

CONTRACT: ID: R-0609IA

# STATE OF NORTH CAROLINA

DEPARTMENT OF TRANSPORTATION

DIVISION OF HIGHWAYS

GEOTECHNICAL ENGINEERING UNIT

## STRUCTURE SUBSURFACE INVESTIGATION

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	STATE	STATE PROJECT REFERENCE NO.	SHEET NO.	TOTAL SHEETS
	N.C.	34345.11(R-0609IA)	1	15
	STATE PROJ. NO.	F.A. PROJ. NO.	DESCRIPTION	
	34345.11	MAF-F-119-1(D)	P.E. CONST.	

### CAUTION NOTICE

THE SUBSURFACE INFORMATION AND THE SUBSURFACE INVESTIGATION ON WHICH IT IS BASED WAS MADE FOR THE PURPOSE OF STUDY, PLANNING AND DESIGN, AND NOT FOR CONSTRUCTION OR PAY PURPOSES. THE VARIOUS FIELD BORING LOGS, ROCK CORES, AND SOIL TEST DATA AVAILABLE MAY BE REVIEWED OR INSPECTED IN RALEIGH BY CONTACTING THE N.C. DEPARTMENT OF TRANSPORTATION, GEOTECHNICAL UNIT @ (919) 250-4088. NEITHER THE SUBSURFACE PLANS AND REPORTS, NOR THE FIELD BORING LOGS, ROCK CORES, OR SOIL TEST DATA IS PART OF THE CONTRACT.

GENERAL SOIL AND ROCK STRATA DESCRIPTIONS AND INDICATED BOUNDARIES ARE BASED ON A GEOTECHNICAL INTERPRETATION OF ALL AVAILABLE SUBSURFACE DATA AND MAY NOT NECESSARILY REFLECT THE ACTUAL SUBSURFACE CONDITIONS BETWEEN BORINGS OR BETWEEN SAMPLED STRATA WITHIN THE BOREHOLE. THE LABORATORY SAMPLE DATA AND THE IN SITU (ON-PLACE) TEST DATA CAN BE RELIED ON ONLY TO THE DEGREE OF RELIABILITY INHERENT IN THE STANDARD TEST METHOD. THE OBSERVED WATER LEVELS OR SOIL MOISTURE CONDITIONS INDICATED IN THE SUBSURFACE INVESTIGATIONS ARE AS RECORDED AT THE TIME OF THE INVESTIGATION. THESE WATER LEVELS OR SOIL MOISTURE CONDITIONS MAY VARY CONSIDERABLY WITH TIME ACCORDING TO CLIMATIC CONDITIONS INCLUDING TEMPERATURES, PRECIPITATION AND WIND, AS WELL AS OTHER NON-CLIMATIC FACTORS.

THE BIDDER OR CONTRACTOR IS CAUTIONED THAT DETAILS SHOWN ON THE SUBSURFACE PLANS ARE PRELIMINARY ONLY AND IN MANY CASES THE FINAL DESIGN DETAILS ARE DIFFERENT. FOR BIDDING AND CONSTRUCTION PURPOSES, REFER TO THE CONSTRUCTION PLANS AND DOCUMENTS FOR FINAL DESIGN INFORMATION ON THIS PROJECT. THE DEPARTMENT DOES NOT WARRANT OR GUARANTEE THE SUFFICIENCY OR ACCURACY OF THE INVESTIGATION MADE, NOR THE INTERPRETATIONS MADE OR OPINION OF THE DEPARTMENT AS TO THE TYPE OF MATERIALS AND CONDITIONS TO BE ENCOUNTERED. THE BIDDER OR CONTRACTOR IS CAUTIONED TO MAKE SUCH INDEPENDENT SUBSURFACE INVESTIGATIONS AS HE DEEMS NECESSARY TO SATISFY HIMSELF AS TO CONDITIONS TO BE ENCOUNTERED ON THIS PROJECT. THE CONTRACTOR SHALL HAVE NO CLAIM FOR ADDITIONAL COMPENSATION OR FOR AN EXTENSION OF TIME FOR ANY REASON RESULTING FROM THE ACTUAL CONDITIONS ENCOUNTERED AT THE SITE DIFFERING FROM THOSE INDICATED IN THE SUBSURFACE INFORMATION.

STATE PROJECT 34345.1.1 I.D. NO. R-0609IA

F.A. PROJECT MAF-F-119-1(1)

COUNTY GUILFORD

PROJECT DESCRIPTION US 311 HIGH POINT  
EAST BELTWAY FROM US 29-70 TO  
I-85 NORTH OF ARCHDALE

SITE DESCRIPTION STRUCTURE NO. 1  
ON SR 1193 (BAKER RD.) OVER  
PROPOSED US 311 AT STATION  
-Y1- 12 + 66.293

INVESTIGATED BY J.B. BARFIELD PERSONNEL J.L. LOVE

CHECKED BY D.N. ARGENBRIGHT B. SCHULL

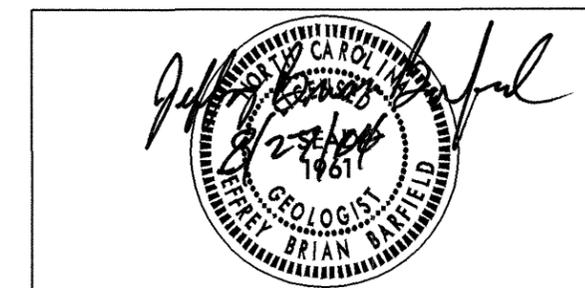
SUBMITTED BY D.N. ARGENBRIGHT D. BOGGS

DATE AUGUST 2004

DRAWN BY: TTW, JBB

NOTE - THE INFORMATION CONTAINED HEREIN IS NOT IMPLIED OR GUARANTEED BY THE N.C. DEPARTMENT OF TRANSPORTATION AS BEING ACCURATE NOR IS IT CONSIDERED TO BE PART OF THE PLANS, SPECIFICATIONS, OR CONTRACT FOR THE PROJECT.

NOTE - BY HAVING REQUESTED THIS INFORMATION THE CONTRACTOR SPECIFICALLY WAIVES ANY CLAIMS FOR INCREASED COMPENSATION OR EXTENSION OF TIME BASED ON DIFFERENCES BETWEEN THE CONDITIONS INDICATED HEREIN AND THE ACTUAL CONDITIONS AT THE PROJECT SITE.



NORTH CAROLINA DEPARTMENT OF TRANSPORTATION  
 DIVISION OF HIGHWAYS  
 GEOTECHNICAL UNIT

ID	STATE PROJECT NO.	SHEET NO.	TOTAL SHEETS
R-06091A	34345.IJ	2	15



SUBSURFACE INVESTIGATION

SOIL AND ROCK LEGEND, TERMS, SYMBOLS, AND ABBREVIATIONS

SOIL DESCRIPTION		GRADATION		ROCK DESCRIPTION		TERMS AND DEFINITIONS																																																																																								
<p>SOIL IS CONSIDERED TO BE THE UNCONSOLIDATED, SEMI-CONSOLIDATED OR WEATHERED EARTH MATERIALS WHICH CAN BE PENETRATED WITH A CONTINUOUS FLIGHT POWER AUGER, AND WHICH YIELDS LESS THAN 100 BLOWS PER 30 cm ACCORDING TO STANDARD PENETRATION TEST (AASHTO T206, ASTM D-1586). SOIL CLASSIFICATION IS BASED ON THE AASHTO SYSTEM AND BASIC DESCRIPTIONS GENERALLY SHALL INCLUDE: CONSISTENCY, COLOR, TEXTURE, MOISTURE, AASHTO CLASSIFICATION, AND OTHER PERTINENT FACTORS SUCH AS MINERALOGICAL COMPOSITION, ANGULARITY, STRUCTURE, PLASTICITY, ETC. EXAMPLE:</p> <p>VERY STIFF, GRAY SILTY CLAY, MOST WITH INTERBEDDED FINE SAND LAYERS, HARD PLASTIC, A-7-6</p>		<p>WELL GRADED - INDICATES A GOOD REPRESENTATION OF PARTICLE SIZES FROM FINE TO COARSE                  UNIFORM - INDICATES THAT SOIL PARTICLES ARE ALL APPROXIMATELY THE SAME SIZE (ALSO POORLY GRADED).                  GAP-GRADED - INDICATES A MIXTURE OF UNIFORM PARTICLES OF TWO OR MORE SIZES.</p> <p>ANGULARITY OF GRAINS</p> <p>THE ANGULARITY OR ROUNDNESS OF SOIL GRAINS ARE DESIGNATED BY THE TERMS: ANGULAR, SUBANGULAR, SUBROUNDED, OR ROUNDED.</p>		<p>HARD ROCK IS NON-COASTAL PLAIN MATERIAL THAT WHEN TESTED, WOULD YIELD SPT REFUSAL. AN INFERRED ROCK LINE INDICATES THE LEVEL AT WHICH NON-COASTAL PLAIN MATERIAL WOULD YIELD SPT REFUSAL.                  SPT REFUSAL IS PENETRATION BY A SPLIT SPOON SAMPLER EQUAL TO OR LESS THAN 2.5 cm PER 50 BLOWS, IN NON-COASTAL PLAIN MATERIAL, THE TRANSITION BETWEEN SOIL AND ROCK IS OFTEN REPRESENTED BY A ZONE OF WEATHERED ROCK.                  ROCK MATERIALS ARE TYPICALLY DIVIDED AS FOLLOWS:</p>		<p>ALLUVIUM (ALLUV.) - SOILS WHICH HAVE BEEN TRANSPORTED BY WATER.                  AUGIFER - A WATER BEARING FORMATION OR STRATA.                  ARENACEOUS - APPLIED TO ROCKS THAT HAVE BEEN DERIVED FROM SAND OR THAT CONTAIN SAND.                  ARGILLACEOUS - APPLIED TO ALL ROCKS OR SUBSTANCES COMPOSED OF CLAY MINERALS, OR HAVING A NOTABLE PROPORTION OF CLAY IN THEIR COMPOSITION, AS SHALE, SLATE, ETC.                  ARTESIAN - GROUND WATER THAT IS UNDER SUFFICIENT PRESSURE TO RISE ABOVE THE LEVEL AT WHICH IT IS ENCOUNTERED, BUT WHICH DOES NOT NECESSARILY RISE TO OR ABOVE THE GROUND SURFACE.                  CALCAREOUS (CALC.) - SOILS WHICH CONTAIN APPRECIABLE AMOUNTS OF CALCIUM CARBONATE.                  COLLUVIUM - ROCK FRAGMENTS MIXED WITH SOIL DEPOSITED BY GRAVITY ON SLOPE OR AT BOTTOM OF SLOPE.                  CORE RECOVERY (REC.) - TOTAL LENGTH OF ALL MATERIAL RECOVERED IN THE CORE BARREL DIVIDED BY TOTAL LENGTH OF CORE RUN AND EXPRESSED AS A PERCENTAGE.                  DIKE - A TABULAR BODY OF IGNEOUS ROCK THAT CUTS ACROSS THE STRUCTURE OF ADJACENT ROCKS OR CUTS MASSIVE ROCK.                  DIP - THE ANGLE AT WHICH A STRATUM OR ANY PLANAR FEATURE IS INCLINED FROM THE HORIZONTAL.                  DIP DIRECTION (DIP AZIMUTH) - THE DIRECTION OR BEARING OF THE HORIZONTAL TRACE OF THE LINE OF DIP, MEASURED CLOCKWISE FROM NORTH.                  FAULT - A FRACTURE OR FRACTURE ZONE ALONG WHICH THERE HAS BEEN DISPLACEMENT OF THE SIDES RELATIVE TO ONE ANOTHER PARALLEL TO THE FRACTURE.                  FISSILE - A PROPERTY OF SPLITTING ALONG CLOSELY SPACED PARALLEL PLANES.                  FLOAT - ROCK FRAGMENTS ON SURFACE NEAR THEIR ORIGINAL POSITION AND DISLOGGED FROM PARENT MATERIAL.                  FLOOD PLAIN (F.P.) - LAND BORDERING A STREAM, BUILT OF SEDIMENTS DEPOSITED BY THE STREAM.                  FORMATION (FM.) - A MAPPABLE GEOLOGIC UNIT THAT CAN BE RECOGNIZED AND TRACED IN THE FIELD.                  JOINT - FRACTURE IN ROCK ALONG WHICH NO APPRECIABLE MOVEMENT HAS OCCURRED.                  LEDGE - A SHELF-LIKE RIDGE OR PROJECTION OF ROCK WHOSE THICKNESS IS SMALL COMPARED TO ITS LATERAL EXTENT.                  LENS - A BODY OF SOIL OR ROCK THAT THINS OUT IN ONE OR MORE DIRECTIONS.                  MOTTLED (MOTL.) - IRREGULARLY MARKED WITH SPOTS OF DIFFERENT COLORS, MOTTLING IN SOILS USUALLY INDICATES POOR AERATION AND LACK OF GOOD DRAINAGE.                  PERCHED WATER - WATER MAINTAINED ABOVE THE NORMAL GROUND WATER LEVEL BY THE PRESENCE OF AN INTERVENING IMPERVIOUS STRATUM.                  RESIDUAL SOIL - SOIL FORMED IN PLACE BY THE WEATHERING OF ROCK.                  ROCK QUALITY DESIGNATION (R.Q.D.) - A MEASURE OF ROCK QUALITY DESCRIBED BY: TOTAL LENGTH OF ROCK SEGMENTS EQUAL TO OR GREATER THAN 10 CENTIMETERS DIVIDED BY THE TOTAL LENGTH OF CORE RUN AND EXPRESSED AS A PERCENTAGE.                  SAPROLITE (SAP.) - RESIDUAL SOIL WHICH RETAINS THE RELIC STRUCTURE OR FABRIC OF THE PARENT ROCK.                  SILL - AN INTRUSIVE BODY OF IGNEOUS ROCK OF APPROXIMATELY UNIFORM THICKNESS AND RELATIVELY THIN COMPARED WITH ITS LATERAL EXTENT, WHICH HAS BEEN EMPLACED PARALLEL TO THE BEDDING OR SCHISTOSITY OF THE INTRUDED ROCKS.                  SLICKENSIDE - POLISHED AND STRIATED SURFACE THAT RESULTS FROM FRICTION ALONG A FAULT OR SLIP PLANE.</p>																																																																																								
<p>SOIL LEGEND AND AASHTO CLASSIFICATION</p> <table border="1"> <tr> <th>GENERAL CLASS.</th> <th colspan="2">GRANULAR MATERIALS (75% PASSING #200)</th> <th colspan="2">SILT-CLAY MATERIALS (75% PASSING #200)</th> <th colspan="2">ORGANIC MATERIALS</th> </tr> <tr> <td>GROUP CLASS.</td> <td>A-1</td> <td>A-3</td> <td>A-2</td> <td>A-4</td> <td>A-5</td> <td>A-7</td> </tr> <tr> <td>SYMBOL</td> <td></td> <td></td> <td></td> <td></td> <td></td> <td></td> </tr> <tr> <td>% PASSING</td> <td>100</td> <td>100</td> <td>100</td> <td>100</td> <td>100</td> <td>100</td> </tr> <tr> <td>LIQUID LIMIT</td> <td>≤ 5</td> <td>≤ 10</td> <td>≤ 15</td> <td>≤ 25</td> <td>≤ 40</td> <td>≤ 60</td> </tr> <tr> <td>PLASTIC INDEX</td> <td>≤ 4</td> <td>≤ 7</td> <td>≤ 10</td> <td>≤ 15</td> <td>≤ 25</td> <td>≤ 40</td> </tr> <tr> <td>GROUP INDEX</td> <td>0</td> <td>0</td> <td>0</td> <td>0</td> <td>0</td> <td>0</td> </tr> <tr> <td>USUAL TYPES OF MAJOR MATERIALS</td> <td>STONE FRAGS, GRAVEL AND SAND</td> <td>FINE SAND</td> <td>SILTY OR CLAYEY GRAVEL AND SAND</td> <td>SILTY SAND</td> <td>CLAYEY SAND</td> <td>CLAYEY SILT</td> </tr> <tr> <td>GEN. 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ARE USED IN DESCRIPTIONS WHENEVER THEY ARE CONSIDERED OF SIGNIFICANCE.</p> <p>COMPRESSIBILITY</p> <p>SLIGHTLY COMPRESSIBLE - LIQUID LIMIT LESS THAN 30                  MODERATELY COMPRESSIBLE - LIQUID LIMIT 31-50                  HIGHLY COMPRESSIBLE - LIQUID LIMIT GREATER THAN 50</p> <p>PERCENTAGE OF MATERIAL</p> <table border="1"> <tr> <th>ORGANIC MATERIAL</th> <th>GRANULAR SOILS</th> <th>SILT-CLAY SOILS</th> <th>OTHER MATERIAL</th> </tr> <tr> <td>TRACE OF ORGANIC MATTER</td> <td>2 - 3%</td> <td>3 - 5%</td> <td>TRACE</td> </tr> <tr> <td>LITTLE ORGANIC MATTER</td> <td>3 - 5%</td> <td>5 - 12%</td> <td>LITTLE</td> </tr> <tr> <td>MODERATELY ORGANIC</td> <td>5 - 10%</td> <td>12 - 20%</td> <td>SOME</td> </tr> <tr> <td>HIGHLY ORGANIC</td> <td>&gt;10%</td> <td>&gt;20%</td> <td>HIGHLY</td> </tr> <tr> <td></td> <td></td> <td></td> <td>35% AND ABOVE</td> </tr> </table>		ORGANIC MATERIAL	GRANULAR SOILS	SILT-CLAY SOILS	OTHER MATERIAL	TRACE OF ORGANIC MATTER	2 - 3%	3 - 5%	TRACE	LITTLE ORGANIC MATTER	3 - 5%	5 - 12%	LITTLE	MODERATELY ORGANIC	5 - 10%	12 - 20%	SOME	HIGHLY ORGANIC	>10%	>20%	HIGHLY				35% AND ABOVE	<p>WEATHERED ROCK (WR)</p> <p>CRYSTALLINE ROCK (CR)</p> <p>NON-CRYSTALLINE ROCK (NCR)</p> <p>COASTAL PLAIN SEDIMENTARY ROCK (CP)</p> <p>WEATHERING</p> <p>FRESH - ROCK FRESH, CRYSTALS BRIGHT, FEW JOINTS MAY SHOW SLIGHT STAINING, ROCK RINGS UNDER HAMMER IF CRYSTALLINE.                  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STATE OF NORTH CAROLINA  
DEPARTMENT OF TRANSPORTATION

Michael F. Easley  
GOVERNOR

P.O. BOX 25201, RALEIGH, N.C. 27611-5201

Lyndo Tippet  
SECRETARY

August 17, 2004

STATE PROJECT: 34345.1.1 (R-0609IA)  
F.A. PROJECT: MAF-F-119-1 (1)  
COUNTY: Guilford  
DESCRIPTION: US 311 High Point East Beltway from US 29-70 to I-85 north of Archdale  
SUBJECT: Geotechnical Report – Structure No. 1 on -Y1- at Sta. 12+66.293 over US 311

**Project Description**

A three span bridge, 77 meters in length, is proposed on -Y1- (SR 1193) over proposed US 311. The project is located in Southwest Guilford County near High Point. The skew is 62° 33' 16".

The subsurface investigation was conducted during June of 2004 using an ATV-mounted CME 45C and B-55 drill machine. Two Standard Penetration Test borings were performed at each of the three proposed bent locations. All borings were advanced until weathered rock or crystalline rock was encountered. B1-A and EB2-A were cored using NXWL to recover samples from crystalline rock. Representative soil samples were obtained for visual classification in the field and selected samples were sent to the Materials and Test Unit for laboratory analysis.

**Physiography and Geology**

The project is located in gently rolling terrain of the Piedmont Physiographic Province. Geologically, the site is located within the Carolina Slate Belt and is underlain by meta-granite and Meta-diorite. The area consists of a mixture of wooded, homes and businesses.

**Soil Properties**

Soils encountered on the project site include roadway embankment and residual soils.

Roadway embankment soils were encountered in all but one of the borings (EB2-B) and range in thickness from 0.45 to 1.65 meters. These soils consist primarily of tan-orange, moist, very loose to medium dense, silty sand (A-2-4) and red-orange, moist, medium stiff, silty clay (A-7-5).

Residual soils were encountered in all of the borings and range in thickness from 0.57 to 3.48 meters. These soils consist primarily of brown to white, dry to moist, loose to medium dense, saprolitic, micaceous, silty sand (A-2-4). Cohesive soils consisted tan-orange, moist, medium stiff to stiff, micaceous, sandy silt (A-4) and tan-orange and orange, black and brown, moist, medium stiff to very stiff, micaceous, sandy and silty clay (A-6, A-7). Residual soils are derived from the underlying weathered rock.

**Rock Properties**

Weathered rock was derived from the underlying meta-granite and meta-diorite and ranges in thickness from 0.23 to 1.10 meters. The top of weathered rock was encountered at elevations ranging from 255.86 to 254.32 meters.

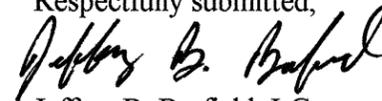
Crystalline rock was encountered at each boring location. The top of the crystalline rock ranges from 254.77 in B1-A to 253.13 in B1-B. Rock core was obtained from B1-A and EB2-A. The crystalline rock in B1-A consisted of a white, pink to gray, slightly to very severely weathered, fresh to moderately hard, closely fractured, meta-granite and a white to dark gray, fresh, hard to very hard, meta-diorite. The crystalline rock encountered in EB2-A consisted of an orange to white, fresh to slightly weathered, hard, moderate to closely fractured, meta-granite. Core recovery (REC) for the meta-granite ranged from 32 to 92 percent and rock quality designation (RQD) ranged from 9 to 61 percent. The core recovery for the meta-diorite was 76 percent and 63 percent rock quality designation (RQD).

**Groundwater**

Groundwater was encountered in three of the six borings. Groundwater elevations ranged from 255.12 to 254.86 meters.

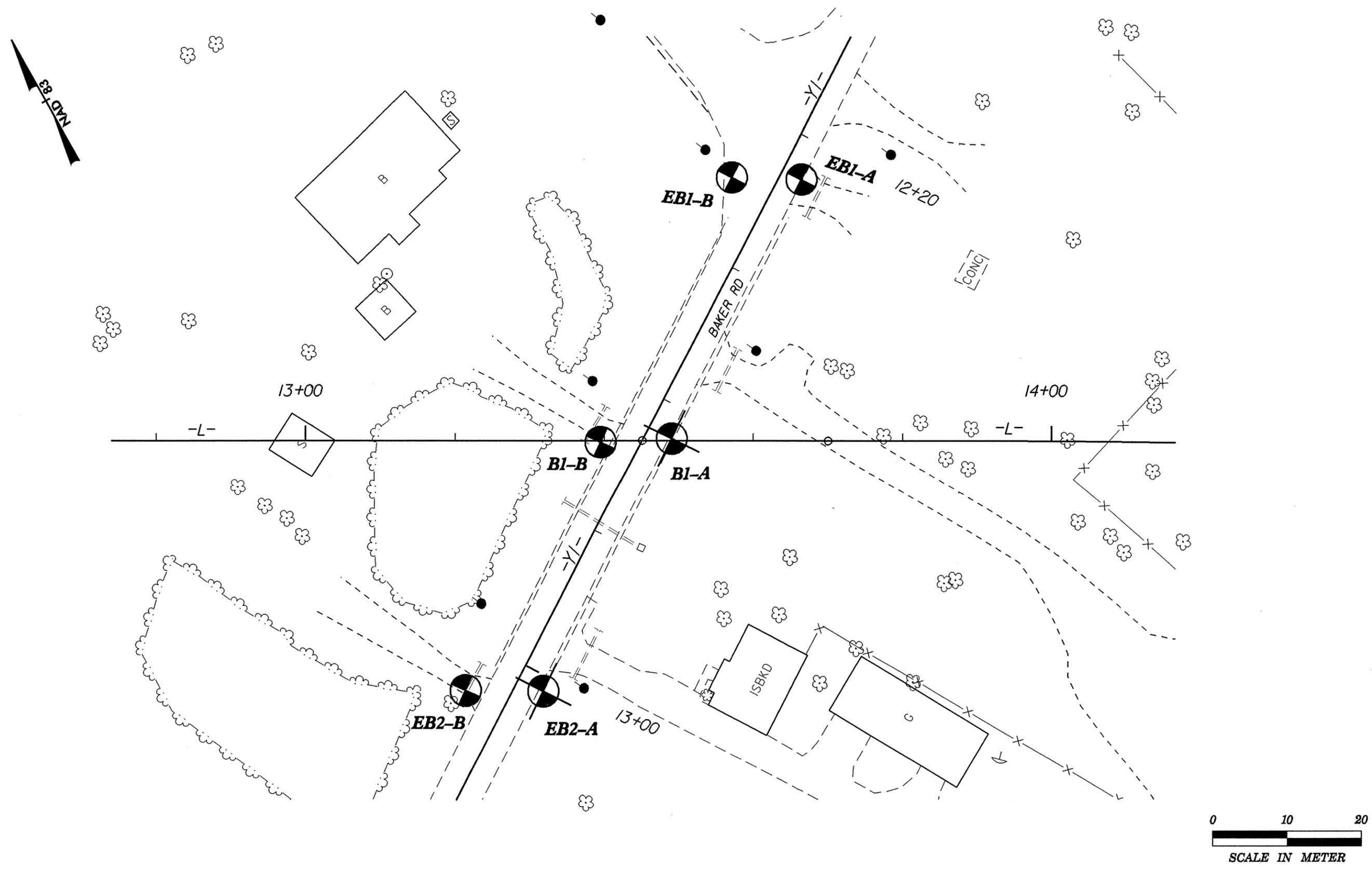
**Notice**

This Geotechnical foundation report is based on the Preliminary General Drawing for Structure No. 1, dated January 14, 2004. If significant changes are made in the design or location of the proposed structure, the subsurface information should be reviewed and modified as necessary.

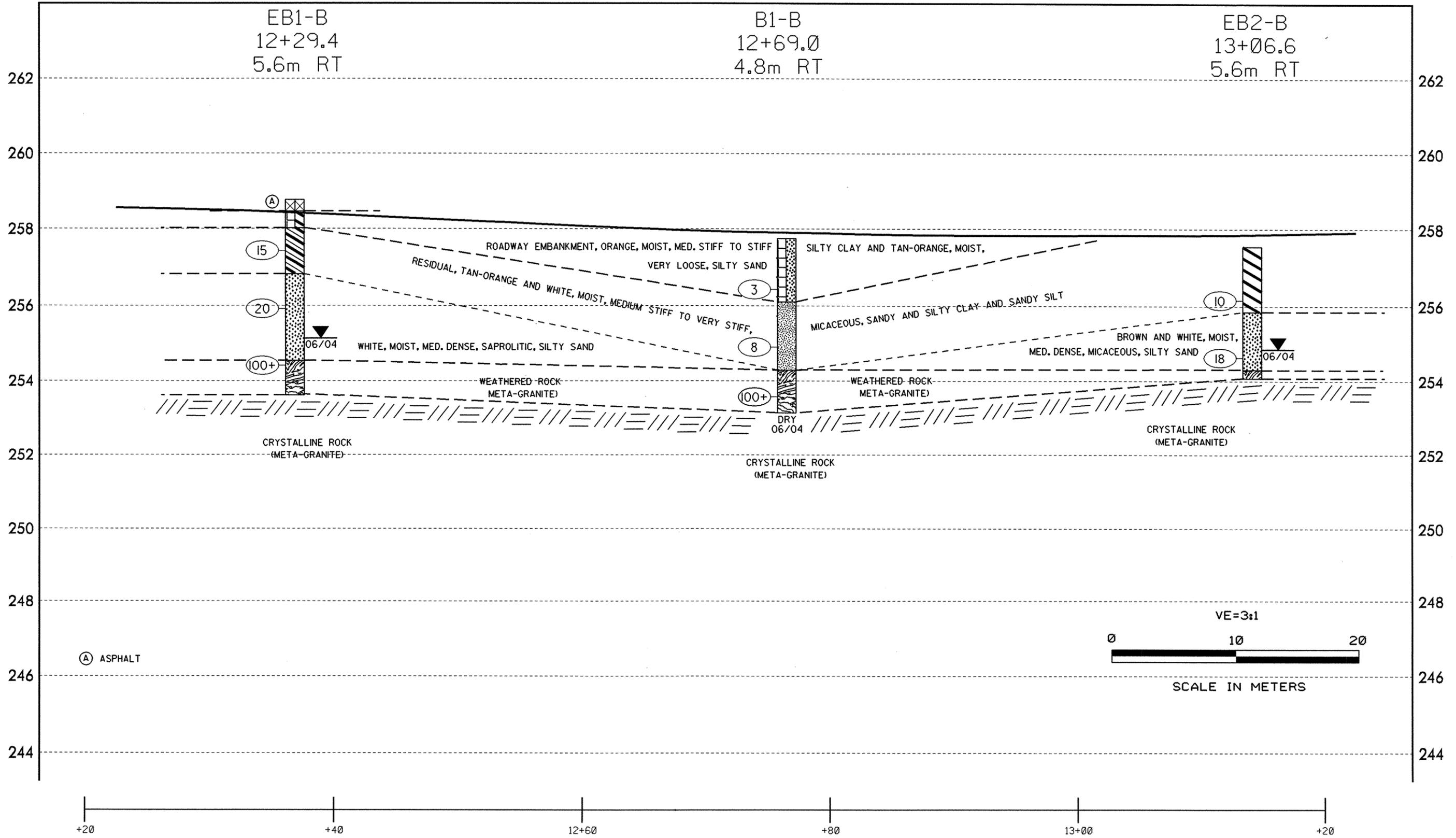
Respectfully submitted,  
  
Jeffrey B. Barfield, LG  
Project Geologist



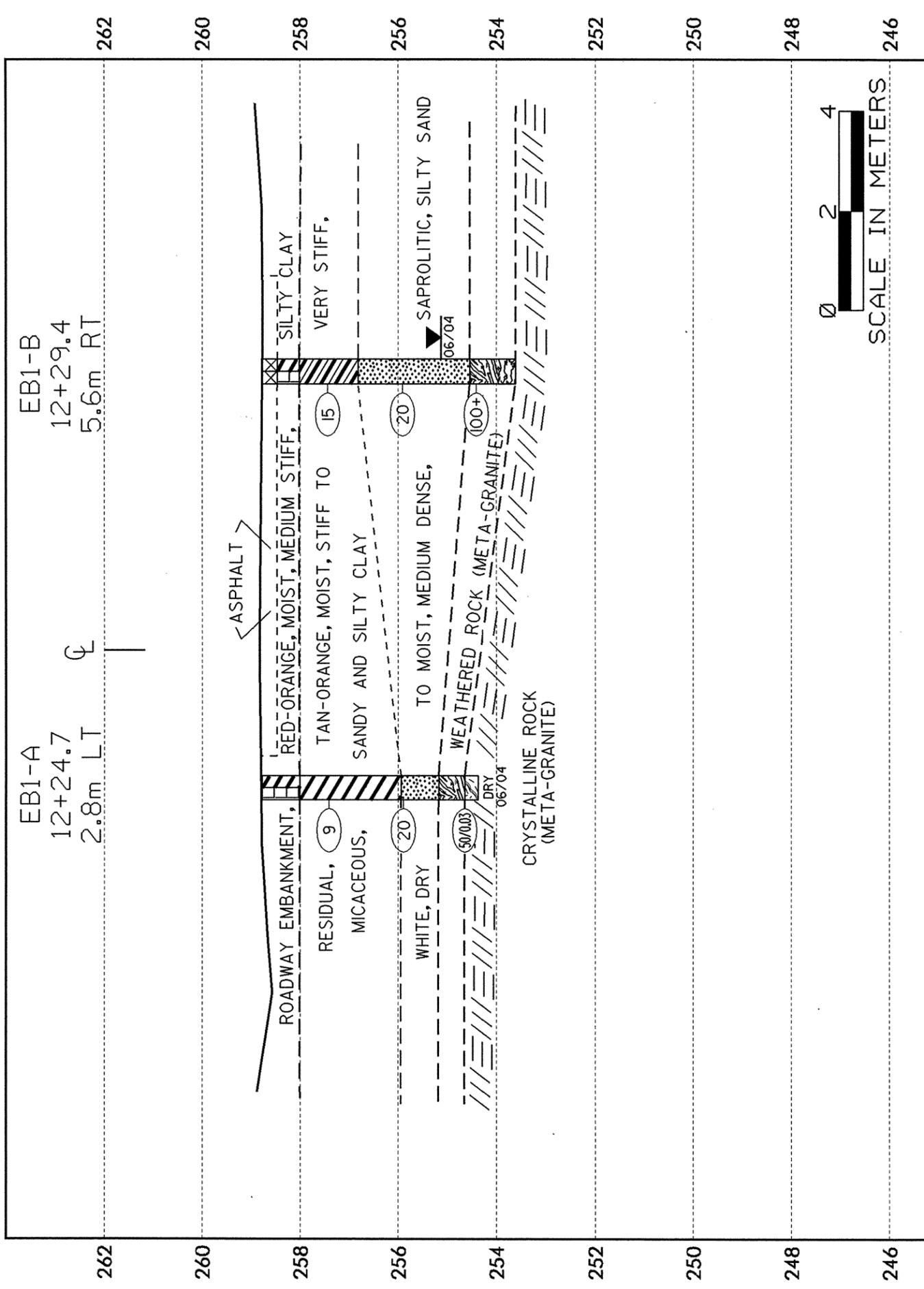
# TEST SITE PLAN



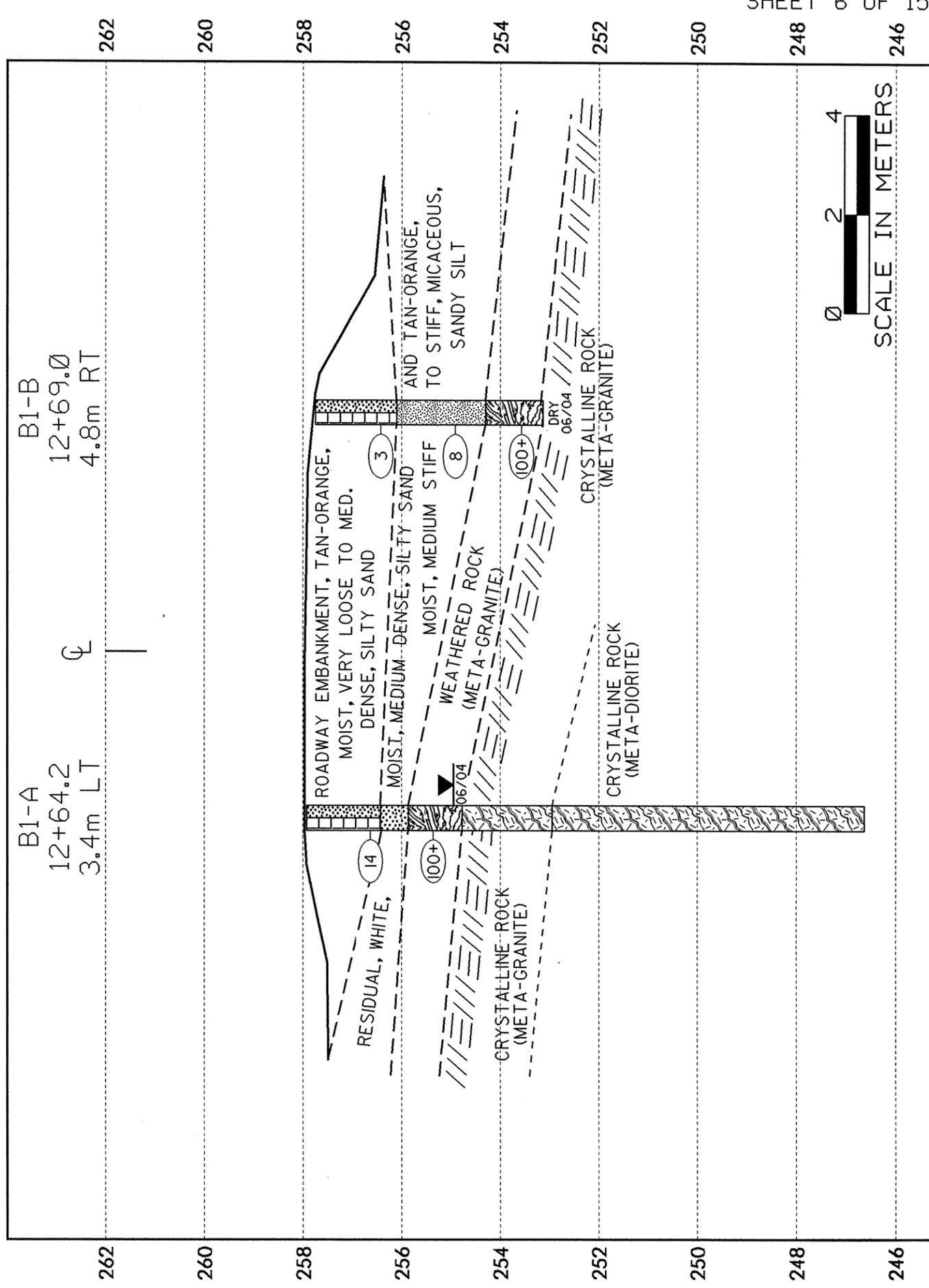
# PROFILE THROUGH BORINGS PROJECTED ALONG -YI-



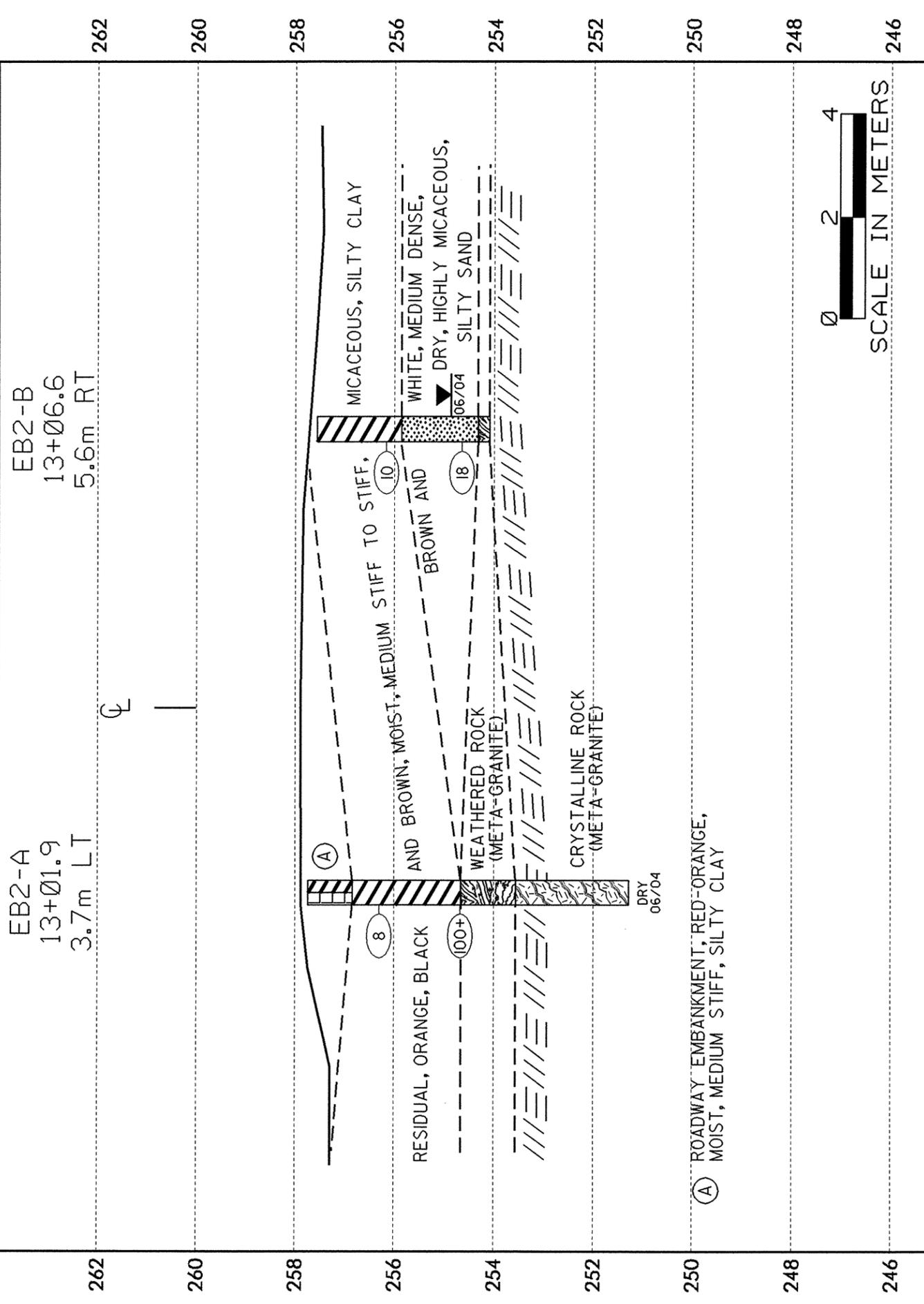
CROSS SECTION THROUGH END BENT I STRUCTURE NO.1, 34345.1.1 (R-0609IA)



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CROSS SECTION THROUGH END BENT 2 STRUCTURE NO. 1, 34345.1.1 (R-Ø6Ø9IA)





NORTH CAROLINA DEPARTMENT OF TRANSPORTATION  
 GEOTECHNICAL UNIT BORING LOG

PROJECT NO. 34345.1.1		ID. R-06091A		COUNTY GUILFORD		GEOLOGIST J.L. LOVE						
SITE DESCRIPTION STRUCTURE NO. 1 ON -Y1- SR 1193 (BAKER RD.) OVER -L- PROPOSED US 311							GROUND WATER					
BORING NO. BI-A		BORING LOCATION 12+64.2		OFFSET 3.4m LT		ALIGNMENT -Y1-						
COLLAR ELEV. 257.93m		NORTHING 243343.4		EASTING 522705.0		0 HR. N/A 24 HR. 2.99m						
TOTAL DEPTH 11.30m		DRILL MACHINE CME-45		DRILL METHOD HSA/NW-CASING/NXWL		HAMMER TYPE MANUAL						
START DATE 6/8/04		COMPLETION DATE 6/9/04		SURFACE WATER DEPTH N/A		DEPTH TO ROCK 3.16m						
ELEV.	DEPTH	BLOW COUNT			PEN. (m)	BLOWS PER 30cm				SAMPLE NUMBER	LOG	SOIL AND ROCK DESCRIPTION
		15cm	15cm	15cm		0	25	50	75			
257.93	1.00	10	7	7	0.30					M		ROADWAY EMBANKMENT, TAN-ORANGE, SILTY SAND WITH SOME GRAVEL
256.00	2.52	100			0.12				100+X	M		RESIDUAL, WHITE, SILTY SAND WEATHERED ROCK (META-GRANITE)
254.00												CRYSTALLINE ROCK (META-GRANITE)
252.00												REC=32%, RQD=9%
250.00												(META-DIORITE)
248.00										RS-4		REC=76%, RQD=63%
246.00												CORING TERMINATED AT ELEVATION 246.63 METERS IN CRYSTALLINE ROCK (META-DIORITE)
244.00												
242.00												
240.00												
238.00												

CORE BORING REPORT							
PROJECT: 34345.1.1		ID: R-06091A		COUNTY: GUILFORD		BORING NO: B1-A	
DESCRIPTION: STRUCTURE NO. 1 ON -Y1- SR 1193 OVER -L- PROPOSED US 311							
LOCATION OF BORING: -Y1-, 12+64.2, 3.4m LT				COMPLETION DATE: 6/9/04			
COLLAR or GROUND ELEVATION: 257.93 m				CORE SIZE: NXWL		GEOLOGIST: J.L. LOVE	
CORE EQUIPMENT: CME-45, NXWL,				DRILLER: BRYAN SCHULL			
ELEV (m)	DEPTH (m)	DRILL RATE (min/0.3m)	RUN (m)	REC (%)	RQD (%)	SAMPLE NUMBER	FIELD CLASSIFICATION and REMARKS
254.77	3.16	1:59					WHITE, SLIGHTLY WEATHERED, MODERATELY HARD, META-GRANITE ALL BUT 0.11m WASHED AWAY
		1:17		0.11	0.00		
		1:24	0.91	(12%)	(0%)		
253.86	4.07						PINK-GRAY, FRESH, HARD, CLOSE FRACTURED, META-GRANITE, (4.55-4.98) SEVERELY WEATHERED, HIGHLY MICACEOUS
253.86	4.07	1:47		0.47	0.16		
		4:15	0.91	(52%)	(18%)		
252.95	4.98						DARK GRAY TO WHITE, FRESH, HARD, META-DIORITE
252.95	4.98	20:00					
		8:30		0.77	0.27		
		14:50	1.52	(51%)	(18%)		
251.43	6.50	17:39					WHITE, FRESH, HARD, META-DIORITE
251.43	6.50	20:00					
		8:30		1.16	0.88		
		14:50	1.52	(76%)	(58%)		
249.91	8.02	17:39					WHITE, FRESH, HARD, VERY THICKLY BEDDED, META-DIORITE
249.91	8.02	15:10					
		12:15		1.40	1.40	RS-4	
		8:22	1.52	(92%)	(92%)		
248.39	9.54	21:20					WHITE AND DARK GRAY, FRESH, VERY HARD, VERY THICKLY BEDDED, META-DIORITE
248.39	9.54	9:40					
		10:32		1.40	1.40		
		18:29	1.52	(92%)	(92%)		
246.87	11.06	29:42					WHITE, FRESH, VERY HARD, META-DIORITE
246.87	11.06	45:49/0.24					
				0.24	0.00		
246.63	11.30						
BOREHOLE TERMINATED AT ELEVATION OF 246.63 METERS, IN META-DIORITE.							







**EB1-A**

<b>SOIL TEST RESULTS</b>															
SAMPLE NO.	OFFSET	STATION	DEPTH INTERVAL	AASHTO CLASS.	L.L.	P.I.	% BY WEIGHT				% PASSING (SIEVES)			% MOISTURE	% ORGANIC
							C.SAND	F.SAND	SILT	CLAY	10	40	200		
S-5	2.8m LT	12+24.7	0.00-0.77	A-7-5(66)	91	57	1.6	3.7	23.6	71.1	100	99	96	-	-
SS-6	2.8m LT	12+24.7	1.07-1.52	A-7-5(22)	57	18	0.6	14.6	50.2	34.6	100	100	92	-	-
SS-7	2.8m LT	12+24.7	2.84-3.04	A-2-4(0)	26	NP	41.3	29.7	2.6	26.4	97	71	34	-	-

**EB1-B**

<b>SOIL TEST RESULTS</b>															
SAMPLE NO.	OFFSET	STATION	DEPTH INTERVAL	AASHTO CLASS.	L.L.	P.I.	% BY WEIGHT				% PASSING (SIEVES)			% MOISTURE	% ORGANIC
							C.SAND	F.SAND	SILT	CLAY	10	40	200		
SS-8	5.6m RT	12+29.4	1.04-1.49	A-6(4)	36	14	18.9	37.0	15.7	28.5	100	92	50	-	-

**B1-B**

<b>SOIL TEST RESULTS</b>															
SAMPLE NO.	OFFSET	STATION	DEPTH INTERVAL	AASHTO CLASS.	L.L.	P.I.	% BY WEIGHT				% PASSING (SIEVES)			% MOISTURE	% ORGANIC
							C.SAND	F.SAND	SILT	CLAY	10	40	200		
SS-3	4.8m RT	12+69.0	1.03-1.48	A-2-4(0)	22	5	29.1	40.7	10.0	20.3	93	81	34	-	-
SS-4	4.8m RT	12+69.0	2.55-3.00	A-4(2)	36	4	11.4	37.4	28.9	22.4	100	96	61	-	-

**EB2-A**

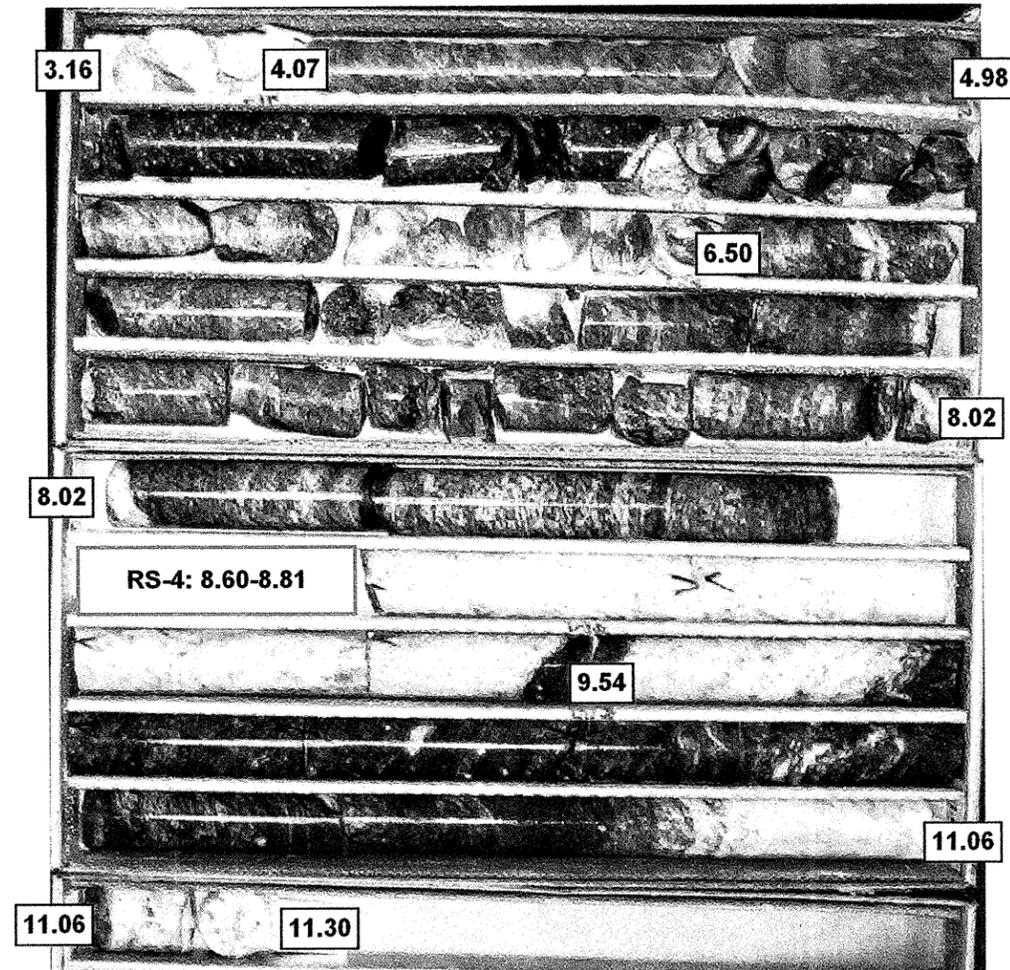
<b>SOIL TEST RESULTS</b>															
SAMPLE NO.	OFFSET	STATION	DEPTH INTERVAL	AASHTO CLASS.	L.L.	P.I.	% BY WEIGHT				% PASSING (SIEVES)			% MOISTURE	% ORGANIC
							C.SAND	F.SAND	SILT	CLAY	10	40	200		
SS-9	3.7m LT	13+01.9	1.14-1.59	A-7-5(15)	51	17	0.8	30.7	40.2	28.3	100	100	77	-	-

**EB2-B**

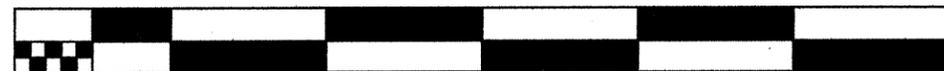
<b>SOIL TEST RESULTS</b>															
SAMPLE NO.	OFFSET	STATION	DEPTH INTERVAL	AASHTO CLASS.	L.L.	P.I.	% BY WEIGHT				% PASSING (SIEVES)			% MOISTURE	% ORGANIC
							C.SAND	F.SAND	SILT	CLAY	10	40	200		
SS-1	5.6m RT	13+06.6	1.10-1.55	A-7-6(13)	49	20	13.8	28.7	27.0	30.5	100	92	66	-	-
SS-2	5.6m RT	13+06.6	2.62-3.07	A-2-4(0)	27	NP	39.8	33.9	16.1	10.2	98	75	32	-	-

CORE PHOTOGRAPHS  
B1-A

BOXES 1, 2 & 3: 3.16 - 11.30 METERS



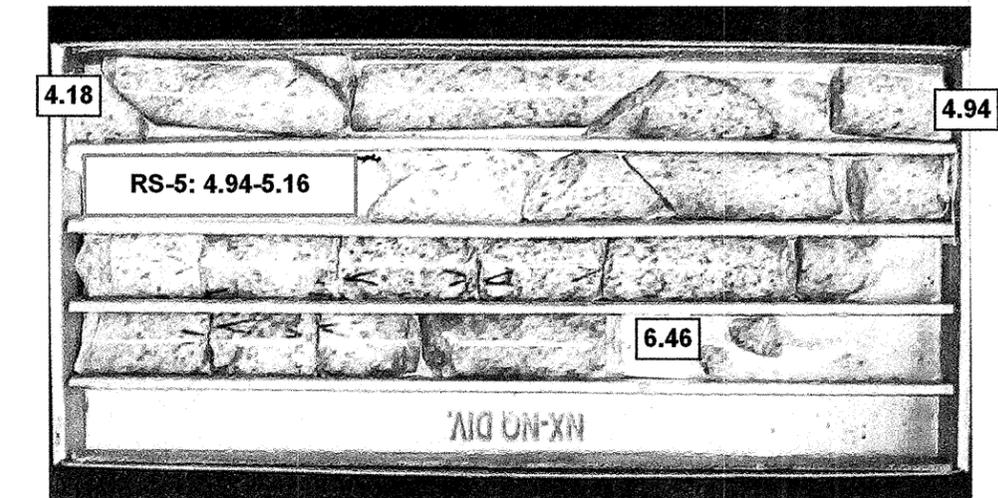
0.0 0.1 0.2 0.3 0.4 0.5 0.6



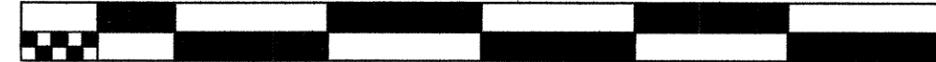
METERS

CORE PHOTOGRAPHS  
EB2-A

BOX 1: 4.18 - 6.46 METERS

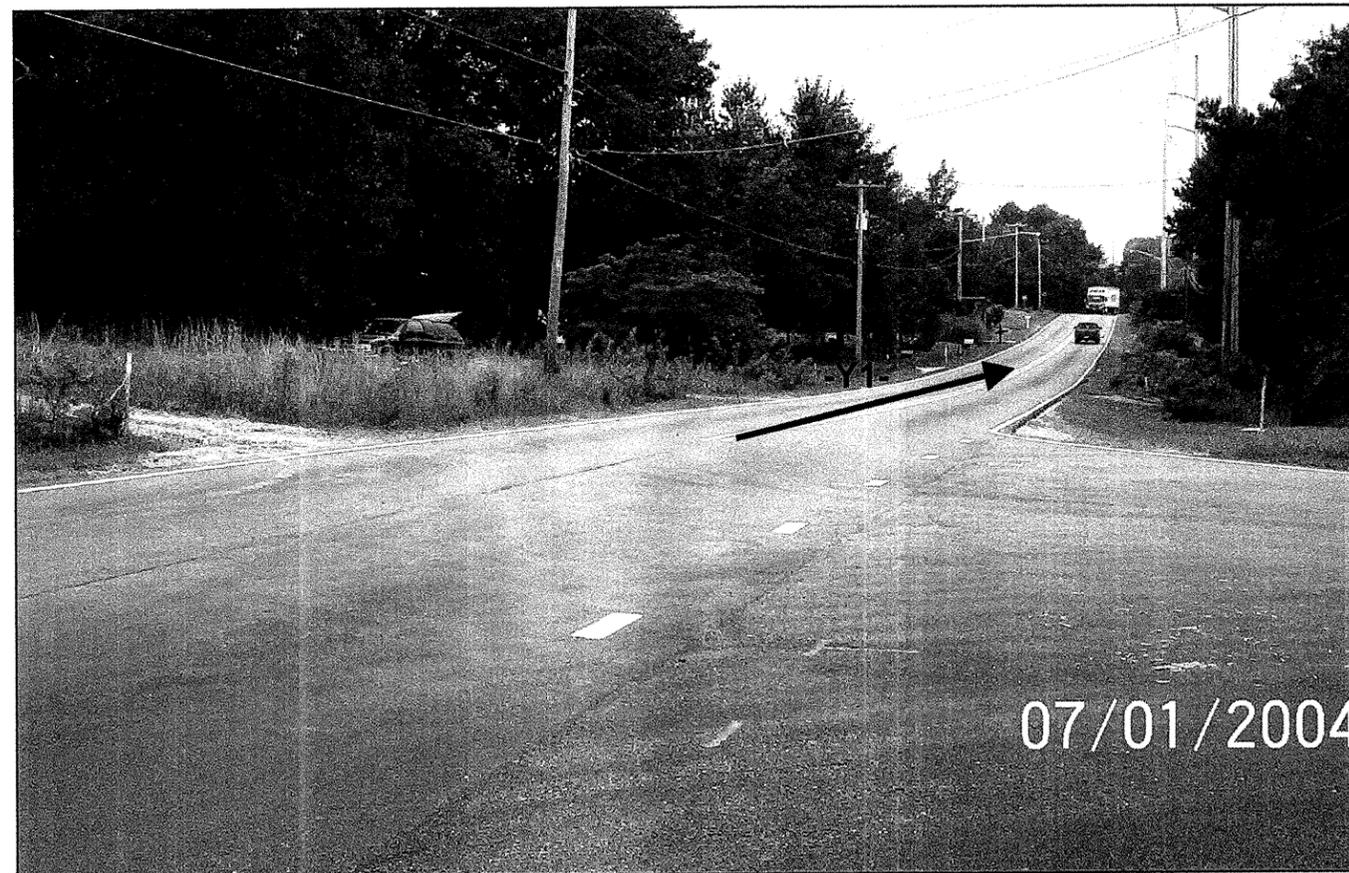


0.0 0.1 0.2 0.3 0.4 0.5 0.6



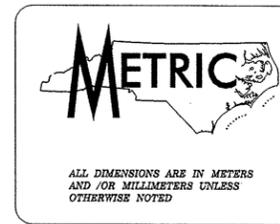
METERS

# ***SITE PHOTOGRAPH***



PROJECT: 34345.1.1 ID. R-0609IA

STATE OF NORTH CAROLINA  
DEPARTMENT OF TRANSPORTATION  
DIVISION OF HIGHWAYS  
GEOTECHNICAL UNIT



STATE	STATE PROJECT REFERENCE NO.	SHEET NO.	TOTAL SHEETS
N.C.	R-0609IA	1	19
STATE PROJ. NO.	F.A. PROJ. NO.	DESCRIPTION	
34345.1.1		P.E. CONST.	

**STRUCTURE  
SUBSURFACE INVESTIGATION**

STATE PROJECT 34345.1.1 I.D. NO. R-0609IA

F.A. PROJECT \_\_\_\_\_

COUNTY GUILFORD

PROJECT DESCRIPTION US 311 HIGH POINT EAST  
BELTWAY FROM US 29-70 TO NORTH OF  
NC 62 NORTH OF ARCHDALE

SITE DESCRIPTION BRIDGE ON SR 1158 (-Y2-)  
OVER US 311 AND MILE BRANCH

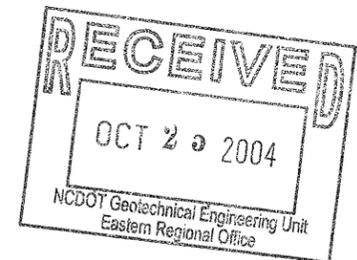
**INVENTORY**

**CAUTION NOTICE**

THE SUBSURFACE INFORMATION AND THE SUBSURFACE INVESTIGATION ON WHICH IT IS BASED WAS MADE FOR THE PURPOSE OF STUDY, PLANNING AND DESIGN, AND NOT FOR CONSTRUCTION OR PAY PURPOSES. THE VARIOUS FIELD BORING LOGS, ROCK CORES, AND SOIL TEST DATA AVAILABLE MAY BE REVIEWED OR INSPECTED IN RALEIGH BY CONTACTING THE N.C. DEPARTMENT OF TRANSPORTATION, GEOTECHNICAL UNIT @ (919) 250-4088. NEITHER THE SUBSURFACE PLANS AND REPORTS, NOR THE FIELD BORING LOGS, ROCK CORES, OR SOIL TEST DATA IS PART OF THE CONTRACT.

GENERAL SOIL AND ROCK STRATA DESCRIPTIONS AND INDICATED BOUNDARIES ARE BASED ON A GEOTECHNICAL INTERPRETATION OF ALL AVAILABLE SUBSURFACE DATA AND MAY NOT NECESSARILY REFLECT THE ACTUAL SUBSURFACE CONDITIONS BETWEEN BORINGS OR BETWEEN SAMPLED STRATA WITHIN THE BOREHOLE. THE LABORATORY SAMPLE DATA AND THE IN SITU (IN-PLACE) TEST DATA CAN BE RELIED ON ONLY TO THE DEGREE OF RELIABILITY INHERENT IN THE STANDARD TEST METHOD. THE OBSERVED WATER LEVELS OR SOIL MOISTURE CONDITIONS INDICATED IN THE SUBSURFACE INVESTIGATIONS ARE AS RECORDED AT THE TIME OF THE INVESTIGATION. THESE WATER LEVELS OR SOIL MOISTURE CONDITIONS MAY VARY CONSIDERABLY WITH TIME ACCORDING TO CLIMATIC CONDITIONS INCLUDING TEMPERATURES, PRECIPITATION AND WIND, AS WELL AS OTHER NON-CLIMATIC FACTORS.

THE BIDDER OR CONTRACTOR IS CAUTIONED THAT DETAILS SHOWN ON THE SUBSURFACE PLANS ARE PRELIMINARY ONLY AND IN MANY CASES THE FINAL DESIGN DETAILS ARE DIFFERENT. FOR BIDDING AND CONSTRUCTION PURPOSES, REFER TO THE CONSTRUCTION PLANS AND DOCUMENTS FOR FINAL DESIGN INFORMATION ON THIS PROJECT. THE DEPARTMENT DOES NOT WARRANT OR GUARANTEE THE SUFFICIENCY OR ACCURACY OF THE INVESTIGATION MADE, NOR THE INTERPRETATIONS MADE OR OPINION OF THE DEPARTMENT AS TO THE TYPE OF MATERIALS AND CONDITIONS TO BE ENCOUNTERED. THE BIDDER OR CONTRACTOR IS CAUTIONED TO MAKE SUCH INDEPENDENT SUBSURFACE INVESTIGATIONS AS HE DEEMS NECESSARY TO SATISFY HIMSELF AS TO CONDITIONS TO BE ENCOUNTERED ON THIS PROJECT. THE CONTRACTOR SHALL HAVE NO CLAIM FOR ADDITIONAL COMPENSATION OR FOR AN EXTENSION OF TIME FOR ANY REASON RESULTING FROM THE ACTUAL CONDITIONS ENCOUNTERED AT THE SITE DIFFERING FROM THOSE INDICATED IN THE SUBSURFACE INFORMATION.



INVESTIGATED BY J.E. BEVERLY PERSONNEL R.W. TODD

CHECKED BY C.B. LITTLE R.S. HINSON

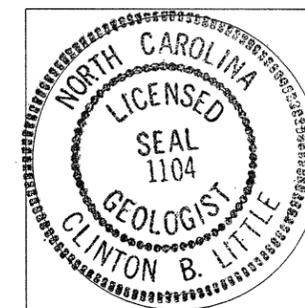
SUBMITTED BY C.B. LITTLE M.L. SMITH

DATE OCTOBER 2004

DRAWN BY: J.K. McCLURE

NOTE - THE INFORMATION CONTAINED HEREIN IS NOT IMPLIED OR GUARANTEED BY THE N.C. DEPARTMENT OF TRANSPORTATION AS BEING ACCURATE NOR IT IS CONSIDERED TO BE PART OF THE PLANS, SPECIFICATIONS, OR CONTRACT FOR THE PROJECT.

NOTE - BY HAVING REQUESTED THIS INFORMATION THE CONTRACTOR SPECIFICALLY WAIVES ANY CLAIMS FOR INCREASED COMPENSATION OR EXTENSION OF TIME BASED ON DIFFERENCES BETWEEN THE CONDITIONS INDICATED HEREIN AND THE ACTUAL CONDITIONS AT THE PROJECT SITE.



SEAL 10-20-04  
[Signature]  
SIGNATURE

NORTH CAROLINA DEPARTMENT OF TRANSPORTATION  
DIVISION OF HIGHWAYS  
GEOTECHNICAL UNIT



## SUBSURFACE INVESTIGATION

### SOIL AND ROCK LEGEND, TERMS, SYMBOLS, AND ABBREVIATIONS

SOIL DESCRIPTION				GRADATION				ROCK DESCRIPTION				TERMS AND DEFINITIONS											
SOIL IS CONSIDERED TO BE THE UNCONSOLIDATED, SEMI-CONSOLIDATED OR WEATHERED EARTH MATERIALS WHICH CAN BE PENETRATED WITH A CONTINUOUS FLIGHT POWER AUGER, AND WHICH YIELDS LESS THAN 100 BLOWS PER 30 cm ACCORDING TO STANDARD PENETRATION TEST (ASTM D-1586). SOIL CLASSIFICATION IS BASED ON THE AASHTO SYSTEM AND BASIC DESCRIPTIONS GENERALLY SHALL INCLUDE: CONSISTENCY, COLOR, TEXTURE, MOISTURE, AASHTO CLASSIFICATION, AND OTHER PERTINENT FACTORS SUCH AS MINERALOGICAL COMPOSITION, ANGULARITY, STRUCTURE, PLASTICITY, ETC. EXAMPLE:  <i>VERY STIFF, GRAY SILTY CLAY, MOST WITH INTERBEDDED FINE SAND LAYERS, HIGHLY PLASTIC, A-7-6</i>				WELL-GRADED- INDICATES A GOOD REPRESENTATION OF PARTICLE SIZES FROM FINE TO COARSE UNIFORM- INDICATES THAT SOIL PARTICLES ARE ALL APPROXIMATELY THE SAME SIZE (ALSO POORLY GRADED). GAP-GRADED- INDICATES A MIXTURE OF UNIFORM PARTICLES OF TWO OR MORE SIZES.  ANGULARITY OF GRAINS  THE ANGULARITY OR ROUNDNESS OF SOIL GRAINS ARE DESIGNATED BY THE TERMS: <u>ANGULAR</u> , <u>SUBANGULAR</u> , <u>SUBROUNDED</u> , OR <u>ROUNDED</u> .				HARD ROCK IS NON-COASTAL PLAIN MATERIAL THAT WHEN TESTED, WOULD YIELD SPT REFUSAL, AN INFERRED ROCK LINE INDICATES THE LEVEL AT WHICH NON-COASTAL PLAIN MATERIAL WOULD YIELD SPT REFUSAL. SPT REFUSAL IS PENETRATION BY A SPLIT SPOON SAMPLER EQUAL TO OR LESS THAN 2.5 cm PER 50 BLOWS. IN NON-COASTAL PLAIN MATERIAL, THE TRANSITION BETWEEN SOIL AND ROCK IS OFTEN REPRESENTED BY A ZONE OF WEATHERED ROCK. ROCK MATERIALS ARE TYPICALLY DIVIDED AS FOLLOWS:  WEATHERED ROCK (WR)  NON-COASTAL PLAIN MATERIAL THAT YIELDS SPT N VALUES > 100 BLOWS PER 30 cm.  CRYSTALLINE ROCK (CR)  FINE TO COARSE GRAIN IGNEOUS AND METAMORPHIC ROCK THAT WOULD YIELD SPT REFUSAL IF TESTED. ROCK TYPE INCLUDES GRANITE, GNEISS, GABBRO, SCHIST, ETC.  NON-CRYSTALLINE ROCK (NCR)  FINE TO COARSE GRAIN METAMORPHIC AND NON-COASTAL PLAIN SEDIMENTARY ROCK THAT WOULD YIELD SPT REFUSAL IF TESTED. ROCK TYPE INCLUDES PHYLLITE, SLATE, SANDSTONE, ETC.  COASTAL PLAIN SEDIMENTARY ROCK (CP)  COASTAL PLAIN SEDIMENTS CEMENTED INTO ROCK, BUT MAY NOT YIELD SPT REFUSAL. ROCK TYPE INCLUDES LIMESTONE, SANDSTONE, CEMENTED SHELL BEDS, ETC.				ALLUVIUM (ALLUV.) - SOILS WHICH HAVE BEEN TRANSPORTED BY WATER. AQUIFER - A WATER BEARING FORMATION OR STRATA. ARENACEOUS - APPLIED TO ROCKS THAT HAVE BEEN DERIVED FROM SAND OR THAT CONTAIN SAND. ARGILLACEOUS - APPLIED TO ALL ROCKS OR SUBSTANCES COMPOSED OF CLAY MINERALS, OR HAVING A NOTABLE PROPORTION OF CLAY IN THEIR COMPOSITION, AS SHALE, SLATE, ETC. ARTESIAN - GROUND WATER THAT IS UNDER SUFFICIENT PRESSURE TO RISE ABOVE THE LEVEL AT WHICH IS ENCOUNTERED, BUT WHICH DOES NOT NECESSARILY RISE TO OR ABOVE THE GROUND SURFACE. CALCAREOUS (CALC.) - SOILS WHICH CONTAIN APPRECIABLE AMOUNTS OF CALCIUM CARBONATE. COLLUVIUM - ROCK FRAGMENTS MIXED WITH SOIL DEPOSITED BY GRAVITY ON SLOPE OR AT BOTTOM OF SLOPE. CORE RECOVERY (REC.) - TOTAL LENGTH OF ALL MATERIAL RECOVERED IN THE CORE BARREL DIVIDED BY TOTAL LENGTH OF CORE RUN AND EXPRESSED AS A PERCENTAGE. DIKE - A TABULAR BODY OF IGNEOUS ROCK THAT CUTS ACROSS THE STRUCTURE OF ADJACENT ROCKS OR CUTS MASSIVE ROCK. DIP - THE ANGLE AT WHICH A STRATUM OR ANY PLANAR FEATURE IS INCLINED FROM THE HORIZONTAL. DIP DIRECTION (DIP AZIMUTH) - THE DIRECTION OR BEARING OF THE HORIZONTAL TRACE OF THE LINE OF DIP, MEASURED CLOCKWISE FROM NORTH. FAULT - A FRACTURE OR FRACTURE ZONE ALONG WHICH THERE HAS BEEN DISPLACEMENT OF THE SIDES RELATIVE TO ONE ANOTHER PARALLEL TO THE FRACTURE. FISSILE - A PROPERTY OF SPLITTING ALONG CLOSELY SPACED PARALLEL PLANES. FLOAT - ROCK FRAGMENTS ON SURFACE NEAR THEIR ORIGINAL POSITION AND DISLOGGED FROM PARENT MATERIAL. FLOOD PLAIN (F.P.) - LAND BORDERING A STREAM, BUILT OF SEDIMENTS DEPOSITED BY THE STREAM. FORMATION (FM) - A MAPPABLE GEOLOGIC UNIT THAT CAN BE RECOGNIZED AND TRACED IN THE FIELD. JOINT - FRACTURE IN ROCK ALONG WHICH NO APPRECIABLE MOVEMENT HAS OCCURRED. LEDGE - A SHELF-LIKE RIDGE OR PROJECTION OF ROCK WHOSE THICKNESS IS SMALL COMPARED TO ITS LATERAL EXTENT. LENS - A BODY OF SOIL OR ROCK THAT THINS OUT IN ONE OR MORE DIRECTIONS. MOTTLED (MOT.) - IRREGULARLY MARKED WITH SPOTS OF DIFFERENT COLORS. MOTTLING IN SOILS USUALLY INDICATES POOR AERATION AND LACK OF GOOD DRAINAGE. PERCHED WATER - WATER MAINTAINED ABOVE THE NORMAL GROUND WATER LEVEL BY THE PRESENCE OF AN INTERVENING IMPERVIOUS STRATUM. RESIDUAL SOIL - SOIL FORMED IN PLACE BY THE WEATHERING OF ROCK. ROCK QUALITY DESIGNATION (R.Q.D.) - A MEASURE OF ROCK QUALITY DESCRIBED BY: TOTAL LENGTH OF ROCK SEGMENTS EQUAL TO OR GREATER THAN 10 CENTIMETERS DIVIDED BY THE TOTAL LENGTH OF CORE RUN AND EXPRESSED AS A PERCENTAGE. SAPROLITE (SAP.) - RESIDUAL SOIL WHICH RETAINS THE RELIC STRUCTURE OR FABRIC OF THE PARENT ROCK. SILL - AN INTRUSIVE BODY OF IGNEOUS ROCK OF APPROXIMATELY UNIFORM THICKNESS AND RELATIVELY THIN COMPARED WITH ITS LATERAL EXTENT, WHICH HAS BEEN EMPLACED PARALLEL TO THE BEDDING OR SCHISTOSITY OF THE INTRUDED ROCKS. SLICKENSIDE - POLISHED AND STRIATED SURFACE THAT RESULTS FROM FRICTION ALONG A FAULT OR SLIP PLANE.											
SOIL LEGEND AND AASHTO CLASSIFICATION				MINERALOGICAL COMPOSITION				COMPRESSION				PERCENTAGE OF MATERIAL											
GENERAL CLASS. GRANULAR MATERIALS (< 75% PASSING #200) SILT-CLAY MATERIALS (> 75% PASSING #200) ORGANIC MATERIALS				MINERAL NAMES SUCH AS QUARTZ, FELDSPAR, MICA, TALC, KAOLIN, ETC. ARE USED IN DESCRIPTIONS WHENEVER THEY ARE CONSIDERED OF SIGNIFICANCE.				SLIGHTLY COMPRESSIBLE MODERATELY COMPRESSIBLE HIGHLY COMPRESSIBLE				LIQUID LIMIT LESS THAN 30 LIQUID LIMIT 31-50 LIQUID LIMIT GREATER THAN 50											
GROUP CLASS. A-1, A-1-b, A-3, A-2-4, A-2-5, A-2-6, A-2-7, A-4, A-5, A-6, A-7, A-7-5, A-7-6, A-3, A-4, A-5, A-6, A-7				ORGANIC MATERIALS				TRACE OF ORGANIC MATTER LITTLE ORGANIC MATTER MODERATELY ORGANIC HIGHLY ORGANIC				GRANULAR SOILS SILT-CLAY SOILS OTHER MATERIAL											
SYMBOL				SOILS WITH LITTLE OR MODERATE AMOUNTS OF ORGANIC MATTER				TRACE 1 - 10% LITTLE 10 - 20% SOME 20 - 35% HIGHLY 35% AND ABOVE				GRANULAR SOILS SILT-CLAY SOILS OTHER MATERIAL											
% PASSING # 10, # 40, # 200				GROUP INDEX				USUAL TYPES OF MAJOR MATERIALS				GENERAL RATING AS A SUBGRADE											
LIQUID LIMIT PLASTIC INDEX				GENERAL RATING AS A SUBGRADE				EXCELLENT TO GOOD				FAIR TO POOR											
P.I. OF A-7-5 ≤ L.L. - 30 : P.I. OF A-7-6 > L.L. - 30				FAIR TO POOR				POOR				UNSATURABLE											
CONSISTENCY OR DENSENESS				MISCELLANEOUS SYMBOLS				GROUND WATER				WEATHERING											
PRIMARY SOIL TYPE COMPACTNESS OR CONSISTENCY RANGE OF STANDARD PENETRATION RESISTANCE (N-VALUE) RANGE OF UNCONFINED COMPRESSIVE STRENGTH (kN/m <sup>2</sup> )				ROADWAY EMBANKMENT WITH SOIL DESCRIPTION SOIL SYMBOL ARTIFICIAL FILL OTHER THAN ROADWAY EMBANKMENTS INFERRED SOIL BOUNDARIES INFERRED ROCK LINE ALLUVIAL SOIL BOUNDARY DIP/DIP DIRECTION OF ROCK STRUCTURES SOUNDING ROD				WATER LEVEL IN BORE HOLE IMMEDIATELY AFTER DRILLING. STATIC WATER LEVEL AFTER 24 HOURS. PERCHED WATER, SATURATED ZONE OR WATER BEARING STRATA SPRING OR SEEPAGE				FRESH ROCK FRESH, CRYSTALS BRIGHT, FEW JOINTS MAY SHOW SLIGHT STAINING. ROCK RINGS UNDER HAMMER IF CRYSTALLINE. VERY SLIGHT (V. SLI.) ROCK GENERALLY FRESH, JOINTS STAINED, SOME JOINTS MAY SHOW THIN CLAY COATINGS IF OPEN, CRYSTALS ON A BROKEN SPECIMEN FACE SHINE BRIGHTLY. ROCK RINGS UNDER HAMMER BLOWS IF OF A CRYSTALLINE NATURE. SLIGHT (SLI.) ROCK GENERALLY FRESH, JOINTS STAINED AND DISCOLORATION EXTENDS INTO ROCK UP TO 2.5 cm. OPEN JOINTS MAY CONTAIN CLAY. IN GRANITOID ROCKS SOME OCCASIONAL FELDSPAR CRYSTALS ARE DULL AND DISCOLORED. CRYSTALLINE ROCKS RING UNDER HAMMER BLOWS. MODERATE (MOD.) SIGNIFICANT PORTIONS OF ROCK SHOW DISCOLORATION AND WEATHERING EFFECTS. IN GRANITOID ROCKS, MOST FELDSPARS ARE DULL AND DISCOLORED, SOME SHOW CLAY. ROCK HAS DULL SOUND UNDER HAMMER BLOWS AND SHOWS SIGNIFICANT LOSS OF STRENGTH AS COMPARED WITH FRESH ROCK. MODERATELY SEVERE (MOD. SEV.) ALL ROCK EXCEPT QUARTZ DISCOLORED OR STAINED. IN GRANITOID ROCKS, ALL FELDSPARS DULL AND DISCOLORED AND A MAJORITY SHOW KADLINIZATION. ROCK SHOWS SEVERE LOSS OF STRENGTH AND CAN BE EXCAVATED WITH A GEOLOGIST'S PICK. ROCK GIVES 'CLUNK' SOUND WHEN STRUCK. IF TESTED, WOULD YIELD SPT REFUSAL. SEVERE (SEV.) ALL ROCKS EXCEPT QUARTZ DISCOLORED OR STAINED. ROCK FABRIC CLEAR AND EVIDENT BUT REDUCED IN STRENGTH TO STRONG SOIL. IN GRANITOID ROCKS ALL FELDSPARS ARE KAOLINIZED TO SOME EXTENT. SOME FRAGMENTS OF STRONG ROCK USUALLY REMAIN. IF TESTED, YIELDS SPT N VALUES > 100 BLOWS PER 30 cm. VERY SEVERE (V. SEV.) ALL ROCK EXCEPT QUARTZ DISCOLORED OR STAINED. ROCK FABRIC ELEMENTS ARE DISCERNIBLE BUT THE MASS IS EFFECTIVELY REDUCED TO SOIL STATUS, WITH ONLY FRAGMENTS OF STRONG ROCK REMAINING. SAPROLITE IS AN EXAMPLE OF ROCK WEATHERED TO A DEGREE SUCH THAT ONLY MINOR VESTIGES OF THE ORIGINAL ROCK FABRIC REMAIN. IF TESTED, YIELDS SPT N VALUES < 100 BLOWS PER 30 cm. COMPLETE ROCK REDUCED TO SOIL. ROCK FABRIC NOT DISCERNIBLE, OR DISCERNIBLE ONLY IN SMALL AND SCATTERED CONCENTRATIONS. QUARTZ MAY BE PRESENT AS DIKES OR STRINGERS. SAPROLITE IS ALSO AN EXAMPLE.				SPT DPT DMT VST TEST BORING AUGER BORING CORE BORING MONITORING WELL PIEZOMETER INSTALLATION SLOPE INDICATOR INSTALLATION SPT N-VALUE SPT REFUSAL				SAMPLE DESIGNATIONS S- BULK SAMPLE SS- SPLIT SPOON SAMPLE ST- SHELBY TUBE SAMPLE RS- ROCK SAMPLE RT- RECOMPACTED TRIAXIAL SAMPLE CBR - CBR SAMPLE			
GENERAL RATING AS A SUBGRADE				ROADWAY EMBANKMENT WITH SOIL DESCRIPTION				SPT DPT DMT VST TEST BORING				SAMPLE DESIGNATIONS											
EXCELLENT TO GOOD				ARTIFICIAL FILL OTHER THAN ROADWAY EMBANKMENTS				AUGER BORING				S- BULK SAMPLE											
FAIR TO POOR				INFERRED SOIL BOUNDARIES				CORE BORING				SS- SPLIT SPOON SAMPLE											
POOR				INFERRED ROCK LINE				MONITORING WELL				ST- SHELBY TUBE SAMPLE											
UNSATURABLE				ALLUVIAL SOIL BOUNDARY				PIEZOMETER INSTALLATION				RS- ROCK SAMPLE											
P.I. OF A-7-5 ≤ L.L. - 30 : P.I. OF A-7-6 > L.L. - 30				DIP/DIP DIRECTION OF ROCK STRUCTURES				SLOPE INDICATOR INSTALLATION				RT- RECOMPACTED TRIAXIAL SAMPLE											
EXCELLENT TO GOOD				SOUNDING ROD				SPT N-VALUE				CBR - CBR SAMPLE											
FAIR TO POOR				ROADWAY EMBANKMENT WITH SOIL DESCRIPTION				SPT REFUSAL				ROCK HARDNESS											
POOR				ARTIFICIAL FILL OTHER THAN ROADWAY EMBANKMENTS				SPT REFUSAL				VERY HARD CANNOT BE SCRATCHED BY KNIFE OR SHARP PICK. BREAKING OF HAND SPECIMENS REQUIRES SEVERAL HARD BLOWS OF THE GEOLOGIST'S PICK.											
UNSATURABLE				INFERRED SOIL BOUNDARIES				SPT REFUSAL				HARD CAN BE SCRATCHED BY KNIFE OR PICK ONLY WITH DIFFICULTY. HARD HAMMER BLOWS REQUIRED TO DETACH HAND SPECIMEN.											
EXCELLENT TO GOOD				INFERRED ROCK LINE				SPT REFUSAL				MODERATELY HARD CAN BE SCRATCHED BY KNIFE OR PICK. GOUGES OR GROOVES TO 6 mm DEEP CAN BE EXCAVATED BY HARD BLOW OF A GEOLOGIST'S PICK. HAND SPECIMENS CAN BE DETACHED BY MODERATE BLOWS.											
FAIR TO POOR				ALLUVIAL SOIL BOUNDARY				SPT REFUSAL				MEDIUM HARD CAN BE GROOVED OR GOUGED 1 mm DEEP BY FIRM PRESSURE OF KNIFE OR PICK POINT. CAN BE EXCAVATED IN SMALL CHIPS TO PIECES 25 mm MAXIMUM SIZE BY HARD BLOWS OF THE POINT OF A GEOLOGIST'S PICK.											
POOR				INFERRED ROCK LINE				SPT REFUSAL				SOFT CAN BE GROVED OR GOUGED READILY BY KNIFE OR PICK. CAN BE EXCAVATED IN FRAGMENTS FROM CHIPS TO SEVERAL CENTIMETERS IN SIZE BY MODERATE BLOWS OF A PICK POINT. SMALL, THIN PIECES CAN BE BROKEN BY FINGER PRESSURE.											
UNSATURABLE				ALLUVIAL SOIL BOUNDARY				SPT REFUSAL				VERY SOFT CAN BE CARVED WITH KNIFE. CAN BE EXCAVATED READILY WITH POINT OF PICK. PIECES 25 mm OR MORE IN THICKNESS CAN BE BROKEN BY FINGER PRESSURE. CAN BE SCRATCHED READILY BY FINGERNAIL.											
EXCELLENT TO GOOD				INFERRED ROCK LINE				SPT REFUSAL				STANDARD PENETRATION TEST (PENETRATION RESISTANCE) (SPT) - NUMBER OF BLOWS (N) OF A 63.5 kg HAMMER FALLING 0.76 METERS REQUIRED TO PRODUCE A PENETRATION OF 30 cm INTO SOIL WITH A 5 cm OUTSIDE DIAMETER SPLIT SPOON SAMPLER. SPT REFUSAL IS LESS THAN 2.5 cm PENETRATION WITH 50 BLOWS.											
FAIR TO POOR				ALLUVIAL SOIL BOUNDARY				SPT REFUSAL				STRATA CORE RECOVERY (SREC) - TOTAL LENGTH OF STRATA MATERIAL RECOVERED DIVIDED BY TOTAL LENGTH OF STRATUM AND EXPRESSED AS A PERCENTAGE.											
POOR				INFERRED ROCK LINE				SPT REFUSAL				STRATA ROCK QUALITY DESIGNATION (S.R.Q.D.) - A MEASURE OF ROCK QUALITY DESCRIBED BY: TOTAL LENGTH OF ROCK SEGMENTS WITHIN A STRATUM EQUAL TO OR GREATER THAN 10 CENTIMETERS DIVIDED BY THE TOTAL LENGTH OF STRATA AND EXPRESSED AS A PERCENTAGE.											
UNSATURABLE				ALLUVIAL SOIL BOUNDARY				SPT REFUSAL				TOPSOIL (T.S.) - SURFACE SOILS USUALLY CONTAINING ORGANIC MATTER.											
EXCELLENT TO GOOD				ALLUVIAL SOIL BOUNDARY				SPT REFUSAL				BENCH MARK: BL-36 BL-PINC 22+4.117											
FAIR TO POOR				ALLUVIAL SOIL BOUNDARY				SPT REFUSAL				ELEVATION: 236.102											
POOR				ALLUVIAL SOIL BOUNDARY				SPT REFUSAL				NOTES:											
UNSATURABLE				ALLUVIAL SOIL BOUNDARY				SPT REFUSAL				FRACURE SPACING											
EXCELLENT TO GOOD				ALLUVIAL SOIL BOUNDARY				SPT REFUSAL				BEDDING											
FAIR TO POOR				ALLUVIAL SOIL BOUNDARY				SPT REFUSAL				INDURATION											
POOR				ALLUVIAL SOIL BOUNDARY				SPT REFUSAL				FOR SEDIMENTARY ROCKS, INDURATION IS THE HARDENING OF THE MATERIAL BY CEMENTING, HEAT, PRESSURE, ETC.											
UNSATURABLE				ALLUVIAL SOIL BOUNDARY				SPT REFUSAL				FRIABLE RUBBING WITH FINGER FREES NUMEROUS GRAINS; GENTLE BLOW BY HAMMER DISINTEGRATES SAMPLE.											
EXCELLENT TO GOOD				ALLUVIAL SOIL BOUNDARY				SPT REFUSAL				MODERATELY INDURATED GRAINS CAN BE SEPARATED FROM SAMPLE WITH STEEL PROBE; BREAKS EASILY WHEN HIT WITH HAMMER.											
FAIR TO POOR				ALLUVIAL SOIL BOUNDARY				SPT REFUSAL				INDURATED GRAINS ARE DIFFICULT TO SEPARATE WITH STEEL PROBE; DIFFICULT TO BREAK WITH HAMMER.											
POOR				ALLUVIAL SOIL BOUNDARY				SPT REFUSAL				EXTREMELY INDURATED SHARP HAMMER BLOWS REQUIRED TO BREAK SAMPLE; SAMPLE BREAKS ACROSS GRAINS.											
UNSATURABLE				ALLUVIAL SOIL BOUNDARY				SPT REFUSAL				EQUIPMENT USED ON SUBJECT PROJECT											
EXCELLENT TO GOOD				ALLUVIAL SOIL BOUNDARY				SPT REFUSAL				HAMMER TYPE:											
FAIR TO POOR				ALLUVIAL SOIL BOUNDARY				SPT REFUSAL				CORE SIZE:											
POOR				ALLUVIAL SOIL BOUNDARY				SPT REFUSAL				HAND TOOLS:											
UNSATURABLE				ALLUVIAL SOIL BOUNDARY				SPT REFUSAL				DRILL UNITS:											
EXCELLENT TO GOOD				ALLUVIAL SOIL BOUNDARY				SPT REFUSAL				ADVANCING TOOLS:											
FAIR TO POOR				ALLUVIAL SOIL BOUNDARY				SPT REFUSAL				OTHER _____											
POOR				ALLUVIAL SOIL BOUNDARY				SPT REFUSAL				OTHER _____											
UNSATURABLE				ALLUVIAL SOIL BOUNDARY				SPT REFUSAL				OTHER _____											
EXCELLENT TO GOOD				ALLUVIAL SOIL BOUNDARY				SPT REFUSAL				OTHER _____											
FAIR TO POOR				ALLUVIAL SOIL BOUNDARY				SPT REFUSAL				OTHER _____											
POOR				ALLUVIAL SOIL BOUNDARY				SPT REFUSAL				OTHER _____											
UNSATURABLE				ALLUVIAL SOIL BOUNDARY				SPT REFUSAL				OTHER _____											
EXCELLENT TO GOOD				ALLUVIAL SOIL BOUNDARY				SPT REFUSAL				OTHER _____											
FAIR TO POOR				ALLUVIAL SOIL BOUNDARY				SPT REFUSAL				OTHER _____											
POOR				ALLUVIAL SOIL BOUNDARY				SPT REFUSAL				OTHER _____											
UNSATURABLE				ALLUVIAL SOIL BOUNDARY				SPT REFUSAL				OTHER _____											
EXCELLENT TO GOOD				ALLUVIAL SOIL BOUNDARY				SPT REFUSAL				OTHER _____											
FAIR TO POOR				ALLUVIAL SOIL BOUNDARY				SPT REFUSAL				OTHER _____											
POOR				ALLUVIAL SOIL BOUNDARY				SPT REFUSAL				OTHER _____											
UNSATURABLE				ALLUVIAL SOIL BOUNDARY				SPT REFUSAL				OTHER _____											
EXCELLENT TO GOOD				ALLUVIAL SOIL BOUNDARY				SPT REFUSAL				OTHER _____											
FAIR TO POOR				ALLUVIAL SOIL BOUNDARY				SPT REFUSAL				OTHER _____											
POOR				ALLUVIAL SOIL BOUNDARY				SPT REFUSAL				OTHER _____											
UNSATURABLE				ALLUVIAL SOIL BOUNDARY				SPT REFUSAL				OTHER _____											
EXCELLENT TO GOOD				ALLUVIAL SOIL BOUNDARY				SPT REFUSAL				OTHER _____											
FAIR TO POOR				ALLUVIAL SOIL BOUNDARY				SPT REFUSAL				OTHER _____											
POOR				ALLUVIAL SOIL BOUNDARY				SPT REFUSAL				OTHER _____											
UNSATURABLE				ALLUVIAL SOIL BOUNDARY				SPT REFUSAL				OTHER _____											
EXCELLENT TO GOOD				ALLUVIAL SOIL BOUNDARY				SPT REFUSAL				OTHER _____											
FAIR TO POOR				ALLUVIAL SOIL BOUNDARY				SPT REFUSAL				OTHER _____											
POOR				ALLUVIAL SOIL BOUNDARY				SPT REFUSAL				OTHER _____											
UNSATURABLE				ALLUVIAL SOIL BOUNDARY				SPT REFUSAL				OTHER _____											
EXCELLENT TO GOOD				ALLUVIAL SOIL BOUNDARY				SPT REFUSAL				OTHER _____											
FAIR TO POOR				ALLUVIAL SOIL BOUNDARY				SPT REFUSAL				OTHER _____											
POOR				ALLUVIAL SOIL BOUNDARY				SPT REFUSAL				OTHER _____											
UNSATURABLE				ALLUVIAL SOIL BOUNDARY				SPT REFUSAL				OTHER _____											
EXCELLENT TO GOOD				ALLUVIAL SOIL BOUNDARY				SPT REFUSAL				OTHER _____											
FAIR TO POOR				ALLUVIAL SOIL BOUNDARY				SPT REFUSAL				OTHER _____											
POOR				ALLUVIAL SOIL BOUNDARY				SPT REFUSAL				OTHER _____											
UNSATURABLE				ALLUVIAL SOIL BOUNDARY				SPT REFUSAL				OTHER _____											
EXCELLENT TO GOOD				ALLUVIAL SOIL BOUNDARY				SPT REFUSAL				OTHER _____											
FAIR TO POOR				ALLUVIAL SOIL BOUNDARY				SPT REFUSAL				OTHER _____											
POOR				ALLUVIAL SOIL BOUNDARY				SPT REFUSAL				OTHER _____											
UNSATURABLE				ALLUVIAL SOIL BOUNDARY				SPT REFUSAL				OTHER _____											
EXCELLENT TO GOOD				ALLUVIAL SOIL BOUNDARY				SPT REFUSAL				OTHER _____											
FAIR TO POOR				ALLUVIAL SOIL BOUNDARY				SPT REFUSAL				OTHER _____											
POOR				ALLUVIAL SOIL BOUNDARY				SPT REFUSAL				OTHER _____											
UNSATURABLE				ALLUVIAL SOIL BOUNDARY				SPT REFUSAL				OTHER _____											
EXCELLENT TO GOOD				ALLUVIAL SOIL BOUNDARY				SPT REFUSAL				OTHER _____											
FAIR TO POOR																							



**Groundwater**

Groundwater measurements taken more than 24 hours after each boring was performed indicate a static groundwater table between elevation 223.4 and 226.9 meters.

Respectfully submitted,

J.E. Beverly, Project Geologist

A handwritten signature in cursive script, reading "J E Beverly". The signature is written in black ink and is positioned below the typed name.

STATE	STATE PROJECT REFERENCE NO.	SHEET NO.	TOTAL SHEETS
N.C.	R-06091B	4	19
STATE PROJ. NO.	F.A. PROJ. NO.	DESCRIPTION	
34345.1.1		P.E.	
CONST.			

LAWSON J. INGRAM  
DB 1847 PG 636  
3.300 HA (CADD)

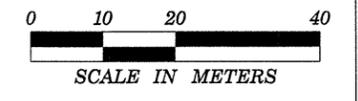
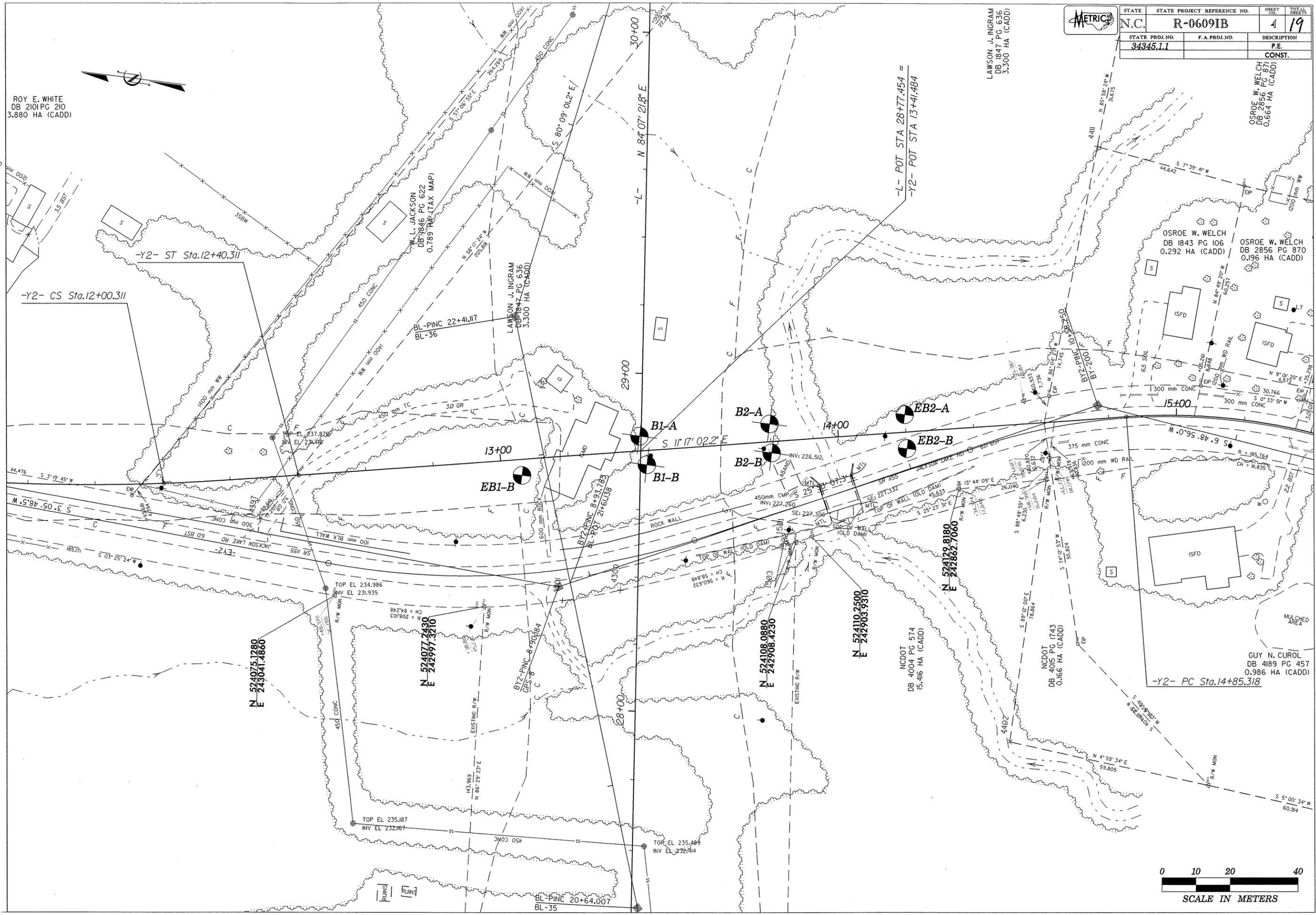
ROY E. WHITE  
DB 2101 PG 210  
3.880 HA (CADD)

OSROE W. WELCH  
DB 2856 PG 871  
0.664 HA (CADD)

OSROE W. WELCH  
DB 1843 PG 106  
0.292 HA (CADD)

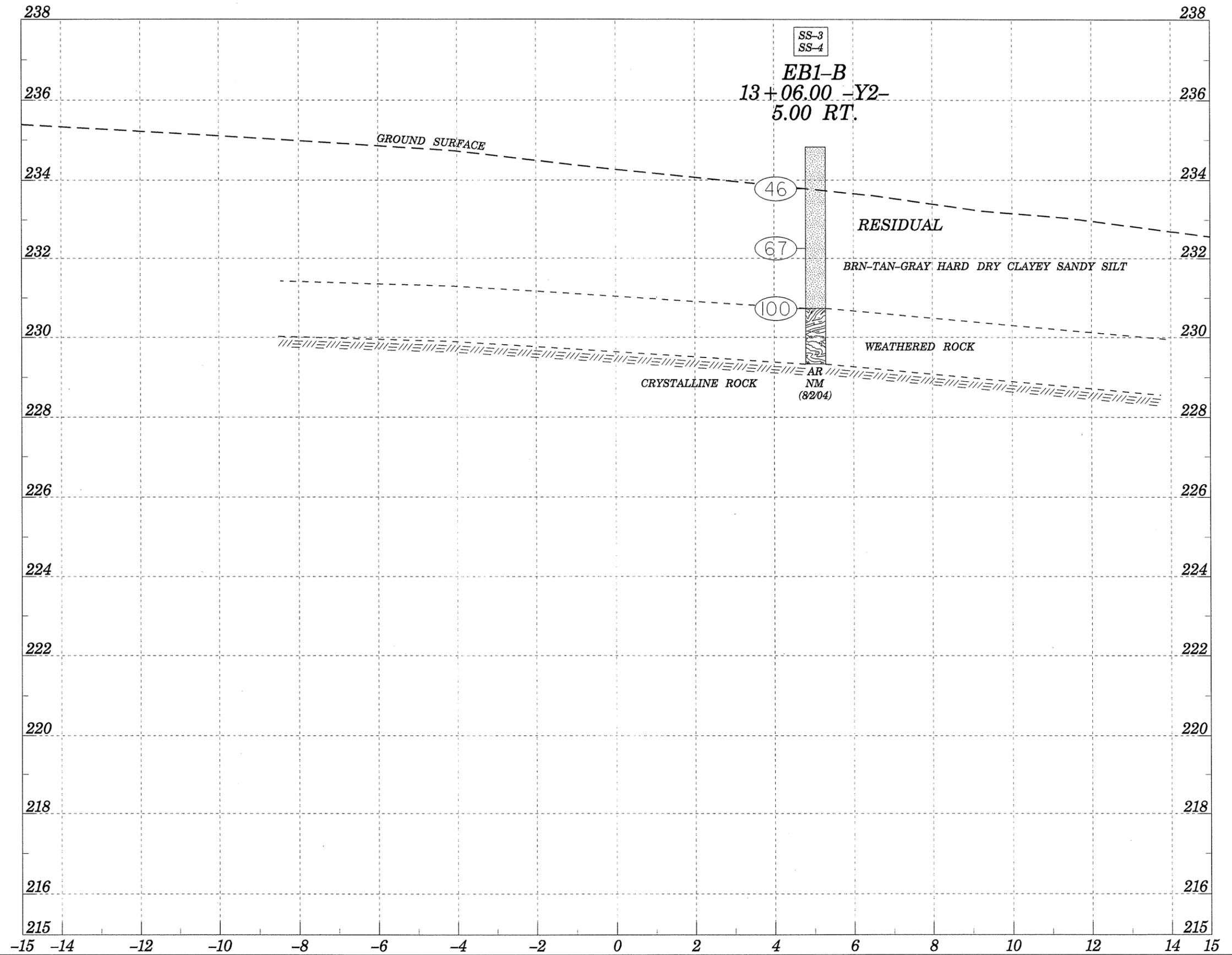
OSROE W. WELCH  
DB 2856 PG 870  
0.196 HA (CADD)

GUY N. CURROL  
DB 4189 PG 457  
0.986 HA (CADD)

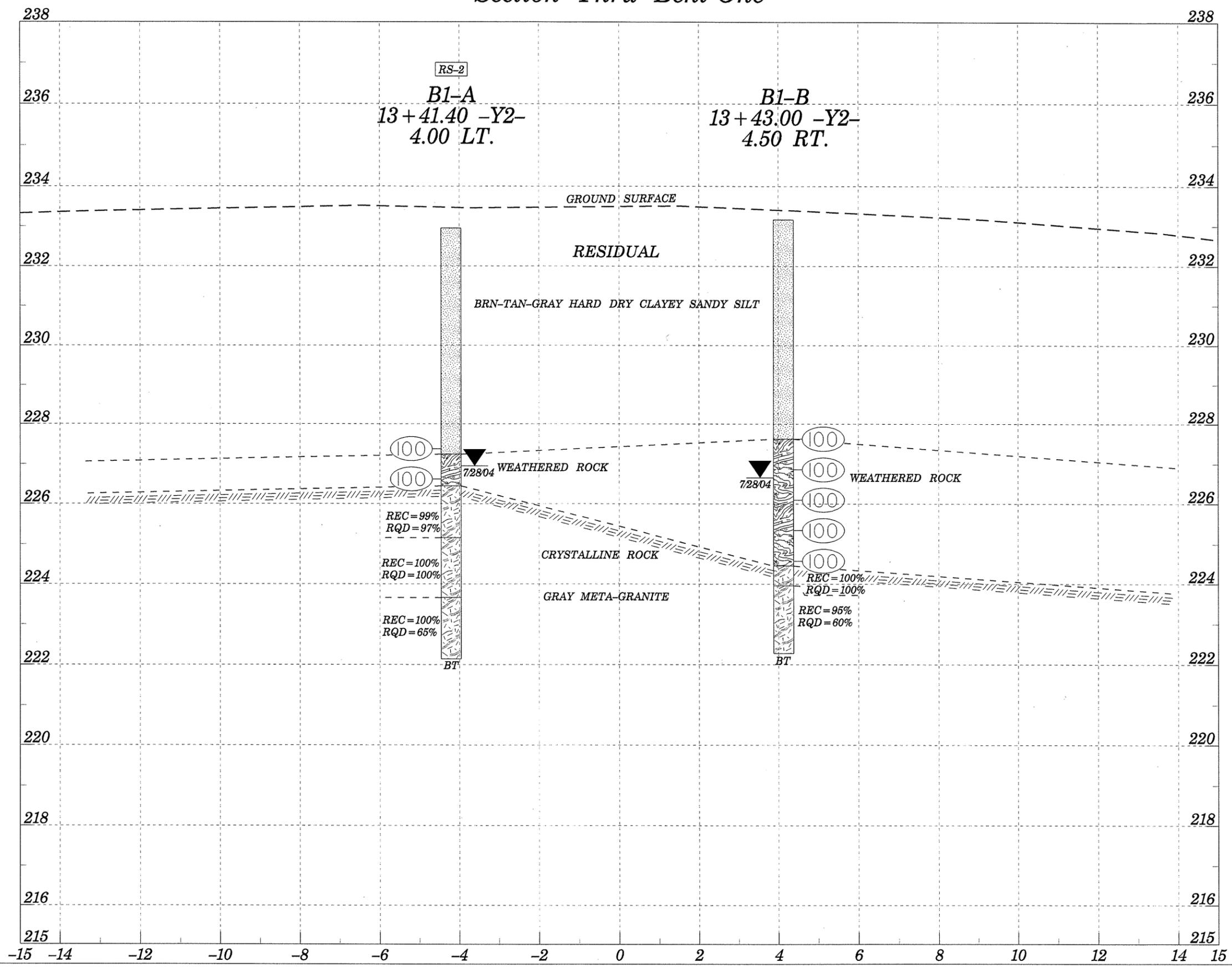


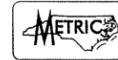
	STATE	STATE PROJECT REFERENCE NO.	SHEET NO.	TOTAL SHEETS
	N.C.	R-0609IA	5	19
	STATE PROJ. NO.	F.A. PROJ. NO.	DESCRIPTION	
	34345.11		P.E.	
			CONST.	

## Section Thru End Bent One



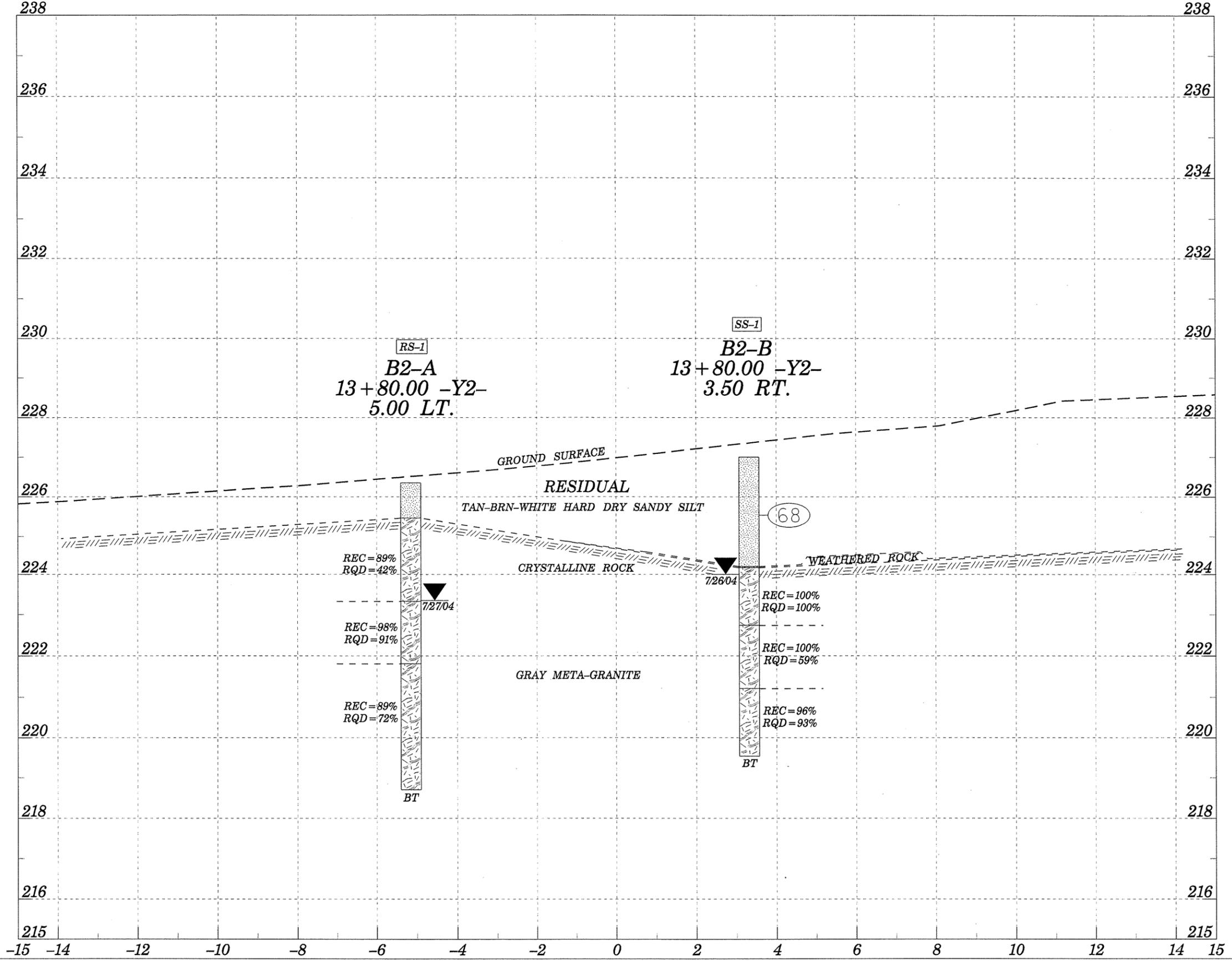
## Section Thru Bent One





STATE	STATE PROJECT REFERENCE NO.	SHEET NO.	TOTAL SHEETS
N.C.	R-06091A	7	19
STATE PROJ. NO.	F. A. PROJ. NO.	DESCRIPTION	
34345.1.1		P.E.	
		CONST.	

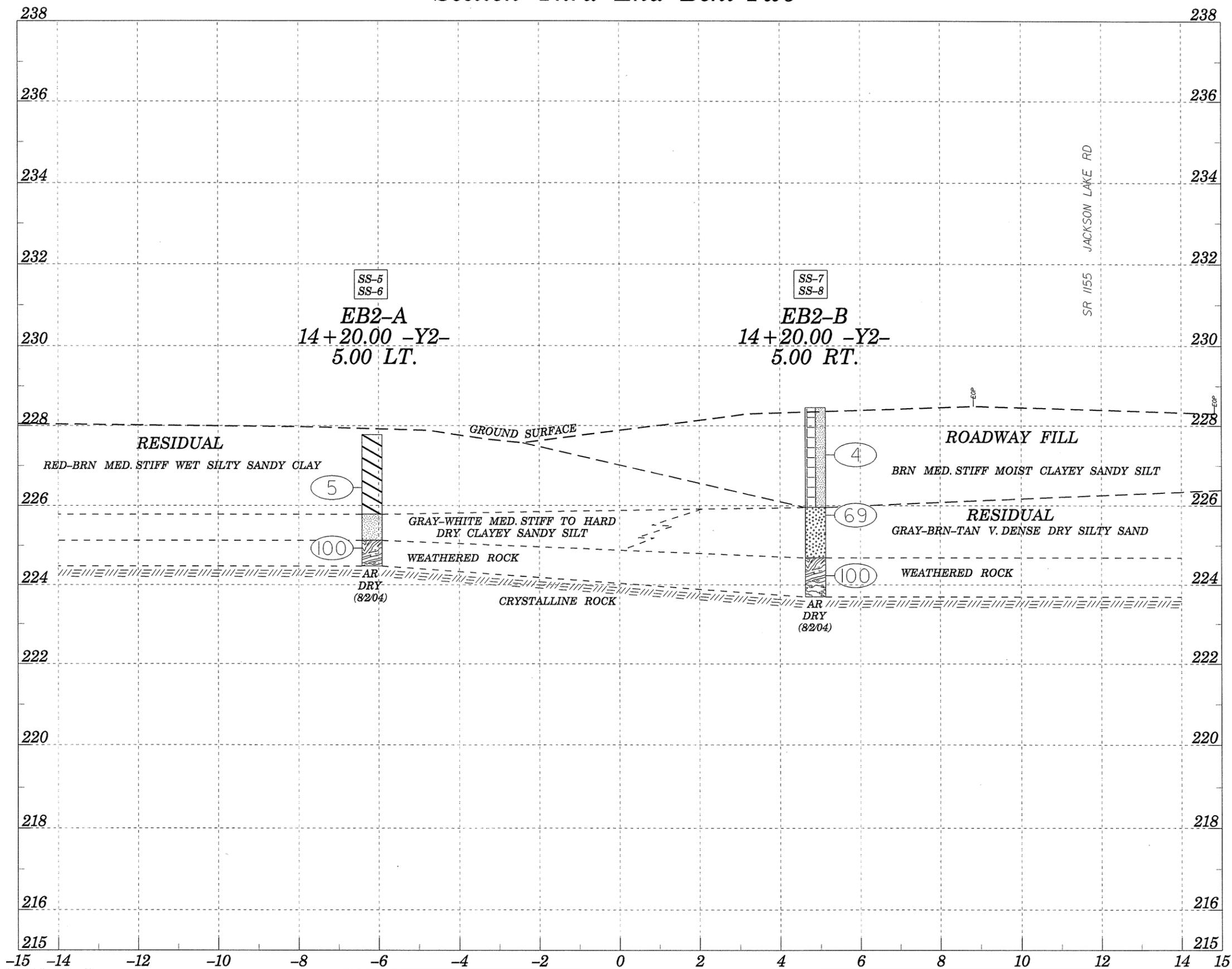
# Section Thru Bent Two





STATE	STATE PROJECT REFERENCE NO.	SHEET NO.	TOTAL SHEETS
N.C.	R-06091A	8	19
STATE PROJ. NO.	F.A. PROJ. NO.	DESCRIPTION	
34345.1.1		P.E. CONST.	

# Section Thru End Bent Two

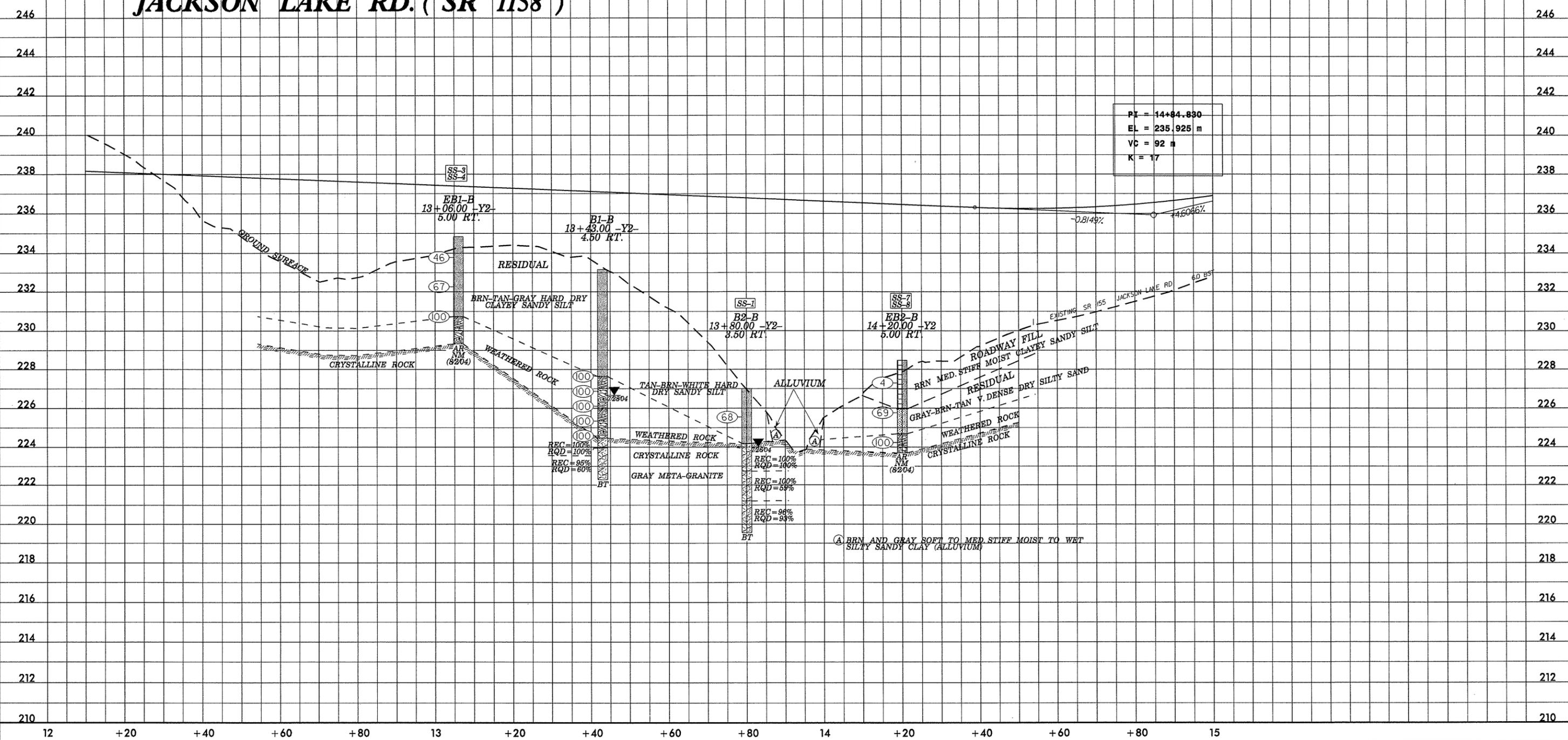




PROJECT REFERENCE NO.	SHEET NO.
R-06091A	9
ROADWAY DESIGN ENGINEER	HYDRAULICS ENGINEER
INCOMPLETE PLANS DO NOT USE FOR R/W ACQUISITION PRELIMINARY PLANS DO NOT USE FOR CONSTRUCTION	
CONST.REV.	
R/W REV.	

# -Y2-

## JACKSON LAKE RD. ( SR 1158 )



NORTH CAROLINA DEPARTMENT OF TRANSPORTATION  
 GEOTECHNICAL UNIT BORING LOG

Sheet 10 of 19

PROJECT NO 34345.1.1		ID R-06091A		COUNTY GUILFORD		GEOLOGIST R.W. TODD									
SITE DESCRIPTION BRIDGE ON SR 1158 (-Y2-) OVER US 311 AND MILE BRANCH							GND WATER								
BORING NO EB1-B		NORTHING 0.00		EASTING 0.00		0 HR N/A									
ALIGNMENT Y2		BORING LOCATION 13+06.000		OFFSET 5.00m RT		24 HR N/A									
COLLAR ELEV 234.83m		TOTAL DEPTH 5.50m		START DATE 8/02/04		COMPLETION DATE 08/02/04									
DRILL MACHINE CME-550			DRILL METHOD H.S. AUGERS			HAMMER TYPE AUTOMATIC									
SURFACE WATER DEPTH N/A			DEPTH TO ROCK 5.50m			Log EB1-B, Page 1 of 1									
ELEV	DEPTH	BLOW CT			PEN (m)	BLOWS PER 30cm					SAMPLE NO	MOI	LOG	SOIL AND ROCK DESCRIPTION	
		15cm	15cm	15cm		0	25	50	75	100					
234.83															
234.00	1.05	13	21	25	0.30					46					RESIDUAL - HARD BROWN-TAN-GRAY CLAYEY SANDY SILT
232.00	2.57	12	25	42	0.30					67					
230.00	4.10	53	47		0.30					100					WEATHERED ROCK
229.33						HOLLOW STEM REFUSAL AT ELEVATION 229.33 METERS ON HARD ROCK									





NORTH CAROLINA DEPARTMENT OF TRANSPORTATION  
 GEOTECHNICAL UNIT BORING LOG

NORTH CAROLINA DEPARTMENT OF TRANSPORTATION  
 DIVISION OF HIGHWAYS  
 GEOTECHNICAL UNIT CORE BORING REPORT

PROJECT NO 34345.1.1		ID R-06091A		COUNTY GUILFORD		GEOLOGIST R.W. TODD						
SITE DESCRIPTION BRIDGE ON SR 1158 (-Y2-) OVER US 311 AND MILE BRANCH							GND WATER					
BORING NO B2-A		NORTHING 0.00		EASTING 0.00		0 HR N/A						
ALIGNMENT Y2		BORING LOCATION 13+80.000		OFFSET 5.00m LT		24 HR 3.00m						
COLLAR ELEV 226.35m		TOTAL DEPTH 7.64m		START DATE 7/27/04		COMPLETION DATE 07/27/04						
DRILL MACHINE CME-550			DRILL METHOD NWCAS/NXWL			HAMMER TYPE AUTOMATIC						
SURFACE WATER DEPTH			DEPTH TO ROCK 0.88m			Log B2-A, Page 1 of 1						
ELEV	DEPTH	BLOW CT			PEN (m)	BLOWS PER 30cm				SAMPLE NO	LOG	SOIL AND ROCK DESCRIPTION
		15cm	15cm	15cm		0	25	50	75			
226.35												Ground Surface
225.00										RUN #1		RESIDUAL - HARD TAN-BROWN-WHITE SANDY SILT
223.00										RS-1 RUN #2		HARD GRANITIC ROCK REC=89% RQD=42%
221.00										RUN #3		GRANITIC ROCK REC=98% RQD=91%
219.00												GRANITIC ROCK REC=89% RQD=72%
218.71												CORING TERMINATED AT ELEVATION 218.71 METERS IN HARD ROCK

PROJECT NO: 34345.1.1      PROJECT ID: R-06091A      COUNTY: GUILFORD      GEOLOGIST: R.W. TODD  
 SITE DESCRIPTION: BRIDGE ON SR 1158 (-Y2) OVER US 311 AND MILE BRANCH      DRILLER: C.L. SMITH  
 BORING NO: B2-A      BORING LOCATION (STA): 13+80 -Y2-      OFFSET: 5.0 LT.  
 COLLAR ELEV: 226.35      PERSONNEL: D.K. BRATTON      CORE SIZE: NXWL  
 TOTAL DEPTH: 7.64      DRILL MACHINE: CME-550      DATE STARTED: 7-27-04  
 TOTAL RUN: 6.76      DRILL EQUIP: NWCAS/NXWL      DATE COMPLETED: 7-27-04

ELEV. (M)	DEPTH (M)	DRILL RATE (MIN/3 m)	RUN NO.	REC % (M)	RQD % (M)	SAMPLE NO.	FIELD CLASSIFICATION AND REMARKS
225.47	0.88		1	89	35	RS-1 (2.82-3.00)	FIRST 0.47 OF RUN IS WHITE SLI. WEATH. HARD FINE GRAINED META-GRANITE W/ V. CLOSE FRACTURE SPACING, THEN @ 1.35 ROCK IS GRAY MOD. WEATH. TO FRESH HARD TO V. HARD META-GRANITE W/ V. CLOSE FRACTURE SPACING
223.33	3.02		2	98	90		GRAY FRESH V. HARD META-GRANITE W/ CLOSE TO MOD. CLOSE FRACTURE SPACING
221.81	4.54		3	89	72		GRAY FRESH V. HARD META-GRANITE W/ CLOSE TO MOD. CLOSE FRACTURE SPACING
218.71	7.64						
NOTES							

NORTH CAROLINA DEPARTMENT OF TRANSPORTATION  
 GEOTECHNICAL UNIT BORING LOG

NORTH CAROLINA DEPARTMENT OF TRANSPORTATION  
 DIVISION OF HIGHWAYS  
 GEOTECHNICAL UNIT CORE BORING REPORT

PROJECT NO 34345.1.1		ID R-06091A		COUNTY GUILFORD		GEOLOGIST R.W. TODD							
SITE DESCRIPTION BRIDGE ON SR 1158 (-Y2-) OVER US 311 AND MILE BRANCH							GND WATER						
BORING NO B2-B		NORTHING 0.00		EASTING 0.00		0 HR N/A							
ALIGNMENT Y2		BORING LOCATION 13+80.000		OFFSET 3.50m RT		24 HR 3.00m							
COLLAR ELEV 227.00m		TOTAL DEPTH 7.46m		START DATE 7/26/04		COMPLETION DATE 07/26/04							
DRILL MACHINE CME-550			DRILL METHOD NWCAS/NXWL			HAMMER TYPE AUTOMATIC							
SURFACE WATER DEPTH			DEPTH TO ROCK 2.83m			Log B2-B, Page 1 of 1							
ELEV	DEPTH	BLOW CT			PEN (m)	BLOWS PER 30cm				SAMPLE NO	LOG	SOIL AND ROCK DESCRIPTION	
		15cm	15cm	15cm		0	25	50	75				100
227.00												Ground Surface	
226.00	1.46	20	31	37	0.30					68	SS-1	DRY	RESIDUAL - HARD TAN-BROWN-WHITE SANDY SILT
224.00											RUN #1	▼	WEATHERED ROCK
											RUN #2		HARD GRANITIC ROCK - REC=100% RQD=100%
222.00											RUN #3		HARD GRANITIC ROCK - REC=100% RQD=59%
220.00													HARD GRANITIC ROCK - REC=96% RQD=93%
219.54													CORING TERMINATED AT ELEVATION 219.54 METERS IN HARD ROCK

PROJECT NO: 34345.1.1      PROJECT ID: R-06091A      COUNTY: GUILFORD      GEOLOGIST: R.W. TODD  
 SITE DESCRIPTION: BRIDGE ON SR 1158 (-Y2-) OVER US 311 AND MILE BRANCH      DRILLER: C.L. SMITH  
 BORING NO: B2-B      BORING LOCATION (STA): 13+80 -Y2-      OFFSET: 3.50 RT.  
 COLLAR ELEV: 227.00      PERSONNEL: D.K. BRATTON      CORE SIZE: NXWL  
 TOTAL DEPTH: 7.46      DRILL MACHINE: CME-550      DATE STARTED: 7-26-04  
 TOTAL RUN: 4.63      DRILL EQUIP: NWCAS/NXWL      DATE COMPLETED: 7-26-04

ELEV. (M)	DEPTH (M)	DRILL RATE (MIN/3 m)	RUN NO.	REC % (M)	RQD % (M)	SAMPLE NO.	FIELD CLASSIFICATION AND REMARKS
224.17	2.83		1	100	87		GRAY V. SLI. WEATH. TO FRESH V. HARD META-GRANITE W/ MOD. CLOSE TO WIDE FRACTURE SPACING
222.74	4.26		2	100	50		GRAY V. SLI. WEATH. TO FRESH V. HARD META-GRANITE W/ V. CLOSE TO CLOSE FRACTURE SPACING
221.21	5.79		3	96	94		GRAY FRESH V. HARD META-GRANITE W/ CLOSE TO WIDE FRACTURE SPACING
219.54	7.46						
NOTES							

NORTH CAROLINA DEPARTMENT OF TRANSPORTATION  
GEOTECHNICAL UNIT BORING LOG

PROJECT NO 34345.1.1		ID R-06091A		COUNTY GUILFORD		GEOLOGIST R.W. TODD								
SITE DESCRIPTION BRIDGE ON SR 1158 (-Y2-) OVER US 311 AND MILE BRANCH							GND WATER							
BORING NO EB2-A		NORTHING 0.00		EASTING 0.00		0 HR N/A								
ALIGNMENT Y2		BORING LOCATION 14+20.000		OFFSET 5.00m LT		24 HR N/A								
COLLAR ELEV 227.78m		TOTAL DEPTH 3.31m		START DATE 8/02/04		COMPLETION DATE 08/02/04								
DRILL MACHINE CME-550			DRILL METHOD H.S. AUGERS			HAMMER TYPE AUTOMATIC								
SURFACE WATER DEPTH			DEPTH TO ROCK 3.31m			Log EB2-A, Page 1 of 1								
ELEV	DEPTH	BLOW CT			PEN (m)	BLOWS PER 30cm				SAMPLE NO	MOI	LOG	SOIL AND ROCK DESCRIPTION	
		15cm	15cm	15cm		0	25	50	75					100
227.78														Ground Surface
227.00	1.33	2	3	2	0.30	5								RESIDUAL - MEDIUM STIFF RED-BROWN SILTY SANDY CLAY
225.00	2.85	2	13	100	0.21									MEDIUM STIFF TO HARD GRAY WHITE CLAYEY SANDY SILT
224.47														WEATHERED ROCK
HOLLOW STEM REFUSAL AT ELEVATION 224.47 FEET ON HARD ROCK														

NORTH CAROLINA DEPARTMENT OF TRANSPORTATION  
GEOTECHNICAL UNIT BORING LOG

PROJECT NO 34345.1.1		ID R-06091A		COUNTY GUILFORD		GEOLOGIST R.W. TODD								
SITE DESCRIPTION BRIDGE ON SR 1158 (-Y2-) OVER US 311 AND MILE BRANCH							GND WATER							
BORING NO EB2-B		NORTHING 0.00		EASTING 0.00		0 HR N/A								
ALIGNMENT Y2		BORING LOCATION 14+20.000		OFFSET 5.00m RT		24 HR N/A								
COLLAR ELEV 228.46m		TOTAL DEPTH 4.76m		START DATE 8/02/04		COMPLETION DATE 08/02/04								
DRILL MACHINE CME-550			DRILL METHOD H.S. AUGERS			HAMMER TYPE AUTOMATIC								
SURFACE WATER DEPTH N/A			DEPTH TO ROCK N/A			Log EB2-B, Page 1 of 1								
ELEV	DEPTH	BLOW CT			PEN (m)	BLOWS PER 30cm				SAMPLE NO	MOI	LOG	SOIL AND ROCK DESCRIPTION	
		15cm	15cm	15cm		0	25	50	75					100
228.46														Ground Surface
228.00	1.18	1	2	2	0.30	4								ROADWAY FILL - MEDIUM STIFF BROWN CLAYEY SANDY SILT
226.00	2.70	16	29	40	0.30									RESIDUAL - VERY DENSE GRAY-BROWN-TAN SILTY SAND
224.00	4.23	100			0.05									WEATHERED ROCK
223.70														WEATHERED ROCK
HOLLOW STEM REFUSAL AT ELEVATION 223.70 METERS ON HARD ROCK														

TEST RESULTS

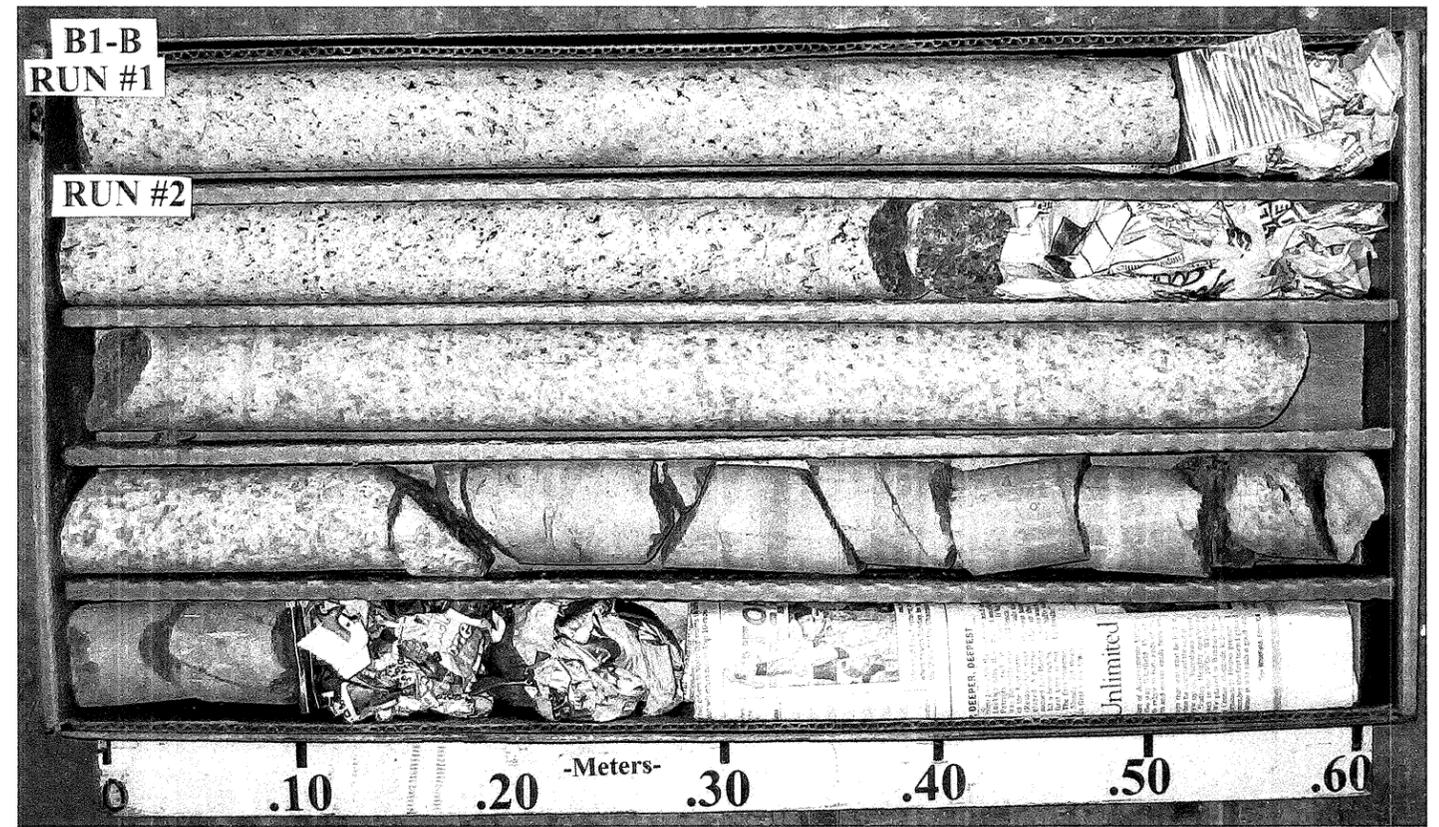
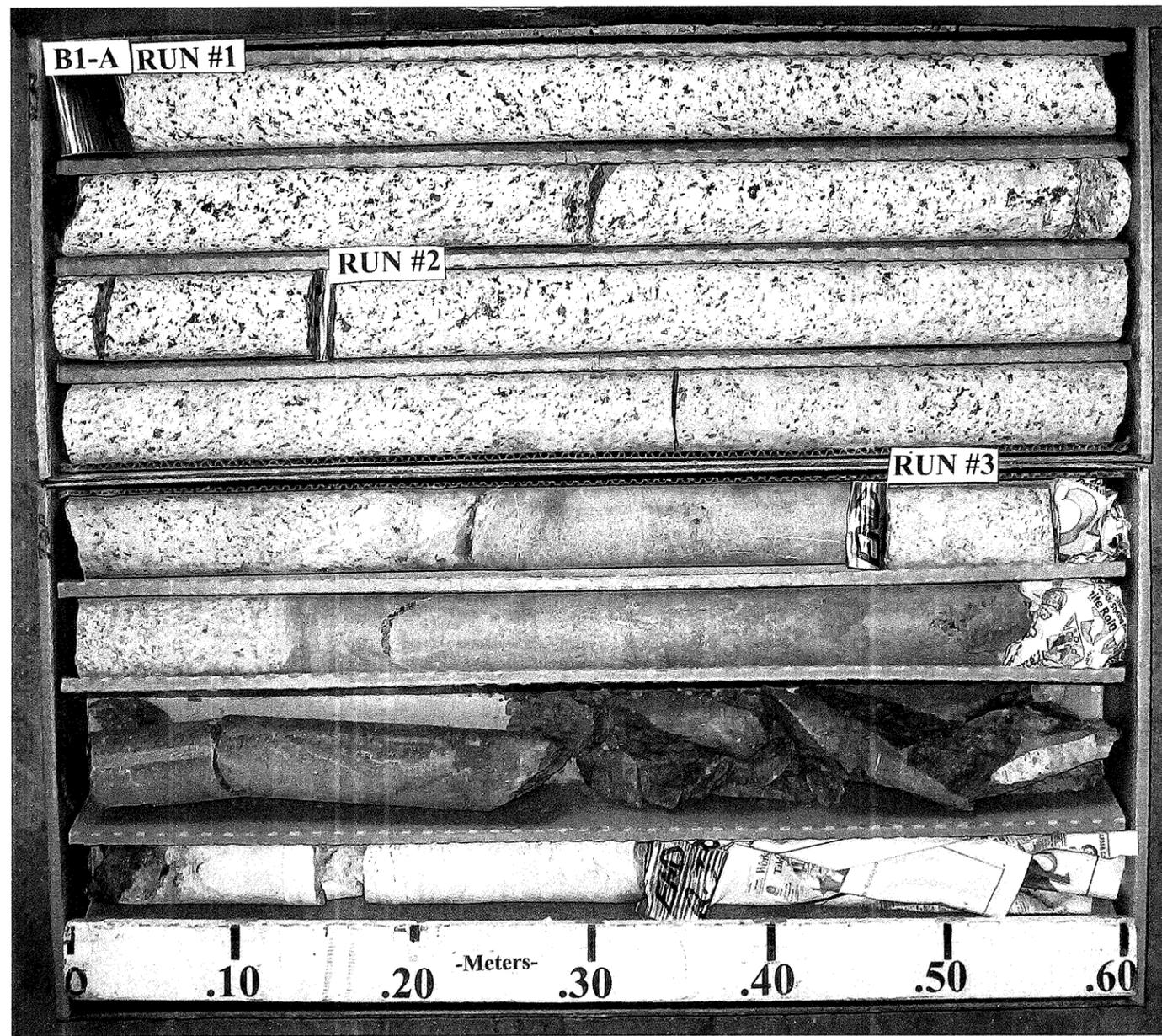
PROJECT: 34345.1.1 R-06091A  
 COUNTY: GUILFORD  
 SITE DESCRIPTION: BRIDGE ON SR 1158 (-Y2-) OVER US 311 AND MILE BRANCH

SOIL SAMPLE RESULTS																
SAMPLE NO.	OFFSET	STATION	DEPTH INTERVAL	AASHTO CLASS	N	L.L.	P.I.	% BY WEIGHT				% PASSING SIEVES			UNIT WT. (d)	VOID RATIO
								C. SAND	F. SAND	SILT	CLAY	10	40	200		
<b>EB1-B</b>																
SS-3	5.00m RT.	13+06.00	1.05-1.50	A-4(0)	46	29	4	27.6	29.8	26.4	16.2	100	86	48		
SS-4			2.57-3.02	A-4(2)	67	35	5	14.4	30.2	43.2	12.2	98	90	62		
<b>B1-B</b>																
SS-2	4.50m RT.	13+43.00	6.30-6.49	A-1-b(0)	100	23	NP	57.2	26.2	12.6	4.1	71	41	14		
<b>B2-B</b>																
SS-1	3.50m RT.	13+80.00	1.46-1.91	A-4(0)	68	25	NP	31.2	26.8	31.8	10.1	98	78	46		
<b>EB2-A</b>																
SS-5	5.00m LT.	14+20.00	1.33-1.78	A-7-6(13)	5	52	23	15.6	28.2	29.8	26.4	100	91	62		
SS-6			2.85-3.21	A-4(0)	100	31	5	28.2	29.4	24.1	18.3	97	79	47		
<b>EB2-B</b>																
SS-7	5.00m RT.	14+20.00	1.18-1.63	A-4(0)	4	23	6	28.6	28.6	22.5	20.3	90	76	43		
SS-8			2.70-3.15	A-2-4(0)	69	25	NP	41.8	31.2	18.9	8.1	94	71	30		

ROCK SAMPLE RESULTS							
SAMPLE NO.	OFFSET	STATION	DEPTH INTERVAL	RQD	UNIT WT	Q(MPa) (MPsi)	E(MPa) (MPsi)
<b>B1-A</b>							
RS-2	4.00m LT.	13+41.40	7.30-7.50	97%		CURRENTLY BEING TESTED	
<b>B2-A</b>							
RS-1	5.00m LT.	13+80.00	2.82-3.00	35%		CURRENTLY BEING TESTED	

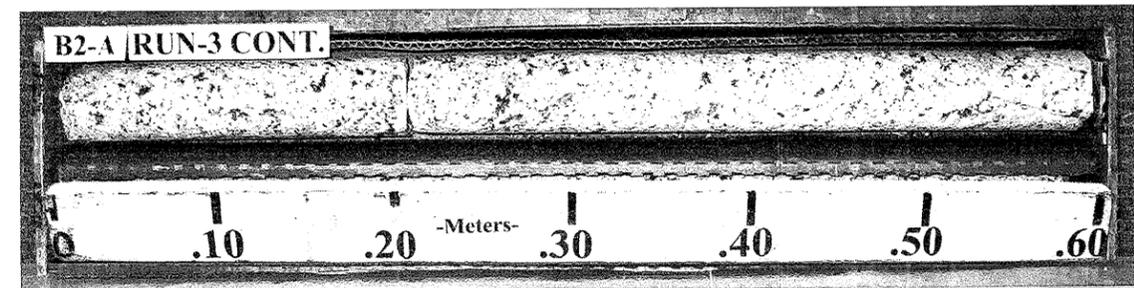
34345.1.1 R-06091A  
GUILFORD COUNTY  
BRIDGE ON SR 1158 (-Y2-) OVER US 311 AND MILE BRANCH

CORE PHOTOS



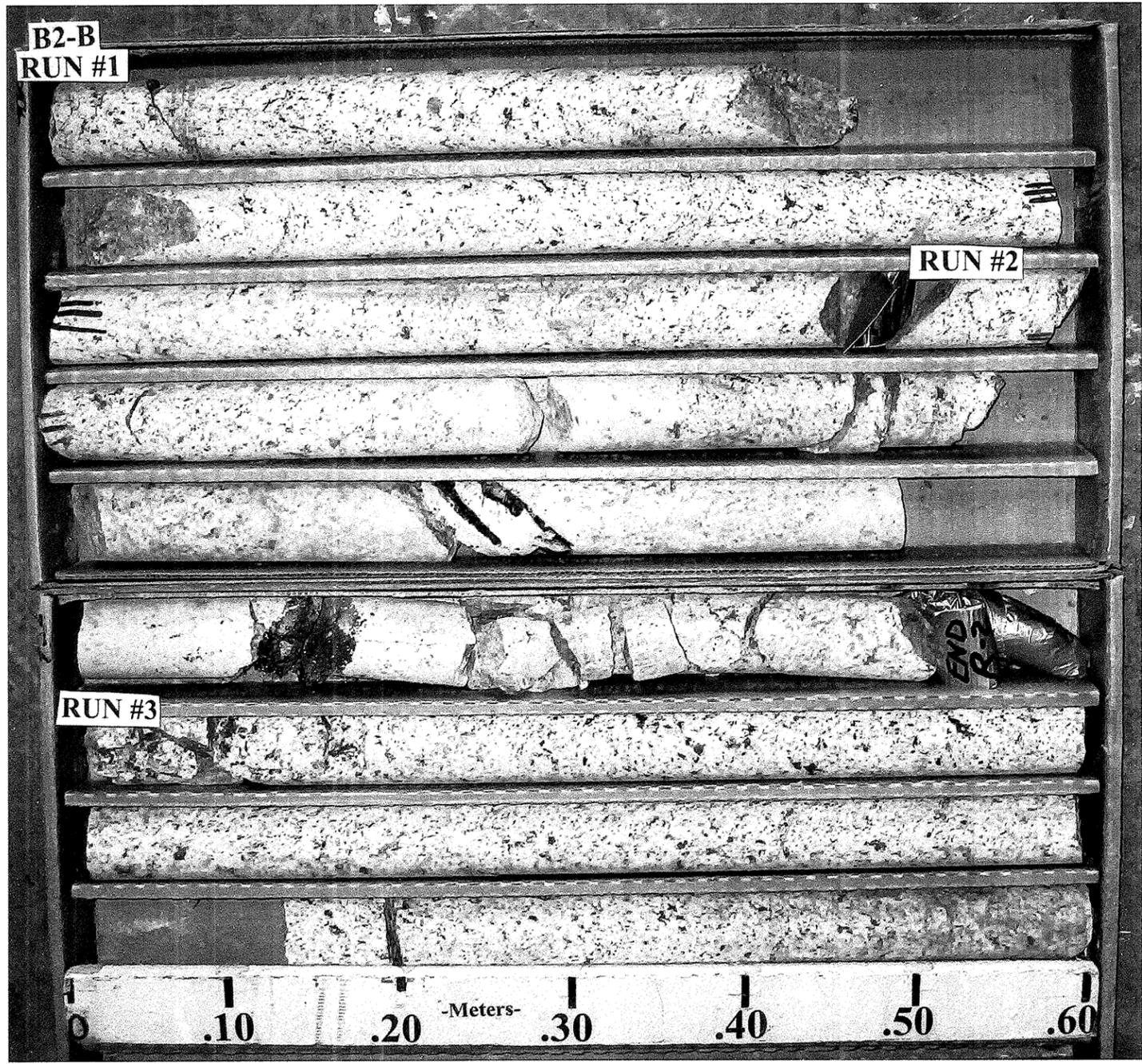
34345.1.1 R-0609IA  
GUILFORD COUNTY  
BRIDGE ON SR 1158 (-Y2-) OVER US 311 AND MILE BRANCH

CORE PHOTOS



34345.1.1 R-0609IA  
GUILFORD COUNTY  
BRIDGE ON SR 1158 (-Y2-) OVER US 311 AND MILE BRANCH

CORE PHOTOS



CONTRACT: ID: R-0609IA

CONTRACT: ID: R-0609IA

# STATE OF NORTH CAROLINA

DEPARTMENT OF TRANSPORTATION

DIVISION OF HIGHWAYS

GEOTECHNICAL ENGINEERING UNIT

## STRUCTURE SUBSURFACE INVESTIGATION

### CONTENTS:

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1	TITLE SHEET
2	LEGEND
3	STRUCTURE INVENTORY REPORT
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5	PROFILE
6,7	CROSS SECTIONS
8-12	BORE LOGS & CORE REPORTS
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	STATE	STATE PROJECT REFERENCE NO.	SHEET NO.	TOTAL SHEETS
	N.C.	34345.1.1 (R-0609IA)	1	15
	STATE PROJ. NO.	F.A. PROJ. NO.	DESCRIPTION	
	34345.1.1	MAF-F-119-1(1)	P.E. CONST.	

### CAUTION NOTICE

THE SUBSURFACE INFORMATION AND THE SUBSURFACE INVESTIGATION ON WHICH IT IS BASED WAS MADE FOR THE PURPOSE OF STUDY, PLANNING AND DESIGN, AND NOT FOR CONSTRUCTION OR PAY PURPOSES. THE VARIOUS FIELD BORING LOGS, ROCK CORES, AND SOIL TEST DATA AVAILABLE MAY BE REVIEWED OR INSPECTED IN RALEIGH BY CONTACTING THE N. C. DEPARTMENT OF TRANSPORTATION, GEOTECHNICAL UNIT @ (919) 250-4088. NEITHER THE SUBSURFACE PLANS AND REPORTS, NOR THE FIELD BORING LOGS, ROCK CORES, OR SOIL TEST DATA IS PART OF THE CONTRACT.

GENERAL SOIL AND ROCK STRATA DESCRIPTIONS AND INDICATED BOUNDARIES ARE BASED ON A GEOTECHNICAL INTERPRETATION OF ALL AVAILABLE SUBSURFACE DATA AND MAY NOT NECESSARILY REFLECT THE ACTUAL SUBSURFACE CONDITIONS BETWEEN BORINGS OR BETWEEN SAMPLED STRATA WITHIN THE BOREHOLE. THE LABORATORY SAMPLE DATA AND THE IN SITU OR-PLACED TEST DATA CAN BE RELIED ON ONLY TO THE DEGREE OF RELIABILITY INHERENT IN THE STANDARD TEST METHOD. THE OBSERVED WATER LEVELS OR SOIL MOISTURE CONDITIONS INDICATED IN THE SUBSURFACE INVESTIGATIONS ARE AS RECORDED AT THE TIME OF THE INVESTIGATION. THESE WATER LEVELS OR SOIL MOISTURE CONDITIONS MAY VARY CONSIDERABLY WITH TIME ACCORDING TO CLIMATIC CONDITIONS INCLUDING TEMPERATURES, PRECIPITATION AND WIND, AS WELL AS OTHER NON-CLIMATIC FACTORS.

THE BIDDER OR CONTRACTOR IS CAUTIONED THAT DETAILS SHOWN ON THE SUBSURFACE PLANS ARE PRELIMINARY ONLY AND IN MANY CASES THE FINAL DESIGN DETAILS ARE DIFFERENT. FOR BIDDING AND CONSTRUCTION PURPOSES, REFER TO THE CONSTRUCTION PLANS AND DOCUMENTS FOR FINAL DESIGN INFORMATION ON THIS PROJECT. THE DEPARTMENT DOES NOT WARRANT OR GUARANTEE THE SUFFICIENCY OR ACCURACY OF THE INVESTIGATION MADE, NOR THE INTERPRETATIONS MADE OR OPINION OF THE DEPARTMENT AS TO THE TYPE OF MATERIALS AND CONDITIONS TO BE ENCOUNTERED. THE BIDDER OR CONTRACTOR IS CAUTIONED TO MAKE SUCH INDEPENDENT SUBSURFACE INVESTIGATIONS AS HE DEEMS NECESSARY TO SATISFY HIMSELF AS TO CONDITIONS TO BE ENCOUNTERED ON THIS PROJECT. THE CONTRACTOR SHALL HAVE NO CLAIM FOR ADDITIONAL COMPENSATION OR FOR AN EXTENSION OF TIME FOR ANY REASON RESULTING FROM THE ACTUAL CONDITIONS ENCOUNTERED AT THE SITE DIFFERING FROM THOSE INDICATED IN THE SUBSURFACE INFORMATION.

STATE PROJECT 34345.1.1 I.D. NO. R-0609IA

F.A. PROJECT MAF-F-119-1(1)

COUNTY GUILFORD

PROJECT DESCRIPTION US 311 HIGH POINT  
EAST BELTWAY FROM US 29-70 TO  
I-85 NORTH OF ARCHDALE

SITE DESCRIPTION STRUCTURE NO. 3  
ON SR 1154 (KERSEY VALLEY RD.)  
OVER PROPOSED US 311 AT  
STATION -Y3- 12 + 61.464

INVESTIGATED BY J.B. BARFIELD PERSONNEL J.L. LOVE

CHECKED BY D.N. ARGENBRIGHT B. SCHULL

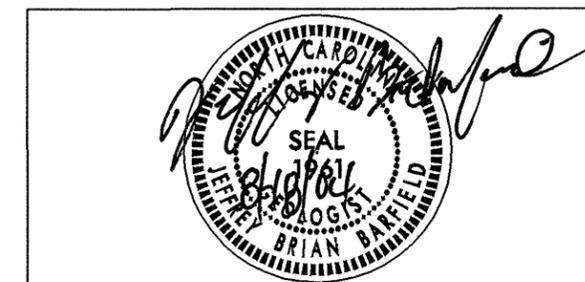
SUBMITTED BY D.N. ARGENBRIGHT D. BOGGS

DATE AUGUST 2004

DRAWN BY: TTW, JBB, DWF, CDC

NOTE - THE INFORMATION CONTAINED HEREIN IS NOT IMPLIED OR GUARANTEED BY THE N. C. DEPARTMENT OF TRANSPORTATION AS BEING ACCURATE NOR IS IT CONSIDERED TO BE PART OF THE PLANS, SPECIFICATIONS, OR CONTRACT FOR THE PROJECT.

NOTE - BY HAVING REQUESTED THIS INFORMATION THE CONTRACTOR SPECIFICALLY WAIVES ANY CLAIMS FOR INCREASED COMPENSATION OR EXTENSION OF TIME BASED ON DIFFERENCES BETWEEN THE CONDITIONS INDICATED HEREIN AND THE ACTUAL CONDITIONS AT THE PROJECT SITE.



NORTH CAROLINA DEPARTMENT OF TRANSPORTATION  
 DIVISION OF HIGHWAYS  
 GEOTECHNICAL UNIT

ID	STATE PROJECT NO.	SHEET NO.	TOTAL SHEETS
R-0609IA	34345.IJ	2	15



SUBSURFACE INVESTIGATION

SOIL AND ROCK LEGEND, TERMS, SYMBOLS, AND ABBREVIATIONS

SOIL DESCRIPTION		GRADATION		ROCK DESCRIPTION		TERMS AND DEFINITIONS	
SOIL IS CONSIDERED TO BE THE UNCONSOLIDATED, SEMI-CONSOLIDATED OR WEATHERED EARTH MATERIALS WHICH CAN BE PENETRATED WITH A CONTINUOUS FLIGHT POWER AUGER, AND WHICH YIELDS LESS THAN 100 BLOWS PER 30 cm ACCORDING TO STANDARD PENETRATION TEST (AASHTO T206, ASTM D-1586). SOIL CLASSIFICATION IS BASED ON THE AASHTO SYSTEM AND BASIC DESCRIPTIONS GENERALLY SHALL INCLUDE: CONSISTENCY, COLOR, TEXTURE, MOISTURE, AASHTO CLASSIFICATION, AND OTHER PERTINENT FACTORS SUCH AS MINERALOGICAL COMPOSITION, ANGULARITY, STRUCTURE, PLASTICITY, ETC. EXAMPLE: <i>VERY STIFF, GRAY SILTY CLAY, MOST WITH INTERBEDDED FINE SAND LAYERS, HEAVY PLASTIC, A-7-6</i>		WELL-GRADED- INDICATES A GOOD REPRESENTATION OF PARTICLE SIZES FROM FINE TO COARSE UNIFORM- INDICATES THAT SOIL PARTICLES ARE ALL APPROXIMATELY THE SAME SIZE (ALSO POORLY GRADED). GAP-GRADED- INDICATES A MIXTURE OF UNIFORM PARTICLES OF TWO OR MORE SIZES.  ANGULARITY OF GRAINS  THE ANGULARITY OR ROUNDNESS OF SOIL GRAINS ARE DESIGNATED BY THE TERMS: ANGULAR, SUBANGULAR, SUBROUNDED, OR ROUNDED.		HARD ROCK IS NON-COASTAL PLAIN MATERIAL THAT WHEN TESTED, WOULD YIELD SPT REFUSAL. AN INFERRED ROCK LINE INDICATES THE LEVEL AT WHICH NON-COASTAL PLAIN MATERIAL WOULD YIELD SPT REFUSAL. SPT REFUSAL IS PENETRATION BY A SPLIT SPOON SAMPLER EQUAL TO OR LESS THAN 2.5 cm PER 50 BLOWS. IN NON-COASTAL PLAIN MATERIAL, THE TRANSITION BETWEEN SOIL AND ROCK IS OFTEN REPRESENTED BY A ZONE OF WEATHERED ROCK. ROCK MATERIALS ARE TYPICALLY DIVIDED AS FOLLOWS:  WEATHERED ROCK (WR)  CRYSTALLINE ROCK (CR)  NON-CRYSTALLINE ROCK (NCR)  COASTAL PLAIN SEDIMENTARY ROCK (CP)		ALLUVIUM (ALLUV.) - SOILS WHICH HAVE BEEN TRANSPORTED BY WATER. AQUIFER - A WATER BEARING FORMATION OR STRATA. ARENACEOUS - APPLIED TO ROCKS THAT HAVE BEEN DERIVED FROM SAND OR THAT CONTAIN SAND. ARGILLACEOUS - APPLIED TO ALL ROCKS OR SUBSTANCES COMPOSED OF CLAY MINERALS, OR HAVING A NOTABLE PROPORTION OF CLAY IN THEIR COMPOSITION, AS SHALE, SLATE, ETC. ARTESIAN - GROUND WATER THAT IS UNDER SUFFICIENT PRESSURE TO RISE ABOVE THE LEVEL AT WHICH IS ENCOUNTERED, BUT WHICH DOES NOT NECESSARILY RISE TO OR ABOVE THE GROUND SURFACE. CALCAREOUS (CALC.) - SOILS WHICH CONTAIN APPRECIABLE AMOUNTS OF CALCIUM CARBONATE. COLLUVIUM - ROCK FRAGMENTS MIXED WITH SOIL DEPOSITED BY GRAVITY ON SLOPE OR AT BOTTOM OF SLOPE. CORE RECOVERY (REC.) - TOTAL LENGTH OF ALL MATERIAL RECOVERED IN THE CORE BARREL DIVIDED BY TOTAL LENGTH OF CORE RUN AND EXPRESSED AS A PERCENTAGE. DIKE - A TABULAR BODY OF IGNEOUS ROCK THAT CUTS ACROSS THE STRUCTURE OF ADJACENT ROCKS OR CUTS MASSIVE ROCK. DIP - THE ANGLE AT WHICH A STRATUM OR ANY PLANAR FEATURE IS INCLINED FROM THE HORIZONTAL. DIP DIRECTION (DIP AZIMUTH) - THE DIRECTION OR BEARING OF THE HORIZONTAL TRACE OF THE LINE OF DIP, MEASURED CLOCKWISE FROM NORTH. FAULT - A FRACTURE OR FRACTURE ZONE ALONG WHICH THERE HAS BEEN DISPLACEMENT OF THE SIDES RELATIVE TO ONE ANOTHER PARALLEL TO THE FRACTURE. FISSILE - A PROPERTY OF SPLITTING ALONG CLOSELY SPACED PARALLEL PLANES. FLOAT - ROCK FRAGMENTS ON SURFACE NEAR THEIR ORIGINAL POSITION AND DISLOGGED FROM PARENT MATERIAL. FLOOD PLAIN (F.P.) - LAND BORDERING A STREAM, BUILT OF SEDIMENTS DEPOSITED BY THE STREAM. FORMATION (FM.) - A MAPPABLE GEOLOGIC UNIT THAT CAN BE RECOGNIZED AND TRACED IN THE FIELD. JOINT - FRACTURE IN ROCK ALONG WHICH NO APPRECIABLE MOVEMENT HAS OCCURRED. LEDGE - A SHELF-LIKE RIDGE OR PROJECTION OF ROCK WHOSE THICKNESS IS SMALL COMPARED TO ITS LATERAL EXTENT. LENS - A BODY OF SOIL OR ROCK THAT THINS OUT IN ONE OR MORE DIRECTIONS. MOTTLED (MOT.) - IRRREGULARLY MARKED WITH SPOTS OF DIFFERENT COLORS. MOTTLING IN SOILS USUALLY INDICATES POOR AERATION AND LACK OF GOOD DRAINAGE. PERCHED WATER - WATER MAINTAINED ABOVE THE NORMAL GROUND WATER LEVEL BY THE PRESENCE OF AN INTERVENING IMPERVIOUS STRATUM. RESIDUAL SOIL - SOIL FORMED IN PLACE BY THE WEATHERING OF ROCK. ROCK QUALITY DESIGNATION (R.Q.D.) - A MEASURE OF ROCK QUALITY DESCRIBED BY: TOTAL LENGTH OF ROCK SEGMENTS EQUAL TO OR GREATER THAN 10 CENTIMETERS DIVIDED BY THE TOTAL LENGTH OF CORE RUN AND EXPRESSED AS A PERCENTAGE. SAPROLITE (SAP.) - RESIDUAL SOIL WHICH RETAINS THE RELIC STRUCTURE OR FABRIC OF THE PARENT ROCK. SILL - AN INTRUSIVE BODY OF IGNEOUS ROCK OF APPROXIMATELY UNIFORM THICKNESS AND RELATIVELY THIN COMPARED WITH ITS LATERAL EXTENT, WHICH HAS BEEN EMPLACED PARALLEL TO THE BEDDING OR SCHISTOSITY OF THE INTRUDED ROCKS. SLICKENSIDE - POLISHED AND STRIATED SURFACE THAT RESULTS FROM FRICTION ALONG A FAULT OR SLIP PLANE. STANDARD PENETRATION TEST (PENETRATION RESISTANCE) (SPT) - NUMBER OF BLOWS (N) OF A 63.5 kg HAMMER FALLING 0.76 METERS REQUIRED TO PRODUCE A PENETRATION OF 30 cm INTO SOIL WITH A 5 cm OUTSIDE DIAMETER SPLIT SPOON SAMPLER. SPT REFUSAL IS LESS THAN 2.5 cm PENETRATION WITH 50 BLOWS. STRATA CORE RECOVERY (SCREC) - TOTAL LENGTH OF STRATA MATERIAL RECOVERED DIVIDED BY TOTAL LENGTH OF STRATUM AND EXPRESSED AS A PERCENTAGE. STRATA ROCK QUALITY DESIGNATION (S.R.Q.D.) - A MEASURE OF ROCK QUALITY DESCRIBED BY: TOTAL LENGTH OF ROCK SEGMENTS WITHIN A STRATUM EQUAL TO OR GREATER THAN 10 CENTIMETERS DIVIDED BY THE TOTAL LENGTH OF STRATA AND EXPRESSED AS A PERCENTAGE. TOPSOIL (T.S.) - SURFACE SOILS USUALLY CONTAINING ORGANIC MATTER.	
SOIL LEGEND AND AASHTO CLASSIFICATION		MINERALOGICAL COMPOSITION		WEATHERING			
GENERAL CLASS. GRANULAR MATERIALS (>35% PASSING #200) SILT-CLAY MATERIALS (>85% PASSING #200) ORGANIC MATERIALS		MINERAL NAMES SUCH AS QUARTZ, FELDSPAR, MICA, TALC, KAOLIN, ETC. ARE USED IN DESCRIPTIONS WHENEVER THEY ARE CONSIDERED OF SIGNIFICANCE.		FRESH ROCK FRESH, CRYSTALS BRIGHT, FEW JOINTS MAY SHOW SLIGHT STAINING, ROCK RINGS UNDER HAMMER IF CRYSTALLINE.			
GROUP CLASS. A-1, A-3, A-2, A-4, A-5, A-6, A-7, A-1, A-2, A-4, A-5, A-6, A-7		COMPRESSIBILITY SLIGHTLY COMPRESSIBLE LIQUID LIMIT LESS THAN 30 MODERATELY COMPRESSIBLE LIQUID LIMIT 31-50 HIGHLY COMPRESSIBLE LIQUID LIMIT GREATER THAN 50		VERY SLIGHT (V. SL.) ROCK GENERALLY FRESH, JOINTS STAINED, SOME JOINTS MAY SHOW THIN CLAY COATINGS IF OPEN, CRYSTALS ON A BROKEN SPECIMEN FACE SHINE BRIGHTLY, ROCK RINGS UNDER HAMMER BLOWS IF OF A CRYSTALLINE NATURE.			
SYMBOL		PERCENTAGE OF MATERIAL		SLIGHT (SL.) ROCK GENERALLY FRESH, JOINTS STAINED AND DISCOLORATION EXTENDS INTO ROCK UP TO 2.5 cm. OPEN JOINTS MAY CONTAIN CLAY. IN GRANITOID ROCKS SOME OCCASIONAL FELDSPAR CRYSTALS ARE DULL AND DISCOLORED. CRYSTALLINE ROCKS RING UNDER HAMMER BLOWS.			
% PASSING		GROUND WATER		MODERATE (MOD.) SIGNIFICANT PORTIONS OF ROCK SHOW DISCOLORATION AND WEATHERING EFFECTS. IN GRANITOID ROCKS, MOST FELDSPARS ARE DULL AND DISCOLORED, SOME SHOW CLAY. ROCK HAS DULL SOUND UNDER HAMMER BLOWS AND SHOWS SIGNIFICANT LOSS OF STRENGTH AS COMPARED WITH FRESH ROCK.			
LIQUID LIMIT PLASTIC INDEX		MISCELLANEOUS SYMBOLS		MODERATELY SEVERE (MOD. SEV.) ALL ROCK EXCEPT QUARTZ DISCOLORED OR STAINED. IN GRANITOID ROCKS, ALL FELDSPARS DULL AND DISCOLORED AND A MAJORITY SHOW KAOLINIZATION. ROCK SHOWS SEVERE LOSS OF STRENGTH AND CAN BE EXCAVATED WITH A GEOLOGIST'S PICK. ROCK GIVES "CLUNK" SOUND WHEN STRUCK. <i>IF TESTED, WOULD YIELD SPT REFUSAL</i>			
GROUP INDEX		ROADWAY EMBANKMENT WITH SOIL DESCRIPTION		SEVERE (SEV.) ALL ROCKS EXCEPT QUARTZ DISCOLORED OR STAINED. ROCK FABRIC CLEAR AND EVIDENT BUT REDUCED IN STRENGTH TO STRONG SOIL. IN GRANITOID ROCKS ALL FELDSPARS ARE KAOLINIZED TO SOME EXTENT. SOME FRAGMENTS OF STRONG ROCK USUALLY REMAIN. <i>IF TESTED, YIELDS SPT N VALUES &gt; 100 BLOWS PER 30 cm</i>			
USUAL TYPES OF MAJOR MATERIALS		SOIL SYMBOL		VERY SEVERE (V. SEV.) ALL ROCK EXCEPT QUARTZ DISCOLORED OR STAINED. ROCK FABRIC ELEMENTS ARE DISCERNIBLE BUT THE MASS IS EFFECTIVELY REDUCED TO SOIL STATUS, WITH ONLY FRAGMENTS OF STRONG ROCK REMAINING. SAPROLITE IS AN EXAMPLE OF ROCK WEATHERED TO A DEGREE SUCH THAT ONLY MINOR VESTIGES OF THE ORIGINAL ROCK FABRIC REMAIN. <i>IF TESTED, YIELDS SPT N VALUES &lt; 100 BLOWS PER 30 cm</i>			
GEN. RATING AS A SUBGRADE		ARTIFICIAL FILL OTHER THAN ROADWAY EMBANKMENTS		COMPLETE ROCK REDUCED TO SOIL. ROCK FABRIC NOT DISCERNIBLE, OR DISCERNIBLE ONLY IN SMALL AND SCATTERED CONCENTRATIONS. QUARTZ MAY BE PRESENT AS DIXES OR STRINGERS. SAPROLITE IS ALSO AN EXAMPLE.			
P.I. OF A-7-5 ≤ L.L. - 30 : P.I. OF A-7-6 > L.L. - 30		INFERRED SOIL BOUNDARIES		ROCK HARDNESS			
CONSISTENCY OR DENSENESS		INFERRED ROCK LINE		VERY HARD CANNOT BE SCRATCHED BY KNIFE OR SHARP PICK. BREAKING OF HAND SPECIMENS REQUIRES SEVERAL HARD BLOWS OF THE GEOLOGIST'S PICK.			
PRIMARY SOIL TYPE		ALLUVIAL SOIL BOUNDARY		HARD CAN BE SCRATCHED BY KNIFE OR PICK ONLY WITH DIFFICULTY. HARD HAMMER BLOWS REQUIRED TO DETACH HAND SPECIMEN.			
COMPACTNESS OR CONSISTENCY		DIP/DIP DIRECTION OF ROCK STRUCTURES		MODERATELY HARD CAN BE SCRATCHED BY KNIFE OR PICK. GOUGES OR GROOVES TO 6 mm DEEP CAN BE EXCAVATED BY HARD BLOW OF A GEOLOGIST'S PICK. HAND SPECIMENS CAN BE DETACHED BY MODERATE BLOWS.			
RANGE OF STANDARD PENETRATION RESISTANCE (N-VALUE)		SOUNDING ROD		MEDIUM HARD CAN BE GROOVED OR GOUGED 1 mm DEEP BY FIRM PRESSURE OF KNIFE OR PICK POINT. CAN BE EXCAVATED IN SMALL CHIPS TO PIECES 25 mm MAXIMUM SIZE BY HARD BLOWS OF THE POINT OF A GEOLOGIST'S PICK.			
RANGE OF UNCONFINED COMPRESSIVE STRENGTH (kN/m <sup>2</sup> )		ABBREVIATIONS		SOFT CAN BE GROVED OR GOUGED READILY BY KNIFE OR PICK. CAN BE EXCAVATED IN FRAGMENTS FROM CHIPS TO SEVERAL CENTIMETERS IN SIZE BY MODERATE BLOWS OF A PICK POINT. SMALL, THIN PIECES CAN BE BROKEN BY FINGER PRESSURE.			
TEXTURE OR GRAIN SIZE		AR - AUGER REFUSAL		VERY SOFT CAN BE CARVED WITH KNIFE. CAN BE EXCAVATED READILY WITH POINT OF PICK. PIECES 25 mm OR MORE IN THICKNESS CAN BE BROKEN BY FINGER PRESSURE. CAN BE SCRATCHED READILY BY FINGER NAIL.			
U.S. STD. SIEVE SIZE OPENING (MM)		BT - BORING TERMINATED		FRACTURE SPACING		BEDDING	
BOULDER (BLDR.)		CL - CLAY		TERM SPACING		TERM THICKNESS	
COBBLE (COB.)		CPT - CONE PENETRATION TEST		VERY WIDE MORE THAN 3 m		VERY THICKLY BEDDED > 1 m	
GRAVEL (GR.)		CSE - COARSE		WIDE 1 TO 3 m		THICKLY BEDDED 0.5 - 1 m	
COARSE SAND (CSE, SD.)		DHT - DILATOMETER TEST		MODERATELY CLOSE 30 TO 100 cm		THINLY BEDDED 0.05 - 0.5 m	
FINE SAND (F. SD.)		DPT - DYNAMIC PENETRATION TEST		CLOSE 5 TO 30 cm		VERY THINLY BEDDED 10 - 50 mm	
SILT (SL.)		e - VOID RATIO		VERY CLOSE LESS THAN 5 cm		THICKLY LAMINATED 2.5 - 10 mm	
CLAY (CL.)		f - FINE				THINLY LAMINATED < 2.5 mm	
GRAIN SIZE		FOSS. - FOSSILIFEROUS		INDURATION			
SOIL MOISTURE - CORRELATION OF TERMS		FRAC. - FRACTURED		FOR SEDIMENTARY ROCKS, INDURATION IS THE HARDENING OF THE MATERIAL BY CEMENTING, HEAT, PRESSURE, ETC.			
SOIL MOISTURE SCALE (ATTERBERG LIMITS)		FRAGS. - FRAGMENTS		FRIABLE RUBBING WITH FINGER FREES NUMEROUS GRAINS; GENTLE BLOW BY HAMMER DISINTEGRATES SAMPLE.			
FIELD MOISTURE DESCRIPTION		MED. - MEDIUM		MODERATELY INDURATED GRAINS CAN BE SEPARATED FROM SAMPLE WITH STEEL PROBE; BREAKS EASILY WHEN HIT WITH HAMMER.			
GUIDE FOR FIELD MOISTURE DESCRIPTION		EQUIPMENT USED ON SUBJECT PROJECT		INDURATED GRAINS ARE DIFFICULT TO SEPARATE WITH STEEL PROBE; DIFFICULT TO BREAK WITH HAMMER.			
- SATURATED - (SAT) USUALLY LIQUID; VERY WET, USUALLY FROM BELOW THE GROUND WATER TABLE		DRILL UNITS:		EXTREMELY INDURATED SHARP HAMMER BLOWS REQUIRED TO BREAK SAMPLE; SAMPLE BREAKS ACROSS GRAINS.			
- WET - (W) SEMISOLID; REQUIRES DRYING TO ATTAIN OPTIMUM MOISTURE		ADVANCING TOOLS:					
- MOIST - (M) SOLID; AT OR NEAR OPTIMUM MOISTURE		MOBILE B- _____					
- DRY - (D) REQUIRES ADDITIONAL WATER TO ATTAIN OPTIMUM MOISTURE		BK-51 _____					
PLASTICITY		CME-45C _____					
PLASTICITY INDEX (PI)		CME-550 _____					
NONPLASTIC 0-5 VERY LOW		PORTABLE HOIST _____					
LOW PLASTICITY 6-15 SLIGHT		OTHER _____					
MED. PLASTICITY 16-25 MEDIUM		OTHER _____					
HIGH PLASTICITY 26 OR MORE HIGH		OTHER _____					
COLOR							
DESCRIPTIONS MAY INCLUDE COLOR OR COLOR COMBINATIONS (TAN, RED, YEL-BRN, BLUE-GRAY) MODIFIERS SUCH AS LIGHT, DARK, STREAKED, ETC. ARE USED TO DESCRIBE APPEARANCE.							



STATE OF NORTH CAROLINA  
DEPARTMENT OF TRANSPORTATION

Michael F. Easley  
GOVERNOR

P.O. BOX 25201, RALEIGH, N.C. 27611-5201

Lyndo Tippett  
SECRETARY

August 18, 2004

STATE PROJECT: 34345.1.1 (R-0609IA)  
F.A. PROJECT: MAF-F-119-1 (1)  
COUNTY: Guilford  
DESCRIPTION: US 311 High Point East Beltway from US 29-70 to I-85 north of Archdale  
SUBJECT: Geotechnical Report – Structure No. 3 on -Y3- at Sta. 12+61.464 over US 311

**Project Description**

A three span bridge, 75 meters in length, is proposed on -Y3- (SR 1154) over proposed US 311. The project is located in Southwest Guilford County near High Point. The skew varies at each bent from 86° 24' 43" to 90° 06' 00"

The subsurface investigation was conducted during June of 2004 using an ATV-mounted CME 45 drill machine. Two Standard Penetration Test borings were performed at each of the three proposed bent locations. All borings were advanced until weathered rock or crystalline rock was encountered. EB1-A and B1-B were cored using NXWL to recover samples from crystalline rock. Representative soil samples were obtained for visual classification in the field and selected samples were sent to the Materials and Test Unit for laboratory analysis.

**Physiography and Geology**

The project is located in gently rolling terrain of the Piedmont Physiographic Province. Geologically, the site is located within the Carolina Slate Belt and is underlain by meta-granite. The area consists of a mixture of wooded land and homes.

**Soil Properties**

Residual soils were the only soils encountered at the project site during this investigation.

Residual soils were encountered in all of the borings and range in thickness from 1.22 to 2.35 meters. These soils consist primarily of green, gray, white and tan, dry to moist, hard, sandy silt (A-4) and orange-tan, moist, stiff, sandy clay (A-6). Residual soils are derived from the underlying weathered rock.

**Rock Properties**

Weathered rock was derived from the underlying metamorphosed granitic rock, and ranges in thickness from 0.66 to 3.77 meters. The top of weathered rock was encountered at elevations ranging from 244.35 to 242.61 meters.

Crystalline rock was encountered at each boring location. The top of the crystalline rock ranges from 243.18 in EB2-B to 238.29 in EB1-A. Rock core was obtained from EB1-A and B1-B. Crystalline rock in both borings was white to gray, pink to gray, fresh to severely weathered, moderately close to very closely fractured and moderately to very hard meta-granite. Two thin lenses (0.18-0.44m) of green to brown, slightly weathered, very close to moderately close fractures, moderately hard diabase was encountered in EB1-A at elevations 238.83 and 232.71 meters. Core recovery (REC) ranged from 89 to 95 percent and rock quality designation (RQD) ranged from 65 to 81 percent.

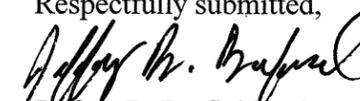
**Groundwater**

Groundwater was encountered in four of the six borings. Groundwater elevations ranged from 242.84 to 240.27 meters.

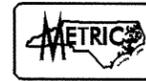
**Notice**

This Geotechnical foundation report is based on the Preliminary General Drawing for Structure No. 3, dated April 7, 2004. If significant changes are made in the design or location of the proposed structure, the subsurface information should be reviewed and modified as necessary.

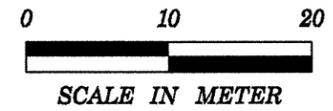
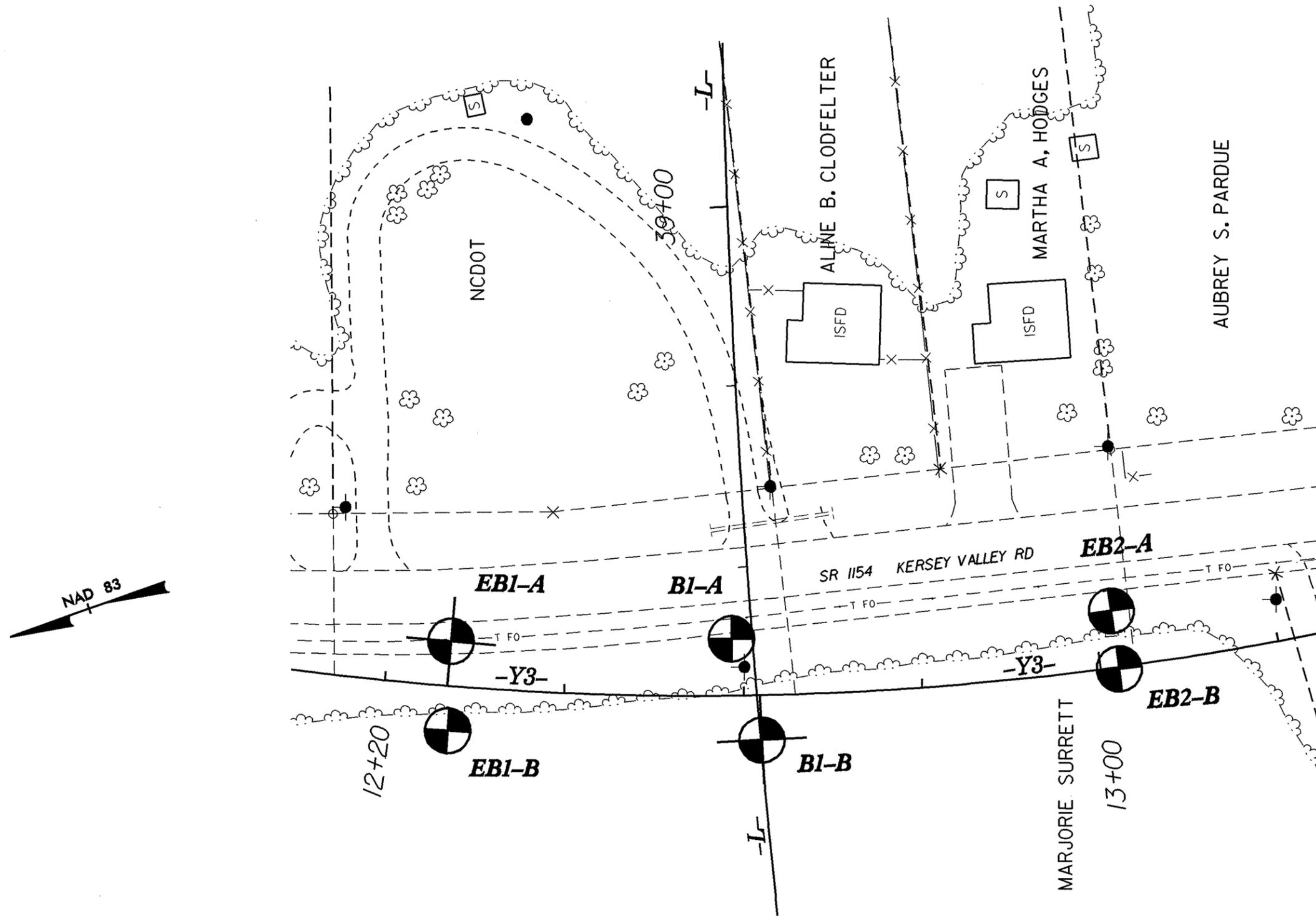
Respectfully submitted,

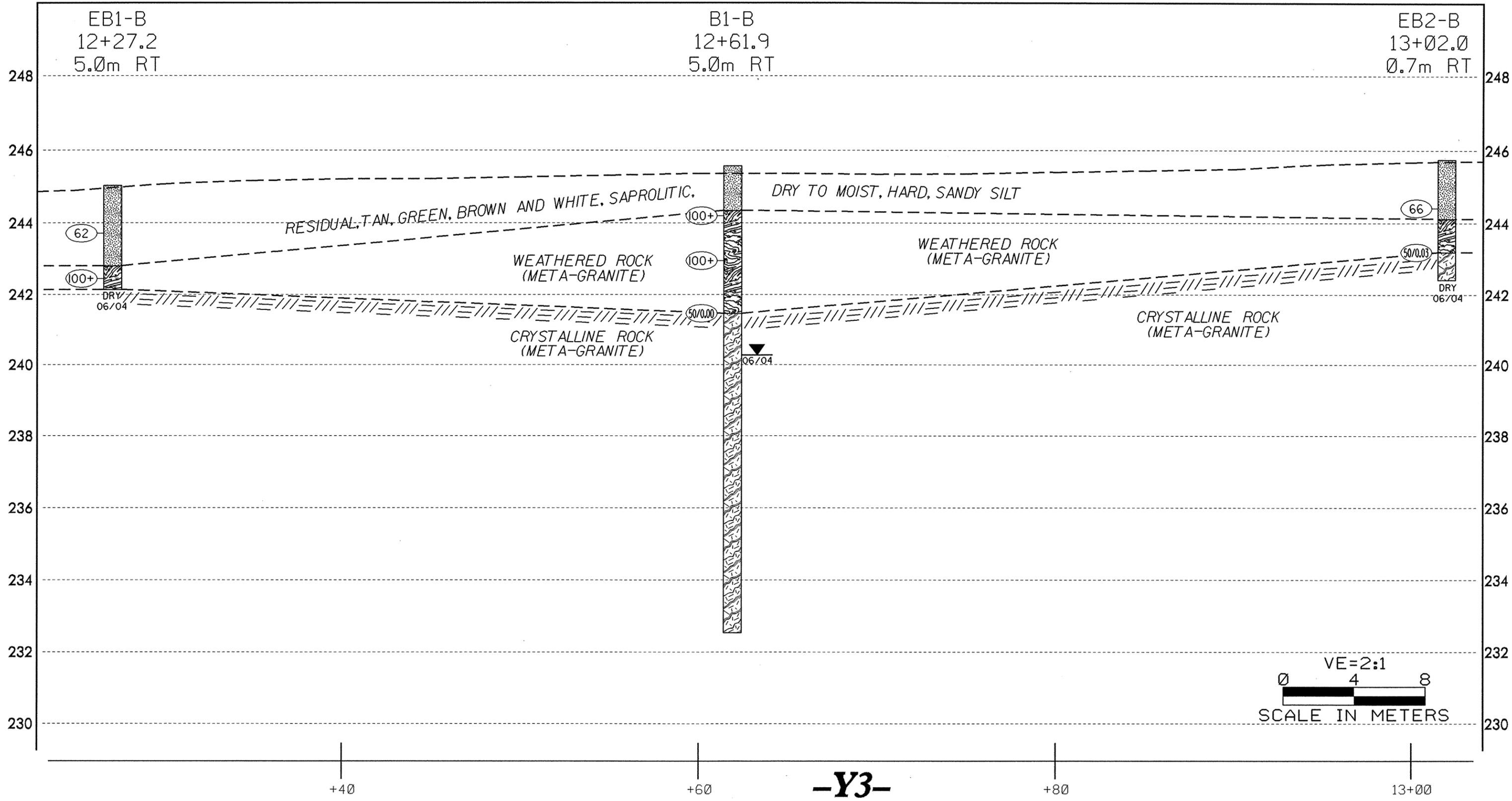
  
Jeffrey B. Barfield, LG  
Project Geologist

# TEST SITE PLAN

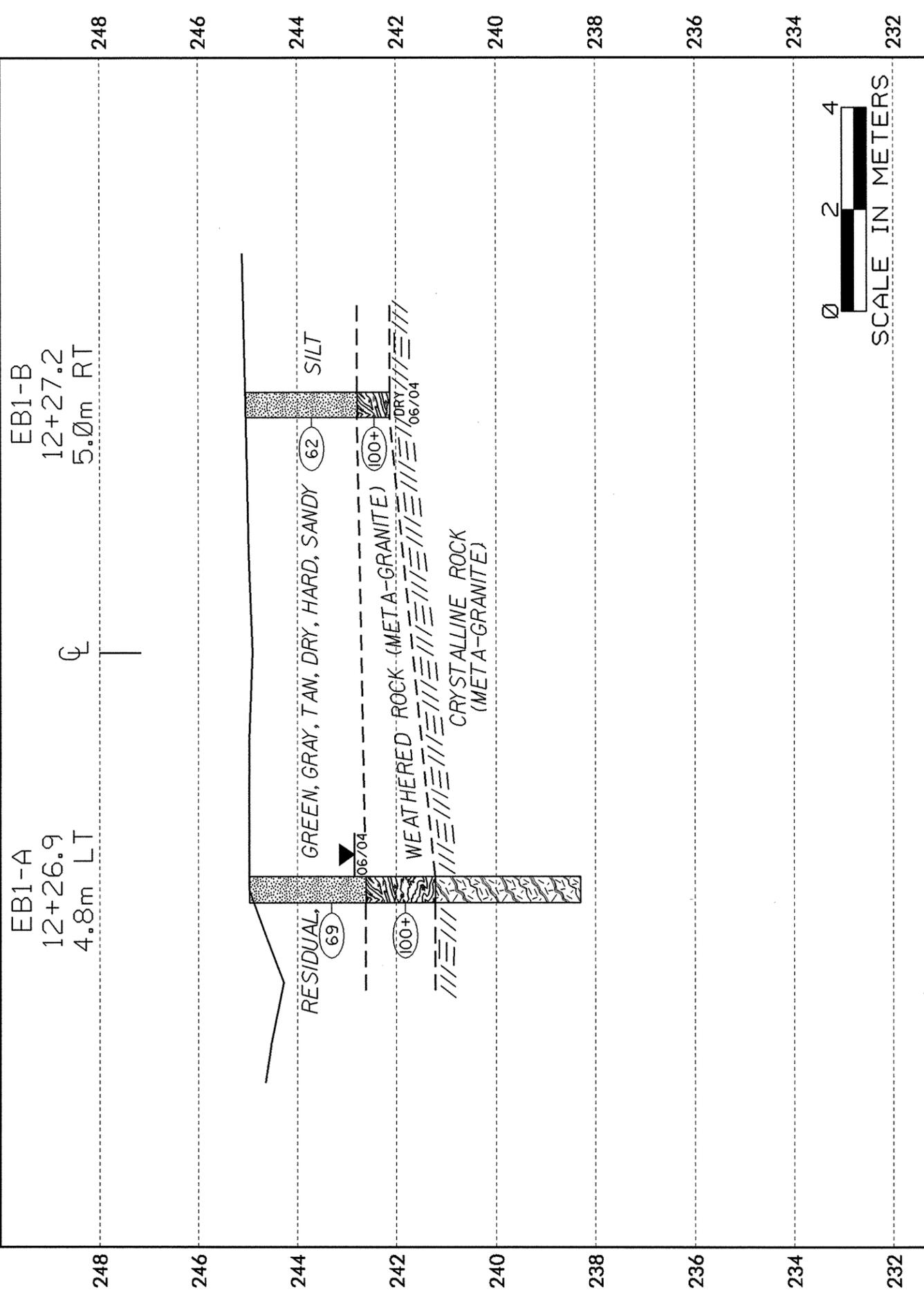


PROJECT REF. NO.	SHEET NO.	TOTAL SHEETS
34345.1.1	4	15

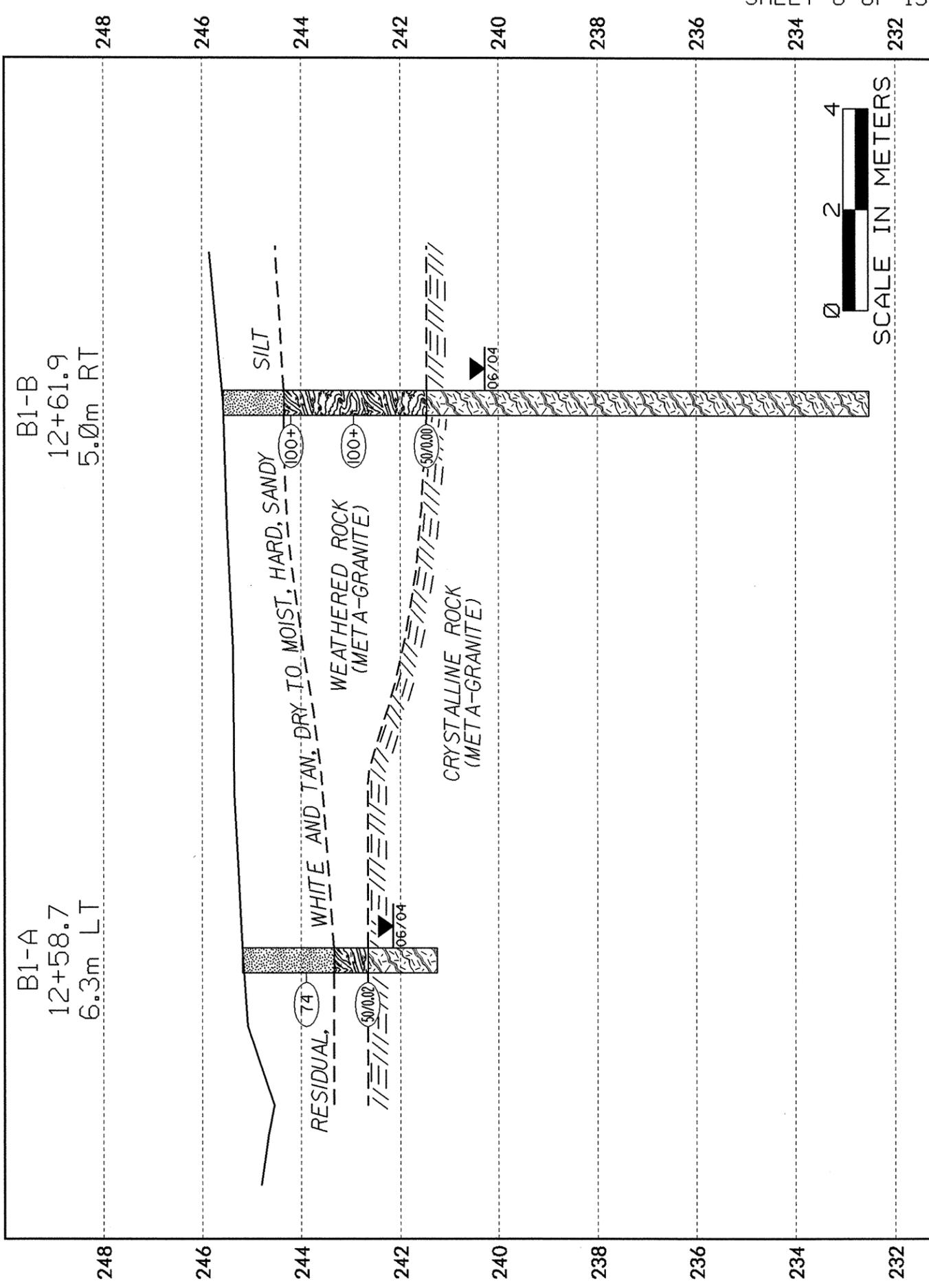




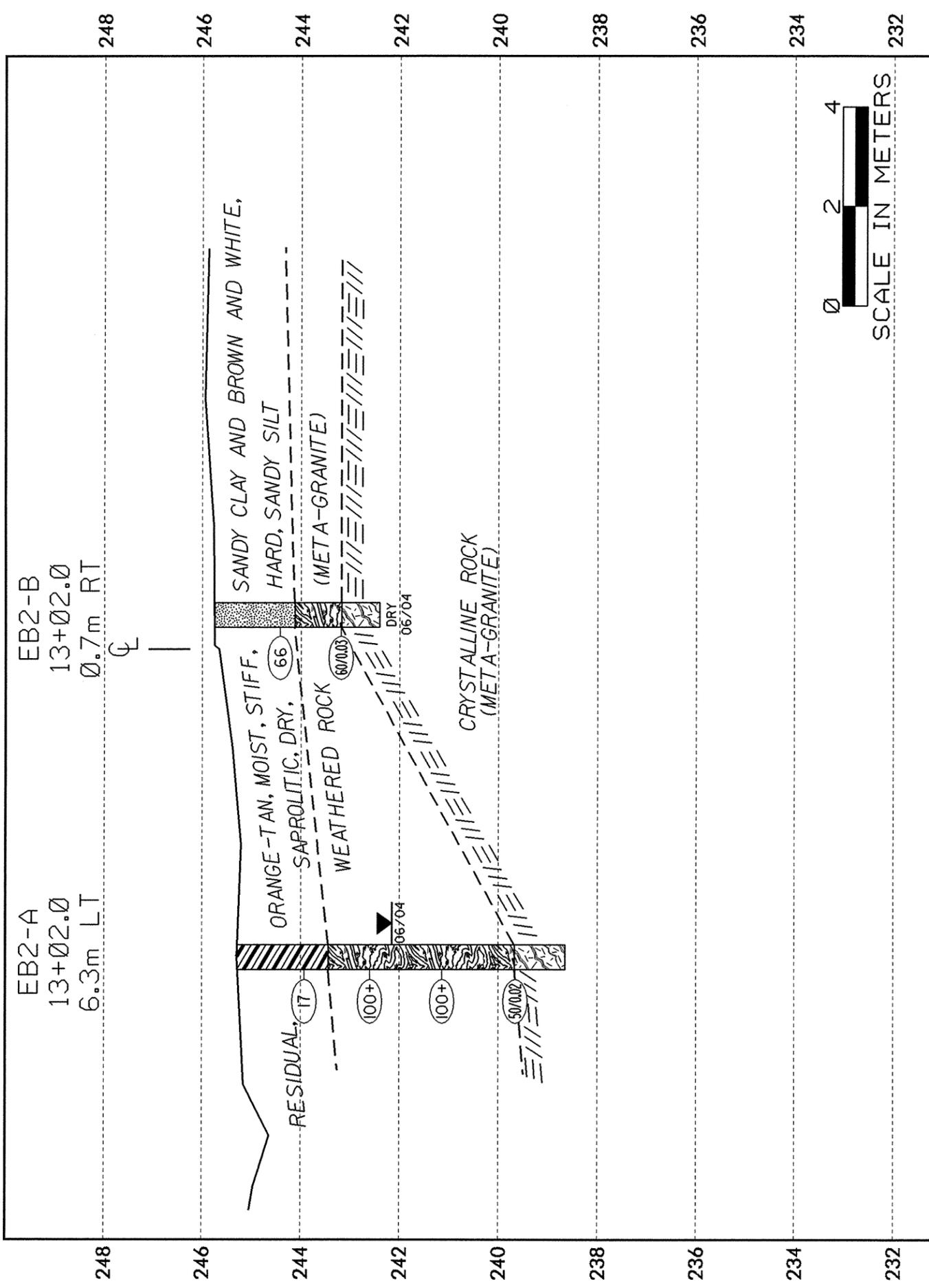
CROSS SECTION THROUGH END BENT I STRUCTURE NO. 3, 34345.1.1 (R-0609IA)



CROSS SECTION THROUGH BENT I STRUCTURE NO. 3, 34345.1.1 (R-0609IA)



CROSS SECTION THROUGH END BENT 2 STRUCTURE NO. 3, 34345.1.1 (R-0609IA)









NORTH CAROLINA DEPARTMENT OF TRANSPORTATION  
 GEOTECHNICAL UNIT BORING LOG

PROJECT NO. 34345.1.1	ID. R-06091A	COUNTY GUILFORD	GEOLOGIST J.L. LOVE/J.B. BARFIELD
SITE DESCRIPTION STRUCTURE NO. 3 ON -Y3- SR 1154 (KERSEY VALLEY RD.) OVER -L- PROPOSED US 311			GROUND WATER
BORING NO. BI-B	BORING LOCATION 12+61.9	OFFSET 5.0m RT	ALIGNMENT -Y3-
COLLAR ELEV. 245.57m	NORTHING 242979.5	EASTING 525085.9	0 HR. N/A
TOTAL DEPTH 13.04m		DRILL MACHINE CME-45	DRILL METHOD NW CASING/NXWL
START DATE 6/3/04	COMPLETION DATE 6/3/04	SURFACE WATER DEPTH N/A	HAMMER TYPE MANUAL
DEPTH TO ROCK 4.11m			

ELEV.	DEPTH	BLOW COUNT			PEN. (m)	BLOWS PER 30cm				SAMPLE NUMBER	LOG	SOIL AND ROCK DESCRIPTION
		15cm	15cm	15cm		0	25	50	75			
245.57	1.07	34	42	58	0.28					SS-4	M	RESIDUAL, TAN, SANDY SILT
244.00	2.59	100			0.09							WEATHERED ROCK (META-GRANITE)
242.00	4.11	50			0.00							CRYSTALLINE ROCK (META-GRANITE)
240.00												
238.00												
236.00												
234.00												
232.00												
230.00												
228.00												
226.00												

CORE BORING REPORT

PROJECT: 34345.1.1	ID: R-06091A	COUNTY: GUILFORD	BORING NO: B1-B
DESCRIPTION: STRUCTURE NO. 3 ON -Y3- SR 1154 OVER -L- PROPOSED US 311			
LOCATION OF BORING: -Y3-, 12+61.9, 5.0m RT		COMPLETION DATE: 6/3/04	
COLLAR or GROUND ELEVATION: 245.57 m		CORE SIZE: NXWL	GEOLOGIST: J.L. LOVE/J.B. BARFIELD
CORE EQUIPMENT: CME-45, NXWL,		DRILLER: BRYAN SCHULL	

ELEV (m)	DEPTH (m)	DRILL RATE (min/0.3m)	RUN (m)	REC (m) (%)	RQD (m) (%)	SAMPLE NUMBER	FIELD CLASSIFICATION and REMARKS
241.19	4.38	9:26					WHITE, VERY SLIGHTLY WEATHERED, VERY HARD, MODERATELY CLOSE FRACTURES, MASSIVE META-GRANITE, TWO FRACTURES @ 20 DEGREES
		13:40		0.64	0.64		
		1:16/0.06	0.66	(97%)	(97%)		
240.53	5.04						WHITE TO GRAY, SLIGHTLY TO FRESHLY WEATHERED, MODERATELY TO VERY HARD, MASSIVE META-GRANITE, THREE HORIZONTAL FRACTURES, TWO @ 25 DEGREES AND THREE @ 50 DEGREES
240.53	5.04	6:57		1.35	0.78		
		4:44	1.52	(89%)	(51%)		
		4:35					
239.01	6.56	3:44					(6.56-6.74m) WHITE TO GRAY, SLIGHTLY TO FRESHLY WEATHERED, MODERATELY TO VERY HARD, (6.74-7.18m) BROWN, SLIGHTLY WEATHERED, MOD. HARD, MOD. CLOSE FRACTURES, THICKLY BEDDED, DIABASE (7.18-8.08m), WHITE TO GRAY, SLIGHTLY TO FRESHLY WEATHERED, MODERATELY TO VERY HARD, MASSIVE, META-GRANITE, THREE HORIZONTAL FRACTURES, THREE @ 30 DEGREES, AND ONE VERTICAL FRACTURE
239.01	6.56	4:00					
		3:29	1.52	1.10	0.44		
		5:58		(72%)	(29%)		
237.49	8.08	2:45					(8.08-8.40m) WHITE AND GRAY, SEVERELY WEATHERED, HARD, VERY CLOSE FRACTURES, META-GRANITE, (8.40-9.60m) WHITE TO GRAY, SLIGHTLY WEATHERED, VERY HARD, CLOSE FRACTURES, MASSIVE META-GRANITE, ONE HORIZONTAL FRACTURE, TWO @ 35 DEGREES AND TWO @ 70 DEGREES
237.49	8.08	3:15					
		3:18	1.52	1.27	0.73		
		4:13		(84%)	(48%)		
235.97	9.60	4:36					WHITE TO GRAY, FRESHLY WEATHERED, VERY HARD, CLOSE FRACTURES, MASSIVE, META-GRANITE, ONE FRACTURE @ 10 DEGREES
235.97	9.60	5:11					
		7:08	1.22	1.22	1.22	RS-1	
		6:15		(100%)	(100%)		
234.75	10.82	4:12					WHITE TO GRAY, FRESHLY WEATHERED, VERY HARD, CLOSE FRACTURES, MASSIVE, META-GRANITE, ONE FRACTURE @ 25 DEGREES
234.75	10.82	8:30					
		1:57/0.09	1.00	0.96	0.88		
		6:15		(96%)	(88%)		
233.75	11.82	7:24					(11.82-12.86m) WHITE TO GRAY, FRESHLY WEATHERED, VERY HARD, CLOSE FRACTURES, MASSIVE, META-GRANITE, (12.86-13.04m) GREEN, SLIGHTLY WEATHERED, MODERATELY HARD, VERY CLOSE FRACTURES, DIABASE, ONE FRACTURE @ 80 DEGREES
233.75	11.82	7:35					
		5:17	1.22	1.18	0.95		
		4:58		(97%)	(78%)		
232.53	13.04	7:27					
		5:10					

BOREHOLE TERMINATED AT ELEVATION OF 232.53 METERS, IN META-GRANITE.



**EB1-A**

<b>SOIL TEST RESULTS</b>															
SAMPLE NO.	OFFSET	STATION	DEPTH INTERVAL	AASHTO CLASS.	L.L.	P.I.	% BY WEIGHT				% PASSING (SIEVES)			%	%
							C.SAND	F.SAND	SILT	CLAY	10	40	200		
SS-3	4.8m LT	12+26.9	1.36-1.81	A-4(4)	38	6	9.6	30.9	47.4	12.2	100	95	68	-	-

**EB1-B**

<b>SOIL TEST RESULTS</b>															
SAMPLE NO.	OFFSET	STATION	DEPTH INTERVAL	AASHTO CLASS.	L.L.	P.I.	% BY WEIGHT				% PASSING (SIEVES)			%	%
							C.SAND	F.SAND	SILT	CLAY	10	40	200		
SS-2	5.0m RT	12+27.2	1.03-1.48	A-4(0)	26	3	20.7	29.3	35.8	14.2	78	68	45	-	-

**B1-A**

<b>SOIL TEST RESULTS</b>															
SAMPLE NO.	OFFSET	STATION	DEPTH INTERVAL	AASHTO CLASS.	L.L.	P.I.	% BY WEIGHT				% PASSING (SIEVES)			%	%
							C.SAND	F.SAND	SILT	CLAY	10	40	200		
SS-5	6.3m LT	12+58.7	1.00-1.27	A-4(2)	31	4	18.3	25.8	43.7	12.2	100	89	64	-	-

**B1-B**

<b>SOIL TEST RESULTS</b>															
SAMPLE NO.	OFFSET	STATION	DEPTH INTERVAL	AASHTO CLASS.	L.L.	P.I.	% BY WEIGHT				% PASSING (SIEVES)			%	%
							C.SAND	F.SAND	SILT	CLAY	10	40	200		
SS-4	5.0m RT	12+61.9	1.07-1.22	A-4(0)	22	NP	30.1	27.2	28.5	14.2	100	84	47	-	-

**EB2-A**

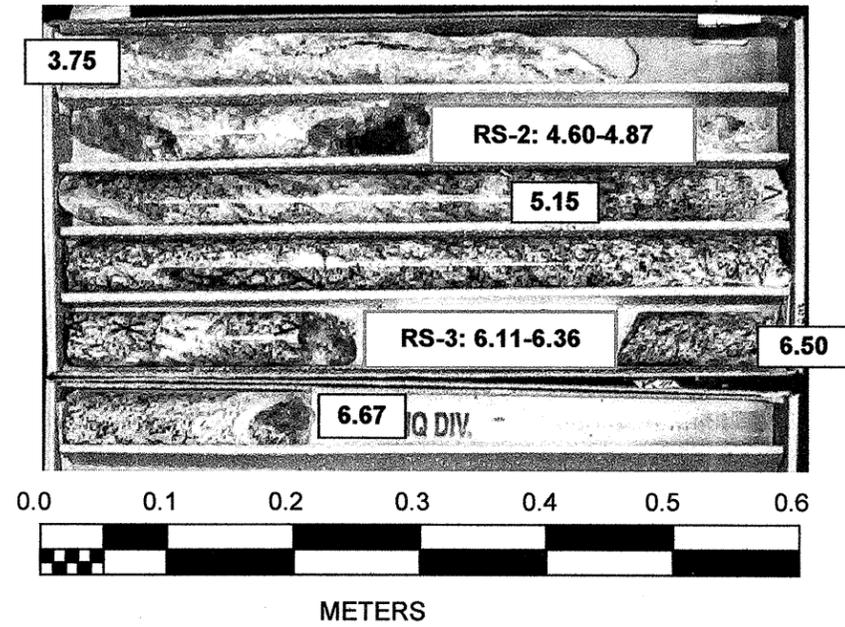
<b>SOIL TEST RESULTS</b>															
SAMPLE NO.	OFFSET	STATION	DEPTH INTERVAL	AASHTO CLASS.	L.L.	P.I.	% BY WEIGHT				% PASSING (SIEVES)			%	%
							C.SAND	F.SAND	SILT	CLAY	10	40	200		
SS-6	6.3m LT	13+02.0	1.07-1.52	A-6(7)	36	13	16.3	24.8	36.6	22.4	99	91	65	-	-

**EB2-B**

<b>SOIL TEST RESULTS</b>															
SAMPLE NO.	OFFSET	STATION	DEPTH INTERVAL	AASHTO CLASS.	L.L.	P.I.	% BY WEIGHT				% PASSING (SIEVES)			%	%
							C.SAND	F.SAND	SILT	CLAY	10	40	200		
SS-1	0.7m RT	13+02.0	1.02-1.47	A-4(0)	23	NP	35.8	29.1	20.9	14.2	95	75	39	-	-

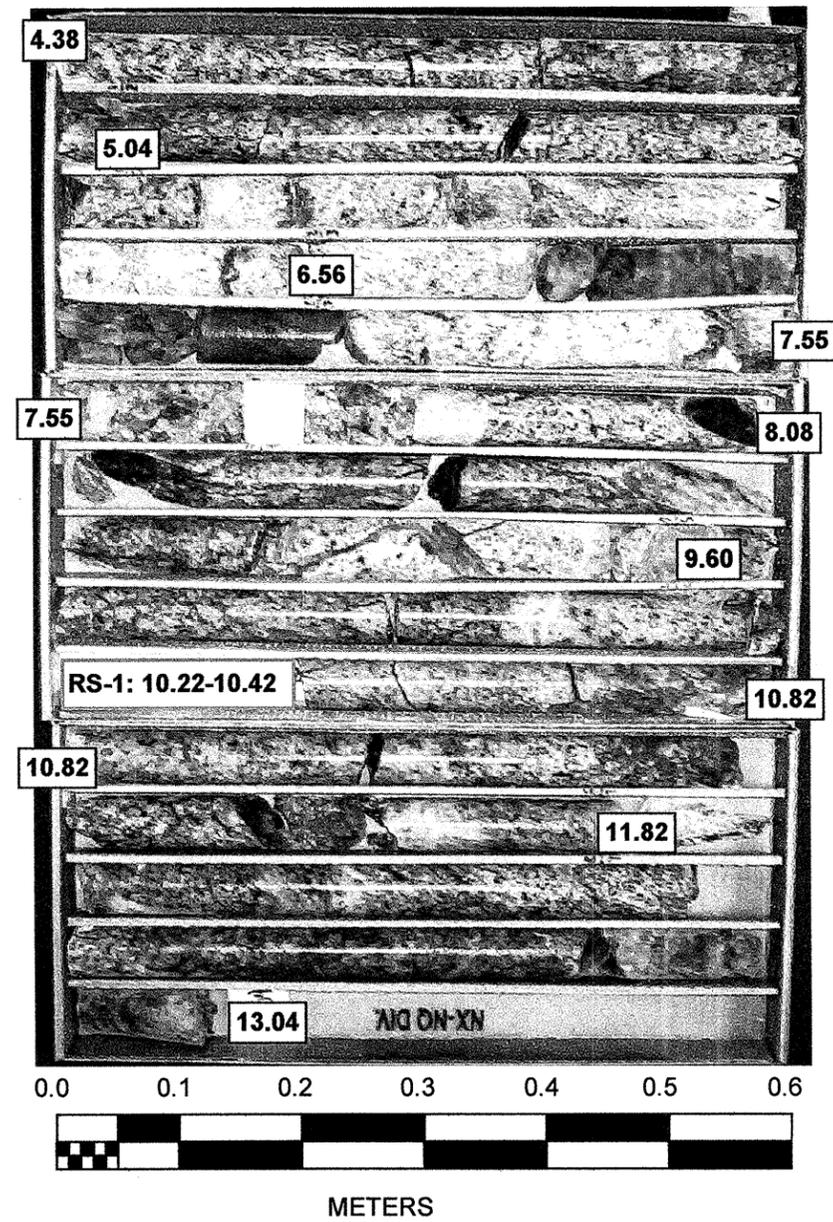
CORE PHOTOGRAPHS  
EB1-A

BOXES 1 & 2: 3.75 - 6.67 METERS

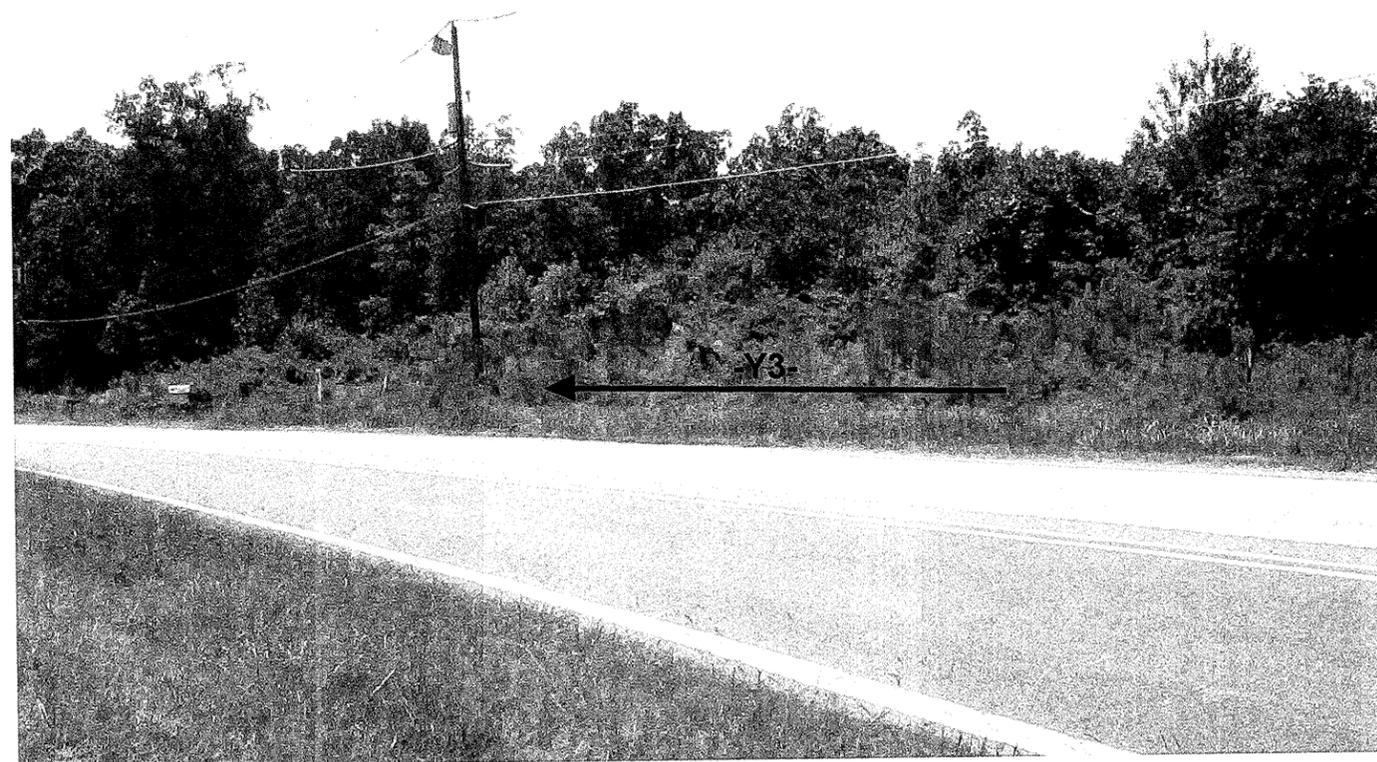


CORE PHOTOGRAPHS  
B1-B

BOX 1, 2 & 3: 4.38 - 13.04 METERS



# ***SITE PHOTOGRAPH***



CONTRACT: ID: R-0609IA

# STATE OF NORTH CAROLINA

DEPARTMENT OF TRANSPORTATION

DIVISION OF HIGHWAYS

GEOTECHNICAL UNIT

## STRUCTURE SUBSURFACE INVESTIGATION

### CONTENTS:

SHEET	DESCRIPTION
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3	STRUCTURE INVENTORY REPORT
4	TEST SITE PLAN
5-6	PROFILES
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16	CORE PHOTOGRAPHS
17	SITE PHOTOGRAPH

	STATE PROJECT REFERENCE NO.	SHEET NO.	TOTAL SHEETS
	N.C. 34345.1.1 (R-0609IA)	1	17
STATE PROJ. NO.	F.A. PROJ. NO.	DESCRIPTION	
		P.E. CONST.	

### CAUTION NOTICE

THE SUBSURFACE INFORMATION AND THE SUBSURFACE INVESTIGATION ON WHICH IT IS BASED WAS MADE FOR THE PURPOSE OF STUDY, PLANNING AND DESIGN, AND NOT FOR CONSTRUCTION OR PAY PURPOSES. THE VARIOUS FIELD BORING LOGS, ROCK CORES, AND SOIL TEST DATA AVAILABLE MAY BE REVIEWED OR INSPECTED IN RALEIGH BY CONTACTING THE N.C. DEPARTMENT OF TRANSPORTATION, GEOTECHNICAL UNIT @ (919) 250-4088. NEITHER THE SUBSURFACE PLANS AND REPORTS, NOR THE FIELD BORING LOGS, ROCK CORES, OR SOIL TEST DATA IS PART OF THE CONTRACT.

GENERAL SOIL AND ROCK STRATA DESCRIPTIONS AND INDICATED BOUNDARIES ARE BASED ON A GEOTECHNICAL INTERPRETATION OF ALL AVAILABLE SUBSURFACE DATA AND MAY NOT NECESSARILY REFLECT THE ACTUAL SUBSURFACE CONDITIONS BETWEEN BORINGS OR BETWEEN SAMPLED STRATA WITHIN THE BOREHOLE. THE LABORATORY SAMPLE DATA AND THE IN SITU (IN-PLACE) TEST DATA CAN BE RELIED ON ONLY TO THE DEGREE OF RELIABILITY INHERENT IN THE STANDARD TEST METHOD. THE OBSERVED WATER LEVELS OR SOIL MOISTURE CONDITIONS INDICATED IN THE SUBSURFACE INVESTIGATIONS ARE AS RECORDED AT THE TIME OF THE INVESTIGATION. THESE WATER LEVELS OR SOIL MOISTURE CONDITIONS MAY VARY CONSIDERABLY WITH TIME ACCORDING TO CLIMATIC CONDITIONS INCLUDING TEMPERATURES, PRECIPITATION AND WIND, AS WELL AS OTHER NON-CLIMATIC FACTORS.

THE BIDDER OR CONTRACTOR IS CAUTIONED THAT DETAILS SHOWN ON THE SUBSURFACE PLANS ARE PRELIMINARY ONLY AND IN MANY CASES THE FINAL DESIGN DETAILS ARE DIFFERENT. FOR BIDDING AND CONSTRUCTION PURPOSES, REFER TO THE CONSTRUCTION PLANS AND DOCUMENTS FOR FINAL DESIGN INFORMATION ON THIS PROJECT. THE DEPARTMENT DOES NOT WARRANT OR GUARANTEE THE SUFFICIENCY OR ACCURACY OF THE INVESTIGATION MADE, NOR THE INTERPRETATIONS MADE OR OPINION OF THE DEPARTMENT AS TO THE TYPE OF MATERIALS AND CONDITIONS TO BE ENCOUNTERED. THE BIDDER OR CONTRACTOR IS CAUTIONED TO MAKE SUCH INDEPENDENT SUBSURFACE INVESTIGATIONS AS HE DEEMS NECESSARY TO SATISFY HIMSELF AS TO CONDITIONS TO BE ENCOUNTERED ON THIS PROJECT. THE CONTRACTOR SHALL HAVE NO CLAIM FOR ADDITIONAL COMPENSATION OR FOR AN EXTENSION OF TIME FOR ANY REASON RESULTING FROM THE ACTUAL CONDITIONS ENCOUNTERED AT THE SITE DIFFERING FROM THOSE INDICATED IN THE SUBSURFACE INFORMATION.

STATE PROJECT 34345.1.1 I.D. NO. R-0609IA

F.A. PROJECT MAF-F-119-1 (1)

COUNTY GUILFORD

PROJECT DESCRIPTION US 311 (HIGH POINT  
EAST BELTWAY) FROM US 29-70  
TO I-85 NORTH OF ARCHDALE

SITE DESCRIPTION STRUCTURE NO. 4  
ON -SBCD- AT STA. 21+25.6 AND  
STRUCTURE NO. 5 ON -Y6SB- (SB I-85)  
AT STA. 27+12.1 OVER US 311

INVESTIGATED BY C. D. CZAJKA PERSONNEL CDC

CHECKED BY N. T. ROBERSON DCB

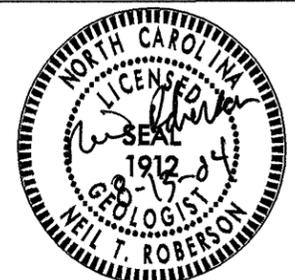
SUBMITTED BY D. N. ARGENBRIGHT RMB

DATE AUGUST, 2004 REM

DRAWN BY: WDF

NOTE - THE INFORMATION CONTAINED HEREIN IS NOT IMPLIED OR GUARANTEED BY THE N.C. DEPARTMENT OF TRANSPORTATION AS BEING ACCURATE NOR IT IS CONSIDERED TO BE PART OF THE PLANS, SPECIFICATIONS, OR CONTRACT FOR THE PROJECT.

NOTE - BY HAVING REQUESTED THIS INFORMATION THE CONTRACTOR SPECIFICALLY WAIVES ANY CLAIMS FOR INCREASED COMPENSATION OR EXTENSION OF TIME BASED ON DIFFERENCES BETWEEN THE CONDITIONS INDICATED HEREIN AND THE ACTUAL CONDITIONS AT THE PROJECT SITE.



NORTH CAROLINA DEPARTMENT OF TRANSPORTATION  
DIVISION OF HIGHWAYS  
GEOTECHNICAL UNIT

ID	STATE PROJECT NO.	SHEET NO.	TOTAL SHEETS
R-0609IA	34345.1J	2	17



SUBSURFACE INVESTIGATION

SOIL AND ROCK LEGEND, TERMS, SYMBOLS, AND ABBREVIATIONS

SOIL DESCRIPTION		GRADATION		ROCK DESCRIPTION		TERMS AND DEFINITIONS																																																																																																																																																																																																																																																																														
<p>SOIL IS CONSIDERED TO BE THE UNCONSOLIDATED, SEMI-CONSOLIDATED OR WEATHERED EARTH MATERIALS WHICH CAN BE PENETRATED WITH A CONTINUOUS FLIGHT POWER AUGER, AND WHICH YIELDS LESS THAN 100 BLOWS PER 30 cm ACCORDING TO STANDARD PENETRATION TEST (AASHTO T206, ASTM D-1586). SOIL CLASSIFICATION IS BASED ON THE AASHTO SYSTEM AND BASIC DESCRIPTIONS GENERALLY SHALL INCLUDE: CONSISTENCY, COLOR, TEXTURE, MOISTURE, AASHTO CLASSIFICATION, AND OTHER PERTINENT FACTORS SUCH AS MINERALOGICAL COMPOSITION, ANGULARITY, STRUCTURE, PLASTICITY, ETC. EXAMPLE:</p> <p>VERY STIFF, GRAY SILTY CLAY, MOST WITH INTERBEDDED FINE SAND LAYERS, HEAVY PLASTIC, A-7-6</p>		<p>WELL GRADED - INDICATES A GOOD REPRESENTATION OF PARTICLE SIZES FROM FINE TO COARSE UNIFORM - INDICATES THAT SOIL PARTICLES ARE ALL APPROXIMATELY THE SAME SIZE (ALSO POORLY GRADED). GAP-GRADED - INDICATES A MIXTURE OF UNIFORM PARTICLES OF TWO OR MORE SIZES.</p> <p>ANGULARITY OF GRAINS</p> <p>THE ANGULARITY OR ROUNDNESS OF SOIL GRAINS ARE DESIGNATED BY THE TERMS: ANGULAR, SUBANGULAR, SUBROUNDED, OR ROUNDED.</p>		<p>HARD ROCK IS NON-COASTAL PLAIN MATERIAL THAT WHEN TESTED, WOULD YIELD SPT REFUSAL. AN INFERRED ROCK LINE INDICATES THE LEVEL AT WHICH NON-COASTAL PLAIN MATERIAL WOULD YIELD SPT REFUSAL. SPT REFUSAL IS PENETRATION BY A SPLIT SPOON SAMPLER EQUAL TO OR LESS THAN 2.5 cm PER 50 BLOWS. IN NON-COASTAL PLAIN MATERIAL, THE TRANSITION BETWEEN SOIL AND ROCK IS OFTEN REPRESENTED BY A ZONE OF WEATHERED ROCK. ROCK MATERIALS ARE TYPICALLY DIVIDED AS FOLLOWS:</p> <p>WEATHERED ROCK (WR)</p> <p>NON-COASTAL PLAIN MATERIAL THAT YIELDS SPT N VALUES &gt; 100 BLOWS PER 30 cm.</p> <p>CRYSTALLINE ROCK (CR)</p> <p>FINE TO COARSE GRAIN IGNEOUS AND METAMORPHIC ROCK THAT WOULD YIELD SPT REFUSAL IF TESTED. ROCK TYPE INCLUDES GRANITE, GNEISS, GABBRO, SCHIST, ETC.</p> <p>NON-CRYSTALLINE ROCK (NCR)</p> <p>FINE TO COARSE GRAIN METAMORPHIC AND NON-COASTAL PLAIN SEDIMENTARY ROCK THAT WOULD YIELD SPT REFUSAL IF TESTED. ROCK TYPE INCLUDES PHYLLITE, SLATE, SANDSTONE, ETC.</p> <p>COASTAL PLAIN SEDIMENTARY ROCK (CP)</p> <p>COASTAL PLAIN SEDIMENTS CEMENTED INTO ROCK, BUT MAY NOT YIELD SPT REFUSAL. ROCK TYPE INCLUDES LIMESTONE, SANDSTONE, CEMENTED SHELL BEDS, ETC.</p>		<p>ALLUVIUM (ALLUV.) - SOILS WHICH HAVE BEEN TRANSPORTED BY WATER. AQUIFER - A WATER BEARING FORMATION OR STRATA. ARENACEOUS - APPLIED TO ROCKS THAT HAVE BEEN DERIVED FROM SAND OR THAT CONTAIN SAND. ARGILLACEOUS - APPLIED TO ALL ROCKS OR SUBSTANCES COMPOSED OF CLAY MINERALS, OR HAVING A NOTABLE PROPORTION OF CLAY IN THEIR COMPOSITION, AS SHALE, SLATE, ETC. ARTESIAN - GROUND WATER THAT IS UNDER SUFFICIENT PRESSURE TO RISE ABOVE THE LEVEL AT WHICH IT IS ENCOUNTERED, BUT WHICH DOES NOT NECESSARILY RISE TO OR ABOVE THE GROUND SURFACE. CALCARIOUS (CALC.) - SOILS WHICH CONTAIN APPRECIABLE AMOUNTS OF CALCIUM CARBONATE. COLLUVIUM - ROCK FRAGMENTS MIXED WITH SOIL DEPOSITED BY GRAVITY ON SLOPE OR AT BOTTOM OF SLOPE. CORE RECOVERY (REC.) - TOTAL LENGTH OF ALL MATERIAL RECOVERED IN THE CORE BARREL DIVIDED BY TOTAL LENGTH OF CORE RUN AND EXPRESSED AS A PERCENTAGE. DIKE - A TABULAR BODY OF IGNEOUS ROCK THAT CUTS ACROSS THE STRUCTURE OF ADJACENT ROCKS OR CUTS MASSIVE ROCK. DIP - THE ANGLE AT WHICH A STRATUM OR ANY PLANAR FEATURE IS INCLINED FROM THE HORIZONTAL. DIP DIRECTION (DIP AZIMUTH) - THE DIRECTION OR BEARING OF THE HORIZONTAL TRACE OF THE LINE OF DIP, MEASURED CLOCKWISE FROM NORTH. FAULT - A FRACTURE OR FRACTURE ZONE ALONG WHICH THERE HAS BEEN DISPLACEMENT OF THE SIDES RELATIVE TO ONE ANOTHER PARALLEL TO THE FRACTURE. FISSILE - A PROPERTY OF SPLITTING ALONG CLOSELY SPACED PARALLEL PLANES. FLOAT - ROCK FRAGMENTS ON SURFACE NEAR THEIR ORIGINAL POSITION AND DISLOADED FROM PARENT MATERIAL. FLOOD PLAIN (F.P.) - LAND BORDERING A STREAM, BUILT OF SEDIMENTS DEPOSITED BY THE STREAM. FORMATION (FM.) - A MAPPABLE GEOLOGIC UNIT THAT CAN BE RECOGNIZED AND TRACED IN THE FIELD. JOINT - FRACTURE IN ROCK ALONG WHICH NO APPRECIABLE MOVEMENT HAS OCCURRED. LEDGE - A SHELF-LIKE RIDGE OR PROJECTION OF ROCK WHOSE THICKNESS IS SMALL COMPARED TO ITS LATERAL EXTENT. LENS - A BODY OF SOIL OR ROCK THAT THINS OUT IN ONE OR MORE DIRECTIONS. MOTTLED (MOT.) - IRREGULARLY MARKED WITH SPOTS OF DIFFERENT COLORS. MOTTLING IN SOILS USUALLY INDICATES POOR AERATION AND LACK OF GOOD DRAINAGE. PERCHED WATER - WATER MAINTAINED ABOVE THE NORMAL GROUND WATER LEVEL BY THE PRESENCE OF AN INTERVENING IMPERVIOUS STRATUM. RESIDUAL SOIL - SOIL FORMED IN PLACE BY THE WEATHERING OF ROCK. ROCK QUALITY DESIGNATION (R.Q.D.) - A MEASURE OF ROCK QUALITY DESCRIBED BY: TOTAL LENGTH OF ROCK SEGMENTS EQUAL TO OR GREATER THAN 10 CENTIMETERS DIVIDED BY THE TOTAL LENGTH OF CORE RUN AND EXPRESSED AS A PERCENTAGE. SAPROLITE (SAP.) - RESIDUAL SOIL WHICH RETAINS THE RELIC STRUCTURE OR FABRIC OF THE PARENT ROCK. SILL - AN INTRUSIVE BODY OF IGNEOUS ROCK OF APPROXIMATELY UNIFORM THICKNESS AND RELATIVELY THIN COMPARED WITH ITS LATERAL EXTENT, WHICH HAS BEEN EMPLACED PARALLEL TO THE BEDDING OR SCHISTOSITY OF THE INTRUDED ROCKS. SLICKENSIDE - POLISHED AND STRIATED SURFACE THAT RESULTS FROM FRICTION ALONG A FAULT OR SLIP PLANE. STANDARD PENETRATION TEST (PENETRATION RESISTANCE) (SPT) - NUMBER OF BLOWS (N) OF A 63.5 kg HAMMER FALLING 0.76 METERS REQUIRED TO PRODUCE A PENETRATION OF 30 cm INTO SOIL WITH A 5 cm OUTSIDE DIAMETER SPLIT SPOON SAMPLER. SPT REFUSAL IS LESS THAN 2.5 cm PENETRATION WITH 50 BLOWS. STRATA CORE RECOVERY (SREC.) - TOTAL LENGTH OF STRATA MATERIAL RECOVERED DIVIDED BY TOTAL LENGTH OF STRATUM AND EXPRESSED AS A PERCENTAGE. STRATA ROCK QUALITY DESIGNATION (S.R.Q.D.) - A MEASURE OF ROCK QUALITY DESCRIBED BY: TOTAL LENGTH OF ROCK SEGMENTS WITHIN A STRATUM EQUAL TO OR GREATER THAN 10 CENTIMETERS DIVIDED BY THE TOTAL LENGTH OF STRATA AND EXPRESSED AS A PERCENTAGE. TOPSOIL (T.S.) - SURFACE SOILS USUALLY CONTAINING ORGANIC MATTER.</p>																																																																																																																																																																																																																																																																														
<p>SOIL LEGEND AND AASHTO CLASSIFICATION</p> <table border="1"> <tr> <th rowspan="2">GENERAL CLASS.</th> <th colspan="7">GRANULAR MATERIALS (&lt; 75% PASSING #200)</th> <th colspan="7">SILT-CLAY MATERIALS (&gt; 75% PASSING #200)</th> <th colspan="3">ORGANIC MATERIALS</th> </tr> <tr> <th>A-1</th> <th>A-3</th> <th colspan="2">A-2</th> <th>A-4</th> <th>A-5</th> <th>A-6</th> <th>A-7</th> <th>A-1, A-2</th> <th>A-4, A-5</th> <th>A-6, A-7</th> <th colspan="2"></th> <th colspan="2"></th> <th colspan="2"></th> </tr> <tr> <td>GROUP CLASS.</td> <td>A-1-a</td> <td>A-1-b</td> <td>A-2-4</td> <td>A-2-5</td> <td>A-2-6</td> <td>A-2-7</td> <td>A-4</td> <td>A-5</td> <td>A-6</td> <td>A-7</td> <td>A-1, A-2</td> <td>A-4, A-5</td> <td>A-6, A-7</td> <td colspan="2"></td> <td colspan="2"></td> </tr> <tr> <td>SYMBOL</td> <td></td> <td colspan="2"></td> <td colspan="2"></td> </tr> <tr> <td>% PASSING</td> <td>50 MX</td> <td>30 MX</td> <td>35 MX</td> <td colspan="2"></td> <td colspan="2"></td> </tr> <tr> <td>LIQUID LIMIT</td> <td>6 MX</td> <td>N.P.</td> <td>40 MX</td> <td>41 MN</td> <td>40 MX</td> <td colspan="2"></td> <td colspan="2"></td> </tr> <tr> <td>PLASTIC INDEX</td> <td>6 MX</td> <td>N.P.</td> <td>40 MX</td> <td>10 MN</td> <td>11 MN</td> <td>11 MN</td> <td>10 MX</td> <td>10 MN</td> <td>11 MN</td> <td>11 MN</td> <td>10 MX</td> <td>10 MN</td> <td>11 MN</td> <td colspan="2"></td> <td colspan="2"></td> </tr> <tr> <td>GROUP INDEX</td> <td>0</td> <td>0</td> <td>0</td> <td>4 MX</td> <td>8 MX</td> <td>12 MX</td> <td>16 MX</td> <td>10 MX</td> <td>12 MX</td> <td>16 MX</td> <td>10 MX</td> <td>12 MX</td> <td>16 MX</td> <td colspan="2"></td> <td colspan="2"></td> </tr> <tr> <td>USUAL TYPES OF MAJOR MATERIALS</td> <td>STONE FRAGS. 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ARE USED IN DESCRIPTIONS WHENEVER THEY ARE CONSIDERED OF SIGNIFICANCE.</p> <p>COMPRESSIBILITY</p> <p>SLIGHTLY COMPRESSIBLE LIQUID LIMIT LESS THAN 30 MODERATELY COMPRESSIBLE LIQUID LIMIT 31-50 HIGHLY COMPRESSIBLE LIQUID LIMIT GREATER THAN 50</p> <p>PERCENTAGE OF MATERIAL</p> <table border="1"> <tr> <th>ORGANIC MATERIAL</th> <th>GRANULAR SOILS</th> <th>SILT-CLAY SOILS</th> <th>OTHER MATERIAL</th> </tr> <tr> <td>TRACE OF ORGANIC MATTER</td> <td>2 - 3%</td> <td>3 - 5%</td> <td>TRACE 1 - 10%</td> </tr> <tr> <td>LITTLE ORGANIC MATTER</td> <td>3 - 5%</td> <td>5 - 12%</td> <td>LITTLE 10 - 20%</td> </tr> <tr> <td>MODERATELY ORGANIC</td> <td>5 - 10%</td> <td>12 - 20%</td> <td>SOME 20 - 35%</td> </tr> <tr> <td>HIGHLY ORGANIC</td> <td>&gt;10%</td> <td>&gt;20%</td> <td>HIGHLY 35% AND ABOVE</td> </tr> </table> <p>GROUND WATER</p> <p>▽ WATER LEVEL IN BORE HOLE IMMEDIATELY AFTER DRILLING. ▽ STATIC WATER LEVEL AFTER 24 HOURS. ▽ PERCHED WATER, SATURATED ZONE OR WATER BEARING STRATA ○ SPRING OR SEEPAGE</p> <p>MISCELLANEOUS SYMBOLS</p> <table border="1"> <tr> <td></td> <td>ROADWAY EMBANKMENT WITH SOIL DESCRIPTION</td> <td></td> <td>SPT TEST BORING</td> <td></td> <td>SAMPLE DESIGNATIONS</td> </tr> <tr> <td></td> <td>SOIL SYMBOL</td> <td></td> <td>AUGER BORING</td> <td></td> <td>S- BULK SAMPLE</td> </tr> <tr> <td></td> <td>ARTIFICIAL FILL OTHER THAN ROADWAY EMBANKMENTS</td> <td></td> <td>CORE BORING</td> <td></td> <td>SS- SPLIT SPOON SAMPLE</td> </tr> <tr> <td></td> <td>INFERRED SOIL BOUNDARIES</td> <td></td> <td>MONITORING WELL</td> <td></td> <td>ST- SHELBY TUBE SAMPLE</td> </tr> <tr> <td></td> <td>INFERRED ROCK LINE</td> <td></td> <td>PIEZOMETER INSTALLATION</td> <td></td> <td>RS- ROCK SAMPLE</td> </tr> <tr> <td></td> <td>ALLUVIAL SOIL BOUNDARY</td> <td></td> <td>SLOPE INDICATOR INSTALLATION</td> <td></td> <td>RT- RECOMPACTED TRIAXIAL SAMPLE</td> </tr> <tr> <td></td> <td>DIP/DIP DIRECTION OF ROCK STRUCTURES</td> <td></td> <td>SPT N-VALUE</td> <td></td> <td>CBR - CBR SAMPLE</td> </tr> <tr> <td></td> <td>SOUNDING ROD</td> <td></td> <td>SPT REFUSAL</td> <td></td> <td></td> </tr> </table> <p>ABBREVIATIONS</p> <table border="1"> <tr> <td>AR - AUGER REFUSAL</td> <td>PMT - PRESSUREMETER TEST</td> </tr> <tr> <td>BT - BORING TERMINATED</td> <td>SD - SAND, SANDY</td> </tr> <tr> <td>CL - CLAY</td> <td>SL - SILT, SILTY</td> </tr> <tr> <td>CPT - CONE PENETRATION TEST</td> <td>SLI - SLIGHTLY</td> </tr> <tr> <td>CSE - COARSE</td> <td>TCR - TRICONE REFUSAL</td> </tr> <tr> <td>DMT - DILATOMETER TEST</td> <td>U - UNIT WEIGHT</td> </tr> <tr> <td>DPT - DYNAMIC PENETRATION TEST</td> <td>U<sub>d</sub> - DRY UNIT WEIGHT</td> </tr> <tr> <td>e - VOID RATIO</td> <td>w - MOISTURE CONTENT</td> </tr> <tr> <td>F - FINE</td> <td>v - VERY</td> </tr> <tr> <td>FOSS. - FOSSILIFEROUS</td> <td>VST - VANE SHEAR TEST</td> </tr> <tr> <td>FRAC. - FRACTURED</td> <td></td> </tr> <tr> <td>FRAGS. - FRAGMENTS</td> <td></td> </tr> <tr> <td>MED. - MEDIUM</td> <td></td> </tr> </table>		ORGANIC MATERIAL	GRANULAR SOILS	SILT-CLAY SOILS	OTHER MATERIAL	TRACE OF ORGANIC MATTER	2 - 3%	3 - 5%	TRACE 1 - 10%	LITTLE ORGANIC MATTER	3 - 5%	5 - 12%	LITTLE 10 - 20%	MODERATELY ORGANIC	5 - 10%	12 - 20%	SOME 20 - 35%	HIGHLY ORGANIC	>10%	>20%	HIGHLY 35% AND ABOVE		ROADWAY EMBANKMENT WITH SOIL DESCRIPTION		SPT TEST BORING		SAMPLE DESIGNATIONS		SOIL SYMBOL		AUGER BORING		S- BULK SAMPLE		ARTIFICIAL FILL OTHER THAN ROADWAY EMBANKMENTS		CORE BORING		SS- SPLIT SPOON SAMPLE		INFERRED SOIL BOUNDARIES		MONITORING WELL		ST- SHELBY TUBE SAMPLE		INFERRED ROCK LINE		PIEZOMETER INSTALLATION		RS- ROCK SAMPLE		ALLUVIAL SOIL BOUNDARY		SLOPE INDICATOR INSTALLATION		RT- RECOMPACTED TRIAXIAL SAMPLE		DIP/DIP DIRECTION OF ROCK STRUCTURES		SPT N-VALUE		CBR - CBR SAMPLE		SOUNDING ROD		SPT REFUSAL			AR - AUGER REFUSAL	PMT - PRESSUREMETER TEST	BT - BORING TERMINATED	SD - SAND, SANDY	CL - CLAY	SL - SILT, SILTY	CPT - CONE PENETRATION TEST	SLI - SLIGHTLY	CSE - COARSE	TCR - TRICONE REFUSAL	DMT - DILATOMETER TEST	U - UNIT WEIGHT	DPT - DYNAMIC PENETRATION TEST	U <sub>d</sub> - DRY UNIT WEIGHT	e - VOID RATIO	w - MOISTURE CONTENT	F - FINE	v - VERY	FOSS. - FOSSILIFEROUS	VST - VANE SHEAR TEST	FRAC. - FRACTURED		FRAGS. - FRAGMENTS		MED. - MEDIUM		<p>ROCK HARDNESS</p> <p>VERY HARD CANNOT BE SCRATCHED BY KNIFE OR SHARP PICK. BREAKING OF HAND SPECIMENS REQUIRES SEVERAL HARD BLOWS OF THE GEOLOGIST'S PICK.</p> <p>HARD CAN BE SCRATCHED BY KNIFE OR PICK ONLY WITH DIFFICULTY. HARD HAMMER BLOWS REQUIRED TO DETACH HAND SPECIMEN.</p> <p>MODERATELY HARD CAN BE SCRATCHED BY KNIFE OR PICK. GOUGES OR GROOVES TO 6 mm DEEP CAN BE EXCAVATED BY HARD BLOW OF A GEOLOGIST'S PICK. HAND SPECIMENS CAN BE DETACHED BY MODERATE BLOWS.</p> <p>MEDIUM HARD CAN BE GROOVED OR GOUGED 1 mm DEEP BY FIRM PRESSURE OF KNIFE OR PICK POINT. CAN BE EXCAVATED IN SMALL CHIPS TO PIECES 25 mm MAXIMUM SIZE BY HARD BLOWS OF THE POINT OF A GEOLOGIST'S PICK.</p> <p>SOFT CAN BE GROOVED OR GOUGED READILY BY KNIFE OR PICK. CAN BE EXCAVATED IN FRAGMENTS FROM CHIPS TO SEVERAL CENTIMETERS IN SIZE BY MODERATE BLOWS OF A PICK POINT. SMALL, THIN PIECES CAN BE BROKEN BY FINGER PRESSURE.</p> <p>VERY SOFT CAN BE CARVED WITH KNIFE. CAN BE EXCAVATED READILY WITH POINT OF PICK. PIECES 25 mm OR MORE IN THICKNESS CAN BE BROKEN BY FINGER PRESSURE. 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STATE OF NORTH CAROLINA  
DEPARTMENT OF TRANSPORTATION

Michael F. Easley  
GOVERNOR

P.O. BOX 25201, RALEIGH, N.C. 27611-5201

Lyndo Tippet  
SECRETARY

August 12, 2004

STATE PROJECT: 34345.1.1 (R-0609IA)  
F.A. PROJECT: MAF-F-119-1 (1)  
COUNTY: Guilford  
DESCRIPTION: US 311 (High Point East Beltway) from US 29-70 to I-85  
North of Archdale  
SUBJECT: Structure No. 4 on -SBCD- at Sta. 21+25.6 and Structure No. 5  
on -Y6SB- (SB I-85) at Sta. 27+12.1 over US 311

**Project Description**

Two bridges with two spans each, 83.6-meters in length with a 90° skew, are proposed on -Y6SB- (SB I-85) and -SBCD- over proposed US 311. The project is located in Guilford County about two miles northeast of Archdale.

The subsurface investigation was conducted during June of 2004 using an ATV-mounted Mobile B-57 drill machine. One Standard Penetration Test boring was performed at each of the six proposed bent locations. All borings were advanced into crystalline or weathered rock. Borings B1-B and EB2-A were cored using NQWL rock coring equipment. Representative soil and rock samples were obtained for visual classification in the field and selected samples were submitted to the Materials and Test Unit for laboratory analysis.

**Physiography and Geology**

The project is located in the gently rolling terrain of the Piedmont Physiographic Province. Geologically, the site is located within the Carolina Slate Belt and is underlain by crystalline meta-granite and diabase dikes. The area consists of a mixture of wooded land, sparse homes and agricultural fields.

**Soil Properties**

Soils encountered at the project site include roadway embankment and residual soils.

Roadway embankment soils are present along existing southbound I-85 at structure no. 5. These soils consist primarily of brown, dry, dense, silty sand (A-2-4) and orange-brown and gray, very stiff, moist, sandy and silty clay (A-6, A-7-6). Numerous weathered rock boulders are also present in the roadway embankment. These soils are 3.44 to 4.13 meters thick and are underlain by weathered and crystalline rock.

SHEET 3 OF 17  
34345.1.1 (R-0609IA)

Residual soils are present at along structure no. 4 and range in thickness from 1.03 to 3.60 meters. These soils consist primarily of orange-brown, dry, medium dense to dense, clayey and silty sand (A-2-4, A-2-6). Residual soils are derived from weathering of the underlying weathered and crystalline rock.

**Rock Properties**

Weathered rock is present in each boring except EB1-A and EB2-A of structure no. 5. Weathered rock is derived from the underlying meta-granite and diabase and ranges in thickness from 1.19 to over 9.77 meters. The top of weathered rock was encountered at elevations ranging from 229.74 to 236.08 meters.

Crystalline rock was encountered at each boring location except B1-A of structure no. 5. The top of crystalline rock ranges in elevation from 226.76 to 236.49 meters. Rock core was obtained from B1-B on structure no. 4 and EB2-A on structure no. 5. Crystalline rock consists of black and white, fresh, hard, moderately closely to widely fractured, meta-granite and gray, moderately severely to moderately weathered, soft to hard, very closely to closely fractured, diabase. Core recovery (REC) ranged from 94 to 95% in borings B1-B and EB2-A respectively. Rock Quality Designation (RQD) ranges from 63% in EB2-A to 92% in B1-B. More detailed rock descriptions can be found in the Core Boring Reports.

**Groundwater**

Groundwater was encountered at two of the boring locations along structure no. 4 with elevations ranging from 227.70 to 231.62 meters.

**Notice**

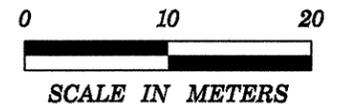
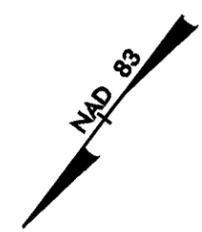
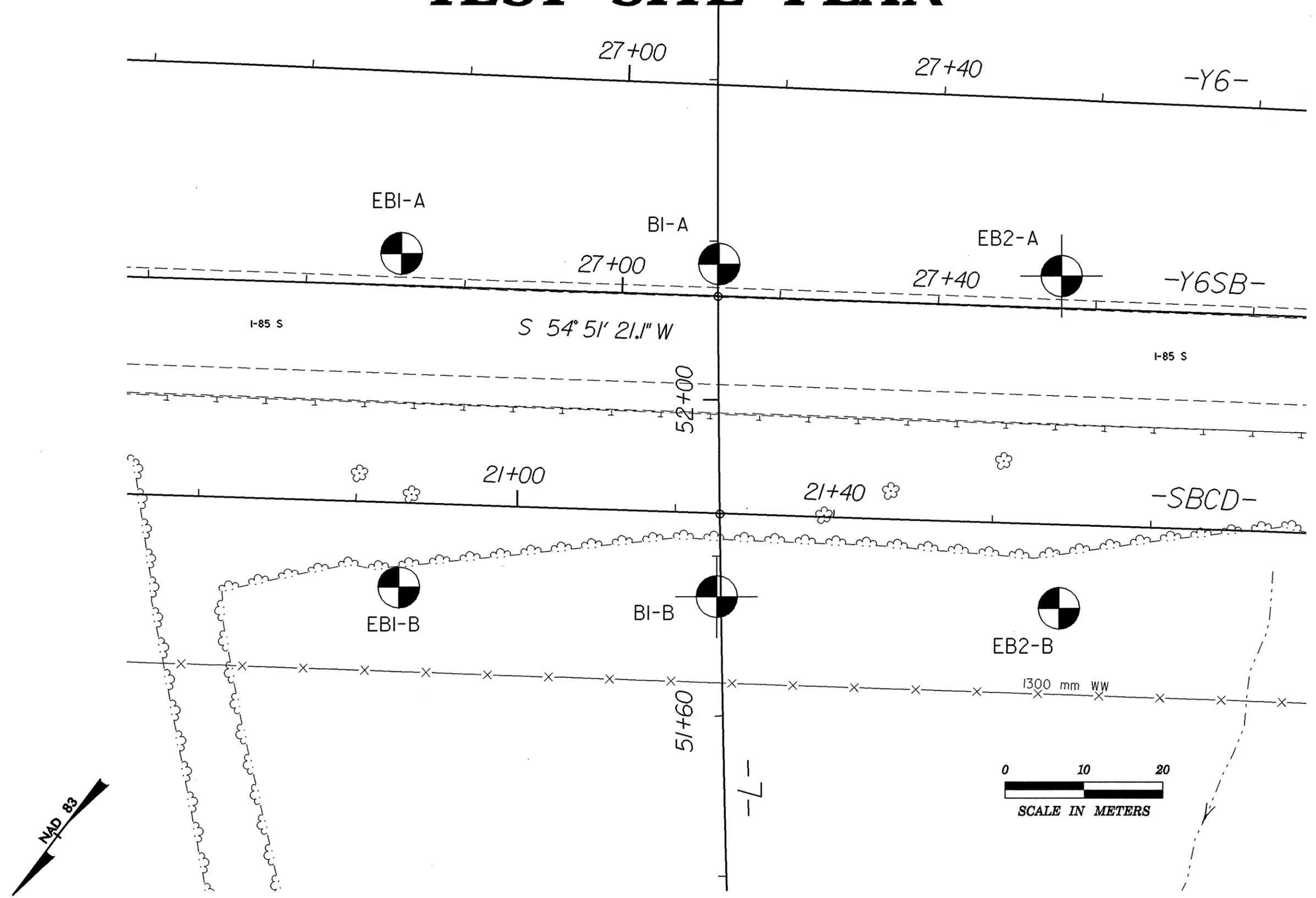
This Geotechnical foundation report is based on the Preliminary General Drawings dated April 6, 2004. If significant changes are made in the design or location of the proposed structure, the subsurface information should be reviewed and modified as necessary.

Respectfully submitted,

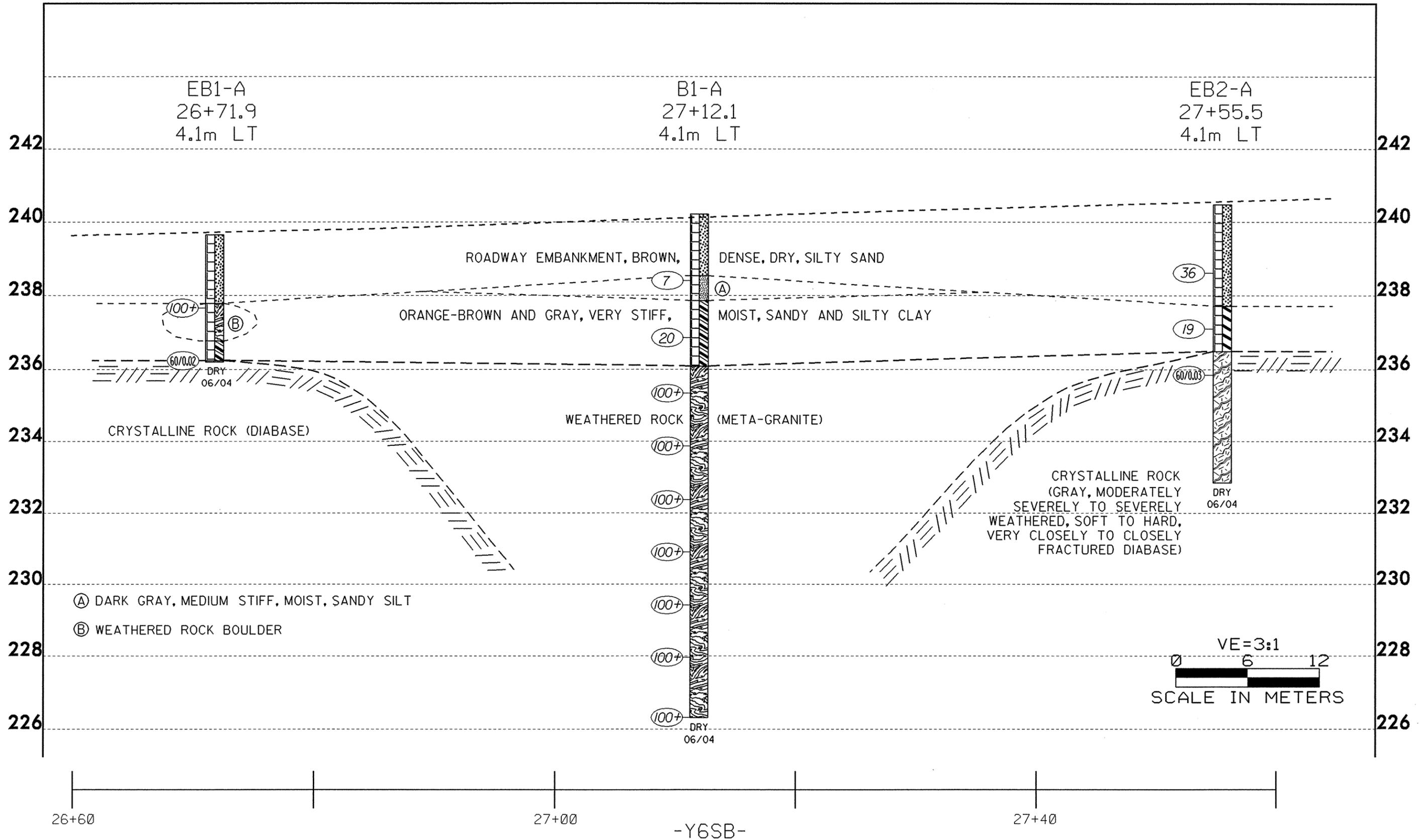
Neil T. Roberson  
Project Geologist

	STATE	STATE PROJECT REFERENCE NO.	SHEET NO.	TOTAL SHEETS
	N.C.	34345.J.I(R-06091A)	4	17
STATE PROJ. NO.	F.A. PROJ. NO.	DESCRIPTION		
		P.E.		
		CONST.		

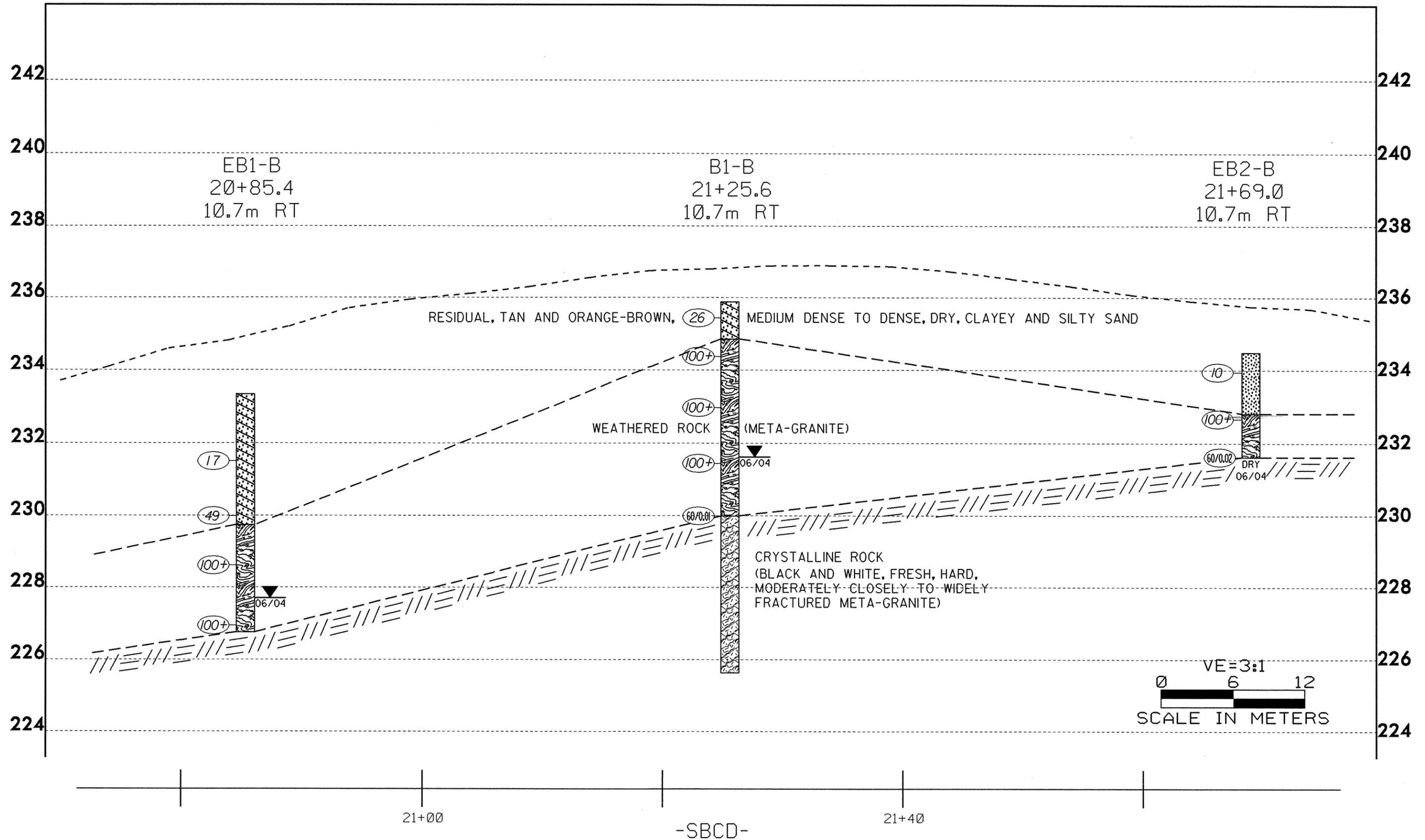
# TEST SITE PLAN



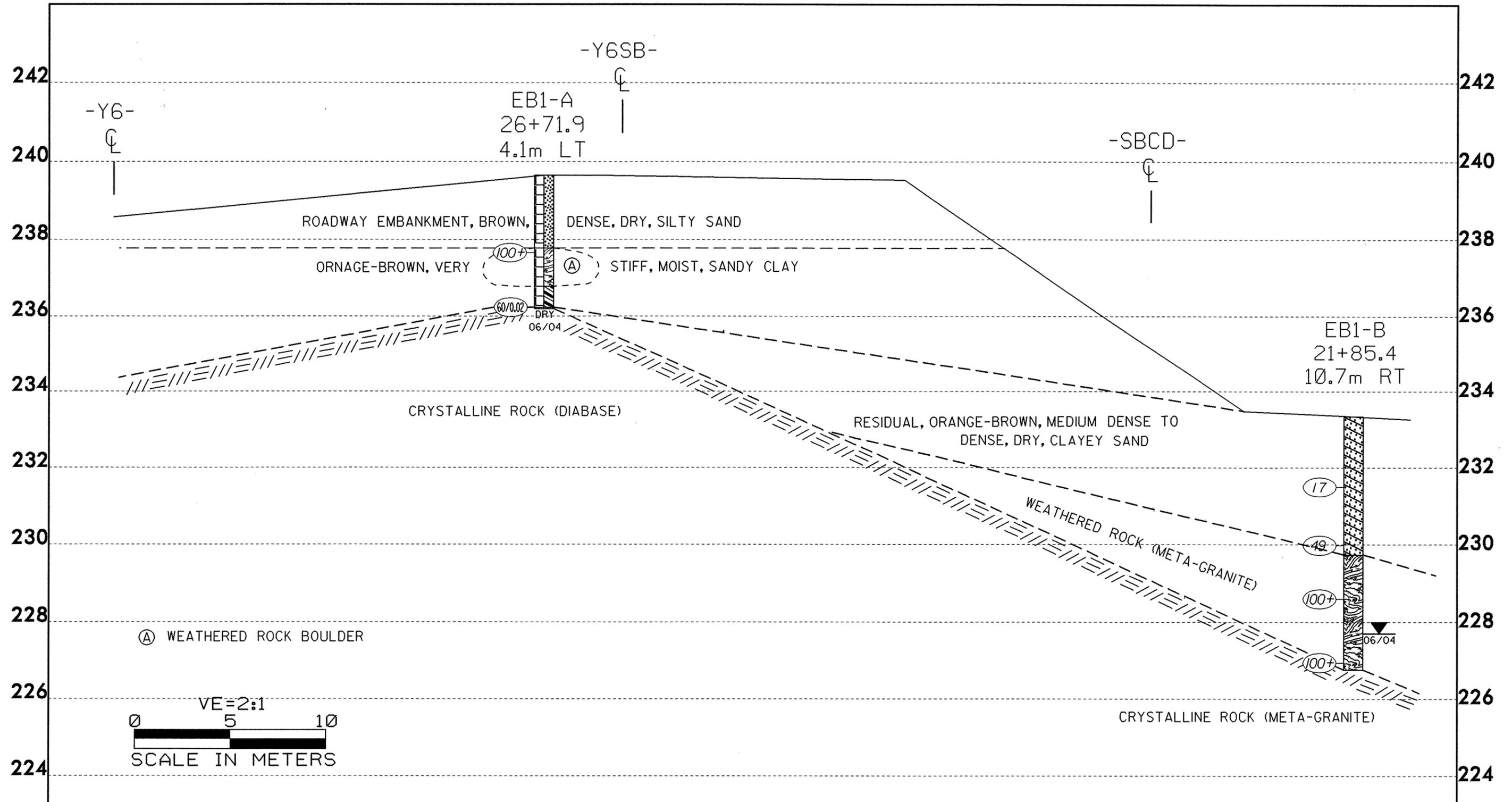
PROFILE THROUGH BORINGS PROJECTED ALONG -Y6SB-



PROFILE THROUGH BORINGS PROJECTED ALONG -SBCD-

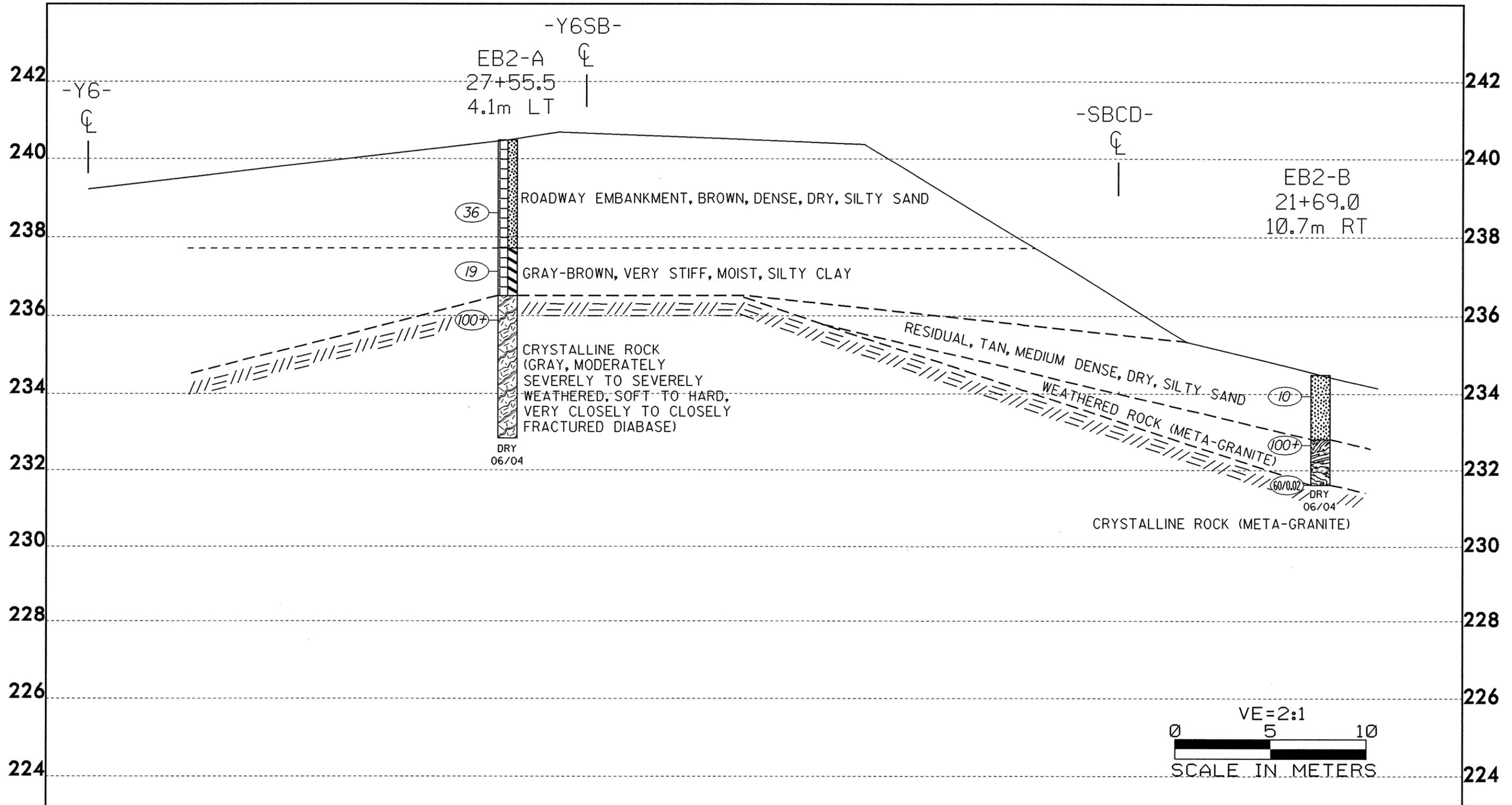


# CROSS SECTION THROUGH END BENT I





# CROSS SECTION THROUGH END BENT 2





NORTH CAROLINA DEPARTMENT OF TRANSPORTATION  
 GEOTECHNICAL UNIT BORING LOG SHEET 11 OF 17

PROJECT NO. 34345.1.1	ID. R-0609IA	COUNTY GUILFORD	GEOLOGIST C.D. CZAJKA
SITE DESCRIPTION STRUCTURE NO. 5 ON -Y6SB- (SB I-85) OVER US 311			GROUND WATER
BORING NO. BI-A	BORING LOCATION 27+12.1	OFFSET 4.1m LT	ALIGNMENT -Y6SB- 0 HR. DRY
COLLAR ELEV. 240.21m	NORTHING 242129.0	EASTING 526139.9	24 HR. DRY
TOTAL DEPTH 13.90m	DRILL MACHINE MOBILE B-57	DRILL METHOD H.S. AUGERS	HAMMER TYPE MANUAL
START DATE 06/16/04	COMPLETION DATE 06/16/04	SURFACE WATER DEPTH N/A	DEPTH TO ROCK N/A

ELEV.	DEPTH	BLOW COUNT			PEN. (m)	BLOWS PER 30cm				SAMPLE NUMBER	LOG	SOIL AND ROCK DESCRIPTION
		15cm	15cm	15cm		0	25	50	75			
240.21												
240.00												
	1.54	5	4	3	0.3						D	ROADWAY EMBANKMENT, BROWN, SILTY SAND
238.00										SS-7	M	DARK GRAY, SANDY SILT
	3.06	5	7	13	0.3						M	ORANGE-BROWN, SANDY CLAY
236.00												
	4.58	15	47	53	0.27				100+ X			
234.00												
	6.10	44	56		0.25				100+ X			
232.00												
	7.62	53	47		0.23				100+ X			
230.00												
	9.14	93	7		0.16				100+ X			
228.00												
	10.66	100			0.13				100+ X			
226.00												
	12.18	100			0.08				100+ X			
224.00												
	13.70	82	18		0.20				100+ X			
222.00												
BORING TERMINATED AT ELEVATION 226.31 METERS IN WEATHERED ROCK (META-GRANITE)												







**PROJ. NO. - 34345.1.1**  
**ID NO. - R-0609IA**  
**COUNTY - GUILFORD**

**B1-A**

<b>SOIL TEST RESULTS</b>															
SAMPLE NO.	OFFSET	STATION	DEPTH INTERVAL	AASHTO CLASS.	L.L.	P.I.	% BY WEIGHT				% PASSING (SIEVES)			% MOISTURE	% ORGANIC
							C.SAND	F.SAND	SILT	CLAY	10	40	200		
SS-7	4.1 LT	27+12.1	1.67-1.99	A-4(0)	22	5	34.0	28.0	15.7	22.3	98	80	41	-	-
SS-8	4.1 LT	27+12.1	3.06-3.51	A-6(2)	26	11	30.2	25.7	19.8	24.3	99	81	48	-	-

**EB2-A**

<b>SOIL TEST RESULTS</b>															
SAMPLE NO.	OFFSET	STATION	DEPTH INTERVAL	AASHTO CLASS.	L.L.	P.I.	% BY WEIGHT				% PASSING (SIEVES)			% MOISTURE	% ORGANIC
							C.SAND	F.SAND	SILT	CLAY	10	40	200		
SS-5	4.1 LT	27+55.5	1.56-2.01	A-2-4(0)	30	10	41.7	23.9	18.1	16.2	84	61	32	-	-
SS-6	4.1 LT	27+55.5	3.08-3.53	A-7-6(26)	56	32	7.7	14.4	29.3	48.6	96	91	78	-	-

**EB2-B**

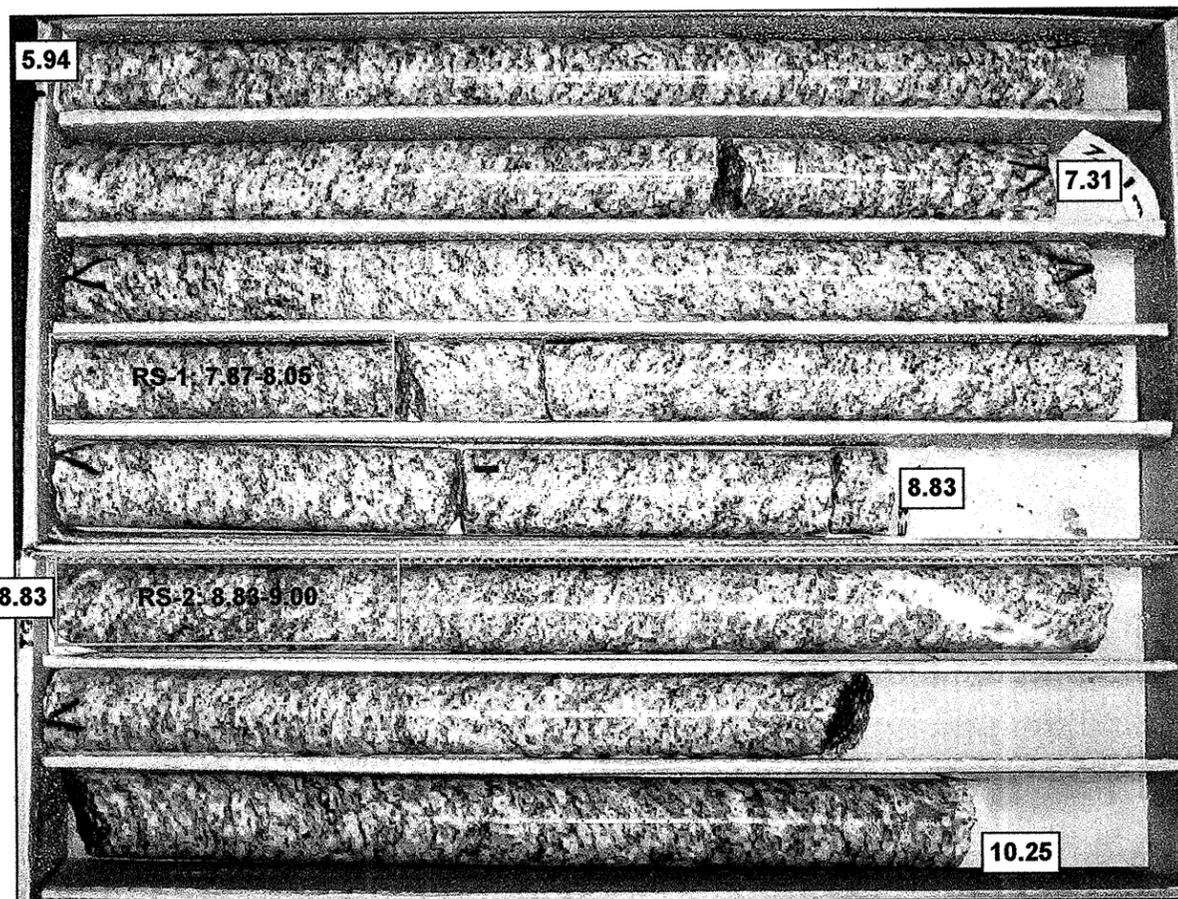
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							C.SAND	F.SAND	SILT	CLAY	10	40	200		
SS-4	10.7 RT	21+69.0	0.15-0.60	A-2-4(0)	21	NP	43.6	27.2	15.1	14.2	94	67	32	-	-

34345.1.1 (R-0509IA)/STRUCTURE NO. 4

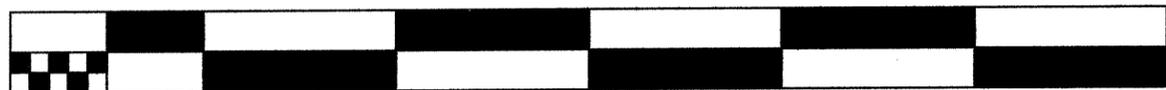
# CORE PHOTOGRAPHS

## B1-B

BOXES 1 & 2: 5.94 - 10.25 METERS



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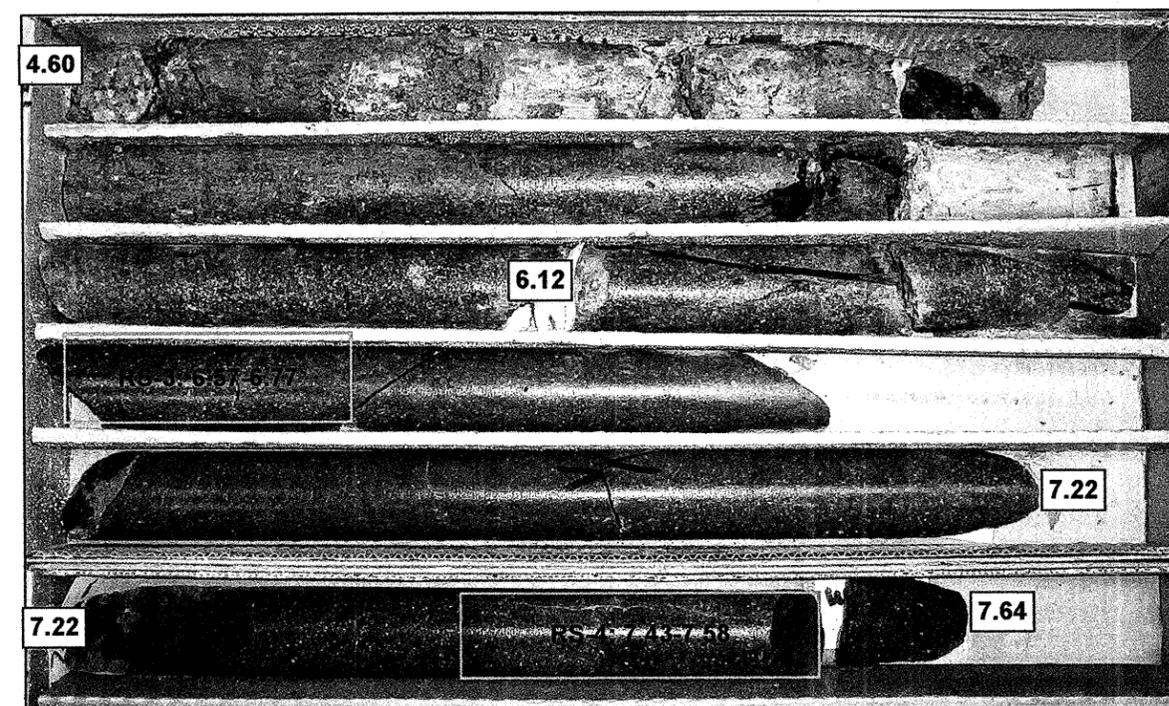
METERS

34345.1.1 (R-0609IA)/STRUCTURE NO. 5

# CORE PHOTOGRAPHS

## EB2-A

BOXES 1 & 2: 4.60 - 7.64 METERS

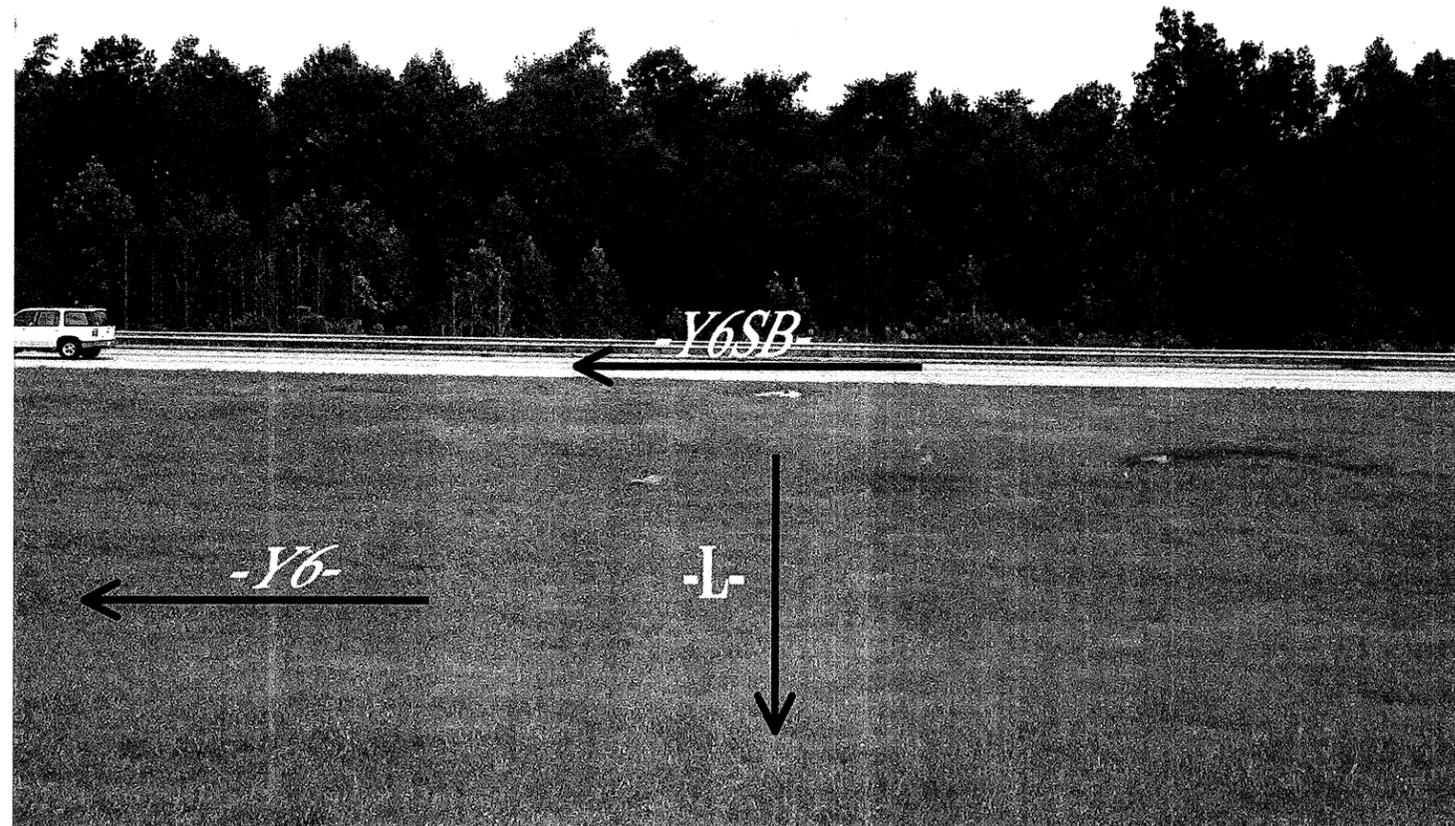


0.0 0.1 0.2 0.3 0.4 0.5 0.6



METERS

# ***SITE PHOTOGRAPH***



CONTRACT: ID: R-06091A

# STATE OF NORTH CAROLINA

DEPARTMENT OF TRANSPORTATION

DIVISION OF HIGHWAYS

GEOTECHNICAL UNIT

## STRUCTURE SUBSURFACE INVESTIGATION

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	STATE	STATE PROJECT REFERENCE NO.	SHEET NO.	TOTAL SHEETS
	N.C.	34345.1.1 (R-06091A)	1	16
	STATE PROJ. NO.	F.A. PROJ. NO.	DESCRIPTION	
			P.E.	
			CONST.	

### CAUTION NOTICE

THE SUBSURFACE INFORMATION AND THE SUBSURFACE INVESTIGATION ON WHICH IT IS BASED WAS MADE FOR THE PURPOSE OF STUDY, PLANNING AND DESIGN, AND NOT FOR CONSTRUCTION OR PAY PURPOSES. THE VARIOUS FIELD BORING LOGS, ROCK CORES, AND SOIL TEST DATA AVAILABLE MAY BE REVIEWED OR INSPECTED IN RALEIGH BY CONTACTING THE N.C. DEPARTMENT OF TRANSPORTATION, GEOTECHNICAL UNIT @ (919) 250-4088. NEITHER THE SUBSURFACE PLANS AND REPORTS, NOR THE FIELD BORING LOGS, ROCK CORES, OR SOIL TEST DATA IS PART OF THE CONTRACT.

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THE BIDDER OR CONTRACTOR IS CAUTIONED THAT DETAILS SHOWN ON THE SUBSURFACE PLANS ARE PRELIMINARY ONLY AND IN MANY CASES THE FINAL DESIGN DETAILS ARE DIFFERENT. FOR BIDDING AND CONSTRUCTION PURPOSES, REFER TO THE CONSTRUCTION PLANS AND DOCUMENTS FOR FINAL DESIGN INFORMATION ON THIS PROJECT. THE DEPARTMENT DOES NOT WARRANT OR GUARANTEE THE SUFFICIENCY OR ACCURACY OF THE INVESTIGATION MADE, NOR THE INTERPRETATIONS MADE OR OPINION OF THE DEPARTMENT AS TO THE TYPE OF MATERIALS AND CONDITIONS TO BE ENCOUNTERED. THE BIDDER OR CONTRACTOR IS CAUTIONED TO MAKE SUCH INDEPENDENT SUBSURFACE INVESTIGATIONS AS HE DEEMS NECESSARY TO SATISFY HIMSELF AS TO CONDITIONS TO BE ENCOUNTERED ON THIS PROJECT. THE CONTRACTOR SHALL HAVE NO CLAIM FOR ADDITIONAL COMPENSATION OR FOR AN EXTENSION OF TIME FOR ANY REASON RESULTING FROM THE ACTUAL CONDITIONS ENCOUNTERED AT THE SITE DIFFERING FROM THOSE INDICATED IN THE SUBSURFACE INFORMATION.

STATE PROJECT 34345.1.1 I.D. NO. R-06091A

F.A. PROJECT MAF-F-119(1)

COUNTY GUILFORD

PROJECT DESCRIPTION US 311 (HIGH POINT EAST BELTWAY) FROM US 29-70 TO I-85 NORTH OF ARCHDALE

SITE DESCRIPTION STRUCTURE NO.6 ON -Y6NB- (NB I-85) AT STA. 27+10.2 AND STRUCTURE NO.7 ON -NBCD- AT STA. 21+32.4 OVER US 311

INVESTIGATED BY N.T. ROBERSON PERSONNEL C.D. CZAJKA

CHECKED BY D.N. ARGENBRIGHT D.C. B

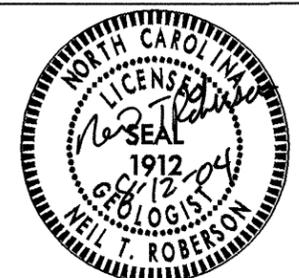
SUBMITTED BY D.N. ARGENBRIGHT R.E. M

DATE AUGUST 2004 R.M. B

DRAWN BY: T.T. WALKER

NOTE - THE INFORMATION CONTAINED HEREIN IS NOT IMPLIED OR GUARANTEED BY THE N.C. DEPARTMENT OF TRANSPORTATION AS BEING ACCURATE NOR IT IS CONSIDERED TO BE PART OF THE PLANS, SPECIFICATIONS, OR CONTRACT FOR THE PROJECT.

NOTE - BY HAVING REQUESTED THIS INFORMATION THE CONTRACTOR SPECIFICALLY WAIVES ANY CLAIMS FOR INCREASED COMPENSATION OR EXTENSION OF TIME BASED ON DIFFERENCES BETWEEN THE CONDITIONS INDICATED HEREIN AND THE ACTUAL CONDITIONS AT THE PROJECT SITE.



**NORTH CAROLINA DEPARTMENT OF TRANSPORTATION**  
**DIVISION OF HIGHWAYS**  
**GEOTECHNICAL UNIT**

ID	STATE PROJECT NO.	SHEET NO.	TOTAL SHEETS
R-06091A	34345.JI	2	16



**SUBSURFACE INVESTIGATION**

**SOIL AND ROCK LEGEND, TERMS, SYMBOLS, AND ABBREVIATIONS**

SOIL DESCRIPTION	GRADATION	ROCK DESCRIPTION	TERMS AND DEFINITIONS
SOIL IS CONSIDERED TO BE THE UNCONSOLIDATED, SEMI-CONSOLIDATED OR WEATHERED EARTH MATERIALS WHICH CAN BE PENETRATED WITH A CONTINUOUS FLIGHT POWER AUGER, AND WHICH YIELDS LESS THAN 100 BLOWS PER 30 cm ACCORDING TO STANDARD PENETRATION TEST (AASHTO T206, ASTM D-1586). SOIL CLASSIFICATION IS BASED ON THE AASHTO SYSTEM AND BASIC DESCRIPTIONS GENERALLY SHALL INCLUDE: CONSISTENCY, COLOR, TEXTURE, MOISTURE, AASHTO CLASSIFICATION, AND OTHER PERTINENT FACTORS SUCH AS MINERALOGICAL COMPOSITION, ANGULARITY, STRUCTURE, PLASTICITY, ETC. EXAMPLE: <i>VERY STIFF, GRAY SILTY CLAY, MOST WITH INTERBEDDED FINE SAND LAYERS, HIGH PLASTIC, A-7-6</i>	<b>WELL GRADED</b> - INDICATES A GOOD REPRESENTATION OF PARTICLE SIZES FROM FINE TO COARSE <b>UNIFORM</b> - INDICATES THAT SOIL PARTICLES ARE ALL APPROXIMATELY THE SAME SIZE (ALSO POORLY GRADED). <b>GAP-GRADED</b> - INDICATES A MIXTURE OF UNIFORM PARTICLES OF TWO OR MORE SIZES. <b>ANGULARITY OF GRAINS</b> THE ANGULARITY OR ROUNDNESS OF SOIL GRAINS ARE DESIGNATED BY THE TERMS: <b>ANGULAR</b> , <b>SUBANGULAR</b> , <b>SUBROUNDED</b> , OR <b>ROUNDED</b> .	<b>HARD ROCK</b> IS NON-COASTAL PLAIN MATERIAL THAT WHEN TESTED, WOULD YIELD SPT REFUSAL. AN INFERRED ROCK LINE INDICATES THE LEVEL AT WHICH NON-COASTAL PLAIN MATERIAL WOULD YIELD SPT REFUSAL. SPT REFUSAL IS PENETRATION BY A SPLIT SPOON SAMPLER EQUAL TO OR LESS THAN 2.5 cm PER 50 BLOWS. IN NON-COASTAL PLAIN MATERIAL, THE TRANSITION BETWEEN SOIL AND ROCK IS OFTEN REPRESENTED BY A ZONE OF WEATHERED ROCK. ROCK MATERIALS ARE TYPICALLY DIVIDED AS FOLLOWS: <b>WEATHERED ROCK (WR)</b> <b>CRYSTALLINE ROCK (CR)</b> <b>NON-CRYSTALLINE ROCK (NCR)</b> <b>COASTAL PLAIN SEDIMENTARY ROCK (CP)</b>	<b>ALLUVIUM (ALLUV.)</b> - SOILS WHICH HAVE BEEN TRANSPORTED BY WATER. <b>AQUIFER</b> - A WATER BEARING FORMATION OR STRATA. <b>ARENACEOUS</b> - APPLIED TO ROCKS THAT HAVE BEEN DERIVED FROM SAND OR THAT CONTAIN SAND. <b>ARGILLACEOUS</b> - APPLIED TO ALL ROCKS OR SUBSTANCES COMPOSED OF CLAY MINERALS, OR HAVING A NOTABLE PROPORTION OF CLAY IN THEIR COMPOSITION, AS SHALE, SLATE, ETC. <b>ARTESIAN</b> - GROUND WATER THAT IS UNDER SUFFICIENT PRESSURE TO RISE ABOVE THE LEVEL AT WHICH IS ENCOUNTERED, BUT WHICH DOES NOT NECESSARILY RISE TO OR ABOVE THE GROUND SURFACE. <b>CALCAREOUS (CALC.)</b> - SOILS WHICH CONTAIN APPRECIABLE AMOUNTS OF CALCIUM CARBONATE. <b>COLLUVIUM</b> - ROCK FRAGMENTS MIXED WITH SOIL DEPOSITED BY GRAVITY ON SLOPE OR AT BOTTOM OF SLOPE. <b>CORE RECOVERY (REC.)</b> - TOTAL LENGTH OF ALL MATERIAL RECOVERED IN THE CORE BARREL DIVIDED BY TOTAL LENGTH OF CORE RUN AND EXPRESSED AS A PERCENTAGE. <b>DIKE</b> - A TABULAR BODY OF IGNEOUS ROCK THAT CUTS ACROSS THE STRUCTURE OF ADJACENT ROCKS OR CUTS MASSIVE ROCK. <b>DIP</b> - THE ANGLE AT WHICH A STRATUM OR ANY PLANAR FEATURE IS INCLINED FROM THE HORIZONTAL. <b>DIP DIRECTION @IP AZIMUTH</b> - THE DIRECTION OR BEARING OF THE HORIZONTAL TRACE OF THE LINE OF DIP, MEASURED CLOCKWISE FROM NORTH. <b>FALLT</b> - A FRACTURE OR FRACTURE ZONE ALONG WHICH THERE HAS BEEN DISPLACEMENT OF THE SIDES RELATIVE TO ONE ANOTHER PARALLEL TO THE FRACTURE. <b>FISSILE</b> - A PROPERTY OF SPLITTING ALONG CLOSELY SPACED PARALLEL PLANES. <b>FLOAT</b> - ROCK FRAGMENTS ON SURFACE NEAR THEIR ORIGINAL POSITION AND DISLOADED FROM PARENT MATERIAL. <b>FLOOD PLAIN (F.P.)</b> - LAND BORDERING A STREAM, BUILT OF SEDIMENTS DEPOSITED BY THE STREAM. <b>FORMATION (FM.)</b> - A MAPPABLE GEOLOGIC UNIT THAT CAN BE RECOGNIZED AND TRACED IN THE FIELD. <b>JOINT</b> - FRACTURE IN ROCK ALONG WHICH NO APPRECIABLE MOVEMENT HAS OCCURRED. <b>LEDGE</b> - A SHELF-LIKE RIDGE OR PROJECTION OF ROCK WHOSE THICKNESS IS SMALL COMPARED TO ITS LATERAL EXTENT. <b>LENS</b> - A BODY OF SOIL OR ROCK THAT THINS OUT IN ONE OR MORE DIRECTIONS. <b>MOTTLED (MOT.)</b> - IRREGULARLY MARKED WITH SPOTS OF DIFFERENT COLORS. MOTTLING IN SOILS USUALLY INDICATES POOR AERATION AND LACK OF GOOD DRAINAGE. <b>PERCHED WATER</b> - WATER MAINTAINED ABOVE THE NORMAL GROUND WATER LEVEL BY THE PRESENCE OF AN INTERVENING IMPERVIOUS STRATUM. <b>RESIDUAL SOIL</b> - SOIL FORMED IN PLACE BY THE WEATHERING OF ROCK. <b>ROCK QUALITY DESIGNATION (R.Q.D.)</b> - A MEASURE OF ROCK QUALITY DESCRIBED BY: TOTAL LENGTH OF ROCK SEGMENTS EQUAL TO OR GREATER THAN 10 CENTIMETERS DIVIDED BY THE TOTAL LENGTH OF CORE RUN AND EXPRESSED AS A PERCENTAGE. <b>SAPROLITE (SAP.)</b> - RESIDUAL SOIL WHICH RETAINS THE RELIC STRUCTURE OR FABRIC OF THE PARENT ROCK. <b>SILL</b> - AN INTRUSIVE BODY OF IGNEOUS ROCK OF APPROXIMATELY UNIFORM THICKNESS AND RELATIVELY THIN COMPARED WITH ITS LATERAL EXTENT, WHICH HAS BEEN EMPLACED PARALLEL TO THE BEDDING OR SCHISTOSITY OF THE INTRUDED ROCKS. <b>SLICKENSIDE</b> - POLISHED AND STRIATED SURFACE THAT RESULTS FROM FRICTION ALONG A FAULT OR SLIP PLANE. <b>STANDARD PENETRATION TEST (PENETRATION RESISTANCE) (SPT)</b> - NUMBER OF BLOWS (NO OF A 63.5 kg HAMMER FALLING 0.76 METERS REQUIRED TO PRODUCE A PENETRATION OF 30 cm INTO SOIL WITH A 5 cm OUTSIDE DIAMETER SPLIT SPOON SAMPLER. SPT REFUSAL IS LESS THAN 2.5 cm PENETRATION WITH 50 BLOWS. <b>STRATA CORE RECOVERY (SREC.)</b> - TOTAL LENGTH OF STRATA MATERIAL RECOVERED DIVIDED BY TOTAL LENGTH OF STRATUM AND EXPRESSED AS A PERCENTAGE. <b>STRATA ROCK QUALITY DESIGNATION (S.R.Q.D.)</b> - A MEASURE OF ROCK QUALITY DESCRIBED BY: TOTAL LENGTH OF ROCK SEGMENTS WITHIN A STRATUM EQUAL TO OR GREATER THAN 10 CENTIMETERS DIVIDED BY THE TOTAL LENGTH OF STRATA AND EXPRESSED AS A PERCENTAGE. <b>TOPSOIL (T.S.)</b> - SURFACE SOILS USUALLY CONTAINING ORGANIC MATTER.
<b>SOIL LEGEND AND AASHTO CLASSIFICATION</b>	<b>MINERALOGICAL COMPOSITION</b>	<b>WEATHERING</b>	
GENERAL CLASS. GRANULAR MATERIALS (<85% PASSING #200) SILT-CLAY MATERIALS (>85% PASSING #200) ORGANIC MATERIALS	MINERAL NAMES SUCH AS QUARTZ, FELDSPAR, MICA, TALC, KAOLIN, ETC. ARE USED IN DESCRIPTIONS WHENEVER THEY ARE CONSIDERED OF SIGNIFICANCE.	<b>FRESH</b> ROCK FRESH, CRYSTALS BRIGHT, FEW JOINTS MAY SHOW SLIGHT STAINING, ROCK RINGS UNDER HAMMER IF CRYSTALLINE. <b>VERY SLIGHT (V. SLI.)</b> ROCK GENERALLY FRESH, JOINTS STAINED, SOME JOINTS MAY SHOW THIN CLAY COATINGS IF OPEN, CRYSTALS ON A BROKEN SPECIMEN FACE SHINE BRIGHTLY, ROCK RINGS UNDER HAMMER BLOWS IF OF A CRYSTALLINE NATURE. <b>SLIGHT (SLI.)</b> ROCK GENERALLY FRESH, JOINTS STAINED AND DISCOLORATION EXTENDS INTO ROCK UP TO 2.5 cm. OPEN JOINTS MAY CONTAIN CLAY. IN GRANITOID ROCKS SOME OCCASIONAL FELDSPAR CRYSTALS ARE DULL AND DISCOLORED, CRYSTALLINE ROCKS RING UNDER HAMMER BLOWS. <b>MODERATE (MOD.)</b> SIGNIFICANT PORTIONS OF ROCK SHOW DISCOLORATION AND WEATHERING EFFECTS. IN GRANITOID ROCKS, MOST FELDSPARS ARE DULL AND DISCOLORED, SOME SHOW CLAY. ROCK HAS DULL SOUND UNDER HAMMER BLOWS AND SHOWS SIGNIFICANT LOSS OF STRENGTH AS COMPARED WITH FRESH ROCK. <b>MODERATELY SEVERE (MOD. SEV.)</b> ALL ROCK EXCEPT QUARTZ DISCOLORED OR STAINED. IN GRANITOID ROCKS, ALL FELDSPARS DULL AND DISCOLORED AND A MAJORITY SHOW KAOLINIZATION. ROCK SHOWS SEVERE LOSS OF STRENGTH AND CAN BE EXCAVATED WITH A GEOLOGIST'S PICK. ROCK GIVES "CLUNK" SOUND WHEN STRUCK. <i>IF TESTED, WOULD YIELD SPT REFUSAL</i> <b>SEVERE (SEV.)</b> ALL ROCKS EXCEPT QUARTZ DISCOLORED OR STAINED, ROCK FABRIC CLEAR AND EVIDENT BUT REDUCED IN STRENGTH TO STRONG SOIL. IN GRANITOID ROCKS ALL FELDSPARS ARE KAOLINIZED TO SOME EXTENT. SOME FRAGMENTS OF STRONG ROCK USUALLY REMAIN. <i>IF TESTED, YIELDS SPT N VALUES &gt; 100 BLOWS PER 30 cm</i> <b>VERY SEVERE (V. SEV.)</b> ALL ROCK EXCEPT QUARTZ DISCOLORED OR STAINED, ROCK FABRIC ELEMENTS ARE DISCERNIBLE BUT THE MASS IS EFFECTIVELY REDUCED TO SOIL STATUS, WITH ONLY FRAGMENTS OF STRONG ROCK REMAINING. SAPROLITE IS AN EXAMPLE OF ROCK WEATHERED TO A DEGREE SUCH THAT ONLY MINOR VESTIGES OF THE ORIGINAL ROCK FABRIC REMAIN. <i>IF TESTED, YIELDS SPT N VALUES &lt; 100 BLOWS PER 30 cm</i> <b>COMPLETE</b> ROCK REDUCED TO SOIL. ROCK FABRIC NOT DISCERNIBLE, OR DISCERNIBLE ONLY IN SMALL AND SCATTERED CONCENTRATIONS. QUARTZ MAY BE PRESENT AS DIXES OR STRINGERS. SAPROLITE IS ALSO AN EXAMPLE.	
<b>COMPRESSIBILITY</b> SLIGHTLY COMPRESSIBLE LIQUID LIMIT 31-50 MODERATELY COMPRESSIBLE LIQUID LIMIT 51-60 HIGHLY COMPRESSIBLE LIQUID LIMIT GREATER THAN 60	<b>PERCENTAGE OF MATERIAL</b> <b>ORGANIC MATERIAL</b> TRACE OF ORGANIC MATTER 2 - 3% LITTLE ORGANIC MATTER 3 - 5% MODERATELY ORGANIC 5 - 10% HIGHLY ORGANIC >10% <b>GRANULAR SOILS</b> SILT-CLAY SOILS OTHER MATERIAL TRACE 1 - 10% LITTLE 10 - 20% SOME 20 - 35% HIGHLY 35% AND ABOVE	<b>GROUND WATER</b> WATER LEVEL IN BORE HOLE IMMEDIATELY AFTER DRILLING. STATIC WATER LEVEL AFTER 24 HOURS. PERCHED WATER, SATURATED ZONE OR WATER BEARING STRATA SPRING OR SEEPAGE	
<b>CONSISTENCY OR DENSENESS</b>	<b>MISCELLANEOUS SYMBOLS</b>	<b>ROCK HARDNESS</b>	
PRIMARY SOIL TYPE COMPACTNESS OR CONSISTENCY RANGE OF STANDARD PENETRATION RESISTANCE (N-VALUE) RANGE OF UNCONFINED COMPRESSIVE STRENGTH (KN/m <sup>2</sup> )	ROADWAY EMBANKMENT WITH SOIL DESCRIPTION SOIL SYMBOL ARTIFICIAL FILL OTHER THAN ROADWAY EMBANKMENTS INFERRED SOIL BOUNDARIES INFERRED ROCK LINE ALLUVIAL SOIL BOUNDARY DIP/DIP DIRECTION OF ROCK STRUCTURES SOUNDING ROD	<b>VERY HARD</b> CANNOT BE SCRATCHED BY KNIFE OR SHARP PICK. BREAKING OF HAND SPECIMENS REQUIRES SEVERAL HARD BLOWS OF THE GEOLOGIST'S PICK. <b>HARD</b> CAN BE SCRATCHED BY KNIFE OR PICK ONLY WITH DIFFICULTY. HARD HAMMER BLOWS REQUIRED TO DETACH HAND SPECIMEN. <b>MODERATELY HARD</b> CAN BE SCRATCHED BY KNIFE OR PICK. GOUGES OR GROOVES TO 6 mm DEEP CAN BE EXCAVATED BY HARD BLOW OF A GEOLOGIST'S PICK. HAND SPECIMENS CAN BE DETACHED BY MODERATE BLOWS. <b>MEDIUM HARD</b> CAN BE GROOVED OR GOUGED 1 mm DEEP BY FIRM PRESSURE OF KNIFE OR PICK POINT. CAN BE EXCAVATED IN SMALL CHIPS TO PIECES 25 mm MAXIMUM SIZE BY HARD BLOWS OF THE POINT OF A GEOLOGIST'S PICK. <b>SOFT</b> CAN BE GROOVED OR GOUGED READILY BY KNIFE OR PICK. CAN BE EXCAVATED IN FRAGMENTS FROM CHIPS TO SEVERAL CENTIMETERS IN SIZE BY MODERATE BLOWS OF A PICK POINT. SMALL, THIN PIECES CAN BE BROKEN BY FINGER PRESSURE. <b>VERY SOFT</b> CAN BE CARVED WITH KNIFE. CAN BE EXCAVATED READILY WITH POINT OF PICK. PIECES 25 mm OR MORE IN THICKNESS CAN BE BROKEN BY FINGER PRESSURE. CAN BE SCRATCHED READILY BY FINGER NAIL.	
<b>TEXTURE OR GRAIN SIZE</b>	<b>ABBREVIATIONS</b>	<b>FRACTURE SPACING</b>	<b>BEDDING</b>
U.S. STD. SIEVE SIZE OPENING (MM) 4 10 40 60 200 270 4.76 2.0 0.42 0.25 0.075 0.053	AR - AUGER REFUSAL BT - BORING TERMINATED CL - CLAY CPT - CONE PENETRATION TEST CSE - COARSE DMT - DILATOMETER TEST DPT - DYNAMIC PENETRATION TEST e - VOID RATIO F - FINE FOSS - FOSSILIFEROUS FRAC - FRACTURED FRAGS - FRAGMENTS MED. - MEDIUM PMT - PRESSUREMETER TEST SD - SAND, SANDY SL - SILT, SILTY SLI - SLIGHTLY TCR - TRICONE REFUSAL γ - UNIT WEIGHT γ <sub>d</sub> - DRY UNIT WEIGHT w - MOISTURE CONTENT v - VERY VST - VANE SHEAR TEST	<b>VERY WIDE</b> MORE THAN 3 m <b>WIDE</b> 1 TO 3 m <b>MODERATELY CLOSE</b> 30 TO 100 cm <b>CLOSE</b> 5 TO 30 cm <b>VERY CLOSE</b> LESS THAN 5 cm	<b>VERY THICKLY BEDDED</b> > 1 m <b>THICKLY BEDDED</b> 0.5 - 1 m <b>THINLY BEDDED</b> 0.05 - 0.5 m <b>VERY THINLY BEDDED</b> 10 - 50 mm <b>THICKLY LAMINATED</b> 2.5 - 10 mm <b>THINLY LAMINATED</b> < 2.5 mm
<b>SOIL MOISTURE - CORRELATION OF TERMS</b>	<b>EQUIPMENT USED ON SUBJECT PROJECT</b>	<b>INDURATION</b>	
SOIL MOISTURE SCALE (ATTERBERG LIMITS) FIELD MOISTURE DESCRIPTION GUIDE FOR FIELD MOISTURE DESCRIPTION	DRILL UNITS: <input checked="" type="checkbox"/> MOBILE B-57 <input type="checkbox"/> BK-51 <input type="checkbox"/> CME-45C <input type="checkbox"/> CME-550 <input type="checkbox"/> PORTABLE HOIST <input type="checkbox"/> OTHER _____ <input type="checkbox"/> OTHER _____	<b>FOR SEDIMENTARY ROCKS, INDURATION IS THE HARDENING OF THE MATERIAL BY CEMENTING, HEAT, PRESSURE, ETC.</b> <b>FRIABLE</b> RUBBING WITH FINGER FREES NUMEROUS GRAINS; GENTLE BLOW BY HAMMER DISINTEGRATES SAMPLE. <b>MODERATELY INDURATED</b> GRAINS CAN BE SEPARATED FROM SAMPLE WITH STEEL PROBE; BREAKS EASILY WHEN HIT WITH HAMMER. <b>INDURATED</b> GRAINS ARE DIFFICULT TO SEPARATE WITH STEEL PROBE; DIFFICULT TO BREAK WITH HAMMER. <b>EXTREMELY INDURATED</b> SHARP HAMMER BLOWS REQUIRED TO BREAK SAMPLE; SAMPLE BREAKS ACROSS GRAINS.	
<b>PLASTICITY</b>			
NONPLASTIC LOW PLASTICITY MED. PLASTICITY HIGH PLASTICITY			
<b>COLOR</b>			
DESCRIPTIONS MAY INCLUDE COLOR OR COLOR COMBINATIONS (TAN, RED, YEL-BRN, BLUE-GRAY) MODIFIERS SUCH AS LIGHT, DARK, STREAKED, ETC. ARE USED TO DESCRIBE APPEARANCE.			



STATE OF NORTH CAROLINA  
DEPARTMENT OF TRANSPORTATION

Michael F. Easley  
GOVERNOR

P.O. BOX 25201, RALEIGH, N.C. 27611-5201

Lyndo Tippet  
SECRETARY

August 12, 2004

STATE PROJECT: 34345.1.1 (R-0609IA)  
F.A. PROJECT: MAF-F-119-1 (1)  
COUNTY: Guilford

DESCRIPTION: US 311 (High Point East Beltway) from US 29-70 to I-85  
North of Archdale

SUBJECT: Structure No. 6 on -Y6NB- (NB I-85) at Sta. 21+25.6 and Structure  
No. 7 on -NBCD- at Sta. 27+12.1 over US 311

**Project Description**

Two bridges with two spans each, 83.6-meters in length with a 90° skew, are proposed on -Y6NB- (NB I-85) and -NBCD- over proposed US 311. The project is located in Guilford County about two miles northeast of Archdale.

The subsurface investigation was conducted during June of 2004 using an ATV-mounted Mobile B-57 drill machine. One Standard Penetration Test boring was performed at each of the six proposed bent locations. All borings were advanced into crystalline or weathered rock. Boring B1-A was cored using NQWL rock coring equipment. Representative soil and rock samples were obtained for visual classification in the field and selected samples were submitted to the Materials and Test Unit for laboratory analysis.

**Physiography and Geology**

The project is located in the gently rolling terrain of the Piedmont Physiographic Province. Geologically, the site is located within the Carolina Slate Belt and is underlain by crystalline meta-granite and diabase dikes. The area consists of a mixture of wooded land, sparse homes and agricultural fields.

**Soil Properties**

Soils encountered at the project site include roadway embankment and residual soils.

Roadway embankment soils are present along existing northbound I-85 at structure no. 6. These soils consist primarily of brown, dry, medium dense to dense, silty sand (A-2-4). Numerous weathered rock boulders are also present in the roadway embankment. These soils are 2.43 to 6.82 meters thick and are underlain by residual soil and weathered rock.

SHEET 3 OF 16  
34345.1.1 (R-0609IA)

Residual soils are present at each boring except for B1-B of structure no. 6, and range in thickness from 0.91 to 4.72 meters. These soils consist primarily of orange and tan-brown, dry to moist, loose to very dense, silty and clayey sand (A-2-4, A-2-6). Residual soils are derived from weathering of the underlying weathered and crystalline rock.

**Rock Properties**

Weathered rock is present at each boring location. Weathered rock is derived from the underlying crystalline meta-granite and diabase and ranges in thickness from 4.30 to 5.42 meters. The top of weathered rock was encountered at elevations ranging from 233.05 to 241.51 meters.

Crystalline rock was encountered at borings B1-A and EB2-A of structure no. 7 and B1-B of structure no. 6. The top of crystalline rock ranges in elevation from 229.90 to 237.21 meters. Rock core was obtained from the interior bent boring of structure no. 7. Crystalline rock consists of light to dark gray, very slightly to slightly weathered, hard, moderately closely to closely fractured diabase. Core recovery (REC) ranges from 94% to 100% and Rock Quality Designation (RQD) ranges from 54% to 94% in B1-A. More detailed rock descriptions can be found in the Core Boring Reports.

**Groundwater**

Groundwater was encountered at two of the boring locations. Groundwater elevations ranged from 233.65 to 238.47 meters.

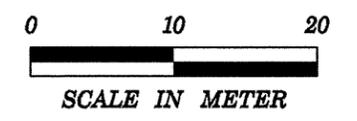
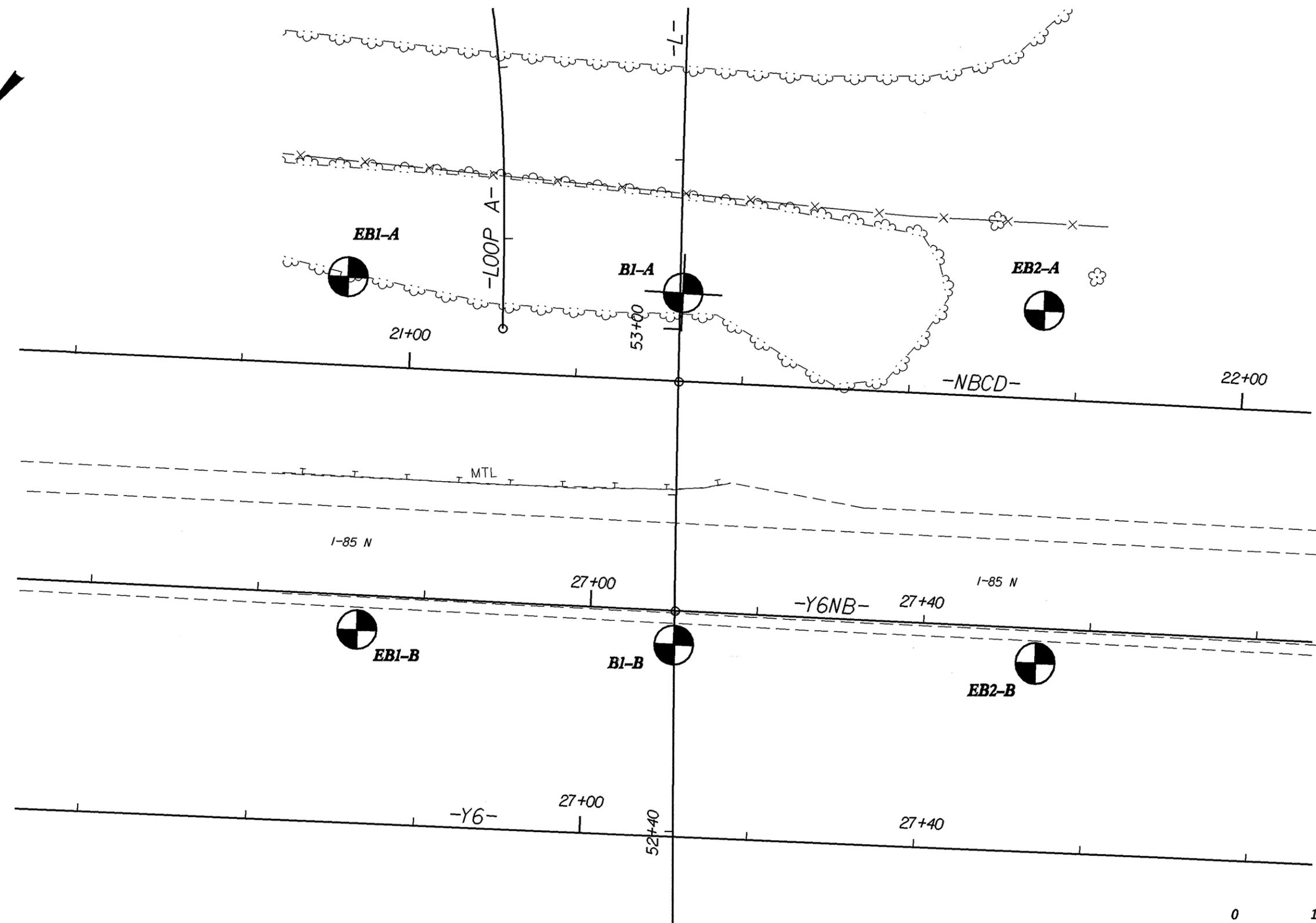
**Notice**

This Geotechnical foundation report is based on the Preliminary General Drawings dated April 6, 2004. If significant changes are made in the design or location of the proposed structure, the subsurface information should be reviewed and modified as necessary.

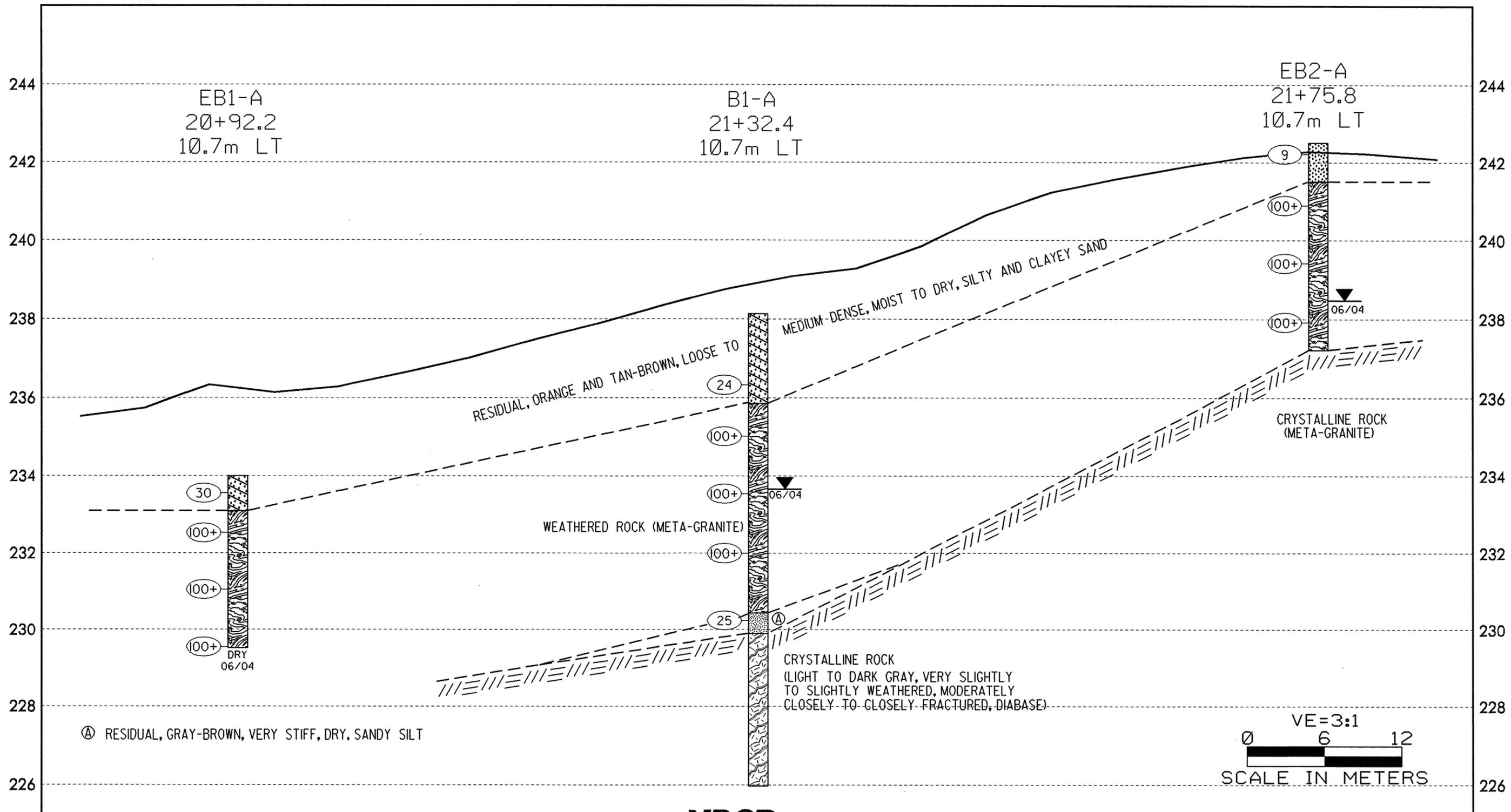
Respectfully submitted,

Neil T. Roberson  
Project Geologist

# TEST SITE PLAN

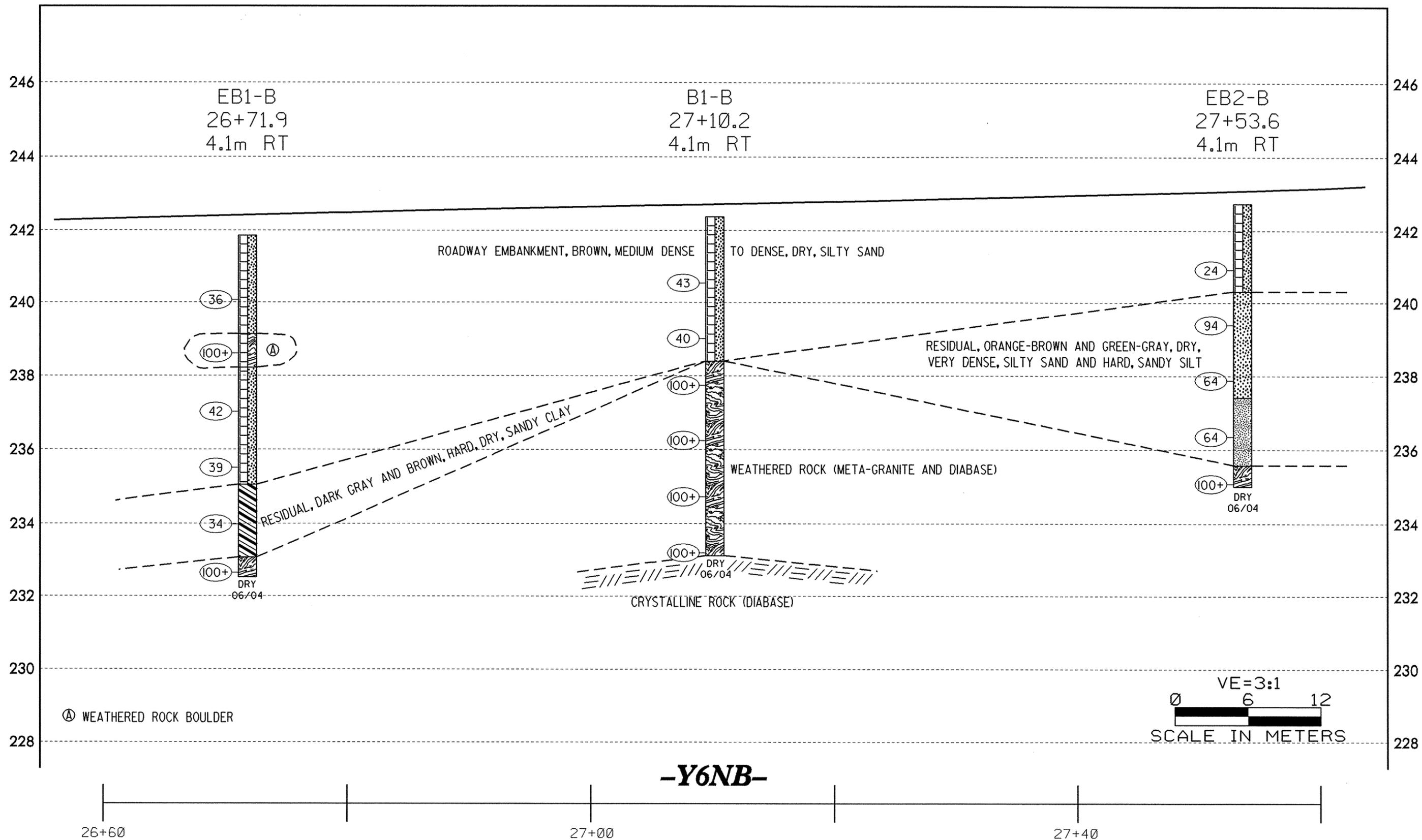


PROFILE THROUGH BORINGS PROJECTED ALONG -NBCD-

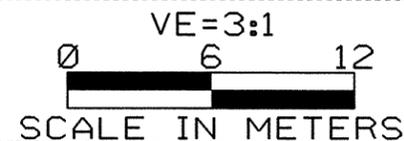


**-NBCD-**

PROFILE THROUGH BORINGS PROJECTED ALONG -Y6NB-

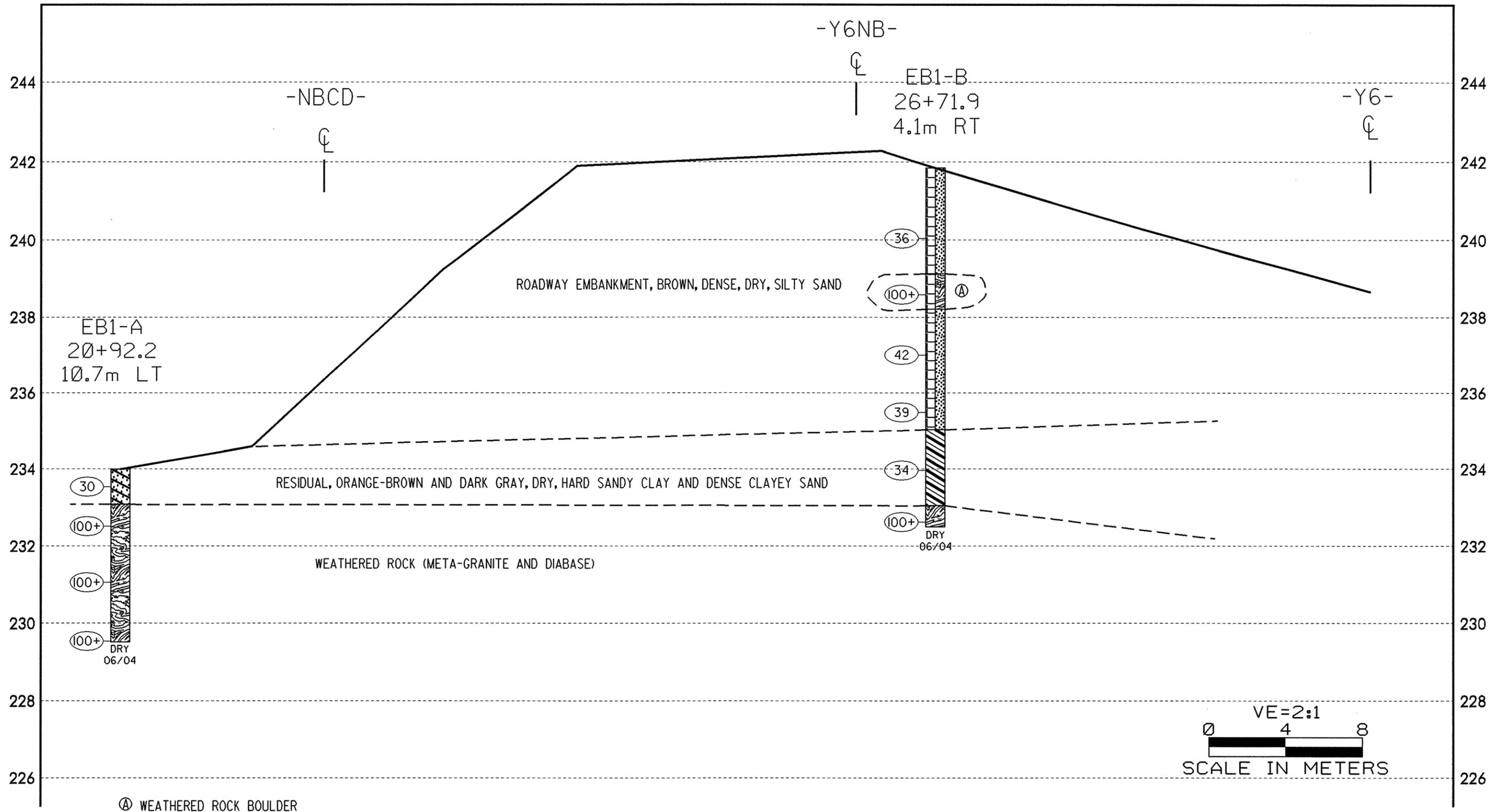


Ⓐ WEATHERED ROCK BOULDER

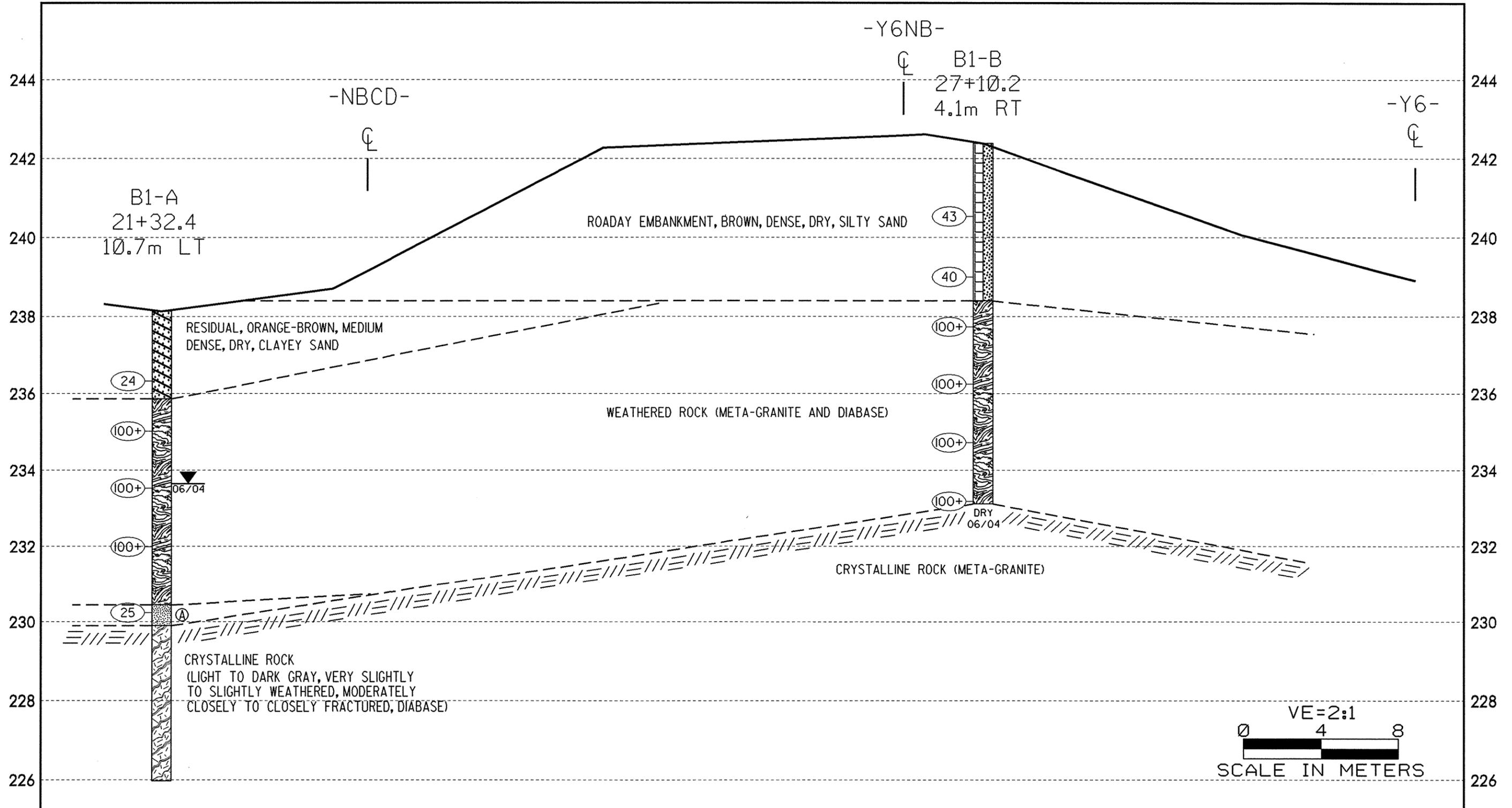


**-Y6NB-**

# CROSS SECTION THROUGH END BENT I

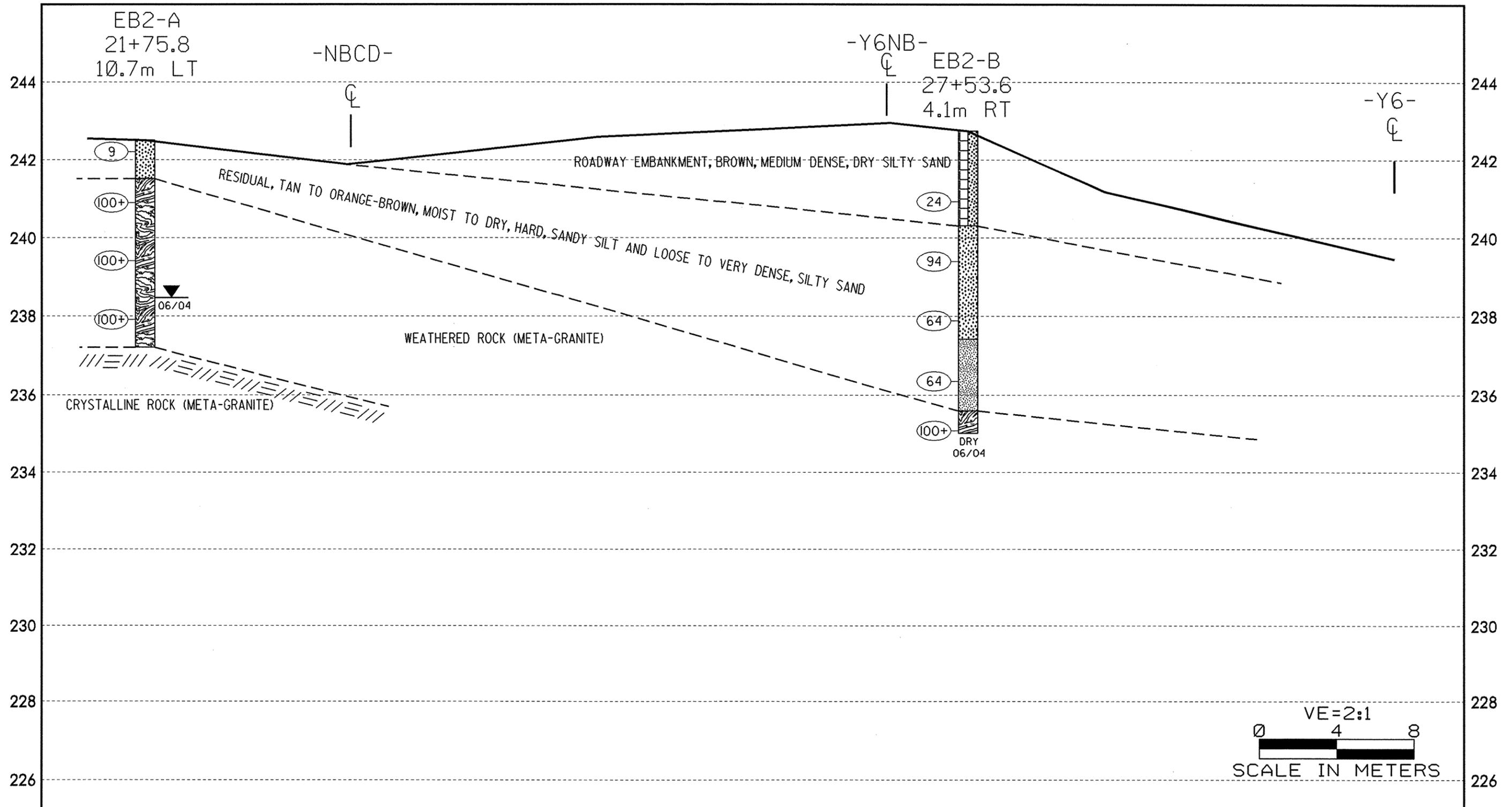


# CROSS SECTION THROUGH BENT I



Ⓐ RESIDUAL, GRAY-BROWN, VERY STIFF, SANDY SILT

# CROSS SECTION THROUGH END BENT 2











**EB1-B**

<b>SOIL TEST RESULTS</b>															
SAMPLE NO.	OFFSET	STATION	DEPTH INTERVAL	AASHTO CLASS.	L.L.	P.I.	% BY WEIGHT				% PASSING (SIEVES)			% MOISTURE	% ORGANIC
							C.SAND	F.SAND	SILT	CLAY	10	40	200		
SS-9	4.1m RT	26+71.9	7.58-8.03	A-6(7)	37	13	7.7	23.9	46.1	22.3	89	85	65	-	-

**BI-A**

<b>SOIL TEST RESULTS</b>															
SAMPLE NO.	OFFSET	STATION	DEPTH INTERVAL	AASHTO CLASS.	L.L.	P.I.	% BY WEIGHT				% PASSING (SIEVES)			% MOISTURE	% ORGANIC
							C.SAND	F.SAND	SILT	CLAY	10	40	200		
SS-2	10.7m LT	21+32.4	1.51-1.96	A-2-6(2)	35	18	40.7	21.1	15.9	22.3	83	61	35	-	-
SS-3	10.7m LT	21+32.4	7.59-8.04	A-4(1)	31	4	10.7	35.9	39.2	14.2	100	96	62	-	-

**EB2-A**

<b>SOIL TEST RESULTS</b>															
SAMPLE NO.	OFFSET	STATION	DEPTH INTERVAL	AASHTO CLASS.	L.L.	P.I.	% BY WEIGHT				% PASSING (SIEVES)			% MOISTURE	% ORGANIC
							C.SAND	F.SAND	SILT	CLAY	10	40	200		
SS-1	10.7m LT	21+75.8	0.00-0.45	A-2-4(0)	22	5	46.2	21.7	17.9	14.2	89	59	32	-	-

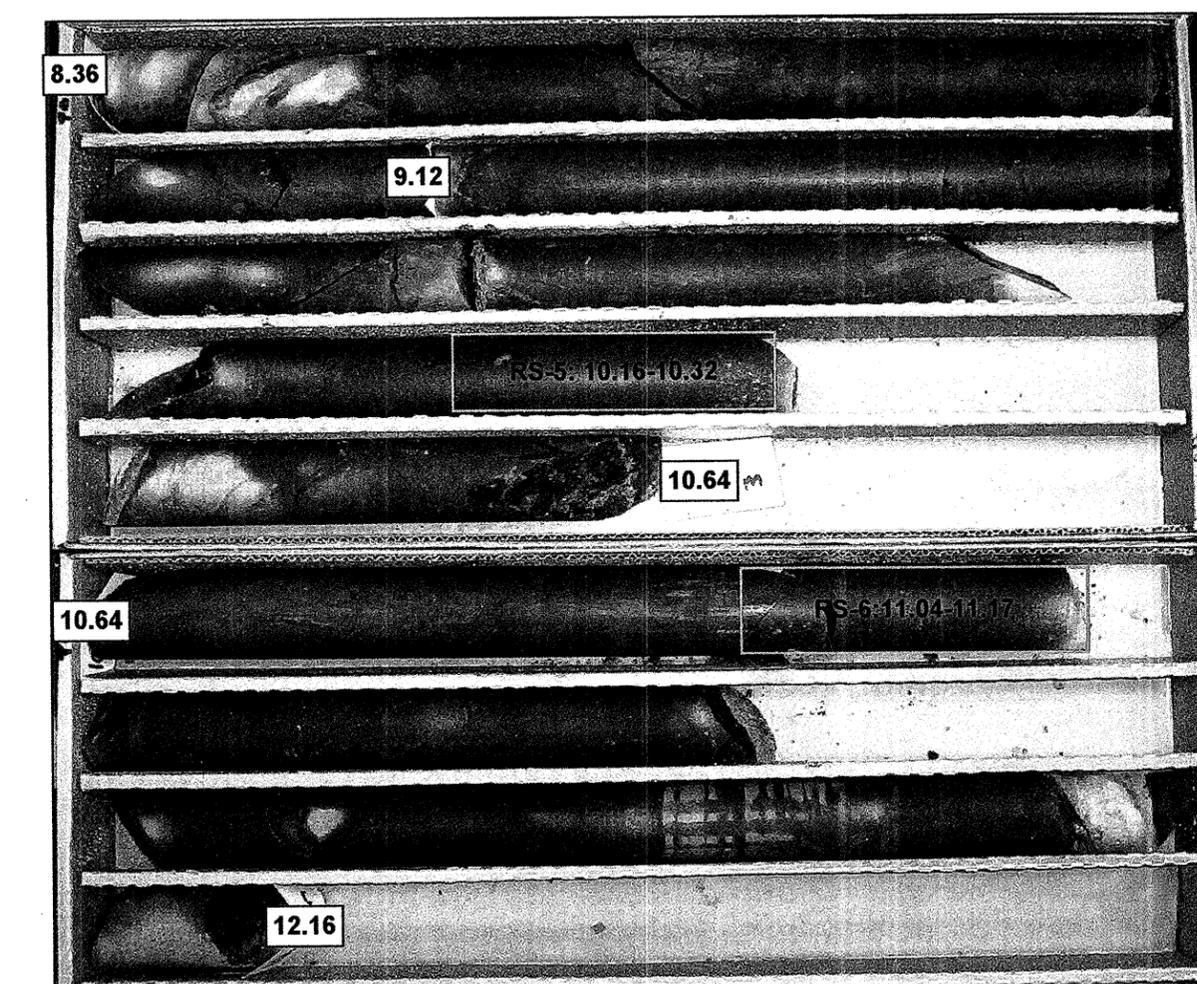
**EB2-B**

<b>SOIL TEST RESULTS</b>															
SAMPLE NO.	OFFSET	STATION	DEPTH INTERVAL	AASHTO CLASS.	L.L.	P.I.	% BY WEIGHT				% PASSING (SIEVES)			% MOISTURE	% ORGANIC
							C.SAND	F.SAND	SILT	CLAY	10	40	200		
SS-10	4.1m RT	27+53.6	3.04-3.49	A-2-4(0)	25	NP	56.1	25.1	10.6	8.1	100	62	23	-	-
SS-11	4.1m RT	27+53.6	6.08-6.53	A-4(7)	40	9	2.4	33.6	49.7	14.2	100	99	75	-	-

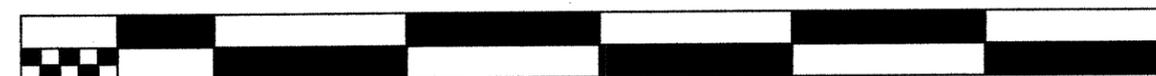
# CORE PHOTOGRAPHS

## B1-A

BOXES 1 & 2: 8.36 - 12.16 METERS

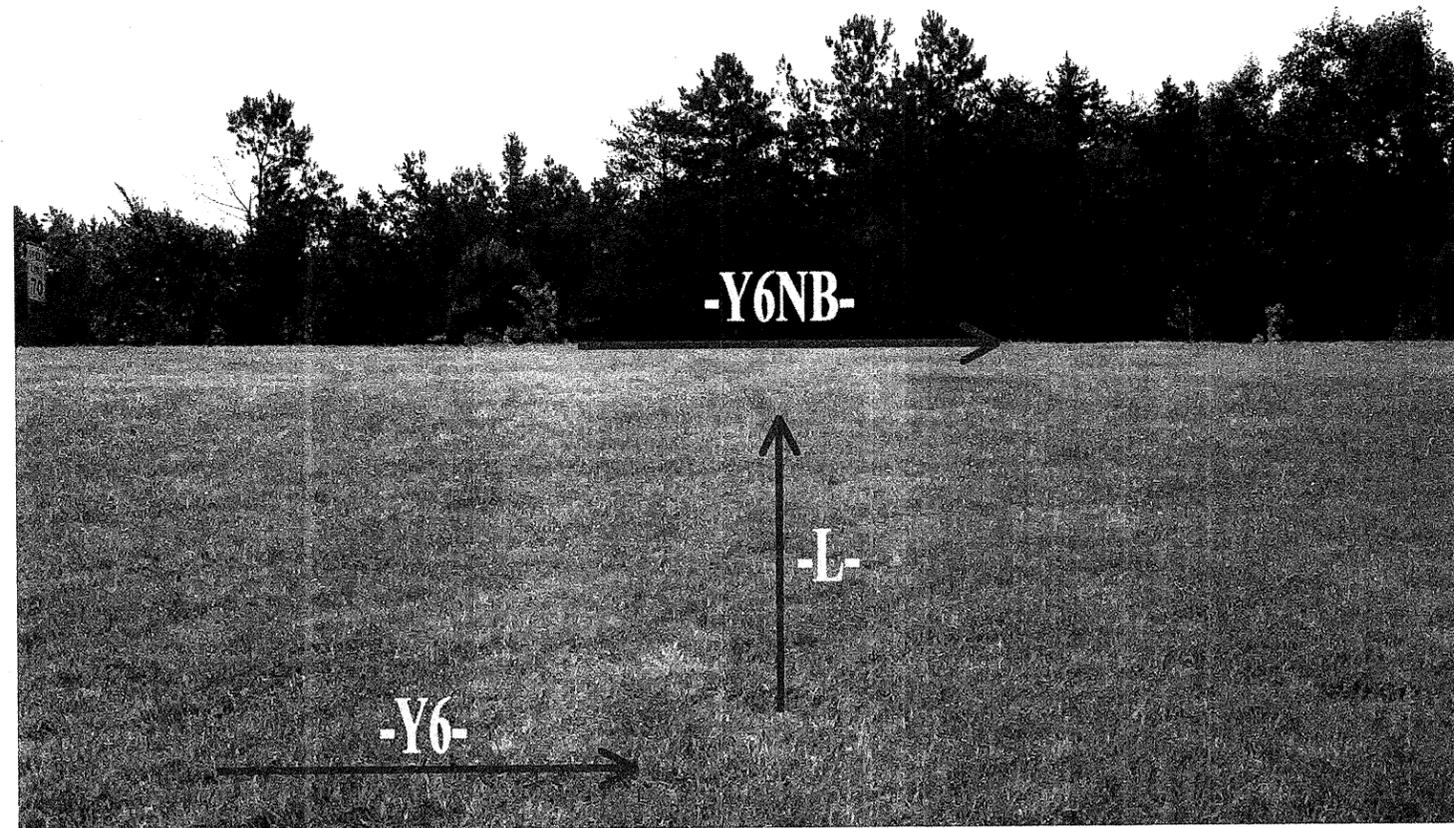


0.0 0.1 0.2 0.3 0.4 0.5 0.6



METERS

# ***SITE PHOTOGRAPH***



CONTRACT: ID: 34345.1.1

# STATE OF NORTH CAROLINA

DEPARTMENT OF TRANSPORTATION

DIVISION OF HIGHWAYS

GEOTECHNICAL UNIT

## STRUCTURE SUBSURFACE INVESTIGATION

### CONTENTS:

SHEET	DESCRIPTION
1	TITLE SHEET
2	LEGEND
3	STRUCTURE INVENTORY REPORT
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	STATE	STATE PROJECT REFERENCE NO.	SHEET NO.	TOTAL SHEETS
	N.C.	34345.1.1 (R-0609IA)	1	14
	STATE PROJ. NO.	F.A. PROJ. NO.	DESCRIPTION	
			P.E.	
			CONST.	

### CAUTION NOTICE

THE SUBSURFACE INFORMATION AND THE SUBSURFACE INVESTIGATION ON WHICH IT IS BASED WAS MADE FOR THE PURPOSE OF STUDY, PLANNING AND DESIGN AND NOT FOR CONSTRUCTION OR PAY PURPOSES. THE VARIOUS FIELD BORING LOGS, ROCK CORES, AND SOIL TEST DATA AVAILABLE MAY BE REVIEWED OR INSPECTED IN RALEIGH BY CONTACTING THE N.C. DEPARTMENT OF TRANSPORTATION, GEOTECHNICAL UNIT @ (919) 250-4088. NEITHER THE SUBSURFACE PLANS AND REPORTS, NOR THE FIELD BORING LOGS, ROCK CORES, OR SOIL TEST DATA IS PART OF THE CONTRACT.

GENERAL SOIL AND ROCK STRATA DESCRIPTIONS AND INDICATED BOUNDARIES ARE BASED ON A GEOTECHNICAL INTERPRETATION OF ALL AVAILABLE SUBSURFACE DATA AND MAY NOT NECESSARILY REFLECT THE ACTUAL SUBSURFACE CONDITIONS BETWEEN BORINGS OR BETWEEN SAMPLED STRATA WITHIN THE BOREHOLE, THE LABORATORY SAMPLE DATA AND THE IN SITU (IN-PLACE) TEST DATA CAN BE RELIED ON ONLY TO THE DEGREE OF RELIABILITY INHERENT IN THE STANDARD TEST METHOD. THE OBSERVED WATER LEVELS OR SOIL MOISTURE CONDITIONS INDICATED IN THE SUBSURFACE INVESTIGATIONS ARE AS RECORDED AT THE TIME OF THE INVESTIGATION. THESE WATER LEVELS OR SOIL MOISTURE CONDITIONS MAY VARY CONSIDERABLY WITH TIME ACCORDING TO CLIMATIC CONDITIONS INCLUDING TEMPERATURES, PRECIPITATION AND WIND, AS WELL AS OTHER NON-CLIMATIC FACTORS.

THE BIDDER OR CONTRACTOR IS CAUTIONED THAT DETAILS SHOWN ON THE SUBSURFACE PLANS ARE PRELIMINARY ONLY AND IN MANY CASES THE FINAL DESIGN DETAILS ARE DIFFERENT. FOR BIDDING AND CONSTRUCTION PURPOSES, REFER TO THE CONSTRUCTION PLANS AND DOCUMENTS FOR FINAL DESIGN INFORMATION ON THIS PROJECT. THE DEPARTMENT DOES NOT WARRANT OR GUARANTEE THE SUFFICIENCY OR ACCURACY OF THE INVESTIGATION MADE, NOR THE INTERPRETATIONS MADE OR OPINION OF THE DEPARTMENT AS TO THE TYPE OF MATERIALS AND CONDITIONS TO BE ENCOUNTERED. THE BIDDER OR CONTRACTOR IS CAUTIONED TO MAKE SUCH INDEPENDENT SUBSURFACE INVESTIGATIONS AS HE DEEMS NECESSARY TO SATISFY HIMSELF AS TO CONDITIONS TO BE ENCOUNTERED ON THIS PROJECT. THE CONTRACTOR SHALL HAVE NO CLAIM FOR ADDITIONAL COMPENSATION OR FOR AN EXTENSION OF TIME FOR ANY REASON RESULTING FROM THE ACTUAL CONDITIONS ENCOUNTERED AT THE SITE DIFFERING FROM THOSE INDICATED IN THE SUBSURFACE INFORMATION.

STATE PROJECT 34345.1.1 I.D. NO. R-0609IA

F.A. PROJECT MAF-F-119-1(1)

COUNTY GUILFORD

PROJECT DESCRIPTION US 311 HIGH  
POINT EAST BELTWAY FROM US 29-70  
TO I-85 NORTH OF ARCHDALE

SITE DESCRIPTION STRUCTURE NO. 8 ON  
-FLY- AT STA. 22+01.8 OVER -L-

INVESTIGATED BY T.P. MOOREFIELD PERSONNEL O.B. OTI

CHECKED BY D.N. ARGENBRIGHT C. A. HENU

SUBMITTED BY D.N. ARGENBRIGHT R.A. BRITTIAN

DATE AUGUST 2004

DRAWN BY: C.D.C.

NOTE - THE INFORMATION CONTAINED HEREIN IS NOT IMPLIED OR GUARANTEED BY THE N.C. DEPARTMENT OF TRANSPORTATION AS BEING ACCURATE NOR IT IS CONSIDERED TO BE PART OF THE PLANS, SPECIFICATIONS, OR CONTRACT FOR THE PROJECT.

NOTE - BY HAVING REQUESTED THIS INFORMATION THE CONTRACTOR SPECIFICALLY WAIVES ANY CLAIMS FOR INCREASED COMPENSATION OR EXTENSION OF TIME BASED ON DIFFERENCES BETWEEN THE CONDITIONS INDICATED HEREIN AND THE ACTUAL CONDITIONS AT THE PROJECT SITE.



*Handwritten signature/initials*

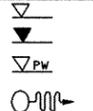
**NORTH CAROLINA DEPARTMENT OF TRANSPORTATION**  
**DIVISION OF HIGHWAYS**  
**GEOTECHNICAL UNIT**

ID	STATE PROJECT NO.	SHEET NO.	TOTAL SHEETS
R-0609IA	34345.1.I STRUCTURE NO. 8	2	14



**SUBSURFACE INVESTIGATION**

**SOIL AND ROCK LEGEND, TERMS, SYMBOLS, AND ABBREVIATIONS**

SOIL DESCRIPTION		GRADATION		ROCK DESCRIPTION		TERMS AND DEFINITIONS							
SOIL IS CONSIDERED TO BE THE UNCONSOLIDATED, SEMI-CONSOLIDATED OR WEATHERED EARTH MATERIALS WHICH CAN BE PENETRATED WITH A CONTINUOUS FLIGHT POWER AUGER, AND WHICH YIELDS LESS THAN 100 BLOWS PER 30 cm ACCORDING TO STANDARD PENETRATION TEST (ASTM D-1586). SOIL CLASSIFICATION IS BASED ON THE AASHTO SYSTEM AND BASIC DESCRIPTIONS GENERALLY SHALL INCLUDE: CONSISTENCY, COLOR, TEXTURE, MOISTURE, AASHTO CLASSIFICATION, AND OTHER PERTINENT FACTORS SUCH AS MINERALOGICAL COMPOSITION, ANGULARITY, STRUCTURE, PLASTICITY, ETC. EXAMPLE: <i>VERY STIFF, GRAY SILTY CLAY, MOST WITH INTERBEDDED FINE SAND LAYERS, HARDY PLASTIC, A-7-6</i>		WELL-GRADED- INDICATES A GOOD REPRESENTATION OF PARTICLE SIZES FROM FINE TO COARSE UNIFORM- INDICATES THAT SOIL PARTICLES ARE ALL APPROXIMATELY THE SAME SIZE (ALSO POORLY GRADED). GAP-GRADED- INDICATES A MIXTURE OF UNIFORM PARTICLES OF TWO OR MORE SIZES.		HARD ROCK IS NON-COASTAL PLAIN MATERIAL THAT WHEN TESTED, WOULD YIELD SPT REFUSAL, AN INFERRED ROCK LINE INDICATES THE LEVEL AT WHICH NON-COASTAL PLAIN MATERIAL WOULD YIELD SPT REFUSAL. SPT REFUSAL IS PENETRATION BY A SPLIT SPOON SAMPLER EQUAL TO OR LESS THAN 2.5 cm PER 50 BLOWS. IN NON-COASTAL PLAIN MATERIAL, THE TRANSITION BETWEEN SOIL AND ROCK IS OFTEN REPRESENTED BY A ZONE OF WEATHERED ROCK. ROCK MATERIALS ARE TYPICALLY DIVIDED AS FOLLOWS:  NON-COASTAL PLAIN MATERIAL THAT YIELDS SPT N VALUES > 100 BLOWS PER 30 cm.  FINE TO COARSE GRAIN IGNEOUS AND METAMORPHIC ROCK THAT WOULD YIELD SPT REFUSAL IF TESTED. ROCK TYPE INCLUDES GRANITE, GNEISS, GABBRO, SCHIST, ETC.  FINE TO COARSE GRAIN METAMORPHIC AND NON-COASTAL PLAIN SEDIMENTARY ROCK THAT WOULD YIELD SPT REFUSAL IF TESTED. ROCK TYPE INCLUDES PHYLLITE, SLATE, SANDSTONE, ETC.  COASTAL PLAIN SEDIMENTS CEMENTED INTO ROCK, BUT MAY NOT YIELD SPT REFUSAL. ROCK TYPE INCLUDES LIMESTONE, SANDSTONE, CEMENTED SHELL BEDS, ETC.		ALLUVIUM (ALLUV.) - SOILS WHICH HAVE BEEN TRANSPORTED BY WATER. AQUIFER - A WATER BEARING FORMATION OR STRATA. ARENACEOUS - APPLIED TO ROCKS THAT HAVE BEEN DERIVED FROM SAND OR THAT CONTAIN SAND. ARGILLACEOUS - APPLIED TO ALL ROCKS OR SUBSTANCES COMPOSED OF CLAY MINERALS, OR HAVING A NOTABLE PROPORTION OF CLAY IN THEIR COMPOSITION, AS SHALE, SLATE, ETC. ARTESIAN - GROUND WATER THAT IS UNDER SUFFICIENT PRESSURE TO RISE ABOVE THE LEVEL AT WHICH IS ENCOUNTERED, BUT WHICH DOES NOT NECESSARILY RISE TO OR ABOVE THE GROUND SURFACE. CALCAREOUS (CALC.) - SOILS WHICH CONTAIN APPRECIABLE AMOUNTS OF CALCIUM CARBONATE. COLLUVIUM - ROCK FRAGMENTS MIXED WITH SOIL DEPOSITED BY GRAVITY ON SLOPE OR AT BOTTOM OF SLOPE. CORE RECOVERY (REC.) - TOTAL LENGTH OF ALL MATERIAL RECOVERED IN THE CORE BARREL DIVIDED BY TOTAL LENGTH OF CORE RUN AND EXPRESSED AS A PERCENTAGE. DIKE - A TABULAR BODY OF IGNEOUS ROCK THAT CUTS ACROSS THE STRUCTURE OF ADJACENT ROCKS OR CUTS MASSIVE ROCK. DIP - THE ANGLE AT WHICH A STRATUM OR ANY PLANAR FEATURE IS INCLINED FROM THE HORIZONTAL. DIP DIRECTION (DIP AZIMUTH) - THE DIRECTION OR BEARING OF THE HORIZONTAL TRACE OF THE LINE OF DIP, MEASURED CLOCKWISE FROM NORTH. FAULT - A FRACTURE OR FRACTURE ZONE ALONG WHICH THERE HAS BEEN DISPLACEMENT OF THE SIDES RELATIVE TO ONE ANOTHER PARALLEL TO THE FRACTURE. FISSILE - A PROPERTY OF SPLITTING ALONG CLOSELY SPACED PARALLEL PLANES. FLOAT - ROCK FRAGMENTS ON SURFACE NEAR THEIR ORIGINAL POSITION AND DISLOADED FROM PARENT MATERIAL. FLOOD PLAIN (F.P.) - LAND BORDERING A STREAM, BUILT OF SEDIMENTS DEPOSITED BY THE STREAM. FORMATION (FM.) - A MAPPABLE GEOLOGIC UNIT THAT CAN BE RECOGNIZED AND TRACED IN THE FIELD. JOINT - FRACTURE IN ROCK ALONG WHICH NO APPRECIABLE MOVEMENT HAS OCCURRED. LEDGE - A SHELF-LIKE RIDGE OR PROJECTION OF ROCK WHOSE THICKNESS IS SMALL COMPARED TO ITS LATERAL EXTENT. LENS - A BODY OF SOIL OR ROCK THAT THINS OUT IN ONE OR MORE DIRECTIONS. MOTTLED (MOT.) - IRREGULARLY MARKED WITH SPOTS OF DIFFERENT COLORS, MOTTLING IN SOILS USUALLY INDICATES POOR AERATION AND LACK OF GOOD DRAINAGE. PERCHED WATER - WATER MAINTAINED ABOVE THE NORMAL GROUND WATER LEVEL BY THE PRESENCE OF AN INTERVENING IMPERVIOUS STRATUM. RESIDUAL SOIL - SOIL FORMED IN PLACE BY THE WEATHERING OF ROCK. ROCK QUALITY DESIGNATION (R.Q.D.) - A MEASURE OF ROCK QUALITY DESCRIBED BY TOTAL LENGTH OF ROCK SEGMENTS EQUAL TO OR GREATER THAN 10 CENTIMETERS DIVIDED BY THE TOTAL LENGTH OF CORE RUN AND EXPRESSED AS A PERCENTAGE. SAPROLITE (SAP.) - RESIDUAL SOIL WHICH RETAINS THE RELIC STRUCTURE OR FABRIC OF THE PARENT ROCK. SILL - AN INTRUSIVE BODY OF IGNEOUS ROCK OF APPROXIMATELY UNIFORM THICKNESS AND RELATIVELY THIN COMPARED WITH ITS LATERAL EXTENT, WHICH HAS BEEN EMPLACED PARALLEL TO THE BEDDING OR SCHISTOSITY OF THE INTRUDED ROCKS. SLICKENSIDE - POLISHED AND STRIATED SURFACE THAT RESULTS FROM FRICTION ALONG A FAULT OR SLIP PLANE. STANDARD PENETRATION TEST (PENETRATION RESISTANCE) (SPT) - NUMBER OF BLOWS (N) OF A 63.5 kg HAMMER FALLING 0.76 METERS REQUIRED TO PRODUCE A PENETRATION OF 30 cm INTO SOIL WITH A 5 cm OUTSIDE DIAMETER SPLIT SPOON SAMPLER. SPT REFUSAL IS LESS THAN 2.5 cm PENETRATION WITH 50 BLOWS. STRATA CORE RECOVERY (SREC.) - TOTAL LENGTH OF STRATA MATERIAL RECOVERED DIVIDED BY TOTAL LENGTH OF STRATUM AND EXPRESSED AS A PERCENTAGE. STRATA ROCK QUALITY DESIGNATION (S.R.Q.D.) - A MEASURE OF ROCK QUALITY DESCRIBED BY TOTAL LENGTH OF ROCK SEGMENTS WITHIN A STRATUM EQUAL TO OR GREATER THAN 10 CENTIMETERS DIVIDED BY THE TOTAL LENGTH OF STRATA AND EXPRESSED AS A PERCENTAGE. TOPSOIL (T.S.) - SURFACE SOILS USUALLY CONTAINING ORGANIC MATTER.							
<b>SOIL LEGEND AND AASHTO CLASSIFICATION</b>		<b>MINERALOGICAL COMPOSITION</b>		<b>WEATHERING</b>		<b>TERMS AND DEFINITIONS</b>							
GENERAL CLASS. GRANULAR MATERIALS (<35% PASSING #200) SILT-CLAY MATERIALS (>35% PASSING #200) ORGANIC MATERIALS		MINERAL NAMES SUCH AS QUARTZ, FELDSPAR, MICA, TALC, KAOLIN, ETC. ARE USED IN DESCRIPTIONS WHENEVER THEY ARE CONSIDERED OF SIGNIFICANCE.		FRESH ROCK FRESH, CRYSTALS BRIGHT, FEW JOINTS MAY SHOW SLIGHT STAINING, ROCK RINGS UNDER HAMMER IF CRYSTALLINE. VERY SLIGHT (V, SL.) ROCK GENERALLY FRESH, JOINTS STAINED, SOME JOINTS MAY SHOW THIN CLAY COATINGS IF OPEN, CRYSTALS ON A BROKEN SPECIMEN FACE SHINE BRIGHTLY, ROCK RINGS UNDER HAMMER BLOWS IF OF A CRYSTALLINE NATURE. SLIGHT (SL.) ROCK GENERALLY FRESH, JOINTS STAINED AND DISCOLORATION EXTENDS INTO ROCK UP TO 2.5 cm. OPEN JOINTS MAY CONTAIN CLAY. IN GRANITOID ROCKS SOME OCCASIONAL FELDSPAR CRYSTALS ARE DULL AND DISCOLORED, CRYSTALLINE ROCKS RING UNDER HAMMER BLOWS. MODERATE (MOD.) SIGNIFICANT PORTIONS OF ROCK SHOW DISCOLORATION AND WEATHERING EFFECTS. IN GRANITOID ROCKS, MOST FELDSPARS ARE DULL AND DISCOLORED, SOME SHOW CLAY. ROCK HAS DULL SOUND UNDER HAMMER BLOWS AND SHOWS SIGNIFICANT LOSS OF STRENGTH AS COMPARED WITH FRESH ROCK. MODERATELY SEVERE (MOD. SEV.) ALL ROCK EXCEPT QUARTZ DISCOLORED OR STAINED, IN GRANITOID ROCKS, ALL FELDSPARS DULL AND DISCOLORED AND A MAJORITY SHOW KAOLINIZATION, ROCK SHOWS SEVERE LOSS OF STRENGTH AND CAN BE EXCAVATED WITH A GEOLOGIST'S PICK, ROCK GIVES "CLUNK" SOUND WHEN STRUCK. <i>IF TESTED, WOULD YIELD SPT REFUSAL</i> SEVERE (SEV.) ALL ROCKS EXCEPT QUARTZ DISCOLORED OR STAINED, ROCK FABRIC CLEAR AND EVIDENT BUT REDUCED IN STRENGTH TO STRONG SOIL. IN GRANITOID ROCKS ALL FELDSPARS ARE KAOLINIZED TO SOME EXTENT, SOME FRAGMENTS OF STRONG ROCK USUALLY REMAIN. <i>IF TESTED, YIELDS SPT N VALUES &gt; 100 BLOWS PER 30 cm</i> VERY SEVERE (V, SEV.) ALL ROCK EXCEPT QUARTZ DISCOLORED OR STAINED, ROCK FABRIC ELEMENTS ARE DISCERNIBLE BUT THE MASS IS EFFECTIVELY REDUCED TO SOIL STATUS, WITH ONLY FRAGMENTS OF STRONG ROCK REMAINING. SAPROLITE IS AN EXAMPLE OF ROCK WEATHERED TO A DEGREE SUCH THAT ONLY MINOR VESTIGES OF THE ORIGINAL ROCK FABRIC REMAIN. <i>IF TESTED, YIELDS SPT N VALUES &lt; 100 BLOWS PER 30 cm</i> COMPLETE ROCK REDUCED TO SOIL, ROCK FABRIC NOT DISCERNIBLE, OR DISCERNIBLE ONLY IN SMALL AND SCATTERED CONCENTRATIONS. QUARTZ MAY BE PRESENT AS DIKES OR STRINGERS, SAPROLITE IS ALSO AN EXAMPLE.		<b>COMPRESSION</b> SLIGHTLY COMPRESSIBLE LIQUID LIMIT LESS THAN 30 MODERATELY COMPRESSIBLE LIQUID LIMIT 31-50 HIGHLY COMPRESSIBLE LIQUID LIMIT GREATER THAN 50		<b>PERCENTAGE OF MATERIAL</b> ORGANIC MATERIAL GRANULAR SOILS SILT-CLAY SOILS OTHER MATERIAL TRACE OF ORGANIC MATTER 2 - 3% 3 - 5% TRACE 1 - 10% LITTLE ORGANIC MATTER 3 - 5% 5 - 12% LITTLE 10 - 20% MODERATELY ORGANIC 5 - 10% 12 - 20% SOME 20 - 35% HIGHLY ORGANIC >10% >20% HIGHLY 35% AND ABOVE		<b>GROUND WATER</b>  WATER LEVEL IN BORE HOLE IMMEDIATELY AFTER DRILLING. STATIC WATER LEVEL AFTER 24 HOURS. PERCHED WATER, SATURATED ZONE OR WATER BEARING STRATA SPRING OR SEEPAGE		<b>WEATHERING</b>	
<b>CONSISTENCY OR DENSITY</b>		<b>MISCELLANEOUS SYMBOLS</b>		<b>WEATHERING</b>		<b>TERMS AND DEFINITIONS</b>							
PRIMARY SOIL TYPE COMPACTNESS OR CONSISTENCY RANGE OF STANDARD PENETRATION RESISTANCE (N-VALUE) RANGE OF UNCONFINED COMPRESSIVE STRENGTH (kN/m <sup>2</sup> )		ROADWAY EMBANKMENT WITH SOIL DESCRIPTION SOIL SYMBOL ARTIFICIAL FILL OTHER THAN ROADWAY EMBANKMENTS INFERRED SOIL BOUNDARIES INFERRED ROCK LINE ALLUVIAL SOIL BOUNDARY DIP/DIP DIRECTION OF ROCK STRUCTURES SOUNDING ROD		SPT TEST BORING AUGER BORING CORE BORING MONITORING WELL PIEZOMETER INSTALLATION SLOPE INDICATOR INSTALLATION SPT N-VALUE SPT REFUSAL		FRESH VERY SLIGHT (V, SL.) SLIGHT (SL.) MODERATE (MOD.) MODERATELY SEVERE (MOD. SEV.) SEVERE (SEV.) VERY SEVERE (V, SEV.) COMPLETE		<b>GROUND WATER</b>					
<b>TEXTURE OR GRAIN SIZE</b>		<b>ABBREVIATIONS</b>		<b>WEATHERING</b>		<b>TERMS AND DEFINITIONS</b>							
U.S. STD. SIEVE SIZE OPENING (mm) 4 10 40 60 200 270 4.76 2.0 0.42 0.25 0.075 0.053		AR - AUGER REFUSAL BT - BORING TERMINATED CL - CLAY CPT - CONE PENETRATION TEST CSE - COARSE DMT - DILATOMETER TEST DPT - DYNAMIC PENETRATION TEST e - VOID RATIO F - FINE FOSS - FOSSILIFEROUS FRAC - FRACTURED FRAGS - FRAGMENTS MED - MEDIUM PMT - PRESSUREMETER TEST SD - SAND, SANDY SL - SILT, SILTY SLI - SLIGHTLY TCR - TRICONE REFUSAL U - UNIT WEIGHT U <sub>d</sub> - DRY UNIT WEIGHT w - MOISTURE CONTENT V - VERY VST - VANE SHEAR TEST		VERY HARD CANNOT BE SCRATCHED BY KNIFE OR SHARP PICK, BREAKING OF HAND SPECIMENS REQUIRES SEVERAL HARD BLOWS OF THE GEOLOGIST'S PICK. HARD CAN BE SCRATCHED BY KNIFE OR PICK ONLY WITH DIFFICULTY, HARD HAMMER BLOWS REQUIRED TO DETACH HAND SPECIMEN. MODERATELY HARD CAN BE SCRATCHED BY KNIFE OR PICK, GOUGES OR GROOVES TO 6 mm DEEP CAN BE EXCAVATED BY HARD BLOW OF A GEOLOGIST'S PICK, HAND SPECIMENS CAN BE DETACHED BY MODERATE BLOWS. MEDIUM HARD CAN BE GROOVED OR GOUGED 1 mm DEEP BY FIRM PRESSURE OF KNIFE OR PICK POINT. CAN BE EXCAVATED IN SMALL CHIPS TO PIECES 25 mm MAXIMUM SIZE BY HARD BLOWS OF THE POINT OF A GEOLOGIST'S PICK. SOFT CAN BE GROOVED OR GOUGED READILY BY KNIFE OR PICK, CAN BE EXCAVATED IN FRAGMENTS FROM CHIPS TO SEVERAL CENTIMETERS IN SIZE BY MODERATE BLOWS OF A PICK POINT, SMALL, THIN PIECES CAN BE BROKEN BY FINGER PRESSURE. VERY SOFT CAN BE CARVED WITH KNIFE, CAN BE EXCAVATED READILY WITH POINT OF PICK, PIECES 25 mm OR MORE IN THICKNESS CAN BE BROKEN BY FINGER PRESSURE, CAN BE SCRATCHED READILY BY FINGER NAIL.		<b>TEXTURE OR GRAIN SIZE</b>		<b>TERMS AND DEFINITIONS</b>					
BOULDER (BLDR.) COBBLE (COB.) GRAVEL (GR.) COARSE SAND (CSE, SD.) FINE SAND (F, SD.) SILT (SL.) CLAY (CL.)		ROADWAY EMBANKMENT WITH SOIL DESCRIPTION SOIL SYMBOL ARTIFICIAL FILL OTHER THAN ROADWAY EMBANKMENTS INFERRED SOIL BOUNDARIES INFERRED ROCK LINE ALLUVIAL SOIL BOUNDARY DIP/DIP DIRECTION OF ROCK STRUCTURES SOUNDING ROD		SPT TEST BORING AUGER BORING CORE BORING MONITORING WELL PIEZOMETER INSTALLATION SLOPE INDICATOR INSTALLATION SPT N-VALUE SPT REFUSAL		VERY HARD CANNOT BE SCRATCHED BY KNIFE OR SHARP PICK, BREAKING OF HAND SPECIMENS REQUIRES SEVERAL HARD BLOWS OF THE GEOLOGIST'S PICK. HARD CAN BE SCRATCHED BY KNIFE OR PICK ONLY WITH DIFFICULTY, HARD HAMMER BLOWS REQUIRED TO DETACH HAND SPECIMEN. MODERATELY HARD CAN BE SCRATCHED BY KNIFE OR PICK, GOUGES OR GROOVES TO 6 mm DEEP CAN BE EXCAVATED BY HARD BLOW OF A GEOLOGIST'S PICK, HAND SPECIMENS CAN BE DETACHED BY MODERATE BLOWS. MEDIUM HARD CAN BE GROOVED OR GOUGED 1 mm DEEP BY FIRM PRESSURE OF KNIFE OR PICK POINT. CAN BE EXCAVATED IN SMALL CHIPS TO PIECES 25 mm MAXIMUM SIZE BY HARD BLOWS OF THE POINT OF A GEOLOGIST'S PICK. SOFT CAN BE GROOVED OR GOUGED READILY BY KNIFE OR PICK, CAN BE EXCAVATED IN FRAGMENTS FROM CHIPS TO SEVERAL CENTIMETERS IN SIZE BY MODERATE BLOWS OF A PICK POINT, SMALL, THIN PIECES CAN BE BROKEN BY FINGER PRESSURE. VERY SOFT CAN BE CARVED WITH KNIFE, CAN BE EXCAVATED READILY WITH POINT OF PICK, PIECES 25 mm OR MORE IN THICKNESS CAN BE BROKEN BY FINGER PRESSURE, CAN BE SCRATCHED READILY BY FINGER NAIL.		<b>TEXTURE OR GRAIN SIZE</b>					
<b>SOIL MOISTURE - CORRELATION OF TERMS</b>		<b>ABBREVIATIONS</b>		<b>WEATHERING</b>		<b>TERMS AND DEFINITIONS</b>							
SOIL MOISTURE SCALE (ATTERBERG LIMITS) FIELD MOISTURE DESCRIPTION GUIDE FOR FIELD MOISTURE DESCRIPTION		AR - AUGER REFUSAL BT - BORING TERMINATED CL - CLAY CPT - CONE PENETRATION TEST CSE - COARSE DMT - DILATOMETER TEST DPT - DYNAMIC PENETRATION TEST e - VOID RATIO F - FINE FOSS - FOSSILIFEROUS FRAC - FRACTURED FRAGS - FRAGMENTS MED - MEDIUM PMT - PRESSUREMETER TEST SD - SAND, SANDY SL - SILT, SILTY SLI - SLIGHTLY TCR - TRICONE REFUSAL U - UNIT WEIGHT U <sub>d</sub> - DRY UNIT WEIGHT w - MOISTURE CONTENT V - VERY VST - VANE SHEAR TEST		VERY HARD CANNOT BE SCRATCHED BY KNIFE OR SHARP PICK, BREAKING OF HAND SPECIMENS REQUIRES SEVERAL HARD BLOWS OF THE GEOLOGIST'S PICK. HARD CAN BE SCRATCHED BY KNIFE OR PICK ONLY WITH DIFFICULTY, HARD HAMMER BLOWS REQUIRED TO DETACH HAND SPECIMEN. MODERATELY HARD CAN BE SCRATCHED BY KNIFE OR PICK, GOUGES OR GROOVES TO 6 mm DEEP CAN BE EXCAVATED BY HARD BLOW OF A GEOLOGIST'S PICK, HAND SPECIMENS CAN BE DETACHED BY MODERATE BLOWS. MEDIUM HARD CAN BE GROOVED OR GOUGED 1 mm DEEP BY FIRM PRESSURE OF KNIFE OR PICK POINT. CAN BE EXCAVATED IN SMALL CHIPS TO PIECES 25 mm MAXIMUM SIZE BY HARD BLOWS OF THE POINT OF A GEOLOGIST'S PICK. SOFT CAN BE GROOVED OR GOUGED READILY BY KNIFE OR PICK, CAN BE EXCAVATED IN FRAGMENTS FROM CHIPS TO SEVERAL CENTIMETERS IN SIZE BY MODERATE BLOWS OF A PICK POINT, SMALL, THIN PIECES CAN BE BROKEN BY FINGER PRESSURE. VERY SOFT CAN BE CARVED WITH KNIFE, CAN BE EXCAVATED READILY WITH POINT OF PICK, PIECES 25 mm OR MORE IN THICKNESS CAN BE BROKEN BY FINGER PRESSURE, CAN BE SCRATCHED READILY BY FINGER NAIL.		<b>TEXTURE OR GRAIN SIZE</b>							
LL LIQUID LIMIT PL PLASTIC LIMIT DM OPTIMUM MOISTURE SL SHRINKAGE LIMIT		AR - AUGER REFUSAL BT - BORING TERMINATED CL - CLAY CPT - CONE PENETRATION TEST CSE - COARSE DMT - DILATOMETER TEST DPT - DYNAMIC PENETRATION TEST e - VOID RATIO F - FINE FOSS - FOSSILIFEROUS FRAC - FRACTURED FRAGS - FRAGMENTS MED - MEDIUM PMT - PRESSUREMETER TEST SD - SAND, SANDY SL - SILT, SILTY SLI - SLIGHTLY TCR - TRICONE REFUSAL U - UNIT WEIGHT U <sub>d</sub> - DRY UNIT WEIGHT w - MOISTURE CONTENT V - VERY VST - VANE SHEAR TEST		VERY HARD CANNOT BE SCRATCHED BY KNIFE OR SHARP PICK, BREAKING OF HAND SPECIMENS REQUIRES SEVERAL HARD BLOWS OF THE GEOLOGIST'S PICK. HARD CAN BE SCRATCHED BY KNIFE OR PICK ONLY WITH DIFFICULTY, HARD HAMMER BLOWS REQUIRED TO DETACH HAND SPECIMEN. MODERATELY HARD CAN BE SCRATCHED BY KNIFE OR PICK, GOUGES OR GROOVES TO 6 mm DEEP CAN BE EXCAVATED BY HARD BLOW OF A GEOLOGIST'S PICK, HAND SPECIMENS CAN BE DETACHED BY MODERATE BLOWS. MEDIUM HARD CAN BE GROOVED OR GOUGED 1 mm DEEP BY FIRM PRESSURE OF KNIFE OR PICK POINT. CAN BE EXCAVATED IN SMALL CHIPS TO PIECES 25 mm MAXIMUM SIZE BY HARD BLOWS OF THE POINT OF A GEOLOGIST'S PICK. SOFT CAN BE GROOVED OR GOUGED READILY BY KNIFE OR PICK, CAN BE EXCAVATED IN FRAGMENTS FROM CHIPS TO SEVERAL CENTIMETERS IN SIZE BY MODERATE BLOWS OF A PICK POINT, SMALL, THIN PIECES CAN BE BROKEN BY FINGER PRESSURE. VERY SOFT CAN BE CARVED WITH KNIFE, CAN BE EXCAVATED READILY WITH POINT OF PICK, PIECES 25 mm OR MORE IN THICKNESS CAN BE BROKEN BY FINGER PRESSURE, CAN BE SCRATCHED READILY BY FINGER NAIL.		<b>TEXTURE OR GRAIN SIZE</b>							
<b>PLASTICITY</b>		<b>ABBREVIATIONS</b>		<b>WEATHERING</b>		<b>TERMS AND DEFINITIONS</b>							
NONPLASTIC LOW PLASTICITY MED. PLASTICITY HIGH PLASTICITY		AR - AUGER REFUSAL BT - BORING TERMINATED CL - CLAY CPT - CONE PENETRATION TEST CSE - COARSE DMT - DILATOMETER TEST DPT - DYNAMIC PENETRATION TEST e - VOID RATIO F - FINE FOSS - FOSSILIFEROUS FRAC - FRACTURED FRAGS - FRAGMENTS MED - MEDIUM PMT - PRESSUREMETER TEST SD - SAND, SANDY SL - SILT, SILTY SLI - SLIGHTLY TCR - TRICONE REFUSAL U - UNIT WEIGHT U <sub>d</sub> - DRY UNIT WEIGHT w - MOISTURE CONTENT V - VERY VST - VANE SHEAR TEST		VERY HARD CANNOT BE SCRATCHED BY KNIFE OR SHARP PICK, BREAKING OF HAND SPECIMENS REQUIRES SEVERAL HARD BLOWS OF THE GEOLOGIST'S PICK. HARD CAN BE SCRATCHED BY KNIFE OR PICK ONLY WITH DIFFICULTY, HARD HAMMER BLOWS REQUIRED TO DETACH HAND SPECIMEN. MODERATELY HARD CAN BE SCRATCHED BY KNIFE OR PICK, GOUGES OR GROOVES TO 6 mm DEEP CAN BE EXCAVATED BY HARD BLOW OF A GEOLOGIST'S PICK, HAND SPECIMENS CAN BE DETACHED BY MODERATE BLOWS. MEDIUM HARD CAN BE GROOVED OR GOUGED 1 mm DEEP BY FIRM PRESSURE OF KNIFE OR PICK POINT. CAN BE EXCAVATED IN SMALL CHIPS TO PIECES 25 mm MAXIMUM SIZE BY HARD BLOWS OF THE POINT OF A GEOLOGIST'S PICK. SOFT CAN BE GROOVED OR GOUGED READILY BY KNIFE OR PICK, CAN BE EXCAVATED IN FRAGMENTS FROM CHIPS TO SEVERAL CENTIMETERS IN SIZE BY MODERATE BLOWS OF A PICK POINT, SMALL, THIN PIECES CAN BE BROKEN BY FINGER PRESSURE. VERY SOFT CAN BE CARVED WITH KNIFE, CAN BE EXCAVATED READILY WITH POINT OF PICK, PIECES 25 mm OR MORE IN THICKNESS CAN BE BROKEN BY FINGER PRESSURE, CAN BE SCRATCHED READILY BY FINGER NAIL.		<b>TEXTURE OR GRAIN SIZE</b>							
<b>COLOR</b>		<b>ABBREVIATIONS</b>		<b>WEATHERING</b>		<b>TERMS AND DEFINITIONS</b>							
DESCRIPTIONS MAY INCLUDE COLOR OR COLOR COMBINATIONS (TAN, RED, YEL-BRN, BLUE-GRAY) MODIFIERS SUCH AS LIGHT, DARK, STREAKED, ETC. ARE USED TO DESCRIBE APPEARANCE.		AR - AUGER REFUSAL BT - BORING TERMINATED CL - CLAY CPT - CONE PENETRATION TEST CSE - COARSE DMT - DILATOMETER TEST DPT - DYNAMIC PENETRATION TEST e - VOID RATIO F - FINE FOSS - FOSSILIFEROUS FRAC - FRACTURED FRAGS - FRAGMENTS MED - MEDIUM PMT - PRESSUREMETER TEST SD - SAND, SANDY SL - SILT, SILTY SLI - SLIGHTLY TCR - TRICONE REFUSAL U - UNIT WEIGHT U <sub>d</sub> - DRY UNIT WEIGHT w - MOISTURE CONTENT V - VERY VST - VANE SHEAR TEST		VERY HARD CANNOT BE SCRATCHED BY KNIFE OR SHARP PICK, BREAKING OF HAND SPECIMENS REQUIRES SEVERAL HARD BLOWS OF THE GEOLOGIST'S PICK. HARD CAN BE SCRATCHED BY KNIFE OR PICK ONLY WITH DIFFICULTY, HARD HAMMER BLOWS REQUIRED TO DETACH HAND SPECIMEN. MODERATELY HARD CAN BE SCRATCHED BY KNIFE OR PICK, GOUGES OR GROOVES TO 6 mm DEEP CAN BE EXCAVATED BY HARD BLOW OF A GEOLOGIST'S PICK, HAND SPECIMENS CAN BE DETACHED BY MODERATE BLOWS. MEDIUM HARD CAN BE GROOVED OR GOUGED 1 mm DEEP BY FIRM PRESSURE OF KNIFE OR PICK POINT. CAN BE EXCAVATED IN SMALL CHIPS TO PIECES 25 mm MAXIMUM SIZE BY HARD BLOWS OF THE POINT OF A GEOLOGIST'S PICK. SOFT CAN BE GROOVED OR GOUGED READILY BY KNIFE OR PICK, CAN BE EXCAVATED IN FRAGMENTS FROM CHIPS TO SEVERAL CENTIMETERS IN SIZE BY MODERATE BLOWS OF A PICK POINT, SMALL, THIN PIECES CAN BE BROKEN BY FINGER PRESSURE. VERY SOFT CAN BE CARVED WITH KNIFE, CAN BE EXCAVATED READILY WITH POINT OF PICK, PIECES 25 mm OR MORE IN THICKNESS CAN BE BROKEN BY FINGER PRESSURE, CAN BE SCRATCHED READILY BY FINGER NAIL.		<b>TEXTURE OR GRAIN SIZE</b>							
<b>EQUIPMENT USED ON SUBJECT PROJECT</b>		<b>ABBREVIATIONS</b>		<b>WEATHERING</b>		<b>TERMS AND DEFINITIONS</b>							
DRILL UNITS: [X] MOBILE B-57 [ ] BK-51 [ ] CME-45C [ ] CME-550 [ ] PORTABLE HOIST [ ] OTHER [ ] OTHER		ADVANCING TOOLS: [ ] CLAY BITS [ ] 152 mm CONTINUOUS FLIGHT AUGER [X] 203 mm HOLLOW AUGERS [ ] HARD FACED FINGER BITS [ ] TUNG-CARBIDE INSERTS [ ] CASING [ ] w/ ADVANCER [ ] TRICONE mm STEEL TEETH [ ] TRICONE mm TUNG-CARB. [X] CORE BIT [ ] OTHER		HAMMER TYPE: [ ] AUTOMATIC [X] MANUAL CORE SIZE: [ ] B [X] NXML [ ] H HAND TOOLS: [ ] POST HOLE DIGGER [ ] HAND AUGER [ ] SOUNDING ROD [ ] VANE SHEAR TEST [ ] OTHER		VERY HARD CANNOT BE SCRATCHED BY KNIFE OR SHARP PICK, BREAKING OF HAND SPECIMENS REQUIRES SEVERAL HARD BLOWS OF THE GEOLOGIST'S PICK. HARD CAN BE SCRATCHED BY KNIFE OR PICK ONLY WITH DIFFICULTY, HARD HAMMER BLOWS REQUIRED TO DETACH HAND SPECIMEN. MODERATELY HARD CAN BE SCRATCHED BY KNIFE OR PICK, GOUGES OR GROOVES TO 6 mm DEEP CAN BE EXCAVATED BY HARD BLOW OF A GEOLOGIST'S PICK, HAND SPECIMENS CAN BE DETACHED BY MODERATE BLOWS. MEDIUM HARD CAN BE GROOVED OR GOUGED 1 mm DEEP BY FIRM PRESSURE OF KNIFE OR PICK POINT. CAN BE EXCAVATED IN SMALL CHIPS TO PIECES 25 mm MAXIMUM SIZE BY HARD BLOWS OF THE POINT OF A GEOLOGIST'S PICK. SOFT CAN BE GROOVED OR GOUGED READILY BY KNIFE OR PICK, CAN BE EXCAVATED IN FRAGMENTS FROM CHIPS TO SEVERAL CENTIMETERS IN SIZE BY MODERATE BLOWS OF A PICK POINT, SMALL, THIN PIECES CAN BE BROKEN BY FINGER PRESSURE. VERY SOFT CAN BE CARVED WITH KNIFE, CAN BE EXCAVATED READILY WITH POINT OF PICK, PIECES 25 mm OR MORE IN THICKNESS CAN BE BROKEN BY FINGER PRESSURE, CAN BE SCRATCHED READILY BY FINGER NAIL.		<b>TEXTURE OR GRAIN SIZE</b>					
<b>FRACTURE SPACING</b>		<b>ABBREVIATIONS</b>		<b>WEATHERING</b>		<b>TERMS AND DEFINITIONS</b>							
TERM SPACING VERY WIDE MORE THAN 3 m WIDE 1 TO 3 m MODERATELY CLOSE 30 TO 100 cm CLOSE 5 TO 30 cm VERY CLOSE LESS THAN 5 cm		AR - AUGER REFUSAL BT - BORING TERMINATED CL - CLAY CPT - CONE PENETRATION TEST CSE - COARSE DMT - DILATOMETER TEST DPT - DYNAMIC PENETRATION TEST e - VOID RATIO F - FINE FOSS - FOSSILIFEROUS FRAC - FRACTURED FRAGS - FRAGMENTS MED - MEDIUM PMT - PRESSUREMETER TEST SD - SAND, SANDY SL - SILT, SILTY SLI - SLIGHTLY TCR - TRICONE REFUSAL U - UNIT WEIGHT U <sub>d</sub> - DRY UNIT WEIGHT w - MOISTURE CONTENT V - VERY VST - VANE SHEAR TEST		VERY HARD CANNOT BE SCRATCHED BY KNIFE OR SHARP PICK, BREAKING OF HAND SPECIMENS REQUIRES SEVERAL HARD BLOWS OF THE GEOLOGIST'S PICK. HARD CAN BE SCRATCHED BY KNIFE OR PICK ONLY WITH DIFFICULTY, HARD HAMMER BLOWS REQUIRED TO DETACH HAND SPECIMEN. MODERATELY HARD CAN BE SCRATCHED BY KNIFE OR PICK, GOUGES OR GROOVES TO 6 mm DEEP CAN BE EXCAVATED BY HARD BLOW OF A GEOLOGIST'S PICK, HAND SPECIMENS CAN BE DETACHED BY MODERATE BLOWS. MEDIUM HARD CAN BE GROOVED OR GOUGED 1 mm DEEP BY FIRM PRESSURE OF KNIFE OR PICK POINT. CAN BE EXCAVATED IN SMALL CHIPS TO PIECES 25 mm MAXIMUM SIZE BY HARD BLOWS OF THE POINT OF A GEOLOGIST'S PICK. SOFT CAN BE GROOVED OR GOUGED READILY BY KNIFE OR PICK, CAN BE EXCAVATED IN FRAGMENTS FROM CHIPS TO SEVERAL CENTIMETERS IN SIZE BY MODERATE BLOWS OF A PICK POINT, SMALL, THIN PIECES CAN BE BROKEN BY FINGER PRESSURE. VERY SOFT CAN BE CARVED WITH KNIFE, CAN BE EXCAVATED READILY WITH POINT OF PICK, PIECES 25 mm OR MORE IN THICKNESS CAN BE BROKEN BY FINGER PRESSURE, CAN BE SCRATCHED READILY BY FINGER NAIL.		<b>TEXTURE OR GRAIN SIZE</b>							
<b>BEDDING</b>		<b>ABBREVIATIONS</b>		<b>WEATHERING</b>		<b>TERMS AND DEFINITIONS</b>							
TERM THICKNESS VERY THICKLY BEDDED > 1 m THICKLY BEDDED 0.5 - 1 m MODERATELY BEDDED 0.05 - 0.5 m VERY THINLY BEDDED 10 - 50 mm THICKLY LAMINATED 2.5 - 10 mm THINLY LAMINATED < 2.5 mm		AR - AUGER REFUSAL BT - BORING TERMINATED CL - CLAY CPT - CONE PENETRATION TEST CSE - COARSE DMT - DILATOMETER TEST DPT - DYNAMIC PENETRATION TEST e - VOID RATIO F - FINE FOSS - FOSSILIFEROUS FRAC - FRACTURED FRAGS - FRAGMENTS MED - MEDIUM PMT - PRESSUREMETER TEST SD - SAND, SANDY SL - SILT, SILTY SLI - SLIGHTLY TCR - TRICONE REFUSAL U - UNIT WEIGHT U <sub>d</sub> - DRY UNIT WEIGHT w - MOISTURE CONTENT V - VERY VST - VANE SHEAR TEST		VERY HARD CANNOT BE SCRATCHED BY KNIFE OR SHARP PICK, BREAKING OF HAND SPECIMENS REQUIRES SEVERAL HARD BLOWS OF THE GEOLOGIST'S PICK. HARD CAN BE SCRATCHED BY KNIFE OR PICK ONLY WITH DIFFICULTY, HARD HAMMER BLOWS REQUIRED TO DETACH HAND SPECIMEN. MODERATELY HARD CAN BE SCRATCHED BY KNIFE OR PICK, GOUGES OR GROOVES TO 6 mm DEEP CAN BE EXCAVATED BY HARD BLOW OF A GEOLOGIST'S PICK, HAND SPECIMENS CAN BE DETACHED BY MODERATE BLOWS. MEDIUM HARD CAN BE GROOVED OR GOUGED 1 mm DEEP BY FIRM PRESSURE OF KNIFE OR PICK POINT. CAN BE EXCAVATED IN SMALL CHIPS TO PIECES 25 mm MAXIMUM SIZE BY HARD BLOWS OF THE POINT OF A GEOLOGIST'S PICK. SOFT CAN BE GROOVED OR GOUGED READILY BY KNIFE OR PICK, CAN BE EXCAVATED IN FRAGMENTS FROM CHIPS TO SEVERAL CENTIMETERS IN SIZE BY MODERATE BLOWS OF A PICK POINT, SMALL, THIN PIECES CAN BE BROKEN BY FINGER PRESSURE. VERY SOFT CAN BE CARVED WITH KNIFE, CAN BE EXCAVATED READILY WITH POINT OF PICK, PIECES 25 mm OR MORE IN THICKNESS CAN BE BROKEN BY FINGER PRESSURE, CAN BE SCRATCHED READILY BY FINGER NAIL.		<b>TEXTURE OR GRAIN SIZE</b>							
<b>INDURATION</b>		<b>ABBREVIATIONS</b>		<b>WEATHERING</b>		<b>TERMS AND DEFINITIONS</b>							
FOR SEDIMENTARY ROCKS, INDURATION IS THE HARDENING OF THE MATERIAL BY CEMENTING, HEAT, PRESSURE, ETC. FRIABLE RUBBING WITH FINGER FREES NUMEROUS GRAINS; GENTLE BLOW BY HAMMER DISINTEGRATES SAMPLE. MODERATELY INDURATED GRAINS CAN BE SEPARATED FROM SAMPLE WITH STEEL PROBE; BREAKS EASILY WHEN HIT WITH HAMMER. INDURATED GRAINS ARE DIFFICULT TO SEPARATE WITH STEEL PROBE; DIFFICULT TO BREAK WITH HAMMER. EXTREMELY INDURATED SHARP HAMMER BLOWS REQUIRED TO BREAK SAMPLE; SAMPLE BREAKS ACROSS GRAINS.		AR - AUGER REFUSAL BT - BORING TERMINATED CL - CLAY CPT - CONE PENETRATION TEST CSE - COARSE DMT - DILATOMETER TEST DPT - DYNAMIC PENETRATION TEST e - VOID RATIO F - FINE FOSS - FOSSILIFEROUS FRAC - FRACTURED FRAGS - FRAGMENTS MED - MEDIUM PMT - PRESSUREMETER TEST SD - SAND, SANDY SL - SILT, SILTY SLI - SLIGHTLY TCR - TRICONE REFUSAL U - UNIT WEIGHT U <sub>d</sub> - DRY UNIT WEIGHT w - MOISTURE CONTENT V - VERY VST - VANE SHEAR TEST		VERY HARD CANNOT BE SCRATCHED BY KNIFE OR SHARP PICK, BREAKING OF HAND SPECIMENS REQUIRES SEVERAL HARD BLOWS OF THE GEOLOGIST'S PICK. HARD CAN BE SCRATCHED BY KNIFE OR PICK ONLY WITH DIFFICULTY, HARD HAMMER BLOWS REQUIRED TO DETACH HAND SPECIMEN. MODERATELY HARD CAN BE SCRATCHED BY KNIFE OR PICK, GOUGES OR GROOVES TO 6 mm DEEP CAN BE EXCAVATED BY HARD BLOW OF A GEOLOGIST'S PICK, HAND SPECIMENS CAN BE DETACHED BY MODERATE BLOWS. MEDIUM HARD CAN BE GROOVED OR GOUGED 1 mm DEEP BY FIRM PRESSURE OF KNIFE OR PICK POINT. CAN BE EXCAVATED IN SMALL CHIPS TO PIECES 25 mm MAXIMUM SIZE BY HARD BLOWS OF THE POINT OF A GEOLOGIST'S PICK. SOFT CAN BE GROOVED OR GOUGED READILY BY KNIFE OR PICK, CAN BE EXCAVATED IN FRAGMENTS FROM CHIPS TO SEVERAL CENTIMETERS IN SIZE BY MODERATE BLOWS OF A PICK POINT, SMALL, THIN PIECES CAN BE BROKEN BY FINGER PRESSURE. VERY SOFT CAN BE CARVED WITH KNIFE, CAN BE EXCAVATED READILY WITH POINT OF PICK, PIECES 25 mm OR MORE IN THICKNESS CAN BE BROKEN BY FINGER PRESSURE, CAN BE SCRATCHED READILY BY FINGER NAIL.		<b>TEXTURE OR GRAIN SIZE</b>							
<b>BENCH MARKS</b>		<b>ABBREVIATIONS</b>		<b>WEATHERING</b>		<b>TERMS AND DEFINITIONS</b>							
BENCH MARK: BL-57, BL-PINC 47+36.2, -L- STA. 54+16.4, 9.9m RT ELEVATION: 237.65 BENCH MARK: BY-7, POT 5+64.7 = BL-58, BL-PINC 48+42.8 -L- STA. 55+23.2, 7.4m RT ELEVATION: 234.40		AR - AUGER REFUSAL BT - BORING TERMINATED CL - CLAY CPT - CONE PENETRATION TEST CSE - COARSE DMT - DILATOMETER TEST DPT - DYNAMIC PENETRATION TEST e - VOID RATIO F - FINE FOSS - FOSSILIFEROUS FRAC - FRACTURED FRAGS - FRAGMENTS MED - MEDIUM PMT - PRESSUREMETER TEST SD - SAND, SANDY SL - SILT, SILTY SLI - SLIGHTLY TCR - TRICONE REFUSAL U - UNIT WEIGHT U <sub>d</sub> - DRY UNIT WEIGHT w - MOISTURE CONTENT V - VERY VST - VANE SHEAR TEST		VERY HARD CANNOT BE SCRATCHED BY KNIFE OR SHARP PICK, BREAKING OF HAND SPECIMENS REQUIRES SEVERAL HARD BLOWS OF THE GEOLOGIST'S PICK. HARD CAN BE SCRATCHED BY KNIFE OR PICK ONLY WITH DIFFICULTY, HARD HAMMER BLOWS REQUIRED TO DETACH HAND SPECIMEN. MODERATELY HARD CAN BE SCRATCHED BY KNIFE OR PICK, GOUGES OR GROOVES TO 6 mm DEEP CAN BE EXCAVATED BY HARD BLOW OF A GEOLOGIST'S PICK, HAND SPECIMENS CAN BE DETACHED BY MODERATE BLOWS. MEDIUM HARD CAN BE GROOVED OR GOUGED 1 mm DEEP BY FIRM PRESSURE OF KNIFE OR PICK POINT. CAN BE EXCAVATED IN SMALL CHIPS TO PIECES 25 mm MAXIMUM SIZE BY HARD BLOWS OF THE POINT OF A GEOLOGIST'S PICK. SOFT CAN BE GROOVED OR GOUGED READILY BY KNIFE OR PICK, CAN BE EXCAVATED IN FRAGMENTS FROM CHIPS TO SEVERAL CENTIMETERS IN SIZE BY MODERATE BLOWS OF A PICK POINT, SMALL, THIN PIECES CAN BE BROKEN BY FINGER PRESSURE. VERY SOFT CAN BE CARVED WITH KNIFE, CAN BE EXCAVATED READILY WITH POINT OF PICK, PIECES 25 mm OR MORE IN THICKNESS CAN BE BROKEN BY FINGER PRESSURE, CAN BE SCRATCHED READILY BY FINGER NAIL.		<b>TEXTURE OR GRAIN SIZE</b>							
<b>NOTES:</b>		<b>ABBREVIATIONS</b>		<b>WEATHERING</b>		<b>TERMS AND DEFINITIONS</b>							
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STATE OF NORTH CAROLINA  
DEPARTMENT OF TRANSPORTATION

Michael F. Easley  
GOVERNOR

P.O. BOX 25201, RALEIGH, N.C. 27611-5201

Lyndo Tippet  
SECRETARY

August 26, 2004

STATE PROJECT: 34345.1.1 (R-0609IA)  
F.A. PROJECT: MAF-F-119-1 (1)  
COUNTY: Guilford

DESCRIPTION: US 311 (High Point East Beltway) from US 29-70 to I-85  
north of Archdale

SUBJECT: Geotechnical Report – Foundation Investigation for Structure No. 8 on -FLY- at Sta. 22+01.8  
over -L-

**Project Description**

A two-span bridge, 100 meters in length, is proposed on -FLY- over the proposed -L- (US 311). The project is located in Guilford County about two miles northeast of Archdale. The skew ranges from 70° 26' 58" at the interior bent to 90° at the end bents.

The subsurface investigation was conducted during June of 2004 using an ATV-mounted Mobile B-57 drill machine with a manual hammer. Standard Penetration Test borings were performed at each of the three proposed bent locations. All borings were advanced into crystalline or weathered rock. Boring B1-B was cored using NXWL rock coring equipment. Representative soil samples were obtained for visual classification in the field and selected samples were submitted to the Materials and Test Unit for laboratory analysis. A rock core sample was also sent to the Materials and Test Unit to determine Unit Weight, Compressive Strength and Young's Modulus.

**Physiography and Geology**

The project is located in the gently rolling terrain of the Piedmont Physiographic Province. Geologically, the site is located within the Carolina Slate Belt and is underlain by metamorphosed granite and gabbro intrusions. The area consists of a mixture of wooded land, sparse homes and agricultural fields.

**Soil Properties**

Residual soil was the only soil type encountered in each boring. The residual soil ranged in thickness from 0.5 to 2.2 meters. These soils consist primarily of tan-brown, hard, dry to moist sandy silt (AASHTO classification of A-4) and tan-brown, loose to very dense, dry to moist coarse sand (A-2-4 and A-1-b). The residual soils are derived from weathering of the underlying weathered and crystalline rock.

**Rock Properties**

Weathered rock is present in each boring except for EB2-A. Weathered rock is derived from the underlying crystalline metamorphosed granite (referred to in this report as meta-granite). The weather rock ranged in thickness from 2.2 meters at End Bent Two to as much as 6.2 meters at End Bent One.

Crystalline rock, consisting of meta-granite and meta-gabbro, was encountered at borings B1-B, EB2-A, and EB2-B. Meta-gabbro rock core was obtained from the B1-B boring. The rock consists of gray, very slightly weathered, hard, moderately closely to closely fractured metamorphosed gabbro (meta-gabbro). Core recovery (REC) ranged from 46% to 100%. Rock Quality Designation (RQD) was highly variable, ranging from 0% to 95%. A more detailed rock description can be found in the B1-B Core Boring Report.

**Groundwater**

Groundwater was encountered at each bent location. Groundwater elevations ranged from 229.5 to 237.0 meters.

**Notice**

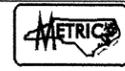
This Geotechnical foundation report is based on the Preliminary General Drawings dated March 24, 2004 which were attached to the "Request for Foundation Recommendations" dated April 2, 2004. If significant changes are made in the design or location of the proposed structure, the subsurface information should be reviewed and modified as necessary.

Prepared by,

A handwritten signature in black ink that reads "Thomas P. Moorefield".

Thomas P. Moorefield, LG  
Project Geologist

# TEST SITE PLAN

	STATE	STATE PROJECT REFERENCE NO.	SHEET NO.	TOTAL SHEETS
	N.C.	34345.1.1(R-06091A) STRUCTURE 8	4	14



BL-57  
BL-PINC 47+36.187  
-L- STA 54+16.424  
OFF 9.925(32.56)RT

-FLY- POC STA. 22+01.765  
-L- POC STA. 54+36.375

21+40  
EBI-A

22+00  
BI-A

EB2-A

EBI-B

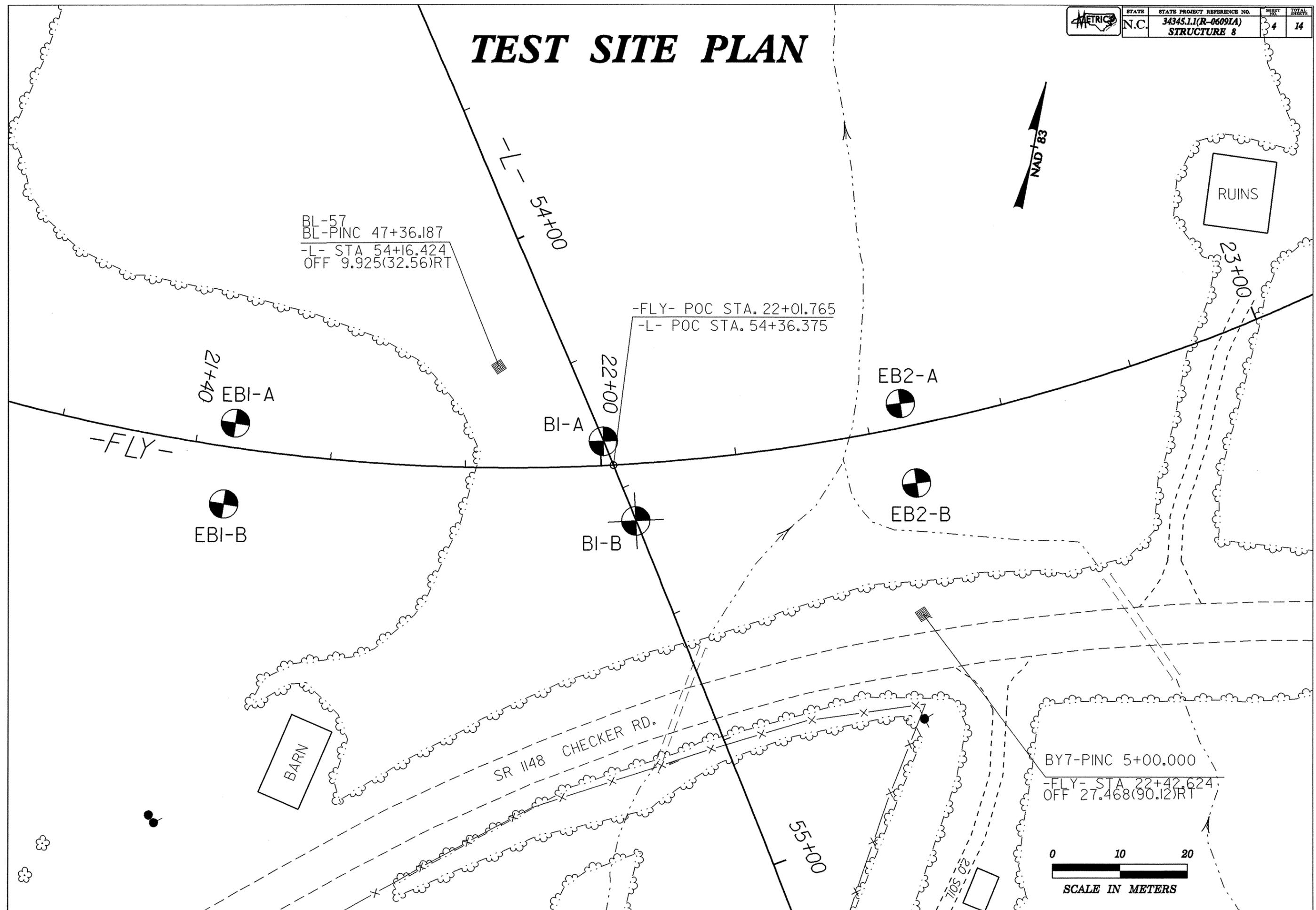
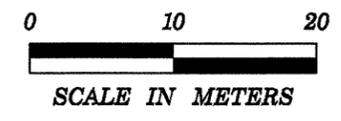
BI-B

EB2-B

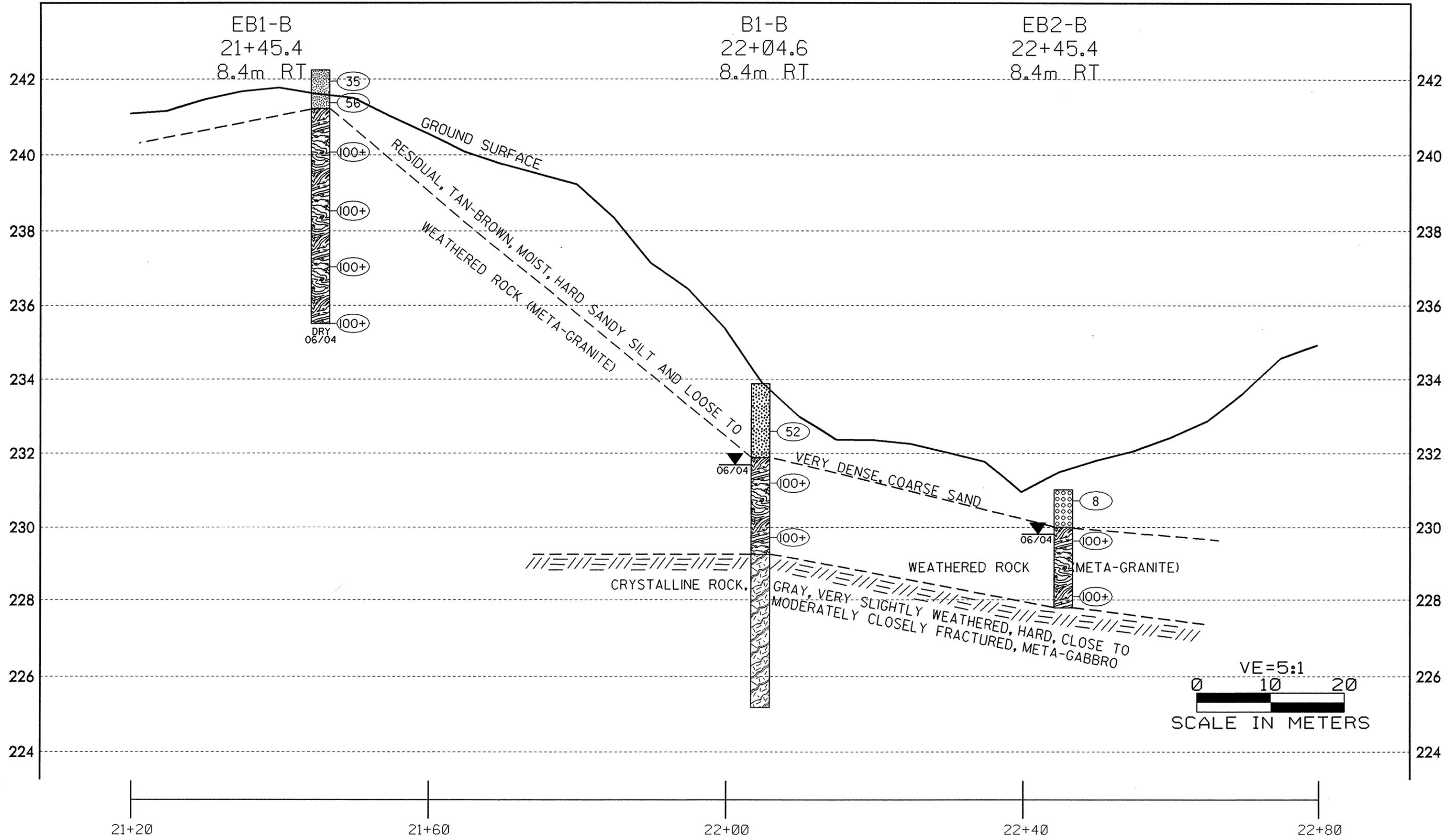
BARN

SR 1148 CHECKER RD.

BY7-PINC 5+00.000  
-FLY- STA 22+42.624  
OFF 27.468(90.12)RT



PROFILE THROUGH BORINGS PROJECTED ALONG -FLY-

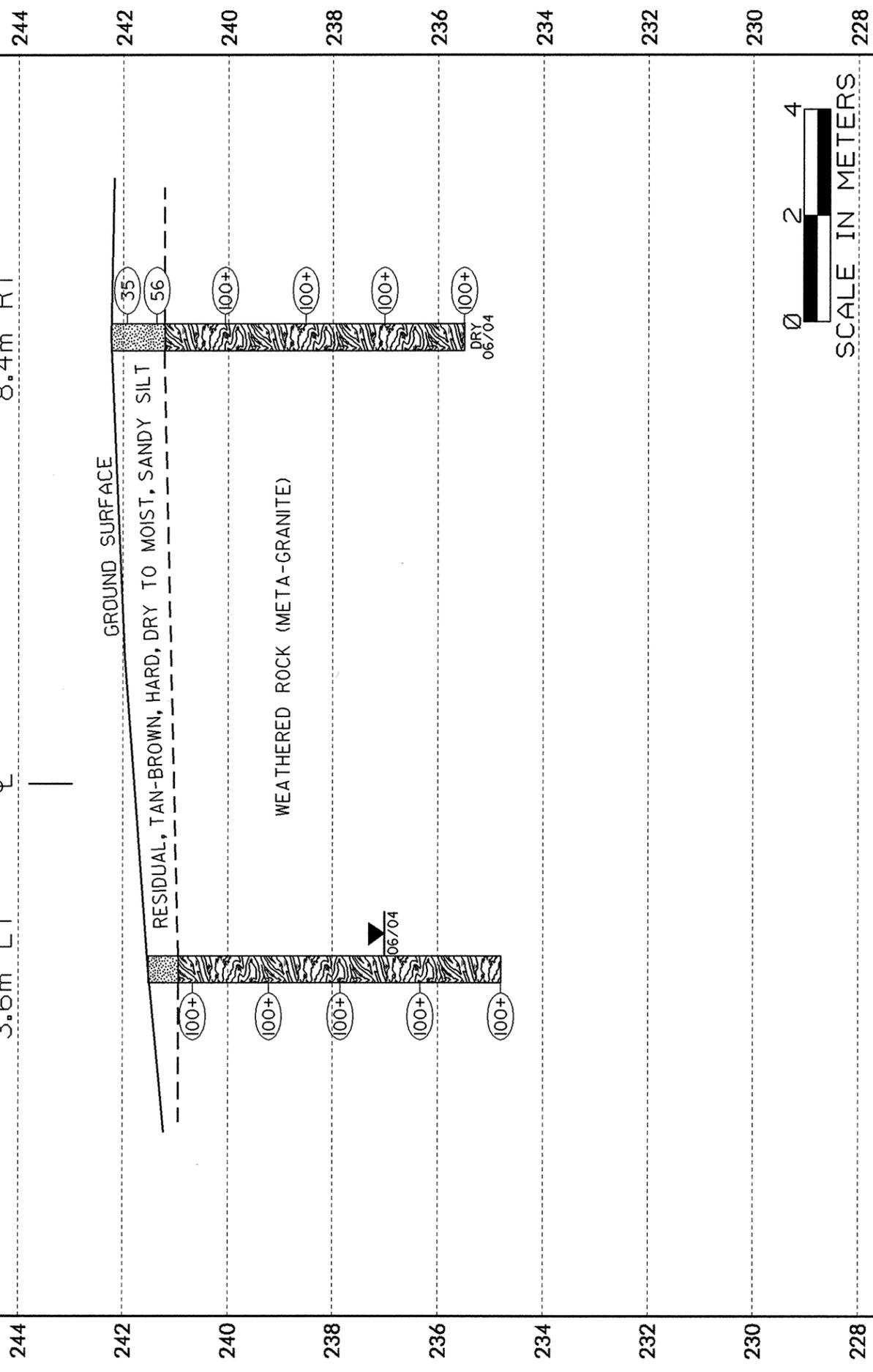


CROSS SECTION THROUGH END BENT I

STRUCTURE NO. 8  
34345.1.1 (R-Ø609IA)

EB1-B  
21+45.4  
8.4m RT

EB1-A  
21+45.4  
3.6m LT

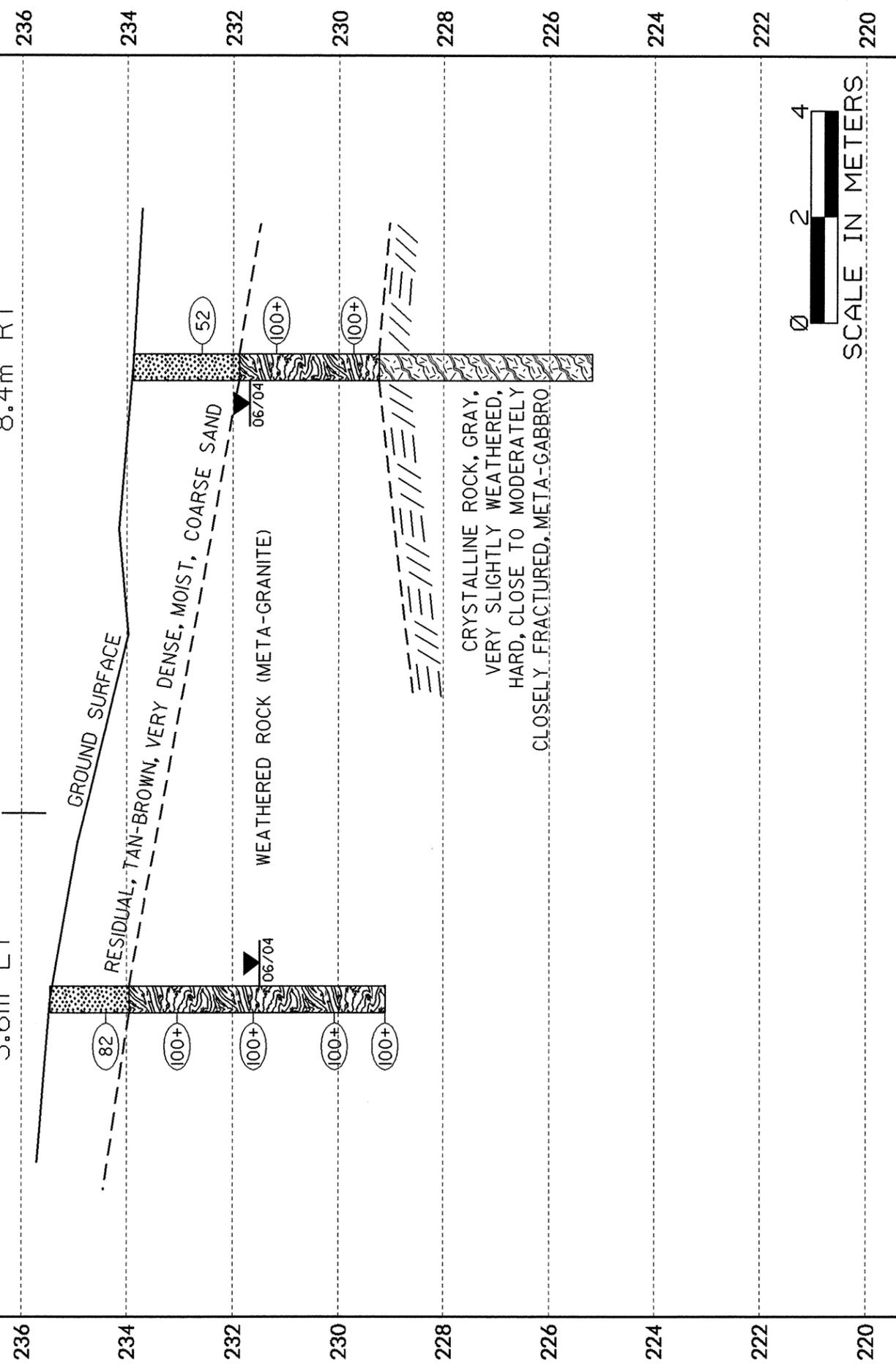


CROSS SECTION THROUGH BENT I

STRUCTURE NO. 8  
34345.1.1 (R-Ø609IA)

B1-B  
22+Ø4.6  
8.4m RT

B1-A  
22+Ø0.5  
3.6m LT



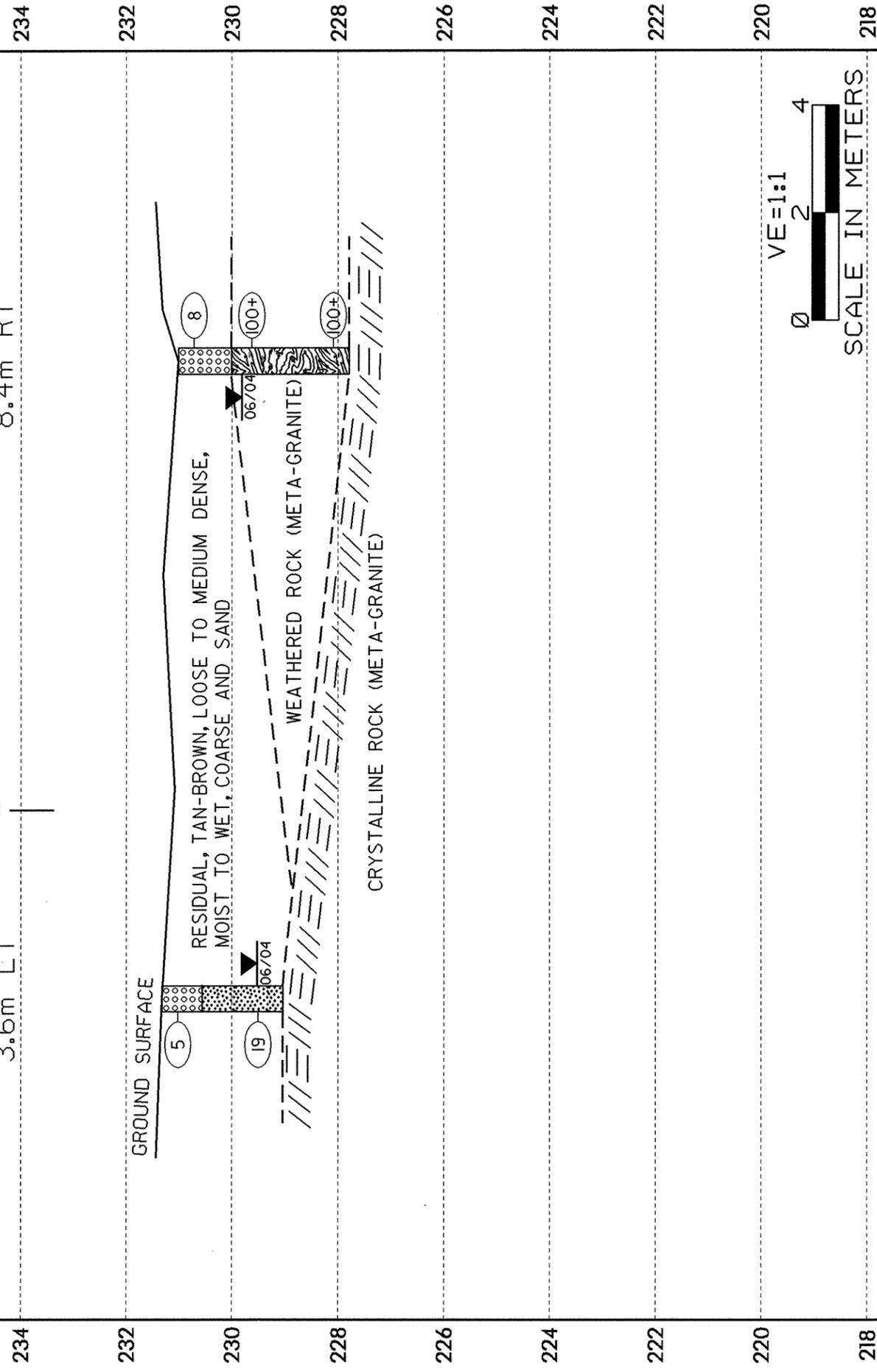
CROSS SECTION THROUGH END BENT 2

STRUCTURE NO. 8  
34345.1.1 (R-0609IA)

EB2-A  
22+45.4  
3.6m LT

EB2-B  
22+45.4  
8.4m RT

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NORTH CAROLINA DEPARTMENT OF TRANSPORTATION  
 GEOTECHNICAL UNIT BORING LOG

NORTH CAROLINA DEPARTMENT OF TRANSPORTATION  
 GEOTECHNICAL UNIT BORING LOG SHEET 8 OF 14

PROJECT NO. 34345.1.1	ID. R-0609IA	COUNTY GUILFORD	GEOLOGIST O. B. OTI
SITE DESCRIPTION STRUCTURE NO. 8 -FLY- OVER -L- (US 311)			GROUND WATER
BORING NO. EBI-A	BORING LOCATION 21+45.4	OFFSET 3.6m LT	ALIGNMENT -FLY-
COLLAR ELEV. 241.50m	NORTHING 241946.0	EASTING 526211.5	0 HR. DRY
TOTAL DEPTH 6.71m		DRILL MACHINE MOBILE B-57	DRILL METHOD H.S. AUGERS
START DATE 6/14/04		COMPLETION DATE 6/14/04	HAMMER TYPE MANUAL
SURFACE WATER DEPTH N/A		DEPTH TO ROCK N/A	

ELEV.	DEPTH	BLOW COUNT			PEN. (m)	BLOWS PER 30cm				SAMPLE NUMBER	LOG	SOIL AND ROCK DESCRIPTION
		15cm	15cm	15cm		0	25	50	75			
241.50	0.57	21	89	0.27							D	RESIDUAL, TAN-BROWN, SANDY SILT
240.00	2.09	50	50	0.20							D	WEATHERED ROCK (META-GRANITE)
238.00	3.61	100		0.05							D	WEATHERED ROCK (META-GRANITE)
236.00	5.13	100		0.05							D	WEATHERED ROCK (META-GRANITE)
234.00	6.65	100		0.06							D	WEATHERED ROCK (META-GRANITE)
BORING TERMINATED AT ELEVATION 234.79M IN WEATHERED ROCK (META-GRANITE)												

PROJECT NO. 34345.1.1	ID. R-0609IA	COUNTY GUILFORD	GEOLOGIST O. B. OTI
SITE DESCRIPTION STRUCTURE NO. 8 -FLY- OVER -L- (US 311)			GROUND WATER
BORING NO. EBI-B	BORING LOCATION 21+45.4	OFFSET 8.4m RT	ALIGNMENT -FLY-
COLLAR ELEV. 242.23m	NORTHING 241934.0	EASTING 526212.1	0 HR. DRY
TOTAL DEPTH 6.73m		DRILL MACHINE MOBILE B-57	DRILL METHOD H.S. AUGERS
START DATE 6/14/04		COMPLETION DATE 6/14/04	HAMMER TYPE MANUAL
SURFACE WATER DEPTH N/A		DEPTH TO ROCK N/A	

ELEV.	DEPTH	BLOW COUNT			PEN. (m)	BLOWS PER 30cm				SAMPLE NUMBER	LOG	SOIL AND ROCK DESCRIPTION
		15cm	15cm	15cm		0	25	50	75			
242.23	0.00	5	10	25	0.30							
242.00	0.57	6	16	40	0.30							RESIDUAL, TAN-BROWN SANDY SILT, SAPROLITIC
240.00	2.09	100		0.08							D	WEATHERED ROCK (META-GRANITE)
238.00	3.61	100		0.10							D	WEATHERED ROCK (META-GRANITE)
236.00	5.13	100		0.08							D	WEATHERED ROCK (META-GRANITE)
234.00	6.65	100		0.08							D	WEATHERED ROCK (META-GRANITE)
BORING TERMINATED AT ELEVATION 235.50M IN WEATHERED ROCK (META-GRANITE)												







**EB1-B**

<b>SOIL TEST RESULTS</b>															
SAMPLE NO.	OFFSET	STATION	DEPTH INTERVAL	AASHTO CLASS.	L.L.	P.I.	% BY WEIGHT				% PASSING (SIEVES)			% MOISTURE	% ORGANIC
							C.SAND	F.SAND	SILT	CLAY	10	40	200		
SS-1	8.4m RT	21+45.4	0.00-0.45	A-4(5)	32	9	6.6	31.2	49.9	12.3	100	96	72	-	-
SS-2	8.4m RT	21+45.4	0.57-1.02	A-4(2)	28	5	9.0	32.0	46.6	12.3	100	95	69	-	-

**B1-A**

<b>SOIL TEST RESULTS</b>															
SAMPLE NO.	OFFSET	STATION	DEPTH INTERVAL	AASHTO CLASS.	L.L.	P.I.	% BY WEIGHT				% PASSING (SIEVES)			% MOISTURE	% ORGANIC
							C.SAND	F.SAND	SILT	CLAY	10	40	200		
SS-3	3.6m LT	22+00.5	0.76-1.21	A-2-4(0)	21	NP	54.4	29.0	10.5	6.2	96	64	20	-	-

**EB2-A**

<b>SOIL TEST RESULTS</b>															
SAMPLE NO.	OFFSET	STATION	DEPTH INTERVAL	AASHTO CLASS.	L.L.	P.I.	% BY WEIGHT				% PASSING (SIEVES)			% MOISTURE	% ORGANIC
							C.SAND	F.SAND	SILT	CLAY	10	40	200		
SS-5	3.6m LT	22+45.4	1.52-1.97	A-2-4(0)	28	7	60.0	20.9	10.9	8.2	69	39	15	-	-

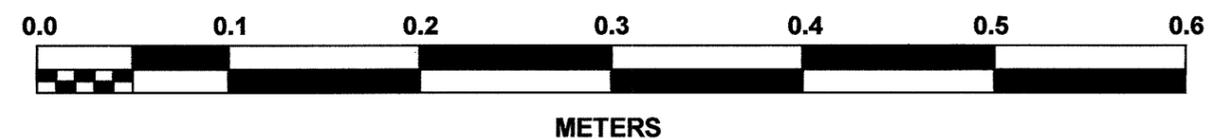
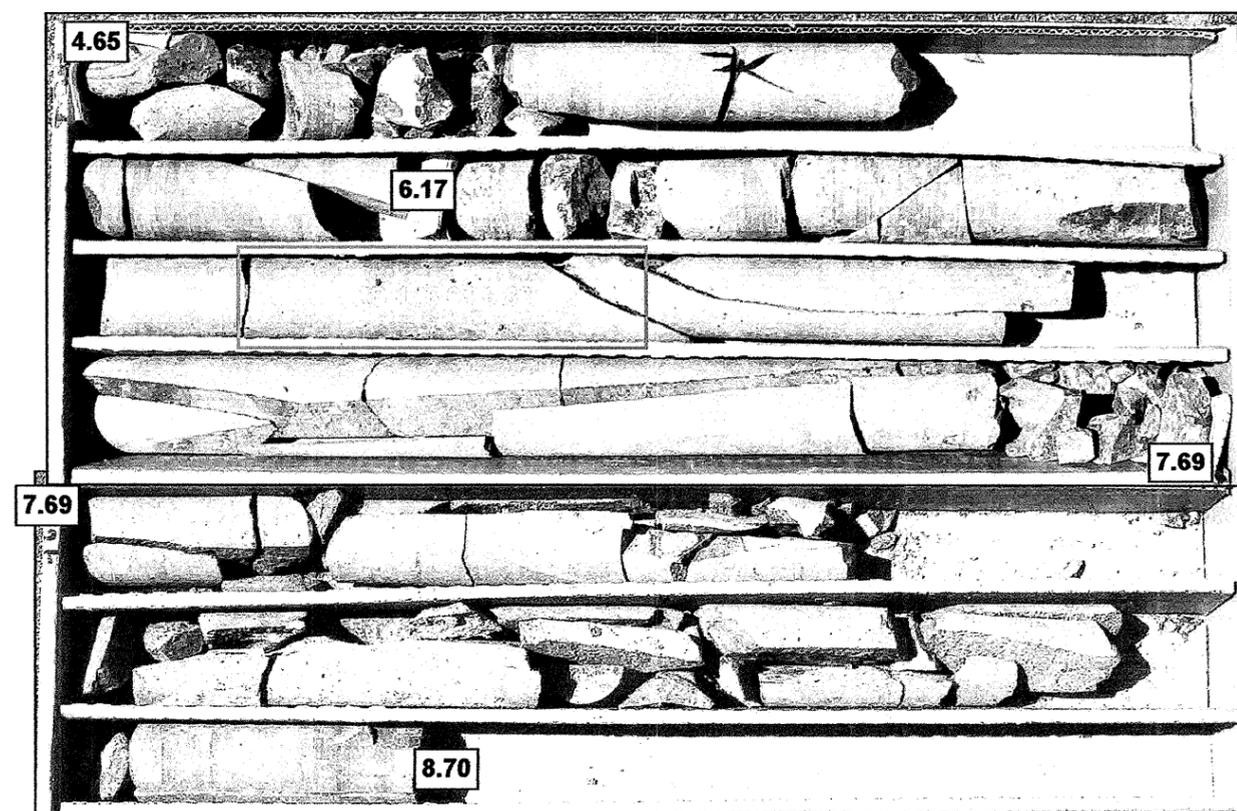
**EB2-B**

<b>SOIL TEST RESULTS</b>															
SAMPLE NO.	OFFSET	STATION	DEPTH INTERVAL	AASHTO CLASS.	L.L.	P.I.	% BY WEIGHT				% PASSING (SIEVES)			% MOISTURE	% ORGANIC
							C.SAND	F.SAND	SILT	CLAY	10	40	200		
SS-4	8.4m RT	22+45.4	0.00-0.45	A-1-b(0)	21	NP	78.0	13.8	4.1	4.1	86	34	8	-	-

# CORE PHOTOGRAPHS

## B1-B

BOXES 1 & 2: 4.65-8.70 METERS





# ***SITE PHOTOGRAPH***



CONTRACT: ID: R-06091A

# STATE OF NORTH CAROLINA

DEPARTMENT OF TRANSPORTATION

DIVISION OF HIGHWAYS

GEOTECHNICAL UNIT

## STRUCTURE SUBSURFACE INVESTIGATION

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	STATE	STATE PROJECT REFERENCE NO.	SHEET NO.	TOTAL SHEETS
	N.C.	34345.1.1 (R-06091A)	1	18
	STATE PROJ. NO.	F.A. PROJ. NO.	DESCRIPTION	
			P.E.	
			CONST.	

### CAUTION NOTICE

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STATE PROJECT 34345.1.1 I.D. NO. R-06091A

F.A. PROJECT MAF-F-119-1(1)

COUNTY GUILFORD

PROJECT DESCRIPTION US 311 HIGH POINT  
EAST BELTWAY FROM US 29-70 TO  
I-85 NORTH OF ARCHDALE

SITE DESCRIPTION STRUCTURE NO. 9 ON  
-FLY- (HIGH POINT EAST BELTWAY)  
OVER -Y6- (I-85) AT STA. 19+26.763 -FLY-

### INVENTORY

INVESTIGATED BY J.I. MILKOVITS, JR N.C. DOT PERSONNEL O.B. OTI

CHECKED BY D.N. ARGENBRIGHT ENGINEERING  
TECTONICS, P.A.  
PERSONNEL

SUBMITTED BY D.N. ARGENBRIGHT

DATE AUGUST 2004 R.F. BAYRON

D.C. BAYRON

DRAWN BY: T.T. WALKER

NOTE - THE INFORMATION CONTAINED HEREIN IS NOT IMPLIED OR GUARANTEED BY THE N.C. DEPARTMENT OF TRANSPORTATION AS BEING ACCURATE NOR IS IT CONSIDERED TO BE PART OF THE PLANS, SPECIFICATIONS, OR CONTRACT FOR THE PROJECT.

NOTE - BY HAVING REQUESTED THIS INFORMATION THE CONTRACTOR SPECIFICALLY WAIVES ANY CLAIMS FOR INCREASED COMPENSATION OR EXTENSION OF TIME BASED ON DIFFERENCES BETWEEN THE CONDITIONS INDICATED HEREIN AND THE ACTUAL CONDITIONS AT THE PROJECT SITE.



NORTH CAROLINA DEPARTMENT OF TRANSPORTATION  
 DIVISION OF HIGHWAYS  
 GEOTECHNICAL UNIT

ID	STATE PROJECT NO.	SHEET NO.	TOTAL SHEETS
R-06091A	34345.1J	2	18



SUBSURFACE INVESTIGATION

SOIL AND ROCK LEGEND, TERMS, SYMBOLS, AND ABBREVIATIONS

SOIL DESCRIPTION		GRADATION		ROCK DESCRIPTION		TERMS AND DEFINITIONS	
SOIL IS CONSIDERED TO BE THE UNCONSOLIDATED, SEMI-CONSOLIDATED OR WEATHERED EARTH MATERIALS WHICH CAN BE PENETRATED WITH A CONTINUOUS FLIGHT POWER AUGER, AND WHICH YIELDS LESS THAN 100 BLOWS PER 30 cm ACCORDING TO STANDARD PENETRATION TEST (AASHTO T206, ASTM D-1586). SOIL CLASSIFICATION IS BASED ON THE AASHTO SYSTEM AND BASIC DESCRIPTIONS GENERALLY SHALL INCLUDE CONSISTENCY, COLOR, TEXTURE, MOISTURE, AASHTO CLASSIFICATION, AND OTHER PERTINENT FACTORS SUCH AS MINERALOGICAL COMPOSITION, ANGULARITY, STRUCTURE, PLASTICITY, ETC. EXAMPLE: <i>VERY STIFF, GRAY SILTY CLAY, MOST WITH INTERBEDDED FINE SAND LAYERS, HIGH PLASTIC, A-7-6</i>		WELL GRADED - INDICATES A GOOD REPRESENTATION OF PARTICLE SIZES FROM FINE TO COARSE UNIFORM - INDICATES THAT SOIL PARTICLES ARE ALL APPROXIMATELY THE SAME SIZE (ALSO POORLY GRADED). GAP-GRADED - INDICATES A MIXTURE OF UNIFORM PARTICLES OF TWO OR MORE SIZES.		HARD ROCK IS NON-COASTAL PLAIN MATERIAL THAT WHEN TESTED, WOULD YIELD SPT REFUSAL. AN INFERRED ROCK LINE INDICATES THE LEVEL AT WHICH NON-COASTAL PLAIN MATERIAL WOULD YIELD SPT REFUSAL. SPT REFUSAL IS PENETRATION BY A SPLIT SPOON SAMPLER EQUAL TO OR LESS THAN 2.5 cm PER 50 BLOWS. IN NON-COASTAL PLAIN MATERIAL, THE TRANSITION BETWEEN SOIL AND ROCK IS OFTEN REPRESENTED BY A ZONE OF WEATHERED ROCK. ROCK MATERIALS ARE TYPICALLY DIVIDED AS FOLLOWS:		ALLUVIUM (ALLUV.) - SOILS WHICH HAVE BEEN TRANSPORTED BY WATER. AQUIFER - A WATER BEARING FORMATION OR STRATA. ARENACEOUS - APPLIED TO ROCKS THAT HAVE BEEN DERIVED FROM SAND OR THAT CONTAIN SAND. ARGILLACEOUS - APPLIED TO ALL ROCKS OR SUBSTANCES COMPOSED OF CLAY MINERALS, OR HAVING A NOTABLE PROPORTION OF CLAY IN THEIR COMPOSITION, AS SHALE, SLATE, ETC. ARTESIAN - GROUND WATER THAT IS UNDER SUFFICIENT PRESSURE TO RISE ABOVE THE LEVEL AT WHICH IS ENCOUNTERED, BUT WHICH DOES NOT NECESSARILY RISE TO OR ABOVE THE GROUND SURFACE. CALCAREOUS (CALC.) - SOILS WHICH CONTAIN APPRECIABLE AMOUNTS OF CALCIUM CARBONATE. COLLUVIUM - ROCK FRAGMENTS MIXED WITH SOIL DEPOSITED BY GRAVITY ON SLOPE OR AT BOTTOM OF SLOPE. CORE RECOVERY (REC.) - TOTAL LENGTH OF ALL MATERIAL RECOVERED IN THE CORE BARREL DIVIDED BY TOTAL LENGTH OF CORE RUN AND EXPRESSED AS A PERCENTAGE. DIKE - A TABULAR BODY OF IGNEOUS ROCK THAT CUTS ACROSS THE STRUCTURE OF ADJACENT ROCKS OR CUTS MASSIVE ROCK. DIP - THE ANGLE AT WHICH A STRATUM OR ANY PLANAR FEATURE IS INCLINED FROM THE HORIZONTAL. DIP DIRECTION @ DIP AZIMUTH - THE DIRECTION OR BEARING OF THE HORIZONTAL TRACE OF THE LINE OF DIP, MEASURED CLOCKWISE FROM NORTH. FAULT - A FRACTURE OR FRACTURE ZONE ALONG WHICH THERE HAS BEEN DISPLACEMENT OF THE SIDES RELATIVE TO ONE ANOTHER PARALLEL TO THE FRACTURE. FISSILE - A PROPERTY OF SPLITTING ALONG CLOSELY SPACED PARALLEL PLANES. FLOAT - ROCK FRAGMENTS ON SURFACE NEAR THEIR ORIGINAL POSITION AND DISLODGED FROM PARENT MATERIAL. FLOOD PLAIN (F.P.) - LAND BORDERING A STREAM, BUILT OF SEDIMENTS DEPOSITED BY THE STREAM. FORMATION (FM.) - A MAPPABLE GEOLOGIC UNIT THAT CAN BE RECOGNIZED AND TRACED IN THE FIELD. JOINT - FRACTURE IN ROCK ALONG WHICH NO APPRECIABLE MOVEMENT HAS OCCURRED. LEDGE - A SHELF-LIKE RIDGE OR PROJECTION OF ROCK WHOSE THICKNESS IS SMALL COMPARED TO ITS LATERAL EXTENT. LENS - A BODY OF SOIL OR ROCK THAT THINS OUT IN ONE OR MORE DIRECTIONS. MOTTLED (MOT.) - IRREGULARLY MARKED WITH SPOTS OF DIFFERENT COLORS. MOTTLING IN SOILS USUALLY INDICATES POOR AERATION AND LACK OF GOOD DRAINAGE. PERCHED WATER - WATER MAINTAINED ABOVE THE NORMAL GROUND WATER LEVEL BY THE PRESENCE OF AN INTERVENING IMPERVIOUS STRATUM. RESIDUAL SOIL - SOIL FORMED IN PLACE BY THE WEATHERING OF ROCK. ROCK QUALITY DESIGNATION (R.Q.D.) - A MEASURE OF ROCK QUALITY DESCRIBED BY TOTAL LENGTH OF ROCK SEGMENTS EQUAL TO OR GREATER THAN 10 CENTIMETERS DIVIDED BY THE TOTAL LENGTH OF CORE RUN AND EXPRESSED AS A PERCENTAGE. SAPROLITE (SAP.) - RESIDUAL SOIL WHICH RETAINS THE RELIC STRUCTURE OR FABRIC OF THE PARENT ROCK. SILL - AN INTRUSIVE BODY OF IGNEOUS ROCK OF APPROXIMATELY UNIFORM THICKNESS AND RELATIVELY THIN COMPARED WITH ITS LATERAL EXTENT, WHICH HAS BEEN EMPLACED PARALLEL TO THE BEDDING OR SCHISTOSITY OF THE INTRUDED ROCKS. SLICKENSIDE - POLISHED AND STRIATED SURFACE THAT RESULTS FROM FRICTION ALONG A FAULT OR SLIP PLANE. STANDARD PENETRATION TEST (PENETRATION RESISTANCE) (SPT) - NUMBER OF BLOWS (NO OF A 63.5 kg HAMMER FALLING 0.76 METERS REQUIRED TO PRODUCE A PENETRATION OF 30 cm INTO SOIL WITH A 5 cm OUTSIDE DIAMETER SPLIT SPOON SAMPLER. SPT REFUSAL IS LESS THAN 2.5 cm PENETRATION WITH 50 BLOWS. STRATA CORE RECOVERY (SREC) - TOTAL LENGTH OF STRATA MATERIAL RECOVERED DIVIDED BY TOTAL LENGTH OF STRATUM AND EXPRESSED AS A PERCENTAGE. STRATA ROCK QUALITY DESIGNATION (S.R.Q.D.) - A MEASURE OF ROCK QUALITY DESCRIBED BY TOTAL LENGTH OF ROCK SEGMENTS WITHIN A STRATUM EQUAL TO OR GREATER THAN 10 CENTIMETERS DIVIDED BY THE TOTAL LENGTH OF STRATA AND EXPRESSED AS A PERCENTAGE. TOPSOIL (T.S.) - SURFACE SOILS USUALLY CONTAINING ORGANIC MATTER.	
SOIL LEGEND AND AASHTO CLASSIFICATION		MINERALOGICAL COMPOSITION		WEATHERING			
GENERAL CLASS. GRANULAR MATERIALS (<5% PASSING #200) SILT-CLAY MATERIALS (>5% PASSING #200) ORGANIC MATERIALS		MINERAL NAMES SUCH AS QUARTZ, FELDSPAR, MICA, TALC, KAOLIN, ETC. ARE USED IN DESCRIPTIONS WHENEVER THEY ARE CONSIDERED OF SIGNIFICANCE.		WEATHERED ROCK (WR) NON-COASTAL PLAIN MATERIAL THAT YIELDS SPT N VALUES > 100 BLOWS PER 30 cm.			
GROUP CLASS. A-1, A-3, A-2, A-4, A-5, A-6, A-7, A-1, A-2, A-4, A-5		COMPRESSIBILITY SLIGHTLY COMPRESSIBLE LIQUID LIMIT LESS THAN 30 MODERATELY COMPRESSIBLE LIQUID LIMIT 31-50 HIGHLY COMPRESSIBLE LIQUID LIMIT GREATER THAN 50		CRYSTALLINE ROCK (CR) FINE TO COARSE GRAIN IGNEOUS AND METAMORPHIC ROCK THAT WOULD YIELD SPT REFUSAL IF TESTED. ROCK TYPE INCLUDES GRANITE, GNEISS, GABBRO, SCHIST, ETC.			
SYMBOL		PERCENTAGE OF MATERIAL		NON-CRYSTALLINE ROCK (NCR) FINE TO COARSE GRAIN METAMORPHIC AND NON-COASTAL PLAIN SEDIMENTARY ROCK THAT WOULD YIELD SPT REFUSAL IF TESTED. ROCK TYPE INCLUDES PHYLLITE, SLATE, SANDSTONE, ETC.			
% PASSING		GROUND WATER		COASTAL PLAIN SEDIMENTARY ROCK (CP) COASTAL PLAIN SEDIMENTS CEMENTED INTO ROCK, BUT MAY NOT YIELD SPT REFUSAL. ROCK TYPE INCLUDES LIMESTONE, SANDSTONE, CEMENTED SHELL BEDS, ETC.			
LIQUID LIMIT PLASTIC INDEX		MISCELLANEOUS SYMBOLS		MODERATELY SEVERE (MOD. SEV.) ALL ROCK EXCEPT QUARTZ DISCOLORED OR STAINED, IN GRANITOID ROCKS, ALL FELDSPARS DULL AND DISCOLORED AND A MAJORITY SHOW KAOLINIZATION. ROCK SHOWS SEVERE LOSS OF STRENGTH AND CAN BE EXCAVATED WITH A GEOLOGIST'S PICK. ROCK GIVES "CLUNK" SOUND WHEN STRUCK. IF TESTED, WOULD YIELD SPT REFUSAL.			
GROUP INDEX		ROADWAY EMBANKMENT WITH SOIL DESCRIPTION		SEVERE (SEV.) ALL ROCKS EXCEPT QUARTZ DISCOLORED OR STAINED, ROCK FABRIC CLEAR AND EVIDENT BUT REDUCED IN STRENGTH TO STRONG SOIL. IN GRANITOID ROCKS ALL FELDSPARS ARE KAOLINIZED TO SOME EXTENT. SOME FRAGMENTS OF STRONG ROCK USUALLY REMAIN. IF TESTED, YIELDS SPT N VALUES > 100 BLOWS PER 30 cm.			
USUAL TYPES OF MAJOR MATERIALS		ARTIFICIAL FILL OTHER THAN ROADWAY EMBANKMENTS		VERY SEVERE (V. SEV.) ALL ROCK EXCEPT QUARTZ DISCOLORED OR STAINED, ROCK FABRIC ELEMENTS ARE DISCERNIBLE BUT THE MASS IS EFFECTIVELY REDUCED TO SOIL STATUS, WITH ONLY FRAGMENTS OF STRONG ROCK REMAINING. SAPROLITE IS AN EXAMPLE OF ROCK WEATHERED TO A DEGREE SUCH THAT ONLY MINOR VESTIGES OF THE ORIGINAL ROCK FABRIC REMAIN. IF TESTED, YIELDS SPT N VALUES < 100 BLOWS PER 30 cm.			
GEN. RATING AS A SUBGRADE		INFERRED SOIL BOUNDARIES		COMPLETE ROCK REDUCED TO SOIL. ROCK FABRIC NOT DISCERNIBLE, OR DISCERNIBLE ONLY IN SMALL AND SCATTERED CONCENTRATIONS. QUARTZ MAY BE PRESENT AS DIKES OR STRINGERS. SAPROLITE IS ALSO AN EXAMPLE.			
P.I. OF A-7-5 ≤ L.L. - 30 + P.I. OF A-7-6 > L.L. - 30		INFERRED ROCK LINE					
CONSISTENCY OR DENSENESS		ALLUVIAL SOIL BOUNDARY					
PRIMARY SOIL TYPE		DIP/DIP DIRECTION OF ROCK STRUCTURES					
COMPACTNESS OR CONSISTENCY		SOUNDING ROD					
RANGE OF STANDARD PENETRATION RESISTANCE (N-VALUE)		ABBREVIATIONS					
RANGE OF UNDEFINED COMPRESSIVE STRENGTH (kN/m <sup>2</sup> )		AR - AUGER REFUSAL					
TEXTURE OR GRAIN SIZE		BT - BORING TERMINATED					
U.S. STD. SIEVE SIZE OPENING (MM)		CL - CLAY					
BOULDER (BLDR.)		CPT - COARSE PENETRATION TEST					
COBBLE (COB.)		CSE - COARSE					
GRAVEL (GR.)		DMT - DILATOMETER TEST					
COARSE SAND (CSE, SD.)		DPT - DYNAMIC PENETRATION TEST					
FINE SAND (F, SD.)		e - VOID RATIO					
SILT (SL.)		F - FINE					
CLAY (CL.)		FOSS. - FOSSILIFEROUS					
GRAIN SIZE		FRAC. - FRACTURED					
SOIL MOISTURE - CORRELATION OF TERMS		FRAGS. - FRAGMENTS					
SOIL MOISTURE SCALE (ATTERBERG LIMITS)		MED. - MEDIUM					
FIELD MOISTURE DESCRIPTION		EQUIPMENT USED ON SUBJECT PROJECT					
GUIDE FOR FIELD MOISTURE DESCRIPTION		DRILL UNITS:					
- SATURATED - (SAT.)		ADVANCING TOOLS:					
USUALLY LIQUID; VERY WET, USUALLY FROM BELOW THE GROUND WATER TABLE		CLAY BITS					
- WET - (W)		152 mm CONTINUOUS FLIGHT AUGER					
SEMISOLID; REQUIRES DRYING TO ATTAIN OPTIMUM MOISTURE		203 mm HOLLOW AUGERS					
- MOIST - (M)		HARD FACED FINGER BITS					
SOLID; AT OR NEAR OPTIMUM MOISTURE		TUNG-CARBIDE INSERTS					
- DRY - (D)		CASING w/ ADVANCER					
REQUIRES ADDITIONAL WATER TO ATTAIN OPTIMUM MOISTURE		TRICONE mm STEEL TEETH					
PLASTICITY		TRICONE mm TUNG-CARB.					
PLASTICITY INDEX (PI)		CORE BIT					
DRY STRENGTH		OTHER					
NONPLASTIC		OTHER					
LOW PLASTICITY							
MED. PLASTICITY							
HIGH PLASTICITY							
COLOR							
DESCRIPTIONS MAY INCLUDE COLOR OR COLOR COMBINATIONS (TAN, RED, YEL-BRN, BLUE-GRAY) MODIFIERS SUCH AS LIGHT, DARK, STREAKED, ETC. ARE USED TO DESCRIBE APPEARANCE.							



STATE OF NORTH CAROLINA  
DEPARTMENT OF TRANSPORTATION

Michael F. Easley  
GOVERNOR

P.O. BOX 25201, RALEIGH, N.C. 27611-5201

Lyndo Tippet  
SECRETARY

August 17, 2004

STATE PROJECT: 34345.1.1 (R-0609IA)  
F.A. PROJECT: MAF-F-119-1 (1)  
COUNTY: Guilford  
DESCRIPTION: US 311 High Point East Beltway from US 29-70 to I-85 north of Archdale  
SUBJECT: Geotechnical Report - Structure Inventory for Structure No. 9 on -FLY- (High Point East Beltway) over -Y6- (I-85) at Station 19+26.763 -FLY-

**Project Description**

A four-span bridge, 161.7 meters in length with variable skew bents, is proposed on -FLY- (High Point East Beltway) over -Y6- (I-85). The project is located in Guilford County about 3.2 kilometers northeast of Archdale.

The subsurface investigation was conducted during June of 2004 using a B-57 drill machine with a manual hammer. Standard Penetration Test borings were performed at each of the five bent locations. All borings were advanced using hollow stem augers until weathered rock and/or crystalline rock was encountered. Interior bent borings B1-B and B2-A were cored using NQWL core equipment to recover rock samples from crystalline rock. Representative soil samples were obtained for visual classification in the field and selected samples were sent to the Materials and Test Unit for laboratory analysis. Three rock core samples were also sent to the Material and Test Unit to determine Unit Weight, Compressive Strength and Young's Modulus.

**Physiography and Geology**

The project is located in the gently rolling terrain of the Piedmont Physiographic Province. The site is located within the Carolina Slate Geologic Belt and is underlain by metamorphosed granitic rock and diabase dikes.

**Soil Properties**

Soils encountered at the project site include roadway embankment and residual soils.

Roadway embankment soils are present at bent 1 and 2 and ranged in thickness from 2.72 to 6.00 meters. These soils consist predominantly of tan-brown-gray, moist, loose to very dense, coarse and silty sand (A-1-b, A-2-4). Residual soils and/or weathered rock underlie embankment soils.

Residual soils were encountered in all borings except at the B1-A location and range in thickness from 0.56 to 5.00 meters. Residual soils consist of tan-brown-gray, moist, medium stiff to hard, sandy clay (A-6) and silty clay (A-7-5). Tan-brown, moist, medium dense to very dense, saprolitic, silty sand (A-2-4) and tan-brown, moist, very stiff to hard, saprolitic, sandy and clayey silt (A-4 and A-5) are also present. Weathered rock underlies residual soils.

**Rock Properties**

Weathered rock was derived from the underlying metamorphosed granite and diabase and ranges in thickness from 0.38 to 5.30 meters. The top of weathered rock was encountered at elevations ranging from 243.28 at EB2-B to 230.40 meters at EB1-B.

Crystalline rock was encountered at each boring location except bent 3 and end bent 2. The top of crystalline rock ranges in elevation from 237.85 at B1-A to 230.02 meters at EB1-B. Rock core was obtained from two of the interior bent borings. Crystalline rock consists of gray-white to brown and dark gray, moderately severe weathered to fresh, moderately hard to very hard, closely to very closely fractured, granite and diabase. Core recovery (REC) ranged from 40% in boring B1-B to 100% in boring B2-A. Rock Quality Designation (RQD) ranges from 0% in B1-B to 100% in B2-A. More detailed rock descriptions can be found in the Core Boring Reports.

**Goundwater**

Groundwater was encountered at two of the boring locations. Groundwater elevations ranged from 238.08 at B1-B to 233.33 meters at B2-A.

**Notice**

This Geotechnical foundation report is based on the Preliminary General Drawing for Bridge no. 9 on -FLY- (High Point East Beltway) over -Y6- (I-85) dated March 19, 2004. If significant changes are made in the design or location of the proposed structure, the subsurface information should be reviewed and modified as necessary.

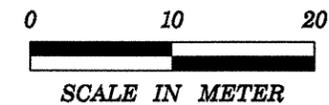
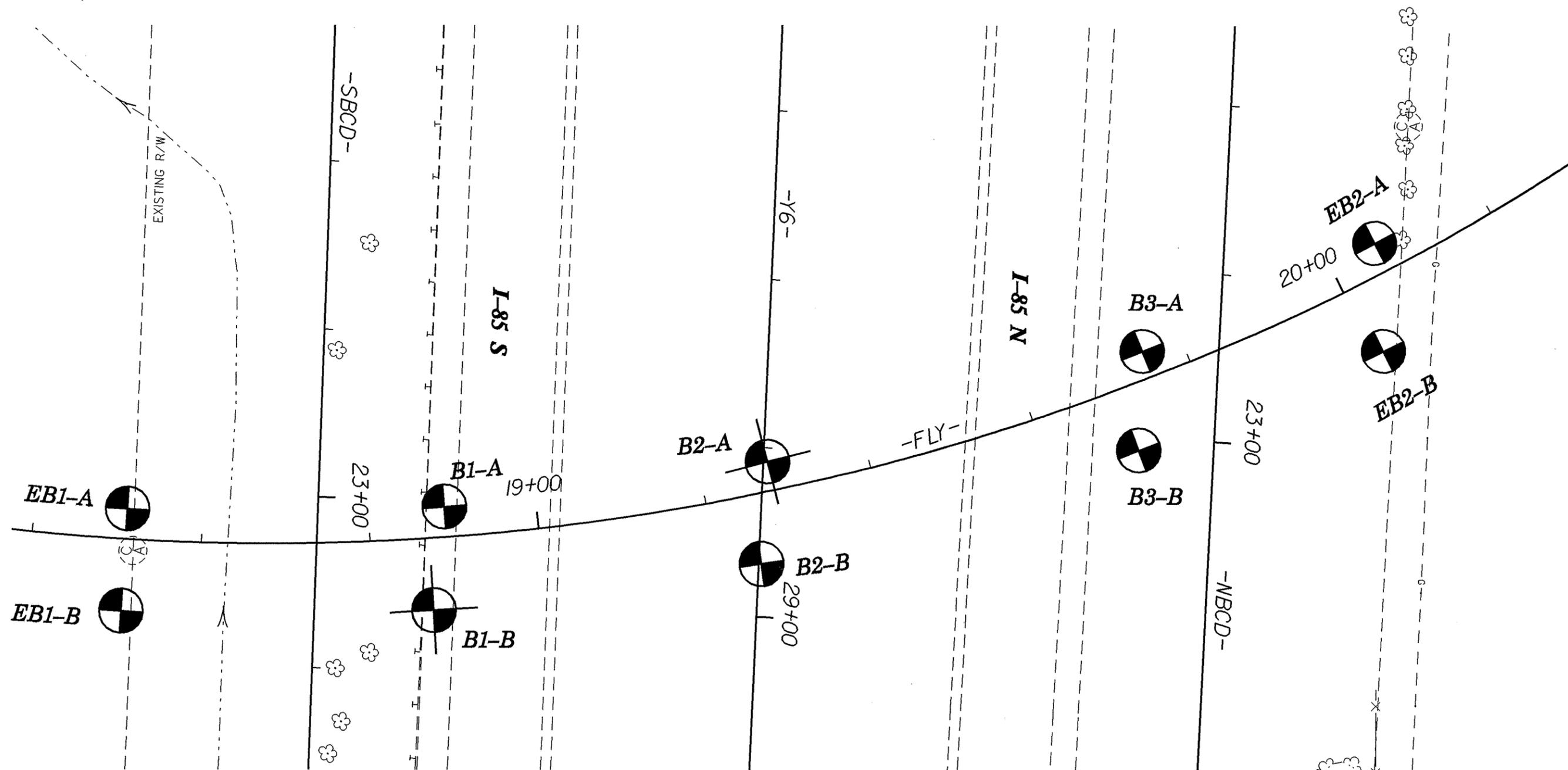
Respectfully submitted,

A handwritten signature in black ink that reads "Joseph I. Milkovits, Jr.".

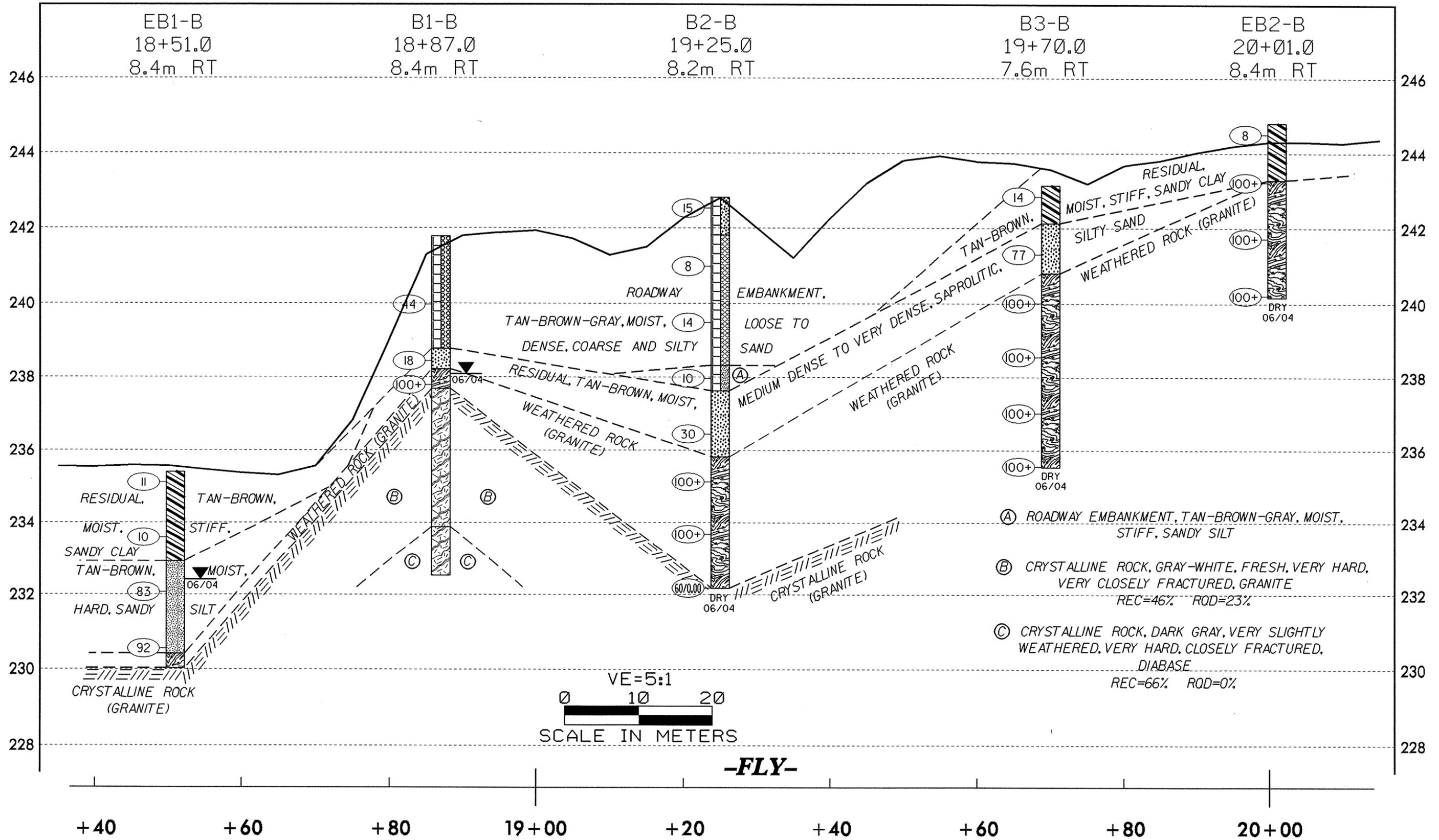
Joseph I. Milkovits, Jr.  
Project Geologist



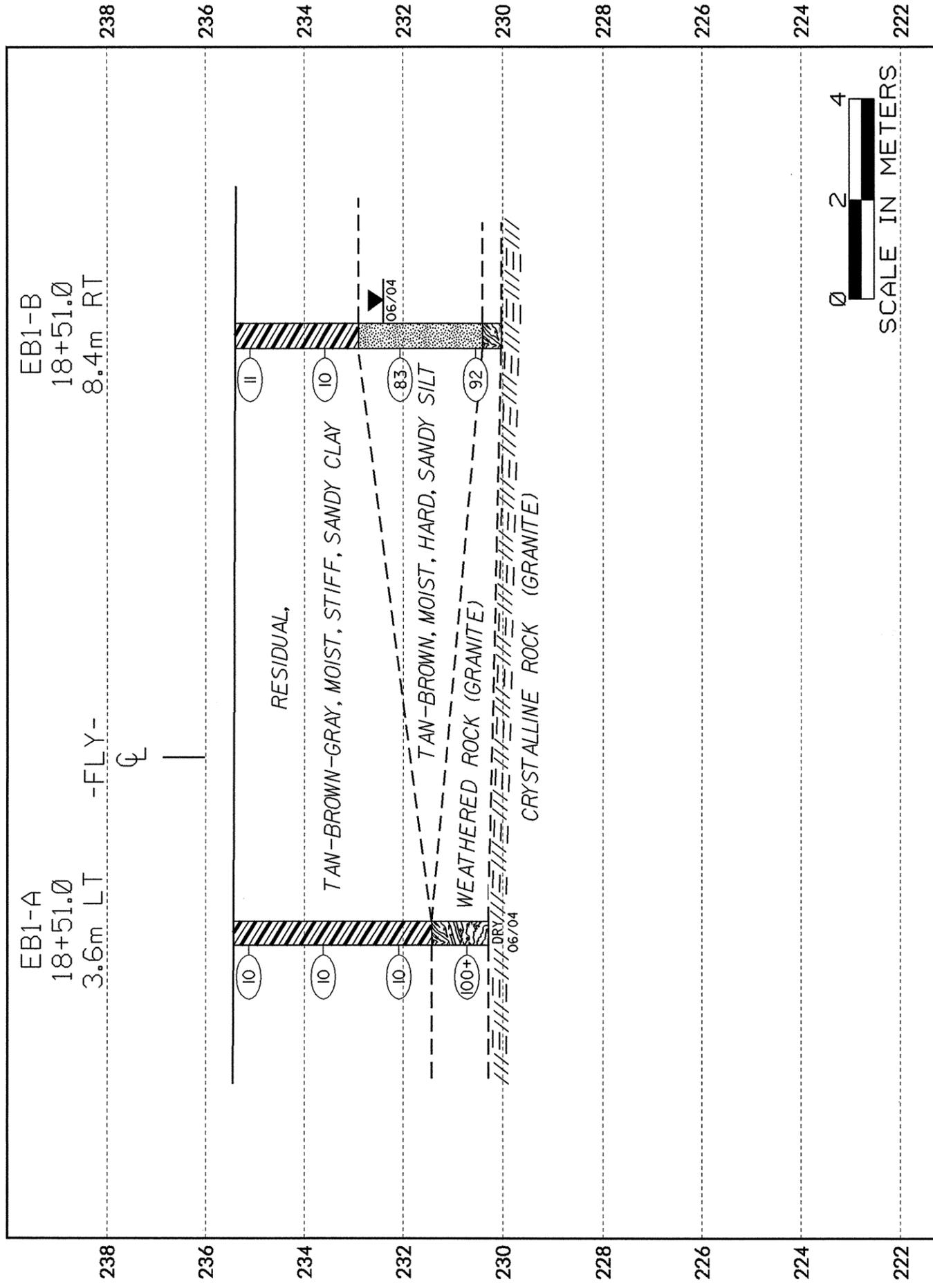
# TEST SITE PLAN



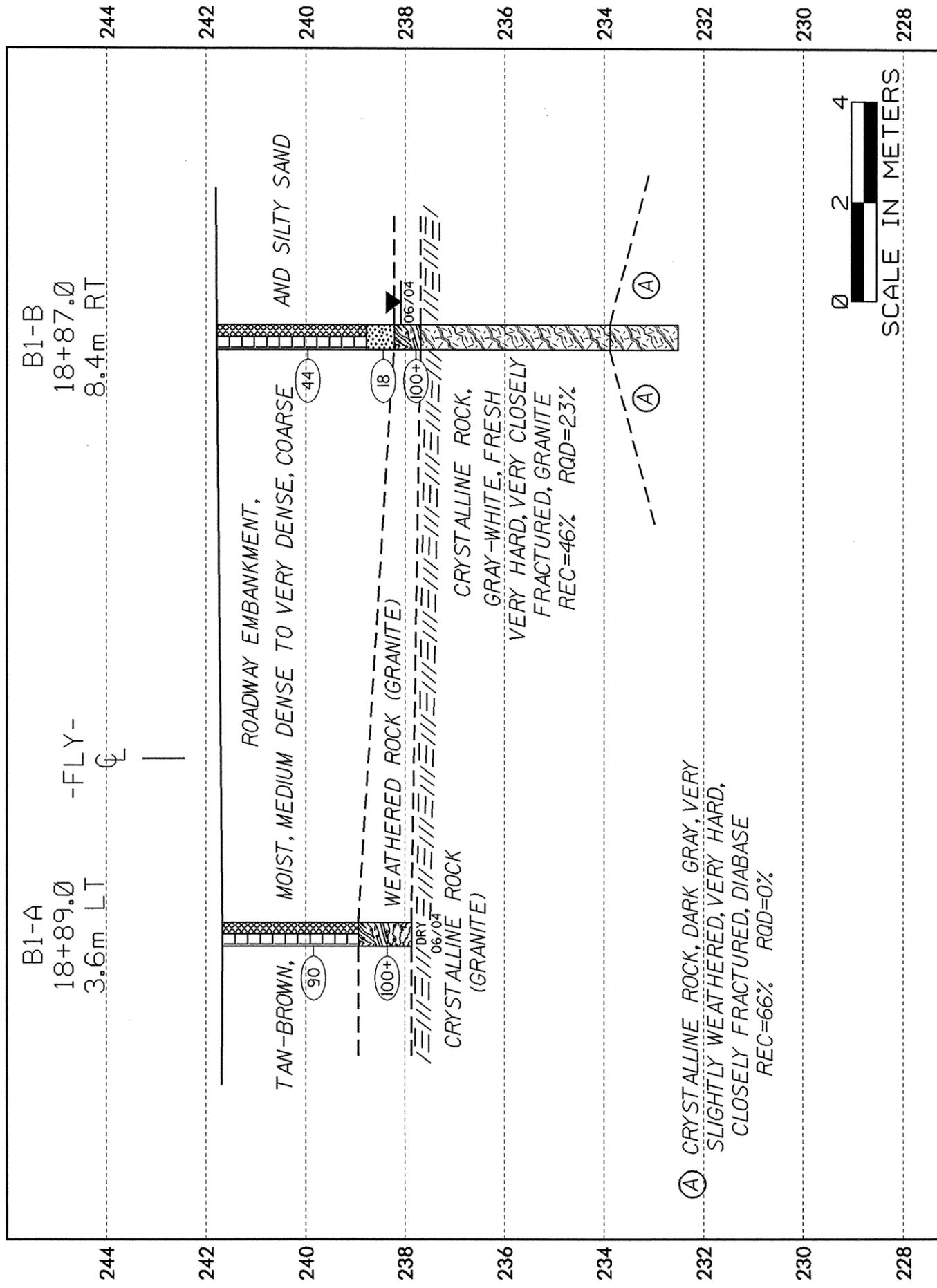
PROFILE THROUGH BORINGS PROJECTED ALONG -FLY-



CROSS SECTION THROUGH END BENT I STRUCTURE NO. 9, 34345.1.1 (R-0609IA)



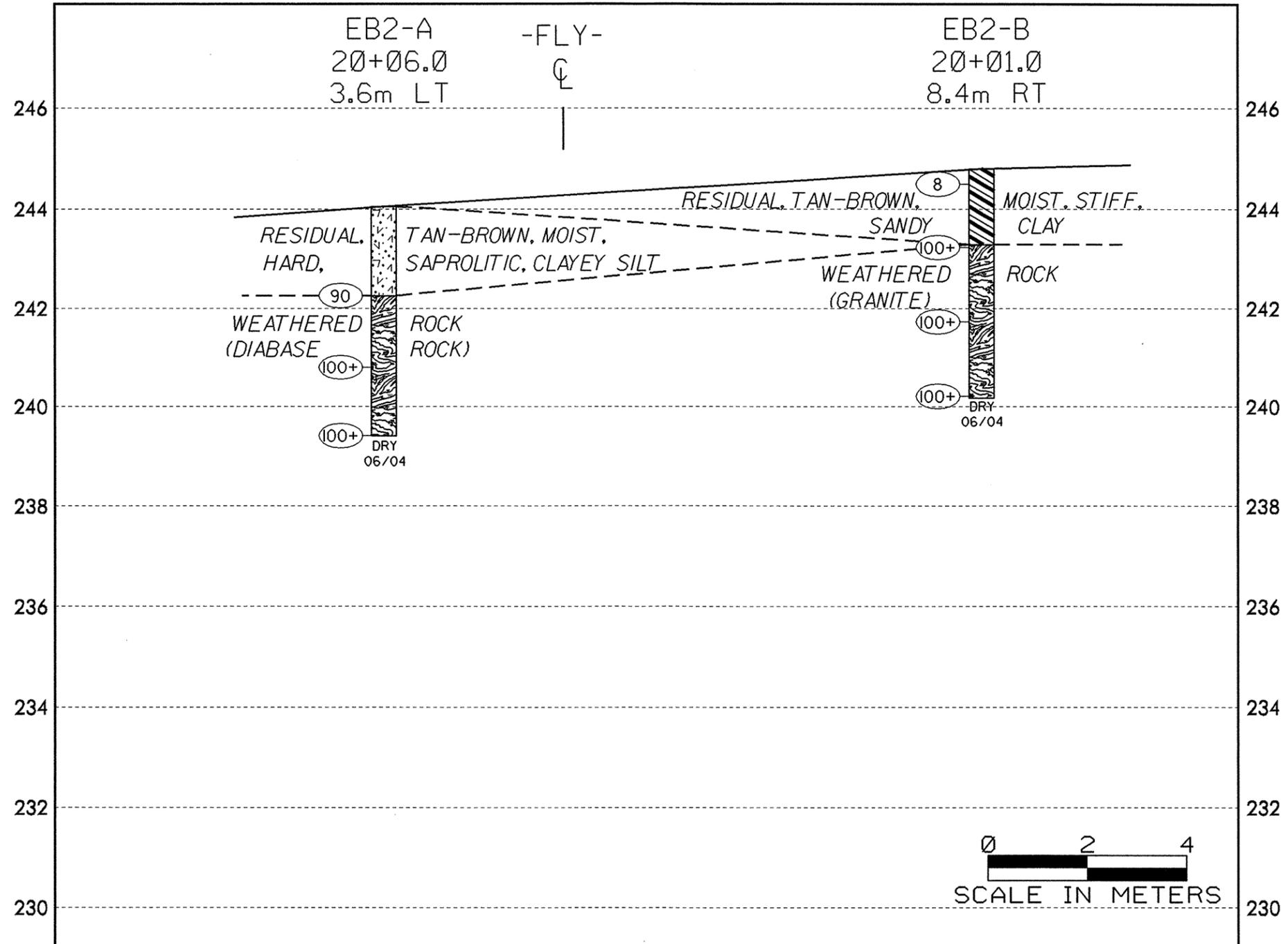
CROSS SECTION THROUGH BENT I STRUCTURE NO. 9, 34345.1.1 (R-0609IA)



(A) CRYSTALLINE ROCK, DARK GRAY, VERY SLIGHTLY WEATHERED, VERY HARD, CLOSELY FRACTURED, DIABASE REC=66% RQD=0%



CROSS SECTION THROUGH END BENT 2 STRUCTURE NO. 9, 34345.1.1 (R-0609IA)



NORTH CAROLINA DEPARTMENT OF TRANSPORTATION  
 GEOTECHNICAL UNIT BORING LOG

NORTH CAROLINA DEPARTMENT OF TRANSPORTATION  
 GEOTECHNICAL UNIT BORING LOG

SHEET 9

PROJECT NO. 34345.1.1		ID. R-06091A		COUNTY GUILFORD		GEOLOGIST O.B. OTI								
SITE DESCRIPTION STRUCTURE NO. 9 ON -FLY- (HIGH POINT EAST BELTWAY) OVER -Y6- (I-85)							GROUND WATER							
BORING NO. EBI-A		BORING LOCATION 18+51.0		OFFSET 3.6m LT	ALIGNMENT -FLY-		0 HR. 3.00m							
COLLAR ELEV. 235.43m		NORTHING 242071.2		EASTING 525963.4		24 HR. FILLED								
TOTAL DEPTH 5.15m		DRILL MACHINE MOBILE B-57		DRILL METHOD H.S. AUGERS		HAMMER TYPE MANUAL								
START DATE 6/9/04		COMPLETION DATE 6/9/04		SURFACE WATER DEPTH N/A		DEPTH TO ROCK 5.15m								
ELEV.	DEPTH	BLOW COUNT			PEN. (m)	BLOWS PER 30cm				SAMPLE NUMBER	MOI.	LOG	SOIL AND ROCK DESCRIPTION	
		15cm	15cm	15cm		0	25	50	75					100
235.43	0.00	5	4	6	0.30									
234.00	1.52	7	6	4	0.30									RESIDUAL, TAN-BROWN-GRAY, SANDY CLAY
232.00	3.04	1	2	8	0.30					SS-19	56%			
	4.56	60	40		0.30									WEATHERED ROCK (GRANITE)
AUGER REFUSAL AT ELEVATION 230.28 METERS ON CRYSTALLINE ROCK (GRANITE)														

PROJECT NO. 34345.1.1		ID. R-06091A		COUNTY GUILFORD		GEOLOGIST O.B. OTI								
SITE DESCRIPTION STRUCTURE NO. 9 ON -FLY- (HIGH POINT EAST BELTWAY) OVER -Y6- (I-85)							GROUND WATER							
BORING NO. EBI-B		BORING LOCATION 18+51.0		OFFSET 8.4m RT	ALIGNMENT -FLY-		0 HR. N/A							
COLLAR ELEV. 235.40m		NORTHING 242064.5		EASTING 525953.4		24 HR. 3.00m								
TOTAL DEPTH 5.38m		DRILL MACHINE MOBILE B-57		DRILL METHOD H.S. AUGERS		HAMMER TYPE MANUAL								
START DATE 6/9/04		COMPLETION DATE 6/9/04		SURFACE WATER DEPTH N/A		DEPTH TO ROCK 5.38m								
ELEV.	DEPTH	BLOW COUNT			PEN. (m)	BLOWS PER 30cm				SAMPLE NUMBER	MOI.	LOG	SOIL AND ROCK DESCRIPTION	
		15cm	15cm	15cm		0	25	50	75					100
235.40	0.00	5	5	6	0.30									
234.00	1.52	5	5	5	0.30									RESIDUAL, TAN-BROWN, SANDY CLAY
232.00	3.04	28	17	66	0.30					SS-18				TAN-BROWN, SANDY SILT
	4.56	38	57	35	0.30									WEATHERED ROCK (GRANITE)
AUGER REFUSAL AT ELEVATION 230.02 METERS ON CRYSTALLINE ROCK (GRANITE)														



# NORTH CAROLINA DEPARTMENT OF TRANSPORTATION GEOTECHNICAL UNIT BORING LOG

PROJECT NO. 34345.1.1		ID. R-0609IA		COUNTY GUILFORD		GEOLOGIST O.B. OTI							
SITE DESCRIPTION STRUCTURE NO. 9 ON -FLY- (HIGH POINT EAST BELTWAY) OVER -Y6- (I-85)							GROUND WATER						
BORING NO. BI-B		BORING LOCATION 18+87.0		OFFSET 8.4m RT	ALIGNMENT -FLY-		0 HR. N/A						
COLLAR ELEV. 241.78m		NORTHING 242039.7		EASTING 525971.9		24 HR. 3.70m							
TOTAL DEPTH 9.26m		DRILL MACHINE MOBILE B-57		DRILL METHOD H.S. AUGERS		HAMMER TYPE MANUAL							
START DATE 6/3/04		COMPLETION DATE 6/3/04		SURFACE WATER DEPTH N/A		DEPTH TO ROCK 4.10m							
ELEV.	DEPTH	BLOW COUNT			PEN. (m)	BLOWS PER 30cm				SAMPLE NUMBER	LOG	SOIL AND ROCK DESCRIPTION	
		15cm	15cm	15cm		0	25	50	75				100
241.78													
240.00	1.52	9	18	26	0.30					44	SS-2	M	ROADWAY EMBANKMENT, TAN-BROWN, COARSE SAND
238.00	3.04	14	9	9	0.30					18	SS-3	M	RESIDUAL, TAN-BROWN, SILTY SAND
236.00	4.00	100			0.01					100 ± X	RS-1	▼	WEATHERED ROCK (GRANITE)
234.00													CRYSTALLINE ROCK, GRAY-WHITE, FRESH, VERY HARD, VERY CLOSELY FRACTURED, GRANITE
232.00													REC=46% ROD=23%
230.00													DARK GRAY, VERY SLIGHTLY WEATHERED, VERY HARD, CLOSELY FRACTURED, DIABASE
228.00													REC=66% ROD=0%
226.00													
224.00													
222.00													

CORING TERMINATED AT ELEVATION 232.52 METERS IN CRYSTALLINE ROCK (DIABASE)

## CORE BORING REPORT

PROJECT: 34345.1.1 ID: R-0609IA COUNTY: GUILFORD BORING NO: B1-B

DESCRIPTION: Structure No. 9 on -FLY- (High Point East Beltway) over -Y6- (I-85)

LOCATION OF BORING: -FLY-, Sta. 18+87, 8.4 m RT COMPLETION DATE: 06/03/04

COLLAR or GROUND ELEVATION: 241.78 m CORE SIZE: NQWL GEOLOGIST: O.B. OTI

CORE EQUIPMENT: B-57, NQWL, H.S. Augers DRILLER: R.F. BAYRON

ELEV (m)	DEPTH (m)	DRILL RATE (min/0.5m)	RUN (m)	REC (m) (%)	RQD (m) (%)	SAMPLE NUMBER	FIELD CLASSIFICATION and REMARKS
237.58	4.20	2:50		0.20 (40%)	0.20 (40%)	RS-1 4.2-4.7m	Gray-white, fresh, very hard, very closely fractured, granite
237.08	4.70						
237.08	4.70	3:10		0.80 (53%)	0.30 (20%)		Gray-white, fresh, very hard, very closely fractured, granite
		3:11					
		4:01	1.52				
235.56	6.22						
235.56	6.22	2:56		0.63 (41%)	0.30 (20%)		Gray-white, fresh, very hard, very closely fractured, granite
		3:21					
		3:30	1.52				
234.04	7.74						
234.04	7.74	3:42		1.00 (66%)	0.00 (0%)		Dark-gray, very slightly weathered, very hard, closely fractured, diabase
		4:01					
		4:21	1.52				
232.52	9.26						

BOREHOLE TERMINATED AT ELEVATION OF 232.52 METERS, IN DIABASIC ROCK.

# NORTH CAROLINA DEPARTMENT OF TRANSPORTATION GEOTECHNICAL UNIT BORING LOG

PROJECT NO. 34345.1.1		ID. R-0609IA		COUNTY GUILFORD		GEOLOGIST O.B. OTI							
SITE DESCRIPTION STRUCTURE NO. 9 ON -FLY- (HIGH POINT EAST BELTWAY) OVER -Y6- (I-85)						GROUND WATER							
BORING NO. B2-A		BORING LOCATION 19+28.0		OFFSET 3.5m LT		ALIGNMENT -FLY-							
COLLAR ELEV. 242.63m		NORTHING 242014.4		EASTING 526013.1		0 HR. N/A							
TOTAL DEPTH 13.64m		DRILL MACHINE MOBILE B-57		DRILL METHOD H.S. AUGERS		HAMMER TYPE MANUAL							
START DATE 6/8/04		COMPLETION DATE 6/8/04		SURFACE WATER DEPTH N/A		DEPTH TO ROCK 9.08m							
ELEV.	DEPTH	BLOW COUNT			PEN. (m)	BLOWS PER 30cm				SAMPLE NUMBER	MOI.	LOG	SOIL AND ROCK DESCRIPTION
		15cm	15cm	15cm		0	25	50	75				
242.65	0.00	3	5	10	0.30								ROADWAY EMBANKMENT, TAN-BROWN, SILTY SAND
242.00	1.52	100			0.07								WEATHERED ROCK (GRANITE BOULDER)
240.00	3.04	8	8	8	0.30								ROADWAY EMBANKMENT, TAN-BROWN, SANDY SILT
238.00	4.52	5	8	9	0.30								RESIDUAL, TAN-BROWN, SANDY CLAY
236.00	6.04	4	5	9	0.30								WEATHERED ROCK (GRANITE)
234.00	7.56	26	74		0.22								CRYSTALLINE ROCK, GRAY-WHITE-BROWN, MODERATELY SEVERE WEATHERED TO FRESH, MODERATELY HARD TO VERY HARD, CLOSELY TO VERY CLOSELY FRACTURED, GRANITE
232.00	9.08	60			0.00								CRYSTALLINE ROCK, GRAY-WHITE-BROWN, MODERATELY SEVERE WEATHERED TO FRESH, MODERATELY HARD TO VERY HARD, CLOSELY TO VERY CLOSELY FRACTURED, GRANITE
228.00													CRYSTALLINE ROCK, GRAY-WHITE-BROWN, MODERATELY SEVERE WEATHERED TO FRESH, MODERATELY HARD TO VERY HARD, CLOSELY TO VERY CLOSELY FRACTURED, GRANITE
226.00													CRYSTALLINE ROCK, GRAY-WHITE-BROWN, MODERATELY SEVERE WEATHERED TO FRESH, MODERATELY HARD TO VERY HARD, CLOSELY TO VERY CLOSELY FRACTURED, GRANITE
224.00													CRYSTALLINE ROCK, GRAY-WHITE-BROWN, MODERATELY SEVERE WEATHERED TO FRESH, MODERATELY HARD TO VERY HARD, CLOSELY TO VERY CLOSELY FRACTURED, GRANITE

CORE BORING REPORT									
PROJECT: 34345.1.1		ID: R-0609IA		COUNTY: GUILFORD		BORING NO: B2-A			
DESCRIPTION: Structure No. 9 on -FLY- (High Point East Beltway) over -Y6- (I-85)									
LOCATION OF BORING: -FLY-, Sta. 19+28, 3.5m LT						COMPLETION DATE: 06/08/04			
COLLAR or GROUND ELEVATION: 242.63 m				CORE SIZE: NQWL		GEOLOGIST: O.B. OTI			
CORE EQUIPMENT: B-57, NQWL, H. S. Augers						DRILLER: R.F. BAYRON			
ELEV (m)	DEPTH (m)	DRILL RATE (min/0.5m)	RUN (m)	REC (%)	RQD (%)	SAMPLE NUMBER	FIELD CLASSIFICATION and REMARKS		
240.93	1.70	4:50					Gray- white, fresh, very hard, very closely fractured, granite boulder		
		7:45		0.50	0.50				
		4:00	1.30	(38%)	(38%)				
239.63	3.00						Material not cored. Material logged as soil and weathered rock on boring log.		
239.63	3.00								
			4.56	N/A	N/A				
235.07	7.56						Gray-tan-brown, very severe weathered, weathered rock (diabase)		
235.07	7.56	2:10							
		1:50		1.00	0.00				
		2:00	1.52	(66%)	(0%)				
233.55	9.08						Gray- white, fresh, very hard, very closely fractured, granite (9.08-9.44)		
233.55	9.08	8:12					White-brown, moderately severe weathered, moderately hard, closely fractured, granite (9.44-9.54)		
		7:56		1.26	0.84		Gray- white, fresh, very hard, very closely fractured, granite (9.54-9.79)		
		10:46	1.52	(83%)	(55%)		White-brown, moderately severe weathered, moderately hard, closely fractured, granite (9.79-10.60)		
232.03	10.60						Gray- white, fresh, very hard, very closely fractured, granite		
232.03	10.60	10:56							
		11:02		1.52	1.52	RS-2	11.18-11.32		
		8:45	1.52	(100%)	(100%)				
230.51	12.12						Gray- white, fresh, very hard, very closely fractured, granite		
230.51	12.12	11:02							
		9:22		1.52	1.52	RS-3	12.57-12.69		
		10:11	1.52	(100%)	(100%)				
228.99	13.64						BOREHOLE TERMINATED AT ELEVATION OF 228.99 METERS, IN GRANITE.		



NORTH CAROLINA DEPARTMENT OF TRANSPORTATION  
 GEOTECHNICAL UNIT BORING LOG

NORTH CAROLINA DEPARTMENT OF TRANSPORTATION  
 GEOTECHNICAL UNIT BORING LOG

SHEET 14

PROJECT NO. 34345.1.1		ID. R-06091A		COUNTY GUILFORD		GEOLOGIST O.B. OTI							
SITE DESCRIPTION STRUCTURE NO. 9 ON -FLY- (HIGH POINT EAST BELTWAY) OVER -Y6- (I-85)							GROUND WATER						
BORING NO. B3-A		BORING LOCATION 19+75.0		OFFSET 3.3m LT		ALIGNMENT -FLY-							
COLLAR ELEV. 243.13m		NORTHING 241986.5		EASTING 526051.0		0 HR. DRY							
TOTAL DEPTH 9.19m		DRILL MACHINE MOBILE B-57		DRILL METHOD H.S. AUGERS		HAMMER TYPE MANUAL							
START DATE 6/7/04		COMPLETION DATE 6/7/04		SURFACE WATER DEPTH N/A		DEPTH TO ROCK N/A							
ELEV.	DEPTH	BLOW COUNT			PEN. (m)	BLOWS PER 30cm				SAMPLE NUMBER	MOI.	LOG	SOIL AND ROCK DESCRIPTION
		15cm	15cm	15cm		0	25	50	75				
243.13													
242.00	1.52	8	30	33	0.30					63			RESIDUAL, TAN-BROWN AND TAN-GRAY, SILTY CLAY
240.00	3.04	11	16	23	0.30					39			RESIDUAL, TAN-BROWN AND TAN-GRAY, SILTY CLAY
238.00	4.56	32	68		0.20					100+	*		WEATHERED ROCK (GRANITIC ROCK INTERBEDDED WITH DIABASE ROCK)
236.00	6.08	40	60		0.23					100+	*		WEATHERED ROCK (GRANITIC ROCK INTERBEDDED WITH DIABASE ROCK)
234.00	7.60	100			0.08					100+	*		WEATHERED ROCK (GRANITIC ROCK INTERBEDDED WITH DIABASE ROCK)
234.00	9.12	100			0.07					100+	*		WEATHERED ROCK (GRANITIC ROCK INTERBEDDED WITH DIABASE ROCK)
BORING TERMINATED AT ELEVATION 233.94 METERS IN WEATHERED ROCK (GRANITE)													

PROJECT NO. 34345.1.1		ID. R-06091A		COUNTY GUILFORD		GEOLOGIST O.B. OTI							
SITE DESCRIPTION STRUCTURE NO. 9 ON -FLY- (HIGH POINT EAST BELTWAY) OVER -Y6- (I-85)							GROUND WATER						
BORING NO. B3-B		BORING LOCATION 19+70.0		OFFSET 7.6m RT		ALIGNMENT -FLY-							
COLLAR ELEV. 243.19m		NORTHING 241980.0		EASTING 526040.9		0 HR. DRY							
TOTAL DEPTH 7.60m		DRILL MACHINE MOBILE B-57		DRILL METHOD H.S. AUGERS		HAMMER TYPE MANUAL							
START DATE 6/4/04		COMPLETION DATE 6/4/04		SURFACE WATER DEPTH N/A		DEPTH TO ROCK N/A							
ELEV.	DEPTH	BLOW COUNT			PEN. (m)	BLOWS PER 30cm				SAMPLE NUMBER	MOI.	LOG	SOIL AND ROCK DESCRIPTION
		15cm	15cm	15cm		0	25	50	75				
243.19	0.00	4	6	8	0.30					14			RESIDUAL, TAN-BROWN, SANDY CLAY
242.00	1.52	29	37	40	0.30					77			TAN-BROWN, SILTY SAND
240.00	3.04	70	30		0.17					100+	*		WEATHERED ROCK (GRANITE)
238.00	4.56	100			0.04					100+	*		WEATHERED ROCK (GRANITE)
236.00	6.08	100			0.08					100+	*		WEATHERED ROCK (GRANITE)
236.00	7.60	100			0.04					100+	*		WEATHERED ROCK (GRANITE)
BORING TERMINATED AT ELEVATION 235.59 METERS IN WEATHERED ROCK (GRANITE)													

NORTH CAROLINA DEPARTMENT OF TRANSPORTATION  
 GEOTECHNICAL UNIT BORING LOG

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 GEOTECHNICAL UNIT BORING LOG

SHEET 15

PROJECT NO. 34345.1.1		ID. R-06091A		COUNTY GUILFORD		GEOLOGIST O.B. OTI							
SITE DESCRIPTION STRUCTURE NO. 9 ON -FLY- (HIGH POINT EAST BELTWAY) OVER -Y6- (I-85)							GROUND WATER						
BORING NO. EB2-A		BORING LOCATION 20+06.0		OFFSET 3.6m LT		ALIGNMENT -FLY-							
COLLAR ELEV. 244.05m		NORTHING 241969.7		EASTING 526083.8		0 HR. DRY							
TOTAL DEPTH 4.74m		DRILL MACHINE MOBILE B-57		DRILL METHOD H.S. AUGERS		HAMMER TYPE MANUAL							
START DATE 6/7/04		COMPLETION DATE 6/7/04		SURFACE WATER DEPTH N/A		DEPTH TO ROCK N/A							
ELEV.	DEPTH	BLOW COUNT			PEN. (m)	BLOWS PER 30cm				SAMPLE NUMBER	MOI.	LOG	SOIL AND ROCK DESCRIPTION
		15cm	15cm	15cm		0	25	50	75				
244.05													
244.00													
	1.52	16	35	55	0.30					90 X	SS-8	M	RESIDUAL, TAN-BROWN, SAPROLITIC, CLAYEY SILT
	3.04	33	67		0.25					100+ X			WEATHERED ROCK (DIABASE ROCK)
	4.56	70	30		0.18					100+ X			WEATHERED ROCK (DIABASE ROCK)
													BORING TERMINATED AT ELEVATION 239.31 METERS IN WEATHERED ROCK (METAVOLCANIC ROCK)

PROJECT NO. 34345.1.1		ID. R-06091A		COUNTY GUILFORD		GEOLOGIST O.B. OTI							
SITE DESCRIPTION STRUCTURE NO. 9 ON -FLY- (HIGH POINT EAST BELTWAY) OVER -Y6- (I-85)							GROUND WATER						
BORING NO. EB2-B		BORING LOCATION 20+01.0		OFFSET 8.4m RT		ALIGNMENT -FLY-							
COLLAR ELEV. 244.80m		NORTHING 241958.7		EASTING 526079.0		0 HR. DRY							
TOTAL DEPTH 4.63m		DRILL MACHINE MOBILE B-57		DRILL METHOD H.S. AUGERS		HAMMER TYPE MANUAL							
START DATE 6/7/04		COMPLETION DATE 6/7/04		SURFACE WATER DEPTH N/A		DEPTH TO ROCK N/A							
ELEV.	DEPTH	BLOW COUNT			PEN. (m)	BLOWS PER 30cm				SAMPLE NUMBER	MOI.	LOG	SOIL AND ROCK DESCRIPTION
		15cm	15cm	15cm		0	25	50	75				
244.80	0.00	3	4	4	0.30					8 X	SS-9	M	RESIDUAL, TAN-BROWN, SANDY CLAY
	1.52	100			0.28					100+ X			WEATHERED ROCK (GRANITE)
	3.04	100			0.07					100+ X			WEATHERED ROCK (GRANITE)
	4.56	100			0.07					100+ X			WEATHERED ROCK (GRANITE)
													BORING TERMINATED AT ELEVATION 240.17 METERS IN WEATHERED ROCK (GRANITE)

**EB1-A**

<b>SOIL TEST RESULTS</b>															
SAMPLE NO.	OFFSET	STATION	DEPTH INTERVAL	AASHTO CLASS.	L.L.	P.I.	% BY WEIGHT				% PASSING (SIEVES)			% MOISTURE	% ORGANIC
							C.SAND	F.SAND	SILT	CLAY	10	40	200		
SS-19	3.6m LT	18+51.0	3.04-3.49	A-6(9)	37	18	20.1	17.6	29.9	32.4	94	81	63	56.2	-

**EB1-B**

<b>SOIL TEST RESULTS</b>															
SAMPLE NO.	OFFSET	STATION	DEPTH INTERVAL	AASHTO CLASS.	L.L.	P.I.	% BY WEIGHT				% PASSING (SIEVES)			% MOISTURE	% ORGANIC
							C.SAND	F.SAND	SILT	CLAY	10	40	200		
SS-17	8.4m RT	18+51.0	0.00-0.45	A-6(5)	34	19	31.8	23.1	16.7	28.4	98	81	48	-	-
SS-18	8.4m RT	18+51.0	3.04-3.49	A-4(4)	36	7	12.0	32.4	43.5	12.2	100	93	66	-	-

**B1-A**

<b>SOIL TEST RESULTS</b>															
SAMPLE NO.	OFFSET	STATION	DEPTH INTERVAL	AASHTO CLASS.	L.L.	P.I.	% BY WEIGHT				% PASSING (SIEVES)			% MOISTURE	% ORGANIC
							C.SAND	F.SAND	SILT	CLAY	10	40	200		
SS-1	3.6m LT	18+89.0	1.52-1.97	A-1-b(0)	22	NP	52.9	25.3	13.7	8.1	70	42	18	-	-

**B1-B**

<b>SOIL TEST RESULTS</b>															
SAMPLE NO.	OFFSET	STATION	DEPTH INTERVAL	AASHTO CLASS.	L.L.	P.I.	% BY WEIGHT				% PASSING (SIEVES)			% MOISTURE	% ORGANIC
							C.SAND	F.SAND	SILT	CLAY	10	40	200		
SS-2	8.4m RT	18+87.0	1.52-1.97	A-1-b(0)	22	NP	49.2	25.5	15.1	10.1	73	48	22	-	-
SS-3	8.4m RT	18+87.0	3.04-3.49	A-2-4(0)	25	NP	54.7	27.0	8.2	10.1	98	60	22	-	-

**B2-A**

<b>SOIL TEST RESULTS</b>															
SAMPLE NO.	OFFSET	STATION	DEPTH INTERVAL	AASHTO CLASS.	L.L.	P.I.	% BY WEIGHT				% PASSING (SIEVES)			% MOISTURE	% ORGANIC
							C.SAND	F.SAND	SILT	CLAY	10	40	200		
SS-10	3.5m LT	19+28.0	3.04-3.49	A-4(0)	26	NP	54.3	20.5	17.1	8.1	74	42	37	-	-
SS-11	3.5m LT	19+28.0	4.52-4.46	A-4(0)	24	4	38.1	28.8	18.9	14.2	97	74	36	-	-
SS-12	3.5m LT	19+28.0	6.04-6.49	A-6(3)	32	16	35.1	22.1	18.5	24.3	93	73	43	-	-

**B2-B**

<b>SOIL TEST RESULTS</b>															
SAMPLE NO.	OFFSET	STATION	DEPTH INTERVAL	AASHTO CLASS.	L.L.	P.I.	% BY WEIGHT				% PASSING (SIEVES)			% MOISTURE	% ORGANIC
							C.SAND	F.SAND	SILT	CLAY	10	40	200		
SS-13	8.2m RT	19+25.0	0.00-0.45	A-2-4(0)	28	6	51.9	21.7	14.3	12.2	89	56	27	-	-
SS-14	8.2m RT	19+25.0	1.52-1.97	A-1-b(0)	23	NP	56.7	20.9	14.3	8.1	75	43	20	-	-
SS-15	8.2m RT	19+25.0	4.56-5.01	A-4(2)	29	10	30.2	24.9	22.6	22.3	97	78	48	-	-
SS-16	8.2m RT	19+25.0	6.08-6.53	A-2-4(0)	30	NP	48.8	27.2	13.9	10.1	100	67	28	-	-

**B3-A**

<b>SOIL TEST RESULTS</b>															
SAMPLE NO.	OFFSET	STATION	DEPTH INTERVAL	AASHTO CLASS.	L.L.	P.I.	% BY WEIGHT				% PASSING (SIEVES)			% MOISTURE	% ORGANIC
							C.SAND	F.SAND	SILT	CLAY	10	40	200		
SS-6	3.3m LT	19+75.0	1.52-1.97	A-7-5(7)	44	11	17.4	26.3	44.1	12.2	100	89	64	-	-
SS-7	3.3m LT	19+75.0	3.04-3.49	A-7-5(10)	48	14	12.2	27.6	46.1	14.2	100	95	69	-	-

**B3-B**

<b>SOIL TEST RESULTS</b>															
SAMPLE NO.	OFFSET	STATION	DEPTH INTERVAL	AASHTO CLASS.	L.L.	P.I.	% BY WEIGHT				% PASSING (SIEVES)			% MOISTURE	% ORGANIC
							C.SAND	F.SAND	SILT	CLAY	10	40	200		
SS-4	7.6m RT	19+70.0	0.00-0.45	A-6(3)	30	13	32.8	24.1	16.7	26.3	100	82	48	-	-
SS-5	7.6m RT	19+70.0	1.52-1.97	A-2-4(0)	27	NP	46.1	30.7	13.1	10.1	100	71	28	-	-

**EB2-A**

<b>SOIL TEST RESULTS</b>															
SAMPLE NO.	OFFSET	STATION	DEPTH INTERVAL	AASHTO CLASS.	L.L.	P.I.	% BY WEIGHT				% PASSING (SIEVES)			% MOISTURE	% ORGANIC
							C.SAND	F.SAND	SILT	CLAY	10	40	200		
SS-8	3.6m LT	20+06.0	1.52-1.97	A-5(2)	44	7	13.0	38.1	38.8	10.1	83	77	50	-	-

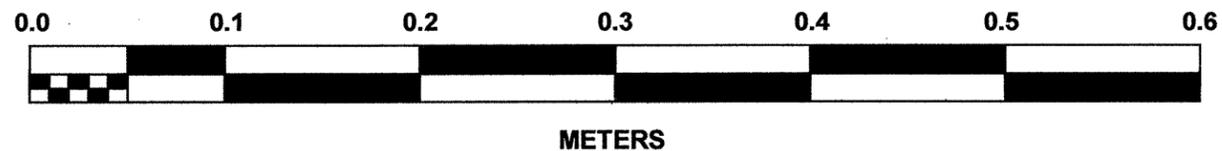
**EB2-B**

<b>SOIL TEST RESULTS</b>															
SAMPLE NO.	OFFSET	STATION	DEPTH INTERVAL	AASHTO CLASS.	L.L.	P.I.	% BY WEIGHT				% PASSING (SIEVES)			% MOISTURE	% ORGANIC
							C.SAND	F.SAND	SILT	CLAY	10	40	200		
SS-9	8.4m RT	20+01.0	0.00-0.45	A-6(2)	30	15	37.1	25.9	10.6	26.3	96	75	39	-	-

# CORE PHOTOGRAPHS

## B1-B

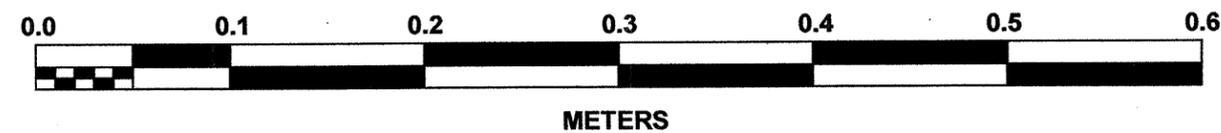
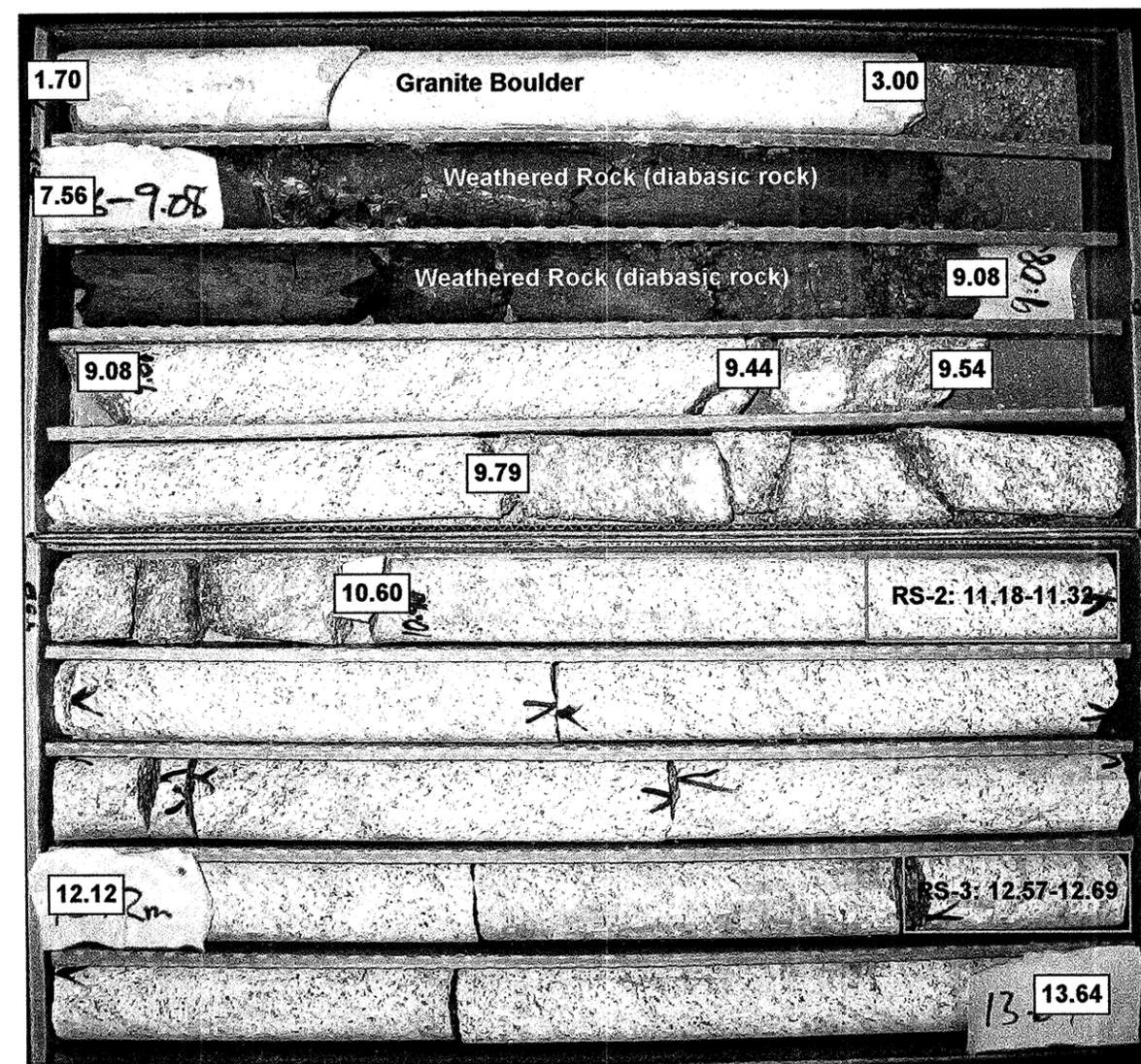
BOX 1 : 4.20 - 9.26 METERS



# CORE PHOTOGRAPHS

## B2-A

BOXES 1 & 2: 1.70 - 3.00 & 7.56 - 13.64 METERS



# SITE PHOTOGRAPH

Structure No. 9 on -FLY- ( High Point East Beltway ) over -Y6- ( I-85 )

