

PROJECT: C201363 ID: B-3872

CONTENTS: II+50 - 14+50 -L-
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STATE OF NORTH CAROLINA

DEPARTMENT OF TRANSPORTATION

DIVISION OF HIGHWAYS

GEOTECHNICAL UNIT

SUBSURFACE INVESTIGATION

STATE PROJECT 33316.1.1 I.D. NO. B-3872
 F.A. PROJECT BRZ-1552(8)
 COUNTY MCDOWELL
 DESCRIPTION APPROACHES TO BRIDGE NO. 195
ON SR-1552 OVER BEAR CREEK

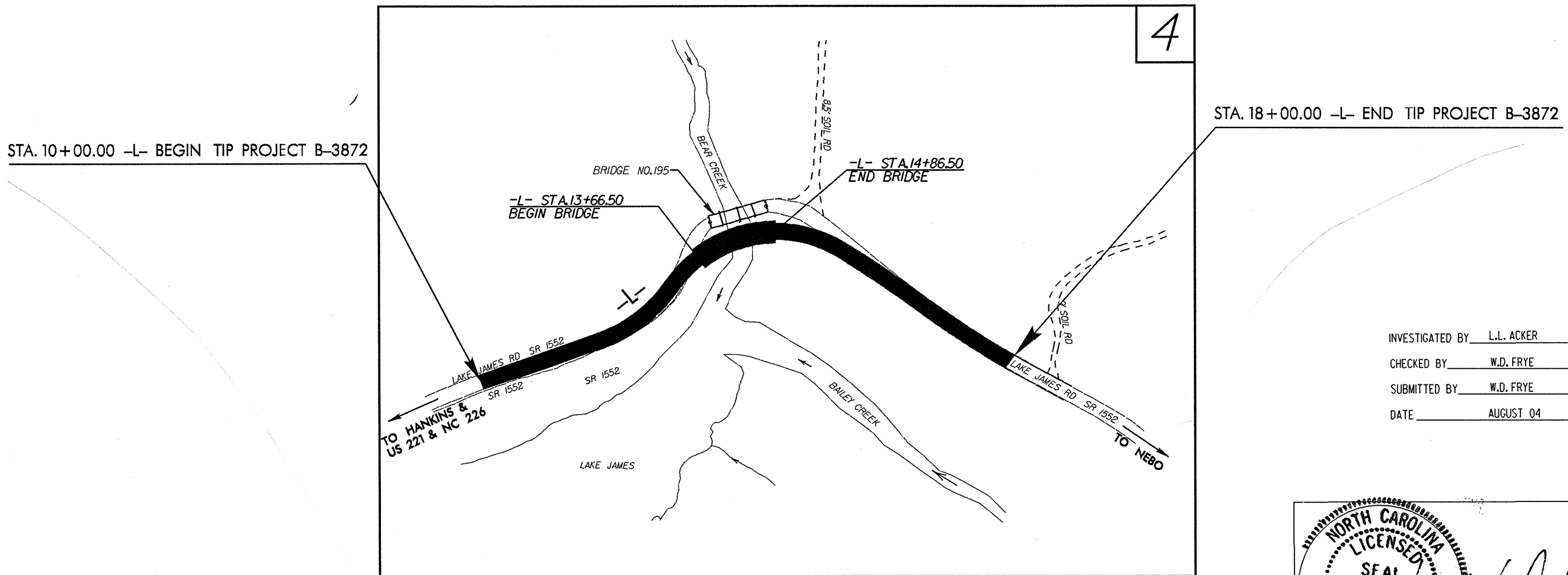
STATE	STATE PROJECT REFERENCE NO.	SHEET NO.	TOTAL SHEETS
N.C.	B-3872	1	
STATE PROJ. NO.	F.A. PROJ. NO.	DESCRIPTION	
33316.1.1	BRZ-1552(8)	PE	
33316.2.2	BRZ-1552(8)	RAW, UTIL	
33316.3.1	BRZ-1552(10)	CONST	

CAUTION NOTICE

THE SUBSURFACE INFORMATION AND THE SUBSURFACE INVESTIGATION ON WHICH IT IS BASED WAS MADE FOR THE PURPOSE OF STUDY, PLANNING AND DESIGN, AND NOT FOR CONSTRUCTION OR PAY PURPOSES. THE VARIOUS FIELD BORING LOGS, ROCK CORES, AND SOIL TEST DATA AVAILABLE MAY BE REVIEWED OR INSPECTED IN RALEIGH BY CONTACTING THE N.C. DEPARTMENT OF TRANSPORTATION, GEOTECHNICAL UNIT @ (919) 250-4088. NEITHER THE SUBSURFACE PLANS AND REPORTS, NOR THE FIELD BORING LOGS, ROCK CORES, OR SOIL TEST DATA IS PART OF THE CONTRACT.

GENERAL SOIL AND ROCK STRATA DESCRIPTIONS AND INDICATED BOUNDARIES ARE BASED ON A GEOTECHNICAL INTERPRETATION OF ALL AVAILABLE SUBSURFACE DATA AND MAY NOT NECESSARILY REFLECT THE ACTUAL SUBSURFACE CONDITIONS BETWEEN BORINGS OR BETWEEN SAMPLED STRATA WITHIN THE BOREHOLE. THE LABORATORY SAMPLE DATA AND THE IN SITU (ON-PLACE) TEST DATA CAN BE RELIED ON ONLY TO THE DEGREE OF RELIABILITY INHERENT IN THE STANDARD TEST METHOD. THE OBSERVED WATER LEVELS OR SOIL MOISTURE CONDITIONS INDICATED IN THE SUBSURFACE INVESTIGATIONS ARE AS RECORDED AT THE TIME OF THE INVESTIGATION. THESE WATER LEVELS OR SOIL MOISTURE CONDITIONS MAY VARY CONSIDERABLY WITH TIME ACCORDING TO CLIMATIC CONDITIONS INCLUDING TEMPERATURES, PRECIPITATION AND WIND, AS WELL AS OTHER NON-CLIMATIC FACTORS.

THE BIDDER OR CONTRACTOR IS CAUTIONED THAT DETAILS SHOWN ON THE SUBSURFACE PLANS ARE PRELIMINARY ONLY AND IN MANY CASES THE FINAL DESIGN DETAILS ARE DIFFERENT. FOR BIDDING AND CONSTRUCTION PURPOSES, REFER TO THE CONSTRUCTION PLANS AND DOCUMENTS FOR FINAL DESIGN INFORMATION ON THIS PROJECT. THE DEPARTMENT DOES NOT WARRANT OR GUARANTEE THE SUFFICIENCY OR ACCURACY OF THE INVESTIGATION MADE, NOR THE INTERPRETATIONS MADE OR OPINION OF THE DEPARTMENT AS TO THE TYPE OF MATERIALS AND CONDITIONS TO BE ENCOUNTERED. THE BIDDER OR CONTRACTOR IS CAUTIONED TO MAKE SUCH INDEPENDENT SUBSURFACE INVESTIGATIONS AS HE DEEMS NECESSARY TO SATISFY HIMSELF AS TO CONDITIONS TO BE ENCOUNTERED ON THIS PROJECT. THE CONTRACTOR SHALL HAVE NO CLAIM FOR ADDITIONAL COMPENSATION OR FOR AN EXTENSION OF TIME FOR ANY REASON RESULTING FROM THE ACTUAL CONDITIONS ENCOUNTERED AT THE SITE DIFFERING FROM THOSE INDICATED IN THE SUBSURFACE INFORMATION.

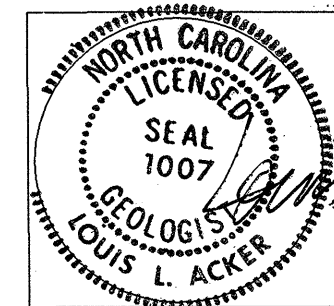


INVESTIGATED BY L.L. ACKER PERSONNEL T.B. DANIEL
 CHECKED BY W.D. FRYE L.A. LANKFORD
 SUBMITTED BY W.D. FRYE J.T. WILLIAMS
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C.J. COFFEY
G.K. ROSE

DRAWN BY: J.W. MANN, L.L. ACKER

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NOTE - BY HAVING REQUESTED THIS INFORMATION THE CONTRACTOR SPECIFICALLY WAIVES ANY CLAIMS FOR INCREASED COMPENSATION OR EXTENSION OF TIME BASED ON DIFFERENCES BETWEEN THE CONDITIONS INDICATED HEREIN AND THE ACTUAL CONDITIONS AT THE PROJECT SITE.



L. Ackers
 SIGNATURE

NORTH CAROLINA DEPARTMENT OF TRANSPORTATION
DIVISION OF HIGHWAYS
GEOTECHNICAL UNIT

ID	STATE PROJECT NO.	SHEET NO.	TOTAL SHEETS
B-3872	33316.1.1	2	27

SUBSURFACE INVESTIGATION

SOIL AND ROCK LEGEND, TERMS, SYMBOLS, AND ABBREVIATIONS

SOIL DESCRIPTION	GRADATION	ROCK DESCRIPTION	TERMS AND DEFINITIONS
SOIL IS CONSIDERED TO BE THE UNCONSOLIDATED, SEMI-CONSOLIDATED OR WEATHERED EARTH MATERIALS WHICH CAN BE PENETRATED WITH A CONTINUOUS FLIGHT POWER AUGER, AND WHICH YIELDS LESS THAN 100 BLOWS PER FOOT ACCORDING TO STANDARD PENETRATION TEST (AASHTO T206, ASTM D-1586). SOIL CLASSIFICATION IS BASED ON THE AASHTO SYSTEM AND BASIC DESCRIPTIONS GENERALLY SHALL INCLUDE: CONSISTENCY, COLOR, TEXTURE, MOISTURE, AASHTO CLASSIFICATION, AND OTHER PERTINENT FACTORS SUCH AS MINERALOGICAL COMPOSITION, ANGULARITY, STRUCTURE, PLASTICITY, ETC. EXAMPLE: VERY STIFF, GRAY SILTY CLAY, MOST WITH INTERBEDDED FINE SAND LAYERS, HIGHLY PLASTIC, A-7-6	WELL GRADED: INDICATES A GOOD REPRESENTATION OF PARTICLE SIZES FROM FINE TO COARSE UNIFORM. INDICATES THAT SOIL PARTICLES ARE ALL APPROXIMATELY THE SAME SIZE. (ALSO POORLY GRADED) GAP-GRADED: INDICATES A MIXTURE OF UNIFORM PARTICLES OF TWO OR MORE SIZES. ANGULARITY OF GRAINS THE ANGULARITY OR ROUNDNESS OF SOIL GRAINS ARE DESIGNATED BY THE TERMS: ANGULAR, SUBANGULAR, SUBROUNDED, OR ROUNDED.	HARD ROCK IS NON-COASTAL PLAIN MATERIAL THAT WHEN TESTED, WOULD YIELD SPT REFUSAL, AN INFERRED ROCK LINE INDICATES THE LEVEL AT WHICH NON-COASTAL PLAIN MATERIAL WOULD YIELD SPT REFUSAL. SPT REFUSAL IS PENETRATION BY A SPLIT SPOON SAMPLER EQUAL TO OR LESS THAN 0.1 FOOT PER 60 BLOWS. IN NON-COASTAL PLAIN MATERIAL, THE TRANSITION BETWEEN SOIL AND ROCK IS OFTEN REPRESENTED BY A ZONE OF WEATHERED ROCK. ROCK MATERIALS ARE TYPICALLY DIVIDED AS FOLLOWS: WEATHERED ROCK (WR) CRYSTALLINE ROCK (CR) NON-CRYSTALLINE ROCK (NCR) COASTAL PLAIN SEDIMENTARY ROCK (CP)	ALLUVIUM (ALLUV.) - SOILS WHICH HAVE BEEN TRANSPORTED BY WATER. AQUIFER - A WATER BEARING FORMATION OR STRATA. ARENACEOUS - APPLIED TO ROCKS THAT HAVE BEEN DERIVED FROM SAND OR THAT CONTAIN SAND. ARGILLACEOUS - APPLIED TO ALL ROCKS OR SUBSTANCES COMPOSED OF CLAY MINERALS, OR HAVING A NOTABLE PROPORTION OF CLAY IN THEIR COMPOSITION, AS SHALE, SLATE, ETC. ARTESIAN - GROUND WATER THAT IS UNDER SUFFICIENT PRESSURE TO RISE ABOVE THE LEVEL AT WHICH IS ENCOUNTERED, BUT WHICH DOES NOT NECESSARILY RISE TO OR ABOVE THE GROUND SURFACE. CALCAREOUS (CALC.) - SOILS WHICH CONTAIN APPRECIABLE AMOUNTS OF CALCIUM CARBONATE. COLLUVIUM - ROCK FRAGMENTS MIXED WITH SOIL DEPOSITED BY GRAVITY ON SLOPE OR AT BOTTOM OF SLOPE. CORE RECOVERY (REC.) - TOTAL LENGTH OF ALL MATERIAL RECOVERED IN THE CORE BARREL DIVIDED BY TOTAL LENGTH OF CORE RUN AND EXPRESSED AS A PERCENTAGE. DIKE - A TABULAR BODY OF IGNEOUS ROCK THAT CUTS ACROSS THE STRUCTURE OF ADJACENT ROCKS OR CUTS MASSIVE ROCK. DIP - THE ANGLE AT WHICH A STRATUM OR ANY PLANAR FEATURE IS INCLINED FROM THE HORIZONTAL. DIP DIRECTION (DIP AZIMUTH) - THE DIRECTION OR BEARING OF THE HORIZONTAL TRACE OF THE LINE OF DIP, MEASURED CLOCKWISE FROM NORTH. FAULT - A FRACTURE OR FRACTURE ZONE ALONG WHICH THERE HAS BEEN DISPLACEMENT OF THE SIDES RELATIVE TO ONE ANOTHER PARALLEL TO THE FRACTURE. FISSILE - A PROPERTY OF SPLITTING ALONG CLOSELY SPACED PARALLEL PLANES. FLOAT - ROCK FRAGMENTS ON SURFACE NEAR THEIR ORIGINAL POSITION AND DISLOGGED FROM PARENT MATERIAL. FLOOD PLAIN (F.P.) - LAND BORDERING A STREAM, BUILT OF SEDIMENTS DEPOSITED BY THE STREAM. FORMATION (FM.) - A MAPPABLE GEOLOGIC UNIT THAT CAN BE RECOGNIZED AND TRACED IN THE FIELD. JOINT - FRACTURE IN ROCK ALONG WHICH NO APPRECIABLE MOVEMENT HAS OCCURRED. LEDGE - A SHELF-LIKE RIDGE OR PROJECTION OF ROCK WHOSE THICKNESS IS SMALL COMPARED TO ITS LATERAL EXTENT. LENS - A BODY OF SOIL OR ROCK THAT THINS OUT IN ONE OR MORE DIRECTIONS. MOTTLED (MOT.) - IRREGULARLY MARKED WITH SPOTS OF DIFFERENT COLORS. MOTTLING IN SOILS USUALLY INDICATES POOR AERATION AND LACK OF GOOD DRAINAGE. PERCHED WATER - WATER MAINTAINED ABOVE THE NORMAL GROUND WATER LEVEL BY THE PRESENCE OF AN INTERVENING IMPERVIOUS STRATUM. RESIDUAL SOIL - SOIL FORMED IN PLACE BY THE WEATHERING OF ROCK. ROCK QUALITY DESIGNATION (R.Q.D.) - A MEASURE OF ROCK QUALITY DESCRIBED BY: TOTAL LENGTH OF ROCK SEGMENTS EQUAL TO OR GREATER THAN 4 INCHES DIVIDED BY THE TOTAL LENGTH OF CORE RUN AND EXPRESSED AS A PERCENTAGE.
SOIL LEGEND AND AASHTO CLASSIFICATION GENERAL CLASS. GRANULAR MATERIALS (<5% PASSING #200) SILT-CLAY MATERIALS (>5% PASSING #200) ORGANIC MATERIALS GROUP CLASS. A-1 A-3 A-2 A-4 A-5 A-6 A-7 A-1, A-2 A-3 A-4, A-5 A-6, A-7 SYMBOL [Diagrams showing soil symbols for various groups]	MINERALOGICAL COMPOSITION MINERAL NAMES SUCH AS QUARTZ, FELDSPAR, MICA, TALC, KAOLIN, ETC. ARE USED IN DESCRIPTIONS WHENEVER THEY ARE CONSIDERED OF SIGNIFICANCE. COMPRESSIBILITY SLIGHTLY COMPRESSIBLE LIQUID LIMIT LESS THAN 30 MODERATELY COMPRESSIBLE LIQUID LIMIT 31-50 HIGHLY COMPRESSIBLE LIQUID LIMIT GREATER THAN 50 PERCENTAGE OF MATERIAL ORGANIC MATERIAL GRANULAR SILT-CLAY OTHER MATERIAL TRACE OF ORGANIC MATTER 2 - 3% 3 - 5% TRACE 1 - 10% LITTLE ORGANIC MATTER 3 - 5% 5 - 12% LITTLE 10 - 20% MODERATELY ORGANIC 5 - 10% 12 - 20% SOME 20 - 35% HIGHLY ORGANIC >10% >20% HIGHLY 35% AND ABOVE	WEATHERING FRESH ROCK FRESH, CRYSTALS BRIGHT, FEW JOINTS MAY SHOW SLIGHT STAINING, ROCK RINGS UNDER HAMMER IF CRYSTALLINE. VERY SLIGHT (V. SL.) ROCK GENERALLY FRESH, JOINTS STAINED, SOME JOINTS MAY SHOW THIN CLAY COATINGS IF OPEN, CRYSTALS ON A BROKEN SPECIMEN FACE SHINE BRIGHTLY. ROCK RINGS UNDER HAMMER BLOWS IF OF A CRYSTALLINE NATURE. SLIGHT (SL.) ROCK GENERALLY FRESH, JOINTS STAINED AND DISCOLORATION EXTENDS INTO ROCK UP TO 1 INCH. OPEN JOINTS MAY CONTAIN CLAY. IN GRANITOID ROCKS SOME OCCASIONAL FELDSPAR CRYSTALS ARE DULL AND DISCOLORED. CRYSTALLINE ROCKS RING UNDER HAMMER BLOWS. MODERATE (MOD.) SIGNIFICANT PORTIONS OF ROCK SHOW DISCOLORATION AND WEATHERING EFFECTS. IN GRANITOID ROCKS, MOST FELDSPARS ARE DULL AND DISCOLORED, SOME SHOW CLAY. ROCK HAS DULL SOUND UNDER HAMMER BLOWS AND SHOWS SIGNIFICANT LOSS OF STRENGTH AS COMPARED WITH FRESH ROCK. MODERATELY SEVERE (MOD. SEV.) ALL ROCK EXCEPT QUARTZ DISCOLORED OR STAINED. IN GRANITOID ROCKS, ALL FELDSPARS DULL AND DISCOLORED AND A MAJORITY SHOW KAOLINIZATION. ROCK SHOWS SEVERE LOSS OF STRENGTH AND CAN BE EXCAVATED WITH A GEOLOGIST'S PICK. ROCK GIVES 'CLUNK' SOUND WHEN STRUCK. IF TESTED, WOULD YIELD SPT REFUSAL. SEVERE (SEV.) ALL ROCKS EXCEPT QUARTZ DISCOLORED OR STAINED. ROCK FABRIC CLEAR AND EVIDENT BUT REDUCED IN STRENGTH TO STRONG SOIL. IN GRANITOID ROCKS ALL FELDSPARS ARE KAOLINIZED TO SOME EXTENT. SOME FRAGMENTS OF STRONG ROCK USUALLY REMAIN. IF TESTED, YIELDS SPT N VALUES > 100 BPF. VERY SEVERE (V. SEV.) ALL ROCK EXCEPT QUARTZ DISCOLORED OR STAINED. ROCK FABRIC ELEMENTS ARE DISCERNIBLE BUT THE MASS IS EFFECTIVELY REDUCED TO SOIL STATUS, WITH ONLY FRAGMENTS OF STRONG ROCK REMAINING. SAPROLITE IS AN EXAMPLE OF ROCK WEATHERED TO A DEGREE SUCH THAT ONLY MINOR VESTIGES OF THE ORIGINAL ROCK FABRIC REMAIN. IF TESTED, YIELDS SPT N VALUES < 100 BPF. COMPLETE ROCK REDUCED TO SOIL, ROCK FABRIC NOT DISCERNIBLE, OR DISCERNIBLE ONLY IN SMALL AND SCATTERED CONCENTRATIONS. QUARTZ MAY BE PRESENT AS DIKES OR STRINGERS. SAPROLITE IS ALSO AN EXAMPLE.	WEATHERING FRESH ROCK FRESH, CRYSTALS BRIGHT, FEW JOINTS MAY SHOW SLIGHT STAINING, ROCK RINGS UNDER HAMMER IF CRYSTALLINE. VERY SLIGHT (V. SL.) ROCK GENERALLY FRESH, JOINTS STAINED, SOME JOINTS MAY SHOW THIN CLAY COATINGS IF OPEN, CRYSTALS ON A BROKEN SPECIMEN FACE SHINE BRIGHTLY. ROCK RINGS UNDER HAMMER BLOWS IF OF A CRYSTALLINE NATURE. SLIGHT (SL.) ROCK GENERALLY FRESH, JOINTS STAINED AND DISCOLORATION EXTENDS INTO ROCK UP TO 1 INCH. OPEN JOINTS MAY CONTAIN CLAY. IN GRANITOID ROCKS SOME OCCASIONAL FELDSPAR CRYSTALS ARE DULL AND DISCOLORED. CRYSTALLINE ROCKS RING UNDER HAMMER BLOWS. MODERATE (MOD.) SIGNIFICANT PORTIONS OF ROCK SHOW DISCOLORATION AND WEATHERING EFFECTS. IN GRANITOID ROCKS, MOST FELDSPARS ARE DULL AND DISCOLORED, SOME SHOW CLAY. ROCK HAS DULL SOUND UNDER HAMMER BLOWS AND SHOWS SIGNIFICANT LOSS OF STRENGTH AS COMPARED WITH FRESH ROCK. MODERATELY SEVERE (MOD. SEV.) ALL ROCK EXCEPT QUARTZ DISCOLORED OR STAINED. IN GRANITOID ROCKS, ALL FELDSPARS DULL AND DISCOLORED AND A MAJORITY SHOW KAOLINIZATION. ROCK SHOWS SEVERE LOSS OF STRENGTH AND CAN BE EXCAVATED WITH A GEOLOGIST'S PICK. ROCK GIVES 'CLUNK' SOUND WHEN STRUCK. IF TESTED, WOULD YIELD SPT REFUSAL. SEVERE (SEV.) ALL ROCKS EXCEPT QUARTZ DISCOLORED OR STAINED. ROCK FABRIC CLEAR AND EVIDENT BUT REDUCED IN STRENGTH TO STRONG SOIL. IN GRANITOID ROCKS ALL FELDSPARS ARE KAOLINIZED TO SOME EXTENT. SOME FRAGMENTS OF STRONG ROCK USUALLY REMAIN. IF TESTED, YIELDS SPT N VALUES > 100 BPF. VERY SEVERE (V. SEV.) ALL ROCK EXCEPT QUARTZ DISCOLORED OR STAINED. ROCK FABRIC ELEMENTS ARE DISCERNIBLE BUT THE MASS IS EFFECTIVELY REDUCED TO SOIL STATUS, WITH ONLY FRAGMENTS OF STRONG ROCK REMAINING. SAPROLITE IS AN EXAMPLE OF ROCK WEATHERED TO A DEGREE SUCH THAT ONLY MINOR VESTIGES OF THE ORIGINAL ROCK FABRIC REMAIN. IF TESTED, YIELDS SPT N VALUES < 100 BPF. COMPLETE ROCK REDUCED TO SOIL, ROCK FABRIC NOT DISCERNIBLE, OR DISCERNIBLE ONLY IN SMALL AND SCATTERED CONCENTRATIONS. QUARTZ MAY BE PRESENT AS DIKES OR STRINGERS. SAPROLITE IS ALSO AN EXAMPLE.
CONSISTENCY OR DENSITY PRIMARY SOIL TYPE COMPACTNESS OR CONSISTENCY RANGE OF STANDARD PENETRATION RESISTANCE (N-VALUE) RANGE OF UNCONFINED COMPRESSIVE STRENGTH (TONS/FT ²) GENERALLY GRANULAR MATERIAL (NON-COHESIVE) VERY LOOSE LOOSE MEDIUM DENSE DENSE VERY DENSE <4 4 TO 10 10 TO 30 30 TO 50 >50 N/A GENERALLY SILT-CLAY MATERIAL (COHESIVE) VERY SOFT SOFT MEDIUM STIFF STIFF VERY STIFF HARD <2 2 TO 4 4 TO 8 8 TO 15 15 TO 30 >30 <0.25 0.25 TO 0.5 0.5 TO 1 1 TO 2 2 TO 4 >4	GROUND WATER WATER LEVEL IN BORE HOLE IMMEDIATELY AFTER DRILLING. STATIC WATER LEVEL AFTER 24 HOURS. PERCHED WATER, SATURATED ZONE OR WATER BEARING STRATA SPRING OR SEEPAGE MISCELLANEOUS SYMBOLS ROADWAY EMBANKMENT WITH SOIL DESCRIPTION SOIL SYMBOL ARTIFICIAL FILL OTHER THAN ROADWAY EMBANKMENTS INFERRED SOIL BOUNDARIES INFERRED ROCK LINE ALLUVIAL SOIL BOUNDARY DIP/DIP DIRECTION OF ROCK STRUCTURES SOUNDING ROD SPT TEST BORING AUGER BORING CORE BORING MONITORING WELL PIEZOMETER INSTALLATION SLOPE INDICATOR INSTALLATION SPT N-VALUE SPT REFUSAL	ROCK HARDNESS VERY HARD CANNOT BE SCRATCHED BY KNIFE OR SHARP PICK. BREAKING OF HAND SPECIMENS REQUIRES SEVERAL HARD BLOWS OF THE GEOLOGIST'S PICK. HARD CAN BE SCRATCHED BY KNIFE OR PICK ONLY WITH DIFFICULTY. HARD HAMMER BLOWS REQUIRED TO DETACH HAND SPECIMEN. MODERATELY HARD CAN BE SCRATCHED BY KNIFE OR PICK. GOUGES OR GROOVES TO 0.25 INCHES DEEP CAN BE EXCAVATED BY HARD BLOW OF A GEOLOGIST'S PICK. HAND SPECIMENS CAN BE DETACHED BY MODERATE BLOWS. MEDIUM HARD CAN BE GROOVED OR GOUGED 0.05 INCHES DEEP BY FIRM PRESSURE OF KNIFE OR PICK POINT. CAN BE EXCAVATED IN SMALL CHIPS TO PEICES 1 INCH MAXIMUM SIZE BY HARD BLOWS OF THE POINT OF A GEOLOGIST'S PICK. SOFT CAN BE GROVED OR GOUGED READILY BY KNIFE OR PICK. CAN BE EXCAVATED IN FRAGMENTS FROM CHIPS TO SEVERAL INCHES IN SIZE BY MODERATE BLOWS OF A PICK POINT. SMALL, THIN PIECES CAN BE BROKEN BY FINGER PRESSURE. VERY SOFT CAN BE CARVED WITH KNIFE. CAN BE EXCAVATED READILY WITH POINT OF PICK. PIECES 1 INCH OR MORE IN THICKNESS CAN BE BROKEN BY FINGER PRESSURE. CAN BE SCRATCHED READILY BY FINGERNAIL.	ROCK HARDNESS SAPROLITE (SAP.) - RESIDUAL SOIL WHICH RETAINS THE RELIC STRUCTURE OR FABRIC OF THE PARENT ROCK. SILL - AN INTRUSIVE BODY OF IGNEOUS ROCK OF APPROXIMATELY UNIFORM THICKNESS AND RELATIVELY THIN COMPARED WITH ITS LATERAL EXTENT, WHICH HAS BEEN EMPLACED PARALLEL TO THE BEDDING OR SCHISTOSITY OF THE INTRUDED ROCKS. SLICKENSIDE - POLISHED AND STRIATED SURFACE THAT RESULTS FROM FRICTION ALONG A FAULT OR SLIP PLANE. STANDARD PENETRATION TEST (PENETRATION RESISTANCE) (SPT) - NUMBER OF BLOWS (N OR B.P.F.) OF A 140 LB. HAMMER FALLING 30 INCHES REQUIRED TO PRODUCE A PENETRATION OF 1 FOOT INTO SOIL WITH A 2 INCH OUTSIDE DIAMETER SPLIT SPOON SAMPLER. SPT REFUSAL IS LESS THAN 0.1 FOOT PENETRATION WITH 60 BLOWS. STRATA CORE RECOVERY (SREC.) - TOTAL LENGTH OF STRATA MATERIAL RECOVERED DIVIDED BY TOTAL LENGTH OF STRATUM AND EXPRESSED AS A PERCENTAGE. STRATA ROCK QUALITY DESIGNATION (S.R.Q.D.) - A MEASURE OF ROCK QUALITY DESCRIBED BY: TOTAL LENGTH OF ROCK SEGMENTS WITHIN A STRATUM EQUAL TO OR GREATER THAN 4 INCHES DIVIDED BY THE TOTAL LENGTH OF STRATA AND EXPRESSED AS A PERCENTAGE. TOPSOIL (T.S.) - SURFACE SOILS USUALLY CONTAINING ORGANIC MATTER.
TEXTURE OR GRAIN SIZE U.S. STD. SIEVE SIZE OPENING (MM) 4 10 40 60 200 270 4.76 2.0 0.42 0.25 0.075 0.053 BOULDER (BLDR.) COBBLE (COB.) GRAVEL (GR.) COARSE SAND (CSE. SD.) FINE SAND (F. SD.) SILT (SL.) CLAY (CL.) GRAIN SIZE MM 305 75 2.0 0.25 0.05 0.005 IN. 12' 3' 2.0 0.25 0.05 0.005	ABBREVIATIONS AR - AUGER REFUSAL BT - BORING TERMINATED CL - CLAY CPT - CONE PENETRATION TEST CSE. - COARSE DMT - DILATOMETER TEST DPT - DYNAMIC PENETRATION TEST e - VOID RATIO F. - FINE FOSS. - FOSSILIFEROUS FRAC. - FRACTURED FRAGS. - FRAGMENTS MED. - MEDIUM PMT - PRESSUREMETER TEST SD. - SAND, SANDY SL. - SILT, SILTY SLI. - SLIGHTLY TCR - TRICONE REFUSAL γ - UNIT WEIGHT γ _d - DRY UNIT WEIGHT w - MOISTURE CONTENT v. - VERY VST - VANE SHEAR TEST	ROCK HARDNESS VERY HARD CANNOT BE SCRATCHED BY KNIFE OR SHARP PICK. BREAKING OF HAND SPECIMENS REQUIRES SEVERAL HARD BLOWS OF THE GEOLOGIST'S PICK. HARD CAN BE SCRATCHED BY KNIFE OR PICK ONLY WITH DIFFICULTY. HARD HAMMER BLOWS REQUIRED TO DETACH HAND SPECIMEN. MODERATELY HARD CAN BE SCRATCHED BY KNIFE OR PICK. GOUGES OR GROOVES TO 0.25 INCHES DEEP CAN BE EXCAVATED BY HARD BLOW OF A GEOLOGIST'S PICK. HAND SPECIMENS CAN BE DETACHED BY MODERATE BLOWS. MEDIUM HARD CAN BE GROOVED OR GOUGED 0.05 INCHES DEEP BY FIRM PRESSURE OF KNIFE OR PICK POINT. CAN BE EXCAVATED IN SMALL CHIPS TO PEICES 1 INCH MAXIMUM SIZE BY HARD BLOWS OF THE POINT OF A GEOLOGIST'S PICK. SOFT CAN BE GROVED OR GOUGED READILY BY KNIFE OR PICK. CAN BE EXCAVATED IN FRAGMENTS FROM CHIPS TO SEVERAL INCHES IN SIZE BY MODERATE BLOWS OF A PICK POINT. SMALL, THIN PIECES CAN BE BROKEN BY FINGER PRESSURE. VERY SOFT CAN BE CARVED WITH KNIFE. CAN BE EXCAVATED READILY WITH POINT OF PICK. PIECES 1 INCH OR MORE IN THICKNESS CAN BE BROKEN BY FINGER PRESSURE. CAN BE SCRATCHED READILY BY FINGERNAIL.	ROCK HARDNESS SAPROLITE (SAP.) - RESIDUAL SOIL WHICH RETAINS THE RELIC STRUCTURE OR FABRIC OF THE PARENT ROCK. SILL - AN INTRUSIVE BODY OF IGNEOUS ROCK OF APPROXIMATELY UNIFORM THICKNESS AND RELATIVELY THIN COMPARED WITH ITS LATERAL EXTENT, WHICH HAS BEEN EMPLACED PARALLEL TO THE BEDDING OR SCHISTOSITY OF THE INTRUDED ROCKS. SLICKENSIDE - POLISHED AND STRIATED SURFACE THAT RESULTS FROM FRICTION ALONG A FAULT OR SLIP PLANE. STANDARD PENETRATION TEST (PENETRATION RESISTANCE) (SPT) - NUMBER OF BLOWS (N OR B.P.F.) OF A 140 LB. HAMMER FALLING 30 INCHES REQUIRED TO PRODUCE A PENETRATION OF 1 FOOT INTO SOIL WITH A 2 INCH OUTSIDE DIAMETER SPLIT SPOON SAMPLER. SPT REFUSAL IS LESS THAN 0.1 FOOT PENETRATION WITH 60 BLOWS. STRATA CORE RECOVERY (SREC.) - TOTAL LENGTH OF STRATA MATERIAL RECOVERED DIVIDED BY TOTAL LENGTH OF STRATUM AND EXPRESSED AS A PERCENTAGE. STRATA ROCK QUALITY DESIGNATION (S.R.Q.D.) - A MEASURE OF ROCK QUALITY DESCRIBED BY: TOTAL LENGTH OF ROCK SEGMENTS WITHIN A STRATUM EQUAL TO OR GREATER THAN 4 INCHES DIVIDED BY THE TOTAL LENGTH OF STRATA AND EXPRESSED AS A PERCENTAGE. TOPSOIL (T.S.) - SURFACE SOILS USUALLY CONTAINING ORGANIC MATTER.
SOIL MOISTURE - CORRELATION OF TERMS SOIL MOISTURE SCALE (ATTERBERG LIMITS) FIELD MOISTURE DESCRIPTION GUIDE FOR FIELD MOISTURE DESCRIPTION LL - LIQUID LIMIT USUALLY LIQUID; VERY WET, USUALLY FROM BELOW THE GROUND WATER TABLE PL - PLASTIC LIMIT SEMISOLID; REQUIRES DRYING TO ATTAIN OPTIMUM MOISTURE OM - OPTIMUM MOISTURE SOLID; AT OR NEAR OPTIMUM MOISTURE SL - SHRINKAGE LIMIT REQUIRES ADDITIONAL WATER TO ATTAIN OPTIMUM MOISTURE	EQUIPMENT USED ON SUBJECT PROJECT DRILL UNITS: MOBILE B- BK-51 CME-45 CME-550 PORTABLE HOIST OTHER OTHER ADVANCING TOOLS: CLAY BITS 6" CONTINUOUS FLIGHT AUGER 8" HOLLOW AUGERS HARD FACED FINGER BITS TUNG-CARBIDE INSERTS CASING w/ ADVANCER TRICONE * STEEL TEETH TRICONE * TUNG-CARB. CORE BIT OTHER HAMMER TYPE: AUTOMATIC MANUAL CORE SIZE: B N H HAND TOOLS: POST HOLE DIGGER HAND AUGER SOUNDING ROD VANE SHEAR TEST OTHER	ROCK HARDNESS VERY HARD CANNOT BE SCRATCHED BY KNIFE OR SHARP PICK. BREAKING OF HAND SPECIMENS REQUIRES SEVERAL HARD BLOWS OF THE GEOLOGIST'S PICK. HARD CAN BE SCRATCHED BY KNIFE OR PICK ONLY WITH DIFFICULTY. HARD HAMMER BLOWS REQUIRED TO DETACH HAND SPECIMEN. MODERATELY HARD CAN BE SCRATCHED BY KNIFE OR PICK. GOUGES OR GROOVES TO 0.25 INCHES DEEP CAN BE EXCAVATED BY HARD BLOW OF A GEOLOGIST'S PICK. HAND SPECIMENS CAN BE DETACHED BY MODERATE BLOWS. MEDIUM HARD CAN BE GROOVED OR GOUGED 0.05 INCHES DEEP BY FIRM PRESSURE OF KNIFE OR PICK POINT. CAN BE EXCAVATED IN SMALL CHIPS TO PEICES 1 INCH MAXIMUM SIZE BY HARD BLOWS OF THE POINT OF A GEOLOGIST'S PICK. SOFT CAN BE GROVED OR GOUGED READILY BY KNIFE OR PICK. CAN BE EXCAVATED IN FRAGMENTS FROM CHIPS TO SEVERAL INCHES IN SIZE BY MODERATE BLOWS OF A PICK POINT. SMALL, THIN PIECES CAN BE BROKEN BY FINGER PRESSURE. VERY SOFT CAN BE CARVED WITH KNIFE. CAN BE EXCAVATED READILY WITH POINT OF PICK. PIECES 1 INCH OR MORE IN THICKNESS CAN BE BROKEN BY FINGER PRESSURE. CAN BE SCRATCHED READILY BY FINGERNAIL.	ROCK HARDNESS SAPROLITE (SAP.) - RESIDUAL SOIL WHICH RETAINS THE RELIC STRUCTURE OR FABRIC OF THE PARENT ROCK. SILL - AN INTRUSIVE BODY OF IGNEOUS ROCK OF APPROXIMATELY UNIFORM THICKNESS AND RELATIVELY THIN COMPARED WITH ITS LATERAL EXTENT, WHICH HAS BEEN EMPLACED PARALLEL TO THE BEDDING OR SCHISTOSITY OF THE INTRUDED ROCKS. SLICKENSIDE - POLISHED AND STRIATED SURFACE THAT RESULTS FROM FRICTION ALONG A FAULT OR SLIP PLANE. STANDARD PENETRATION TEST (PENETRATION RESISTANCE) (SPT) - NUMBER OF BLOWS (N OR B.P.F.) OF A 140 LB. HAMMER FALLING 30 INCHES REQUIRED TO PRODUCE A PENETRATION OF 1 FOOT INTO SOIL WITH A 2 INCH OUTSIDE DIAMETER SPLIT SPOON SAMPLER. SPT REFUSAL IS LESS THAN 0.1 FOOT PENETRATION WITH 60 BLOWS. STRATA CORE RECOVERY (SREC.) - TOTAL LENGTH OF STRATA MATERIAL RECOVERED DIVIDED BY TOTAL LENGTH OF STRATUM AND EXPRESSED AS A PERCENTAGE. STRATA ROCK QUALITY DESIGNATION (S.R.Q.D.) - A MEASURE OF ROCK QUALITY DESCRIBED BY: TOTAL LENGTH OF ROCK SEGMENTS WITHIN A STRATUM EQUAL TO OR GREATER THAN 4 INCHES DIVIDED BY THE TOTAL LENGTH OF STRATA AND EXPRESSED AS A PERCENTAGE. TOPSOIL (T.S.) - SURFACE SOILS USUALLY CONTAINING ORGANIC MATTER.
PLASTICITY NONPLASTIC LOW PLASTICITY MED. PLASTICITY HIGH PLASTICITY PLASTICITY INDEX (PI) DRY STRENGTH VERY LOW SLIGHT MEDIUM HIGH 0-5 6-15 16-25 26 OR MORE	FRACTURE SPACING TERM SPACING VERY WIDE MORE THAN 10 FEET WIDE 3 TO 10 FEET MODERATELY CLOSE 1 TO 3 FEET CLOSE 0.16 TO 1 FEET VERY CLOSE LESS THAN 0.16 FEET	ROCK HARDNESS VERY HARD CANNOT BE SCRATCHED BY KNIFE OR SHARP PICK. BREAKING OF HAND SPECIMENS REQUIRES SEVERAL HARD BLOWS OF THE GEOLOGIST'S PICK. HARD CAN BE SCRATCHED BY KNIFE OR PICK ONLY WITH DIFFICULTY. HARD HAMMER BLOWS REQUIRED TO DETACH HAND SPECIMEN. MODERATELY HARD CAN BE SCRATCHED BY KNIFE OR PICK. GOUGES OR GROOVES TO 0.25 INCHES DEEP CAN BE EXCAVATED BY HARD BLOW OF A GEOLOGIST'S PICK. HAND SPECIMENS CAN BE DETACHED BY MODERATE BLOWS. MEDIUM HARD CAN BE GROOVED OR GOUGED 0.05 INCHES DEEP BY FIRM PRESSURE OF KNIFE OR PICK POINT. CAN BE EXCAVATED IN SMALL CHIPS TO PEICES 1 INCH MAXIMUM SIZE BY HARD BLOWS OF THE POINT OF A GEOLOGIST'S PICK. SOFT CAN BE GROVED OR GOUGED READILY BY KNIFE OR PICK. CAN BE EXCAVATED IN FRAGMENTS FROM CHIPS TO SEVERAL INCHES IN SIZE BY MODERATE BLOWS OF A PICK POINT. SMALL, THIN PIECES CAN BE BROKEN BY FINGER PRESSURE. VERY SOFT CAN BE CARVED WITH KNIFE. CAN BE EXCAVATED READILY WITH POINT OF PICK. PIECES 1 INCH OR MORE IN THICKNESS CAN BE BROKEN BY FINGER PRESSURE. CAN BE SCRATCHED READILY BY FINGERNAIL.	ROCK HARDNESS SAPROLITE (SAP.) - RESIDUAL SOIL WHICH RETAINS THE RELIC STRUCTURE OR FABRIC OF THE PARENT ROCK. SILL - AN INTRUSIVE BODY OF IGNEOUS ROCK OF APPROXIMATELY UNIFORM THICKNESS AND RELATIVELY THIN COMPARED WITH ITS LATERAL EXTENT, WHICH HAS BEEN EMPLACED PARALLEL TO THE BEDDING OR SCHISTOSITY OF THE INTRUDED ROCKS. SLICKENSIDE - POLISHED AND STRIATED SURFACE THAT RESULTS FROM FRICTION ALONG A FAULT OR SLIP PLANE. STANDARD PENETRATION TEST (PENETRATION RESISTANCE) (SPT) - NUMBER OF BLOWS (N OR B.P.F.) OF A 140 LB. HAMMER FALLING 30 INCHES REQUIRED TO PRODUCE A PENETRATION OF 1 FOOT INTO SOIL WITH A 2 INCH OUTSIDE DIAMETER SPLIT SPOON SAMPLER. SPT REFUSAL IS LESS THAN 0.1 FOOT PENETRATION WITH 60 BLOWS. STRATA CORE RECOVERY (SREC.) - TOTAL LENGTH OF STRATA MATERIAL RECOVERED DIVIDED BY TOTAL LENGTH OF STRATUM AND EXPRESSED AS A PERCENTAGE. STRATA ROCK QUALITY DESIGNATION (S.R.Q.D.) - A MEASURE OF ROCK QUALITY DESCRIBED BY: TOTAL LENGTH OF ROCK SEGMENTS WITHIN A STRATUM EQUAL TO OR GREATER THAN 4 INCHES DIVIDED BY THE TOTAL LENGTH OF STRATA AND EXPRESSED AS A PERCENTAGE. TOPSOIL (T.S.) - SURFACE SOILS USUALLY CONTAINING ORGANIC MATTER.
COLOR DESCRIPTIONS MAY INCLUDE COLOR OR COLOR COMBINATIONS (TAN, RED, YEL-BRN, BLUE-GRAY) MODIFIERS SUCH AS LIGHT, DARK, STREAKED, ETC. ARE USED TO DESCRIBE APPEARANCE.	BEDDING TERM THICKNESS VERY THICKLY BEDDED > 4 FEET THICKLY BEDDED 1.5 - 4 FEET THINLY BEDDED 0.16 - 1.5 FEET VERY THINLY BEDDED 0.03 - 0.16 FEET THICKLY LAMINATED 0.008 - 0.03 FEET THINLY LAMINATED < 0.008 FEET	ROCK HARDNESS VERY HARD CANNOT BE SCRATCHED BY KNIFE OR SHARP PICK. BREAKING OF HAND SPECIMENS REQUIRES SEVERAL HARD BLOWS OF THE GEOLOGIST'S PICK. HARD CAN BE SCRATCHED BY KNIFE OR PICK ONLY WITH DIFFICULTY. HARD HAMMER BLOWS REQUIRED TO DETACH HAND SPECIMEN. MODERATELY HARD CAN BE SCRATCHED BY KNIFE OR PICK. GOUGES OR GROOVES TO 0.25 INCHES DEEP CAN BE EXCAVATED BY HARD BLOW OF A GEOLOGIST'S PICK. HAND SPECIMENS CAN BE DETACHED BY MODERATE BLOWS. MEDIUM HARD CAN BE GROOVED OR GOUGED 0.05 INCHES DEEP BY FIRM PRESSURE OF KNIFE OR PICK POINT. CAN BE EXCAVATED IN SMALL CHIPS TO PEICES 1 INCH MAXIMUM SIZE BY HARD BLOWS OF THE POINT OF A GEOLOGIST'S PICK. SOFT CAN BE GROVED OR GOUGED READILY BY KNIFE OR PICK. CAN BE EXCAVATED IN FRAGMENTS FROM CHIPS TO SEVERAL INCHES IN SIZE BY MODERATE BLOWS OF A PICK POINT. SMALL, THIN PIECES CAN BE BROKEN BY FINGER PRESSURE. VERY SOFT CAN BE CARVED WITH KNIFE. CAN BE EXCAVATED READILY WITH POINT OF PICK. PIECES 1 INCH OR MORE IN THICKNESS CAN BE BROKEN BY FINGER PRESSURE. CAN BE SCRATCHED READILY BY FINGERNAIL.	ROCK HARDNESS SAPROLITE (SAP.) - RESIDUAL SOIL WHICH RETAINS THE RELIC STRUCTURE OR FABRIC OF THE PARENT ROCK. SILL - AN INTRUSIVE BODY OF IGNEOUS ROCK OF APPROXIMATELY UNIFORM THICKNESS AND RELATIVELY THIN COMPARED WITH ITS LATERAL EXTENT, WHICH HAS BEEN EMPLACED PARALLEL TO THE BEDDING OR SCHISTOSITY OF THE INTRUDED ROCKS. SLICKENSIDE - POLISHED AND STRIATED SURFACE THAT RESULTS FROM FRICTION ALONG A FAULT OR SLIP PLANE. STANDARD PENETRATION TEST (PENETRATION RESISTANCE) (SPT) - NUMBER OF BLOWS (N OR B.P.F.) OF A 140 LB. HAMMER FALLING 30 INCHES REQUIRED TO PRODUCE A PENETRATION OF 1 FOOT INTO SOIL WITH A 2 INCH OUTSIDE DIAMETER SPLIT SPOON SAMPLER. SPT REFUSAL IS LESS THAN 0.1 FOOT PENETRATION WITH 60 BLOWS. STRATA CORE RECOVERY (SREC.) - TOTAL LENGTH OF STRATA MATERIAL RECOVERED DIVIDED BY TOTAL LENGTH OF STRATUM AND EXPRESSED AS A PERCENTAGE. STRATA ROCK QUALITY DESIGNATION (S.R.Q.D.) - A MEASURE OF ROCK QUALITY DESCRIBED BY: TOTAL LENGTH OF ROCK SEGMENTS WITHIN A STRATUM EQUAL TO OR GREATER THAN 4 INCHES DIVIDED BY THE TOTAL LENGTH OF STRATA AND EXPRESSED AS A PERCENTAGE. TOPSOIL (T.S.) - SURFACE SOILS USUALLY CONTAINING ORGANIC MATTER.
	INDURATION FOR SEDIMENTARY ROCKS, INDURATION IS THE HARDENING OF THE MATERIAL BY CEMENTING, HEAT, PRESSURE, ETC. FRIABLE RUBBING WITH FINGER FREES NUMEROUS GRAINS; GENTLE BLOW BY HAMMER DISINTEGRATES SAMPLE. MODERATELY INDURATED GRAINS CAN BE SEPARATED FROM SAMPLE WITH STEEL PROBE; BREAKS EASILY WHEN HIT WITH HAMMER. INDURATED GRAINS ARE DIFFICULT TO SEPARATE WITH STEEL PROBE; DIFFICULT TO BREAK WITH HAMMER. EXTREMELY INDURATED SHARP HAMMER BLOWS REQUIRED TO BREAK SAMPLE; SAMPLE BREAKS ACROSS GRAINS.	ROCK HARDNESS VERY HARD CANNOT BE SCRATCHED BY KNIFE OR SHARP PICK. BREAKING OF HAND SPECIMENS REQUIRES SEVERAL HARD BLOWS OF THE GEOLOGIST'S PICK. HARD CAN BE SCRATCHED BY KNIFE OR PICK ONLY WITH DIFFICULTY. HARD HAMMER BLOWS REQUIRED TO DETACH HAND SPECIMEN. MODERATELY HARD CAN BE SCRATCHED BY KNIFE OR PICK. GOUGES OR GROOVES TO 0.25 INCHES DEEP CAN BE EXCAVATED BY HARD BLOW OF A GEOLOGIST'S PICK. HAND SPECIMENS CAN BE DETACHED BY MODERATE BLOWS. MEDIUM HARD CAN BE GROOVED OR GOUGED 0.05 INCHES DEEP BY FIRM PRESSURE OF KNIFE OR PICK POINT. CAN BE EXCAVATED IN SMALL CHIPS TO PEICES 1 INCH MAXIMUM SIZE BY HARD BLOWS OF THE POINT OF A GEOLOGIST'S PICK. SOFT CAN BE GROVED OR GOUGED READILY BY KNIFE OR PICK. CAN BE EXCAVATED IN FRAGMENTS FROM CHIPS TO SEVERAL INCHES IN SIZE BY MODERATE BLOWS OF A PICK POINT. SMALL, THIN PIECES CAN BE BROKEN BY FINGER PRESSURE. VERY SOFT CAN BE CARVED WITH KNIFE. CAN BE EXCAVATED READILY WITH POINT OF PICK. PIECES 1 INCH OR MORE IN THICKNESS CAN BE BROKEN BY FINGER PRESSURE. CAN BE SCRATCHED READILY BY FINGERNAIL.	ROCK HARDNESS SAPROLITE (SAP.) - RESIDUAL SOIL WHICH RETAINS THE RELIC STRUCTURE OR FABRIC OF THE PARENT ROCK. SILL - AN INTRUSIVE BODY OF IGNEOUS ROCK OF APPROXIMATELY UNIFORM THICKNESS AND RELATIVELY THIN COMPARED WITH ITS LATERAL EXTENT, WHICH HAS BEEN EMPLACED PARALLEL TO THE BEDDING OR SCHISTOSITY OF THE INTRUDED ROCKS. SLICKENSIDE - POLISHED AND STRIATED SURFACE THAT RESULTS FROM FRICTION ALONG A FAULT OR SLIP PLANE. STANDARD PENETRATION TEST (PENETRATION RESISTANCE) (SPT) - NUMBER OF BLOWS (N OR B.P.F.) OF A 140 LB. HAMMER FALLING 30 INCHES REQUIRED TO PRODUCE A PENETRATION OF 1 FOOT INTO SOIL WITH A 2 INCH OUTSIDE DIAMETER SPLIT SPOON SAMPLER. SPT REFUSAL IS LESS THAN 0.1 FOOT PENETRATION WITH 60 BLOWS. STRATA CORE RECOVERY (SREC.) - TOTAL LENGTH OF STRATA MATERIAL RECOVERED DIVIDED BY TOTAL LENGTH OF STRATUM AND EXPRESSED AS A PERCENTAGE. STRATA ROCK QUALITY DESIGNATION (S.R.Q.D.) - A MEASURE OF ROCK QUALITY DESCRIBED BY: TOTAL LENGTH OF ROCK SEGMENTS WITHIN A STRATUM EQUAL TO OR GREATER THAN 4 INCHES DIVIDED BY THE TOTAL LENGTH OF STRATA AND EXPRESSED AS A PERCENTAGE. TOPSOIL (T.S.) - SURFACE SOILS USUALLY CONTAINING ORGANIC MATTER.
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STATE OF NORTH CAROLINA
DEPARTMENT OF TRANSPORTATION

MICHAEL F. EASLEY
GOVERNOR

LYNDO TIPPETT
SECRETARY

August, 2004

STATE PROJECT: 33316.1.1
TIP NO: B-3872
F. A. PROJECT: BRZ-1552(8)

DESCRIPTION: Bridge No. 195 on SR 1552 over Bear Creek – Approaches

SUBJECT: Geotechnical Report – Inventory

Site Description

This project is located in the foothills of eastern McDowell County, on the north shore of Lake James. The area is sparsely populated and dedicated chiefly to timber plantations and recreation on Lake James. Bridge 195 is a one-lane structure located on a sharp curve at a point about 100 feet upstream of the confluence of Bear Creek and Bailey Creek at the Lake James shore line.

The existing approach back station from the bridge is bounded on the Left Side by a cut about 50 feet high, with hard rock in its lower part. The lakeshore is on the Right Side at the base of an embankment. The approach up station from the bridge is bounded on the Left Side by a low cut in soil and on the Right Side by the floodplain of Bailey Creek.

This project runs on alignment -LREV- from Station 10+00 to Station 18+00. Temporary pavement on a widened approach to the existing bridge will allow traffic to be maintained on the bridge while construction is underway. The widening will involve a Left Side cut about 50 feet high in soil and rock, another Left Side cut about 20 feet high in soil, and fill 12 feet or less in depth.

A retaining wall is planned for the proposed Right Side embankment beside Bear Creek from Station 13+00 to the new bridge abutment at Station 13+59.

This project has undergone several revisions since the initial plan of 2002. The Geotechnical Engineering Unit conducted a subsurface investigation in August and September, 2002, for plans in effect at that time, with borings for alignments -L- and -DET-, including a retaining wall. Those borings are located favorably for investigation

The embankment soils found on this project are of two kinds. Most embankment soil located back from the bridge as well as up station from it is derived from residual soil and rock, and as such it is composed of red-brown to orange-brown, micaceous, sandy silt and silty sand with abundant fresh to weathered rock fragments and boulders (A-1-b, A-2-4, A-4). Some of the embankment soil located up station from the bridge is derived from the alluvial terrace deposits and is composed of red to orange sandy clay and silt with very hard, well rounded cobbles and boulders (A-1-b, A-6, A-4).

The bedrock at this site is for the most part mica gneiss to mica schist with well developed foliation that dips at moderate angles to the southeast. The bedrock from the beginning of the project to approximately Station 11+00 is a fine-grained, highly fractured mylonite gneiss with pods of alaskite.

Geotechnical Descriptive Analysis

Station 10+00 to 12+00

Construction on this segment involves only minimal excavation and grading. Hard rock is exposed in the existing Left Side cut and it will be encountered in ditch line excavations. Most of the roadway is underlain by weathered rock, and the Right Side shoulder is stony embankment material.

Station 12+00 to 14+00

Plans call for widening the existing roadway in this segment to accommodate its use as a detour. The widening will involve additional excavation of the existing Left Side cut, leaving an exposed face about 50 feet high.

The position of the hard rock line can be predicted using information from borings on the slope above the cut, the exposed rock line in the existing cut, and some natural outcrops at the base of the slope on the Right Side. The hard rock line should be about 15 to 20 feet above the ditch line from Station 12+25 to Station 13+25, and 5 to 10 feet above the ditch line beyond there. Material in the cut above hard rock will include about 5 to 15 feet of weathered rock overlain by thick residual soils comprising beds of medium stiff to stiff clay (A-6) and micaceous silt (A-4) and dense to very dense, micaceous silty sand (A-2-4).

A retaining wall is to be constructed on the Right Side from Station 13+00 to the bridge abutment at Station 13+59. Three borings located on the shoulder of the road in the vicinity of the proposed wall found 5 to 11 feet of embankment soil overlying weathered rock. Hard rock was encountered at depths between 9 and 18 feet.

of the current alignment. Additional borings were made in April, 2004 for the retaining wall in the current plan. A total of 15 borings were made using a CME 45 track-mounted power drilling machine and a CME 550 power drilling machine. All borings were made with 8-inch hollow-stem augers. Standard Penetration Tests were made at regular intervals. A total of 20 soil samples were submitted to a DOT laboratory for tests of quality or moisture.

Items of Special Geotechnical Concern

Hard rock in Cut

Hard rock will be encountered in a cut on the Left Side of -LREV- between Stations 12+25 and 13+75.

Soil and Rock Characteristics

There is a considerable diversity of soils on this project. Residual soils underlie the slopes above the road on the Left Side. Those soils grade downward to micaceous weathered rock and hard rock which are exposed in the existing cut back station from the bridge. Natural rock outcrops can be seen also at the base of the slope along the lakeshore and in the creek channels. Soils up station of the bridge include flood plain alluvium of Bailey Creek as well as Pleistocene or older alluvial terrace deposits of the Catawba River. Those terrace soils underlie most of the roadway beyond the bridge, and they overlie saprolite.

The residual soils comprise a clay-silt cap 4 to 5 feet thick at the ground surface and saprolite in the subsurface. The surficial soils are yellow-brown to orange-brown, dry to moist, soft to medium stiff sandy, silty clay (A-6) and sandy, clayey silt (A-4).

Saprolite is composed of brown or red-brown to gray, stiff to hard, micaceous sandy silt (A-4), and brown to gray-brown, medium dense to very dense, micaceous, silty sand (A-2-4). Those soils comprise a bed 16 to 20 feet thick between the surficial, fine soil and the weathered rock line in borings on the slope on the Left Side, back station of the bridge. Saprolite of similar composition is exposed in the Left Side cut up station of the bridge, and it also underlies alluvial flood plain and terrace soils.

Alluvium on the Baily Creek flood plain comprises about 5 to 7 feet of brown, loose, silty sand (A-2-4) and basal gravel (A-1-b).

Alluvial terrace soils are red or orange to yellow brown, soft to medium stiff, moist to wet, sandy clay or silt with suspended pebbles and cobbles (A-6, A-4); and brown, loose to medium dense, silty sand, gravel and boulders (A-1-b). The terrace soils are notable for the abundance of very hard, well-rounded, white to tan, cobbles and boulders of meta-quartzite within the alluvial beds, and concentrated on the ground surface in some places as lag deposits.

Station 15+00 to 17+00

Construction in this segment will involve a cut on the Left Side and fill on the Right Side. The Left Side cut, which is part of the proposed detour widening, will have an exposed cut face about 20 feet high. No borings have been made for this cut; however, inspection of the existing cut face indicates that the excavation will encounter residual soils similar to soils in other borings on the project. The soils consist of red-brown clay and silt grading downward into stiff or medium dense, brown, micaceous sandy silt to silty sand saprolite.

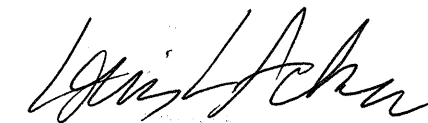
Excavation of a small portion of the cut beyond Station 16+50 will encounter alluvial terrace soil in the upper half of the cut overlying saprolite. The terrace soil consists of red, sandy clay with suspended, well rounded cobbles and small boulders.

The fill on the Right Side is to be about 12 feet high near the bridge abutment, decreasing rapidly to less than 5 feet at Station 16+00. Alluvial terrace soils 3 to 6 feet thick are below and to the Right of the existing embankment slope, where the new fill is to be placed. They are composed of red-brown to yellow-brown, soft to medium stiff sandy clay and silt with hard, rounded cobbles and boulders (A-6, A-1-b). The terrace soils overlie saprolite consisting of stiff to hard sandy silt (A-4) and medium dense to dense silty sand (A-2-4). Groundwater has not been found within 6 feet of the ground surface at any point in this segment.

Station 17+00 to 18+00

This segment involves very minimal construction activity. The existing roadway is underlain by clayey alluvial terrace soils with hard, well-rounded cobbles and boulders (A-6, A-1-b). The existing shallow embankments are derived from those terrace soils.

Respectfully submitted,



Louis L. Acker, LG
Project Geologist

EARTHWORK SUMMARIES

Volumes in Cubic Yards

PROJECT B-3872

COUNTY McDowell

COMPUTED BY: AS

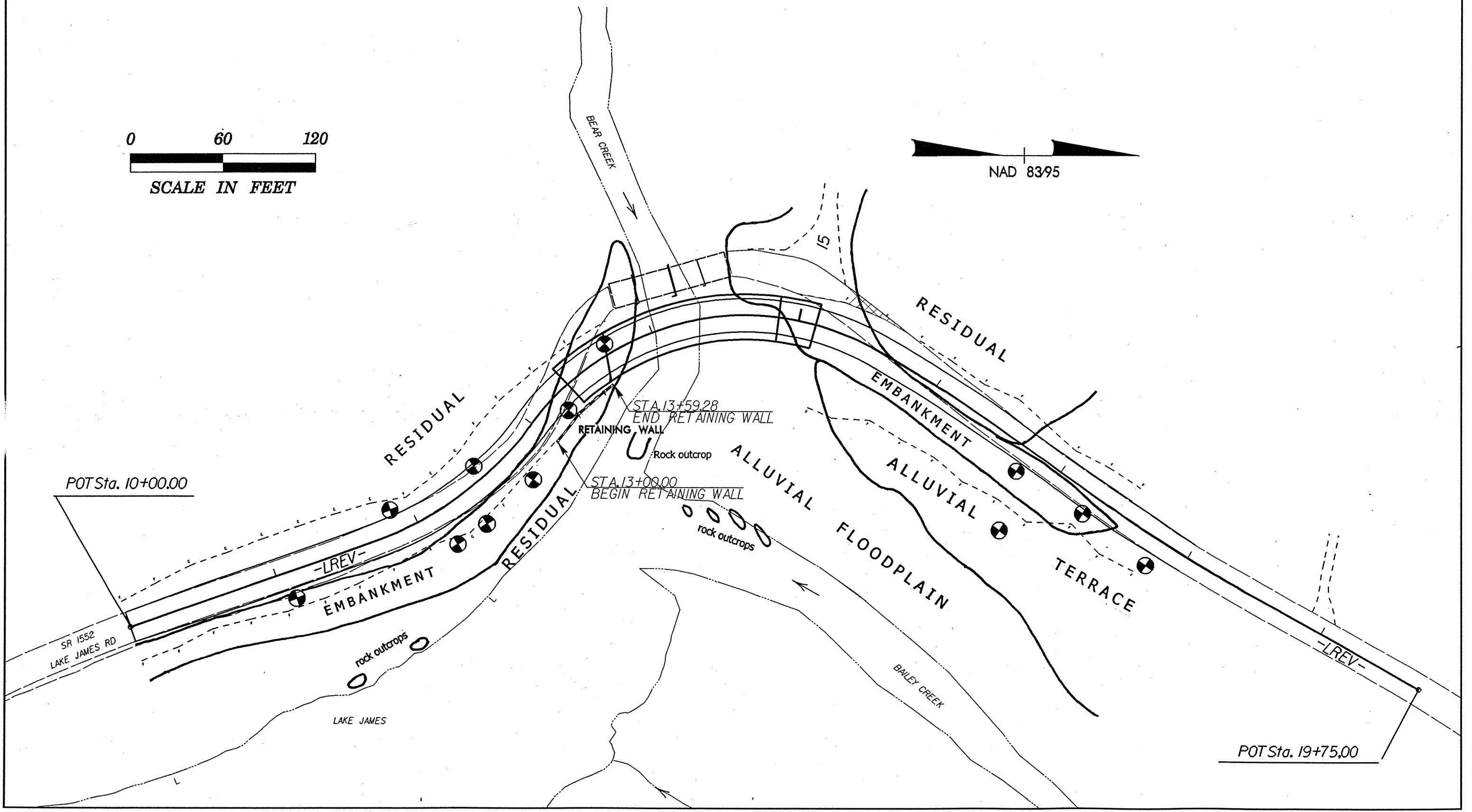
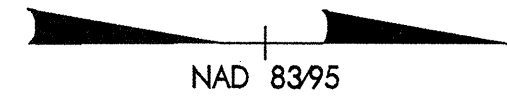
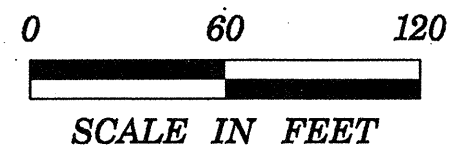
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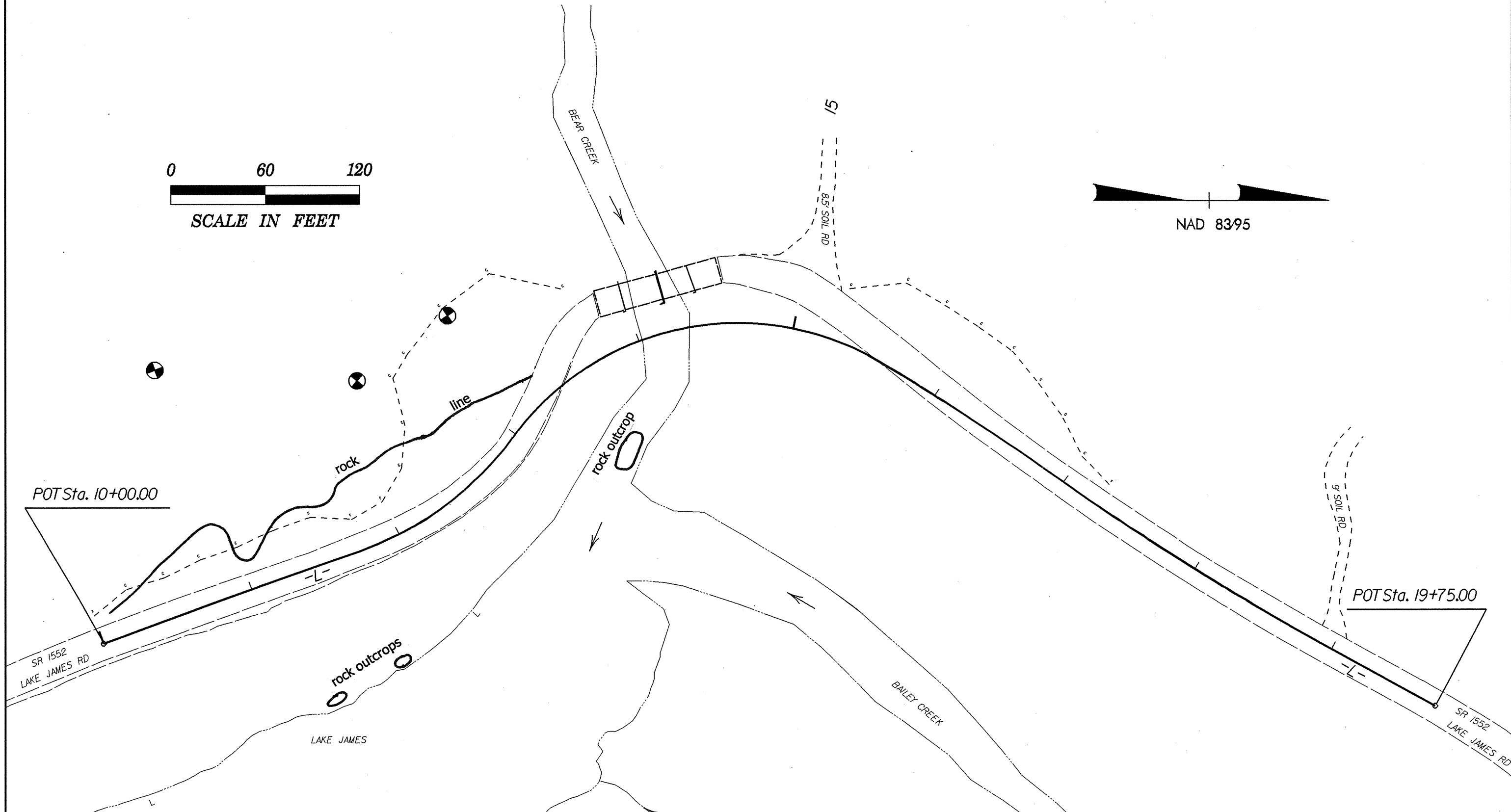
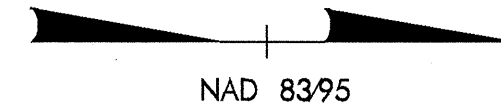
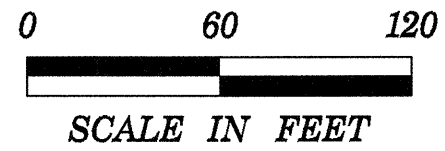
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LINE	STATION	STATION	TOTAL EXCAV. (UNCL.)	ROCK EXCAV.	UNDERCUT	UNSUIT. EXCAV.	SUITABLE EXCAV.	TOTAL EMB.	ROCK EMB.	EARTH EMB.	EMBANK. +15%	BORROW	SUITABLE WASTE	ROCK WASTE	TOTAL WASTE
Summary No. 1															
Phase 1 Temporary Pavement Widening															
-L-	11+75.00	13+75.00	4526	1106			3420	3	3		3		3420	1103	4523
-L-	15+40.00	16+35.00	614				614	7		7	8		606		606
Summary No. 1 Total			5140	1106			4034	10	3	7	11		4026	1103	5129
Summary No. 2															
Phase 2 Rt. Side -L-															
-L-	10+00.00	13+66.50	42				42	255		255	293	251			
-L-	14+86.50	18+00.00	4				4	1336		1336	1536	1532			
Summary No. 2 Total			46				46	1591		1591	1829	1783			
Summary No. 3															
Phase 3 Lt. Side -L-															
-L-	10+00.00	13+66.50	75	39			36	144	39	105	160	85			
-L-	14+86.50	18+00.00	16				16	21		21	24	8			
Summary No. 3 Total			91	39			52	165	39	126	184	93			
Summaries Total			5277	1145			4132	1766	42	1724	2024	1876	4026	1103	5129
Deduction for Clearing and Grubbing			-120				-120					120	-120		-120
Rock Waste to be used in lieu of Borrow									1103	-1103		-1103		-1103	-1103
Adjustment for Rock Waste											-276	-276			
Suitable Waste to be used in lieu of Borrow												-617	-617		-617
Project Totals			5157	1145			4012	1766	1145	621	1748	0	3289		3289
Say			5200												
Estimated Undercut		500 CY													
NOTE: Earthwork quantities are calculated by the Roadway Design Unit. These earthwork quantities are based in part on subsurface data provided by the Geotechnical Engineering Unit.															
SHEET TOTALS:															

PLAN VIEW -LREV-



TEMPORARY PAVEMENT DETAIL



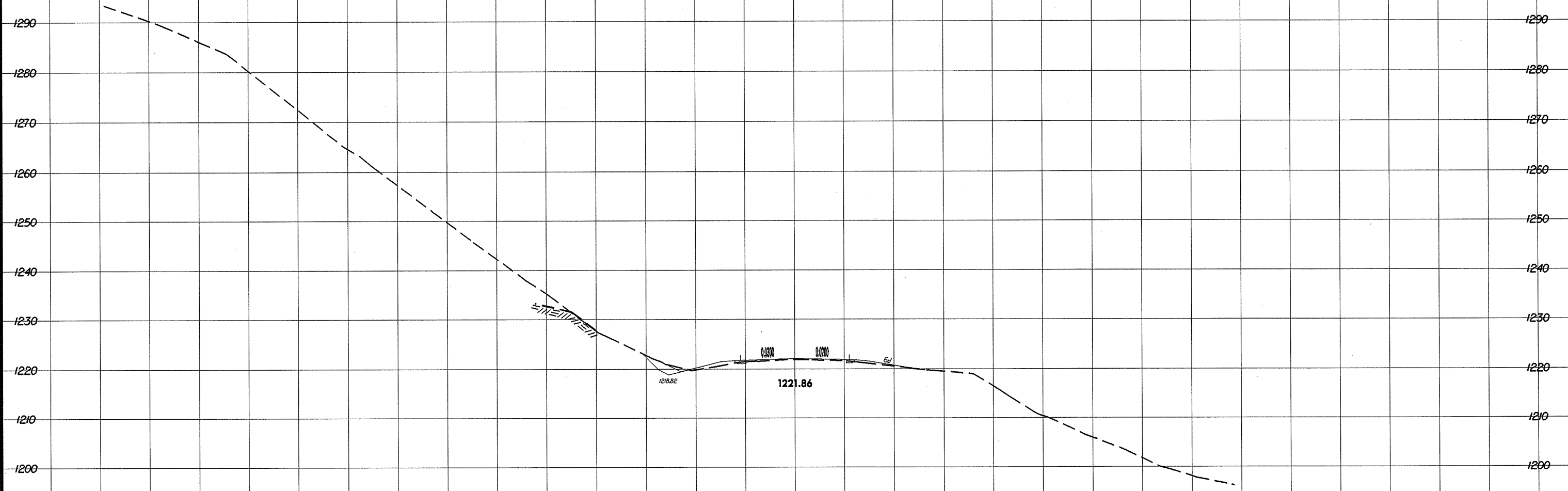
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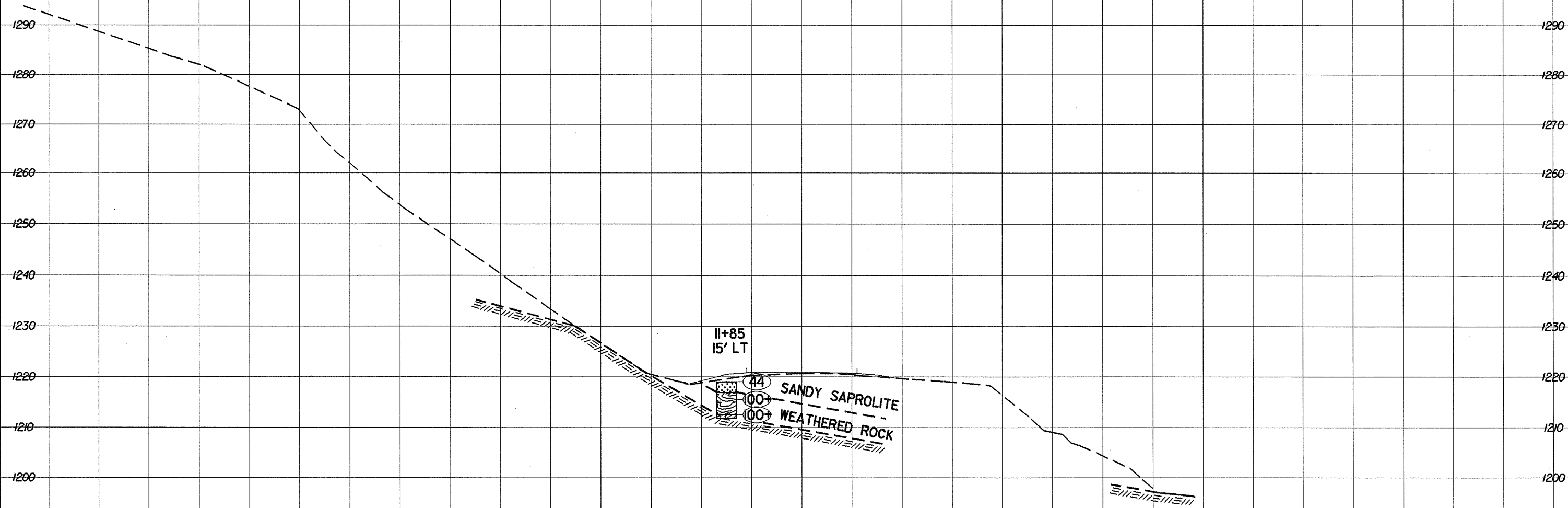
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1290 1280 1270 1260 1250 1240 1230 1220 1210 1200

1290 1280 1270 1260 1250 1240 1230 1220 1210 1200

11+85
15' LT

44 SANDY SAPROLITE
100 WEATHERED ROCK

11+75.00
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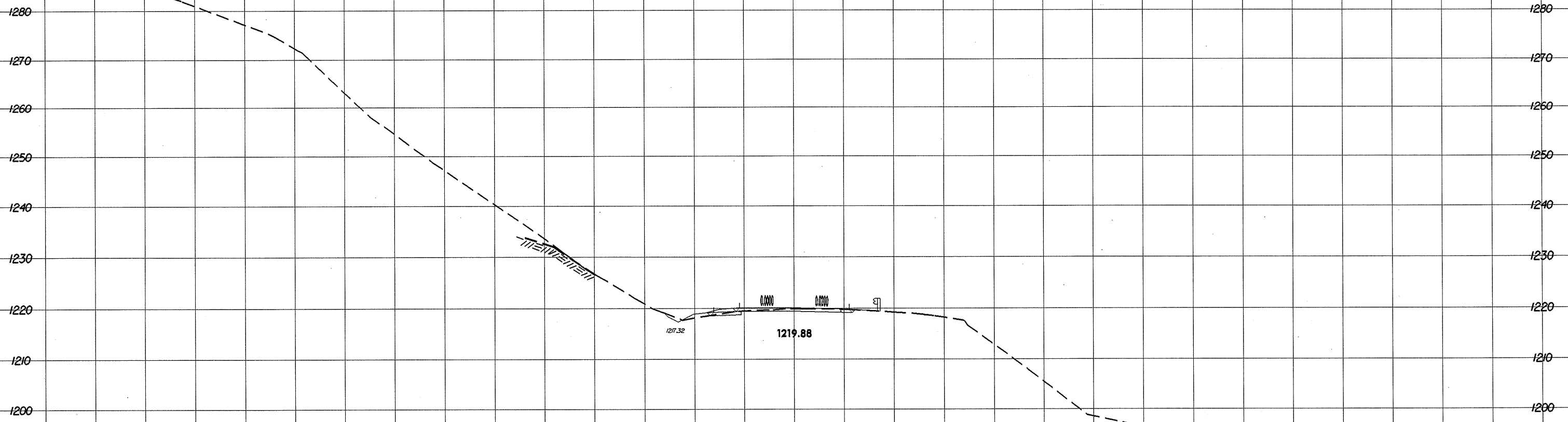
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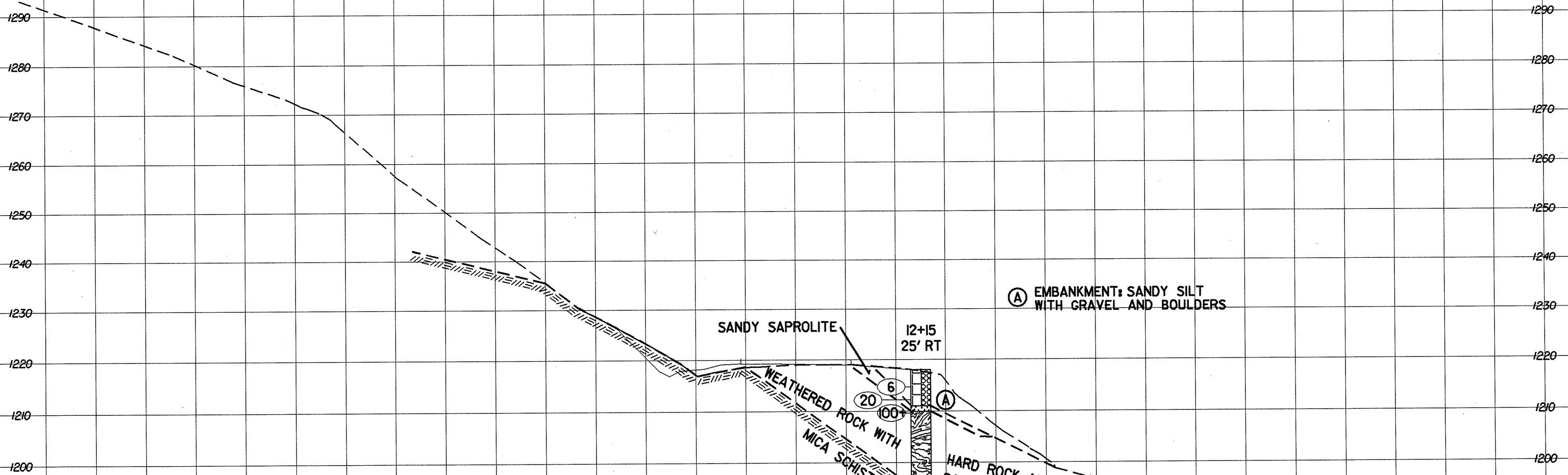
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11 OF 27

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SANDY SAPROLITE

12+15
25' RT

(A) EMBANKMENT: SANDY SILT
WITH GRAVEL AND BOULDERS

WEATHERED ROCK WITH
MICA SCHIST

HARD ROCK AND
SAPROLOTE SEAMS

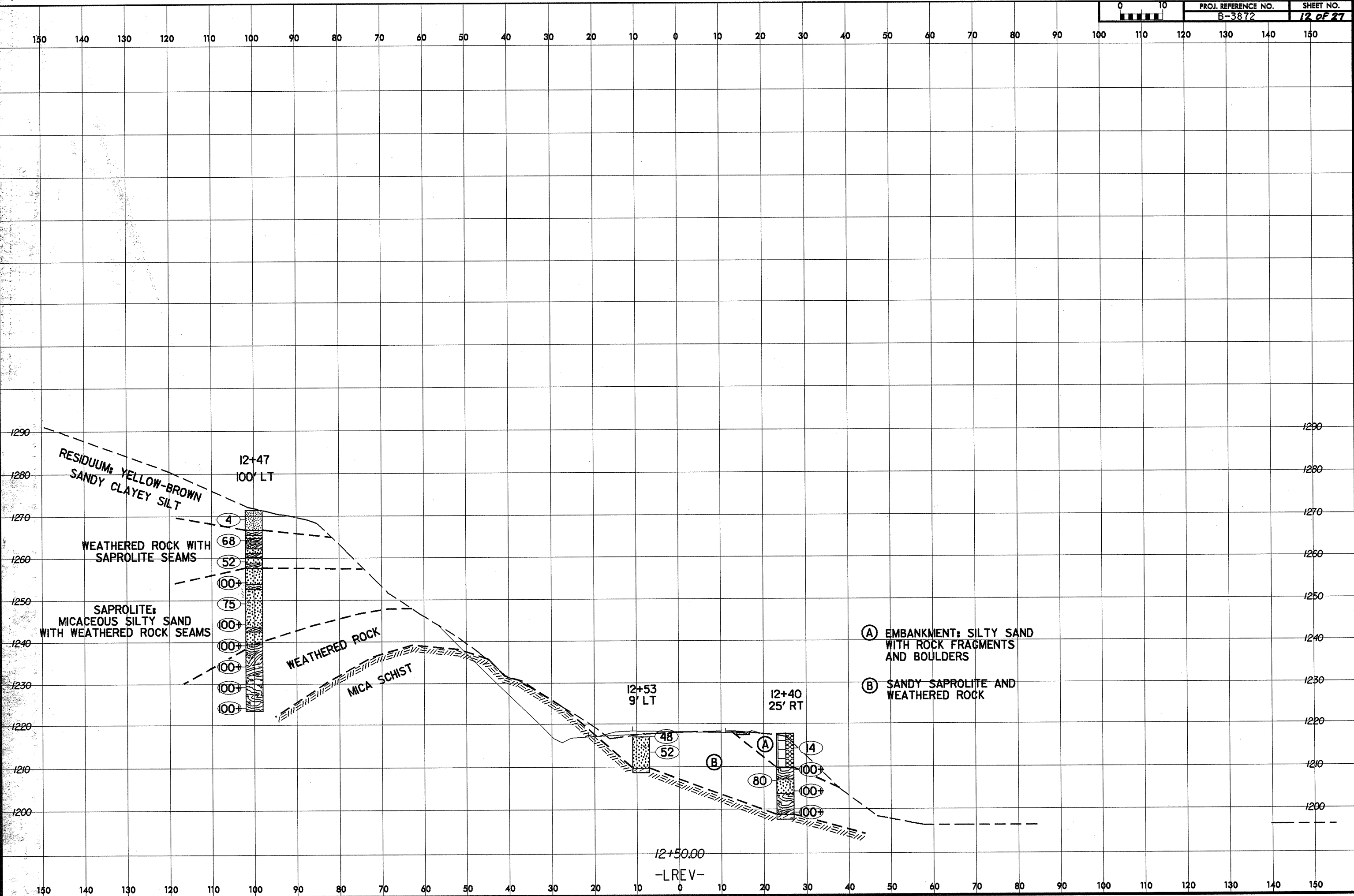
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JOB: B-3872
SHEET: 11 OF 27

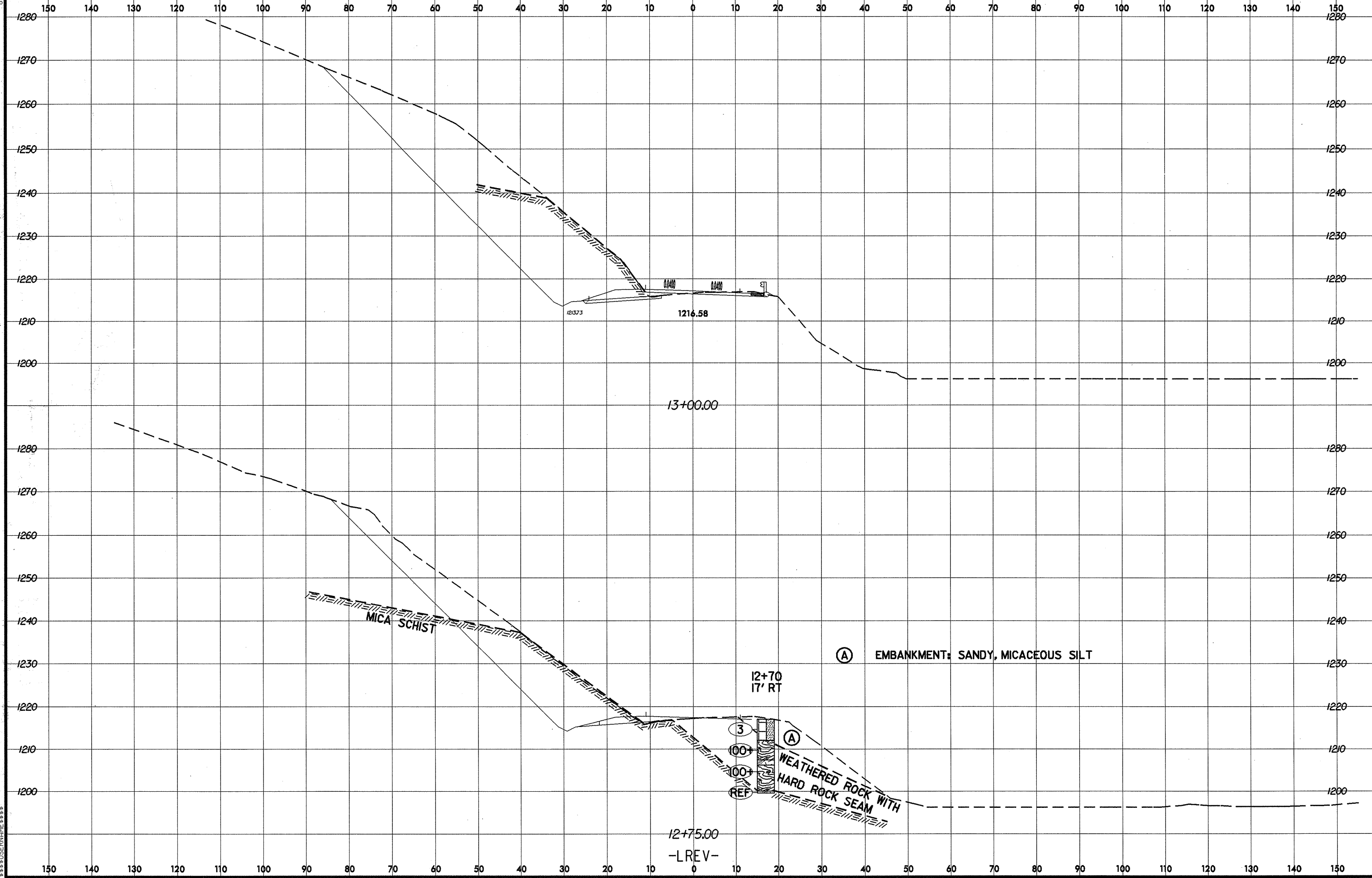
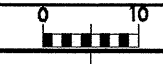
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8/23/99
*****SYSTEM TIME*****
*****SUPERVISOR NAME*****



12+50.00
-LREV-

8/23/99



MICA SCHIST

(A) EMBANKMENT: SANDY, MICACEOUS SILT

12+70
17' RT

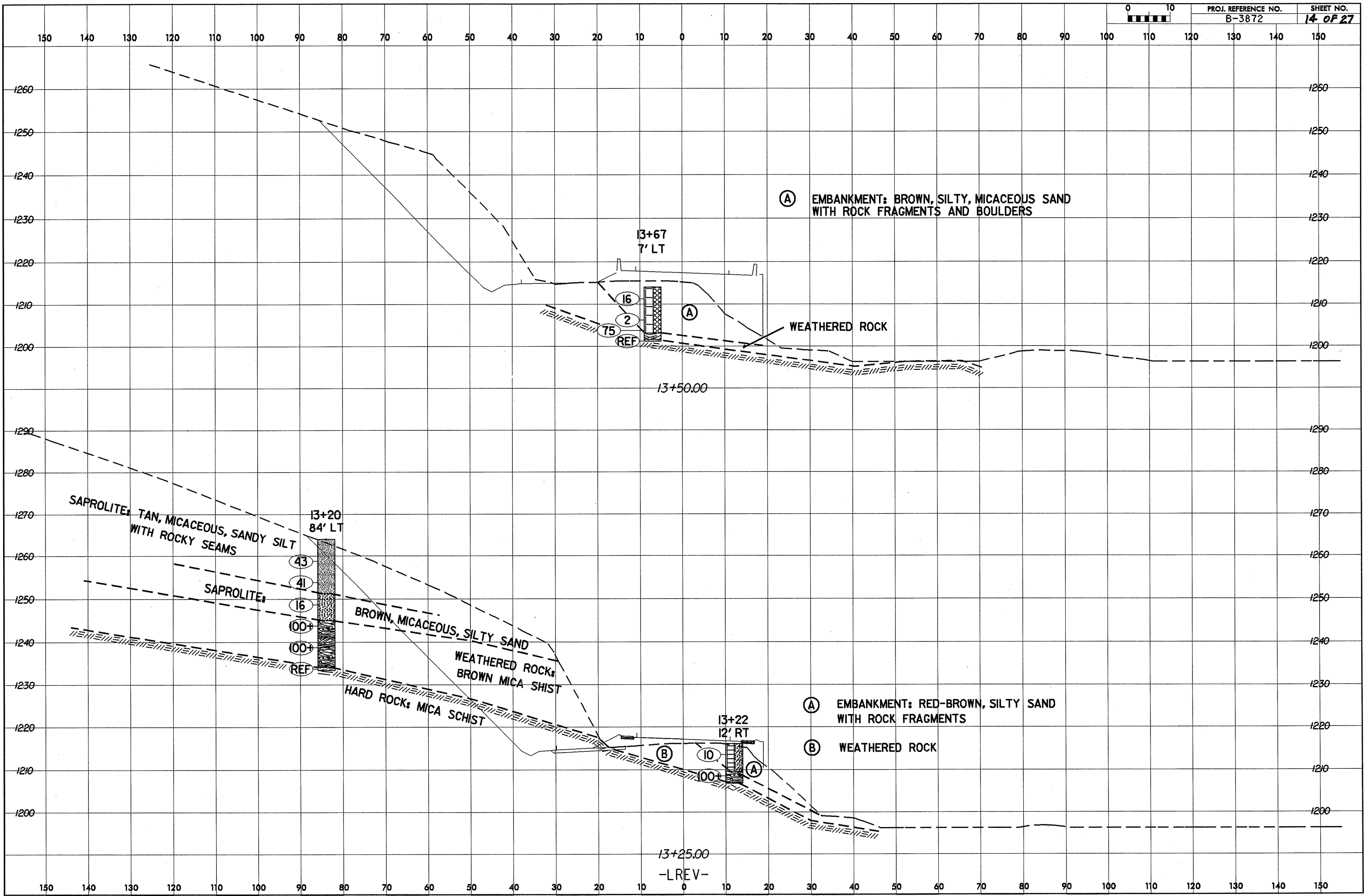
3
00
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REF

(A) WEATHERED ROCK WITH
HARD ROCK SEAM

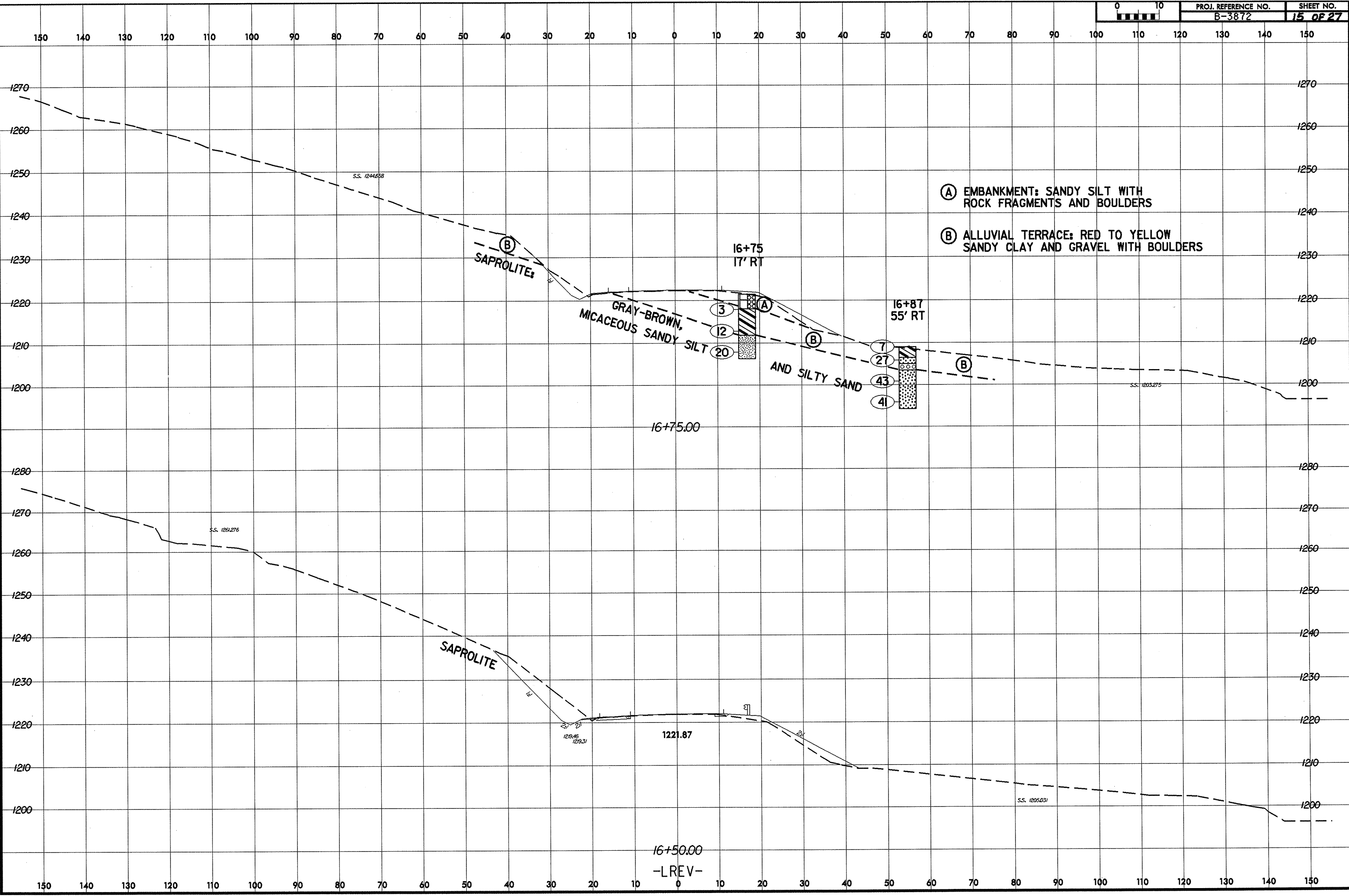
12+75.00

-LREV-

SECTION ON CHAIN LINE

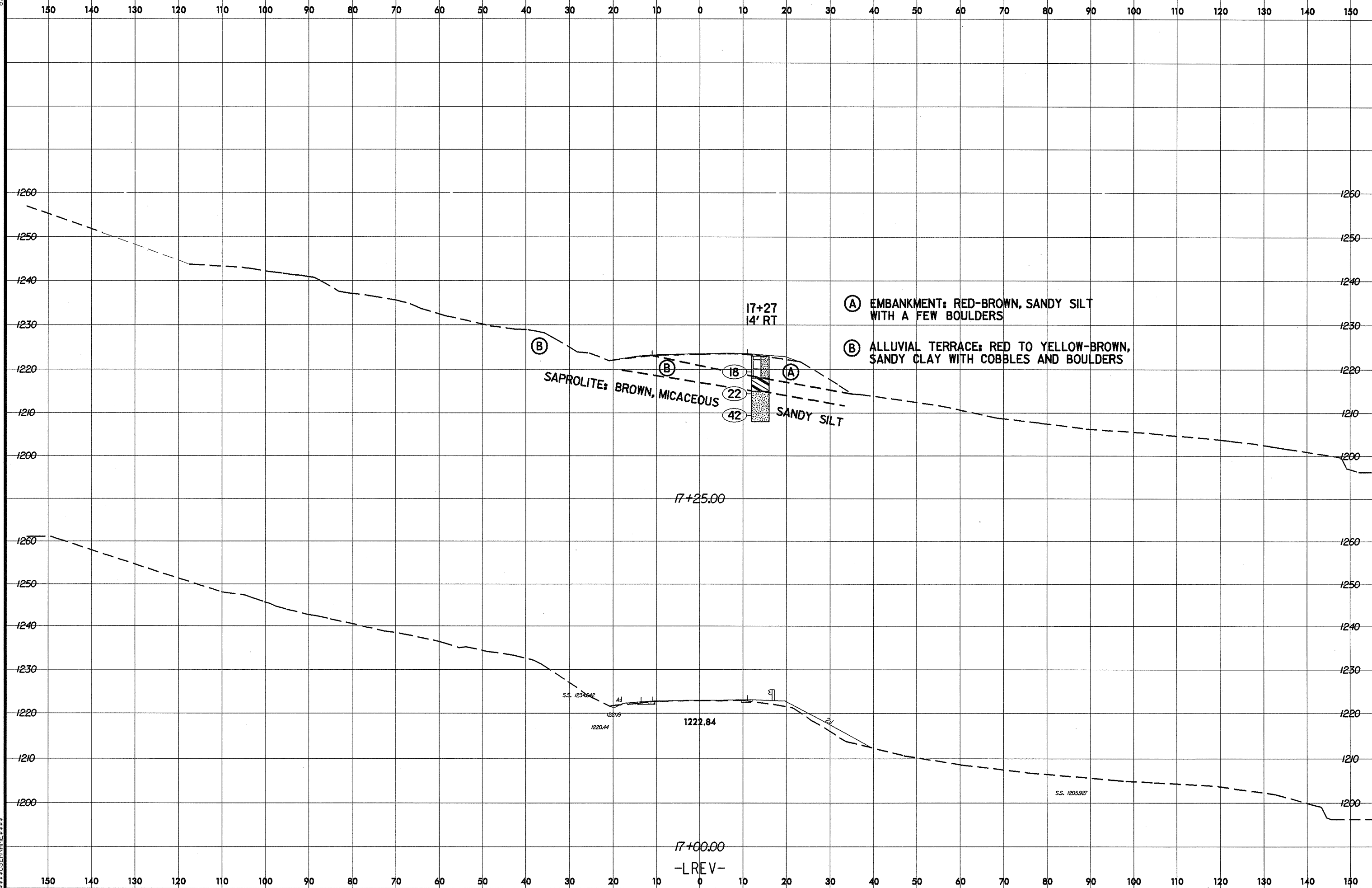


8/23/99



*****SYSTEMTIME*****
*****SERVERNAME*****

8/23/99



SYSTEME

17+00.00
-LREV-

S.S. 1237.642
41
1220.44

1222.84

S.S. 1205.927

17+27
14' RT

(A) EMBANKMENT: RED-BROWN, SANDY SILT
WITH A FEW BOULDERS

(B) ALLUVIAL TERRACE: RED TO YELLOW-BROWN,
SANDY CLAY WITH COBBLES AND BOULDERS

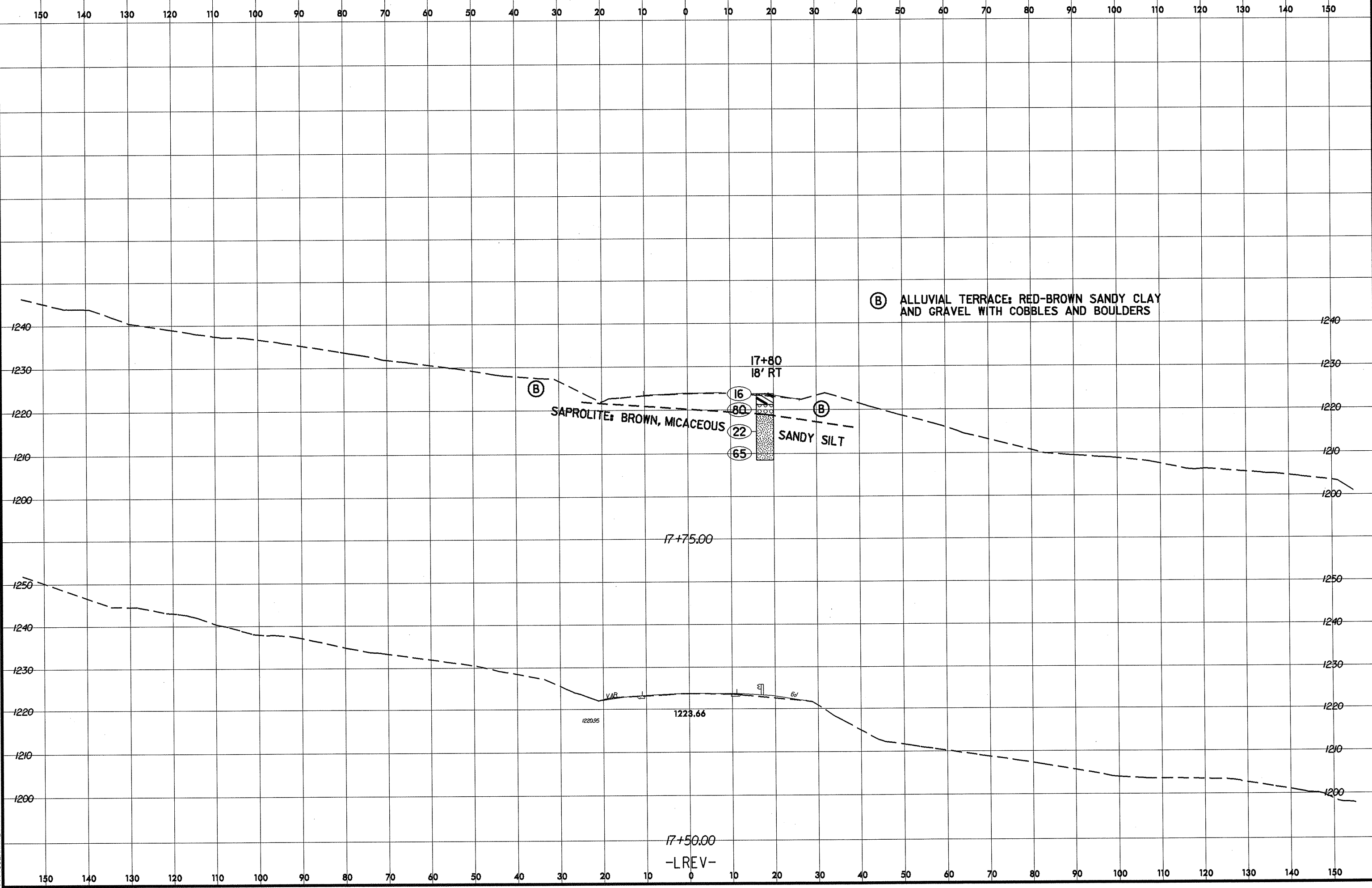
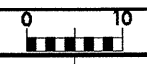
(B) SAPROLITE: BROWN, MICACEOUS

(A) SANDY SILT

18
22
42

17+25.00

8/23/99



(B) ALLUVIAL TERRACE: RED-BROWN SANDY CLAY AND GRAVEL WITH COBBLES AND BOULDERS

17+80
18' RT

(B) SAPROLITE: BROWN, MICACEOUS

(B) SANDY SILT

- (16)
- (80)
- (22)
- (65)

17+75.00

1220.95

1223.66

17+50.00

-LREV-

SYSTEM TIME: 8/23/99 10:00:00 AM
DESIGNER: J. L. BROWN
USER: JLB

NORTH CAROLINA DEPARTMENT OF TRANSPORTATION
GEOTECHNICAL UNIT BORING LOG

PROJECT NO 33316.1.1		ID B-3872		COUNTY MCDOWELL		GEOLOGIST L.L. ACKER							
SITE DESCRIPTION APPROACHES TO BR 195 ON SR-1552 OVER LAKE JAMES CREEK							GND WATER						
BORING NO BORING 3		NORTHING 0.00		EASTING 0.00		0 HR N/A							
ALIGNMENT -LREV-		BORING LOCATION 10+91.000		OFFSET 154.00ft LT		24 HR N/A							
COLLAR ELEV 1290.00ft		TOTAL DEPTH 26.10ft		START DATE 9/01/02		COMPLETION DATE 09/03/02							
DRILL MACHINE CME-45			DRILL METHOD H.S. AUGERS			HAMMER TYPE AUTOMATIC							
SURFACE WATER DEPTH N/A			DEPTH TO ROCK N/A			Log BORING 3, Page 1 of 1							
ELEV	DEPTH	BLOW CT			PEN (ft)	BLOWS PER FOOT				SAMPLE NO	LOG	SOIL AND ROCK DESCRIPTION	
		6in	6in	6in		0	25	50	75				100
1290.00	0.00	1	1	1	1.0								Ground Surface
	3.60	4	8	10	1.0	2							RESIDUAL: ORANGE BROWN, SOFT TO MEDIUM STIFF, SANDY, SILTY CLAY
	8.60	3	4	6	1.0	10							SAPROLITE: RED-BROWN TO GRAY, STIFF, MICACEOUS, SANDY SILT
1280.00	13.60	3	5	8	1.0	13							
	18.60	6	6	9	1.0	15							
1270.00	23.60	30	70		0.2								WEATHERED ROCK: GRAY-BROWN BIOTITE GNEISS
1263.90													HARD ROCK: GRAY-BROWN BIOTITE GNEISS
TERMINATED BORING IN HARD ROCK AT ELEVATION 1263.9 FEET													

NORTH CAROLINA DEPARTMENT OF TRANSPORTATION
GEOTECHNICAL UNIT BORING LOG

PROJECT NO 33316.1.1		ID B-3872		COUNTY MCDOWELL		GEOLOGIST L.L. ACKER							
SITE DESCRIPTION APPROACHES TO BR 195 ON SR-1552 OVER LAKE JAMES CREEK							GND WATER						
BORING NO BORING 10		NORTHING 0.00		EASTING 0.00		0 HR N/A							
ALIGNMENT -LREV-		BORING LOCATION 11+08.000		OFFSET 18.00ft RT		24 HR N/A							
COLLAR ELEV 1221.00ft		TOTAL DEPTH 14.70ft		START DATE 9/06/02		COMPLETION DATE 09/06/02							
DRILL MACHINE CME-45			DRILL METHOD H.S. AUGERS			HAMMER TYPE AUTOMATIC							
SURFACE WATER DEPTH			DEPTH TO ROCK N/A			Log BORING 10, Page 1 of 1							
ELEV	DEPTH	BLOW CT			PEN (ft)	BLOWS PER FOOT				SAMPLE NO	LOG	SOIL AND ROCK DESCRIPTION	
		6in	6in	6in		0	25	50	75				100
1221.00													Ground Surface
	3.60	4	25	56	1.0								EMBANKMENT: SILTY SAND WITH ROCK FRAGMENTS AND BOULDERS
	8.60	100			0.1								SAPROLITE: BROWN, MICACEOUS, SILTY SAND
1210.00	13.60	19	45	55	1.0								WEATHERED ROCK WITH HARD ROCK SEAMS
1206.30													TERMINATED BORING IN WEATHERED ROCK AT ELEVATION 1206.3 FEET

NORTH CAROLINA DEPARTMENT OF TRANSPORTATION
 GEOTECHNICAL UNIT BORING LOG

PROJECT NO 33316.1.1		ID B-3872		COUNTY MCDOWELL		GEOLOGIST L.L. ACKER							
SITE DESCRIPTION APPROACHES TO BR 195 ON SR-1552 OVER LAKE JAMES CREEK							GND WATER						
BORING NO BORING 4		NORTHING 0.00		EASTING 0.00		0 HR N/A							
ALIGNMENT -LREV-		BORING LOCATION 11+85.000		OFFSET 15.00ft LT		24 HR N/A							
COLLAR ELEV 1219.00ft		TOTAL DEPTH 7.20ft		START DATE 9/04/02		COMPLETION DATE 09/04/02							
DRILL MACHINE CME-45			DRILL METHOD H.S. AUGERS			HAMMER TYPE AUTOMATIC							
SURFACE WATER DEPTH N/A			DEPTH TO ROCK 6.50ft			Log BORING 4, Page 1 of 1							
ELEV	DEPTH	BLOW CT			PEN (ft)	BLOWS PER FOOT				SAMPLE NO	LOG	SOIL AND ROCK DESCRIPTION	
		6in	6in	6in		0	25	50	75				100
1219.00	0.00	3	16	28	1.0								Ground Surface
	3.50	100			0.3								SAPROLITE: LIGHT BROWN, MICACEOUS, SILTY SAND
1211.80	6.50	100			0.3								WEATHERED ROCK: MICA SCHIST
TERMINATED BORING IN HARD ROCK AT ELEVATION 1212.5 FEET													

NORTH CAROLINA DEPARTMENT OF TRANSPORTATION
 GEOTECHNICAL UNIT BORING LOG

PROJECT NO 33316.1.1		ID B-3872		COUNTY MCDOWELL		GEOLOGIST L.L. ACKER							
SITE DESCRIPTION APPROACHES TO BR 195 ON SR-1552 OVER LAKE JAMES CREEK							GND WATER						
BORING NO BORING 12		NORTHING 0.00		EASTING 0.00		0 HR N/A							
ALIGNMENT -LREV-		BORING LOCATION 12+15.000		OFFSET 25.00ft RT		24 HR N/A							
COLLAR ELEV 1218.00ft		TOTAL DEPTH 22.70ft		START DATE 9/11/02		COMPLETION DATE 09/11/02							
DRILL MACHINE CME 550			DRILL METHOD H.S. AUGERS			HAMMER TYPE AUTOMATIC							
SURFACE WATER DEPTH			DEPTH TO ROCK 22.70ft			Log BORING 12, Page 1 of 1							
ELEV	DEPTH	BLOW CT			PEN (ft)	BLOWS PER FOOT				SAMPLE NO	LOG	SOIL AND ROCK DESCRIPTION	
		6in	6in	6in		0	25	50	75				100
1218.00													Ground Surface
	3.40	3	3	3	1.0								EMBANKMENT: ORANGE-BROWN, SANDY SILT WITH GRAVEL AND BOULDERS
	5.90	4	8	12	1.0								SAPROLITE: BROWN-GRAY, MICACEOUS, SILTY SAND WEATHERED ROCK: MICA SCHIST WITH SEAMS OF HARD ROCK AND SAPROLITE
1210.00	8.40	19	40	60	0.8								
1200.00													
1195.30													TERMINATED BORING ON HARD ROCK AT ELEVATION 1195.3 FEET

NORTH CAROLINA DEPARTMENT OF TRANSPORTATION
GEOTECHNICAL UNIT BORING LOG

PROJECT NO 33316.1.1		ID B-3872		COUNTY MCDOWELL		GEOLOGIST L.L. ACKER								
SITE DESCRIPTION APPROACHES TO BR 195 ON SR-1552 OVER LAKE JAMES CREEK							GND WATER							
BORING NO BORING 2		NORTHING 0.00		EASTING 0.00		0 HR N/A								
ALIGNMENT -LREV-		BORING LOCATION 12+47.000		OFFSET 100.00ft LT		24 HR 21.70ft								
COLLAR ELEV 1271.50ft		TOTAL DEPTH 48.10ft		START DATE 8/27/02		COMPLETION DATE 08/29/02								
DRILL MACHINE CME-45			DRILL METHOD H.S. AUGERS			HAMMER TYPE AUTOMATIC								
SURFACE WATER DEPTH N/A			DEPTH TO ROCK N/A			Log BORING 2, Page 1 of 2								
ELEV	DEPTH	BLOW CT			PEN (ft)	BLOWS PER FOOT				SAMPLE NO	LOG	SOIL AND ROCK DESCRIPTION		
		6in	6in	6in		0	25	50	75				100	
1271.50														
1270.00	2.30	2	2	2	1.0					4	SS-1	M	RESIDUUM: YELLOW-BROWN, SANDY, CLAYEY SILT	
	7.30	58	43	25	1.0					58		M	WEATHERED ROCK WITH SANDY SAPROLITE SEAMS	
1260.00	12.30	76	27	25	1.0					52	SS-2	M	SAPROLITE: GRAY-BROWN, MICACEOUS, SILTY SAND	
	17.30	100			0.3					100		M	WEATHERED ROCK	
1250.00	22.30	23	43	32	1.0					75	SS-3	M/W	SAPROLITE: GRAY-BROWN, MICACEOUS, SILTY SAND WITH WEATHERED ROCK SEAMS AT BASE	
	27.30	26	32	68	0.9					100				
1240.00	32.30	11	29	71	0.9					100			WEATHERED ROCK: MICA SCHIST AND FELDSPATHIC GNEISS	
	37.30	35	65		0.5					100				
1230.00	42.30	37	63		0.5					100				
1223.40	47.30	45	55		0.3					100				
						Continued on the next page.								

NORTH CAROLINA DEPARTMENT OF TRANSPORTATION
GEOTECHNICAL UNIT BORING LOG

PROJECT NO 33316.1.1		ID B-3872		COUNTY MCDOWELL		GEOLOGIST L.L. ACKER							
SITE DESCRIPTION APPROACHES TO BR 195 ON SR-1552 OVER LAKE JAMES CREEK							GND WATER						
BORING NO BORING 2		NORTHING 0.00		EASTING 0.00		0 HR N/A							
ALIGNMENT -LREV-		BORING LOCATION 12+47.000		OFFSET 100.00ft LT		24 HR 21.70ft							
COLLAR ELEV 1271.50ft		TOTAL DEPTH 48.10ft		START DATE 8/27/02		COMPLETION DATE 08/29/02							
DRILL MACHINE CME-45			DRILL METHOD H.S. AUGERS			HAMMER TYPE AUTOMATIC							
SURFACE WATER DEPTH N/A			DEPTH TO ROCK N/A			Log BORING 2, Page 2 of 2							
ELEV	DEPTH	BLOW CT			PEN (ft)	BLOWS PER FOOT				SAMPLE NO	LOG	SOIL AND ROCK DESCRIPTION	
		6in	6in	6in		0	25	50	75				100
1223.40													
													TERMINATED BORING IN WEATHERED ROCK AT ELEVATION 1223.4 FEET

NORTH CAROLINA DEPARTMENT OF TRANSPORTATION
 GEOTECHNICAL UNIT BORING LOG

PROJECT NO 33316.1.1		ID B-3872		COUNTY MCDOWELL		GEOLOGIST L.L. ACKER							
SITE DESCRIPTION APPROACHES TO BR 195 ON SR-1552 OVER LAKE JAMES CREEK						GND WATER							
BORING NO BORING 13		NORTHING 0.00		EASTING 0.00		0 HR 0.00ft							
ALIGNMENT -LREV-		BORING LOCATION 12+70.000		OFFSET 17.00ft RT		24 HR N/A							
COLLAR ELEV 1217.10ft		TOTAL DEPTH 17.50ft		START DATE 4/06/04		COMPLETION DATE 04/06/04							
DRILL MACHINE CME 550		DRILL METHOD H.S. AUGERS		HAMMER TYPE AUTOMATIC									
SURFACE WATER DEPTH N/A		DEPTH TO ROCK 17.00ft		Log BORING 13, Page 1 of 1									
ELEV	DEPTH	BLOW CT			PEN (ft)	BLOWS PER FOOT				SAMPLE NO	LOG	SOIL AND ROCK DESCRIPTION	
		6in	6in	6in		0	25	50	75				100
1217.10													
	2.40	2	2	1	1.0								EMBANKMENT: BROWN, SANDY, MICACEOUS SILT
1210.00	7.40	40	60		0.4					100	X		WEATHERED ROCK
	12.40	100			0.4					100	X		HARD ROCK WEATHERED ROCK
1199.80	17.40	60			0.1					60	X		HARD ROCK
													TERMINATED BORING IN HARD ROCK AT ELEVATION 1199.6 FEET.

NORTH CAROLINA DEPARTMENT OF TRANSPORTATION
 GEOTECHNICAL UNIT BORING LOG

PROJECT NO 33316.1.1		ID B-3872		COUNTY MCDOWELL		GEOLOGIST L.L. ACKER							
SITE DESCRIPTION APPROACHES TO BR 195 ON SR-1552 OVER LAKE JAMES CREEK						GND WATER							
BORING NO BORING 1		NORTHING 0.00		EASTING 0.00		0 HR N/A							
ALIGNMENT -LREV-		BORING LOCATION 13+20.000		OFFSET 84.00ft LT		24 HR N/A							
COLLAR ELEV 1264.00ft		TOTAL DEPTH 30.60ft		START DATE 9/03/02		COMPLETION DATE 09/03/02							
DRILL MACHINE CME-45		DRILL METHOD H.S. AUGERS		HAMMER TYPE AUTOMATIC									
SURFACE WATER DEPTH		DEPTH TO ROCK 28.40ft		Log BORING 1, Page 1 of 1									
ELEV	DEPTH	BLOW CT			PEN (ft)	BLOWS PER FOOT				SAMPLE NO	LOG	SOIL AND ROCK DESCRIPTION	
		6in	6in	6in		0	25	50	75				100
1264.00													
	5.30	18	15	28	1.0								SAPROLITE: TAN, MICACEOUS, SANDY SILT WITH ROCKY SEAMS
1260.00	10.30	5	16	25	1.0					SS-4 MS-1	D		
	15.30	5	8	8	1.0								SAPROLITE: BROWN, MICACEOUS, SILTY SAND
1250.00	20.30	23	42	58	0.8								WEATHERED ROCK: BROWN MICA SCHIST
1240.00	25.30	24	76		0.3								
1233.40	30.30	100			0.0								HARD ROCK: BROWN MICA SCHIST
													TERMINATED BORING IN HARD ROCK AT ELEVATION 1233.4 FEET

NORTH CAROLINA DEPARTMENT OF TRANSPORTATION
 GEOTECHNICAL UNIT BORING LOG

NORTH CAROLINA DEPARTMENT OF TRANSPORTATION
 GEOTECHNICAL UNIT BORING LOG

PROJECT NO 33316.1.1		ID B-3872		COUNTY MCDOWELL		GEOLOGIST L.L. ACKER						
SITE DESCRIPTION APPROACHES TO BR 195 ON SR-1552 OVER LAKE JAMES CREEK							GND WATER					
BORING NO BORING 14		NORTHING 0.00		EASTING 0.00		0 HR 0.00ft						
ALIGNMENT -LREV-		BORING LOCATION 13+22.000		OFFSET 12.00ft RT		24 HR N/A						
COLLAR ELEV 1216.10ft		TOTAL DEPTH 9.30ft		START DATE 4/06/04		COMPLETION DATE 04/06/04						
DRILL MACHINE CME 550			DRILL METHOD H.S. AUGERS			HAMMER TYPE AUTOMATIC						
SURFACE WATER DEPTH N/A			DEPTH TO ROCK 9.30ft			Log BORING 14, Page 1 of 1						
ELEV	DEPTH	BLOW CT			PEN (ft)	BLOWS PER FOOT				SAMPLE NO	LOG MOI	SOIL AND ROCK DESCRIPTION
		6in	6in	6in		0	25	50	75			
1216.10												
	2.60	2	6	4	1.0							Ground Surface
	7.60	100			0.2							EMBANKMENT: RED-BROWN, SILTY SAND WITH ROCK FRAGMENTS
												WEATHERED ROCK WITH THIN SOFT SEAMS
												TERMINATED BORING ON HARD ROCK AT ELEVATION 1206.8 FEET.

PROJECT NO 33316.1.1		ID B-3872		COUNTY MCDOWELL		GEOLOGIST L.L. ACKER						
SITE DESCRIPTION APPROACHES TO BR 195 ON SR-1552 OVER LAKE JAMES CREEK							GND WATER					
BORING NO BORING 15		NORTHING 0.00		EASTING 0.00		0 HR 0.00ft						
ALIGNMENT -LREV-		BORING LOCATION 13+67.000		OFFSET 7.00ft LT		24 HR N/A						
COLLAR ELEV 1213.90ft		TOTAL DEPTH 12.70ft		START DATE 4/06/04		COMPLETION DATE 04/06/04						
DRILL MACHINE CME 550			DRILL METHOD H.S. AUGERS			HAMMER TYPE AUTOMATIC						
SURFACE WATER DEPTH N/A			DEPTH TO ROCK 12.30ft			Log BORING 15, Page 1 of 1						
ELEV	DEPTH	BLOW CT			PEN (ft)	BLOWS PER FOOT				SAMPLE NO	LOG MOI	SOIL AND ROCK DESCRIPTION
		6in	6in	6in		0	25	50	75			
1213.90												
	2.70	12	12	4	1.0							Ground Surface
	7.70	1	1	1	1.0							EMBANKMENT: BROWN, SILTY MICACEOUS SAND WITH ROCK FRAGMENTS AND BOULDERS
	10.20	2	6	69	1.0							
	12.70	60			0.1							WEATHERED ROCK
												HARD ROCK
												TERMINATED BORING IN HARD ROCK AT ELEVATION 1201.6 FEET.

NORTH CAROLINA DEPARTMENT OF TRANSPORTATION
 GEOTECHNICAL UNIT BORING LOG

PROJECT NO 33316.1.1		ID B-3872		COUNTY MCDOWELL		GEOLOGIST L.L. ACKER							
SITE DESCRIPTION APPROACHES TO BR 195 ON SR-1552 OVER LAKE JAMES CREEK							GND WATER						
BORING NO BORING 7		NORTHING 0.00		EASTING 0.00		0 HR N/A	24 HR N/A						
ALIGNMENT -LREV-		BORING LOCATION 16+75.000		OFFSET 17.00ft RT									
COLLAR ELEV 1221.50ft		TOTAL DEPTH 15.20ft		START DATE 9/06/02		COMPLETION DATE 09/06/02							
DRILL MACHINE CME-45			DRILL METHOD H.S. AUGERS			HAMMER TYPE AUTOMATIC							
SURFACE WATER DEPTH			DEPTH TO ROCK N/A			Log BORING 7, Page 1 of 1							
ELEV	DEPTH	BLOW CT			PEN (ft)	BLOWS PER FOOT				SAMPLE NO	LOG	SOIL AND ROCK DESCRIPTION	
		6in	6in	6in		0	25	50	75				100
1221.50													
1220.00	3.70	1	1	2	1.0	3					SS-13	M	EMBANKMENT: RED-BROWN, SANDY SILT, GRAVEL AND BOULDERS
	8.70	2	4	8	1.0	12					SS-14	M	ALLUVIUM: RED-BROWN, SANDY, SILTY CLAY WITH A LITTLE GRAVEL
1210.00													
1206.30	13.70	5	9	11	1.0	20						M	SAPROLITE: GRAY-BROWN, MICACEOUS, SANDY SILT
TERMINATED BORING IN SAPROLITE AT ELEVATION 1206.3 FEET													

NORTH CAROLINA DEPARTMENT OF TRANSPORTATION
 GEOTECHNICAL UNIT BORING LOG

PROJECT NO 33316.1.1		ID B-3872		COUNTY MCDOWELL		GEOLOGIST L.L. ACKER							
SITE DESCRIPTION APPROACHES TO BR 195 ON SR-1552 OVER LAKE JAMES CREEK							GND WATER						
BORING NO BORING 6		NORTHING 0.00		EASTING 0.00		0 HR N/A	24 HR 9.40ft						
ALIGNMENT -LREV-		BORING LOCATION 16+87.000		OFFSET 55.00ft RT									
COLLAR ELEV 1209.00ft		TOTAL DEPTH 14.70ft		START DATE 9/05/02		COMPLETION DATE 09/05/02							
DRILL MACHINE CME-45			DRILL METHOD H.S. AUGERS			HAMMER TYPE AUTOMATIC							
SURFACE WATER DEPTH			DEPTH TO ROCK N/A			Log BORING 6, Page 1 of 1							
ELEV	DEPTH	BLOW CT			PEN (ft)	BLOWS PER FOOT				SAMPLE NO	LOG	SOIL AND ROCK DESCRIPTION	
		6in	6in	6in		0	25	50	75				100
1209.00	0.00	2	3	4	1.0								
	3.20	2	7	20	1.0	7					SS-9	M	ALLUVIUM: RED, SANDY CLAY
											SS-10	M	ALLUVIUM: YELLOW-BROWN SILTY SAND
1200.00	8.20	9	18	25	1.0	43							ALLUVIUM: YELLOW-BROWN, SILTY SAND AND GRAVEL TO BOULDERS
	13.20	7	17	24	1.0	41						D	SAPROLITE: GRAY-BROWN, MICACEOUS, SILTY SAND
1194.30													TERMINATED BORING IN SAPROLITE AT ELEVATION 1194.3 FEET

JJL
NORTH CAROLINA DEPARTMENT OF TRANSPORTATION
DIVISION OF HIGHWAYS-MATERIALS AND TESTS UNIT E
SOILS TEST REPORT-SOILS LABORATORY

T.I.P. ID #: _____

REPORT ON SAMPLES OF: Soil for Classification

PROJECT:	8.2872001	COUNTY:	McDowell	Owner:	--
DATE SAMPLED:	8-27-02	DATE RECEIVED:	9-10-02	DATE REPORTED:	10-9-02
SAMPLED FROM:	Rdw - Detour	SAMPLED BY:	L L Acker		
SUBMITTED BY:	W D Frye	2002	STANDARD SPECIFICATION		
LABORATORY:	Asheville				

TEST RESULTS

Project Sample No.	SS-1	MS-2	SS-2	MS-2	SS-3	MS-3	SS-4	MS-4
Lab Sample No. A-	138998	138999	139000	139001	139002	139003	139004	139005
HiCAMS Sample #	--	--	--	--	--	--	--	--
Retained #4 Sieve %	--	--	--	--	--	--	--	--
Passing #10 Sieve %	90		92		95		95	
Passing #40 Sieve %	77		70		73		79	
Passing #200 Sieve %	43		28		30		40	

MINUS #10 FRACTION

Soil Mortar - 100%								
Coarse Sand -Ret. #60	25		39		38		28	
Fine Sand - Ret. #270	37		39		39		36	
Silt 0.05-0.005 mm %	22		18		19		28	
Clay < 0.005 mm %	16		4		4		8	
Passing # 40 Sieve %	--		--		--		--	
Passing # 200 Sieve %	--		--		--		--	

Liquid Limit	33		31		30		35	
Plastic Index	7		NP		NP		NP	
AASHTO Classification	A-4 (1)		A-2-4 (0)		A-2-4 (0)		A-4 (1)	
Quantity								
Texture								
Station	14+00 Lt	15+10 Lt	14+00 Lt	15+1- Lt	14+00 Lt	13+00 Lt	15+10 Lt	13+00 :t
Hole No.								
Depth (ft) From:	1.0	3.8	12.3	13.8	22.3	8.6	3.8	18.6
To:	3.0	5.3	13.8	15.3	23.8	10.1	5.3	20.1
% Moisture		6.2		8.2		16.9		14.5

Remarks:
A-138999 thru A-139017

CC:
W D Frye ✓
J J Lail
File

SOILS ENGINEER: *Jessie*

JJL
NORTH CAROLINA DEPARTMENT OF TRANSPORTATION
DIVISION OF HIGHWAYS-MATERIALS AND TESTS UNIT E
SOILS TEST REPORT-SOILS LABORATORY

T.I.P. ID #: _____

REPORT ON SAMPLES OF: Soil for Classification

PROJECT:	8.2872001	COUNTY:	McDowell	Owner:	--
DATE SAMPLED:	9-3-02	DATE RECEIVED:	9-10-02	DATE REPORTED:	10-9-02
SAMPLED FROM:	Rdw	SAMPLED BY:	L L Acker		
SUBMITTED BY:	W D Frye	2002	STANDARD SPECIFICATION		
LABORATORY:	Asheville				

TEST RESULTS

Project Sample No.	SS-5	SS-6	SS-7	SS-8	SS-9	SS-10	SS-11	SS-12
Lab Sample No. A-	139006	139007	139008	139009	139010	139011	139012	139013
HiCAMS Sample #	--	--	--	--	--	--	--	--
Retained #4 Sieve %	--	--	--	--	--	--	--	--
Passing #10 Sieve %	95	90	91	87	97	92	89	97
Passing #40 Sieve %	76	78	79	67	94	82	85	87
Passing #200 Sieve %	35	55	45	33	59	33	49	41

MINUS #10 FRACTION

Soil Mortar - 100%								
Coarse Sand -Ret. #60	33	21	26	36	6	21	9	18
Fine Sand - Ret. #270	39	22	30	32	41	52	48	56
Silt 0.05-0.005 mm %	20	22	32	20	14	13	25	24
Clay < 0.005 mm %	8	35	12	12	39	14	18	2
Passing # 40 Sieve %	--	--	--	--	--	--	--	--
Passing # 200 Sieve %	--	--	--	--	--	--	--	--

Liquid Limit	31	40	34	40	36	24	25	36
Plastic Index	NP	14	NP	NP	17	NP	4	NP
AASHTO Classification	A-2-4 (0)	A-6 (6)	A-4 (2)	A-2-4 (0)	A-6(8)	A-2-4 (0)	A-4 (3)	A-4 (1)
Quantity								
Texture								
Station	15+10 Lt	13+00 Lt	13+00 Lt	13+00 Lt	16+00 Rt	16+00 Rt	16+50 CL	16+50 CL
Hole No.	Detour	kDetour	Detour	Detour	-L-	-L-	-L-	-L-
Depth (ft) From:	13.8	0.0	8.6	18.6	0.0	3.2	0.0	13.6
To:	15.3	1.5	10.1	20.1	1.5	4.7	3.0	15.1

Remarks:
A-138999 thru A-139017

CC:
W D Frye
J J Lail
File

SOILS ENGINEER: _____

JLJ
 NORTH CAROLINA DEPARTMENT OF TRANSPORTATION
 DIVISION OF HIGHWAYS-MATERIALS AND TESTS UNIT
 SOILS TEST REPORT-SOILS LABORATORY

T.I.P. ID #:	
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REPORT ON SAMPLES OF:	Soil for Classification
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PROJECT:	8.2872001	COUNTY:	McDowell	Owner:	--
DATE SAMPLED:	0-6-02	DATE RECEIVED:	9-10-02	DATE REPORTED:	10-9-02
SAMPLED FROM:	Rdw -L-	SAMPLED BY:	L L Acker		
SUBMITTED BY:	W D Frye			2002	STANDARD SPECIFICATION
LABORATORY:	Asheville				

TEST RESULTS

Project Sample No.	SS-13	SS-14	SS-15	SS-16				
Lab Sample No. A-	139014	139015	139016	139017				
HiCAMS Sample #	--	--	--	--				
Retained #4 Sieve %	--	--	--	--				
Passing #10 Sieve %	91	97	100	64				
Passing #40 Sieve %	86	89	91	44				
Passing #200 Sieve %	55	58	44	24				

MINUS #10 FRACTION

Soil Mortar - 100%								
Coarse Sand -Ret. #60	10	14	20	42				
Fine Sand - Ret. #270	39	36	49	28				
Silt 0.05-0.005 mm %	25	21	27	20				
Clay < 0.005 mm %	26	29	4	10				
Passing # 40 Sieve %	--	--	--	--				
Passing # 200 Sieve %	--	--	--	--				

Liquid Limit	33	37	35	24				
Plastic Index	11	15	NP	NP				
AASHTO Classification	A-6 (5)	A-6 (6)	A-4 (2)	A-1-b (0)				
Quantity								
Texture								
Station	16+00 Lt	16+00 Lt	16+00 Lt	11+10 Rt				
Hole No.								
Depth (ft) From:	3.7	8.7	13.7	4.8				
To:	5.2	9.6	15.2	8.6				

Remarks:

A-138999 thru A-139017

CC:

W D Frye	
J J Lail	File

SOILS ENGINEER:	
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