1111413	12+60 to 13+40 6 13+40 to 19+00 6-7 19+00 to 19+60 7-8 19+60 to 20+80 8 20+80 to 22+00 8,9 22+00 to 25+00 9-11 32+00 to 32+40 11 32+00 to 32+40 11 32+00 to 10+51 14 -YI- 10+00 to 10+51 14 -YI- 10+00 to 11+66 14 -Y2- 10+20 to 18+40 15-18 18+40 to 22+40 8,18 22+40 to 27+60 8,14,1 27+60 to 29+00 14,20 29+00 to 30+74 20 -Y3- 10+00 to 11+89 6 Y3B- 10+00 to 11+89 6 Y3B- 10+00 to 11+80 17 11+80 to 13+68 17 -Y8- 10+00 to 10+51 5				
-)	$ \begin{array}{c ccccccccccccccccccccccccccccccccccc$				
		12+60	to.	13 + 40	6
		13 + 40	to.	19 + 00	6-7
		19+00	to.	19 + 60	7-8
		19+60	to 2	20 + 80	8
		20+80	to	22 + 00	8
		22+00	to	25 + 00	8,9
		25 + 00	to	32 + 00	
		32 + 00	to	32 + 40	11
-LTR	ANS-	7+40	to	11 + 00	4,5
-D	R1-	10+00	to .	10 + <i>5</i> 1	
-7	71-	10+00	to.	12 + 80	12
		12+80	to .	19 + 07	12-14
-Y	1.A.–	10+00	to .	<i>11+66</i>	14
-Y	72-	10+20	to.	18 + 40	15–18
		18+40	to .	22 + 40	8,18
		22 + 40	to	27 + 60	8,14,1
- Y	73–				
-Y	3 <i>A</i>	11+00	to 2	11+89	
-Y	3 <i>B</i> -	10+00	to.	12 + 64	18
-Y	74-				
-Y	75-	10+00	to.	10 + 20	17
		10+20	to.	11 + 80	17
		11+80	to 2	13 + 68	17
-Y	78-	10+00	to.	10 + 51	5
	12+60 to 13+40				

STATION

SHEET NUMBERS

PROFILE X-SECTS.

28-30

31–32

33–35

21

21

21

21

21,22

22 23

25 24

24

25 26

27

27

23 23 25

25

26,27 | 59-65

27 66-69

PLAN

CONTENTS:

LINE

.2809A

STATE OF NORTH CAROLINA DEPARTMENT OF TRANSPORTATION **DIVISION OF HIGHWAYS** GEOTECHNICAL ENGINEERING UNIT

ROADWAY SUBSURFACE INVESTIGATION

__ F.A. PROJ. _*STP-98(1)* __ | **.**D. **R-2809A** STATE PROJ. **34503.1.1** COUNTY WAKE PROJECT DESCRIPTION NC 98 (WAKE FOREST BYPASS) FROM WEST OR SR 1923 (THOMPSON MILL ROAD)

INVENTORY

TO WEST OF US 1 (CAPITAL BLVD.)



ALL DIMENSIONS IN HESE PLANS ARE IN METERS OR MILLIMETERS JNLESS OTHERWISE SHOWN

STATE	STATE PROJECT REFERENCE NO.			SHEET NO.	TOTAL SHBBTS
N.C.	R-	-2809A		1	73
8TAT	E PROJ.NO.	F.A. PROL NO.	T	DESCRIP	TION
345	503.1.1	STP-98(1)	T	PE	
345	03.2.5	STP-98(2)		R/W &	UTIL
34503.3.7		STP-98(23)	98(23)		
	1		İ		

CAUTION NOTICE

THE SUBSURFACE INFORMATION AND THE SUBSURFACE INVESTIGATION ON WHICH IT IS BASED WAS MADE FOR THE PURPOSE OF STUDY, PLANNING AND DESIGN, AND NOT FOR CONSTRUCTION OR PAY PURPOSES. THE VARBURS FEELD BORNIC LOSS, ROCK CORES, AND SOL TEST DATA AVAILABLE MAY BE REVIEWED OR INSPECTED IN RALEIGH BY CONTACTING THE N. C. DEPARTMENT OF TRANSPORTATION, GOTTENDAGLAUTH OF GRADE ADOBE, NETTER THE SUBSURFACE PLANS AND REPORTS, NOR THE FIELD BORRIG LOGS, ROCK CORES, OR SOIL TEST DATA IS PART OF THE CONTRACT.

CENERAL SOIL AND ROCK STRATA DESCRIPTIONS AND INDICATED BOUNDARIES ARE BASED ON A GEOTECHNICAL INTERPRETATION OF ALL AVAILABLE SUBSURFACE DATA AND MAY NOT RECESSARLY REFLECT THE ACTUAL SUBSURFACE CONDITIONS BETWEEN BORNESS OR BETWEEN SAMPLED STRATA WITHIN THE BORNICLE, THE LABORATORY SAMPLE DATA AND THE NISTU INFRACED ISST DATA CAN BE RELED ON ONLY TO THE DEGREE OF RELIABILITY INHERENT IN THE STANDARD TEST METHOD. THE OBSERVED WATER LEVELS OR SOLW, MOSTLEME CONDITIONS MOCLATED IN THE SUBSURFACE INVESTIGATIONS ARE AS RECORDED AT THE TIME OF THE NIVESTIGATION THESE WATER LEVELS OR SOIL MOISTURE CONDITIONS AND VARY CONSISPERAL WITH TIME ACCORDING TO CLIMATIC CONDITIONS NICLUDING TEMPERATURES, PRECIPITATION AND WIND, AS WELL AS OTHER NON-CLIMATIC FACTORS,

THE BIDDER OR CONTRACTOR IS CAUTIONED THAT DETAILS SHOWN ON THE SUBSURFACE PLANS
ARE PRELIMINARY ONLY AND IN MAINY CASES THE FRAIL DESIGN DETAILS ARE DFFERENT, FOR BODING
MIS CONSTRUCTION PURPOSES, REFER TO THE CONSTRUCTION PLANS AND DOCUMENTS FOR FRAIL DESIGN
NFORMATION ON THIS PROJECT. THE DEPARTMENT DOES NOT WARRANT OR GUARANTEE THE SUFFICIENCY
OR ACCURACY OF THE WVESTIGATION MADE, NOT PIE INTERPRETATIONS MADE OR OPHONIO OF THE
PERAFITMENT AS TO THE TYPE OF MATERIALS AND CONSTITIONS TO BE ENCOUNTERED. THE BIDDER OR
OUTTAGATOR IS CAUTIONED TO MAKE SUCH INDEPONENT SUBSURPERS INVESTIGATIONS AS SE DEEMS
MECESSARY TO SAITSFY HANSLEF AS TO CONDITIONS TO BE ENCOUNTERED ON THIS PROJECT. THE

25 70-73 25 WAKE UNION CHURCH RD PLAN SHEET TIGATION. HAYEBURY CT. -Y8- SR 1999 NA PA TO DURHAM -L- STA. 7+40.000 BEGIN TIP PROJECT R-2809A BEGIN CONSTRUCTION PPLENOOK EXISTING RELOCATED FALLS OF NEUSE RD. FALLS OF NEUSE RD GARDEN HILL DR. TO RALEIGH MOUNT AIN HIGH ROAD -L- STA. 32 +40.000 END TIP PROJECT R-2809A DENOTES EXISTING TRAFFIC SIGNAL -L- STA. 35+00.000 * DENOTES PROPOSED TRAFFIC SIGNAL END CONSTRUCTION

PERSONNEL I. L. PEDRO

F. COX

E. RIVERA H. R. CONLEY

M. L. REEDER

INVESTIGATED BY J. L. PEDRO

CHECKED BY N.T. ROBERSON

SUBMITTED BY N. T. ROBERSON

DATE FEBRUARY 2006

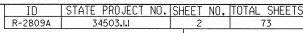
THE INFORMATION CONTAINED HEREIN IS NOT IMPLIED OR GUARANTEED BY THE N. C. DEPARTMENT OF TRANSPORTATION AS BEING ACCURATE NOR IT IS CONSIDERED TO BE PART OF THE PLANS, SPECIFICATIONS, OR CONTRACT FOR THE PROJECT.

DRAWN BY: W. D. FIELDS, J. L. PEDRO, T. T. WALKER

NORTH CAROLINA DEPARTMENT OF TRANSPORTATION

DIVISION OF HIGHWAYS

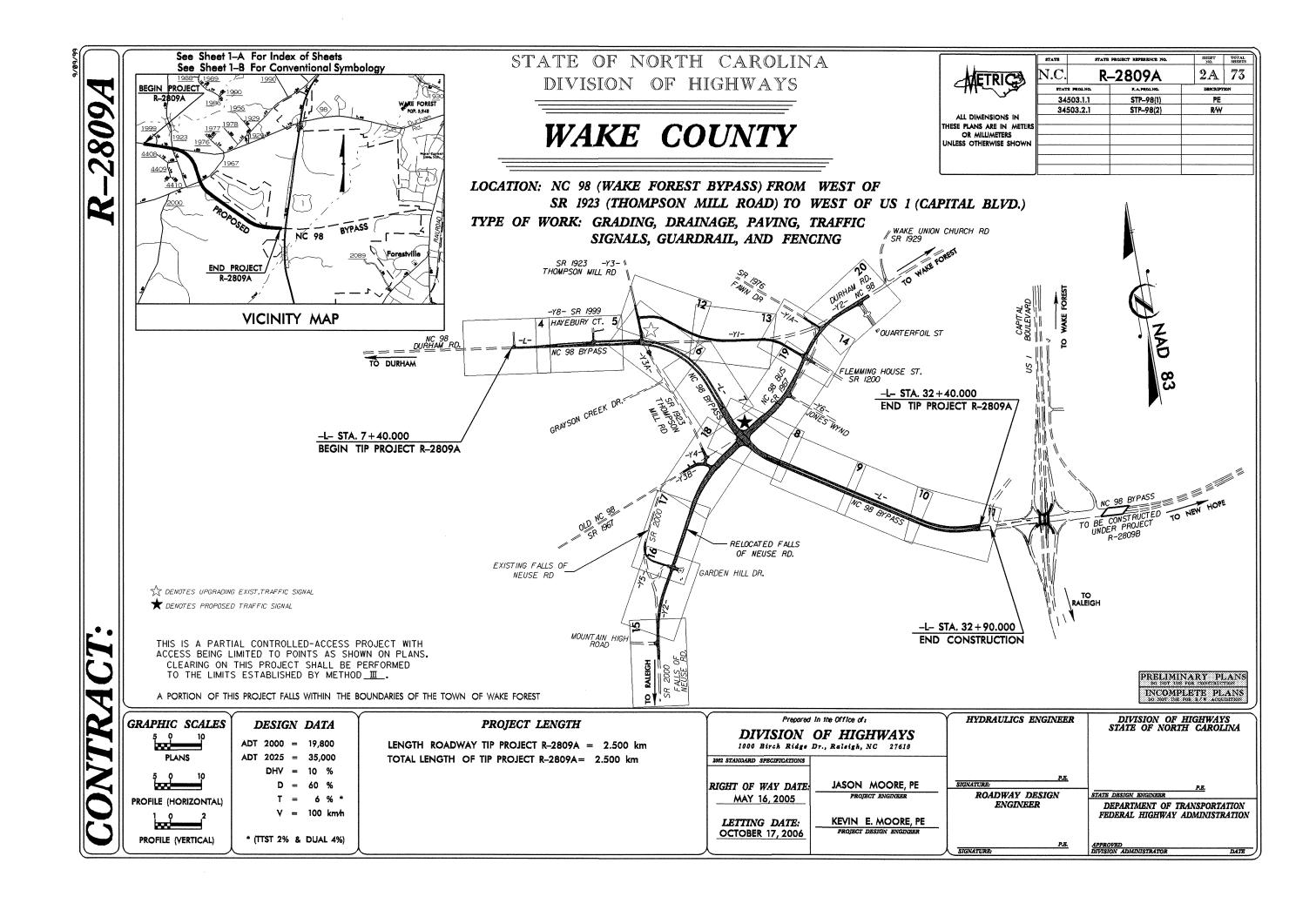
GEOTECHNICAL UNIT





SUBSURFACE INVESTIGATION

	SOIL AND ROCK LEGEND, TERM	IS, SYMBOLS, AND ABBREVIATIONS	
SOIL DESCRIPTION	GRADATION	ROCK DESCRIPTION	TERMS AND DEFINITIONS
SOIL IS CONSIDERED TO BE THE UNCONSOLIDATED, SEMI-CONSOLIDATED OR WEATHERED EARTH MATERIALS WHICH CAN BE PENETRATED WITH A CONTINUOUS FLIGHT POWER AUGER, AND WHICH YIELDS LESS THAN 100 BLOWS PER 30 cm aCCORDING TO STANDARD PER ITERATION TEST (AGASTHO T-SEGS, ASTM D-1586S, SOIL CLASSICIATION IS BASED ON THE AASHTO SYSTEM AND BASIC DESCRIPTIONS GENERALLY SHALL INCLUDE; CONSISTENCY, COLOR, TEXTURE, MOISTURE, AASHTO CLASSIFICATION, AND OTHER PERTINENT FACTORS SUCH AS MINERALDGICAL COMPOSITION, ANOULARITY, STRUCTURE, PLASTICITY, ETC. EXAMPLE. VERY STAFF, GRW SATY CLA. MOST WITH INTERBEDDED FINE SWO LAVERS, MURRY PLASTC, A-7-6	WELL GRADED INDICATES A GOOD REPRESENTATION OF PARTICLE SIZES FROM FINE TO COARSE UNIFORM— INDICATES THAT SOIL PARTICLES ARE ALL APPROXIMATELY THE SAME SIZE (ALSO POORLY GRADED), GAP-GRADED—INDICATES A MIXTURE OF UNIFORM PARTICLES OF TWO OR MORE SIZES. ANGULARITY OF GRAINS THE ANGULARITY OR ROUNDNESS OF SOIL GRAINS ARE DESIGNATED BY THE TERMS; ANGULAR, SUBANGULAR, SUBROUNDED, OR ROUNDED.	HARD ROCK IS NON-COASTAL PLAIN MATERIAL THAT WHEN TESTED, WOULD YIELD ST REFUSAL, AN INFERRED ROCK LINE INDICATES THE LEVEL AT WHICH NON-COASTAL PLAIN MATERIAL WOULD YIELD ST REFUSAL. SPT REFUSAL IS PENETRATION BY A SPLIT SPOON SAMPLER EQUAL TO OR LESS THAN 2.5 cm PER 50 BLOWS. IN NON-COASTAL PLAIN MATERIAL, THE TRANSITION BETWEEN SOIL AND ROCK IS OFTEN REPRESENTED BY A ZONE OF WEATHERED ROCK. ROCK MATERIALS ARE TYPICALLY DIVIDED AS FOLOWS: WEATHERED TO NON-COASTAL PLAIN MATERIAL THAT YIELDS SPT N VALUES > 100 BLOWS ROCK (WR)	ALLUVIUM (ALLUV.) - SOILS WHICH HAVE BEEN TRANSPORTED BY WATER. AOUIFER - A WATER BEARING FORMATION OR STRATA. ARENACEOUS - APPLIED TO ROCKS THAT HAVE BEEN DERIVED FROM SAND OR THAT CONTAIN SAND. AREILACEOUS - APPLIED TO ALL ROCKS OR SUBSTANCES COMPOSED OF CLAY MINERALS, OR HAVING A NOTABLE PROPORTION OF CLAY IN THEIR COMPOSITION, AS SHALE, SLATE, ETC. ARTESIAN - GROUND WATER THAT IS UNDER SUFFICIENT PRESSURE TO RISE ABOVE THE LEVEL AT WHICH IS
SOIL LEGEND AND AASHTO CLASSIFICATION GENERAL GRANULAR MATERIALS SILT-CLAY MATERIALS ORGANIC MATERIALS CLASS. (\$25% PASSING *200) (\$85% PASSING *200) ORGANIC MATERIALS	MINERAL OGICAL COMPOSITION MINERAL NAMES SUCH AS QUARTZ, FELDSPAR, MICA, TALC, KAQLIN, ETC. ARE USED IN DESCRIPTIONS WHENEVER THEY ARE CONSIDERED OF SIGNIFICANCE.	CRYSTALLINE ROCK (CR) FINE TO COARSE GRAIN IGNEOUS AND METAMORPHIC FOCK THAT WOLLD YIELD SPT REFUSAL IF TESTED, ROCK TYPE INCLUDES GRANITE, GRIESS, GABRIS, CHIST, CT.	ENCOUNTERED, BUT WHICH DOES NOT NECESSARILY RISE TO DR ABOVE THE GROUND SURFACE. CALCAREDUS (CALC.) - SOILS WHICH CONTAIN APPRECIABLE AMOUNTS OF CALCIUM CARBONATE. COLLUVIUM - ROCK FRAGMENTS MIXED WITH SOIL DEPOSITED BY GRAVITY ON SLOPE OR AT BOTTOM OF SLOPE.
GROUP A-1 A-3 A-2 A-4 A-5 A-6 A-7 A-1, A-2 A-4, A-5 CLASS. A-1-a A-1-b A-2-4 A-2-5 A-2-6 A-2-7 A-3 A-6, A-7 A-1, A-2 A-4, A-5 A-6 A-7 A-1, A-2 A-4, A-5 A-6, A-7 A-1, A-2 A-1, A-2 A-6, A-7 A-1, A-2 A-1, A-1, A-2 A-1, A-1, A-1, A-1, A-1, A-1, A-1, A-1,	COMPRESSIBILITY SLIGHTLY COMPRESSIBLE LIQUID LIMIT LESS THAN 30 MODERATELY COMPRESSIBLE LIQUID LIMIT 31-50 HIGHLY COMPRESSIBLE LIQUID LIMIT GREATER THAN 50	NON-CRYSTALLINE FINE TO COARSE GRAIN METAMORPHIC AND NON-COASTAL PLAIN SEDIMENTARY ROCK THAT WOULD YELD SY REFUSAL IF TESTED, ROCK TYPE SEDIMENTARY ROCK THAT WOULD YELD SY REFUSAL IF TESTED, ROCK TYPE INCLUDES PHYLLITE, SLATE, SANDSTONE, ETC. COASTAL PLAIN SEDIMENTS CEMENTED INTO ROCK, BUT MAY NOT YIELD SEDIMENTARY SEDIMENTARY SEDIMENTARY	CORE RECOVERY (REC.) - TOTAL LENGTH OF ALL MATERIAL RECOVERED IN THE CORE BARREL DIVIDED BY TOTAL LENGTH OF CORE RUN AND EXPRESSED AS A PERCENTAGE. DIKE - A TABBLEAR BODY OF IGNEOUS ROCK THAT CUTS ACROSS THE STRUCTURE OF ADJACENT ROCKS OR CUTS MASSIVE ROCK.
Z PASSING	WATER LEVEL IN BORE HOLE IMMEDIATELY AFTER DRILLING.	FRESH ROCK FRESH, CRYSTALLS BRIGHT, FEW JOINTS MAY SHOW SLIGHT STAINING, ROCK RINGS UNDER HAMMER IF CRYSTALLINE. VERY SLIGHT (V. SLI.) CRYSTALLS ON A BROKEN SPECIMEN FACE SHINE BRIGHTLY, ROCK RINGS UNDER HAMMER BLOWS IF OF A CRYSTALLINE RATURE. SLIGHT ROCK GENERALLY FRESH, JOINTS STAINED AND DISCOLORATION EXTENDS INTO ROCK UP TO CS. JOINTS MAY CONTAIN CAY. IN GRANITOID ROCKS SOME OCCASIONAL FELDSPAR CRYSTALLS RAE DULL AND DISCOLORACION ROCKS SOME OCCASIONAL FELDSPAR CRYSTALS ARE DULL AND DISCOLORACION ROCKS SOME OCCASIONAL FELDSPAR CRYSTALS ARE DULL AND DISCOLORACION ROCKS SOME OCCASIONAL FELDSPAR	DIP - THE ANGLE AT WHICH A STRATUM OR ANY PLANAR FEATURE IS INCLINED FROM THE HORIZONTAL. DIP DIRECTION (DIP AZIMUTH) - THE DIRECTION OR BEARING OF THE HORIZONTAL TRACE OF THE LINE OF DIP, MEASURED CLOCKWISE FROM NORTH. FAULT - A FRACTURE OR FRACTURE ZONE ALONG WHICH THERE HAS BEEN DISPLACEMENT OF THE SIDES RELATIVE TO DNE ANDTHER PARALLEL TO THE FRACTURE. FISSILE - A PROPERTY OF SPLITTING ALONG CLOSELY SPACED PARALLEL PLANES. FLOAT - ROCK FRAGMENTS ON SURFACE NEAR THEIR ORIGINAL POSITION AND DISLODGED FROM PARENT MATERIAL FLOOD PLAIN (F.P.) - LAND BORDERING A STREAM, BUILT OF SEDIMENTS DEPOSITED BY THE STREAM.
GENERATING SAND SAND GRAVEL AND SAND SOILS SOILS MATTER	STATIC WATER LEVEL AFTER 24 HOURS. PERCHED WATER, SATURATED ZONE OR WATER BEARING STRATA SPRING OR SEEPAGE	MODERATE MODERA	FORMATION (FM.) - A MAPPABLE GEOLOGIC UNIT THAT CAN BE RECOGNIZED AND TRACED IN THE FIELD. JOINT - FRACTURE IN ROCK ALONG WHICH NO APPRECIABLE MOVEMENT HAS OCCURRED. LEDGE - A SHELF-LIKE RIDGE OR PROJECTION OF ROCK WHOSE THICKNESS IS SMALL COMPARED TO ITS LATERAL EXTENT. LENS - A BODY OF SOIL OR ROCK THAT THINS OUT IN ONE OR MORE DIRECTIONS.
CONSISTENCY OR DENSENESS PRIMARY SOIL TYPE COMPACTNESS OR CONSISTENCY COMPACTNESS OR CONSISTENCY COMPACTNESS OR CONSISTENCY COMPACTNESS OR COMPACTNESS	MISCELLANEOUS SYMBOLS ROADWAY EMBANKMENT WITH SOIL DESCRIPTION MISCELLANEOUS SYMBOLS PAT 1PT 1 DESTRUCT SAMPLE DESIGNATIONS	SEVERE AND DISCOLORED AND A MAJORITY SHOW KAOLINIZATION, ROCK SHOWS SEVERE LOSS OF STRENGTH AND CAN BE EXCAVATED WITH A GEOLOGIST'S PICK, ROCK GIVES 'CLUNK' SOUND WHEN STRUCK. IF TESTED, MOULD YIELD SPT REFUSAL SEVERE ALL ROCKS EXCEPT QUARTZ DISCOLORED OR STAINED, ROCK FABRIC CLEAR AND EVIDENT BUT REDUCED	MOTILEO (MOT.) - IRREGULARLY MARKED WITH SPOTS OF DIFFERENT COLORS. MOTILING IN SOILS USUALLY INDICATES POOR AERATION AND LACK OF GOOD DRAINAGE. PERCHED WATER - WATER MAINTAINED ABOVE THE NORMAL GROUND WATER LEVEL BY THE PRESENCE OF AN INTERVENING IMPERVIOUS STRATUM.
GENERALLY	SOIL SYMBOL AUGER BORING S- BULK SAMPLE ARTIFICIAL FILL OTHER THAN ROADWAY EMBANKMENTS CORE BORING SS- SPLIT SPOON SAMPLE ST- SHELBY TUBE SAMPLE SAMPLE SAMPLE SAMPLE SAMPLE	IN STRENGTH TO STRONG SOIL. IN GRANTIOID ROCKS ALL FELDPARS ARE KAOLINIZED TO SOME EXTENT, SOME FRAGMENTS OF STRONG ROCK USUALLY REMAIN. 15 TESTED, YIELDS ST N VALUES > 180 BLOWS PER 30 cm. VERY SEVERE (V. SEV.) 14 ROSK EXCEPT QUARTZ DISCOLORED OR STAINED, ROCK FABRIC ELEMENTS ARE DISCERNIBLE BUT THE MASS IS EFFECTIVELY REDUCED TO SOIL STATUS, WITH ONLY FRAMMENTS OF STRONG ROCK REMAININ SAPPOLITE IS AN EXAMPLE OF ROCK WEATHERED TO A DEGREE SUCH THAT ONLY MINOR VESTIGES OF THE	RESIDUAL SOIL - SOIL FORMED IN PLACE BY THE WEATHERING OF ROCK. ROCK QUALITY DESIGNATION (R.O.D.) - A MEASURE OF ROCK QUALITY DESCRIBED BY TOTAL LENGTH OF ROCK SEGMENTS EQUAL TO OR GREATER THAN 10 CENTIMETERS DIVIDED BY THE TOTAL LENGTH OF CORE RUN
GENERALLY SOFT 2 TO 4 25 TO 50	INFERRED ROCK LINE ALLUVIAL SOIL BOUNDARY PIEZOMETER INSTALLATION RT- RECOMPACTED SLOPE INDICATOR TRIAXIAL SAMPLE	ORIGINAL ROCK FABRIC REMAIN. <u>IF TESTED, YIELDS SPT N VALUES < 188 BLOWS PER 30 cm</u> , COMPLETE ROCK REDUCED TO SOIL. ROCK FABRIC NOT DISCERNIBLE, OR DISCERNIBLE ONLY IN SMALL AND SCATTERED CONCENTRATIONS. QUARTZ MAY BE PRESENT AS DIKES OR STRINGERS. SAPROLITE IS ALSO AN EXAMPLE.	SILL - AN INTRUSIVE BODY OF IGNEOUS ROCK OF APPROXIMATELY UNIFORM THICKNESS AND RELATIVELY THIN COMPARED MITH ITS LATERAL EXTENT, WHICH HAS BEEN EMPLACED PARALLEL TO THE BEDDING OR SCHISTOSITY OF THE INTRUDED ROCKS.
HARD >30 >400 TEXTURE OR GRAIN SIZE	25/905 DIP/DIP DIRECTION OF ROCK STRUCTURES SPT N-VALUE SOUNDING ROD SPT REFUGAL	ROCK HARDNESS VERY HARD CANNOT BE SCRATCHED BY KNIFE OR SHARP PICK, BREAKING OF HAND SPECIMENS REQUIRES	SLICKENSIDE - POLISHED AND STRIATED SURFACE THAT RESULTS FROM FRICTION ALONG A FAULT OR SLIP PLANE. STANDARD PENETRATION TEST (PENETRATION RESISTANCE) (SPT) - NUMBER OF BLOWS (IN OF A 63.5 kg HAMMER
U.S. STD. SIEVE SIZE	ABBREVIATIONS	SEVERAL HARD BLOWS OF THE GEOLOGISTS PICK. HARD CAN BE SCRATCHED BY KNIFE OR PICK ONLY WITH DIFFICULTY, HARD HAMMER BLOWS REQUIRED TO DETACH HAND SPECIMEN.	FALLING 0.76 METERS REQUIRED TO PRODUCE A PENETRATION OF 30 cm INTO SOIL WITH A 5 cm outside diameter split spoon sampler. Spt refusal is less than 2.5 cm penetration WITH 50 BLOWS. STRATA CORE RECOVERY (SREC.) - TOTAL LENGTH OF STRATA MATERIAL RECOVERED DIVIDED BY TOTAL LENGTH
GRAIN MM 305 75 2.0 0.25 0.05 0.005	AR - AUGER REFUSAL PMT - PRESSUREMETER TEST BT - BORING TERMINATED SD SAND, SANDY CL CLAY SL SILT, SILTY CPT - CONE PENETRATION TEST SLI SLIGHTLY CSE - COARSE TCR - TRICONE REFUSAL	MODERATELY HARD CAN BE SCRATCHED BY KNIFE OR PICK, GOUGES OR GROOVES TO 6 mm DEEP CAN BE EXCAVATED BY HARD BLOW OF A GEOLOGISTS PICK, HAND SPECIMENS CAN BE DETACHED BY MODERATE BLOWS. MEDIUM CAN BE GROOVED OR GOUGED 1 mm DEEP BY FIRM PRESSURE OF KNIFE OR PICK POINT.	OF STRATUM AND EXPRESSED AS A PERCENTAGE. STRATA ROCK QUALITY DESIGNATION (S.R.D.D.) - A MEASURE OF ROCK QUALITY DESCRIBED BY: TOTAL LENGTH OF ROCK SEGMENTS WITHIN A STRATUM EQUAL TO OR GREATER THAN 10 CENTIMETERS DIVIDED BY THE TOTAL LENGTH OF STRATA AND EXPRESSED AS A PERCENTAGE.
SOIL MOISTURE - CORRELATION OF TERMS SOIL MOISTURE SCALE FIELD MOISTURE GUIDE FOR FIELD MOISTURE DESCRIPTION GUIDE FOR FIELD MOISTURE DESCRIPTION	F FINE W - MOISTURE CONTENT	HARD CAN BE EXCAVATED IN SMALL CHIPS TO PIECES 25 mm MAXIMUM SIZE BY HARD BLOWS OF THE POINT OF A GEOLOGISTS PICK. SOFT CAN BE GROVED OR GOUGED READILY BY KNIFE OR PICK, CAN BE EXCAVATED IN FRAGMENTS FROM CHIPS TO SEVERAL CENTIMETERS IN SIZE BY MODERATE BLOWS OF A PICK POINT. SMALL, THIN PIECES CAN BE BROKEN BY FINGER PRESSURE.	TOPSOIL (T.S.) - SURFACE SOILS USUALLY CONTAINING ORGANIC MATTER. BENCH MARK:
- SATURATED - USUALLY LIQUID, VERY WET, USUALLY FROM BELOW THE GROUND WATER TABLE PLASTIC SEMISOLID; REQUIRES DRYING TO	FRAC FRACTURED VST - VANE SHEAR TEST FRAGS FRAGMENTS MED MEDIUM	VERY CAN BE CARVED WITH KNIFE. CAN BE EXCAVATED READILY WITH POINT OF PICK, PIECES 25 mm SOFT OR MORE IN THICKNESS CAN BE BROKEN BY FINGER PRESSURE. CAN BE SCRATCHED READILY BY FINGERNAIL.	ELEVATION: NOTES:
PLASTIC LIMIT - WEI - (W) ATTAIN OPTIMUM MOISTURE	DRILL UNITS: ADVANCING TOOLS: ADVANCING TOOLS: TX AUTOMATIC MANUAL	FRACTURE SPACING BEDDING	
OM OPTIMUM MOISTURE - MOIST - (M) SULID: AT OR NEAR DEFIIMUM MOISTURE SHRINKAGE LIMIT - DRY - (D) REQUIRES ADDITIONAL WATER TO ATTAIN OPTIMUM MOISTURE	MOBILE B-	WIDE	
PLASTICITY PLASTICITY INDEX (PI) DRY STRENGTH	CME-45C HARD FACED FINGER BITS -N	INDURATION FOR SEDIMENTARY ROCKS, INDURATION IS THE HARDENING OF THE MATERIAL BY CEMENTING, HEAT, PRESSURE, ETC.	1
NONPLASTIC 0-5 VERY LOW LOW PLASTICITY 6-15 SLIGHT MED. PLASTICITY 16-25 MEDIUM HIGH PLASTICITY 26 OR MORE HIGH	TONG-CARBIDE INSERTS	FRIABLE RUBBING WITH FINGER FREES NUMEROUS GRAINS; GENTLE BLOW BY HAMMER DISINTEGRATES SAMPLE. MODERATELY INDURATED GRAINS CAN BE SEPARATED FROM SAMPLE WITH STEEL PROBE;	
COLOR DESCRIPTIONS MAY INCLUDE COLOR OR COLOR COMBINATIONS (TAN, RED, YEL-BRN, BLUE-GRAY) MODIFIERS SUCH AS LIGHT, DARK, STREAKED, ETC. ARE USED TO DESCRIBE APPEARANCE.	X DTHER D-50 TRICONE MIN TUNGCARB. HAND AUGER CORE BIT SOUNDING ROD VANE SHEAR TEST	BREAKS EASILY WHEN HIT WITH HAMMER. INDURATED GRAINS ARE DIFFICULT TO SEPARATE WITH STEEL PROBE; DIFFICULT TO BREAK WITH HAMMER. EXTREMELY INDURATED SHARP HAMMER BLOWS REQUIRED TO BREAK SAMPLE;	
	OTHER	SAMPLE BREAKS ACROSS GRAINS.	REVISED 09/15/00





STATE OF NORTH CAROLINA

DEPARTMENT OF TRANSPORTATION

Michael F. Easley GOVERNOR P.O. BOX 25201, RALEIGH, N.C. 27611-5201

Lyndo Tippett Secretary

February 1, 2006

STATE PROJECT:

34503.1.1 (R-2809A)

FEDERAL PROJECT:

DESCRIPTION:

STP-98(1) Wake

COUNTY:

NC 98 (Wake Forest Bypass) from west of SR 1923 (Thompson Mill Road)

to west of US 1 (Capital Boulevard)

SUBJECT:

Geotechnical Report – Inventory

Project Description

This westernmost portion of the Wake Forest Bypass project consists of a new four lane roadway (-L-), which will tie into the existing NC 98 (Durham Road) approximately 2.7 kilometers west of US 1 (Capital Boulevard). The project begins 0.6 kilometers west of the intersection of NC 98 and Thompson Mill Road (SR 1923), and ends just west of the US 1 at R-2809B (currently under construction). The project includes an intersection with NC 98 Bypass (-L-) and Falls of the Neuse Road (-Y2-) and also upgrades to several SR routes. Some of the State Roads (all Y lines) are being re-aligned to accommodate the relocation of -Y2-. A noise wall is to be constructed left of -L- from 30+60 to 31+80. The project is approximately 2.5 kilometers in length.

The geotechnical field investigation was conducted from March to July 2005. A track-mounted D-50 drill and an ATV-mounted CME-550 with automatic hammers were used during the investigation. Standard Penetration Tests were performed in selected borings and additional borings were advanced using continuous flight augers. Representative soil samples were collected for visual classification in the field and for laboratory analysis by the Materials and Tests Unit.

The following alignments, totaling 6.8 kilometers, were investigated. Subsurface soil profiles, or cross-sections, of these alignments are included in this report.

<u>Line</u>	<u>S1</u>	tatio	<u>n</u>
-L-	7+40	to	32+40
-Y1-	10+00	to	19+07
-Y1A-	10+00	to	11+66
-Y2-	10+20	to	30+74

-Y3-	10+00	to	12+49
-Y3A-	11+00	to	11+89
-Y3B-	10+00	to	12+64
-Y4-	10+00	to	11+01
-Y5-	10+14	to	13+68

Areas of Special Geotechnical Interest

1) <u>Highly Plastic Clay Soils</u>: Areas containing highly plastic (PI>25) clay soils are noted below:

<u>Alignment</u>	<u>Station</u>	Offset(m)
- L-	12+20	12 LT
- L-	15+20	20 RT
-L-	18+90 to 19+50	RT
- L-	20+70 to 21+70	CL
-L-	22+95 to 24+10	CL
-L-	25+75 to 26+90	CL
- L-	27+50 to 29+45	CL
-Y1-	10+70 to 11+90	CL
-Y1-	17+00	7.5 LT
-Y2-	12+00 to 13+30	CL
-Y2-	15+00	CL
-Y2-	15+65 to 16+68	CL
-Y2-	17+60	CL
-Y2-	18+90 to 19+30	CL
-Y2-	20+50 to 21+30	CL
-Y2-	23+30 to 25+85	CL
-Y2-	28+20 to 29+00	RT
-Y3B-	10+70	19 RT
-Y5-	10+85 to 11+48	CL
-Y5-	11+50	CL
-Y5-	11+80	CL

A discussion of these highly plastic clay soils is located below in the section titled "Soil Properties".

2) <u>Water Wells:</u> No water wells were found within the proposed construction limits on the project. However, one well was located within 1 meter of the proposed construction limits at the following location:

<u>Alignment</u>	<u>Station</u>	Offset(m)
-Y1A-	10+60	9.5 RT

Physiography and Geology

The project is located southwest of the town of Wake Forest, within the Piedmont Physiographic Province. Residual soils are derived from the underlying Raleigh Belt bedrock. The bedrock in this area is composed

of injected granite gneiss and lineated, felsic, mica schist and gneiss. The project corridor includes areas of dense woods and scattered homes. Several subdivisions occur towards the town limits of Wake Forest and at the beginning of Falls of the Neuse Road (-Y2-).

Soil Properties

Soils encountered during this investigation are separated into four categories based on origin, roadway embankment, artificial fill, alluvial, and residual soils.

Roadway Embankment Soils: Roadway embankment soils were encountered in small amounts associated with several existing roadways on the project. Most of the embankment soils occur where the relocated lines tie back into the existing roadways. Roadway embankment soils are present on -L-, -Y1-, -Y2-, -Y3-, -Y3B-, -Y4-, and -Y5-. The embankment soils in these areas primarily consist of red-orange and brown, medium stiff to stiff, dry to moist, sandy and silty clay (A-6 and A-7).

Artificial Fill Soils: An area of artificial fill soil is present at the end of -Y2-, from right of 27+60 to 28+50 (see Plan Sheets No. 14, 20) where a gravel driveway leading to a utility easement has been constructed. The fill soils consist of loose gravel and very stiff, silty clay (A-7-6) with some asphalt debris.

<u>Alluvial Soils</u>: Alluvial soils are present in several small creeks throughout the project. Alluvial soils are 2.0 meters thick or less, consisting of stiff to very stiff, silty and sandy clay (A-7-5 and A-6) (see Plan Sheet Nos. 17 and Profile Sheet No. 26).

Residual Soils: Residual soils are derived from the in-place weathering of the underlying bedrock. They consist primarily of tan, brown, red, and orange, medium stiff to very stiff, dry to moist, sandy and silty clay (A-6 and A-7). Most of the residual, silty clays on the project have plasticity indices from 15 to 35, generally decreasing with depth. Residual, highly plastic "cap" clays occur at the ground surface over several areas of the project. Areas containing highly plastic soils (plasticity indices greater than 25) are listed above in the section "Areas of Special Geotechnical Interest". Lessor amounts of tan, orange and brown, stiff, dry to moist, sandy silt (A-4 and A-5) and tan, pink, and orange, loose to medium dense, dry to moist, silty sand and sand (A-2-4 and A-2-5) are also present. Most of the residual soils exhibit a saprolitic texture, and mica contents for these soils generally range from moderate to abundant amounts.

Rock Properties

Weathered rock: Weathered rock is encountered in seven borings on -L-, -Y1-, and -Y3- (see Profile Sheet No. 21-24 and Cross Section Sheet No. 28, 37, 43, 47, 51, and 53) near the center and at the end of the project. The weathered rock consists of severely weathered granite with coarse-grained mica, which is derived from the underlying bedrock. Weathered rock grades into crystalline rock (see Profile Sheet No. 24).

<u>Crystalline rock:</u> Injected granite gneiss and lineated, felsic, mica schist and gneiss of the Raleigh Belt underlies the project area. Areas of crystalline rock yielding auger refusal occurred at -Y1- 10+25, CL and 11+40, 8m RT.

Noise Wall

A noise wall will be constructed left of -L- Sta. 30+60 to 31+80 (see Plan Sheet No. 11 and Cross Section Sheet No. 49-51). Residual soils and weathered rock were encountered at the wall location.

SHEET 3A OF 73 34503.1.1 (R-2809A)

The soils primarily consist of orange-brown and tan-pink, loose to medium dense, dry, saprolitic, silty sand and sand (A-2-4 and A-2-5); and tan-pink and orange, stiff, dry, micaceous, saprolitic, sandy silt (A-5). Other soil present is red-orange, stiff, moist, silty clay (A-7-5). Weathered rock occurs at a depth of 2.1 meters at 31+60, 25 LT. Groundwater was not encountered in the noise wall borings.

Groundwater

Groundwater was encountered in less than 10% of the borings throughout the project, and generally occurs at depths greater than 5.0 meters. The borings where groundwater was encountered were located near streambeds in the low-lying areas. All creeks were dry at the time of the investigation.

Prepared by,

Jaime Love Pedro Engineering Geologist

Jaime Love Pedro

UNDISTURBED SAMPLES

Undisturbed "Shelby" tube samples were taken at the following locations to provide data regarding in situ soil strength.

Sample No.	<u>Location</u>	Depth (m)	<u>Test</u>
ST-1	28+40, 40m LT, -L-	4.40 - 4.90	Triaxial CU

BULK SAMPLES

The following bulk samples were taken for tests to determine the engineering properties of the soil.

Sample No.	<u>Location</u>	Depth (m)	<u>Test</u>
RT-1	13+40, 32m RT, -L-	4.50 - 9.50	Recompacted Triaxial CU
CBR-1	13+40, 32m RT, -L-	4.50 - 9.50	California Bearing Ratio

COMPUTED BY:	HLE	DATE:	8/1/07	
CHECKED BY:	TRM	DATE:	8/6/07	

SHEET NO. PROJECT NO. R-2809A ALTERNATE 1 3B OF 73

EARTHWORK BALANCE SHEET IN CUBIC METERS

					IN CODIC		т					Dool/	OLUTADI E	UNSUITABLE	TOTAL
	UNCLASSIFIED	ROCK	UNDERCUT	UNSUITABLE	SUITABLE	TOTAL	EARTH	ROCK	EMB'T		SELECT	ROCK	SUITABLE	WASTE	WASTE
LOCATION	EXCAVATION	EXCAVATION	EXCAVATION	EARTH EXCAVATION	EARTH EXCAVATION	EMB'T	EMBANKMENT	EMB'T	+ % 20	BORROW	BORROW	WASTE	WASTE	WASIE	WASIE
L (LT) 7+40.000 TO 13+60.000	88	0	0	0	88	37331	37331	.0	44797	44709	0	0	0	0	Ņ
L (RT) 7+40.000 TO 13+60.000	2709	0	0	0	2709	3750	3750	0	4500	1791	0	0	0	0	0
L 13+60.000 TO 15+60.000	8572	0	0	0	8572	8039	8039	0	9647	1075	0	0	0	0 '	0 ·
Y8 10+20.000 TO 10+50.000	29	0	0	0	29	42	42	0	50	21	0	0	0	0	0
Y3 10+00.000 TO 12+40.000	54	0	0	0	54	5441	5441	0	6529	6475	0	0	0	0	0
Y1 10+00.000 TO 18+40.000	8853	0	0	3953	4900	21446	21446	0	25735	20835	0	0	0	3953	3953
Y1 18+40.000 TO 19+00.000	19	0	0	0	19	242	242	0	290	271	0	0	0	0	0
Y3A 11+00.000 TO 11+50.000	719	0	0	0	719	7	7	0	8	0	0	0	711	0	711
Y3A 11+00.000 TO 11+30.000	0	0	0	0	0	0	0	0	0	0	0	0	. 0	0	0
SUBTOTALS NO 1	21043	0	0	3953	17090	76298	76298	Ō	91556	75177	0	0	711	3953	4664
		**************************************		6694	12499	86551	86551	0	103861	91362	0	0	0	7766	7766
L 15+60.000 TO 24+00.000	19193	0	1072 0	0	324	163	163	- ŏ	196	0	0	0	128	0	128
Y2 (LT.) 8+60.000 TO 12+00.000	324		0	0	775	15	15	0	18	0	0	0	757	0	757
Y2 (RT.) 8+60.000 TO 12+00.000	775	0		1767	31530	17284	17284	Ö	20741	0	0	0	10789	4203	14992
Y2 12+00.000 TO 21+40.000	33297 16494	0	2436 0	0	16494	16506	16506	0	19807	3313	0	0	0	0	0
Y 21+80.000 TO 28+60.000	16494	0	0	0	26	135	135	0	162	136	0	0	0	0	0
Y2 (LT.) 28+60.000 TO 30+73.920 Y2 (RT.) 28+60.000 TO 30+73.920	101	0	0	0	101	132	132	0	158	57	0	0	0	0	0
	121	0	0	0	121	23	23	.0	28	0	0	0	93	0	93
Y5 10+20.000 TO 10+60.000	1975	0	152	1688	287	1179	1179	0	1415	1128	0	0	0	1840	1840
Y5 10+80.000 TO 12+70.000	1393	0	0	0	1393	178	178	0	214	0	0	0	1179	0	1179
Y3B 10+20.000 TO 11+60.000	559	0	0	0	559	39	39	0	47	0	0	0	512	0	512
Y4 10+04.500 TO 10+80.000	3	0	0	0	3	5	5	0	6	3	0	0	0	0	0
Y6 10+15.000 TO 10+25.000	87	0	0	0	87	390	390	0	468	381	0	0	0	0	0
Y1A 10+20.000 TO 11+00.000	0	0	0	ō	0	0	0	. 0	0	0	0	0	0	0	0
CUDTOTAL C. NO.2	74348	0	3660	10149	64199	122600	122600	0	147121	96380	0	0	13458	13809	27267
SUBTOTALS NO 2		34	0	41013	105122	7626	7592	34	9144	0	0	0	96012	41013	137025
L 24+00.000 TO 32+60.000	146169	0	0	0	0	24667	24667	0	29600	29600	0	0	0	0	. 0
L 23+80 TO 31+40 (EARTH BERM)	0 1501	0	0	77	1424	8	8	0	10	0	0	0	1414	77	1491
Y9 10+25.000 TO 10+70.000	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
CUDTOTAL NO 2	147670	34	0	41090	106546	32301	32267	34	38754	29600	0	0	97.426	41090	138516
SUBTOTAL NO 3			,,,,,,,,,,,,,,,,,,,,,,,,,,,,,,,,,,,,,,			100	100	0	120	120	0	0	i o	T o	0
- (LT) 84+64 (R-2809C SUPPLEMENTAL WORK)	0	0	0 .	0	0	0	0	0	0	0	0	0	0	0	0
	0	0	0	0	0					<u> </u>		0	1 0	1 0	0
SUBTOTAL NO 4	0	0	0	0	0	100	100	0	120	120	0	U	j o		
PROJECT SUBTOTALS	243061	34	3660	55192	187835	231299	231265	34	277551	201277	0	0	111595	58852	170447
LOSS DUE TO CLEAR. & GRUB	-7700				-7700					7700			0	10050	0
ADDITIONAL UNDERCUT EXCAV.			10050	0	0	10050	10050	0	12060	12060	0		0	10050	10050 100
EST. FOR DRIVEWAYS	100				100	0	0	0 .	0	0			100	.	100
EST. FOR PAV'T REMOVAL						1570	1570	0	1884	1884	<u> </u>			_	
ROCK TO REPLACE BORROW							0	0	0	0		0	<u> </u>		
ADJUST FOR ROCK WASTE						<u> </u>			0	0	-		444605	<u> </u>	111605
WASTE IN LIEU OF BORROW							1,1100		44400	-111695	<u> </u>	<u> </u>	-111695	-	-111695
SHOULDER CONSTRUCTION				_			11100		11100	11100	 		 	-	
LESS SELECT GRANURAL MAT'L			<u> </u>				270		224	0 324		 			
ROADSIDE ENVIRONMENTAL	0						270		324	324					
				55.00	400007	242040	254255	34	302919	122650		0	0	68902	68902
PROJECT TOTALS	235461	34	: 13710	55192	180235	242919	254255	34	302919		 	1		00902	- 00002
REPLACE TOP SOIL BOR. PITS										6133					
										400700					
GRAND TOTALS	235461		13710	ļ		<u> </u>			<u> </u>	128783	0	 			
SAY	235500	1	13800				<u> </u>	<u> </u>	<u> </u>	128800	0	<u></u>	1	<u> </u>	

PAVEMENT STRUCTURE VOLUME : DRAINAGE DITCH EXCAVATION : SHOULDER BORROW: UNDERCUT EXCAVATION

20,727 CUBIC METERS 3,420 CUBIC METERS CUBIC METERS CUBIC METERS

(Contingency Item)

EARTHWORK QUANTITIES ARE CALCULATED BY THE ROADWAY DESIGN UNIT. THESE EARTHWORK QUANTITIES

ARE BASED IN PART ON SUBSURFACE DATA PROVIDED BY THE GEOTECHNICAL ENGINEERING UNIT.

COMPUTED BY: HLE DATE: 8/1/07 CHECKED BY: DATE: _____8/6/07_ _TRM_

SHEET NO. PROJECT NO. R-2809A ALTERNATE 2 3C OF 73

EARTHWORK BALANCE SHEET IN CUBIC METERS

					IN CUBIC	MEIEKS					· · · · · · · · · · · · · · · · · · ·				
LOCATION	UNCLASSIFIED EXCAVATION	ROCK EXCAVATION	UNDERCUT EXCAVATION	UNSUITABLE EARTH EXCAVATION	SUITABLE EARTH EXCAVATION	TOTAL EMB'T	EARTH EMBANKMENT	ROCK EMB'T	EMB'T + % 20	BORROW	SELECT BORROW	ROCK WASTE	SUITABLE WASTE	UNSUITABLE WASTE	TOTAL WASTE
L (LT) 7+40.000 TO 13+60.000	88	0	0	0	88	38015	38015	. 0	45618	45530	0	0	0	0	0
L (RT) 7+40.000 TO 13+60.000	2701	0	0	0	2701	3789	3789	0	4547	1846	0	0	0	0	0
L 13+60.000 TO 15+60.000	8208	0	0	0	8208	8572	8572	0 /	10286	2078	0	0	0	0	0
Y8 10+20.000 TO 10+50.000	29	0	0	0	29	42	42	0	50	21	0	0	0	0	0
Y3 10+00.000 TO 12+40.000	54	0	0	0	54	5441	5441	0	6529	6475	0	0	0	0	0
Y1 10+00.000 TO 18+40.000	8264	0	0	3953	4311	22536	22536	0	27043	22732	0	0	0	3953	3953
Y1 18+40.000 TO 19+00.000	16	0	0	0	16	273	273	0	328	312	0	0	0	0	0
Y3A 11+00.000 TO 11+50.000	719	0	0	0	719	7	7	0	8	0	0	0	711	0	711
194 11100:000 10 11130:000	0	0	0	0	0	0	. 0	0	0	0	0	0	0	0	0
CURTOTAL S. NO.1	20079	0	0	3953	16126	78675	78675	0	94409	78994	0	0	711	3953	4664
SUBTOTALS NO 1		and the state of t		·			89125	0	106950	95465	0	0	0	7546	7546
L 15+60.000 TO 24+00.000	17991	0	1040	6506	11485	89125 152	<u> </u>	0	182	0	0	0	82	0	82
Y2 (LT.) 8+60.000 TO 12+00.000	264	0 .	0	0	264		152			0	0	0	422	0	422
Y2 (RT.) 8+60.000 TO 12+00.000	441	0	0	0	441	16	16	0	19 21794	0	0	0	8360	3725	12085
Y2 12+00.000 TO 21+40.000	31478	0	2401	1324	30154	18162	18162	0	21176	5711	0	0	0	0	0
Y 21+80.000 TO 28+60.000	15465	0	0	0	15465	17647	17647	0	21176	214	0	0.	0	0	0
Y2 (LT.) 28+60.000 TO 30+73.920	7	0	0	0 .	44	184 160	184 160	0	192	148	0	0	0	0	0
Y2 (RT.) 28+60.000 TO 30+73.920	44	0	0	0		23	23	0	28	0	0	0	93	0	93
Y5 10+20.000 TO 10+60.000	121	0	0	0	121	1179	1179	0	1415	1128	0	0	0	1840	1840
Y5 10+80.000 TO 12+70.000	1975	0	152	1688	287		179	0	214	0	0	0	1179	0	1179
Y3B 10+20.000 TO 11+60.000	1393	0	0	0	1393	178 39	39	0	47	0	0	0	512	1 0	512
Y4 10+04.500 TO 10+80.000	559	0	0	0	559	5 5	5	0	6	3	0	0	0	0	0
Y6 10+15.000 TO 10+25.000	3	0	0	0	3 87	390	390	0	468	381	0	0	0	0	0
Y1A 10+20.000 TO 11+00.000	87	0	0	0		0	0	0	0	0	0	0	0	0	0
	0	0	0	0	0								10648	13111	23759
SUBTOTALS NO 2	69828	0	3593	9518	60310	127260	127260	0	152712	103050	0	0			·
L 24+00.000 TO 32+60.000	143352	34	0	41025	102293	8152	8118	34	9776	0	0	0	92551	41025	133576
L 23+80 TO 31+40 (EARTH BERM)	0	0	0	0	0	24667	24667	0	29600	29600	0	0	0	0	0
Y9 10+25.000 TO 10+70.000	1382	0	0	77	1305	8	8	0	10	0	0	0	1295	77	1372
	0	0	0 .	0	0	0	0	0	0	0	0	0	0	0	0
SUBTOTAL NO 3	144734	34	0	41102	103598	32827	32793	34	39386	29600	0	0	93846	41102	134948
L- (LT) 84+64 (R-2809C SUPPLEMENTAL WORK)	0	0	T 0	0	0	100	100	0	120	120	0	0	0	0	0
L- (L1) 04: 04 (11-20030 CO11 ELIMEIVINE VVOING	0	0	0	0	0	0	0	0	0	0	0	0	. 0	0	0
SUBTOTAL NO 4	0	0	0	0	0	100	100	0	120	120	0	0	0	0	0
	234641	34	3593	54573	180034	238862	238828	34	286627	211764	I 0	0	105205	58166	163371
PROJECT SUBTOTALS LOSS DUE TO CLEAR. & GRUB	-7700	34	. 3333	04070	-7700					7700	1	1	0		0
ADDITIONAL UNDERCUT EXCAV.	-1700		10050	0	0	10050	10050	0	12060	12060	0		0	10050	10050
EST. FOR DRIVEWAYS	100		10000	 	100	0	0	0	0	0			100		100
EST. FOR DRIVEWAYS	100		+			1570	1570	0	1884	1884					
ROCK TO REPLACE BORROW			-	 			0	0	0	0		0			
ADJUST FOR ROCK WASTE			 						0	0	1				
WASTE IN LIEU OF BORROW			-							-105305			-105305		-105305
SHOULDER CONSTRUCTION					<u> </u>		4500	1	4500	4500		1			
LESS SELECT GRANURAL MAT'L										0					
ROADSIDE ENVIRONMENTAL	0		 	1			270		324	324					
ROADSIDE ENVIRONMENTAL															
PROJECT TOTALS	227041	34	13643	54573	172434	250482	255218	34	305395	132927	0	0	0	68216	68216
	221041	J4	10070	0 707 0	,,,,,,,,,,,,,,,,,,,,,,,,,,,,,,,,,,,,,,,		1			6646					
REPLACE TOP SOIL BOR. PITS										30-10					
										139573	0				
GRAND TOTALS	227041	1	13643	•	1	i .				139573	1 13		I I	1	

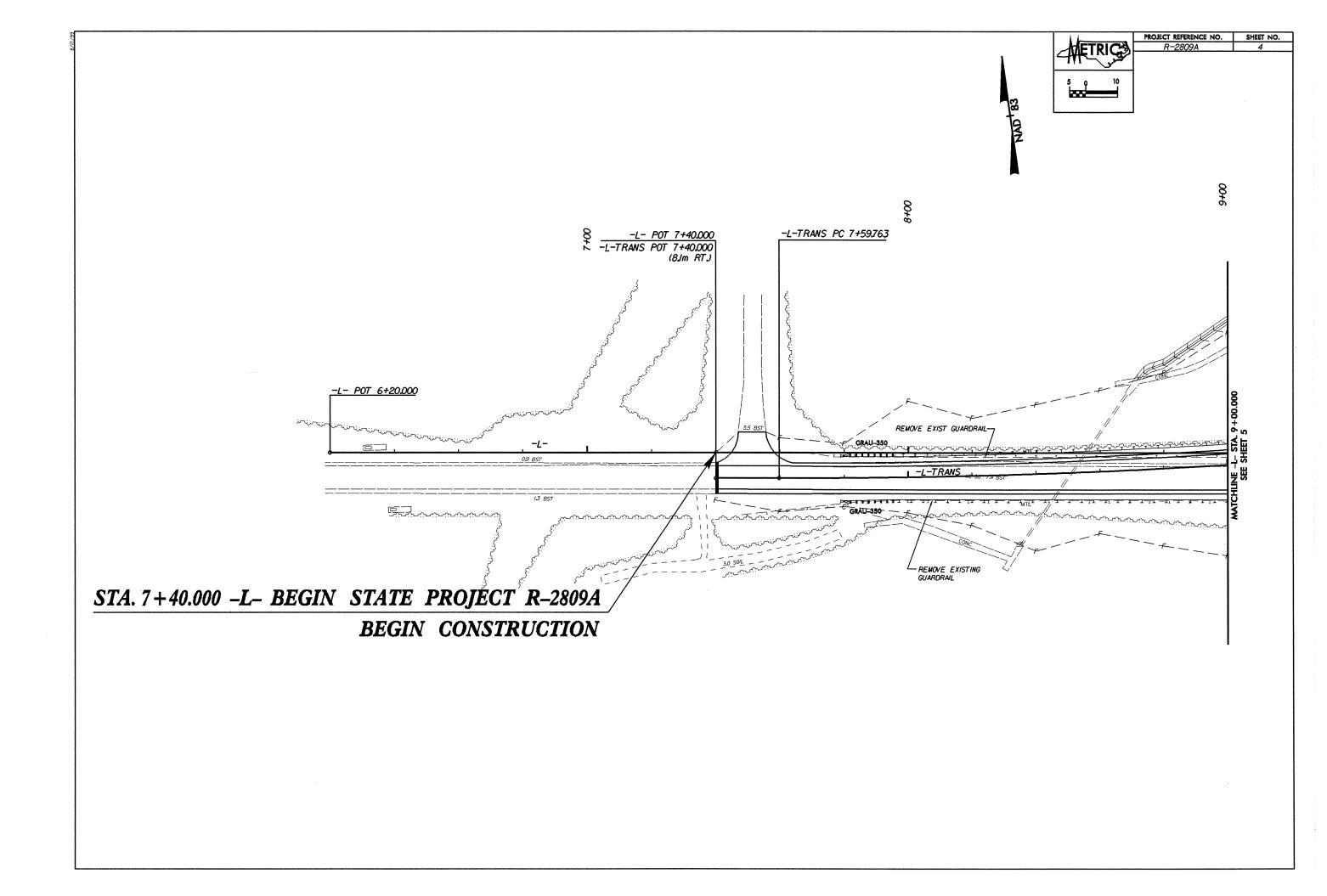
PAVEMENT STRUCTURE VOLUME :
DRAINAGE DITCH EXCAVATION :
SHOULDER BORROW:
UNDERCUT EXCAVATION

13,103 CUBIC METERS 3,420 CUBIC METERS CUBIC METERS CUBIC METERS 0

(Contingency Item)

EARTHWORK QUANTITIES ARE CALCULATED BY THE ROADWAY DESIGN UNIT. THESE EARTHWORK QUANTITIES

ARE BASED IN PART ON SUBSURFACE DATA PROVIDED BY THE GEOTECHNICAL ENGINEERING UNIT.



PROJECT REFERENCE NO. SHEET NO. R-2809A R/W SHEET NO. -Y8- PT 10+82.639 क्रिक्क्क्क् -Y8- PC 10+52.114 -L-TRANS PT ||+00,3|| -L- POT ||+00,000 (4,500 LT.) -PROPOSED G/R -L-TRANS PRC 9+30.037 GRAU-350 ____REMOVE EXIST GUARDRAIL-GRAU-350

