CONTENTS: 13+00-23+00 -L-

STATE OF NORTH CAROLINA

DEPARTMENT OF TRANSPORTATION

GEOTECHNICAL NOV 1 9 2004

N.C. B-3701 1 17 STATE PROJ.NO. BRZ-1309(2) 33241.1.1 RW, UTILI. CONST. BRZ-1309(2) BRZ-1309(2) 33241.2.1 33241.3.1 CAUTION NOTICE

THE SUBSURFACE INFORMATION AND THE SUBSURFACE INVESTIGATION ON WHICH IT IS BASED WAS MADE FOR THE PURPOSE OF STUDY, PLANNING AND DESIGN, AND NOT FOR CONSTRUCTION OR PAY PURPOSES. THE VARIOUS FIELD BORING LOGS, ROCK CORES, AND SOIL TEST DATA AVAILABLE MAY BE REVIEWED OR INSPECTED IN RALEIGH BY CONTACTING THE N.C. DEPARTMENT OF TRANSPORTATION, GEOTECHNICAL UNIT @ (9)9) 250-4088. NEITHER THE SUBSURFACE PLANS AND REPORTS, NOR THE FIELD

GENERAL SOIL AND ROCK STRATA DESCRIPTIONS AND INDICATED BOUNDARIES ARE BASED ON A
GEOTECHNICAL INTERPRETATION OF ALL AVAILABLE SUBSURFACE DATA AND MAY NOT NECESSARILY
REFLECT THE ACTUAL SUBSURFACE CONDITIONS BETWEEN BORINGS OR BETWEEN SAMPLED STRATA

REFLECT THE ACTUAL SUBSURFACE CONDITIONS BETWEEN BORINGS OR BETWEEN SAMPLED STRATA WITHIN THE BOREHOLE. THE LABORATORY SAMPLE DATA AND THE IN SITU IN-PLACED TEST DATA CAN BE RELIED ON ONLY TO THE DEGREE OF RELIABILITY INHERENT IN THE STANDARD TEST METHOD. THE OBSERVED WATER LEVELS OR SOIL MOISTURE CONDITIONS INDICATED IN THE SUBSURFACE INVESTIGATIONS ARE AS RECORDED AT THE TIME OF THE INVESTIGATION. THESE WATER LEVELS OR SOIL MOISTURE CONDITIONS MAY VARY CONSIDERABLY WITH TIME ACCORDING TO CLIMATIC CONDITIONS INCLUDING TEMPERATURES, PRECIPITATION AND WIND, AS WELL AS OTHER NON-CLIMATIC FACTORS.

INFORMATION ON THIS PROJECT. THE DEPARTMENT DOES NOT WARRANT OR GUARANTEE THE SUFFICIENCY OR ACCURACY OF THE INVESTIGATION MADE, NOR THE INTERPRETATIONS MADE OR OPINION OF THE DEPARTMENT AS TO THE TYPE OF MATERIALS AND CONDITIONS TO BE ENCOUNTERED. THE BIDDER OR

CONTRACTOR IS CAUTIONED TO MAKE SUCH INDEPENDENT SUBSURFACE INVESTIGATIONS AS HE DEEMS
NECESSARY TO SATISFY HIMSELF AS TO CONDITIONS TO BE ENCOUNTERED ON THIS PROJECT. THE
CONTRACTOR SHALL HAVE NO CLAIM FOR ADDITIONAL COMPENSATION OR FOR AN EXTENSION OF TIME FOR

ANY REASON RESULTING FROM THE ACTUAL CONDITIONS ENCOUNTERED AT THE SITE DIFFERING FROM THOSE INDICATED IN THE SUBSURFACE INFORMATION.

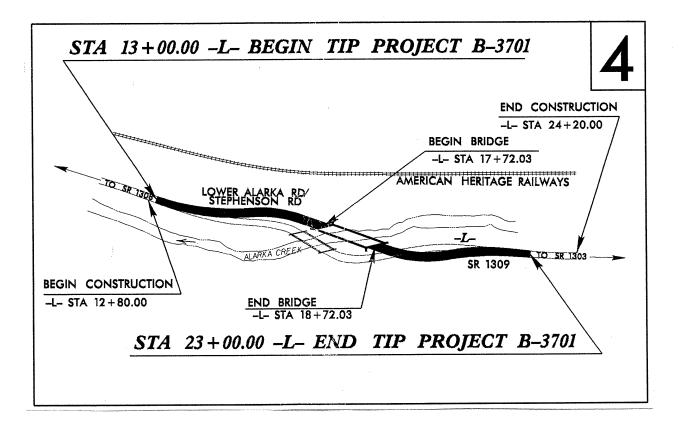
THE RIDDER OR CONTRACTOR IS CAUTIONED THAT DETAILS SHOWN ON THE SUBSURFACE PLANS ARE PRELIMINARY ONLY AND IN MANY CASES THE FINAL DESIGN DETAILS ARE DIFFERENT, FOR BIDDING AND CONSTRUCTION PURPOSES, REFER TO THE CONSTRUCTION PLANS AND DOCUMENTS FOR FINAL DESIGN

BORING LOGS, ROCK CORES, OR SOIL TEST DATA IS PART OF THE CONTRACT.

DIVISION OF HIGHWAYS GEOTECHNICAL UNIT

SUBSURFACE INVESTIGATION

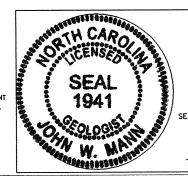
STATE PROJECT 332413.1 I.D. NO. B-3701 BRZ-1309(2) F.A. PROJECT COUNTY____ SWAIN DESCRIPTION APPROACHES TO BRIDGE NO. 106 ON SR-1309 OVER ALARKA CR.



J.W. MANN INVESTIGATED BY___ W.D. FRYE FLORENCE & HUTCHESON, INC W.D. FRYE CONSULTING ENGINEERS NOVEMBER 2004

NOTE - THE INFORMATION CONTAINED HEREIN IS NOT IMPLIED OR GUARANTEED BY THE N. C. DEPARTMENT OF TRANSPORTATION AS BEING ACCURATE NOR IT IS CONSIDERED TO BE PART OF THE PLANS,

NOTE - BY HAVING REQUESTED THIS INFORMATION THE CONTRACTOR SPECIFICALLY WAIVES ANY CLAIMS FOR INCREASED COMPENSATION OR EXTENSION OF TIME BASED ON DIFFERENCES BETWEEN THE CONDITIONS INDICATED HEREIN AND THE ACTUAL CONDITIONS AT THE PROJECT SITE.



DRAWN BY: J.W. MANN

NORTH CAROLINA DEPARTMENT OF TRANSPORTATION

DIVISION OF HIGHWAYS

GEOTECHNICAL UNIT

SUBSURFACE INVESTIGATION

	SOIL AND RO	OCK LEGEND, TERMS,	SYMBOLS, AND ABBREVIATIONS		
SOIL DESCRIPTION	GRADATION		ROCK DESCRIPTION		TERMS AND DEFINITIONS
SOIL IS CONSIDERED TO BE THE UNCONSOLIDATED, SEMI-CONSOLIDATED OR WEATHERED EARTH MATERIALS	WELL GRADED- INDICATES A GOOD REPRESENTATION OF PARTICLE SIZES I UNIFORM- INDICATES THAT SOIL PARTICLES ARE ALL APPROXIMATELY THE	FROM FINE TO COARSE E SAME SIZE.(ALSO	HARD ROCK IS NON-COASTAL PLAIN MATERIAL THAT WHEN TESTED, WOUL ROCK LINE INDICATES THE LEVEL AT WHICH NON-COASTAL PLAIN MATER	TAL WOULD YIELD SPT REFUSAL.	ALLUVIUM (ALLUV.) - SOILS WHICH HAVE BEEN TRANSPORTED BY WATER.
WHICH CAN BE PENETRATED WITH A CONTINUOUS FLIGHT POWER AUGER, AND WHICH YIELDS LESS THAN 100 BLOWS PER FOOT ACCORDING TO STANDARD PENETRATION TEST (AASHTO 1208, ASTM 0-1586). SOIL	POORLY GRADED) GAP-GRADED- INDICATES A MIXTURE OF UNIFORM PARTICLES OF TWO OR I		SPT REFUSAL IS PENETRATION BY A SPLIT SPOON SAMPLER EDUAL TO IN NON-COASTAL PLAIN MATERIAL, THE TRANSITION BETWEEN SOIL AND	OR LESS THAN 0.1 FOOT PER 60 BLOWS. ROCK IS OFTEN REPRESENTED BY A ZON	ACCUSER - A WATER BEARING FORMATION OR STRATA. ARENACEOUS - APPLIED TO ROCKS THAT HAVE BEEN DERIVED FROM SAND OR THAT CONTAIN SAND.
CLASSIFICATION IS BASED ON THE AASHTO SYSTEM AND BASIC DESCRIPTIONS GENERALLY SHALL INCLUDE: CONSISTENCY, COLOR, TEXTURE, MOISTURE, AASHTO CLASSIFICATION, AND OTHER PERTINENT FACTORS SUCH	ANGULARITY OF GRAINS		OF WEATHERED ROCK. ROCK MATERIALS ARE TYPICALLY DIVIDED AS FOLOWS:		ARGILLACEDUS - APPLIED TO ALL ROCKS OR SUBSTANCES COMPOSED OF CLAY MINERALS,
AS MINERALOGICAL COMPOSITION, ANGULARITY, STRUCTURE, PLASTICITY, ETC. EXAMPLE: VERY STIFF, GRAY SUTY CLAY, MOST WITH WITERBEDDED FINE SAND LAYERS, HIGHLY PUSTIC, A-7-6	THE ANGULARITY OR ROUNDNESS OF SOIL GRAINS ARE DESIGNATED BY TH SUBANGULAR, SUBROUNDED, OR ROUNDED.		VEATHERED NON-COASTAL PLAIN MATERIAL THAT	YIELDS SPT N VALUES > 100 BLOWS	OR HAVING A NOTABLE PROPORTION OF CLAY IN THEIR COMPOSITION, AS SHALE, SLATE, ETC.
SOIL LEGEND AND AASHTO CLASSIFICATION	MINERALOGICAL COMPOSITI	ION	RUCK (WR) PER FOOT. RYSTALLINE FINE TO COARSE GRAIN IGNEOUS AND	METAMORPHIC ROCK THAT	ARTESIAN - GROUND WATER THAT IS UNDER SUFFICIENT PRESSURE TO RISE ABOVE THE LEVEL AT WHICH IS IS ENCOUNTERED, BUT WHICH DOES NOT NECESSARILY RISE TO OR ABOVE THE
GENERAL GRANULAR MATERIALS SILT-CLAY MATERIALS ORGANIC MATERIAL (195% PASSING *200) (195% PASSING *200)	MINERAL NAMES SUCH AS QUARTZ, FELDSPAR, MICA, TALC, KAOLIN, ETC. ARE WHENEVER THEY ARE CONSIDERED OF SIGNIFICANCE.	USED IN DESCRIPTIONS	WOULD YIELD SPT REFUSAL IF TESTE GNEISS, GABBRO, SCHIST, ETC.		GROUND SURFACE. CALCAREOUS (CALC.) - SDILS WHICH CONTAIN APPRECIABLE AMOUNTS OF CALCIUM CARBONATE.
GROUP A-1 A-3 A-2 A-4 A-5 A-6 A-7 A-1, A-2 A-4, A-5	COMPRESSIBILITY		ON-CRYSTALLINE FINE TO COARSE GRAIN METAMORPHIC	AND NON-COASTAL PLAIN D SPT REFUSAL IF TESTED, ROCK TYPE	COLLUVIUM - ROCK FRAGMENTS MIXED WITH SOIL DEPOSITED BY GRAVITY ON SLOPE OR AT BOTTOM
CLASS. A-1-a A-1-b A-2-4 A-2-5 A-2-6 A-2-7 A-7-6 A-3 A-6, A-7	SLIGHTLY COMPRESSIBLE LIQUID LIMI MODERATELY COMPRESSIBLE LIQUID LIMI	T LESS THAN 30	OCK (NCR) SEDIMENTAL PHALITE, SANDSTON OASTAL PLAIN COASTAL PLAIN COASTAL PLAIN SEDIMENTS CEMENTED		OF SLOPE. CORE RECOVERY (REC.) - TOTAL LENGTH OF ALL MATERIAL RECOVERED IN THE CORE BARREL DIVIDED BY TOTAL
SYMBOL 000000000000000000000000000000000000	HIGHLY COMPRESSIBLE LIQUID LIMI	T GREATER THAN 50	EDIMENTARY ROCK SPT REFUSAL, ROCK TYPE INCLUDES L SHELL BEDS, ETC.		LENGTH OF CORE RUN AND EXPRESSED AS A PERCENTAGE.
% PASSING SILT-GRANULAR SILT-CLAY	PERCENTAGE OF MATERIA ORGANIC MATERIAL GRANULAR SILT-CLAY GRANULAR SILT-CLAY		WEATHERING		DIKE - A TABULAR BODY OF IGNEOUS ROCK THAT CUTS ACROSS THE STRUCTURE OF ADJACENT ROCKS OR CUTS MASSIVE ROCK.
# 40 30 MX50 MX51 MN SOILS SOI	SULS SULS	OTHER MATERIAL RACE 1 - 10%	RESH ROCK FRESH, CRYSTALS BRIGHT, FEW JOINTS MAY SHOW SLIG HAMMER IF CRYSTALLINE.	SHT STAINING. ROCK RINGS UNDER	DIP - THE ANGLE AT WHICH A STRATUM OR ANY PLANAR FEATURE IS INCLINED FROM THE
L100JD LINIT 48 MX41 MN 48 MX41 MN 48 MX 41 MN 48 MX 41 MN SOILS WITH	LITTLE ORGANIC MATTER 3 - 5% 5 - 12% L	ITTLE 10 - 20%	HAMMER IF CRISTALLINE. ERY SLIGHT ROCK GENERALLY FRESH, JOINTS STAINED, SOME JOINTS MAY	SHOW THIN CLAY COATINGS IF OPEN,	HORIZONTAL. DIP_DIRECTION (DIP_AZIMUTH) - THE DIRECTION OR BEARING OF THE HORIZONTAL TRACE OF
PLASTIC INDEX 6 MX N.P. 10 MX 10 MX 11 MN 11 MN 10 MX 10 MX 11 MN 11 MN LITTLE OR	GHLY HIGHLY ORGANIC >10% >20% H	IGHLY 35% AND ABOVE	/.SLI.) CRYSTALS ON A BROKEN SPECIMEN FACE SHINE BRIGHTLY. RI OF A CRYSTALLINE NATURE.	OCK RINGS UNDER HAMMER BLOWS IF	THE LINE OF DIP, MEASURED CLOCKWISE FROM NORTH.
AMOUNTS OF	IGANIC GROUND WATER ULS WATER LEVEL IN BORE HOLE IMMEDIATELY AFTE		LIGHT ROCK GENERALLY FRESH, JOINTS STAINED AND DISCOLORATIO		FAULT - A FRACTURE OR FRACTURE ZONE ALONG WHICH THERE HAS BEEN DISPLACEMENT OF THE SIDES RELATIVE TO ONE ANOTHER PARALLEL TO THE FRACTURE.
OF MAJOR GRAVEL AND GR	WATER LEVEL IN BORE HOLE IMMEDIATELY AFTE.	R DRILLING.	1 INCH. OPEN JOINTS MAY CONTAIN CLAY. IN GRANITOID ROC CRYSTALS ARE DULL AND DISCOLORED. CRYSTALLINE ROCKS		FISSILE - A PROPERTY OF SPLITTING ALONG CLOSELY SPACED PARALLEL PLANES.
MATERIALS SHAU	\\\\\\\\\\\\\\\\\\\\\\\\\\\\\\\\\\\\\\	I .	(ODERATE SIGNIFICANT PORTIONS OF ROCK SHOW DISCOLORATION AND MOD.) GRANITOID ROCKS, MOST FELDSPARS ARE DULL AND DISCOLOR		FLOAT - ROCK FRAGMENTS ON SURFACE NEAR THEIR ORIGINAL POSITION AND DISLODGED FROM PARENT MATERIAL.
	SUITABLE TENENES WITTER, SATISTATES ZONE ON WITTER BET	ARING STRATA	DULL SOUND UNDER HAMMER BLOWS AND SHOWS SIGNIFICANT WITH FRESH ROCK.		FLOOD PLAIN (F.P.) - LAND BORDERING A STREAM, BUILT OF SEDIMENTS DEPOSITED BY
P.I. OF A-7-5 ≤ L.L 30 : P.I. OF A-7-6 > L.L 30	SPRING OR SEEPAGE		ODERATELY ALL ROCK EXCEPT QUARTZ DISCOLORED OR STAINED. IN GRA	NITOID ROCKS, ALL FELDSPARS DULL	THE STREAM.
CONSISTENCY OR DENSENESS RANGE OF STANDARD RANGE OF UNCONFIL	MISCELLANEOUS SYMBOL		EVERE AND DISCOLORED AND A MAJORITY SHOW KAOLINIZATION. ROI 40D. SEV.) AND CAN BE EXCAVATED WITH A GEOLOGIST'S PICK. ROCK GI		FORMATION (FM.) - A MAPPABLE GEOLOGIC UNIT THAT CAN BE RECOGNIZED AND TRACED IN THE FIELD.
PRIMARY SOIL TYPE COMPACTNESS OR CONSISTENCY PENETRATION RESISTENCE COMPRESSIVE STREET (N-VALUE) (TONS/FT?)		RING SAMPLE DESIGNATIONS	IF TESTED, WOULD YIELD SPT REFUSAL		JOINT - FRACTURE IN ROCK ALONG WHICH NO APPRECIABLE MOVEMENT HAS OCCURRED.
GENERALLY VERY LOOSE 44	SOIL SYMBOL AUGER BORING	DESIGNATIONS	EVERE ALL ROCKS EXCEPT QUARTZ DISCOLORED OR STAINED. ROCK SEV.) IN STRENGTH TO STRONG SOIL. IN GRANITOID ROCKS ALL FE		D <u>LEDGE</u> - A SHELF-LIKE RIDGE OR PROJECTION OF ROCK WHOSE THICKNESS IS SMALL COMPARED TO ITS LATERAL EXTENT.
GRANULAR LUUSE 4 TO 10 N/A	ARTIFICIAL FILL OTHER THAN	' S- BULK SAMPLE SS- SPLIT SPOON	EXTENT. SOME FRAGMENTS OF STRONG ROCK USUALLY REMA! IF TESTED, YIELDS SPT N VALUES > 100 BPF	IN.	LENS - A BODY OF SOIL OR ROCK THAT THINS OUT IN ONE OR MORE DIRECTIONS.
MATERIAL DENSE 30 TO 50 VERY DENSE 500	ROADWAY EMBANKMENTS - CORE BORING	SAMPLE V	ERY SEVERE ALL ROCK EXCEPT QUARTZ DISCOLORED OR STAINED. ROCK F		MOTTLED (MOT.) - IRREGULARLY MARKED WITH SPOTS OF DIFFERENT COLORS, MOTTLING IN SOILS USUALLY INDICATES POOR AERATION AND LACK OF GOOD DRAINAGE.
VERY SOFT <2 <0.25	INFERRED SOIL BOUNDARIES MONITORING W	CAMPI E	7. SEV.) THE MASS IS EFFECTIVELY REDUCED TO SOIL STATUS, WITH REMAINING. SAPROLITE IS AN EXAMPLE OF ROCK WEATHERED	TO A DEGREE SUCH THAT ONLY MINOR	PERCHED WATER - WATER MAINTAINED ABOVE THE NORMAL GROUND WATER LEVEL BY THE PRESENCE OF AN
GENERALLY SOFT 2 TO 4 0.25 TO 0.5 SILT-CLAY MEDIUM STIFF 4 TO 8 0.5 TO 1	INFERRED ROCK LINE A PIEZOMETER	RS- ROCK SAMPLE	VESTIGES OF THE ORIGINAL ROCK FABRIC REMAIN. IF TEST		INTERVENING IMPERVIOUS STRATUM
MATERIAL STIFF 8 TO 15 1 TO 2	TTTT ALLUVIAL SOIL BOUNDARY INSTALLATION	RT- RECOMPACTED	OMPLETE ROCK REDUCED TO SOIL. ROCK FABRIC NOT DISCERNIBLE, OR SCATTERED CONCENTRATIONS. QUARTZ MAY BE PRESENT AS I		RESIDUAL SOIL - SOIL FORMED IN PLACE BY THE WEATHERING OF ROCK. ROCK QUALITY DESIGNATION (R.Q.D.) - A MEASURE OF ROCK QUALITY DESCRIBED BY TOTAL LENGTH OF
(COHESIVE) VERY STIFF 15 TO 30 2 TO 4 HARD >30 >4	25/025 DIP/DIP DIRECTION OF SLOPE INDICATION	i on	ALSO AN EXAMPLE. ROCK HARDNESS		ROCK SEGMENTS EQUAL TO OR GREATER THAN 4 INCHES DIVIDED BY THE TOTAL LENGTH OF CORE RUN AND EXPRESSED AS A PERCENTAGE.
TEXTURE OR GRAIN SIZE	ROCK STRUCTURES - SPT N-VALUE		VERY HARD CANNOT BE SCRATCHED BY KNIFE OR SHARP PICK, BREAKIN	IC OF HAND SPECIMENS DECITIOES	SAPROLITE (SAP.) - RESIDUAL SOIL WHICH RETAINS THE RELIC STRUCTURE OR FABRIC OF THE
U.S. STD. SIEVE SIZE 4 10 40 60 200 270 OPENING (MM) 4.76 2.0 0.42 0.25 0.075 0.053	• - SOUNDING ROD REF— SPT REFUSAL		SEVERAL HARD BLOWS OF THE GEOLOGISTS PICK.	O OF TIME STEETHERS REGULES	PARENT ROCK. SILL - AN INTRUSIVE BODY OF IGNEOUS ROCK OF APPROXIMATELY UNIFORM THICKNESS AND
POULDED CORRE CRAVEL COARSE FINE STIT	ABBREVIATIONS AY ABBREVIATIONS		HARD CAN BE SCRATCHED BY KNIFE OR PICK ONLY WITH DIFFICU TO DETACH HAND SPECIMEN.	LTY. HARD HAMMER BLOWS REQUIRED	RELATIVELY THIN COMPARED WITH ITS LATERAL EXTENT, WHICH HAS BEEN EMPLACED PARALLEL TO THE BEDDING OR SCHISTOSITY OF THE INTRUDED ROCKS
(BLDR.) (COB.) (GR.) (SE. SD.) (F. SD.) (SL.)	-AT AR - AUGER REFUSAL PMT - PRESS BT - BORING TERMINATED SD SAND, S CL CLAY SL SILT, S		MODERATELY CAN BE SCRATCHED BY KNIFE OR PICK, GOUGES OR GROOVE HARD EXCAVATED BY HARD BLOW OF A GEOLOGISTS PICK, HAND		SLICKENSIDE - POLISHED AND STRIATED SURFACE THAT RESULTS FROM FRICTION ALONG A FAULT OR SLIP PLANE.
GRAIN MM 305 75 2.0 0.25 0.05 0.005 SIZE IN 12' 3'	CPT - CONE PENETRATION TEST SLI, S CPT - CORE ENETRATION TEST TRICO CSE COARSE TCR - TRICO	TLY	BY MODERATE BLOWS. MEDIUM CAN BE GROOVED OR GOUGED 0.05 INCHES DEEP BY FIRM I	PRESSURE OF KNIFE OR PICK POINT.	STANDARD PENETRATION TEST (PENETRATION RESISTANCE) (SPT) - NUMBER OF BLOWS (N OR B.P.F.) OF
SOIL MOISTURE - CORRELATION OF TERMS	DMT - DILATOMETER TEST		HARD CAN BE EXCAVATED IN SMALL CHIPS TO PEICES 1 INCH MA POINT OF A GEOLOGISTS PICK.	XIMUM SIZE BY HARD BLOWS OF THE	A 140 LB. HAMMER FALLING 30 INCHES REDUIRED TO PRODUCE A PENETRATION OF 1 FOOT INTO SOIL WITH A 2 INCH OUTSIDE DIAMETER SPLIT SPOON SAMPLER. SPT REFUSAL IS LESS THAN 0.1 FOOT PENETRATION
SOIL MOISTURE SCALE FIELD MOISTURE GUIDE FOR FIELD MOISTURE DESC (ATTERBERG LIMITS) DESCRIPTION	PTION e - VOID RATIO	UNIT WEIGHT	SOFT CAN BE GROVED OR GOUGED READILY BY KNIFE OR PICK. C		WITH 60 BLOWS. STRATA CORE RECOVERY (SREC.) - TOTAL LENGTH OF STRATA MATERIAL RECOVERED DIVIDED BY TOTAL LENGTH
- SATURATED - USUALLY LIQUID: VERY WET, USUAL	F FINE W - MOISTUF Y FOSS FOSSILIFEROUS V VERY		FROM CHIPS TO SEVERAL INCHES IN SIZE BY MODERATE BE PIECES CAN BE BROKEN BY FINGER PRESSURE.	LOWS OF A PICK POINT, SMALL, THIN	OF STRATUM AND EXPRESSED AS A PERCENTAGE.
(SAT.) FROM BELDW THE GROUND WATER	ABLE FRAC FRACTURED VST - VANE FRAGS FRAGMENTS	SHEAR TEST	VERY CAN BE CARVED WITH KNIFE. CAN BE EXCAVATED READILY SOFT OR MORE IN THICKNESS CAN BE BROKEN BY FINGER PRESSU		STRATA ROCK QUALITY DESIGNATION (S.R.O.D.) - A MEASURE OF ROCK QUALITY DESCRIBED BY: TOTAL LENGTH OF ROCK SEGMENTS WITHIN A STRATUM EQUAL TO OR GREATER THAN 4 INCHES DIVIDED BY THE TOTAL LENGTH OF STRATA AND EXPRESSED AS A PERCENTAGE.
PLASTIC SEMISOLID; REQUIRES DRYING TO	MED MEDIUM EQUIPMENT USED ON SUBJECT	PROJECT	FINGERNAIL. FRACTURE SPACING	BEDDING	TOPSOIL (T.S.) - SURFACE SOILS USUALLY CONTAINING ORGANIC MATTER.
(PI) PLASTIC LIMIT ATTAIN OPTIMUM MOISTURE		HAMMER TYPE:	TERM SPACING TERM	THICKNESS	BENCH MARK:
OM OPTIMUM MOISTURE - MOIST - (M) SOLID; AT OR NEAR OPTIMUM MO	STURE	AUTOMATIC X MANUAL	VERY WIDE MORE THAN 10 FEET VERY THICKLY BEDG	DED 1.5 - 4 FEET	
SL SHRINKAGE LIMIT	——— MOBILE B-		MODERATELY CLOSE 1 TO 3 FEET THINLY BEDDE		ELEVATION:
REQUIRES ADDITIONAL WATER TO - DRY - (D) ATTAIN OPTIMUM MOISTURE	BK-51 6° CONTINUOUS FLIGHT AUGER 8° HOLLOW AUGERS	CORE SIZE:	CLOSE 0.16 TO 1 FEET THICKLY LAMIN VERY CLOSE LESS THAN 0.16 FEET THICKLY LAMIN THINLY LAMIN	INATED 0.008 - 0.03 FEET	NOTES:
PLASTICITY	X CME-45 HARD FACED FINGER BITS	-B	INDURATION	19125	
PLASTICITY INDEX (PI) DRY STRENGTH	TUNG,-CARBIDE INSERTS	1	OR SEDIMENTARY ROCKS, INDURATION IS THE HARDENING OF THE MATERIAL	BY CEMENTING, HEAT, PRESSURE, ETC.	
NONPLASTIC 0-5 VERY LOW LOW PLASTICITY 6-15 SLIGHT	CME-550 X CASING X W/ ADVANCER		FRIABLE RUBBING WITH FINGER FREES GENTLE BLOW BY HAMMER DIS		
MED. PLASTICITY 16-25 MEDIUM	PORTABLE HOIST TRICONE *STEEL TEETH	HAND TOOLS: POST HOLE DIGGER		ROM SAMPLE WITH STEEL PROBE:	
HIGH PLASTICITY 26 OR MORE HIGH COLOR	TRICONE *TUNGCARB.	HAND AUGER	BREAKS EASILY WHEN HIT WIT	TH HAMMER.	
CULUK DESCRIPTIONS MAY INCLUDE COLOR OR COLOR COMBINATIONS (TAN, RED, YEL-BRN, BLUE-GRAY	OTHER CORE BIT	SOUNDING ROD	INDURATED GRAINS ARE DIFFICULT TO SE DIFFICULT TO BREAK WITH HA		
MODIFIERS SUCH AS LIGHT, DARK, STREAKED, ETC. ARE USED TO DESCRIBE APPEARANCE.	OTHEROTHER	VANE SHEAR TEST	EXTREMELY INDURATED SHARP HAMMER BLOWS REGULF	RED TO BREAK SAMPLE;	
		OTHER	SAMPLE BREAKS ACROSS GRAI	INS.	DEVICED 09 /K /00



STATE OF NORTH CAROLINA DEPARTMENT OF TRANSPORTATION

MICHAEL F. EASLEY
GOVERNOR

LYNDO TIPPETT SECRETARY

November 2004

STATE PROJECT:

33241.1.1 (B-3701)

F. A. PROJECT:

BRZ-1309(2)

COUNTY:

Swain

DESCRIPTION:

Approaches to Bridge No. 106 on SR-1309 over Alarka Creek

SUBJECT:

Geotechnical Report - Inventory

Project Description

This project is located in southwestern Swain County north of the community of Lauada and southwest of Bryson City. The project consists of the relocation of approaches to Bridge No. 106 approximately 70 feet upstream from the existing crossing. Construction will primarily entail the excavation of cut slopes left and right of centerline approaching 65 feet in height. Embankment placement is minimal. Two retaining walls are proposed to prevent the side slopes of fills from toeing up in the floodplain of Alarka Creek. Total length of this project is 0.189 miles.

A geotechnical investigation was conducted by private consultants during August and September of 2004 utilizing a CME-45 skid rig equipped with a manual hammer for Standard Penetration Testing (SPT). NXWL drill rod was used for the retrieval of rock core specimens.

The following survey lines were investigated.

<u>Line</u>	Station Interval
-L-	13+00 – 23+00
-DRIVE-	10+00 – 13+00

MAILING ADDRESS:
NC DEPARTMENT OF TRANSPORTATION
GEOTECHNICAL ENGINEERING UNIT
1589 MAIL SERVICE CENTER
RALEIGH NC 27699-1589

TELEPHONE: 919-250-4088 FAX: 919-250-4237

WEBSITE: WWW.DOH.DOT.STATE.NC.US

LOCATION: CENTURY CENTER COMPLEX BUILDING B 1020 BIRCH RIDGE DRIVE RALEIGH NC 27610

Areas of Special Geotechnical Interest

1) Hard Rock: Hard rock dominates the majority of the proposed cut slopes. Hard rock was found or can be expected at or above grade in the following areas.

Line	Station Interval
-L-	14+50 – 16+50
-L-	19+00-23+00
-DRIVE-	10+50-12+00

Physiography and Geology

The project area is located in the Blue Ridge physiographic province. The terrain in the area is mountainous. The corridor is incised into a gorge with topographic relief over 300 feet. The area is drained primarily by Alarka Creek which flows into Fontana Lake approximately 3 miles from the beginning of the project.

The extensive rock along the corridor is identified as a biotite granitic gneiss unit that lies as an unconformity in the Blue Ridge Belt. Locally the rock approaches a quartz-monzonitic composition, rich in feldspar, quartz, and lesser mica. However, this rock unit is noted for its heterogeneity.

Rock Properties

Retrieved core specimens revealed generally high recovery numbers, but variable RQD (Rock Quality Designation) values. The core averaged fair (approximately 50%). Rock in the area has wavy foliation, and is closely fractured.

Geotechnical Descriptive Analysis of the Project

For descriptive purposes, this project is divided into segments based on cross-section development.

1) <u>-L- Stations 13+00 to 17+00</u>

This section involves cut slopes to be constructed to the left of the alignment centerline. Slopes along this interval will approach a height of 65 feet, and will be built out of approximately 10 feet of colluvium underlain by hard rock. The colluvium consists of sandy soil and large boulders. It should be noted that due to the size of boulders, excavation of the colluvium should be treated in the same manner as that of hard rock. Rock in this section is moderately severe to very slightly weathered and closely fractured at angles from 35 to 65 degrees.

2) <u>-L- Stations 17+00 to 19+50</u>

Bridge Span and approach fills.

3) <u>-L- Stations 19+50 to 23+00</u>

Cuts will be developed right of centerline through this interval, with a maximum height of \pm 50 feet. As in the aforementioned cut interval, these excavations will be constructed in approximately 10 feet of colluvium deposited over hard rock. Boulders are not as widespread in this section, and hard rock possesses the same previously described characteristics.

4) -DRIVE- Stations 10+00 to 13+00

A drive is to be constructed to provide access to a private road, resulting in up to 60-foot high cuts built to the left of this alignment. Again, these excavations will be developed in 10 feet of colluvium and moderately weathered to fresh, moderately close to closely fractured hard rock.

5) Retaining Wall Stations 19+50, 21+50 to 23+00

The retaining walls will be founded in existing embankment and on hard rock. Embankment materials are comprised of 10 to 20 feet of loose to very dense sand, rock fragments, and boulders. The embankment has been placed on colluvial boulders, saprolite, medium dense sand, and hard rock. Coring was required to penetrate the colluvium.

Respectfully Submitted,

John W. Mann, LG

Project Engineering Geologist

PROJECT: B-3701	-	-	COUNTY:	Swain		EARTHWO	RK ESTIMA	TE				TIP: B-37	01	
	EXCAVATION				EMBANKMENT				WASTE					
	TOTAL	ROCK	UNDERCUT	UNSUITABLE	SUITABLE	TOTAL	ROCK	EARTH	EMBANKMENT	BORROW	ROCK	SUITABLE	UNSUITABLE	TOTAL
LOCATION	EXCAVATION	EXCAV.	EXCAV.	EXCAV.	EXCAV.	EMBANKMENT	EMBANK.	EMBANKMENT	PLUS 15%		WASTE	WASTE	WASTE	WASTE
L 13+00 TO 17+72.03	4299	903			3396	298	238		238		665	3396	1	406
L 18+72.03 TO 23+00	5230	2968			2262	514	411		411		2557	2262		4819
DRIVE 10+11 TO 13+00	8562	5314			3248	15	12		12		5302	3248		8550
SUB-TOTAL 1	18091	9185			8906		662		662		8523	8906		1742
								· · · · · · · · · · · · · · · · · · ·						
				· · · · · · · · · · · · · · · · · · ·										
										:	,			
											`			
											·			
					,									
							,							
TOTAL	18091	9185			8906	827	662		662	÷	8523	8906		17429
LOSS DUE TO CLEARING & GRUBBING	-1450				-1450							-1450		-1450
ADJUSTMENT FOR ROCK SWELL										5	. 0404			
OVV LLL	·										2131			213
PROJECT TOTALS EST. 5% FOR REPLACING TO	16641	9185		www.gr.	7456	827	662		662	÷	10654	7456		18110
GRAND TOTAL	16641	9185)		7456	827	662		662		10654	7456		1811
CAV	40700													
SAY EST_UNDERCUT PER GEOT	16700				and the second second			A A DOMESTIC OF THE STATE OF TH						18200

EST. UNDERCUT PER GEOTECHNICAL RECOMMENDATION: 500 CY

⁻L- & -DRIVE- PAVEMENT STRUCTURE VOLUME: 600 CY

POSS DE POSS D			PROJECT REFERENCE N B-3701	NO. SHEET NO. 4 0F 17
STA 13+0000 -L- BEGIN STATE PROJECT B-3701 STA 23+00.00 -L- END STATE PROJECT B-3701 STA 23+00.00 -L- END STATE PROJECT B-3701 BY SECOND STATE PROJECT B-3701 STA 23+00.00 -L- END STATE PROJECT B-3701 BY SECOND STATE PROJECT B-3701 STA 23+00.00 -L- END STATE PROJECT B-3701				
STA 13+60.00 -L- BEGIN STATE PROJECT B-3701 STA 13+60.00 -L- BEGIN STATE PROJECT B-3701 STA 23+00.00 -L- END STATE PROJECT B-3701 STA 23+00.00 -L- END STATE PROJECT B-3701 STA 23+00.00 -L- END STATE PROJECT B-3701		BRIDGE APPROACHES SITE PL	.AN	
STA 13+60.00 -L- BEGIN STATE PROJECT B-3701 STA 13+60.00 -L- BEGIN STATE PROJECT B-3701 STA 23+00.00 -L- END STATE PROJECT B-3701 STA 23+00.00 -L- END STATE PROJECT B-3701 STA 23+00.00 -L- END STATE PROJECT B-3701	Q			
PSS ANGUE THE STATE OF THE STA			•	t
PSS ANGUE THE STATE OF THE STA				
FIGURE SINCE STATE OF THE STAT	WOODS WOODS WOODS WOODS WOODS WOODS WOODS WOODS		B-3701 STA 23+00.00 -L- END STATE PROJECT B-3701	AND
SEM SHOOL SEM SHOOL	See 10 28 8 22 10 280 See 10 280	WOODS	je programa po programa por pro	
DOME PTSO, IN-OUR DESCRIPTION DOME TYPE IN THOUSE DESCRIPTION DO		WEN CONTRACTOR CONTRAC	PROM HERITAGE RAUMAYS MA 71	woods
ORNE PTSIA ISTOLIE END CONSTRUCTION O 50 100 SCALE IN FEST	12 W. 11	William Woods William		_L_ <u>POT Sta. 23 +13 1</u> 0
ORNE PTSIA ISTOLIE END CONSTRUCTION O 50 100 SCALE IN FEST	Sandrand Comment Comme	ABAND B	T- POC Sta. 19+56.00= ALARKA CREEK	
DRIVE PTSIO. I3+02/P END CONSTRUCTION O 50 100 SCALE IN FEET	Marina and a second a second and a second and a second and a second and a second an	AARKA CII	and the second	- Chrome
DRIVE PTSIO. I3+02/P END CONSTRUCTION O 50 100 SCALE IN FEET		MOODS MOODS MANAGEM 8000S	COME	SR 1309 15.5' BST TO SR 1303
DRNE PTSto. 13+02JP END CONSTRUCTION SCALE IN FEET		38 MIL.	18 dayer my	marinaina an an an an
DRIVE PTSIa. I3+02J2 O 50 100 END CONSTRUCTION SCALE IN FEET	sisteriorio de la companya della companya della companya de la companya della com	and the second s		WELL O HOUSE
DRIVE PTSto. 13+02J2 END CONSTRUCTION SCALE IN FEET			WOODS	
SCALE IN FEET				
성'·		DRIVE PTSIO. 13+02J2	SCALE IN FEET	I_I
			선 	i (}

