

STATE OF NORTH CAROLINA DEPARTMENT OF TRANSPORTATION

ROY COOPER
GOVERNOR

J. ERIC BOYETTE
SECRETARY

November 04, 2020

U.S. Army Corps of Engineers Asheville Regulatory Field Office 151 Patton Avenue, Room 208 Asheville, NC 28801-5006

ATTN: Mr. Eric Alsmeyer

NCDOT Regulatory Coordinator

Subject: Request for Modification to Individual Section 404 and Section 401Water Quality

Certification for the Winston-Salem Northern Beltway from US 52 to US 311 (New Walkertown Road), Forsyth County. Federal Aid Project No. NHP-0074(213), TIP Nos.

U-2579D, E& F. Division 9.

Reference: Section 404 and 401 Individual Permit Application requested October 2, 2017 for Winston

Salem Northern Beltway from I-40 to US 52 (R-2247B Phase 2, CA, CB, CD, D, EA, EB, and EC), from US 52 to US 311 north (U-2579C Phase 2, D, E, and F) and from I-40

Business to US 311 South (U-2579AA and AB)

- Modification Application for U-2579D, E, F / U-2579C Phase 2, dated January 7, 2019.

- Modification Application for R-2247EB, dated June 26, 2019.
- Modification Application for U-2579D, E, F, dated October 17, 2019.

Section 404 Individual Permit issued for Winston Salem Northern Beltway from I-40 to US 52 (R-2247B Phase 2, CA, CB, CD, D, EA, EB, and EC), from US 52 to US 311 north (U-2579C Phase 2, D, E, and F) and from I-40 Business to US 311 South (U-2579AA and AB) January 22, 2018 (SAW-2017-02112)

- Permit Modification for U-2579D, E, F / U-2579C Phase 2, dated March 4, 2019.
- Permit Modification for R-2247EB, dated August 13, 2019.
- Permit Modification for U-2579D, E, F, dated January 29, 2020.

Section 401 Water Quality Certification issued November 14, 2017 (WQC004131); for Winston Salem Northern Beltway from I-40 to US 52 (R- 2247B Phase 2, CA, CB, CD, D, EA, EB, and EC), from US 52 to US 311 north (U-2579C Phase 2, D, E, and F) and from I-40 Business to US 311 South (U- 2579AA and AB); Revision 1 dated November 27, 2017, Revision 2 dated December 21, 2017, Revision 3 dated February 7, 2019, Revision 4 dated August 13, 2019, Revision 5 dated November 14, 2019.

Telephone: (336) 896-2330

Customer Service: 1-877-368-4968

Website: www.ncdot.gov

Dear Sir:

The North Carolina Department of Transportation (NCDOT) requests modification to the existing Section 404 Individual Permit and Section 401 Water Quality Certification for the proposed Winston Salem Northern Beltway from US 52 to US 311 (TIP U-2759 D, E, and F) in Forsyth County. This project is currently under construction.

This modification will result in no changes to wetland impacts previous documented. It will result in 31 linear feet of additional permanent stream fill impacts and 20 linear feet of additional temporary stream impacts. The total project impacts for U-2579D, E, and F include 12,111 linear feet of permanent stream impacts of which 405 linear feet is bank stabilization, 615 linear feet of temporary stream impacts, 1.66 acres of permanent wetland impacts, no temporary wetland impacts, and no hand clearing in wetlands.

Please see the enclosed revised permit drawings and updated impact summary sheets which reflect the changes in impacts.

Permit Impact Site Modification

Site 18A (Roadway Plan Sheet 20): Site 18A impacts changed as a result of the decision to remove a 24" reinforced concrete pipe (RCP) located to the south of the Winston Salem Northern Beltway roadway alignment and to the west of Phelps Drive (Y6C). The parcels (5 total) where Stream ES-S68 (UT to Mill Creek) and the existing 24" RCP are located were obtained by NCDOT during the right of way process. Access to the parcel is being removed along with the homes and sheds that occurred on the parcel(s). This site was reviewed for onsite mitigation potential; however, due to a number of constraints, it was determined that the best solution was to remove the 24" RCP and replace it with a standard base ditch with embedded rip rap. The main constraint and reason to use the standard base ditch is due to the steep slope of the channel. There will be 20 linear feet of temporary impacts at this location in order to dewater the existing stream around during construction. Removal of the 24" RCP will result in 141 linear feet of channel being daylighted. There will be 7 linear feet of permanent bank stabilization impacts to the channel beyond the pipe to tie the channel in at the inlet location and 24 linear feet of permanent impact to correct the existing scour and tie at the pipe outlet. No additional impacts will occur at this site.

Site 26 (Roadway Plan Sheets 24/25): Site 26 impacts have not changed from the modification dated January 2020; however, the design has changed due to the noise walls. A ditch is required along the toe of fill that will tie to the existing bank stabilization at the inlet of ES-S24. The updated plan sheets are attached.

Compensatory Mitigation

At site 18A, compensatory mitigation is proposed for the 24' of new linear feet of permanent impacts to correct the existing scour and tie at the pipe outlet. No compensatory mitigation is proposed for the 7' of permanent bank stabilization impacts. Table 1 below illustrates the current summary of stream impacts requiring mitigation which has changed since the last permit modification request dated October 17, 2019.

Table 1. Summary of Stream Impacts by Project TIP Requiring Mitigation

| Section | Design Stage | Stream Impacts Requiring Mitigation from Phased IP Application (lf) | Final Stream Impacts Requiring Mitigation (lf) ¹ | Difference in Impacts Requiring Mitigation (lf) |
|--|--------------|---|---|---|
| R-2247EC | Permitted | 814 | 814 | 0 |
| R-2247CD | Permitted | 611 | 611 | 0 |
| U-2579C Phase 2 (U-2579D, E &F Sites 29 & 30) | Permitted | 3,409 | 3,178 | - 231 |
| U-2579AA | Preliminary | 3,019 | 3,019 | 0 |
| U-2579AB | Preliminary | 10,351 | 10,351 | 0 |
| U-2579D | Permitted | 5,043 | 4345+* | - 698 |
| U-2579E | Permitted | 4,864 | 4,817 | - 47 |
| U-2579F | Permitted | 2,544 | 2,502 | - 42 |
| R-2247EA | Preliminary | 0 | 774 ⁺ | 774 + |
| R-2247EB | Final | 5,819 + | 5,323 + | - 496 |
| | Totals | 36,474 + | 35,710 ² | - 764 |

¹ Final impacts for each section in the preliminary design stage will be updated with future modifications.

Thank you for your assistance with this project. If you have any questions or need additional information, please contact me at either aeuliss@ncdot.gov or (336) 747-7800.

Sincerely,

Amy Euliss

Amy Culiss

NCDOT Division 9 Environmental Officer

cc:

Bill Barrett, Environmental Analysis Unit

Michael Shumsky, PE, Project Manager, NCDOT Design Build Unit

Marcus Kiser, PE, NCDOT Division 9 Resident Engineer

Phil Suggs, NCDOT Division 9 and 10 Roadside Environmental Filed Operations Engineer

Beth Harmon, Division of Mitigation Services

Amy Chapman, NCDENR-DWR

Dave Wanucha, NCDENR-DWR

² Assumes the preliminary impacts for future sections.

⁺ Revised, per Permit Modification for R-2247EB [NCDOT's R-2247EB Modification application, dated June 26, 2019; USACE's R-2247EB Phased Modification (Action ID SAW-2017-02112), dated August 13, 2019] and Permit Modification for U-2579DEF [NCDOT's U-2579DEF Modification application, dated October 17, 2019; USACE'sU-2579DEF Modification (Action ID SAW-2017-02112), dated January 29, 2020].

^{*}Revised, per the current permit modification for U-2579DEF, dated November 4, 2020.

North Carolina Department of Transportation Environmental Analysis Unit (EAU) Raleigh, North Carolina

UT to Mill Creek and Mill Creek
On-site Stream Mitigation Feasibility Report for
Winston-Salem Northern Beltway
Forsyth County, North Carolina

T.I.P. Number U-2579 D, E, and F WBS No. 34839.3.GV7

May 22, 2020

1.0 BASELINE INFORMATION

The project is located within USGS Hydrologic Cataloging Unit 03040101, and NC Division of Water Resources (NCDWR) sub-basin 03-07-04 of the Yadkin Pee-Dee River Basin, and is part of the of the South Atlantic-Gulf region. The potential mitigation site is located along and to the west of Phelps Drive approximately 0.35 mile east of Baux Mountain Road in Forsyth County on the north side of Winston-Salem, on land that was purchased by the North Carolina Department of Transportation (NCDOT). The project is within the Piedmont physiographic province, specifically near the line between the Northern Inner Piedmont and the Southern Outer Piedmont Ecoregions. Land use within the watershed is primarily residential development, forested areas, and agricultural. The existing stream length, drainage area, and jurisdictional status are summarized in Table 1.

Table 1. Summary of Existing Stream Length and Drainage Area.

| Stream Reach | Existing Length (LF) | Drainage Area | Intermittent/Perennial Status |
|---------------------|----------------------|---------------|---|
| UT to Mill Creek | 419 | 17.7 ac | Stream reach considered intermittent by agencies during impact assessments. Status = <u>Intermittent</u> |
| Mill Creek | 285 | 3.45 sq. mi. | Stream reach considered perennial by agencies during impact assessments. Status = Perennial |

The project includes analyzing the feasibility of restoration, enhancement, and preservation of an unnamed tributary (UT) to Mill Creek as well as enhancement and restoration to areas along Mill Creek. Mill Creek has been assigned Stream Index Number12-94-7 (NCDWR 2020) and is designated a warm water stream with a classification of C. Class C waters are protected for secondary recreation, fishing, and aquatic life.

Most of the native plant communities have been removed from the site as the area has been mowed and maintained for the nearby residence and outbuildings which have now been removed through NCDOTs acquisition of the property.

NCDOT is analyzing the potential for on-site mitigation for jurisdictional stream impacts associated with Transportation Improvement Program (TIP) U-2579DEF as part of the construction of the Winston-Salem Northern Beltway. The project would serve as on-site mitigation through the restoration, enhancement, and preservation of 705 linear feet of streams if deemed feasible.

2.0 SITE SELECTION

TIP U-2579 DEF involves construction of the Winston-Salem Northern Beltway from US 52 to US 311 (New Walkertown Road). The stream restoration, enhancement and preservation site was selected due to its location within the U-2579DEF corridor and NCDOTs purchase of the property related to the project. The potential mitigation area includes approximately 1.5 acres within a Right of Way held by NCDOT. Within this area, the NCDOT reviewed the potential for restoration of the UT to Mill Creek, restoration of Mill Creek at the crossing of Phelps Dr, and enhancement along Mill Creek. These options are referred to as options 1, 2 and 3, respectfully. These options are summarized in table 2.

3.0 OPTIONS REVIEWED

Option 1: Restoration of Unnamed Tributary to Mill Creek

The existing UT to Mill Creek begins immediately downstream of a 24" pipe within the footprint of the U-2579DEF road construction project. The UT extends over 600 lf from this point to where it ties to Mill Creek. The far upstream extent of the UT is marked by severe channel and floodplain incision immediately downstream of the 24" pipe where the channel begins. From this point, moving downstream, the floodplain begins to widen allowing the channel some meander pattern progressing south toward Mill Creek. Around the midway point along the UT, the channel proceeds through another 24" reinforced concrete pipe for 141 lf.

Approximately 180 lf of the upper portion of the UT will be filled in by the new roadway facility (permit site 18). Extensive Class I Rip Rap outlet protection is called for in the construction plans to address stability concerns due to a 42" cross pipe and 18" pipe system outlet at this location. This mitigation feasibility analysis reviewed restoring the UT to a natural channel design from the terminus of the existing impact at permit site 18 to the tie in with Mill Creek.

The feasibility analysis reviewed three options for the Unnamed Tributary to Mill Creek. These are referred to as Options 1a, 1b, and 1c below.

• Option 1a proposes to remove 141 ft of existing 24" pipe and Provide full restoration of the UT to Mill Creek by grading a new floodplain, installing a number of grade control structures and steps to stabilize the channel, and plantings. See Figures 1 thru 3.

- Option 1b incorporates restoration, enhancement, and preservation of UT to Mill Creek. This option attempts to strike a balance between the costs of full restoration and obtaining mitigation credits. See Figures 4 thru 6.
- Option 1c, is based on removing 141 linear feet of existing 24" pipe and replacing it with a standard base ditch with embedded riprap for stability. This option does not propose mitigation credits for this work. Instead, it avoids a potential issue with the existing pipe in the future by removing it prior to access being removed or restricted. See Figures 7 and 8.

Issues Discovered with Options 1a and 1b

The feasibility analysis has several constraints which would make options 1a and 1b appear cost prohibitive on the UT to Mill Creek. These challenges include long term stability, grade control, cost associated with necessary cut to achieve adequate floodplain, and buffer limitations due to utility easements.

Stability and Grade Control Concerns: First and foremost was the channel slope which is 3.8% or more specifically the UT to Mill Creek drops 19 feet in elevation over 500 feet in length. In order to address that slope and provide full restoration on the UT to Mill Creek would take significant grading and a significant number of structures in order to stabilize the channel. Because of the slope of the channel, there is also a high potential for head cutting if any proposed structures were to fail within the restored channel. There is an approximately 8 foot drop to get from the UT elevation in the floodplain to the elevation of Mill Creek which poses a challenge for stability and would require a substantial structure and some grading if you left the channel at existing grade and simply tried to tie it in at Mill Creek. Even by doing that there is still a steep grade on the channel which would require a number of structures to stabilize the channel.

The existing 24" pipe to be removed has a slope of 2.5%. As shown in the profile drawing (Figure 6), there is notable drop-down in channel elevation as water enters the inlet side of the pipe. At the outlet end there is a 2' drop from pipe outlet to channel bottom. This outlet drop is stabilized by a small concrete pad.

Grade control structures would be needed for stability of the channel and prevention of potential head-cutting in the channel. In addition, the new channel will need to be steeper than existing pipe to avoid drastic elevation changes at the upstream and downstream ends. As seen in profile drawings for Options 1a and 1b (Figures 3 and 6) the longitudinal slopes would need to be 3.5-4% to better tie with the channel upstream and limit amount of grade change at the downstream end. Grade control needs to be achieved through the use of rock cross vanes. These have been shown on the mitigation plan drawings for Options 1a and 1b (Figures 1 and 4).

With the amount of grade change in this section, as well as the overall site, trying to achieve any riffle-pool sequence would be problematic as the riffle sections would need to be very short in

length and severely steep. Therefore a typical section resembling that of a constructed riffle with adequately sized stone to be embedded to channel depth would be required.

At the locations noted on Figures 4 and 7, the existing channel braids with some channel flow accessing the floodplain of Mill Creek. If we were to take this channel offline we would face challenges with grade change and tying a new channel into Mill Creek in a stable fashion.

Floodplain Concerns: In addition to the slope and stability issues noted above, another item that is of concern creating and maintaining the floodplain.

In the full restoration option (Option 1a) floodplain would need to be created due to channel elevations and topography. For studying the feasibility of this option we set the longitudinal grade by mirroring the existing channel grade for the UT and then stepping the proposed channel down after reaching the top of bank for Mill Creek (See Figure 3, profile). Proceeding in this fashion kept the longitudinal grade as low as possible (aided by drop structures near pipe removal) and limited the amount of floodplain excavation needed. Even still, we estimated approximately 1060 CY of grading.

Another concern is with potential backwater impacts to the channel that could occur due to high flows in Mill Creek. As mentioned above about stepping a new channel down to tie back in with Mill Creek, we would need to accomplish this with grade controls structures. There would be potential for these to degrade or fail due to effects of higher flows in Mill Creek.

Other unanticipated impacts that could occur include potentially impacting wetlands that exist in the floodplain of Mill Creek that were not previously mapped as jurisdictional to the west of the existing tributary.

Option 2: Restoration to Mill Creek at Phelps Drive

The two options reviewed at the crossing of Phelps Drive included restoration using natural channel design (option 2a) and stabilizing with conventional means as approved in the permit application (option 2b).

Option 2a: Option 2a includes restoration along Mill Creek where the existing culvert will be removed using natural channel design methods. The intent would be to lay back the slopes and remove the existing roadbed/fill to accomplish this. The slopes would be matted and planted with livestakes. The existing scour hole would be filled in and banks repaired to tie in with the new opening from removal of the culvert and the existing channel downstream. The goal would be to install a minimum of two structures to assist in stabilizing the channel grade where the culvert is removed and scour hole is repaired. There would not be any relocation of the channel with this option.

Option 2b: Option 2b includes removal of the existing culvert and stabilizing the area with riprap as detailed and illustrated in the permitted plans.

Issues Discovered with Options 2a

The main issue discovered when analyzing the options of restoration along Mill Creek include the potential to increase instability along the reach and the existing utility easement along the left bank of the channel. There is a potential to impact the utility easement by laying back the channel to a stable slope depending on the final designed slope. In addition, when laying back the slopes in just a short section of the channel there would be additional stress added to those banks when the flow necks back down to the existing channel and could create a potential area of erosion or scour. The channel has a large drainage area and storms or other events could significantly impact the stability of the reach during the time it takes to stabilize the plantings. This could lead to future maintenance issues and/or additional sedimentation into Mill Creek in the interim.

Option 3: Enhancements Along Mill Creek

Option 3 includes enhancing the riparian buffer along Mill Creek. This option includes planting of native bareroot tree and shrub species. The goal would be to plant a minimum 50-foot buffer if possible; however, there is an existing utility easement that would not allow that width to the south or along the left bank of Mill Creek. The minimum width of buffer along Mill Creek would be 35 feet along the utility easement.

Table 2. Summary of Restoration and/or Enhancement Options 1, 2 and 3 and Proposed Mitigation Amounts.

| Mitigation Area | Option | Existing Length | Potential Credits | Restoration Approach | | | |
|--------------------|--------------------------|--------------------|--|--|--|--|--|
| UT to Mill | UT to Mill 1a 420 LF 420 | | Restoration of entire UT to Mill Creek using | | | | |
| Creek | Tu | 120 21 | (1:1) | natural channel design. | | | |
| UT to Mill | | | 22 | Preservation will consist of maintaining the | | | |
| Creek | 1b | 87 LF | (4:1 ratio) | stream reach and existing stream riparian buffer | | | |
| CICCK | | | (4.1 18110) | as it currently is in the upper section of UT. | | | |
| | | | | Restoration will consist of removing the | | | |
| UT to Mill | 1b | 156 LF | 156 | existing 24" RCP and replacing it with a stable | | | |
| Creek | | | (1:1 ratio) | 3' wide base channel and building a 10' wide | | | |
| | | | | floodplain bench on each side. | | | |
| | | | | Enhancement will consist removing the | | | |
| | | | | existing trees which are growing in portions of | | | |
| UT to Mill | 1b | 177 LF | 44 | the channel and planting a native riparian | | | |
| Creek | | 1//LI | (4:1 ratio) | species typical of a bottomland hardwood | | | |
| | | | | forest. Any invasive tree/shrub species noted | | | |
| | | | | will be cut and treated with herbicide. | | | |
| | | | 89 | Restoration of the channel in the area of an | | | |
| Mill Creek | 2a | 89 LF | (1:1 ratio) | existing culvert and repair of adjacent scour | | | |
| | | | (1.1 14110) | hole utilizing natural channel design. | | | |

| Mill Creek | 2b | 89 LF | 0 | Removal of existing culvert and install rip rap channel per permitted plans. |
|------------|----|--------|-------------------|--|
| Mill Creek | 3 | 196 LF | 49 (4:1 ratio) | Enhancement will consist of planting a native riparian buffer with species typical of a bottomland hardwood forest. Any invasive tree/shrub species noted will be cut and treated with herbicide. Full 50 ft wide buffer is not possible due to existing utility easement. |

4.0 PROJECT COST ESTIMATES FOR UT TO MILL CREEK

Three cost estimates are provided below based on the three options that have been reviewed for the potential mitigation site (Table 3). The first cost estimate provided is based on providing a full restoration of the UT to Mill Creek by grading a new floodplain, installing a number of grade control structures and steps to stabilize the channel, and plantings. In the table below this is Option 1a.

Option 1b below is based on the proposed plan detailed in the previous sections of this document which incorporate restoration, enhancement, and preservation of UT to Mill Creek. This option attempts to strike a balance between the costs of full restoration and obtaining mitigation credits. The Table below illustrates the cost estimate for Option 2.

The final option, Option 1c, is based on removing 141 linear feet of existing 24" pipe and replacing it with a standard base ditch. This option does not propose mitigation credits for this work. Instead, it avoids a potential issue with the existing pipe in the future by removing it prior to access being removed or restricted.

Table 3. Cost Estimates of Potential Options for UT to Mill Creek.

| Option | Description | Potential Mitigation Credits | Estimated Cost of Construction ¹ |
|---|---|------------------------------------|--|
| Option 1a | Full Restoration of UT to Mill Creek | 420 | \$180,000 |
| Option 1b | Combination of Restoration, Enhancement, and Preservation to UT to Mill Creek | 222 | \$111,000 |
| Option 1c | No Restoration – Pipe Removal on UT to Mill Creek | 0 | \$41,000 ² |
| Option 2a Full Restoration – Mill Creek | | 89 | \$110,500 |
| Option 2b | No Restoration – Pipe Removal on Mill Creek | 0 | Included in the cost of the D-B contract |

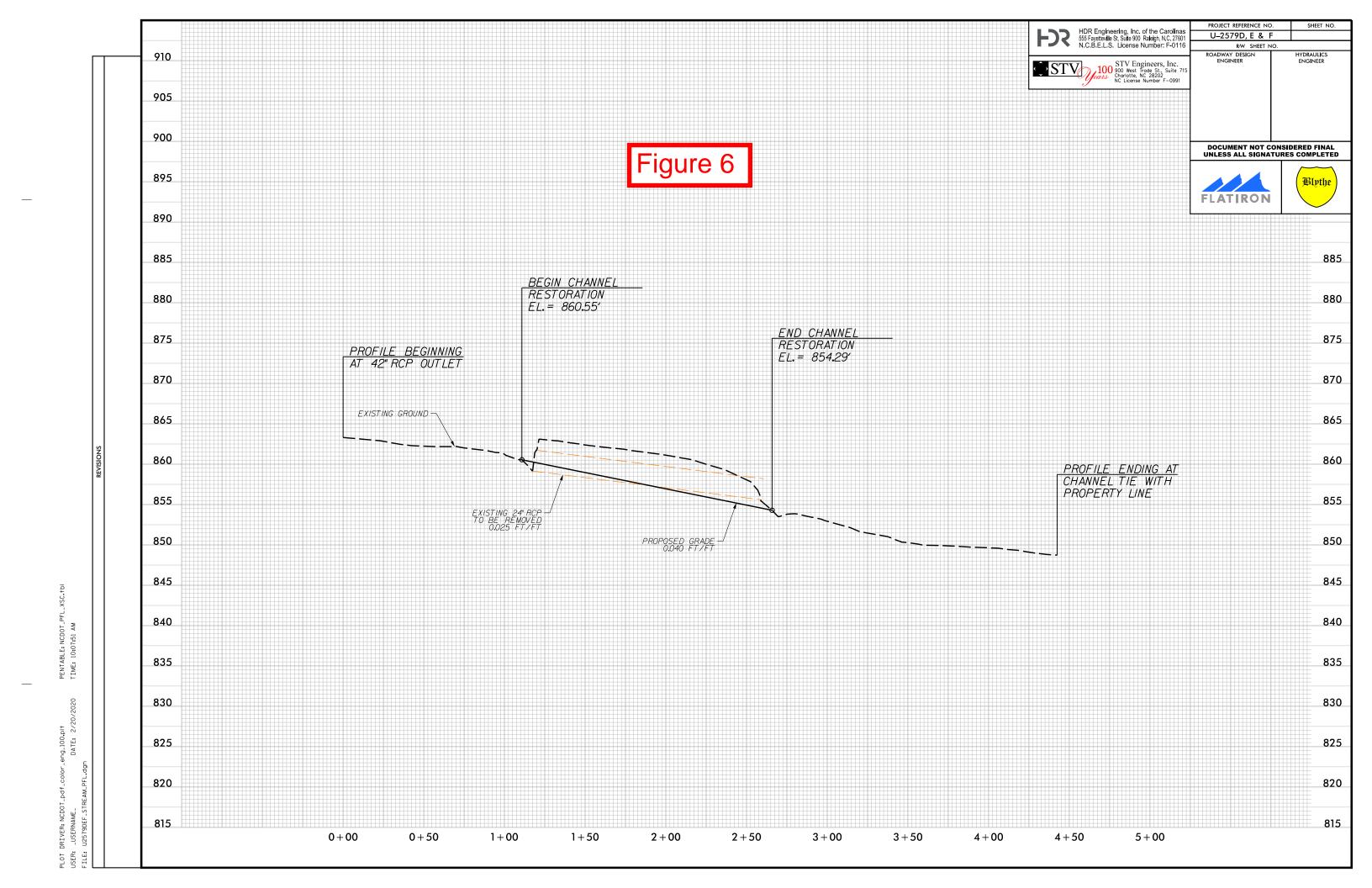
| Option 3 Enhancemen | t – Mill Creek 49 | \$7,700 |
|---------------------|-------------------|---------|
|---------------------|-------------------|---------|

¹ Please note: This is a preliminary Rough Oder of Magnitude Price only and final pricing would need to be submitted following issuance of RFC plans for the selected option.

Recommendations

Based on the above description of options regarding mitigation at the UT to Mill Creek and Mill Creek at Phelps Drive, NCDOT has determined the best path forward as Option 1c which is to remove the pipe on the UT to Mill Creek and stabilize the area and 2b which is removing the pipe on Mill Creek and proceeding with the permitted plans for stabilizing the channel. The decision to move in this direction was based on a number of factors such as cost of construction when weighed against the potential mitigation credits, long term monitoring and maintenance of the site, and utility encroachments. The overall benefit of doing full restoration would not provide significant ecological uplift on either reach and could introduce instability of either reach following construction.

² Assumes work would be completed by the D-B contractor rather than an approved mitigation contractor.



MATCHLINE
-Y6A- STA 13+00
SEE SHEET 51 PROJECT REFERENCE NO. SHEET NO. HDR Engineering, Inc. of the Carolina 555 Fayetteville St, Sulle 900 Raleigh, N.C. 2760 N.C.B.E.L.S. License Number: F-011 U-2579D, E & F 20 185 R/W SHEET NO HYDRAULICS ENGINEER ROADWAY DESIGN ENGINEER STV Engineers, Inc.

100 900 West Trade St., Suite
Charlotte, No. 28202
NC License Number F-0991 185 DOCUMENT NOT CONSIDERED FINAL UNLESS ALL SIGNATURES COMPLETED 605 Blythe SITE 19 FLATIRON ES-S19 183 182 655 606 604 184 (181) 269+00 604 50 8 ³N 50" 55′₁10.0" W 8 3 -L- STA SHEET 21 누동 BEGIN MATCHLINE SEE HLINE NW16 **(** ES-S68-186 SITE 18 DETAIL 20-12 STANDARD BASE DITCH 187 188 Min. D=2Ft. Max. d=1.5Ft. B=3Ft. SEE SHEET (180) SITE 18A DETAIL 20-10 DETAIL 20-11
FLOODPLAIN EXCAVATION DETAIL 20–8
BANK STABILIZATION AT
JURISDICTIONAL STREAM MILL CREEK (ES-S20) CLASS 'I'

RIP RAP

NATURAL STREAM

BED ELEVATION VARIES' Type of Liner = CL | Rip-Rap STA, 24+34 -Y6C- LT STA. 271+50 -L- RT STA. 280+78 -L- RT MILL CREEK STA. 279 + 50 -L- LT SITE 20 DETAIL 20-4 LATERAL BASE DITCH DETAIL 20-3 (ES-S20) **LEGEND**: Min. D = 3.0Ft.

Max. d = 3.0Ft.

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MATCHLINE
-Y6A- STA 13+00
SEE SHEET 51 PROJECT REFERENCE NO. SHEET NO. HDR Engineering, Inc. of the Carolina 555 Fayetteville St, Sulle 900 Raleigh, N.C. 2760 N.C.B.E.L.S. License Number: F-011 U-2579D, E & F 20 185 R/W SHEET NO. HYDRAULICS ENGINEER ROADWAY DESIGN ENGINEER John STV Engineers, Inc. 900 West Trade St., Suite 1 Charlotte, NC 28202 NC License Number F-0991 185 DOCUMENT NOT CONSIDERED FINAL UNLESS ALL SIGNATURES COMPLETED 605 Blythe SITE 19 FLATIRON ES-S19 183 655 606 604 184 (181) 269+00 604 50 N 50" 55" 10.0" W 281 -L- STA SHEET 21 누동 BEGIN MATCHLINE SEE CHLINE **NW16** ES-S68 186 SITE 18 (187) 188 Min. D=2Ft. Max. d=1.5Ft. B=3Ft. SEE SHEET 20A (180) SITE 18A DETAIL 20-10 DETAIL 20-11
FLOODPLAIN EXCAVATION DETAIL 20–8
BANK STABILIZATION AT
JURISDICTIONAL STREAM MILL CREEK (ES-S20) Type of Liner = CL | Rip-Rap STA, 24+34 -Y6C- LT MILL CREEK STA. 271+50 -L- RT STA. 280+78 -L- RT STA. 279 + 50 -L- LT SITE 20 DETAIL 20-4 LATERAL BASE DITCH DETAIL 20-3 (ES-S20) **LEGEND**: PERMIT DRAWING SHEET 61 OF 97 Min. D = 1.5Ft. Max. d = 1.5Ft. B = 6.0Ft. b = 5.0Ft. DENOTES TEMPORARY
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PROJECT REFERENCE NO. SHEET NO. COAPUIC SCALE HDR Engineering, Inc. of the Carolinas 555 Fayetteville St, Suite 900 Raleigh, N.C. 2760 N.C.B.E.L.S. License Number: F-0116 U-2579D, E & F 24 RW SHEET NO HYDRAULICS ENGINEER ROADWAY DESIGN ENGINEER STV Engineers, Inc. 900 West Trade St., Suite 7 Charlotte, NC 28202, NC License Number F-0991 GEOTEXTILE B Min. D = 2.0Ft. Max. d = 2.0Ft. Type of Liner = Class 'I' Rip-Rap B = 5.0Ft. FROM _L_ STA. 335+73 TO STA. 337+50 LT (191A) LEGEND: DETAIL 24-6
OUTLET CHANNEL PROTECTION
(Not to Scale) Natural 8 DENOTES IMPACTS IN SURFACE WATER DOCUMENT NOT CONSIDERED FINAL UNLESS ALL SIGNATURES COMPLETED (191) DENOTES TEMPORARY
IMPACTS IN SURFACE WATER ES-\$49 **ES-S50** Blythe ES-W23 FLATIRON Type of Liner - CL 'II' RIP RAP STA. 335 + 81 -L- LT 704 SITE 25 2' LATERAL BASE DITO CLASS 'I' RIPRAP EST 159 TONS WINSTON-SALEM CITY LIMITS 192 ES-S24 47" WW FENCE ES-S23 SITE 26 50 50 -L- STA 337 SHEET 25 FS 2GI-A က 32 40 ST, 구분 \$ 29°00′ 58.0″ E I MATCHLINE SEE MATCHLINE SEE BEGIN JS 704 FS 2GI 189 2' BERM DITCH & CLASS (I' RIPRAP EST 454 TONS 768 SY GEOTEXTILE SEE DETAIL 24–5 83 2' BERM DITCH CLASS 'B' RIPRAP EST 58 TONS 100 SY GEOTEXTILE SEE DETAIL 24–12 47" WW FENCE BERM DITCH SEE DETAIL 24-9 803 2' BERM DITCH SEE DETAIL 24–10 2' LATERAL BASE DITCH — CLASS 'I' RIPRAP EST 60 TONS 101 SY GEOTEXTILE SEE DETAIL 24-1 DDE = 206 CY (704) 190 N 21-24 18.8" W 190 PERMIT DRAWING SHEET 72 OF 97

PROJECT REFERENCE NO. GRAPHIC SCALE SHEET NO. HDR Engineering, Inc. of the Carolina 555 Fayetteville St, Suite 900 Raleigh, N.C. 2760 N.C.B.E.L.S. License Number: F-0110 U-2579D, E & F 24 RW SHEET NO. HYDRAULICS ENGINEER ROADWAY DESIGN ENGINEER STV Engineers, Inc. 900 West Trade St., Suite 7 Charlotte, No. 28202 NC License Number F-0991 FROM _L_ STA. 335+73 TO STA. 337+50 LT LEGEND: (191A) DETAIL 24-6
OUTLET CHANNEL PROTECTION
(Not to Scale) ______Notural f DENOTES IMPACTS IN SURFACE WATER DOCUMENT NOT CONSIDERED FINAL UNLESS ALL SIGNATURES COMPLETED (191) DENOTES TEMPORARY
IMPACTS IN SURFACE WATER ES-S49 **ES-S50** Blythe **ES-W23** FLATIRON of Liner - CL 'II' RIP RAP STA. 335 + 81 -L- LT 704 SITE 25 WINSTON-SALEM CITY LIMITS 192 ES-S24 ES-S23 SITE 26 50 50 -L- STA 337-SHEET 25 FS ↓ 2GI–A 323 -L- ST/ SHEET MATCHLINE SEE MATCHLINE SEE BEGIN JS. 704 FS 189 2' BERM DITCH CLASS (I' RIPRAP EST 454 TONS 768 SY GEOTEXTILE SEE DETAIL 24-5 2 BERM DITCH CLASS 'B' RIPRAP EST 58 TONS 100 SY GEOTEXTILE SEE DETAIL 24-12 47" WW FENCE BERM DITCH SEE DETAIL 24-9 704 2' LATERAL BASE DITCH -CLASS-'I' RIPRAP EST 60 TONS 101 SY GEOTEXTILE N 21° 24′ 18.8° W PERMIT DRAWING SHEET <u>73</u> OF <u>97</u>

15+00 PROJECT REFERENCE NO. SHEET NO. HDR Engineering, Inc. of the Carolina 555 Fayetteville St, Suite 900 Raleigh, N.C. 2760 N.C.B.E.L.S. License Number: F-011 U-2579D, E & F 25 RW SHEET NO HYDRAULICS ENGINEER 생생 ROADWAY DESIGN ENGINEER 100 STV Engineers, Inc.
900 West Trade St., Suite 7
Charlotte, NC 28202
NC License Number F-0991 © 611) GRAPHIC SCALE 198 DOCUMENT NOT CONSIDERED FINAL UNLESS ALL SIGNATURES COMPLETED # 192 뜻 661 뜌 Blythe FLATIRON 199 땱 N 85 28 58 W 350+00 340+00 677 LAND 80 95 673 608 S 32" 34',27.5" W 610 2/ 20+00 232 \bigcirc N 82 27' 231" W + N. 351-15+00 NW19-1B BERM DITCH SEE DETAIL 25-9 MATCHLINE 4 337+50 S ΔÃ SHEET ST MATCHLINE 608 **MANAGE** 15" RCP-III 193 TOE PROTECTION SEE DETAIL 25-7 # 610 ES-S24 - 2/ LATERAL BA F ST CLASS 11' RIPRAL EST 69 TONS 117 SY GEOTEXTILE SEE DETAIL 25-3 DDE = 304 CY 190 _L_ +17.06 175.00^RT _L_ +71.59 214.35' RT MATCHINE 00 NATCHINE 00 YISEE SHEET 52 47" WW FENCE 602 (190A) LATERAL 'V' DITCH SEE DETAIL 25-2 DDE = 47 CY 193 SESTEMONS 26 612 345+00 LATERAL 'V' DITCE SEE DETAIL 25-2 DDE = 9 CY BEGIN JS #[#] BEGIN JS DETAIL 25-8
BANK STABILIZATION AT
JURISDICTIONAL STREAM
(Not to Scale) DETAIL 25-7
TOE PROTECTION
(Not to Scale) NATURAL BED -ELEVAT**I**ON LEGEND: DENOTES IMPACTS IN SURFACE WATER PERMIT DRAWING SHEET 74 OF 97 Type of Liner= Class 'I' Rip-Rap Type of Liner = CL | Rip-Rap denotes temporary impacts in surface water FROM -L- STA. 337+50 TO STA. 338+52 RT FROM -L- STA. 338+53 TO STA. 340+50 RT STA. 338 + 75 -L- RT

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900 West Trade St., Suite 7
Charlotte, NC 28202
NC License Number F-0991 611 GRAPHIC SCALE 198 DOCUMENT NOT CONSIDERED FINAL UNLESS ALL SIGNATURES COMPLETED 192 661 Blythe FLATIRON (199) N 85 28 58 W 350+00 677 LAND 800 95 673 608 S 32" 34" 27.5" W 610 232 N 82 27 23. W + 15+00 351-NW19-1B BERM DITCH SEE DETAIL 25-9 MATCHLINE 4 337+50 S ΔÃ SHEET ST MATCHLINE 608 193 610 ESUS24 190 <u>-L-</u> +71.59 214.35′ RT MATCHUNE 00 NATCHUNE 00 YISEE SHEET 52 47" WW FENCE 602 (190A) LATERAL 'V' DITCH-SEE DETAIL 25-2 DDE = 47 CY **ES-S51** SEST EONS 26 193 612 345+00 BEGIN JS BEGIN JS DETAIL 25–8
BANK STABILIZATION AT
JURISDICTIONAL STREAM
(Not to Scale) DETAIL 25-7
TOE PROTECTION
(Not to Scale) NATURAL BED ELEVATION LEGEND: DENOTES IMPACTS IN SURFACE WATER PERMIT DRAWING SHEET 75 OF 97 Type of Liner = Class 'I' Rip-Rap Type of Liner = CL | Rip-Rap denotes temporary impacts in surface water FROM -L- STA. 337+50 TO STA. 338+52 RT FROM -L- STA. 338+53 TO STA. 340+50 RT STA. 338 + 75 -L- RT

| | | | | , | WETLAND | DEDMIT IN | ACT CLIM | IMADV | | | | |
|----------|------------------------|------------------------|-----------|----------|-----------------------|-------------------|----------|-----------|---------|-------------|----------|---------|
| | | ĺ | | | WETLAND FLAND IMPA | PERMIT IMF CTS | ACI SUM | IIVIAK Y | SURFA | CE WATER IN | //PACTS | |
| | | | | | | | Hand | | | Existing | Existing | |
| | | | Permanent | Temp. | Excavation | Mechanized | Clearing | Permanent | Temp. | Channel | Channel | Natural |
| Site | Station | Structure | Fill In | Fill İn | in | Clearing | in | SW | SW | Impacts | Impacts | Stream |
| No. | (From/To) | Size / Type | Wetlands | Wetlands | Wetlands | in Wetlands | Wetlands | impacts | impacts | Permanent | Temp. | Design |
| | | | (ac) | (ac) | (ac) | (ac) | (ac) | (ac) | (ac) | (ft) | (ft) | (ft) |
| 11 | 173+80 -L- | 48" RCP | | | | | | 0.04 | | 615 | | |
| 11A** | 174+23 -L- RT | 48" RCP | 0.08 | | | | | | | | | |
| 12 | 203+75 - 207+10 -L- RT | ROADWAY FILL | | | | | | 0.02 | < 0.01 | 410 | 10 | |
| 12A | 205+69 -L- RT | FILL IN WETLAND | 0.08 | | | | | | | | | |
| 13 | 207+24 -L- | 5x6 CULVERT | | | | | | 0.03 | < 0.01 | 337 | 22 | |
| 13A | 209+14 -L- LT | 4' BASE DITCH | | | | < 0.01 | | | | | | |
| 14 | 222+50 - 223+56 -L- RT | 48" RCP | | | | | | 0.01 | < 0.01 | 220 | 11 | |
| 14A | 222+65 -L- LT | POND | | | | | | 3.06 | | | | |
| 15 | 242+86/244+20 -L- | 5'x6' RCBC | | | | | | 0.05 | < 0.01 | 520 | 15 | |
| 15A | 243+60/244+00 -L- | 5'x6' RCBC | 0.04 | | | | | | | | | |
| 16 | 253+92/255+58 -L- | 42" RCP AND DITCH | | | | | | 0.05 | < 0.01 | 513 | 20 | |
| 17 | 261+02/262+96 -L- | 6'x6' RCBC | | | | | | 0.08 | < 0.01 | 928 | 23 | |
| 18 | 275+75/276+78 -L- | 42" RCP | | | | | | 0.03 | < 0.01 | 161 | 11 | |
| 18A | 277+00 RT -L- | PIPE REMOVAL AND DITCH | | | | | | < 0.01 | < 0.01 | 24 | 20 | |
| 18A | 277+00 RT -L- | BANK STABILIZATION | | | | | | < 0.01 | | 7 | | |
| 19 | 279+30/280+86 -L- | 5'x7' RCBC | | | | | | 0.04 | < 0.01 | 250 | 11 | |
| 19 | 280+86/281+71 -L- | 5' STANDARD BASE DITCH | | | | | | < 0.01 | < 0.01 | 91 | 15 | |
| 20 | 280+61/281+66 -L- | REMOVE 2 - 72" CMP | | | | | | 0.02 | < 0.01 | 35 | 31 | |
| 20A | 285+56/285+81 -L- | BANK STABILIZATION | | | | | | < 0.01 | < 0.01 | 16 | 21 | |
| 20B | 285+56/285+81 -L- | BANK STABILIZATION | | | | | | < 0.01 | < 0.01 | 20 | 21 | |
| 20C | 284+44/285+33 -L- | TEMPORARY WORK PAD | | | | | | | 0.06 | | 165 | |
| 21 | 285+19/285+65 -L- | CHANNEL CHANGE | | | | | | < 0.01 | | 133 | | |
| 21 | 285+19 -L- | BANK STABILIZATION | | | | | | < 0.01 | < 0.01 | 10 | 10 | |
| 22 | 303+57/304+09 -L- | 30" RCP/ 36" RCP | | | | | | < 0.01 | < 0.01 | 97 | 7 | |
| TOTALS S | SHEET 2*: | | 0.20 | | | < 0.01 | | 3.47 | 0.09 | 4387 | 413 | 0 |

^{*}Rounded totals are sum of actual impacts

NOTES:

-**Wetland ES-W33 is total take

-Site 24 is a Channel Relocation

NC DEPARTMENT OF TRANSPORTATION
DIVISION OF HIGHWAYS

11/03/2020
FORSYTH
U-2579D, E & F
34839.3.GV7
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| | | | | WI | FTI AND PE | ERMIT IMPAG | CT SUMM | ARY | | | | |
|----------|-----------------------|------------------------------|-----------|----------|------------|-------------|------------------|-----------|---------|---------------------|---------------------|---------|
| | | | | | TLAND IMPA | | <u> </u> | | SURFA | CE WATER IN | IPACTS | |
| | | | Permanent | Temp. | Excavation | Mechanized | Hand Clearing | Permanent | Temp. | Existing Channel | Existing Channel | Natural |
| Site | Station | Structure | Fill In | Fill In | in | Clearing | in | SW | SW | Impacts | Impacts | Stream |
| No. | (From/To) | Size / Type | Wetlands | Wetlands | Wetlands | in Wetlands | Wetlands | impacts | impacts | Permanent | Temp. | Design |
| | | | (ac) | (ac) | (ac) | (ac) | (ac) | (ac) | (ac) | (ft) | (ft) | (ft) |
| 23 | 316+25/317+41 -L- | 6'x6' RCBC | | | | | | 0.04 | | 363 | | |
| 23 | 317+40/317+49 -L- | BANK STABILIZATION | | | | | | < 0.01 | < 0.01 | 28 | 10 | |
| 24 | 316+84/317+01 -L- | CHANNEL CHANGE | | | | | | < 0.01 | < 0.01 | 120 | 10 | |
| 25 | 323+23/325+22 -L- | 24" RCP | | | | | | 0.02 | < 0.01 | 294 | 10 | |
| 26 | 335+66/338+74 -L- | 6'x6' RCBC | | | | | | 0.05 | < 0.01 | 523 | 10 | |
| 26 | 338+82 -L- | BANK STABILIZATION | | | | | | < 0.01 | < 0.01 | 29 | 8 | |
| 27 | 356+07/357+64 -L- | 7'x7' RCBC | | | | | | 0.05 | | 328 | | |
| 27 | 356+07 -L- LT | BANK STABILIZATION | | | | | | < 0.01 | < 0.01 | 35 | 8 | |
| 27 | 357+63 -L- RT | BANK STABILIZATION | | | | | | < 0.01 | < 0.01 | 37 | 12 | |
| 28** | 397+37/400+85 -L- | ROADWAY FILL | 0.26 | | | | | | | | | |
| 29** | 398+61/405+73 -L- | ROADWAY FILL / 5' BASE DITCH | | | | | | 0.06 | | 906 | | |
| 29** | 398+61/405+73 -L- | BANK STABILIZATION | | | | | | < 0.01 | < 0.01 | 40 | 10 | |
| 29A** | 401+96 -L- | ROADWAY FILL | < 0.01 | | | | | | | | | |
| 29B** | 402+33/405+59 -L- | ROADWAY FILL / 5' BASE DITCH | 0.26 | | 0.03 | 0.01 | | | | | | |
| 30*** | 409+52/411+61 -L- | 54" RCP | | | | | | 0.05 | < 0.01 | 563 | 14 | |
| 30*** | 409+53 -L- | BANK STABILIZATION | | | | | | < 0.01 | < 0.01 | 15 | 10 | |
| 31 | 49+59 - 50+28 -Y3- LT | 2' BASE DITCH | | | | | | < 0.01 | | 57 | | |
| 32 | 35+68/37+32 -Y6- | BANK STABILIZATION | | | | | | 0.03 | < 0.01 | 172 | 19 | |
| 33 | 173+50 -L- LT | CHANNEL IMPROVEMENT TIE-IN | | | | | | < 0.01 | < 0.01 | 20 | 21 | |
| 34 | 357+38/357+49 -L- | BANK STABILIZATION | | | | | | < 0.01 | < 0.01 | 24 | 11 | |
| | | TOTAL SHEET 1 | 0.87 | | < 0.01 | 0.01 | | 0.49 | < 0.01 | 5694 | 49 | |
| | | TOTAL SHEET 2 | 0.20 | | | < 0.01 | | 3.47 | 0.09 | 4387 | 413 | |
| | | TOTAL SHEET 3 | 0.53 | | 0.03 | 0.01 | | 0.34 | 0.02 | 3554 | 153 | |
| TOTALS A | ALL SHEETS*: | | 1.60 | 0.00 | 0.03 | 0.03 | 0.00 | 4.31 | 0.11 | 13635 | 615 | |

^{*}Rounded totals are sum of actual impacts

NOTES

- ISOLATED WETLANDS SITE
- SITE 24 IS A CHANNEL RELOCATION

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SHEET 97 OF 97

^{**} SITE 28, 29, 29A, AND 29B - SITE 1 IN U-2579C PHASE 2 PERMIT

^{***} SITE 30 - SITE 2 IN U-2579C PHASE 2 PERMIT