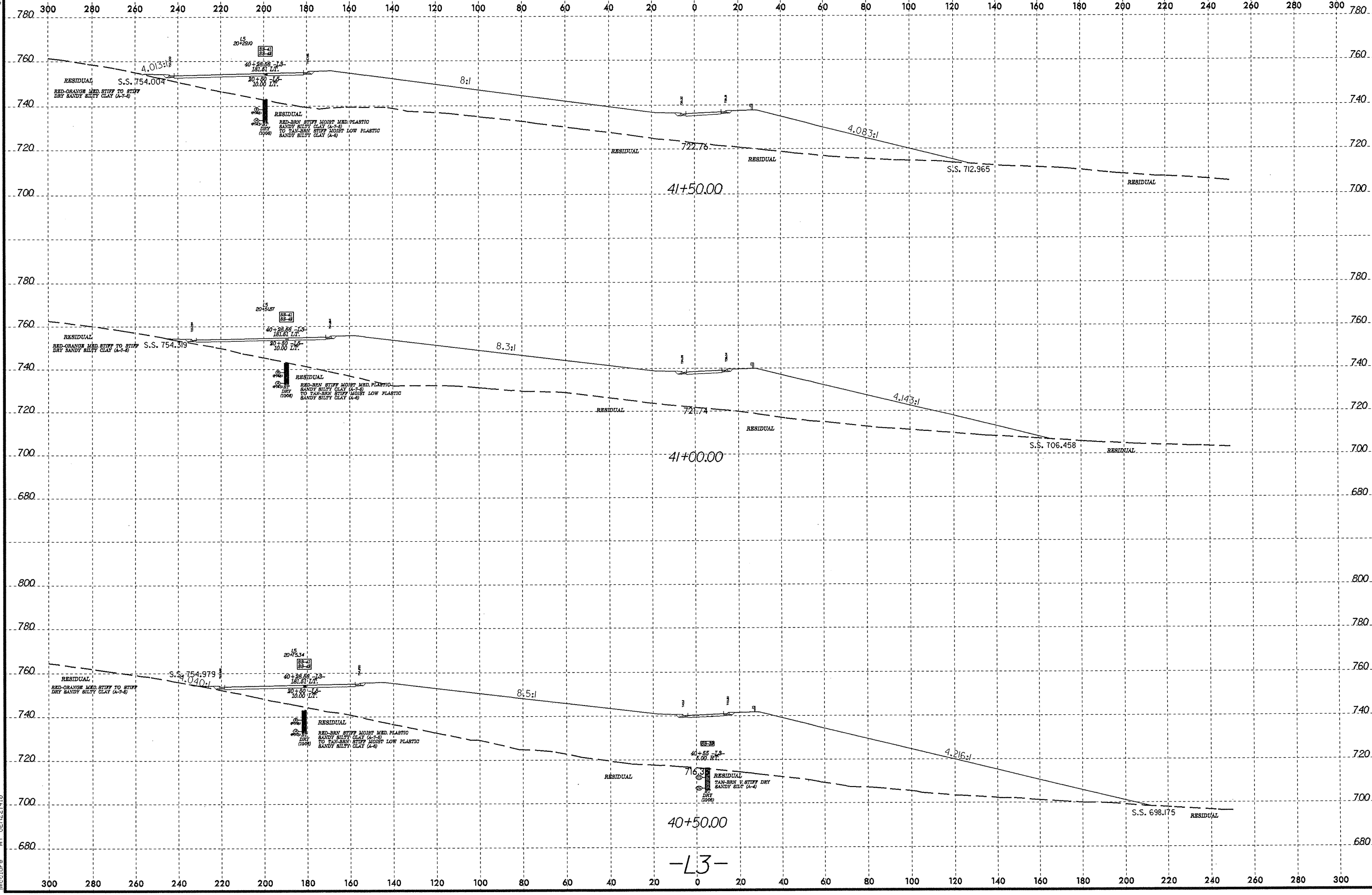
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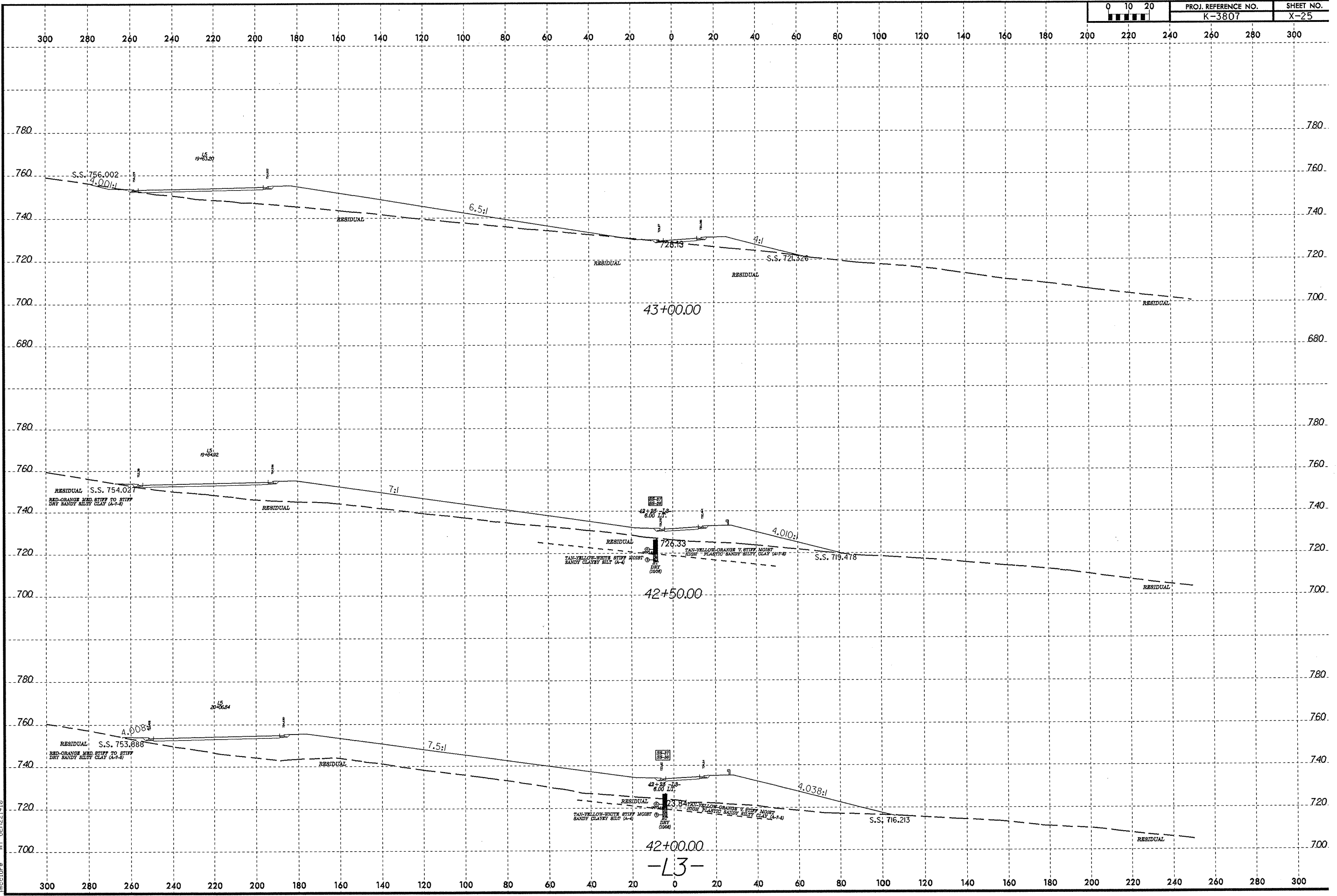
39+50.00
-L3-

09-JAN-2007 10:41
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09-JAN-2007 10:42
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8/23/99

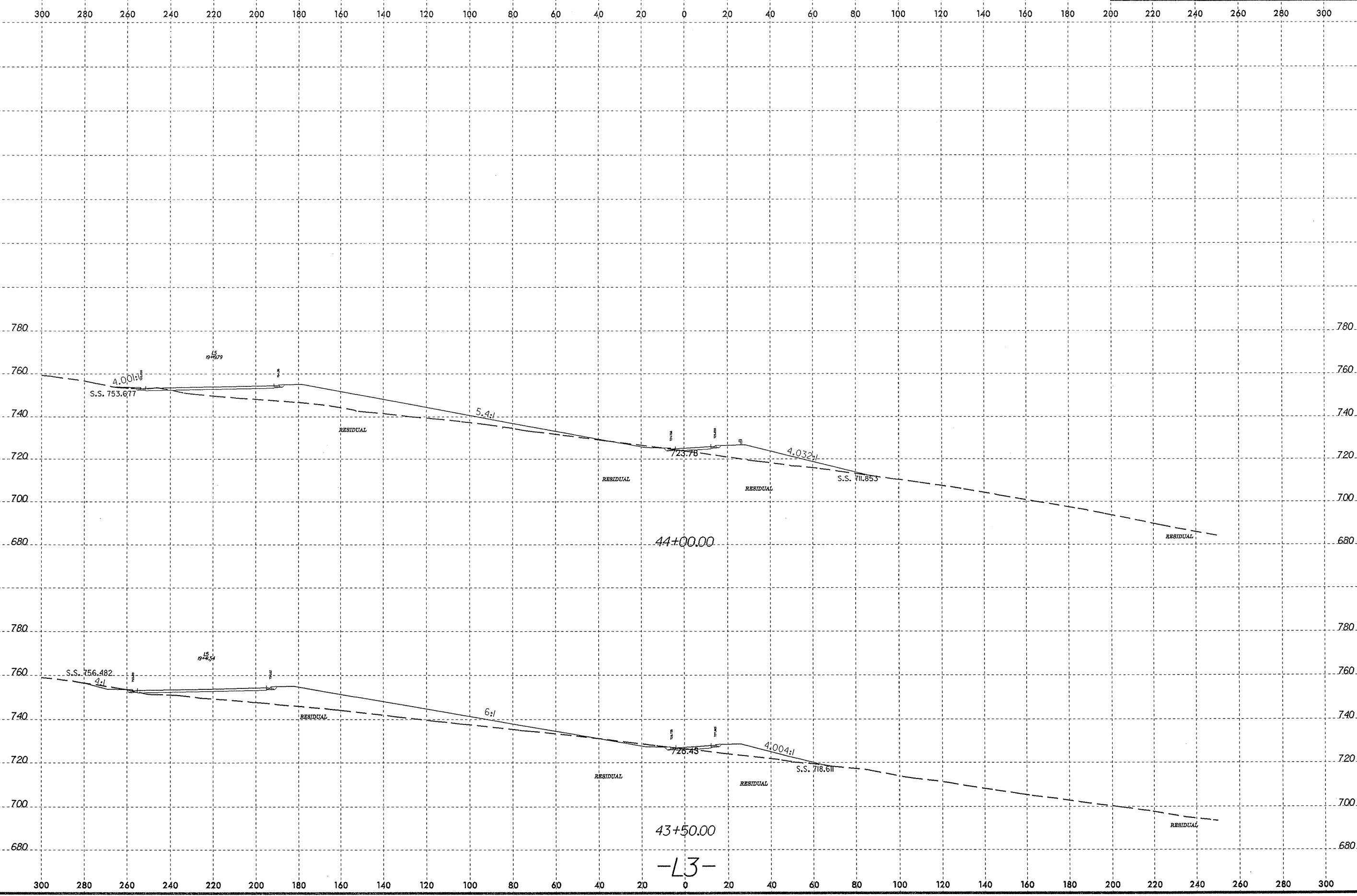


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-L3-

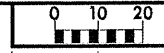
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	K-3807	X-26



44+00.00
 43+50.00
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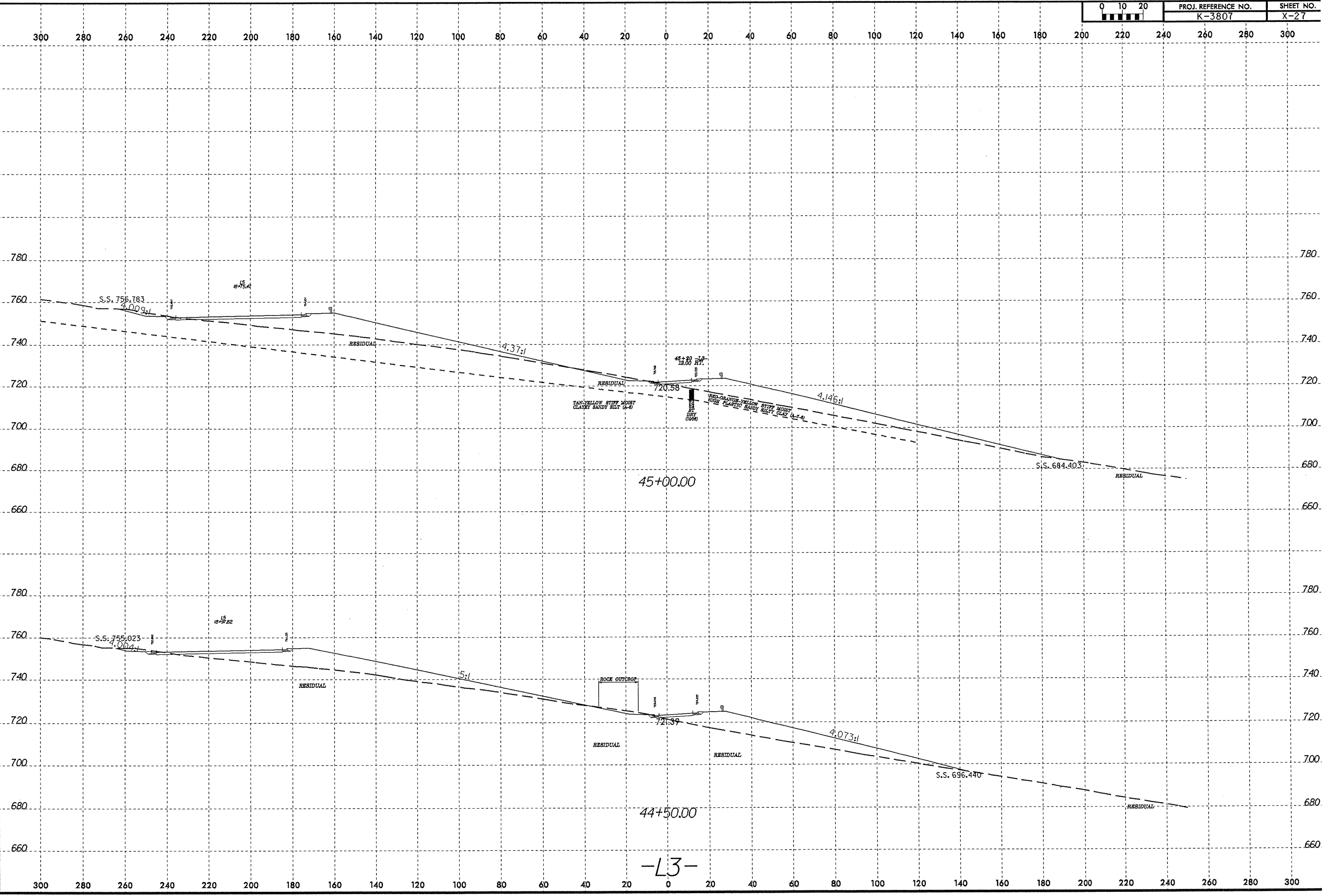
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 10/22/06 10:10

09-JAN-2007 10:43
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AI 08/22/10
09-jan-2007 10:43
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me\2110
AI 08/22/10



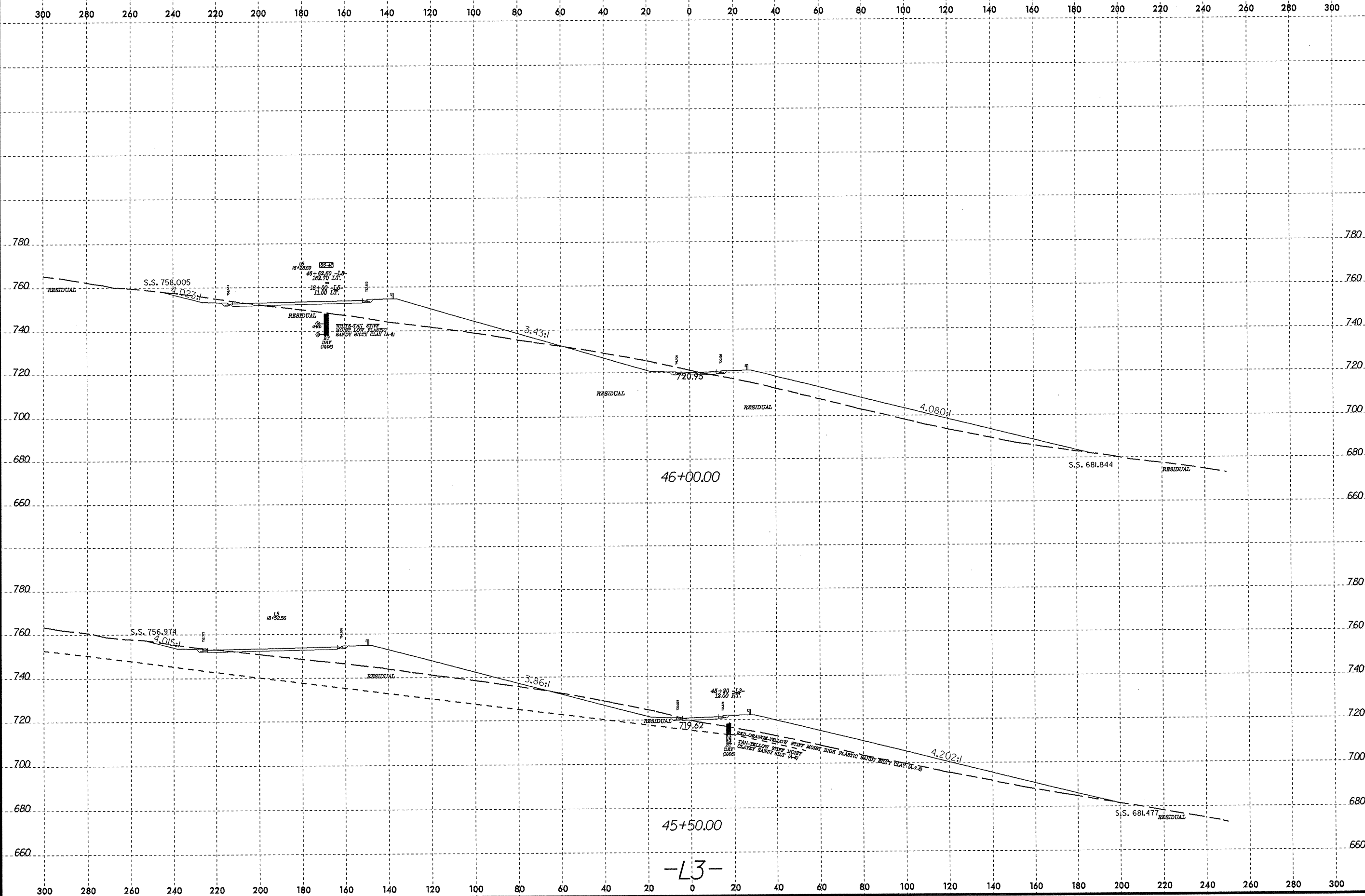
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K-3807

SHEET NO.
X-27



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8/23/99



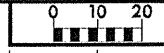
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45+50.00

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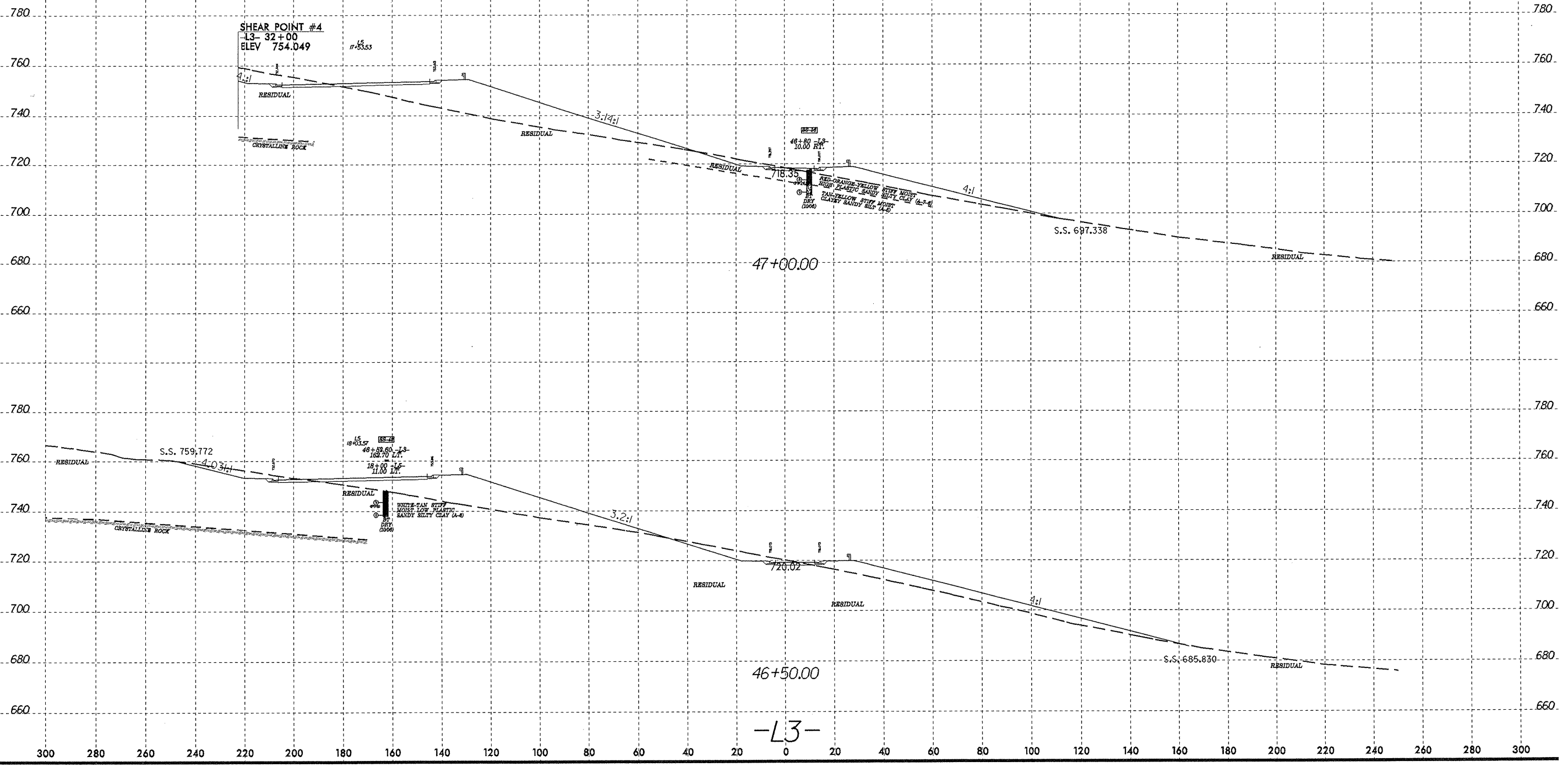
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8/23/99



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K-3807	X-29

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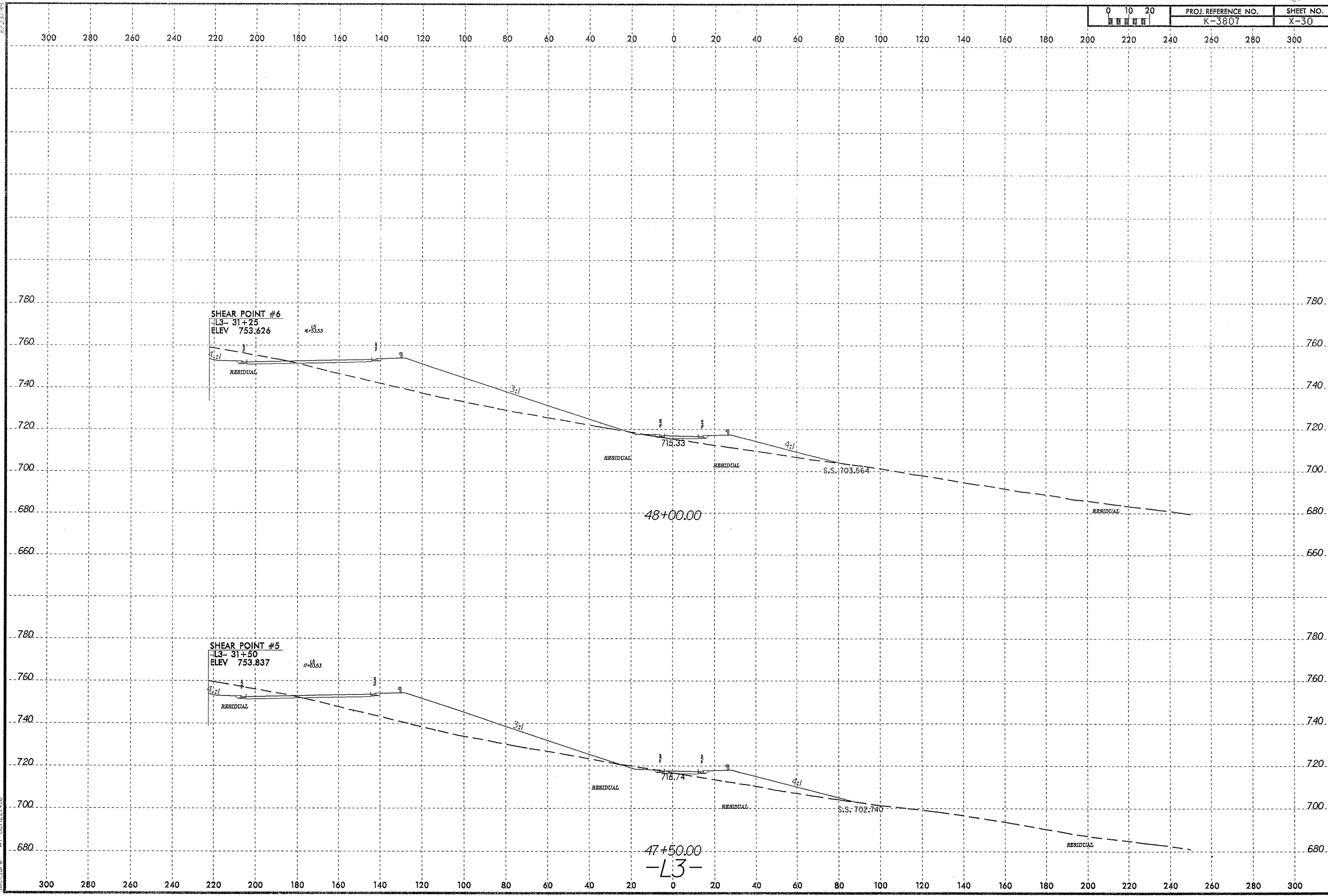
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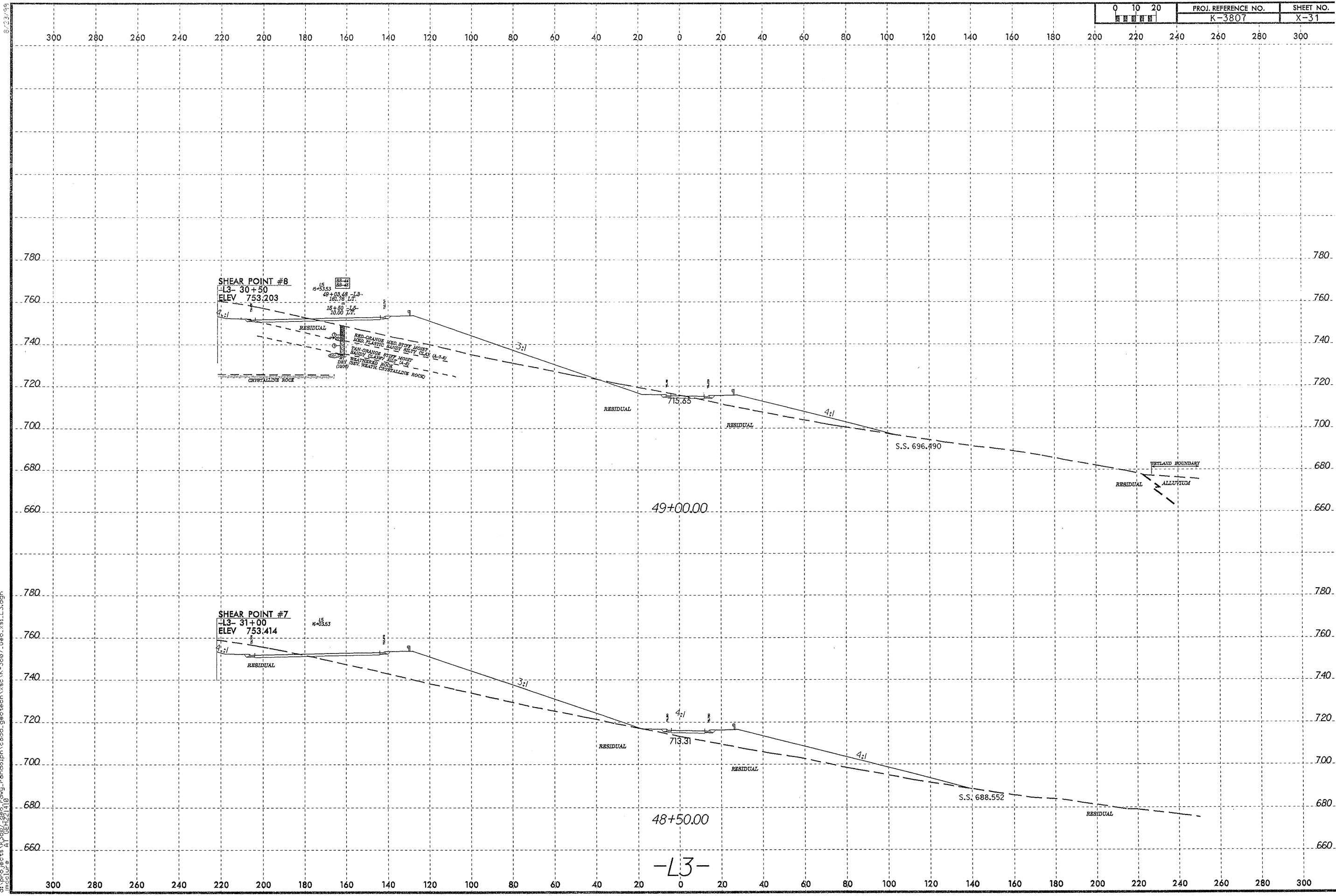
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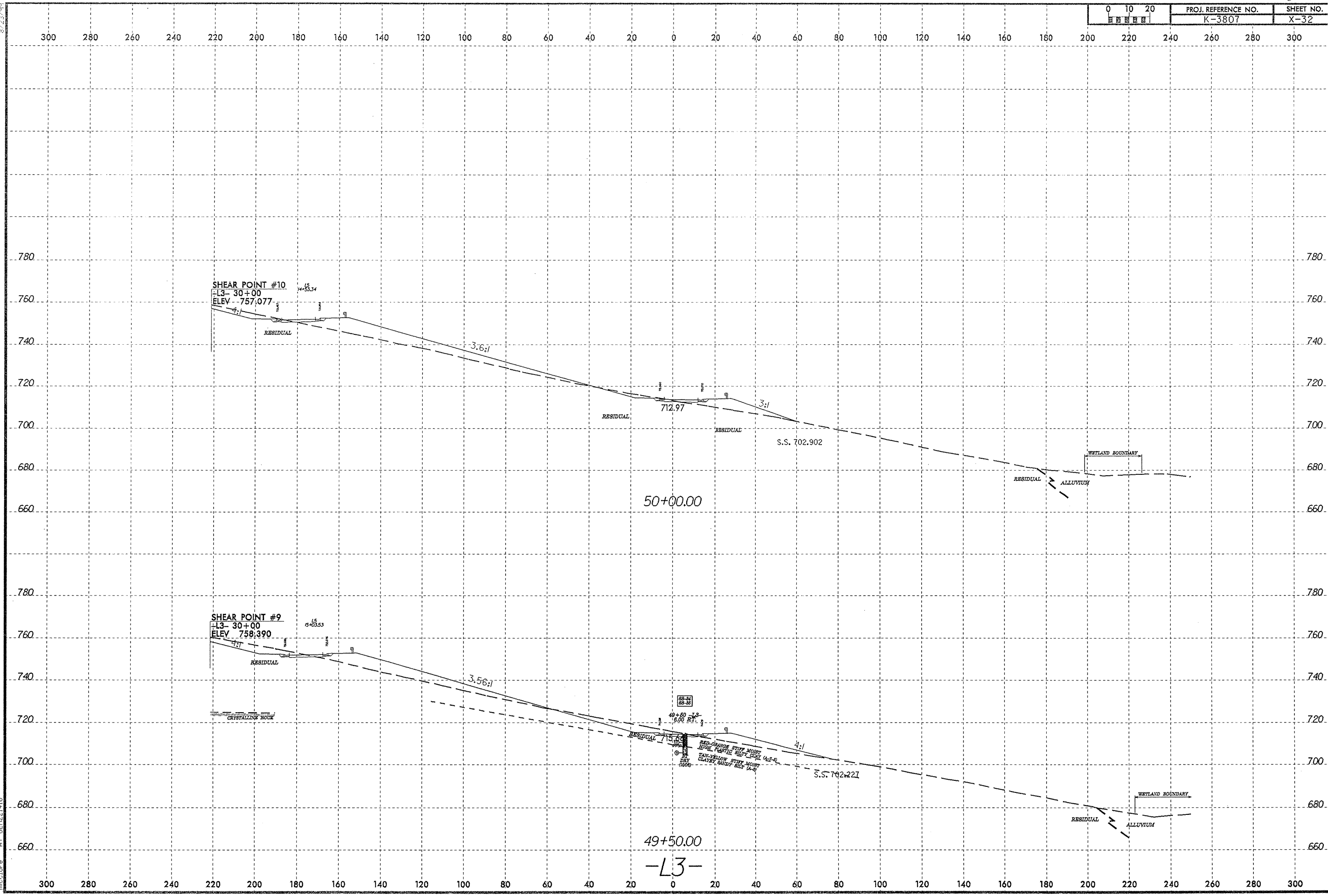


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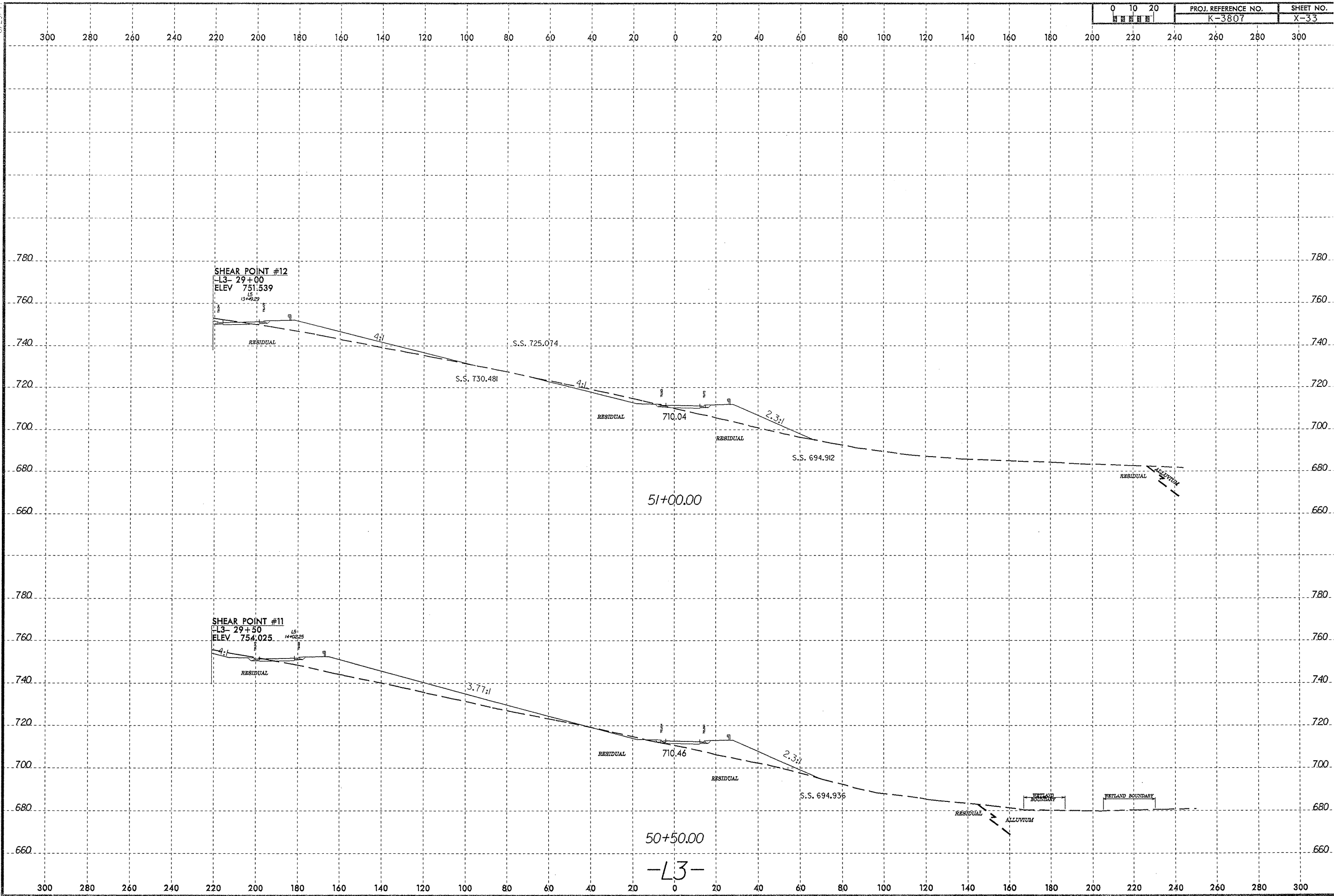
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0 10 20	PROJ. REFERENCE NO.	SHEET NO.
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SHEAR POINT #12
 L3- 29+00
 ELEV 751.539

SHEAR POINT #11
 L3- 29+50
 ELEV 754.025

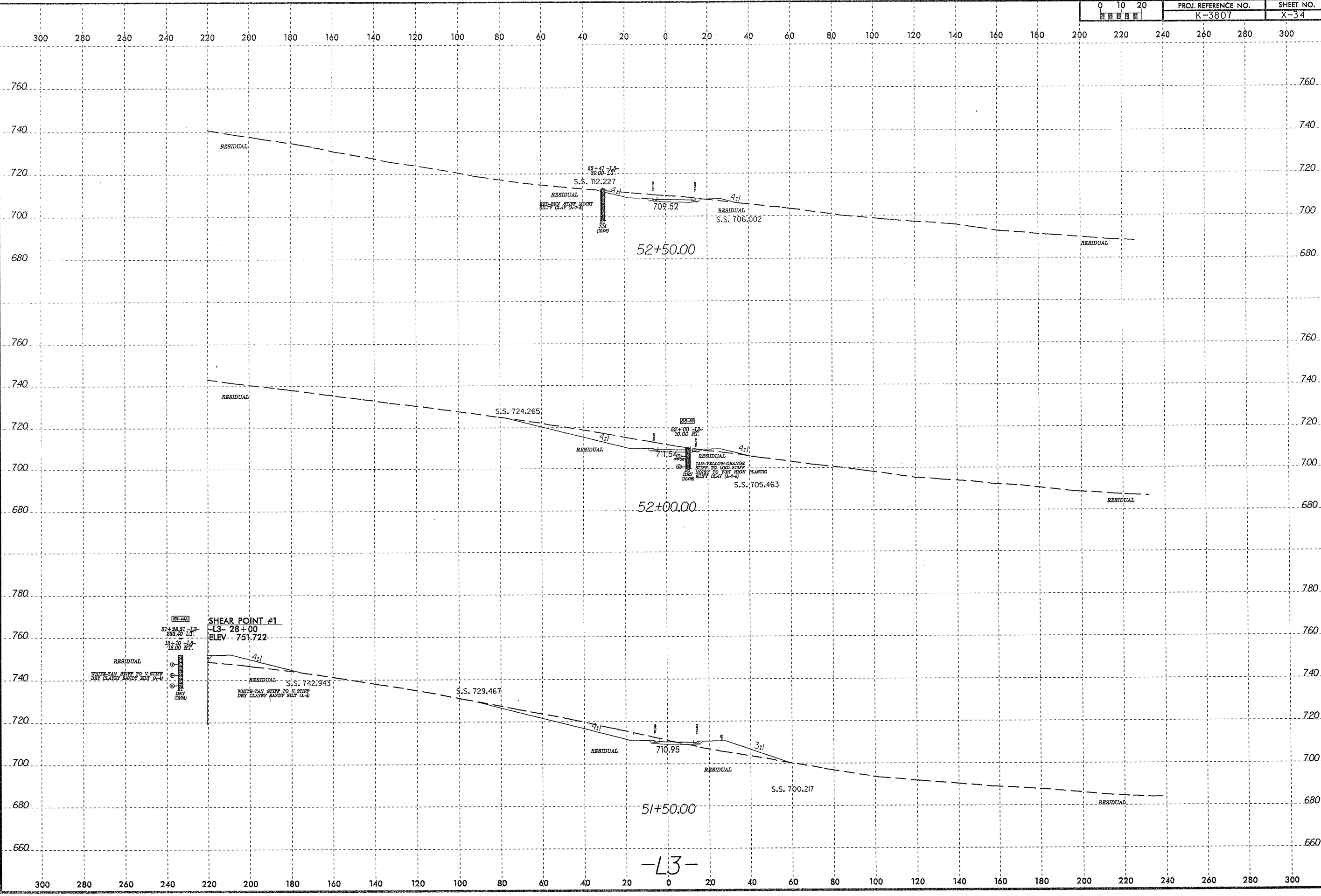
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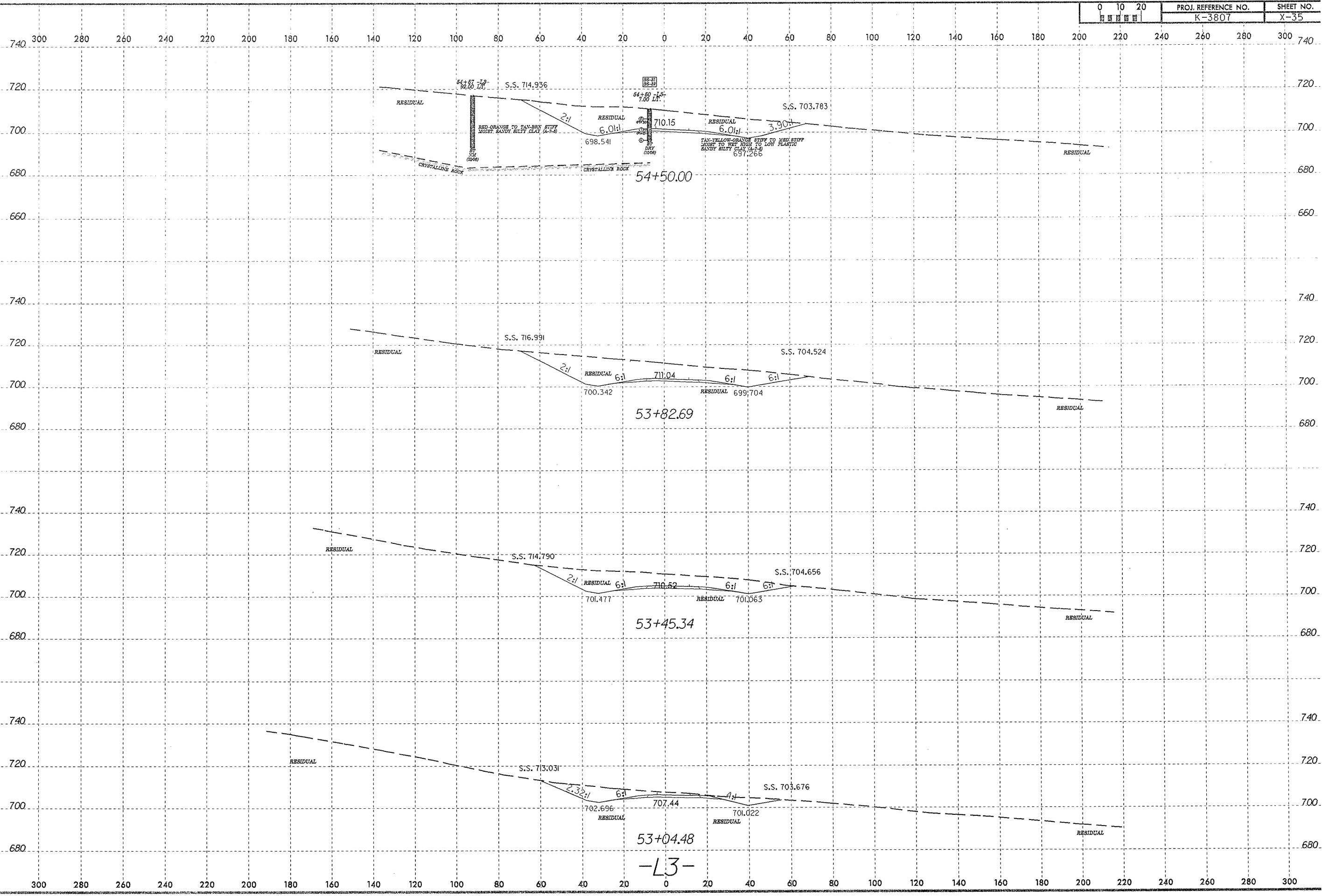
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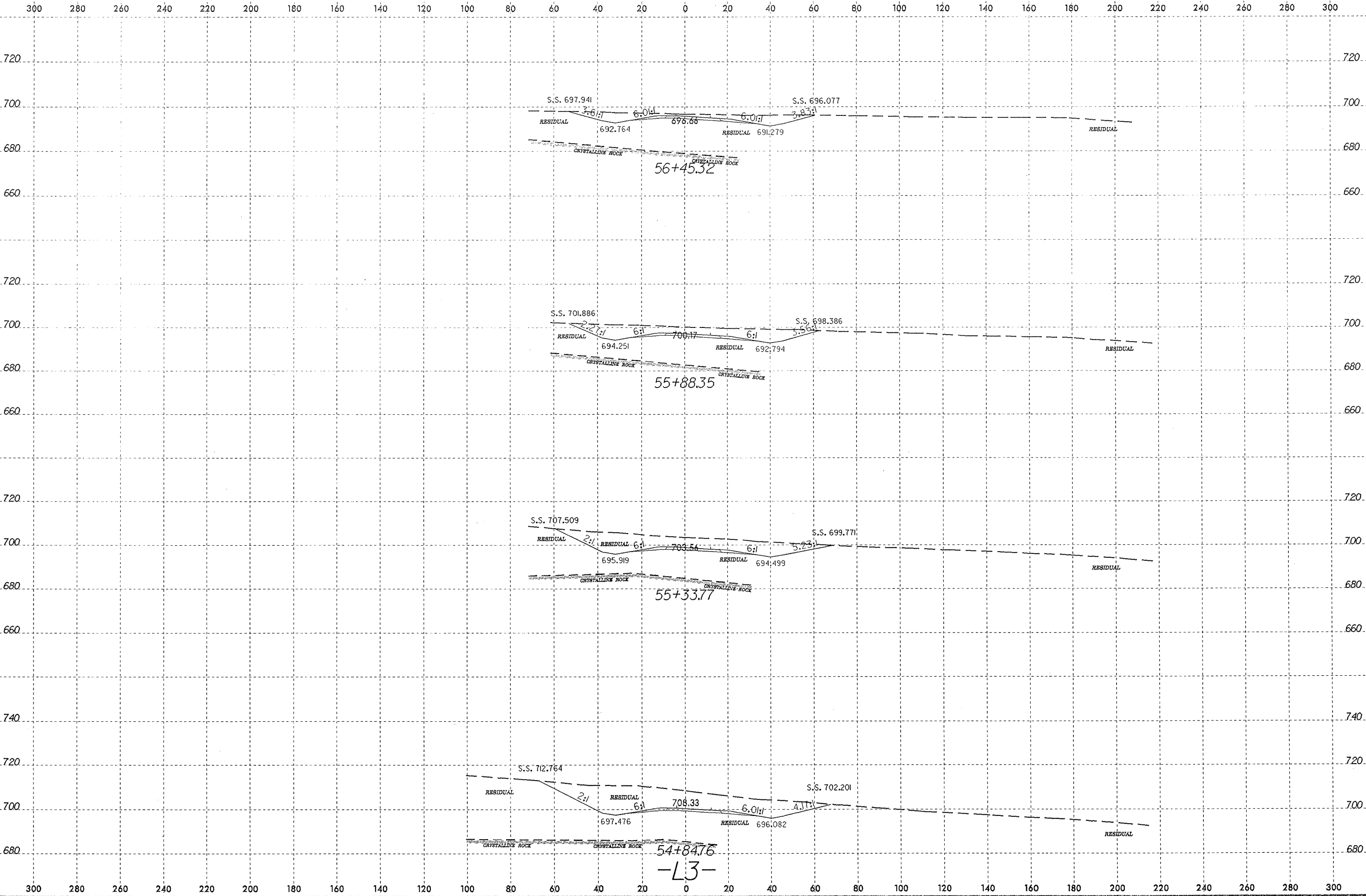
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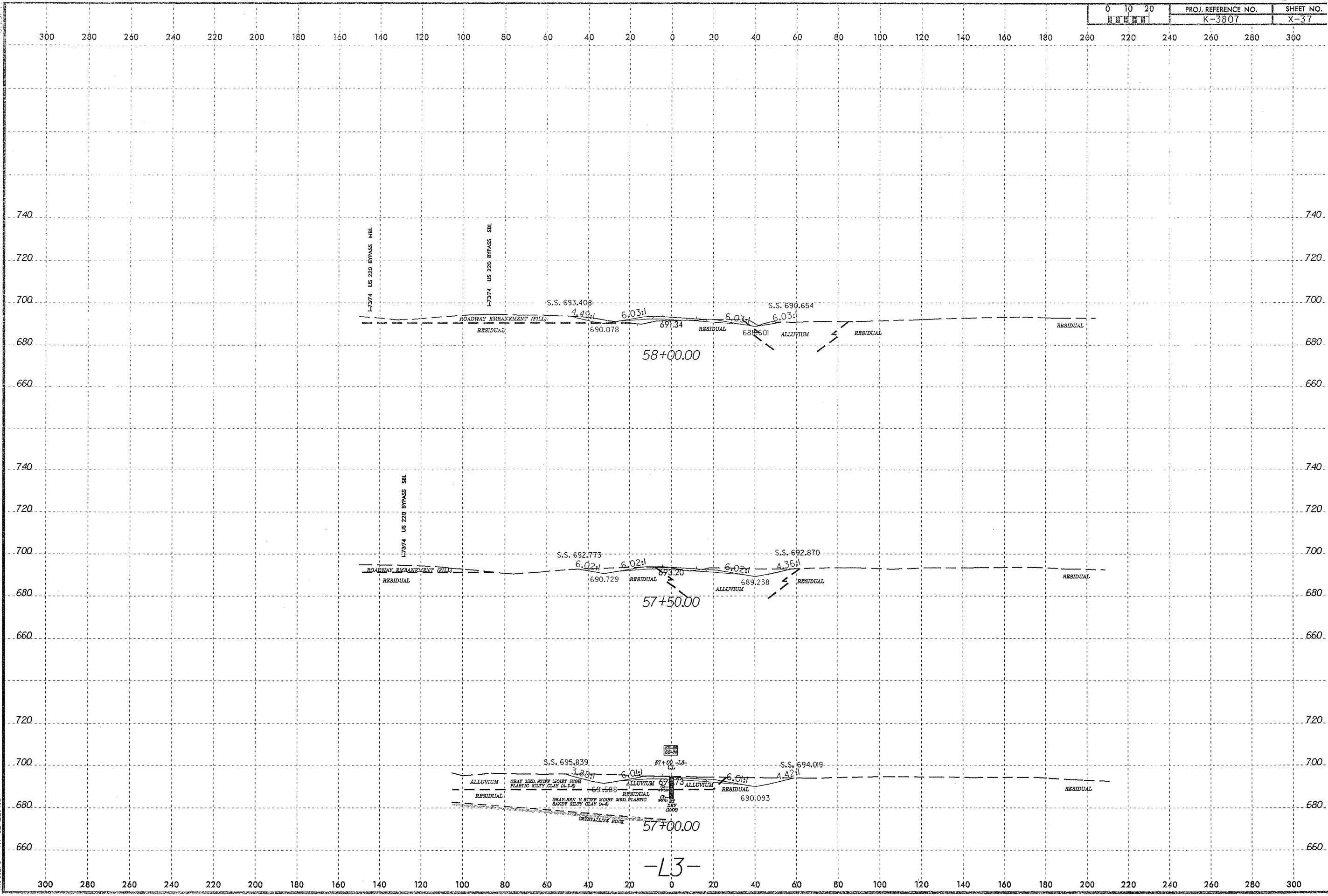
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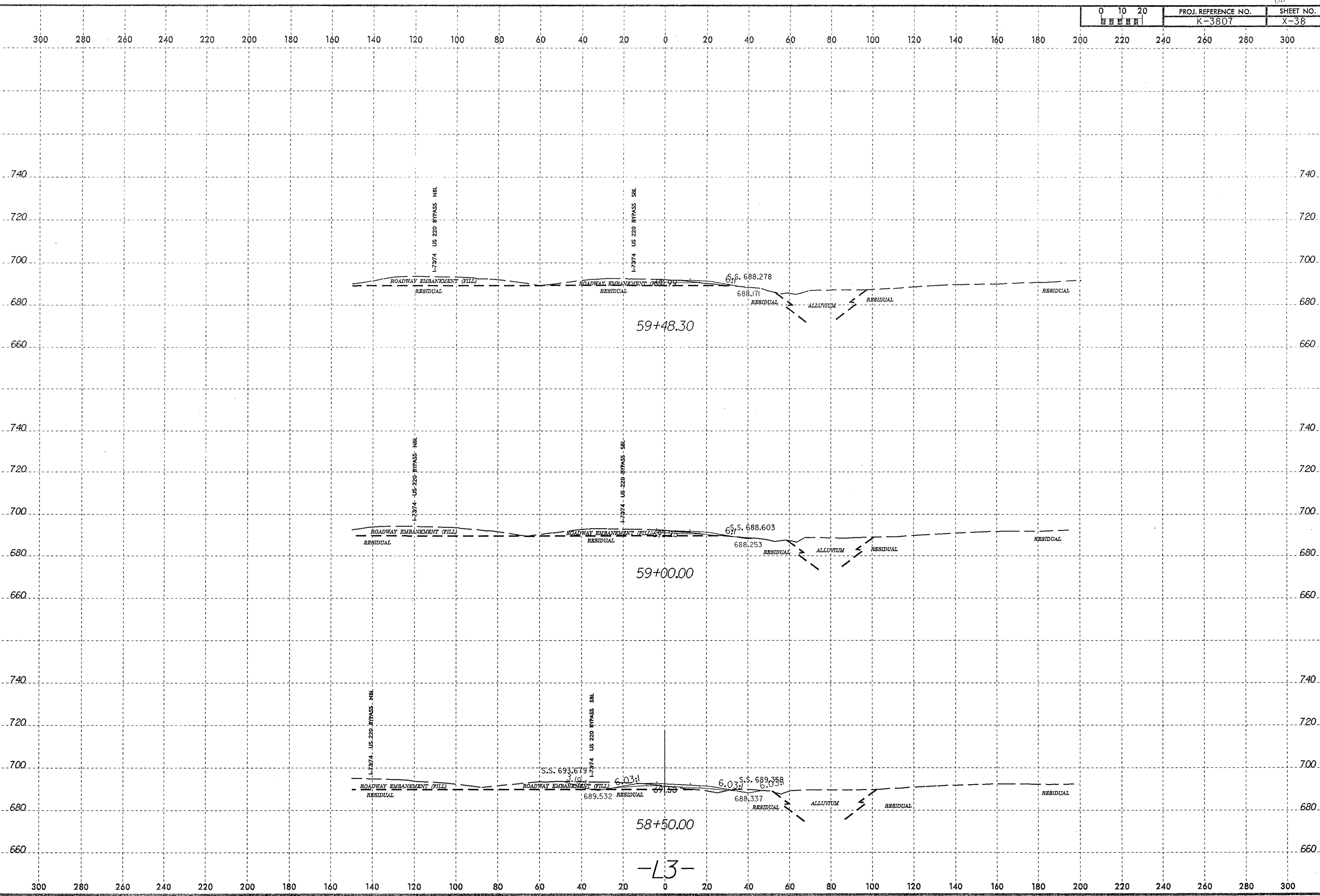
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22-DEC-2006 09:47
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SOIL TEST RESULTS

SAMPLE NO.	OFFSET	STATION	DEPTH INTERVAL	AASHTO CLASS.	L.L.	P.I.	% BY WEIGHT				% PASSING (SIEVES)			%	%
							C.SAND	F.SAND	SILT	CLAY	10	40	200		
SS-1	10 RT	16+00 -L1-	3.20-4.20	A-7-5(14)	47	15	9.0	17.6	32.3	41.0	100	94	79	-	-
SS-2	10 RT	16+00 -L1-	8.20-9.20	A-7-5(32)	63	25	0.6	4.1	29.6	65.6	100	100	97	-	-
SS-3	10 RT	16+00 -L1-	13.20-14.20	A-7-5(14)	47	14	2.3	20.5	30.1	47.2	100	100	83	-	-
SS-4	5 LT	18+90 -L1-	4.30-5.30	A-5(8)	46	10	15.8	15.4	33.9	34.9	96	86	71	-	-
SS-5	5 LT	18+90 -L1-	9.30-10.30	A-4(2)	36	7	28.3	13.3	31.7	26.7	90	69	54	-	-
SS-6	CL	22+00 -L1-	4.30-5.30	A-7-5(42)	77	35	0.8	7.6	19.8	71.8	100	100	94	-	-
SS-7	CL	22+00 -L1-	9.30-10.30	A-7-5(26)	73	22	4.1	17.0	27.6	51.3	100	99	84	-	-
SS-8	CL	22+00 -L1-	14.30-15.30	A-7-5(12)	57	14	14.2	23.6	25.3	36.9	100	92	68	-	-
SS-9	12 LT	25+02 -L1-	4.70-5.70	A-7-5(28)	65	22	0.8	10.1	33.7	55.4	100	100	93	-	-
SS-10	12 LT	25+02 -L1-	14.70-15.70	A-7-5(17)	58	17	4.7	25.6	40.9	28.7	100	99	79	-	-
SS-11	13 RT	28+10 -L1-	4.90-5.90	A-7-5(30)	60	25	0.8	9.2	38.7	51.3	100	100	94	-	-
SS-12	13 RT	28+10 -L1-	9.90-10.90	A-7-5(26)	61	25	2.5	18.1	44.6	34.9	100	99	86	-	-
SS-13	60 RT	34+00 -L1-	3.60-4.60	A-7-6(21)	49	23	10.7	7.0	27.0	55.4	100	91	85	-	-
S-14	40 RT	32+00 -L1-	0.00-19.00	A-4(0)	25	4	32.6	18.3	26.6	22.6	84	69	43	-	-
SS-15	5 RT	15+05 -L2-	4.80-5.80	A-4(0)	33	1	17.8	29.1	32.5	20.5	100	91	59	-	-
S-16	98 LT	14+30 -L2-	0.00-7.40	A-4(0)	22	NP	24.0	19.7	35.8	20.5	95	79	58	-	-
S-17	98 LT	14+30 -L2-	7.40-27.40	A-4(4)	34	10	21.5	26.3	21.4	30.8	100	86	57	-	-
S-18	30 LT	15+50 -L2-	0.00-18.00	A-7-6(10)	42	16	17.4	17.0	26.6	39.0	95	83	67	-	-
S-19	30 LT	15+50 -L2-	18.00-27.70	A-5(7)	42	10	17.0	17.6	34.6	30.8	100	87	70	-	-
S-20	99 RT	12+36 -L2-	5.00-18.10	A-7-5(45)	74	37	0.6	5.5	22.1	71.8	100	100	97	-	-
S-21	99 RT	12+36 -L2-	18.10-28.10	A-7-5(20)	54	20	1.0	22.6	35.4	41.0	100	100	84	-	-
S-22	CL	18+50 -L2-	3.00-17.00	A-4(2)	29	7	18.3	15.6	35.4	30.8	76	66	54	-	-
S-23	CL	18+50 -L2-	17.00-18.20	A-6(13)	39	13	2.3	14.6	42.2	41.0	100	98	92	-	-
SS-24	100 LT	22+00 -L1-	3.90-4.90	A-7-5(30)	64	32	11.5	6.8	26.4	55.4	98	90	82	-	-
SS-25	100 LT	22+00 -L1-	8.90-9.90	A-4(6)	40	8	11.9	26.1	37.4	24.6	100	92	69	-	-
SS-26	135 LT	22+00 -L1-	3.50-4.50	A-4(2)	37	8	20.3	20.1	30.9	28.7	72	62	47	-	-
SS-27	105 LT	22+50 -L1-	3.50-4.50	A-7-5(45)	71	39	2.9	3.9	27.6	65.6	100	98	95	-	-
SS-28	105 LT	22+50 -L1-	8.50-9.50	A-7-5(13)	45	11	4.5	13.3	53.4	28.7	100	98	88	-	-
SS-29	CL	57+00 -L3-	4.30-5.30	A-7-6(59)	82	59	6.2	5.9	26.4	61.5	100	95	90	-	-
SS-30	CL	57+00 -L3-	9.30-10.30	A-6(10)	40	19	12.7	23.0	41.7	22.6	90	82	64	-	-
SS-31	7LT	54+50 -L3-	4.00-5.00	A-7-5(30)	65	30	9.0	10.5	31.3	49.2	100	93	84	-	-
SS-32	7LT	54+50 -L3-	9.00-10.00	A-7-5(13)	53	12	8.8	18.1	44.4	28.7	100	94	79	-	-
SS-33	10 RT	52+00 -L3-	3.60-4.60	A-7-5(42)	75	38	6.8	6.2	23.5	63.6	100	94	90	-	-
SS-34	6 RT	49+50 -L3-	3.40-4.40	A-7-5(62)	90	51	0.4	3.7	22.1	73.8	100	100	98	-	-
SS-35	6 RT	49+50 -L3-	8.40-9.40	A-5(7)	41	9	12.5	22.6	42.4	22.6	100	91	72	-	-
SS-36	10 RT	46+80 -L3-	3.40-4.40	A-7-6(25)	54	26	10.7	5.5	40.7	43.1	100	91	86	-	-
SS-37	6 LT	42+25 -L3-	4.10-5.10	A-7-5(34)	64	34	9.8	4.3	38.7	47.2	100	92	88	-	-
SS-38	6 LT	42+25 -L3-	9.10-10.10	A-4(7)	38	8	13.3	16.6	45.4	24.6	100	90	77	-	-
SS-39	5 RT	40+55 -L3-	3.90-4.50	A-4(2)	37	4	15.6	34.3	41.9	8.2	100	94	59	-	-
SS-40	27 LT	38+00 -L3-	2.90-3.90	A-7-5(15)	49	16	7.4	15.2	34.4	43.1	100	96	81	-	-
SS-41	10 LT	20+50 -L5-	3.80-4.80	A-7-5(16)	49	18	2.5	31.4	43.6	22.6	100	99	79	-	-
SS-42	10 LT	20+50 -L5-	8.80-9.80	A-6(10)	40	13	3.1	31.8	42.6	22.6	100	99	74	-	-
SS-43	11 LT	18+00 -L5-	4.00-5.00	A-6(10)	40	11	8.4	15.8	47.1	28.7	100	97	81	-	-
SS-44A	15 RT	13+10 -L5-	4.00-5.00	A-5(6)	41	8	10.5	23.6	39.3	26.7	100	95	71	-	-
SS-44	10 LT	15+50 -L5-	3.60-4.60	A-7-5(20)	52	20	1.6	19.5	44.0	34.9	100	100	86	-	-
SS-45	10 LT	15+50 -L5-	8.60-9.60	A-5(11)	44	10	2.3	22.2	51.0	24.6	100	99	84	-	-
SS-45A	CL	12+50 -L4-	4.00-5.00	A-4(1)	34	8	34.3	26.9	24.5	14.4	95	73	43	-	-
SS-46	7 LT	15+07 -L4-	4.30-5.30	A-7-5(26)	57	21	1.8	5.3	45.6	47.2	100	99	95	-	-
SS-47	7 LT	15+07 -L4-	9.30-10.30	A-5(9)	46	4	0.4	10.3	64.7	24.6	100	100	95	-	-
SS-48	12 LT	17+50 -L4-	4.30-5.30	A-7-6(12)	42	14	12.1	13.7	31.1	43.1	100	92	78	-	-
SS-49	12 LT	17+50 -L4-	14.30-15.30	A-7-5(34)	66	26	0.6	4.9	39.1	55.4	100	100	98	-	-
SS-50	12 LT	17+50 -L4-	19.30-20.30	A-7-5(8)	45	11	10.5	28.5	38.5	22.6	100	95	69	-	-
SS-51	8 RT	20+00 -L4-	4.50-5.50	A-7-5(22)	54	22	7.8	11.3	33.7	47.2	100	94	84	-	-
SS-52	8 RT	20+00 -L4-	9.50-10.50	A-7-6(16)	47	22	8.4	21.3	27.2	43.1	100	94	74	-	-
SS-53	CL	36+50 -L3-	3.90-4.90	A-7-5(16)	48	14	8.0	7.4	53.8	30.8	100	94	88	-	-
SS-54	CL	34+00 -L3-	3.80-4.80	A-7-5(32)	67	30	7.2	8.4	31.1	53.3	100	94	87	-	-
SS-55	CL	34+00 -L3-	8.80-9.80	A-7-5(11)	48	11	8.2	18.9	46.3	26.7	98	91	79	-	-
SS-56	13 RT	31+50 -L3-	8.80-9.80	A-5(8)	48	5	8.6	13.3	59.6	18.5	97	90	81	-	-
SS-57	8 LT	29+00 -L3-	3.90-4.90	A-7-6(16)	48	21	12.1	19.1	33.9	34.9	100	93	74	-	-
SS-58	8 LT	29+00 -L3-	8.80-9.80	A-4(6)	40	6	9.8	16.6	55.1	18.5	100	93	80	-	-
SS-59	CL	26+50 -L3-	3.70-4.70	A-4(3)	37	4	13.7	25.2	46.7	14.4	100	92	68	-	-
SS-60	CL	26+50 -L3-	8.70-9.70	A-4(2)	34	4	14.2	28.7	42.8	14.4	100	93	64	-	-
SS-61	12 LT	19+00 -L3-	4.50-5.50	A-7-5(31)	67	29	4.9	12.1	27.6	55.4	100	97	87	-	-
SS-62	10 RT	17+40 -L3-	4.60-5.60	A-4(7)	38	10	10.7	20.5	38.1	30.8	100	96	73	-	-
SS-63	132 RT	17+08 -L2-	4.30-5.30	A-6(10)	40	11	7.4	15.1	47.4	30.1	99	94	83	-	-
SS-64	50 RT	17+32 -L2-	9.30-10.30	A-7-6(13)	42	14	7.2	12.0	44.6	36.1	96	92	82	-	-

SOIL TEST RESULTS

SAMPLE NO.	OFFSET	STATION	DEPTH INTERVAL	AASHTO CLASS.	L.L.	P.I.	% BY WEIGHT				% PASSING (SIEVES)			%	%
							C.SAND	F.SAND	SILT	CLAY	10	40	200		
SS-65	50 RT	17+32 -L2-	14.50-15.50	A-6(8)	39	16	18.5	20.5	32.9	28.1	95	84	63	-	-
SS-66	50 RT	17+32 -L2-	19.50-20.50	A-7-6(21)	50	27	11.0	12.4	38.4	38.2	96	89	77	-	-
SS-67	50 RT	17+32 -L2-	24.50-25.50	A-7-6(5)	43	19	42.6	11.0	24.3	22.1	93	62	45	-	-
SS-68	50 RT	17+32 -L2-	29.50-30.50	A-7-5(24)	60	19	0.6	12.0	41.2	46.2	100	100	92	-	-
SS-69	220 RT	16+60 -L2-	4.80-5.80	A-7-6(14)	45	17	10.2	12.0	43.6	34.1	95	88	78	-	-
SS-70	220 RT	16+60 -L2-	14.80-15.80	A-6(1)	34	13	29.7	19.1	31.1	20.1	69	54	38	-	-
SS-71	220 RT	16+60 -L2-	19.80-20.80	A-4(4)	31	8	17.3	13.7	36.9	32.1	92	80	67	-	-
SS-72	220 RT	16+60 -L2-	29.80-30.80	A-7-5(56)	86	54	6.2	7.2	24.3	62.2	100	96	89	-	-
SS-73	115 RT	20+80 -L5-	0.50-1.50	A-7-5(18)	50	15	1.4	11.8	52.6	34.1	100	99	93	-	-
SS-74	150 RT	19+12 -L5-	11.30-12.30	A-4(2)	33	6	28.5	22.1	35.3	14.1	99	84	54	-	-