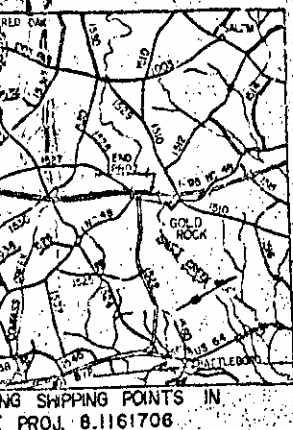


NASH COUNTY

LOCATION: I-95 FROM APPROXIMATELY 317'± SOUTH OF SR.1604
NORTHEASTERLY TO SR.1522, ± 1 MILE SOUTH OF
GOLD ROCK.

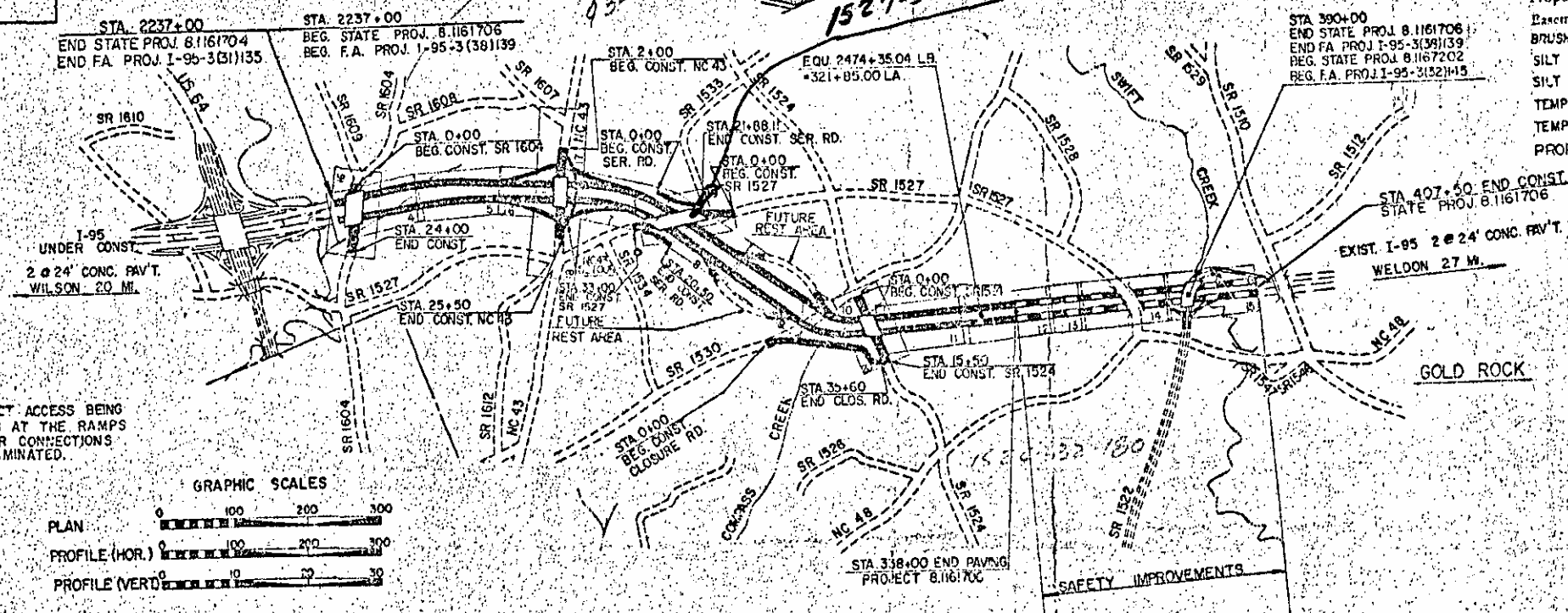
TYPE: THIS CONTRACT INCLUDES GRADING, DRAINAGE, PAVING,
STRUCTURES, AND SAFETY IMPROVEMENTS.



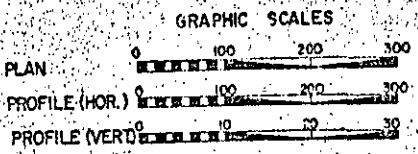
TOTAL LENGTH FA PROJECT I-95-3(38)139 = 5.786 MILES
TOTAL LENGTH STATE PROJECT 8.1161706 = 5.786 MILES

DESIGN DATA

1975 ADT	15,600 - 16,000
1995 ADT	30,000 - 31,000
DHV	11 %
D	60 %
T	23 %
V	70 MPH



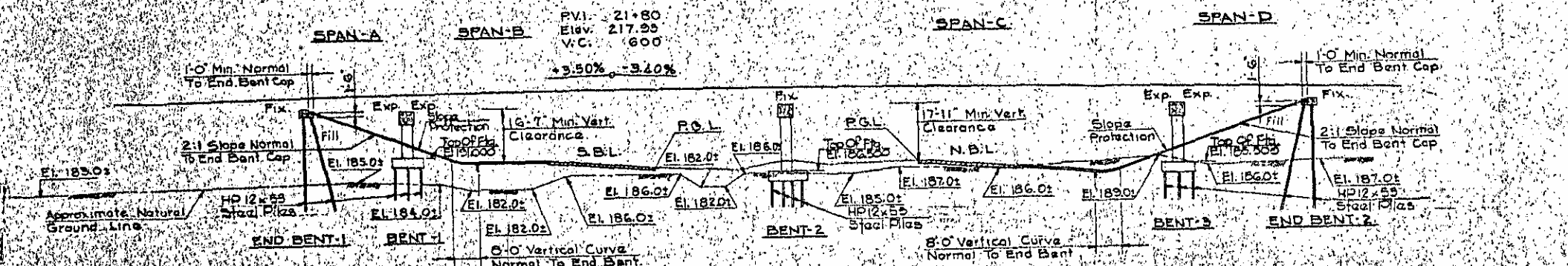
CONTROLLED ACCESS PROJECT ACCESS BEING
POINTS SHOWN ON PLANS AT THE RAMPS
SEPARATIONS. ALL OTHER CONNECTIONS
CROSSINGS ARE TO BE ELIMINATED.



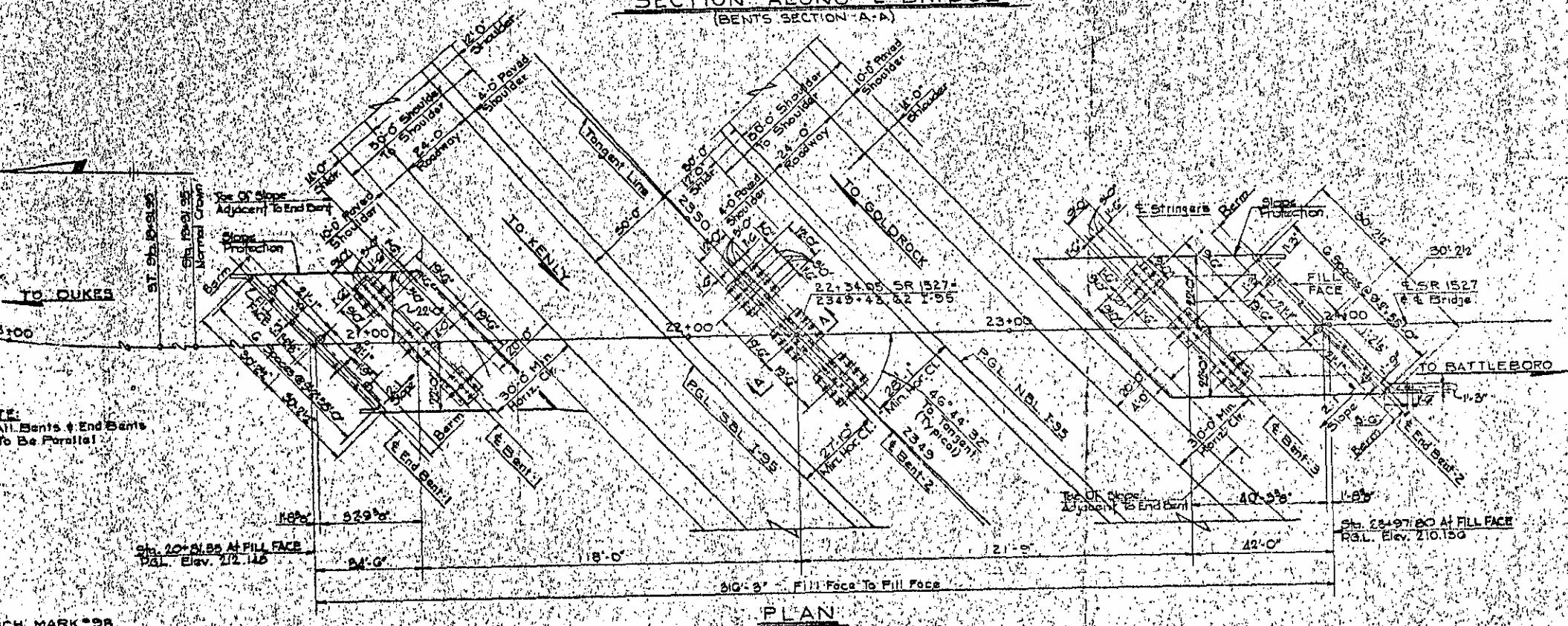
MOON MORTON
RUMMEL KLEPPER & KAHL
NASH COUNTY, N.C.

1972 STANDARD SPECIFICATIONS THE R/W ON THIS PROJECT IS AS SHOWN ON PLANS

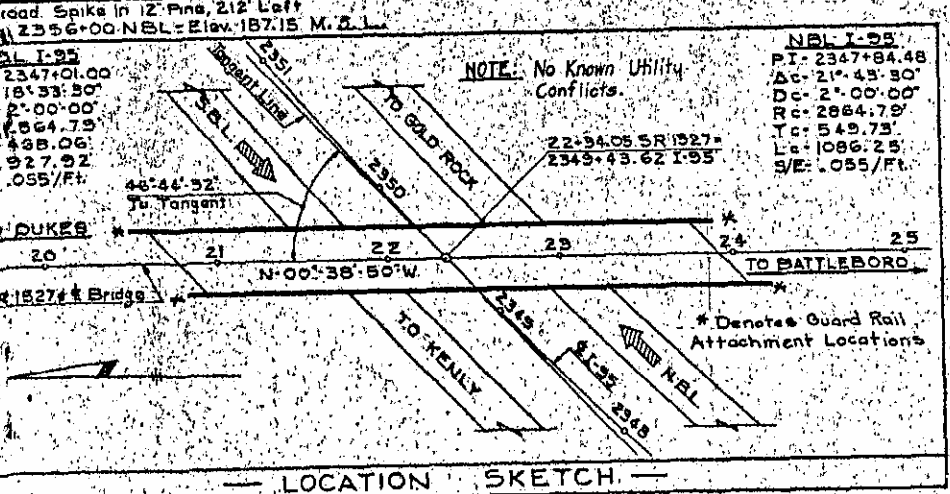
County L.
City or T.
Exist. R.
Rights of
Survey L.
Property
Exist. Pr.
Proposed
Existing
Railroad
Control
Slope St.
Bridge
Culvert
Woods
Telepho
Tower L.
Power P.
Propose
Guard R.
Sanitary
Water L.
Gas Lin
Propose
Easeme
BRUSH
SILT C
SILT F
TEMPO
PROR



SECTION ALONG E BRIDGE
 (BENTS SECTION A-A)



PLAN



LOCATION SKETCH

NOTES

ASSUMED LIVE LOAD: HS 15-44

REFERENCE TO SHEET S-N: For Other Data And General Notes See Sheet S-N

PILE CAPACITIES: Piles Are Designed For Bearing Capacities Of 30-Tons Per Pile

EXCAVATION: No Work Shall Be Started On This Bridge Until After The Roadway Section has been Graded. The Roadway Contractor Will Be Required To Remove The Existing Pavement And Scarify The Subgrade To A Minimum Depth Of 2'-0" Within The Area Of End Bent Piles.

PILES DRIVEN THRU FILL AT END BENTS: See Specifications

STRUCTURAL STEEL: All Structural Steel Shall Be ASTM A36 Grade.

PAINTING STRUCTURAL STEEL: Structural Steel For This Structure Shall Be Painted In Accordance With Paint System 4. See Special Provisions For galvanized High Strength Bolts. See Special Provisions.

REINFORCING STEEL LENGTHS ARE BASED ON THE FOLLOWING MINIMUM SPLICE LENGTHS

BAR SIZE	SPLICE DISTANCE
# 4	1'-3"
# 5	1'-9"
# 6	2'-0"
# 7	2'-3"
# 8	2'-6"
# 9	2'-9"
# 10	3'-3"
# 11	3'-6"

TOTAL BILL OF MATERIAL

	CLASS AA CONCRETE	CLASS A CONCRETE	REINFORCING STEEL	STRUCTURAL STEEL	HP1253 STEEL PILES	LINSEED OIL CONCRETE PROTECTION	FOUNDATION EXCAVATION	4" SLOPE PROTECTION	1-BAR METAL RAILING	BRIDGE APPROACH SLABS
	CU. YDS.	CU. YDS.	LBS.	APPROX. LBS.	NO. LIN. FT.	GALLONS	CU. YDS.	SQ. YDS.	LIN. FT.	LUMP SUM
SUPERSTRUCTURE	384.0		86,024	325,200		29		210		
END BENT NO. 1		20.8	3,808		10	725				
BENT NO. 1		63.0	10,023		27	1,458	100			
BENT NO. 2		91.7	17,438		48	2,370	150			
BENT NO. 3		75.3	10,861		27	1,169	130			
END BENT NO. 2		23.6	4,379		11	545		270		
CURVED END BLOCK		0.8	85							
TOTALS	384.6	274.2	182,618	325,200	123	6,267	29	380	480	621.83 LUMP SUM

1527-32-170

PROJECT NO. 1161700

NASH COUNTY

STATION: 2349+43.62 I-95
 22+34.05 S.R. 1527

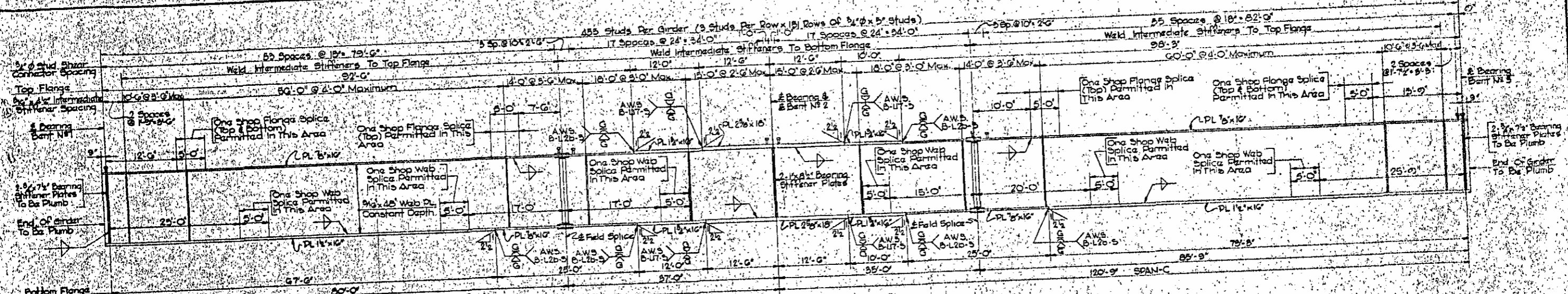
STATE OF NORTH CAROLINA
STATE HIGHWAY COMMISSION
 GENERAL DRAWING FOR BRIDGE
 ON S.R. 1527 OVER PROJECT 1-95
 BETWEEN DUKE AND BATTLEBORO

RUMMEL, KLEPPER & KAHL
 CONSULTING ENGINEERS
 RALEIGH, NORTH CAROLINA

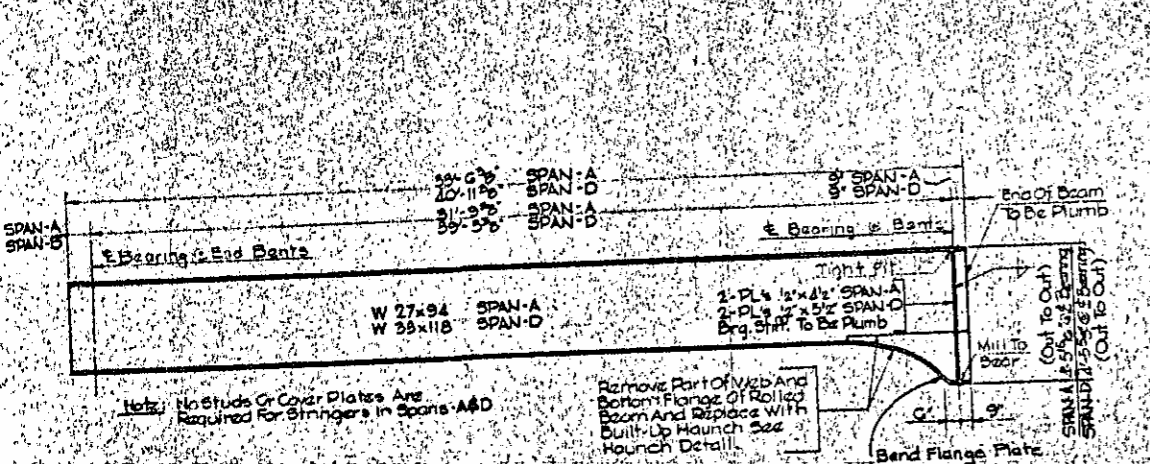
REVISIONS

NO.	BY	DATE	NO.	BY	DATE
1			2		
2			3		
3			4		

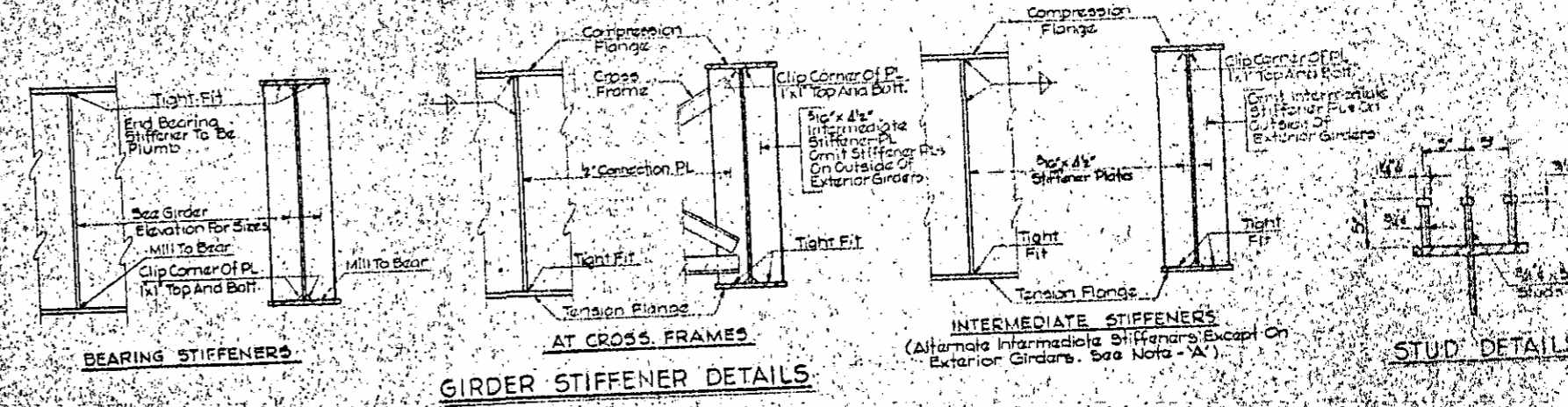
5-37
 76



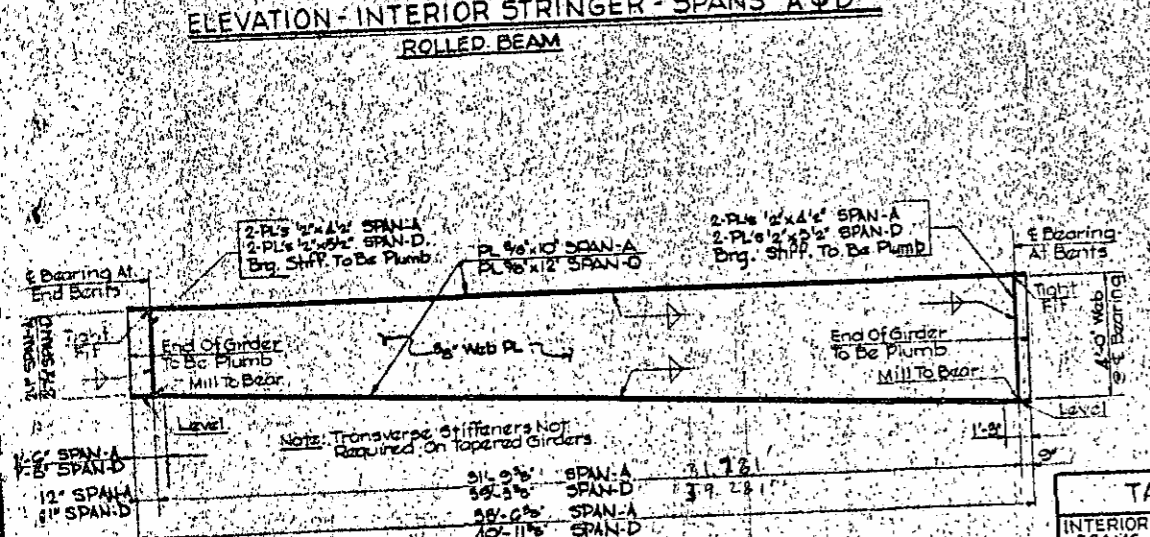
GIRDER ELEVATION



ELEVATION - INTERIOR STRINGER - SPANS A & D
ROLLED BEAM



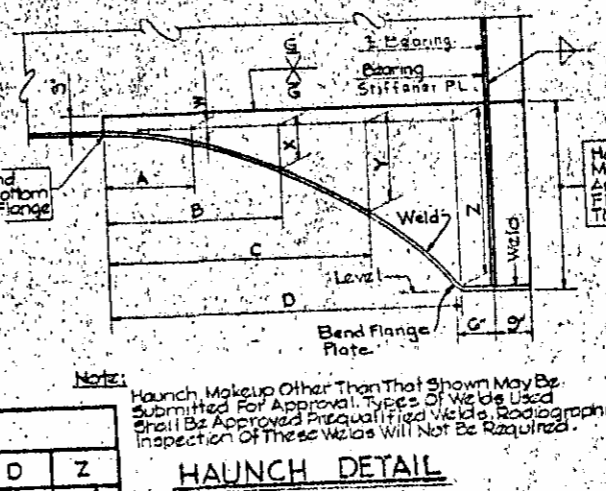
GIRDER STIFFENER DETAILS



ELEVATION - EXTERIOR STRINGER - SPANS A & D
TAPERED GIRDER

TABLE OF DIMENSIONS

	A	W	B	X	C	Y	D	Z
INTERIOR BEAMS								
SPAN-A	1'-11 1/2"	1'-10"	2'-9 1/2"	0'-9 1/2"	3'-4 1/2"	1'-3 1/4"	4'-0 3/8"	2'-3 1/2"
SPAN-D	10'	14'	14'-8 1/2"	3'-4"	2'-7 1/2"	11 1/2"	5'-5 3/4"	1'-8 1/2"



HAUNCH DETAIL

Note - A
Single Intermediate Transverse Stiffeners Shall Be Used On All Girders Of Spans B & C. Place Intermediate Stiffeners On Alternate Sides Of The Web Of Interior Girders And On The Inside Of The Web Of Exterior Girders.

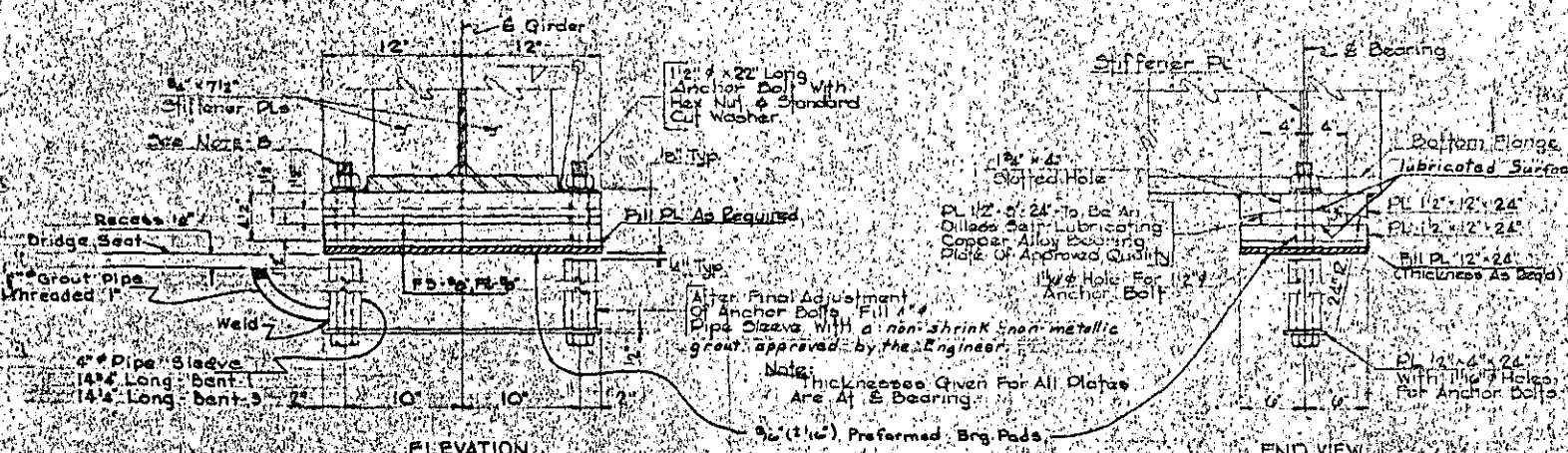
PROJECT NO. B 1161700
NASH COUNTY
STATION 27+99.43 62 1-95
27+34.05 S. R. 1527

STATE OF NORTH CAROLINA
STATE HIGHWAY COMMISSION
S. R. 1527 UNDERPASS
SUPERSTRUCTURE
DETAILS

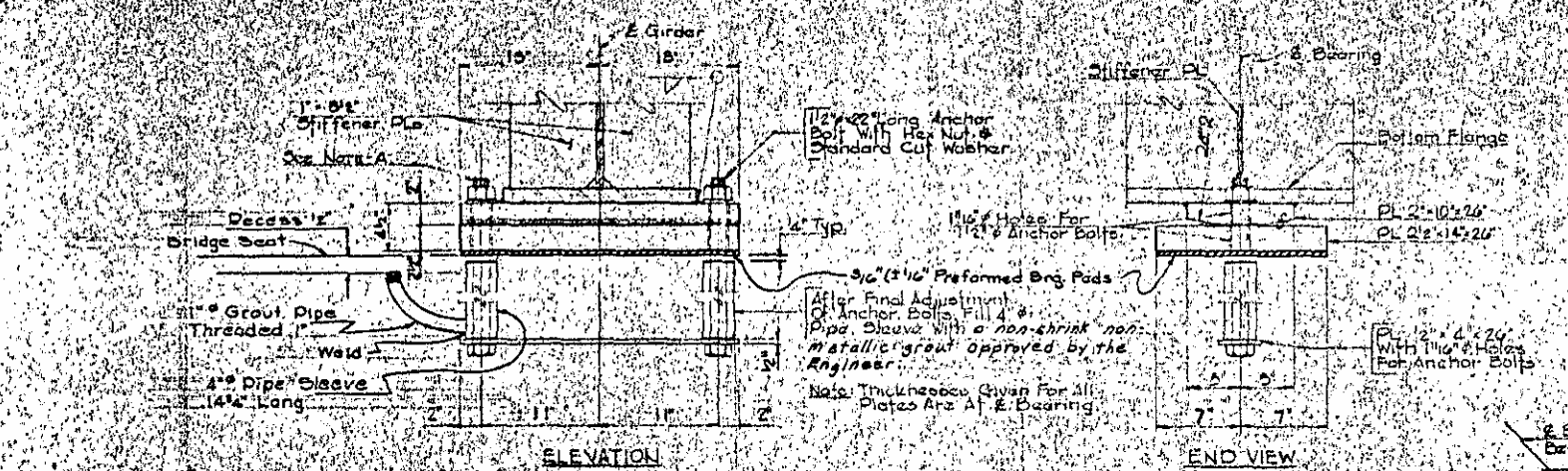
NO.	DATE	REVISIONS

3-41
76

RUMMEL, KLEPPER & KAHL
CONSULTING ENGINEERS
RALEIGH, NORTH CAROLINA

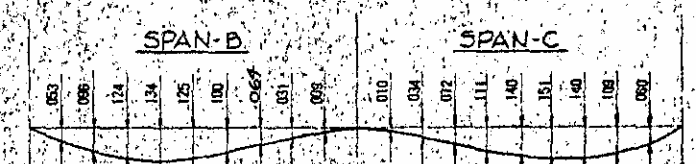


EXPANSION BEARING ASSEMBLY EB-3
NOTE B: At All Expansion Bearings, Thread Of The Nut And Bolt Shall Be Burred With A Sharp Pointed Tool.



FIXED BEARING ASSEMBLY FB-3
NOTE A: At All Fixed Bearings, Nuts For Anchor Bolts Are To Be Tightened Finger Tight And Then Bucked One (2) Turns. The Thread Of The Nut And Bolt Shall Then Be Burred With A Sharp Pointed Tool.

BEARING ASSEMBLIES REQUIRED			
MARK	LOCATION	NO REQ'D	
EB-3	BENT 1	5	
	BENT 3	5	
FB-3	BENT 2	5	
FILL PLATES REQUIRED			
MARK	REQ. ASS'Y	NO REQ'D	THICKNESS
F3	EB-3	1	3/8"
F4	EB-3	1	3/8"



DEFLECTIONS DUE TO POURS 1 THRU 3

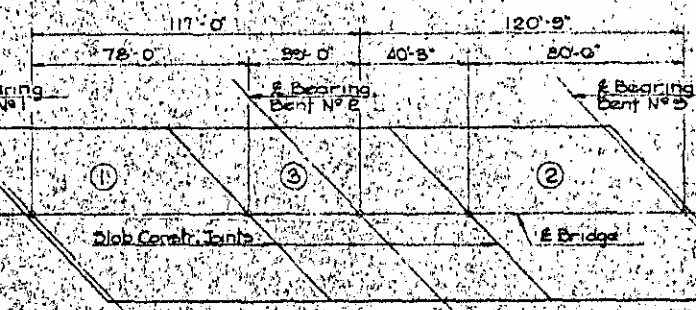


DEFLECTIONS DUE TO POURS 1 AND 2



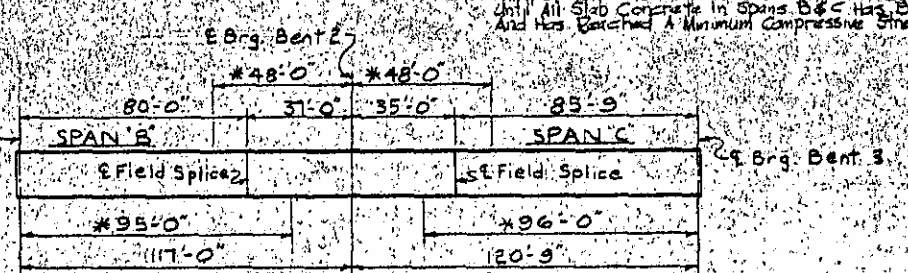
DEFLECTIONS DUE TO POUR 1

Note: All Deflection Ordinates Are In Feet And Are Given At The Tenth Points Of The Span Between Bearings For Interior Girders Only. Deflections Are For Weight Of Concrete Slab Only.



ROADWAY SLAB POURING SEQUENCE

Previously Cast Concrete Units In Spans B & C Shall Attain A Minimum Compressive Strength Of 3000 p.s.i. Before Additional Concrete Pours Are Made. Poured Concrete In Spans B & C Shall Not Be cast Until All Slab Concrete In Spans B & C Has Been Cast And Has Reached A Minimum Compressive Strength Of 3000 p.s.i.



* Charpy V-Notch Tests will be required for top or bottom flange plates, which falls within limits shown in Span B and Span C. Also Charpy V-Notch Tests will be required for all web plates, web splice plates, flange splice plates in Span B and Span C, bottom flange plates of the girders in Span A & Span D and all rolled beams in Span A and D. For Charpy V-Notch Tests, See Special Provisions.

NOTES

DAMBER: Girders Shall Be Fabricated With Camber To Compensate For The Deflection Caused By The Combined Weights Of The Structural Steel And The Superimposed Dead Loads And The Vertical Curve Ordinates. Cambered Girder Lengths Shall Be Adjusted And Bearings Are To Be Placed On The Cambered Girder So As To Be Aligned With The Anchors After Dead Load Deflection Has Occurred. Shop Plans Shall Be Prepared Accordingly.

FIELD CONNECTIONS: All Field Connections Not Welded Shall Be Made With 3/4" High Strength Bolts Unless Otherwise Noted.

SHOP SPLICES: All Shop Spllices In Flange And Web Plates Shall Be Made Prior To Welding Flange Plates To Web Plates. No Spllices Other Than Those Shown On The Plans Will Be Permitted In The Flange Plates. However, Additional Shop Web Spllices Will Be Allowed Within The Areas Shown In The Details. The Location Of These Spllices Shall Be Shown On The Shop Plans.

WELDING: All Welding Shall Conform To The Latest AWS Specification For Welded Highway And Railroad Bridges And The Special Provisions.

STUDS: For Description Of Studs See Special Provisions.

FIELD SPLICES: All Bolts In Field Spllices Are 3/4" High Strength Bolts. Spacing Of Studs On Top Flange Spllice Plate May Be Adjusted If Necessary To Clear Bolts. However, The Total Number Of Studs Required On Spllice Plate Shall Not Be Less Than That Required By Using Normal Spacing.

STRUCTURAL STEEL: All Structural Steel To Be ASTM A36.

STRUCTURAL STEEL ERECTION: Erection Of Structural Steel Shall Be Completed For Spans B & C Before False Work Or Form Work For Spans B & C Are Placed.

BEARING ASSEMBLIES: For Requirements Of Self-Lubricating Plate, See Special Provisions.

All Bearing Assemblies Except Self-Lubricating Plate Shall Be Galvanized.

Masonry Plates Shall Be Straight And Smooth On Both Sides. No Surface Finish Required.

At The Contractors Option, Fill Plates May Be Combined With Masonry Jts. For Location Of Bolts In Beams And Girders To Accommodate 'K' Bars, See Superstructure Sections & Details.

SHIPPING DETAILS: Shipping details for beams and girders shall be submitted for approval, indicating the top flange location during shipment, and in all cases showing the web vertical. The method of shipment, position of the vehicle, and attachments to the beams or girders or any shipping restraints shall be clearly detailed.

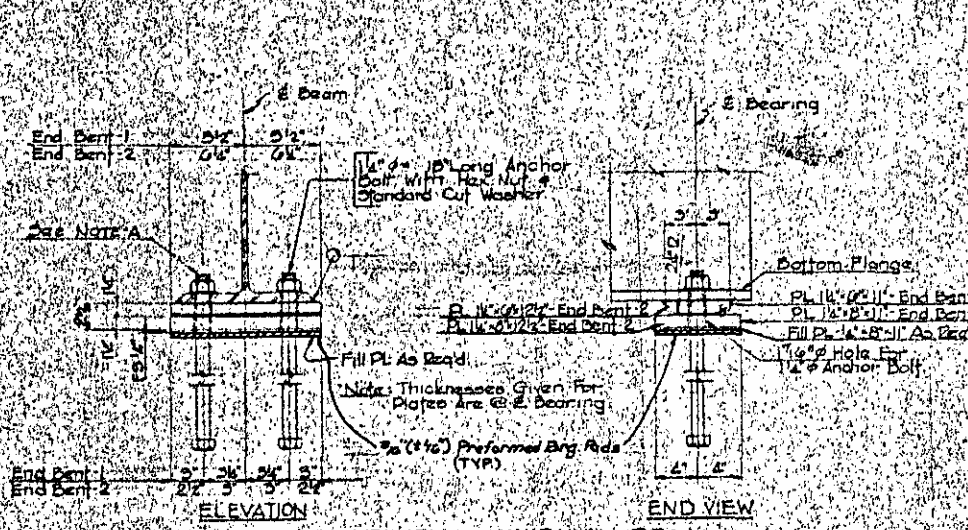
PROJECT NO. 8-1161706

HASH COUNTY

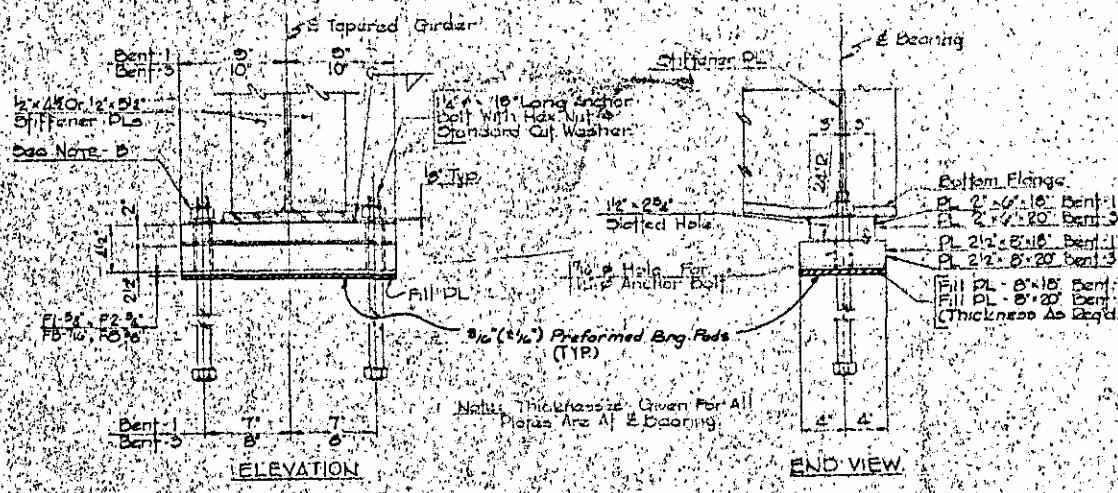
STATION 2298+43.62-1-55
22+34.05, S. R. 1577

STATE OF NORTH CAROLINA
STATE HIGHWAY COMMISSION
R-1527 UNDERPASS
SUPERSTRUCTURE
BEARING SHOES

RUMMEL, KLEPPER & KAHL,
CONSULTING ENGINEERS
RALEIGH, NORTH CAROLINA



FIXED BEARING ASSEMBLY - FB-1
 INTERIOR - ROLLED BEAMS - SPANS A & D



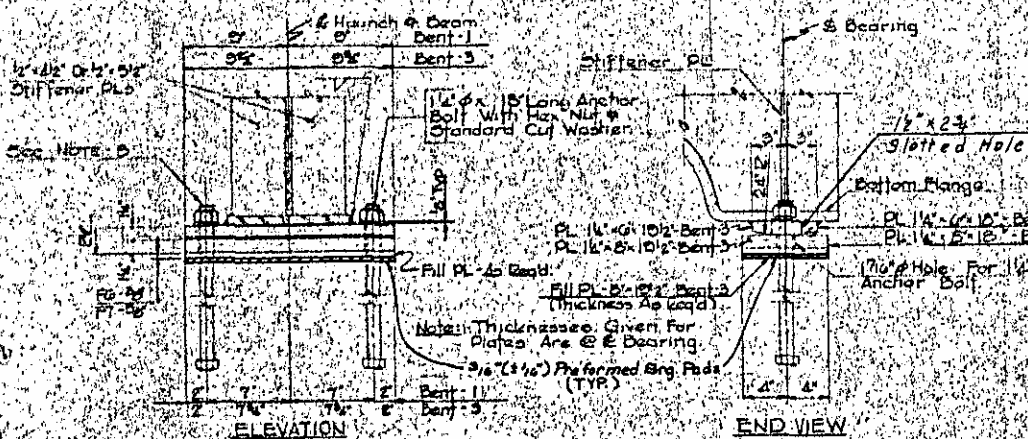
EXPANSION BEARING ASSEMBLY EB-2
 EXTERIOR - TAPERED GIRDER - SPANS A & D

Note A: At Fixed Bearings, Nuts for Anchor Bolts Are to be Tightened Finger Tight and then Backed Off 1/2 Turn. The Threads of the Nut and Bolt shall then be Burned With a Sharp Pointed Tool.
Note B: At Expansion Bearings, The Threads of the Nut and Bolt shall be Burned With a Sharp Pointed Tool.

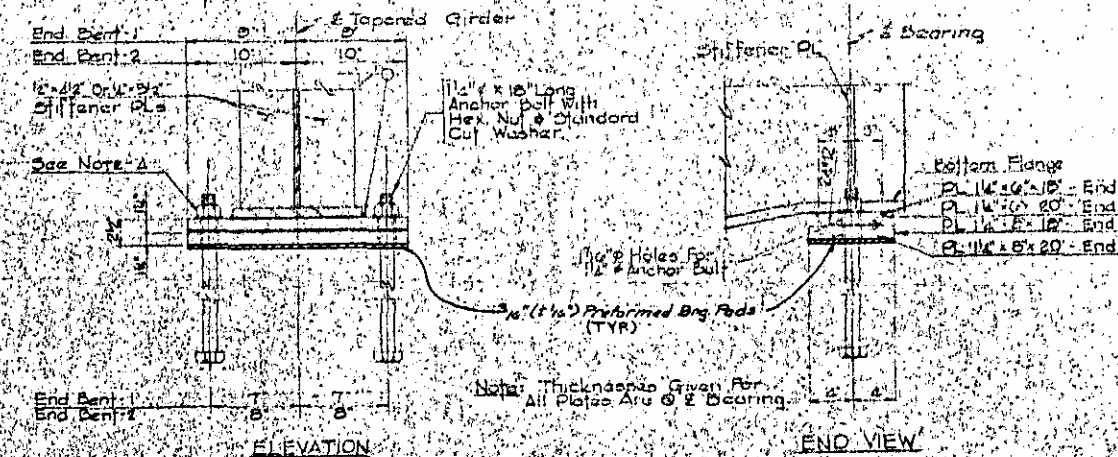
Note: In Spans A and D All Bearing Assemblies shall be Galvanized. Masonry Plates for Spans A and D To Be Straight and Smooth on Both Sides. No Surface Finish Required.

BEARING ASSEMBLIES REQUIRED		
MARUM	LOCATION	NO. REQ'D
BB-1	BENT-1	3
EB-1	BENT-5	3
EB-2	BENT-1	2
EB-2	BENT-5	2
FB-1	END BENT-1	3
FB-2	END BENT-1	2
FB-2	END BENT-2	2

FILL PLATES REQUIRED			
MARUM	BRG. ASSY	INTERD. THICKNESS	
F1	EB-2 Bent-1	1 1/2"	3"
F2	EB-2 Bent-1	1 1/2"	2"
F3	EB-2 Bent-3	1 1/2"	3"
F4	EB-1 Bent-3	1 1/2"	3"
F5	EB-1 Bent-5	1 1/2"	3"
F6	EB-2 Bent-5	1 1/2"	3"
F7	FB-1 End Bent-2	1 1/2"	1"



EXPANSION BEARING ASSEMBLY - EB-1
 INTERIOR - ROLLED BEAMS - SPANS A & D



FIXED BEARING ASSEMBLY FB-2
 EXTERIOR - TAPERED GIRDER - SPANS A & D

PROJECT NO. 8-11617

NASH COUNTY

STATION: 27+00.43 62 1-95
 27+34.05 S.R. 1527

STATE OF NORTH CAROLINA
 STATE HIGHWAY COMMISSION

S. R. 1527 UNDERPASS
 SUPERSTRUCTURE
 BEARING SHOES

RUMMEL, KLEPPER & KAHL
 CONSULTING ENGINEERS
 RALEIGH, NORTH CAROLINA

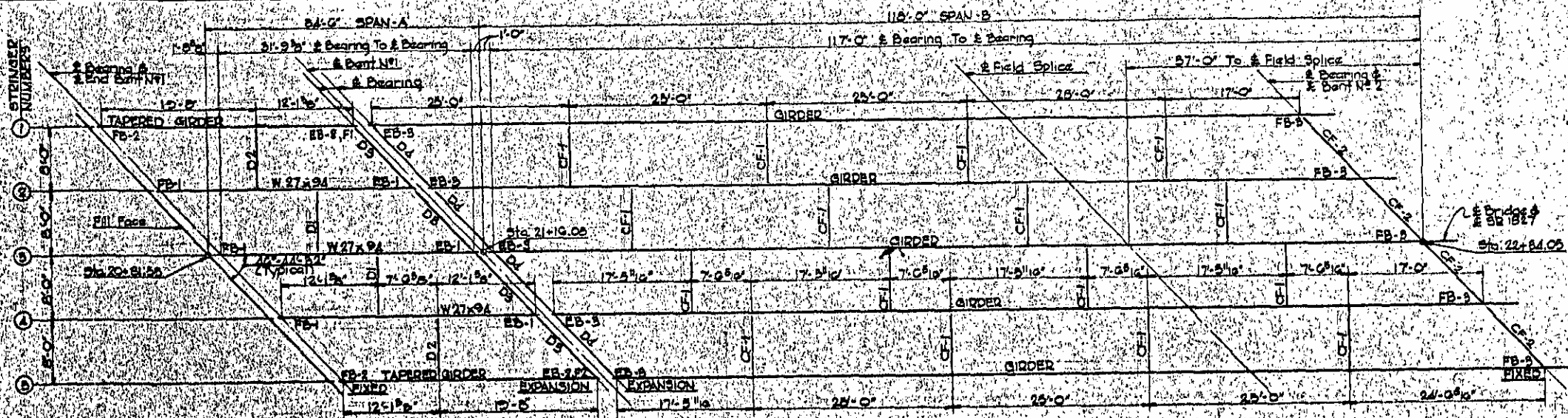
REV.	BY	DATE	DESCRIPTION

Revised 13.12.2011

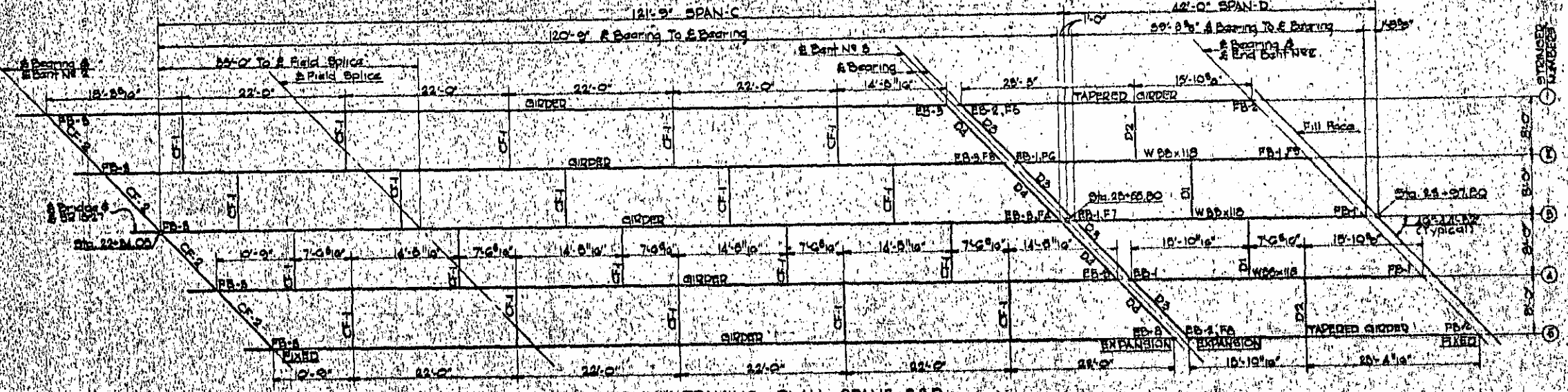
BILL OF MATERIAL

4. N.C. 8.1161206
 5(97) 39

NUMBER			SIZE	TYPE	LENGTH	WEIGHT			BAR	NUMBER			SIZE	TYPE	LENGTH	WEIGHT			BAR	NUMBER			SIZE	TYPE	LENGTH	WEIGHT		
SPAN-A	SPAN-B&C	SPAN-D				SPAN-A	SPAN-B&C	SPAN-D		SPAN-A	SPAN-B&C	SPAN-D				SPAN-A	SPAN-B&C	SPAN-D		SPAN-A	SPAN-B&C	SPAN-D				SPAN-A	SPAN-B&C	SPAN-D
1	412	10	#5	①	10'-2"	41	17,250	670	a01	2	2	2	#5	②	0'-0"	12	12	12	a11	2	2	2	#5	Str.	9'-9"	20	20	20
2	411	15	#5	①	37'-8"	41	19,147	889	a02	2	2	2	#5	②	0'-0"	12	12	12	a12	2	2	2	#5	Str.	9'-2"	20	20	20
3	410	20	#5	①	57'-4"	78	78	78	a03	2	2	2	#5	②	0'-0"	12	12	12	a13	2	2	2	#5	Str.	8'-8"	18	18	18
4	409	25	#5	①	50'-4"	76	76	76	a04	2	2	2	#5	②	0'-0"	10	10	10	a14	2	2	2	#5	Str.	8'-2"	16	16	16
5	408	30	#5	①	58'-8"	74	74	74	a05	2	2	2	#5	②	0'-0"	10	10	10	a15	2	2	2	#5	Str.	7'-7"	14	14	14
6	407	35	#5	①	58'-8"	74	74	74	a06	2	2	2	#5	②	0'-0"	8	8	8	a16	2	2	2	#5	Str.	6'-7"	14	14	14
7	406	40	#5	①	58'-8"	74	74	74	a07	2	2	2	#5	②	0'-0"	8	8	8	a17	2	2	2	#5	Str.	6'-0"	12	12	12
8	405	45	#5	①	58'-8"	72	72	72	a08	2	2	2	#5	②	0'-0"	8	8	8	a18	2	2	2	#5	Str.	5'-6"	12	12	12
9	404	50	#5	①	58'-8"	70	70	70	a09	2	2	2	#5	②	0'-0"	8	8	8	a19	2	2	2	#5	Str.	4'-11"	10	10	10
10	403	55	#5	①	58'-8"	70	70	70	a10	2	2	2	#5	②	0'-0"	8	8	8	a20	2	2	2	#5	Str.	4'-5"	10	10	10
11	402	60	#5	①	58'-8"	68	68	68	a11	2	2	2	#5	②	0'-0"	8	8	8	a21	2	2	2	#5	Str.	3'-11"	8	8	8
12	401	65	#5	①	58'-8"	66	66	66	a12	2	2	2	#5	②	0'-0"	8	8	8	a22	2	2	2	#5	Str.	3'-5"	8	8	8
13	400	70	#5	①	58'-8"	66	66	66	a13	2	2	2	#5	②	0'-0"	8	8	8	a23	2	2	2	#5	Str.	2'-10"	6	6	6
14	399	75	#5	①	58'-8"	64	64	64	a14	2	2	2	#5	②	0'-0"	8	8	8	a24	2	2	2	#5	Str.	2'-4"	4	4	4
15	398	80	#5	①	58'-8"	64	64	64	a15	2	2	2	#5	②	0'-0"	8	8	8	a25	2	2	2	#5	Str.	1'-9"	4	4	4
16	397	85	#5	①	58'-8"	64	64	64	a16	2	2	2	#5	②	0'-0"	8	8	8	a26	2	2	2	#5	Str.	1'-9"	4	4	4
17	396	90	#5	①	58'-8"	62	62	62	a17	2	2	2	#5	②	0'-0"	8	8	8	a27	2	2	2	#5	Str.	1'-9"	4	4	4
18	395	95	#5	①	58'-8"	62	62	62	a18	2	2	2	#5	②	0'-0"	8	8	8	a28	2	2	2	#5	Str.	1'-9"	4	4	4
19	394	100	#5	①	58'-8"	60	60	60	a19	2	2	2	#5	②	0'-0"	8	8	8	a29	2	2	2	#5	Str.	1'-9"	4	4	4
20	393	105	#5	①	58'-8"	60	60	60	a20	2	2	2	#5	②	0'-0"	8	8	8	a30	2	2	2	#5	Str.	1'-9"	4	4	4
21	392	110	#5	①	58'-8"	58	58	58	a21	2	2	2	#5	②	0'-0"	8	8	8	a31	2	2	2	#5	Str.	1'-9"	4	4	4
22	391	115	#5	①	58'-8"	58	58	58	a22	2	2	2	#5	②	0'-0"	8	8	8	a32	2	2	2	#5	Str.	1'-9"	4	4	4
23	390	120	#5	①	58'-8"	56	56	56	a23	2	2	2	#5	②	0'-0"	8	8	8	a33	2	2	2	#5	Str.	1'-9"	4	4	4
24	389	125	#5	①	58'-8"	56	56	56	a24	2	2	2	#5	②	0'-0"	8	8	8	a34	2	2	2	#5	Str.	1'-9"	4	4	4
25	388	130	#5	①	58'-8"	54	54	54	a25	2	2	2	#5	②	0'-0"	8	8	8	a35	2	2	2	#5	Str.	1'-9"	4	4	4
26	387	135	#5	①	58'-8"	54	54	54	a26	2	2	2	#5	②	0'-0"	8	8	8	a36	2	2	2	#5	Str.	1'-9"	4	4	4
27	386	140	#5	①	58'-8"	52	52	52	a27	2	2	2	#5	②	0'-0"	8	8	8	a37	2	2	2	#5	Str.	1'-9"	4	4	4
28	385	145	#5	①	58'-8"	52	52	52	a28	2	2	2	#5	②	0'-0"	8	8	8	a38	2	2	2	#5	Str.	1'-9"	4	4	4
29	384	150	#5	①	58'-8"	50	50	50	a29	2	2	2	#5	②	0'-0"	8	8	8	a39	2	2	2	#5	Str.	1'-9"	4	4	4
30	383	155	#5	①	58'-8"	50	50	50	a30	2	2	2	#5	②	0'-0"	8	8	8	a40	2	2	2	#5	Str.	1'-9"	4	4	4
31	382	160	#5	①	58'-8"	48	48	48	a31	2	2	2	#5	②	0'-0"	8	8	8	a41	2	2	2	#5	Str.	1'-9"	4	4	4
32	381	165	#5	①	58'-8"	48	48	48	a32	2	2	2	#5	②	0'-0"	8	8	8	a42	2	2	2	#5	Str.	1'-9"	4	4	4
33	380	170	#5	①	58'-8"	46	46	46	a33	2	2	2	#5	②	0'-0"	8	8	8	a43	2	2	2	#5	Str.	1'-9"	4	4	4
34	379	175	#5	①	58'-8"	46	46	46	a34	2	2	2	#5	②	0'-0"	8	8	8	a44	2	2	2	#5	Str.	1'-9"	4	4	4
35	378	180	#5	①	58'-8"	44	44	44	a35	2	2	2	#5	②	0'-0"	8	8	8	a45	2	2	2	#5	Str.	1'-9"	4	4	4
36	377	185	#5	①	58'-8"	44	44	44	a36	2	2	2	#5	②	0'-0"	8	8	8	a46	2	2	2	#5	Str.	1'-9"	4	4	4
37	376	190	#5	①	58'-8"	42	42	42	a37	2	2	2	#5	②	0'-0"	8	8	8	a47	2	2	2	#5	Str.	1'-9"	4	4	4
38	375	195	#5	①	58'-8"	42	42	42	a38	2	2	2	#5	②	0'-0"	8	8	8	a48	2	2	2	#5	Str.	1'-9"	4	4	4
39	374	200	#5	①	58'-8"	40	40	40	a39	2	2	2	#5	②	0'-0"	8	8	8	a49	2	2	2	#5	Str.	1'-9"	4	4	4
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41	372	210	#5	①	58'-8"	38	38	38	a41	2	2	2	#5	②	0'-0"	8	8	8	a51	2	2	2	#5	Str.	1'-9"	4	4	4
42	371	215	#5	①	58'-8"	38	38	38	a42	2	2	2	#5	②	0'-0"	8	8	8	a52	2	2	2	#5	Str.	1'-9"	4	4	4
43	370	220	#5	①	58'-8"	36	36	36	a43	2	2	2	#5	②	0'-0"	8	8	8	a53	2	2	2	#5	Str.	1'-9"	4	4	4
44	369	225	#5	①	58'-8"	36	36	36	a44	2	2	2	#5	②	0'-0"	8	8	8	a54	2	2	2	#5	Str.	1'-9"	4	4	4
45	368	230	#5	①	58'-8"	34	34	34	a45	2	2	2	#5	②	0'-0"	8	8	8	a55	2	2	2	#5	Str.	1'-9"	4	4	4
46	367	235	#5	①	58'-8"	34	34	34	a46	2	2	2	#5	②	0'-0"	8	8	8	a56	2	2	2	#5	Str.	1'-9"	4	4	4
47	366	240	#5	①	58'-8"	32	32	32	a47	2	2	2	#5	②	0'-0"	8	8	8	a57	2	2	2	#5	Str.	1'-9"	4	4	4
48	365	245	#5	①	58'-8"	32	32	32	a48	2	2	2	#5	②	0'-0"	8	8	8	a58	2	2	2	#5	Str.	1'-9"	4	4	4
49	364	250	#5	①	58'-8"	30	30	30	a49	2	2	2	#5	②	0'-0"	8	8	8	a59	2	2	2	#5	Str.	1'-9"	4	4	4
50	363	255	#5	①	58'-8"	30	30	30	a50	2	2	2	#5	②	0'-0"	8	8	8	a60	2	2	2	#5	Str.	1'-9"	4	4	4
51	362	260	#5	①	58'-8"	28	28	28	a51	2	2	2	#5	②	0'-0"	8	8	8	a61	2	2	2	#5	Str.	1'-9"	4	4	4
52	361	265	#5	①	58'-8"	28	28	28	a52	2	2																	



FRAMING PLAN - SPANS A & B



FRAMING PLAN - SPANS C & D

PROJECT NO. 8-1161706
 NASH COUNTY
 STATION: 23+94.05 S. R. 1527

DEAD LOAD DEFLECTION AND CAMBER SCHEDULE

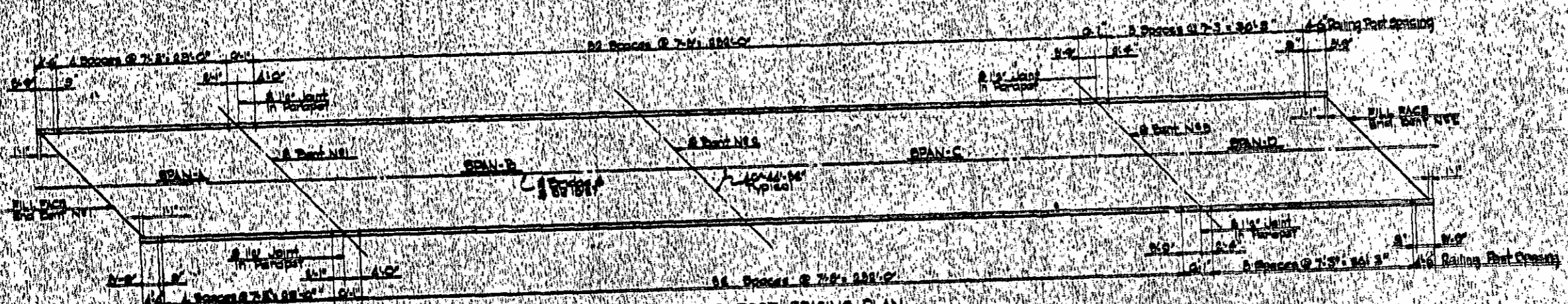
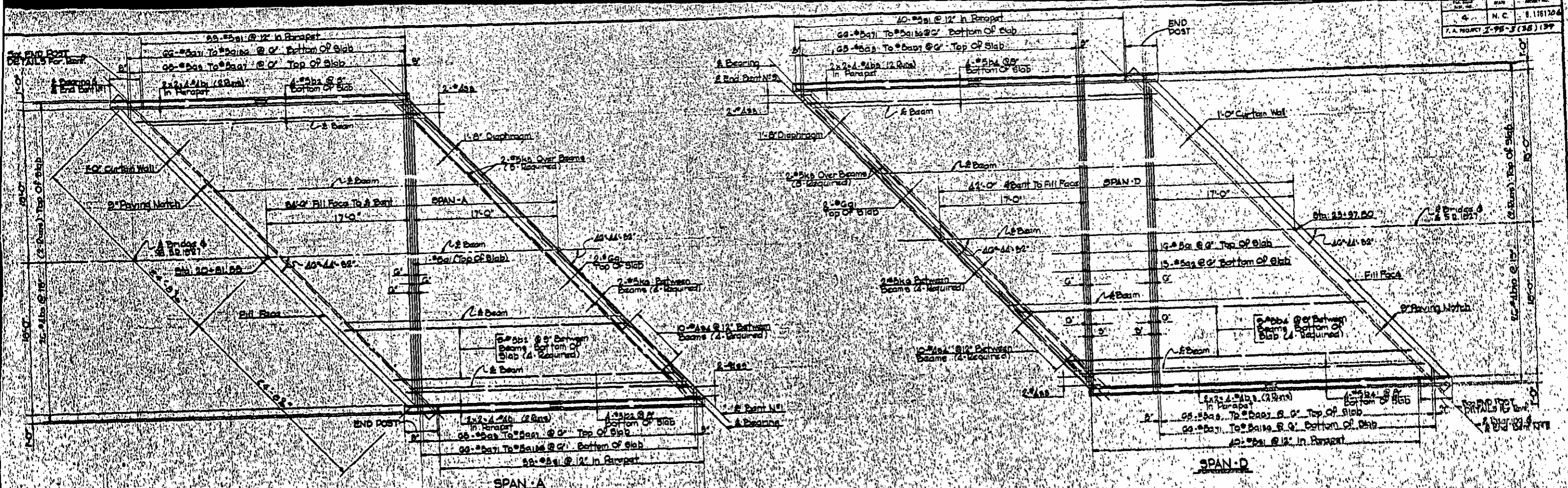
	SPAN A		SPAN B												SPAN C												SPAN D															
	MID POINT		1/10 SPAN		2/10 SPAN		3/10 SPAN		4/10 SPAN		5/10 SPAN		6/10 SPAN		7/10 SPAN		8/10 SPAN		9/10 SPAN		10/10 SPAN		11/10 SPAN		12/10 SPAN		13/10 SPAN		14/10 SPAN		MID POINT											
	INT.	EXT.	INT.	EXT.	INT.	EXT.	INT.	EXT.	INT.	EXT.	INT.	EXT.	INT.	EXT.	INT.	EXT.	INT.	EXT.	INT.	EXT.	INT.	EXT.	INT.	EXT.	INT.	EXT.	INT.	EXT.	INT.	EXT.												
DEFLECTION DUE TO WT. OF STEEL	002	001	014	014	026	026	033	033	038	038	033	033	026	026	016	016	008	008	002	002	000	000	003	003	011	011	022	022	034	034	042	042	045	045	041	041	032	032	018	018	003	002
DEFLECTION DUE TO WT. OF SLAB	018	013	060	049	082	069	118	114	128	121	118	112	091	088	058	058	027	026	007	006	000	000	012	011	038	037	078	078	118	114	148	141	167	161	148	140	112	109	082	059	023	020
DEFLECTION DUE TO WT. OF COMPOSITE LOADS	004	003	010	010	018	018	023	024	026	027	025	026	022	022	016	016	009	009	003	003	000	000	004	004	011	011	018	020	026	027	030	031	031	032	028	028	021	022	011	012	008	005
TOTAL DEAD LOAD DEFLECTION	024	017	074	073	126	133	174	171	188	184	174	171	139	136	080	080	044	043	012	011	000	000	019	018	080	079	119	117	178	176	216	214	239	226	214	208	165	163	091	089	032	027
VERTICAL CURVE ORDINATE	010	010	072	072	128	128	188	188	192	188	188	188	171	169	108	108	128	128	072	072	000	000	077	077	136	136	178	178	204	204	213	213	204	204	178	178	138	136	077	077	023	023
REQUIRED BEAM CAMBER	030	032	148	145	264	261	342	339	360	376	374	371	331	328	258	258	172	171	084	083	000	000	066	066	198	195	288	288	362	370	431	427	437	432	383	388	301	299	168	168	055	060

All Deflections Are in Feet

RUMMEL, KLEPPER & KAHL
 CONSULTING ENGINEERS
 RALEIGH, NORTH CAROLINA

REVISIONS					
NO.	BY	DATE	NO.	BY	DATE

Sheet No. 5-46
 Total 76

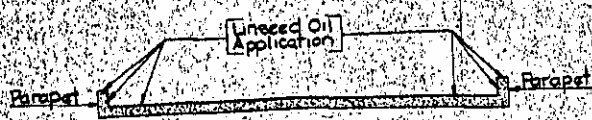
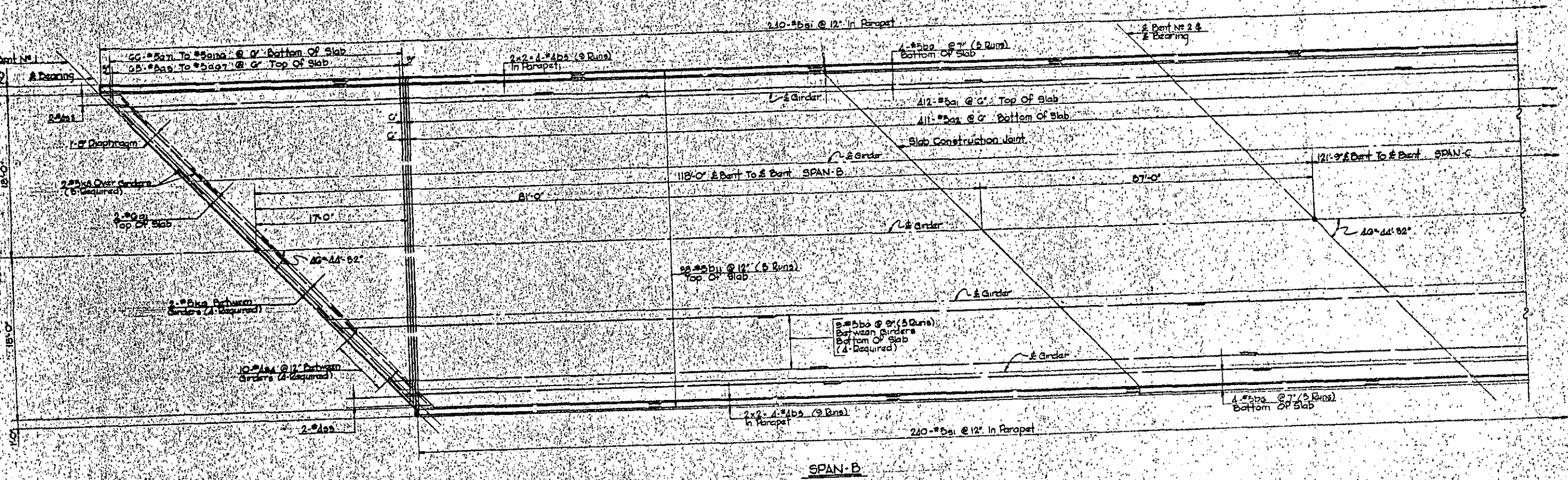


RAILING POST SPACING PLAN
 RAILING POST SPACING PLAN

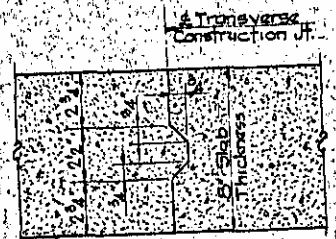
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 N.C. COUNTY
 STATION: 29+97.00 - 30+01.00

RUMMEL, KLEPPER & KAHL
 CONSULTING ENGINEERS
 RALEIGH, NORTH CAROLINA

STATE OF NORTH CAROLINA STATE HIGHWAY COMMISSION					
SUPERSTRUCTURE					
CHECK PLAN SPANS A & D					
REVISIONS					
NO.	BY	DATE	NO.	BY	DATE
1					
2					



SKETCH SHOWING LIMIT OF LINSEED OIL APPLICATION



NOTE: Reinforcing Steel Not Shown. Longitudinal And Transverse Reinforcing Steel Shall Be Continuous Thru Joint.

TRANSVERSE CONSTRUCTION JOINT IN DECK SLAB

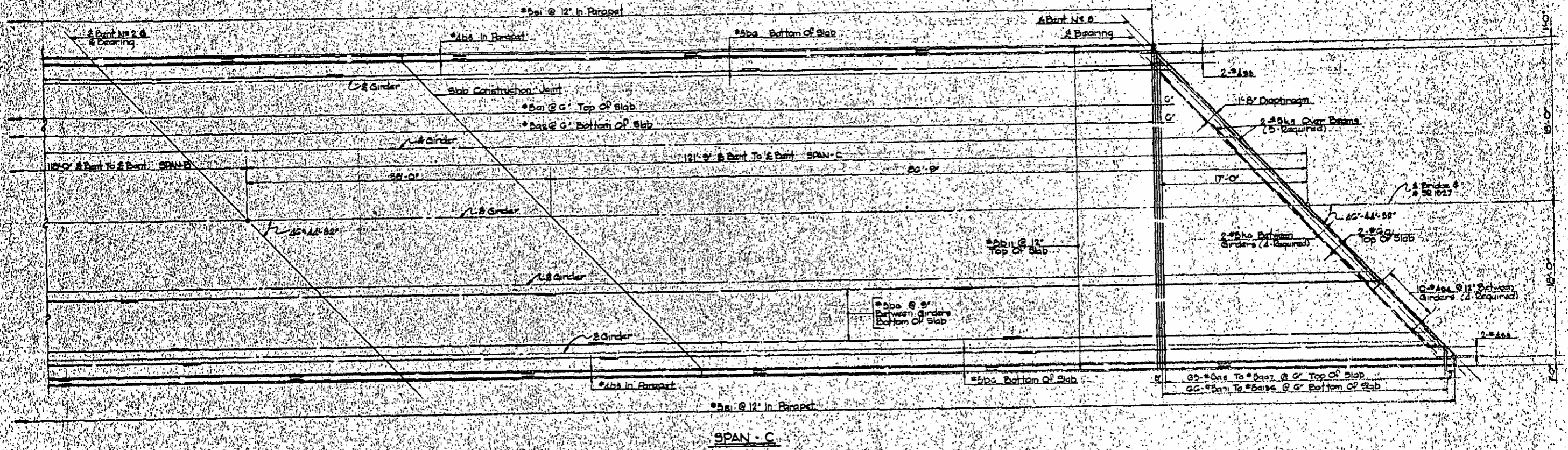
PROJECT NO. 8 11617
 NASH COUNTY
 STATION: 2349+43.62 I-95
 22+34.05 S.R. 1527

STATE OF NORTH CAROLINA
STATE HIGHWAY COMMISSION
 RALEIGH
 S. R. 1527 UNDERPASS
 SUPERSTRUCTURE
 DECK PLAN SPAN B

RUMMEL, KLEPPER & KAHL
 CONSULTING ENGINEERS
 RALEIGH, NORTH CAROLINA

REVISIONS					
NO.	BY	DATE	NO.	BY	DATE
1			2		
2			3		
			4		

TOTAL SHEETS: 5-49
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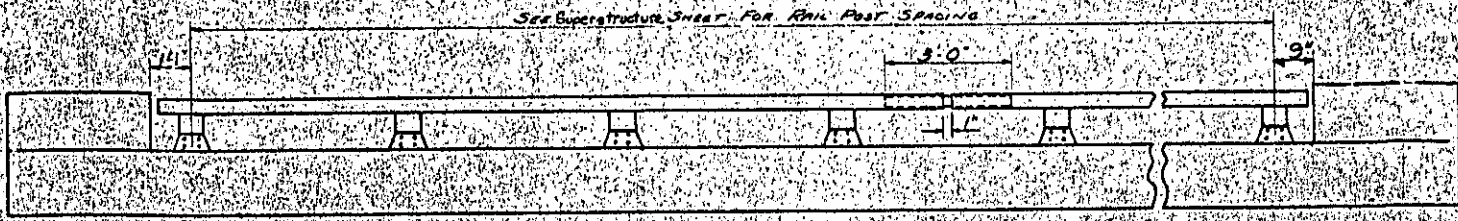
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 NASH COUNTY
 STATION: 2349+43.62 1-05
 22+34.05 S. R. 1527

STATE OF NORTH CAROLINA
STATE HIGHWAY COMMISSION
 S. R. 1527 UNDERPASS
 SUPERSTRUCTURE
 DECK PLAN SPAN C

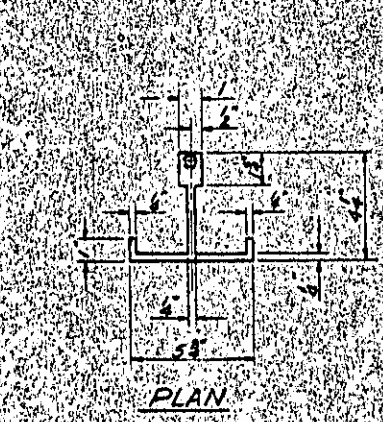
RUMMEL, KLEPPER & KAHL
 CONSULTING ENGINEERS
 RALEIGH, NORTH CAROLINA

REVISIONS					
NO.	BY	DATE	NO.	BY	DATE
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2			2		

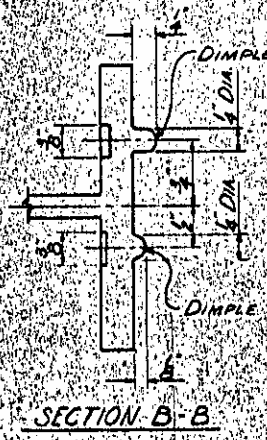
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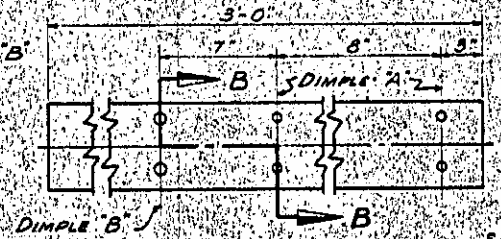
ELEVATION



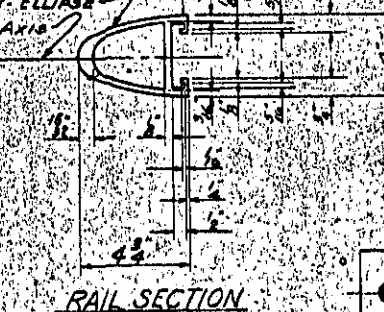
PLAN



SECTION B-B



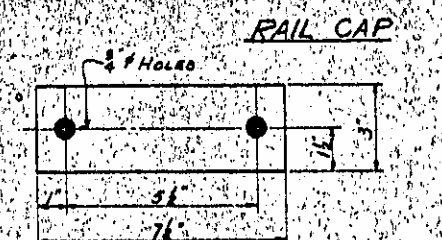
BAR SECTION
EXPANSION BAR DETAILS



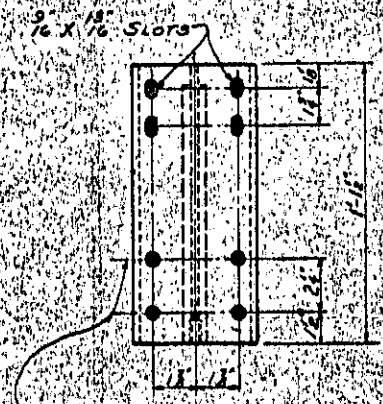
RAIL SECTION



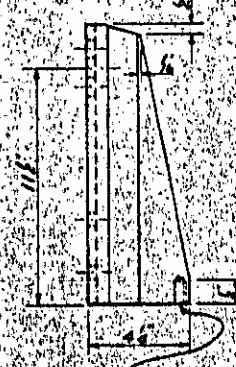
SECTION THRU PARAPET & RAIL



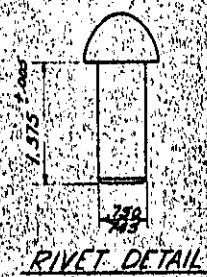
RAIL CAP



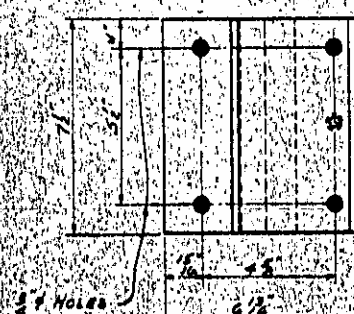
FRONT ELEVATION



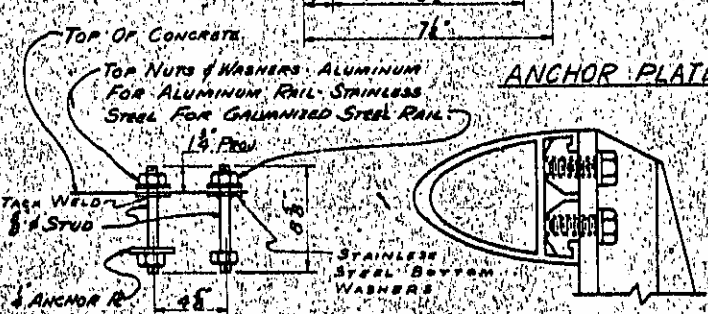
SIDE ELEVATION



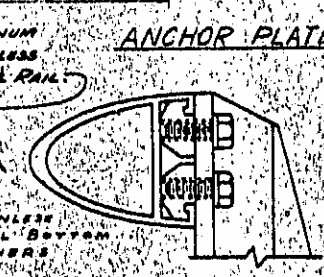
RIVET DETAIL



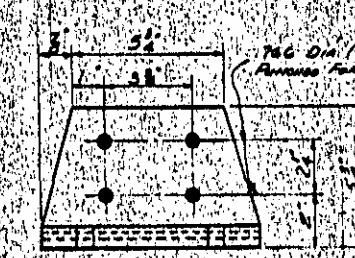
PLAN



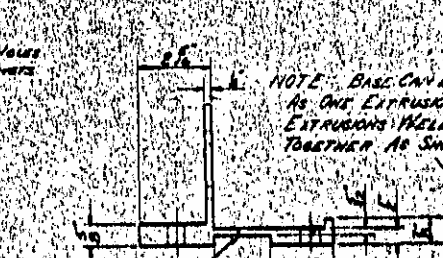
ANCHOR ASSEMBLY



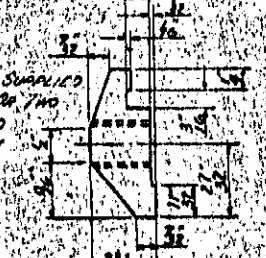
CLAMP & RAIL ASSEMBLY



FRONT ELEVATION



SIDE ELEVATION



CLAMP BAR DETAIL
(2 REQUIRED PER POST)

AT THE CONTRACTOR'S OPTION METAL RAIL MAY BE EITHER ALUMINUM OR GALVANIZED STEEL IN ACCORDANCE WITH THE REQUIREMENTS OF THE GENERAL NOTES AND THE FOLLOWING SPECIFICATIONS FOR THE ALTERNATE MATERIALS; HOWEVER THE CONTRACTOR WILL BE REQUIRED TO USE THE SAME RAIL MATERIAL ON ALL STRUCTURES ON THE PROJECT FOR WHICH METAL RAIL IS DESIGNATED.

ALUMINUM RAILS

MATERIAL FOR POSTS, BASES, (RAILS), EXPANSION BARS, (CLAMP BARS) SHALL BE A.S.T.M. B-221 ALLOY COING.

MATERIAL FOR ALUMINUM WASHER SHALL BE A.S.T.M. B-203 ALLOY ALCLAD 2024-T3.

MATERIAL FOR RIVETS SHALL BE A.S.T.M. B-916 ALLOY COING.

RIVETS SHALL BE 5/16" BUTT HEAD & CONE POINT COLD DRIVEN AS PER DRAWING.

MATERIAL FOR ALUMINUM NUTS SHALL BE A.S.T.M. B-211 ALLOY COING-YE. THE BASE OF RAIL POSTS, OR ANY OTHER ALUMINUM SURFACE IN CONTACT WITH CONCRETE SHALL BE THOROUGHLY COATED WITH AN ALUMINUM IMPREGNATED PAINTING COMPOUND OF APPROVED QUALITY. MATERIAL FOR SHIMS TO BE A.S.T.M. B-209 ALLOY COING-YE.

GENERAL NOTES

1. RAILING SHALL BE CONTINUOUS FROM END POST TO END POST OF BRIDGE. EACH JOINT IN RAIL LENGTH SHALL BE SPICED AS DETAILED. PANEL LENGTHS OF RAIL SHALL BE ATTACHED TO A MINIMUM OF THREE POSTS.
2. END OF RAIL TO CLEAR FACE OF CONCRETE END POST BY 1/2".
3. MATERIAL FOR ANCHOR STUDS SHALL BE TYPE 430 STAINLESS STEEL WITH MINIMUM 70,000 P.S.I. ULTIMATE STRENGTH.
4. CERTIFIED MILL REPORTS ARE REQUIRED FOR RAILS & POSTS. SHOP INSPECTION IS NOT REQUIRED.
5. METAL RAIL POSTS TO BE SET NORMAL TO CURVE BARS.
6. METHOD OF MEASUREMENT FOR METAL RAIL AND LENGTH OF METAL RAILS TO BE PER 1978 STANDARD SPECIFICATIONS, SUB ARTICLE 460-4 (R).
7. CURVED RAIL USABR. VERTICAL RAILS ARE TO BE USED ON BRIDGES ON HORIZONTAL AND/OR VERTICAL CURVATURE. THE CONTRACTOR SHALL AT HIS OPTION HAVE THE RAILS ADJUSTED IN THE SHOP OR IN THE FIELD. IN EITHER EVENT THE RAILS SHALL CONFORM WITHOUT BUCKLING OR KINKING TO THE REQUIRED CURVATURE IN A MANNER ACCEPTABLE TO THE ENGINEER.
8. TO insure future identification of the fabricator, a permanent identifying mark shall be placed on each post. The method of marking and location shall be such that it does not detract from the appearance of the post.
9. SHIMS TO BE USED AS NECESSARY FOR POST ALIGNMENT.
10. ALLOY 6061-T6 may be substituted for alloy 6061-T6 where applicable.

GALVANIZED STEEL RAILS

MATERIALS AND GALVANIZING ARE TO CONFORM TO THE FOLLOWING SPECIFICATIONS.

POSTS, POST BASES, RAILS, EXPANSION BARS AND CLAMP BARS: A.S.T.M. A36 GRADE STRUCTURAL STEEL - GALVANIZED TO A.S.T.M. A-123.

RIVETS: RIVETS SHALL MEET THE REQUIREMENTS OF A.S.T.M. A602 FOR GRADE 1 RIVETS.

NUTS & WASHERS FOR TOP END OF ANCHOR ASSEMBLY FOR STEEL RAIL SHALL BE TYPE 430 STAINLESS STEEL.

THE CUT ENDS OF GALVANIZED STEEL RAILING, AFTER GALVANIZING, SHALL BE GIVEN TWO COATS OF ZINC RICH PAINT MEETING THE REQUIREMENTS OF FEDERAL SPECIFICATION MIL-P-26913 USAF TYPE I OR OF FEDERAL SPECIFICATION TT-P-641.

SHIMS: SHIMS SHALL MEET THE REQUIREMENTS OF ASTM A570 FOR GRADE C OR D61 FOR GRADE C, AND SHALL BE GALVANIZED IN ACCORDANCE WITH ASTM A123.

CLOSURE BARS: CLOSURE BARS SHALL MEET THE REQUIREMENTS OF ASTM A246 GRADE C AND SHALL BE GALVANIZED IN ACCORDANCE WITH ASTM A123.

PAV LENGTH: 621.89

PROJECT NO. 8.181700

NASH COUNTY
 2349-43.82 I-95

STATION: 22+74.05 P.R. 1527

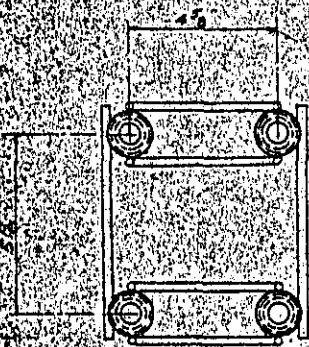
NOTE: MINOR VARIATIONS IN DETAILS OF METAL RAIL WILL BE CONSIDERED. DETAILS OF SUCH VARIATIONS, IF DESIRED, SHALL BE SUBMITTED FOR APPROVAL.

STATE OF NORTH CAROLINA
 STATE HIGHWAY COMMISSION
 STANDARD
 1 BAR
 METAL RAIL

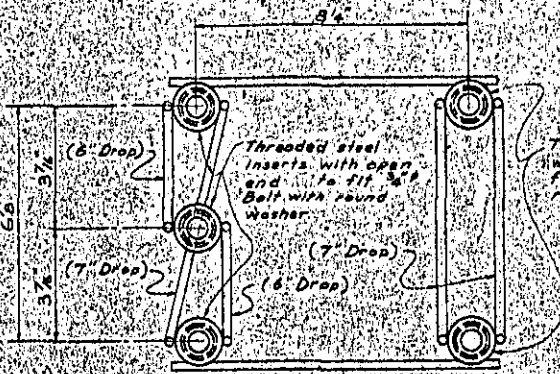
JULY 1968

GENERAL NOTES

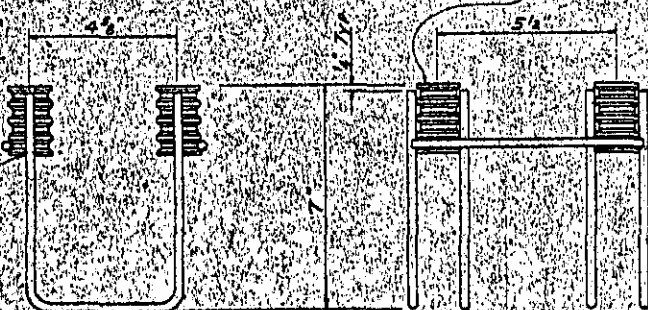
This Preset Anchor Assembly may be used in lieu of the anchorage shown on the Standard Metal Rail Sheet. The cost of the Preset Anchor Assembly with bolts and washers complete in place shall be included in the price bid for Lin. Ft. Metal Rail. The wire gauge and threaded steel inserts to be of sufficient strength to insure load anchoring capacity as specified in the AASHTO Specifications. The Preset Anchor Units to be hot dipped galvanized to conform to requirements of A.S.T.M. A 123. Anchor Bolts to be either high tensile steel conforming to A.S.T.M. A 442 and galvanized to conform to A.S.T.M. 153 or stainless steel Type 430 with a minimum 70,000 p.s.i. ultimate strength. Bolts to be tightened one-half turn with the wrench from a finger-tight position.



PLAN

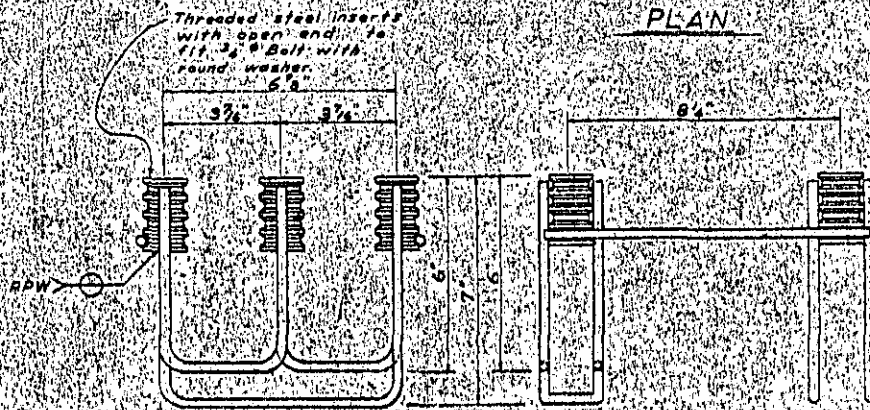


PLAN



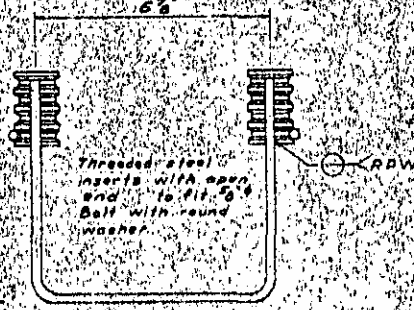
ELEVATION

SIDE VIEW



LEFT SIDE VIEW

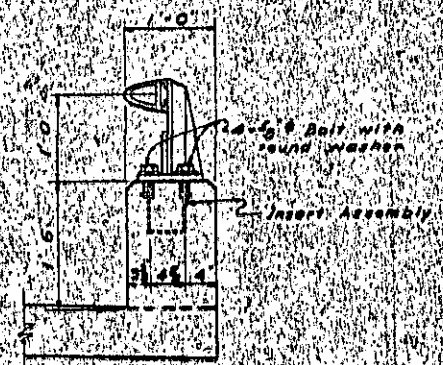
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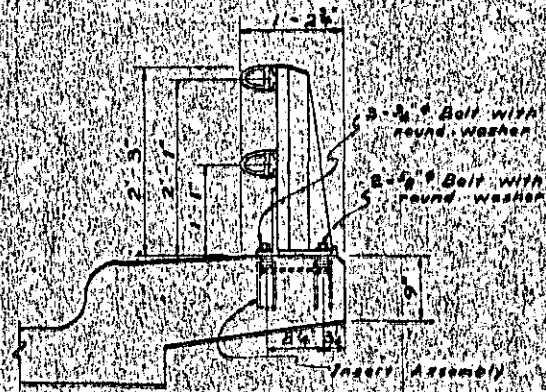
RIGHT SIDE VIEW

4-BOLT PRESET ANCHOR
FOR 1-BAR METAL RAIL

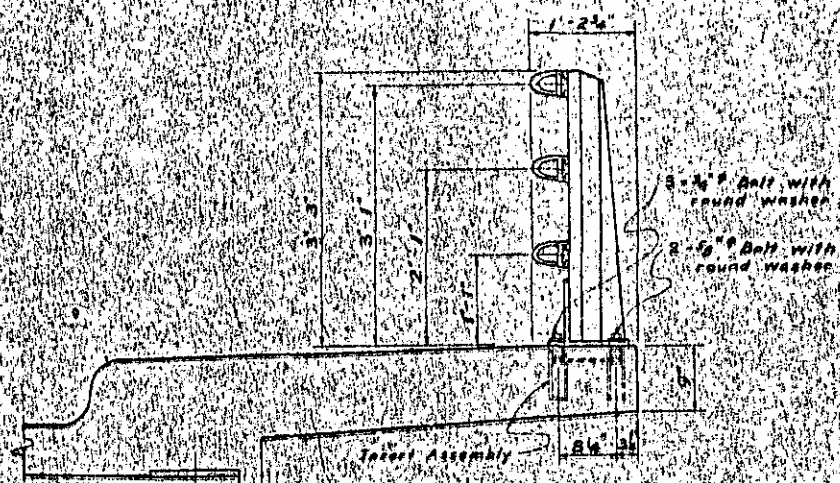
5-BOLT PRESET ANCHOR
FOR 2 OR 3-BAR METAL RAIL



SECTION THRU PARAPET & RAIL



SECTION THRU CURB & RAIL



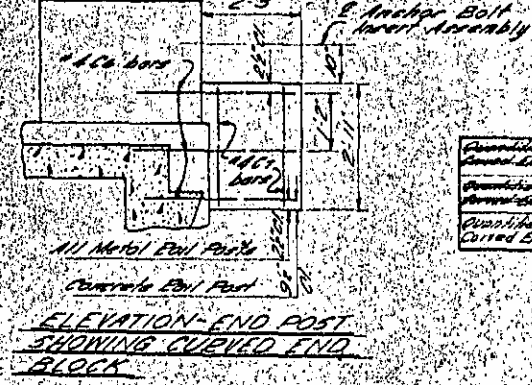
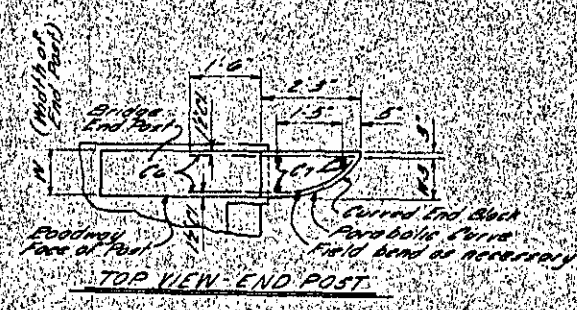
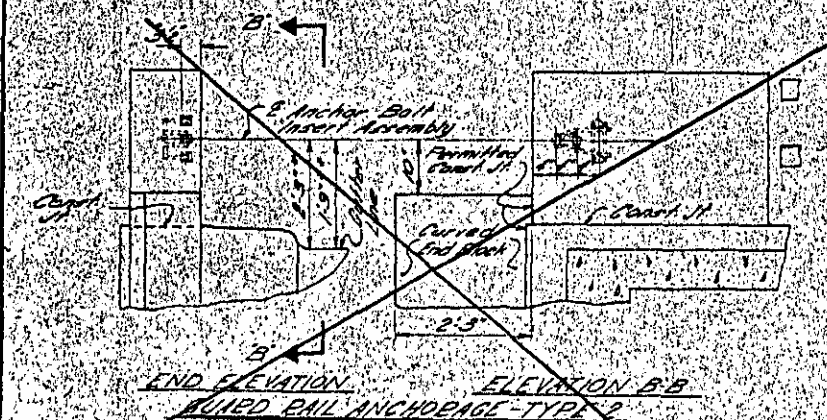
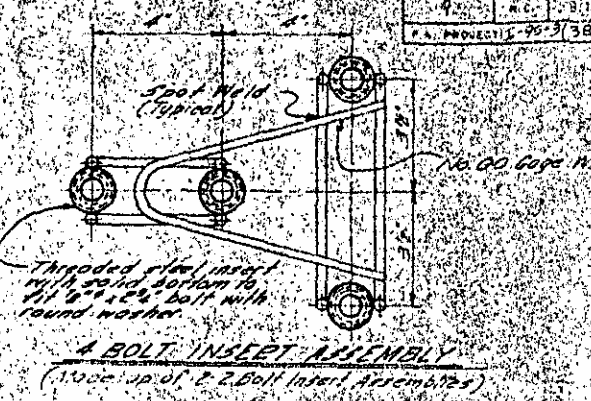
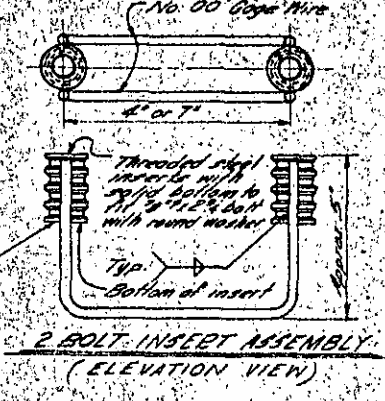
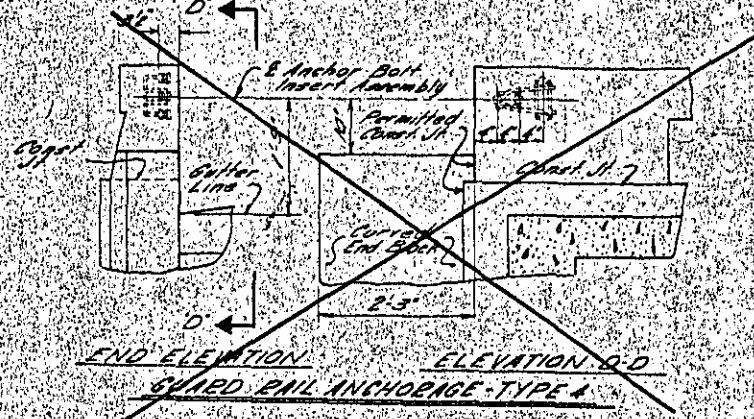
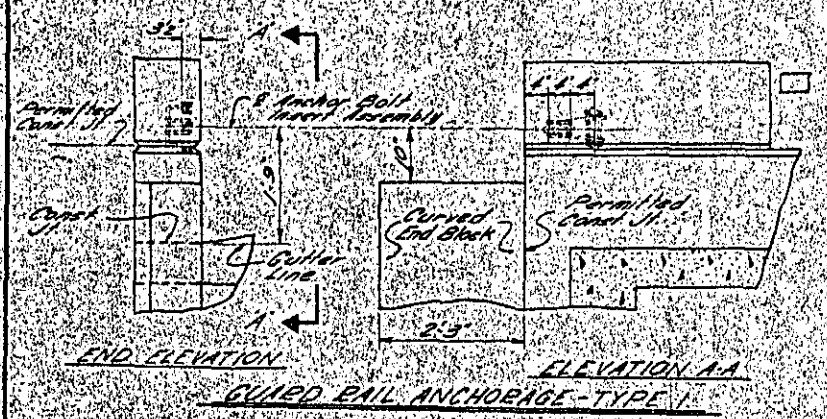
SECTION THRU SIDEWALK & RAIL

Revision No. 1 Revised to change Preset Anchor for 1-Bar Metal Rail 8-4-71 By J.A.V. / BX J.L.S.
 Revision No. 2 Revised to change data concerning tightening of bolts 3-18-71 By J.A.V. / BX J.L.S.
 Revision No. 3 Revised to change weld symbol 8-19-71 By J.A.V. / BX J.L.S.

PROJECT NO. B-118170
 WASH. COUNTY
 STATION 2348+62.05

STATE OF NORTH CAROLINA
STATE HIGHWAY COMMISSION
 STANDARD
 METAL RAIL
 OPTIONAL PRESET
 ANCHOR UNITS
 DECEMBER 1, 1970

REVISIONS					
NO.	BY	DATE	NO.	BY	DATE



BILL OF MATERIAL FOR CURVED END BLOCK

Qty.	No.	Size	Length	Height
6	6	2.5"	5.75"	7.5"
2	4	2.5"	2.75"	7"

Reinforcing Steel - 8 lbs.
 2x4" Anchor Bolt - 2 lbs.
 2x4" Curved End Block - 4 lbs.
 2x4" Parabolic Curve - 4 lbs.
 2x4" Field bend as necessary - 4 lbs.
 2x4" Metal End Post - 4 lbs.
 Concrete End Post - 8 lbs.

GENERAL NOTES

The cost of the 4 Bolt Insert Assembly Unit consisting of the insert assembly and 4 1/2" x 4" bolts with washers complete in place shall be included in the unit contract price and for clear of concrete.

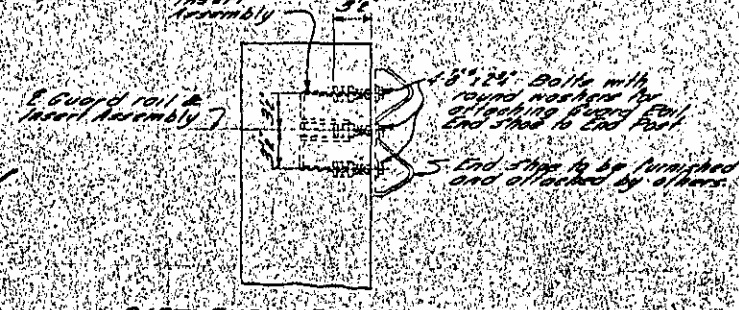
The excavation and backfill for curved end block will not be measured or paid for as a separate item. The entire part of this work shall be included in the unit price bid for clear of concrete.

The anchor unit shall be assembled in the shop. Bolt threads may be recut as necessary to insure fit.

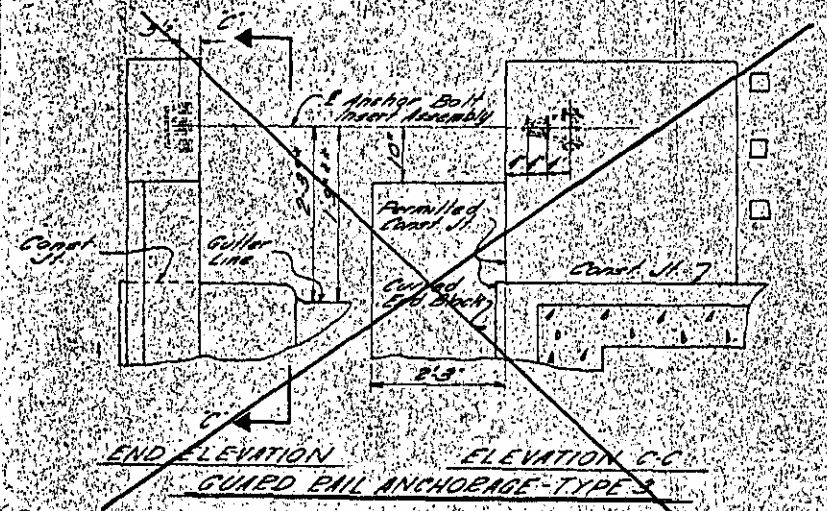
The 1/2" bolts and washers shall conform to the requirements of A.S.T.M. A307 and shall be galvanized to conform to the requirements of A.S.T.M. A153.

If the contractor option stainless steel bolts and washers may be used on an alternate for the galvanized bolts and washers. They shall conform to or exceed the mechanical requirements of A.S.T.M. A307. The use of this alternate shall be approved by the Engineer.

The threaded steel inserts shall conform to the requirements of A.S.T.M. A192 with a minimum tensile strength of 60000 psi.



NOTE GUARD RAIL ANCHORAGE AND CURVED END BLOCKS ARE TO BE CAST AT ALL END POSTS.



2.5' For 2" Curb & Gutter approaching bridge when offset distance from bridge end post to face of curb is over 9'

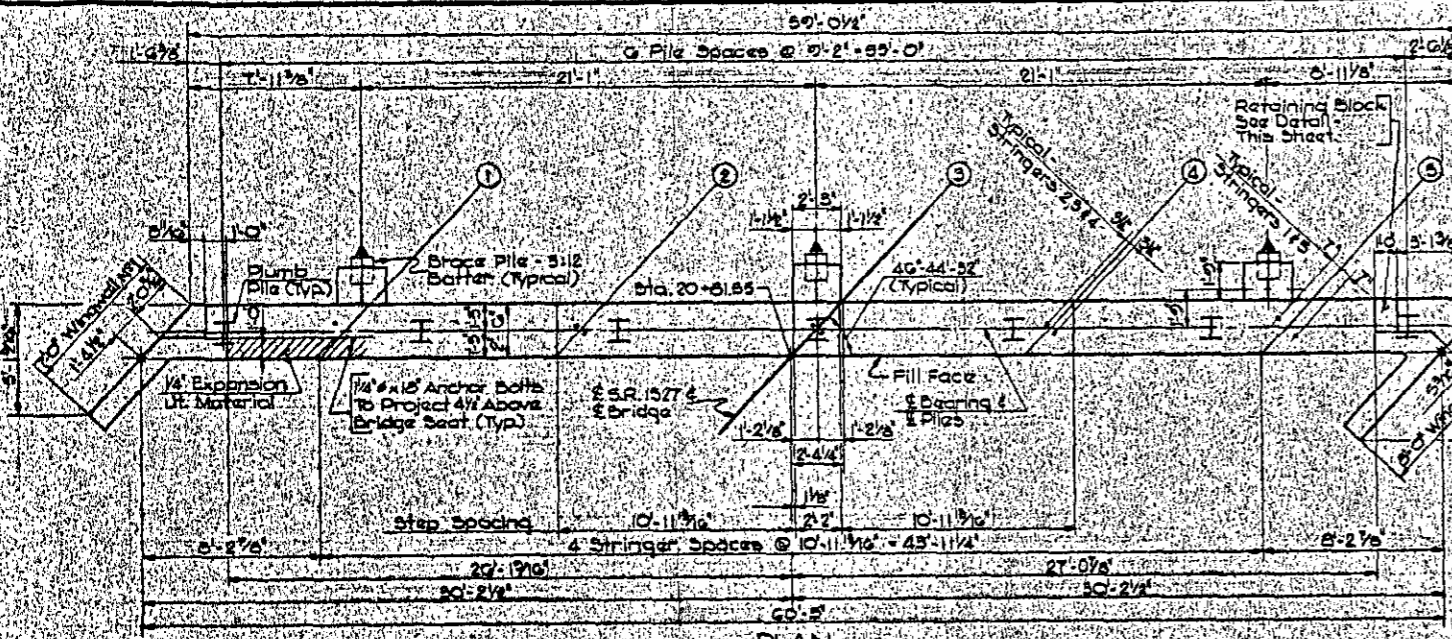
1.5' For no Curb & Gutter and 2" curb & Gutter approaching bridge when offset distance from bridge end post to face of curb is 9' or less

ASSEMBLED BY [Signature]
 CHECKED BY [Signature]

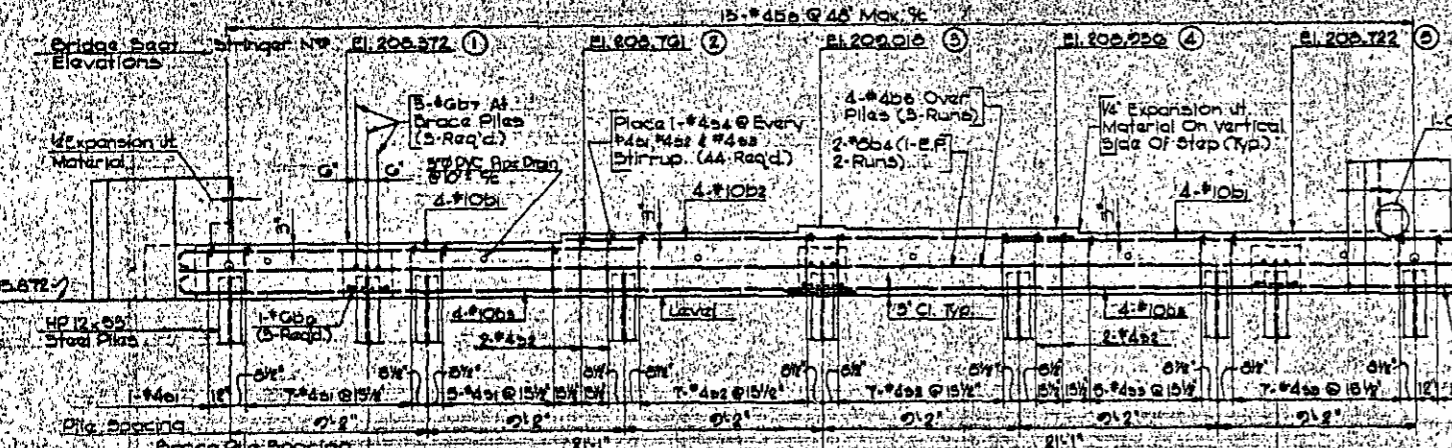
PROJECT No. 2-1161700
 COUNTY
 STATION 209-90 2 1-95
 28 21 05 8 0 1527

STATE OF NORTH CAROLINA
 STATE HIGHWAY COMMISSION
 STANDARD GUARD RAIL ANCHORAGE FOR BRIDGE END POST

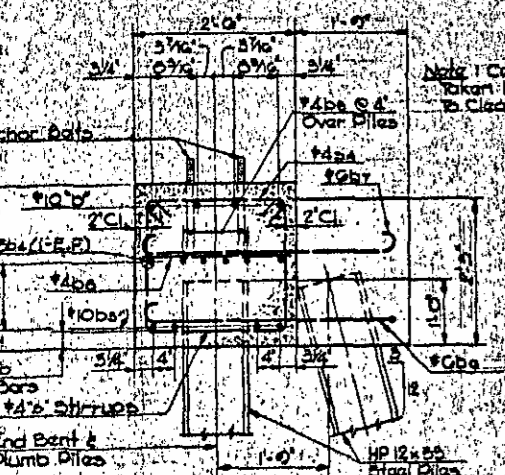
DATE: 1-95
 REVISIONS



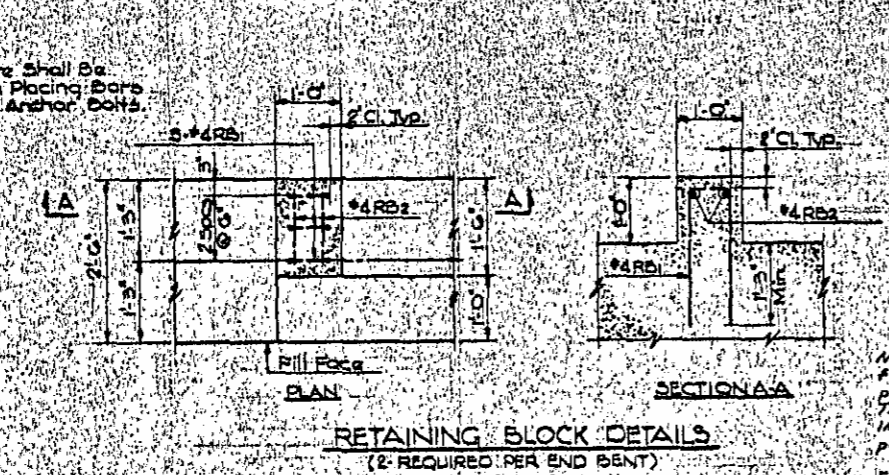
PLAN
END BENT N#1



ELEVATION
END BENT N#1



TYPICAL SECTION THRU CAP

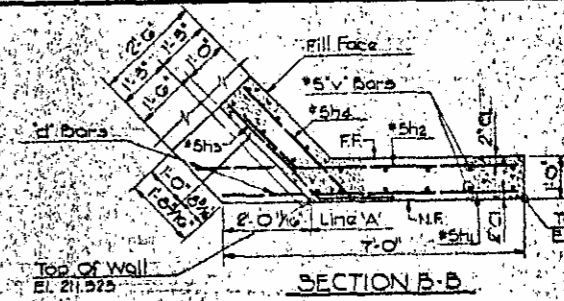


RETAINING BLOCK DETAILS
(2 REQUIRED PER END BENT)

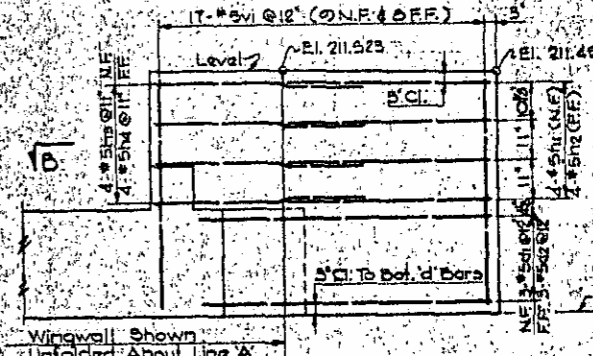
Note: No separate payment will be made for furnishing and installing the P.V.C. plastic pipe drains, hardware, cloth and fasteners. The entire cost of this work shall be included in the unit contract price bid for the several pay items.

For Pile splice details, see End Bent No. 2.

For Bill of Material, see End Bent No. 2.

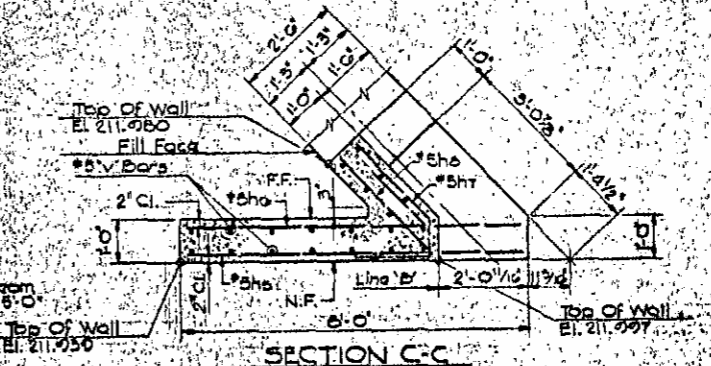


SECTION B-B

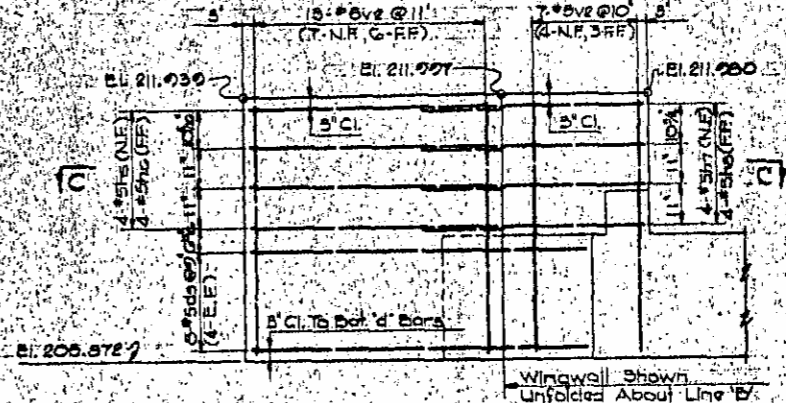


ELEVATION-WINGWALL N#1

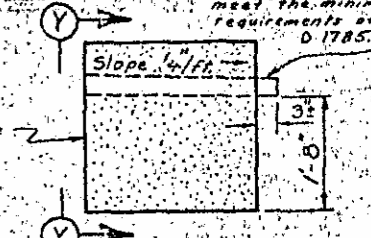
3" P.V.C. Plastic Pipe Drains. This pipe shall meet the minimum requirements of A.S.T.M. D 1785.



SECTION C-C



ELEVATION-WINGWALL N#2



SECTION THRU CAP

6" Square aluminum or galvanized steel with #4 mesh hardware cloth of commercial quality. Anchor firmly to Fill Face.



VIEW Y-Y
PIPE DRAIN DETAILS

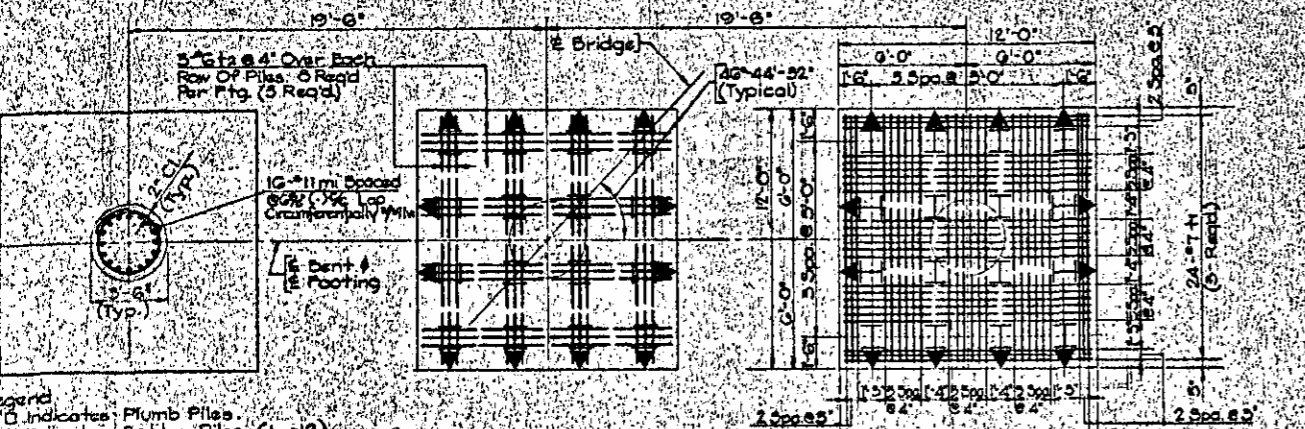
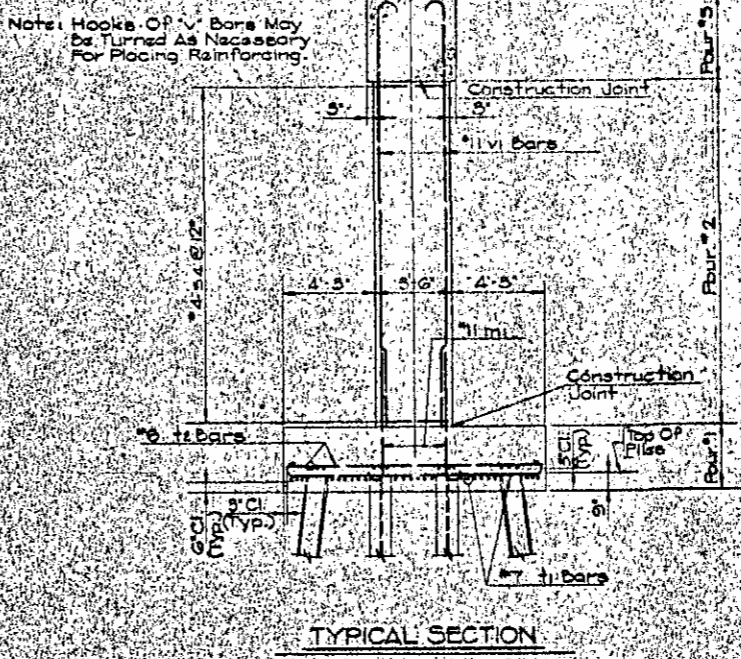
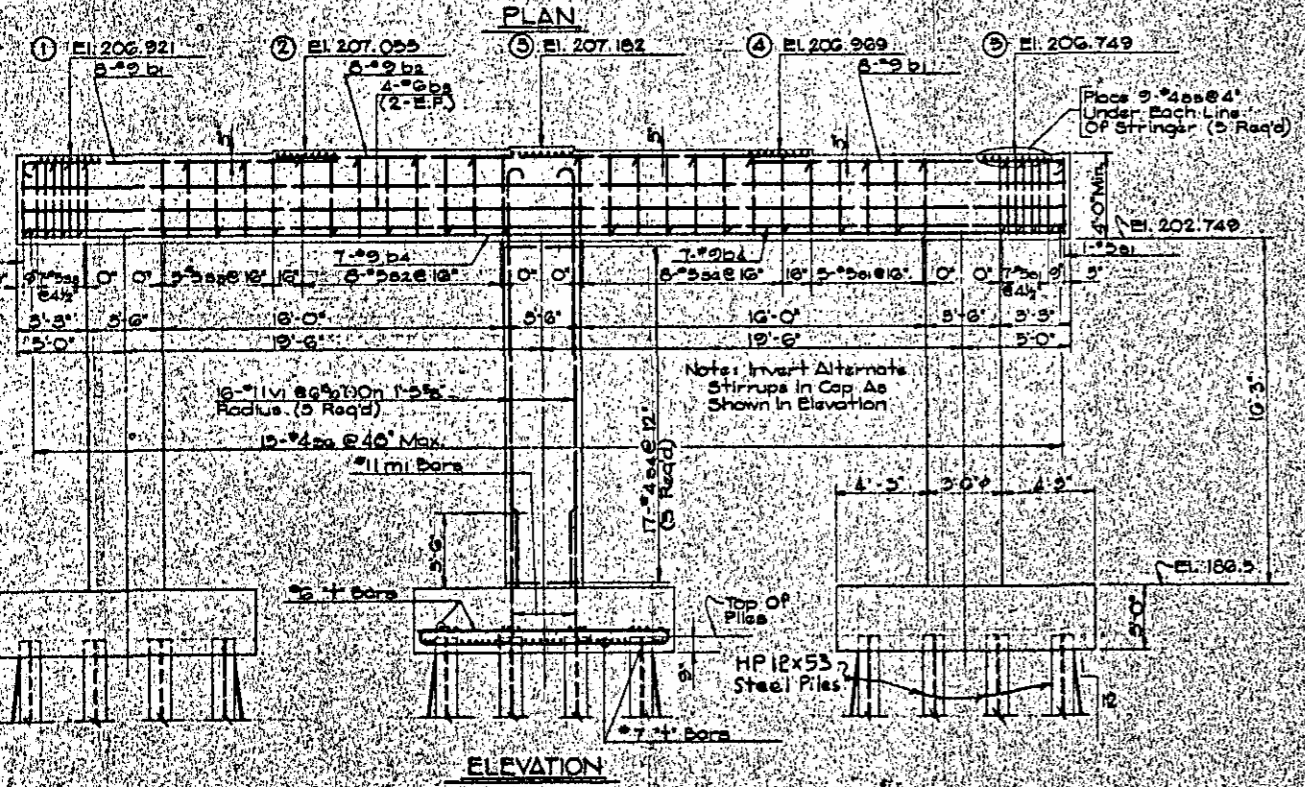
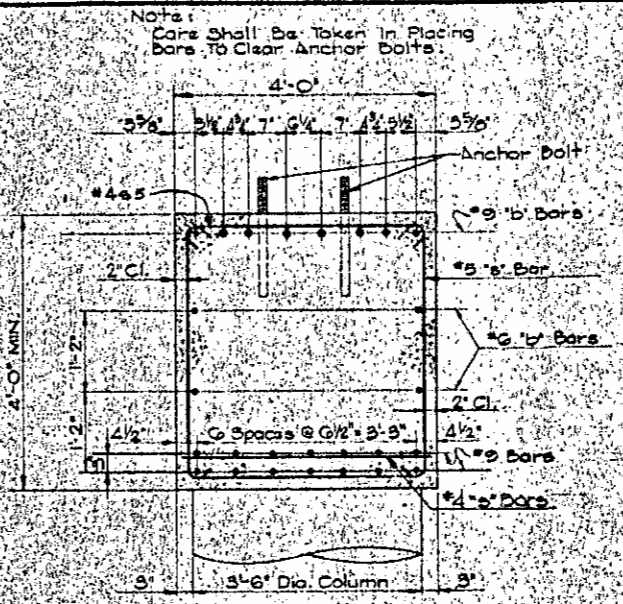
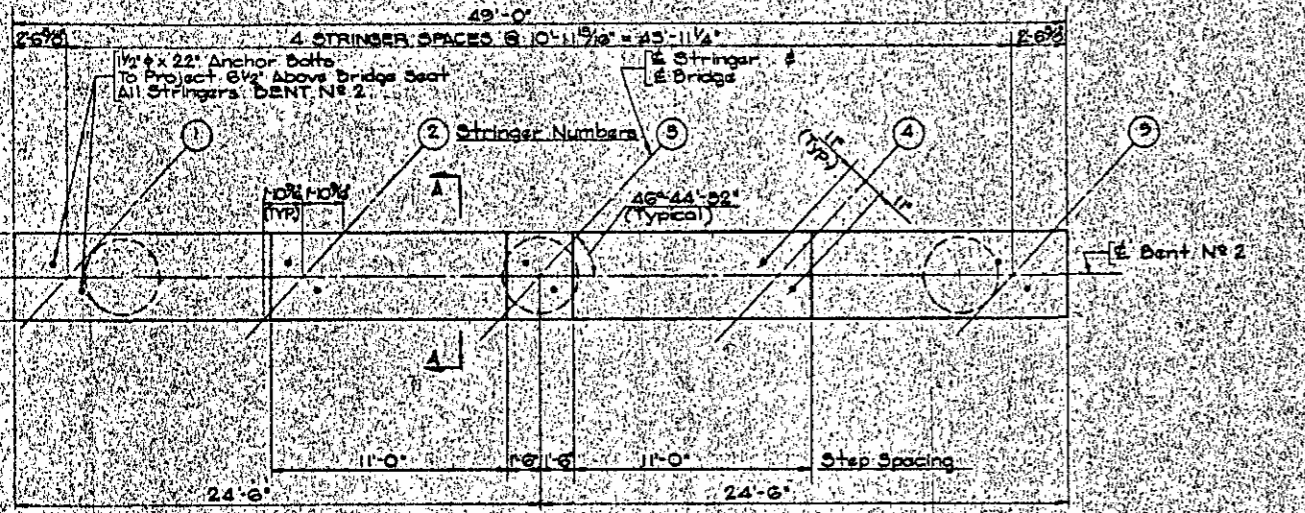
PROJECT NO. 8.11617
NASH COUNTY
STATION: 2249+43.62 I-95
22+34.05 S.R. 1527

STATE OF NORTH CAROLINA
STATE HIGHWAY COMMISSION

S. R. 1527 UNDERPASS
SUBSTRUCTURE
END BENT NO. 1

RUMMEL, KLEPPER & KAHL
CONSULTING ENGINEERS

REVISIONS					
NO.	BY	DATE	NO.	BY	DATE



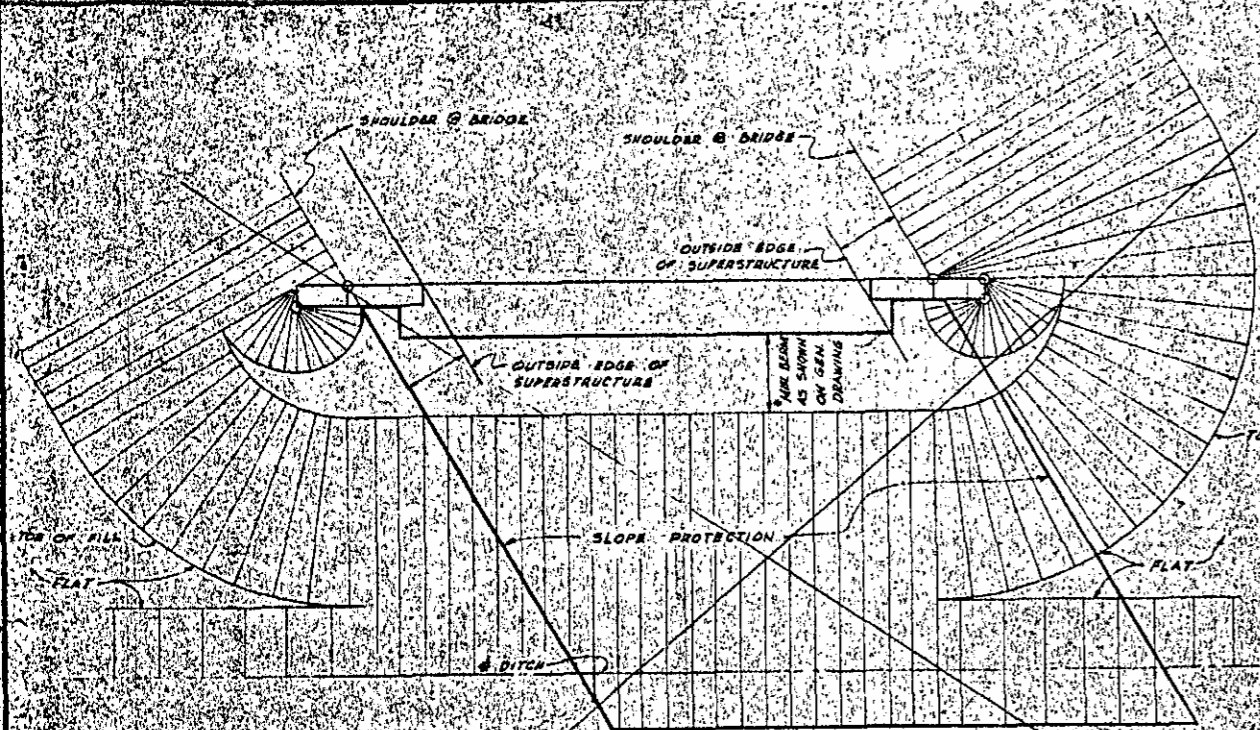
BAR TYPES		BILL OF MATERIAL			
BAR	NO.	SIZE	TYPE	LENGTH	WEIGHT
b1	16	#9	(2)	15'-0"	837
b2	6	#9	Str.	24'-0"	660
b3	4	#9	Str.	48'-0"	270
b4	14	#9	Str.	48'-0"	2,300
mi	45	#11	(2)	7'-2"	1,028
mi	15	#5	(3)	11'-11"	162
mi	16	#5	(3)	12'-4"	206
mi	18	#5	(3)	12'-5"	168
mi	51	#4	(3)	11'-5"	583
mi	45	#4	(4)	4'-5"	133
mi	18	#4	Str.	3'-0"	33
mi	144	#7	(1)	13'-0"	5,526
mi	72	#6	Str.	11'-0"	1,244
mi	45	#11	(2)	20'-11"	5,254
Reinforcing Steel				Lbs.	17,458
HP 12x53 Steel Piles				Nr.	45
Four #11 Castings				C.Y.	2.370
Four #9 Castings				C.Y.	17.4
Four #5 Caps				C.Y.	26.8
TOTAL				C.Y.	47.7

PROJECT NO. 8.116170
 NASH COUNTY
 STATION: 2349+93.62-1-95
 22+34.05 S.R. 1527

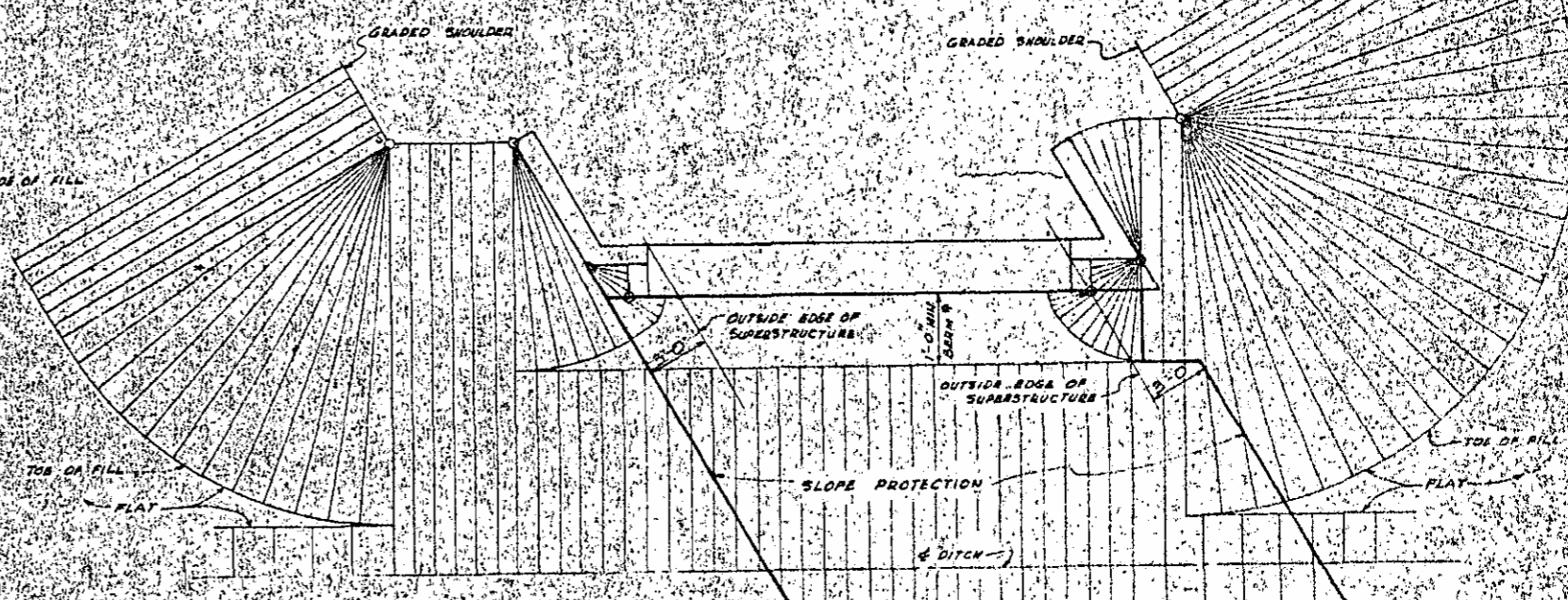
STATE OF NORTH CAROLINA
 STATE HIGHWAY COMMISSION
 S. R. 1527 UNDERPASS
 SUBSTRUCTURE
 BENT NO. 2

RUMMEL, KLÉPPER & KAHL
 CONSULTING ENGINEERS

REVISIONS				
NO.	BY	DATE	NO.	BY
1			2	

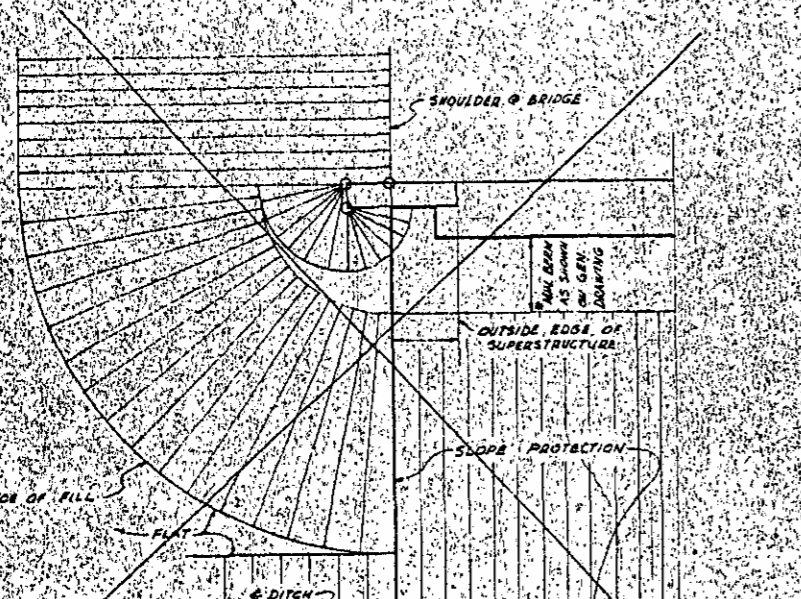


END BENT WITH EAR WALLS - SKEWED



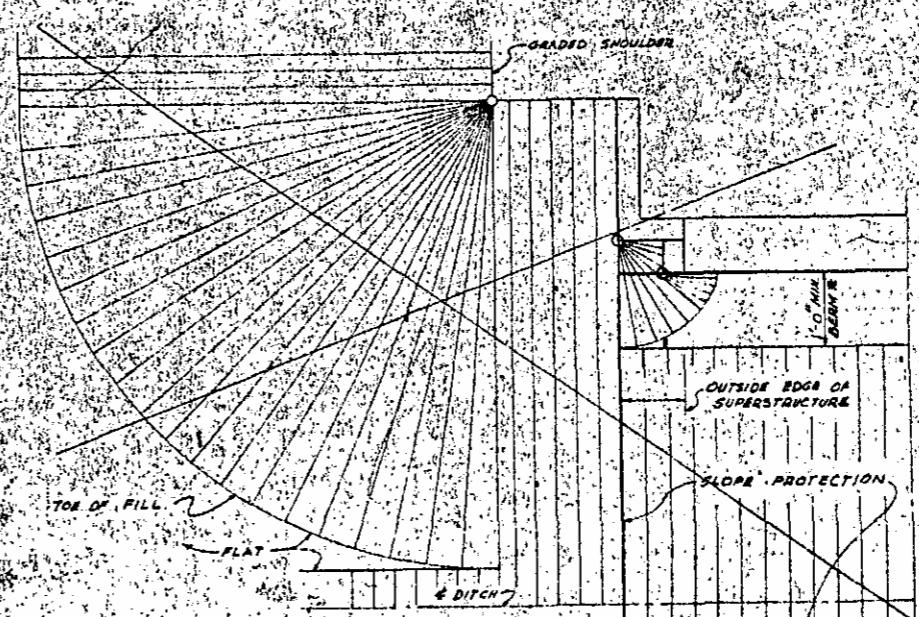
END BENT WITH SWEEP BACK WINGS - SKEWED

NOTE: VARY BERM WIDTH AS NECESSARY TO FIT DITCH ALIGNMENT.



HALF PLAN END BENT WITH EAR WALLS - 90°

NOTE: OTHER SIDE SIMILAR



HALF PLAN END BENT WITH SWEEP BACK WINGS - 90°

NOTE: OTHER SIDE SIMILAR

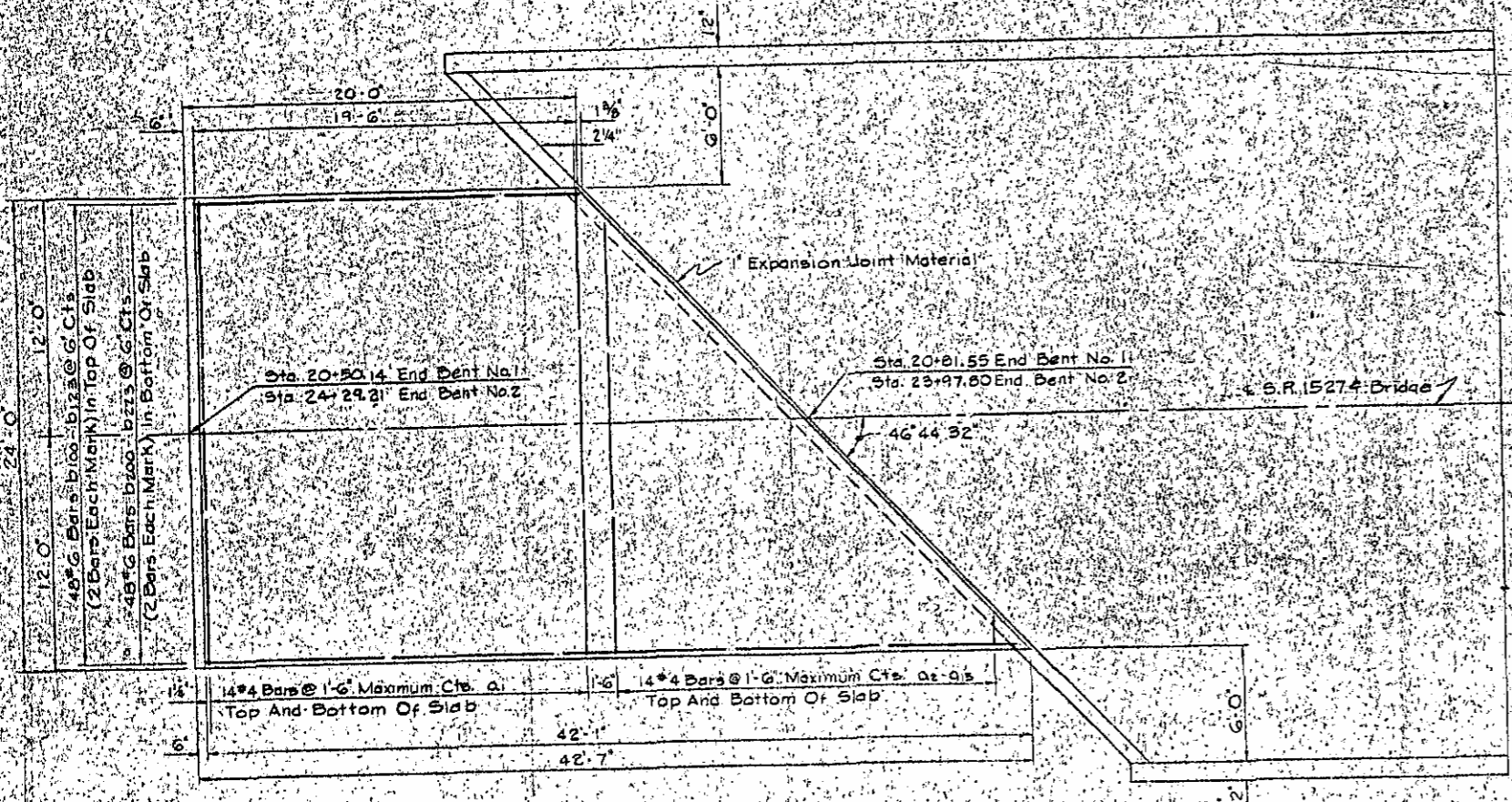
PROJECT NO. 25-367
 WASH COUNTY
 STATION: 2743+43.62
 2234.66 6.7

SHEET 2 OF 2
 STATE OF NORTH CAROLINA
 STATE HIGHWAY COMMISSION
 STANDARD
 SLOPE PROTECTION PAVING
 DETAILS
 FEBRUARY, 1944

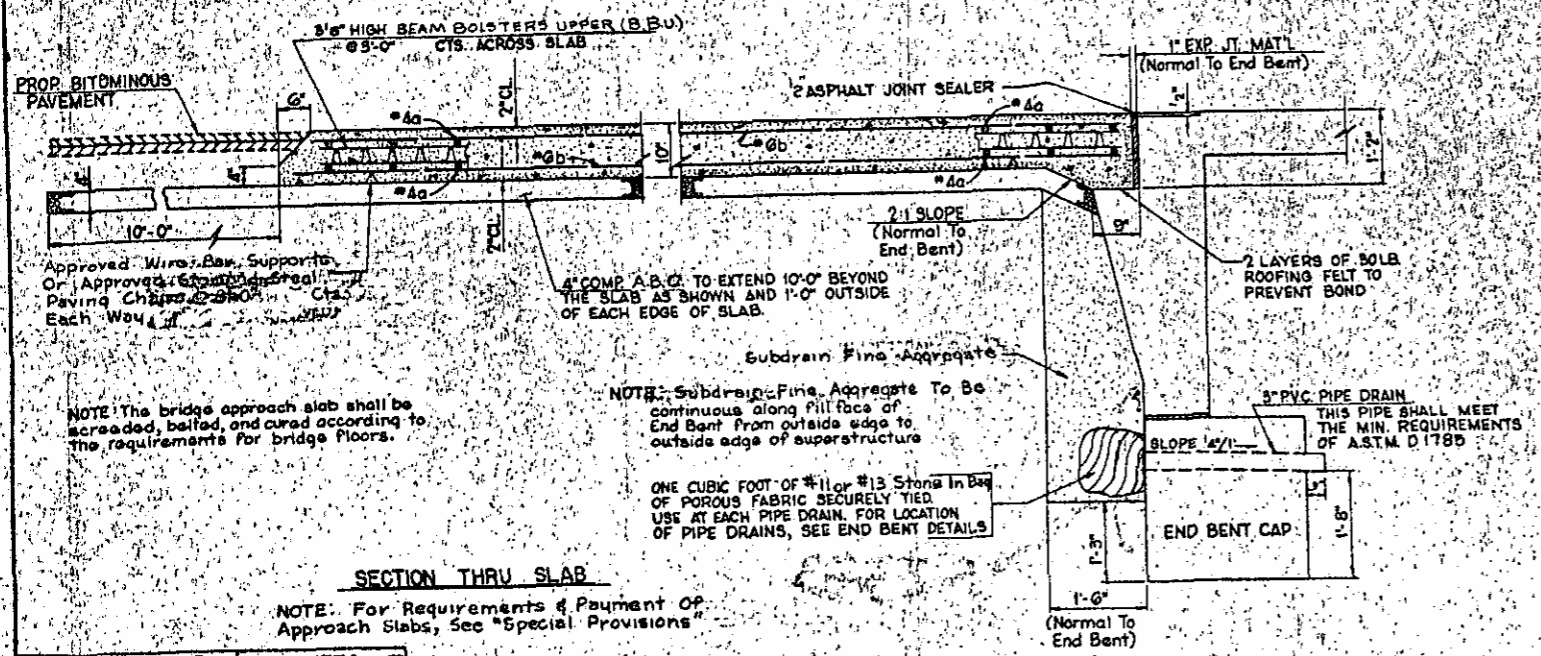
REVISED BY	DATE
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ASSEMBLED BY: [Signature] DATE: 10-3-33
 CHECKED BY: [Signature] DATE: 10-3-33
 DRAWN BY: F.D. ALFORD, JR. DATE: FEB '34

Rev. No. 3 To change min. berm from 3'-6" to 1'-0" on End Bents with Swept Back Wings. ✓ O.W.R.
 REV. NO. 2 TO ELIMINATE 90° CORNER AT THE END SLOPE FOR SKEWED BRIDGES. ✓ G.T.R.
 REV. NO. 1 TO TAKE OUT DIMENSIONS FROM OUTSIDE EDGE OF SUPERSTRUCTURE TO OUTSIDE SLOPE PROTECTION LINE.



PLAN @ END BENT NO. 1
 Approach Slab @ End Bent #2 Similar By Rotation



NOTE: The bridge approach slab shall be screeded, banded, and cured according to the requirements for bridge floors.
 NOTE: Subdrain Fine Aggregate To Be continuous along full face of End Bent from outside edge to outside edge of superstructure
 ONE CUBIC FOOT OF #11 or #13 Stone In Bag OF POROUS FABRIC SECURELY TIED USE AT EACH PIPE DRAIN. FOR LOCATION OF PIPE DRAINS, SEE END BENT DETAILS
 NOTE: For Requirements of Payment of Approach Slabs, See "Special Provisions"

BILL OF MATERIAL											
FOR ONE UNIT — TWO REQUIRED											
BAR	NO.	SIZE	TYPE	LENGTH	WEIGHT	BAR	NO.	SIZE	TYPE	LENGTH	WEIGHT
a1	2	4	Str.	23-2	443	b201	2	6	Str.	28-3	85
a2	2			22-1	30	b202	2			24-3	38
a3	2			20-6	27	b203	2			30-2	91
a4	2			18-10	25	b204	2			31-1	93
a5	2			17-3	23	b205	2			32-0	96
a6	2			15-8	21	b206	2			33-0	99
a7	2			14-1	19	b207	2			33-11	102
a8	2			12-6	17	b208	2			34-10	105
a9	2			10-11	15	b209	2			35-10	108
a10	2			9-4	12	b210	2			36-9	110
a11	2			7-9	10	b211	2			37-5	113
a12	2			6-1	8	b212	2			38-7	116
a13	2			4-6	6	b213	2			34-7	119
a14	2			2-11	4	b214	2			40-6	122
a15	2	4	Str.	1-4	2	b215	2	6	Str.	41-5	124
b100	2	6	Str.	19-5	58	Quantities For One Unit					
b101	2			20-4	61	Class A-A Conc. Cu Yds. 23.5					
b102	2			21-3	64	Reinforcing Steel Lbs. = 5049					
b103	2			22-2	67						
b104	2			23-2	70	Two Units Road at This Structure					
b105	2			24-1	72	Location					
b106	2			25-0	75	Total Quantities					
b107	2			26-0	78	Class A-A Conc. Cu Yds. = 47.0					
b108	2			26-11	81	Reinforcing Steel Lbs. = 10018					
b109	2			27-10	84						
b110	2			28-10	87						
b111	2			29-9	90						
b112	2			30-2	92						
b113	2			31-7	95						
b114	2			32-7	98						
b115	2			33-6	101						
b116	2			34-5	103						
b117	2			35-5	106						
b118	2			36-4	109						
b119	2			37-3	112						
b120	2			38-2	115						
b121	2			39-2	118						
b122	2			40-1	120						
b123	2	6	Str.	41-0	123						
b200	2	6	Str.	19-10	60						
b201	2			20-9	62						
b202	2			21-8	65						
b203	2			22-7	68						
b204	2			23-7	71						
b205	2			24-6	74						
b206	2			25-5	76						
b207	2			26-5	80						
b208	2	6	Str.	27-4	82						

PROJECT NO. 811G1706
 WASH COUNTY
 STATION 23+49.52 TO 23+52.19
 22+34.05 S.R. 1527

STATE OF NORTH CAROLINA
STATE HIGHWAY COMMISSION
 RALEIGH
 BRIDGE APPROACH SLAB
 FOR
 FLEXIBLE PAVEMENT
 OVERPASS @ S.R. 1527

REVISIONS				SHEET NO.	
NO.	BY	DATE	NO.	BY	DATE
1			3		
2			4		

TOTAL SHEETS: 76