

09/28/19

See Sheet 1A For Index of Sheets

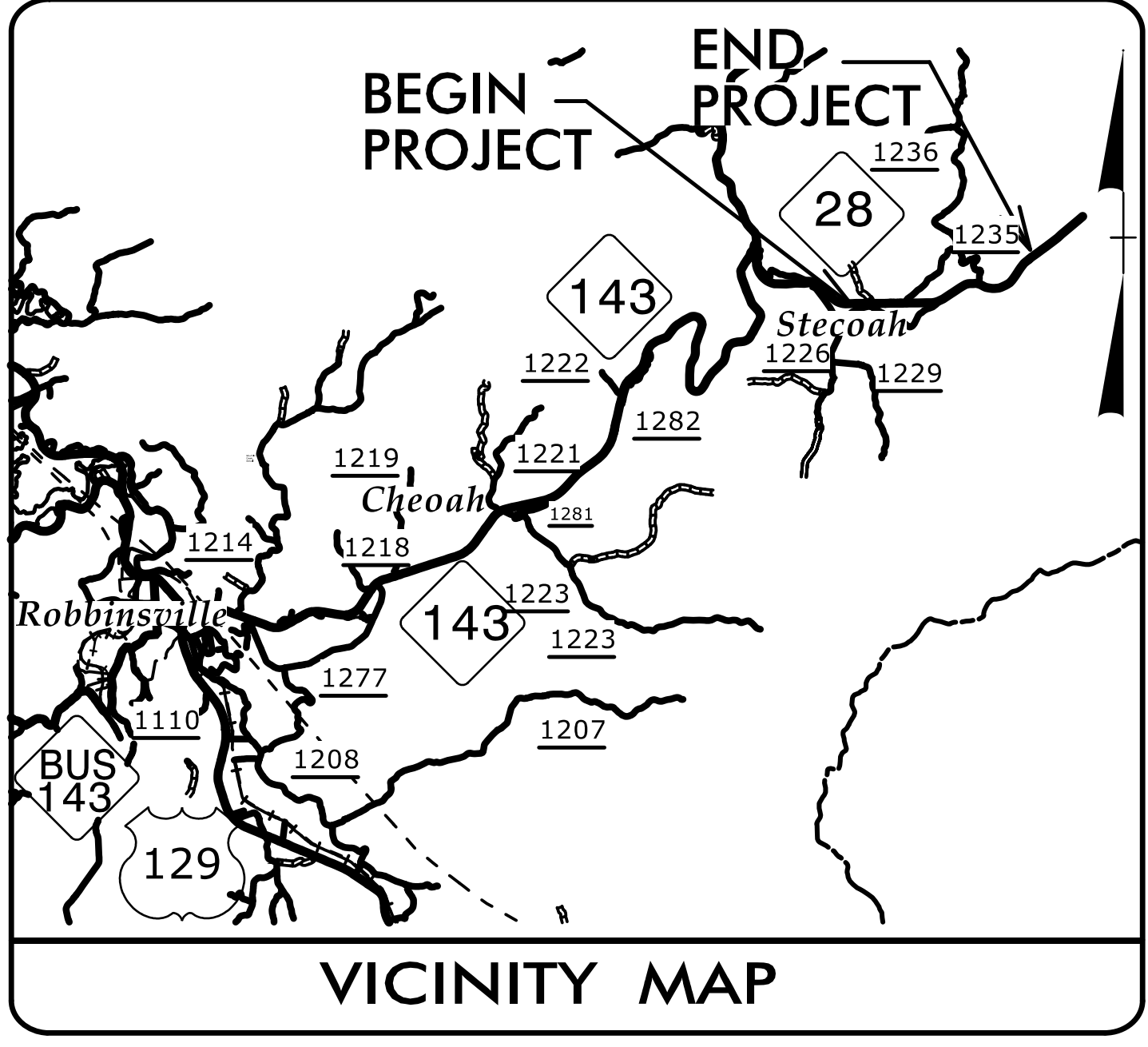
STATE OF NORTH CAROLINA
DIVISION OF HIGHWAYS

STATE	STATE PROJECT REFERENCE NO.	SHEET NO.	TOTAL SHEETS
N.C.	A-0009CD	1	
STATE PROJ. NO.	F.A. PROJ. NO.	DESCRIPTION	
32572.1.15		PE	
32572.3.19	0028011	CONST.	

GRAHAM COUNTY

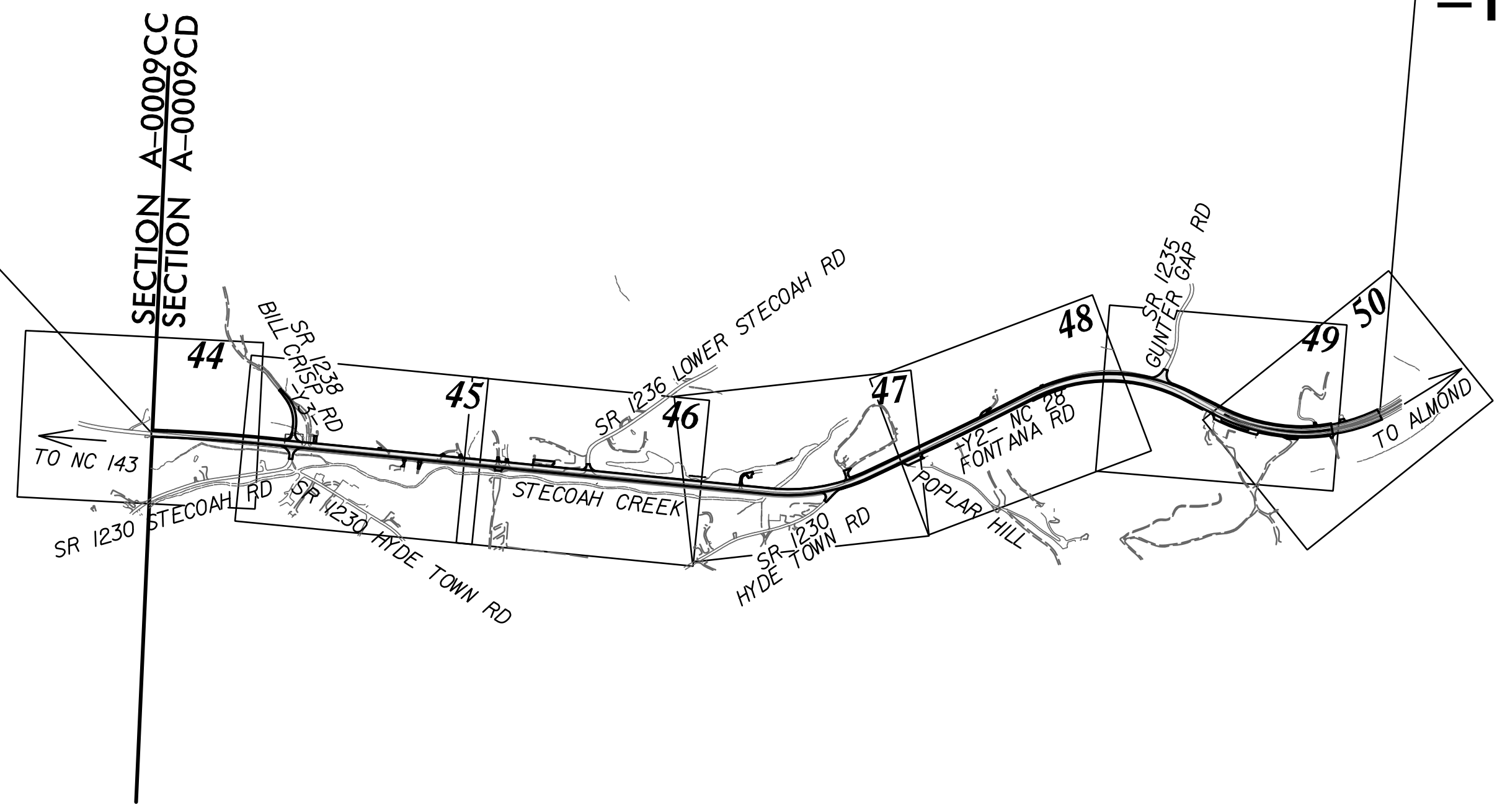
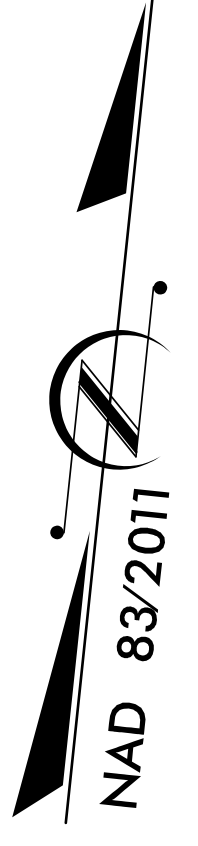
LOCATION: NC 28 FROM WEST OF SR 1228 (STECOAH RD) TO SR 1235 (GUNTER GAP RD)

TYPE OF WORK: GRADING, PAVING, DRAINAGE, CULVERTS, & RETAINING WALLS



BEGIN TIP PROJECT A-0009CD
-Y2- STA. 89 + 80.00

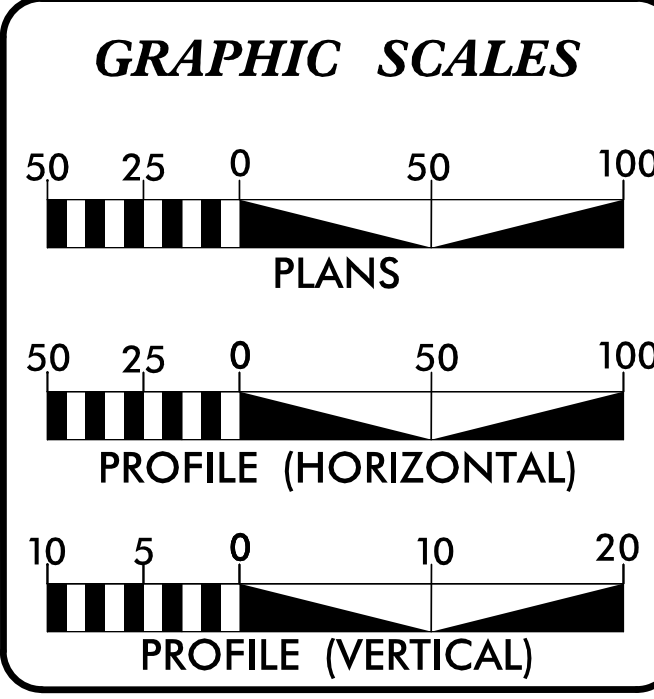
END TIP PROJECT A-0009CD
-Y2- STA. 169 + 60.00



TIP PROJECT: A-0009CD

CONTRACT: C205159

DOCUMENT NOT CONSIDERED FINAL
UNLESS ALL SIGNATURES COMPLETED



-Y2- DESIGN DATA

ADT 2026 =	4100
ADT 2045 =	5200
K =	11 %
D =	67.5 %
T =	7 % *
V =	50 MPH
* TTST = 2% DUAL = 5%	
FUNC CLASS =	
RURAL ARTERIAL	
REGIONAL TIER	

PROJECT LENGTH

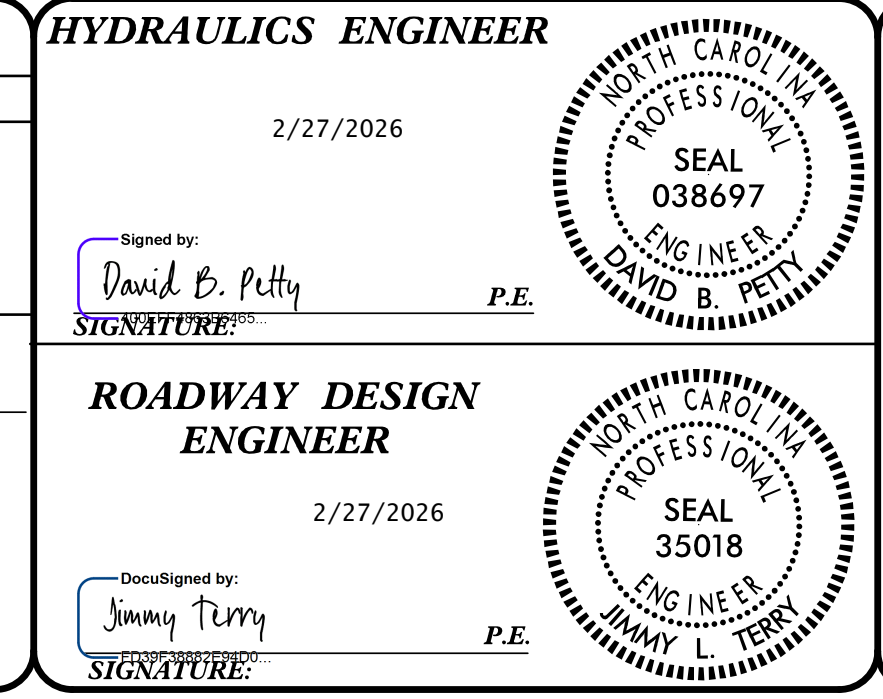
LENGTH ROADWAY TIP	PROJECT A-0009CD	=	1.511 MILES
TOTAL LENGTH TIP	PROJECT A-0009CD	=	1.511 MILES

NC DOT CONTACT: JEANETTE WHITE, PE

PLANS PREPARED BY:	PLANS PREPARED FOR:
TGS ENGINEERS 201 W. MARION ST SHELBY, NC 28150 PH (704) 476-0003 CORP. LICENSE NO.: C-0275	NORTH CAROLINA DEPARTMENT OF TRANSPORTATION DIVISION 14 252 Webster Rd Sylva, NC 28779
RIGHT OF WAY DATE:	JIMMY L. TERRY, PE PROJECT ENGINEER
AUGUST 12, 2021	
LETTING DATE:	AUSTIN TURNER, PE PROJECT DESIGN ENGINEER
MAY 19, 2026	
2024 STANDARD SPECIFICATIONS	

HYDRAULICS ENGINEER

2/27/2026	
Signed by: David B. Petty	P.E.
SIGNATURE:	
2/27/2026	
Signed by: Jimmy Terry	P.E.
SIGNATURE:	



2/20/2026 X:\NCDOT\VA-0009\Roadway\Proj\A-0009CD Plan Sheets\A-0009CD_Rdy_rsh.dgn User:smelvin

8/17/1999
2/20/2026
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Insert.mxd

INDEX OF SHEETS	
SHEET NUMBER	SHEET
1	TITLE SHEET
1A	INDEX OF SHEETS, GENERAL NOTES, AND STANDARD DRAWINGS
1B	CONVENTIONAL SYMBOLS
2A-1 THRU 2A-4	PAVEMENT SCHEDULE AND TYPICAL SECTIONS
2B-1	ROADWAY DETAIL - LAYOUT OF MOND. CONC. ISLAND, 8" CONC. CURB AND MISC. DRIVES
2B-2	ROADWAY DETAIL - TIMBER RAIL FENCE AT MUP
2C-1	SPECIAL DETAILS - CONCRETE SIDEWALK
2C-2 THRU 2C-4	SPECIAL DETAILS - GUARDRAIL PLACEMENT
2C-5	SPECIAL DETAILS - HANDRAIL ON WALL
2C-6	SPECIAL DETAILS - WOODRAIL IN BACK OF GUARDRAIL
2C-7 THRU 2C-8	SPECIAL DETAILS - METHOD OF PIPE INSTALLATION
2D-1	DRAINAGE DETAIL - TEMPORARY STEEL COVER
2D-2	DRAINAGE DETAIL - CONVERT DROP INLET OR JB TO CATCH BASIN
2G-1	GEOTECHNICAL DETAILS - ROCK EMBANKMENTS- POND DETAIL
2G-2	GEOTECHNICAL DETAILS - ROCK EMBANKMENTS- WIDENED FILL DETAIL
2G-3	GEOTECHNICAL DETAILS - GEOTEXTILE FOR EMBANKMENT STABILIZATION
2G-4	GEOTECHNICAL DETAILS - HORIZONTAL DRAINS
2G-5	GEOTECHNICAL DETAILS - ROCK SLOPE MATERIALS
2G-6	GEOTECHNICAL DETAILS - TENSIONED SPOT ROCK BOLTS - 75 KIPS
2G-7 THRU 2G-9	GEOTECHNICAL DETAILS - STANDARD TEMPORARY WALL
2G-10	GEOTECHNICAL DETAILS - STANDARD TEMPORARY SHORING
3B-1	EARTHWORK SUMMARY & ASPHALT PAVEMENT REMOVAL
3B-2	GUARDRAIL SUMMARY
3D-1 THRU 3D-5	DRAINAGE SUMMARIES
3D-6	SUMMARY OF STORMWATER CONTROL MEASURES
3G-1	GEOTECHNICAL SUMMARIES
3P-1	PARCEL INDEX SHEET
44 THRU 50	PLAN SHEETS
74 THRU 77	PROFILE SHEETS
RW-01 THRU RW-49	SURVEY CONTROL SHEETS
TMP-1 THRU TMP-16	TRAFFIC MANAGEMENT PLANS
PMP-1 THRU PMP-4	PAVEMENT MARKING PLANS
EC-1 THRU EC-18	EROSION CONTROL PLANS
RF-1	REFORESTATION DETAIL SHEET
RF-2 & RF-3	STREAMBANK REFORESTATION DETAIL SHEETS
SIGN-1 THRU SIGN-8	SIGNING PLANS
UO-1 THRU UO-8	UTILITIES BY OTHERS PLANS
X-1	CROSS-SECTION INDEX
X-1A	CROSS-SECTION SUMMARY SHEET
X-2 THRU X-92	CROSS-SECTIONS
STRUCTURE TITLE SHEET	
STRUCTURE - INDEX OF SHEETS	
C1-1 THRU C1-6	CULVERT PLANS - SINGLE 12 FT X 5 FT CONCRETE BOX CULVERT
C2-1 THRU C2-7	CULVERT PLANS - DOUBLE 6 FT X 6 FT CONCRETE BOX CULVERT
STANDARD NOTES	
W41-1 THRU W41-3	RETAINING WALL #41

GENERAL NOTES

GENERAL NOTES: 2024 SPECIFICATIONS
EFFECTIVE: 01-16-2024
REVISED:

GRADE LINE:
GRADING AND SURFACING OR RESURFACING AND WIDENING:
THE GRADE LINES SHOWN DENOTE THE FINISHED ELEVATION OF THE PROPOSED SURFACING AT GRADE POINTS SHOWN ON THE TYPICAL SECTIONS. WHERE NO GRADE LINES ARE SHOWN, THE PROFILES SHOWN DENOTE THE TOP ELEVATION OF THE EXISTING PAVEMENT ALONG THE CENTER LINE OF SURVEY ON WHICH THE PROPOSED RESURFACING WILL BE PLACED. GRADE LINES MAY BE ADJUSTED BY THE ENGINEER IN ORDER TO SECURE A PROPER TIE-IN.

CLEARING:
CLEARING ON THIS PROJECT SHALL BE PERFORMED TO THE LIMITS ESTABLISHED BY METHOD II.

SUPERELEVATION:
ALL CURVES ON THIS PROJECT SHALL BE SUPERELEVATED IN ACCORDANCE WITH STD. NO. 225.04 AND STD. NO. 225.05 USING THE RATE OF SUPERELEVATION AND RUNOFF SHOWN ON THE PLANS. SUPERELEVATION IS TO BE REVOLVED ABOUT THE GRADE POINTS SHOWN ON THE TYPICAL SECTIONS.

SHOULDER CONSTRUCTION:
ASPHALT, EARTH, AND CONCRETE SHOULDER CONSTRUCTION ON THE HIGH SIDE OF SUPERELEVATED CURVES SHALL BE IN ACCORDANCE WITH STD. NO. 560.01

SIDE ROADS:
THE CONTRACTOR WILL BE REQUIRED TO DO ALL NECESSARY WORK TO PROVIDE SUITABLE CONNECTIONS WITH ALL ROADS, STREETS, AND DRIVES ENTERING THIS PROJECT. THIS WORK WILL BE PAID FOR AT THE CONTRACT UNIT PRICE FOR THE PARTICULAR ITEMS INVOLVED.

SUBSURFACE DRAINS:
SUBSURFACE DRAINS SHALL BE CONSTRUCTED IN ACCORDANCE WITH STD. NO. 815.02 AT LOCATIONS DIRECTED BY THE ENGINEER.

DRIVEWAYS:
DRIVEWAYS SHALL BE CONSTRUCTED IN ACCORDANCE WITH STD. 848.02 USING 3 FOOT RADII OR RADII AS SHOWN ON THE PLANS. LOCATIONS OF DRIVES WILL BE AS SHOWN ON THE PLANS OR AS DIRECTED BY THE ENGINEER.

STREET TURNOUT:
STREET RETURNS SHALL BE CONSTRUCTED IN ACCORDANCE WITH STD. NO. 848.04 USING THE RADII NOTED ON PLANS.

GUARDRAIL:
THE GUARDRAIL LOCATIONS SHOWN ON THE PLANS MAY BE ADJUSTED DURING CONSTRUCTION AS DIRECTED BY THE ENGINEER. THE CONTRACTOR SHOULD CONSULT WITH THE ENGINEER PRIOR TO ORDERING GUARDRAIL MATERIAL.


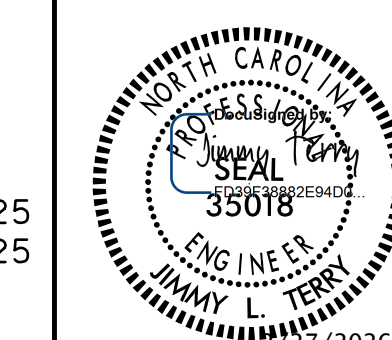
TEMPORARY SHORING:
SHORING REQUIRED FOR THE MAINTENANCE OF TRAFFIC NOT SHOWN ON THE PLANS WILL BE PAID FOR AT THE CONTRACT PRICE FOR "TEMPORARY SHORING".

UTILITIES:
UTILITY OWNERS ON THIS PROJECT ARE DUKE ENERGY, FRONTIER, AND ZITO MEDIA
ANY RELOCATION OF EXISTING UTILITIES WILL BE ACCOMPLISHED BY OTHERS.

RIGHT-OF-WAY MARKERS:
ALL RIGHT-OF-WAY MARKERS ON THIS PROJECT SHALL BE PLACED BY OTHERS.

CURB RAMPS
CURB RAMPS ARE SHOWN ON THE PLANS AT APPROXIMATE LOCATIONS.
CONSTRUCT ALL CURB RAMPS ACCORDANCE WITH STD 848.06.

ROCK
ROCK IS ANTICIPATED BETWEEN 106+25 TO 106+75, 133+25 TO 137+75, AND 150+25 TO 160+75. BLASTING MAY BE REQUIRED FOR EXCAVATION ON THE PROJECT. SEE SECTION 220 OF THE STANDARD SPECIFICATIONS AND IF APPLICABLE, ROCK BLASTING PROVISION.

 TGS ENGINEERS 201 W. MARION ST. STE 200 SHELBY, NC 28150 PH (704) 476-0003 CORP. LICENSE NO.: C-0275	DOCUMENT NOT CONSIDERED FINAL UNLESS ALL SIGNATURES COMPLETED	PROJECT REFERENCE NO. A-0009CD	SHEET NO. 1A
			

STANDARD DRAWINGS

EFF. 08-11-2025
REV.11-26-2025

2024 ROADWAY ENGLISH STANDARD DRAWINGS

The following Roadway Standards as appear in "Roadway Standard Drawings" Contracts Standards and Development Unit - N. C. Department of Transportation - Raleigh, N. C., Dated January 16, 2024 are applicable to this project and by reference hereby are considered a part of these plans:

STD.NO.	TITLE
DIVISION 2 - EARTHWORK	
200.02	Method of Clearing - Method II
225.02	Guide for Grading Subgrade - Secondary and Local
225.04	Method of Obtaining Superlevation - Two Lane Pavement
225.05	Method of Obtaining Superlevation - Divided Highways
225.06	Method of Grading Sight Distance at Intersections
DIVISION 3 - PIPE CULVERTS	
300.01	Method of Pipe Installation (Use Details in Lieu of Standards for Sheets 1 and 2 of 2)
310.10	Driveway Pipe Construction
DIVISION 5 - SUBGRADE, BASES AND SHOULDERS	
560.01	Method of Shoulder Construction - High Side of Superelevated Curve - Method I
DIVISION 6 - ASPHALT BASES AND PAVEMENTS	
654.01	Pavement Repairs
DIVISION 8 - INCIDENTALS	
815.02	Subsurface Drain
838.27	Reinforced Concrete Endwall - for Single 60" Pipe 90 Skew
838.45	Notes for Reinforced Concrete Endwall - Std. Dwg 838.21 thru 838.40
838.57	Reinforced Brick Endwall - for Single 60" Pipe 90 Skew
838.75	Notes for Reinforced Brick Endwall - Std. Dwg 838.51 thru 838.70
838.80	Precast Endwalls - 12" thru 72" Pipe 90 Skew
840.00	Concrete Base Pad for Drainage Structures
840.01	Brick Catch Basin - 12" thru 54" Pipe
840.02	Concrete Catch Basin - 12" thru 54" Pipe
840.03	Frame, Grates and Hood - for Use on Standard Catch Basin
840.14	Concrete Drop Inlet - 12" thru 30" Pipe
840.15	Brick Drop Inlet - 12" thru 30" Pipe
840.16	Drop Inlet Frame and Grates - for use with Std. Dwg 840.14 and 840.15
840.18	Concrete Grated Drop Inlet Type 'B' - 12" thru 36" Pipe
840.19	Concrete Grated Drop Inlet Type 'D' - 12" thru 36" Pipe
840.20	Frames and Wide Slot Flat Grates
840.22	Frames and Wide Slot Sag Grates
840.25	Anchorage for Frames - Brick or Concrete or Precast
840.27	Brick Grated Drop Inlet Type 'B' - 12" thru 36" Pipe
840.28	Brick Grated Drop Inlet Type 'D' - 12" thru 36" Pipe
840.29	Frames and Narrow Slot Flat Grates
840.31	Concrete Junction Box - 12" thru 66" Pipe
840.32	Brick Junction Box - 12" thru 66" Pipe
840.35	Traffic Bearing Grated Drop Inlet - for Cast Iron Double Frame and Grates
840.45	Precast Drainage Structure
840.46	Traffic Bearing Precast Drainage Structure
840.54	Manhole Frame and Cover
840.66	Drainage Structure Steps
840.71	Concrete and Brick Pipe Plug
840.72	Pipe Collar
846.01	Concrete Curb, Gutter and Curb & Gutter
846.04	Drop Inlet Installation in Shoulder Berm Gutter
848.01	Concrete Sidewalk (Use Detail in Lieu of Standard for Sheet 1 of 1)
848.02	Driveway Turnout - Radius Type
848.04	Street Turnout
848.06	Curb Ramp (Use Details in Lieu of Standards for Sheets 9 and 10 of 13)
848.07	Concrete Sidepath / Shared Use Path / Greenway Construction
850.01	Concrete Paved Ditches
850.10	Guide for Berm Drainage Outlet - 15" and 18" Pipe
852.01	Concrete Islands
852.04	Method for Placement of Drop Inlets in Grassed Median - Using 1'-6" Curb and Gutter
852.06	Method for Placement of Drop Inlets in Concrete Islands
854.02	Double Faced Concrete Barrier - Types T, T1 and T2
857.01	Precast Reinforced Concrete Barrier - 41" Single Faced
862.01	Guardrail Placement (Use Details in Lieu of Standards for Sheets 4, 6, 11, 12, and 14 of 15)
862.02	Guardrail Installation (Use Detail in Lieu of Standard for Sheet 5 of 9)
862.03	Structure Anchor Units (Use Detail in Lieu of Standards for Sheet 6 and 8 of 9)
862.04	Anchoring End of Guardrail - for B-77 and B-83 Anchor Units
876.01	Rip Rap in Channels and Ditches
876.02	Guide for Rip Rap at Pipe Outlets
876.04	Drainage Ditches with Class 'B' Rip Rap





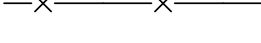








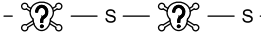
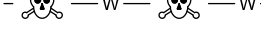
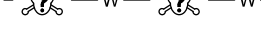






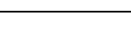
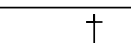

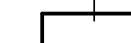




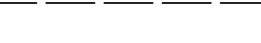









Note: Not to Scale

STATE OF NORTH CAROLINA, DIVISION OF HIGHWAYS

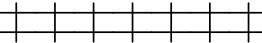

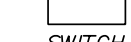
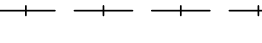

CONVENTIONAL PLAN SHEET SYMBOLS

A-0009CD
IB













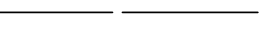













BOUNDARIES AND PROPERTY:

State Line	_____
County Line	_____
Township Line	_____
City Line	_____
Reservation Line	_____
Property Line	_____
Existing Iron Pin (EIP)	_____ 
Computed Property Corner	_____ 
Existing Concrete Monument (ECM)	_____ 
Parcel / Sequence Number	_____ 
Existing Fence Line	_____ 
Proposed Woven Wire Fence	_____ 
Proposed Chain Link Fence	_____ 
Proposed Barbed Wire Fence	_____ 
Existing Wetland Boundary	_____ 
Proposed Wetland Boundary	_____ 
Existing Endangered Animal Boundary	_____ 
Existing Endangered Plant Boundary	_____ 
Existing Historic Property Boundary	_____ 
Known Contamination Area: Soil	_____ 
Potential Contamination Area: Soil	_____ 
Known Contamination Area: Water	_____ 
Potential Contamination Area: Water	_____ 
Contaminated Site: Known or Potential	_____ 
BUILDINGS AND OTHER CULTURE:	
Gas Pump Vent or U/G Tank Cap	_____ 
Sign	_____ 
Well	_____ 
Small Mine	_____ 
Foundation	_____ 
Area Outline	_____ 
Cemetery	_____ 
Building	_____ 
School	_____ 
Church	_____ 
Dam	_____ 
HYDROLOGY:	
Stream or Body of Water	_____ 
Hydro, Pool or Reservoir	_____ 
Jurisdictional Stream	_____ 
Buffer Zone 1	_____ 
Buffer Zone 2	_____ 
Flow Arrow	_____ 
Disappearing Stream	_____ 
Spring	_____ 
Wetland	_____ 
Proposed Lateral, Tail, Head Ditch	_____ 
False Sump	_____ 

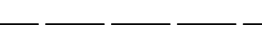
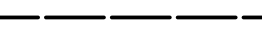









RAILROADS:

Standard Gauge	_____ 
RR Signal Milepost	_____ 
Switch	_____ 
RR Abandoned	_____ 
RR Dismantled	_____ 



RIGHT OF WAY & PROJECT CONTROL:


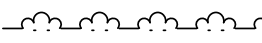
Primary Horiz Control Point	_____ 
Primary Horiz and Vert Control Point	_____ 
Secondary Horiz and Vert Control Point	_____ 
Vertical Benchmark	_____ 
Existing Right of Way Monument	_____ 
Proposed Right of Way Monument (Rebar and Cap)	_____ 
Proposed Right of Way Monument (Concrete)	_____ 
Existing Permanent Easement Monument	_____ 
Proposed Permanent Easement Monument (Rebar and Cap)	_____ 
Existing C/A Monument	_____ 
Proposed C/A Monument (Rebar and Cap)	_____ 
Proposed C/A Monument (Concrete)	_____ 
Existing Right of Way Line	_____ 
Proposed Right of Way Line	_____ 
Existing Control of Access Line	_____ 
Proposed Control of Access Line	_____ 
Proposed ROW and CA Line	_____ 
Existing Easement Line	_____ 
Proposed Temporary Construction Easement	_____ 
Permanent Construction Easement	_____ 
Proposed Temporary Drainage Easement	_____ 
Proposed Permanent Drainage Easement	_____ 
Proposed Permanent Drainage/Utility Easement	_____ 
Proposed Permanent Utility Easement	_____ 
Proposed Temporary Utility Easement	_____ 
Proposed Aerial Utility Easement	_____ 

ROADS AND RELATED FEATURES:

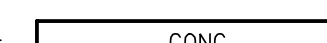



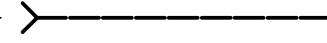
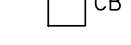



Existing Edge of Pavement	_____ 
Existing Curb	_____ 
Proposed Slope Stakes Cut	_____ 
Proposed Slope Stakes Fill	_____ 
Proposed Curb Ramp	_____ 
Existing Metal Guardrail	_____ 
Proposed Guardrail	_____ 
Existing Cable Guiderail	_____ 
Proposed Cable Guiderail	_____ 
Equality Symbol	_____ 
Pavement Removal	_____ 

VEGETATION:














Single Tree	_____ 
Single Shrub	_____ 

Hedge	_____ 
Woods Line	_____ 
Orchard	_____ 
Vineyard	_____ 

















EXISTING STRUCTURES:

MAJOR:	
Bridge, Tunnel or Box Culvert	_____ 
Bridge Wing Wall, Head Wall and End Wall	_____ 
MINOR:	
Head and End Wall	_____ 
Pipe Culvert	_____ 
Footbridge	_____ 
Drainage Box: Catch Basin, DI or JB	_____ 
Paved Ditch Gutter	_____ 
Storm Sewer Manhole	_____ 
Storm Sewer	_____ 










UTILITIES: * SUE - Subsurface Utility Engineering
LOS - Level of Service - A,B,C or D (Accuracy)

POWER:	
Existing Power Pole	_____ 
Proposed Power Pole	_____ 
Existing Joint Use Pole	_____ 
Proposed Joint Use Pole	_____ 
Power Manhole	_____ 
Power Line Tower	_____ 
Power Transformer	_____ 
U/G Power Cable Hand Hole	_____ 
H-Frame Pole	_____ 
U/G Power Line Test Hole (SUE - LOS A)*	_____ 
U/G Power Line (SUE - LOS B)*	_____ 
U/G Power Line (SUE - LOS C)*	_____ 
U/G Power Line (SUE - LOS D)*	_____ 











TELEPHONE:

Existing Telephone Pole	_____ 
Proposed Telephone Pole	_____ 
Telephone Manhole	_____ 
Telephone Pedestal	_____ 
Telephone Cell Tower	_____ 
U/G Telephone Cable Hand Hole	_____ 
U/G Telephone Test Hole (SUE - LOS A)*	_____ 
U/G Telephone Cable (SUE - LOS B)*	_____ 
U/G Telephone Cable (SUE - LOS C)*	_____ 
U/G Telephone Cable (SUE - LOS D)*	_____ 
U/G Telephone Conduit (SUE - LOS B)*	_____ 
U/G Telephone Conduit (SUE - LOS C)*	_____ 
U/G Telephone Conduit (SUE - LOS D)*	_____ 
U/G Fiber Optics Cable (SUE - LOS B)*	_____ 
U/G Fiber Optics Cable (SUE - LOS C)*	_____ 
U/G Fiber Optics Cable (SUE - LOS D)*	_____ 








WATER:

Water Manhole	_____ 
Water Meter	_____ 
Water Valve	_____ 
Water Hydrant	_____ 
U/G Water Line Test Hole (SUE - LOS A)*	_____ 
U/G Water Line (SUE - LOS B)*	_____ 
U/G Water Line (SUE - LOS C)*	_____ 
U/G Water Line (SUE - LOS D)*	_____ 
Above Ground Water Line	_____ 









TV:

TV Pedestal	_____ 
TV Tower	_____ 
U/G TV Cable Hand Hole	_____ 
U/G TV Test Hole (SUE - LOS A)*	_____ 
U/G TV Cable (SUE - LOS B)*	_____ 
U/G TV Cable (SUE - LOS C)*	_____ 
U/G TV Cable (SUE - LOS D)*	_____ 
U/G Fiber Optic Cable (SUE - LOS B)*	_____ 
U/G Fiber Optic Cable (SUE - LOS C)*	_____ 
U/G Fiber Optic Cable (SUE - LOS D)*	_____ 












GAS:

Gas Valve	_____ 
Gas Meter	_____ 
U/G Gas Line Test Hole (SUE - LOS A)*	_____ 
U/G Gas Line (SUE - LOS B)*	_____ 
U/G Gas Line (SUE - LOS C)*	_____ 
U/G Gas Line (SUE - LOS D)*	_____ 
Above Ground Gas Line	_____ 

SANITARY SEWER:

Sanitary Sewer Manhole	_____ 
Sanitary Sewer Cleanout	_____ 
U/G Sanitary Sewer Line	_____ 
Above Ground Sanitary Sewer	_____ 
SS Force Main Line Test Hole (SUE - LOS A)*	_____ 
SS Force Main Line (SUE - LOS B)*	_____ 
SS Force Main Line (SUE - LOS C)*	_____ 
SS Force Main Line (SUE - LOS D)*	_____ 

MISCELLANEOUS:

Utility Pole	_____ 
Utility Pole with Base	_____ 
Utility Located Object	_____ 
Utility Traffic Signal Box	_____ 
Utility Unknown U/G Line (SUE - LOS B)*	_____ 
U/G Tank; Water, Gas, Oil	_____ 
Underground Storage Tank, Approx. Loc.	_____ 
A/G Tank; Water, Gas, Oil	_____ 
Geoenvironmental Boring	_____ 
Abandoned According to Utility Records	_____ 
End of Information	_____ 

AATUR
E.O.I.

6/2/2025

FINAL PAVEMENT SCHEDULE

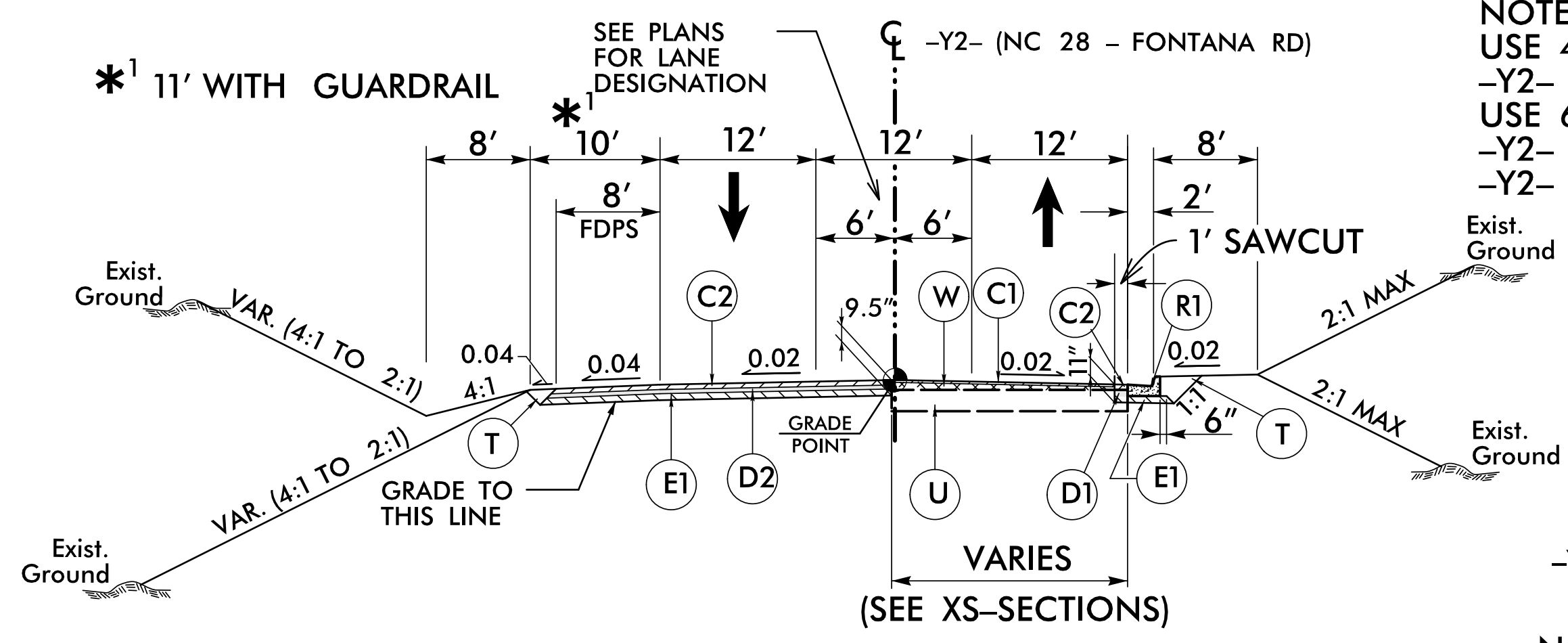
(NOV. 21, 2025)

C1	PROP. APPROX. 1 1/2" ASPHALT CONCRETE SURFACE COURSE, TYPE S9.5C, AT AN AVERAGE RATE OF 168 LBS. PER SQ. YD.
C2	PROP. APPROX. 3" ASPHALT CONCRETE SURFACE COURSE, TYPE S9.5C, AT AN AVERAGE RATE OF 168 LBS. PER SQ. YD. IN EACH OF TWO LAYERS.
C3	PROP. VAR. DEPTH ASPHALT CONCRETE SURFACE COURSE, TYPE S9.5C, AT AN AVERAGE RATE OF 112 LBS. PER SQ. YD. PER 1" DEPTH. TO BE PLACED IN LAYERS NOT TO EXCEED 1 1/2" IN DEPTH.
D1	PROP. APPROX. 4" ASPHALT CONCRETE INTERMEDIATE COURSE, TYPE I19.0C, AT AN AVERAGE RATE OF 456 LBS. PER SQ. YD.
D2	PROP. APPROX. 2 1/2" ASPHALT CONCRETE INTERMEDIATE COURSE, TYPE I19.0C, AT AN AVERAGE RATE OF 285 LBS. PER SQ. YD.
D3	PROP. VAR. DEPTH ASPHALT CONCRETE INTERMEDIATE COURSE, TYPE I19.0C, AT AN AVERAGE RATE OF 114 LBS. PER SQ. YD. PER 1" DEPTH. TO BE PLACED IN LAYERS NOT LESS THAN 2 1/2" OR GREATER THAN 4" IN DEPTH.
E1	PROP. APPROX. 4" ASPHALT CONCRETE BASE COURSE, TYPE B25.0C, AT AN AVERAGE RATE OF 456 LBS. PER SQ. YD.
E2	PROP. VAR. DEPTH ASPHALT CONCRETE BASE COURSE, TYPE B25.0C, AT AN AVERAGE RATE OF 114 LBS. PER SQ. YD. PER 1" DEPTH. TO BE PLACED IN LAYERS NOT LESS THAN 3" OR GREATER THAN 5 1/2" IN DEPTH.
K	CLASS IV SUBGRADE STABILIZATION
J1	PROP. 6" AGGREGATE BASE COURSE.
N	GEOTEXTILE FOR SUBGRADE STABILIZATION
R1	2'-6" CONCRETE CURB AND GUTTER.
R3	8" X 12" CONCRETE CURB
R4	SHOULDER BERM GUTTER
R5	5" MONOLITHIC CONCRETE ISLAND (SURFACE-MOUNT)
T	EARTH MATERIAL.
U	EXISTING PAVEMENT.
V1	MILLING, 0 TO 3" DEPTH, SEE THIS SHEET FOR DETAIL
V2	1 1/2" MILLING
W	WEDGING EXISTING PAVEMENT, SEE THIS SHEET FOR DETAILS
Y1	4" CONCRETE MULTI-USE PATH

PAVEMENT EDGE SLOPES ARE 1:1 UNLESS SHOWN OTHERWISE.

NOTE: ANYWHERE ALONG THE PROJECT IN WHICH LESS THAN 3" OF OVERLAY HAS BEEN CALLED FOR THE CONTRACTOR SHALL MILL EXISTING PAVEMENT AND PROVIDE A MINIMUM OF TWO FULL LAYERS OF S9.5C TO SATISFY RIDEABILITY REQUIREMENTS PER CONTRACT FOR -Y2-.

PROJECT REFERENCE NO. A-0009CD	SHEET NO. 2A-1
ROADWAY DESIGN ENGINEER JIMMY L. TERRY 041986	PAVEMENT DESIGN ENGINEER MATTHEW BRUNER 041986
DOCUMENT NOT CONSIDERED FINAL UNLESS ALL SIGNATURES COMPLETED	
TGS ENGINEERS 201 W. MARION ST SHELBY, NC 28150 PH (704) 476-0003 CORP. LICENSE NO.: C-0275	

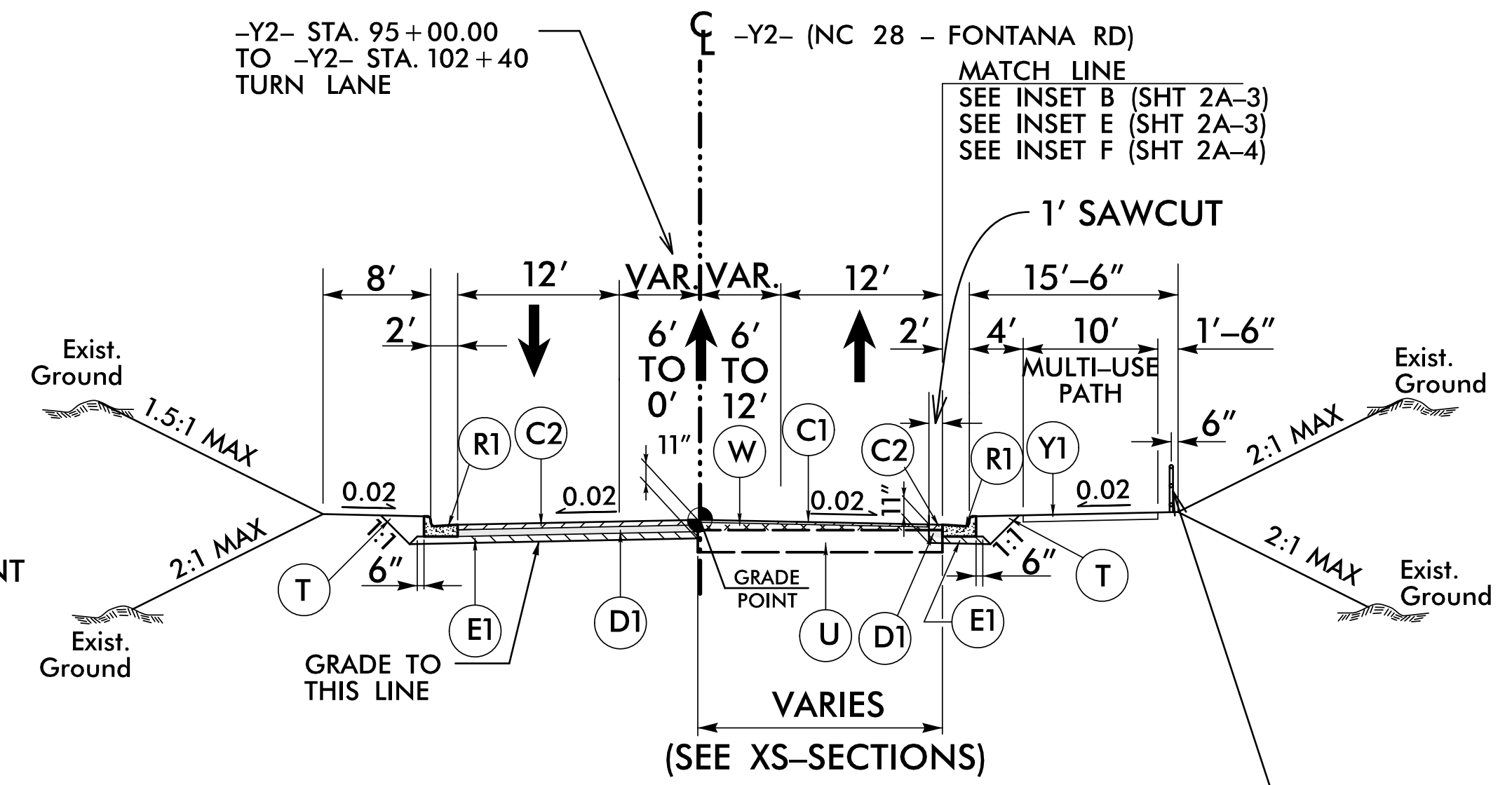


NOTE:
 USE 4' BERM :
 -Y2- STA. 92+50 TO 93+00.00, RT
 USE 6' BERM :
 -Y2- STA. 89+90.00 TO 92+00.00, RT
 -Y2- STA. 93+50.00, RT

USE TYPICAL SECTION NO. 1

-Y2- STA. 89+90.00 TO -Y2- STA. 98+65.27

NOTE: DETAIL SHT. 2C-4
 GUARDRAIL 6" FROM FACE OF CURB.



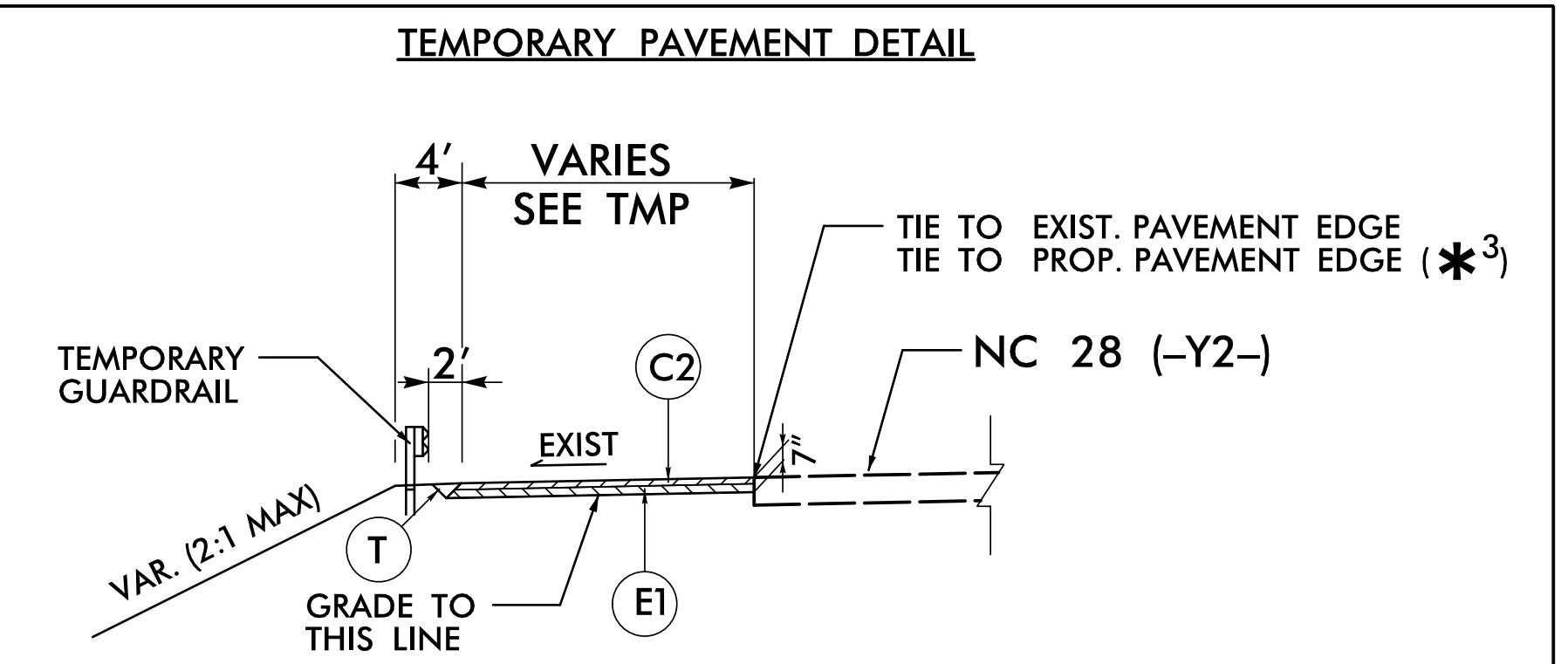
USE TYPICAL SECTION NO. 2

-Y2- STA. 98+65.27 TO -Y2- STA. 135+21.96

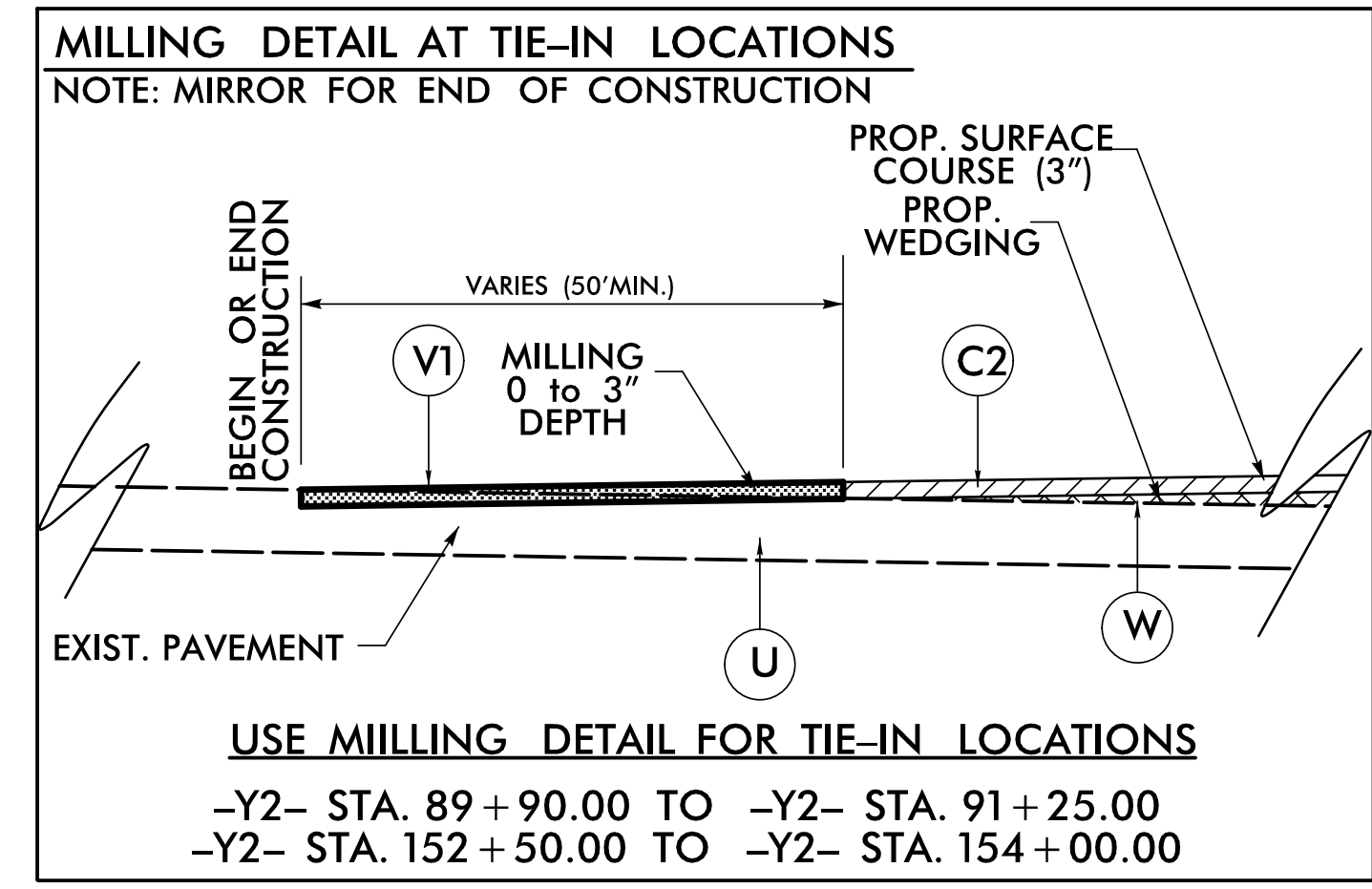
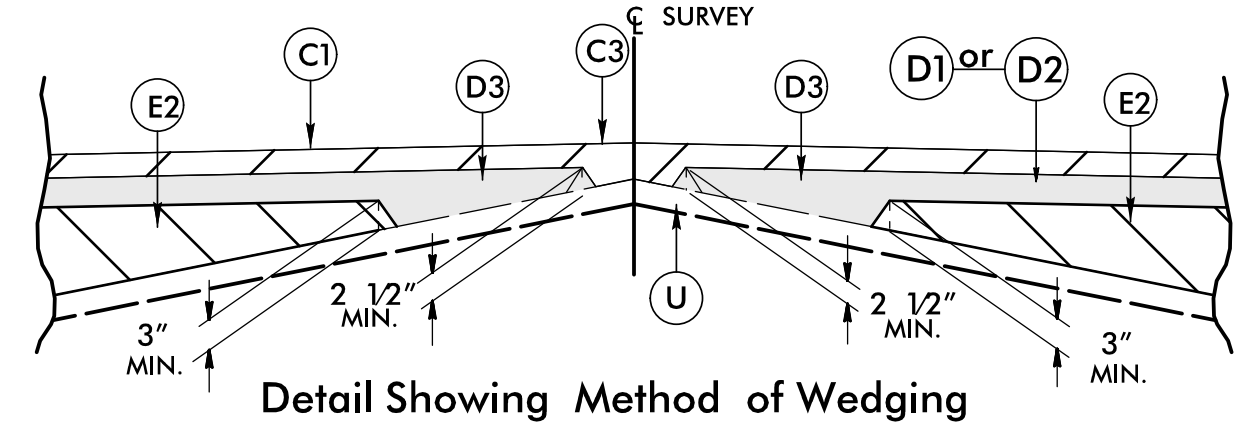
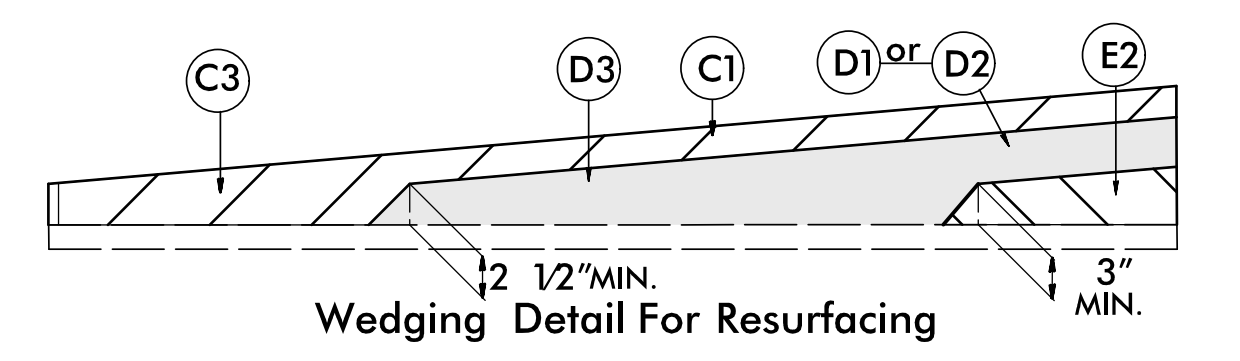
NOTE: USE SHOULDER SECTION AND STANDARD DITCH (SEE TYPICAL SECTION NO. 1)

-Y2- STA. 134+09± TO -Y2- STA. 135+21.96, LT

NOTE: SEE DETAIL SHT. 2C-4
 GUARDRAIL 6" FROM FACE OF CURB.
 AT MUP PLACE GUARDRAIL IN LINE WITH FACE OF CURB



-Y2- STA. 97+60± TO -Y2- STA. 102+15±, RT (PHASE I, STEP 3)
 -Y2- STA. 98+97± TO -Y2- STA. 102+75±, LT (PHASE I, STEP 5)*3
 -Y2- STA. 101+30± TO -Y2- STA. 101+80±, RT (PHASE I, STEP 3)



USE MILLING DETAIL FOR TIE-IN LOCATIONS

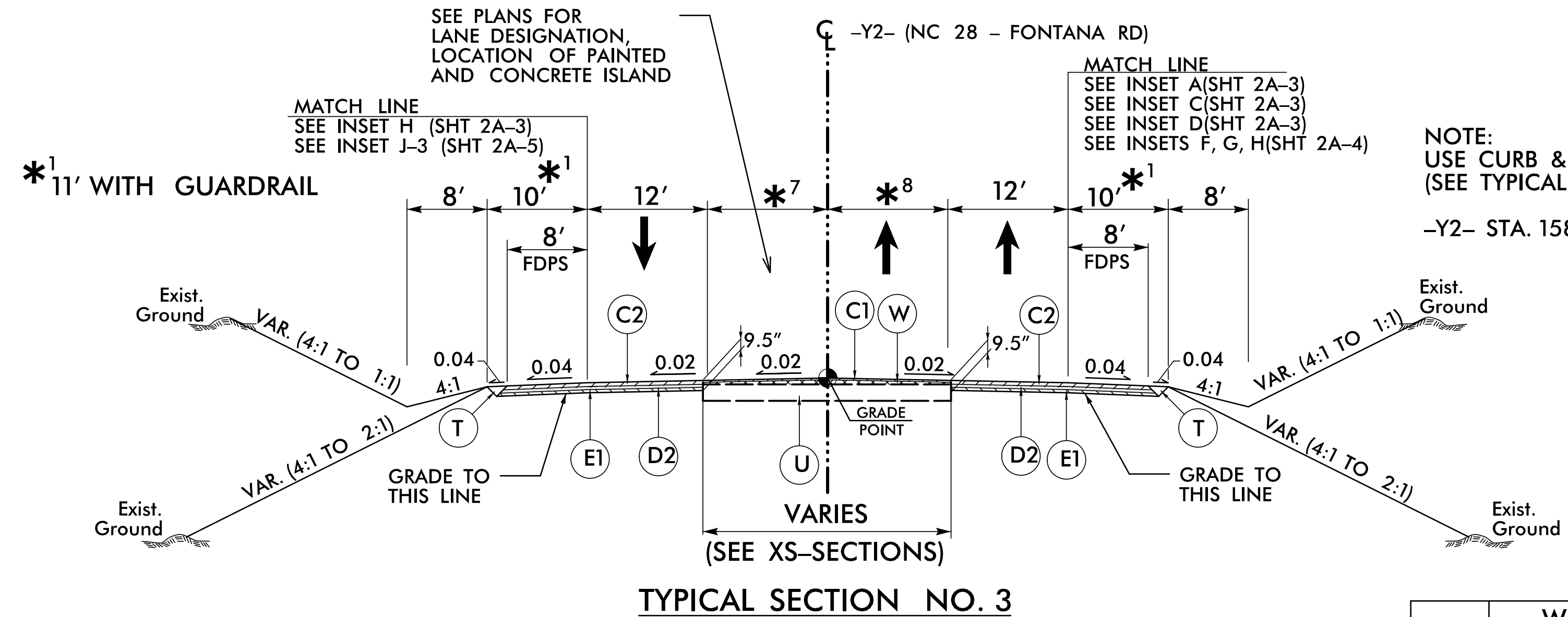
-Y2- STA. 89+90.00 TO -Y2- STA. 91+25.00
 -Y2- STA. 152+50.00 TO -Y2- STA. 154+00.00

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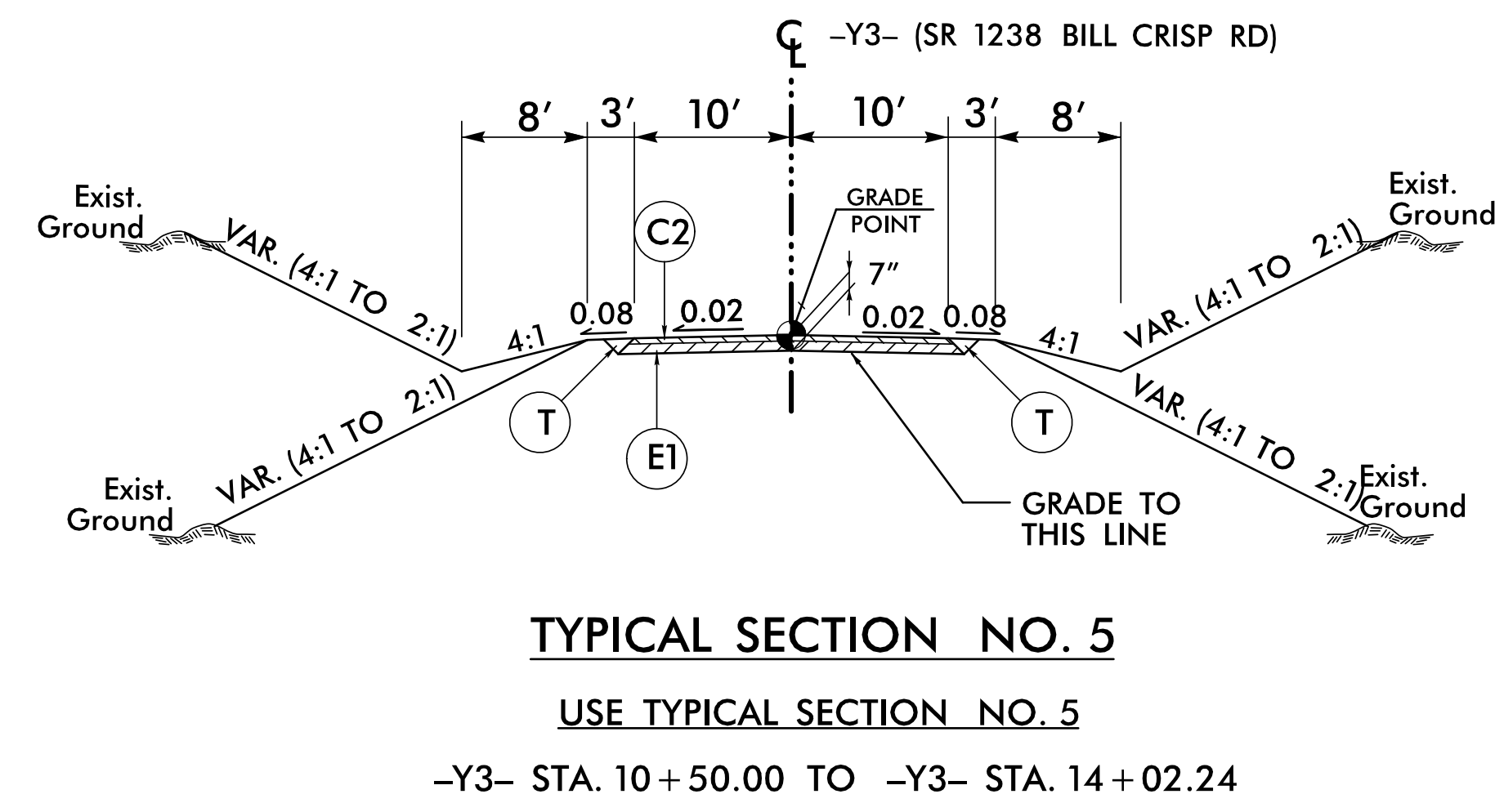
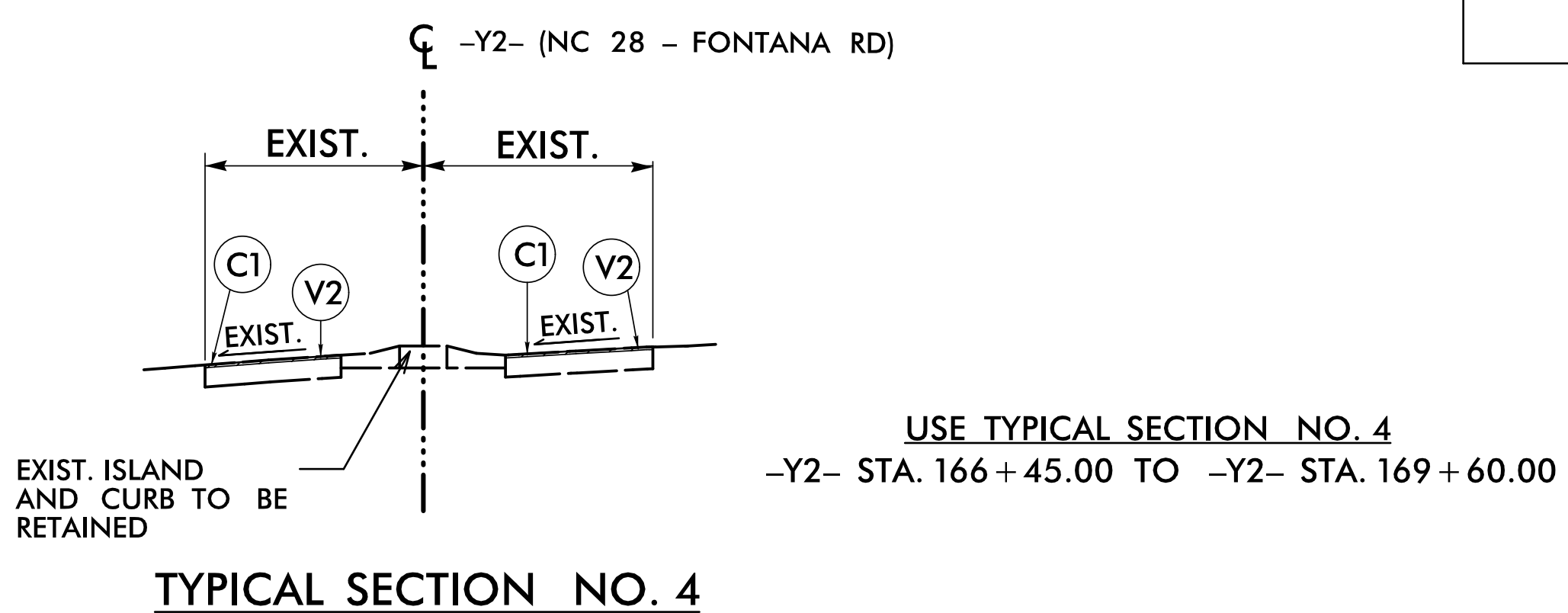
6/2/2019

PAVEMENT SCHEDULE			
C1	1.5" S9.5C	T	EARTH MATERIAL
C2	3" S9.5C	U	EXISTING PAVEMENT
D2	2 1/2" I19.0C	V2	1 1/2" MILLING
E1	4" B25.0C	W	WEDGING

PAVEMENT EDGE SLOPES ARE 1:1 UNLESS SHOWN OTHERWISE.
SEE SHEET 2A-1 FOR DETAILED PAVEMENT SCHEDULE.



	WIDTH	STA TO STA
*7	0'	-Y2- STA. 135+21.96 TO -Y2- STA. 148+20.00
	0' TO 19'	-Y2- STA. 148+20.00 TO -Y2- STA. 158+65.00
	19'	-Y2- STA. 158+65.00 TO -Y2- STA. 158+96.87
	19' TO 16'	-Y2- STA. 158+96.87 TO -Y2- STA. 164+50.00
*8	16' TO 18'-7"	-Y2- STA. 164+50.00 TO -Y2- STA. 166+45.00
	12'	-Y2- STA. 135+21.96 TO -Y2- STA. 158+96.87
	12' TO 15'	-Y2- STA. 158+96.87 TO -Y2- STA. 164+50.00
	15' TO 17'-4"	-Y2- STA. 164+50.00 TO -Y2- STA. 166+45.00



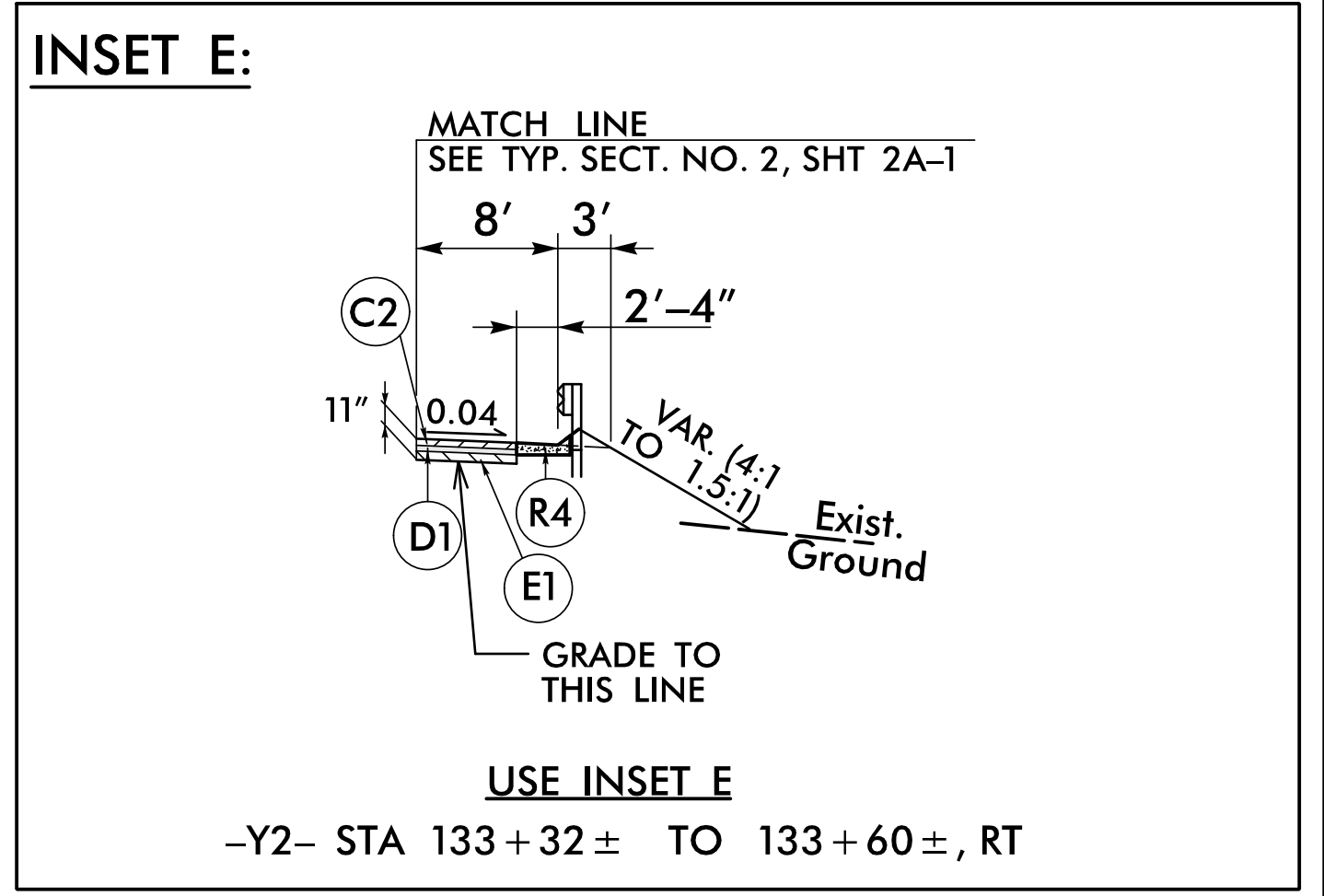
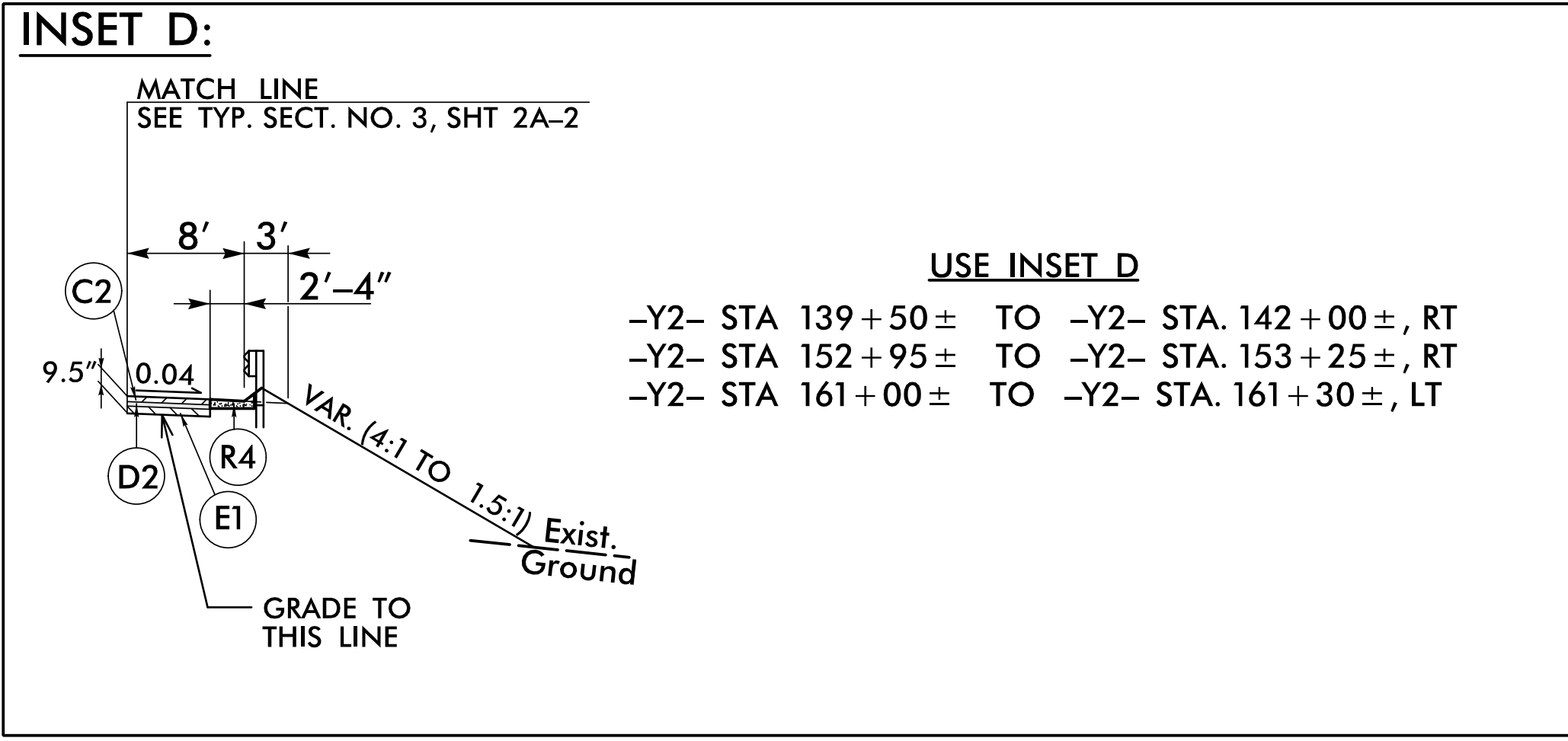
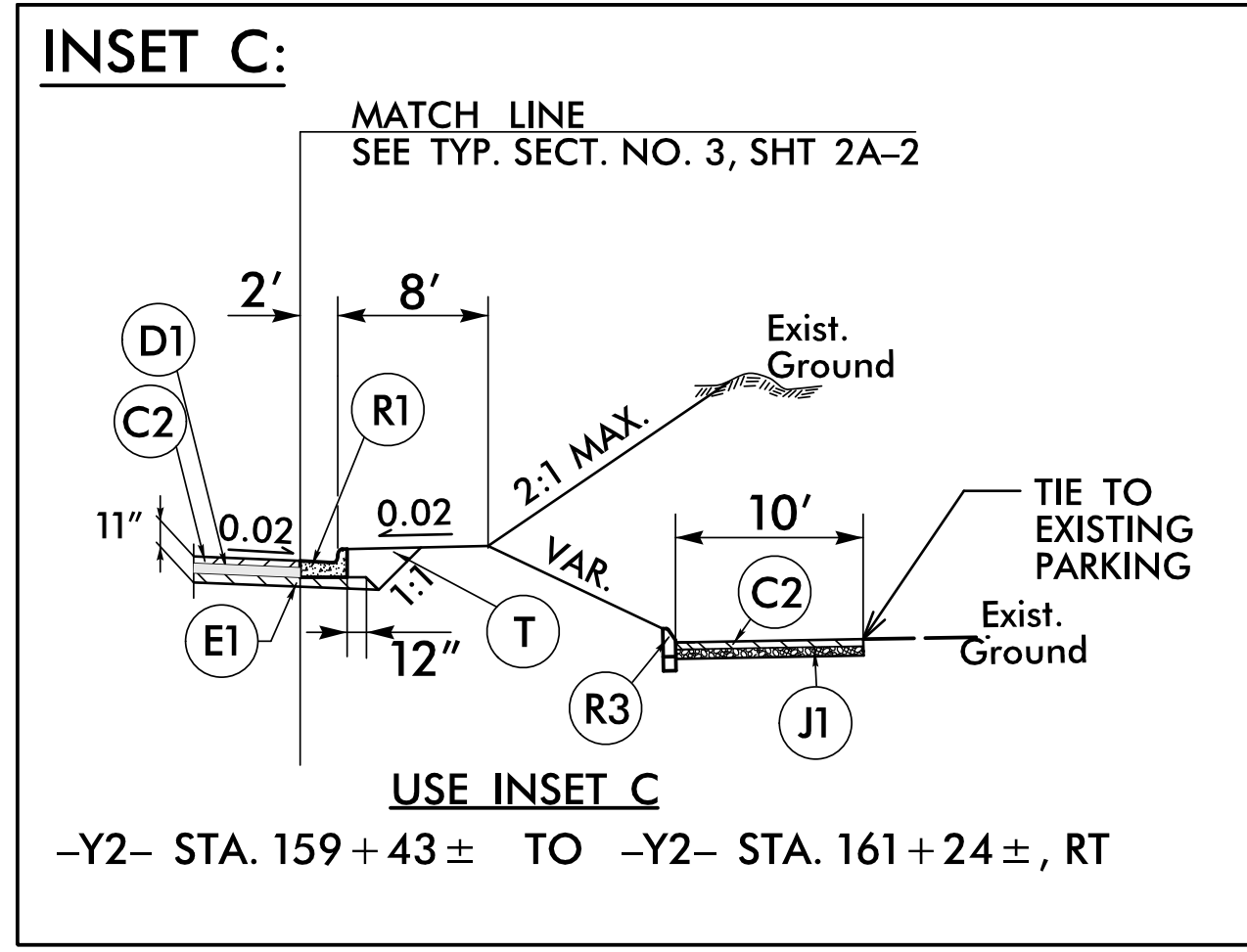
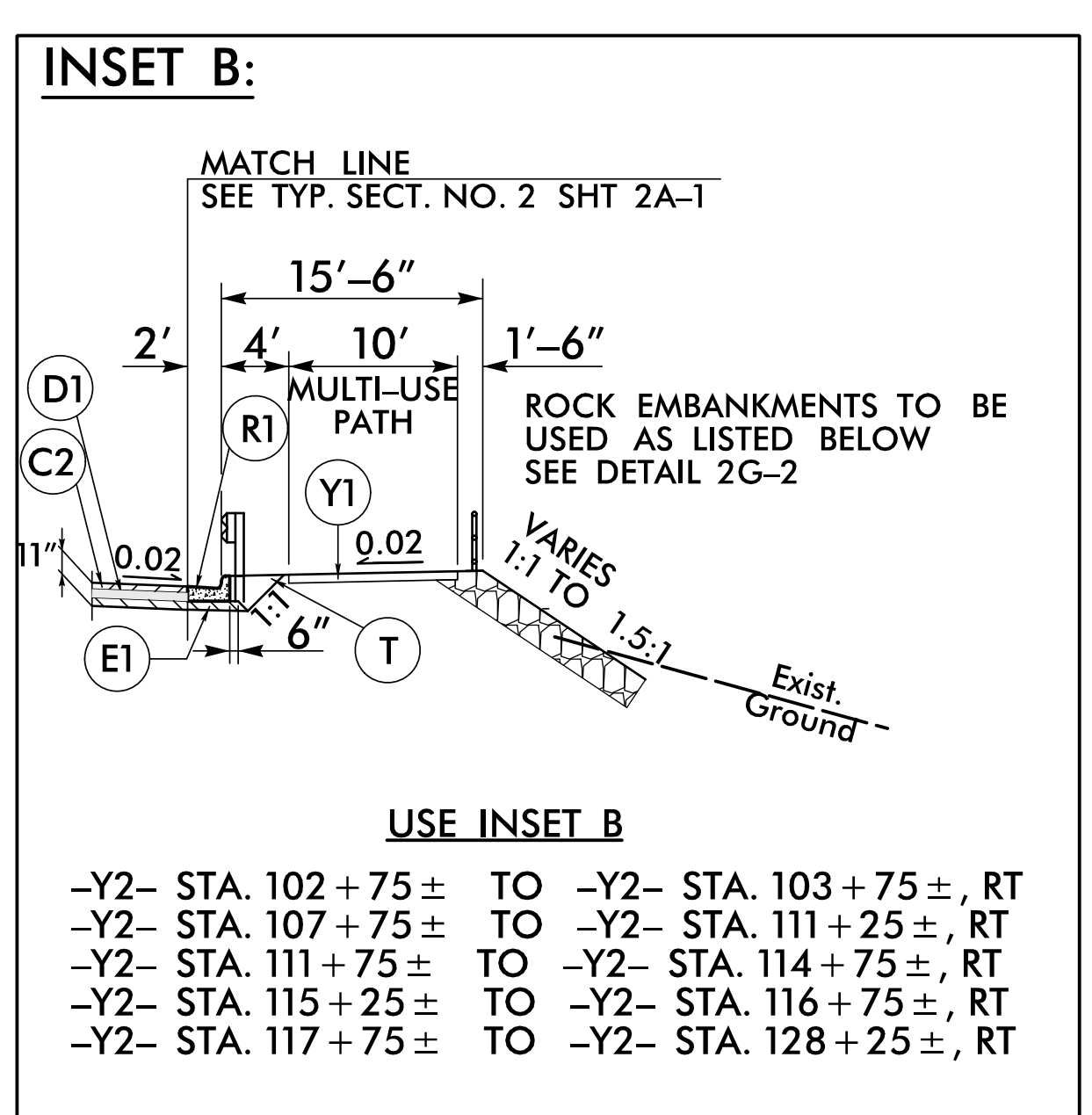
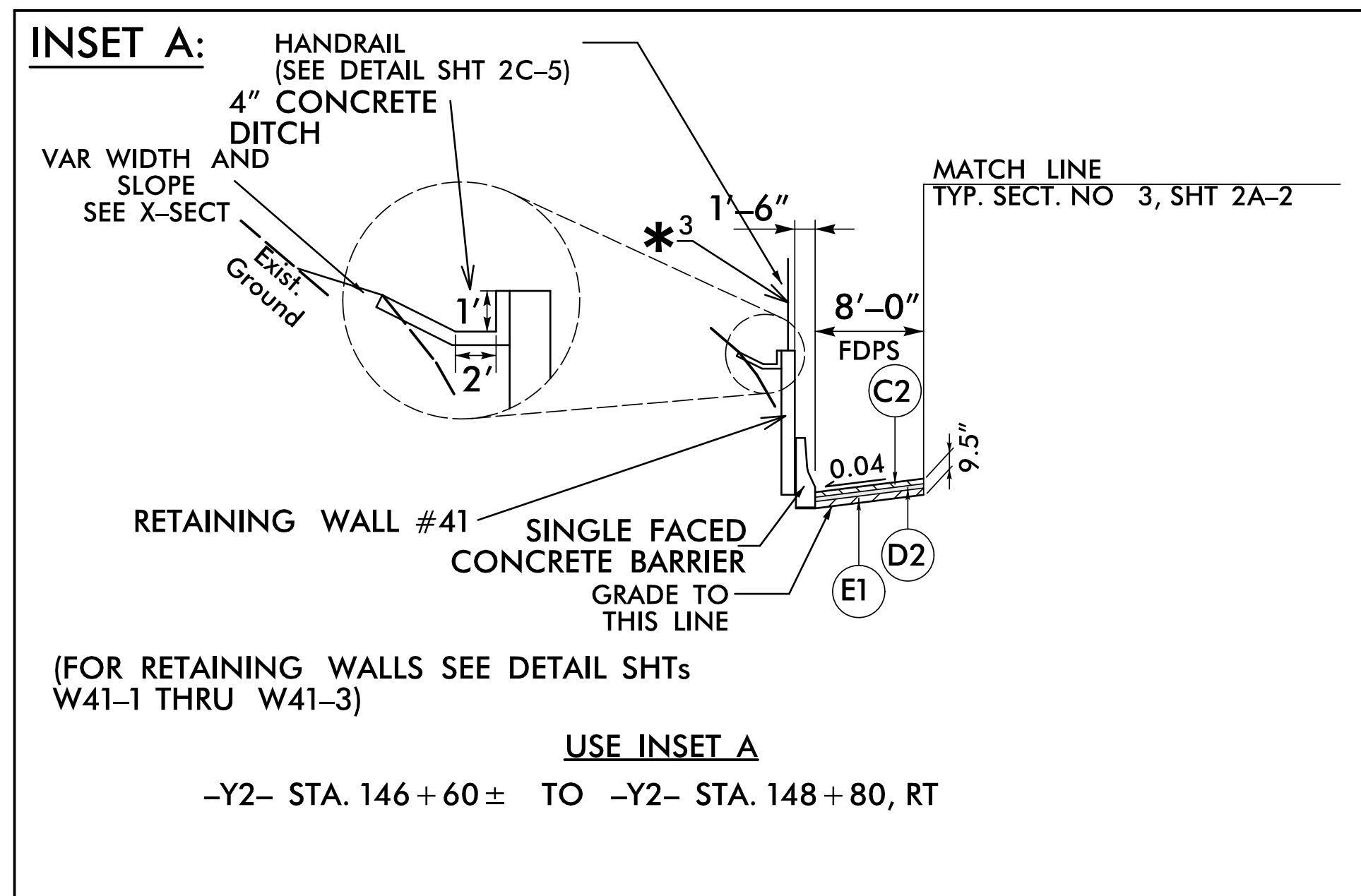
PROJECT REFERENCE NO. A-0009CD	SHEET NO. 2A-2
ROADWAY DESIGN ENGINEER JIMMY L. TERRY 35018	PAVEMENT DESIGN ENGINEER MATTHEW BRUMER 041986
DOCUMENT NOT CONSIDERED FINAL UNLESS ALL SIGNATURES COMPLETED	
TGS ENGINEERS 201 W. MARION ST SHELBY, NC 28150 PH (704) 476-0003 CORP. LICENSE NO.: C-0275	

2/20/2019
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6/2/2019

PAVEMENT SCHEDULE	
C2	3" S9.5C
D1	4" I19.0C
D2	2 1/2" I19.0C
E1	4" B25.0C
J1	6" ABC
R1	2'-6" C&G
R3	8" X 12" CONCRETE CURB
R4	SHOULDER BERM GUTTER
T	EARTH MATERIAL
Y1	4" CONC MULTI-USE PATH

PAVEMENT EDGE SLOPES ARE 1:1 UNLESS SHOWN OTHERWISE. SEE SHEET 2A-1 FOR DETAILED PAVEMENT SCHEDULE.

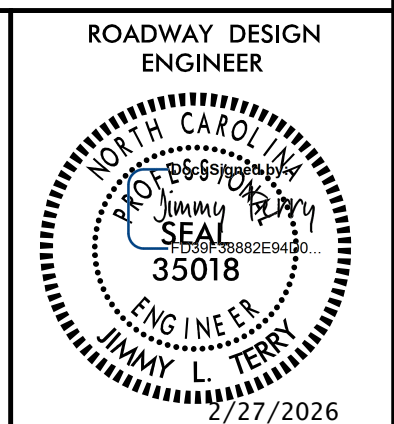


PROJECT REFERENCE NO. A-0009CD	SHEET NO. 2A-3
ROADWAY DESIGN ENGINEER	PAVEMENT DESIGN ENGINEER
DOCUMENT NOT CONSIDERED FINAL UNLESS ALL SIGNATURES COMPLETED	
TGS ENGINEERS 201 W. MARION ST SHELBY, NC 28150 PH (704) 476-0003 CORP. LICENSE NO.: C-0275	

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 User: jterry

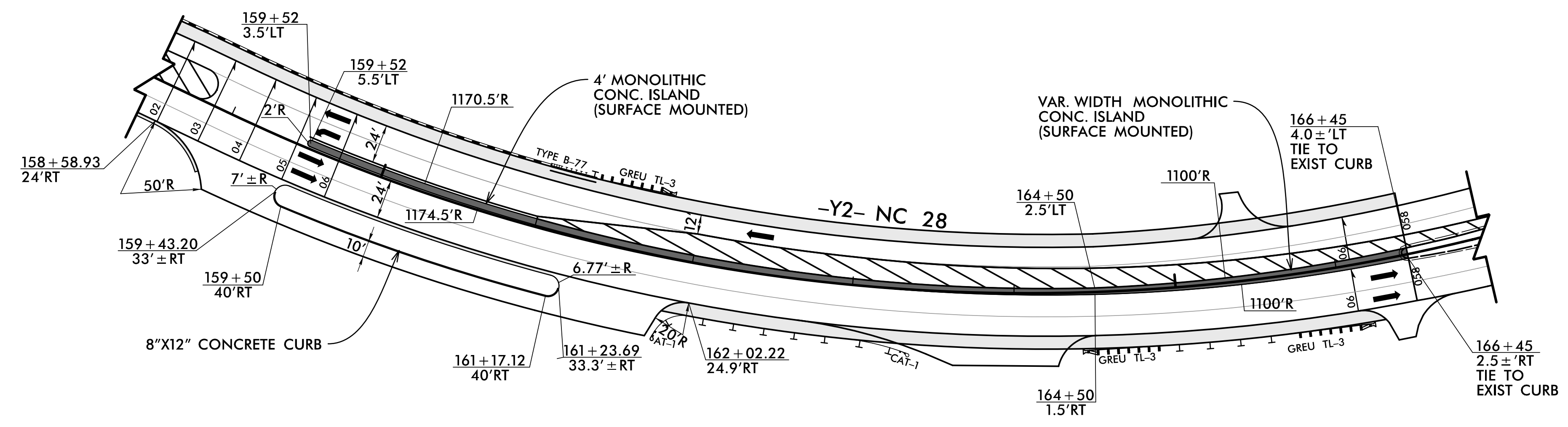
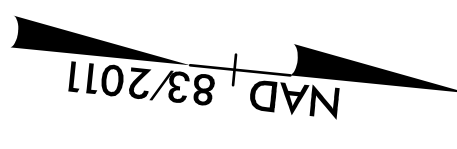
8/17/99

PROJECT REFERENCE NO.	SHEET NO.
A-0009CD	2B-1
RW SHEET NO.	



**DOCUMENT NOT CONSIDERED FINAL
UNLESS ALL SIGNATURES COMPLETED**

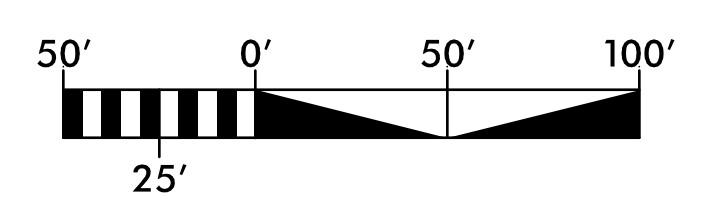
TGS ENGINEERS
201 W. MARION ST., STE 200
SHELBY, NC 28150
PH: (704) 476-0003
CORP. LICENSE NO.: C-0275



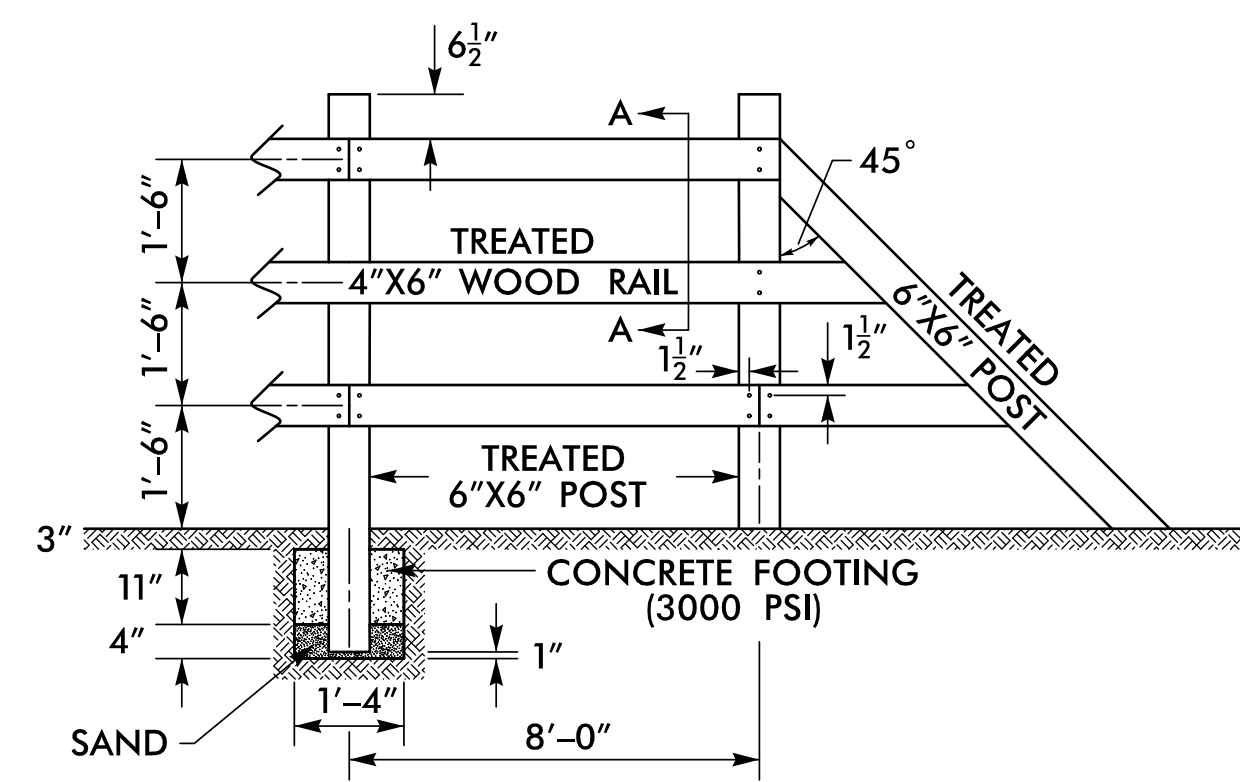
SEE PLAN SHEETS NO. 49 AND 50 FOR PLAN

REVISIONS

I:\7/2026\17/2026\A-0009\Roadway\Proj\A-0009CD_Plan Sheets\A-0009CD_Rdw_Detail_Sht_02B-1(Conc Island Layout).dgn

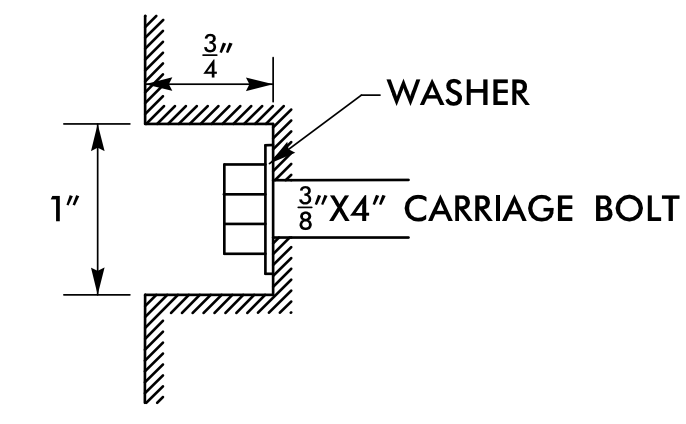
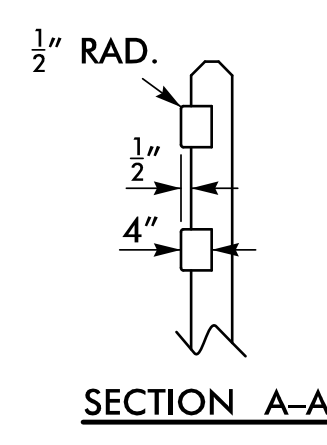


**LAYOUT OF MONOLITHIC CONCRETE ISLANDS,
8" CONCRETE CURB AND MISC. DRIVES**



TIMBER RAIL - DETAIL

NOTE: 1 1/4" NOTCH IN 6"X6" POSTS
 NOTE: 1 3/4" NOTCH IN 4"X6" RAIL

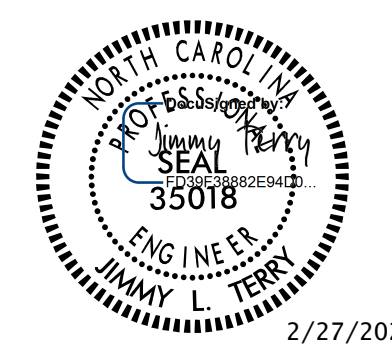


NOTE: BOLTS TO BE SPACED AS SHOWN

TIMBER RAIL TO BE CONSTRUCTED AT THE FOLLOWING LOCATIONS IN CONJUNCTION WITH THE TYPICAL SECTIONS AND THE DIRECTION OF THE ENGINEER.

- Y2- STA 98+97.00 TO -Y2- STA 105+65.00, RT (MIN. 1' FROM EDGE OF MULTI-USE PATH)
- Y2- STA 105+89.00 TO -Y2- STA 106+64.00, RT (MIN. 1' FROM EDGE OF MULTI-USE PATH)
- Y2- STA 106+88.00 TO -Y2- STA 111+49.00, RT (MIN. 1' FROM EDGE OF MULTI-USE PATH)
- Y2- STA 111+95.00 TO -Y2- STA 132+25.00, RT (MIN. 1' FROM EDGE OF MULTI-USE PATH)

I:\7\2026\A-0009CD\Roadway\Proj\A-0009CD\Plan Sheets\A-0009CD_Rdwy_Detail_Sht_02B-2\Timber Rail Fence at MUP.dgn
 User:smelvin



2/27/2026

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**TIMBER RAIL
 at MULTI-USE PATH**

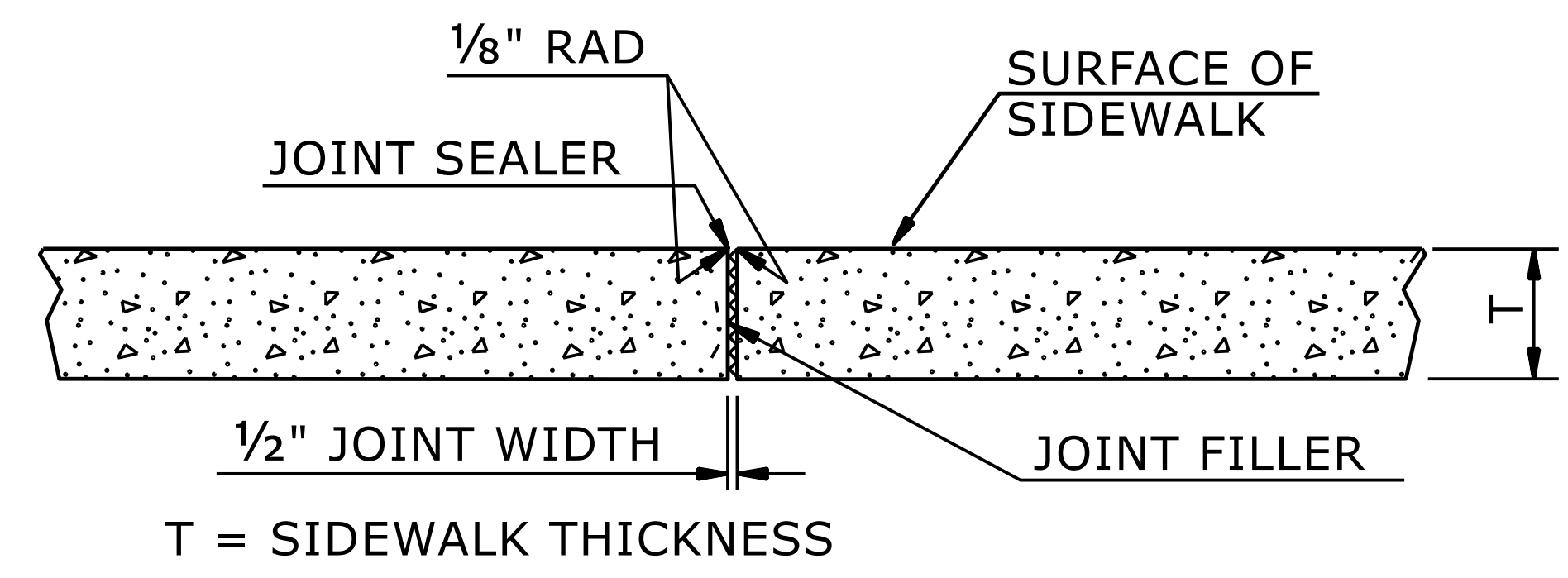
ORIGINAL BY: C.B. PRUETT DATE: JULY 2020
 MODIFIED BY: S. G. MELVIN DATE: JUNE 2022
 CHECKED BY: DATE:
 FILE SPEC.:

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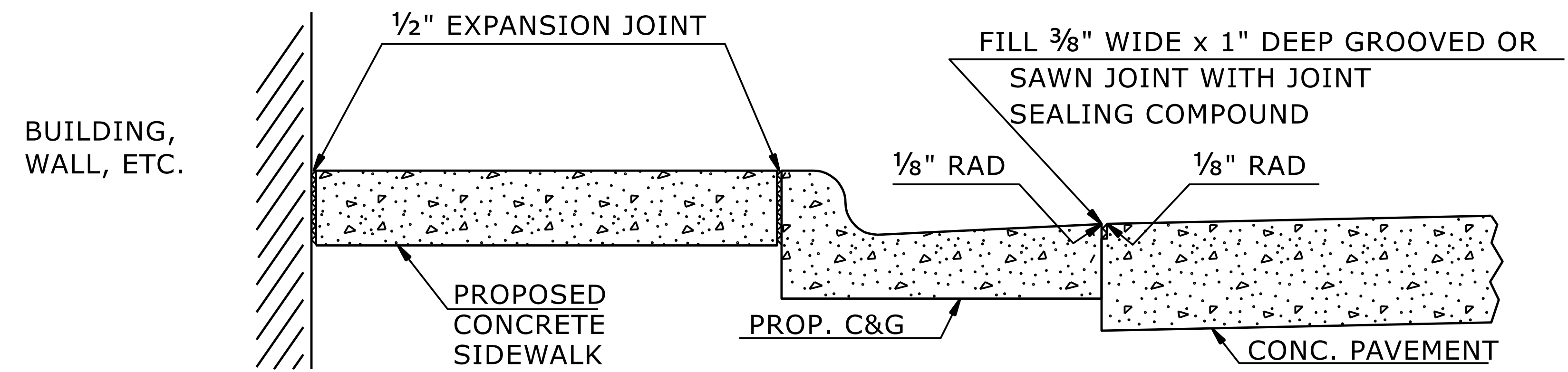
CONSTRUCT STANDARD SIDEWALK 5' WIDE AND 4" THICK UNLESS OTHERWISE DENOTED ON PLANS.

PLACE A GROOVE JOINT 1" DEEP WITH 1/8" RADII IN THE CONCRETE SIDEWALK AT 5' INTERVALS. ONE 1/2" EXPANSION JOINT WILL BE REQUIRED AT 50' INTERVALS. A 1/2" EXPANSION JOINT WILL BE REQUIRED WHERE THE SIDEWALK JOINS ANY RIGID STRUCTURE.

SEE STD. DWG. 848.06 FOR CURB RAMP LOCATION REQUIREMENTS AND CONSTRUCTION GUIDELINES.



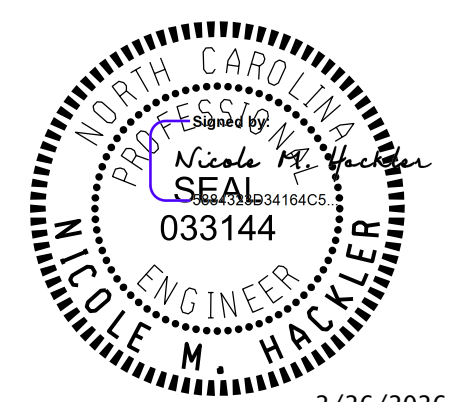
TRANSVERSE EXPANSION JOINT IN SIDEWALK



DETAILS SHOWING JOINTS IN CONCRETE SIDEWALK

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DIVISION OF HIGHWAYS
RALEIGH, N.C.

ROADWAY DETAIL DRAWING FOR
CONCRETE SIDEWALK



2/26/2026

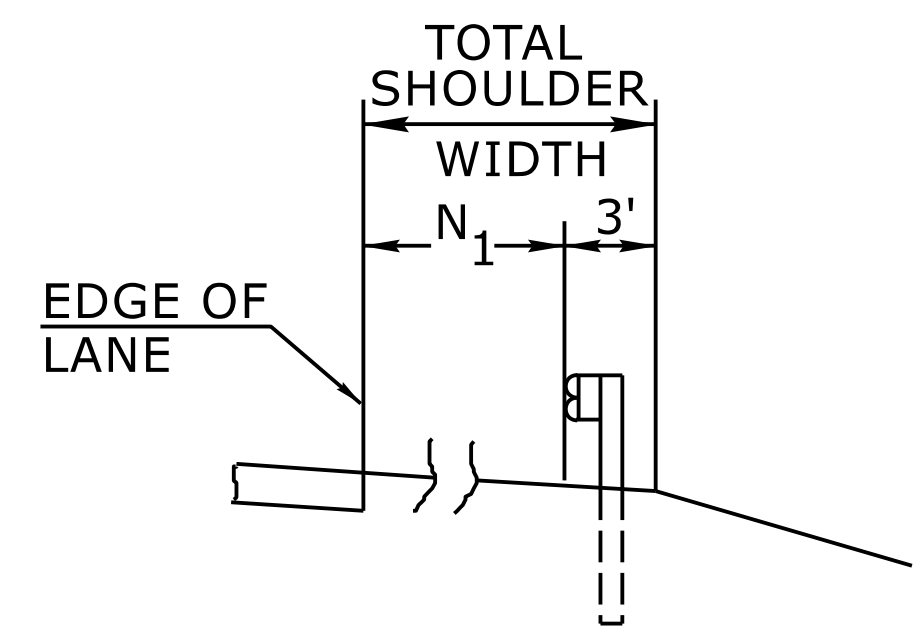
SHEET 1 OF 1
848D01

DOCUMENT NOT CONSIDERED FINAL
UNLESS ALL SIGNATURES COMPLETED

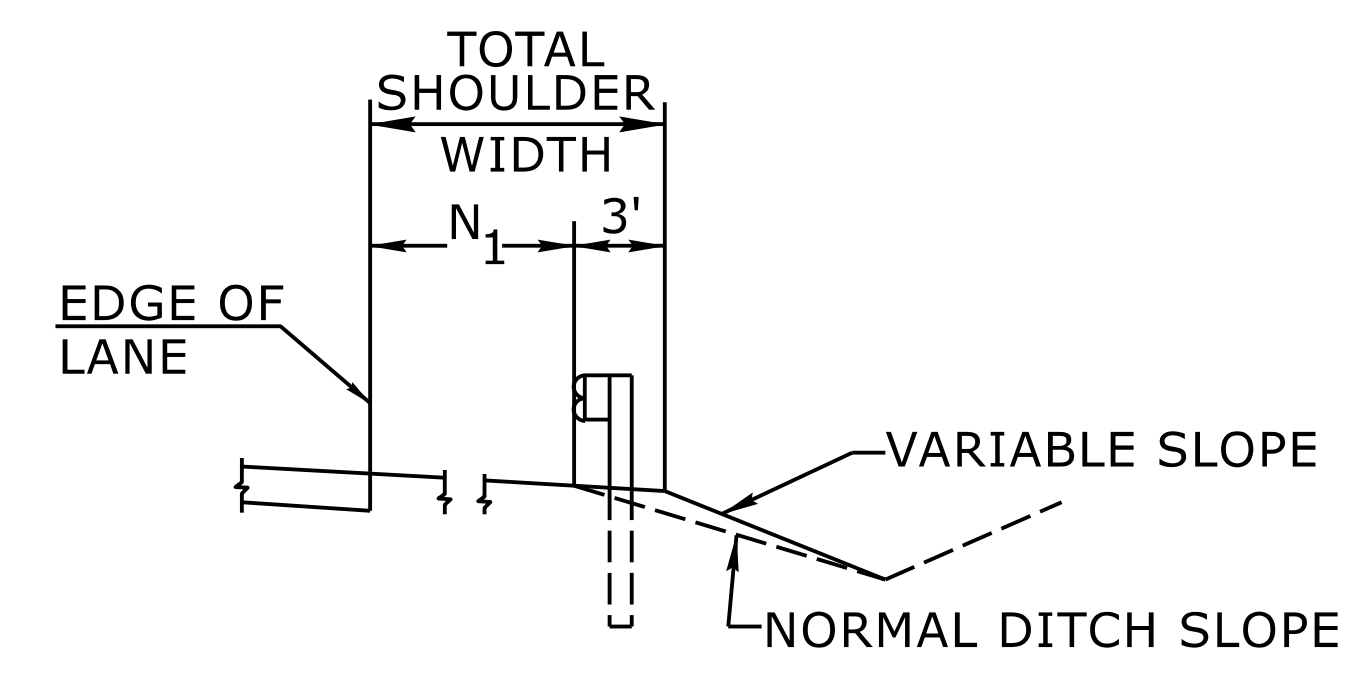
**CONTRACTS STANDARDS
AND DEVELOPMENT UNIT**
Office 919-707-6950 FAX 919-250-4119

SEE TITLE BLOCK

ORIGINAL BY: S.CALHOUN	DATE: 7-25-2024
MODIFIED BY: _____	DATE: _____
CHECKED BY: _____	DATE: _____
FILE SPEC.: _____	

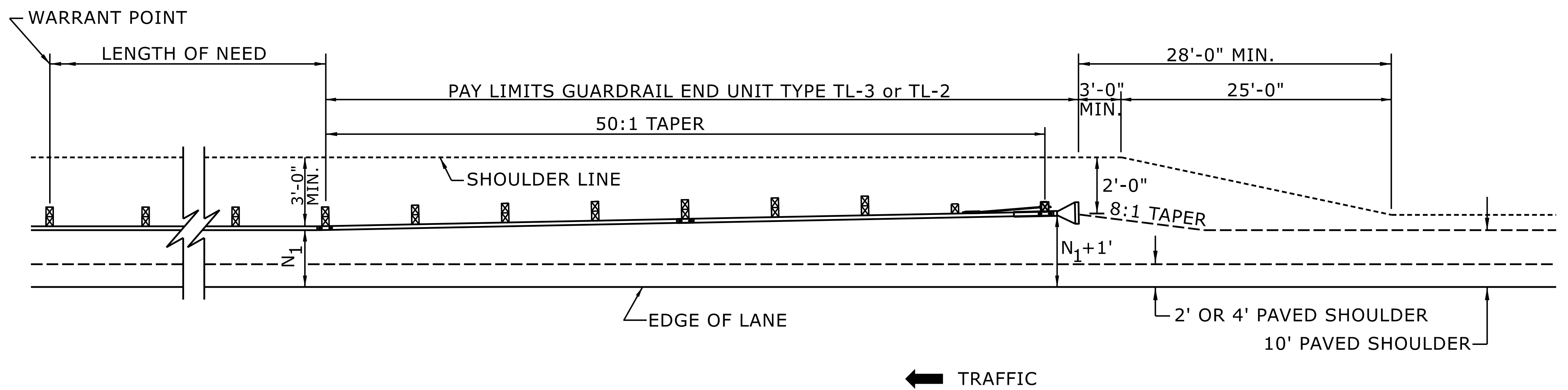


FILL SECTION



CUT SECTION

"N₁" = DISTANCE FROM EDGE OF LANE TO FACE OF GUARDRAIL WHERE GUARDRAIL IS PARALLEL TO LANE.

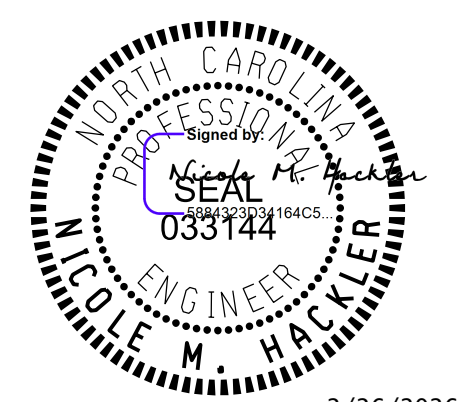


FOR POSTED SPEEDS ≥ 45mph USE GREU TYPE TL-3
FOR POSTED SPEEDS < 45mph USE GREU TYPE TL-2

DETAIL OF BEGINNING OF GUARDRAIL IN CUT OR FILL SECTION

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DIVISION OF HIGHWAYS
RALEIGH, N.C.

ROADWAY DETAIL DRAWING FOR
GUARDRAIL PLACEMENT



2/26/2026

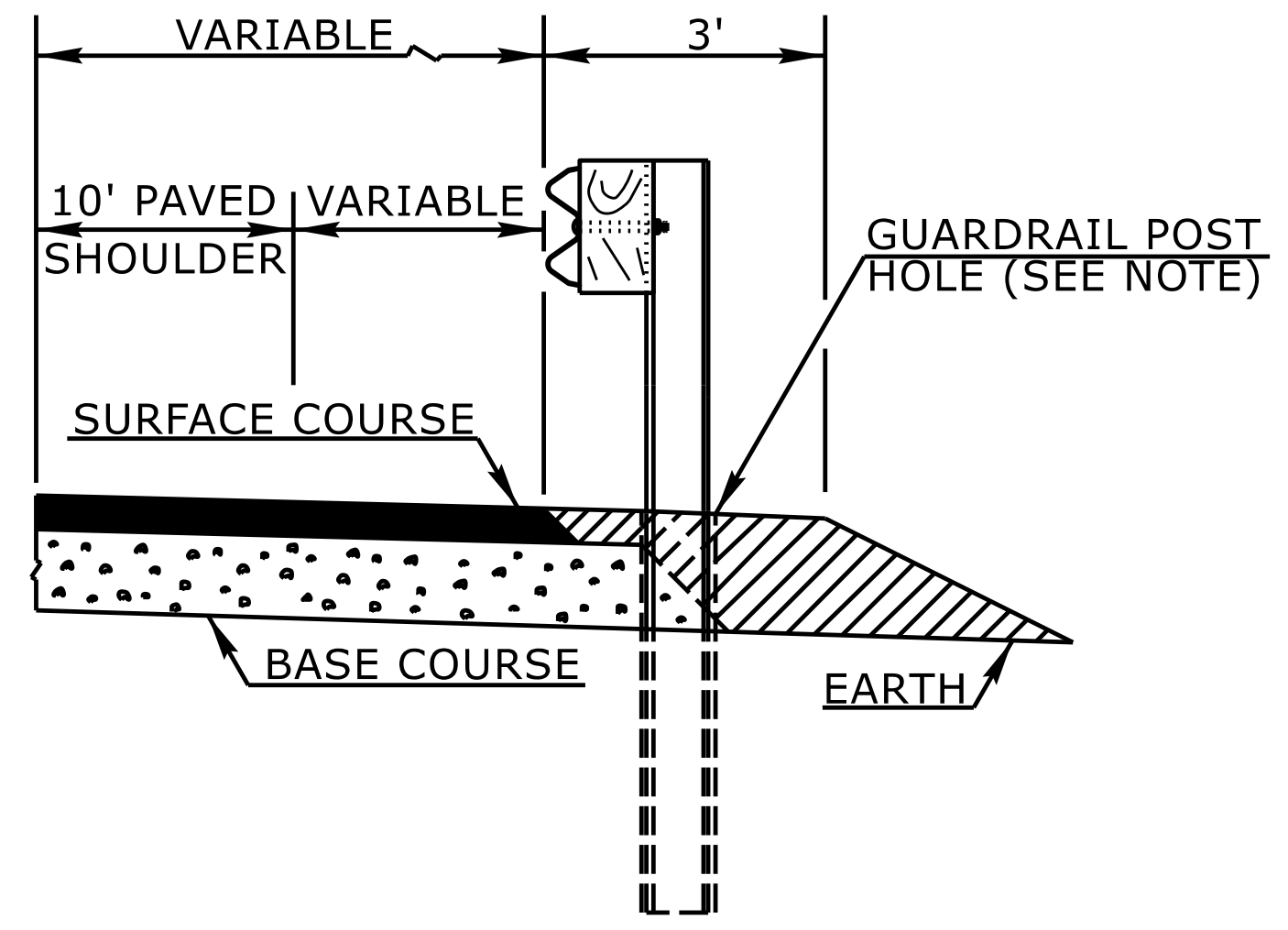
SHEET 6 OF 15
862D01

DOCUMENT NOT CONSIDERED FINAL
UNLESS ALL SIGNATURES COMPLETED

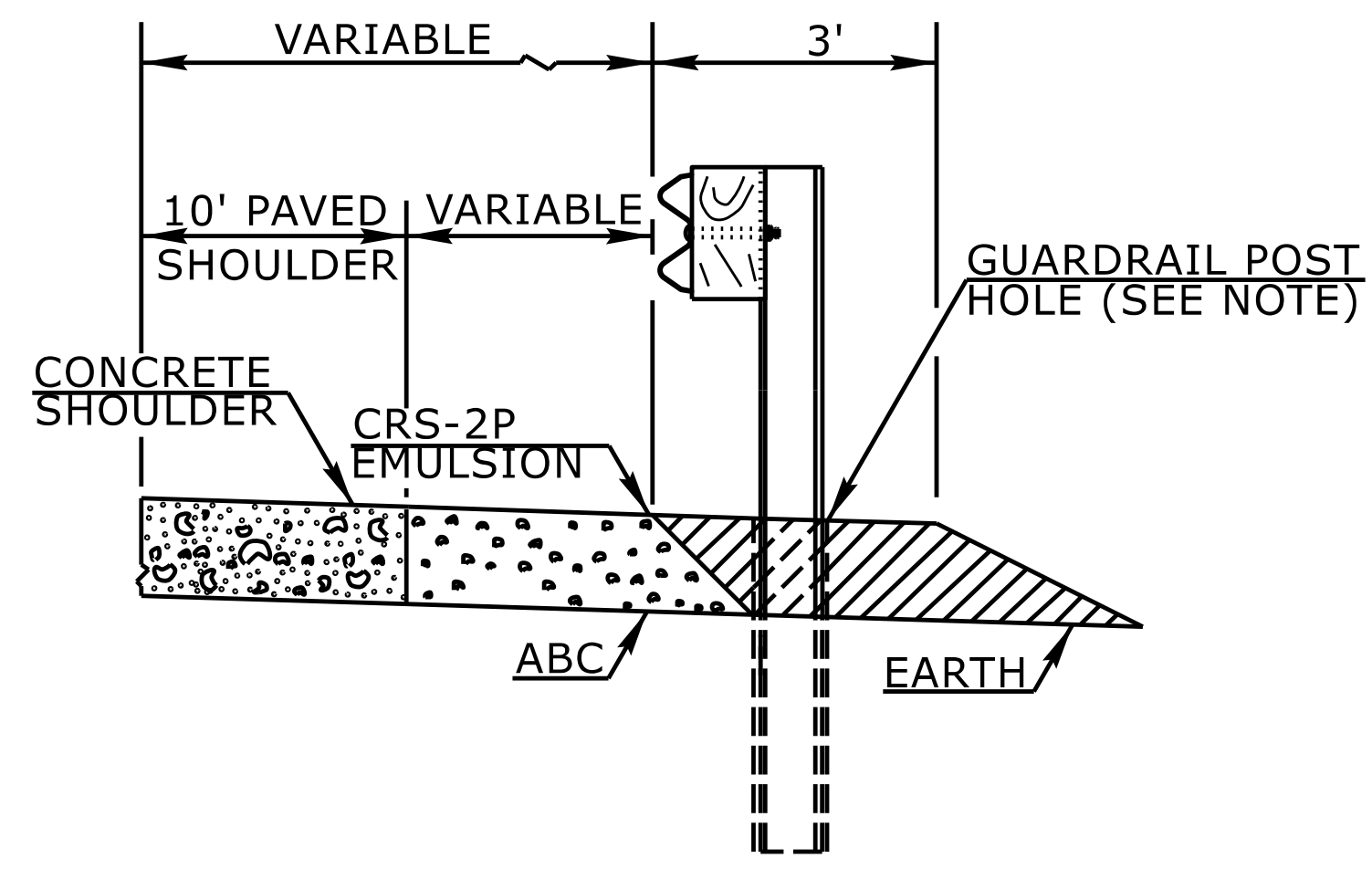
**CONTRACTS STANDARDS
AND DEVELOPMENT UNIT**
Office 919-707-6950 FAX 919-250-4119

SEE TITLE BLOCK

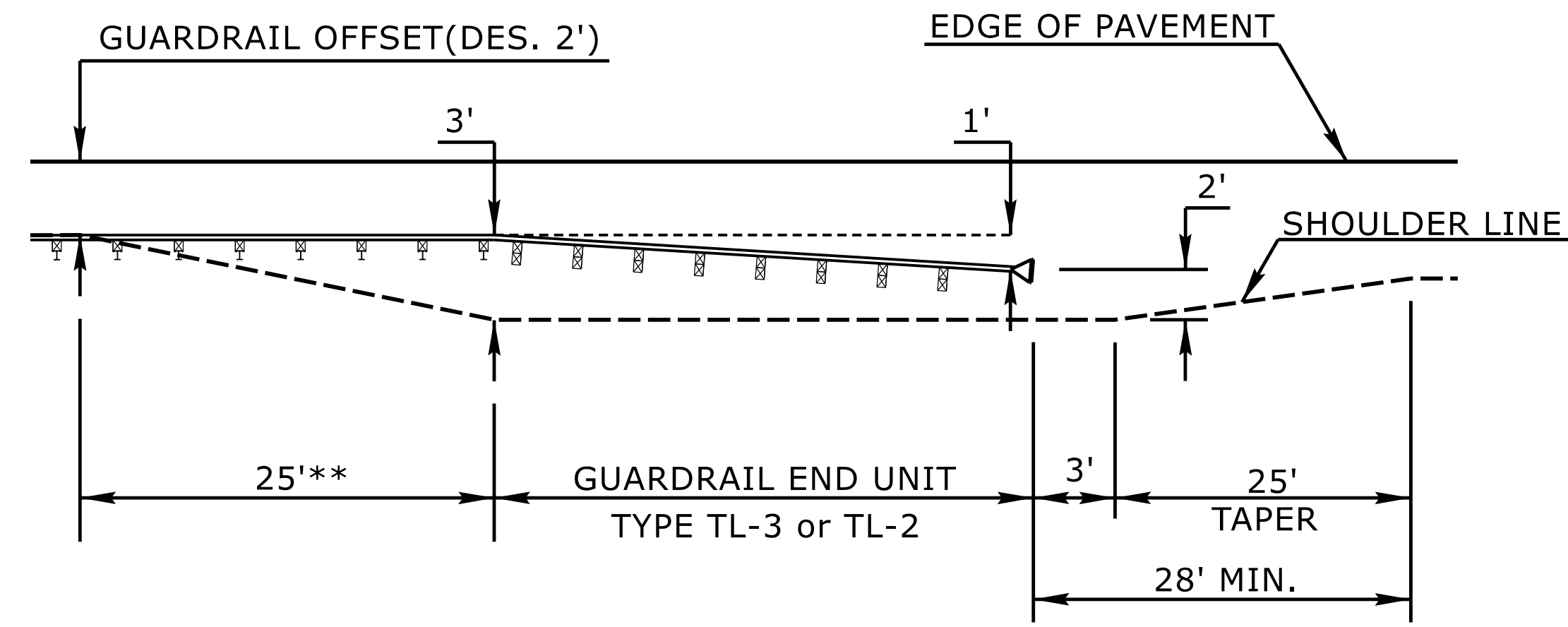
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MODIFIED BY:	DATE:
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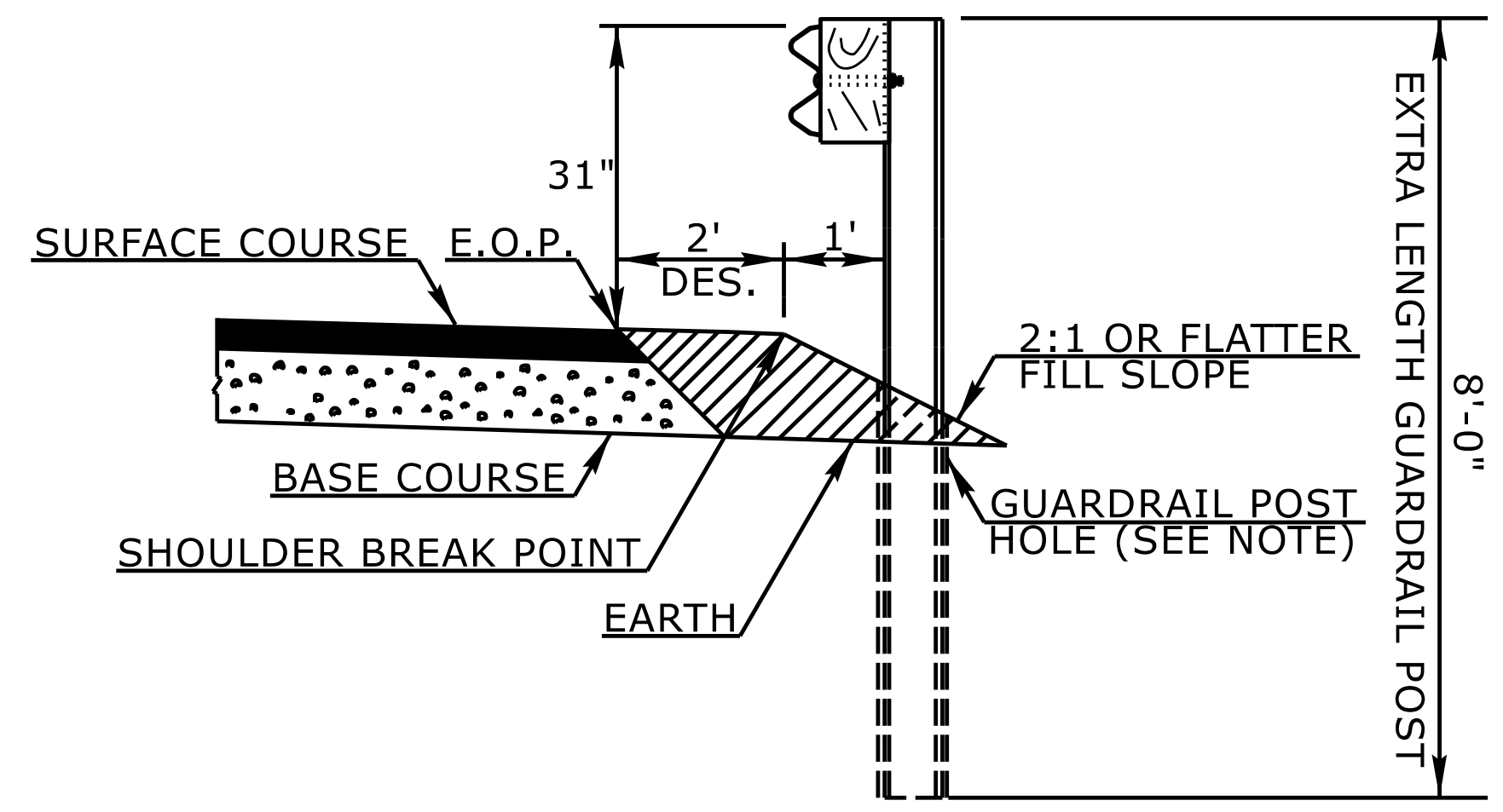
FLEXIBLE PAVED SHOULDER



CONCRETE PAVED SHOULDER



**8' GUARDRAIL POST ON 2:1 SLOPE-END UNIT TRANSITION*
PLAN VIEW**



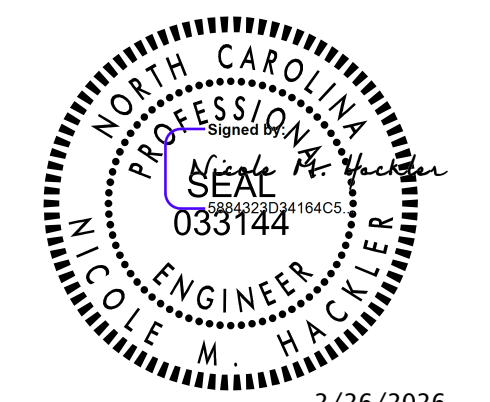
8' GUARDRAIL POST ON 2:1 SLOPE*

* THE 8' GUARDRAIL POST ON 2:1 SLOPE DETAIL IS INTENDED FOR USE ONLY IN SEVERELY CONSTRAINED AREAS WITH A POSTED SPEED ≤ 60 MPH. GUARDRAIL END UNITS MAY NOT BE PLACED ON THE 2:1 SLOPE AND MUST TRANSITION TO THE SHOULDER.
** 8' GUARDRAIL POST SHOULD BE USED IN THIS RANGE

NOTE:
WHEN WOODEN GUARDRAIL POSTS ARE USED, DRILL HOLES THROUGH EARTH MATERIAL AND BASE COURSE. THE POST MAY THEN BE DRIVEN TO THE PROPER DEPTH. DRILL THE HOLE OF SUFFICIENT SIZE TO ACCOMMODATE THE PARTICULAR POST BEING USED. BACKFILL AND TAMP HOLES USING THE EXCAVATED MATERIAL.

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RALEIGH, N.C.

ROADWAY DETAIL DRAWING FOR
GUARDRAIL PLACEMENT



2/26/2026

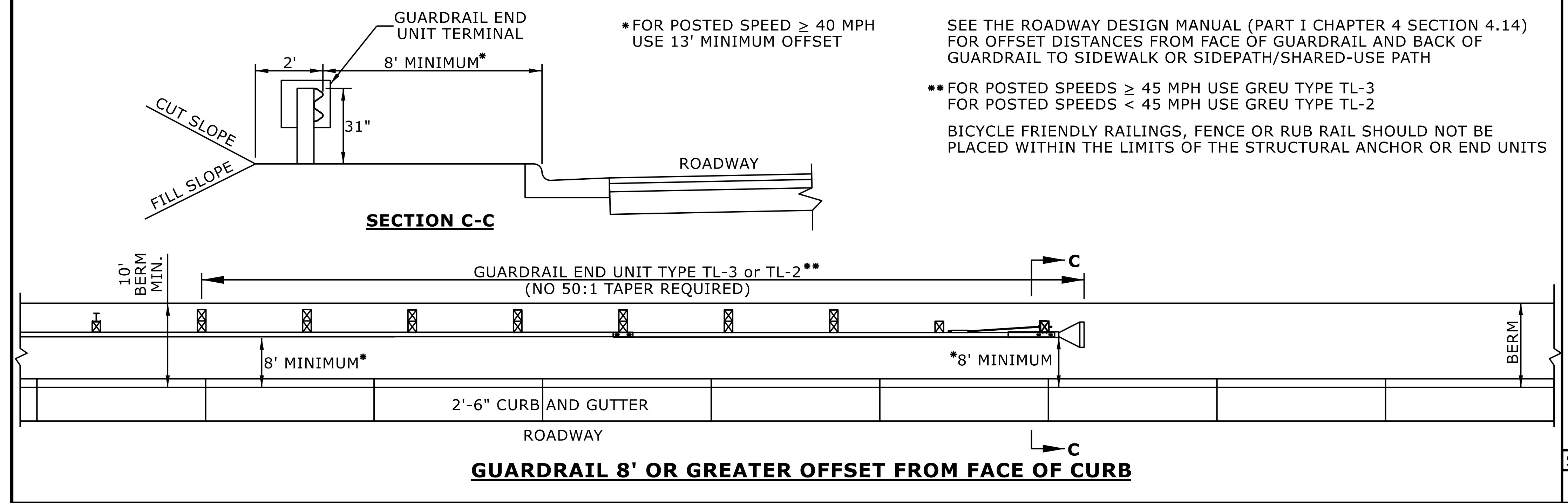
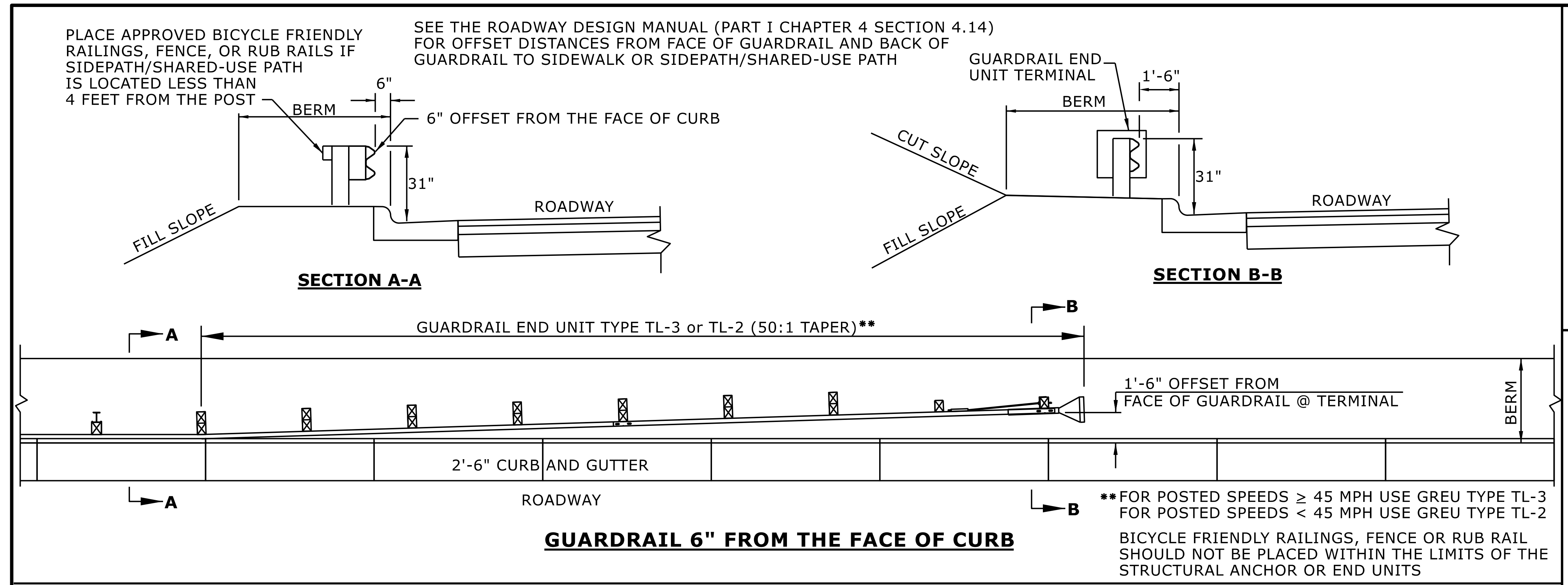
SHEET 11 OF 15
862D01

DOCUMENT NOT CONSIDERED FINAL
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**CONTRACTS STANDARDS
AND DEVELOPMENT UNIT**
Office 919-707-6950 FAX 919-250-4119

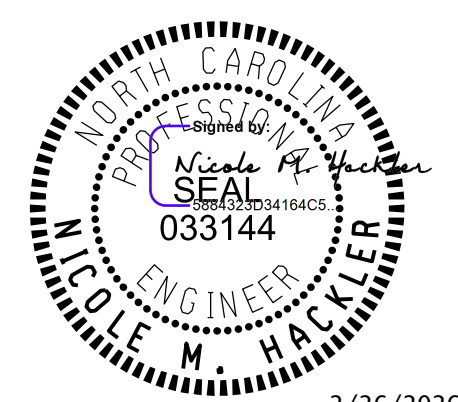
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ORIGINAL BY: L.SMITH DATE: 10-14-2025
MODIFIED BY: DATE:
CHECKED BY: DATE:
FILE SPEC.: DATE:



STATE OF NORTH CAROLINA
DEPT. OF TRANSPORTATION
DIVISION OF HIGHWAYS
RALEIGH, N.C.

ROADWAY DETAIL DRAWING FOR
GUARDRAIL PLACEMENT
GUARDRAIL TREATMENT AT CURB AND GUTTER



2/26/2026

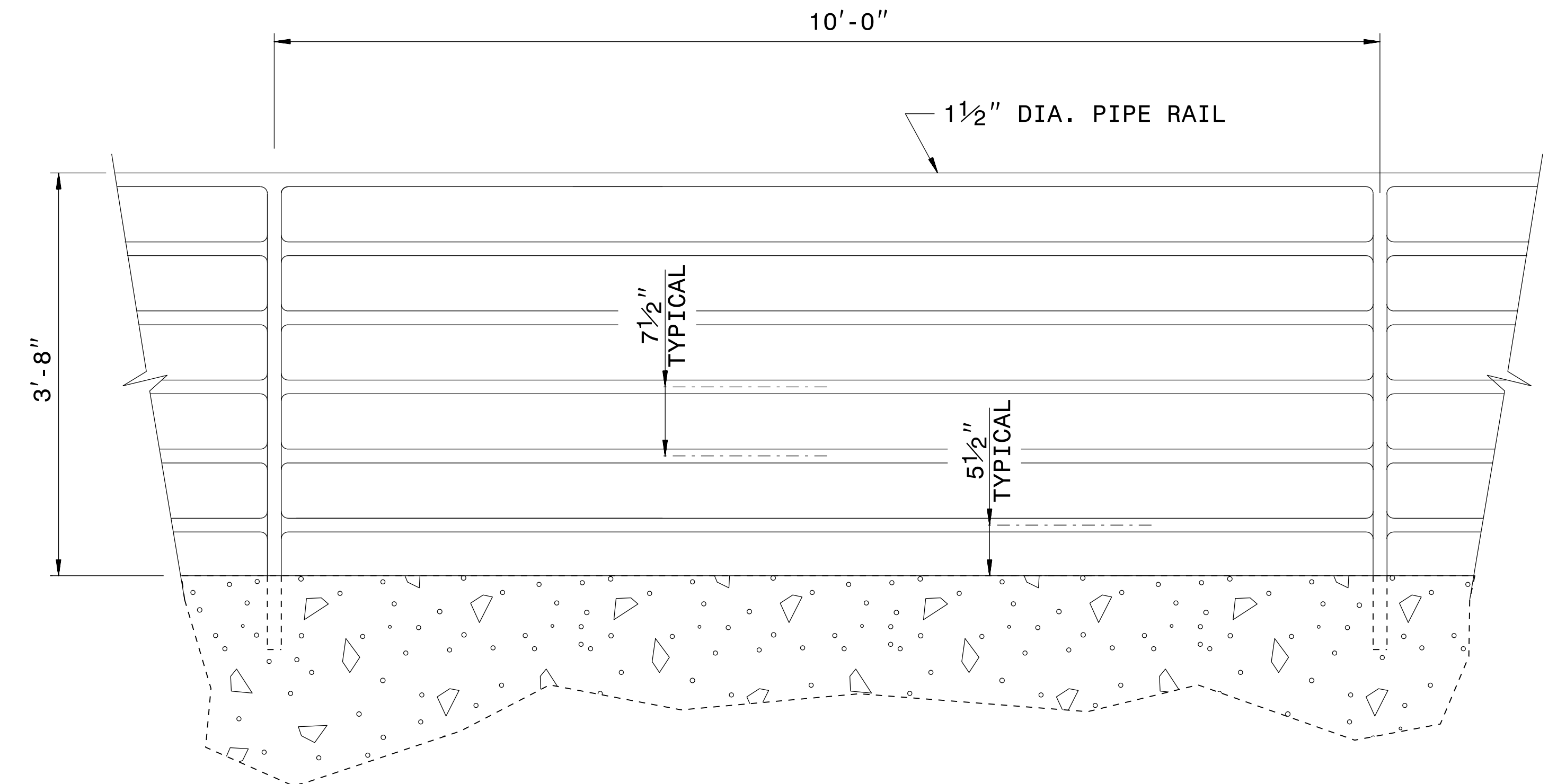
SHEET 12 OF 15
862D01

DOCUMENT NOT CONSIDERED FINAL
UNLESS ALL SIGNATURES COMPLETED

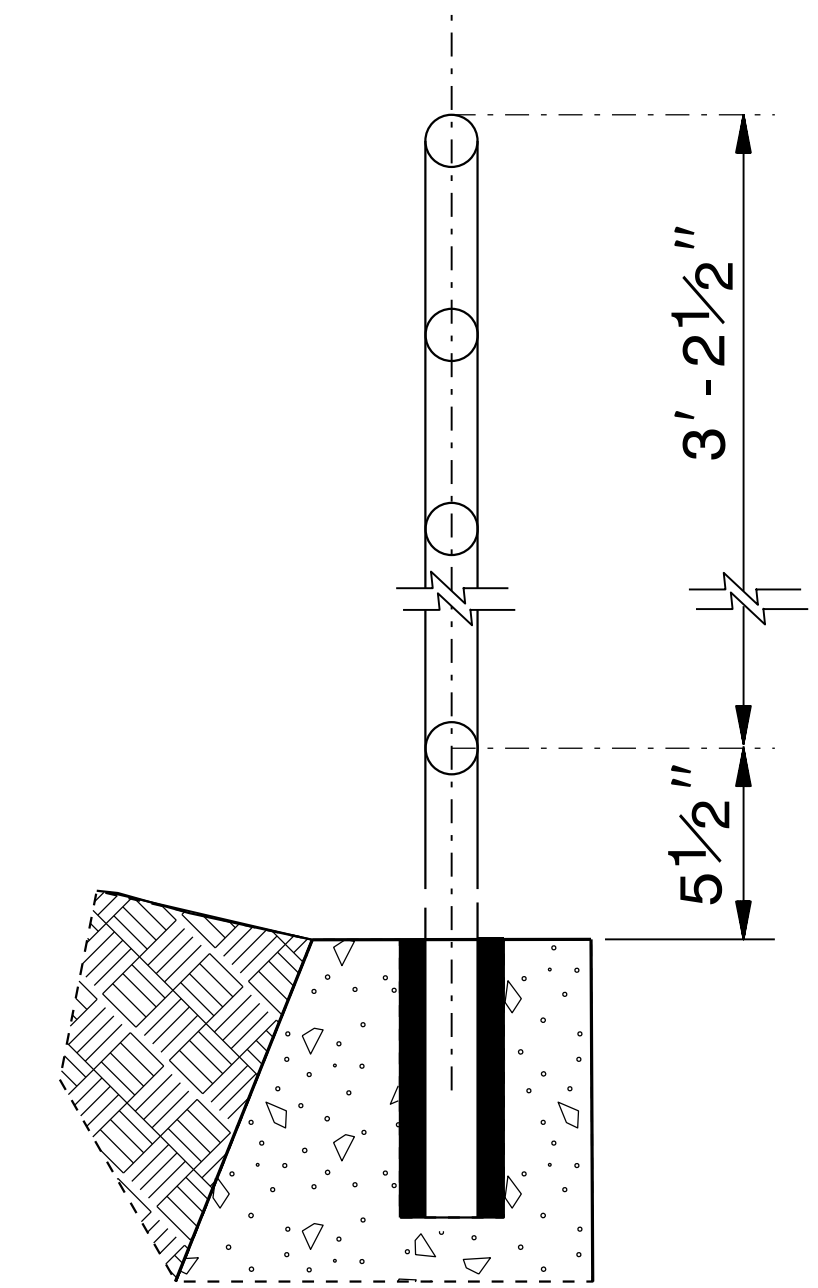
CONTRACTS STANDARDS AND DEVELOPMENT UNIT
Office 919-707-6950 FAX 919-250-4119

SEE TITLE BLOCK

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MODIFIED BY: _____ DATE: _____
CHECKED BY: _____ DATE: _____
FILE SPEC.: _____



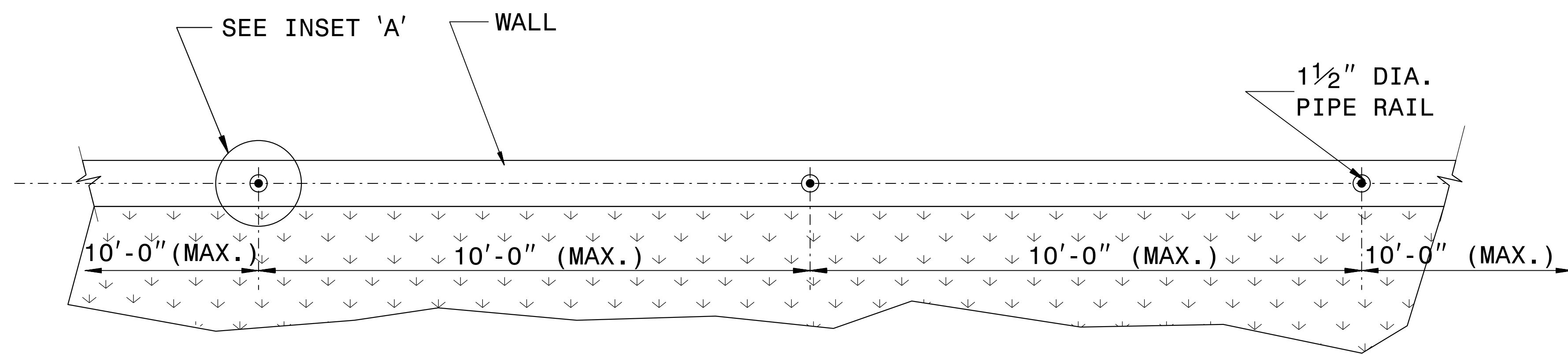
ELEVATION OF HANDRAIL



INSET 'A'

NOTES:

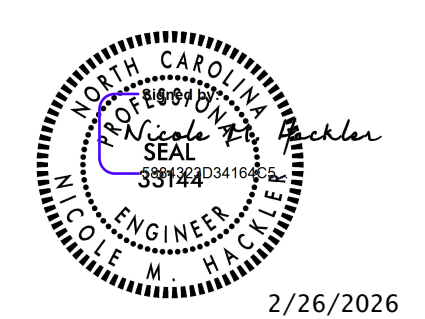
- CONSTRUCT PROPOSED STEEL PIPE RAIL 1 1/2" DIAMETER SCHEDULE 40 PLAIN END GALVANIZED STEEL PIPE MEETING THE REQUIREMENTS OF ASTM A53.
- EMBED PIPE RAIL INTO PROPOSED WALL WITH CHEMICAL OR CONCRETE GROUT ANCHORING SYSTEM PER THE WALL MANUFACTURER'S RECOMMENDATIONS.
- REPAIR GALVANIZING IN ACCORDANCE WITH SECTION 1076 OF THE NCDOT STANDARD SPECIFICATIONS.
- PAINT, IF REQUIRED BY THE ENGINEER, IN ACCORDANCE WITH SECTION 1080 OF THE STANDARD SPECIFICATIONS.
- CENTER THE PROPOSED RAILING ON TOP OF THE WALL WITH POST SPACING SYMMETRICAL ABOUT THE CENTER-LINE OF THE WALL.
- WELD IN ACCORDANCE WITH ARTICLE 1072-18 OF THE STANDARD SPECIFICATIONS.
- SUBMIT THE ATTACHMENT OF THE HANDRAIL TO THE RETAINING WALL TO THE CONTRACTS AND STANDARDS OFFICE FOR APPROVAL.



PLAN VIEW

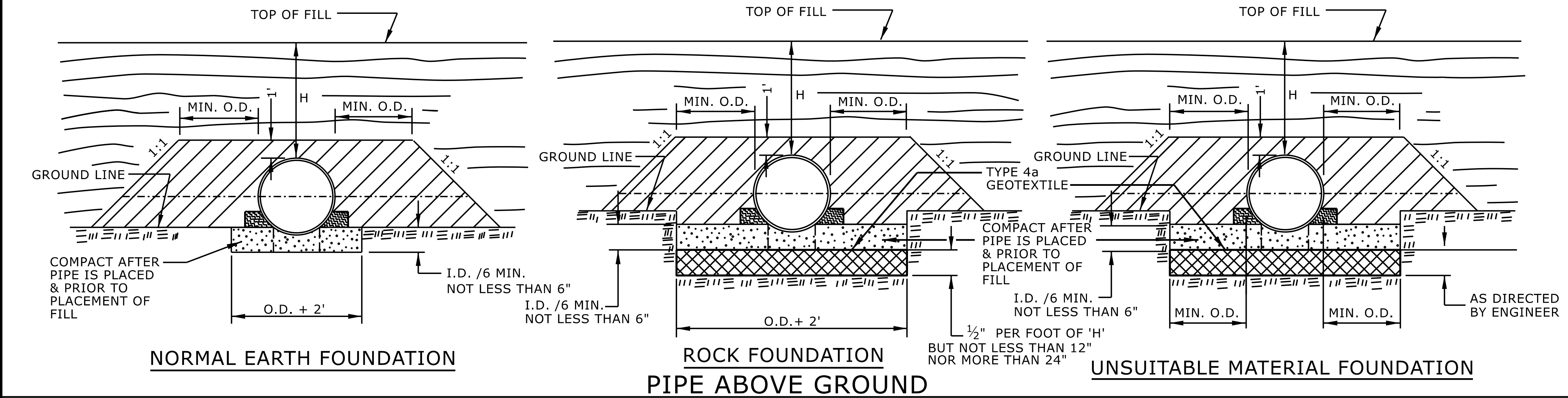
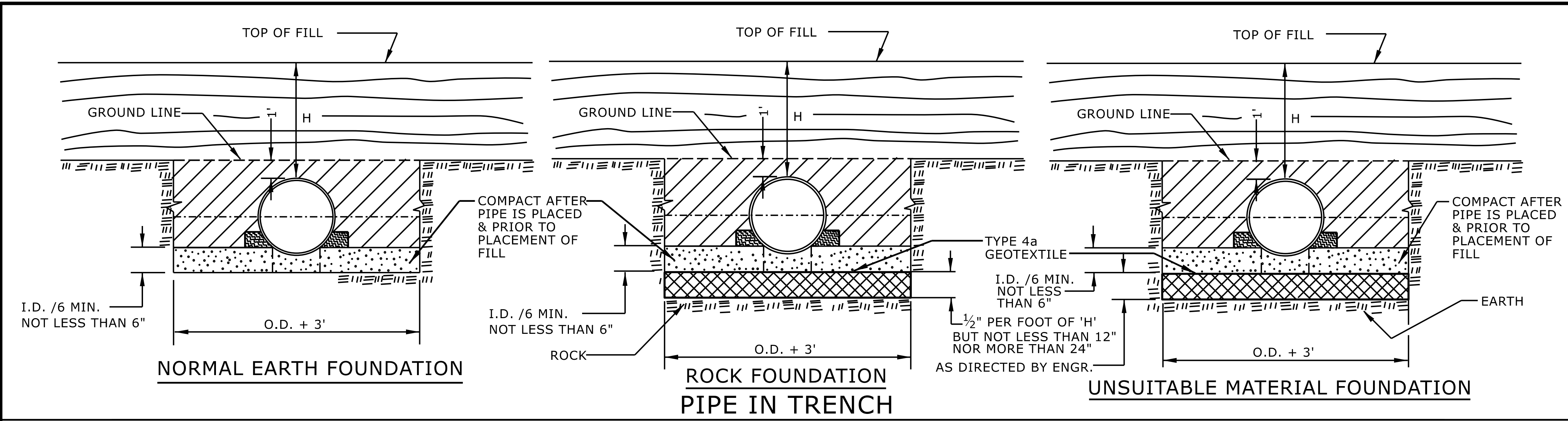
24-MAY-2018 14:10 S:\Contracts\Special Details\Howerton\Handrail on Retaining Wall.dgn jhowerton AT USD-292595

DOCUMENT NOT CONSIDERED FINAL UNLESS ALL SIGNATURES COMPLETED



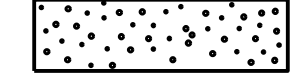


2/26/2026

CONTRACT STANDARDS AND DEVELOPMENT UNIT	
Office 919-707-6950 FAX 919-250-4119	
DETAIL OF PIPE HANDRAIL MOUNTED ON A WALL	
ORIGINAL BY: E.E. WARD	DATE: 12-99
MODIFIED BY:	DATE:
CHECKED BY:	DATE:
FILE SPEC.: jhowerton/handrail_on_retaining_wall.dgn	

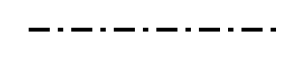
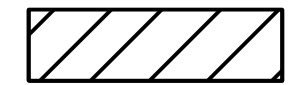
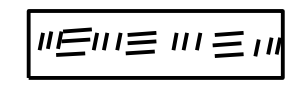



GENERAL NOTES:
 I.D. = THE MAXIMUM HORIZONTAL INSIDE DIAMETER DIMENSION.
 O.D. = THE MAXIMUM HORIZONTAL OUTSIDE DIAMETER DIMENSION.
 H = THE FILL HEIGHT MEASURED VERTICALLY AT ANY POINT ALONG THE PIPE FROM THE TOP OF THE PIPE TO THE TOP OF THE EMBANKMENT AT THAT POINT.

 APPROVED SUITABLE LOCAL MATERIAL.
 TAKE CARE TO FULLY COMPACT HAUNCH ZONE OF PIPE BACKFILL.
 LOOSELY PLACED SELECT MATERIAL CLASS III OR CLASS II, TYPE 1 FOR PIPE BEDDING. LEAVE SECTION DIRECTLY BENEATH PIPE UNCOMPACTED AS PIPE SEATING AND BACKFILL WILL ACCOMPLISH COMPACTION.

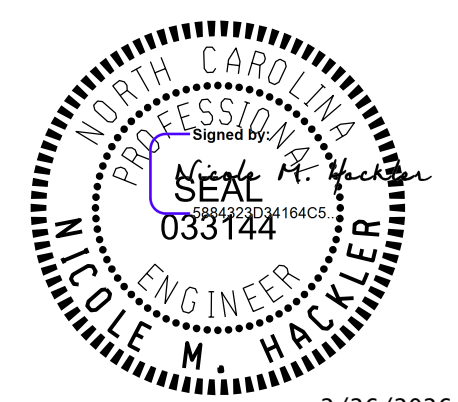
DO NOT OPERATE HEAVY EQUIPMENT OVER ANY PIPE CULVERT UNTIL THE PIPE CULVERT HAS BEEN PROPERLY BACKFILLED AND COVERED WITH AT LEAST 3 FEET OF APPROVED MATERIAL.

REFER TO NCDOT PIPE MATERIAL SELECTION GUIDE AND STANDARD SPECIFICATIONS FOR ALLOWABLE PIPE FILL HEIGHTS AND PIPE SPECIFICATIONS.

 SPRINGLINE OF PIPE
 SELECT BACKFILL MATERIAL CLASS III OR CLASS II, TYPE 1 ABOVE AND BELOW SPRINGLINE.
 UNDISTURBED EARTH MATERIAL
 SELECT MATERIAL CLASS V OR VI FOR FOUNDATION CONDITIONING. ENCAPSULATE WITH TYPE IV GEOTEXTILE AS DIRECTED BY THE ENGINEER.

STATE OF NORTH CAROLINA
DEPT. OF TRANSPORTATION
DIVISION OF HIGHWAYS
RALEIGH, N.C.

ROADWAY DETAIL DRAWING FOR
METHOD OF PIPE INSTALLATION
FLEXIBLE PIPE



2/26/2026

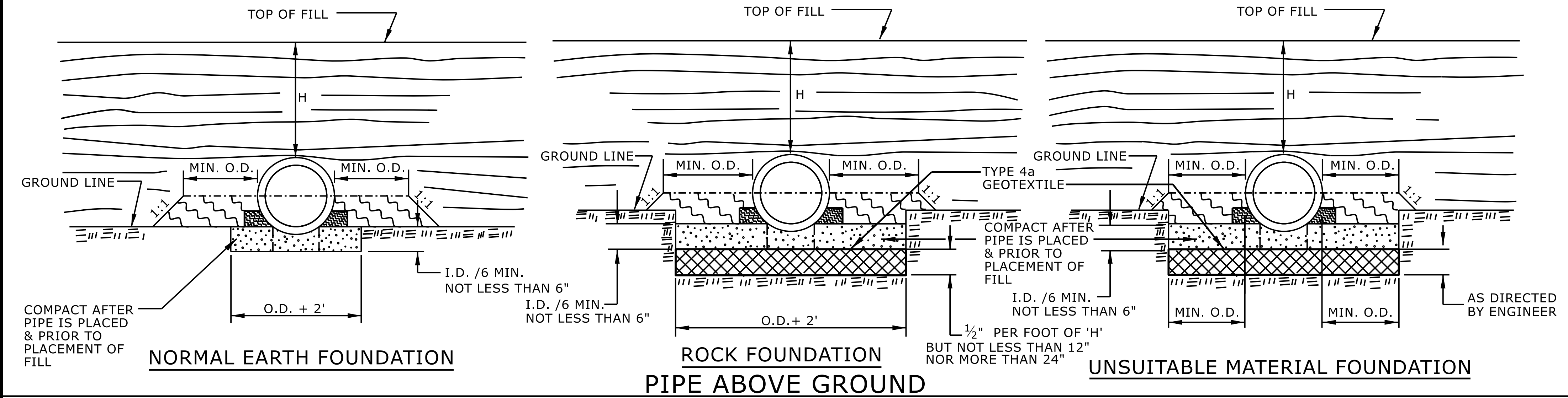
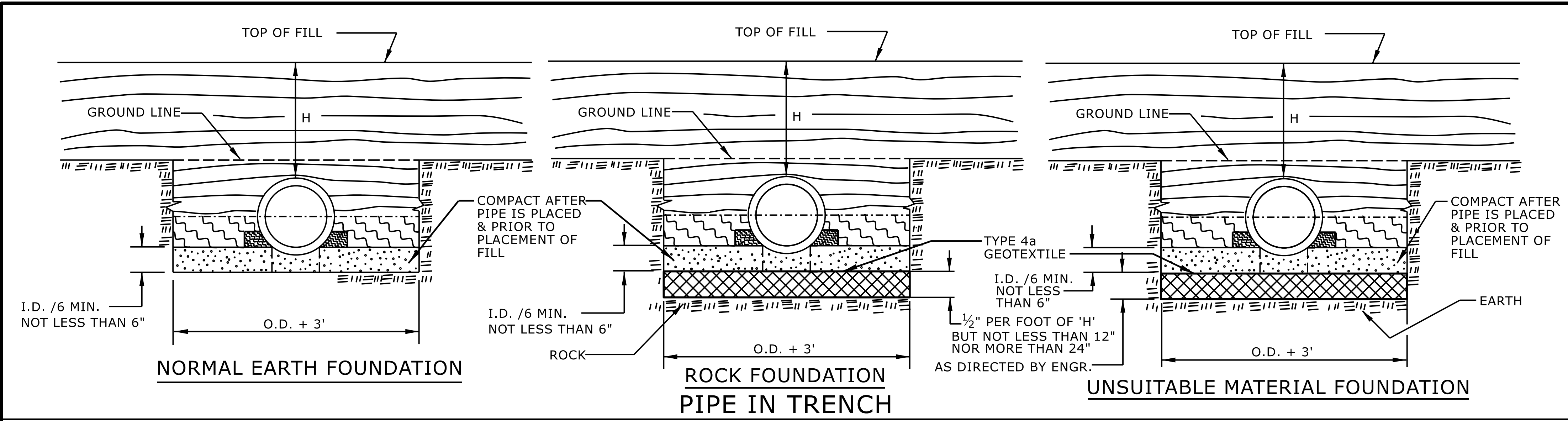
SHEET 1 OF 2
300.01

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UNLESS ALL SIGNATURES COMPLETED



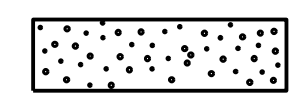
**CONTRACTS STANDARDS
AND DEVELOPMENT UNIT**
Office 919-707-6950 FAX 919-250-4119

SEE TITLE BLOCK

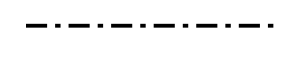

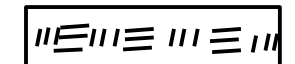

ORIGINAL BY: S.CALHOUN DATE: 7-25-2024
MODIFIED BY: DATE: _____
CHECKED BY: DATE: _____
FILE SPEC: _____



GENERAL NOTES:
 I.D. = THE MAXIMUM HORIZONTAL INSIDE DIAMETER DIMENSION.
 O.D. = THE MAXIMUM HORIZONTAL OUTSIDE DIAMETER DIMENSION.
 H = THE FILL HEIGHT MEASURED VERTICALLY AT ANY POINT ALONG THE PIPE FROM THE TOP OF THE PIPE TO THE TOP OF THE EMBANKMENT AT THAT POINT.

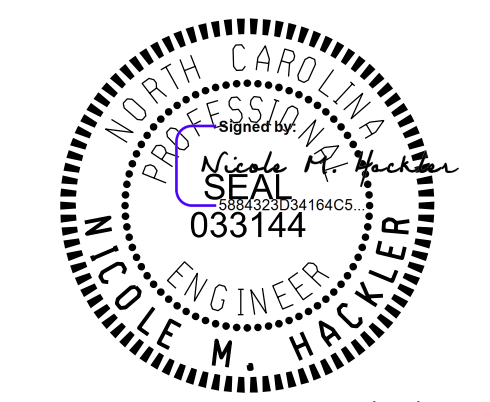
 APPROVED SUITABLE LOCAL MATERIAL.
 TAKE CARE TO FULLY COMPACT HAUNCH ZONE OF PIPE BACKFILL.
 LOOSELY PLACED SELECT MATERIAL CLASS III OR CLASS II, TYPE 1 FOR PIPE BEDDING. LEAVE SECTION DIRECTLY BENEATH PIPE UNCOMPACTED AS PIPE SEATING AND BACKFILL WILL ACCOMPLISH COMPACTION.

DO NOT OPERATE HEAVY EQUIPMENT OVER ANY PIPE CULVERT UNTIL THE PIPE CULVERT HAS BEEN PROPERLY BACKFILLED AND COVERED WITH AT LEAST 3 FEET OF APPROVED MATERIAL.
 REFER TO NCDOT PIPE MATERIAL SELECTION GUIDE AND STANDARD SPECIFICATIONS FOR ALLOWABLE PIPE FILL HEIGHTS AND PIPE SPECIFICATIONS.

-  SPRINGLINE OF PIPE
-  SELECT BACKFILL MATERIAL CLASS III OR CLASS II, BELOW SPRINGLINE.
-  UNDISTURBED EARTH MATERIAL
-  SELECT MATERIAL CLASS V OR VI FOR FOUNDATION CONDITIONING. ENCAPSULATE WITH TYPE IV GEOTEXTILE AS DIRECTED BY THE ENGINEER.

STATE OF NORTH CAROLINA
 DEPT. OF TRANSPORTATION
 DIVISION OF HIGHWAYS
 RALEIGH, N.C.

ROADWAY DETAIL DRAWING FOR
METHOD OF PIPE INSTALLATION
 RIGID PIPE



2/26/2026

SHEET 2 OF 2
300.01

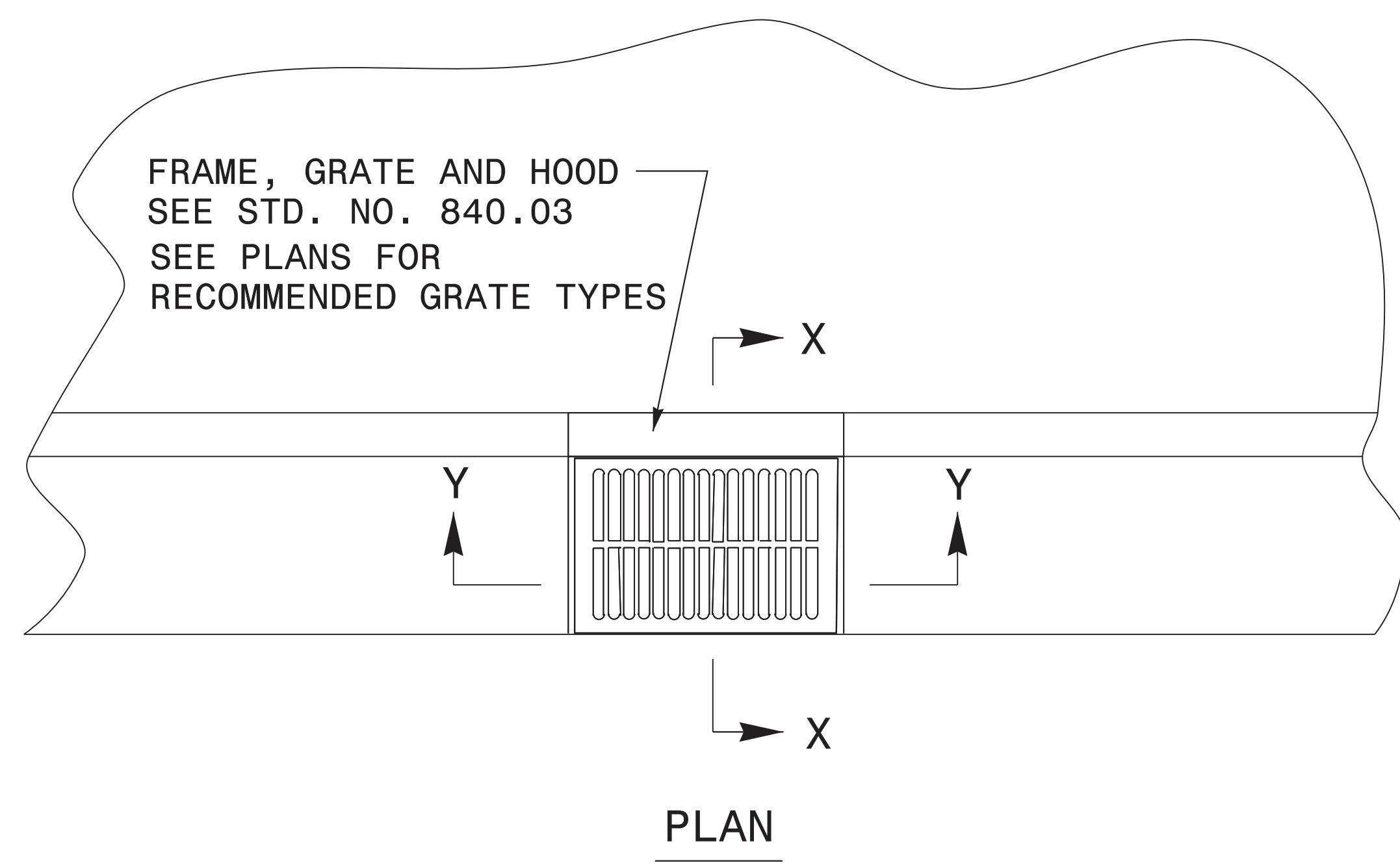
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CONTRACTS STANDARDS AND DEVELOPMENT UNIT
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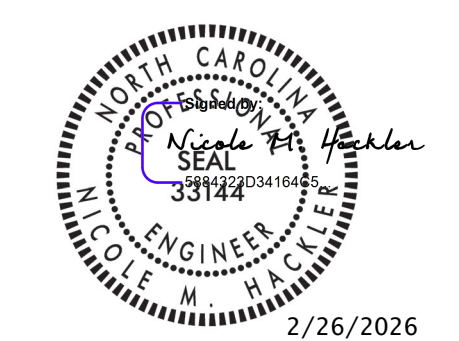
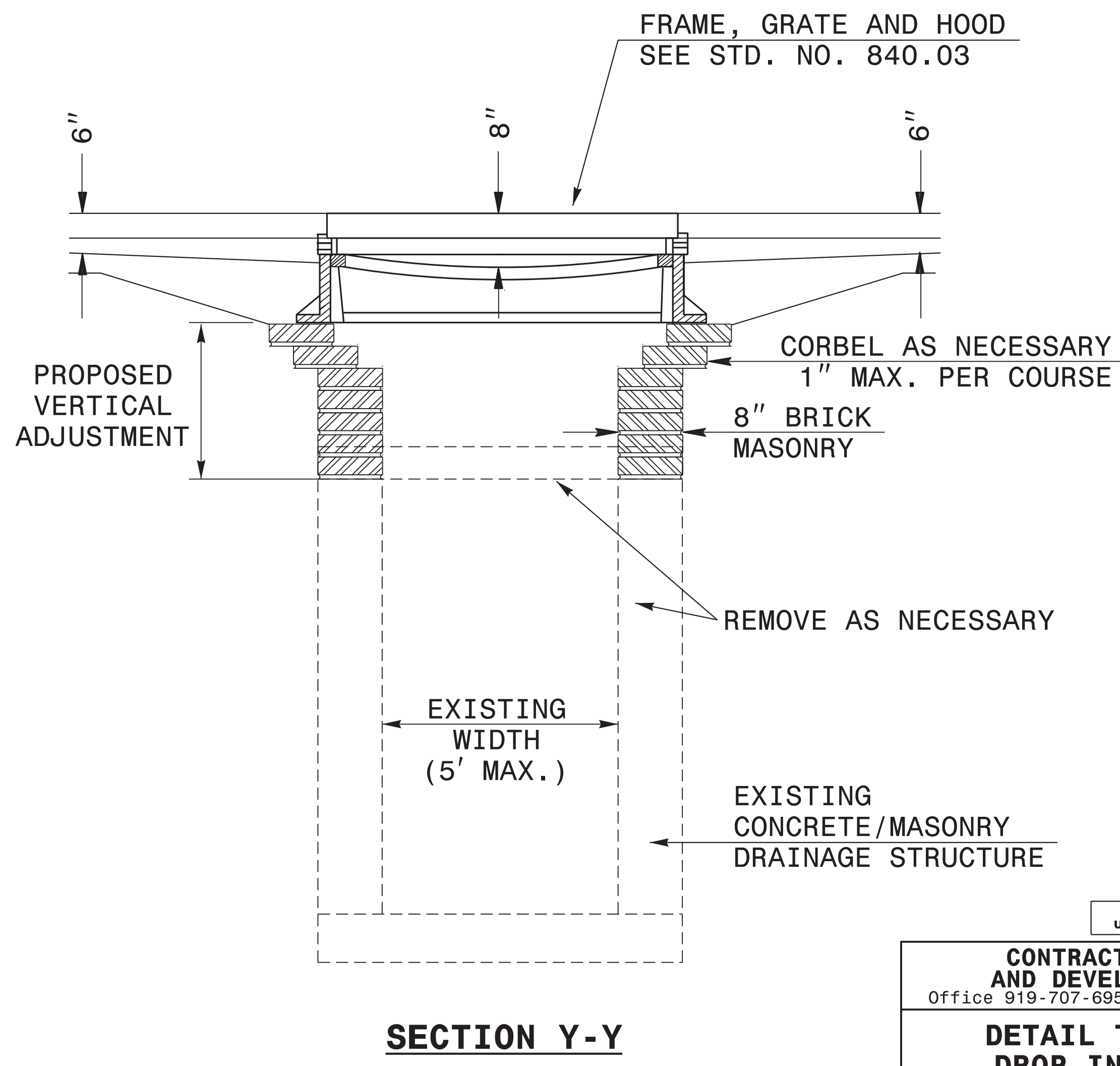
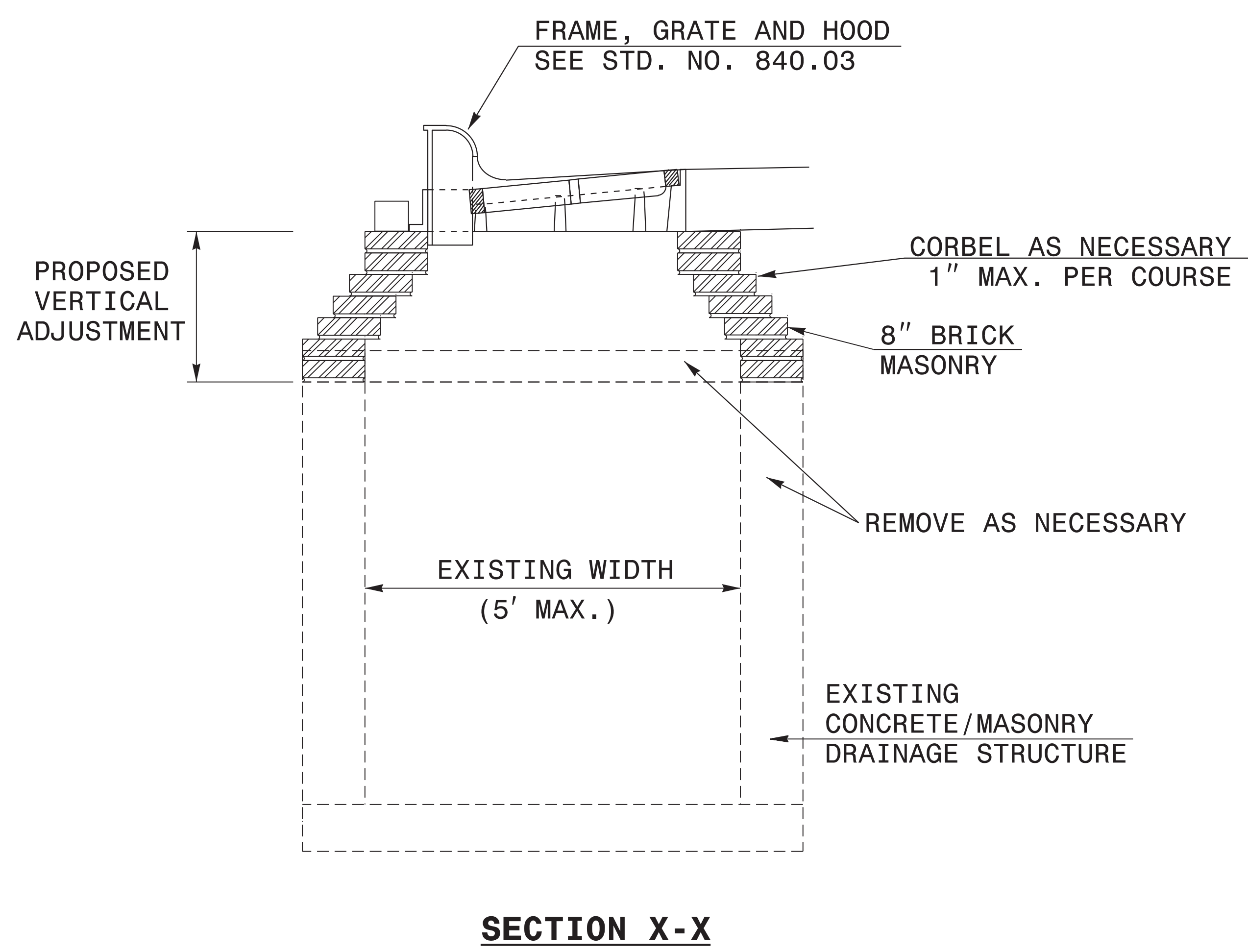
ORIGINAL BY: S.CALHOUN DATE: 7-25-2024
 MODIFIED BY: DATE: _____
 CHECKED BY: DATE: _____
 FILE SPEC.: _____

5/14/99



GENERAL NOTES:

- THE ROADWAY PLANS INDICATE STRUCTURES TO BE CONVERTED.
- AFTER REMOVAL, STORE GRATES AND FRAMES AS DIRECTED BY THE ENGINEER.
- 4" SOLID CLAY BRICK, JUMBO BRICK, CONCRETE, OR 4" SOLID CONCRETE BLOCK MAY BE USED FOR VERTICAL ADJUSTMENT OF THE STRUCTURE.
- CONVERT IN ACCORDANCE WITH SECTION 859 OF THE STANDARD SPECIFICATIONS.



2/26/2026


DOCUMENT NOT CONSIDERED FINAL UNLESS ALL SIGNATURES COMPLETED

CONTRACT STANDARDS AND DEVELOPMENT UNIT
Office 919-707-6950 FAX 919-250-4119

DETAIL TO CONVERT DROP INLET OR JB TO CATCH BASIN

ORIGINAL BY: E.E. WARD DATE: 11-97
 MODIFIED BY: DATE:
 CHECKED BY: DATE:
 FILE SPEC.: DS37:usr\details\stand\jbtocb.dgn

26-JUN-2017 10:42 S:\Contracts\Special Details\convert DI or JB to CB.dgn .jhoverton AT USD-292595

PROJECT REFERENCE NO. A-0009CD	SHEET NO. 2G-1
GEOTECHNICAL ENGINEER  D. Matthew Brewer 1/15/2023	ENGINEER
DOCUMENT NOT CONSIDERED FINAL UNLESS ALL SIGNATURES COMPLETED	

FOR USE IN THE FOLLOWING LOCATIONS,
OR AS DIRECTED BY THE ENGINEER.

STATIONS:
-Y2- 120+50 TO 122+00, LT

NOTES:

FOR ROCK EMBANKMENTS, SEE ROCK EMBANKMENTS SPECIAL PROVISION

CONSTRUCT ROCK EMBANKMENTS AS SHOWN IN THE DETAIL AND IN ACCORDANCE WITH THE ROCK EMBANKMENTS SPECIAL PROVISION

CONSTRUCT ROCK EMBANKMENTS A MINIMUM OF 2 FT. ABOVE POND SURFACE WATER ELEVATION.

USE SELECT MATERIAL, CLASS VII, RIP RAP CLASS A, RIP RAP CLASS B, AND SELECT MATERIAL, CLASS VI TO CONSTRUCT ROCK EMBANKMENTS AS SHOWN. RIP RAP CLASS A AND SELECT MATERIAL, CLASS VI SHALL BE USED TO CHOKE OFF VOIDS IN SELECT MATERIAL, CLASS VII BEFORE PLACING SEPARATION GEOTEXTILES AND SOIL EMBANKMENT FILL

SELECT MATERIAL, CLASS VII AND RIP RAP CLASS A AND B SHALL MEET THE REQUIREMENTS OF SECTION 1042 OF THE STANDARD SPECIFICATIONS

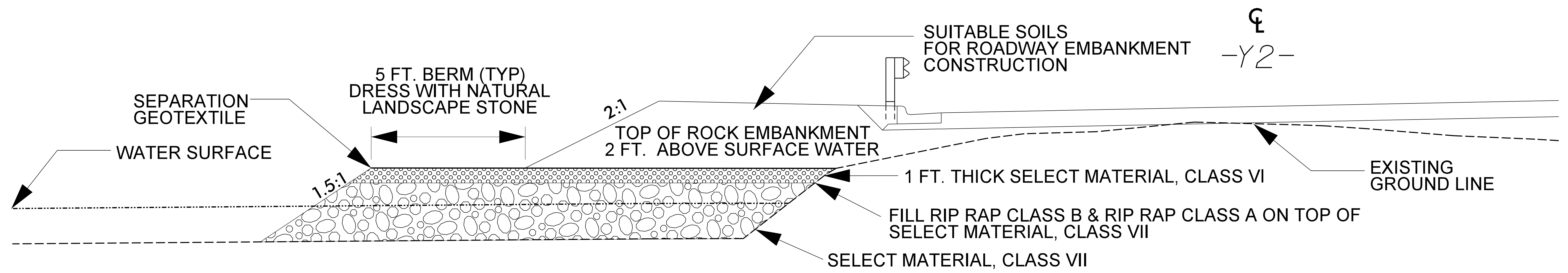
SELECT MATERIAL, CLASS VI SHALL MEET THE GRADATION REQUIREMENTS IN SECTION 1016 OF THE STANDARD SPECIFICATIONS

INSTALL SEPARATION GEOTEXTILE ON TOP OF ROCK EMBANKMENTS IN ACCORDANCE WITH THE ROCK EMBANKMENTS SPECIAL PROVISION AND ARTICLE 270-3 OF THE STANDARD SPECIFICATIONS

PLACE APPROXIMATELY 6 TO 10 INCHES OF NATURAL LANDSCAPE STONE ON THE SURFACE OF EXPOSED GEOTEXTILE AND SELECT MATERIAL, CLASS VII ON THE ROCK EMBANKMENT BERM. FOR NATURAL LANDSCAPE STONE, SEE SPECIAL PROVISION.

ESTIMATED QUANTITIES ROCK EMBANKMENTS	
SELECT MATERIAL, CLASS VII FOR ROCK EMBANKMENTS	2,260 TON
SELECT MATERIAL, CLASS VI FOR ROCK EMBANKMENTS	400 TON
RIP RAP CLASS B	400 TON
RIP RAP CLASS A	400 TON
GEOTEXTILE FOR ROCK EMBANKMENTS	690 SY

ESTIMATED QUANTITIES LANDSCAPING NATURAL STONE	
NATURAL LANDSCAPE STONE	50 TON



ROCK EMBANKMENTS TYPICAL DETAILS

NOT TO SCALE


PREPARED BY: D. MATTHEW BREWER, P.E.	DATE: 1/15/23
REVIEWED BY: ROBERT E. KRAL, P.E.	DATE: 1/15/23

Prepared in the Office of:



**CAROLINAS
GEOTECHNICAL
GROUP**
2400 CROWNPOINT EXECUTIVE DRIVE
SUITE 800
CHARLOTTE, NC 28227
(980) 339-8684

GEOTECHNICAL CONSTRUCTION DETAILS - ROCK EMBANKMENTS - POND DETAIL					
REVISIONS					
NO.	BY	DATE	NO.	BY	DATE
1			3		
2			4		

PROJECT REFERENCE NO. A-0009CD	SHEET NO. 2G-2
GEOTECHNICAL ENGINEER  D. Matthew Brewer 1/15/2023 SIGNATURE DATE	ENGINEER
DOCUMENT NOT CONSIDERED FINAL UNLESS ALL SIGNATURES COMPLETED	

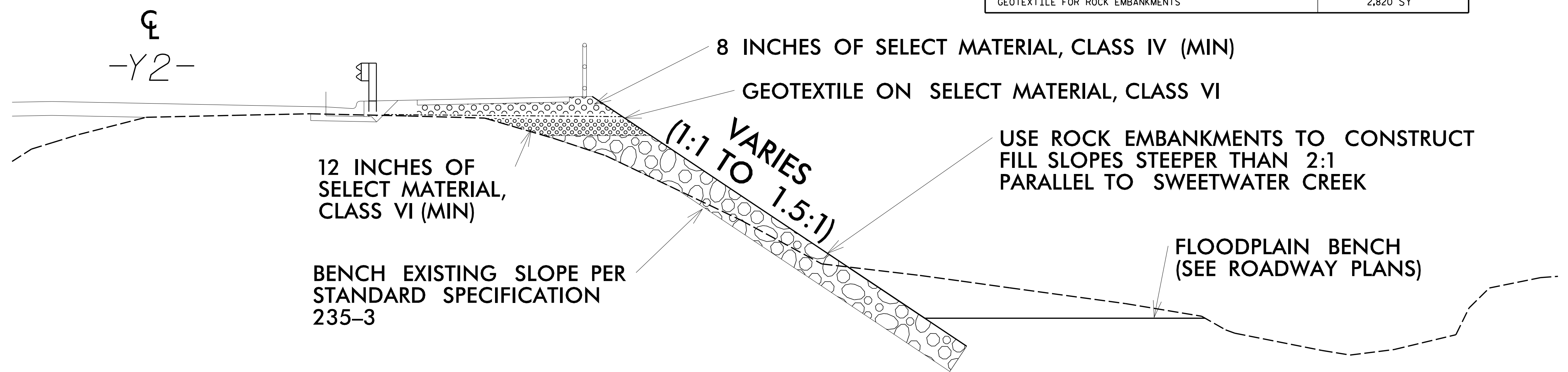
FOR USE IN THE FOLLOWING LOCATIONS,
OR AS DIRECTED BY THE ENGINEER.

- STATIONS:**
- Y2- 102+75 TO 103+75, RT
 - Y2- 107+75 TO 111+25, RT
 - Y2- 111+75 TO 114+75, RT
 - Y2- 115+25 TO 116+75, RT
 - Y2- 117+75 TO 128+25, RT

NOTES:

- FOR ROCK EMBANKMENTS, SEE ROCK EMBANKMENTS SPECIAL PROVISION
- CONSTRUCT ROCK EMBANKMENTS AS SHOWN IN THE DETAIL AND IN ACCORDANCE WITH THE ROCK EMBANKMENTS SPECIAL PROVISION
- USE SELECT MATERIAL, CLASS VII, RIP RAP CLASS A, RIP RAP CLASS B, AND SELECT MATERIAL, CLASS VI TO CONSTRUCT ROCK EMBANKMENTS AS SHOWN. RIP RAP CLASS A AND SHALL BE USED TO CHOKE OFF VOIDS IN SELECT MATERIAL, CLASS VII BEFORE PLACING SEPARATION GEOTEXTILES AND SELECT MATERIAL, CLASS IV.
- SELECT MATERIAL, CLASS VII AND RIP RAP CLASS A AND B SHALL MEET THE REQUIREMENTS OF SECTION 1042 OF THE STANDARD SPECIFICATIONS
- SELECT MATERIAL, CLASS VI SHALL MEET THE GRADATION REQUIREMENTS IN SECTION 1016 OF THE STANDARD SPECIFICATIONS
- SELECT MATERIAL, CLASS IV SHALL MEET THE GRADATION REQUIREMENTS IN SECTION 1016 OF THE STANDARD SPECIFICATIONS
- INSTALL SEPARATION GEOTEXTILE ON TOP OF ROCK EMBANKMENTS IN ACCORDANCE WITH THE ROCK EMBANKMENTS SPECIAL PROVISION AND ARTICLE 270-3 OF THE STANDARD SPECIFICATIONS

ESTIMATED QUANTITIES ROCK EMBANKMENTS	
SELECT MATERIAL, CLASS VII FOR ROCK EMBANKMENTS	3,010 TON
SELECT MATERIAL, CLASS VI FOR ROCK EMBANKMENTS	940 TON
SELECT MATERIAL, CLASS IV FOR ROCK EMBANKMENTS	910 TON
RIP RAP CLASS B	450 TON
RIP RAP CLASS A	450 TON
GEOTEXTILE FOR ROCK EMBANKMENTS	2,820 SY



PREPARED BY: D. MATTHEW BREWER, P.E.	DATE: 1/15/23
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**GEOTECHNICAL
CONSTRUCTION DETAILS -
ROCK EMBANKMENTS -
WIDENED FILL DETAIL**

REVISIONS					
NO.	BY	DATE	NO.	BY	DATE
1			3		
2			4		

PROJECT REFERENCE NO. A-0009CD	SHEET NO. 2G-3
GEOTECHNICAL ENGINEER  D. Matthew Brewer 1/15/2023 DATE	ENGINEER _____ SIGNATURE _____ DATE
DOCUMENT NOT CONSIDERED FINAL UNLESS ALL SIGNATURES COMPLETED	

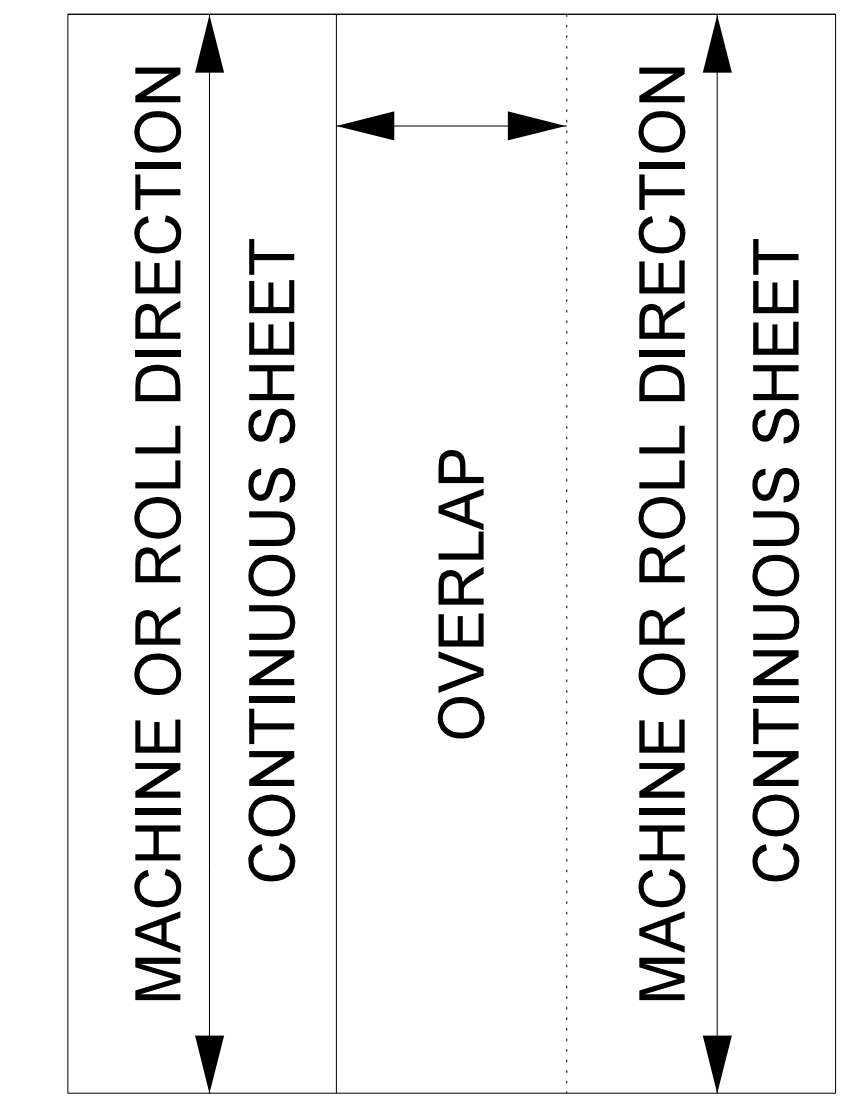
FOR USE IN THE FOLLOWING LOCATIONS,
OR AS DIRECTED BY THE ENGINEER

STATIONS:
-Y2- 90+75 TO 99+25, LT

NOTES:

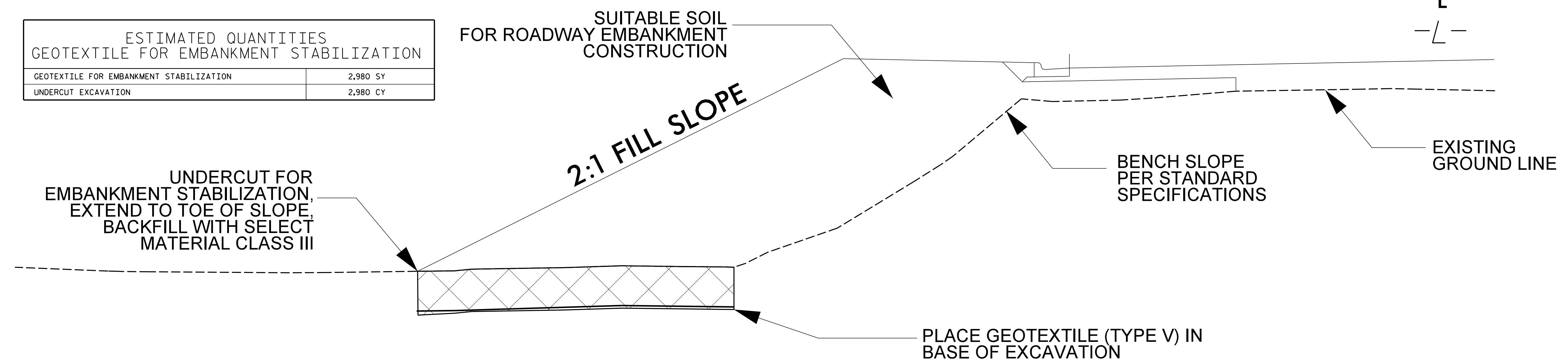
- PERFORM UNDERCUT EXCAVATION IN THE AREAS NOTED. UNDERCUT SHALL EXTEND FROM TOE OF EXISTING SLOPE TO TOE OF PROPOSED SLOPE. UNDERCUT SHALL EXTEND TO A DEPTH OF 3 FEET BELOW EXISTING GRADE.
- PLACE GEOTEXTILE TYPE V FOR EMBANKMENT STABILIZATION. FOR GEOTEXTILE FOR EMBANKMENT STABILIZATION, SEE SPECIAL PROVISION
- PLACE THE GEOTEXTILE FOR EMBANKMENT STABILIZATION WITHOUT ANY CREASES OR WRINKLES
- THE TERMS ROLL AND MACHINE DIRECTION ARE USED INTERCHANGEABLY
- GEOTEXTILE FOR EMBANKMENT STABILIZATION PLACEMENT SHALL BE CONTINUOUS IN THE MACHINE DIRECTION PERPENDICULAR TO -L- AS SHOWN IN THE PLANS AND OF SUFFICIENT LENGTH TO COVER THE AREA INDICATED. THE GEOTEXTILE SHALL BE PLACED IN THE BOTTOM OF THE UNDERCUT EXCAVATION
- NO SEAMS OR JOINTS ARE ALLOWED IN THE MACHINE/ROLL DIRECTION
- THE MINIMUM OVERLAP BETWEEN ADJACENT GEOTEXTILE OF THE SAME MACHINE/ROLL DIRECTION IS 18 INCHES UNLESS SEWING IS USED TO ACHIEVE THE REQUIRED SEAM STRENGTH
- BACKFILL UNDERCUT WITH SELECT MATERIAL, CLASS III

18 INCHES MIN OVERLAP
OR SEE MANUFACTURER
GUIDELINES IF SEWN.



GEOTEXTILE FOR EMBANKMENT STABILIZATION OVERLAP DETAIL
NOT TO SCALE

ESTIMATED QUANTITIES GEOTEXTILE FOR EMBANKMENT STABILIZATION	
GEOTEXTILE FOR EMBANKMENT STABILIZATION	2,980 SY
UNDERCUT EXCAVATION	2,980 CY



GEOTEXTILE FOR EMBANKMENT STABILIZATION WITH UNDERCUT TYPICAL DETAILS
NOT TO SCALE

PREPARED BY: D. MATTHEW BREWER, P.E.	DATE: 1/15/23
REVIEWED BY: ROBERT E. KRAL, P.E.	DATE: 1/15/23


Prepared in the Office of:

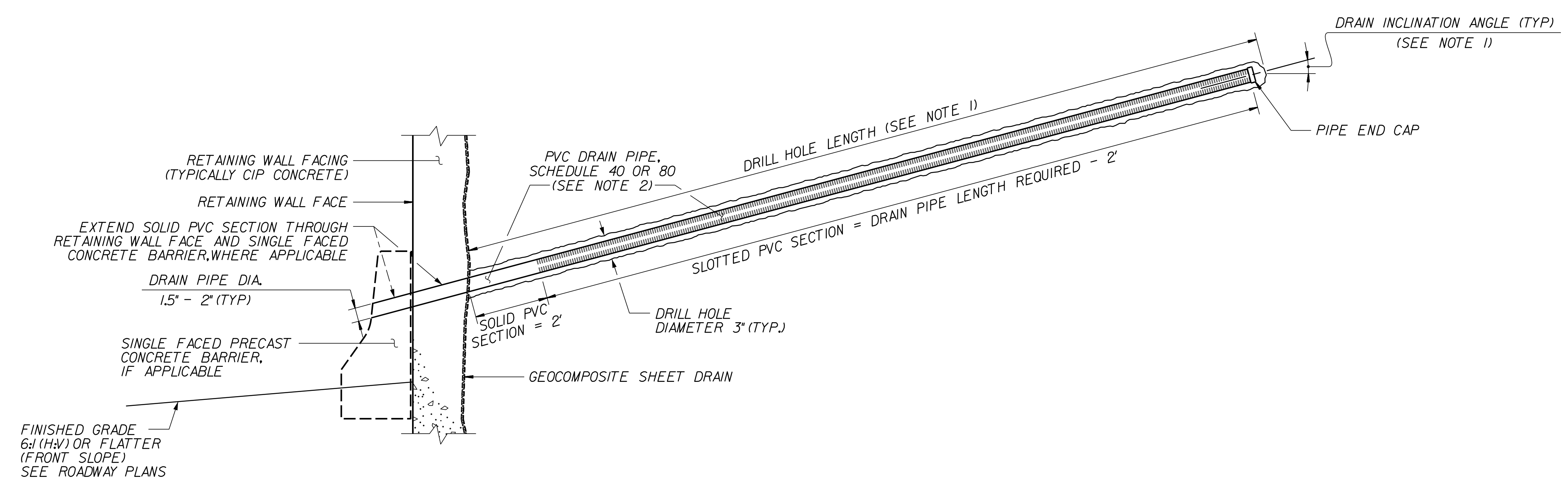


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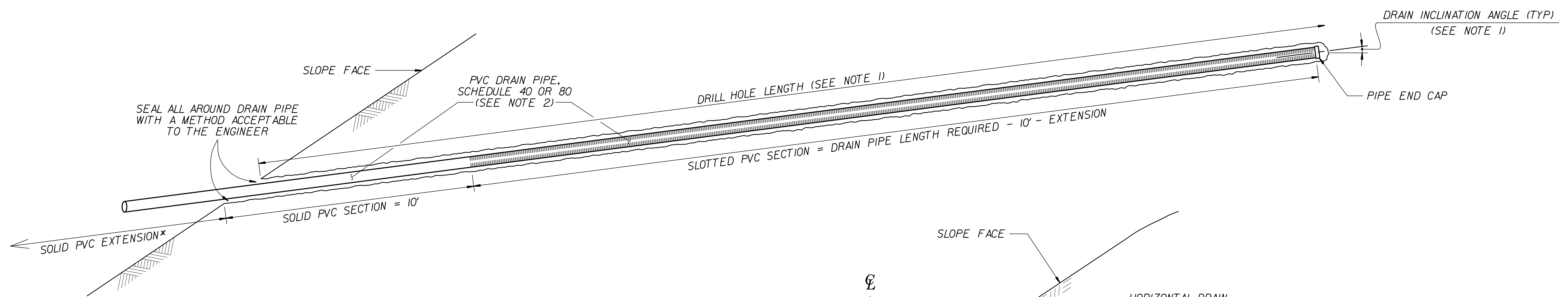
**GEOTECHNICAL CONSTRUCTION
DETAILS - GEOTEXTILE FOR
EMBANKMENT STABILIZATION**

REVISIONS					
NO.	BY	DATE	NO.	BY	DATE
1			3		
2			4		

PROJECT REFERENCE NO. A-0009CD		SHEET NO. 2G-4	
GEOTECHNICAL ENGINEER  D. Matthew Brewer 1/15/2023 SIGNATURE DATE		ENGINEER SIGNATURE DATE	
DOCUMENT NOT CONSIDERED FINAL UNLESS ALL SIGNATURES COMPLETED			



RETAINING WALL HORIZONTAL DRAIN



SLOPE HORIZONTAL DRAIN

*EXTEND SOLID PVC SECTION TO CONNECT PIPE TO A DRAINAGE SYSTEM OR DISCHARGE WATER AS DIRECTED

EXAMPLE CROSS-SECTION WITH SLOPE HORIZONTAL DRAIN
*SEE NOTE 1 FOR DRAIN ELEVATIONS ABOVE (OR BELOW) GRADE

NOTES:

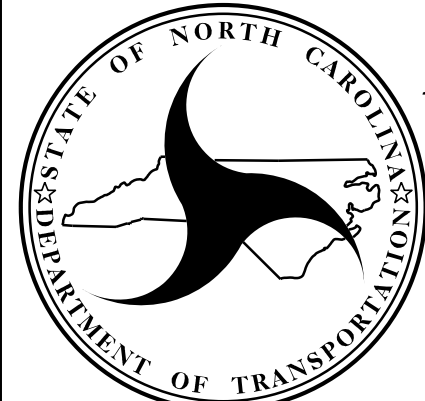
1. SEE ROADWAY SUMMARY SHEETS FOR APPROXIMATE KNOWN HORIZONTAL DRAIN LOCATIONS, ELEVATIONS, INCLINATION AND LENGTHS. ADDITIONAL DRAINS MAY BE REQUIRED AS DETERMINED BY THE ENGINEER.
2. DRAIN PIPES MAY BE OMITTED FOR SOME HORIZONTAL DRAINS. SEE ROADWAY SUMMARY SHEETS FOR DRAIN PIPE REQUIREMENTS INCLUDING THOSE DRAINS WITHOUT PIPES.
3. FOR HORIZONTAL DRAINS, SEE HORIZONTAL DRAINS SPECIAL PROVISION.

PREPARED BY: D. MATTHEW BREWER, P.E.	DATE: 1/15/23
REVIEWED BY: ROBERT E. KRAL, P.E.	DATE: 1/15/23

Prepared in the Office of:



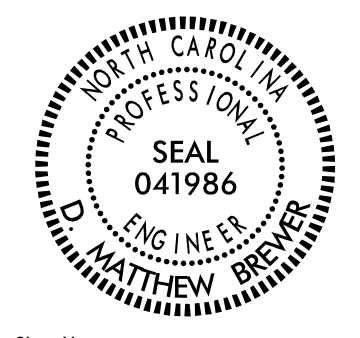
CAROLINIAN GEOTECHNICAL GROUP
2400 CROWNPOINT EXECUTIVE DRIVE
SUITE 800
CHARLOTTE, NC 28227
(980) 339-8684



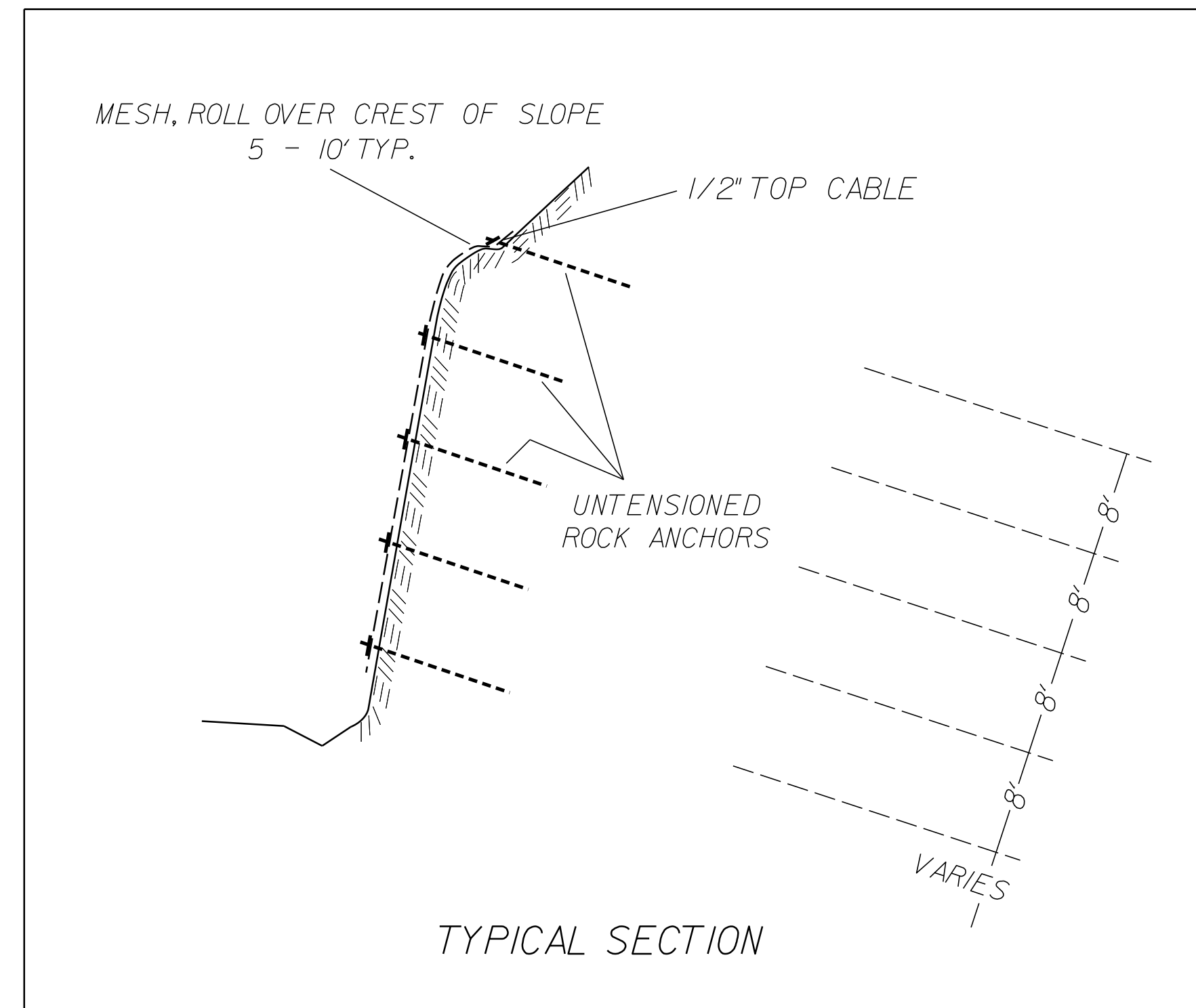
NORTH CAROLINA DEPARTMENT OF TRANSPORTATION DIVISION OF HIGHWAYS

GEOTECHNICAL ENGINEERING UNIT

GEOTECHNICAL CONSTRUCTION DETAILS - HORIZONTAL DRAINS					
REVISIONS					
NO.	BY	DATE	NO.	BY	DATE
1			3		
2			4		

GEOTECHNICAL ENGINEER  SEAL 041986 MATTHEW BREWER ENGINEER	ENGINEER _____ SIGNATURE DATE
DocuSigned by: D. Matthew Brewer 04/09/2026 _____ SIGNATURE DATE	

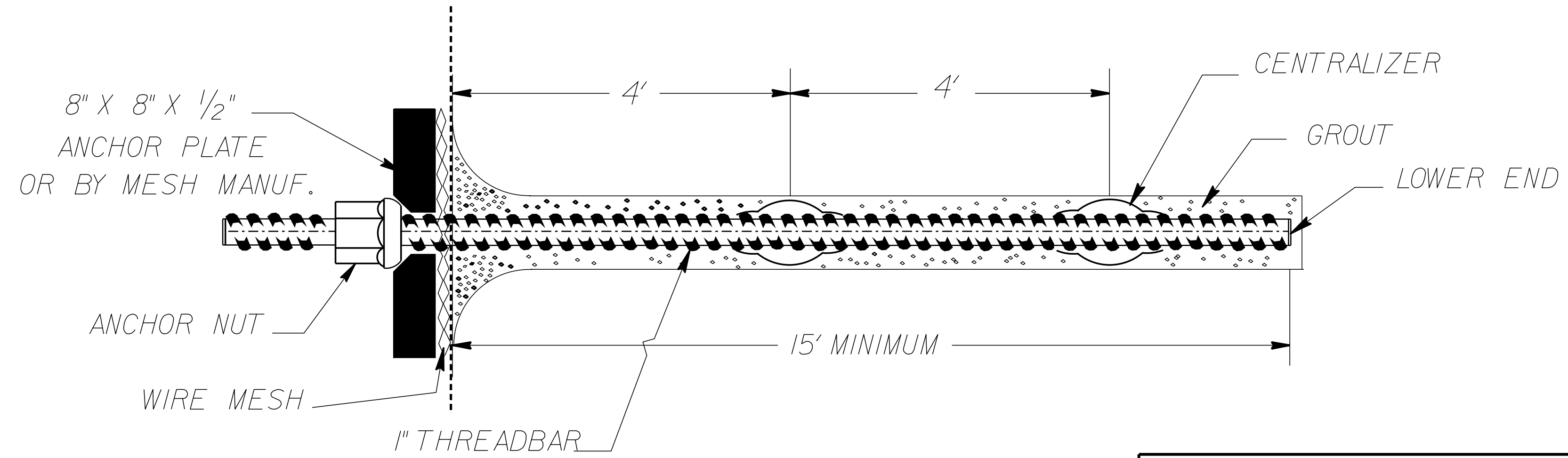
DOCUMENT NOT CONSIDERED FINAL UNLESS ALL SIGNATURES COMPLETED



MESH: SEE SPECIAL PROVISION "SLOPE STABILIZATION" "WIRE MESH" TYPE 1 OR TYPE 2


DETAIL OF WIRE MESH
N.T.S.

- NOTES:
- 1) WIRE MESH SHALL BE INSTALLED ON SLOPES AT LOCATIONS AS DIRECTED BY THE ENGINEER.

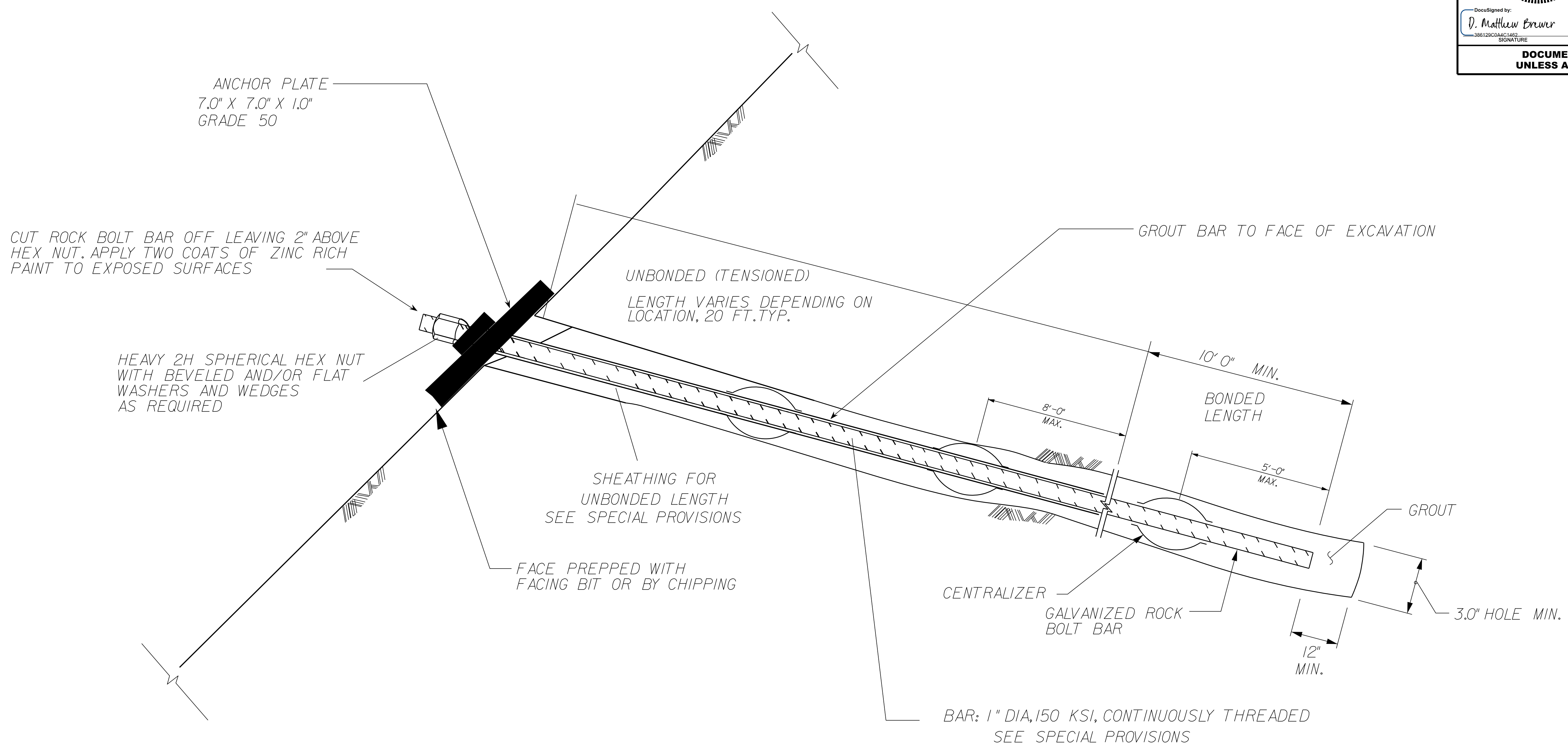


ANCHOR FOR WIRE MESH
N.T.S.

ESTIMATED QUANTITIES	
WIRE MESH (TYPE 1)	750 SY
WIRE MESH (TYPE 2)	750 SY
UNTENSIONED ROCK ANCHOR FOR WIRE MESH	3,000 LF

PROJECT REFERENCE NO. A-0009CD		SHEET NO. 2G-6	
GEOTECHNICAL ENGINEER  D. Matthew Brewer 1/15/2023		ENGINEER _____ _____ _____	
DOCUMENT NOT CONSIDERED FINAL UNLESS ALL SIGNATURES COMPLETED			

NOTE: GALVANIZE ALL ACCESSORIES INCLUDING PLATES, WASHERS, WEDGES, CLIPES, CLAMPS, WIRES, RINGS, AND ALL INCIDENTALS NECESSARY FOR INSTALLATION, TO THE REQUIREMENTS OF ASTM A-153-80



TYPICAL SECTION

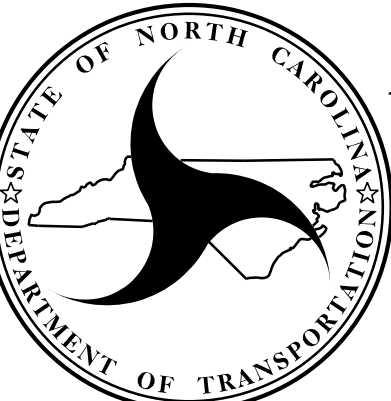
ESTIMATED QUANTITIES	
TENSIONED SPOT ROCK BOLTS - 75 KIPS	3,060 LF

PREPARED BY: D. MATTHEW BREWER, P.E.	DATE: 1/15/23
REVIEWED BY: ROBERT E. KRAL, P.E.	DATE: 1/15/23

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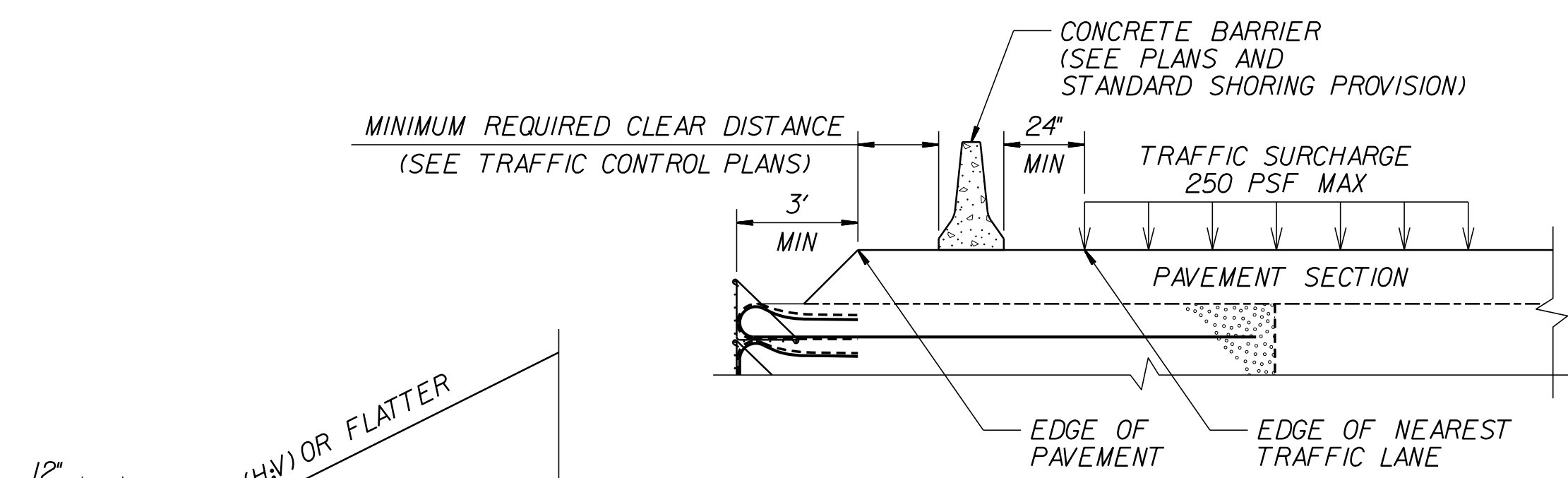


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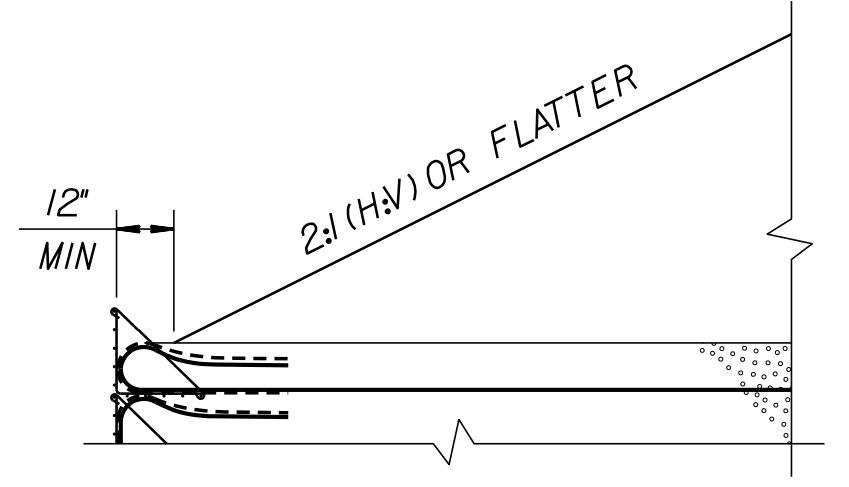
**GEOTECHNICAL
ENGINEERING UNIT**

GEOTECHNICAL CONSTRUCTION DETAILS - TENSIONED SPOT ROCK BOLTS - 75 KIPS					
REVISIONS					
NO.	BY	DATE	NO.	BY	DATE
1			3		
2			4		

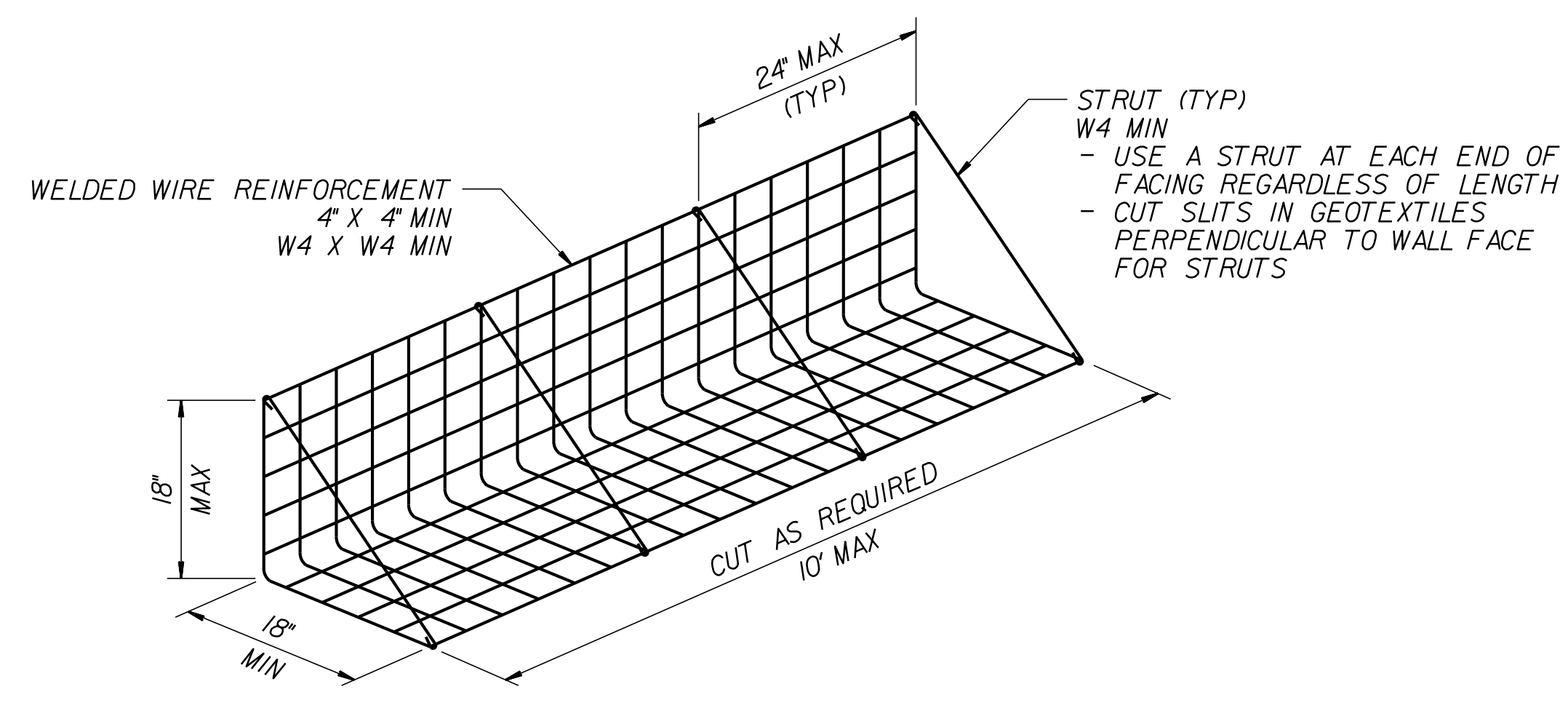
PROJECT REFERENCE NO. A-0009CD	SHEET NO. 2G-7
GEOTECHNICAL ENGINEER  D. Matthew Brumr 1/15/2023	ENGINEER
DOCUMENT NOT CONSIDERED FINAL UNLESS ALL SIGNATURES COMPLETED	



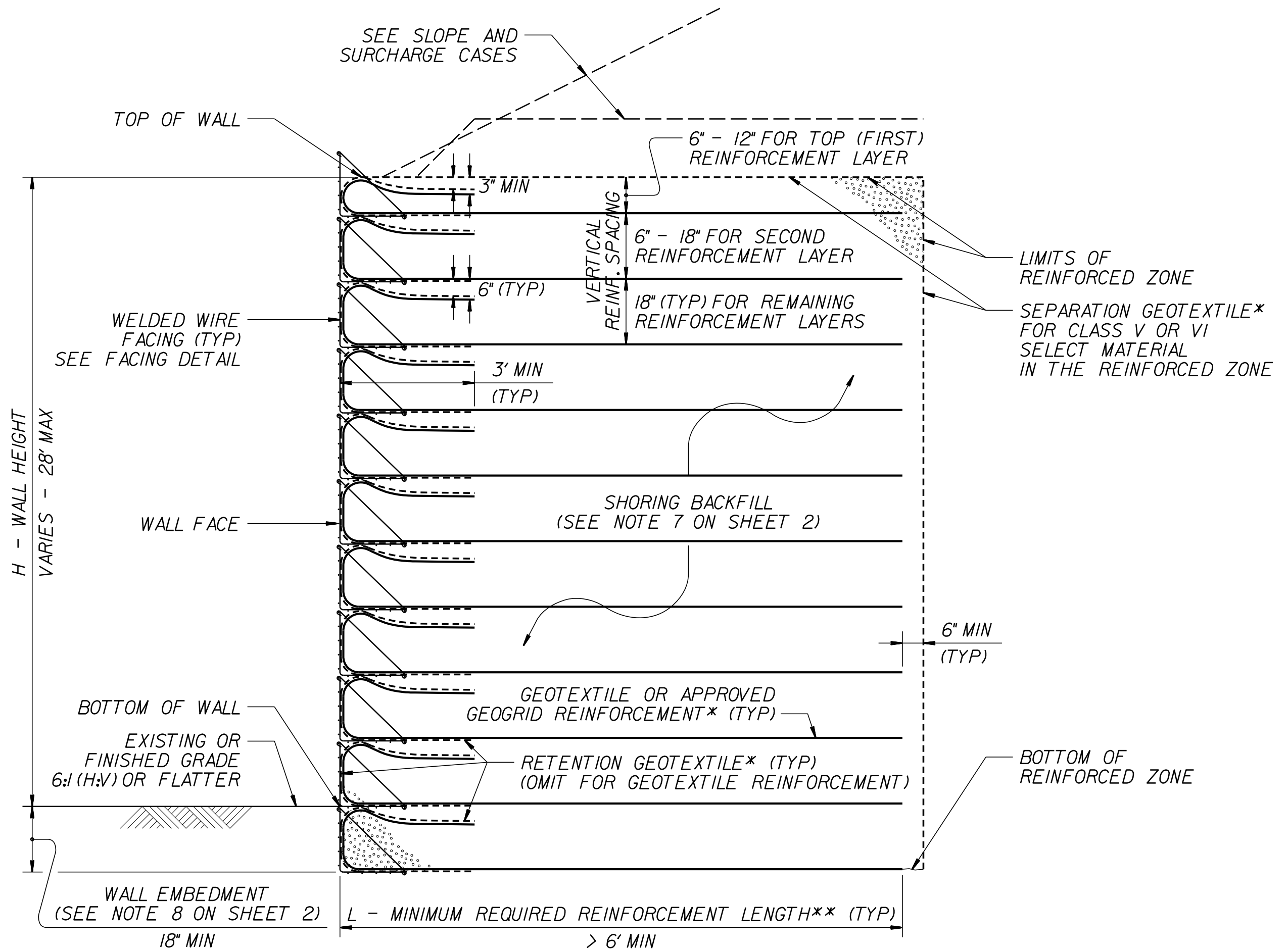
SURCHARGE CASE



SLOPE CASE

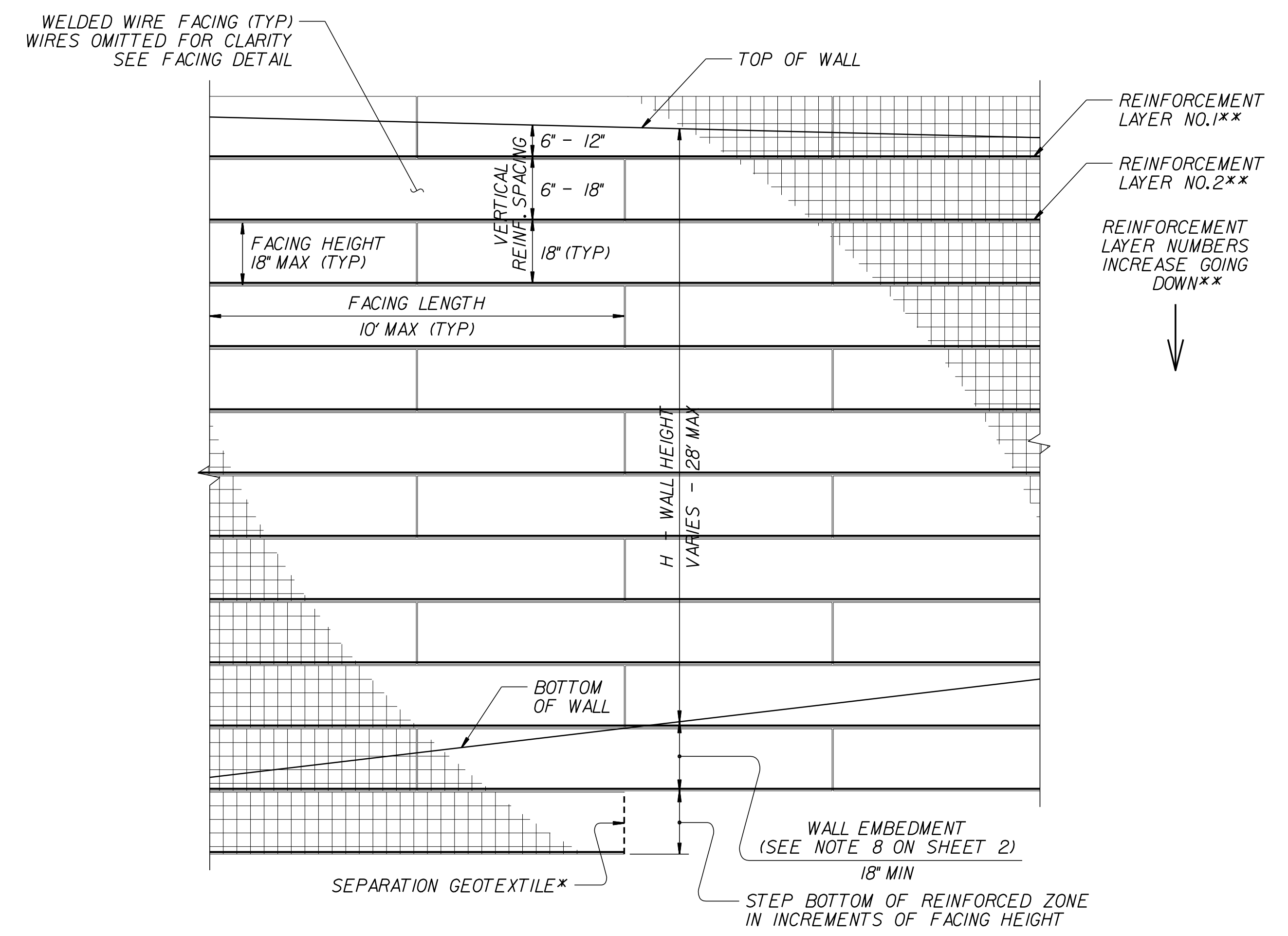


FACING DETAIL



STANDARD TEMPORARY WALL

(FOR STANDARD TEMPORARY WALLS ON STRUCTURES, SEE TEMPORARY WALL ON STRUCTURE DETAIL ON SHEET 2.)
 *SEE GEOSYNTHETIC PLACEMENT DETAILS ON SHEET 2.
 **SEE REINFORCEMENT TABLES ON SHEET 3.



STANDARD TEMPORARY WALL – PARTIAL ELEVATION


*SEE GEOSYNTHETIC PLACEMENT DETAILS ON SHEET 2.
 **SEE REINFORCEMENT TABLES ON SHEET 3.

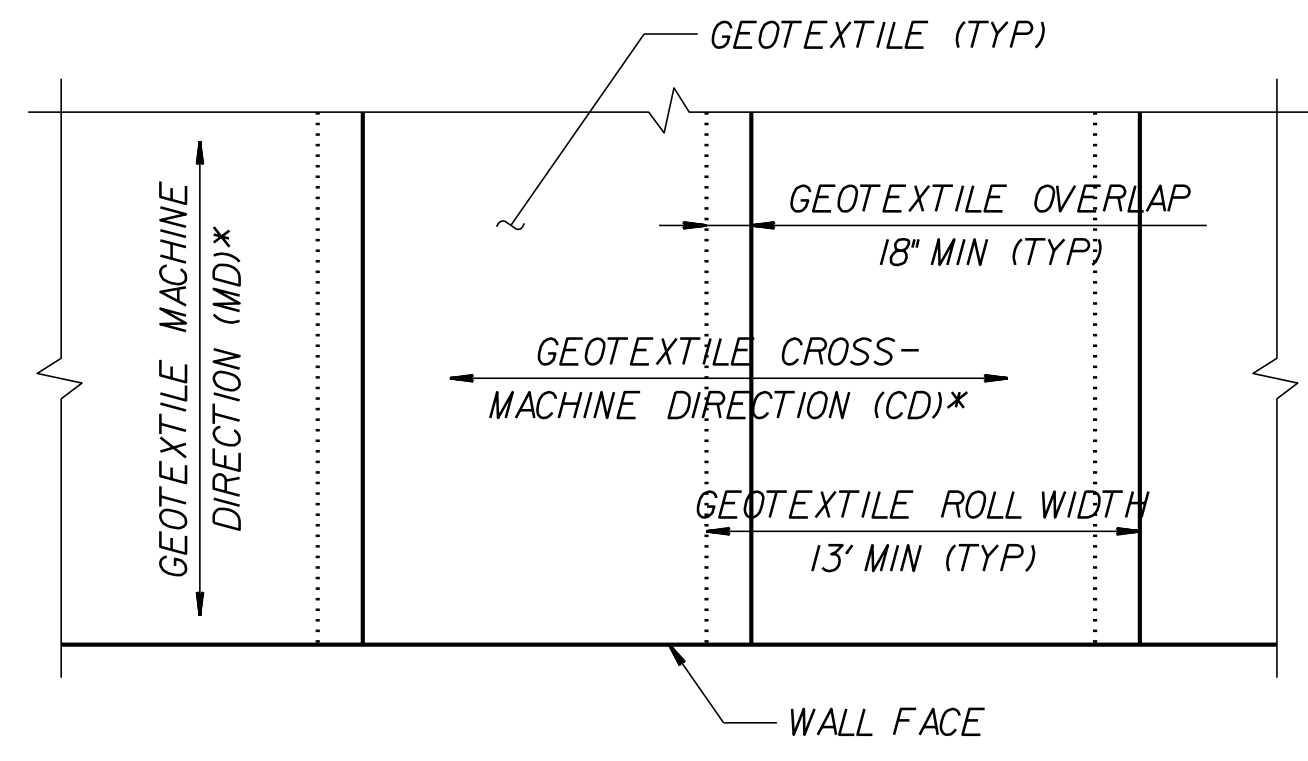


NORTH CAROLINA
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**GEOTECHNICAL
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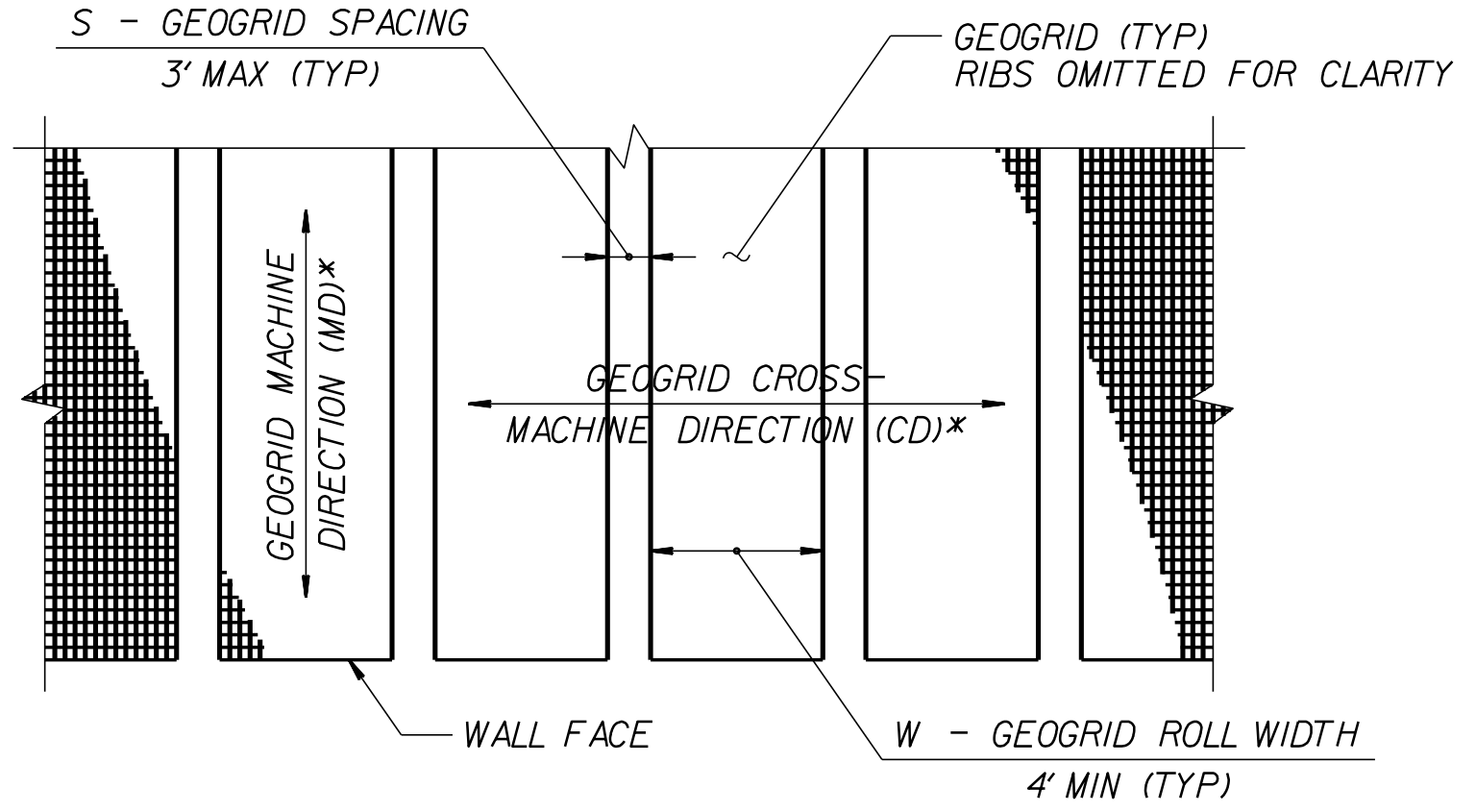
STANDARD DETAIL NO. 1801.02

STANDARD
 TEMPORARY WALL
 SHEET 1 OF 3

PROJECT REFERENCE NO. A-0009CD	SHEET NO. 2G-8
 GEOTECHNICAL ENGINEER	ENGINEER
DOCUMENT NOT CONSIDERED FINAL UNLESS ALL SIGNATURES COMPLETED	

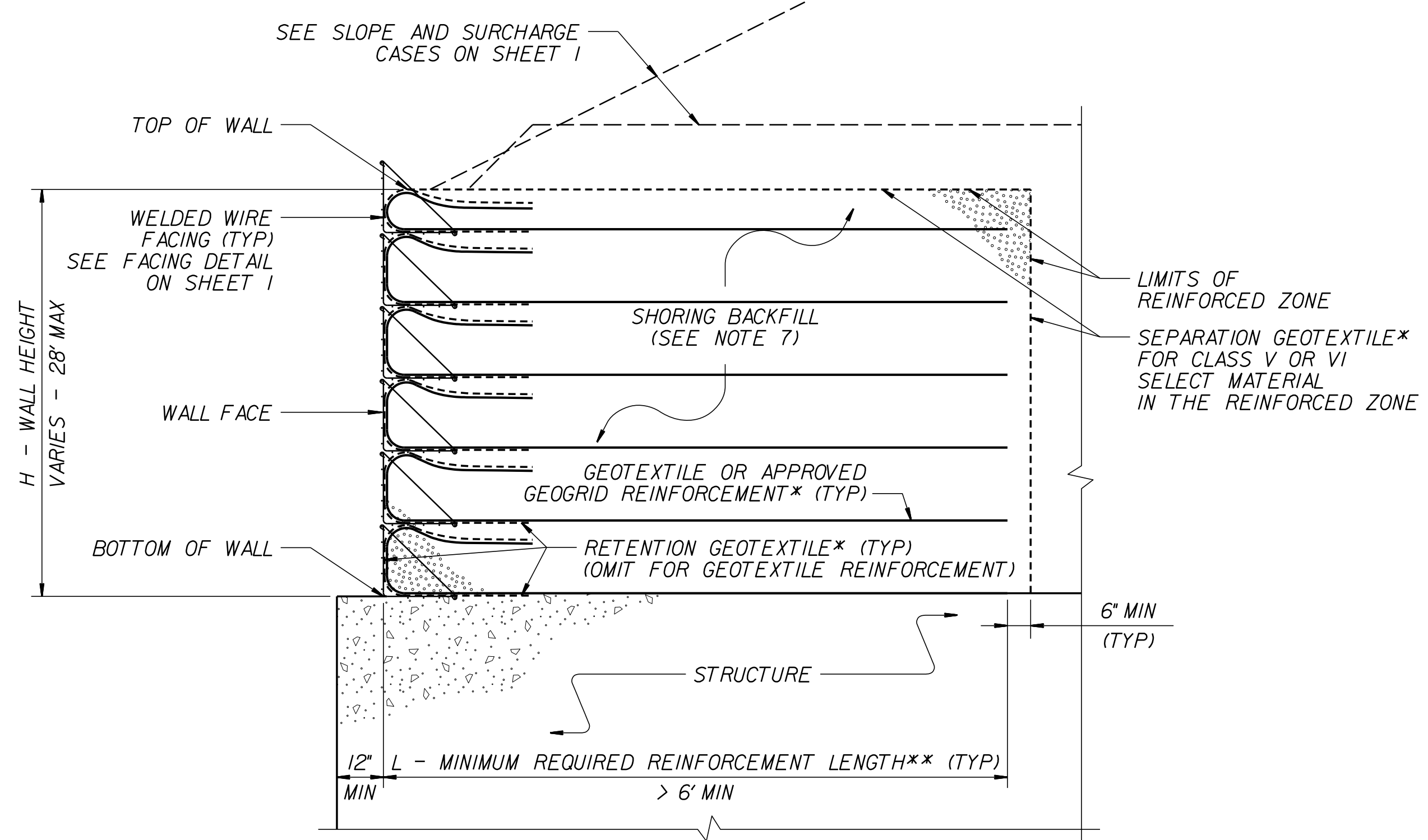


GEOTEXTILE PLACEMENT
(100% COVERAGE MIN FOR GEOTEXTILE REINFORCEMENT)



GEOGRID PLACEMENT
(80% COVERAGE MIN FOR GEOGRID REINFORCEMENT - $\frac{W}{W+S} \times 100 \geq 80\%$, SEE NOTE 11)

GEOSYNTHETIC PLACEMENT DETAILS
(PLAN VIEW)
*SEE NOTE 12.



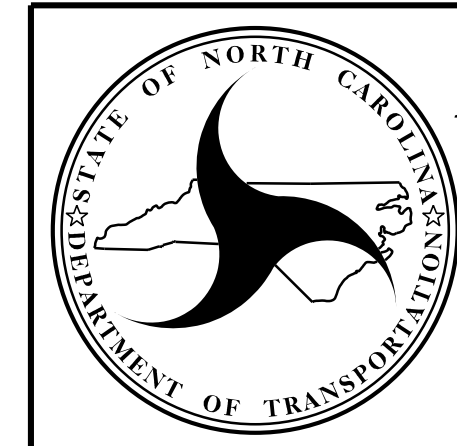
TEMPORARY WALL ON STRUCTURE DETAIL
*SEE GEOSYNTHETIC PLACEMENT DETAILS.
**SEE REINFORCEMENT TABLES ON SHEET 3.

NOTES:

1. AT THE CONTRACTOR'S OPTION, USE STANDARD TEMPORARY WALLS AS NOTED IN THE PLANS.
2. FOR STANDARD TEMPORARY WALLS, SEE STANDARD SHORING PROVISION.
3. STANDARD TEMPORARY WALLS ARE BASED ON THE FOLLOWING IN-SITU ASSUMED SOIL PARAMETERS:
UNIT WEIGHT, $\gamma = 120$ PCF
FRICTION ANGLE, $\phi = 30$ DEGREES
COHESION, $c = 0$ PSF
4. DO NOT USE STANDARD TEMPORARY WALLS IF ASSUMED SOIL PARAMETERS ARE NOT APPLICABLE.
5. DO NOT USE STANDARD TEMPORARY WALLS WHEN VERY LOOSE OR SOFT SOIL OR MUCK IS BELOW TEMPORARY WALLS.
6. USE GROUNDWATER ELEVATION NOTED IN THE PLANS. IF NO GROUNDWATER ELEVATION IS SHOWN IN THE PLANS, ASSUME GROUNDWATER DEPTH IS LESS THAN 7' BELOW BOTTOM OF REINFORCED ZONE. DO NOT USE STANDARD TEMPORARY WALLS IF GROUNDWATER OR FLOOD ELEVATION IS ABOVE BOTTOM OF REINFORCED ZONE.
7. DO NOT USE A-2-4 SOIL FOR STANDARD TEMPORARY WALLS AROUND CULVERTS OR IN THE REINFORCED ZONE OF STANDARD TEMPORARY WALLS FOR SLOPE CASES. DO NOT USE SELECT MATERIAL, CLASS VI IN THE REINFORCED ZONE OF STANDARD TEMPORARY WALLS WITH GEOTEXTILE REINFORCEMENT.
8. WALL EMBEDMENT IS NOT REQUIRED FOR STANDARD TEMPORARY WALLS ON STRUCTURES OR ROCK AS DETERMINED BY THE ENGINEER.
9. DO NOT USE MORE THAN 4 DIFFERENT REINFORCEMENT STRENGTHS FOR EACH STANDARD TEMPORARY WALL.
10. GEOGRIDS FOR GEOGRID REINFORCEMENT ARE APPROVED FOR SHORT TERM DESIGN STRENGTHS (3-YEAR DESIGN LIFE) IN THE MD AND CD BASED ON MATERIAL TYPE. THE LIST OF APPROVED GEOGRIDS WITH DESIGN STRENGTHS IS AVAILABLE FROM: connect.ncdot.gov/resources/Geological/Pages/Products.aspx
DEFINE MATERIAL TYPE FROM THE WEBSITE ABOVE FOR SHORING BACKFILL AS FOLLOWS:

MATERIAL TYPE	SHORING BACKFILL
BORROW	A-2-4 SOIL
FINE AGGREGATE	CLASS II, TYPE I OR CLASS III SELECT MATERIAL
COARSE AGGREGATE	CLASS V OR VI SELECT MATERIAL

11. FOR GEOGRID REINFORCEMENT WITH LESS THAN 100% COVERAGE, STAGGER REINFORCEMENT SO GEOGRIDS ARE CENTERED OVER GAPS IN THE REINFORCEMENT LAYER BELOW.
12. AT THE CONTRACTOR'S OPTION, REINFORCEMENT MAY BE INSTALLED WITH THE MD PARALLEL TO THE WALL FACE IF BOTH OF THE FOLLOWING CONDITIONS OCCUR:
- W (REINFORCEMENT ROLL WIDTH) \geq (MINIMUM REQUIRED REINFORCEMENT LENGTH) + 4.5' AND
- REINFORCEMENT STRENGTH IN CD \geq MINIMUM REQUIRED REINFORCEMENT STRENGTH IN MD.
13. SUBMIT A "STANDARD TEMPORARY WALL SELECTION FORM" AT LEAST 7 DAYS BEFORE STARTING TEMPORARY WALL CONSTRUCTION. STANDARD SHORING SELECTION FORMS ARE AVAILABLE FROM: connect.ncdot.gov/resources/Geological/Pages/Geotech_Forms_Details.aspx
14. DO NOT PLACE SHORING BACKFILL OR REINFORCEMENT UNTIL EXCAVATION DIMENSIONS AND FOUNDATION MATERIAL ARE APPROVED.
15. FOR STANDARD TEMPORARY WALLS WITH PILE FOUNDATIONS IN THE REINFORCED ZONE, DRIVE PILES THROUGH REINFORCEMENT AFTER CONSTRUCTING TEMPORARY WALLS.
16. DO NOT SPLICE OR OVERLAP REINFORCEMENT SO SEAMS ARE PARALLEL TO THE WALL FACE.
17. CONTACT THE ENGINEER WHEN EXISTING OR FUTURE OBSTRUCTIONS SUCH AS FOUNDATIONS, PAVEMENTS, PIPES, INLETS OR UTILITIES WILL INTERFERE WITH REINFORCEMENT.
18. FOR STANDARD TEMPORARY WALLS WITH INTERIOR ANGLES LESS THAN 90 DEGREES, WRAP GEOSYNTHETICS AT ACUTE CORNERS AS DIRECTED BY THE ENGINEER.
19. FOR STANDARD TEMPORARY WALLS WITH TOP OF WALL WITHIN 5' OF FINISHED GRADE, REMOVE TOP FACING AND INCORPORATE TOP REINFORCEMENT LAYER INTO FILL WHEN PLACING FILL IN FRONT OF WALL.

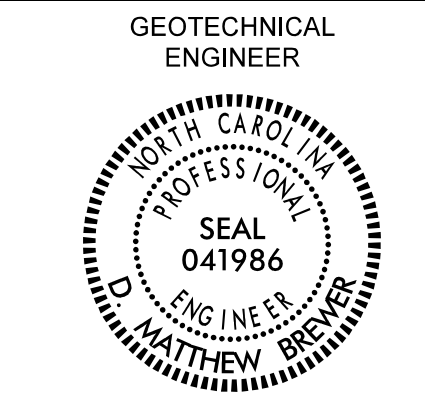


NORTH CAROLINA
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GEOTECHNICAL
ENGINEERING UNIT

STANDARD DETAIL NO. 1801.02

STANDARD
TEMPORARY WALL
SHEET 2 OF 3

PROJECT REFERENCE NO. A-0009CD	SHEET NO. 2G-9
 GEOTECHNICAL ENGINEER	ENGINEER
DOCUMENT NOT CONSIDERED FINAL UNLESS ALL SIGNATURES COMPLETED	

SLOPE OR SURCHARGE CASE	GROUNDWATER DEPTH BELOW BOTTOM OF REINFORCED ZONE (SEE NOTE 6 ON SHEET 2) (FT)	SHORING BACKFILL TYPE IN THE REINFORCED ZONE (SEE NOTE 7 ON SHEET 2)	H - WALL HEIGHT (FT)																									
			< 4	5	6	7	8	9	10	11	12	13	14	15	16	17	18	19	20	21	22	23	24	25	26	27	28	
SLOPE CASE	> 0	CLASS II, TYPE I, CLASS III, CLASS V OR CLASS VI SELECT MATERIAL	6	6	7	8	9	11	12	13	13	14	15	16	17	18	19	20	21	22	23	24	24	25	26	27	27	
SURCHARGE CASE	> 0 TO 7 FOR H < 20' > 0 TO 10 FOR H ≥ 20'	ALL SHORING BACKFILL TYPES	6	7	7	8	8	9	9	10	11	11	12	12	13	14	14	15	16	17	17	18	19	19	20	21	22	
		A-2-4 SOIL	6	6	7	8	8	9	9	10	11	11	12	12	13	14	14	15	16	16	17	18	18	19	20	20	21	
		CLASS II, TYPE I OR CLASS III SELECT MATERIAL	6	6	7	7	8	8	9	10	10	11	11	12	12	13	14	15	15	16	16	17	17	18	18	19	20	
	> 7 FOR H < 20' > 10 FOR H ≥ 20'	CLASS V OR CLASS VI SELECT MATERIAL	6	6	7	7	8	8	9	9	10	10	11	12	13	13	14	14	15	15	16	17	17	18	19	19		

L - MINIMUM REQUIRED REINFORCEMENT LENGTH (FT)
(FOR ALL REINFORCEMENT TYPES)

WALL HEIGHT (H) + WALL EMBEDMENT (FT)	NUMBER OF REINFORCEMENT LAYERS*
2.5 - 4	3
4 - 5.5	4
5.5 - 7	5
7 - 8.5	6
8.5 - 10	7
10 - 11.5	8
11.5 - 13	9
13 - 14.5	10
14.5 - 16	11
16 - 17.5	12
17.5 - 19	13
19 - 20.5	14
20.5 - 22	15
22 - 23.5	16
23.5 - 25	17
25 - 26.5	18
26.5 - 28	19
28 - 29.5	20

*BASED ON VERTICAL REINFORCEMENT SPACING SHOWN ON SHEET 1.

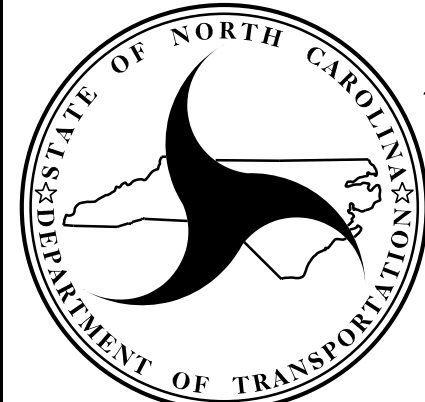
REINFORCEMENT LAYER NUMBER*	SHORING BACKFILL TYPE IN THE REINFORCED ZONE (SEE NOTE 7 ON SHEET 2)				
	SLOPE CASE		SURCHARGE CASE		
	CLASS II, TYPE I OR CLASS III SELECT MATERIAL	CLASS V SELECT MATERIAL	A-2-4 SOIL	CLASS II, TYPE I OR CLASS III SELECT MATERIAL	CLASS V SELECT MATERIAL
1	2400	2400	2400	2400	2400
2	2400	2400	2400	2400	2400
3	2400	2400	2400	2400	2400
4	2400	2400	2500	2400	2400
5	2500	2400	3000	2400	2400
6	3000	2400	3500	2800	2400
7	3500	2700	4000	3200	2600
8	4000	3100	4500	3600	2900
9	4500	3500	5000	4000	3200
10	5000	3900	5500	4400	3500
11	5500	4300	6000	4800	3800
12	6000	4700	6500	5200	4100
13	6500	5100	7000	5600	4400
14	7000	5400	7500	6000	4700
15	7500	5800	8000	6400	5000
16	8000	6200	8500	6800	5300
17	8500	6600	9000	7200	5600
18	9000	7000	9500	7600	5900
19	9500	7400	10000	8000	6200
20	10000	7800	10500	8400	6500

GEOTEXTILE REINFORCEMENT
ULTIMATE TENSILE STRENGTH (LB/FT)

REINFORCEMENT LAYER NUMBER*	SHORING BACKFILL TYPE IN THE REINFORCED ZONE (SEE NOTE 7 ON SHEET 2)				
	SLOPE CASE		SURCHARGE CASE		
	CLASS II, TYPE I OR CLASS III SELECT MATERIAL	CLASS V OR CLASS VI SELECT MATERIAL	A-2-4 SOIL	CLASS II, TYPE I OR CLASS III SELECT MATERIAL	CLASS V OR CLASS VI SELECT MATERIAL
1	240	200	340	290	240
2	380	310	520	430	350
3	530	420	700	570	460
4	690	550	870	720	570
5	860	690	1050	860	680
6	1030	830	1220	1000	790
7	1200	970	1400	1150	900
8	1370	1110	1580	1290	1010
9	1550	1240	1750	1430	1120
10	1720	1380	1930	1580	1230
11	1890	1520	2100	1720	1340
12	2060	1660	2280	1860	1450
13	2240	1800	2450	2010	1560
14	2410	1940	2630	2150	1670
15	2580	2080	2800	2290	1780
16	2750	2220	2980	2440	1890
17	2930	2360	3160	2580	2000
18	3100	2500	3330	2720	2110
19	3270	2640	3510	2860	2220
20	3440	2780	3690	3000	2330

GEOGRID REINFORCEMENT
SHORT-TERM DESIGN STRENGTH (LB/FT)
(SEE NOTE 10 ON SHEET 2.)


MINIMUM REQUIRED REINFORCEMENT STRENGTH IN MD
(SEE NOTE 9 ON SHEET 2.)
*SEE PARTIAL ELEVATION ON SHEET 1 FOR REINFORCEMENT LAYER NUMBERING.



NORTH CAROLINA
DEPARTMENT OF TRANSPORTATION
DIVISION OF HIGHWAYS

GEOTECHNICAL
ENGINEERING UNIT

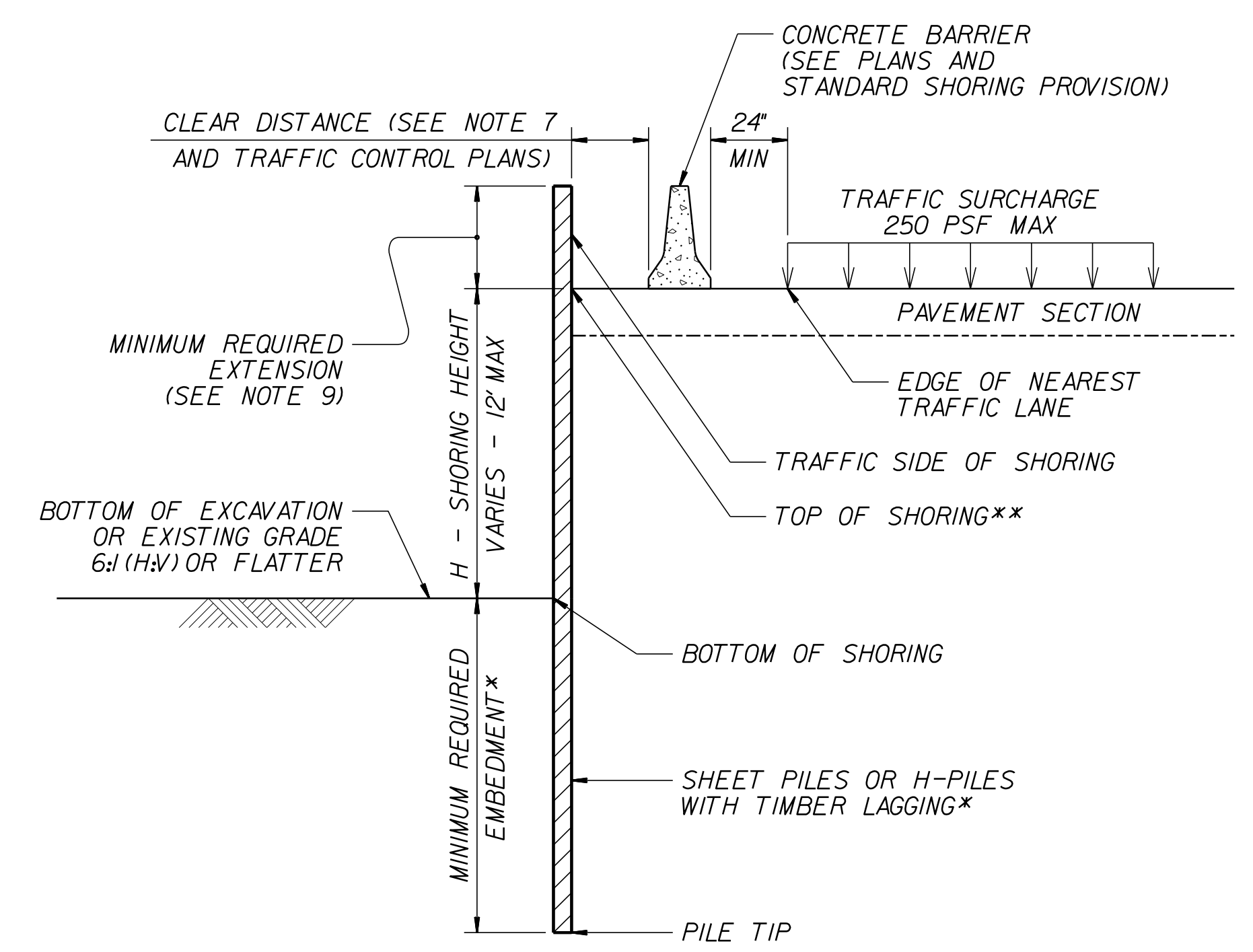
STANDARD DETAIL NO. 1801.02
STANDARD TEMPORARY WALL SHEET 3 OF 3
DATE: 11-19-13

PROJECT REFERENCE NO. A-0009CD	SHEET NO. 2G-10
GEOTECHNICAL ENGINEER  D. Matthew Brewer 1/15/2023	ENGINEER
DOCUMENT NOT CONSIDERED FINAL UNLESS ALL SIGNATURES COMPLETED	

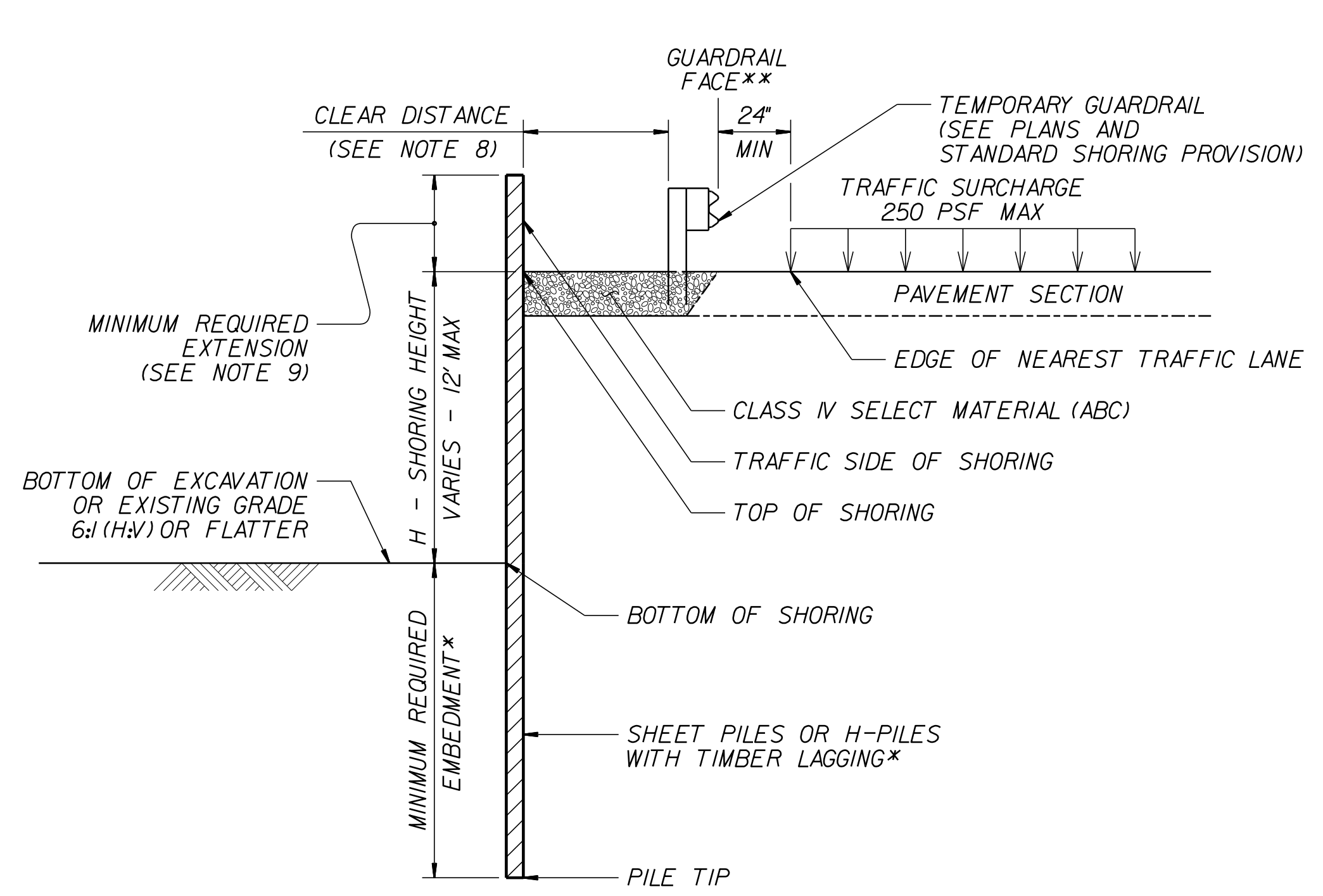
GROUNDWATER CONDITION (SEE NOTE 6)	H SHORING HEIGHT (FT)	SLOPE OR SURCHARGE CASE WITH NO TRAFFIC IMPACT					SURCHARGE CASE WITH TRAFFIC IMPACT				
		SHEET PILES		H-PILES WITH TIMBER LAGGING			SHEET PILES		H-PILES WITH TIMBER LAGGING		
		MINIMUM REQUIRED EMBEDMENT (FT)	MINIMUM REQUIRED SECTION MODULUS (IN ³ /FT)	MINIMUM REQUIRED EMBEDMENT* (FT) (SEE NOTE 10)			MINIMUM REQUIRED EMBEDMENT (FT)	MINIMUM REQUIRED SECTION MODULUS (IN ³ /FT)	MINIMUM REQUIRED EMBEDMENT* (FT) (SEE NOTE 10)		
				HP 10x42	HP 12x53	HP 14x73			HP 10x42	HP 12x53	HP 14x73
GROUNDWATER ELEVATION BETWEEN BOTTOM OF SHORING AND PILE TIP	< 6	11.5	4.5	11.5	11.5	11.5	16.0	12.0	13.0	13.0	13.0
	7	13.0	7.0	13.0	13.0	13.0	17.0	14.5	14.5	14.5	14.5
	8	15.0	10.0	--	15.0	15.0	18.0	17.0	--	15.5	15.5
	9	17.0	14.0	--	17.0	17.0	19.0	20.0	--	17.0	17.0
	10	18.5	19.5	--	--	18.5	20.0	23.5	--	--	18.5
	11	20.5	26.0	--	--	--	21.0	28.0	--	--	20.0
12	22.5	33.0	--	--	--	22.0	33.0	--	--	21.5	
GROUNDWATER ELEVATION BELOW PILE TIP	< 6	7.5	3.0	8.0	8.0	8.0	11.0	10.0	9.5	9.5	9.5
	7	8.5	4.5	9.5	9.5	9.5	12.0	12.0	10.5	10.5	10.5
	8	10.0	6.5	10.5	10.5	10.5	12.5	14.0	11.5	11.5	11.5
	9	11.0	9.5	--	12.0	12.0	13.5	16.5	--	12.5	12.5
	10	12.5	13.0	--	--	13.5	14.0	19.5	--	13.5	13.5
	11	13.5	17.0	--	--	14.5	15.0	22.5	--	--	14.5
12	15.0	21.5	--	--	16.0	16.0	25.5	--	--	15.5	

- NOTES:**
- AT THE CONTRACTOR'S OPTION, USE STANDARD TEMPORARY SHORING AS NOTED IN THE PLANS.
 - FOR STANDARD TEMPORARY SHORING, SEE STANDARD SHORING PROVISION.
 - STANDARD TEMPORARY SHORING IS BASED ON THE FOLLOWING IN-SITU ASSUMED SOIL PARAMETERS:
UNIT WEIGHT, $\gamma = 120$ PCF
FRICTION ANGLE, $\phi = 30$ DEGREES
COHESION, $c = 0$ PSF
 - DO NOT USE STANDARD TEMPORARY SHORING IF ASSUMED SOIL PARAMETERS ARE NOT APPLICABLE.
 - DO NOT USE STANDARD TEMPORARY SHORING WHEN VERY LOOSE OR SOFT SOIL OR MUCK IS WITHIN THE EMBEDMENT DEPTH.
 - USE GROUNDWATER ELEVATION NOTED IN THE PLANS. IF NO GROUNDWATER ELEVATION IS SHOWN IN THE PLANS, USE "GROUNDWATER ELEVATION BETWEEN BOTTOM OF SHORING AND PILE TIP" FOR GROUNDWATER CONDITION. DO NOT USE STANDARD TEMPORARY SHORING IF GROUNDWATER IS ABOVE BOTTOM OF SHORING.
 - AT THE CONTRACTOR'S OPTION OR IF AVAILABLE CLEAR DISTANCE IS LESS THAN THE MINIMUM REQUIRED FOR CONCRETE BARRIER, SET BARRIER NEXT TO AND UP AGAINST TRAFFIC SIDE OF PILES AND USE "SURCHARGE CASE WITH TRAFFIC IMPACT".
 - AT THE CONTRACTOR'S OPTION OR IF AVAILABLE CLEAR DISTANCE IS LESS THAN 4' FOR TEMPORARY GUARDRAIL, ATTACH GUARDRAIL TO TRAFFIC SIDE OF PILES AS SHOWN IN THE PLANS AND USE "SURCHARGE CASE WITH TRAFFIC IMPACT".
 - MINIMUM REQUIRED EXTENSION IS 6' FOR "SLOPE OR SURCHARGE CASE WITH NO TRAFFIC IMPACT" AND 32' FOR "SURCHARGE CASE WITH TRAFFIC IMPACT".
 - MINIMUM REQUIRED EMBEDMENT FOR H-PILES WITH TIMBER LAGGING IS BASED ON DRIVEN H-PILES AT MAXIMUM 6' SPACING. AT THE CONTRACTOR'S OPTION, EMBEDMENT DEPTHS MAY BE REDUCED BY 25% FOR DRILLED-IN H-PILES.
 - SUBMIT A "STANDARD TEMPORARY SHORING SELECTION FORM" AT LEAST 7 DAYS BEFORE STARTING TEMPORARY SHORING CONSTRUCTION. UP TO 3 SHORING LOCATIONS MAY BE INCLUDED ON EACH FORM. STANDARD SHORING SELECTION FORMS ARE AVAILABLE FROM:
connect.ncdot.gov/resources/Geological/Pages/Geotech_Forms_Details.aspx
 - CONTACT THE ENGINEER IF PILES DO NOT ATTAIN THE MINIMUM REQUIRED EMBEDMENT.

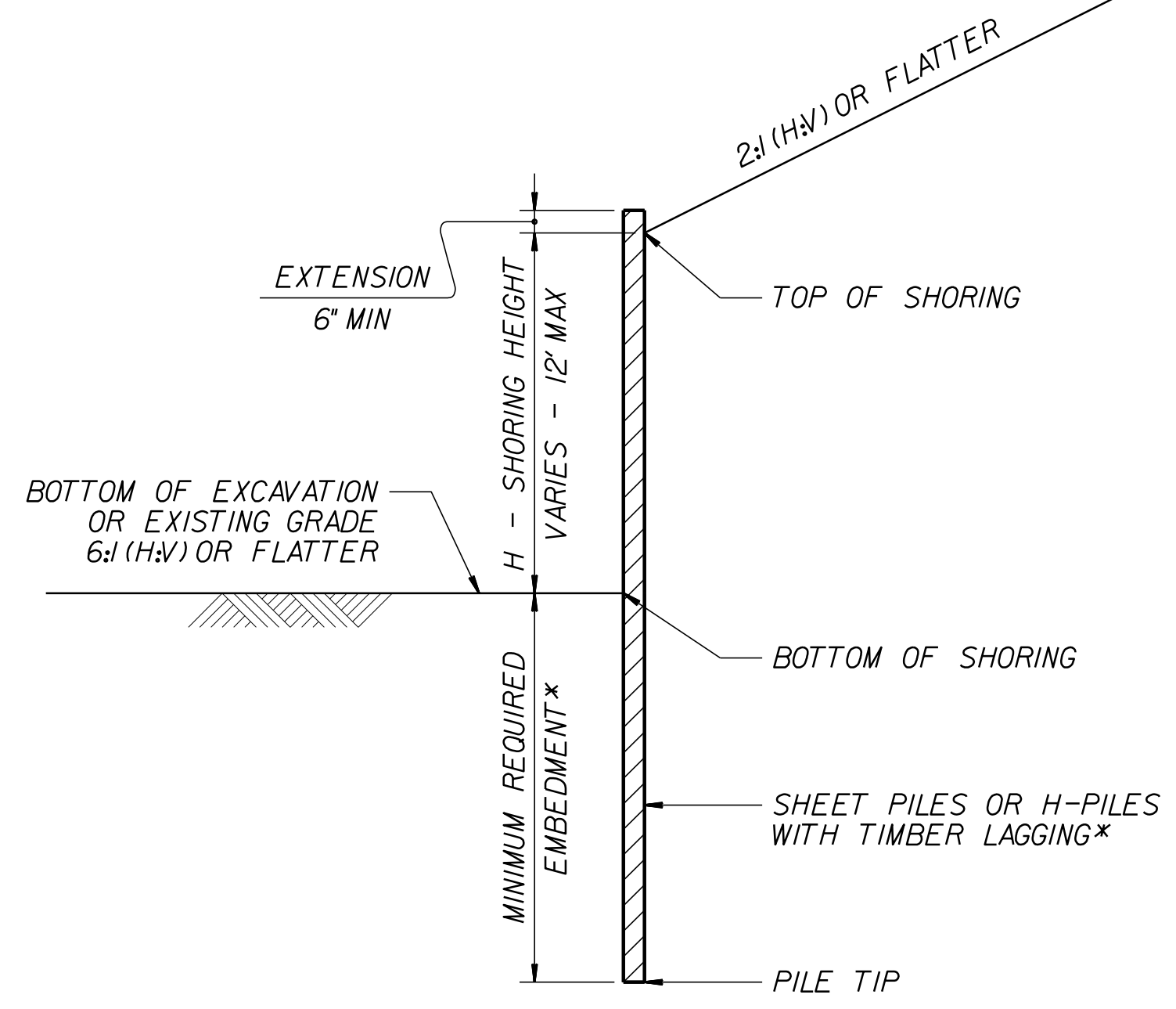
MINIMUM REQUIRED EMBEDMENT AND SECTION MODULUS
***DO NOT USE H-PILES WITH TIMBER LAGGING FOR GROUNDWATER CONDITION, SHORING HEIGHT AND H-PILE SIZE SHOWN IF MINIMUM REQUIRED EMBEDMENT IS "--".**



CONCRETE BARRIER
****TOP OF SHORING = EDGE OF PAVEMENT**

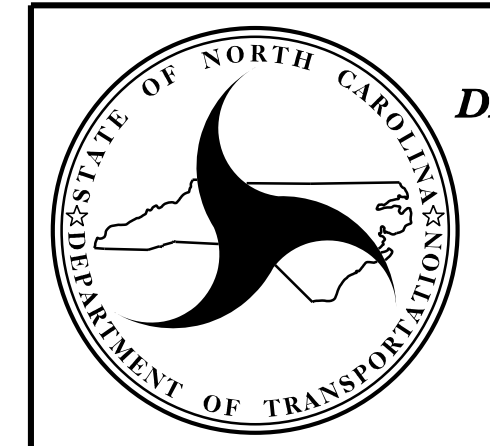


TEMPORARY GUARDRAIL
****GUARDRAIL FACE = EDGE OF PAVEMENT**



STANDARD TEMPORARY SHORING (SLOPE CASE)
***SEE TABLE ABOVE.**

STANDARD TEMPORARY SHORING (SURCHARGE CASE)
***SEE TABLE ABOVE.**



NORTH CAROLINA
 DEPARTMENT OF TRANSPORTATION
 DIVISION OF HIGHWAYS
GEOTECHNICAL ENGINEERING UNIT

STANDARD DETAIL NO. 1801.01

STANDARD TEMPORARY SHORING

COMPUTED BY: SGM DATE: 1/6/2026
 CHECKED BY: JLT DATE: 1/13/26

PROJECT NO. SHEET NO.
 A-0009CD 3B-1

**STATE OF NORTH CAROLINA
 DIVISION OF HIGHWAYS**

**SUMMARY OF EARTHWORK
 IN CUBIC YARDS**

Station	Station	Uncl. Excav.	Undercut	Embank. +%	Borrow	Waste
-Y2- 89+80.00	-Y2- 100+00.00	102		8,221	8,119	
-Y3- 10+50.00	-Y3- 14+02.24	2,881		1,334		1,547
-Y2- 90+75.00	-Y2- 99+25.00		2,980			2,980
SUBTOTAL 1		2,983	2,980	9,555	8,119	4,527
-Y2- 100+00.00	-Y2- 130+00.00	7,170		14,190	7,020	
-Y2- 117+75.00	-Y2- 130+00.00		5,076			5,076
SUBTOTAL 2		7,170	5,076	14,190	7,020	5,076
-Y2- 130+00.00	-Y2- 158+00.00	357,395		15,504		341,891
-Y2- 130+00.00	-Y2- 133+75.00		1,554			1,554
-Y2- 138+25.00	-Y2- 141+75.00		2,320			2,320
SUBTOTAL 3		357,395	3,874	15,504		345,765
-Y2- 158+00.00	-Y2- 169+60.00	54,499		670		53,829
SUBTOTAL 4		54,499		670		53,829
TOTAL		422,047	11,930	39,919	15,139	409,197
LOSS DUE TO CLEARING & GRUBBING		-18,000				-18,000
ADDITIONAL UNDERCUT (CONTINGENCY -PER GEOTECH)			750			750
ADDITIONAL UNDERCUT (PER CFI)			10,000			10,000
HARD ROCK WASTE TO REPLACE BORROW					-10,531	-10,531
ADJUST ROCK SWELL				-2,633	-2,633	
ELIMINATE EARTH SHRINKAGE FACTOR				-1,975	-1,975	
ADJUST ROCK SWELL				-1,476		1,476
ELIMINATE EARTH SHRINKAGE FACTOR				-1,107		1,107
APPLY ROCK SWELL FACTOR TO PERMANENTLY WASTED HARD ROCK						29,576
APPLY ROCK SHRINKAGE TO PERMANENTLY WASTED HARD ROCK						22,182
GRAND TOTALS:		404,047	22,680	32,728	0	445,757
SAY		410,000	22,680			

**PAVEMENT REMOVAL SUMMARY
 IN SQUARE YARDS**

SURVEY LINE	Station	Station	LOCATION LT/RT/CL	ASPHALT REMOVAL
-Y2-	89+80	98+47	RT	817.54
-Y2-	92+10	95+00	RT	138.13
-Y2-	98+80	132+27	RT	4,590.52
-Y2-	139+46	154+49	LT	1,957.62
-Y2-	155+33	156+13	LT	46.67
SR 1238 Bill Crisp Rd (Lt of -Y2- 99+50)				163.48
TOTAL:				7,713.96
SAY:				7,750

Note: Earthwork quantities are calculated by TGS Engineers. These earthwork quantities are based in part on subsurface data provided by the Geotechnical Engineering Unit.

EST. DDE = 800 CUBIC YARDS
 SELECT GRANULAR MATERIAL, CLASS III = 13,630 CY
 PAVEMENT STRUCTURE VOLUME = 2,300 CY

Quantities are approximate only. The Resident Engineer will recross-section the work accurately when the project is staked out. These cross-section notes will be used in computing the final quantities for which the contractor will be paid.

COMPUTED BY: D. Matthew Brewer DATE: 1/5/2026
 CHECKED BY: Robert E. Kral DATE: 1/5/2026

(9-17-24)

PROJECT NO.	SHEET NO.
A-0009CD	3G-1

STATE OF NORTH CAROLINA DIVISION OF HIGHWAYS

SUMMARY OF SUBSURFACE DRAINAGE

LINE	Station	Station	Location LT/RT/CL	Drain Type* UD/BD/SD	LF
-Y2-	123+50	128+50	LT to RT	SD	1000
CONTINGENCY				SD	200
				TOTAL LF:	1200

*UD = Underdrain
 *BD = Blind Drain
 *SD = Subsurface Drain

SUMMARY OF AGGREGATE SUBGRADE/STABILIZATION

LINE	Station	Station	Aggregate Type* ASU(1/2)/AST	Aggregate Thickness INCHES [8" for ASU(2)]	Shallow Undercut CY	Class IV Subgrade Stabilization TONS	Geotextile for Subgrade Stabilization SY	Stabilizer Aggregate TONS	Class IV Aggregate Stabilization TONS
-Y2-	90+00	95+00	ASU (1)	12		690	1580		
-Y2-	118+00	126+50	ASU (1)	12		1180	2750		
-Y2-	127+50	129+50	ASU (1)	12		270	580		
-Y2-	137+50	141+50	ASU (1)	12		420	950		
CONTINGENCY			ASU (1)	12	150	300	450	150	0
CONTINGENCY			ASU (1)	12	150	300	450		
TOTAL CY/TONS/SY:					300	1290**	2430**	150	0

*ASU(1/2) = Aggregate Subgrade (Type 1 or 2)
 *AST = Aggregate Stabilization

**Total tons of "Class IV Subgrade Stabilization" and total square yards of "Geotextile for Subgrade Stabilization" are only the estimated quantities for ASU(1/2)/AST and may only represent a portion of the subgrade stabilization and geotextile quantities shown in the Item Sheets of the Proposal.

SUMMARY OF PRE-SPLITTING OF ROCK

LINE	Beginning Rock Cut Slope (H:V)	Approx. Station	Ending Rock Cut Slope (H:V)	Approx. Station	Location LT/RT	Pre-splitting of Rock SY
-Y2-	0.5:1	134+00	0.5:1	137+50	RT	1750
-Y2-	0.5:1	153+50	0.5:1	158+25	RT	1850
-Y2-	0.5:1	156+50	0.5:1	160+00	LT	4000
TOTAL SY:						7600

SUMMARY OF HORIZONTAL DRAINS

LINE	Approximate Station	Location LT/RT	Elevation Above or Below Grade (+/-) FT	Inclination Angle DEGREES	PVC Pipe Schedule 40/80 or NO PIPE	Horizontal Drain FT	Horizontal Drain W/O Pipe FT
CONTINGENCY						210	
TOTAL FT:						210	0

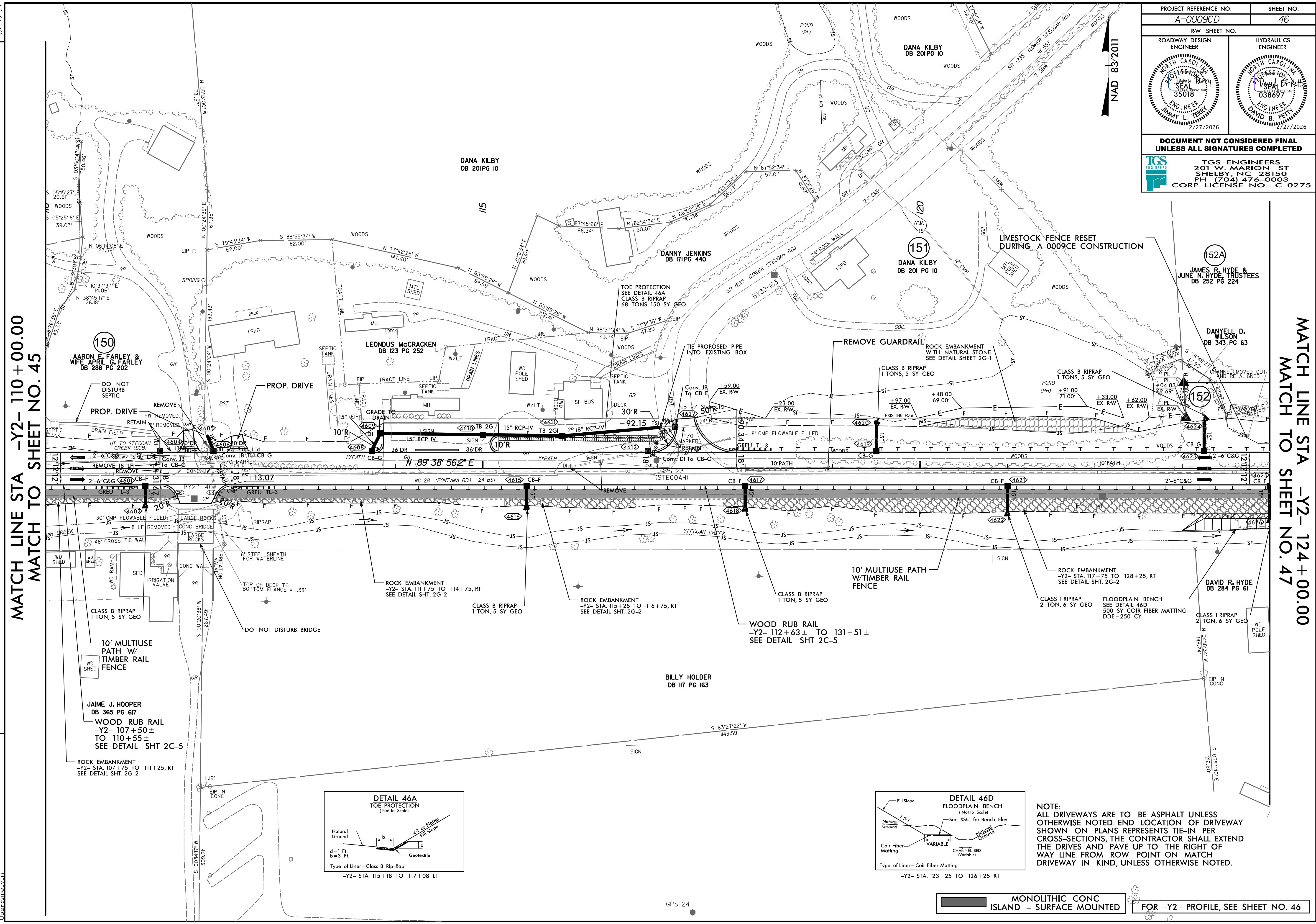
SUMMARY OF REINFORCED SOIL SLOPES AND SLOPE EROSION CONTROL

LINE	Beginning Slope/ RSS (H:V)	Approx. Station	Ending Slope/ RSS (H:V)	Approx. Station	Location LT/RT	Reinforced Soil Slope (RSS) SY	Geocells SY	Coir Fiber Mat SY	Matting for Erosion Control SY
-Y2-	1.5:1	100+75	1.5:1	102+75	LT			2930	
-Y2-	1.5:1	104+25	1.5:1	106+75	LT			2350	
-Y2-	1.5:1	133+75	1.5:1	137+75	RT			12500	
-Y2-	1.5:1	145+25	1.5:1	147+25	RT			1140	
-Y2-	1.5:1	148+25	1.5:1	158+25	RT			25850	
-Y2-	1.5:1	156+25	1.5:1	162+25	LT			11000	
TOTAL SY:						0	0	55770*	0

*Total square yards of "Coir Fiber Mat" is only the estimated quantity for slopes steeper than 2:1 (H:V) and may only represent a portion of the coir fiber mat quantity shown in the Item Sheets of the Proposal.

**Total square yards of "Matting for Erosion Control" is only the estimated quantity for RSS and may only represent a portion of the matting quantity shown in the Item Sheets of the Proposal.

PROJECT REFERENCE NO. A-0009CD		SHEET NO. 46	
ROADWAY DESIGN ENGINEER		HYDRAULICS ENGINEER	
DOCUMENT NOT CONSIDERED FINAL UNLESS ALL SIGNATURES COMPLETED			
 TGS ENGINEERS 201 W. MARION ST SHELBY, NC 28150 PH: (704) 476-0003 CORP. LICENSE NO.: C-0275			

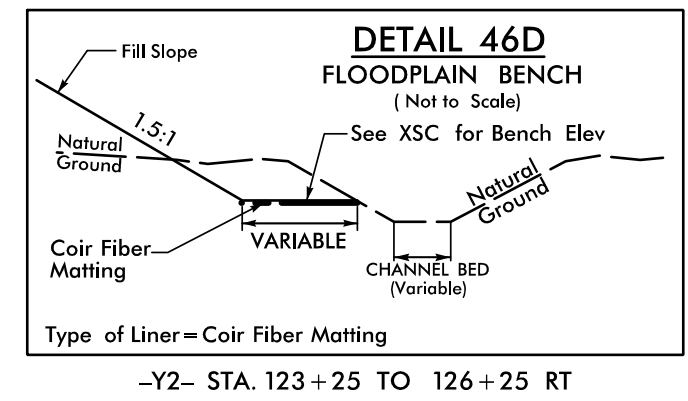
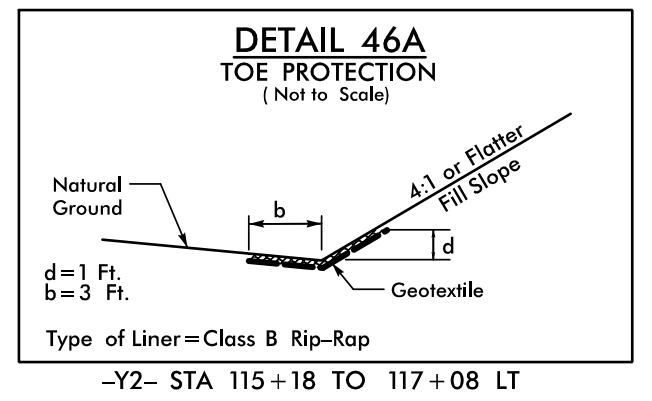


MATCH LINE STA -Y2- 110 + 00.00
 MATCH TO SHEET NO. 45

MATCH LINE STA -Y2- 124 + 00.00
 MATCH TO SHEET NO. 47

REVISIONS

2/24/2026
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 jhooper

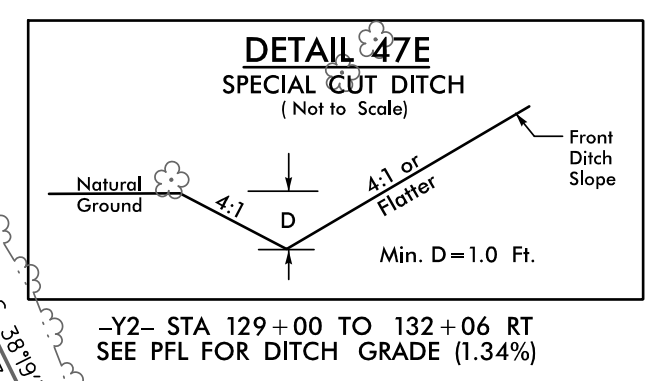
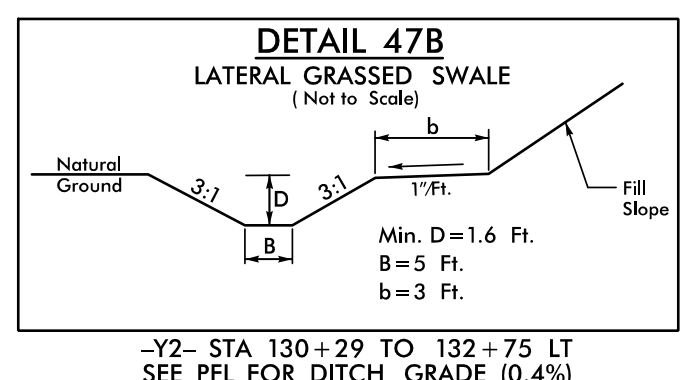
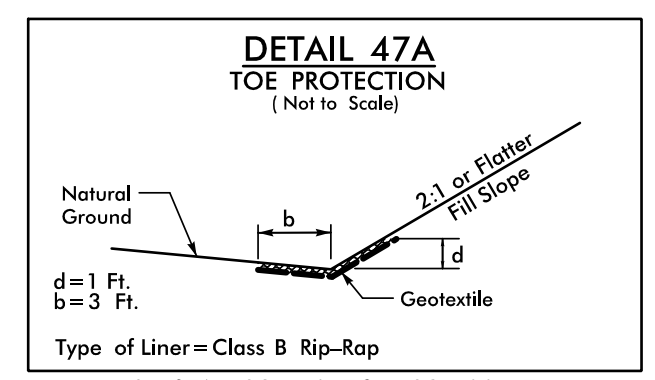


NOTE:
ALL DRIVEWAYS ARE TO BE ASPHALT UNLESS OTHERWISE NOTED. END LOCATION OF DRIVEWAY SHOWN ON PLANS REPRESENTS TIE-IN PER CROSS-SECTIONS, THE CONTRACTOR SHALL EXTEND THE DRIVES AND PAVE UP TO THE RIGHT OF WAY LINE. FROM ROW POINT ON MATCH DRIVEWAY IN KIND, UNLESS OTHERWISE NOTED.

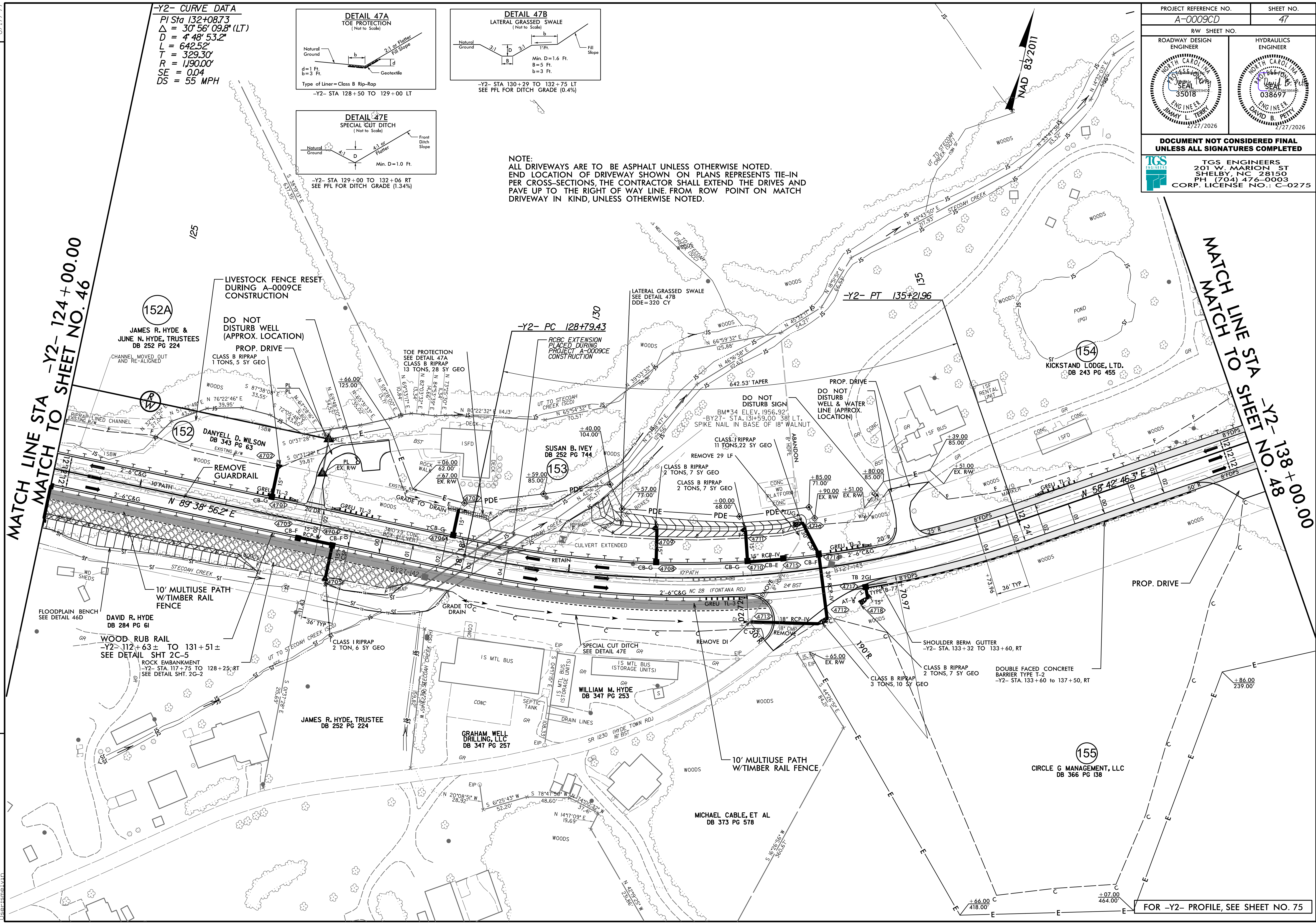
MONOLITHIC CONC ISLAND - SURFACE MOUNTED
 FOR -Y2- PROFILE, SEE SHEET NO. 46

PROJECT REFERENCE NO. A-0009CD		SHEET NO. 47	
RW SHEET NO.		HYDRAULICS ENGINEER	
ROADWAY DESIGN ENGINEER		ENGINEER	
<p>DOCUMENT NOT CONSIDERED FINAL UNLESS ALL SIGNATURES COMPLETED</p>			
<p>TGS ENGINEERS 201 W. MARION ST SHELBY, NC 28150 PH (704) 476-0003 CORP. LICENSE NO.: C-0275</p>			

-Y2- CURVE DATA
 PI Sta 132+08.73
 $\Delta = 30^{\circ}56'09.8''$ (LT)
 $D = 4^{\circ}48'53.2''$
 $L = 642.52'$
 $T = 329.30'$
 $R = 1,190.00'$
 $SE = 0.04$
 $DS = 55$ MPH



NOTE:
 ALL DRIVEWAYS ARE TO BE ASPHALT UNLESS OTHERWISE NOTED.
 END LOCATION OF DRIVEWAY SHOWN ON PLANS REPRESENTS TIE-IN PER CROSS-SECTIONS, THE CONTRACTOR SHALL EXTEND THE DRIVES AND PAVE UP TO THE RIGHT OF WAY LINE FROM ROW POINT ON MATCH DRIVEWAY IN KIND, UNLESS OTHERWISE NOTED.



MATCH LINE STA -Y2- 124+00.00
 MATCH TO SHEET NO. 46

MATCH LINE STA SHEET NO. 48
 -Y2- 138+00.00

FOR -Y2- PROFILE, SEE SHEET NO. 75

REVISIONS

2/24/2026 A-0009CD\Roadway\Proj\A-0009CD_Plan_Sheets\A-0009CD_Plan_sht_47.dgn
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 2/24/2026 A-0009CD\Roadway\Proj\A-0009CD_Plan_Sheets\A-0009CD_Plan_sht_47.dgn
 Sheets\A-0009CD_Plan_Sheets\A-0009CD_Plan_sht_47.dgn

8/17/99

-L- CURVE DATA
 PI Sta 167+46.16
 $\Delta = 72^\circ 53' 32.5" (LT)$
 $D = 4' 58" 56.1"$
 $L = 1,463.04'$
 $T = 849.28'$
 $R = 1,150.00'$
 $SE = 0.06$
 $DS = 55 MPH$

PROP MONOLITHIC CONC ISLAND - SURFACE MOUNTED

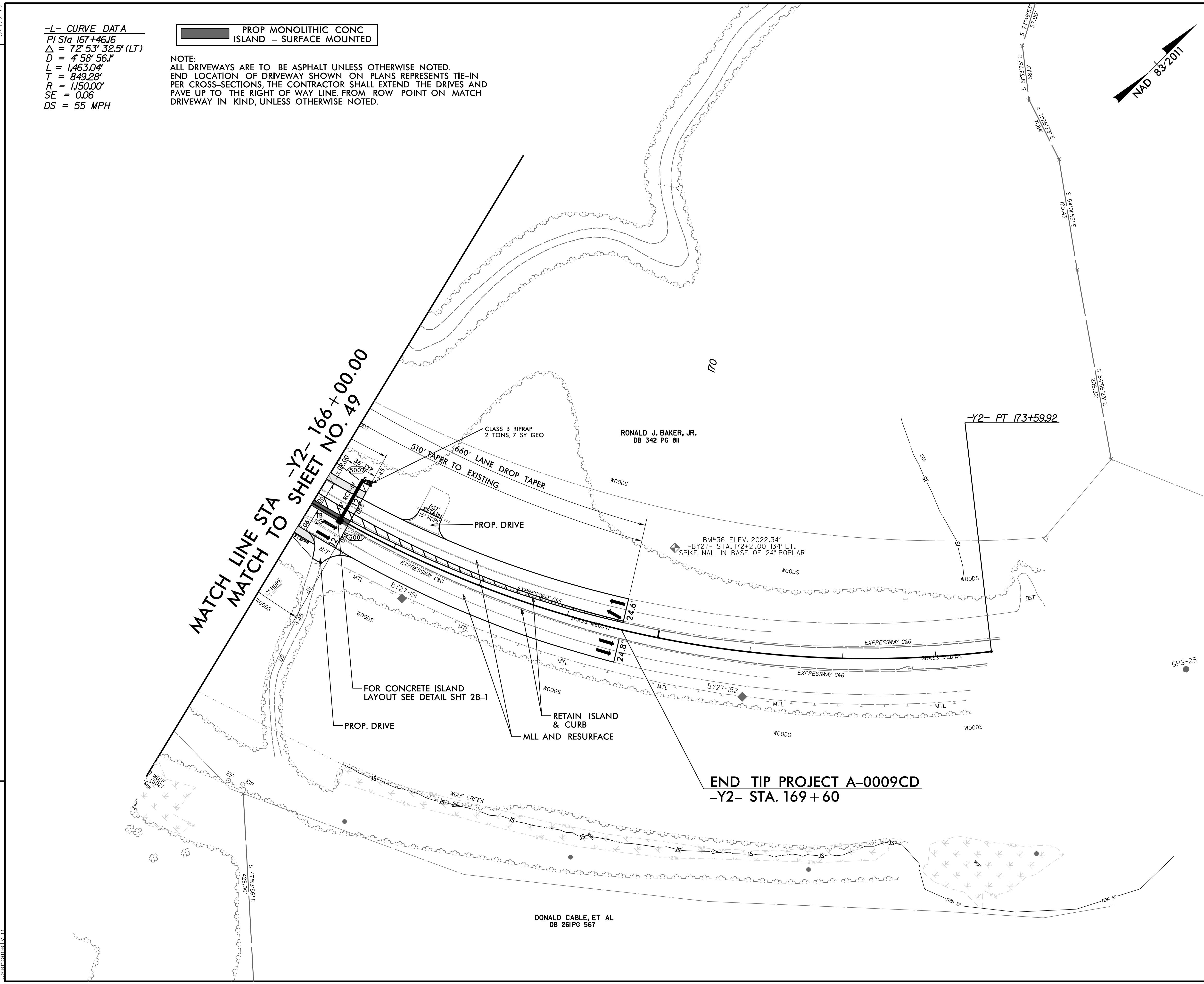
NOTE:
 ALL DRIVEWAYS ARE TO BE ASPHALT UNLESS OTHERWISE NOTED.
 END LOCATION OF DRIVEWAY SHOWN ON PLANS REPRESENTS TIE-IN PER CROSS-SECTIONS, THE CONTRACTOR SHALL EXTEND THE DRIVES AND PAVE UP TO THE RIGHT OF WAY LINE FROM ROW POINT ON MATCH DRIVEWAY IN KIND, UNLESS OTHERWISE NOTED.

PROJECT REFERENCE NO. A-0009CD	SHEET NO. 50
ROADWAY DESIGN ENGINEER JIMMY L. TERRY 35018 2/27/2026	HYDRAULICS ENGINEER DAVID B. PETTY 038697 2/27/2026
DOCUMENT NOT CONSIDERED FINAL UNLESS ALL SIGNATURES COMPLETED	
TGS ENGINEERS 201 W. MARION ST SHELBY, NC 28150 PH: (704) 476-0003 CORP. LICENSE NO.: C-0275	



REVISIONS

1/7/2026
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 License: TGS

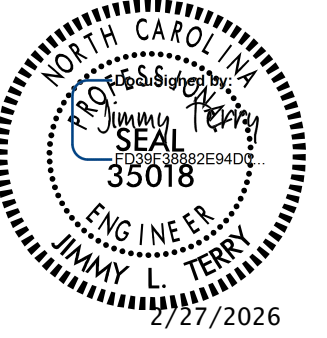
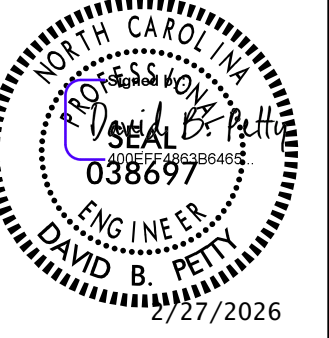



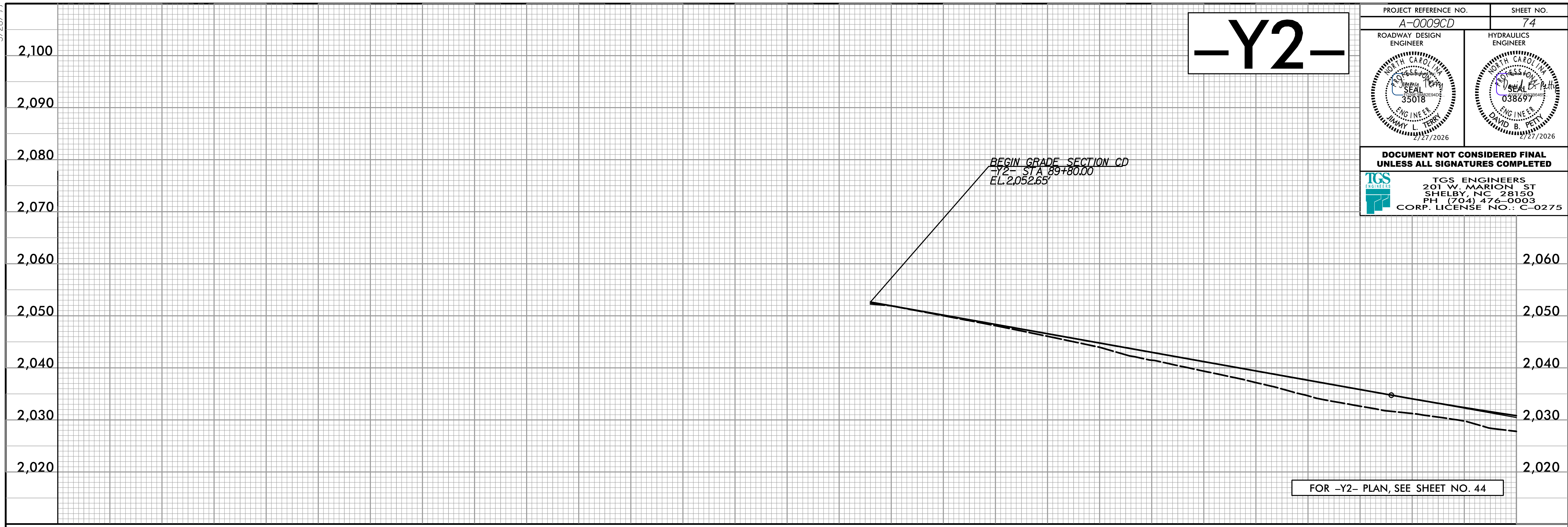
END TIP PROJECT A-0009CD
-Y2- STA. 169 + 60

FOR -Y2- PROFILE, SEE SHEET NO. 77

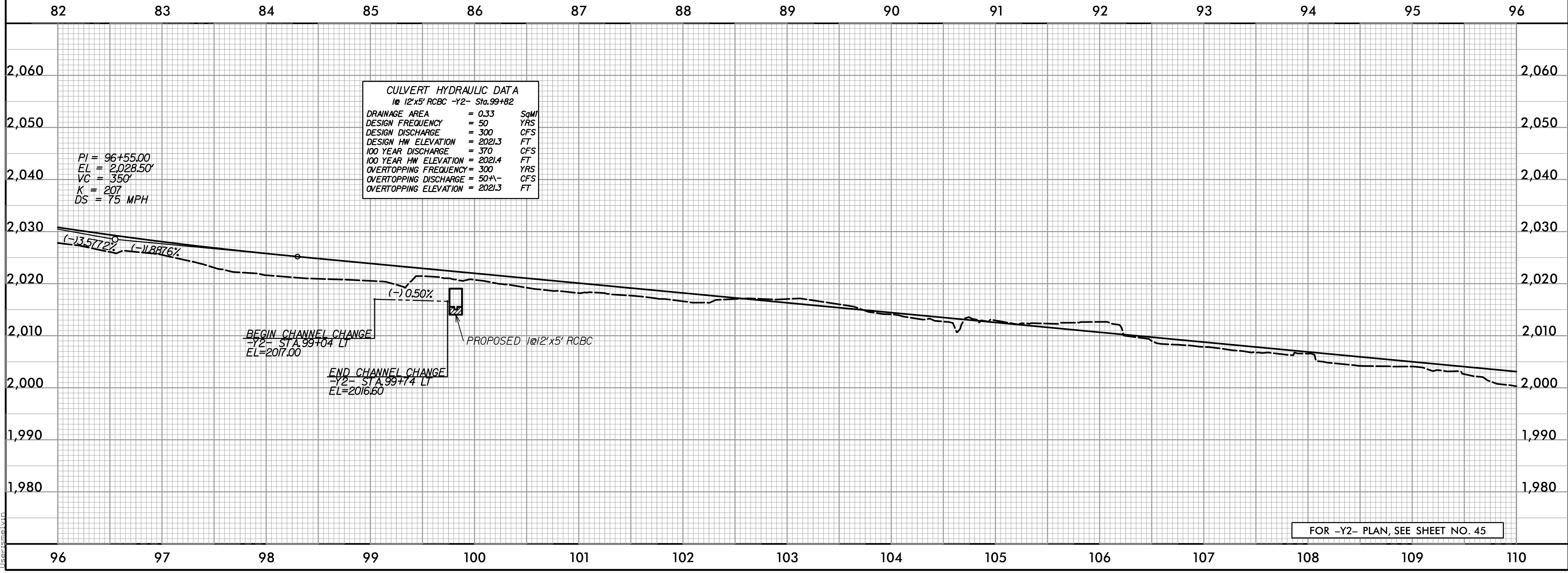
5/28/99

-Y2-

PROJECT REFERENCE NO. A-0009CD	SHEET NO. 74
ROADWAY DESIGN ENGINEER 	HYDRAULICS ENGINEER 
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FOR -Y2- PLAN, SEE SHEET NO. 44

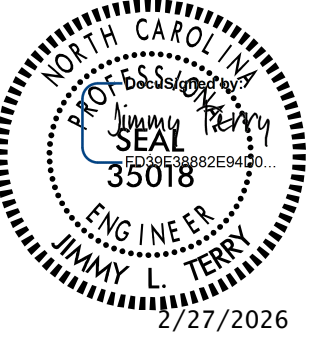
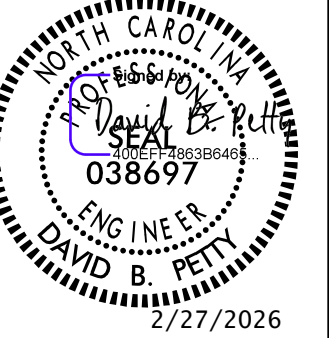



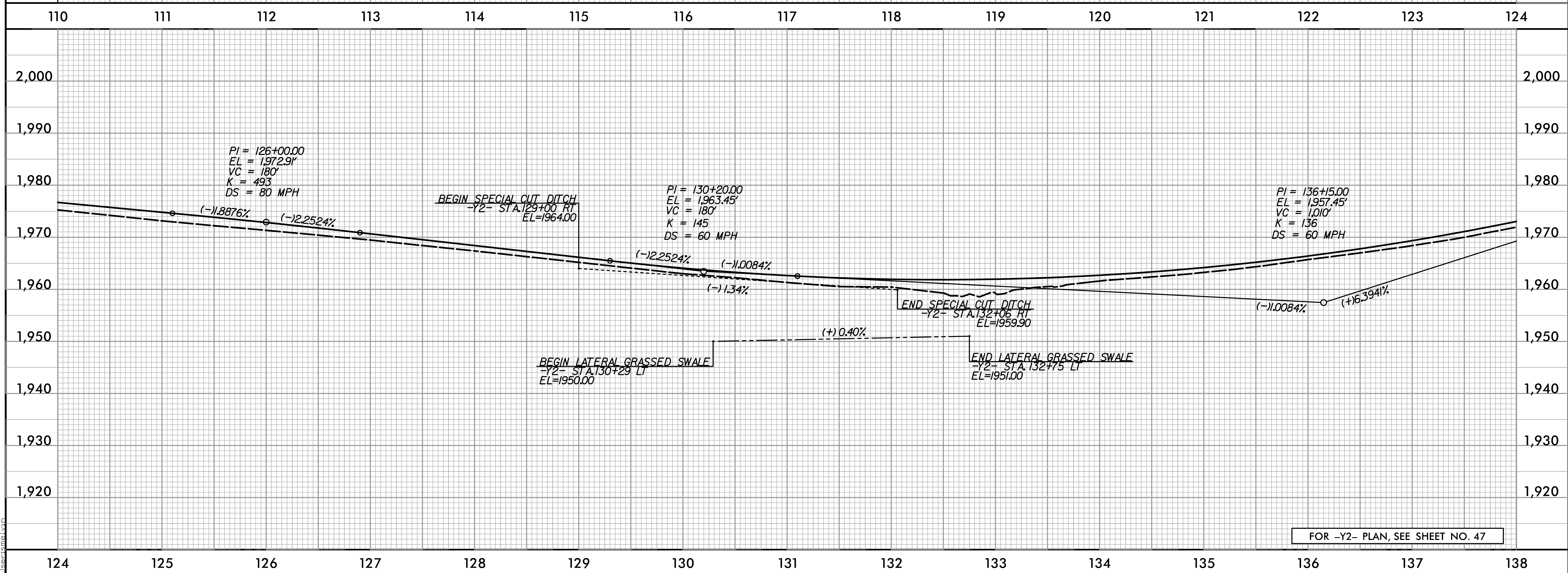
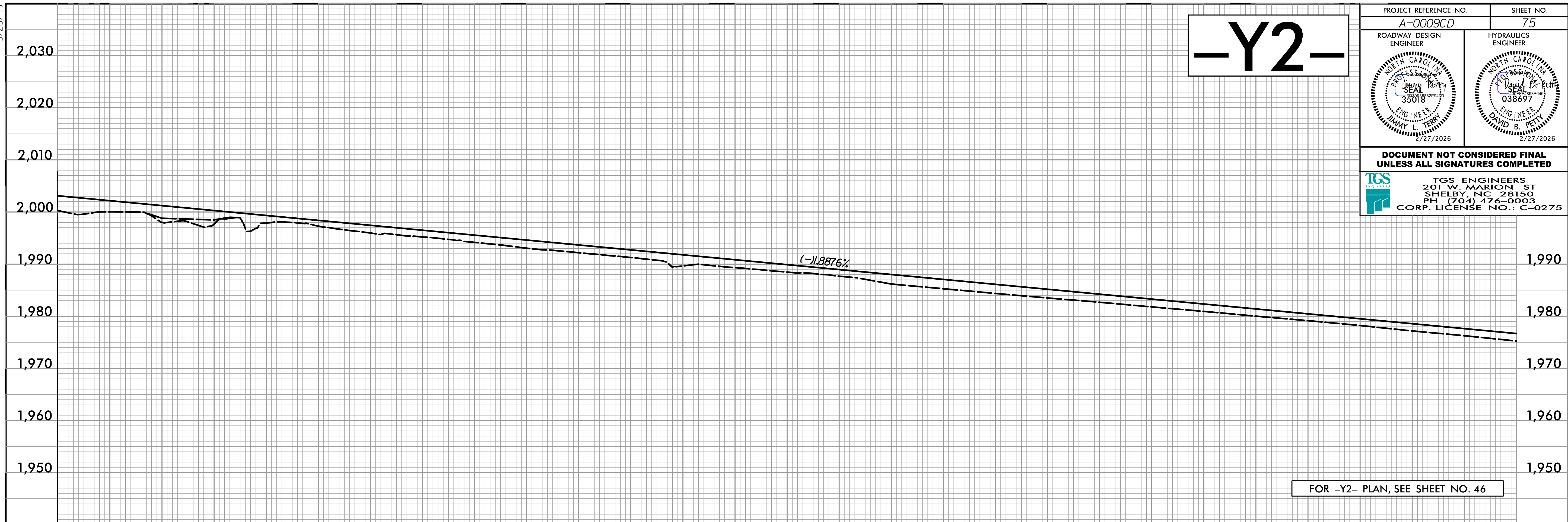
FOR -Y2- PLAN, SEE SHEET NO. 45

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5/28/24

-Y2-

PROJECT REFERENCE NO. A-0009CD	SHEET NO. 75
ROADWAY DESIGN ENGINEER 	HYDRAULICS ENGINEER 
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5/28/2026

-Y2-

PROJECT REFERENCE NO. A-0009CD	SHEET NO. 76
ROADWAY DESIGN ENGINEER	HYDRAULICS ENGINEER
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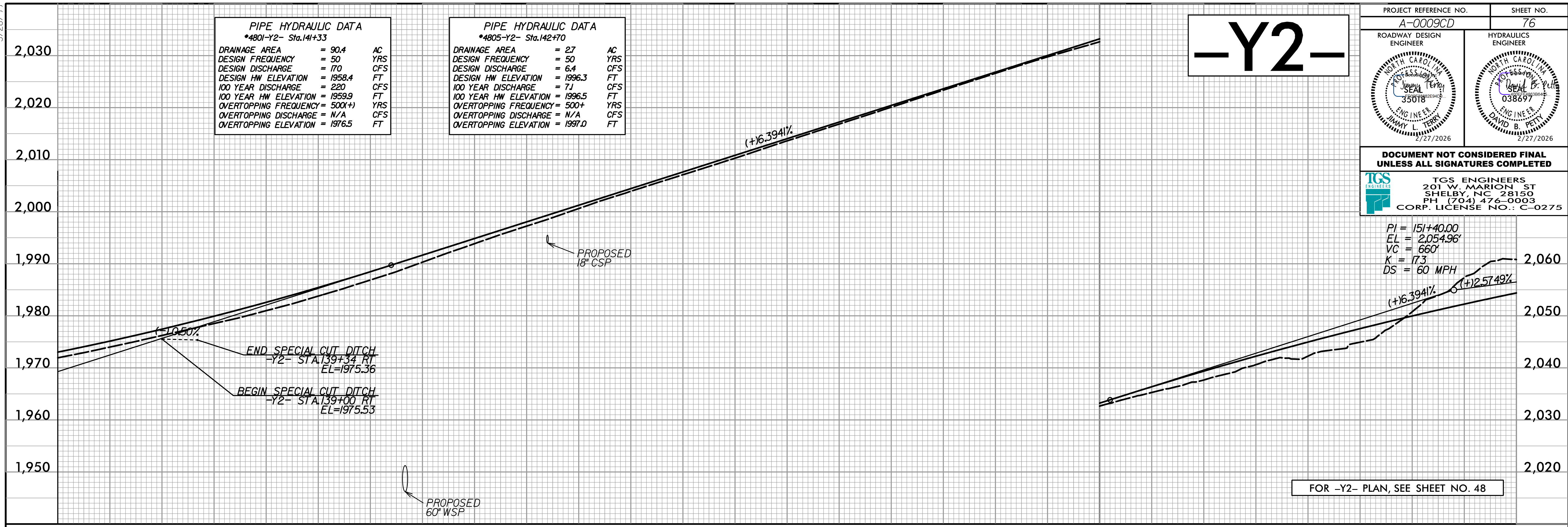
PIPE HYDRAULIC DATA
*4801-Y2- Sta.141+33

DRAINAGE AREA	= 90.4	AC
DESIGN FREQUENCY	= 50	YRS
DESIGN DISCHARGE	= 170	CFS
DESIGN HW ELEVATION	= 1958.4	FT
100 YEAR DISCHARGE	= 220	CFS
100 YEAR HW ELEVATION	= 1959.9	FT
OVERTOPPING FREQUENCY (500+)	= N/A	YRS
OVERTOPPING DISCHARGE	= N/A	CFS
OVERTOPPING ELEVATION	= 1976.5	FT

PIPE HYDRAULIC DATA
*4805-Y2- Sta.142+70

DRAINAGE AREA	= 27	AC
DESIGN FREQUENCY	= 50	YRS
DESIGN DISCHARGE	= 6.4	CFS
DESIGN HW ELEVATION	= 1996.3	FT
100 YEAR DISCHARGE	= 7.1	CFS
100 YEAR HW ELEVATION	= 1996.5	FT
OVERTOPPING FREQUENCY (500+)	= N/A	YRS
OVERTOPPING DISCHARGE	= N/A	CFS
OVERTOPPING ELEVATION	= 1997.0	FT

PI = 151+40.00
EL = 2,054.96'
VC = 660'
K = 173
DS = 60 MPH

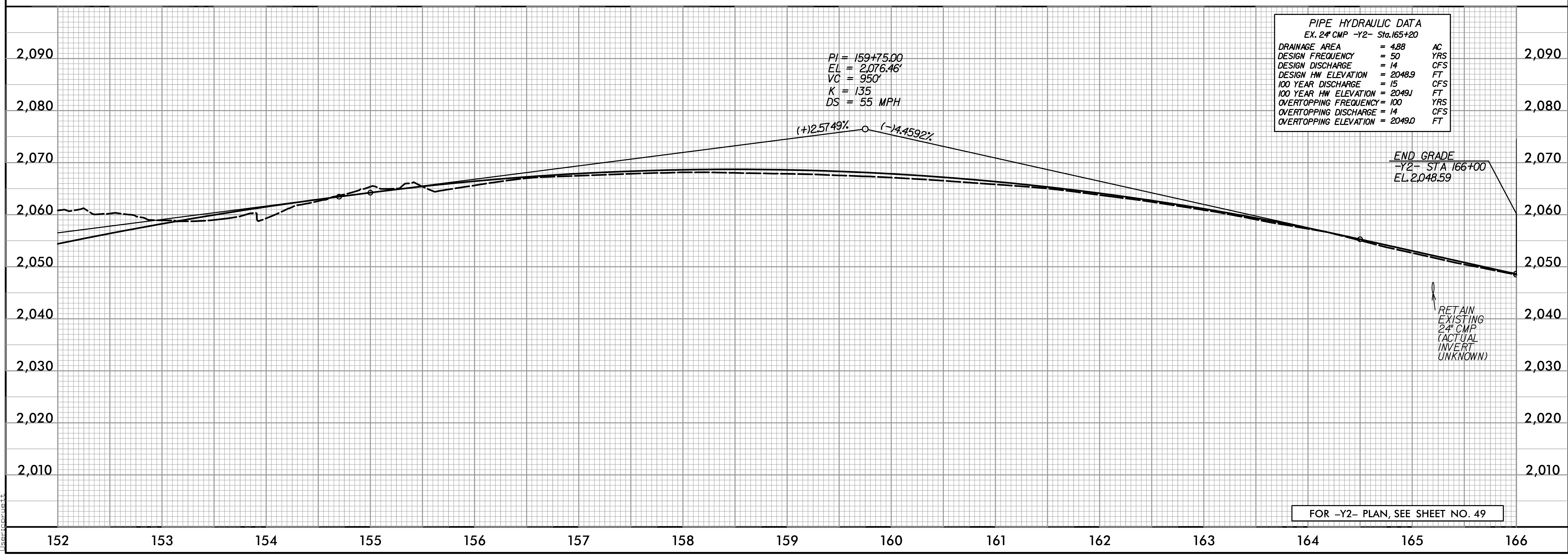


FOR -Y2- PLAN, SEE SHEET NO. 48

PIPE HYDRAULIC DATA
EX. 24" CMP -Y2- Sta.165+20

DRAINAGE AREA	= 488	AC
DESIGN FREQUENCY	= 50	YRS
DESIGN DISCHARGE	= 14	CFS
DESIGN HW ELEVATION	= 2048.9	FT
100 YEAR DISCHARGE	= 15	CFS
100 YEAR HW ELEVATION	= 2049.1	FT
OVERTOPPING FREQUENCY (100)	= 100	YRS
OVERTOPPING DISCHARGE	= 14	CFS
OVERTOPPING ELEVATION	= 2049.0	FT

PI = 159+75.00
EL = 2,076.46'
VC = 950'
K = 135
DS = 55 MPH



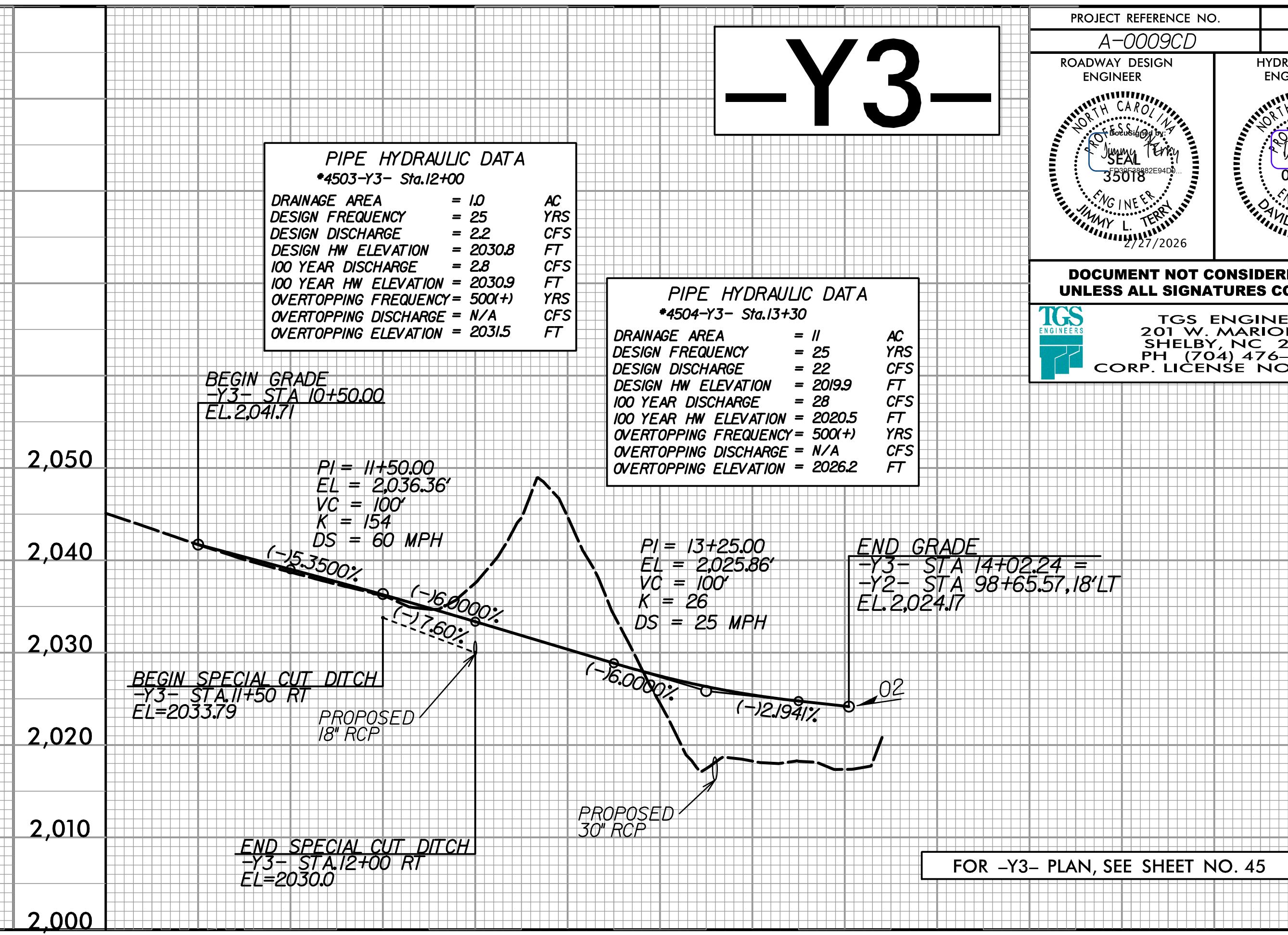
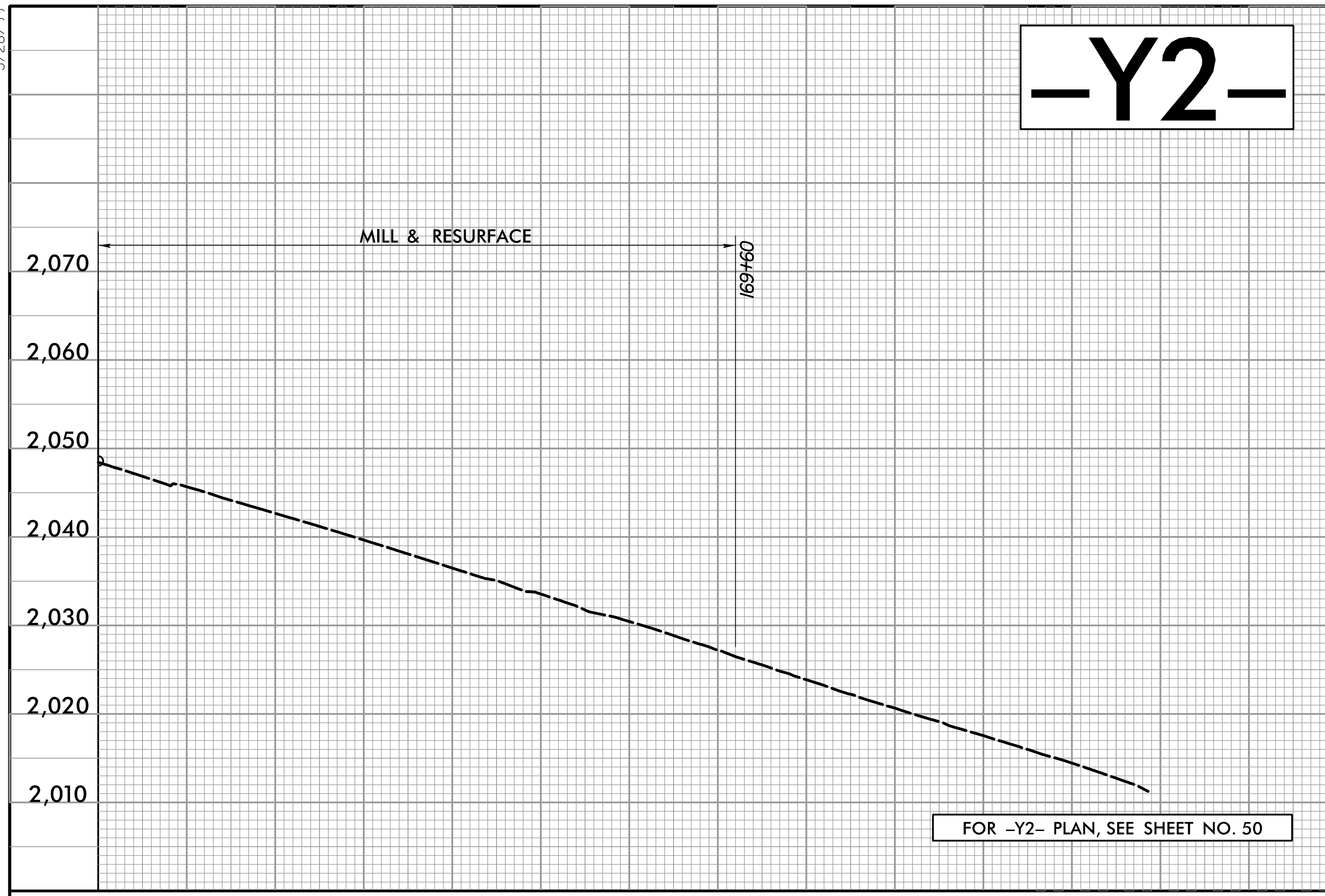
FOR -Y2- PLAN, SEE SHEET NO. 49

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5/28/99

-Y2-

-Y3-



PIPE HYDRAULIC DATA
*4503-Y3- Sta.12+00

DRAINAGE AREA	= 10	AC
DESIGN FREQUENCY	= 25	YRS
DESIGN DISCHARGE	= 2.2	CFS
DESIGN HW ELEVATION	= 2030.8	FT
100 YEAR DISCHARGE	= 2.8	CFS
100 YEAR HW ELEVATION	= 2030.9	FT
OVERTOPPING FREQUENCY (500X+)	= N/A	YRS
OVERTOPPING DISCHARGE	= N/A	CFS
OVERTOPPING ELEVATION	= 2031.5	FT

PIPE HYDRAULIC DATA
*4504-Y3- Sta.13+30

DRAINAGE AREA	= 11	AC
DESIGN FREQUENCY	= 25	YRS
DESIGN DISCHARGE	= 2.2	CFS
DESIGN HW ELEVATION	= 2019.9	FT
100 YEAR DISCHARGE	= 2.8	CFS
100 YEAR HW ELEVATION	= 2020.5	FT
OVERTOPPING FREQUENCY (500X+)	= N/A	YRS
OVERTOPPING DISCHARGE	= N/A	CFS
OVERTOPPING ELEVATION	= 2026.2	FT

PROJECT REFERENCE NO. A-0009CD	SHEET NO. 77
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