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STATE OF NORTH CAROLINA

DEPARTMENT OF TRANSPORTATION

DIVISION OF HIGHWAYS GEOTECHNICAL UNIT

STRUCTURE SUBSURFACE INVESTIGATION

STATE PROJECT 33271.1	.1 I.D. NO. B-3818
F.A. PROJECT	
COUNTY CALDWELL	
PROJECT DESCRIPTION	BRIDGE # 3 ON
NC 90 OVER LOST	
-	
SITE DESCRIPTION BRID	GE # 3 ON
NC 90 OVER LOST C	

STATE	PROJ. NO.	F. A. PROJ. NO.		P.E.	
N.C.		l.1.1 (B-3818)	1	I	

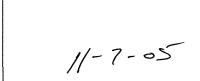
CAUTION NOTICE

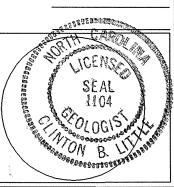
THE SUBSURFACE INFORMATION AND THE SUBSURFACE INVESTIGATION ON WHICH IT IS BASED WAS MADE FOR THE PURPOSE OF STUDY, PLANNING AND DESIGN, AND NOT FOR CONSTRUCTION OR PAY PURPOSES. THE VARIOUS FIELD BORING LOSS, ROCK CORES, AND SOIL TEST DATA AVAILABLE MAY BE REVIEWED OR INSPECTED IN RALEIGH BY CONTACTING THE N. C, DEPARTMENT OF TRANSPORTATION, GEOTECHNICAL LINIT @ (919) 250-4088. NEITHER THE SUBSURFACE PLANS AND REPORTS, NOR THE FIELD BORING LOGS, ROCK CORES, OR SOIL TEST DATA IS PART OF THE CONTRACT.

GENERAL SOIL AND ROCK STRATA DESCRIPTIONS AND INDICATED BOUNDARIES ARE BASED ON A GEOTECHNICAL INTERPRETATION OF ALL AVAILABLE SUBSURFACE DATA AND MAY NOT NECESSARILY REFLECT THE ACTUAL SUBSURFACE CONDITIONS BETWEEN BORINGS OR RETWEEN SAMPLED STRATA WITHIN THE BOREHOLE. THE LABORATORY SAMPLE DATA AND THE IN SITU (IN-PLACE) TEST DATA CAN BE RELIED ON ONLY TO THE DEGREE OF RELIABILITY INMERENT IN THE STANDARD TEST METHOD. THE OBSERVED WATER LEVELS OR SOIL MOISTURE CONDITIONS INDICATED IN THE SUBSURFACE INVESTIGATIONS ARE AS RECORDED AT THE TIME OF THE INVESTIGATION. THESE WATER LEVELS OR SOIL MOISTURE CONDITIONS MAY VARY CONSIDERABLY WITH TIME ACCORDING TO CLIMATIC CONDITIONS INCLUDING TEMPERATURES, PRECIPITATION AND WIND, AS WELL AS OTHER NON-CLIMATIC FACTORS.

THE BIDDER OR CONTRACTOR IS CALITIONED THAT DETAILS SHOWN ON THE SURSURFACE PLANS ARE PRELIMINARY ONLY AND IN MANY CASES THE FINAL DESIGN DETAILS ARE DIFFERENT, FOR BIDDING AND CONSTRUCTION PLANS AND DOCUMENTS FOR FINAL DESIGN INFORMATION ON THIS PROJECT. THE DEPARTMENT DOES NOT WARRANT OR GUARANTEE THE SUFFICIENCY OR ACCURACY OF THE INVESTIGATION MADE, NOR THE INTERPRETATIONS MADE OR OPINION OF THE DEPARTMENT AS TO THE TYPE OF MATERIALS AND CONDITIONS TO BE ENCOUNTERED. THE BIDDER OR CONTRACTOR IS CAUTIONED TO MAKE SUCH INDEPENDENT SUBSURFACE INVESTIGATIONS AF DEEMS NECESSARY TO SATISFY HIMSELF AS TO CONDITIONS TO BE ENCOUNTERED ON THIS PROJECT. THE CONTRACTOR SHALL HAVE NO CLAIM FOR ADDITIONAL COMPENSATION OR FOR AN EXTENSION OF TIME FOR ANY REASON RESULTING FROM THE ACTUAL CONDITIONS ENCOUNTERED AT THE SITE DIFFERING FROM THOSE INDICATED IN THE SUBSURFACE INFORMATION.

INVESTIGATED BY J.E. BEVERLY PERSONNEL R.W. TODD C.B. LITTLE M.L. SMITH SUBMITTED BY C.B. LITTLE C.E. BURRIS NOVEMBER 2005





NOTE - THE INFORMATION CONTAINED HEREIN IS NOT IMPLIED OR GUARANTEED BY THE N. C. DEPARTMENT OF TRANSPORTATION AS BEING ACCURATE NOR IT IS CONSIDERED TO BE PART OF THE PLANS. SPECIFICATIONS, OR CONTRACT FOR THE PROJECT.

FOR INCREASED COMPENSATION OR EXTENSION OF TIME BASED ON DIFFERENCES BETWEEN THE CONDITIONS INDICATED HEREIN AND THE ACTUAL CONDITIONS AT THE PROJECT SITE.

DRAWN BY: J.E. BEVERLY

NOTE - BY HAVING REQUESTED THIS INFORMATION THE CONTRACTOR SPECIFICALLY WAIVES ANY CLAIMS

NORTH CAROLINA DEPARTMENT OF TRANSPORTATION

DIVISION OF HIGHWAYS
GEOTECHNICAL UNIT

SUBSURFACE INVESTIGATION

	SOIL AND ROCK LE	EGEND, TERMS,	SYMBOLS, AND ABBREY	VIATIONS	
SOIL DESCRIPTION	GRADATION		ROCI	< DESCRIPTION	TERMS AND DEFINITIONS
SOIL IS CONSIDERED TO BE THE UNCONSOLIDATED, SEMI-CONSOLIDATED OR WEATHERED EARTH MATERIALS WHICH CAN BE PENETRATED WITH A CONTINUOUS FLIGHT POWER ALGER, AND WHICH YIELDS LESS THAN 100 BLOWS PER FOOT ACCOPDING TO STANDARD PENETRATION TEST (AASHTO T206, ASTM D-1586), SOIL CLASSIFICATION IS BASED ON THE AASHTO SYSTEM AND BASIC DESCRIPTIONS GENERALLY SHALL INCLUDE; CONSISTENCY, COLOR, TEXTURE, MOISTURE, AASHTO CLASSIFICATION, AND OTHER PERTINENT FACTORS SUCH AS MINERALOGICAL COMPOSITION, AND CHURCHLY, STEMPLES. VERY STAFF, GRAY SUTY CLA, WOST WITH WITEREEDEDE FIME SWID LIVERS, MOSILY PLOSTIC, A-7-6	MELL GRADED- INDICATES A GOOD REPRESENTATION OF PARTICLE SIZES FROM FINE TO UNIFORM- INDICATES THAT SOIL PARTICLES ARE ALL APPROXIMETLY THE SAME SIZE. (#POORLY GRADED- INDICATES A MIXTURE OF UNIFORM PARTICLES OF TWO OR MORE SIZES. ANGULARITY OF GRAINS THE ANGULARITY OR ROUNDRESS OF SOIL GRAINS ARE DESIGNATED BY THE TERMS; ANGUSULAR, SUBROUNDED, OR ROUNDED.	GULAR, WE	HARD ROCK IS NON-COASTAL PLAIN MATERIAL. ROCK LINE INDICATES THE LEVEL AT WHICH NO SPT REFUSAL IS PENETRATION BY A SPLIT SPI IN NON-COASTAL PLAIN MATERIAL, THE TRANS: OF WEATHERED ROCK. ROCK MATERIALS ARE TYPICALLY DIVIDED AS F EATHERED NON-COASTAL	THAT WHEN TESTED, WOULD YIELD SPT REFUSAL AN INFERRED DN-CDASTAL PLAIN MATERIAL WOULD YIELD SPT REFUSAL. DON SAMPLER EDUAL TO OR LESS THAN 0.1 FOOT PER 60 BLOWS, ITTON BETWEEN SOIL AND ROCK IS OFTEN REPRESENTED BY A ZONE	ALLUVIUM (ALLUV.) - SOILS WHICH HAVE BEEN TRANSPORTED BY WATER. ADUJFER - A WATER BEARING FORMATION OR STRATA. ARENACEOUS - APPLIED TO ROCKS THAT HAVE BEEN DERIVED FROM SAND OR THAT CONTAIN SAND. ARGILLACEOUS - APPLIED TO ALL ROCKS OR SUBSTANCES COMPOSED OF CLAY MINERALS, OR HAVING A NOTABLE PROPORTION OF CLAY IN THEIR COMPOSITION, AS SHALE, SLATE, ETC.
SOIL LEGEND AND AASHTO CLASSIFICATION GENERAL GRANULAR MATERIALS SILT-CLAY MATERIALS ORGANIC MATERIALS CLASS. (1957, PASSING *200) (1987, PASSING *200) ORGANIC MATERIALS	MINERALOGICAL COMPOSITION MINERAL NAMES SUCH AS QUARTZ, FELDSPAR, MICA, TALC, KAOLIN, ETC. ARE USED IN DESC MHENEVER THEY ARE CONSIDERED OF SIGNIFICANCE.	CR	RYSTALLINE FINE TO COA	NRSE GRAIN IGNEOUS AND METAMDRPHIC ROCK THAT D SPT REFUSAL IF TESTED, ROCK TYPE INCLUDES GRANITE, NRO, SCHIST, ETC.	ARTESIAN - GROUND WATER THAT IS UNDER SUFFICIENT PRESSURE TO RISE ABOVE THE LEVEL AT WHICH IS IS ENCOUNTERED, BUT WHICH DOES NOT NECESSARILY RISE TO OR ABOVE THE GROUND SURFACE.
CLASS. (35% PASSING *200) (85% PASSING *200) GROUP A-1 A-3 A-2 A-4 A-5 A-6 A-7 A-1, A-2 A-4, A-5 CLASS. A-1-6 A-1-6 A-2-4 A-2-5 A-2-6 A-2-7 A-3 A-6, A-7	COMPRESSIBILITY SLIGHTLY COMPRESSIBLE LIQUID LIMIT LESS THAN 3	1 3Ø RO	IN-CRYSTALLINE FINE TO COA ICK (NCR) SEDIMENTARY INCLUDES PH	RSE GRAIN METAMORPHIC AND NON-COASTAL PLAIN ROCK THAT WOULD YEILD SPT REFUSAL IF TESTED, ROCK TYPE YLLITE, SLATE, SANDSTONE, ETC.	CALCAREOUS (CALC.) - SOILS WHICH CONTAIN APPRECIABLE AMOUNTS OF CALCIUM CARBONATE. COLLUVIUM - ROCK FRAGMENTS MIXED WITH SOIL DEPOSITED BY GRAVITY ON SLOPE OR AT BOTTOM OF SLOPE.
SYMBOL 20080000000000000000000000000000000000	MODERATELY COMPRESSIBLE LIQUID LIMIT 31-50 HIGHLY COMPRESSIBLE LIQUID LIMIT GREATER TH. PERCENTAGE OF MATERIAL		DIMENTARY ROCK SPT REFUSAL	IN SEDIMENTS CEMENTED INTO ROCK, BUT MAY NOT YIELD . ROCK TYPE INCLUDES LIMESTONE, SANDSTONE, CEMENTED ETC.	CORE RECOVERY (REC.) - TOTAL LENGTH OF ALL MATERIAL RECOVERED IN THE CORE BARREL DIVIDED BY TOTAL LENGTH OF CORE RUN AND EXPRESSED AS A PERCENTAGE.
# 10 50 MX GRANULAR CLAY MUCK	ORGANIC MATERIAL GRANULAR SILT- CLAY	(EDIA)	<u> </u>	/EATHERING	DIKE - A TABULAR BODY OF IGNEOUS ROCK THAT CUTS ACROSS THE STRUCTURE OF ADJACENT ROCKS OR CUTS MASSIVE ROCK.
■ 200 15 MX 25 MX 10 MX 35 MX 35 MX 35 MX 35 MX 36 MN	TRACE OF ORGANIC MATTER 2 - 3% 3 - 5% TRACE 1 LITTLE ORGANIC MATTER 3 - 5% 5 - 12% LITTLE 10	1 - 10% 0 - 20%	HAMMER IF CRYSTALLINE.	JOINTS MAY SHOW SLIGHT STAINING, ROCK RINGS UNDER	$\overline{ ext{DIP}}$ - THE ANGLE AT WHICH A STRATUM OR ANY PLANAR FEATURE IS INCLINED FROM THE HORIZONTAL.
FRSTIC INDEX 6 MX N.P. 10 MX 1	HIGHLY ORGANIC >10% >20% HIGHLY 35%		SLI.) CRYSTALS ON A BROKEN SPECIMEN OF A CRYSTALLINE NATURE.	FAINED, SOME JOINTS MAY SHOW THIN CLAY COATINGS IF OPEN, FACE SHINE BRIGHTLY. ROCK RINGS UNDER HAMMER BLOWS IF	DIP DIRECTION (DIP AZIMUTH) - THE DIRECTION OR BEARING OF THE HORIZONTAL TRACE OF THE LINE OF DIP, MEASURED CLOCKWISE FROM NORTH.
USUAL TYPES STONE FRAGS. OF HAJOR GRAVEL AND SAND GRAVEL AND SAND SOULS SOULD SOULS SOULS SOULS SOULS SOULS SOULS SOULS SOULS	✓ WATER LEVEL IN BORE HOLE IMMEDIATELY AFTER DRILLING.		LI.) 1 INCH. OPEN JOINTS MAY CONTAIN	TAINED AND DISCOLORATION EXTENDS INTO ROCK UP TO CLAY. IN GRANITOID ROCKS SOME OCCASIONAL FELDSPAR RED. CRYSTALLINE ROCKS RING UNDER HAMMER BLOWS.	FAULT - A FRACTURE OR FRACTURE ZONE ALONS WHICH THERE HAS BEEN DISPLACEMENT OF THE SIDES RELATIVE TO ONE ANOTHER PARALLEL TO THE FRACTURE. FISSILE - A PROPERTY OF SPLITTING ALONG CLOSELY SPACED PARALLEL PLANES.
MATERIALS SAND STATE TO GOOD FAIR TO POOR MAGUITAGE	STATIC WATER LEVEL AFTER 24 HOURS. PERCHED WATER, SATURATED ZONE OR WATER BEARING STRATA		DDERATE SIGNIFICANT PORTIONS OF ROCK SHOOD.) GRANITOID ROCKS, MOST FELDSPARS	IOW DISCOLORATION AND WEATHERING EFFECTS. IN ARE DULL AND DISCOLORED, SOME SHOW CLAY, ROCK HAS	FLOAT - ROCK FRAGMENTS ON SURFACE NEAR THEIR ORIGINAL POSITION AND DISLODGED FROM PARENT MATERIAL.
SUBGRADE P.I. OF A-7-5 ≤ L.L 30 : P.I. OF A-7-6 > L.L 30	SPRING OR SEEPAGE	MDI	WITH FRESH ROCK.	AND SHOWS SIGNIFICANT LOSS OF STRENGTH AS COMPARED RED OR STAINED. IN GRANITOID ROCKS, ALL FELDSPARS DULL	FLOOD PLAIN (F.P.) - LAND BORDERING A STREAM, BUILT OF SEDIMENTS DEPOSITED BY THE STREAM.
CONSISTENCY OR DENSENESS RANGE OF STRINDARD RANGE OF UNCONFINED PRIMARY SOIL TYPE COMPACTNESS OR PRETENTION RESISTENCE COMPRESSIVE STRENGTH	MISCELLANEOUS SYMBOLS III ROADWAY EMBANKMENT PRI PET PET PET PONNE CONTROLLED PONNE CONTRO	SEY (MC	VERE AND DISCOLORED AND A MAJORITY DD. SEV.) AND CAN BE EXCAVATED WITH A GE	SHOW KAOLINIZATION. ROCK SHOWS SEVERE LOSS OF STRENGTH OLOGIST'S PICK. ROCK GIVES "CLUNK" SDUND WHEN STRUCK.	FORMATION (FM.) - A MAPPABLE GEOLOGIC UNIT THAT CAN BE RECOGNIZED AND TRACED IN THE FIELD.
CUNSISTENCY (N-VALUE) (TONS/F12)	SI BURING SI	SAMPLE SIGNATIONS SE	VERE ALL ROCKS EXCEPT QUARTZ DISCOL	<u>sal</u> .Ored or stained, rock fabric clear and evident but reduce!	JOINT - FRACTURE IN ROCK ALONG WHICH NO APPRECIABLE MOVEMENT HAS OCCURRED.
GENERALLY	SOIL SYMBOL AUGER BORING S- BL			GRANITOID ROCKS ALL FELDSPARS ARE KAOLINIZED TO SOME	LEDGE - A SHELF-LIKE RIDGE OR PROJECTION OF ROCK WHOSE THICKNESS IS SMALL COMPARED TO ITS LATERAL EXTENT.
MATERIAL MEDIUM DENSE 10 TO 30 N/H (NON-COHESIVE) DENSE 30 TO 50 VERY DENSE >50 VERY SOFT <2 (0.25	ROADWAY EMBANKMENTS ST- SH ST- SH MONITORING WELL ST- SH MONITORING WELL	SHELBY TUBE (V. SAMPLE	IF TESTED, YIELDS SPT N VALUES RY SEVERE ALL ROCK EXCEPT QUARTZ DISCOLO SEV.) THE MASS IS EFFECTIVELY REDUCE REMAINING, SAPROLITE IS AN EXAM		LENS - A BODY OF SOIL OR ROCK THAT THINS OUT IN ONE OR MORE DIRECTIONS. MOTTLED (MOT.) - IRREGULARLY MARKED WITH SPOTS OF DIFFERENT COLORS. MOTTLING IN SOILS UBUALLY INDICATES POOR AERATION AND LACK OF GOOD DRAINAGE. PERCHED WATER MAINTAINED ABOVE THE NORMAL GROUND WATER LEVEL BY THE PRESENCE OF AN INTERVENING IMPERVIOUS STRATUM.
SILT-CLAY MEDIUM STIFF 4 TO 8 0.5 TO 1 MATERIAL STIFF 8 TO 15 1 TO 2	ALLUVIAL SOIL BOUNDARY PIEZOMETER INSTALLATION RT- RI	RECOMPACTED	MPLETE ROCK REDUCED TO SOIL. ROCK FABR	RIC NOT DISCERNIBLE, OR DISCERNIBLE ONLY IN SMALL AND IZ MAY BE PRESENT AS DIKES OR STRINGERS, SAPROLITE IS	RESIDUAL SOIL - SOIL FORMED IN PLACE BY THE WEATHERING OF ROCK.
(COHESIVE) VERY STIFF 15 TO 30 2 TO 4 >4 >4	25/025 DIP/DIP DIRECTION OF INSTALLATION CBR -	TRIAXIAL SAMPLE - CBR SAMPLE	ALSO AN EXAMPLE.	CK HARDNESS	ROCK QUALITY DESIGNATION (R.D.D.) - A MEASURE OF ROCK QUALITY DESCRIBED BY: TOTAL LENGTH OF ROCK SEGMENTS EQUAL TO OR GREATER THAN 4 INCHES DIVIDED BY THE TOTAL LENGTH OF CORE RUN AND EXPRESSED AS A PERCENTAGE.
U.S. STD. SIEVE SIZE 4 10 40 60 200 270	→ SPT N-VALUE → - SDUNDING ROD (REF)— SPT REFUSAL	vi	ERY HARD CANNOT BE SCRATCHED BY KNIFE SEVERAL HARD BLOWS OF THE GET	OR SHARP PICK. BREAKING OF HAND SPECIMENS REDUIRES	SAPROLITE (SAP.) - RESIDUAL SOIL WHICH RETAINS THE RELIC STRUCTURE OR FABRIC OF THE PARENT ROCK.
DPENING (MM)	ABBREVIATIONS			PICK ONLY WITH DIFFICULTY, HARD HAMMER BLOWS REQUIRED	SILL - AN INTRUSIVE BODY OF IGNEOUS ROCK OF APPROXIMATELY UNIFORM THICKNESS AND RELATIVELY THIN COMPARED WITH ITS LATERAL EXTENT, WHICH HAS BEEN EMPLACED PARALLEL TO THE BEDDING OR SCHISTOSITY OF THE INTRUDED ROCKS
GRAIN MM 305 75 2,0 0.25 0.05 0.005	AR - AUGER REFUSAL PMT - PRESSUREMETER TE BT - BORING TERMINATED SD SAND, SANDY CL CLAY SL SILT, SILTY	i m		PICK. GOUGES OR GROOVES TO 0.25 INCHES DEEP CAN BE GEOLOGISTS PICK. HAND SPECIMENS CAN BE DETACHED	SLICKENSIDE - POLISHED AND STRIATED SURFACE THAT RESULTS FROM FRICTION ALONG A FAULT OR SLIP PLANE.
SIZE IN. 12* 3* SOIL MOISTURE - CORRELATION OF TERMS	CPT - CONE PENETRATION TEST SLI SLIGHTLY CSE COARSE TCR - TRICONE REFUSAL DMT - DILATOMETER TEST DPT - DYNAMIC PENETRATION TEST - UNIT WEIGHT		MEDIUM CAN BE GROOVED OR GOUGED 0.05 HARD CAN BE EXCAVATED IN SMALL CHI	INCHES DEEP BY FIRM PRESSURE OF KNIFE OR PICK POINT. PS TO PEICES 1 INCH MAXIMUM SIZE BY HARD BLOWS OF THE	STANDARD PENETRATION TEST (PENETRATION RESISTANCE) (SPT) - NUMBER OF BLOWS (N OR B.P.F.) OF A 140 LB. HAMMER FALLING 30 INCHES REQUIRED TO PRODUCE A PENETRATION OF J FOOT INTO SOIL WITH A 2 INCH OUTSIDE DIAMETER SPLIT SPOON SAMPLER, SPT REFUSAL IS LESS THAN 0,1 FOOT PENETRATION
SOIL MOISTURE SCALE FIELD MOISTURE GUIDE FOR FIELD MOISTURE DESCRIPTION GUIDE FOR FIELD MOISTURE DESCRIPTION	DPT - DYNAMIC PENETRATION TEST	s		ILY BY KNIFE OR PICK. CAN BE EXCAVATED IN FRAGMENTS IN SIZE BY MODERATE BLOWS OF A PICK POINT, SMALL, THIN	HIZ THEN BUSINES ELEMENTER SPLIT SPUBNISHMELER, SPT REPUSHL IS LESS THAN WITHOUT PENETHALION WITH 60 BLOWS. STRATA CORE RECOVERY (SREC.) - TOTAL LENGTH OF STRATA MATERIAL RECOVERED DIVIDED BY TOTAL LENGTH
- SATURATED - USUALLY LIQUID; VERY WET, USUALLY (SAT.) FROM BELOW THE GROUND WATER TABLE LL LIQUID LIMIT	FOSS FOSSILIFEROUS V VERY	· · ·	PIECES CAN BE BROKEN BY FINGE VERY CAN BE CARVED WITH KNIFE. CAN		OF STRATUM AND EXPRESSED AS A PERCENTAGE. STRATA ROCK QUALITY DESIGNATION (S.R.O.D.) - A MEASURE OF ROCK QUALITY DESCRIBED BY: TOTAL LENGTH OF ROCK SEGMENTS WITHIN A STRATUM EDUAL TO OR GREATER THAN 10 CENTIMETERS DIVIDED
PLASTIC SEMISOLID; REQUIRES DRYING TO	MED MEDIUM		FINGERNAIL.		BY THE TOTAL LENGTH OF STRATA AND EXPRESSED AS A PERCENTAGE. TOPSOIL (T.S.) - SURFACE SOILS USUALLY CONTAINING ORGANIC MATTER.
(PI) PLASTIC LIMIT ATTAIN OPTIMUM MOISTURE	EQUIPMENT USED ON SUBJECT PROJECT	TVDE	FRACTURE SPACING IERM SPACING	BEDDING IERM IHICKNESS	BENCH MARK: TBM: SET IN SE CORNER OF BRIDGE (-L- 14+10, RT.)
OM OPTIMUM MOISTURE - MOIST - (M) SOLID; AT OR NEAR OPTIMUM MOISTURE	AUTO	TOMATIC MANUAL	VERY WIDE MORE THAN 10 FEET WIDE 3 TO 10 FEET	VERY THICKLY BEDDED > 4 FEET THICKLY BEDDED 1.5 - 4 FEET	
SL SHRINKAGE LIMIT	6* CONTINUOUS FLIGHT AUGER CORE SIZE	ZE:	MODERATELY CLOSE 1 TO 3 FEET CLOSE 0.16 TO 1 FEET	THINLY BEDDED 0.16 - 1.5 FEET VERY THINLY BEDDED 0.03 - 0.16 FEET	ELEVATION: I569.65 (EST.)
- DRY - (D) ATTAIN OPTIMUM MOISTURE	BK-51		VERY CLOSE LESS THAN 0.16 FEET	THINLY LAMINATED < 0.008 FEET	NOTES:
PLASTICITY	CME-45C HARD FACED FINGER BITS -N_			NDURATION	-
PLASTICITY INDEX (PI) DRY STRENGTH NONPLASTIC 0-5 VERY LOW	TUNG -CAPRIDE INSERTS	j r ur	DURR	DENING OF THE MATERIAL BY CEMENTING, HEAT, PRESSURE, ETC.	
LOW PLASTICITY 6-15 SLIGHT	CASING W/ ADVANCER HAND TO			ING WITH FINGER FREES NUMERDUS GRAINS: LE BLOW BY HAMMER DISINTEGRATES SAMPLE.	
MED. PLASTICITY 16-25 MEDIUM HIGH PLASTICITY 26 OR MORE HIGH	PORTABLE HOIST TRICONE STEEL TEETH POS	OST HOLE DIGGER		NS CAN BE SEPARATED FROM SAMPLE WITH STEEL PROBE: KS EASILY WHEN HIT WITH HAMMER.	
COLOR		OUNDING ROD		NS ARE DIFFICULT TO SEPARATE WITH STEEL PROBE:	
DESCRIPTIONS MAY INCLUDE COLOR OR COLOR COMBINATIONS (TAN, RED, YEL-BRN, BLUE-GRAY) MODIFIERS SUCH AS LIGHT, DARK, STREAKED, ETC. ARE USED TO DESCRIBE APPEARANCE.	OTHER OTHER VAN	ANE SHEAR TEST	EXTREMELY INDURATED SHAR	ICULT TO BREAK WITH HAMMER. IP HAMMER BLOWS REQUIRED TO BREAK SAMPLE; I'LE BREAKS ACROSS GRAINS.	
Landard Control of the Control of th			SAMI	LE DREHNO HURUOS UNHINO.	

STATE PROJECT NO. SHEET NO. TOTAL SHEETS
33271.1.1 2 1/

ID B-3818



STATE OF NORTH CAROLINA DEPARTMENT OF TRANSPORTATION

MICHAEL F. EASLEY

P.O. BOX 25201, RALEIGH, N.C. 27611-5201 LYNDO TIPPETT

GOVERNOR

SECRETARY

November 4, 2005

STATE PROJECT: 33271.1.1 (B-3818)

COUNTY:

Caldwell

DESCRIPTION:

Bridge #3 on NC 90 over Lost Cove Creek

SUBJECT:

Geotechnical Report - Bridge Foundation Investigation

This is a proposed bridge replacement for bridge number 9 on NC 90 over Lost Cove Creek. The new structure location will be upstream from the existing structure. Proposed structure is a 120' single span design with steel plate girders. Recommended roadway width is 24' on a 90-degree skew angle.

Three foundation test borings were performed utilizing a CME-550 drill machine, hollow stem augers, and an automatic drop hammer. The field investigation for this project was conducted in September of 2005.

Physiography/Geology

The project is located in the NW corner of Caldwell County in proximity to the town of Edgemont. Lost Cove Creek is a high quality water trout stream. The site area is gently sloping and heavily wooded. Geologically this area is underlain by Middle Proterozic age Biotite Granite Gneiss associated with the Grandfather Mountain Window.

Site specific soils noted during our investigation include existing roadway fill associated with NC 90 and alluvial loose to dense silty coarse sand with gravel (A-1-a, A-1-b). All boring locations (3 total) achieved auger refusal on hard rock at the base of alluvium.

Foundation Materials

End Bent 1:

This bent is located south of Lost Cove Creek. One boring was performed at the proposed centerline for this bent location and encountered 11.5 feet of brown alluvial loose to medium dense silty coarse sand and gravel (A-1-a, A-1-b) overlying hard rock. Elevation for the hard rock contact is 1550.05'.

2

End Bent 2:

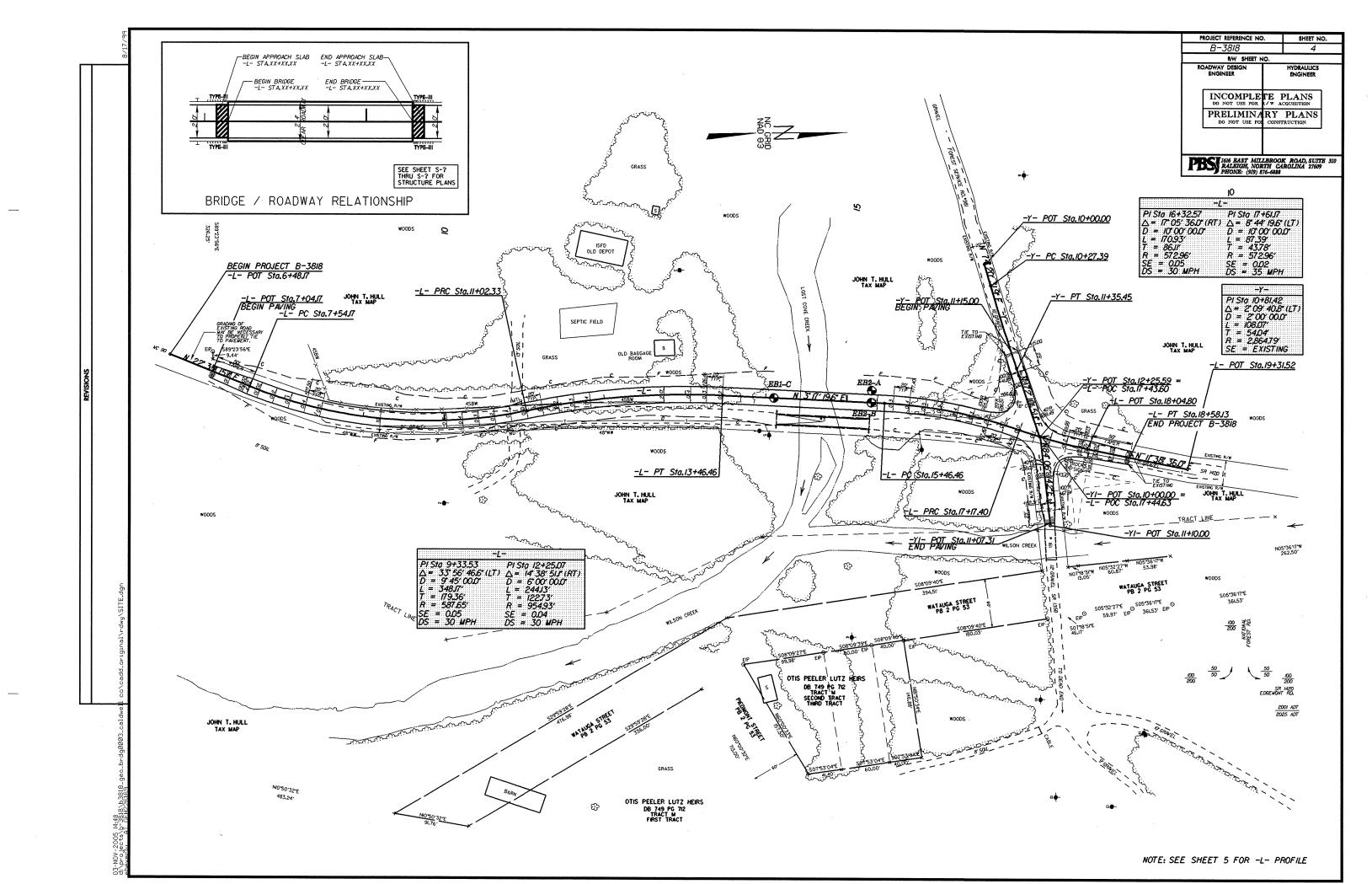
This bent is located north of Lost Cove Creek. Two borings were performed for this bent location and encountered 6 to 6.9 feet of brown alluvial loose to dense silty coarse sand and gravel (A-1-a, A-1-b) overlying hard rock. Elevation for the hard rock contact is 1553.24' at boring EB2-A and 1553.11' for boring EB2-C.

Groundwater

Groundwater measurements taken more than 24 hours after each boring was performed indicate the static groundwater table lies at approximate elevation 1557.2 feet.

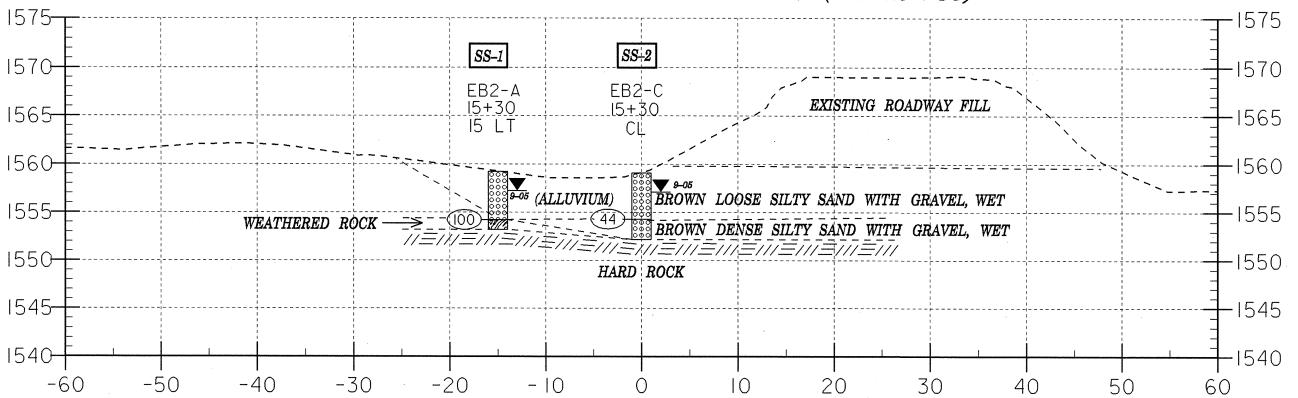
Respectfully submitted,

J.E. Beverly, Project Geologist

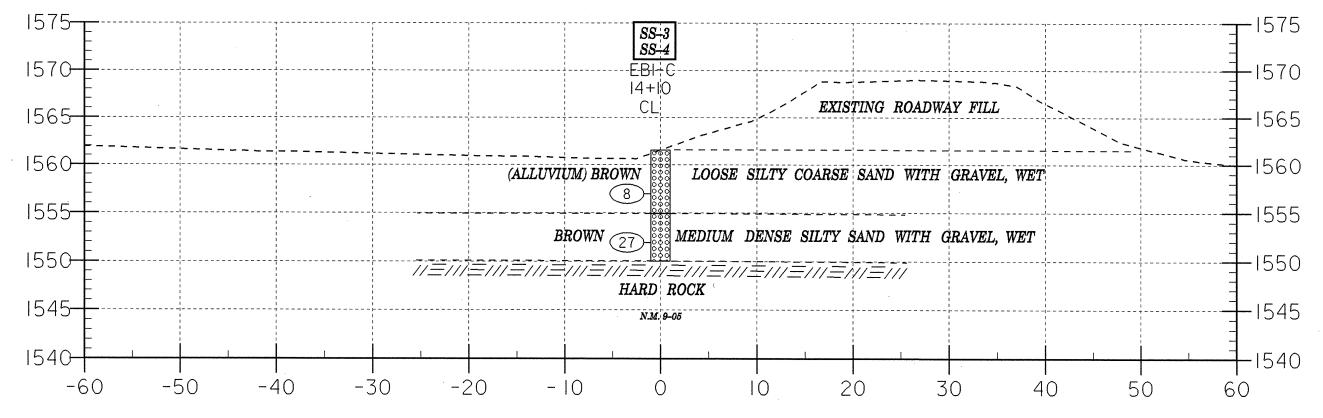


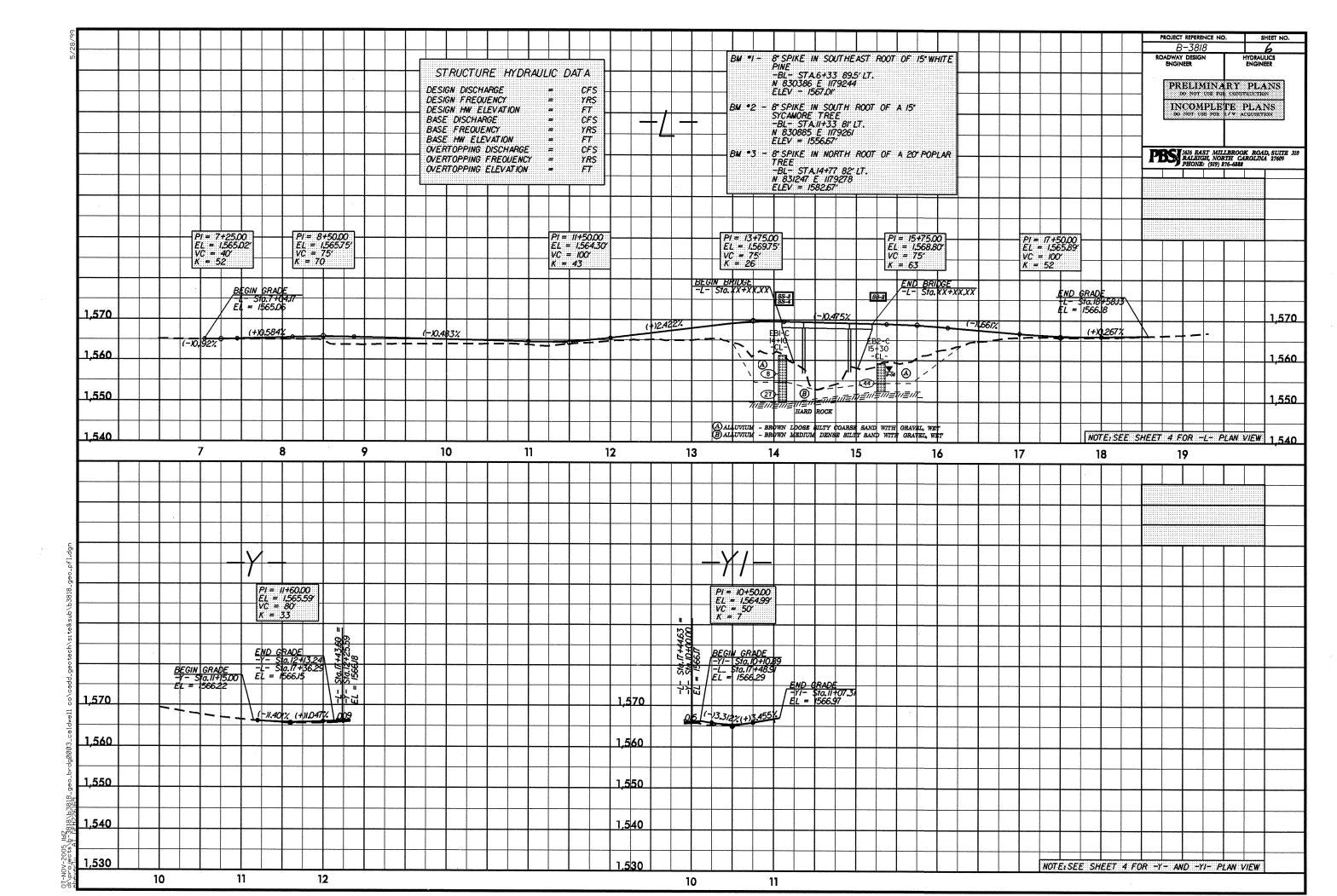


SECTION THRU EB2-A AND EB2-C (-L-15+30)



SECTION THROUGH EB1-C (-L- STA. 14+10)





NORTH CAROLINA DEPARTMENT OF TRANSPORTATION GEOTECHNICAL UNIT BORING LOG

						GEOT	ECHN	IICAL I	UNIT B	ORING	LOG			
PROJECT	Γ NO 332	71.1.1			ID B-3				LDWELL		7		GIST R W TODD	
SITE DES	CRIPTIO	N BR	IDGE				ER LOST COVE CREEK							GND WATER
BORING						HING 0.00	0.00 EAST			EASTING	0.00			0 HR N/A
ALIGNMI	ENT L					NG LOCAT		+10.000		OFFSET				24 HR N/A
COLLAR	ELEV 15	61.55	ft			L DEPTH			START DA	ATE 9/01/0			COMPLETION DA	
DRILL M.	ACHINE	CME-	550				T	DRILL METHOD H.S. AUGERS					HAMMER TYPE	
SURFACE	E WATER	DEPT	Ή				1		CK 11.50f	~~~			Log EB1-C, Page 1 of 1	
ELEV	DEPTH	В	LOW	СТ	PEN	Е	BLOWS F			SAMPLE	Y/	1		D ROCK
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NORTH CAROLINA DEPARTMENT OF TRANSPORTATION GEOTECHNICAL UNIT BORING LOG

***************************************						GEOT	ECHN	IICAL (JNIT B	ORING	LOG				
	ΓNO 3327				ID B-3	3818	COU	NTY CAI	DWELL				GIST R W TODD		
	SCRIPTION		DGE	3 ON	NC 90	OVER LO								GND WATER	
	NO EB2-A	•		1	NORT	HING 0.00	0			EASTING	0.00			0 HR N/A	
ALIGNMI					BORIN	NG LOCAT	TION 15-	+30.000		OFFSET	15.00ff	LT		24 HR 2.00ft	
	ELEV 15				<u> FOTAJ</u>	L DEPTH	6.00ft		START DA	ATE 9/01/0)5		COMPLETION DA	ATE 09/01/05	
	ACHINE (DRILL	METHO	D H.S. AL	JGERS			HAMMER TYPE	AUTOMATIC	
SURFACE	E WATER					T		TO ROC					Log EB2-A, Page 1 of 1		
ELEV	DEPTH	1 .	LOW		PEN	L		PER FOC		SAMPLE	MO	[5]		ID ROCK	
	<u> </u>	bin	6in	6in	(ft)	0 2	25 5	50	75 10	NO NO	MO	Ğ	DESCR	RIPTION	
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1559.24			 	+	+-		Ground	Surface		SS-1	10/	0000			
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1553.24				 '	10.5				<u> </u>	K		9000 35#	(WEATHER		
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Sheet 8

NORTH CAROLINA DEPARTMENT OF TRANSPORTATION GEOTECHNICAL UNIT BORING LOG

			140	JKI	ПС				UNIT B					TATION	
PROJECT	NO 3327	1.1.1			ID B-3				LDWELL	_		T		IST R W TODD	
SITE DES	CRIPTION	N BRI	DGE	3 ON	NC 90	OVER L						4			GND WATER
BORING I	NO EB2-C	<u> </u>]	NORT	HING 0.0	00			E	ASTING	0.00			0 HR N/A
ALIGNMI	ENT L			1	BORIN	G LOCA	TION 15	+30.000		0	FFSET (0.00ft			24 HR 2.00ft
COLLAR	ELEV 15	59.111	t		TOTA	L DEPTH	6.90ft		START DA	ΑT	E 9/01/0	5		COMPLETION DA	ATE 09/01/05
DRILL M	ACHINE (CME-	550				DRILL	метно	D H.S. AL	JG	ERS			HAMMER TYPE	AUTOMATIC
SURFACE	WATER				T				CK 6.90ft			,	*	Log EB2-C, Page 1 of 1	
ELEV	DEPTH	1	OW (PEN		BLOWS F				SAMPLE	Y /	G		ID ROCK
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1552.21							X			Ш	SS-2	W	0000	BROWN DENS	
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NORTH CAROLINA DEPARTMENT OF TRANSPORTATION **DIVISION OF HIGHWAY** MATERIALS & TESTS UNIT

SOILS LABORATORY

T. I. P. No.

B-3818

	REPORT ON SAM	PLES OF	SOILS FOR QUALITY		
Project	8.1731601	County	CALDWELL	Owner	
Date: Sampled	9/5/05	Received	9/8/05	Reported	9/12/2005
Sampled from	BRIDGE		Ву	J P ROGE	RS
Submitted by	N WAINAINA			1995	Standard Specifications

725720 TO 725724 11/3/05

TEST RESULTS

Proj. Sample No.	SS-1	SS-2	SS-3	SS-4	SS-5	
Lab. Sample No.	725720	725721	725722	725723	725724	
Retained #4 Sieve %	50	25	27	43		
Passing #10 Sieve %	40	60	59	42	100	
Passing #40 Sieve %	27	35	31	24	40	
Passing #200 Sieve %	12	. 15	.I. X	10	9	

MINUS NO. 10 FRACTION

SOIL MORTAR - 100%							
Coarse Sand Ret - #60	%	43.2	55.8	60.0	54.5	79.2	
Fine Sand Ret - #270	%	31.7	27.9	24.7	26.7	13.3	
Silt 0.05 - 0.005 mm	%	21.0	12.3	12.2	16.8	6.5	
Clay < 0.005 mm	%	4.0	4.0	3.0	2.0	1.0	
Passing #40 Sieve	%	-	-	-	<u>-</u>	-	
Passing #200 Sieve	%	~				-	

L. L.	24	26	23	23	26	
P. I.	NP	NP	NP	NP	NP	
AASHTO Classification	A-1-a(0)	A-1-b(0)	A-1-l ₁ (0)	A-1-a(0)	A-1-b(0)	
Station						
	·					
LOCATION	EB2-A	EB2-B	EB1-C	EB1-C	Crk Bank	
Depth (Ft)	0.00	4.80	4.60	9.60		
	to 5.00	6.30	6.10	11.10		

cc: JPROGERS Soils File

Soils Engineer

GEOTECHNICAL UNIT FIELD SCOUR REPORT

PROJECT: 33271.1.1 TIP NO.: B-3818 COUNTY: CALDWELL
DESCRIPTION(1): BRIDGE # 3ON NC 90 OVER LOST COVE CREEK
♦ INFORMATION ON EXISTING BRIDGES Information obtained from ☐ Field Inspection ☐ Microfilm (Reel: Position:) Other
COUNTY BRIDGE NO. 3 BRIDGE LENGTH 115' NO. BENTS 3 NO. BENTS IN: CHANNEL 1 FLOODPLAIN 4
FOUNDATION TYPE: REINFORCED CONCRETE ARCH BRIDGE ON FOOTINGS
EVIDENCE OF SCOUR(2):
ABUTMENTS OR END BENT SLOPES: NONE
INTERIOR BENTS: MODERATE SCOUR AT BOTH INTERIOR FOOTINGS IN CHANNEL
CHANNEL BED: NONE
CHANNEL BANKS: NONE
• EXISTING SCOUR PROTECTION:
TYPE(3): SAND BAGS
EXTENT(4): BOTH END BENT SLOPES COVERED WITH SAND BAGS
EFFECTIVENESS(5): GOOD
OBSTRUCTIONS(6) (DAMS, DEBRIS, ETC.): NONE NOTED
• DESIGN INFORMATION
CHANNEL BED MATERIAL(7) (Sample Results Attached): ROCK AND COBBLES
CHANNEL BANK MATERIAL(8) (Sample Results Attached): SAND - (SS-5 = A-1-b)
CHANNEL BANK COVER(10): TREES WITH SOME LEANING TOWARD CREEK
FLOOD PLAIN WIDTH(11): APPROXIMATELY 500'
FLOOD PLAIN COVER(12): WOODS
STREAM IS: DEGRADING AGGRADING (13)
OTHER OBSERVATIONS AND COMMENTS: WATER AND SITE ARE BOTH VERY CLEAN

SHEET /O OF //

DESIGN INFORMATION CONT.

CHANNEL MIGRATION TENDENCY(14): SLIGHT - POSSIBLY TO SOUTHEAST

GEOTECHNICAL ADJUSTED SCOUR ELEVATIONS (15): SINCE PROPOSED BRIDGE DESIGN IS A SINGLE SPAN (WITH NO INTERIOR

BENTS) THEN SCOUR SHOULD NOT BE AN ISSUE.

ACCORDING TO THE HYDRO REPORT THE 500 YEAR CONTRACTION SCOUR ELEVATION IS 1549' IN THE CREEK CHANNEL.

REPORTED BY: R.W. TODD / JEB DATE: 11-4-05

INSTRUCTIONS

- (1) GIVE THE DESCRIPTION OF THE SPECIFIC SITE GIVING ROUTE NUMBER AND BODY OF WATER CROSSED.
- (2) NOTE ANY EVIDENCE OF SCOUR AT THE EXISTING END BENTS OR ABUTMENTS (UNDERMINING, SLOUGHING, SCOUR LOCATIONS DEGRADATIONS, ETC.)
- (3) NOTE ANY EXISTING SCOUR PROTECTION (RIPRAP, ETC.)
- (4) DESCRIBE THE EXTENT OF ANY EXISTING SCOUR PROTECTION.
- (5) DESCRIBE WHETHER OR NOT THE SCOUR PROTECTION APPEARS TO BE WORKING.
- (6) NOTE ANY DAMS, FALLEN TREES, DEBRIS AT BENTS, ETC.
- (7) DESCRIBE THE CHANNEL BED MATERIAL; A SAMPLE SHOULD BE TAKEN FOR GRAIN SIZE DISTRIBUTION, ATTACH LAB RESULTS.
- (8) DESCRIBE THE CHANNEL BANK MATERIAL; A SAMPLE SHOULD BE TAKEN FOR GRAIN SIZE DISTRIBUTION, ATTACH LAB RESULTS.
- (9) DESCRIBE THE FOUNDATION BEARING MATERIAL
- (10) DESCRIBE THE BANK COVERING (GRASS, TREES, RIPRAP, NONE, ETC.)
- (11) GIVE THE APPROXIMATE FLOOD PLAIN WIDTH (ESTIMATE).
- (12) DESCRIBE THE FLOOD PLAIN COVERING (GRASS, TREES, CROPS, ETC.)
- (13) CHECK THE APPROPRIATE SPACE AS TO WHETHER THE STREAM IS DEGRADING OR AGGRADING.
- (14) DESCRIBE THE POTENTIAL OF THE BODY OF WATER TO MIGRATE LATERALLY DURING THE LIFE OF THE BRIDGE (APPROXIMATELY 100 YEARS)
- (15) GIVE THE GEOTECHNICAL ADJUSTED SCOUR ELEVATION EXPECTED OVER THE LIFE OF THE BRIDGE (APPROXIMATELY 100 YEARS). THIS CAN BE GIVEN AS AN ELEVATION RANGE ACROSS THE SITE, OR ON A BENT BY BENT BASIS WHERE VARIATIONS EXIST. DISCUSS RELATIONSHIP BETWEEN THE HYDRAULICS THEORETICAL SCOUR AND THE GEOTECHNICAL ADJUSTED SCOUR ELEVATION. IF THE GEOTECHNICAL ADJUSTED SCOUR ELEVATION IS DEPENDENT ON SCOUR COUNTER MEASURES, EXPLAIN. (RIPRAP ARMORING ON SLOPES, ETC.) THE GEOTECHNICAL ADJUSTED SCOUR ELEVATION IS BASED ON THE ERODABILITY OF MATERIALS WITH CONSIDERATION FOR JOINTING, FOLIATION, BEDDING ORIENTATION AND FREQUENCY; CORE RECOVERY PERCENTAGE; PERCENT RQD; DIFFERENTIAL WEATHERING; SHEAR STRENGTH; OBSERVATIONS AT EXISTING STRUCTURES; OTHER TESTS DEEMED APPROPRIATE; AND OVERALL GEOLOGIC CONDITIONS AT THE SITE.

rev. 9-03

33271.1.1 (B-3818) CALDWELL COUNTY BRIDGE # 3 ON NC 90 OVER LOST COVE CREEK

SITE PHOTOS



Looking Sorth along NC 90 (Creek flow right to left)



North
Looking South along NC 90 (Creek flow left to right)