

NOTE: SEE SHEET 1A FOR PLAN SHEET LAYOUT AT TIME OF INVESTIGATION

STATE OF NORTH CAROLINA
DEPARTMENT OF TRANSPORTATION
DIVISION OF HIGHWAYS
GEOTECHNICAL ENGINEERING UNIT

STATE	STATE PROJECT REFERENCE NO.	SHEET NO.	TOTAL SHEETS
N.C.	B-4101	1	10
W.D. NO.	F.A. PROJ. NO.	DESCRIPTION	
33457.1.1	BRSTP-1741(3)	P.E.	
33457.2.1	BRSTP-1741(3)	R/W, UTIL	
33457.3.1	BRSTP-1741(3)	CONST	

CONTENTS

LINE	STATION	PLAN	PROFILE	XSECT
-L-	17+00 TO 26+50	4	5	7-8
-DET-	10+00 TO 18+76.75	4	6	

ROADWAY
SUBSURFACE INVESTIGATION

PROJ. REFERENCE NO. 33457.1.1 (B-4101) F.A. PROJ. BRSTP-1741(3)
COUNTY DAVIDSON
PROJECT DESCRIPTION BRIDGE 141 OVER SPURGEON CREEK ON SR 1741

CAUTION NOTICE

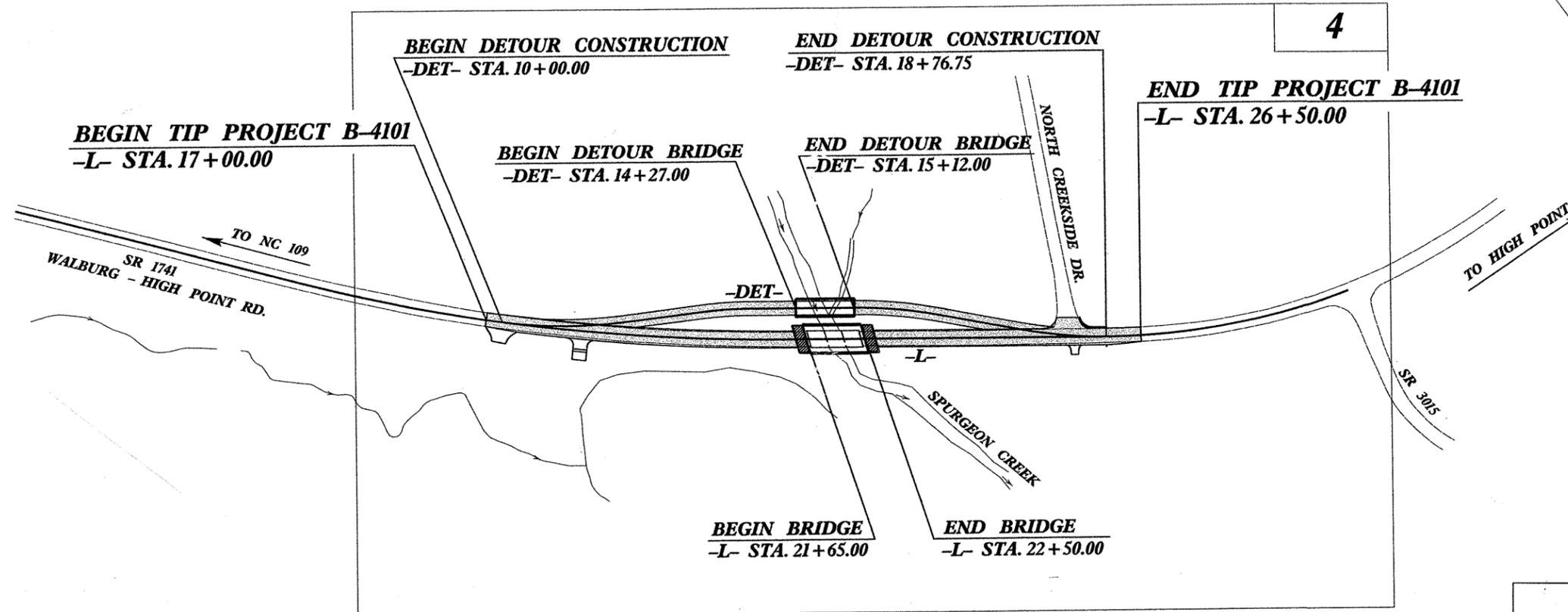
THE SUBSURFACE INFORMATION AND THE SUBSURFACE INVESTIGATION ON WHICH IT IS BASED WERE MADE FOR THE PURPOSE OF STUDY, PLANNING, AND DESIGN, AND NOT FOR CONSTRUCTION OR PAY PURPOSES. THE VARIOUS FIELD BORING LOGS, ROCK CORES, AND SOIL TEST DATA AVAILABLE MAY BE REVIEWED OR INSPECTED IN RALEIGH BY CONTACTING THE N.C. DEPARTMENT OF TRANSPORTATION, GEOTECHNICAL ENGINEERING UNIT AT (919) 250-4088. NEITHER THE SUBSURFACE PLANS AND REPORTS, NOR THE FIELD BORING LOGS, ROCK CORES, OR SOIL TEST DATA ARE PART OF THE CONTRACT.

GENERAL SOIL AND ROCK STRATA DESCRIPTIONS AND INDICATED BOUNDARIES ARE BASED ON A GEOTECHNICAL INTERPRETATION OF ALL AVAILABLE SUBSURFACE DATA AND MAY NOT NECESSARILY REFLECT THE ACTUAL SUBSURFACE CONDITIONS BETWEEN BORINGS OR BETWEEN SAMPLED STRATA WITHIN THE BOREHOLE. THE LABORATORY SAMPLE DATA AND THE IN SITU (IN-PLACE) TEST DATA CAN BE RELIED ON ONLY TO THE DEGREE OF RELIABILITY INHERENT IN THE STANDARD TEST METHOD. THE OBSERVED WATER LEVELS OR SOIL MOISTURE CONDITIONS INDICATED IN THE SUBSURFACE INVESTIGATIONS ARE AS RECORDED AT THE TIME OF THE INVESTIGATION. THESE WATER LEVELS OR SOIL MOISTURE CONDITIONS MAY VARY CONSIDERABLY WITH TIME ACCORDING TO CLIMATIC CONDITIONS INCLUDING TEMPERATURES, PRECIPITATION, AND WIND, AS WELL AS OTHER NON-CLIMATIC FACTORS.

THE BIDDER OR CONTRACTOR IS CAUTIONED THAT DETAILS SHOWN ON THE SUBSURFACE PLANS ARE PRELIMINARY ONLY AND IN MANY CASES THE FINAL DESIGN DETAILS ARE DIFFERENT. FOR BIDDING AND CONSTRUCTION PURPOSES, REFER TO THE CONSTRUCTION PLANS AND DOCUMENTS FOR FINAL DESIGN INFORMATION ON THIS PROJECT. THE DEPARTMENT DOES NOT WARRANT OR GUARANTEE THE SUFFICIENCY OR ACCURACY OF THE INVESTIGATION MADE, NOR THE INTERPRETATIONS MADE, OR OPINION OF THE DEPARTMENT AS TO THE TYPE OF MATERIALS AND CONDITIONS TO BE ENCOUNTERED. THE BIDDER OR CONTRACTOR IS CAUTIONED TO MAKE SUCH INDEPENDENT SUBSURFACE INVESTIGATIONS AS HE DEEMS NECESSARY TO SATISFY HIMSELF AS TO CONDITIONS TO BE ENCOUNTERED ON THIS PROJECT. THE CONTRACTOR SHALL HAVE NO CLAIM FOR ADDITIONAL COMPENSATION OR FOR AN EXTENSION OF TIME FOR ANY REASON RESULTING FROM THE ACTUAL CONDITIONS ENCOUNTERED AT THE SITE DIFFERING FROM THOSE INDICATED IN THE SUBSURFACE INFORMATION.

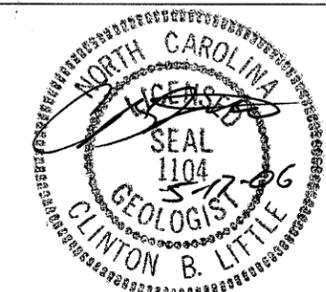
INVENTORY

CONTRACT: C201757 B-4101



- PERSONNEL
- LITTLE
 - MURRAY
 - ESTEP
 - HARPER

INVESTIGATED BY MURRAY
CHECKED BY LITTLE
SUBMITTED BY LITTLE
DATE APRIL 2006



DRAWN BY: LITTLE

NOTE - THE INFORMATION CONTAINED HEREIN IS NOT IMPLIED OR GUARANTEED BY THE N. C. DEPARTMENT OF TRANSPORTATION AS BEING ACCURATE NOR IS IT CONSIDERED TO BE PART OF THE PLANS, SPECIFICATIONS, OR CONTRACT FOR THE PROJECT.

NOTE - BY HAVING REQUESTED THIS INFORMATION THE CONTRACTOR SPECIFICALLY WAIVES ANY CLAIMS FOR INCREASED COMPENSATION OR EXTENSION OF TIME BASED ON DIFFERENCES BETWEEN THE CONDITIONS INDICATED HEREIN AND THE ACTUAL CONDITIONS AT THE PROJECT SITE.

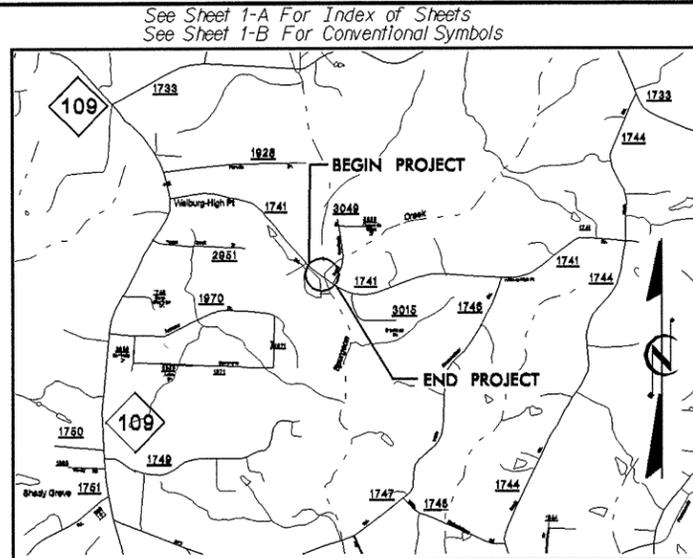
STATE	STATE PROJECT REFERENCE NO.	SHEET NO.	TOTAL SHEETS
N.C.	B-4101	1A	10
VEN. NO.	F.A. PROJ. NO.	DESCRIPTION	
33457.1.1	BRSTP-1741(3)	PE	

STATE OF NORTH CAROLINA
DIVISION OF HIGHWAYS

DAVIDSON COUNTY

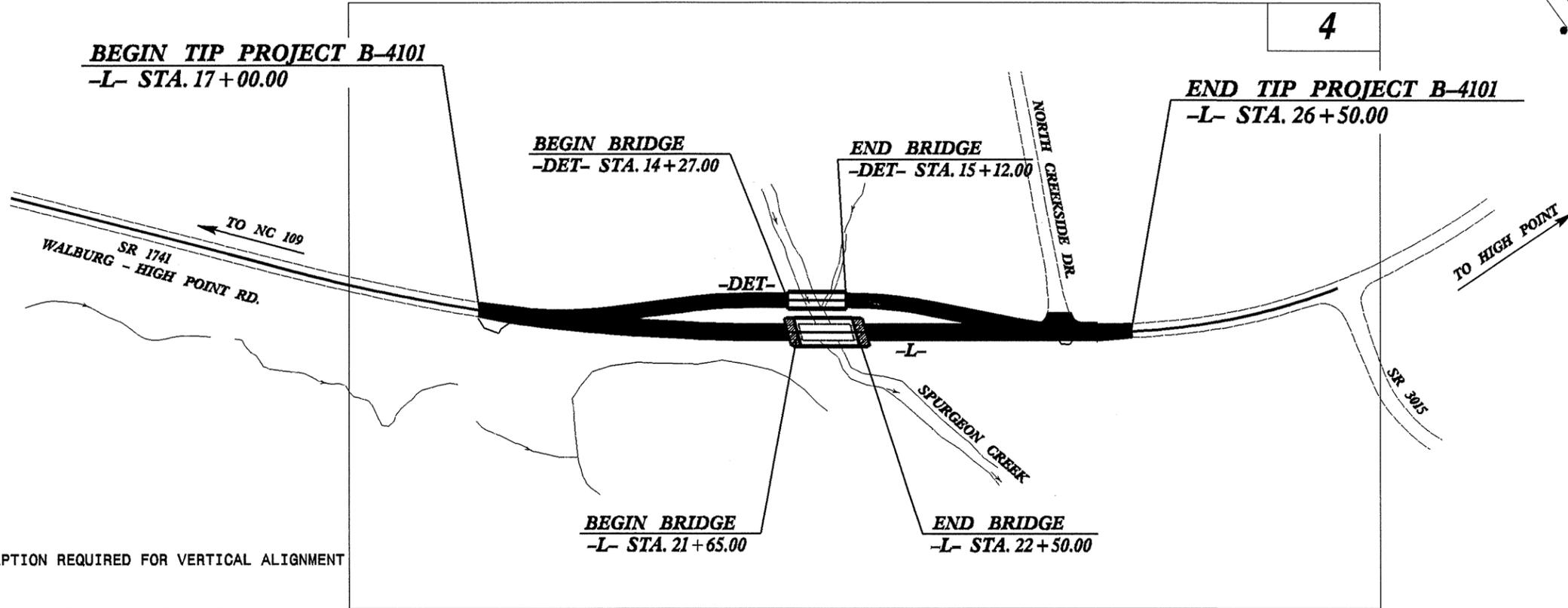
LOCATION: BRIDGE NO. 141 OVER SPURGEON CREEK ON SR 1741 (WALBURG-HIGH POINT RD.) IN THOMASVILLE

TYPE OF WORK: GRADING, DRAINAGE, PAVING & STRUCTURES



VICINITY MAP

(THIS PROJECT IS NOT WITHIN ANY MUNICIPAL BOUNDARIES.)



** DESIGN EXCEPTION REQUIRED FOR VERTICAL ALIGNMENT

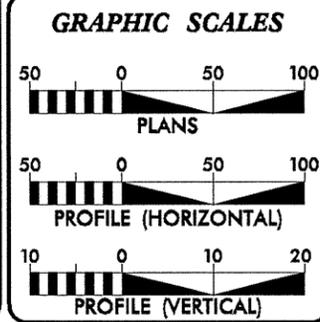
NCDOT CONTACT: CATHY HOUSER, P.E.
ROADWAY DESIGN - ENGINEERING COORDINATION

CLEARING ON THIS PROJECT SHALL BE PERFORMED TO THE LIMITS ESTABLISHED BY METHOD ____.

INCOMPLETE PLANS
DO NOT USE FOR R/W ACQUISITION
PRELIMINARY PLANS
DO NOT USE FOR CONSTRUCTION

TIP PROJECT: B-4101

CONTRACT:



DESIGN DATA

ADT 2007 =	2,580
ADT 2027 =	4,380
DHV =	10 %
D =	55 %
T =	4 % *
** V =	60 MPH
* TTST 1%	DUAL 3%

PROJECT LENGTH

LENGTH ROADWAY TIP PROJECT B-4101	=	0.164 MI.
LENGTH STRUCTURES TIP PROJECT B-4101	=	0.016 MI.
TOTAL LENGTH OF TIP PROJECT B-4101	=	0.180 MI.

Prepared in the Office of:

KO & ASSOCIATES, P.C.
Consulting Engineers
1011 Schaub Dr., Suite 202, Raleigh, NC 27606
(919) 851-9066

2002 STANDARD SPECIFICATIONS

RIGHT OF WAY DATE: APRIL 21, 2006

LETTING DATE: APRIL 17, 2007

DAVID C. WALLER, PE
PROJECT ENGINEER

STEPHEN R. WHITLEY, PE
PROJECT DESIGN ENGINEER

HYDRAULICS ENGINEER

SIGNATURE: _____ P.E.

ROADWAY DESIGN ENGINEER

SIGNATURE: _____ P.E.

**DIVISION OF HIGHWAYS
STATE OF NORTH CAROLINA**

STATE HIGHWAY DESIGN ENGINEER

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NORTH CAROLINA DEPARTMENT OF TRANSPORTATION
DIVISION OF HIGHWAYS
GEOTECHNICAL ENGINEERING UNIT

PROJECT REFERENCE NO. B-4101	SHEET NO. 2
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SUBSURFACE INVESTIGATION

SOIL AND ROCK LEGEND, TERMS, SYMBOLS, AND ABBREVIATIONS

SOIL DESCRIPTION	GRADATION	ROCK DESCRIPTION	TERMS AND DEFINITIONS
SOIL IS CONSIDERED TO BE THE UNCONSOLIDATED, SEMI-CONSOLIDATED, OR WEATHERED EARTH MATERIALS THAT CAN BE PENETRATED WITH A CONTINUOUS FLIGHT POWER AUGER, AND YIELD LESS THAN 100 BLOWS PER FOOT ACCORDING TO STANDARD PENETRATION TEST (AASHTO T206, ASTM D-1586). SOIL CLASSIFICATION IS BASED ON THE AASHTO SYSTEM. BASIC DESCRIPTIONS GENERALLY SHALL INCLUDE: CONSISTENCY, COLOR, TEXTURE, MOISTURE, AASHTO CLASSIFICATION, AND OTHER PERTINENT FACTORS SUCH AS MINERALOGICAL COMPOSITION, ANGULARITY, STRUCTURE, PLASTICITY, ETC. EXAMPLE: <i>VERY STIFF, GRAY, SILTY CLAY, MOIST WITH INTERBEDDED FINE SAND LAYERS, HIGHLY PLASTIC, A-7-6</i>	WELL GRADED - INDICATES A GOOD REPRESENTATION OF PARTICLE SIZES FROM FINE TO COARSE. UNIFORM - INDICATES THAT SOIL PARTICLES ARE ALL APPROXIMATELY THE SAME SIZE. (ALSO POORLY GRADED) GAP-GRADED - INDICATES A MIXTURE OF UNIFORM PARTICLES OF TWO OR MORE SIZES. THE ANGULARITY OR ROUNDNESS OF SOIL GRAINS IS DESIGNATED BY THE TERMS: ANGULAR, SUBANGULAR, SUBROUNDED, OR ROUNDED.	HARD ROCK IS NON-COASTAL PLAIN MATERIAL THAT IF TESTED, WOULD YIELD SPT REFUSAL. AN INFERRED ROCK LINE INDICATES THE LEVEL AT WHICH NON-COASTAL PLAIN MATERIAL WOULD YIELD SPT REFUSAL. SPT REFUSAL IS PENETRATION BY A SPLIT SPOON SAMPLER EQUAL TO OR LESS THAN 0.1 FOOT PER 60 BLOWS. IN NON-COASTAL PLAIN MATERIAL, THE TRANSITION BETWEEN SOIL AND ROCK IS OFTEN REPRESENTED BY A ZONE OF WEATHERED ROCK. ROCK MATERIALS ARE TYPICALLY DIVIDED AS FOLLOWS: WEATHERED ROCK (WR) NON-COASTAL PLAIN MATERIAL THAT WOULD YIELD SPT N VALUES > 100 BLOWS PER FOOT IF TESTED. CRYSTALLINE ROCK (CR) FINE TO COARSE GRAIN IGNEOUS AND METAMORPHIC ROCK THAT WOULD YIELD SPT REFUSAL IF TESTED. ROCK TYPE INCLUDES GRANITE, GNEISS, GABBRO, SCHIST, ETC. NON-CRYSTALLINE ROCK (NCR) FINE TO COARSE GRAIN METAMORPHIC AND NON-COASTAL PLAIN SEDIMENTARY ROCK THAT WOULD YIELD SPT REFUSAL IF TESTED. ROCK TYPE INCLUDES PHYLLITE, SLATE, SANDSTONE, ETC. COASTAL PLAIN SEDIMENTARY ROCK (CP) COASTAL PLAIN SEDIMENTS CEMENTED INTO ROCK, BUT MAY NOT YIELD SPT REFUSAL. ROCK TYPE INCLUDES LIMESTONE, SANDSTONE, CEMENTED SHELL BEDS, ETC.	ALLUVIUM (ALLUV.) - SOILS THAT HAVE BEEN TRANSPORTED BY WATER. AQUIFER - A WATER BEARING FORMATION OR STRATA. ARENACEOUS - APPLIED TO ROCKS THAT HAVE BEEN DERIVED FROM SAND OR THAT CONTAIN SAND. ARGILLACEOUS - APPLIED TO ALL ROCKS OR SUBSTANCES COMPOSED OF CLAY MINERALS, OR HAVING A NOTABLE PROPORTION OF CLAY IN THEIR COMPOSITION, AS SHALE, SLATE, ETC. ARTESIAN - GROUND WATER THAT IS UNDER SUFFICIENT PRESSURE TO RISE ABOVE THE LEVEL AT WHICH IT IS ENCOUNTERED, BUT WHICH DOES NOT NECESSARILY RISE TO OR ABOVE THE GROUND SURFACE. CALCAREOUS (CALC.) - SOILS THAT CONTAIN APPRECIABLE AMOUNTS OF CALCIUM CARBONATE. COLLUVIUM - ROCK FRAGMENTS MIXED WITH SOIL DEPOSITED BY GRAVITY ON SLOPE OR AT BOTTOM OF SLOPE. CORE RECOVERY (REC.) - TOTAL LENGTH OF ALL MATERIAL RECOVERED IN THE CORE BARREL DIVIDED BY TOTAL LENGTH OF CORE RUN AND EXPRESSED AS A PERCENTAGE. DIKE - A TABULAR BODY OF IGNEOUS ROCK THAT CUTS ACROSS THE STRUCTURE OF ADJACENT ROCKS OR CUTS MASSIVE ROCK. DIP - THE ANGLE AT WHICH A STRATUM OR ANY PLANAR FEATURE IS INCLINED FROM THE HORIZONTAL. DIP DIRECTION (DIP AZIMUTH) - THE DIRECTION OR BEARING OF THE HORIZONTAL TRACE OF THE LINE OF DIP, MEASURED CLOCKWISE FROM NORTH. FAULT - A FRACTURE OR FRACTURE ZONE ALONG WHICH THERE HAS BEEN DISPLACEMENT OF THE SIDES RELATIVE TO ONE ANOTHER PARALLEL TO THE FRACTURE. FISSILE - A PROPERTY OF SPLITTING ALONG CLOSELY SPACED PARALLEL PLANES. FLOAT - ROCK FRAGMENTS ON SURFACE NEAR THEIR ORIGINAL POSITION AND DISLODGED FROM PARENT MATERIAL. FLOOD PLAIN (FP) - LAND BORDERING A STREAM, BUILT OF SEDIMENTS DEPOSITED BY THE STREAM. FORMATION (FM) - A MAPPABLE GEOLOGIC UNIT THAT CAN BE RECOGNIZED AND TRACED IN THE FIELD. JOINT - FRACTURE IN ROCK ALONG WHICH NO APPRECIABLE MOVEMENT HAS OCCURRED. LEDGE - A SHELF-LIKE RIDGE OR PROJECTION OF ROCK WHOSE THICKNESS IS SMALL COMPARED TO ITS LATERAL EXTENT. LENS - A BODY OF SOIL OR ROCK THAT THINS OUT IN ONE OR MORE DIRECTIONS. MOTTLED (MOT.) - IRREGULARLY MARKED WITH SPOTS OF DIFFERENT COLORS. MOTTLING IN SOILS USUALLY INDICATES POOR AERATION AND LACK OF GOOD DRAINAGE. PERCHED WATER - WATER MAINTAINED ABOVE THE NORMAL GROUND WATER LEVEL BY THE PRESENCE OF AN INTERVENING IMPERVIOUS STRATUM. RESIDUAL (RES.) SOIL - SOIL FORMED IN PLACE BY THE WEATHERING OF ROCK. ROCK QUALITY DESIGNATION (RQD) - A MEASURE OF ROCK QUALITY DESCRIBED BY TOTAL LENGTH OF ROCK SEGMENTS EQUAL TO OR GREATER THAN 4 INCHES DIVIDED BY THE TOTAL LENGTH OF CORE RUN AND EXPRESSED AS A PERCENTAGE. SAPROLITE (SAP.) - RESIDUAL SOIL THAT RETAINS THE RELIC STRUCTURE OR FABRIC OF THE PARENT ROCK. SILL - AN INTRUSIVE BODY OF IGNEOUS ROCK OF APPROXIMATELY UNIFORM THICKNESS AND RELATIVELY THIN COMPARED WITH ITS LATERAL EXTENT, THAT HAS BEEN EMPLACED PARALLEL TO THE BEDDING OR SCHISTOSITY OF THE INTRUDED ROCKS. SLICKENSIDE - POLISHED AND STRIATED SURFACE THAT RESULTS FROM FRICTION ALONG A FAULT OR SLIP PLANE. STANDARD PENETRATION TEST (PENETRATION RESISTANCE) (SPT) - NUMBER OF BLOWS (N OR BPF) OF A 140 LB. HAMMER FALLING 30 INCHES REQUIRED TO PRODUCE A PENETRATION OF 1 FOOT INTO SOIL WITH A 2 INCH OUTSIDE DIAMETER SPLIT SPOON SAMPLER. SPT REFUSAL IS PENETRATION EQUAL TO OR LESS THAN 0.1 FOOT PER 60 BLOWS. STRATA CORE RECOVERY (SREC.) - TOTAL LENGTH OF STRATA MATERIAL RECOVERED DIVIDED BY TOTAL LENGTH OF STRATUM AND EXPRESSED AS A PERCENTAGE. STRATA ROCK QUALITY DESIGNATION (SRQD) - A MEASURE OF ROCK QUALITY DESCRIBED BY TOTAL LENGTH OF ROCK SEGMENTS WITHIN A STRATUM EQUAL TO OR GREATER THAN 4 INCHES DIVIDED BY THE TOTAL LENGTH OF STRATA AND EXPRESSED AS A PERCENTAGE. TOPSOIL (TS.) - SURFACE SOILS USUALLY CONTAINING ORGANIC MATTER.
SOIL LEGEND AND AASHTO CLASSIFICATION	MINERALOGICAL COMPOSITION	WEATHERING	
GENERAL CLASS. GRANULAR MATERIALS (<= 35% PASSING #200) SILT-CLAY MATERIALS (> 35% PASSING #200) ORGANIC MATERIALS	MINERAL NAMES SUCH AS QUARTZ, FELDSPAR, MICA, TALC, KAOLIN, ETC. ARE USED IN DESCRIPTIONS WHENEVER THEY ARE CONSIDERED OF SIGNIFICANCE.	FRESH ROCK FRESH, CRYSTALS BRIGHT, FEW JOINTS MAY SHOW SLIGHT STAINING. ROCK RINGS UNDER HAMMER IF CRYSTALLINE. VERY SLIGHT (V SLI.) ROCK GENERALLY FRESH, JOINTS STAINED, SOME JOINTS MAY SHOW THIN CLAY COATINGS IF OPEN. CRYSTALS ON A BROKEN SPECIMEN FACE SHINE BRIGHTLY. ROCK RINGS UNDER HAMMER BLOWS IF OF A CRYSTALLINE NATURE. SLIGHT (SLI.) ROCK GENERALLY FRESH, JOINTS STAINED AND DISCOLORATION EXTENDS INTO ROCK UP TO 1 INCH. OPEN JOINTS MAY CONTAIN CLAY. IN GRANITOID ROCKS SOME OCCASIONAL FELDSPAR CRYSTALS ARE DULL AND DISCOLORED. CRYSTALLINE ROCKS RING UNDER HAMMER BLOWS. MODERATE (MOD.) SIGNIFICANT PORTIONS OF ROCK SHOW DISCOLORATION AND WEATHERING EFFECTS. IN GRANITOID ROCKS, MOST FELDSPARS ARE DULL AND DISCOLORED, SOME SHOW CLAY. ROCK HAS DULL SOUND UNDER HAMMER BLOWS AND SHOWS SIGNIFICANT LOSS OF STRENGTH AS COMPARED WITH FRESH ROCK. MODERATELY SEVERE (MOD. SEV.) ALL ROCK EXCEPT QUARTZ DISCOLORED OR STAINED. IN GRANITOID ROCKS, ALL FELDSPARS DULL AND DISCOLORED AND A MAJORITY SHOW KAOLINIZATION. ROCK SHOWS SEVERE LOSS OF STRENGTH AND CAN BE EXCAVATED WITH A GEOLOGIST'S PICK. ROCK GIVES "CLUNK" SOUND WHEN STRUCK. IF TESTED, WOULD YIELD SPT REFUSAL. SEVERE (SEV.) ALL ROCK EXCEPT QUARTZ DISCOLORED OR STAINED. ROCK FABRIC CLEAR AND EVIDENT BUT REDUCED IN STRENGTH TO STRONG SOIL. IN GRANITOID ROCKS ALL FELDSPARS ARE KAOLINIZED TO SOME EXTENT. SOME FRAGMENTS OF STRONG ROCK USUALLY REMAIN. IF TESTED, YIELDS SPT N VALUES > 100 BPF. VERY SEVERE (V SEV.) ALL ROCK EXCEPT QUARTZ DISCOLORED OR STAINED. ROCK FABRIC ELEMENTS ARE DISCERNIBLE BUT THE MASS IS EFFECTIVELY REDUCED TO SOIL STATUS, WITH ONLY FRAGMENTS OF STRONG ROCK REMAINING. SAPROLITE IS AN EXAMPLE OF ROCK WEATHERED TO A DEGREE SUCH THAT ONLY MINOR VESTIGES OF THE ORIGINAL ROCK FABRIC REMAIN. IF TESTED, YIELDS SPT N VALUES < 100 BPF. COMPLETE ROCK REDUCED TO SOIL. ROCK FABRIC NOT DISCERNIBLE, OR DISCERNIBLE ONLY IN SMALL AND SCATTERED CONCENTRATIONS. QUARTZ MAY BE PRESENT AS DIKES OR STRINGERS. SAPROLITE IS ALSO AN EXAMPLE.	
COMPRESSION	PERCENTAGE OF MATERIAL	GROUND WATER	
SLIGHTLY COMPRESSIBLE LIQUID LIMIT LESS THAN 31 MODERATELY COMPRESSIBLE LIQUID LIMIT EQUAL TO 31-50 HIGHLY COMPRESSIBLE LIQUID LIMIT GREATER THAN 50	ORGANIC MATERIAL GRANULAR SOILS SILT - CLAY SOILS OTHER MATERIAL TRACE OF ORGANIC MATTER 2 - 3% 3 - 5% TRACE 1 - 10% LITTLE ORGANIC MATTER 3 - 5% 5 - 12% LITTLE 10 - 20% MODERATELY ORGANIC 5 - 10% 12 - 20% SOME 20 - 35% HIGHLY ORGANIC >10% >20% HIGHLY 35% AND ABOVE	WATER LEVEL IN BORE HOLE IMMEDIATELY AFTER DRILLING STATIC WATER LEVEL AFTER 24 HOURS PERCHED WATER, SATURATED ZONE, OR WATER BEARING STRATA SPRING OR SEEP	
CONSISTENCY OR DENSENESS	MISCELLANEOUS SYMBOLS	ROCK HARDNESS	
PRIMARY SOIL TYPE COMPACTNESS OR CONSISTENCY RANGE OF STANDARD PENETRATION RESISTANCE (N-VALUE) RANGE OF UNCONFINED COMPRESSIVE STRENGTH (TONS/FT ²)	ROADWAY EMBANKMENT (RE) WITH SOIL DESCRIPTION SOIL SYMBOL ARTIFICIAL FILL (AF) OTHER THAN ROADWAY EMBANKMENT INFERRED SOIL BOUNDARY INFERRED ROCK LINE ALLUVIAL SOIL BOUNDARY DIP & DIP DIRECTION OF ROCK STRUCTURES SOUNDING ROD	VERY HARD CANNOT BE SCRATCHED BY KNIFE OR SHARP PICK. BREAKING OF HAND SPECIMENS REQUIRES SEVERAL HARD BLOWS OF THE GEOLOGIST'S PICK. HARD CAN BE SCRATCHED BY KNIFE OR PICK ONLY WITH DIFFICULTY. HARD HAMMER BLOWS REQUIRED TO DETACH HAND SPECIMEN. MODERATELY HARD CAN BE SCRATCHED BY KNIFE OR PICK. GOUGES OR GROOVES TO 0.25 INCHES DEEP CAN BE EXCAVATED BY HARD BLOW OF A GEOLOGIST'S PICK. HAND SPECIMENS CAN BE DETACHED BY MODERATE BLOWS. MEDIUM HARD CAN BE GROOVED OR GOUGED 0.05 INCHES DEEP BY FIRM PRESSURE OF KNIFE OR PICK POINT. CAN BE EXCAVATED IN SMALL CHIPS TO PIECES 1 INCH MAXIMUM SIZE BY HARD BLOWS OF THE POINT OF A GEOLOGIST'S PICK. SOFT CAN BE GROOVED OR GOUGED READILY BY KNIFE OR PICK. CAN BE EXCAVATED IN FRAGMENTS FROM CHIPS TO SEVERAL INCHES IN SIZE BY MODERATE BLOWS OF A PICK POINT. SMALL, THIN PIECES CAN BE BROKEN BY FINGER PRESSURE. VERY SOFT CAN BE CARVED WITH KNIFE. CAN BE EXCAVATED READILY WITH POINT OF PICK. PIECES 1 INCH OR MORE IN THICKNESS CAN BE BROKEN BY FINGER PRESSURE. CAN BE SCRATCHED READILY BY FINGER NAIL.	SAPROLITE (SAP.) - RESIDUAL SOIL THAT RETAINS THE RELIC STRUCTURE OR FABRIC OF THE PARENT ROCK. SILL - AN INTRUSIVE BODY OF IGNEOUS ROCK OF APPROXIMATELY UNIFORM THICKNESS AND RELATIVELY THIN COMPARED WITH ITS LATERAL EXTENT, THAT HAS BEEN EMPLACED PARALLEL TO THE BEDDING OR SCHISTOSITY OF THE INTRUDED ROCKS. SLICKENSIDE - POLISHED AND STRIATED SURFACE THAT RESULTS FROM FRICTION ALONG A FAULT OR SLIP PLANE. STANDARD PENETRATION TEST (PENETRATION RESISTANCE) (SPT) - NUMBER OF BLOWS (N OR BPF) OF A 140 LB. HAMMER FALLING 30 INCHES REQUIRED TO PRODUCE A PENETRATION OF 1 FOOT INTO SOIL WITH A 2 INCH OUTSIDE DIAMETER SPLIT SPOON SAMPLER. SPT REFUSAL IS PENETRATION EQUAL TO OR LESS THAN 0.1 FOOT PER 60 BLOWS. STRATA CORE RECOVERY (SREC.) - TOTAL LENGTH OF STRATA MATERIAL RECOVERED DIVIDED BY TOTAL LENGTH OF STRATUM AND EXPRESSED AS A PERCENTAGE. STRATA ROCK QUALITY DESIGNATION (SRQD) - A MEASURE OF ROCK QUALITY DESCRIBED BY TOTAL LENGTH OF ROCK SEGMENTS WITHIN A STRATUM EQUAL TO OR GREATER THAN 4 INCHES DIVIDED BY THE TOTAL LENGTH OF STRATA AND EXPRESSED AS A PERCENTAGE. TOPSOIL (TS.) - SURFACE SOILS USUALLY CONTAINING ORGANIC MATTER.
TEXTURE OR GRAIN SIZE	ABBREVIATIONS	FRACTURE SPACING	BEDDING
U.S. STD. SIEVE SIZE OPENING (MM) 4 10 40 60 200 270 2.00 0.42 0.25 0.075 0.053	AR - AUGER REFUSAL BT - BORING TERMINATED CL - CLAY CPT - CONE PENETRATION TEST CSE - COARSE DMT - DILATOMETER TEST DPT - DYNAMIC PENETRATION TEST e - VOID RATIO F - FINE FOSS. - FOSSILIFEROUS FRAC. - FRACTURED, FRACTURES FRAGS. - FRAGMENTS HI. - HIGHLY MED. - MEDIUM MICA - MICACEOUS MOD. - MODERATELY NP - NON PLASTIC ORG. - ORGANIC PMT - PRESSUREMETER TEST SAP. - SAPROLITIC SD. - SAND, SANDY SL. - SILT, SILTY SLI. - SLIGHTLY TCR - TRICONE REFUSAL # - MOISTURE CONTENT v - VERY VST - VANE SHEAR TEST WEA. - WEATHERED w - UNIT WEIGHT w _d - DRY UNIT WEIGHT	VERY WIDE MORE THAN 10 FEET WIDE 3 TO 10 FEET MODERATELY CLOSE 1 TO 3 FEET CLOSE 0.16 TO 1 FEET VERY CLOSE LESS THAN 0.16 FEET	VERY THICKLY BEDDED > 4 FEET THICKLY BEDDED 1.5 - 4 FEET THINLY BEDDED 0.16 - 1.5 FEET VERY THINLY BEDDED 0.03 - 0.16 FEET THICKLY LAMINATED 0.008 - 0.03 FEET THINLY LAMINATED < 0.008 FEET
SOIL MOISTURE - CORRELATION OF TERMS	EQUIPMENT USED ON SUBJECT PROJECT	INDURATION	
SOIL MOISTURE SCALE (ATTERBERG LIMITS) FIELD MOISTURE DESCRIPTION GUIDE FOR FIELD MOISTURE DESCRIPTION	DRILL UNITS: MOBILE B- BK-51 CME-45C CME-550 PORTABLE HOIST ADVANCING TOOLS: CLAY BITS 6" CONTINUOUS FLIGHT AUGER 8" HOLLOW AUGERS HARD FACED FINGER BITS TUNG-CARBIDE INSERTS CASING w/ ADVANCER TRICONE STEEL TEETH TRICONE TUNG-CARB. CORE BIT HAMMER TYPE: AUTOMATIC MANUAL CORE SIZE: B N H HAND TOOLS: POST HOLE DIGGER HAND AUGER SOUNDING ROD VANE SHEAR TEST	FOR SEDIMENTARY ROCKS, INDURATION IS THE HARDENING OF THE MATERIAL BY CEMENTING, HEAT, PRESSURE, ETC. FRIABLE RUBBING WITH FINGER FREES NUMEROUS GRAINS; GENTLE BLOW BY HAMMER DISINTEGRATES SAMPLE. MODERATELY INDURATED GRAINS CAN BE SEPARATED FROM SAMPLE WITH STEEL PROBE; BREAKS EASILY WHEN HIT WITH HAMMER. INDURATED GRAINS ARE DIFFICULT TO SEPARATE WITH STEEL PROBE; DIFFICULT TO BREAK WITH HAMMER. EXTREMELY INDURATED SHARP HAMMER BLOWS REQUIRED TO BREAK SAMPLE; SAMPLE BREAKS ACROSS GRAINS.	BENCH MARK: BL-4 22+49.93 -L- 14.95' LT. ELEVATION: 783.30 FT. NOTES:
PLASTICITY			
NONPLASTIC LOW PLASTICITY MED. PLASTICITY HIGH PLASTICITY			
COLOR			
DESCRIPTIONS MAY INCLUDE COLOR OR COLOR COMBINATIONS (TAN, RED, YELLOW-BROWN, BLUE-GRAY). MODIFIERS SUCH AS LIGHT, DARK, STREAKED, ETC. ARE USED TO DESCRIBE APPEARANCE.			



STATE OF NORTH CAROLINA
DEPARTMENT OF TRANSPORTATION

MICHAEL F. EASLEY
GOVERNOR

LYNDO TIPPETT
SECRETARY

April 12, 2006

STATE PROJECT: 33457.1.1 (B-4101)
FEDERAL PROJECT: BRSTP-1741(3)
COUNTY: Davidson
DESCRIPTION: Bridge No. 141 over Spurgeon Creek
On SR 1741 (Walburg – High Point Road)

SUBJECT: Geotechnical Report - Inventory

PROJECT DESCRIPTION

The project is located in northeastern Davidson County, west of High Point. The project consists of a bridge replacement with an on-site detour. This report addresses the roadway approaches for the replacement bridge plus the detour roadway. The alignment runs west to east from Station 17+00 to 26+50 –L- and 10+00 to 18+76.75 –DET-, for a total length of lines investigated of 1826.75' or 0.346 miles.

The Geotechnical investigation consisted of four Standard Penetration Test (SPT) borings for the bridge foundation and one hand auger boring with Vane Shear Tests and two undisturbed (Shelby Tube) samples obtained by hand. An additional boring obtained for the preliminary planning documents is also included. It was located very close to boring EB1-A and is labeled "PDEA" on the plan and profile sheets. The PDEA boring was conducted in May 2004 with 8" Hollow Stem augers, the remaining borings were conducted in February 2006 with "NW" casing, roller cone, and water.

AREAS OF SPECIAL GEOTECHNICAL INTEREST

Alluvial Soils: The alluvial soils on the western approach are very soft and saturated. The worst area is within the area delimited as wetlands on the roadway plans. The soils would not support the drill rig. Tests indicate the soils are weak and compressible. The AASHTO classification is A-7-5. The Group Index is 18 to 35 and the fines content is 78 to 91%.

PHYSIOGRAPHY AND GEOLOGY

The project is located in the piedmont region of North Carolina, within the Charlotte Geologic Belt. The rock units in the area are mapped as metamorphosed mafic igneous rocks and meta-granite. No rock cores were obtained during the investigation. Samples of saprolite were consistent with the mapped units. Elevations on the project range from highs of about 790 feet at either end, about 775 across the floodplain, and 768 in the stream channel.

SOIL PROPERTIES

Residual Soils

Residual soils occur below the alluvium and outside of the floodplain area. They were described as:

- 1) Olive-tan-brown micaceous silty or clayey sand with vertical foliation probably derived from mafic metavolcanics (altered basalts)
- 2) Tan-white or red-brown silty sands that appear granitic in nature.

The granitic soils were predominant. On the western approach, the residual soil layer was thin (2-4') and graded quickly to weathered rock. The soils were deeper on the eastern approach (20' to 30') and terminated abruptly on rock.

Artificial/Roadway Fill Soils

Existing fill soils average ten feet in height across the floodplain. They were described as red-brown medium stiff silty clay.

Alluvial Soils

The majority of the floodplain falls on the western side of the stream but alluvial soils were noted in the End Bent Two borings. Please refer to the section on Areas of Special Interest.

GROUNDWATER

Static groundwater was measured in the boreholes between elevations 770 – 775. Water was on the ground surface in the wetland area (Station 12+00 to 14+00 –DET-).

Respectfully submitted,

Clint Little
Regional Geologist

STATION	STATION	EXCAVATION				EMBANKMENT			BORROW	WASTE		
		TOTAL UNCLASS.	ROCK	UNDERCUT	UNSUIT. UNCLASS.	SUITABLE UNCLASS.	TOTAL	ROCK		EARTH	EMBANK. (+) 15 %	ROCK
SUMMARY NO. 1												
	DETOUR (-L- 17+24 TO 21+50)	27				27	2395	2395	2755	2728		
	TOTAL SUMMARY NO. 1	27				27	2395	2395	2755	2728		
SUMMARY NO. 2												
	DETOUR (-L- 22+35 TO 25+25)	177				177	411	411	473	296		
	TOTAL SUMMARY NO. 2	177				177	411	411	473	296		
	SUB-TOTAL (SUMMARY 1 THRU 2)	204				204	2806	2806	3228	3024		
	EST. BORROW FOR SHOULDER CONST.						207	207	239	239		
	DETOUR TOTALS	204				204	3013	3013	3467	3263		
SUMMARY NO. 3												
	-L- 17+00 TO 21+00 LEFT	4				4	88	88	102	98		
	TOTAL SUMMARY NO. 3	4				4	88	88	102	98		
SUMMARY NO. 4												
	-L- 17+00 TO 21+00 RIGHT	16				16	658	658	757	741		
	TOTAL SUMMARY NO. 4	16				16	658	658	757	741		
SUMMARY NO. 5												
	-L- 21+00 TO 21+65(BRIDGE)	0				0	278	278	320	320		
	TOTAL SUMMARY NO. 5	0				0	278	278	320	320		
SUMMARY NO. 6												
	-L- 22+50(BRIDGE) TO 24+00	0				0	614	614	707	707		
	TOTAL SUMMARY NO. 6	0				0	614	614	707	707		
SUMMARY NO. 7												
	-L- 24+00 TO 26+50 LEFT	5				5	1	1	2		3	
	TOTAL SUMMARY NO. 7	5				5	1	1	2		3	
SUMMARY NO. 8												
	-L- 24+00 TO 26+50 RIGHT	14				14	159	159	183	169		
	TOTAL SUMMARY NO. 8	14				14	159	159	183	169		
	SUB-TOTAL (SUMMARY 3 THRU 8)	39				39	1798	1798	2071	2035	3	
	EST. BORROW IN LIEU OF WASTE									-3	-3	
	-L- TOTALS	39				39	1798	1798	2071	2032		
SUMMARY NO. 9												
	DETOUR REMOVAL(-L- 18+50 TO 21+50)	2181				2181	0	0	0		2181	
	TOTAL SUMMARY NO. 9	2181				2181	0	0	0		2181	
SUMMARY NO. 10												
	DETOUR REMOVAL(-L- 22+50 TO 25+50)	420				420	30	30	35		385	
	TOTAL SUMMARY NO. 10	420				420	30	30	35		385	
	SUB-TOTAL (SUMMARY 9 THRU 10)	2601				2601	30	30	35		2566	
	DETOUR REMOVAL TOTALS	2601				2601	30	30	35		2566	
	PROJECT TOTALS	2844				2844	4841	4841	5573	5295	2566	
	ESTIMATE 5% TO REPLACE TOPSOIL IN BORROW PIT									265		
	GRAND TOTAL	2844								5560	2566	
	SAY	2850								5600	2600	

ADDITIONAL UNDERCUT = 1000 CY
 SELECT GRANULAR MATERIAL = 1500 CY
 DDE = 30 CY

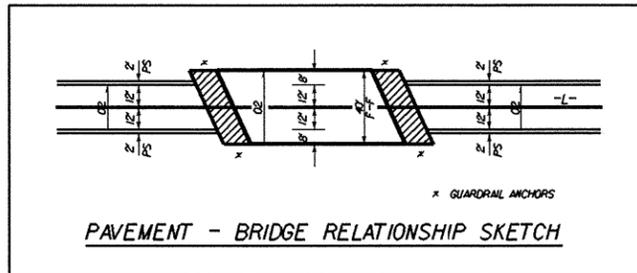
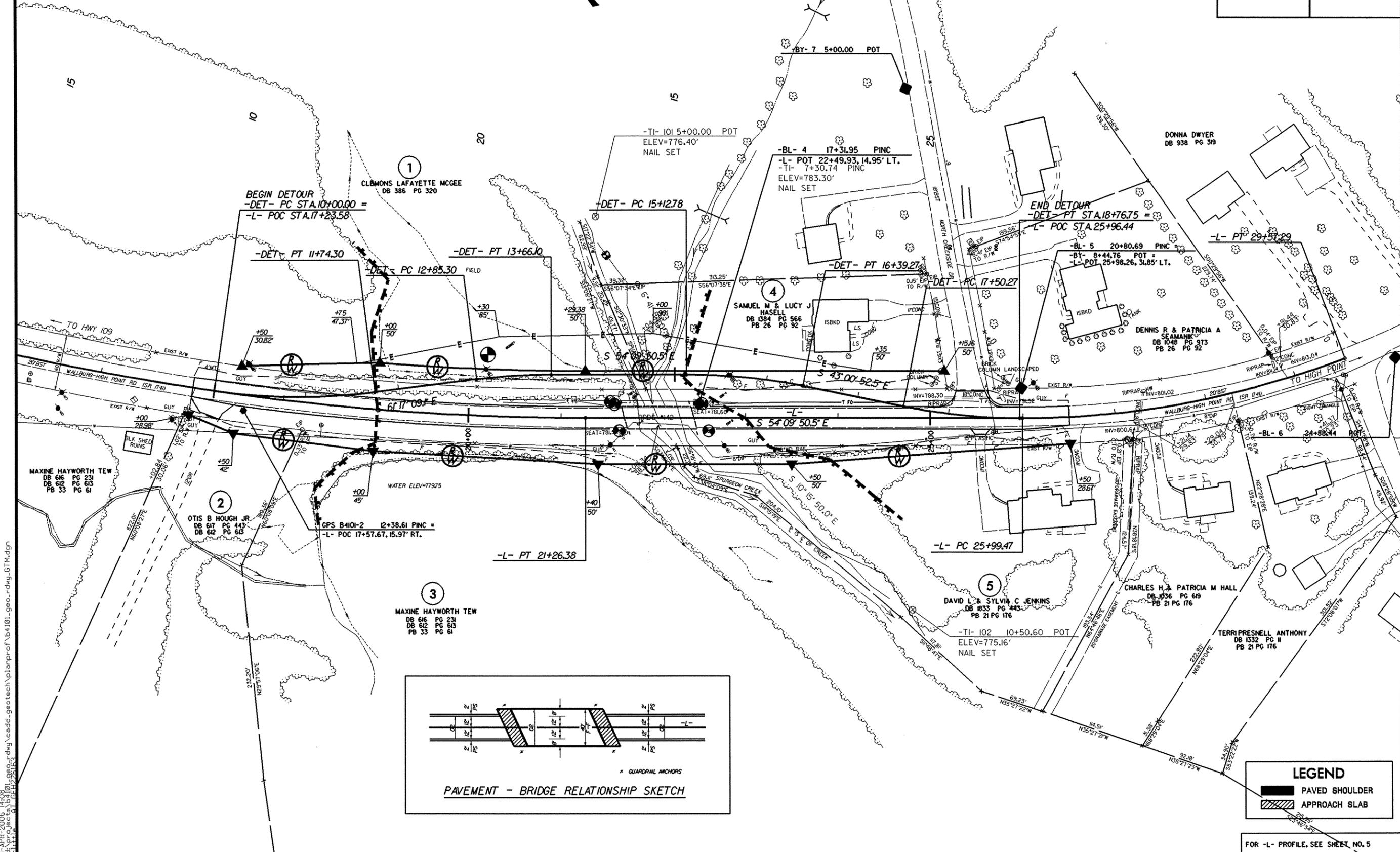
8/17/99

KO & ASSOCIATES, P.C.
Consulting Engineers
1011 BEAVER DR., SUITE 202 RALEIGH, N.C. 27606
(919) 883-6066

PROJECT REFERENCE NO. B-4101		SHEET NO. 4	
RW SHEET NO.			
ROADWAY DESIGN ENGINEER		HYDRAULICS ENGINEER	
PRELIMINARY PLANS DO NOT USE FOR CONSTRUCTION			
INCOMPLETE PLANS DO NOT USE FOR R/W ACQUISITION			

DETOUR

PI Sta 10+87.68 Δ = 15° 21' 51" (LT) D = 8' 48" 53.0" L = 174.30' T = 87.68' R = 650.00'	PI Sta 13+25.75 Δ = 7° 07' 18.5" (RT) D = 8' 48" 53.0" L = 80.79' T = 40.45' R = 650.00'	PI Sta 15+76.22 Δ = 1° 08' 58.0" (RT) D = 8' 48" 53.0" L = 126.49' T = 63.44' R = 650.00'	PI Sta 18+13.71 Δ = 1° 08' 58.0" (LT) D = 8' 48" 53.0" L = 126.49' T = 63.44' R = 650.00'
---	---	--	--



LEGEND

	PAVED SHOULDER
	APPROACH SLAB

FOR -L- PROFILE, SEE SHEET NO. 5

IF-APR-2006 14:08
C:\projects\10101\9906\dwg\cadd\geotech\planprcf\10101-geo-r\dwy_67M.dgn
11/11/99 8:11:18 AM

5/14/99

IT-MAY-2006 08:42
d:\projects\4101-geo-rdw\cadd-geotech\planproj\4101-geo-rdw\l-det1.dgn

PROJECT REFERENCE NO. B-4101	SHEET NO. 6
ROADWAY DESIGN ENGINEER	HYDRAULICS ENGINEER
INCOMPLETE PLANS DO NOT USE FOR A/W ACQUISITION PRELIMINARY PLANS DO NOT USE FOR CONSTRUCTION	

TEST RESULTS			
Proj. Sample No.	HA-1	HA-2	HA-3
Lab. Sample No.	727531	727532	727533
Retained #4 Sieve	%	-	-
Passing #10 Sieve	%	100	100
Passing #40 Sieve	%	99	99
Passing #200 Sieve	%	92	83

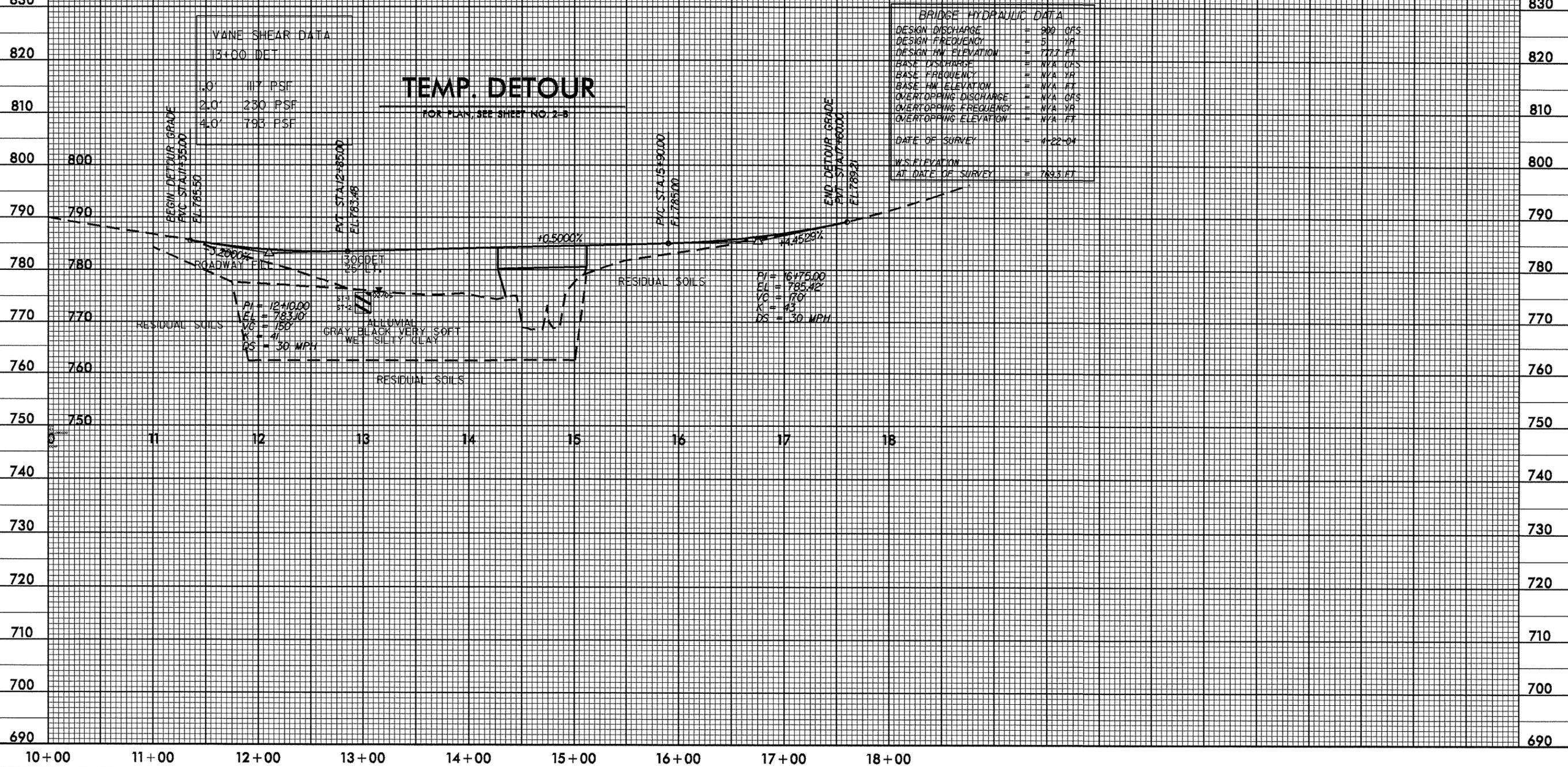
Proj. Sample No.		ST-1
Lab. Sample No.		728879
Retained #4 Sieve	%	
Passing #10 Sieve	%	100
Passing #40 Sieve	%	99
Passing #200 Sieve	%	89

MINUS NO. 10 FRACTION			
SOIL MORTAR - 100%			
Coarse Sand Ret - #60	%	2.0	8.9
Fine Sand Ret - #270	%	9.1	12.9
Silt 0.05 - 0.005 mm	%	44.6	33.9
Clay < 0.005 mm	%	44.4	44.4
Passing #40 Sieve	%	-	-
Passing #200 Sieve	%	-	-

SOIL MORTAR - 100%	
Coarse Sand Ret - #60	% 4.1
Fine Sand Ret - #270	% 11.1
Silt 0.05 - 0.005 mm	% 36.2
Clay < 0.005 mm	% 48.6
Passing #40 Sieve	%
Passing #200 Sieve	%

L.L.	68	52	55
P.I.	30	18	21
AASHTO Classification	A-7-5(35)	A-7-5(18)	A-7-5(25)
Station	13+00	13+00	13+00
OFFSET	25 LT	25 LT	25 LT
Hole No.			
Depth (Ft)	0.00	1.00	3.00
	to	1.00	3.00
		3.00	4.00

L.L.	53
P.I.	21
AASHTO Classification	A-7-5(22)
Station	20+25 - L-
OFFSET	57 LT.
Hole No.	
Depth (Ft)	1.9-3.9'
	to



BRIDGE HYDRAULIC DATA	
DESIGN DISCHARGE	= 900 GFS
DESIGN FREQUENCY	= 5 YR
DESIGN HW ELEVATION	= 777 FT
BASE DISCHARGE	= NYA GFS
BASE FREQUENCY	= NYA YR
BASE HW ELEVATION	= NYA FT
OVERTOPPING DISCHARGE	= NYA GFS
OVERTOPPING FREQUENCY	= NYA YR
OVERTOPPING ELEVATION	= NYA FT
DATE OF SURVEY	= 4-22-04
W.S. ELEVATION	
AT DATE OF SURVEY	= 769.5 FT

VANE SHEAR DATA	
13+00 DET	
1.0'	117 PSF
2.0'	230 PSF
4.0'	793 PSF

TEMP. DETOUR

FOR PLAN, SEE SHEET NO. 2-B

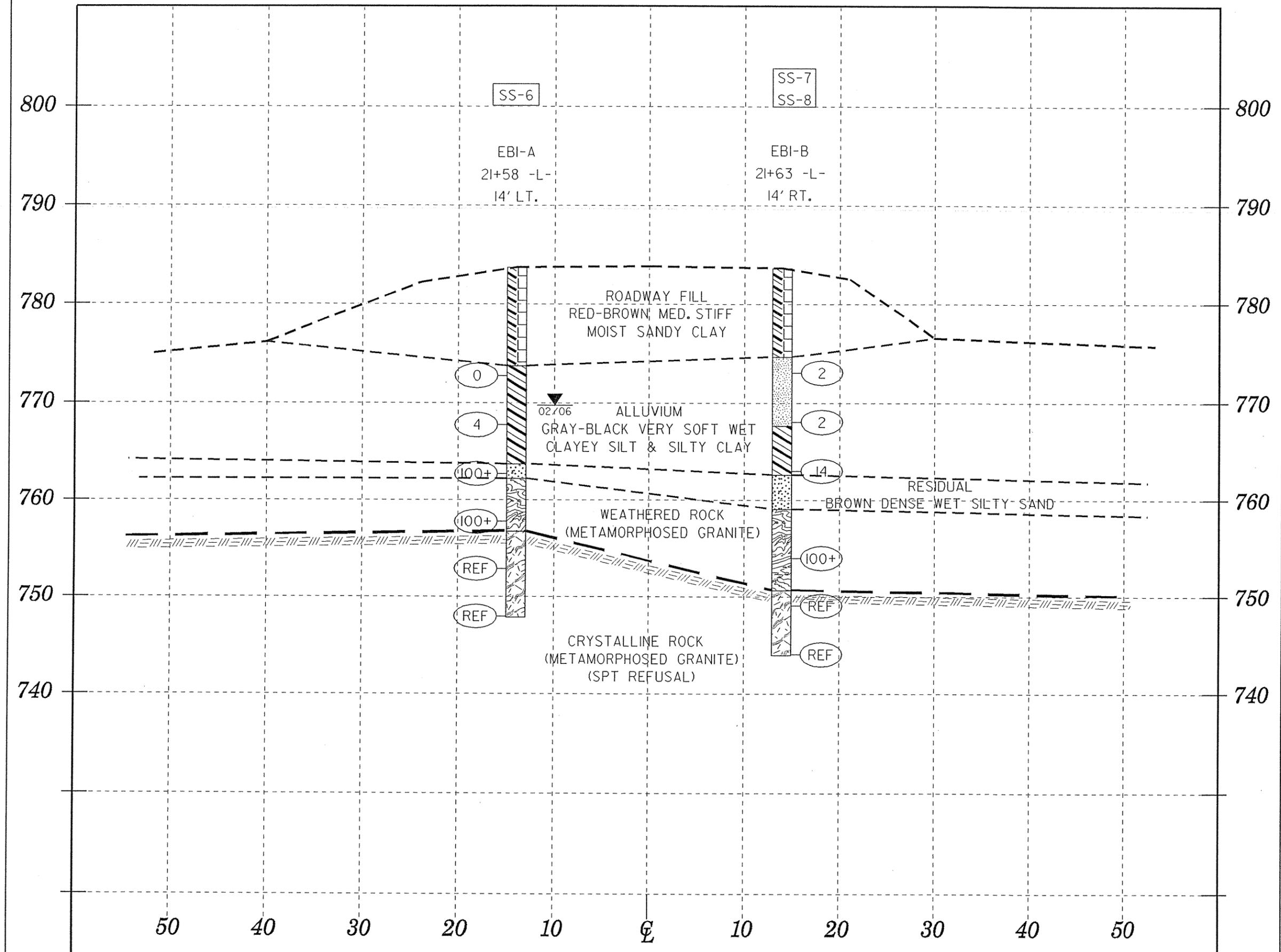
PI = 16+75.00
 EL = 785.42'
 VC = 170'
 K = 43
 DS = 30 MPH

PI = 12+10.00
 EL = 783.10'
 VC = 150'
 K = 41
 DS = 30 MPH

ALLUMIAL
 GRAY BLACK VERY SOFT
 WET SILTY CLAY

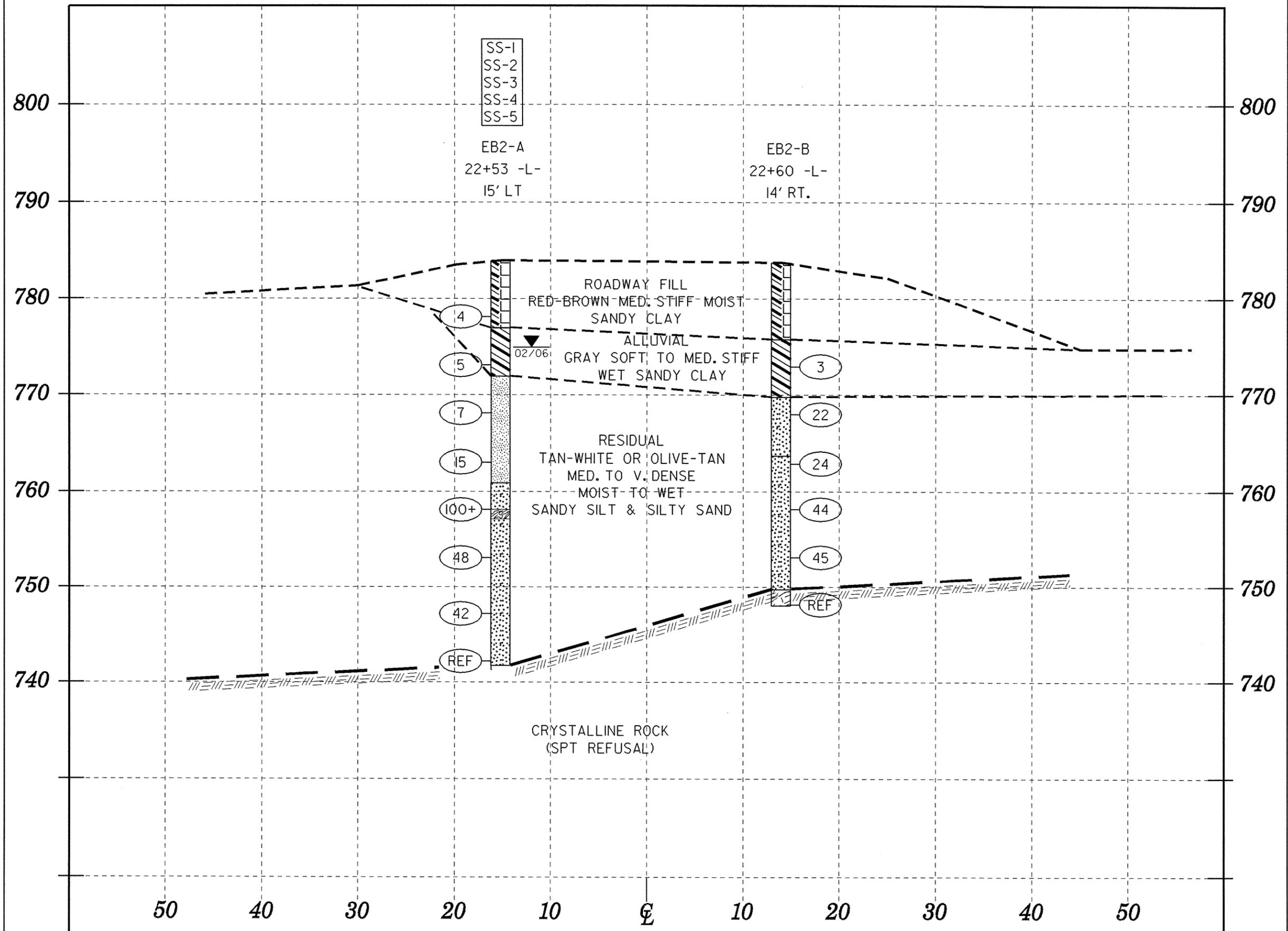
SECTION THROUGH EB1-A & EB1-B

PROJECT B-4101
COUNTY DAVIDSON



SECTION THROUGH EB2-A & EB2-B

PROJECT B-4101
COUNTY DAVIDSON



NORTH CAROLINA DEPARTMENT OF TRANSPORTATION
 DIVISION OF HIGHWAY
 MATERIALS & TESTS UNIT
 SOILS LABORATORY

T. I. P. No. B-4101

REPORT ON SAMPLES OF SOILS FOR QUALITY

Project 3345711 County DAVIDSON Owner _____
 Date: Sampled 2/8/06 Received 2/10/06 Reported 2/14/2006
 Sampled from _____ By C C MURRAY
 Submitted by N WAINAINA 1995 Standard Specifications

728287 TO 728294
 3/6/06

TEST RESULTS

Proj. Sample No.	SS-1	SS-2	SS-3	SS-4	SS-5	SS-6
Lab. Sample No.	728287	728288	728289	728290	728291	728292
Retained #4 Sieve %	1	-	-	-	-	-
Passing #10 Sieve %	95	99	99	99	97	97
Passing #40 Sieve %	71	87	92	92	81	80
Passing #200 Sieve %	40	56	46	43	30	55

MINUS NO. 10 FRACTION

SOIL MORTAR - 100%						
Coarse Sand Ret - #60 %	35.3	19.4	17.6	16.3	32.5	23.7
Fine Sand Ret - #270 %	27.4	31.7	46.4	50.7	43.1	23.7
Silt 0.05 - 0.005 mm %	18.9	28.5	27.9	28.9	20.3	22.0
Clay < 0.005 mm %	18.4	20.4	8.2	4.1	4.1	30.6
Passing #40 Sieve %	-	-	-	-	-	-
Passing #200 Sieve %	-	-	-	-	-	-

L. L.	37	38	36	35	31	33
P. I.	13	12	NP	NP	NP	15
AASHTO Classification	A-6(2)	A-6(5)	A-4(0)	A-4(0)	A-2-4(0)	A-6(5)
Station	22+53	22+53	22+53	22+53	22+53	21+58
OFFSET	15 LT	14 LT				
ALIGNMENT	L	L	L	L	L	L
Depth (Ft)	5.80	10.80	15.80		30.80	11.00
to	7.30	12.30	17.30		32.30	12.50

cc: C C MURRAY
 Soils File

9/10

NORTH CAROLINA DEPARTMENT OF TRANSPORTATION
 DIVISION OF HIGHWAY
 MATERIALS & TESTS UNIT
 SOILS LABORATORY

T. I. P. No. _____

REPORT ON SAMPLES OF SOILS FOR QUALITY

Project 3345711 County DAVIDSON Owner _____
 Date: Sampled 2/8/06 Received 2/10/06 Reported 2/14/2006
 Sampled from _____ By C C MURRAY
 Submitted by N WAINAINA 1995 Standard Specifications

728287 TO 728294
 3/6/06

TEST RESULTS

Proj. Sample No.	SS-7	SS-8			
Lab. Sample No.	728293	728294			
Retained #4 Sieve %	-	-			
Passing #10 Sieve %	100	97			
Passing #40 Sieve %	99	80			
Passing #200 Sieve %	71	52			

MINUS NO. 10 FRACTION

SOIL MORTAR - 100%					
Coarse Sand Ret - #60 %	5.9	25.5			
Fine Sand Ret - #270 %	31.7	25.7			
Silt 0.05 - 0.005 mm %	35.9	20.1			
Clay < 0.005 mm %	26.6	28.6			
Passing #40 Sieve %	-	-			
Passing #200 Sieve %	-	-			

L. L.	35	33			
P. I.	10	13			
AASHTO Classification	A-4(6)	A-6(4)			
Station	21+63	21+63			
	14 RT	14 RT			
Hole No.	L	L			
Depth (Ft)	10.60	15.60			
to	12.10	17.10			

NORTH CAROLINA DEPARTMENT OF TRANSPORTATION
DIVISION OF HIGHWAY
MATERIALS & TESTS UNIT
SOILS LABORATORY

T. I. P. No. B-4101

REPORT ON SAMPLES OF SOILS FOR QUALITY

Project 33457.1.1 County DAVIDSON Owner _____
Date: Sampled 3/20/06 Received 3/31/06 Reported 04/04/06
Sampled from BRIDGE By C B LITTLE
Submitted by R PRIVETTE T-4926 1995 Standard Specifications

728879 TO 728879
4/5/06

TEST RESULTS

Proj. Sample No.		ST-1					
Lab. Sample No.		728879					
Retained #4 Sieve	%	-					
Passing #10 Sieve	%	100					
Passing #40 Sieve	%	99					
Passing #200 Sieve	%	89					

MINUS NO. 10 FRACTION

SOIL MORTAR - 100%							
Coarse Sand Ret - #60	%	4.1					
Fine Sand Ret - #270	%	11.1					
Silt 0.05 - 0.005 mm	%	36.2					
Clay < 0.005 mm	%	48.6					
Passing #40 Sieve	%	-					
Passing #200 Sieve	%	-					

L. L.	53						
P. I.	21						
AASHTO Classification	A-7-5(22)						
Station	20+25						
OFFSET	57 LT						
Hole No.							
Depth (Ft)							
	to						

cc: R L PRIVETTE
STEVE BROWN
STEVE WIRTH
CHRIS CHEN
Soils File

Soils Engineer

NORTH CAROLINA DEPARTMENT OF TRANSPORTATION
DIVISION OF HIGHWAY
MATERIALS & TESTS UNIT
SOILS LABORATORY

T. I. P. No. _____

REPORT ON SAMPLES OF SOILS FOR QUALITY

Project 33457.1.1 County DAVIDSON Owner _____
Date: Sampled 3/20/06 Received 4/17/06 Reported 4/19/06
Sampled from _____ By C B LITTLE
Submitted by R L PRIVETTE T-4927 1995 Standard Specifications

729274 TO 729274
4/25/06

TEST RESULTS

Proj. Sample No.		ST-2					
Lab. Sample No.		729274					
Retained #4 Sieve	%	-					
Passing #10 Sieve	%	100					
Passing #40 Sieve	%	100					
Passing #200 Sieve	%	92					

MINUS NO. 10 FRACTION

SOIL MORTAR - 100%							
Coarse Sand Ret - #60	%	1.0					
Fine Sand Ret - #270	%	12.9					
Silt 0.05 - 0.005 mm	%	51.7					
Clay < 0.005 mm	%	34.3					
Passing #40 Sieve	%	-					
Passing #200 Sieve	%	-					

L. L.	48						
P. I.	18						
AASHTO Classification	A-7-5(20)						
Station	20+25						
OFFSET	55						
ALIGNMENT	L						
Depth (Ft)	2.00						
	to	4.00					

cc: R L PRIVETTE
STEVE BROWN
STEVE WIRTH
CHRIS CHEN
Soils File

Soils Engineer