CONTENTS: SHEET DESCRIPTION

6-7

TITLE SHEET

TEST SITE PLAN

CROSS SECTIONS

SOIL TEST RESULTS SCOUR REPORT

SITE PHOTOGRAPH

BORING LOGS

STRUCTURE INVENTORY REPORT

LEGEND

PROFILE

STATE OF NORTH CAROLINA

DEPARTMENT OF TRANSPORTATION

DIVISION OF HIGHWAYS

GEOTECHNICAL UNIT

STRUCTURE SUBSURFACE INVESTIGATION

STATE PROJECT 33276.1.1 I.D. NO. B-3824 F.A. PROJECT_*BRZ-1525(4)* COUNTY_ CHATHAM PROJECT DESCRIPTION BRIDGE NO. 88 ON -L- (SR 1525) OVER FERRELL'S CREEK AT - L - STATION 13 + 35

STATE STATE PROJECT REFERENCE NO. SHEET TOTAL SHEETS N.C. 33276.1.1 (B-3824) 1 15

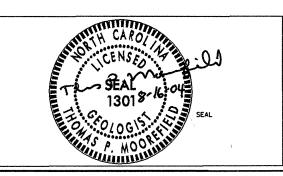
CAUTION NOTICE

THE SUBSURFACE INFORMATION AND THE SUBSURFACE INVESTIGATION ON WHICH IT IS BASED WAS MADE FOR THE PURPOSE OF STUDY, PLANNING AND DESKIN, AND NOT FOR CONSTRUCTION OR PAY PURPOSES, THE VARIOUS FELD BORING LOGS, ROCK CORES, AND SOIL TEST DATA AVAILABLE MAY BE REVIEWED OR INSPECTED IN RALEIGH BY CONTACTING THE N. C, DEPARTMENT OF TRANSPORTATION, GEOTECHNICAL UNIT & (919) 250-4088. NEITHER THE SUBSURFACE PLANS AND REPORTS, NOR THE FIELD RING LOGS, ROCK CORES, OR SOIL TEST DATA IS PART OF THE CONTRACT.

GENERAL SOIL AND ROCK STRATA DESCRIPTIONS AND INDICATED BOUNDARIES ARE BASED ON A GEOTECHNICAL INTERPRETATION OF ALL AVAILABLE SUBSURFACE DATA AND MAY NOT NECESSARILY REFLECT THE ACTUAL SUBSURFACE CONDITIONS BETWEEN BORINGS OR BETWEEN SAMPLED STRATA WITHIN THE BORENOLE. THE LABORATORY SAMPLE DATA AND THE IN SITU (IN-PLACE) TEST DATA CAN BE RELED ON ONLY TO THE DEGREE OF RELIABILITY INHERENT IN THE STANDARD TEST METHOD. THE OBSERVED WATER LEVELS OR SOIL MOISTURE CONDITIONS INDICATED IN THE SUBSURFACE INVESTIGATIONS ARE AS RECORDED AT THE TIME OF THE INVESTIGATION, THESE WATER LEVELS OR SOIL MOISTURE CONDITIONS MAY VARY CONSIDERABLY WITH TIME ACCORDING TO CLIMATIC CONDITIONS INCLUDING TEMPERATURES, PRECIPITATION AND WIND, AS WELL AS OTHER NON-CLIMATIC FACTORS.

THE BIDDER OR CONTRACTOR IS CAUTIONED THAT DETAILS SHOWN ON THE SUBSURFACE PLANS ARE PRELIMINARY ONLY AND IN MANY CASES THE FINAL DESIGN DETAILS ARE DIFFERENT. FOR BIDDING AND CONSTRUCTION PURPOSES, REFER TO THE CONSTRUCTION PLANS AND DOCUMENTS FOR FINAL DESIGN INFORMATION ON THIS PROJECT. THE DEPARTMENT DOES NOT WARRANT OR GUARANTEE THE SUFFICIENCY OR ACCURACY OF THE INVESTIGATION MADE, NOR THE INTERPRETATIONS MADE OR OPINION OF THE DEPARTMENT AS TO THE TYPE OF MATERIALS AND CONDITIONS TO BE ENCOUNTERED. THE BUDDER OR CONTRACTOR IS CAUTIONED TO MAKE SUCH INDEPENDENT SUBSURFACE INVESTIGATIONS AS HE DEEMS NECESSARY TO SATISFY HIMSELF AS TO CONDITIONS TO BE ENCOUNTERED ON THIS PROJECT. THE CONTRACTOR SHALL HAVE NO CLAIM FOR ADDITIONAL COMPENSATION OR FOR AN EXTENSION OF TIME FOR ANY REASON RESULTING FROM THE ACTUAL CONDITIONS ENCOUNTERED AT THE SITE DIFFERING FROM THOSE INDICATED IN THE SUBSURFACE INFORMATION.

INVESTIGATED BY T.P. MOOREFIELD PERSONNEL J.L. LOVE J.B. BARFIELD CHECKED BY D.N. ARGENBRIGHT O.B. OTI SUBMITTED BY D.N. ARGENBRIGHT DATE AUGUST 2004 L.B. MADISON H.R. CONLEY T.N. BENNEKIN J.T. BAGWELL C.E. POPE D. W. DIXON



NOTE - THE INFORMATION CONTAINED HEREIN IS NOT IMPLIED OR GUARANTEED BY THE N. C. DEPARTMENT OF TRANSPORTATION AS BEING ACCURATE NOR IT IS CONSIDERED TO BE PART OF THE PLANS.

NOTE - BY HAVING REQUESTED THIS INFORMATION THE CONTRACTOR SPECIFICALLY WAIVES ANY CLAIMS FOR INCREASED COMPENSATION OR EXTENSION OF TIME BASED ON DIFFERENCES BETWEEN THE CONDITIONS INDICATED HEREIN AND THE ACTUAL CONDITIONS AT THE PROJECT SITE.

DRAWN BY: J.L. LOVE, T.T. WALKER

NORTH CAROLINA DEPARTMENT OF TRANSPORTATION

DIVISION OF HIGHWAYS

GEOTECHNICAL UNIT

SUBSURFACE INVESTIGATION

| | | | | SOIL AND RO | OCK LEGEND, TERM | 1S, SYMBOLS, | AND ABBREV | 'IATIONS | | |
|--|---|---|---|---|--|---------------------------------|---|---|---------------------------------------|---|
| | SOIL DESCRIPTION | | | GRADATION | | | ROCK | DESCRIPTION | | TERMS AND DEFINITIONS |
| | NSOLIDATED, SEMI-CONSOLIDATED OR WEAT | | UNIFORM- INDICATES THAT SOIL | DOD REPRESENTATION OF PARTICLE SIZES I L PARTICLES ARE ALL APPROXIMATELY THE | FROM FINE TO COARSE E SAME SIZE.(ALSO | ROCK LINE INDICAT | TES THE LEVEL AT WHICH NON | HAT WHEN TESTED, WOULD YIELD SPT R N-COASTAL PLAIN MATERIAL WOULD YIE | I D SPT REFUSAL | ALLUVIUM (ALLUV.) - SDILS WHICH HAVE BEEN TRANSPORTED BY WATER. |
| 100 BLOWS PER FOOT ACCORDING TO S | ONTINUOUS FLIGHT POWER AUGER, AND WHIC STANDARD PENETRATION TEST (AASHTO T20 | 96. ASTM D-1586), SOIL | POORLY GRADED) GAP-GRADED- INDICATES A MIX | TURE OF UNIFORM PARTICLES OF TWO OR | MORE SIZES. | SPT REFUSAL IS P | ENETRATION BY A SPLIT SPOO | ON SAMPLER EQUAL TO OR LESS THAN TION BETWEEN SOIL AND ROCK IS OFTE | 0.1 FOOT PER 60 BLOWS | AQUIFER - A WATER BEARING FORMATION OR STRATA. |
| CLASSIFICATION IS BASED ON THE AAS CONSISTENCY, COLOR, TEXTURE, MOISTUR | SHTO SYSTEM AND BASIC DESCRIPTIONS GE RE, AASHTO CLASSIFICATION, AND OTHER PE | SENERALLY SHALL INCLUDE: ERTINENT FACTORS SUCH | | ANGULARITY OF GRAINS | S | OF WEATHERED ROO | CK. ARE TYPICALLY DIVIDED AS FO | | IN REPRESENTED BY A ZUNE | ARENACEOUS - APPLIED TO ROCKS THAT HAVE BEEN DERIVED FROM SAND OR THAT CONTAIN SAND. |
| AS MINERALOGICAL COMPOSITION, ANGUL | LARITY, STRUCTURE. PLASTICITY, ETC. EXAM | IPLE: | | S OF SOIL GRAINS ARE DESIGNATED BY THE | HE TERMS; ANGULAR, | WEATHERED | 127212 | | | ARGILLACEOUS - APPLIED TO ALL ROCKS OR SUBSTANCES COMPOSED OF CLAY MINERALS, OR HAVING A NOTABLE PROPORTION OF CLAY IN THEIR COMPOSITION, AS SHALE, SLATE, ETC. |
| | CLN, MOIST WITH INTERBEDDED FINE SAND LAVERS, HIGHLY I | | SUBANGULAR, SUBROUNDED, DR | | · · · · · · · · · · · · · · · · · · · | ROCK (WR) | PER FOOT. | PLAIN MATERIAL THAT YIELDS SPT N | VALUES > 100 BLOWS | ARTESIAN - GROUND WATER THAT IS UNDER SUFFICIENT PRESSURE TO RISE ABOVE THE LEVEL |
| GENERAL GRANULAR MATER | ND AND AASHTO CLASSIFI(RIALS SILT-CLAY MATERIALS | | | MINERALOGICAL COMPOSITI | | CRYSTALLINE | FINE TO COAR | SE GRAIN IGNEOUS AND METAMORPHIC | ROCK THAT | AT WHICH IS IS ENCOUNTERED, BUT WHICH DOES NOT NECESSARILY RISE TO OR ABOVE THE |
| CLASS. (S5% PASSING #2 | | ORGANIC MATERIALS | WHENEVER THEY ARE CONSIDER | | OSED IN DESCRIPTIONS | ROCK (CR) | GNEISS, GABBRI | | | GROUND SURFACE. CALCAREOUS (CALC.) - SOILS WHICH CONTAIN APPRECIABLE AMOUNTS OF CALCIUM CARBONATE. |
| GROUP A-1 A-3 | A-2 A-4 A-5 A-6 A-7 | | | COMPRESSIBILITY | | NON-CRYSTALLINE | FINE TO COARS | SE GRAIN METAMORPHIC AND NON-COAS ROCK THAT WOULD YEILD SPT REFUSAL | TAL PLAIN | COLLUVIUM - ROCK FRAGMENTS MIXED WITH SOIL DEPOSITED BY GRAVITY ON SLOPE OR AT BOTTOM |
| 000000000000000000000000000000000000000 | A-2-5 A-2-6 A-2-7 A-7-6 | A-3 A-6, A-7 | SLIGHTLY COMPRESSI MODERATELY COMPRE | | T LESS THAN 30 | ROCK (NCR) COASTAL PLAIN | INCLUDES PHYL | LLITE, SLATE, SANDSTONE, ETC. | | OF SLOPE. |
| SYMBOL 8000000000000000000000000000000000000 | | | HIGHLY COMPRESSIBL | | T GREATER THAN 50 | SEDIMENTARY ROCK | SPT REFUSAL. | N SEDIMENTS CEMENTED INTO ROCK, BU ROCK TYPE INCLUDES LIMESTONE, SAN | DSTONE, CEMENTED | CORE RECOVERY (REC.) - TOTAL LENGTH OF ALL MATERIAL RECOVERED IN THE CORE BARREL DIVIDED BY TOTAL LENGTH OF CORE RUN AND EXPRESSED AS A PERCENTAGE. |
| % PASSING * 10 50 MX | | GRANULAR SILT- MUCK, | | PERCENTAGE OF MATERIA | AL | - (CF) | SHELL BEDS, E | EATHERING | | DIKE - A TABULAR BODY OF IGNEOUS ROCK THAT CUTS ACROSS THE STRUCTURE OF ADJACENT |
| # 40 30 MX50 MX51 MN | | SOILS CLAY PEAT | UNGANIE MATERIAL | GRANULAR SILT- CLAY SOILS SOILS | OTHER MATERIAL | FRESH ROCK F | | JOINTS MAY SHOW SLIGHT STAINING. R | 2004 24400 14425 | ROCKS OR CUTS MASSIVE ROCK. |
| | 85 MX35 MX35 MX36 MN36 MN36 MN36 MN | IN SOLES | TRACE OF ORGANIC MATTER LITTLE ORGANIC MATTER | | RACE 1 - 10% ITTLE 10 - 20% | | R IF CRYSTALLINE. | JUINIS MAT SHOW SLIGHT STAINING, H | KULK KINGS UNDER | DIP - THE ANGLE AT WHICH A STRATUM OR ANY PLANAR FEATURE IS INCLINED FROM THE HORIZONTAL. |
| LIDUID LIMIT 40 MX41 | 13 MN 48 MX41 MN 48 MX41 MN 48 MX41 MN 8 MX11 MN 11 MN 18 MX 18 MX 11 MN 11 MN | JOILS WITH | MODERATELY ORGANIC | 5 - 10% 12 - 20% S | OME 20 - 35% | VERY SLIGHT ROCK O | SENERALLY FRESH, JOINTS STA | NED, SOME JOINTS MAY SHOW THIN CL | LAY COATINGS IF OPEN, | DIP DIRECTION (DIP AZIMUTH) - THE DIRECTION OR BEARING OF THE HORIZONTAL TRACE OF |
| GROUP INDEX 0 0 0 | | - HIGHLY | HIGHLY ORGANIC | | IGHLY 35% AND ABOVE | (V. SL).) CRYSTA | RLS UN A BRUKEN SPECIMEN F CRYSTALLINE NATURE. | ACE SHINE BRIGHTLY, ROCK RINGS UND | ER HAMMER BLOWS IF | THE LINE OF DIP, MEASURED CLOCKWISE FROM NORTH. |
| INCIDAL TYPES STONE EDAGS | | AMOUNTS OF SOILS | ✓ WATER L | GROUND WATER | D. DDI 4 TAG | SLIGHT ROCK O | GENERALLY FRESH, JOINTS STA | INED AND DISCOLORATION EXTENDS IN | TO ROCK UP TO | FAULT - A FRACTURE OR FRACTURE ZONE ALONG WHICH THERE HAS BEEN DISPLACEMENT OF THE SIDES RELATIVE TO ONE ANOTHER PARALLEL TO THE FRACTURE. |
| OF THOUSE OWNER WAS COND CRAVE | Y OR CLAYEY SILTY CLAYEY VEL AND SAND SOILS SOILS | MATTER | _ | EVEL IN BORE HOLE IMMEDIATELY AFTE | R DRILLING. | (SLI.) I INCH. CRYSTA | OPEN JOINTS MAY CONTAIN C ALS ARE DULL AND DISCOLORE | CLAY. IN GRANITOID ROCKS SOME OCCA: CD. CRYSTALLINE ROCKS RING UNDER H | SIONAL FELDSPAR AMMER BLOWS. | FISSILE - A PROPERTY OF SPLITTING ALONG CLOSELY SPACED PARALLEL PLANES. |
| MATERIALS SAND SHIND OTHER | | | ∇7 s | ATER LEVEL AFTER 24 HOURS. | | MODERATE SIGNIFI | ICANT PORTIONS OF ROCK SHOP | W DISCOLORATION AND WEATHERING EF | FECTS, IN | FLOAT - ROCK FRAGMENTS ON SURFACE NEAR THEIR ORIGINAL POSITION AND DISLODGED FROM |
| AS A EXCELLENT TO GI | GOOD FAIR TO POOR | FAIR TO POOR UNSUITABLE | | WATER, SATURATED ZONE OR WATER BEA | ARING STRATA | (MOD.) GRANIT | OID ROCKS, MOST FELDSPARS # SOUND UNDER HAMMER BLOWS # | ARE DULL AND DISCOLORED, SOME SHOW AND SHOWS SIGNIFICANT LOSS OF STRE | √ CLAY. ROCK HAS ENGTH AS COMPARED | PARENT MATERIAL. |
| | 5 ≤ L.L 30 : P.I. OF A-7-6 > L.L | 130 | O∭► SPRING OF | R SEEPAGE | | WITH F | RESH ROCK. | | | FLOOD PLAIN (F.P.) - LAND BORDERING A STREAM, BUILT OF SEDIMENTS DEPOSITED BY THE STREAM. |
| | NSISTENCY OR DENSENESS | 3 | | MISCELLANEOUS SYMBOL | S | MODERATELY ALL RO SEVERE AND DI | OCK EXCEPT QUARTZ DISCOLORE SCOLORED AND A MAJORITY SH | ED OR STAINED. IN GRANITOID ROCKS, HOW KAOLINIZATION. ROCK SHOWS SEVE | ALL FELDSPARS DULL | FORMATION (FM.) - A MAPPABLE GEOLOGIC UNIT THAT CAN BE RECOGNIZED AND TRACED IN |
| | TNESS OR RANGE OF STANDARD PENETRATION RESISTENCE | RANGE OF UNCONFINED COMPRESSIVE STRENGTH | □ ROADWAY EMBANKM | MENT SPT CPT DPT DMT TEST BOR | ING SAMPLE | (MOD. SEV.) AND CA | IN BE EXCAVATED WITH A GEO TED. WOULD YIELD SPT REFUSA | LOGIST'S PICK. ROCK GIVES "CLUNK" SO | OUND WHEN STRUCK. | THE FIELD. |
| CONSIS | STENCY (N-VALUE) | (TONS/FT ²) | WITH SOIL DESCRI | IPTION VST PHT 1231 BUR | DESIGNATIONS | 1 | | RED OR STAINED ROCK FABRIC CLEAR | AND EVIDENT DUT DEDUCED | JOINT - FRACTURE IN ROCK ALONG WHICH NO APPRECIABLE MOVEMENT HAS OCCURRED. |
| GENERALLY VERY L | | | SOIL SYMBOL | AUGER BORING | S- BULK SAMPLE | (SEV.) IN STR | ENGTH TO STRONG SOIL. IN G | RANITOID ROCKS ALL FELDSPARS ARE | KAOLINIZED TO SOME | LEDGE - A SHELF-LIKE RIDGE OR PROJECTION OF ROCK WHOSE THICKNESS IS SMALL COMPARED TO ITS LATERAL EXTENT. |
| MATERIAL MEDIUM | 1 DENSE 10 TO 30 | N/A | ARTIFICIAL FILL (| | SS- SPLIT SPOON | | T. SOME FRAGMENTS OF STRON STED. YIELDS SPT N VALUES > | | | LENS - A BODY OF SOIL OR ROCK THAT THINS OUT IN ONE OR MORE DIRECTIONS. |
| (NON-COHESIVE) DENS | | | ROADWAY EMBANKM | MENTS - CORE BORING | SAMPLE | VERY SEVERE ALL RO | OCK EXCEPT QUARTZ DISCOLORE | ED OR STAINED. ROCK FABRIC ELEMENT | TS ARE DISCERNIBLE BUT | MOTTLED (MOT.) - JRREGULARLY MARKED WITH SPOTS OF DIFFERENT COLORS. MOTTLING IN |
| VERY S | | ⟨0.25 | INFERRED SOIL BO | OUNDARIES MONITORING W | ST- SHELBY TUBE SAMPLE | (V. SEV.) THE MA | ASS IS EFFECTIVELY REDUCED ING. SAPROLITE IS AN EXAMPL | TO SOIL STATUS, WITH ONLY FRAGMEN LE OF ROCK WEATHERED TO A DEGREE | ITS OF STRONG ROCK | SOILS UGUALLY INDICATES POOR AERATION AND LACK OF GOOD DRAINAGE. PERCHED WATER - WATER MAINTAINED ABOVE THE NORMAL GROUND WATER LEVEL BY THE PRESENCE OF AN |
| GENERALLY SOFT SILT-CLAY MEDIUM | | 0.25 TO 0.5 | SIEIN INFERRED ROCK LI | | RS- ROCK SAMPLE | VESTIG | ES OF THE ORIGINAL ROCK FA | ABRIC REMAIN. <u>IF TESTED, YIELDS SP</u> | T N VALUES < 100 BPF | INTERVENING IMPERVIOUS STRATUM. |
| MATERIAL STIFE | F 8 TO 15 | 0.5 TO 1 1 TO 2 | ← ← ← ← ← ← ← ← ← ← ← ← ← | △ INCTALLATION | III NECOLII ACTED | COMPLETE ROCK R | EDUCED TO SOIL. ROCK FABRIO | C NOT DISCERNIBLE, OR DISCERNIBLE OF MAY BE PRESENT AS DIKES OR STRIN | ONLY IN SMALL AND | RESIDUAL SOIL - SOIL FORMED IN PLACE BY THE WEATHERING OF ROCK. |
| (COHESIVE) VERY S HARD | | 2 TO 4 >4 | 25/025 DIP/DIP DIRECTION | N OF SLOPE INDICAT | | ALSO A | N EXAMPLE. | THE DE TRESERT AS DIRES ON STREET | OCENS. SHPROLITE 15 | ROCK QUALITY DESIGNATION (R.Q.D.) - A MEASURE OF ROCK QUALITY DESCRIBED BY: TOTAL LENGTH OF ROCK SEGMENTS EQUAL TO OR GREATER THAN 4 INCHES DIVIDED BY THE TOTAL LENGTH OF CORE RUN AN |
| Т | EXTURE OR GRAIN SIZE | 1 | ROCK STRUCTURES | | CBR - CBR SAMPLE | | ROCI | K HARDNESS | | EXPRESSED AS A PERCENTAGE. |
| U.S. STD. SIEVE SIZE | 4 10 40 60 200 | 0 270 | - SOUNDING ROD | REF SPT REFUSAL | | VERY HARD CANNO | OT BE SCRATCHED BY KNIFE OF | R SHARP PICK, BREAKING OF HAND SPE | ECIMENS REQUIRES | SAPROLITE (SAP.) - RESIDUAL SOIL WHICH RETAINS THE RELIC STRUCTURE OR FABRIC OF THE PARENT ROCK. |
| | 4.76 2.0 0.42 0.25 0.07 | | | | | 3 | RAL HARD BLOWS OF THE GEOL RE SCRATCHED BY KNIEF OR PI | .UGISTS PICK. ICK ONLY WITH DIFFICULTY. HARD HAM | MED DI OUE DECUTDED | SILL - AN INTRUSIVE BODY OF IGNEOUS ROCK OF APPROXIMATELY UNIFORM THICKNESS AND |
| BOULDER COBBLE G | GRAVEL COARSE FINE | | | ABBREVIATIONS | | | TACH HAND SPECIMEN. | TON ONE! WITH DIFFICULTY, THAT THAT | INER BLOWS REGULARD | RELATIVELY THIN COMPARED WITH ITS LATERAL EXTENT, WHICH HAS BEEN EMPLACED PARALLEL TO THE BEDDING OR SCHISTOSITY OF THE INTRUDED ROCKS |
| | (GR.) SAND SANI (CSE. SD.) (F. S | AD (CI.) | AR - AUGER REFUS BT - BORING TERM | | SUREMETER TEST ANDY | MODERATELY CAN B | BE SCRATCHED BY KNIFE OR PI | ICK. GOUGES OR GROOVES TO 0.25 INC | HES DEEP CAN BE | SLICKENSIDE - POLISHED AND STRIATED SURFACE THAT RESULTS FROM FRICTION ALONG A FAULT OR |
| GRAIN MM 305 75 | 2.0 0.25 | 0.05 0.005 | CL CLAY CPT - CONE PENET | SL SILT, S | ILTY | | DOERATE BLOWS. | EDLOGISTS PICK. HAND SPECIMENS CAN | BE DETACHED | SLIP PLANE. |
| SIZE IN. 12° 3° | | | CSE COARSE | TCR - TRICO | | MEDIUM CAN B HARD CAN B | BE GROOVED OR GOUGED 0.05 I | INCHES DEEP BY FIRM PRESSURE OF K | NIFE OR PICK POINT. | STANDARD PENETRATION TEST (PENETRATION RESISTANCE) (SPT) - NUMBER OF BLOWS (N OR B.P.F.) OF A 140 LB. HAMMER FALLING 30 INCHES REQUIRED TO PRODUCE A PENETRATION OF 1 FOOT INTO SOIL WITH |
| SOIL MOISTURE SCALE | TURE - CORRELATION OF | | DMT - DILATOMETE DPT - DYNAMIC PE | NETRATION TEST / - UNII | | | OF A GEOLOGISTS PICK. | S TO PEICES 1 INCH MAXIMUM SIZE BY | HARD BLOWS OF THE | A 2 INCH OUTSIDE DIAMETER SPLIT SPOON SAMPLER. SPT REFUSAL IS LESS THAN 0.1 FOOT PENETRATION |
| (ATTERBERG LIMITS) | DESCRIPTION GUIDE FOR | R FIELD MOISTURE DESCRIPTION | e - VOID RATIO F FINE | $\gamma_{ m d}$ - DRY U W - MOISTUR | | | | Y BY KNIFE OR PICK. CAN BE EXCAVAT N SIZE BY MODERATE BLOWS OF A PIC | | WITH 60 BLOWS. STRATA CORE RECOVERY (SREC.) - TOTAL LENGTH OF STRATA MATERIAL RECOVERED DIVIDED BY TOTAL LENGTH |
| | - SATURATED - USUALLY L | LIQUID; VERY WET, USUALLY | FOSS FOSSILIFE | ROUS V VERY | | | S CAN BE BROKEN BY FINGER | | K PUINI. SMALL, IHIN | DF STRATUM AND EXPRESSED AS A PERCENTAGE. |
| LL_ LIQUID LIMIT | (SAT.) FROM BELL | OW THE GROUND WATER TABLE | FRAC FRACTURED FRAGS FRAGMENT | | SHEAR TEST | VERY CAN B | E CARVED WITH KNIFE. CAN BE | E EXCAVATED READILY WITH POINT OF | PICK. PIECES 1 INCH | STRATA ROCK QUALITY DESIGNATION (S.R.O.D.) - A MEASURE OF ROCK QUALITY DESCRIBED BY: TOTAL LENGTH OF ROCK SEGMENTS WITHIN A STRATUM EQUAL TO OR GREATER THAN 10 CENTIMETERS DIVIDED |
| PLASTIC | SEMISOI ID | D. REQUIRES DRYING TO | MED MEDIUM | | | SUF1 OR MU FINGER | | OKEN BY FINGER PRESSURE. CAN BE SO | CRATCHED READILY BY | BY THE TOTAL LENGTH OF STRATA AND EXPRESSED AS A PERCENTAGE. |
| RANGE < | | D;REQUIRES DRYING TO PTIMUM MOISTURE | EGUI | PMENT USED ON SUBJECT | PROJECT | FRACTU | JRE SPACING | BEDDIN | G | TOPSOIL (T.S.) - SURFACE SOILS USUALLY CONTAINING ORGANIC MATTER. |
| PLL PLASTIC LIMIT | | | DRILL UNITS: | ADVANCING TOOLS: | HAMMER TYPE: | IERM | SPACING | TERM | THICKNESS | BENCH MARK: BM #I-RR Spike in Base of Powerpole |
| OM _ OPTIMUM MOISTURE | - MOIST - (M) SOLID; AT | T OR NEAR OPTIMUM MOISTURE | | CLAY BITS | AUTOMATIC MANUAL | VERY WIDE WIDE | MORE THAN 10 FEET 3 TO 10 FEET | VERY THICKLY BEDDED THICKLY BEDDED | > 4 FEET 1.5 - 4 FEET | 33.61FT RT of -L- Sta. II+81.44 |
| SL _ SHRINKAGE LIMIT | | | MOBILE B | 6° CONTINUOUS FLIGHT AUGER | | MODERATELY CLOS | SE 1 TO 3 FEET | THINLY BEDDED VERY THINLY BEDDED | 0.16 - 1.5 FEET 0.03 - 0.16 FEET | ELEVATION: 368.78' |
| | | ADDITIONAL WATER TO PTIMUM MOISTURE | BK-51 | 8" HOLLOW AUGERS | CORE SIZE: | CLOSE VERY CLOSE | 0.16 TO 1 FEET LESS THAN 0.16 FEET | THICKLY LAMINATED | 0.008 - 0.03 FEET | NOTES: |
| | PLASTICITY | | | | | | TNI | THINLY LAMINATED DURATION | < 0.008 FEET | 1 |
| | PLASTICITY PLASTICITY INDEX (PI) | DRY STRENGTH | CME-45C | HARD FACED FINGER BITS | | FOR SEDIMENTARY ROC | | ENING OF THE MATERIAL BY CEMENTING | S. HEAT, PRESSURE, FTC. | 1 |
| NONPLASTIC | 0-5 | VERY LOW | | TUNGCARBIDE INSERTS | П-н | 1 | | NG WITH FINGER FREES NUMEROUS GRAD | | |
| LOW PLASTICITY MED. PLASTICITY | 6-15 16-25 | SLIGHT MEDIUM | | CASING W/ ADVANCER | HAND TOOLS: | FRIABLE | | E BLOW BY HAMMER DISINTEGRATES SA | | |
| HIGH PLASTICITY | 26 OR MORE | HIGH | PORTABLE HOIST | TRICONE STEEL TEETH | POST HOLE DIGGER | MODERATEL | | CAN BE SEPARATED FROM SAMPLE WI EASILY WHEN HIT WITH HAMMER. | ITH STEEL PROBE; | |
| | COLOR | | OTHER | TRICONE * TUNG,-CARB. | HAND AUGER | | | | TEEL DOORS | |
| | DLOR OR COLOR COMBINATIONS (TAN, RE | | | CORE BIT | SOUNDING ROD | INDURATED | | S ARE DIFFICULT TO SEPARATE WITH S CULT TO BREAK WITH HAMMER. | DIECE PRUBE; | |
| MODIFIERS SUCH AS LIGHT, DAR | RK, STREAKED, ETC. ARE USED TO DESCR | RIBE APPEARANCE. | OTHER | OTHER | VANE SHEAR TEST OTHER | EXTREMELY | | HAMMER BLOWS REQUIRED TO BREAK S | SAMPLE; | |
| 1 | | | | | | ı | SAMPLE | E BREAKS ACROSS GRAINS. | | 1 |

 ID
 STATE PROJECT NO.
 SHEET NO.
 TOTAL SHEETS

 B-3824
 33276.1.1
 2
 15



STATE OF NORTH CAROLINA DEPARTMENT OF TRANSPORTATION

Michael F. Easley GOVERNOR P.O. BOX 25201, RALEIGH, N.C. 27611-5201

Lyndo Tippett Secretary

August 19, 2004

STATE PROJECT:

33276.1.1 (B-3824)

FEDERAL PROJECT:

BRZ-1525(4)

COUNTY:

Chatham

DESCRIPTION:

Bridge No. 88 on SR 1525 over Ferrell's Creek

SUBJECT:

Geotechnical Report – Foundation Investigation for Structure on

-L- (SR 1525) over Ferrell's Creek at -L- Station 13+35

Note: This inventory report supersedes the previously submitted report of August 28, 2003.

Project Description

This project consists of a 144-foot long three span bridge to be constructed over Ferrell's Creek. The proposed bridge has a 60° skew and will replace the existing 116-foot long structure. The project is located approximately 7 miles north of Pittsboro.

This site was originally investigated in July 2003 for a single span structure. In May 2004, the field crew returned to investigate for the proposed three span structure discussed in this report. The July 2003 borings were advanced using a BK 51 ATV-mounted drill machine with automatic hammer. The August 2004 borings were drilled using a CME 550 ATV-mounted drill machine with automatic hammer. Standard Penetration Tests were performed at each bent location. Representative soil samples were collected for visual classification in the field and for laboratory analysis by the Materials and Tests Unit.

Physiography and Geology

The project is located in gently rolling terrain of the central Piedmont Physiographic Province in rural northern Chatham County. The project occurs within the Carolina Slate Belt geologic province. The underlying bedrock consists of metamorphosed granite and diorite intrusions.

SHEET 3 OF 15

Soil Properties

Soils encountered at the project site include roadway embankment, alluvial, and residual soil.

The roadway embankment fill soil consists of very soft to medium stiff, moist to wet sandy clay (AASHTO classification of A-6) and silty sandy clay (A-7-6). The fill soil is approximately 10 to 18 feet thick at End Bent One, 2 feet thick at Bent Two, and 10 to 14 feet thick at End Bent Two.

The alluvial soil underlies the roadway embankment in each boring and ranges from 9 to 15 feet in thickness. The alluvial soil consists of interbedded very soft to soft, moist to wet, sandy silt (A-4) and silty sandy clay (A-6 and A-7-6) with trace organic matter, overlying 1.5 to 12 feet of gray coarse alluvial sand (A-3). A lag gravel deposit was encountered at the base of the alluvial soils and yielded N-values of 100+. The alluvial soil overlies either residual soil or weathered rock.

The residual soil consists of silty coarse sand derived from the in-place weathering of the underlying bedrock. These soils include medium dense to very dense, dry silty coarse sand (A-2-4) and coarse sand (A-1-b). The residual soils grade into weathered rock with depth.

Rock Properties

The weathered rock was derived from the underlying metamorphosed granite and diorite bedrock. Weathered rock was encountered below the residual soil at Bent One, Bent Two, and the right side of End Bent One. The weathered rock is interbedded with very dense residual sand in boring EB2-B.

Groundwater

Groundwater was encountered at each bent location. Groundwater elevations ranged from 358.0 to 361.5 feet.

Undisturbed Sample

An undisturbed sample of alluvial soil was retrieved from the interval 21.0 to 23.0 feet in boring EB2-B. The sample was submitted to the Materials and Test Unit for consolidation testing. See Settlement Test Report dated August 22, 2003 for results.

Notice

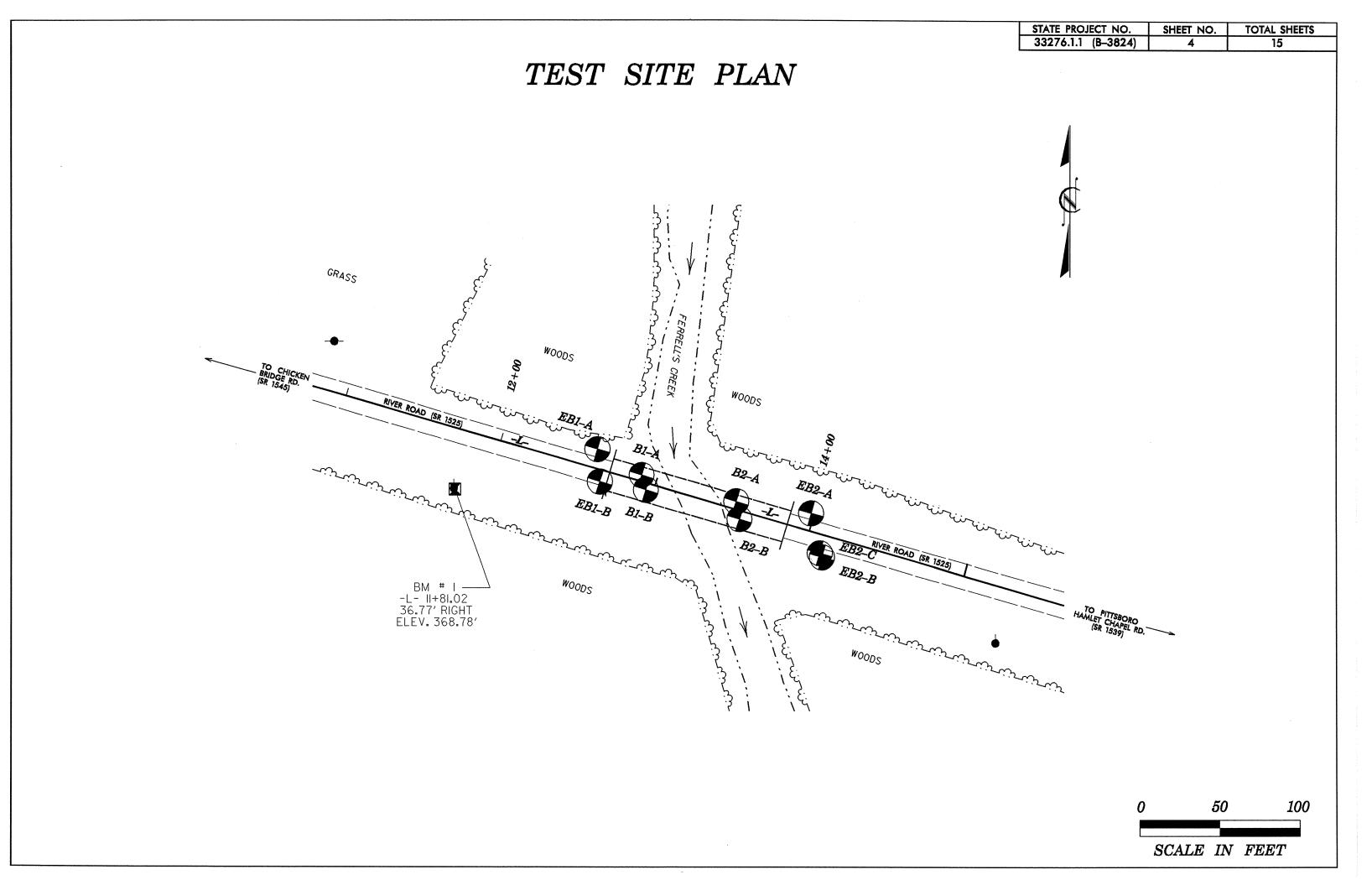
This Geotechnical foundation report is based on the bent locations provided in the Bridge Survey & Hydraulic Design Report dated April 23, 2003 and the memo "Revised Request for Foundation Recommendations" dated April 29, 2004. If significant changes are made in the design, or location of the proposed structure, the subsurface information should be reviewed and modified as necessary.

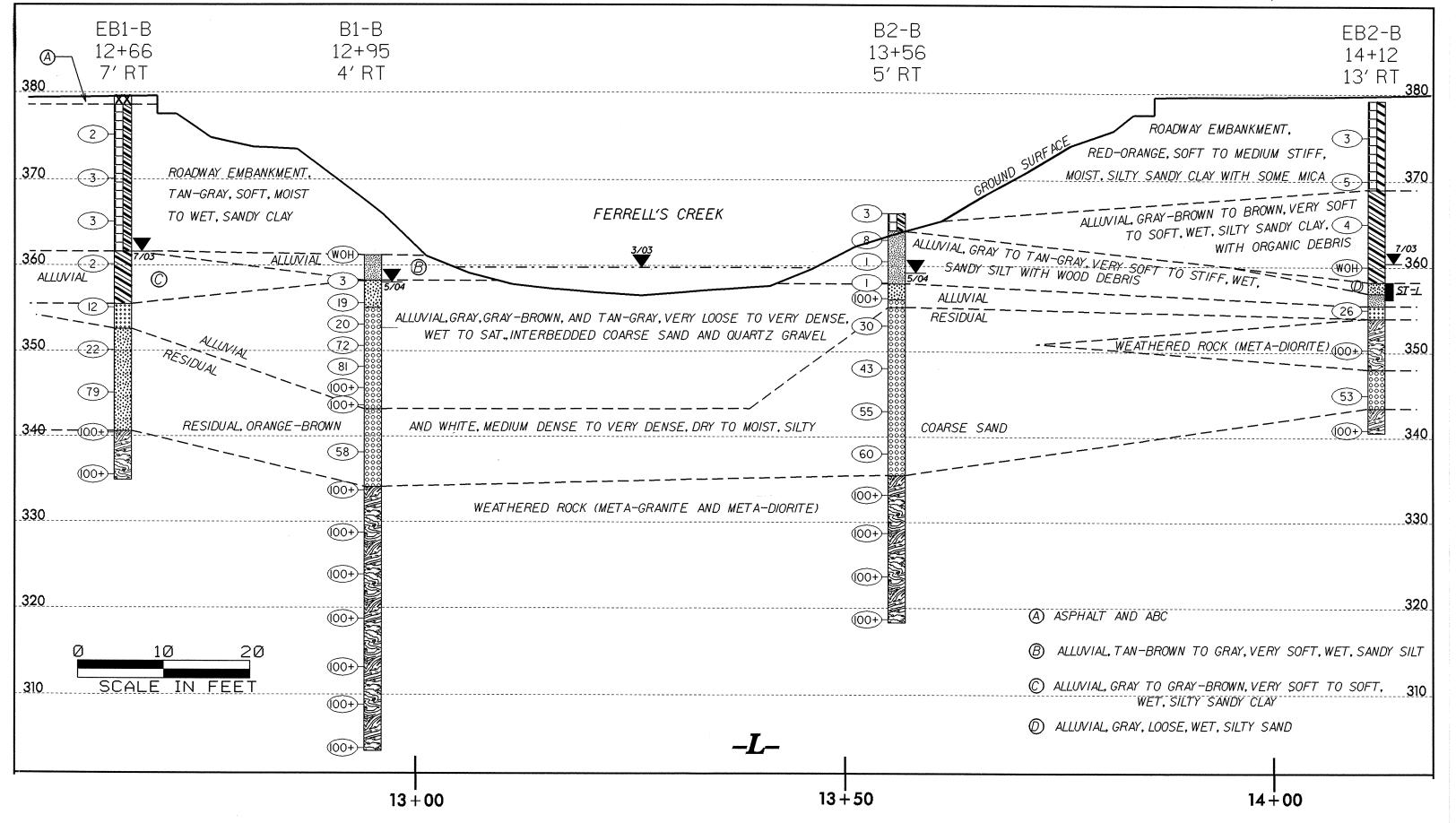
Prepared by,

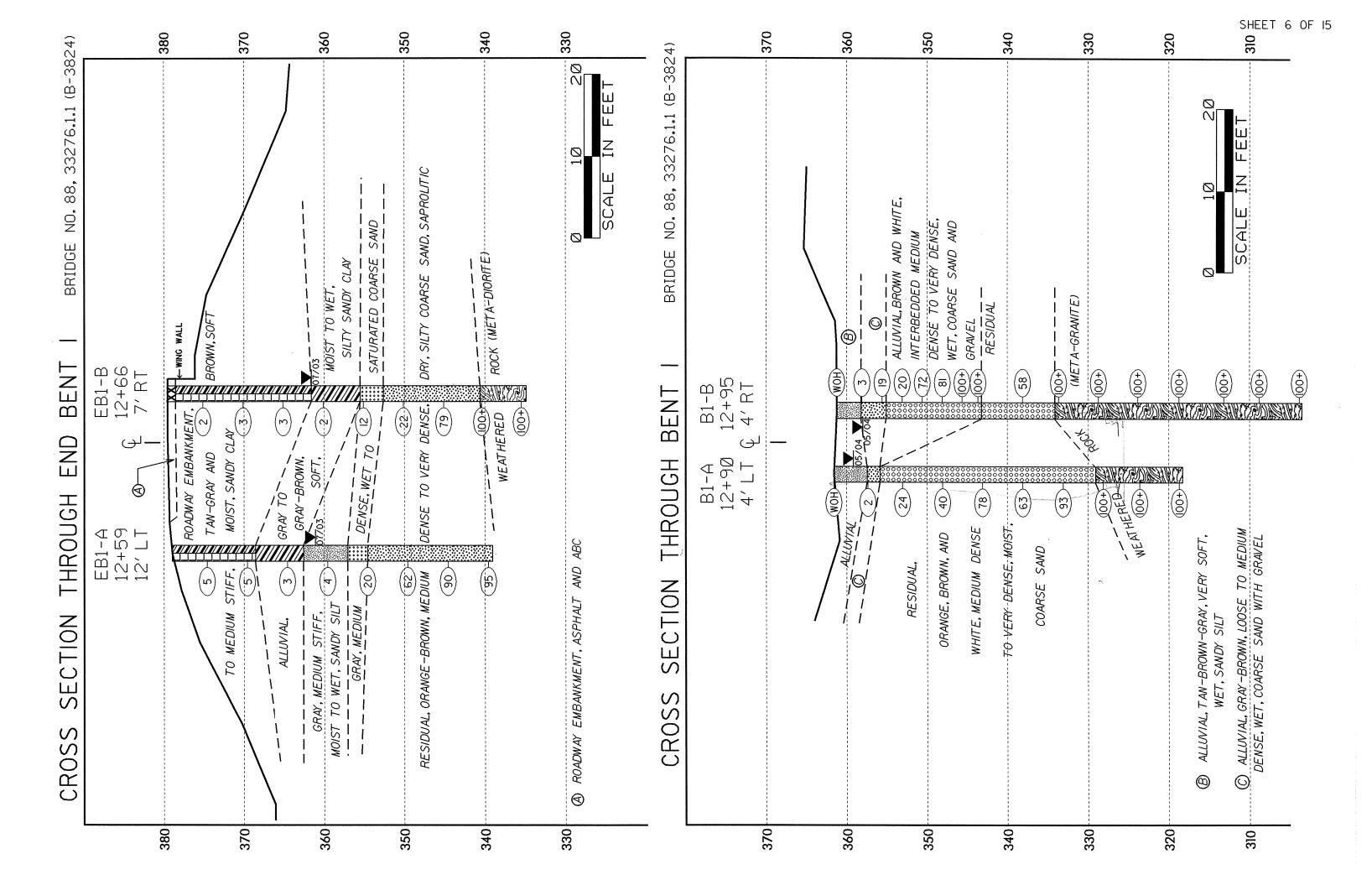
Thomas P. Moorefield, LG

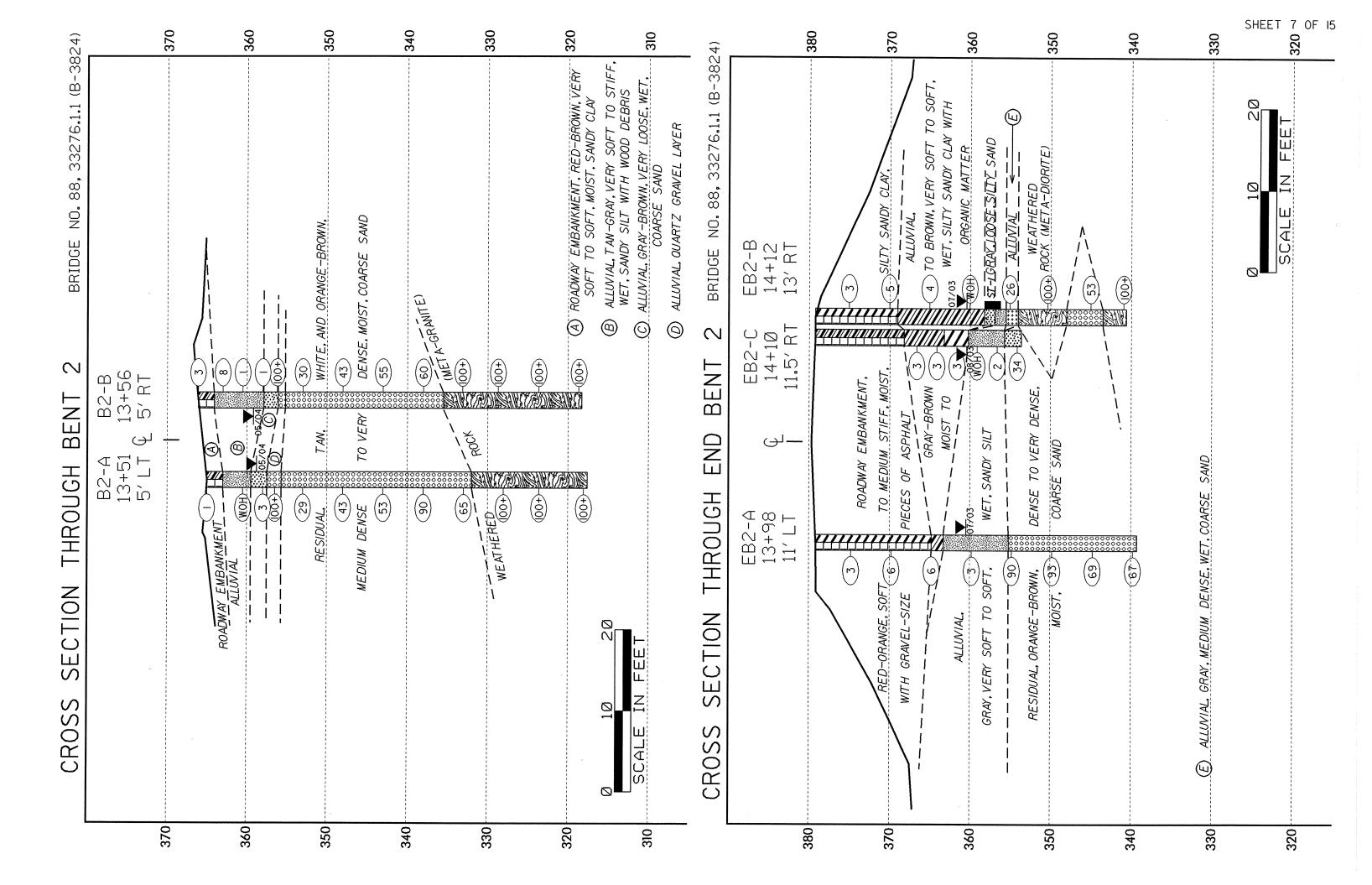
The P. Monfel

Project Geologist









North Carolina Department of transportation | North Carolina Department of transportation GEOTECHNICAL UNIT BORING LOG

| PROJECT NO. 33276.1.1 ID. | B-3824 COUNTY CHATHA | M GEOLOGIST | J.L. LOVE | PROJECT NO. 33276.I.I ID. B-3824 COUNTY CHATHAM GEOLOGIST J.B. BARFIELD |
|---|---|---------------------------------------|---|--|
| SITE DESCRIPTION BRIDGE NO. 88 | 8 ON -L- (SR 1525) OVER FE | ~~~~ | GROUND WATER | |
| BORING NO. EBI-A BORING LOC | | r 12' LT ALIGNMENT | | O' BORING NO. EBI-B BORING LOCATION 12+66 OFFSET 7' RT ALIGNMENT -L- 0 HR. 18.0' |
| | NORTHING 755868 | EASTING 1943227 | | MAPINO 13 10220 Print 1010 |
| TOTAL DEPTH 39.9' DRILL MACE | | THOD H.S. AUGERS | HAMMER TYPE AUTOMATIC | |
| | | RFACE WATER DEPTH N/A | | |
| ELEV. DEPTH BLOW COUNT PEN. (FT.) 0.510.510.5 (FT.) | BLOWS PER FOOT 0 25 50 75 10 | SAMPLE V O O O NUMBER MOI. G | SOIL AND ROCK DESCRIPTION | ELEV. DEPTH BLOW COUNT PEN. BLOWS PER FOOT SAMPLE ON NUMBER MOI. G SOIL AND ROCK DESCRIPTION |
| 379.0 | | | | 379.6 ASPHALT AND ABC |
| 375.0 = 3.4 1 2 3 1.0 | X5 | SS-7 M | ROADWAY EMBANKMENT, AN-GRAY AND BROWN, SANDY | |
| 370.0 = 8.4 2 2 3 1.0 | X5 | м | CLAY ALLUVIAL, | M |
| 365.0 = 13.4 1 1 2 1.0 | X3 | SS-8 M GI | RAY-BROWN, SILTY SANDY CLAY | |
| 360.0 = 18.4 1 1 3 1.0 | X4 | M | GRAY, SANDY SILT | 360.0 = 18.5 1 1 1 1.0 |
| $\frac{1}{355.0} = 23.4 \text{ WOH } 9 = 11 = 1.0$ | 20 | W W W W W W W W W W | GRAY, COARSE SAND | 355.0 = 23.5 1 2 10 1.0 = -12 = |
| 350.0 = 28.4 4 32 30 1.0 | x62 | SS-IO D | RESIDUAL, | 350.0 = 28.5 1 10 12 1.0 |
| 345.0 = 33.4 5 40 50 1.0 | 90 x | D ORAN | IGE-BROWN, SILTY COARSE SAND | SAND 345.0 = 33.5 11 44 35 1.0 = |
| 340.0 = 38.4 14 45 50 1.0 | BORING TERMINATED AT | D | | 340.0 = 38.5 14 50 50 0.9 |
| 335.0 + | ELEVATION 339 IFEET IN RESIDUAL COARSE SAND | | | 335.0 = 43.5 11 63 37 0.7 |
| 330.0 ± | | | | 330.0 = |
| 325.0 ± | | | | 325.0 = 1 |
| 320.0 ± | | | | 320.0 = |
| 315.0 + | | | | 315.0 = 1 |
| 310.0 + | | | | 310.0 = 1 |
| 305.0 + | | | | 305.0 = 1 |
| 300.0 + | | | | 300.0 + |

NORTH CAROLINA DEPARTMENT OF TRANSPORTATION NORTH CAROLINA DEPARTMENT OF TRANSPORTATION GEOTECHNICAL UNIT BORING LOG

| PROJECT NO. 33276.I.I ID. B-3824 COUNTY CHATHAN | | PROJECT NO. 33276.I.I ID. B-3824 COUNTY CHATHAM GEOLOGIST J.L. LOVE |
|--|--|---|
| SITE DESCRIPTION BRIDGE NO. 88 ON -L- (SR 1525) OVER FER | | SITE DESCRIPTION BRIDGE NO. 88 ON -L- (SR 1525) OVER FERRELL'S CREEK GROUND I |
| BORING NO. BI-A BORING LOCATION 12+90 OFFSET | | BORING NO. BI-B BORING LOCATION 12+95 OFFSET 4'RT ALIGNMENT -L- O HR. NA |
| COLLAR ELEVATION 361.6' NORTHING 755852 | | COLLAR ELEVATION 361.2' NORTHING 755842 EASTING 1943257 24 Hr. 3. |
| | · · · · · · · · · · · · · · · · · · · | TOTAL DEPTH 57.7' DRILL MACHINE CME-550 DRILL METHOD H.S. AUGERS HAMMER TYPE AUTOM |
| | | START DATE 5/25/04 COMPLETION DATE 5/26/04 SURFACE WATER DEPTH N/A DEPTH TO ROCK N/A |
| ELEV. DEPTH BLOW COUNT PEN. BLOWS PER FOOT (FT.) 0.5/0.5/0.5 (FT.) 0 25 50 75 100 | SAMPLE SOIL AND ROCK DESCRIPTION | ELEV. DEPTH BLOW COUNT PEN. BLOWS PER FOOT SAMPLE OF SOIL AND ROCK DESCRIPTION |
| | | ± |
| 361.6 360.0 | SS-20 ALLUVIAL, SS-21 W TAN-GRAY, SANDY SILT | 361.2 - 0.0 WUHWUHWUH I.0 X 0 SS-31 W ALLUVIAL, SS-32 SS-32 TAN-BROWN AND GRAY SANDY |
| 355 0 1 | SS-22 BROWN, COARSE SAND SS-23 M 888 | 355.0 + 7.1 9 10 10 1.0 - X19 SS-34 W GRAY-BROWN, COARSE SAND W QUARTZ GRAVEL |
| 350.0 + 12.5 16 22 18 1.0 | M 0000 | 9.6 35 37 35 I.0 |
| 345.0 | RESIDUAL, | 14.6 33 49 51 0.9 51 0.9 COARSE SAND AND GRAVEL |
| + 17.5 40 38 40 1.0 + +6 + +6 + +6 + +6 + +6 +6 + +6 | W 800 URANGE-BROWN, WHITE, AND BLACK, | T 17.1 38 52 48 0.9 D 0.00 0.00 |
| 340.0 + 22.5 15 25 38 1.0 | M 60000 60000 90000 | |
| 335.0 27.5 23 33 60 1.0 | M 0000 0000 0000 0000 | 335.0 |
| 330.0 + 32.5 27 55 45 0.9 | 0000 | 330.0 + 32.1 45 55 0.8 |
| 325.0 = 37.5 40 60 0.9 | M WEATHERED ROCK (META-GRANITE) | 335.0 |
| 320.0 + 42.5 65 35 0.8 | | |
| BORING JERMINATED AT ELEVATION 318.3 FEET IN | | 315.0 + 47.1 25 30 70 0.9 |
| 310.0 TWEATHERED ROCK | | 310.0 |
| | | 305.0 |
| 305.0 | | BORING TERMINATED AT |
| 300.0 + | | 300.0 - ELEVATION 303.5 FEET - IN WEATHERED ROCK - |
| 295.0 | | 295.0 ± |
| 290.0 = | | 290.0 = |
| 285.0 | | 285.0 |

\top north carolina department of transportation $\overline{}$ north carolina department of transportation $\overline{}$ GEOTECHNICAL UNIT BORING LOG

| PROJECT | NO. | 332 | 76.1 | .1 | | D. B | -382 | 4 | COUN | NTY CH | ATHAN | M | G | EOLO | GIST | J.L. | LOVE | | w | PROJECT | NO. 3 | 3276 | . . | | ID. | 3-382 | 4 | COUN' | ry c | НАТНА | ДМ | (| GEOLOG | IST | J.L. L0 | | ET 10 OF 1 |
|-----------|-------------|----------------|--------------------|----------------|---------------|-------------|--|-------------|-----------------|----------|-------------------|----------------|----------|---------------------------------|------|----------|-----------------|------------------|----------|--------------------------------|--------------|------|--|--------------|--------|--|-------------------|-------|-----------------|--------|----------------|--------|--------|-------|--------------------|----------------|---------------------|
| SITE DESC | CRIP | rion | BR | DGE | NO. | 88 | ON - | ·L- (SI | R 15 | 25) OVE | R FER | RELL'S | CRE | EΚ | | | | GROUN | D WATER | SITE DESC | RIPTION | BI | RIDG | E NO | 0.88 | ON - | -L- (S | R 152 | 5) OV | ER FE | RRELL'S | CRE | EEK | | | | ROUND WATI |
| BORING 1 | NO. | B2- | Α | BOI | RING | LOCA | TION | 13+5 | 51 | 0 | FFSET | 5′ RT | A | LIGN | MENT | -L- | | 0 HR. | N/A | BORING N | 0. B2 | 2-B | В | ORIN(| G LOC. | ATION | 13+5 | 56 | | OFFSET | 5′RT | 1 | ALIGNM | ENT | -L- | 0 | HR. N/A |
| COLLAR | ELEV | ATION | 1 3 | 65.0 | , | NO | RTHIN | G 75 | 583 | 5 | | EASTIN | G | 1943 | 3313 | | | 24 HR. | 6.2′ | COLLAR E | LEVATI | ON : | 366. | .1′ | N | ORTHIN | G 75 | 55824 | | | EASTI | NG | 1943 | 315 | | 24 | HR. 6.9' |
| TOTAL D |)EPTH | I 47. | 4′ | DR | LL M | ACHI | NE CN | E-550 | | DRILL | , METHO | OD H.S. | AUGE | ERS | | HAM | MER TY | PE AUT | OMATIC | TOTAL DE | PTH 4 | 7.8′ | D | RILL | MACH | ine ca | 1E-550 | | DRI | LL MET | HOD H.S. | AUG | ERS | | HAMME | R TYPE | AUTOMATIC |
| START D | ATE | 5/2 | 4/0 | 4 | C | OMPLI | ETION | DATE | 5/24 | 4/04 | SURF | ACE WAT | ER DE | PTH | N/A | DE | PTH TO | ROCK N | I/A | START DA | TE 5 | /20/ | Q4 | | COMP | LETION | DATE | 5/20 | /04 | SU | RFACE WA' | rer di | EPTH 1 | 1/A | DEPTI | H TO RO | CK N/A |
| ELEV. | 1 | PTHE | | | 1 | | | LOWS | | R FOOT | | SAMPLE | 1 / | 161 | | SOI | L AND | ROCK | | ELEV. | DEPTH | 1 | | | | | BLOWS | | | | SAMPLE | . 1 / | / L | | SOIL | AND R | OCK |
| | <u> (F</u> | T.) | 0.5 ₁ 0 |).5j0 |) <u>.5</u> ∜ | FT.) |) | 25 | 50 | 75 | 100 | NUMBER | MO | 11. Ğ | ļ | DE | ESCRIP | TION | | | (FT.) | 0.5 | <u> 10.5</u> | <u> 10.5</u> | (FT.) | <u> </u> | 25 | 50 | 75 | 100 | NUMBER | MC WC | DI. Ğ | | DESC | CRIPTIC |)N |
| | ‡ | | | | | | | _ | _ - | | | | | | | | | | | - | Ė | | | İ | | | - | | | | | | | | | | |
| 365.0 - | +, | 0.0 | /OHV | MH | + | | | | | | | | Н | | | RUAL | DWAY EN | MBANKMEN | JT. | 366.1 - 365.0 - | 0.0 | ╂ | | 1-5 | 1.0 | | | | | | 166.04 | N | | | DO A DIM A V | TAID AND | ZNENT |
| , | + | | | ı | | | | | | | | | w | | | | BROWN, S | SANDY CL | | 365.0 - | 2.1 | 1 | 4 | 4 | i.0 | X 8 | - | | | | SS-24 SS-25 | | | ≀ RED | ROADWAY -BROWN, | SANDY (| CLAY WITH PIECES |
| 360.0 - | | 3 . 5 V | - 1 | - 1 | | (٥٠١ | [<u>-</u> - | | | | | | " | | CRAY | Y CAND | ALLU\ Y SILT | /IAL. W/ WOOD | NERRIC | | 4.6 | WOH | 1 | 0 | 1.0 | XT | -1 | | | | 33 23 | | . M | | SOME AS | <u>PHALT F</u> | PIECES |
| 300.0 | # (| 6.0 h | /OH | 1 | 2 | 1.0 | X-2- | _ | | | | SS-29 | W | | | V | | SE SAND | <u> </u> | 360.0 - | _ | WOH | 1 | 1 | 1.0 | ¥T | _ | | | | SS-26 | 45% | · 💹 | 1AT | N-GRAY, | SANDY S | SILT WITH |
| 755.0 | ‡ ; | 8.5 | 60 | | - 10 | 0.0 | | | _ - | 60/ | <u>0.0</u> X | | | 000 000 000 000 000 | | 1, | '* | | | | † | | 1 | 1 | | | - | | | 100+- | , 33 20 | W | | | | D DEBR | |
| 355.0 - | + | 11.0 | 14 | 15 | 14 | 1.0 | | - ‡ - 2 | 9- - | | | SS-30 | W | 0000 | _ | QUAF | RTZ GRA | VEL LAY | ER/ | 355.0 - | _ | 25 | 1 | Į. | 0.7 | | | | | | | | 000 | \G | | | RSE SAND |
| | Ŧ | | | | | | | | - | | | | | 000 | | | | | | - | 12.1 | 10 | 14 | 16 | 1.0 | | - | θ- | | | SS-27 | M | 000 | \ | GRAVI | EL LAYE | R |
| 350.0 - | 士, | 6.0 | 101 | 19 | 24 | | | | V 43 | | | | l w | 000 | | | | | | 350 . 0 - | E | | | | | | | (| | | | | 0000 | | | | |
| | ‡ ' | 0.0 | 10 | 1 ' | _ | '•' | F | -} | 7 | | | | | 0000 | | | RESIDI | JAL. | | 350.0 - | 17.1 | 13 | 20 | 23 | 1.0 | | | X 43 | | | | М | 0000 | | | | |
| 345.0 - | # | | | | | | | _ | | | | | | 0000 | TA | N. WHIT | | ORANGE-E | BROWN. | | | | | | | | | -7- | | | | | 000 | | R | ESIDUAL | |
| | + 2 | 21.0 | 9 | 22 | 31 | 1.0 | | _ | - X | 53 | | | W | 000 | | | COARSE | | * - | 345.0 - | 22.1 | 22 | 30 | 25 | 1.0 | | - | | - 55 | | | | 000 | ORA | | | ARSE SAND |
| 740.0 | ‡ | | | 1 | | | | - | | | | | | 000 | | | , | | | - | <u> </u> | | | | | | - | f_ | | | | | 0000 | 01.7 | | o 1, 00. | |
| 340.0 - | \pm 2 | 6.0 | 28 | 42 | 48 | 1.0 | | - | | |)×- | | D | 000 | } | | | | | 340.0 - | F 27, | 1,7 | 200 | | | | - | | f = = - | | 66.00 | | 000 | | | | |
| | ‡ | | | | | | <u> </u> | | | | | | | 000 | | | | | | | [21.1 | 17 | 30 | 130 | 1.0 | | | | k 60 | | SS-28 | | 000 | | | | |
| 335.0 - | +: | 31.0 | 18 | 29 | 36 | 1.0 | | | | X-65- | | | D | 000 | | | | | | 335.0 | | - | | | | | | -1- | | | | | 900 | | | | |
| | ‡ ` | | | - | | | | | | | | | - | 9777 | | | | | | 555.0 | 32.1 | 33 | 57 | 43 | 0.7 | | - | | | 160+3 | | | | | | | |
| 330.0 - | ‡, | 6.0 | 27 | 20 | | | | - | | |)0+X | | | | | | | | | | F | | | | | | - | | | | | | S | | | | |
| | Ī 3 | 06.0 | 2/ | 36 | 02 | او.ق | | - | | | X | | | S | | | | | | 330.0 - | 37.1 | 70 | 30 | | 0.6 | | _ | | | 100+-> | k | | | | | | |
| 325.0 - | _ | | | | | | | -] | | | | | | | WEA | N TUEDER | ם סטרוי | (META-GI | DAMITEL | | E | | | | | | -} | | | | | | | WEAT | HERED F | ROCK (ME | TA-GRANITE) |
| 325.0 - | ‡ 4 | 41.0 | 25 | 75 | - 1 | 0.8 | <u> </u> | _ | | 40 | 20+ -* | | | | "" | ATHENEL | NOCK | TWIL I A GI | VAINTE/ | 325.0 - | 421 | 55 | 45 | | 0.7 | | -} | | | 100+> | | | S | | | | |
| 700.0 | ‡ | | | | | | | - | | | | | | S | | | | | | - | '~" | | ' | | 0.1 | | _ | | | | | | | | | | |
| 320.0 - | \pm 4 | 16.0 | 31 | 40 | 6Ø | 0.9 | | _ | | | 00+X | | | | | | | | | 320.0 - | <u> </u> | | | | 7 | | - | - | | | | | | | | | |
| | # | | | $\neg \dagger$ | | | BOF | UNG_T | ERMI | INATED | AI | | | - VIL | 1 | | | | | - | 4/. | 65 | <u>35</u> | + | 10.7 | | _ | ==== | | 100+-> | | | - | | | | |
| 315.0 - | + | | | | | | 1 | 1 | i . | 317.6 FE | 1 | | | | | | | | | 315.0 | L | | | | | | RING _ | l l | 1 | | | | | | | | |
| | ‡ | | | | | . | 1 | 1 | 1 | RED_BOC | | | | | | | | | | 515.0 | <u> </u> | | | | | | EVATI | | | | | | | | | | |
| 310.0 - | + | | | | | | <u> </u> | CMETA | T-GR | ANITE | | | | | | , | | | | | ‡ | | | | | 1 | WEAT | - 1 | | | | | | | | | |
| | Ŧ | | | | | | | - † | | | | | | | | | | | | 310.0 - | F | | | | | | | | ממובע | | | | | | | | |
| 305.0 - | \pm | | | | | | F | | | | | | | | | | | | | - | F | | | | | | | | | | | | | | | | • |
| 303.0 | ‡ | | | | | | | -} | | | | | | | | | | | | 305.0 | F | | | | | | -} | | | | | | | | | | |
| | ‡ | | | | | | | - | _ _ | | | | | | | | | | | - | <u> </u> | | | | | | - | | | | | | | | | | |
| 300.0 - | ‡ | | | | | | | | _ - | | | | | | | | | | | 300.0 - | <u> </u> | | | | | | _ | | | | | | | | | | |
| | Ŧ | | | | | | | - | - - | | | | | | | | | | | | ‡ | | | | | | _ | | | | | | | | | | |
| 295.0 - | \pm | | | | | l | | _ | - - | | | | | | | | | | | | ‡ | | | | | | - | | | | | | | | | | |
| | Ŧ | | | | j. | W | <i></i> | - | - - | | | | | | | | | | | 295.0 | - | | | | | | - | | | | | | | | | | |
| 290.0 - | + | | | | 1 | trans. | | | -1- | | | | | | | | | | | | E | | | | | | | | | | | | | | | | |
| | ‡ | | | | | | <u> </u> | | [- | | | | | | | | | | | 290.0 | | | | ' | | | | | | | | | | | | | |
| <u> </u> | | | | | | | L | | | | | | 1 | | 1 | | | | | JIL | L | | 1 | | | | | | | | | | | | | | |

North Carolina Department of transportation | North Carolina Department of transportation GEOTECHNICAL UNIT BORING LOG

| The property of the property o | DDATEOR NA 7707C LL ID D 7004 CONTINUE CLUTTURE CONTINUE | SHEET 11 OF 15 |
|--|---|------------------------|
| PROJECT NO. 33276.I.I ID. B-3824 COUNTY CHATHAM GEOLOGIST J.L. LOVE | PROJECT NO. 33276.I.I ID. B-3824 COUNTY CHATHAM GEOLOGIST J.L. LOV | |
| WITE DEBORM TION | IND WATER SITE DESCRIPTION BRIDGE NO. 88 ON -L- (SR 1525) OVER FERRELL'S CREEK | GROUND WATER |
| | 22.0' BORING NO. EB2-B BORING LOCATION 14+12 OFFSET 13' RT ALIGNMENT -L- | 0 HR. 20.2' |
| | . 18.7' COLLAR ELEVATION 379.2' NORTHING 755801 EASTING 1943367 | 24 HR. 18.8′ |
| TOTAL DEPTH 39.9' DRILL MACHINE BK-51 DRILL METHOD H.S. AUGERS HAMMER TYPE AU | TOMATIC TOTAL DEPTH 38.6' DRILL MACHINE BK-51 DRILL METHOD H.S. AUGERS HAMMEI | R TYPE AUTOMATIC |
| START DATE 7/17/03 COMPLETION DATE 7/17/03 SURFACE WATER DEPTH N/A DEPTH TO ROCK | | H TO ROCK N/A |
| SOIL AND ROCK | ELEV. DEPTH BLOW COUNT PEN. BLOWS PER FOOT SAMPLE SOIL | AND ROCK |
| ELEV. (FT.) 0.510.510.5 (FT.) 0 25 50 75 100 NUMBER MOI. G DESCRIPTION | CLEV. (FT.) 0.510.510.5 (FT.) 0 25 50 75 100 NUMBER MOI. G DESC | CRIPTION |
| 379.2 | 379.2 | |
| | T Z O O O O O O O O O O O O O O O O O O | EMBANKMENT, |
| 375.0 + 3.4 2 1 2 1.0 X 3 + + M ROADWAY EMBANKME | | SILTY SANDY CLAY |
| THE RED-ORANGE, SILTY SAND | | |
| 370 0 ± 8.4 2 2 4 1.0 - + 6 - M WITH SOME MICA AI | \\\\\\\\\\\\\\\\\\\\\\\\\\\\\\\\\\\\\ | SOME MICA |
| 370.0 + 8.4 2 2 4 1.0 | 11 \$ / (1) 1 | |
| | | LLUVIAL, |
| 365.0 + 13.4 2 3 3 1.0 + 6 - + + M | $ _{365.0} + _{3.2} _{1} _{2} _{2} _{1.0} _{x-4-++$ | LAY AND SILTY CLAY |
| 365.0 ± 13.4 2 3 3 1.0 7.6 M M ALLUVIAL, GRAY, SANDY | CLAY | 1 |
| | 11 | ORGANIC MATTER |
| $360.0 \pm 18.4 \text{ WOH}$ 1 2 1.0 $\pm 3.7 \pm 1.7 \pm 1$ | 360.0 + 18.2 WOT WOT 1 1.0 + 10.1 1.0 + 10.1 | |
| <u> </u> | | SILTY SAND |
| $\begin{vmatrix} 1 & 1 & 1 & 1 & 1 & 1 & 1 & 1 & 1 & 1 $ | $ _{700}$ + 23.2 1 3 23 1.0 $ _{}$ 26 - + | SANDY SILT |
| 355.0 | 355.0 + | WN, COARSE SAND |
| | WEATHEREI | D ROCK (DIORITE) |
| $350.0 \pm 28.4 = 5 = 28 = 65 = 1.0 = 28 = 1.0 = 28 = 28 = 28 = 28 = 28 = 28 = 28 = 2$ | 350.0 + 20.2 10 70 0.3 350.0 | / NOOK (DIONITE) |
| TRESPOSAL, | | CIBLIAL |
| + 77 4 25 4 7 2 1 0 1 + + V -to | | SIDUAL, |
| $\begin{vmatrix} 345.0 & + & 35.4 \end{vmatrix}$ | | OWN, COARSE SAND |
| | | HERED ROCK DIORITE) |
| 340.0 = 38.4 20 35 32 1.0 = = = = = = = = = = M 67 = = = M 688 | 340.0 +- | IOIII L7 |
| BORING TERMINATED AT | T BORING IERMINATED AT | |
| | | |
| 335.0 + | 335.0 = | |
| | | |
| 330.0 + | 330.0 + | |
| | | |
| | | |
| 325.0 + | | |
| <u> </u> | | |
| 320.0 + | 320.0 + | |
| | | |
| <u>+ </u> | | |
| 315.0 + | 315.0 + | |
| | | |
| 710 0 + | | |
| 310.0 + | 310.0 + | |
| | | |
| 305.0 + | 305.0 + | |
| | | |
| | | |
| 300.0 + | | |
| | | |

NORTH CAROLINA DEPARTMENT OF TRANSPORTATION GEOTECHNICAL UNIT BORING LOG

| PROJEC | r no . 33 | 3276 | .1.1 | | ID. | 3-3824 | COUNT | ry (| CHATHA | <u></u> М | GE | OLO | GIST | J.L. LOVE | - |
|----------------|------------------|------|-------|-------|------|-------------|----------------|-------|--------|---------------------------|----------|-----|-------|------------------|--|
| SITE DE | SCRIPTION | BF | RIDGI | E NO | 88 . | ON -L- (| SR 152 | 5) 0\ | /ER FE | RRELL'S | CREE | ΪK | | | GROUND WATER |
| BORING | NO. EB | 2-C | B | ORING | LOC | ATION 14+ | 10 | | OFFSET | I2' RT | AL | IGN | MENT | -L- | 0 HR. N/A |
| | ELEVATIO | | 379. | | | | 55803 | | | EASTIN(| | | 3365 | | 24 HR. 18.8′ |
| | DEPTH 25 | | | | | INE CME-45C | | | | OD H.S. | | | | | TYPE AUTOMATIC |
| START | DATE 8/ | | | | | LETION DATE | | | | FACE WATE | ER DEP | TH | N/A | DEPTH ' | TO ROCK N/A |
| ELEV | DEPTH (FT.) | 1 | | | | | 50 50 | F00 | | SAMPLE NUMBER | MOI. | O G | | SOIL AN DESCR | ID ROCK IPTION |
| 379.2 | + | | | | | | | | | | | | | | |
| | ‡ | | | | | | | | | | М | | | ROADWAY E | MBANKMENT, |
| 375.0 | + | | | | | | | | | | | | RED | -ORANGE, SO | OFT TO MEDIUM |
| | Ŧ | | | | | | | | | | | | S | TIFF. SILTY | SANDY CLAY |
| 370.0 | \pm | | | | | | |] | | | | | | • | |
| | ± 11 . 6 | 1 | 1 | 2 | 1.0 | X-3 | - | | | SS-20 | 18% | | | ALLU' | VIAI |
| 365 . 0 | ± 14.1 | 1 | 2 | 1 | 1.0 | | - | | | | М | | RF | | SANDY CLAY |
| | 16.6 | 1 | 2 | 1 | 1.0 | X3 | | | | SS-2I | 34% | | | | LTY CLAY WITH |
| 360 . 0 | + | 1 | 1 | l | | | | | | SS-22 | 23% | | | | OODY DEBRIS |
| 200.0 | + | | | | | 1 1+ | | | | SS-23 | 23% W | | | GRAY SA | ANDY SILT |
| | ‡ 21 . 6 | | | l | 1.0 | X-2 | | | | SS-24 | W | | | | |
| 355.0 | ± 24.1 | 5 | 14 | 20 | 1.0 | X | 34 - | | | SS-24 SS-25A SS-25B | M | | - 004 | | DUAL, |
| | ‡ | | | | | BORING | | | 1 | | | | | | GREEN, AND PINK, / TIC COARSE SAND/ |
| 350.0 | 丰 | | | | | ELEVATIO | 1 | 1 | 1 | | | | DEIA | DE, SALIVULI | TIC COARSE SAND |
| | Ŧ | | | | | RESIDUAL | _COAI | RSE | SAND | | | | | | |
| 345.0 | <u>+</u> | | | | | | | | | | | | | | |
| 0 10.0 | ‡ | | | | | | - | | | | | | | | |
| 740.0 | ‡ | | | | | | | | | | | | | | |
| 340.0 | Ŧ | | | | | | | | | | | | | | |
| | Ī | | | | | | | | | | | | | | |
| 335.0 | + | | | | | | | | | | | | | | |
| | ‡ | | | | | | | | | | | | | | |
| 330.0 | + | | | | | | | | | | | | | | |
| | Ŧ | | | | | | | | | | | | | | |
| 325.0 | <u>±</u> | | | | | | |] | | | | | | | |
| J2J•U | ‡ | | | | | | |] | | | | | | | |
| | ‡ | | | | | | | | | | | | | | |
| 320.0 | + | | | | | | | | | | | | | | |
| | ‡ | | | | | | | | | | | | | | |
| 315.0 | + | | | | | | | | | | | | | | |
| | Ŧ | | | | | | | | | | | | | | |
| 310.0 | + | | | | | | | | | | | | | | |
| 5,010 | ‡ | | | | | | | | | | | | | | |
| 705.5 | ‡ | | | | | | | | | | | | | | |
| 305.0 | + | | | | | | - | | | | | | | | |
| | Ŧ | | | | | | | | | | | | | | |
| 300.0 | + | | | | | | | | | | | | | | |
| | _ T | | | L | | | † - | | | | | | | | |

SHEET 12 OF 15

PROJ. NO. - 33276.1.1 ID NO. - B-3824 COUNTY -CHATHAM

EB1-A

| EDI-A | | | | | | | | | | | | | | | |
|--------|--------|---------|-----------|----------|------|------|--------|--------|--------|------|-------|---------|--------|----------|---------|
| | | | S | OIL I | TE. | ST | RE | SUI | TS | | | | | | |
| SAMPLE | | | DEPTH | AASHTO | | | | % BY V | /EIGHT | | % PAS | SING (S | IEVES) | % | % |
| NO. | OFFSET | STATION | INTERVAL | CLASS. | L.L. | P.I. | C.SAND | F.SAND | SILT | CLAY | 10 | 40 | 200 | MOISTURE | ORGANIC |
| SS-7 | 12' LT | 12+59 | 3.4-4.9 | A-6(8) | 40 | 23 | 29.0 | 20.3 | 18.2 | 32.5 | 94 | 80 | 50 | • | - |
| SS-8 | 12' LT | 12+59 | 13.4-14.9 | A-6(1) | 25 | 11 | 28.2 | 31.5 | 15.9 | 24.4 | 100 | 91 | 44 | - | - |
| SS-9 | 12' LT | 12+59 | 24.4-24.9 | A-2-4(0) | 31 | NP | 55.8 | 21.7 | 16.3 | 6.1 | 97 | 55 | 26 | - | - |
| SS-10 | 12' LT | 12+59 | 28.4-29.9 | A-2-4(0) | 28 | NP | 54.3 | 24.1 | 14.5 | 7.1 | 99 | 59 | 25 | - | • |

EB1-B

| | | | S | COIL T | TE. | ST | RE | SUI | TS | | | | | | |
|--------|--------|---------|-----------|--------|------|------|--------|--------|--------|------|-------|---------|--------|----------|---------|
| SAMPLE | | | DEPTH | AASHTO | | | | % BY W | VEIGHT | | % PAS | SING (S | IEVES) | % | % |
| NO. | OFFSET | STATION | INTERVAL | CLASS. | L.L. | P.I. | C.SAND | F.SAND | SILT | CLAY | 10 | 40 | 200 | MOISTURE | ORGANIC |
| SS-11 | 7' RT | 12+66 | 24.1-25.0 | A-3(0) | 29 | NP | 66.0 | 26.6 | 5.4 | 2.0 | 100 | 74 | 9 | | |

B1-A

| DI-A | | | | | | | | | | | | | | | |
|--------|--------|---------|----------|----------|------|------|--------|--------|------|------|-----|----|-----|----------|---------|
| | | | S | OIL T | TE. | ST | RE | SUL | TS | | | | , | | |
| SAMPLE | | | | | | | | | | | | | | | |
| NO. | OFFSET | STATION | INTERVAL | CLASS. | L.L. | P.I. | C.SAND | F.SAND | SILT | CLAY | 10 | 40 | 200 | MOISTURE | ORGANIC |
| SS-20 | 4' LT | 12+90 | 0.0-1.5 | A-4(0) | 22 | 3 | 13.1 | 45.1 | 25.7 | 16.2 | 100 | 97 | 47 | | |
| SS-21 | 4' LT | 12+90 | 3.2-4.1 | A-4(0) | 20 | 2 | 12.9 | 40.0 | 32.9 | 14.1 | 100 | 98 | 54 | - | • |
| SS-22 | 4' LT | 12+90 | 4.1-4.7 | A-2-4(0) | 26 | NP | 67.1 | 22.2 | 6.7 | 4.0 | 96 | 60 | 12 | | |
| SS-23 | 4' LT | 12+90 | 7.5-9.0 | A-1-b(0) | 30 | NP | 56.2 | 24.8 | 10.9 | 8.1 | 73 | 41 | 17 | | • |

B1-B

| | | | S | OIL 7 | TE | ST | RE | SUI | LTS | | | | | | |
|--------|--------|---------|-----------|----------|------|------|--------|--------|--------|------|-------|---------|---------|----------|---------|
| SAMPLE | | | DEPTH | AASHTO | | | | % BY W | /EIGHT | | % PAS | SING (S | SIEVES) | % | % |
| NO. | OFFSET | STATION | INTERVAL | CLASS. | L.L. | P.I. | C.SAND | F.SAND | SILT | CLAY | 10 | 40 | 200 | MOISTURE | ORGANIC |
| SS-31 | 4' RT | 12+95 | 0.0-1.0 | A-4(0) | 24 | 5 | 20.6 | 41.8 | 19.4 | 18.2 | 99 | 93 | 41 | • | |
| SS-32 | 4' RT | 12+95 | 1.0-1.5 | A-4(0) | 20 | NP | 6.1 | 49.1 | 26.7 | 18.2 | 100 | 99 | 53 | 26.5 | • |
| SS-33 | 4' RT | 12+95 | 2.3-3.0 | A-4(2) | 25 | 7 | 6.9 | 40.0 | 34.9 | 18.2 | 100 | 98 | 62 | • | - |
| SS-34 | 4' RT | 12+95 | 3.0-3.5 | A-2-4(0) | 29 | NP | 62.4 | 28.5 | 5.1 | 4.0 | 99 | 72 | 11 | • | • |
| SS-35 | 4' RT | 12+95 | 22.1-23.6 | A-1-b(0) | 28 | NP | 58.0 | 24.2 | 13.7 | 4.0 | 61 | 33 | 14 | | |

B2-A

| | | | S | OIL 7 | TE. | ST | RE | SUL | TS | | | | | | |
|--------|--------|---------|-----------|----------|------|------|--------|--------|------|------|-----|----|-----|----------|---------|
| SAMPLE | | | | | | | | | | | | | | | |
| NO. | OFFSET | STATION | INTERVAL | CLASS. | L.L. | P.I. | C.SAND | F.SAND | SILT | CLAY | 10 | 40 | 200 | MOISTURE | ORGANIC |
| SS-29 | 5' LT | 13+51 | 6.0-7.5 | A-2-4(0) | 25 | NP | 57.2 | 28.3 | 6.5 | 8.1 | 100 | 85 | 16 | | • |
| SS-30 | 5' LT | 13+51 | 11.0-12.5 | A-1-b(0) | 26 | NP | 62.0 | 21.8 | 12.1 | 4.0 | 72 | 35 | 14 | - | |

B2-B

| | | | S | OIL T | TE. | ST | RE | SUI | LTS | | | | | | |
|--------|--------|---------|-----------|----------|------|------|--------|--------|--------|------|-------|---------|--------|----------|---------|
| SAMPLE | | | DEPTH | AASHTO | | | | % BY V | VEIGHT | | % PAS | SING (S | IEVES) | % | % |
| NO. | OFFSET | STATION | INTERVAL | CLASS. | L.L. | P.I. | C.SAND | F.SAND | SILT | CLAY | 10 | 40 | 200 | MOISTURE | ORGANIC |
| SS-24 | 5' RT | 13+56 | 0.0-1.5 | A-6(2) | 36 | 16 | 32.1 | 18.6 | 19.0 | 30.3 | 78 | 63 | 40 | - | |
| SS-25 | 5' RT | 13+56 | 2.1-3.6 | A-4(1) | 26 | 8 | 29.7 | 26.3 | 21.8 | 22.2 | 99 | 82 | 47 | - | - |
| SS-26 | 5' RT | 13+56 | 7.1-8.6 | A-4(3) | 26 | 9 | 4.2 | 41.4 | 24.0 | 30.3 | 100 | 100 | 59 | 45.3 | - |
| SS-27 | 5' RT | 13+56 | 12.1-13.6 | A-1-b(0) | 26 | NP | 63.4 | 21.8 | 10.7 | 4.0 | 82 | 41 | 15 | - | - |
| SS-28 | 5' RT | 13+56 | 27.1-28.6 | A-1-b(0) | 26 | NP | 64.0 | 19.6 | 12.3 | 4.0 | 85 | 42 | 17 | - | • |

EB2-A

| SOIL TEST RESULTS | | | | | | | | | | | | | | | |
|-------------------|--------|---------|-----------|--------|------|------|--------|--------|--------|------|-------|---------|---------|----------|---------|
| SAMPLE | | | DEPTH | AASHTO | | | | % BY W | /EIGHT | | % PAS | SING (S | SIEVES) | % | % |
| NO. | OFFSET | STATION | INTERVAL | CLASS. | L.L. | P.I. | C.SAND | F.SAND | SILT | CLAY | 10 | 40 | 200 | MOISTURE | ORGANIC |
| SS-6 | 11' LT | 13+98 | 18.4-19.9 | A-4(0) | 18 | NP | 5.5 | 49.5 | 28.7 | 16.2 | 100 | 99 | 52 | - | • |

EB2-C

| UDZ-C | | | | | | | | | | | | | | | |
|--------|--------|---------|-----------|-----------|------|------|--------|--------|--------|------|-------|---------|--------|----------|---------|
| | • | | S | OIL 7 | TE. | ST | RE | SUI | LTS | | | | | | |
| SAMPLE | | | DEPTH | AASHTO | | | | % BY V | VEIGHT | | % PAS | SING (S | IEVES) | % | % |
| NO. | OFFSET | STATION | INTERVAL | CLASS. | L.L. | P.I. | C.SAND | F.SAND | SILT | CLAY | 10 | 40 | 200 | MOISTURE | ORGANIC |
| SS-20 | 12' RT | 14+10 | 11.6-13.1 | A-6(9) | 40 | 22 | 31.9 | 18.4 | 17.4 | 32.3 | 100 | 79 | 56 | 18.0 | - |
| SS-21 | 12' RT | 14+10 | 16.6-17.6 | A-7-6(20) | 43 | 20 | 2.2 | 7.3 | 42.0 | 48.5 | 100 | 99 | 93 | 34.2 | 6.3 |
| SS-22 | 12' RT | 14+10 | 19.1-2.10 | A-4(0) | 20 | 3 | 7.9 | 42.2 | 25.7 | 24.2 | 100 | 99 | 59 | 22.8 | - |
| SS-23 | 12' RT | 14+10 | 21.6-22.2 | A-4(0) | 22 | 2 | 8.9 | 50.9 | 20.0 | 20.2 | 100 | 99 | 47 | | - |
| SS-24 | 12' RT | 14+10 | 22.2-23.1 | A-4(0) | 24 | 4 | 10.3 | 45.7 | 23.8 | 20.2 | 100 | 98 | 61 | - | |
| SS-25A | 12' RT | 14+10 | 24.1-24.8 | A-2-4(0) | 24 | NP | 56.4 | 23.8 | 13.7 | 6.1 | 96 | 58 | 22 | - | - |
| SS-25B | 12' RT | 14+10 | 24.8-25.6 | A-2-4(0) | 27 | NP | 58.0 | 21.2 | 14.7 | 6.1 | 90 | 51 | 23 | | - |

EB2-B

| | | | S | OIL 7 | TE. | ST | RE | SUI | LTS | | | | | | |
|---------|--------|---------|-----------|-----------|------|------|--------|--------|--------|------|-------|---------|--------|----------|---------|
| SAMPLE | | | DEPTH | AASHTO | | | | % BY V | VEIGHT | | % PAS | SING (S | IEVES) | % | % |
| NO. | OFFSET | STATION | INTERVAL | CLASS. | L.L. | P.I. | C.SAND | F.SAND | SILT | CLAY | 10 | 40 | 200 | MOISTURE | ORGANIC |
| SS-1 | 13' RT | 14+12 | 3.2-4.7 | A-7-6(15) | 49 | 26 | 22.3 | 16.2 | 22.8 | 38.6 | 100 | 86 | 65 | | - |
| SS-2 | 13' RT | 14+12 | 13.2-14.7 | A-6(2) | 28 | 12 | 36.1 | 22.5 | 19.0 | 22.3 | 97 | 76 | 43 | - | - |
| SS-3 | 13' RT | 14+12 | 18.2-19.7 | A-6(11) | 33 | 14 | 1.6 | 19.5 | 38.3 | 40.6 | 100 | 100 | 83 | | |
| ST-1 #1 | 12' RT | 14+15 | 21.0-21.7 | A-2-4(0) | 20 | NP | 21.3 | 54.7 | 9.9 | 14.1 | 100 | 96 | 28 | - | - |
| ST-1 #2 | 12' RT | 14+15 | 21.7-22.3 | A-2-4(0) | 20 | NP | 14.1 | 60.8 | 11.1 | 14.1 | 100 | 99 | 30 | • | |
| ST-1 #3 | 12' RT | 14+15 | 22.3-23.0 | A-4(0) | 21 | 2 | 8.9 | 54.9 | 18.1 | 18.1 | 100 | 99 | 42 | | - |
| SS-4 | 13' RT | 14+12 | 23.2-24.7 | A-3(0) | 25 | NP | 62.9 | 31.3 | 3.8 | 2.0 | 100 | 81 | 8 | - | - |
| SS-5 | 13' RT | 14+12 | 33.2-34.7 | A-1-b(0) | 28 | NP | 61.7 | 20.3 | 13.9 | 4.1 | 91 | 48 | 19 | - | |

GEOTECHNICAL UNIT FIELD SCOUR REPORT

| PROJECT: 8.2522101 ID: B-3824 COUNTY: Chatham |
|--|
| DESCRIPTION(1): Bridge No. 88 on -L- (SR 1525) over Ferrell's Creek |
| |
| INFORMATION ON EXISTING BRIDGE Information obtained from: |
| BR. NO.: 88 BR. LENGTH: 116 ft. NO. BENTS: 4 NO. BENTS IN: CHANNEL: 2 FLOODPLAIN: 2 |
| FOUNDATION TYPE:Timber piles |
| |
| EVIDENCE OF SCOUR(2): |
| ABUTMENTS OR END BENT SLOPES: None visible |
| INTERIOR BENTS: One to two feet of local scour around piles. |
| |
| CHANNEL BED: None visible |
| CHANNEL BANKS: Eroding channel banks, approximately 2 to 3 feet in height. |
| EXISTING SCOUR PROTECTION: |
| TYPE(3):Timber abutments at both end bents.Concrete reinforcement around the base of Bent 1 piles. |
| EXTENT(4): Abutments extend approximately 10 feet from the outside edge of the bridge. |
| EFFECTIVENESS(5): Effective |
| OBSTRUCTIONS(6) (DAMS,DEBRIS,ETC.): Small limbs and branches around Bent 1 |
| DESIGN INFORMATION |
| CHANNEL BED MATERIAL(7) (SAMPLE RESULTS ATTACHED): Alluvial, tan-gray, soft to medium |
| stiff, sandy silt (SS-6 and SS-25) and brown, very loose, coarse sand (SS-22). |
| CHANNEL BANK MATERIAL(8) (SAMPLE RESULTS ATTACHED): Alluvial gray, very soft, sandy clay (SS-3) |
| and tan-brown and gray, very soft, sandy silt (SS-31 and 32). |
| CHANNEL BANK COVER(10): Trees, shrubs, and brush |
| FLOOD PLAIN WIDTH(11): +/- 150 feet |
| FLOOD PLAIN COVER(12): Grass, trees, and brush |

SHEET 14 OF 1

| | SHEET 14 O |
|---|--|
| DES | SIGN INFORMATION CONT. |
| STR | EAM IS: DEGRADINGX_ AGGRADING (13) |
| OTH | IER OBSERVATIONS AND COMMENTS: |
| | |
| | |
| | |
| CHA | NNEL MIGRATION TENDENCY (14): West, towards End Bent 1 |
| GEC | DTECHNICALLY ADJUSTED SCOUR ELEVATIONS(15): |
| | Elevation (feet) |
| | Bent 1 355.5 |
| | Bent 2 355.5 |
| | The GASE scour elevations agree with the Hydraulic Unit's predicted scour elevations. |
| | |
| | REPORTED BY: The P. Morfeld DATE: August 16, 2004 |
| (1) (2) (3) (4) (5) (6) (7) (8) (9) (10) (11) (12) (13) (14) (15) | INSTRUCTIONS GIVE THE DESCRIPTION OF THE SPECIFIC SITE GIVING ROUTE NUMBER AND BODY OF WATER CROSSED. NOTE ANY EVIDENCE OF SCOUR AT THE EXISTING END BENTS OR ABUTMENTS (UNDERMINING, SLOUGHING, SCOUR LOCATIONS, DEGRADATIONS, ETC.) NOTE ANY EXISTING SCOUR PROTECTION (RIR RAP, ETC.) DESCRIBE THE EXTENT OF ANY EXISTING SCOUR PROTECTION. DESCRIBE THE EXTENT OF ANY EXISTING SCOUR PROTECTION. DESCRIBE WHETHER OR NOT THE SCOUR PROTECTION APPEARS TO BE WORKING. NOTE ANY DAMS, FALLEN TREES, DEBRIS AT BENTS,ETC. DESCRIBE THE CHANNEL BED MATERIAL: A SAMPLE SHOULD BE TAKEN FOR GRAIN SIZE DISTRIBUTION, DESCRIBE THE CHANNEL BANK MATERIAL: A SAMPLE SHOULD BE TAKEN FOR GRAIN SIZE DISTRIBUTION, ATTACH LAB RESULTS. DESCRIBE THE FOUNDATION BEARING MATERIAL, DESCRIBE THE FOUNDATION BEARING MATERIAL, DESCRIBE THE FLOOD PLAIN COVERING (GRASS, TREES, RIP RAP, NONE, ETC. GIVE THE APPROXIMATE FLOOD PLAIN WIDTH (ESTIMATE). DESCRIBE THE FLOOD PLAIN COVERING (GRASS, TREES, CROPS, ETC.) CHECK THE APPROPRIATE SPACE AS TO WHETHER THE STREAM IS DEGRADING OR AGGRADING DESCRIBE THE POTENTIAL OF THE BODY OF WATER TO MIGRATE LATERALLY DURING THE LIFE OF THE BRIDGE (APPROXIMATELY 100 YEARS). GIVE THE GEOTECHNICALLY ADJUSTED SCOUR ELEVATION EXPECTED OVER THE LIFE OF THE BRIDGE (APPROXIMATELY 100 YEARS). THIS CAN BE GIVEN AS AN ELEVATION RANGE ACROSS THE SITE, OR ON A BENT BY BENT BASIS WHERE VARIATIONS EXIST. DISCUSS THE RELATIONSHIP BETWEEN THE HYDRAULICS THEORETICAL SCOUR AND THE GEOTECHNICALLY ADJUSTED SCOUR ELEVATION IS DEPENDENT ON SCOUR COUNTER MEASURES, EXPLAIN. (RIPRAP ARMORING ON SLOPES, ETC.) THE GEOTECHNICALLY ADJUSTED SCOUR ELEVATION IS BASED ON THE ERODABILITY OF MATERIALS WITH CONSIDERATION FOR JOINTING, FOLIATION, BEDDING ORIENTATION AND FREQUENCY; CORE RECOVERY PERCENTAGE; PERCENTAGE; PERCENTAGE; DERCENTAGE; OF PERCENTAGE; OF COHER PERCENTAGE; OF COHER PERCENTAGE; OF CHERCUTURES; OTHER TESTS DEEMED APPROPRIATE; AND OVERALL GEOLOGIC CONDITIONS AT THE SITE. |

SITE PHOTOGRAPHS

Bridge No. 88 on -L- (SR 1525) over Ferrell's Creek

