PROJECT: 8.2080101 ID: B-3858

| STATE | STATE PROJECT REFERENCE NO. | SHEET NO. | TOTAL SHEETS |
|---|---|---|---|
| N.C. | B-3858 | 1 | 9 |
| STATE PROJ. NO. | F. A. PROJ. NO. | DESCRIPTION | |
| 8.2080101 | | P.E. | |
| | | CONST. | |

# STATE OF NORTH CAROLINA

## DEPARTMENT OF TRANSPORTATION

## DIVISION OF HIGHWAYS

## GEOTECHNICAL UNIT

# STRUCTURE SUBSURFACE INVESTIGATION

CONTENTS:



STATE PROJECT 8.2080101 I.D. NO. B-3858

F.A. PROJECT BRZ-1110(3)

COUNTY HYDE

PROJECT DESCRIPTION BRIDGE NO. 6 ON RELOCATED SR 1110 OVER LAKE LANDING CANAL

SITE DESCRIPTION BRIDGE NO 6 ON RELOCATED SR 1110 OVER LAKE LANDING CANAL AT -L- STA. 12+17.5

## CAUTION NOTICE

THE SUBSURFACE INFORMATION AND THE SUBSURFACE INVESTIGATION ON WHICH IT IS BASED WAS MADE FOR THE PURPOSE OF STUDY, PLANNING AND DESIGN, AND NOT FOR CONSTRUCTION OR PAY PURPOSES. THE VARIOUS FIELD BORING LOGS, ROCK CORES, AND SOIL TEST DATA AVAILABLE MAY BE REVIEWED OR INSPECTED IN RALEIGH BY CONTACTING THE N. C. DEPARTMENT OF TRANSPORTATION, GEOTECHNICAL UNIT @ (919) 250-4088. NEITHER THE SUBSURFACE PLANS AND REPORTS, NOR THE FIELD BORING LOGS, ROCK CORES, OR SOIL TEST DATA IS PART OF THE CONTRACT.

GENERAL SOIL AND ROCK STRATA DESCRIPTIONS AND INDICATED BOUNDARIES ARE BASED ON A GEOTECHNICAL INTERPRETATION OF ALL AVAILABLE SUBSURFACE DATA AND MAY NOT NECESSARILY REFLECT THE ACTUAL SUBSURFACE CONDITIONS BETWEEN BORINGS OR BETWEEN SAMPLED STRATA WITHIN THE BOREHOLE. THE LABORATORY SAMPLE DATA AND THE IN SITU (IN-PLACE) TEST DATA CAN BE RELIED ON ONLY TO THE DEGREE OF RELIABILITY INHERENT IN THE STANDARD TEST METHOD. THE OBSERVED WATER LEVELS OR SOIL MOISTURE CONDITIONS INDICATED IN THE SUBSURFACE INVESTIGATIONS ARE AS RECORDED AT THE TIME OF THE INVESTIGATION. THESE WATER LEVELS OR SOIL MOISTURE CONDITIONS MAY VARY CONSIDERABLY WITH TIME ACCORDING TO CLIMATIC CONDITIONS INCLUDING TEMPERATURES, PRECIPITATION AND WIND, AS WELL AS OTHER NON-CLIMATIC FACTORS.

THE BIDDER OR CONTRACTOR IS CAUTIONED THAT DETAILS SHOWN ON THE SUBSURFACE PLANS ARE PRELIMINARY ONLY AND IN MANY CASES THE FINAL DESIGN DETAILS ARE DIFFERENT. FOR BIDDING AND CONSTRUCTION PURPOSES, REFER TO THE CONSTRUCTION PLANS AND DOCUMENTS FOR FINAL DESIGN INFORMATION ON THIS PROJECT. THE DEPARTMENT DOES NOT WARRANT OR GUARANTEE THE SUFFICIENCY OR ACCURACY OF THE INVESTIGATION MADE, NOR THE INTERPRETATIONS MADE OR OPINION OF THE DEPARTMENT AS TO THE TYPE OF MATERIALS AND CONDITIONS TO BE ENCOUNTERED. THE BIDDER OR CONTRACTOR IS CAUTIONED TO MAKE SUCH INDEPENDENT SUBSURFACE INVESTIGATIONS AS HE DEEMS NECESSARY TO SATISFY HIMSELF AS TO CONDITIONS TO BE ENCOUNTERED ON THIS PROJECT. THE CONTRACTOR SHALL HAVE NO CLAIM FOR ADDITIONAL COMPENSATION OR FOR AN EXTENSION OF TIME FOR ANY REASON RESULTING FROM THE ACTUAL CONDITIONS ENCOUNTERED AT THE SITE DIFFERING FROM THOSE INDICATED IN THE SUBSURFACE INFORMATION.

INVESTIGATED BY FMW PERSONNEL KBM

CHECKED BY FMW FMW LWD

SUBMITTED BY RRW RRW RES

DATE APRIL, 2003

DRAWN BY: DRA

NOTE - THE INFORMATION CONTAINED HEREIN IS NOT IMPLIED OR GUARANTEED BY THE N. C. DEPARTMENT OF TRANSPORTATION AS BEING ACCURATE NOR IT IS CONSIDERED TO BE PART OF THE PLANS, SPECIFICATIONS, OR CONTRACT FOR THE PROJECT.

NOTE - BY HAVING REQUESTED THIS INFORMATION THE CONTRACTOR SPECIFICALLY WAIVES ANY CLAIMS FOR INCREASED COMPENSATION OR EXTENSION OF TIME BASED ON DIFFERENCES BETWEEN THE CONDITIONS INDICATED HEREIN AND THE ACTUAL CONDITIONS AT THE PROJECT SITE.

NORTH CAROLINA LICENSED SEAL 254 GEOLOGIST ROBERT RAY WHITE

SEAL

Robert Ray White

SIGNATURE

| ID | STATE PROJECT NO. | SHEET NO. | TOTAL SHEETS |
|---|---|---|---|
| B-3858 | 8.2080101 | 2 | 9 |

# NORTH CAROLINA DEPARTMENT OF TRANSPORTATION
## DIVISION OF HIGHWAYS
## GEOTECHNICAL UNIT

# SUBSURFACE INVESTIGATION

## SOIL AND ROCK LEGEND, TERMS, SYMBOLS, AND ABBREVIATIONS

### SOIL DESCRIPTION

SOIL IS CONSIDERED TO BE THE UNCONSOLIDATED, SEMI-CONSOLIDATED OR WEATHERED EARTH MATERIALS WHICH CAN BE PENETRATED WITH A CONTINUOUS FLIGHT POWER AUGER, AND WHICH YIELDS LESS THAN 100 BLOWS PER FOOT ACCORDING TO STANDARD PENETRATION TEST (AASHTO T206, ASTM D-1586). SOIL CLASSIFICATION IS BASED ON THE AASHTO SYSTEM AND BASIC DESCRIPTIONS GENERALLY SHALL INCLUDE: CONSISTENCY, COLOR, TEXTURE, MOISTURE, AASHTO CLASSIFICATION, AND OTHER PERTINENT FACTORS SUCH AS MINERALOGICAL COMPOSITION, ANGULARITY, STRUCTURE, PLASTICITY, ETC. EXAMPLE:

*VERY STIFF, GRAY SILTY CLAY, MOIST WITH INTERBEDDED FINE SAND LAYERS, HIGHLY PLASTIC, A-7-6*

### SOIL LEGEND AND AASHTO CLASSIFICATION

| GENERAL CLASS. | GRANULAR MATERIALS (≤35% PASSING #200) | | | | | | | SILT-CLAY MATERIALS (>35% PASSING #200) | | | | ORGANIC MATERIALS | | |
|---|---|---|---|---|---|---|---|---|---|---|---|---|---|---|
| GROUP CLASS. | A-1 | | A-3 | A-2 | | | | A-4 | A-5 | A-6 | A-7 | A-1, A-2<br>A-3 | A-4, A-5<br>A-6, A-7 | |
| | A-1-a | A-1-b | | A-2-4 | A-2-5 | A-2-6 | A-2-7 | | | | A-7-5<br>A-7-6 | | | |
| SYMBOL | | | | | | | | | | | | | | |
| % PASSING<br>#10<br>#40<br>#200 | 50 MX<br>30 MX<br>15 MX | 50 MX<br>25 MX | 51 MN<br>10 MX | 35 MX | 35 MX | 35 MX | 35 MX | 36 MN | 36 MN | 36 MN | 36 MN | GRANULAR SOILS | SILT-CLAY SOILS | MUCK, PEAT |
| LIQUID LIMIT | 6 MX | | N.P. | 40 MX | 41 MN | 40 MX | 41 MN | 40 MX | 41 MN | 40 MX | 41 MN | SOILS WITH LITTLE OR MODERATE AMOUNTS OF ORGANIC MATTER | | HIGHLY ORGANIC SOILS |
| PLASTIC INDEX | | | | 10 MX | 10 MX | 11 MN | 11 MN | 10 MX | 10 MX | 11 MN | 11 MN | | | |
| GROUP INDEX | 0 | | 0 | 0 | | 4 MX | | 8 MX | 12 MX | 16 MX | No MX | | | |
| USUAL TYPES OF MAJOR MATERIALS | STONE FRAGS. GRAVEL AND SAND | | FINE SAND | SILTY OR CLAYEY GRAVEL AND SAND | | | | SILTY SOILS | | CLAYEY SOILS | | | | |
| GEN. RATING AS A SUBGRADE | EXCELLENT TO GOOD | | | | | | | FAIR TO POOR | | | | FAIR TO POOR | POOR | UNSUITABLE |

P.I. OF A-7-5 ≤ L.L. - 30 : P.I. OF A-7-6 > L.L. - 30

### CONSISTENCY OR DENSENESS

| PRIMARY SOIL TYPE | COMPACTNESS OR CONSISTENCY | RANGE OF STANDARD PENETRATION RESISTENCE (N-VALUE) | RANGE OF UNCONFINED COMPRESSIVE STRENGTH ($TONS/FT^2$) |
|---|---|---|---|
| GENERALLY GRANULAR MATERIAL (NON-COHESIVE) | VERY LOOSE | <4 | N/A |
| | LOOSE | 4 TO 10 | |
| | MEDIUM DENSE | 10 TO 30 | |
| | DENSE | 30 TO 50 | |
| | VERY DENSE | >50 | |
| GENERALLY SILT-CLAY MATERIAL (COHESIVE) | VERY SOFT | <2 | <0.25 |
| | SOFT | 2 TO 4 | 0.25 TO 0.5 |
| | MEDIUM STIFF | 4 TO 8 | 0.5 TO 1 |
| | STIFF | 8 TO 15 | 1 TO 2 |
| | VERY STIFF | 15 TO 30 | 2 TO 4 |
| | HARD | >30 | >4 |

### TEXTURE OR GRAIN SIZE

| U.S. STD. SIEVE SIZE | 4 | 10 | 40 | 60 | 200 | 270 |
|---|---|---|---|---|---|---|
| OPENING (MM) | 4.76 | 2.0 | 0.42 | 0.25 | 0.075 | 0.053 |

| BOULDER (BLDR.) | COBBLE (COB.) | GRAVEL (GR.) | COARSE SAND (CSE. SD.) | FINE SAND (F. SD.) | SILT (SL.) | CLAY (CL.) |
|---|---|---|---|---|---|---|

| GRAIN SIZE | MM | 305 | 75 | 2.0 | 0.25 | 0.05 | 0.005 |
|---|---|---|---|---|---|---|---|
| | IN. | 12" | 3" | | | | |

### SOIL MOISTURE - CORRELATION OF TERMS

| SOIL MOISTURE SCALE (ATTERBERG LIMITS) | FIELD MOISTURE DESCRIPTION | GUIDE FOR FIELD MOISTURE DESCRIPTION |
|---|---|---|
| | - SATURATED - (SAT.) | USUALLY LIQUID; VERY WET, USUALLY FROM BELOW THE GROUND WATER TABLE |
| LL - LIQUID LIMIT | | |
| PLASTIC RANGE (PI) | - WET - (W) | SEMISOLID; REQUIRES DRYING TO ATTAIN OPTIMUM MOISTURE |
| PL - PLASTIC LIMIT | | |
| OM - OPTIMUM MOISTURE | - MOIST - (M) | SOLID; AT OR NEAR OPTIMUM MOISTURE |
| SL - SHRINKAGE LIMIT | | |
| | - DRY - (D) | REQUIRES ADDITIONAL WATER TO ATTAIN OPTIMUM MOISTURE |

### PLASTICITY

| | PLASTICITY INDEX (PI) | DRY STRENGTH |
|---|---|---|
| NON PLASTIC | 0-5 | VERY LOW |
| LOW PLASTICITY | 6-15 | SLIGHT |
| MED. PLASTICITY | 16-25 | MEDIUM |
| HIGH PLASTICITY | 26 OR MORE | HIGH |

### COLOR

DESCRIPTIONS MAY INCLUDE COLOR OR COLOR COMBINATIONS (TAN, RED, YEL-BRN, BLUE-GRAY) MODIFIERS SUCH AS LIGHT, DARK, STREAKED, ETC. ARE USED TO DESCRIBE APPEARANCE.

### GRADATION

WELL GRADED- INDICATES A GOOD REPRESENTATION OF PARTICLE SIZES FROM FINE TO COARSE
UNIFORM- INDICATES THAT SOIL PARTICLES ARE ALL APPROXIMATELY THE SAME SIZE. (ALSO POORLY GRADED)
GAP-GRADED- INDICATES A MIXTURE OF UNIFORM PARTICLES OF TWO OR MORE SIZES.

### ANGULARITY OF GRAINS

THE ANGULARITY OR ROUNDNESS OF SOIL GRAINS ARE DESIGNATED BY THE TERMS: ANGULAR, SUBANGULAR, SUBROUNDED, OR ROUNDED.

### MINERALOGICAL COMPOSITION

MINERAL NAMES SUCH AS QUARTZ, FELDSPAR, MICA, TALC, KAOLIN, ETC. ARE USED IN DESCRIPTIONS WHENEVER THEY ARE CONSIDERED OF SIGNIFICANCE.

### COMPRESSIBILITY

| | |
|---|---|
| SLIGHTLY COMPRESSIBLE | LIQUID LIMIT LESS THAN 30 |
| MODERATELY COMPRESSIBLE | LIQUID LIMIT 31-50 |
| HIGHLY COMPRESSIBLE | LIQUID LIMIT GREATER THAN 50 |

### PERCENTAGE OF MATERIAL

| ORGANIC MATERIAL | GRANULAR SOILS | SILT- CLAY SOILS | OTHER MATERIAL | |
|---|---|---|---|---|
| TRACE OF ORGANIC MATTER | 2 - 3% | 3 - 5% | TRACE | 1 - 10% |
| LITTLE ORGANIC MATTER | 3 - 5% | 5 - 12% | LITTLE | 10 - 20% |
| MODERATELY ORGANIC | 5 - 10% | 12 - 20% | SOME | 20 - 35% |
| HIGHLY ORGANIC | >10% | >20% | HIGHLY | 35% AND ABOVE |

### GROUND WATER

- WATER LEVEL IN BORE HOLE IMMEDIATELY AFTER DRILLING.
- STATIC WATER LEVEL AFTER 24 HOURS.
- PW - PERCHED WATER, SATURATED ZONE OR WATER BEARING STRATA
- SPRING OR SEEPAGE

### MISCELLANEOUS SYMBOLS

- ROADWAY EMBANKMENT WITH SOIL DESCRIPTION
- SOIL SYMBOL
- ARTIFICIAL FILL OTHER THAN ROADWAY EMBANKMENTS
- INFERRED SOIL BOUNDARIES
- INFERRED ROCK LINE
- ALLUVIAL SOIL BOUNDARY
- 25/025 - DIP/DIP DIRECTION OF ROCK STRUCTURES
- ● - SOUNDING ROD
- SPT CPT DPT DMT VST PMT - TEST BORING
- AUGER BORING
- CORE BORING
- MW - MONITORING WELL
- PIEZOMETER INSTALLATION
- SLOPE INDICATOR INSTALLATION
- SPT N-VALUE
- REF - SPT REFUSAL

SAMPLE DESIGNATIONS
S- BULK SAMPLE
SS- SPLIT SPOON SAMPLE
ST- SHELBY TUBE SAMPLE
RS- ROCK SAMPLE
RT- RECOMPACTED TRIAXIAL SAMPLE
CBR - CBR SAMPLE

### ABBREVIATIONS

AR - AUGER REFUSAL
BT - BORING TERMINATED
CL. - CLAY
CPT - CONE PENETRATION TEST
CSE. - COARSE
DMT - DILATOMETER TEST
DPT - DYNAMIC PENETRATION TEST
e - VOID RATIO
F. - FINE
FOSS. - FOSSILIFEROUS
FRAC. - FRACTURED
FRAGS. - FRAGMENTS
MED. - MEDIUM
PMT - PRESSUREMETER TEST
SD. - SAND, SANDY
SL. - SILT, SILTY
SLI. - SLIGHTLY
TCR - TRICONE REFUSAL
$\gamma$ - UNIT WEIGHT
$\gamma_d$ - DRY UNIT WEIGHT
W - MOISTURE CONTENT
V. - VERY
VST - VANE SHEAR TEST

### EQUIPMENT USED ON SUBJECT PROJECT

DRILL UNITS:
- ☐ MOBILE B-___
- ☐ BK-51
- ■ CME-45B
- ☐ CME-550
- ☐ PORTABLE HOIST
- ☐ OTHER _____
- ☐ OTHER _____

ADVANCING TOOLS:
- ☐ CLAY BITS
- ☐ 6" CONTINUOUS FLIGHT AUGER
- ☐ 8" HOLLOW AUGERS
- ☐ HARD FACED FINGER BITS
- ☐ TUNG.-CARBIDE INSERTS
- ■ CASING ☐ W/ ADVANCER
- ■ TRICONE 2½ " STEEL TEETH
- ☐ TRICONE _____ " TUNG.-CARB.
- ☐ CORE BIT
- ☐ OTHER _____

HAMMER TYPE:
- ■ AUTOMATIC ☐ MANUAL

CORE SIZE:
- ☐ -B_____
- ☐ -N_____
- ☐ -H_____

HAND TOOLS:
- ☐ POST HOLE DIGGER
- ☐ HAND AUGER
- ☐ SOUNDING ROD
- ☐ VANE SHEAR TEST
- ☐ OTHER _____

### ROCK DESCRIPTION

HARD ROCK IS NON-COASTAL PLAIN MATERIAL THAT WHEN TESTED, WOULD YIELD SPT REFUSAL. AN INFERRED ROCK LINE INDICATES THE LEVEL AT WHICH NON-COASTAL PLAIN MATERIAL WOULD YIELD SPT REFUSAL. SPT REFUSAL IS PENETRATION BY A SPLIT SPOON SAMPLER EQUAL TO OR LESS THAN 0.1 FOOT PER 60 BLOWS. IN NON-COASTAL PLAIN MATERIAL, THE TRANSITION BETWEEN SOIL AND ROCK IS OFTEN REPRESENTED BY A ZONE OF WEATHERED ROCK.
ROCK MATERIALS ARE TYPICALLY DIVIDED AS FOLOWS:

| | |
|---|---|
| WEATHERED ROCK (WR) | NON-COASTAL PLAIN MATERIAL THAT YIELDS SPT N VALUES > 100 BLOWS PER FOOT. |
| CRYSTALLINE ROCK (CR) | FINE TO COARSE GRAIN IGNEOUS AND METAMORPHIC ROCK THAT WOULD YIELD SPT REFUSAL IF TESTED. ROCK TYPE INCLUDES GRANITE, GNEISS, GABBRO, SCHIST, ETC. |
| NON-CRYSTALLINE ROCK (NCR) | FINE TO COARSE GRAIN METAMORPHIC AND NON-COASTAL PLAIN SEDIMENTARY ROCK THAT WOULD YEILD SPT REFUSAL IF TESTED. ROCK TYPE INCLUDES PHYLLITE, SLATE, SANDSTONE, ETC. |
| COASTAL PLAIN SEDIMENTARY ROCK (CP) | COASTAL PLAIN SEDIMENTS CEMENTED INTO ROCK, BUT MAY NOT YIELD SPT REFUSAL. ROCK TYPE INCLUDES LIMESTONE, SANDSTONE, CEMENTED SHELL BEDS, ETC. |

### WEATHERING

| | |
|---|---|
| FRESH | ROCK FRESH, CRYSTALS BRIGHT, FEW JOINTS MAY SHOW SLIGHT STAINING. ROCK RINGS UNDER HAMMER IF CRYSTALLINE. |
| VERY SLIGHT (V. SLI.) | ROCK GENERALLY FRESH, JOINTS STAINED, SOME JOINTS MAY SHOW THIN CLAY COATINGS IF OPEN, CRYSTALS ON A BROKEN SPECIMEN FACE SHINE BRIGHTLY. ROCK RINGS UNDER HAMMER BLOWS IF OF A CRYSTALLINE NATURE. |
| SLIGHT (SLI.) | ROCK GENERALLY FRESH, JOINTS STAINED AND DISCOLORATION EXTENDS INTO ROCK UP TO 1 INCH. OPEN JOINTS MAY CONTAIN CLAY. IN GRANITOID ROCKS SOME OCCASIONAL FELDSPAR CRYSTALS ARE DULL AND DISCOLORED. CRYSTALLINE ROCKS RING UNDER HAMMER BLOWS. |
| MODERATE (MOD.) | SIGNIFICANT PORTIONS OF ROCK SHOW DISCOLORATION AND WEATHERING EFFECTS. IN GRANITOID ROCKS, MOST FELDSPARS ARE DULL AND DISCOLORED, SOME SHOW CLAY. ROCK HAS DULL SOUND UNDER HAMMER BLOWS AND SHOWS SIGNIFICANT LOSS OF STRENGTH AS COMPARED WITH FRESH ROCK. |
| MODERATELY SEVERE (MOD. SEV.) | ALL ROCK EXCEPT QUARTZ DISCOLORED OR STAINED. IN GRANITOID ROCKS, ALL FELDSPARS DULL AND DISCOLORED AND A MAJORITY SHOW KAOLINIZATION. ROCK SHOWS SEVERE LOSS OF STRENGTH AND CAN BE EXCAVATED WITH A GEOLOGIST'S PICK. ROCK GIVES "CLUNK" SOUND WHEN STRUCK. *IF TESTED, WOULD YIELD SPT REFUSAL* |
| SEVERE (SEV.) | ALL ROCKS EXCEPT QUARTZ DISCOLORED OR STAINED. ROCK FABRIC CLEAR AND EVIDENT BUT REDUCED IN STRENGTH TO STRONG SOIL. IN GRANITOID ROCKS ALL FELDSPARS ARE KAOLINIZED TO SOME EXTENT. SOME FRAGMENTS OF STRONG ROCK USUALLY REMAIN. *IF TESTED, YIELDS SPT N VALUES > 100 BPF* |
| VERY SEVERE (V. SEV.) | ALL ROCK EXCEPT QUARTZ DISCOLORED OR STAINED. ROCK FABRIC ELEMENTS ARE DISCERNIBLE BUT THE MASS IS EFFECTIVELY REDUCED TO SOIL STATUS, WITH ONLY FRAGMENTS OF STRONG ROCK REMAINING. SAPROLITE IS AN EXAMPLE OF ROCK WEATHERED TO A DEGREE SUCH THAT ONLY MINOR VESTIGES OF THE ORIGINAL ROCK FABRIC REMAIN. *IF TESTED, YIELDS SPT N VALUES < 100 BPF* |
| COMPLETE | ROCK REDUCED TO SOIL. ROCK FABRIC NOT DISCERNIBLE, OR DISCERNIBLE ONLY IN SMALL AND SCATTERED CONCENTRATIONS. QUARTZ MAY BE PRESENT AS DIKES OR STRINGERS. SAPROLITE IS ALSO AN EXAMPLE. |

### ROCK HARDNESS

| | |
|---|---|
| VERY HARD | CANNOT BE SCRATCHED BY KNIFE OR SHARP PICK. BREAKING OF HAND SPECIMENS REQUIRES SEVERAL HARD BLOWS OF THE GEOLOGISTS PICK. |
| HARD | CAN BE SCRATCHED BY KNIFE OR PICK ONLY WITH DIFFICULTY. HARD HAMMER BLOWS REQUIRED TO DETACH HAND SPECIMEN. |
| MODERATELY HARD | CAN BE SCRATCHED BY KNIFE OR PICK. GOUGES OR GROOVES TO 0.25 INCHES DEEP CAN BE EXCAVATED BY HARD BLOW OF A GEOLOGISTS PICK. HAND SPECIMENS CAN BE DETACHED BY MODERATE BLOWS. |
| MEDIUM HARD | CAN BE GROOVED OR GOUGED 0.05 INCHES DEEP BY FIRM PRESSURE OF KNIFE OR PICK POINT. CAN BE EXCAVATED IN SMALL CHIPS TO PEICES 1 INCH MAXIMUM SIZE BY HARD BLOWS OF THE POINT OF A GEOLOGISTS PICK. |
| SOFT | CAN BE GROVED OR GOUGED READILY BY KNIFE OR PICK. CAN BE EXCAVATED IN FRAGMENTS FROM CHIPS TO SEVERAL INCHES IN SIZE BY MODERATE BLOWS OF A PICK POINT. SMALL, THIN PIECES CAN BE BROKEN BY FINGER PRESSURE. |
| VERY SOFT | CAN BE CARVED WITH KNIFE. CAN BE EXCAVATED READILY WITH POINT OF PICK. PIECES 1 INCH OR MORE IN THICKNESS CAN BE BROKEN BY FINGER PRESSURE. CAN BE SCRATCHED READILY BY FINGERNAIL. |

### FRACTURE SPACING

| TERM | SPACING |
|---|---|
| VERY WIDE | MORE THAN 10 FEET |
| WIDE | 3 TO 10 FEET |
| MODERATELY CLOSE | 1 TO 3 FEET |
| CLOSE | 0.16 TO 1 FEET |
| VERY CLOSE | LESS THAN 0.16 FEET |

### BEDDING

| TERM | THICKNESS |
|---|---|
| VERY THICKLY BEDDED | > 4 FEET |
| THICKLY BEDDED | 1.5 - 4 FEET |
| THINLY BEDDED | 0.16 - 1.5 FEET |
| VERY THINLY BEDDED | 0.03 - 0.16 FEET |
| THICKLY LAMINATED | 0.008 - 0.03 FEET |
| THINLY LAMINATED | < 0.008 FEET |

### INDURATION

FOR SEDIMENTARY ROCKS, INDURATION IS THE HARDENING OF THE MATERIAL BY CEMENTING, HEAT, PRESSURE, ETC.

| | |
|---|---|
| FRIABLE | RUBBING WITH FINGER FREES NUMEROUS GRAINS; GENTLE BLOW BY HAMMER DISINTEGRATES SAMPLE. |
| MODERATELY INDURATED | GRAINS CAN BE SEPARATED FROM SAMPLE WITH STEEL PROBE; BREAKS EASILY WHEN HIT WITH HAMMER. |
| INDURATED | GRAINS ARE DIFFICULT TO SEPARATE WITH STEEL PROBE; DIFFICULT TO BREAK WITH HAMMER. |
| EXTREMELY INDURATED | SHARP HAMMER BLOWS REQUIRED TO BREAK SAMPLE; SAMPLE BREAKS ACROSS GRAINS. |

### TERMS AND DEFINITIONS

ALLUVIUM (ALLUV.) - SOILS WHICH HAVE BEEN TRANSPORTED BY WATER.

AQUIFER - A WATER BEARING FORMATION OR STRATA.

ARENACEOUS - APPLIED TO ROCKS THAT HAVE BEEN DERIVED FROM SAND OR THAT CONTAIN SAND.

ARGILLACEOUS - APPLIED TO ALL ROCKS OR SUBSTANCES COMPOSED OF CLAY MINERALS, OR HAVING A NOTABLE PROPORTION OF CLAY IN THEIR COMPOSITION, AS SHALE, SLATE, ETC.

ARTESIAN - GROUND WATER THAT IS UNDER SUFFICIENT PRESSURE TO RISE ABOVE THE LEVEL AT WHICH IS IS ENCOUNTERED, BUT WHICH DOES NOT NECESSARILY RISE TO OR ABOVE THE GROUND SURFACE.

CALCAREOUS (CALC.) - SOILS WHICH CONTAIN APPRECIABLE AMOUNTS OF CALCIUM CARBONATE.

COLLUVIUM - ROCK FRAGMENTS MIXED WITH SOIL DEPOSITED BY GRAVITY ON SLOPE OR AT BOTTOM OF SLOPE.

CORE RECOVERY (REC.) - TOTAL LENGTH OF ALL MATERIAL RECOVERED IN THE CORE BARREL DIVIDED BY TOTAL LENGTH OF CORE RUN AND EXPRESSED AS A PERCENTAGE.

DIKE - A TABULAR BODY OF IGNEOUS ROCK THAT CUTS ACROSS THE STRUCTURE OF ADJACENT ROCKS OR CUTS MASSIVE ROCK.

DIP - THE ANGLE AT WHICH A STRATUM OR ANY PLANAR FEATURE IS INCLINED FROM THE HORIZONTAL.

DIP DIRECTION (DIP AZIMUTH) - THE DIRECTION OR BEARING OF THE HORIZONTAL TRACE OF THE LINE OF DIP, MEASURED CLOCKWISE FROM NORTH.

FAULT - A FRACTURE OR FRACTURE ZONE ALONG WHICH THERE HAS BEEN DISPLACEMENT OF THE SIDES RELATIVE TO ONE ANOTHER PARALLEL TO THE FRACTURE.

FISSILE - A PROPERTY OF SPLITTING ALONG CLOSELY SPACED PARALLEL PLANES.

FLOAT - ROCK FRAGMENTS ON SURFACE NEAR THEIR ORIGINAL POSITION AND DISLODGED FROM PARENT MATERIAL.

FLOOD PLAIN (F.P.) - LAND BORDERING A STREAM, BUILT OF SEDIMENTS DEPOSITED BY THE STREAM.

FORMATION (FM.) - A MAPPABLE GEOLOGIC UNIT THAT CAN BE RECOGNIZED AND TRACED IN THE FIELD.

JOINT - FRACTURE IN ROCK ALONG WHICH NO APPRECIABLE MOVEMENT HAS OCCURRED.

LEDGE - A SHELF-LIKE RIDGE OR PROJECTION OF ROCK WHOSE THICKNESS IS SMALL COMPARED TO ITS LATERAL EXTENT.

LENS - A BODY OF SOIL OR ROCK THAT THINS OUT IN ONE OR MORE DIRECTIONS.

MOTTLED (MOT.) - IRREGULARLY MARKED WITH SPOTS OF DIFFERENT COLORS. MOTTLING IN SOILS USUALLY INDICATES POOR AERATION AND LACK OF GOOD DRAINAGE.

PERCHED WATER - WATER MAINTAINED ABOVE THE NORMAL GROUND WATER LEVEL BY THE PRESENCE OF AN INTERVENING IMPERVIOUS STRATUM.

RESIDUAL SOIL - SOIL FORMED IN PLACE BY THE WEATHERING OF ROCK.

ROCK QUALITY DESIGNATION (R.Q.D.) - A MEASURE OF ROCK QUALITY DESCRIBED BY: TOTAL LENGTH OF ROCK SEGMENTS EQUAL TO OR GREATER THAN 4 INCHES DIVIDED BY THE TOTAL LENGTH OF CORE RUN AND EXPRESSED AS A PERCENTAGE.

SAPROLITE (SAP.) - RESIDUAL SOIL WHICH RETAINS THE RELIC STRUCTURE OR FABRIC OF THE PARENT ROCK.

SILL - AN INTRUSIVE BODY OF IGNEOUS ROCK OF APPROXIMATELY UNIFORM THICKNESS AND RELATIVELY THIN COMPARED WITH ITS LATERAL EXTENT, WHICH HAS BEEN EMPLACED PARALLEL TO THE BEDDING OR SCHISTOSITY OF THE INTRUDED ROCKS

SLICKENSIDE - POLISHED AND STRIATED SURFACE THAT RESULTS FROM FRICTION ALONG A FAULT OR SLIP PLANE.

STANDARD PENETRATION TEST (PENETRATION RESISTANCE) (SPT) - NUMBER OF BLOWS (N OR B.P.F.) OF A 140 LB. HAMMER FALLING 30 INCHES REQUIRED TO PRODUCE A PENETRATION OF 1 FOOT INTO SOIL WITH A 2 INCH OUTSIDE DIAMETER SPLIT SPOON SAMPLER. SPT REFUSAL IS LESS THAN 0.1 FOOT PENETRATION WITH 60 BLOWS.

STRATA CORE RECOVERY (SREC.) - TOTAL LENGTH OF STRATA MATERIAL RECOVERED DIVIDED BY TOTAL LENGTH OF STRATUM AND EXPRESSED AS A PERCENTAGE.

STRATA ROCK QUALITY DESIGNATION (S.R.Q.D.) - A MEASURE OF ROCK QUALITY DESCRIBED BY: TOTAL LENGTH OF ROCK SEGMENTS WITHIN A STRATUM EQUAL TO OR GREATER THAN 10 CENTIMETERS DIVIDED BY THE TOTAL LENGTH OF STRATA AND EXPRESSED AS A PERCENTAGE.

TOPSOIL (T.S.) - SURFACE SOILS USUALLY CONTAINING ORGANIC MATTER.

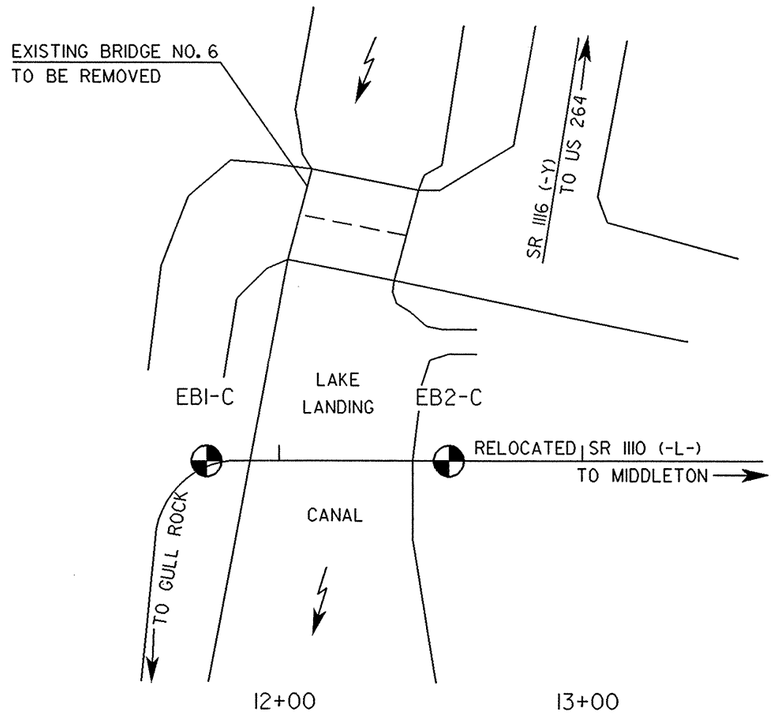
BENCH MARK BM# 9 IN SW CORNER OF PUMPING STATION AT WYSOCKING BAY 288.17' RT OF -FL-
STA. 14+76.25 ELEVATION: 3.84'

NOTES:

REVISED 09/15/00

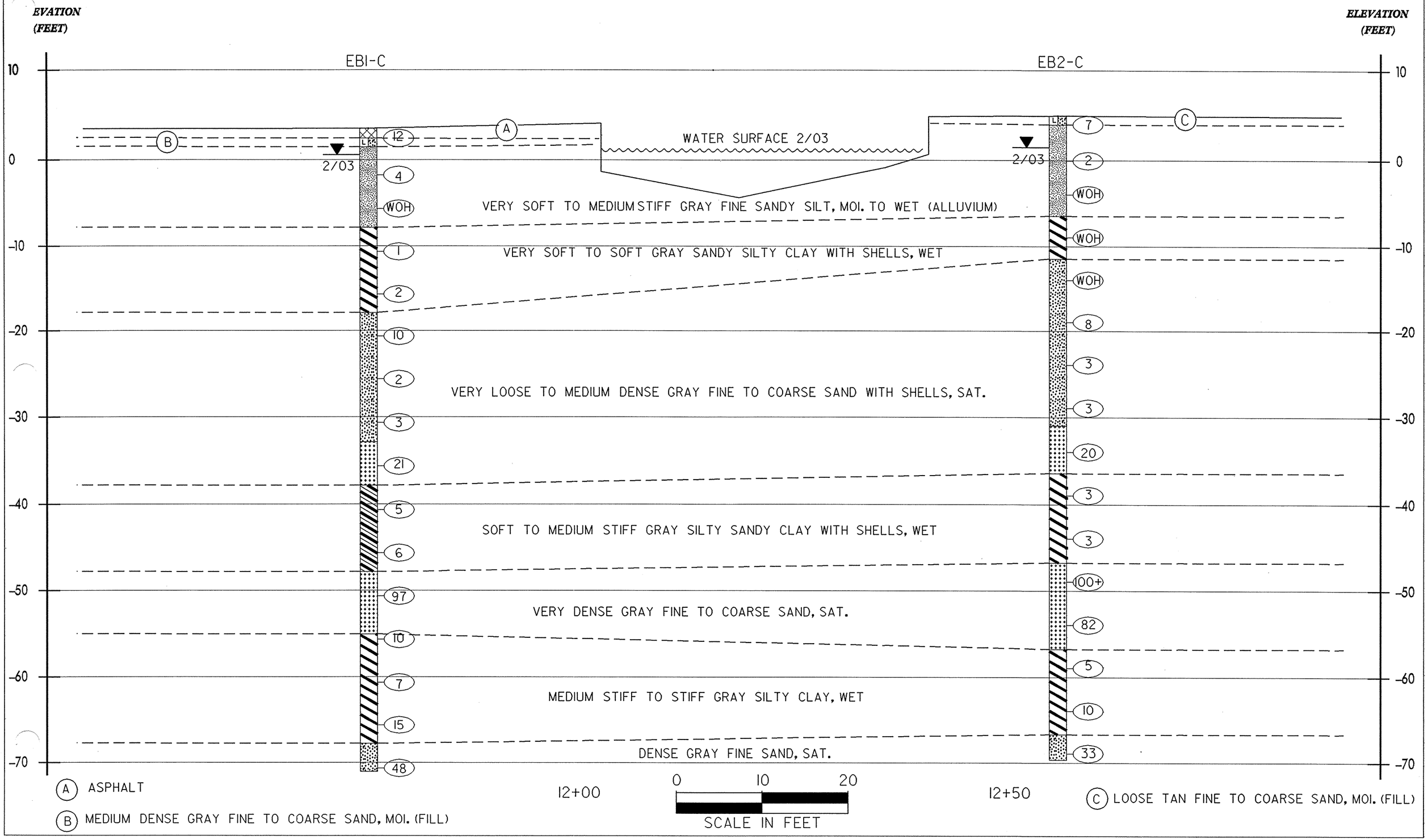
| STATE PROJECT NO. | COUNTY | BRIDGE NO. | SHEET NO. | TOTAL SHEETS |
|---|---|---|---|---|
| 8.2080101 | HYDE | 6 | 3 | 9 |

# RELOCATED SR 1110 (-L-) OVER LAKE LANDING CANAL


| STATE PROJECT NO. | COUNTY | BRIDGE NO. | SHEET NO. | TOTAL SHEETS |
|---|---|---|---|---|
| 8.2080101 | HYDE | 6 | 4 | 9 |

# SOIL PROFILE ALONG RELOCATED SR 1110 (-L-) OVER LAKE LANDING CANAL




(A) ASPHALT

(B) MEDIUM DENSE GRAY FINE TO COARSE SAND, MOI. (FILL)

(C) LOOSE TAN FINE TO COARSE SAND, MOI. (FILL)

# NORTH CAROLINA DEPARTMENT OF TRANSPORTATION
## GEOTECHNICAL UNIT BORING LOG

SHEET 1 OF 2

| | | | |
|---|---|---|---|
| **PROJECT NO.** 8.2080101 | **ID.** B-3858 | **COUNTY** HYDE | **GEOLOGIST** K.B. MILLER |
| **SITE DESCRIPTION** BRIDGE NO. 6 ON RELOCATED SR 1110 OVER LAKE LANDING CANAL | | | **GROUND WATER** |
| **BORING NO.** EBI-C | **BORING LOCATION** 11+76 | **OFFSET** CL | **ALIGNMENT** -L- | **0 HR.** N.M. |
| **COLLAR ELEVATION** 3.5' | **NORTHING** 0.00 | **EASTING** 0.00 | **24 HR.** 3.0' |
| **TOTAL DEPTH** 74.6' | **DRILL MACHINE** CME-45B | **DRILL METHOD** ROTARY W/MUD | **HAMMER TYPE** AUTOMATIC |
| **START DATE** 2/13/03 | **COMPLETION DATE** 2/13/03 | **SURFACE WATER DEPTH** | |

| ELEV. | DEPTH (FT.) | BLOW COUNT 0.5' | 0.5' | 0.5' | PEN. (FT.) | BLOWS PER FOOT 0 25 50 75 100 | SAMPLE NUMBER | ▼ MOI. | LOG | SOIL AND ROCK DESCRIPTION |
|---|---|---|---|---|---|---|---|---|---|---|
| 3.5 | 1.1 | 6 | 8 | 4 | 1 | X 12 | | | | ASPHALT |
| 0.0 | 4.2 | 1 | 2 | 2 | 1 | X 4 | SS-1 | ▼ | | GRAY FINE TO COARSE SAND, MOI. (FILL) |
| -5.0 | 8.1 | WOH | WOH | WOH | 1 | X 0 | | | | GRAY FINE SANDY SILT, WET (ALLUVIUM) |
| -10.0 | 13.1 | WOH | WOH | 1 | 1 | X 1 | SS-2 | | | GRAY SANDY SILTY CLAY WITH SHELLS, WET |
| -15.0 | 18.1 | WOH | 1 | 1 | 1 | X 2 | | | | |
| -20.0 | 23.1 | 3 | 6 | 4 | 1 | X 10 | | | | |
| 25.0 | 28.1 | WOH | 1 | 1 | 1 | X 2 | | | | GRAY FINE TO COARSE SAND WITH SHELLS, SAT. |
| -30.0 | 33.1 | 1 | 1 | 2 | 1 | X 3 | SS-3 | | | |
| -35.0 | 38.1 | 12 | 13 | 8 | 1 | X 21 | SS-4 | | | |
| -40.0 | 43.1 | WOH | 2 | 3 | 1 | X 5 | SS-5 | | | GRAY SILTY SANDY CLAY WITH SHELLS, WET |
| -45.0 | 48.1 | 1 | 3 | 3 | 1 | X 6 | | | | |
| -50.0 | 53.1 | 27 | 45 | 52 | 1 | 97 X | SS-6 | | | TAN FINE TO COARSE SAND, SAT. |
| -55.0 | 58.1 | 16 | 5 | 5 | 1 | X 10 | SS-7 | | | |
| -60.0 | 63.1 | 1 | 3 | 4 | 1 | X 7 | | | | GRAY SILTY CLAY, WET |
| -65.0 | 68.1 | 3 | 5 | 10 | 1 | X 15 | | | | |
| -70.0 | 73.1 | 14 | 23 | 25 | 1 | X 48 | SS-8 | | | GRAY FINE SAND, SAT. |

# NORTH CAROLINA DEPARTMENT OF TRANSPORTATION
## GEOTECHNICAL UNIT BORING LOG

SHEET 2 OF 2

| | | | |
|---|---|---|---|
| **PROJECT NO.** 8.2080101 | **ID.** B-3858 | **COUNTY** HYDE | **GEOLOGIST** K.B. MILLER |
| **SITE DESCRIPTION** BRIDGE NO. 6 ON RELOCATED SR 1110 OVER LAKE LANDING CANAL | | | **GROUND WATER** |
| **BORING NO.** EBI-C | **BORING LOCATION** 11+76 | **OFFSET** CL | **ALIGNMENT** -L- | **0 HR.** N.M. |
| **COLLAR ELEVATION** 3.5' | **NORTHING** 0.00 | **EASTING** 0.00 | **24 HR.** 3.0' |
| **TOTAL DEPTH** 74.6' | **DRILL MACHINE** CME-45B | **DRILL METHOD** ROTARY W/MUD | **HAMMER TYPE** AUTOMATIC |
| **START DATE** 2/13/03 | **COMPLETION DATE** 2/13/03 | **SURFACE WATER DEPTH** | |

| ELEV. | DEPTH (FT.) | BLOW COUNT 0.5' | 0.5' | 0.5' | PEN. (FT.) | BLOWS PER FOOT 0 25 50 75 100 | SAMPLE NUMBER | ▼ MOI. | LOG | SOIL AND ROCK DESCRIPTION |
|---|---|---|---|---|---|---|---|---|---|---|
| | | | | | | BORING TERMINATED AT ELEV. -71.1' IN DENSE SAND | | | | |

# NORTH CAROLINA DEPARTMENT OF TRANSPORTATION
## GEOTECHNICAL UNIT BORING LOG

SHEET 1 OF 2

| | | | |
|---|---|---|---|
| PROJECT NO. 8.2080101 | ID. B-3858 | COUNTY HYDE | GEOLOGIST K.B. MILLER |
| SITE DESCRIPTION BRIDGE NO. 6 ON RELOCATED SR 1110 OVER LAKE LANDING CANAL | | | GROUND WATER |
| BORING NO. EB2-C | BORING LOCATION 12+56 | OFFSET CL | ALIGNMENT -L- | 0 HR. N.M. |
| COLLAR ELEVATION 5.1′ | NORTHING 0.00 | EASTING 0.00 | 24 HR. 3.6′ |
| TOTAL DEPTH 74.7′ | DRILL MACHINE CME-45B | DRILL METHOD ROTARY W/MUD | HAMMER TYPE AUTOMATIC |
| START DATE 2/13/03 | COMPLETION DATE 2/13/03 | SURFACE WATER DEPTH | |

| ELEV. | DEPTH (FT.) | BLOW COUNT 0.5′ | 0.5′ | 0.5′ | PEN. (FT.) | BLOWS PER FOOT 0 25 50 75 100 | SAMPLE NUMBER | MOI. | LOG | SOIL AND ROCK DESCRIPTION |
|---|---|---|---|---|---|---|---|---|---|---|
| 5.1′ | 0.0 | 2 | 3 | 4 | 1 | X 7 | | | | TAN FINE TO COARSE SAND, MOI. (FILL) |
| 0.0 | 4.1 | 1 | 1 | 1 | 1 | X 2 | SS-9 | 40% | | GRAY FINE SANDY SILT, MOI. TO WET (ALLUVIUM) |
| -5.0 | 8.2 | WOH | WOH | WØH | 1 | X 0 | | | | |
| -10.0 | 13.2 | WOH | WOH | WOH | 1 | X 0 | | | | GRAY SANDY SILTY CLAY, WET |
| -15.0 | 18.2 | WOH | WOH | WOH | 1 | X 0 | SS-10 | | | |
| 20.0 | 23.2 | 1 | 3 | 5 | 1 | X 8 | | | | |
| -25.0 | 28.2 | 1 | 2 | 1 | 1 | X 3 | SS-11 | | | GRAY FINE TO COARSE SAND, SAT. |
| -30.0 | 33.2 | 1 | 1 | 2 | 1 | X 3 | | | | |
| -35.0 | 38.2 | 2 | 8 | 12 | 1 | X 20 | SS-12 | | | |
| -40.0 | 43.2 | WOH | 1 | 2 | 1 | X 3 | SS-13 | 90% | | GRAY SILTY CLAY WITH SHELLS, WET |
| -45.0 | 48.2 | WOH | 1 | 2 | 1 | X 3 | | | | |
| -50.0 | 53.2 | 39 | 54 | 46 | 0.9 | 100+ X | SS-14 | | | TAN FINE SAND, SAT. |
| -55.0 | 58.2 | 28 | 40 | 42 | 1 | X 82 | | | | |
| -60.0 | 63.2 | 2 | 2 | 3 | 1 | X 5 | SS-15 | | | GRAY SILTY CLAY, WET |
| -65.0 | 68.2 | 5 | 5 | 5 | 1 | X 10 | | | | |
| -70.0 | 73.2 | 9 | 16 | 17 | 1 | X 33 | SS-16 | | | GRAY FINE SAND, SAT. |

# NORTH CAROLINA DEPARTMENT OF TRANSPORTATION
## GEOTECHNICAL UNIT BORING LOG

SHEET 2 OF 2

| | | | |
|---|---|---|---|
| PROJECT NO. 8.2080101 | ID. B-3858 | COUNTY HYDE | GEOLOGIST K.B. MILLER |
| SITE DESCRIPTION BRIDGE NO. 6 ON RELOCATED SR 1110 OVER LAKE LANDING CANAL | | | GROUND WATER |
| BORING NO. EB2-C | BORING LOCATION 12+56 | OFFSET CL | ALIGNMENT -L- | 0 HR. N.M. |
| COLLAR ELEVATION 5.1 | NORTHING 0.00 | EASTING 0.00 | 24 HR. 3.6′ |
| TOTAL DEPTH 74.7′ | DRILL MACHINE CME-45B | DRILL METHOD ROTARY W/MUD | HAMMER TYPE AUTOMATIC |
| START DATE 2/13/03 | COMPLETION DATE 2/13/03 | SURFACE WATER DEPTH | |

| ELEV. | DEPTH (FT.) | BLOW COUNT 0.5′ | 0.5′ | 0.5′ | PEN. (FT.) | BLOWS PER FOOT 0 25 50 75 100 | SAMPLE NUMBER | MOI. | LOG | SOIL AND ROCK DESCRIPTION |
|---|---|---|---|---|---|---|---|---|---|---|
| | | | | | | BORING TERMINATED AT ELEV. -69.6′ IN DENSE SAND | | | | |

# B-3858

## Bridge No. 6 on Relocated SR 1110 over Lake Landing Canal

| HOLE # | SAMPLE # | PASS 10 | PASS 40 | PASS 200 | CSESAND | FINESAND | SI | CL | LL | PI | CLASS | DEPTH | MOIST. | ORG. |
|---|---|---|---|---|---|---|---|---|---|---|---|---|---|---|
| EB1-C | SS-1 | 100 | 100 | 76 | 0.2 | 36.5 | 51.3 | 12.1 | 22 | NP | A4(0) | 4.2-5.7 | | |
| | SS-2 | 100 | 100 | 94 | 0.4 | 15.5 | 41.8 | 42.3 | 46 | 24 | A76(25) | 13.1-14.6 | | |
| | SS-3 | 100 | 95 | 24 | 9.5 | 69.5 | 11.0 | 10.1 | 25 | 2 | A24(0) | 33.1-34.6 | | |
| | SS-4 | 98 | 89 | 8 | 39.3 | 53.9 | 3.8 | 3.0 | 19 | NP | A3(0) | 38.1-39.6 | | |
| | SS-5 | 100 | 99 | 58 | 1.0 | 49.3 | 25.5 | 24.2 | 30 | 12 | A6(4) | 43.1-44.6 | | |
| | SS-6 | 100 | 95 | 7 | 26.6 | 68.4 | 4.0 | 1.0 | 17 | NP | A3(0) | 53.1-54.6 | | |
| | SS-7 | 100 | 98 | 88 | 4.4 | 15.9 | 41.4 | 38.3 | 55 | 32 | A76(31) | 58.6-59.1 | | |
| | SS-8 | 100 | 100 | 20 | 10.5 | 71.4 | 8.1 | 10.1 | 21 | NP | A24(0) | 73.1-74.6 | | |
| EB2-C | SS-9 | 100 | 100 | 84 | 0.6 | 33.0 | 52.3 | 14.1 | 34 | 6 | A4(6) | 4.1-5.6 | 39.5 | |
| | SS-10 | 100 | 99 | 16 | 2.8 | 83.2 | 4.9 | 9.1 | 19 | NP | A24(0) | 18.2-19.7 | | |
| | SS-11 | 95 | 82 | 22 | 29.2 | 49.7 | 9.0 | 12.1 | 22 | 5 | A24(0) | 28.2-29.7 | | |
| | SS-12 | 94 | 77 | 10 | 48.1 | 42.3 | 5.5 | 4.0 | 18 | NP | A3(0) | 38.2-39.7 | | |
| | SS-13 | 100 | 99 | 97 | 0.8 | 8.1 | 44.8 | 46.3 | 70 | 45 | A76(50) | 43.2-44.7 | 89.7 | |
| | SS-14 | 100 | 98 | 8 | 8.7 | 85.3 | 5.0 | 1.0 | 22 | NP | A3(0) | 53.2-54.1 | | |
| | SS-15 | 100 | 100 | 92 | 1.2 | 13.9 | 38.6 | 46.3 | 65 | 43 | A76(44) | 63.2-64.7 | | |
| | SS-16 | 100 | 99 | 15 | 14.0 | 73.2 | 6.7 | 6.0 | 22 | NP | A24(0) | 73.2-74.7 | | |

Rev. 5/91

## GEOTECHNICAL UNIT FIELD SCOUR REPORT

PROJECT: 8.2080101 ID: B-3858 COUNTY: Hyde

DESCRIPTION (1): Bridge No. 6 on Relocated SR 1110 over Lake Landing Canal

**INFORMATION ON EXISTING BRIDGES** Information obtained from X field inspection
____ microfilm (Reel:____ Position:____)
____ other ____

COUNTY BRIDGE NO. 6 BRIDGE LENGTH 35' NO. BENTS 2 NO. BENTS IN CHANNEL N/A FLOOD PLAIN N/A

FOUNDATION TYPE: N/A

EVIDENCE OF SCOUR (2):

ABUTMENTS OR END BENT SLOPES: N/A

INTERIOR BENTS: N/A

CHANNEL BED: N/A

CHANNEL BANKS: N/A

**EXISTING SCOUR PROTECTION:**

TYPE (3): Wooden end walls

EXTENT (4): 15' from outside edge of bridge

EFFECTIVENESS (5): Appears satisfactory

**OBSTRUCTIONS** (6) (DAMS, DEBRIS, ETC.): None noted

**DESIGN INFORMATION**

CHANNEL BED MATERIAL (7) (SAMPLE RESULTS ATTACHED): N/A – No interior bents

CHANNEL BANK MATERIAL (8) (SAMPLE RESULTS ATTACHED): Very soft to soft fine sandy silt (SS-1, SS-9)

FOUNDATION BEARING MATERIAL (9): Very dense fine to coarse sand

CHANNEL BANK COVER (10): Marsh grasses

**DESIGN INFORMATION CONT.**

FLOOD PLAIN WIDTH (11): N/A – Man made canal

FLOOD PLAIN COVER (12): N/A

STREAM IS ___DEGRADING ___AGGRADING _X_EQUILIBRIUM (13)

OTHER OBSERVATIONS AND COMMENTS: ____

CHANNEL MIGRATION TENDENCY (14): N/A

GEOTECHNICALLY ADJUSTED SCOUR ELEVATION (15): N/A – no interior bents

REPORTED BY: Fred M Nixon IV DATE: 4-10-03

**INSTRUCTIONS**

(1) GIVE THE DESCRIPTION OF THE SPECIFIC SITE GIVING ROUTE NUMBER AND BODY OF WATER CROSSED.
(2) NOTE ANY EVIDENCE OF SCOUR AT THE EXISTING END BENTS OR ABUTMENTS (UNDERMINING, SLOUGHING, SCOUR LOCATIONS, DEGRADATIONS, ETC.)
(3) NOTE ANY EXISTING SCOUR PROTECTION (RIP RAP, ETC.)
(4) DESCRIBE THE EXTENT OF ANY EXISTING SCOUR PROTECTION.
(5) DESCRIBE WHETHER OR NOT THE SCOUR PROTECTION APPEARS TO BE WORKING.
(6) NOTE ANY DAMS, FALLEN TREES, DEBRIS AT BENTS, ETC.
(7) DESCRIBE THE CHANNEL BED MATERIAL: A SAMPLE SHOULD BE TAKEN FOR GRAIN SIZE DISTRIBUTION. ATTACH LAB RESULTS.
(8) DESCRIBE THE CHANNEL BANK MATERIAL: A SAMPLE SHOULD BE TAKEN FOR GRAIN SIZE DISTRIBUTION. ATTACH LAB RESULTS.
(9) DESCRIBE THE FOUNDATION BEARING MATERIAL.
(10) DESCRIBE THE BANK COVERING (GRASS, TREES, RIP RAP, NONE, ETC.)
(11) GIVE THE APPROXIMATE FLOOD PLAIN WIDTH (ESTIMATE).
(12) DESCRIBE THE FLOOD PLAIN COVERING (GRASS, TREES, CROPS, ETC.)
(13) CHECK THE APPROPRIATE SPACE AS TO WHETHER THE STREAM IS DEGRADING, AGGRADING, OR EQUILIBRIUM.
(14) DESCRIBE THE POTENTIAL OF THE BODY OF WATER TO MIGRATE LATERALLY DURING THE LIFE OF THE BRIDGE (APPROXIMATELY 100 YEARS).
(15) GIVE THE GEOTECHNICALLY ADJUSTED SCOUR ELEVATION EXPECTED OVER THE LIFE OF THE BRIDGE (APPROXIMATELY 100 YEARS). THIS CAN BE GIVEN AS AN ELEVATION RANGE ACROSS THE SITE, OR ON A BENT BY BENT BASIS WHERE VARIATIONS EXIST. DISCUSS RELATIONSHIP BETWEEN THE HYDRAULICS THEORETICAL SCOUR AND THE GEOTECHNICALLY ADJUSTED SCOUR ELEVATION. THE GEOTECHNICALLY ADJUSTED SCOUR ELEVATION IS BASED ON THE ERODABILITY OF MATERIALS WITH CONSIDERATION FOR JOINTING, FOLIATION, BEDDING ORIENTATION AND FREQUENCY; CORE RECOVERY PERCENTAGE; PERCENTAGE RQD; DIFFERENTIAL WEATHERING; SHEAR STRENGTH; OBSERVATIONS AT EXISTING STRUCTURES; OTHER TESTS DEEMED APPROPRIATE; AND OVERALL GEOLOGIC CONDITIONS AT THE SITE.

**8.2080101 B-3858**
**Hyde Co.**
**Bridge No. 6 on Relocated SR 1110 over Great Ditch (Lake Landing Canal)**

**Looking east towards End Bent 2**