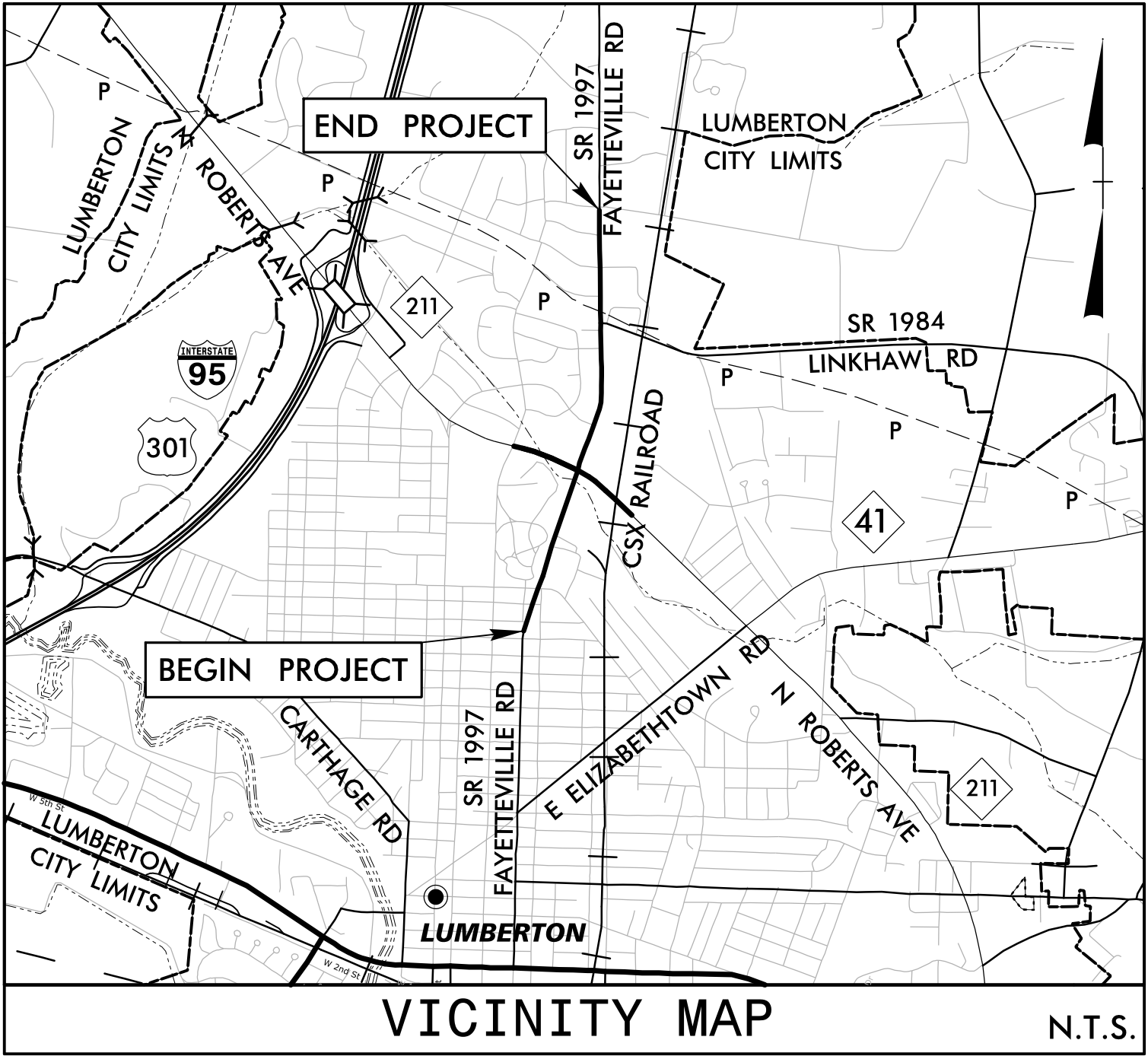


TIP PROJECT: U-5797

CONTRACT: C205036



FINAL PLANS

STATE OF NORTH CAROLINA
DIVISION OF HIGHWAYS

ROBESON COUNTY

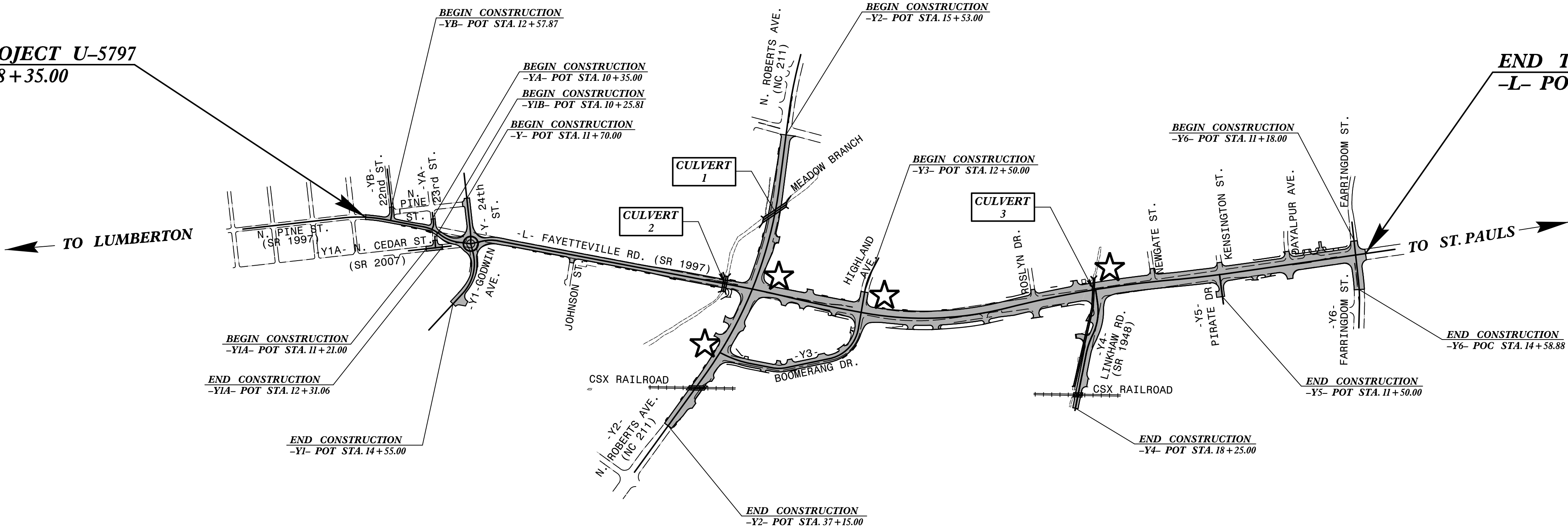
LOCATION: SR 1997 (FAYETTEVILLE ROAD) FROM FARRINGDOM STREET TO EAST 22ND STREET.

TYPE OF WORK: GRADING, PAVING, DRAINAGE, CULVERTS, AND SIGNALS

STATE	STATE PROJECT REFERENCE NO.	SHEET NO.	TOTAL SHEETS
N.C.	U-5797	1	
STATE PROJ. NO.	F.A. PROJ. NO.	DESCRIPTION	
44367.1.1	N/A	PE	
44367.2.1	N/A	R.O.W.	
44367.2.2	N/A	UTIL	
44367.3.1	N/A	CONSTRUCTION	



BEGIN TIP PROJECT U-5797
-L- POC STA. 18+35.00



END TIP PROJECT U-5797
-L- POT STA. 87+20.00



STRUCTURES

DOCUMENT NOT CONSIDERED FINAL
UNLESS ALL SIGNATURES COMPLETED

DESIGN DATA

ADT 2025 = 34,600
ADT 2045 = 41,600
K = 55 %
D = 9 %
T = 4 % *
V = 50 MPH
* TTST =1% DUAL =3%
FUNC CLASS =
URBAN ARTERIAL
REGIONAL TIER

PROJECT LENGTH

LENGTH OF ROADWAY TIP PROJECT U-5797 = 1.299 MILES
LENGTH OF STRUCTURE TIP PROJECT U-5797 = 0.005 MILES
TOTAL LENGTH OF TIP PROJECT U-5797 = 1.304 MILES

NCDOT CONTACT: CRAIG A. FREEMAN JR., PE
NCDOT DIVISION 6

P.O. Box 1150
558 Gillespie Street (Delivery)
Fayetteville, NC 28302
910-364-0603 Office
910-364-0841 Direct
cafreeman2@ncdot.gov

PLANS PREPARED FOR THE NCDOT BY:

stv STV Engineers, Inc.
1600 Perimeter Park Dr., Suite 225
Morrisville, NC 27560
NC License Number F-0991

RIGHT OF WAY DATE:
July 22, 2022

JASON T. GRISCOM, PE
PROJECT ENGINEER

LETTING DATE:
October 21, 2025

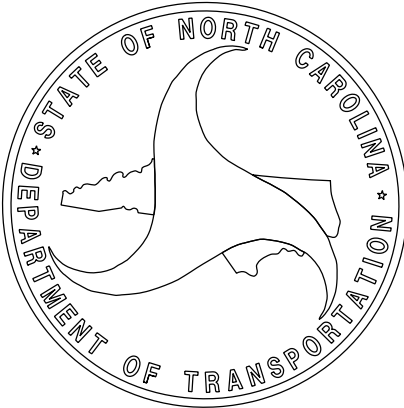
SPENCER G. HENSLEY, PE
PROJECT DESIGNER

STRUCTURES ENGINEER



Signed by:
Jason Griscorn
P.E.

P.E.



REVISIONS						SHEET C1-1
NO.	BY:	DATE:	NO.	BY:	DATE:	
1			3			TOTAL SHEETS 12
2			4			

3/6/2025

R:\Structures\04_Culverts\Culvert 20+80 -Y2- (Culvert 1)\02_Ustation\03_Final Plans\03_U-5797_SML_CUL_002_770C01.dgn

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1/23/2025

hensleg

DRAWN BY : SGH

CHECKED BY : MLO

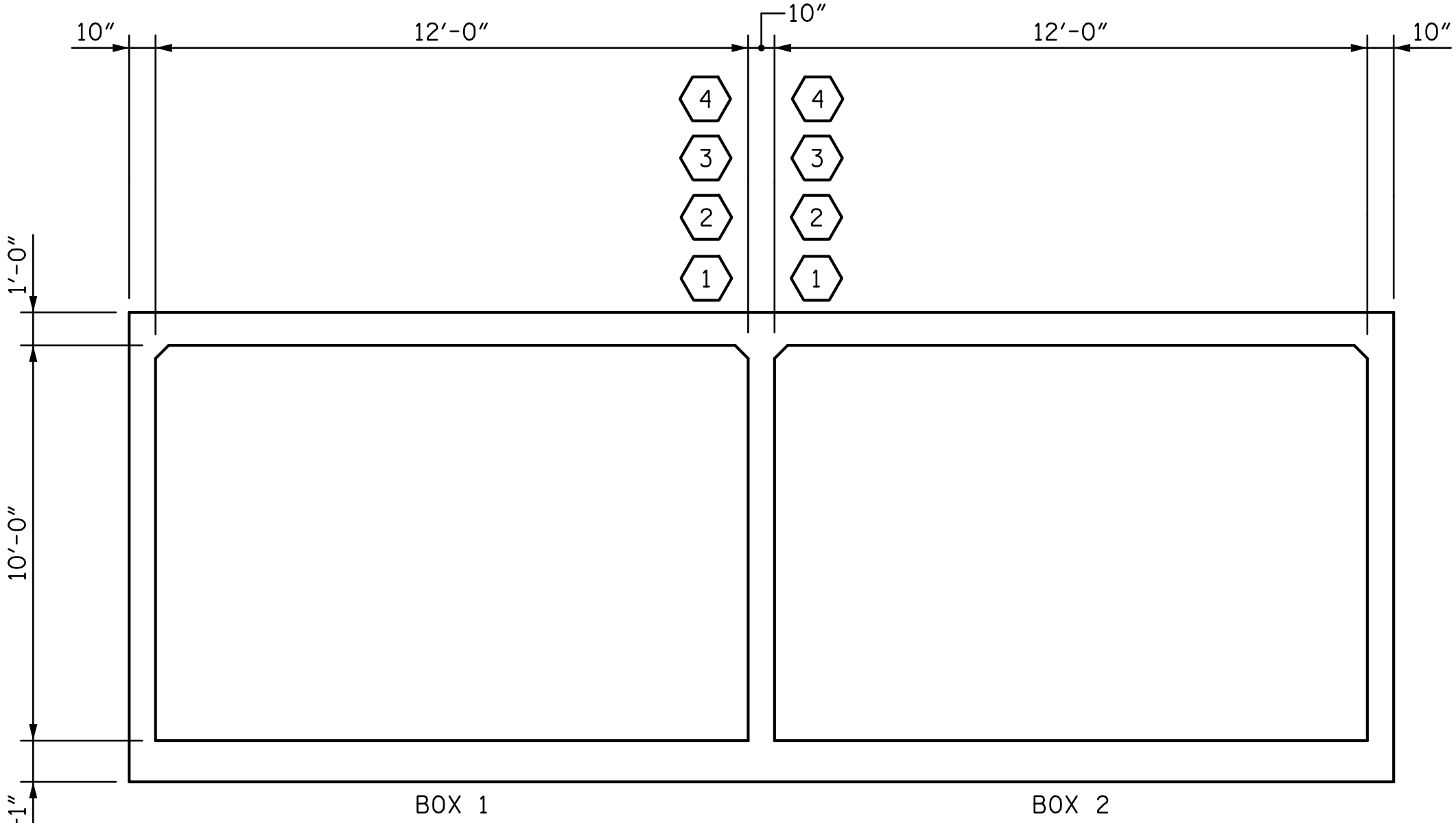
DESIGN ENGINEER OF RECORD : J. GRISCOM

DATE : 11-24

DATE : 11-24

DATE : 11-24

LOAD AND RESISTANCE FACTOR RATING (LRFR) SUMMARY FOR REINFORCED CONCRETE BOX CULVERTS															
LOAD TYPE	VEHICLE	WEIGHT (W) (TONS)	CONTROLLING LOAD RATING ⬡	MINIMUM RATING FACTORS (RF)	TONS = W × RF	STRENGTH I LIMIT STATE									COMMENT NUMBER
						LIVE-LOAD FACTORS (γ _{LL})	MOMENT				SHEAR				
							RATING FACTOR	BOX NO.	ELEMENT TYPE	DISTANCE FROM LEFT END OF ELEMENT (ft)	RATING FACTOR	BOX NO.	ELEMENT TYPE	DISTANCE FROM LEFT END OF ELEMENT (ft)	
DESIGN LOAD RATING	HL-93 (INVENTORY)	N/A	⬡1	1.04	--	1.75	1.23	1 & 2	EXTERIOR WALL	11.08'	1.04	1 & 2	TOP SLAB	12.83'/13.67'	
	HL-93 (OPERATING)	N/A		1.35	--	1.35	1.60	1 & 2	EXTERIOR WALL	11.08'	1.35	1 & 2	TOP SLAB	12.83'/13.67'	
	HS-20 (INVENTORY)	36,000	⬡2	1.39	50,040	1.75	1.60	1 & 2	EXTERIOR WALL	11.08'	1.39	1 & 2	TOP SLAB	12.83'/13.67'	
	HS-20 (OPERATING)	36,000		1.80	64,800	1.35	2.07	1 & 2	EXTERIOR WALL	11.08'	1.80	1 & 2	TOP SLAB	12.83'/13.67'	
LEGAL LOAD RATING	SINGLE VEHICLE (SV)	SNSH		2.46	33,210	1.40	2.46	1 & 2	EXTERIOR WALL	6.06'	2.63	1 & 2	TOP SLAB	12.83'/13.67'	
		SNGARBS2		2.41	48,200	1.40	2.41	1 & 2	EXTERIOR WALL	11.08'	2.41	1 & 2	TOP SLAB	12.83'/13.67'	
		SNAGRIS2		2.46	54,120	1.40	2.46	1 & 2	EXTERIOR WALL	606'	2.52	1 & 2	TOP SLAB	12.83'/13.67'	
		SNCOTTS3	⬡3	1.42	38,695	1.40	1.62	1 & 2	EXTERIOR WALL	11.08'	1.42	1 & 2	TOP SLAB	12.83'/13.67'	
		SNAGGRS4		1.49	52,038	1.40	1.82	1 & 2	EXTERIOR WALL	11.08'	1.49	1 & 2	TOP SLAB	12.83'/13.67'	
		SNS5A		1.67	59,369	1.40	2.02	1 & 2	EXTERIOR WALL	11.08'	1.67	1 & 2	TOP SLAB	12.83'/13.67'	
		SNS6A		1.51	60,325	1.40	1.73	1 & 2	EXTERIOR WALL	11.08'	1.51	1 & 2	TOP SLAB	12.83'/13.67'	
		SNS7B		1.43	60,060	1.40	1.82	1 & 2	EXTERIOR WALL	11.08'	1.43	1 & 2	TOP SLAB	12.83'/13.67'	
	TRUCK TRACTOR SEMI-TRAILER (TTST)	TNAGRIT3		1.89	62,370	1.40	2.46	1 & 2	EXTERIOR WALL	6.06'	1.89	1 & 2	BOTTOM SLAB	12.83'/13.67'	
		TNT4A		1.63	53,912	1.40	1.81	1 & 2	EXTERIOR WALL	11.08'	1.63	1 & 2	TOP SLAB	12.83'/13.67'	
		TNT6A		1.56	64,896	1.40	1.83	1 & 2	EXTERIOR WALL	11.08'	1.56	1 & 2	TOP SLAB	12.83'/13.67'	
		TNT7A		1.65	69,300	1.40	1.81	1 & 2	EXTERIOR WALL	11.08'	1.65	1 & 2	TOP SLAB	12.83'/13.67'	
		TNT7B		1.50	63,000	1.40	1.77	1 & 2	EXTERIOR WALL	11.08'	1.50	1 & 2	TOP SLAB	12.83'/13.67'	
		TNAGRIT4		1.58	67,940	1.40	1.81	1 & 2	EXTERIOR WALL	11.08'	1.58	1 & 2	TOP SLAB	12.83'/13.67'	
		TNAGT5A		1.58	71,100	1.40	2.07	1 & 2	EXTERIOR WALL	11.08'	1.58	1 & 2	TOP SLAB	12.83'/13.67'	
		TNAGT5B		1.48	66,600	1.40	1.81	1 & 2	EXTERIOR WALL	11.08'	1.48	1 & 2	BOTTOM SLAB	12.83'/13.67'	
EMERGENCY VEHICLE (EV)	EV2		1.84	52,900	1.30	2.09	1 & 2	EXTERIOR WALL	11.08'	1.84	1 & 2	TOP SLAB	12.83'/13.67'		
	EV3		⬡4	1.23	52,890	1.30	1.52	1 & 2	EXTERIOR WALL	11.08'	1.23	1 & 2	TOP SLAB	12.83'/13.67'	



LRFR SUMMARY
(LOOKING DOWNSTREAM)

LOAD FACTORS:

DESIGN LOAD RATING FACTORS

LOAD TYPE	MAX FACTOR	MIN FACTOR
DC	1.25	0.90
DW	1.50	0.65
EV	1.30	0.90
EH	1.35	0.90
ES	1.35	0.90
LS	1.75	--
WA	1.00	--

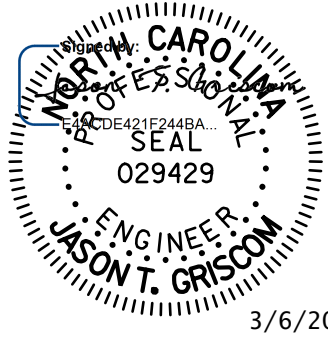
NOTE:

RATING FACTORS ARE BASED ON THE STRENGTH I LIMIT STATE.

1. FOR WALL ELEMENTS THE LEFT END OF ELEMENT IS LOCATED FROM THE BOTTOM OF THE BOTTOM SLAB.

COMMENTS:

⬡ CONTROLLING LOAD RATING
⬡1 DESIGN LOAD RATING (HL-93)
⬡2 DESIGN LOAD RATING (HS-20)
⬡3 LEGAL LOAD RATING **
⬡4 EMERGENCY VEHICLE LAOD RATING **
** SEE CHART FOR VEHICLE TYPE



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2151 Howkins St., Suite 1400
Charlotte, NC 28203
NC License Number F-0991

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PROJECT NO. U-5797

ROBESON COUNTY

STATION: 20+80.00 -Y2-

SHEET 2 OF 12

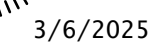
STATE OF NORTH CAROLINA
DEPARTMENT OF TRANSPORTATION
RALEIGH

LRFR SUMMARY FOR
REINFORCED CONCRETE
BOX CULVERTS
(NON-INTERSTATE TRAFFIC)

REVISIONS					SHEET NO.
NO.	BY:	DATE:	NO.	BY:	DATE:
1			3		
2			4		
					TOTAL SHEETS 12

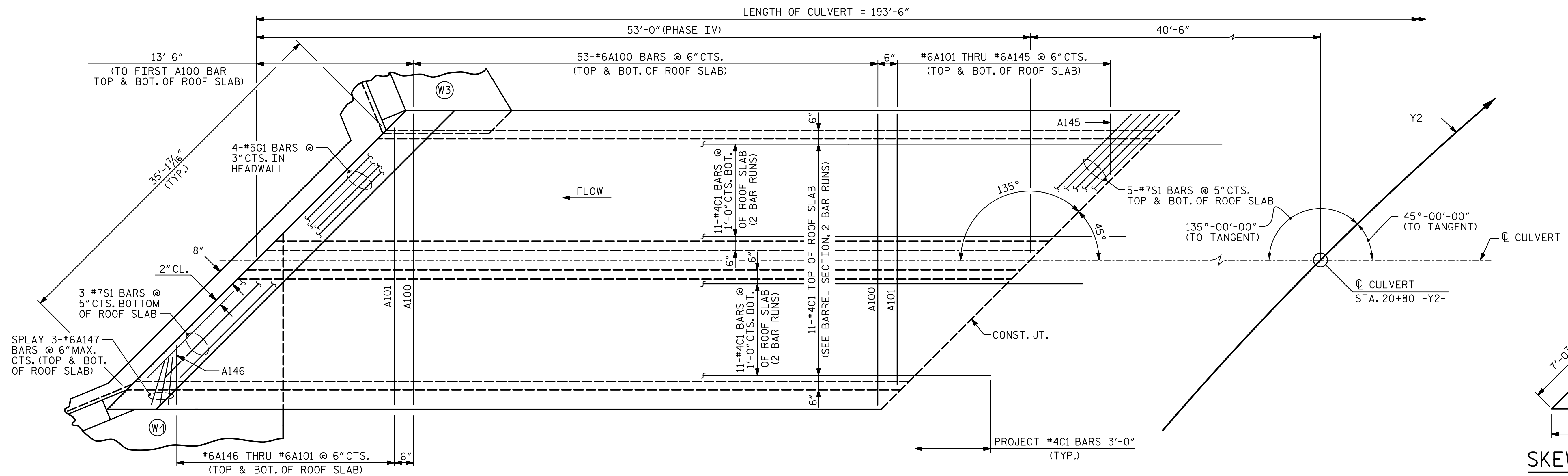


REVISIONS						SHEET NO. C1-3
NO.	BY:	DATE:	NO.	BY:	DATE:	
1			3			TOTAL SHEETS 12
2			4			

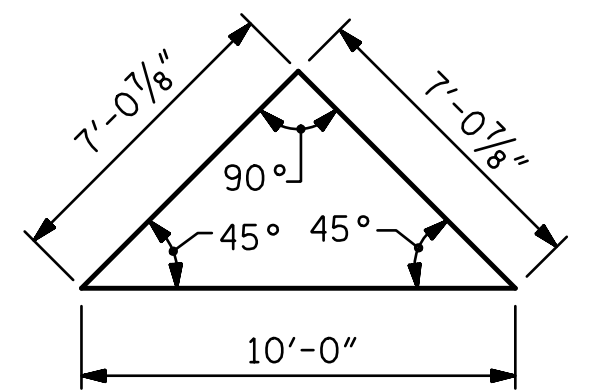


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PLAN ROOF SLAB - PHASE IV - STAGE 2

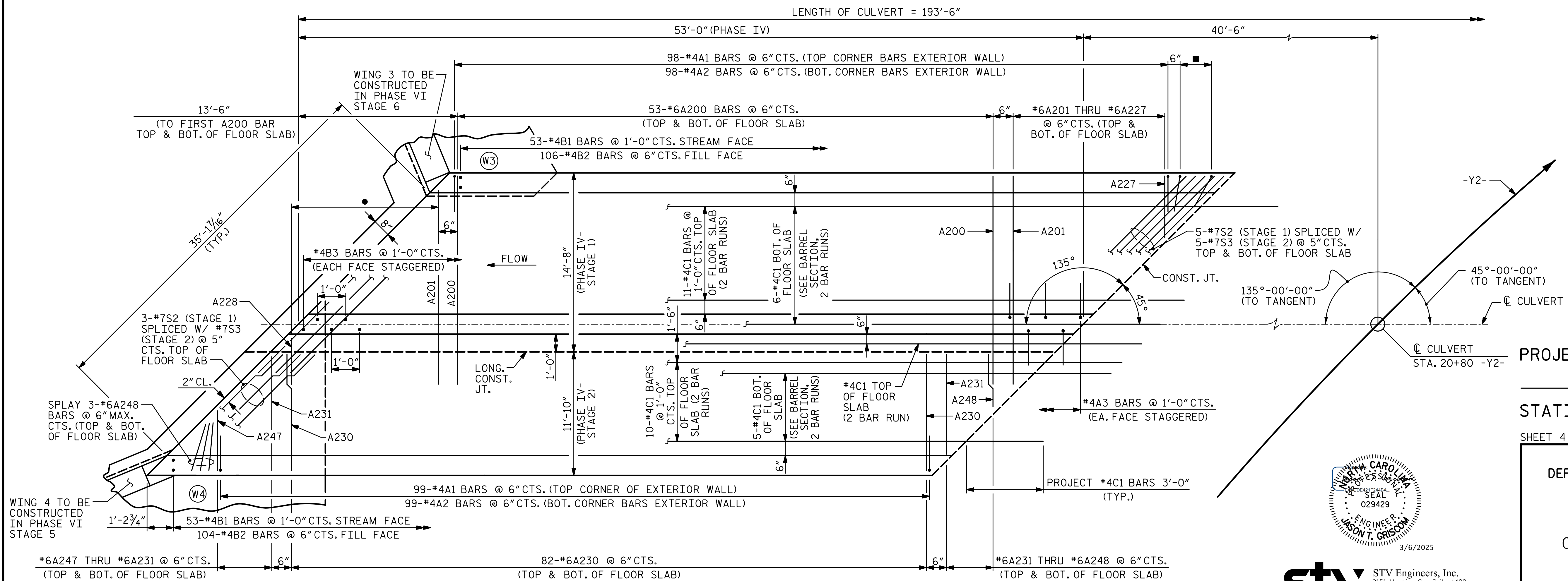


SKEW TRIANGLE

NOTES:

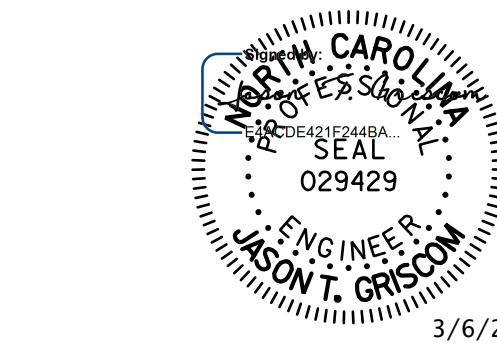
SPLICE A2XX BARS IN STAGE 1
2'-9" MIN. WITH A2XX BARS IN
STAGE 2.

- #6A201 THRU #6A228 @ 6"CTS.
(TOP & BOT. OF FLOOR SLAB)
- SPLAY 7-#4A1 BARS @ 6"CTS. (TOP
CORNER BARS EXTERIOR WALL)
& 7-#4A2 BARS @ 6"CTS. (BOT.
CORNER BARS EXTERIOR WALL)



PLAN FLOOR SLAB AND WALLS - PHASE IV - STAGES 1 & 2

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 CHECKED BY : MLO DATE : 11-24
 DESIGN ENGINEER OF RECORD : J. GRISCOM DATE : 11-24



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Charlotte, NC 28203
NC License Number F-09991

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PROJECT NO. U-5797

ROBESON COUNTY

STATION: 20+80.00 -Y2-

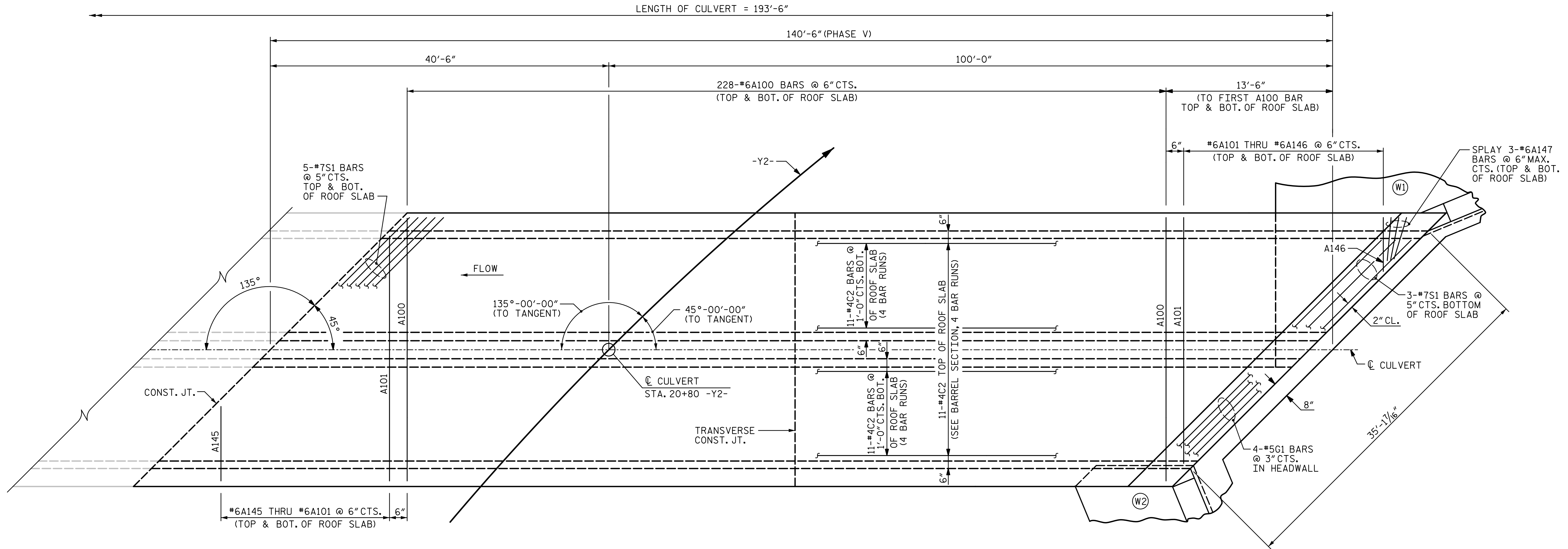
SHEET 4 OF 12

STATE OF NORTH CAROLINA
DEPARTMENT OF TRANSPORTATION
RALEIGH

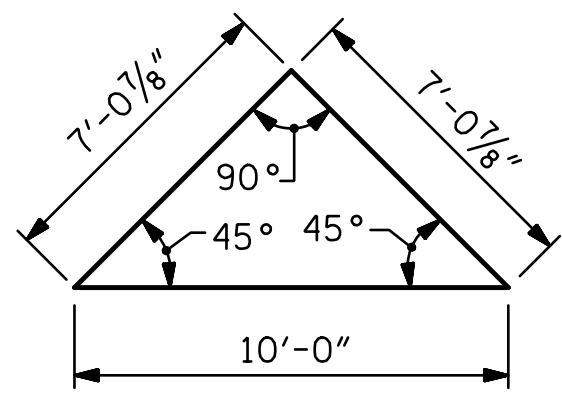
DOUBLE 12'-0" X 10'-0"
CONCRETE BOX CULVERT
45°-00'-00" SKEW

REVISIONS						SHEET NO. C1-4
NO.	BY:	DATE:	NO.	BY:	DATE:	
1			3			TOTAL SHEETS 12
2			4			

SPLICE #4C2 BARS 2'-5"
MIN. TO #4C1 BARS IN
PHASE IV - STAGES 1 & 2



PLAN ROOF SLAB - PHASE V - STAGE 4

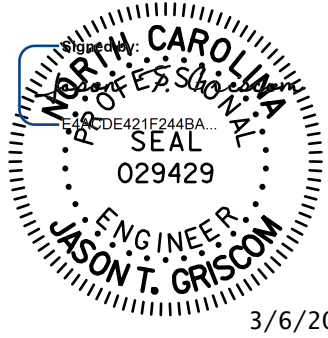


SKEW TRIANGLE

SHEET 5 OF 12

DOUBLE 12'-0" X 10'-0"
CONCRETE BOX CULVERT
45°-00'-00" SKEW

REVISIONS						SHEET N C1-5
NO.	BY:	DATE:	NO.	BY:	DATE:	
1			3			TOTAL SHEETS 12
2			4			



3/6/2025



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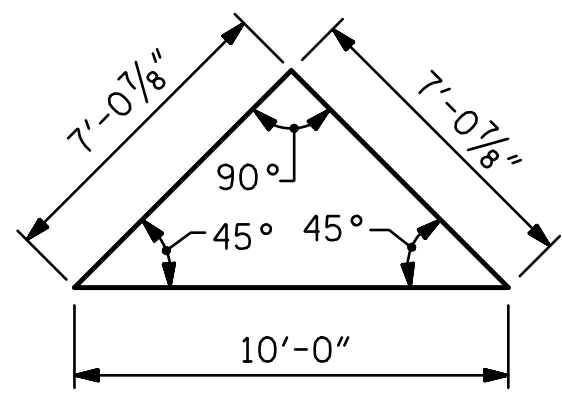
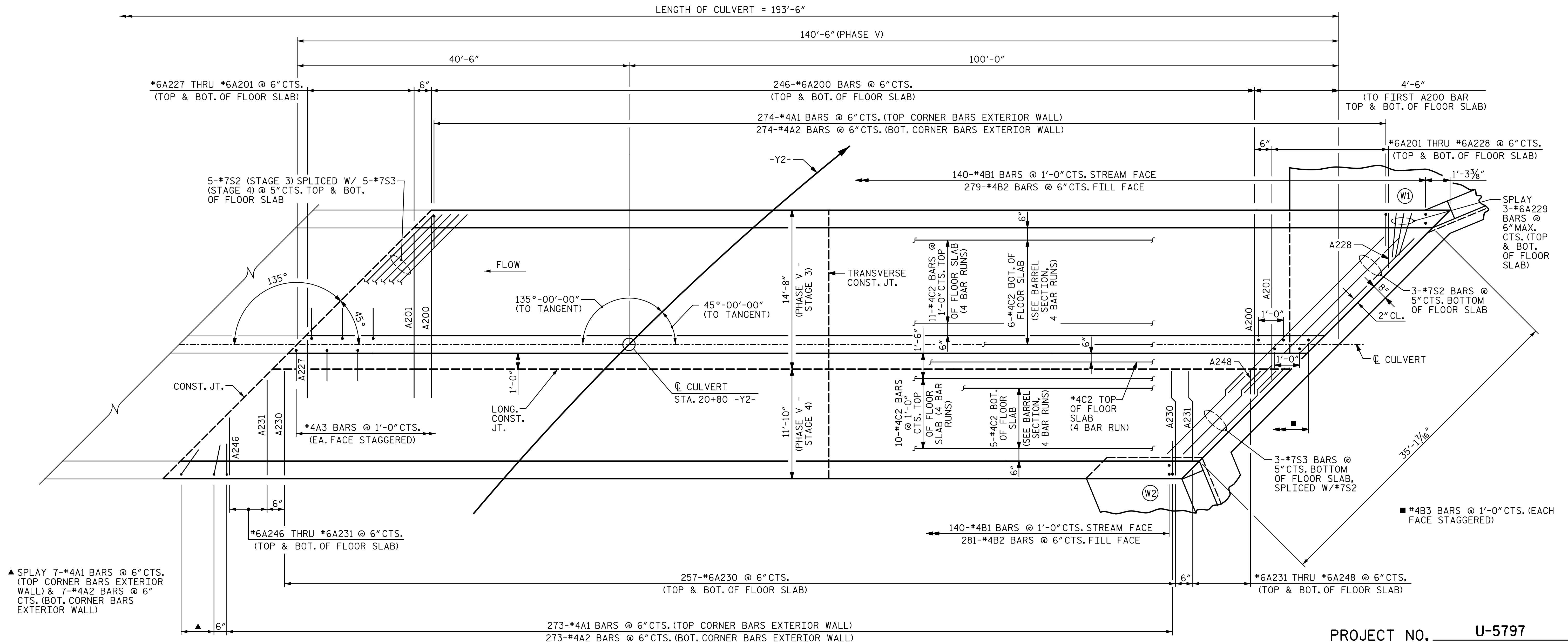
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CHECKED BY :	MLO	DATE :	11-24
DESIGN ENGINEER OF RECORD :	J. GRISCOM	DATE :	11-24

NOTES:

SPLICE #4C2 BARS 2'-5" MIN.
TO #4C1 BARS IN PHASE IV -
STAGES 1 & 2.

SPLICE A2XX BARS IN STAGE
3 2'-9" MIN. WITH A2XX BARS
IN STAGE 4.



SKEW TRIANGLE

PLAN FLOOR SLAB AND WALLS - PHASE V - STAGES 3 & 4

PROJECT NO. U-5797

ROBESON COUNTY

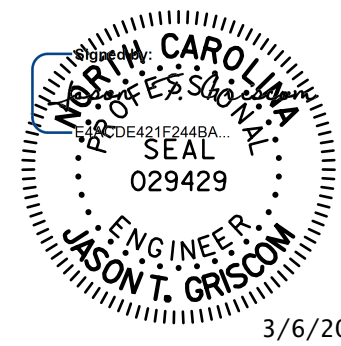
STATION: 20+80.00 -Y2-

SHEET 6 OF 12

STATE OF NORTH CAROLINA
DEPARTMENT OF TRANSPORTATION
RALEIGH

DOUBLE 12'-0" X 10'-0"
CONCRETE BOX CULVERT
45°-00'-00" SKEW

REVISIONS						SHEET NO. C1-6
NO.	BY:	DATE:	NO.	BY:	DATE:	
1			3			TOTAL SHEETS 12
2			4			



3/6/2025

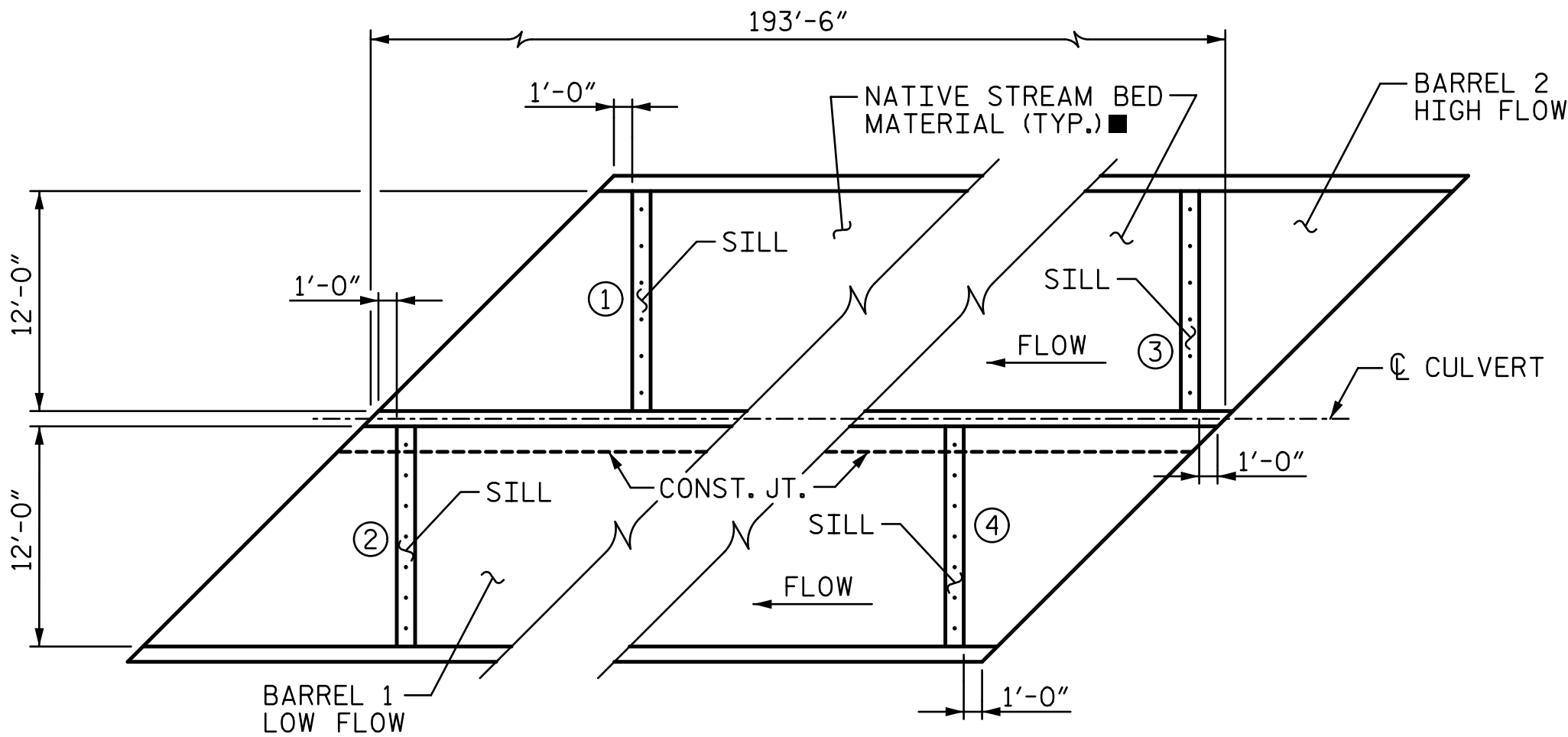


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CHECKED BY :	MLO	DATE :	11-24
DESIGN ENGINEER OF RECORD :	J. GRISCOM	DATE :	11-24

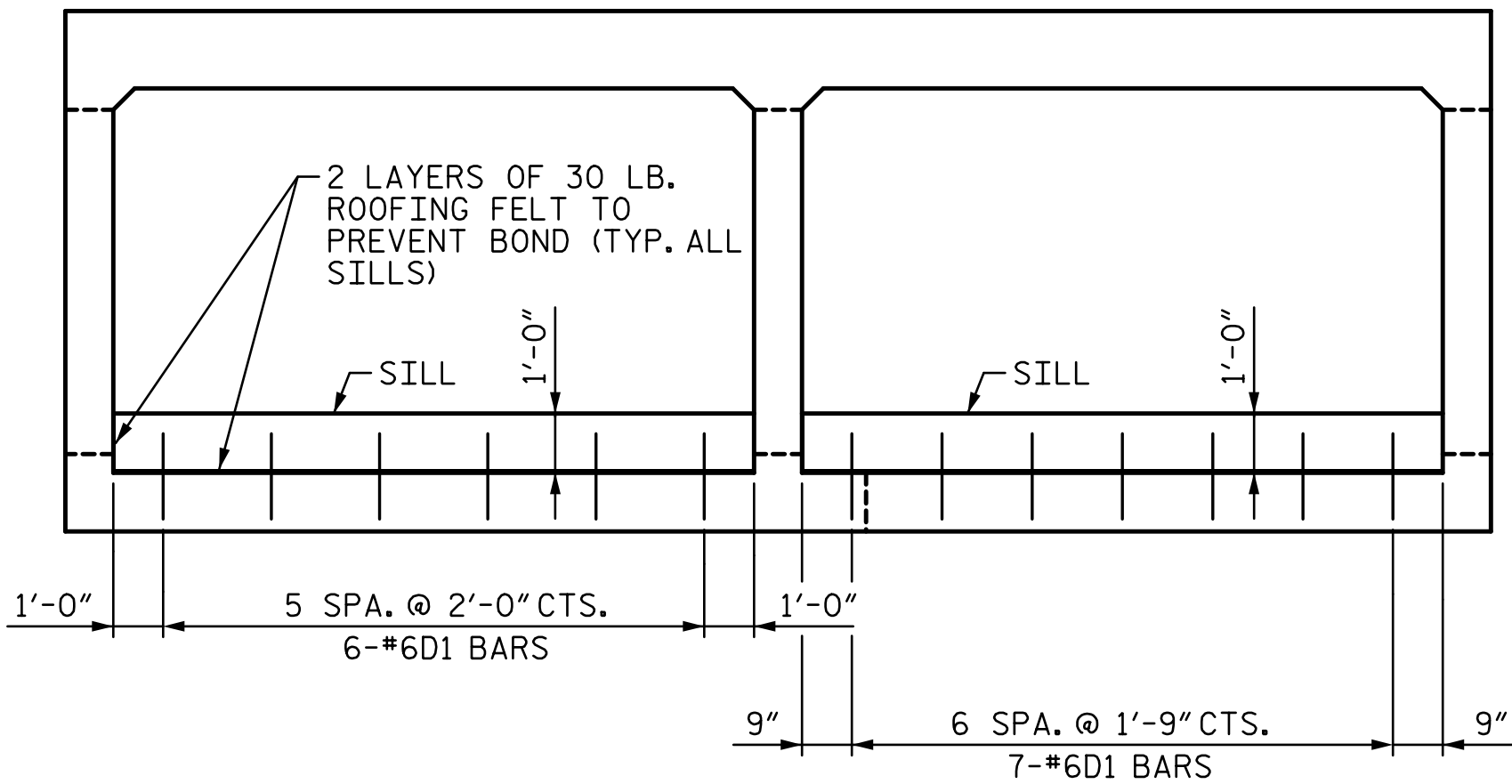
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1/23/2025 1:36:27 PM hnslesg



PLAN VIEW - LOCATION OF SILLS

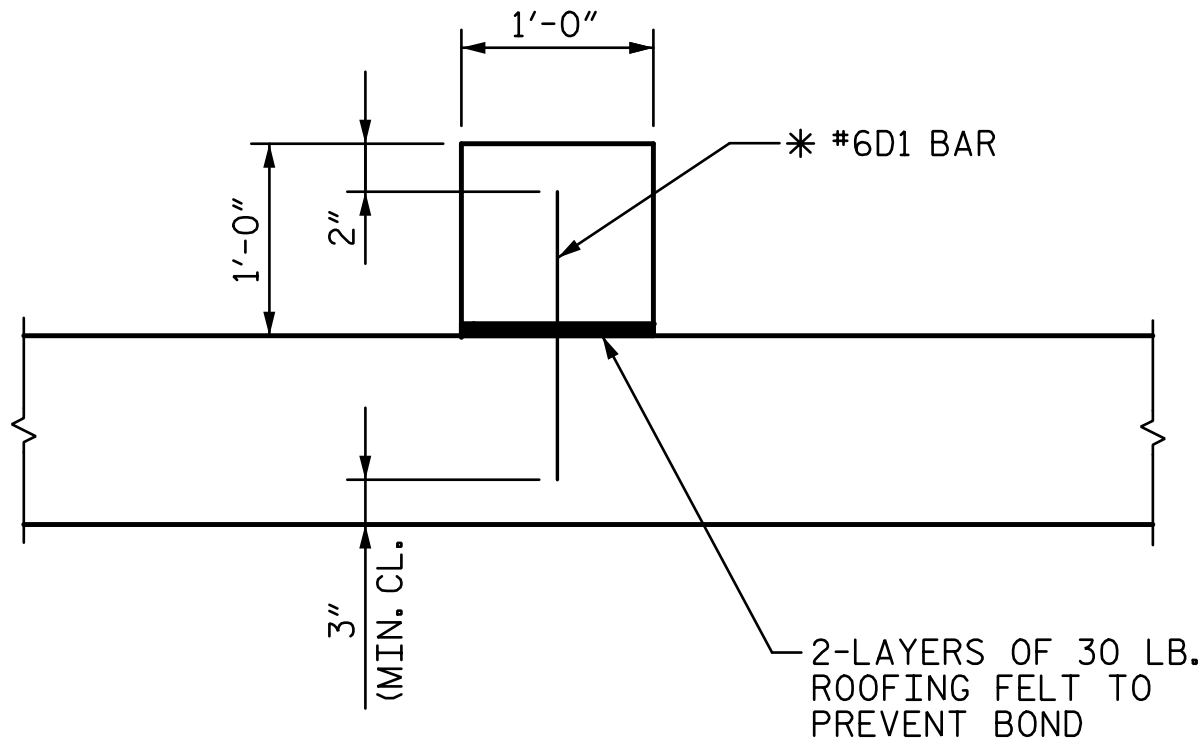
■ NATIVE STREAM BED MATERIAL SHALL BE USED TO BACKFILL THE CULVERT TO SILL HEIGHT BETWEEN SILLS. (SEE NOTE ON SHEET 1 OF 12)

- ① SILL TO BE CONSTRUCTED IN PHASE IV - STAGE 1
- ② SILL TO BE CONSTRUCTED IN PHASE IV - STAGE 2
- ③ SILL TO BE CONSTRUCTED IN PHASE V - STAGE 3
- ④ SILL TO BE CONSTRUCTED IN PHASE V - STAGE 4



ELEVATION

LOOKING UPSTREAM
ELEVATION AT OUTLET SHOWN, ELEVATION AT INLET SIMILAR.

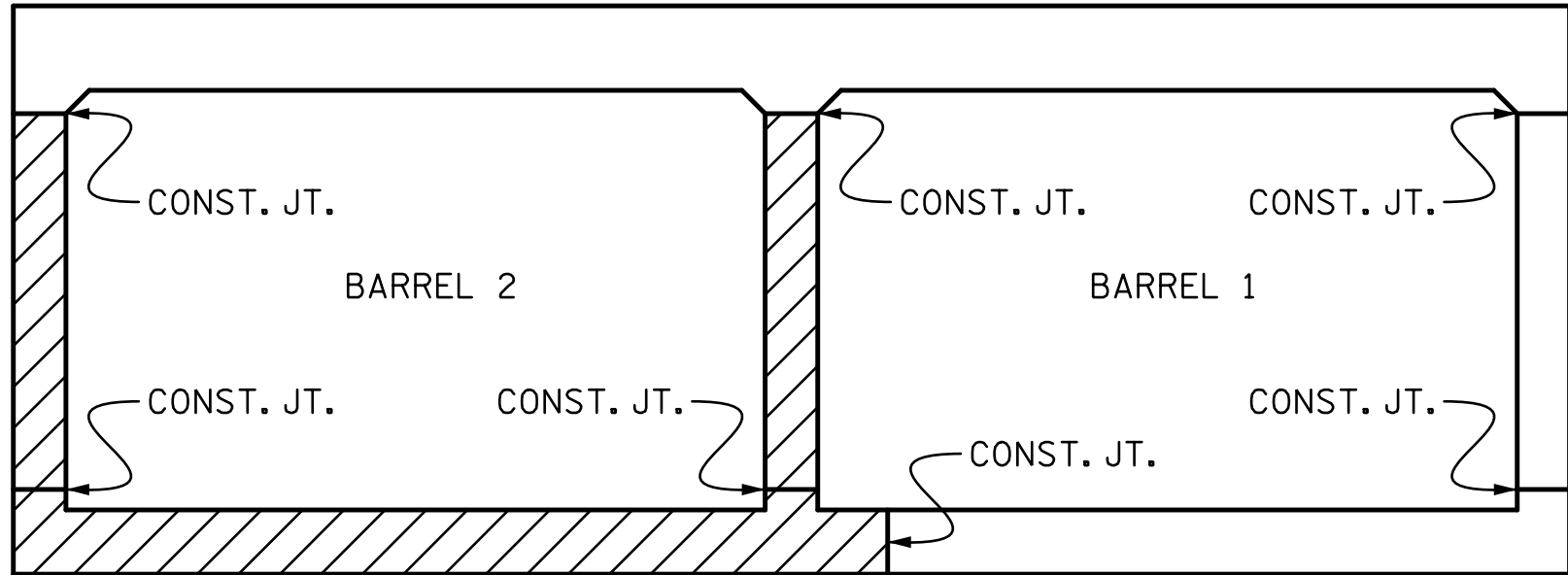


SECTION THROUGH SILL

* DOWELS MAY BE PUSHED INTO GREEN CONCRETE AFTER SLAB HAS BEEN FLOATED FINISHED.

PHASE IV - STAGE 1
OR
PHASE V - STAGE 3

PHASE IV - STAGE 2
OR
PHASE V - STAGE 4



- PHASE IV - STAGE 1 AND PHASE IV - STAGE 2
POUR 1: PHASE IV - STAGE 1 - FLOOR SLAB (INCLUDING PART OF WING 3 FOOTING) AND 4" OF VERTICAL WALLS/WING
POUR 2: PHASE IV - STAGE 1 - REMAINING PORTIONS OF EXTERIOR WALL AND INTERIOR WALL
POUR 3: PHASE IV - STAGE 2 - FLOOR SLAB (INCLUDING PART OF WING 4 FOOTING) AND 4" OF VERTICAL WALL/WING
POUR 4: PHASE IV - STAGE 2 - REMAINING PORTION OF EXTERIOR WALL
POUR 5: PHASE IV - STAGE 2 - ENTIRE ROOF SLAB AND OUTLET HEADWALL

- PHASE V - STAGE 3 AND PHASE V - STAGE 4
POUR 1: PHASE V - STAGE 3 - FLOOR SLAB (INCLUDING WING 1 FOOTING) AND 4" OF VERTICAL WALLS/WING
POUR 2: PHASE V - STAGE 3 - REMAINING PORTIONS OF EXTERIOR WALL, WING 1 AND INTERIOR WALL
POUR 3: PHASE V - STAGE 4 - FLOOR SLAB (INCLUDING WING 2 FOOTING) AND 4" OF VERTICAL WALL/WING
POUR 4: PHASE V - STAGE 4 - REMAINING PORTION OF EXTERIOR WALL AND WING 2
POUR 5: PHASE V - STAGE 4 - ENTIRE ROOF SLAB AND INLET HEADWALL

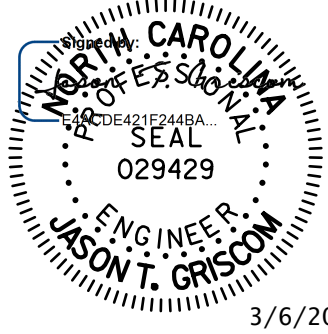
- PHASE VI - STAGE 5 AND PHASE VI - STAGE 6
POUR 1: PHASE VI - STAGE 5 - REMAINING PORTION OF WING 4
POUR 2: PHASE VI - STAGE 6 - REMAINING PORTION OF WING 3

CULVERT CONSTRUCTION SEQUENCE

LOOKING UPSTREAM

PROJECT NO. U-5797
ROBESON COUNTY
STATION: 20+80.00 -Y2-

SHEET 7 OF 12



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Charlotte, NC 28203
NC License Number F-0991

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CHECKED BY : MLO DATE : 11-24
DESIGN ENGINEER OF RECORD : J. GRISCOM DATE : 11-24

REVISIONS					SHEET NO.
NO.	BY:	DATE:	NO.	BY:	DATE:
1			3		
2			4		
					C1-7
					TOTAL SHEETS 12

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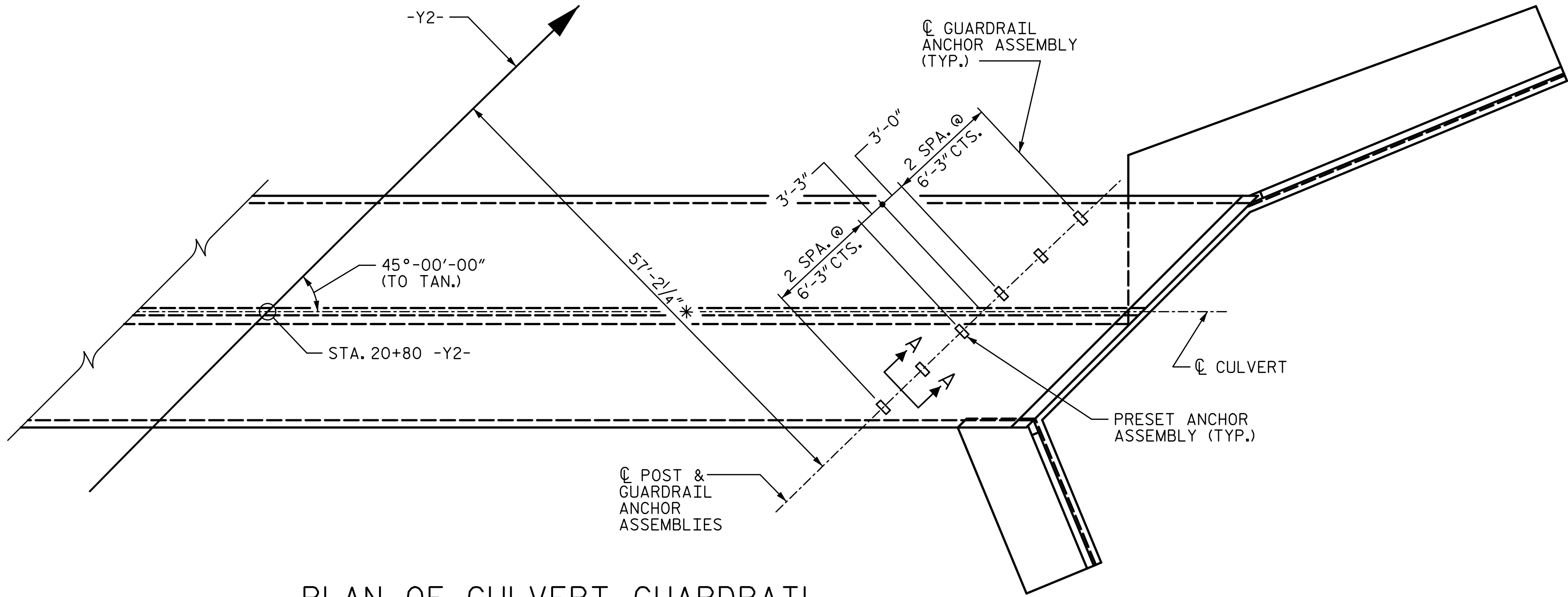
BILL OF MATERIAL PHASE IV - STAGE 1					
BAR	NO.	SIZE	TYPE	LENGTH	WEIGHT
A1	105	#4	1	5'-10"	409
A2	105	#4	1	5'-10"	409
A3	106	#4	1	5'-10"	413
A200	142	#6	STR	17'-3"	3,679
A201	4	#6	STR	16'-10"	98
A202	4	#6	STR	16'-4"	98
A203	4	#6	STR	15'-10"	95
A204	4	#6	STR	15'-4"	92
A205	4	#6	STR	14'-10"	89
A206	4	#6	STR	14'-4"	86
A207	4	#6	STR	13'-10"	83
A208	4	#6	STR	13'-4"	80
A209	4	#6	STR	12'-10"	77
A210	4	#6	STR	12'-4"	74
A211	4	#6	STR	11'-10"	71
A212	4	#6	STR	11'-4"	68
A213	4	#6	STR	10'-10"	65
A214	4	#6	STR	10'-4"	62
A215	4	#6	STR	9'-10"	59
A216	4	#6	STR	9'-4"	56
A217	4	#6	STR	8'-10"	53
A218	4	#6	STR	8'-4"	50
A219	4	#6	STR	7'-10"	47
A220	4	#6	STR	7'-4"	44
A221	4	#6	STR	6'-10"	41
A222	4	#6	STR	6'-4"	38
A223	4	#6	STR	5'-10"	35
A224	4	#6	STR	5'-4"	32
A225	4	#6	STR	4'-10"	29
A226	4	#6	STR	4'-4"	26
A227	4	#6	STR	3'-10"	23
A228	2	#6	STR	3'-4"	10
B1	53	#4	STR	11'-8"	413
B2	106	#4	STR	9'-4"	661
B3	106	#4	STR	11'-8"	826
C1	78	#4	STR	29'-2"	1,520
D1	6	#6	STR	1'-8"	15
S2	13	#7	STR	24'-0"	638
PHASE IV - STAGE 1 REINFORCING STEEL 10,667 LBS.					
PHASE IV - STAGE 1 QUANTITIES					
REINFORCING STEEL BARREL PORTION OF WING 3 TOTAL 10,667 LBS. 148 LBS. 10,815 LBS.					
CLASS A CONCRETE BARREL @ 1.185 CY/FT SILL END CURTAIN WALL PORTION OF WINGS 3 & 4 TOTAL 62.8 CY 0.4 CY 1.3 CY 1.8 CY 66.3 CY					

BILL OF MATERIAL PHASE IV - STAGE 2					
BAR	NO.	SIZE	TYPE	LENGTH	WEIGHT
A1	99	#4	1	5'-10"	386
A2	99	#4	1	5'-10"	386
A100	106	#6	STR	26'-2"	4,166
A101	4	#6	STR	25'-10"	155
A102	4	#6	STR	25'-4"	152
A103	4	#6	STR	24'-10"	149
A104	4	#6	STR	24'-4"	146
A105	4	#6	STR	23'-10"	143
A106	4	#6	STR	23'-4"	140
A107	4	#6	STR	22'-10"	137
A108	4	#6	STR	22'-4"	134
A109	4	#6	STR	21'-10"	131
A110	4	#6	STR	21'-4"	128
A111	4	#6	STR	20'-10"	125
A112	4	#6	STR	20'-4"	122
A113	4	#6	STR	19'-10"	119
A114	4	#6	STR	19'-4"	116
A115	4	#6	STR	18'-10"	113
A116	4	#6	STR	18'-4"	110
A117	4	#6	STR	17'-10"	107
A118	4	#6	STR	17'-4"	104
A119	4	#6	STR	16'-10"	101
A120	4	#6	STR	16'-4"	98
A121	4	#6	STR	15'-10"	95
A122	4	#6	STR	15'-4"	92
A123	4	#6	STR	14'-10"	89
A124	4	#6	STR	14'-4"	86
A125	4	#6	STR	13'-10"	83
A126	4	#6	STR	13'-4"	80
A127	4	#6	STR	12'-10"	77
A128	4	#6	STR	12'-4"	74
A129	4	#6	STR	11'-10"	71
A130	4	#6	STR	11'-4"	68
A131	4	#6	STR	10'-10"	65
A132	4	#6	STR	10'-4"	62
A133	4	#6	STR	9'-10"	59
A134	4	#6	STR	9'-4"	56
A135	4	#6	STR	8'-10"	53
A136	4	#6	STR	8'-4"	50
A137	4	#6	STR	7'-10"	47
A138	4	#6	STR	7'-4"	44
A139	4	#6	STR	6'-10"	41
A140	4	#6	STR	6'-4"	38
A141	4	#6	STR	5'-10"	35
A142	4	#6	STR	5'-4"	32
A143	4	#6	STR	4'-10"	29
A144	4	#6	STR	4'-4"	26
A145	4	#6	STR	3'-10"	23
A146	2	#6	STR	3'-4"	10
A147	6	#6	STR	2'-10"	26
A230	164	#6	STR	11'-8"	2,874
A231	4	#6	STR	11'-4"	68
A232	4	#6	STR	10'-10"	65
A233	4	#6	STR	10'-4"	62
A234	4	#6	STR	9'-10"	59
A235	4	#6	STR	9'-4"	56
A236	4	#6	STR	8'-10"	53
A237	4	#6	STR	8'-4"	50
A238	4	#6	STR	7'-10"	47
A239	4	#6	STR	7'-4"	44
A240	4	#6	STR	6'-10"	41
A241	4	#6	STR	6'-4"	38
A242	4	#6	STR	5'-10"	35
A243	4	#6	STR	5'-4"	32
A244	4	#6	STR	4'-10"	29
A245	4	#6	STR	4'-4"	26
A246	4	#6	STR	3'-10"	23
A247	4	#6	STR	3'-4"	20
A248	8	#6	STR	2'-10"	34

PHASE IV - STAGE 2 REINFORCING STEEL 17,582 LBS.			
PHASE IV - STAGE 2 QUANTITIES			
REINFORCING STEEL BARREL PORTION OF WING 4 TOTAL 17,582 LBS. 1,887 LBS. 19,469 LBS.			
CLASS A CONCRETE BARREL @ 1.794 CY/FT COUNTERFORT SILL END CURTAIN WALL PORTION OF WING 4 HEADWALL TOTAL 95.1 CY 1.1 CY 0.4 CY 1.0 CY 5.5 CY 1.7 CY 104.8 CY			

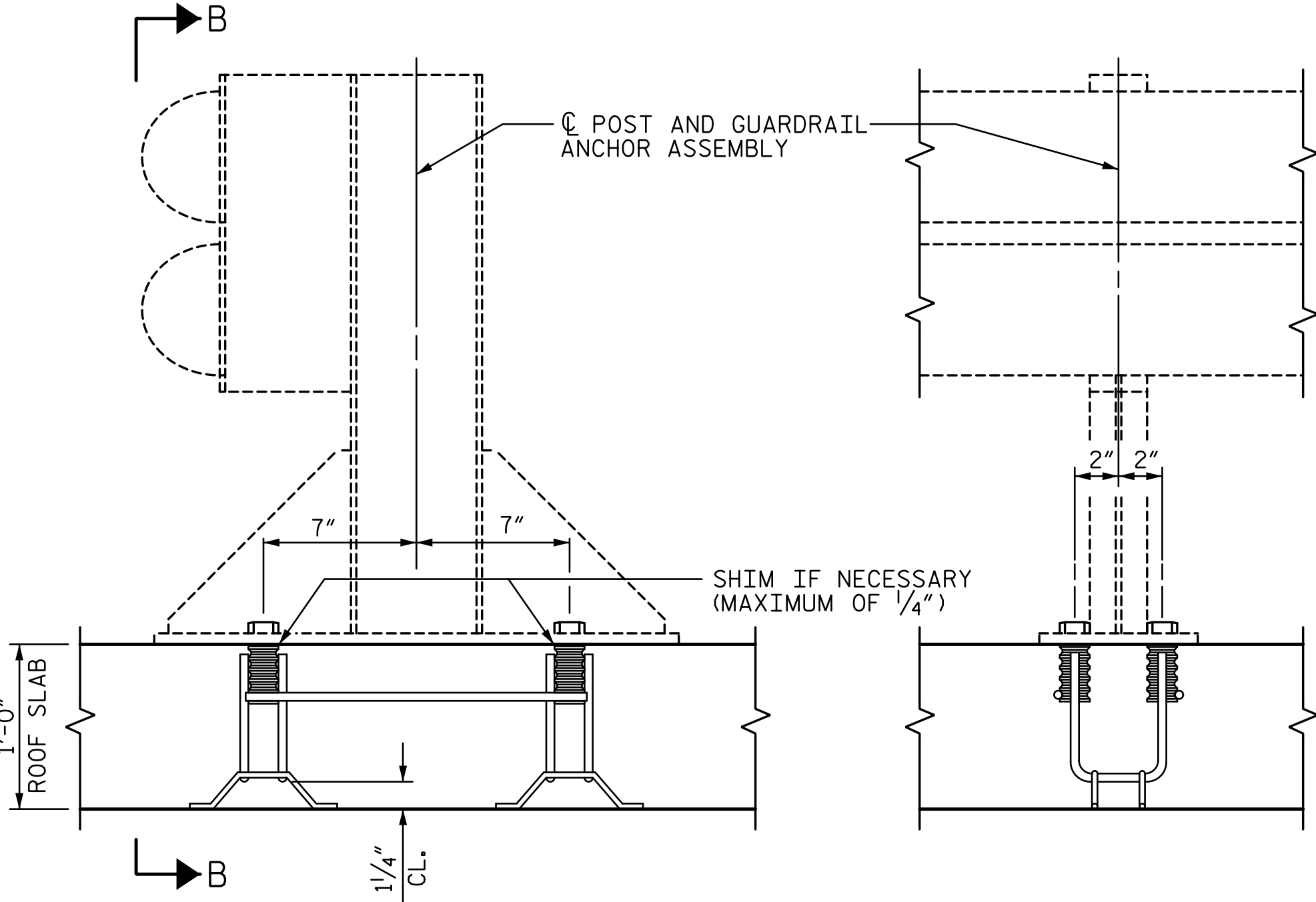
BILL OF MATERIAL PHASE V - STAGE 3					
BAR	NO.	SIZE	TYPE	LENGTH	WEIGHT
A1	274	#4	1	5'-10"	1,068
A2	274	#4	1	5'-10"	1,068
A3	282	#4	1	5'-10"	1,099
A200	492	#6	STR	17'-3"	12,747
A201	4	#6	STR	16'-10"	101
A202	4	#6	STR	16'-4"	98
A203	4	#6	STR	15'-10"	95
A204	4	#6	STR	15'-4"	92
A205	4	#6	STR	14'-10"	89
A206	4	#6	STR	14'-4"	86
A207	4	#6	STR	13'-10"	83
A208	4	#6	STR	13'-4"	80
A209	4	#6	STR	12'-10"	77
A210	4	#6	STR	12'-4"	74
A211	4	#6	STR	11'-10"	71
A212	4	#6	STR	11'-4"	68
A213	4	#6	STR	10'-10"	65
A214	4	#6	STR	10'-4"	62
A215	4	#6	STR	9'-10"	59
A216	4	#6	STR	9'-4"	56
A217	4	#6	STR	8'-10"	53
A218	4	#6	STR	8'-4"	50
A219	4	#6	STR	7'-10"	47
A220	4	#6	STR	7'-4"	44
A221	4	#6	STR	6'-10"	41
A222	4	#6	STR	6'-4"	38
A223	4	#6	STR	5'-10"	35
A224	4	#6	STR	5'-4"	32
A225	4	#6	STR	4'-10"	29
A226	4	#6	STR	4'-4"	26
A227	4	#6	STR	3'-10"	23
A228	2	#6	STR	3'-4"	10
A229	6	#6	STR	2'-10"	26
B1	140	#4	STR	11'-8"	1,091
B2	279	#4	STR	9'-4"	1,739
B3	282	#4	STR	11'-8"	2,198
C2	156	#4	STR	36'-10"	3,838
D1	6	#6	STR	1'-8"	15
S2	13	#7	STR	24'-0"	638
PHASE V - STAGE 3 REINFORCING STEEL 27,211 LBS.					
PHASE V - STAGE 3 QUANTITIES					
REINFORCING STEEL BARREL WING 1 TOTAL 27,211 LBS. 4,712 LBS. 31,923 LBS.					
CLASS A CONCRETE BARREL @ 1.185 CY/FT COUNTERFORT SILL END CURTAIN WALL WING 1 TOTAL 166.5 CY 1.1 CY 0.4 CY 3.6 CY 35.2 CY 206.8 CY					

BILL OF MATERIAL PHASE V					
BAR	NO.	SIZE	TYPE	LENGTH	WEIGHT
A1	280	#4	1	5'-10"	1,091
A2	280	#4	1	5'-10"	1,091
A100	456	#6	STR	26'-2"	17,922
A101	4	#6	STR	25'-10"	155
A102	4	#6	STR	25'-4"	152
A103	4	#6	STR	24'-10"	149
A104	4	#6	STR	24'-4"	146
A105	4	#6	STR	23'-10"	143
A106	4	#6	STR	23'-4"	140
A107	4	#6	STR	22'-10"	137
A108	4	#6	STR	22'-4"	134
A109	4	#6	STR	21'-10"	131
A110	4	#6	STR	21'-4"	128
A111	4	#6	STR	20'-10"	125
A112	4	#6	STR	20'-4"	122
A113	4	#6	STR	19'-10"	119
A114	4	#6	STR	19'-4"	116
A115	4	#6	STR	18'-10"	113
A116	4	#6	STR	18'-4"	110
A117	4	#6	STR	17'-10"	107
A118	4	#6	STR	17'-4"	104
A119	4	#6	STR	16'-10"	101
A120	4	#6	STR	16'-4"	98
A121	4	#6	STR	15'-10"	95
A122	4	#6	STR	15'-4"	92
A123	4	#6	STR	14'-10"	89
A124	4	#6	STR	14'-4"	86
A125	4	#6	STR	13'-10"	83
A126	4	#6	STR	13'-4"	80
A127	4	#6	STR	12'-10"	77
A128	4	#6	STR	12'-4"	74
A129	4	#6	STR	11'-10"	71
A130	4	#6	STR	11'-4"	68
A131	4	#6	STR	10'-10"	65
A132	4	#6	STR	10'-4"	62
A133	4	#6	STR	9'-10"	59
A134	4	#6	STR	9'-4"	56
A135	4	#6	STR	8'-10"	53
A136	4	#6	STR	8'-4"	50
A137	4	#6	STR	7'-10"	47
A138	4	#6	STR	7'-4"	44
A139	4	#6	STR	6'-10"	41
A140	4	#6	STR	6'-4"	38
A141	4	#6	STR	5'-10"	35
A142	4	#6	STR	5'-4"	32
A143	4	#6	STR	4'-10"	29
A144	4	#6	STR	4'-4"	26
A145	4	#6	STR	3'-10"	23
A146	2	#6	STR	3'-4"	10
A147	6	#6	STR	2'-10"	26
A230	514	#6	STR	11'-8"	9,007
A231	4	#6	STR	11'-4"	68
A232	4	#6	STR	10'-10"	65
A233	4	#6	STR	10'-4"	62
A234	4	#6	STR	9'-10"	59
A235	4	#6	STR	9'-4"	56
A236	4	#6	STR	8'-10"	53
A237	4	#6	STR	8'-4"	50
A238	4	#6	STR	7'-10"	47
A239	4	#6	STR	7'-4"	44
A240	4	#6	STR	6'-10"	41
A241	4	#6	STR	6'-4"	38
A242	4	#6	STR	5'-10"	35
A243	4	#6	STR	5'-4"	32
A244	4	#6	STR	4'-10"	29
A245	4	#6	STR	4'-4"	26
A246	4	#6	STR	3'-10"	23
A247	2	#6	STR	3'-4"	10
A248	2	#6	STR	2'-10"	9



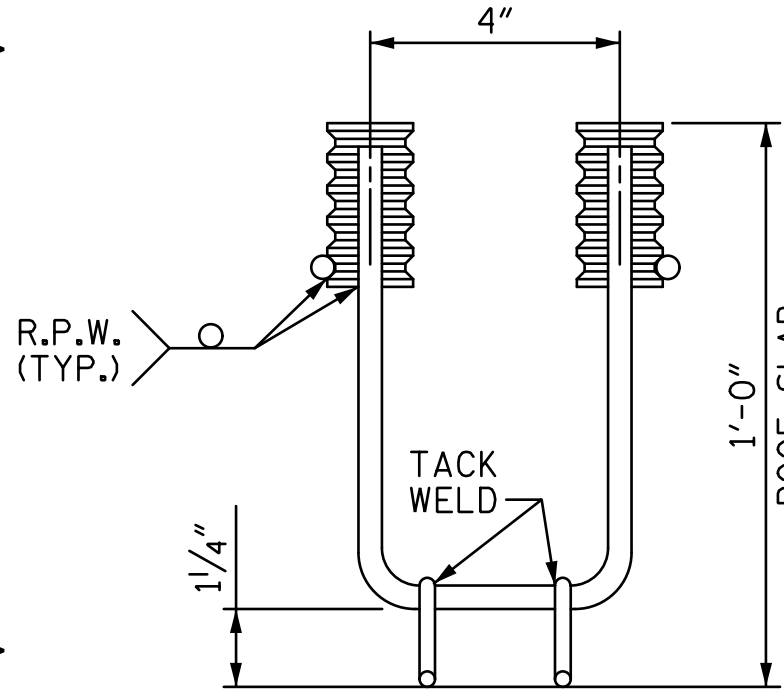
PLAN OF CULVERT GUARDRAIL ANCHOR ASSEMBLY SPACING

* THIS DIMENSION TO BE CONFIRMED BY THE ENGINEER IN THE FIELD.

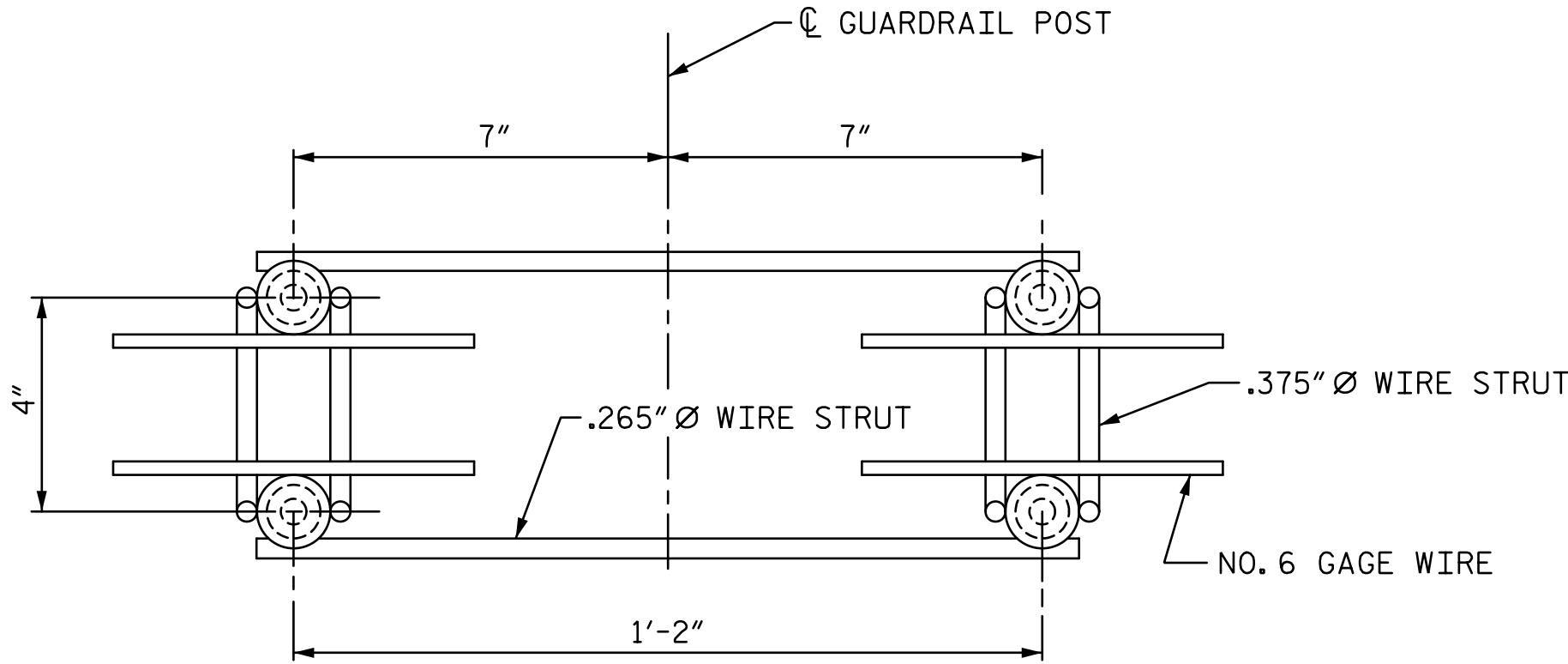


SECTION A-A

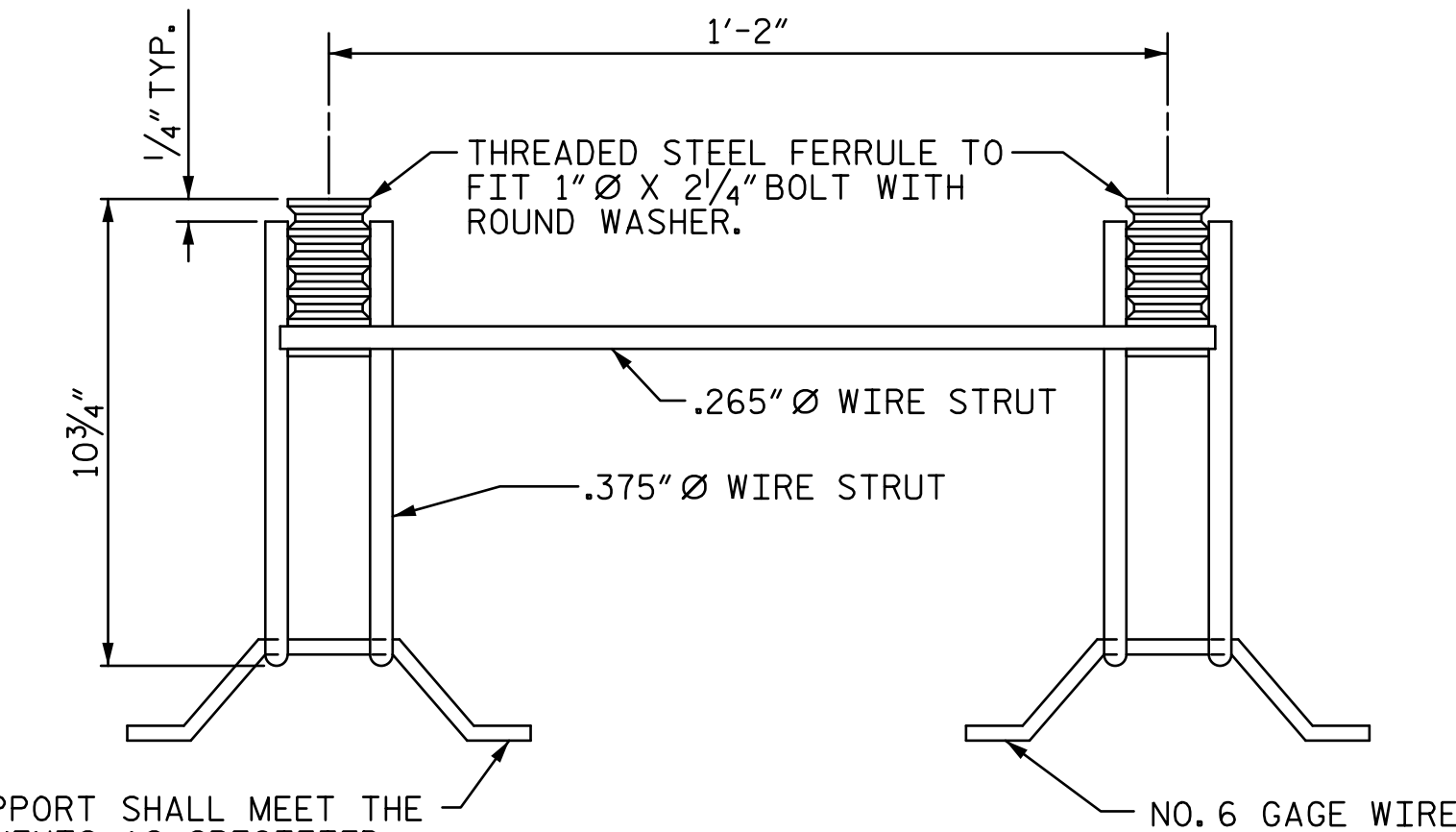
SECTION B-B



ELEVATION



PLAN



SIDE VIEW

THIS SUPPORT SHALL MEET THE REQUIREMENTS AS SPECIFIED FOR SUPPORTS FOR REINFORCING STEEL. SEE SPECIFICATIONS.

GUARDRAIL ANCHOR ASSEMBLY FOR CULVERTS

NOTES

THE GUARDRAIL ANCHOR ASSEMBLY FOR CULVERTS SHALL CONSIST OF THE FOLLOWING COMPONENTS :

- FERRULES SHALL BE MADE FROM STEEL MEETING THE REQUIREMENTS OF AASHTO M169, GRADE 12L14 AND SHALL HAVE A MINIMUM LENGTH OF THREADS OF 2 1/2".
- 4 - 1" Ø X 2 1/4" BOLTS WITH WASHERS, BOLTS SHALL CONFORM TO THE REQUIREMENTS OF ASTM A307. BOLTS AND WASHERS SHALL BE GALVANIZED. (AT THE CONTRACTOR'S OPTION, STAINLESS STEEL BOLTS AND WASHERS MAY BE USED AS AN ALTERNATE FOR THE 1" Ø X 2 1/4" GALVANIZED BOLTS AND WASHERS. THEY SHALL CONFORM TO OR EXCEED THE MECHANICAL REQUIREMENTS OF ASTM A307. THE USE OF THIS ALTERNATE SHALL BE APPROVED BY THE ENGINEER.)
- WIRE STRUTS SHOWN IN THE GUARDRAIL ANCHOR ASSEMBLY FOR CULVERTS DETAIL ARE MINIMUM ALLOWABLE SIZE AND SHALL HAVE A MINIMUM TENSILE STRENGTH OF 100,000 P.S.I. AS AN OPTION, A 7/16" Ø WIRE STRUT WITH A MINIMUM TENSILE STRENGTH OF 90,000 PSI IS ACCEPTABLE.

GUARDRAIL ANCHOR ASSEMBLY WITH BOLTS SHALL BE ASSEMBLED IN THE SHOP. BOLT THREADS MAY BE RECUT AS NECESSARY TO INSURE FIT.

THE COST OF THE GUARDRAIL ANCHOR ASSEMBLY FOR CULVERTS COMPLETE IN PLACE, SHALL BE INCLUDED IN THE UNIT CONTRACT PRICE BID FOR CLASS "A" CONCRETE.

FERRULES TO BE PLUGGED DURING POURING OF SLAB AS RECOMMENDED BY THE MANUFACTURER.

AT THE CONTRACTOR'S OPTION, FERRULES WITH OPEN OR CLOSED ENDS MAY BE USED.

PAYMENT FOR GUARDRAIL, POSTS, AND POST BASE PLATES IS INCLUDED IN ROADWAY PAY ITEMS.

SLAB REINFORCING STEEL MAY BE SHIFTED AS NECESSARY TO CLEAR GUARDRAIL ANCHOR ASSEMBLY. CARE SHOULD BE TAKEN TO KEEP THE SHIFTING OF REINFORCING STEEL TO A MINIMUM.

THE CONTRACTOR MAY USE ADHESIVELY ANCHORED ANCHOR BOLTS IN PLACE OF GUARDRAIL ANCHOR ASSEMBLY. LEVEL TWO FIELD TESTING IS REQUIRED, AND THE YIELD LOAD OF THE 1" Ø BOLT IS 21.8 KIPS. FOR ADHESIVELY ANCHORED ANCHOR BOLTS OR DOWELS, SEE STANDARD SPECIFICATIONS.

PROJECT NO. U-5797

ROBESON COUNTY

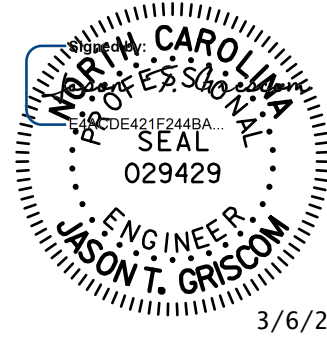
STATION: 20+80.00 -Y2-

SHEET 9 OF 12

STATE OF NORTH CAROLINA
DEPARTMENT OF TRANSPORTATION
RALEIGH

ANCHORAGE DETAILS FOR
GUARDRAIL ANCHOR ASSEMBLY
FOR CULVERTS

REVISIONS					SHEET NO.
NO.	BY:	DATE:	NO.	BY:	DATE:
1			3		
2			4		
					TOTAL SHEETS 12

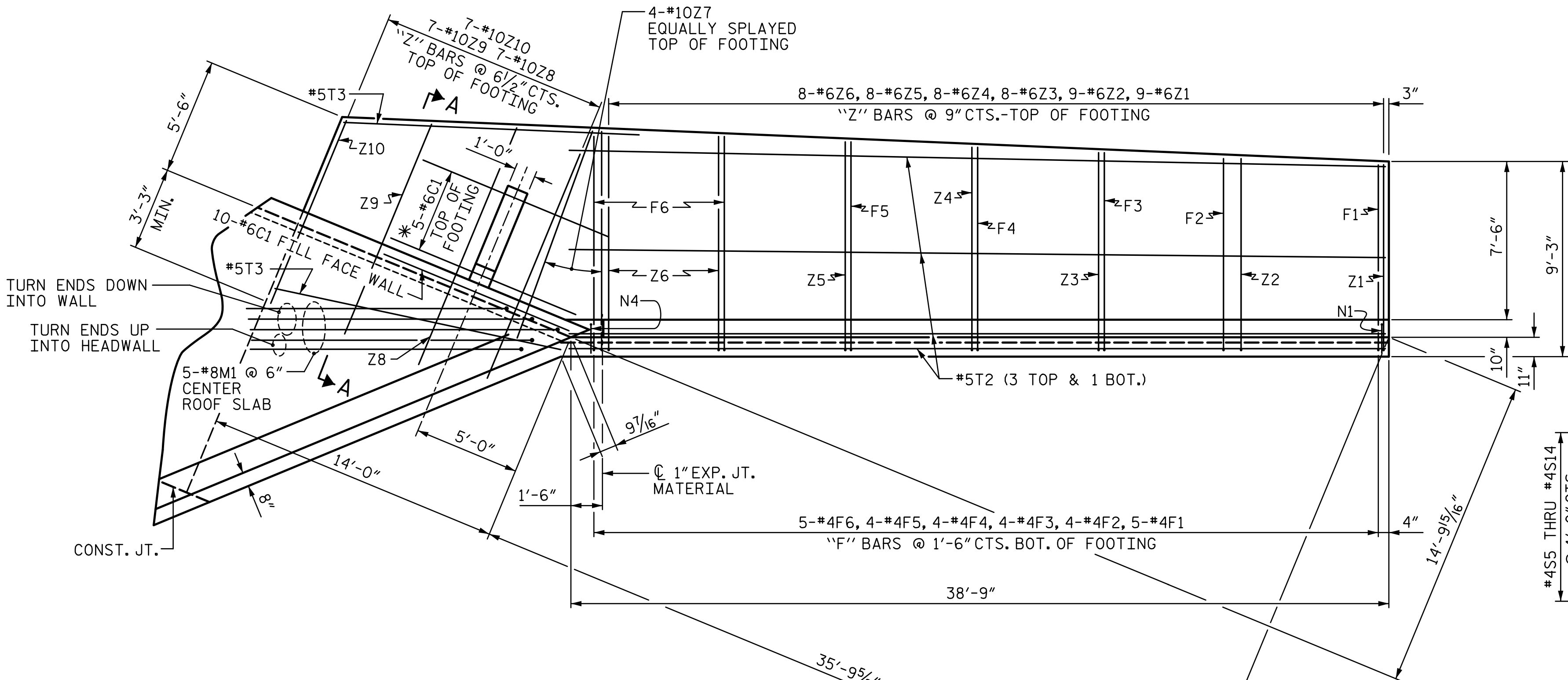


stv STV Engineers, Inc.
2151 Howkins St., Suite 1400
Charlotte, NC 28203
NC License Number F-0991

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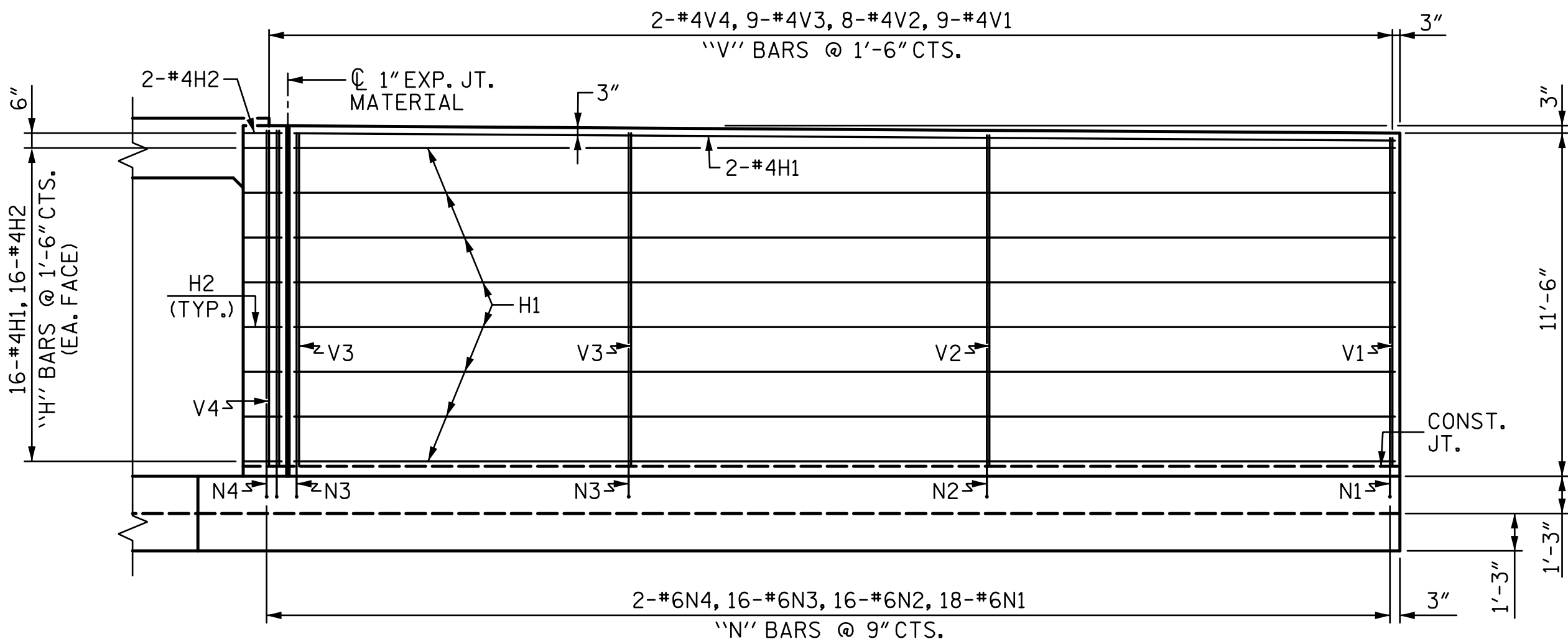
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CHECKED BY : MLO DATE : 11-24
DESIGN ENGINEER OF RECORD : J. GRISCOM DATE : 11-24

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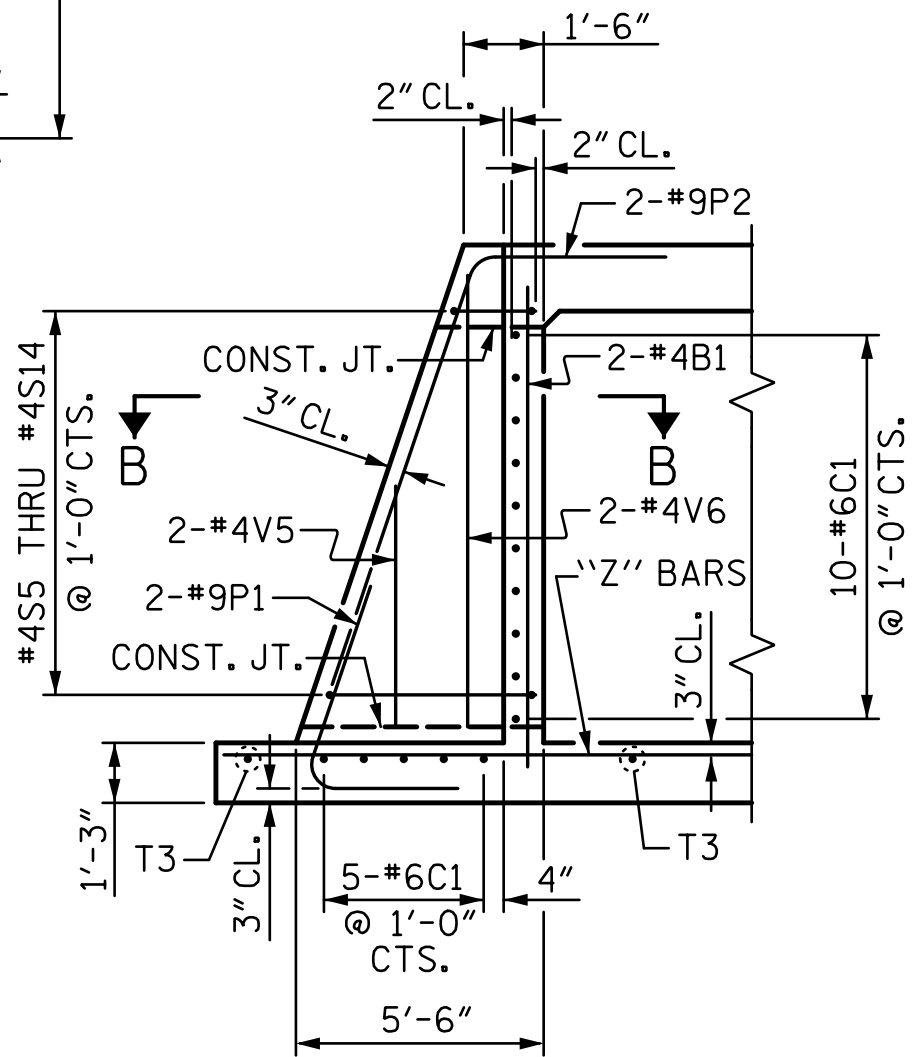


PLAN W1

* CENTER ALL #6C1 BARS ON CL COUNTERFORT

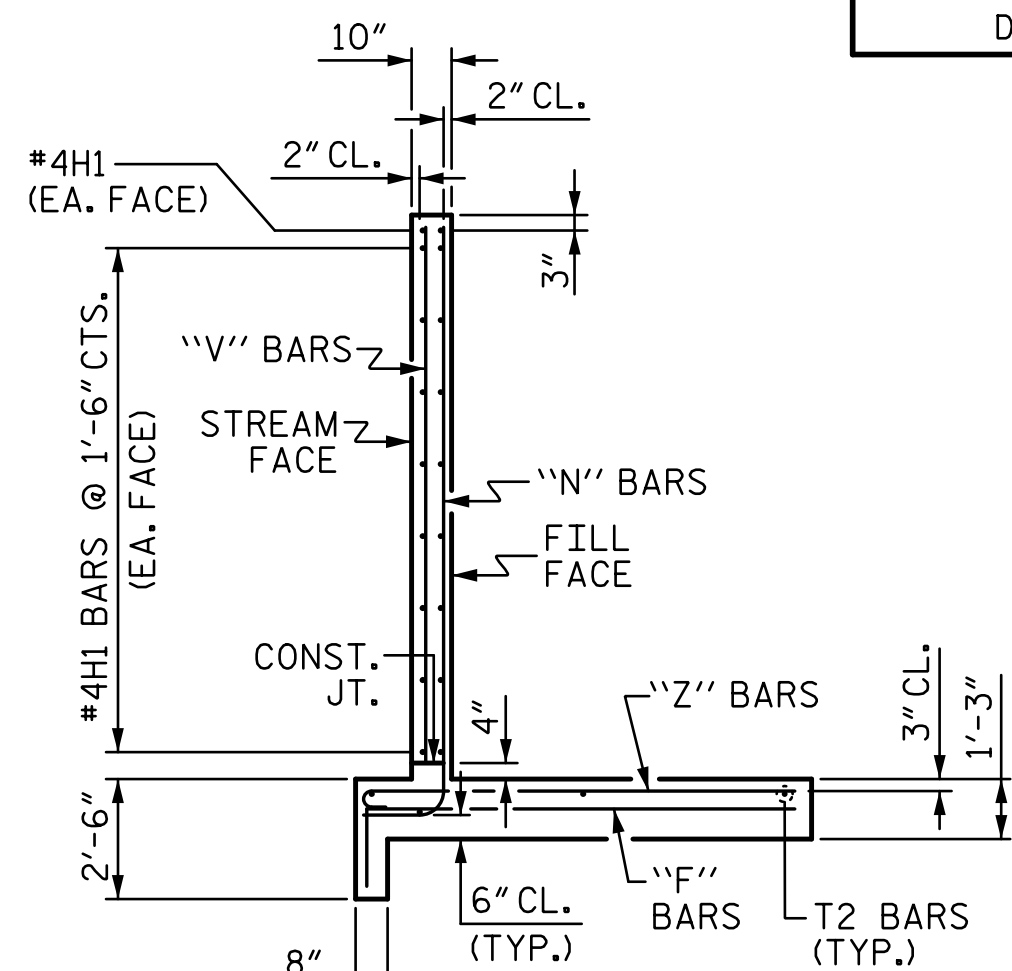


ELEVATION W1

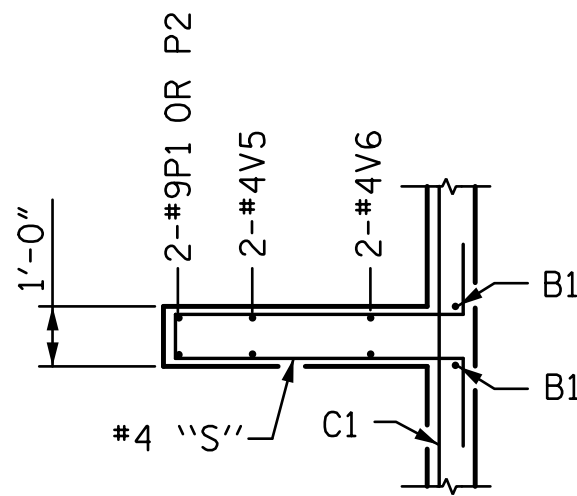


SECTION A-A

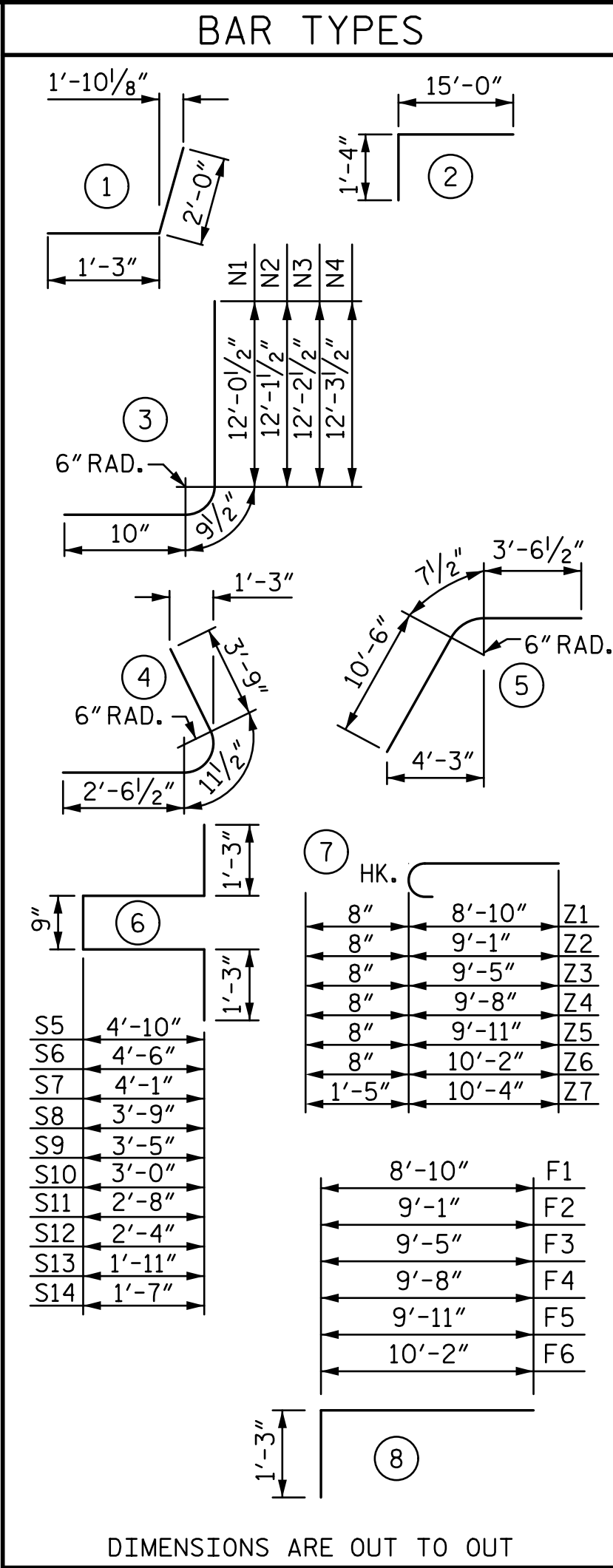
STANDARD REINFORCING STEEL IN BARREL NOT SHOWN



WING SECTION



SECTION B-B

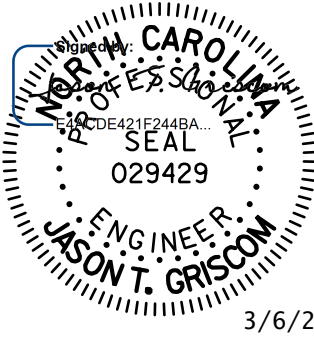


DIMENSIONS ARE OUT TO OUT

BILL OF MATERIAL					
BAR	NO.	SIZE	TYPE	LENGTH	WEIGHT
B1	2	#4	STR	11'-0"	15
C1	15	#6	STR	10'-0"	225
H1	18	#4	STR	36'-10"	443
H2	18	#4	1	3'-3"	39
M1	5	#8	2	16'-4"	218
N1	18	#6	3	13'-8"	369
N2	16	#6	3	13'-9"	330
N3	16	#6	3	13'-10"	332
N4	2	#6	3	13'-11"	42
P1	2	#9	4	7'-3"	49
P2	2	#9	5	14'-8"	100
S5	1	#4	6	12'-11"	9
S6	1	#4	6	12'-3"	8
S7	1	#4	6	11'-5"	8
S8	1	#4	6	10'-9"	7
S9	1	#4	6	10'-1"	7
S10	1	#4	6	9'-3"	6
S11	1	#4	6	8'-7"	6
S12	1	#4	6	7'-11"	5
S13	1	#4	6	7'-1"	5
S14	1	#4	6	6'-5"	4
F1	5	#4	8	10'-1"	34
F2	4	#4	8	10'-4"	28
F3	4	#4	8	10'-8"	29
F4	4	#4	8	10'-11"	29
F5	4	#4	8	11'-2"	30
F6	5	#4	8	11'-5"	38
T2	4	#5	STR	38'-8"	161
T3	2	#5	STR	14'-0"	29
V1	9	#4	STR	11'-0"	66
V2	8	#4	STR	11'-1"	59
V3	9	#4	STR	11'-2"	67
V4	2	#4	STR	11'-3"	15
V5	2	#4	STR	5'-0"	7
V6	2	#4	STR	9'-6"	13
Z1	9	#6	7	9'-6"	128
Z2	9	#6	7	9'-9"	132
Z3	8	#6	7	10'-1"	121
Z4	8	#6	7	10'-4"	124
Z5	8	#6	7	10'-7"	127
Z6	8	#6	7	10'-10"	130
Z7	4	#10	7	11'-9"	202
Z8	7	#10	STR	11'-6"	346
Z9	7	#10	STR	10'-1"	304
Z10	7	#10	STR	8'-10"	266
REINFORCING STEEL				4,712 LBS.	

PROJECT NO. U-5797
ROBESON COUNTY
STATION: 20+80.00 -Y2-

SHEET 10 OF 12



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2151 Howkins St., Suite 1400
Charlotte, NC 28203
NC License Number F-0991

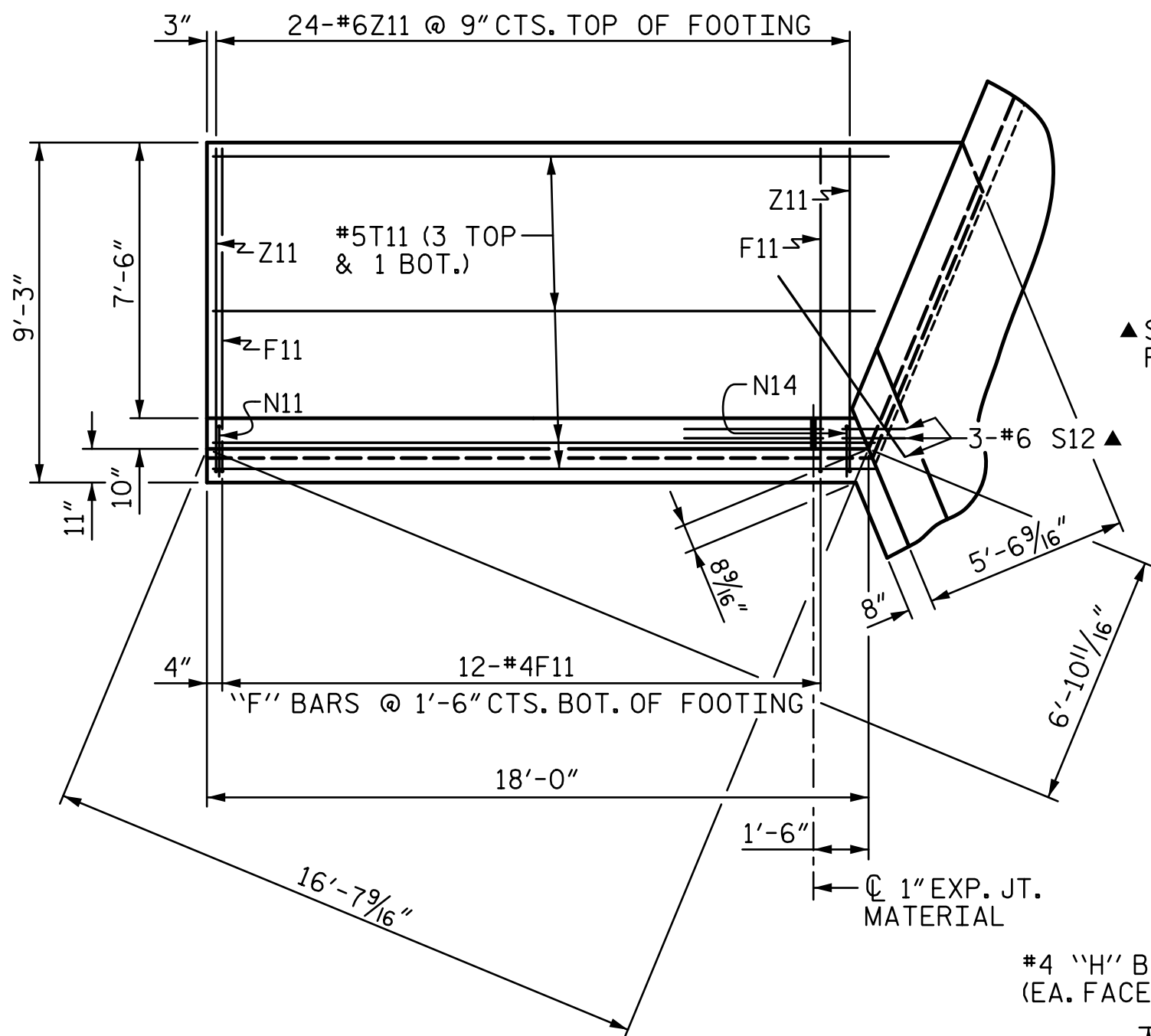
DOCUMENT NOT CONSIDERED
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SIGNATURES COMPLETED

STATE OF NORTH CAROLINA
DEPARTMENT OF TRANSPORTATION
RALEIGH
WING 1 FOR
CONCRETE BOX CULVERT
H = 10'-0" SLOPE = 3:1
45° SKEW

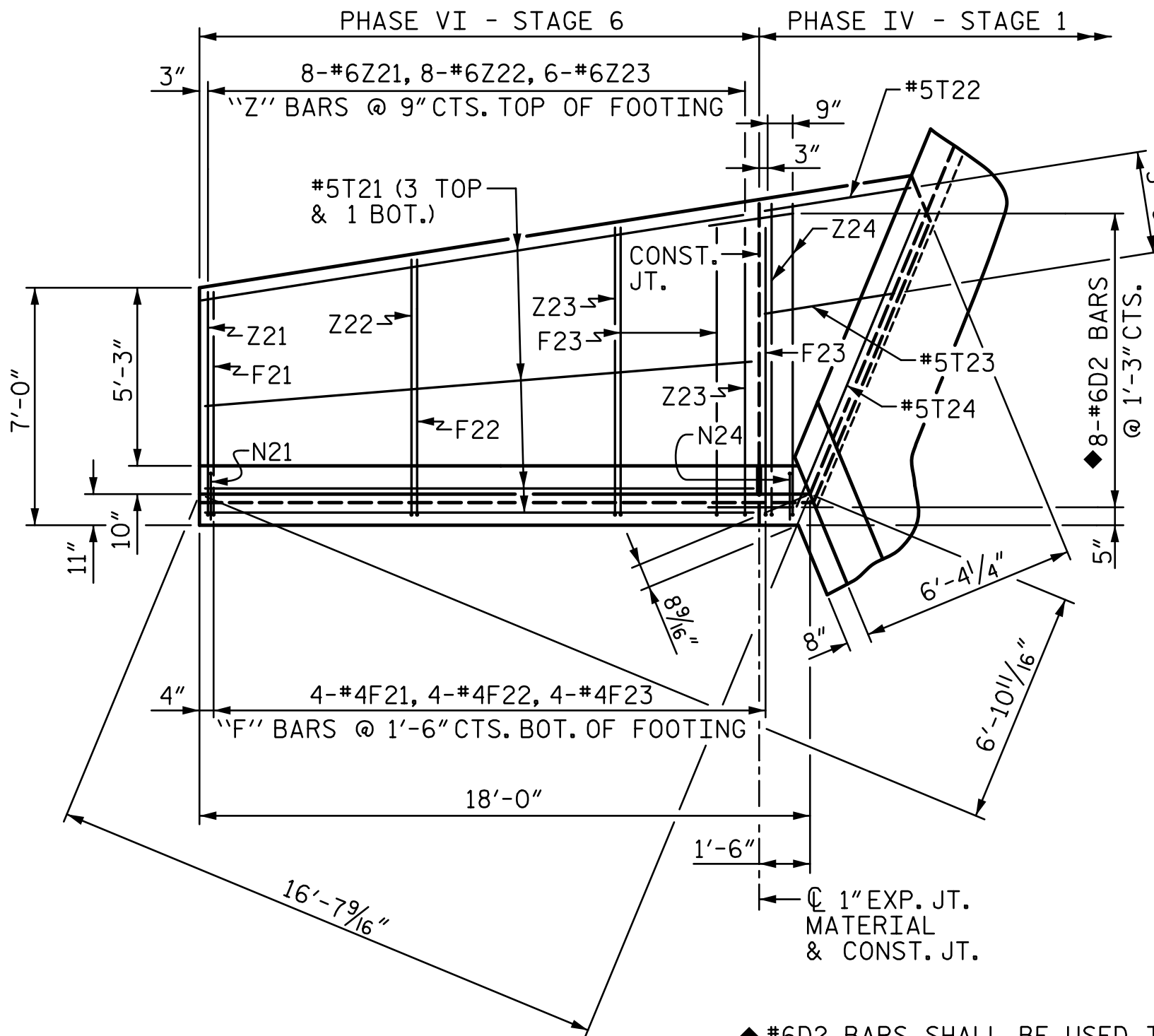
REVISIONS				SHEET NO.
NO.	BY:	DATE:	NO.	C1-10
1			3	TOTAL SHEETS 12
2			4	

DRAWN BY :	SGH	DATE :	11-24
CHECKED BY :	MLO	DATE :	11-24
DESIGN ENGINEER OF RECORD :	J. GRISCOM	DATE :	11-24

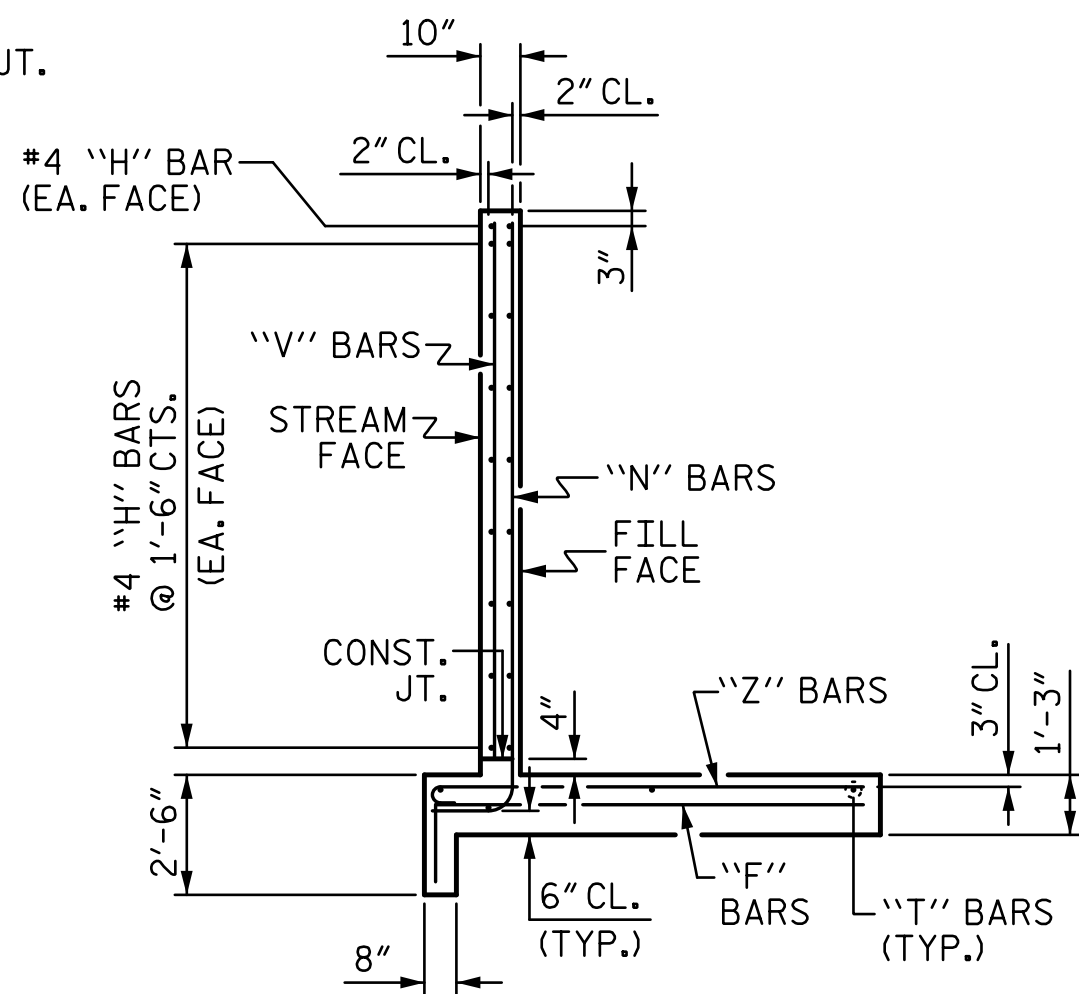
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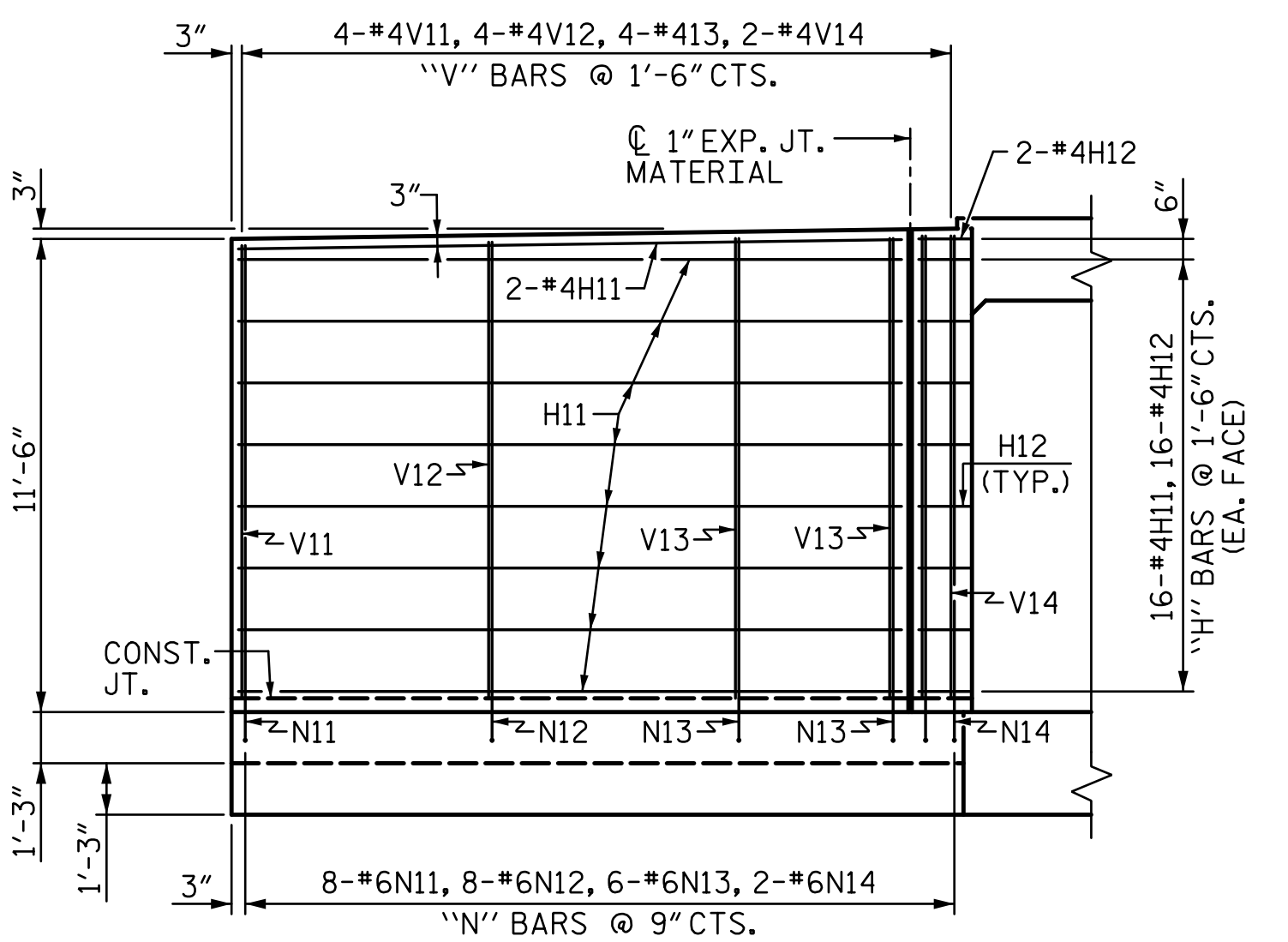
PLAN W2



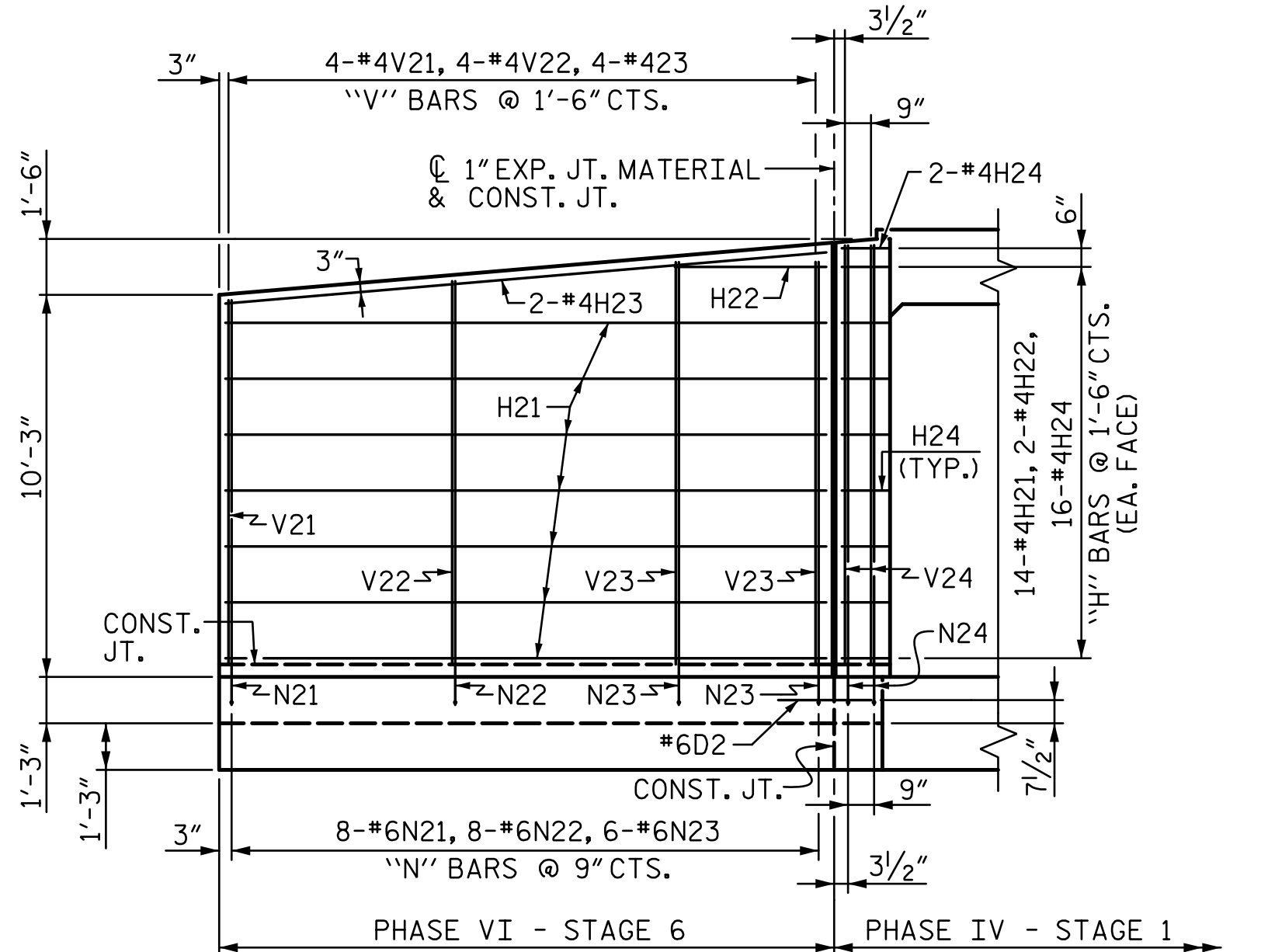
PLAN W3



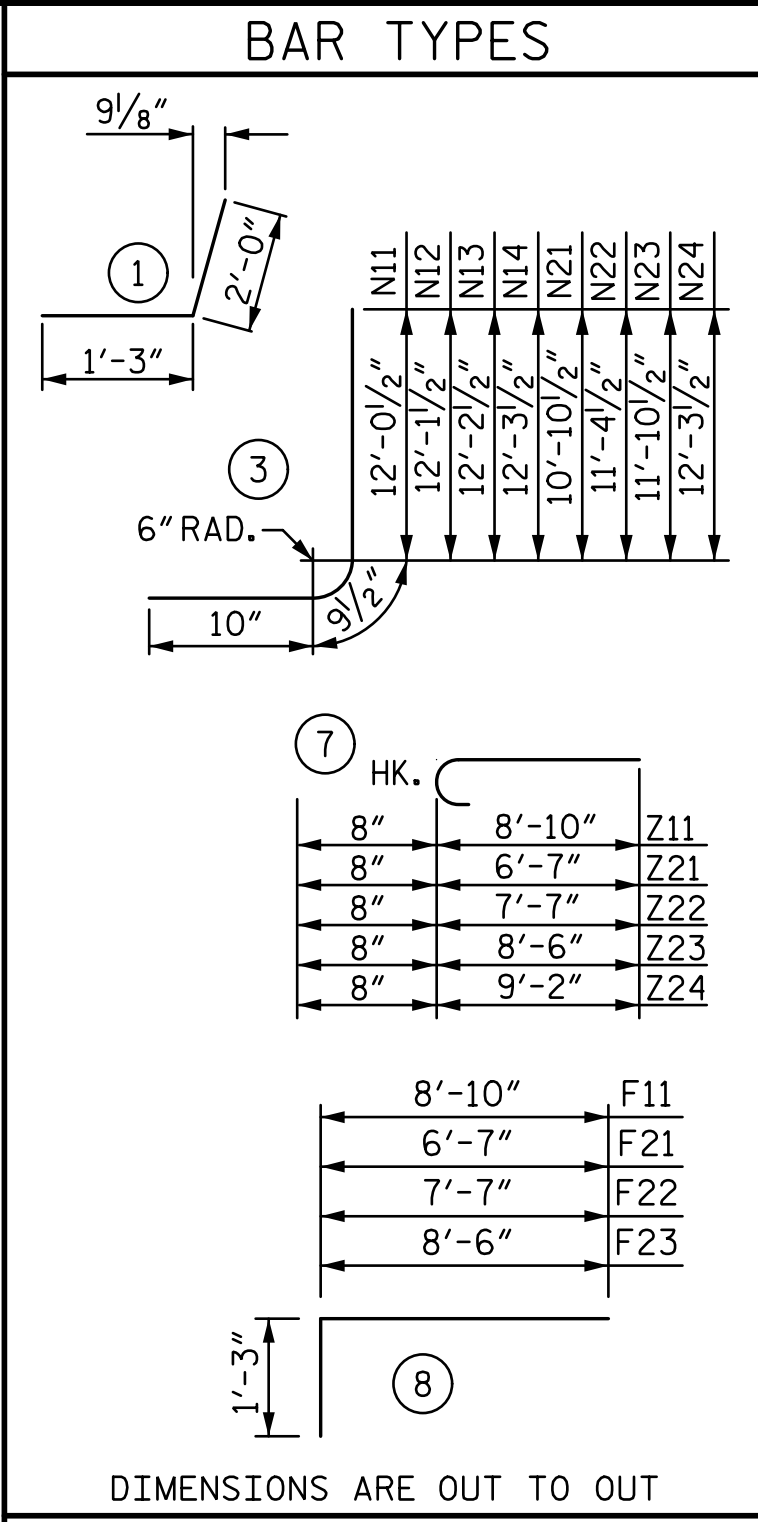
WING SECTION



ELEVATION W2



ELEVATION W3



DIMENSIONS ARE OUT TO OUT

BILL OF MATERIAL FOR WING 3 (PHASE IV - STAGE 1)

BAR	NO.	SIZE	TYPE	LENGTH	WEIGHT
F23	1	#4	8	9'-9"	7
H24	16	#4	1	3'-3"	35
N24	2	#6	3	13'-11"	42
T24	1	#5	STR	9'-5"	10
V24	2	#4	STR	11'-2"	15
Z24	2	#6	7	9'-10"	30
REINFORCING STEEL FOR WING 3 (PHASE IV - STAGE 1)					148 LBS.

BILL OF MATERIAL FOR WING 2

BAR	NO.	SIZE	TYPE	LENGTH	WEIGHT
F11	12	#4	8	10'-1"	81
H11	18	#4	STR	16'-1"	193
H12	18	#4	1	3'-3"	39
N11	8	#6	3	13'-8"	164
N12	8	#6	3	13'-9"	165
N13	6	#6	3	13'-10"	125
N14	2	#6	3	13'-11"	42
S12	3	#6	STR	6'-0"	27
T11	4	#5	STR	18'-0"	75
V11	4	#4	STR	11'-0"	29
V12	4	#4	STR	11'-1"	30
V13	4	#4	STR	11'-2"	30
V14	2	#4	STR	11'-3"	15
Z11	24	#6	7	9'-6"	342
REINFORCING STEEL FOR WING 2					1,357 LBS.

BILL OF MATERIAL FOR WING 3 (PHASE VI - STAGE 6)

BAR	NO.	SIZE	TYPE	LENGTH	WEIGHT
F21	4	#4	8	7'-10"	21
F22	4	#4	8	8'-10"	24
F23	3	#4	8	9'-9"	20
D2	8	#6	STR	3'-3"	39
H21	14	#4	STR	16'-1"	150
H22	2	#4	STR	4'-0"	5
H23	2	#4	STR	16'-2"	22
N21	8	#6	3	12'-6"	150
N22	8	#6	3	13'-0"	156
N23	6	#6	3	13'-6"	122
T21	4	#5	STR	16'-2"	67
V21	4	#4	STR	9'-9"	26
V22	4	#4	STR	10'-3"	27
V23	4	#4	STR	10'-9"	29
Z21	8	#6	7	7'-3"	87
Z22	8	#6	7	8'-3"	99
Z23	6	#6	7	9'-2"	83
REINFORCING STEEL FOR WING 3 (PHASE VI - STAGE 6)					1,127 LBS.

PROJECT NO. U-5797
ROBESON COUNTY
STATION: 20+80.00 -Y2-

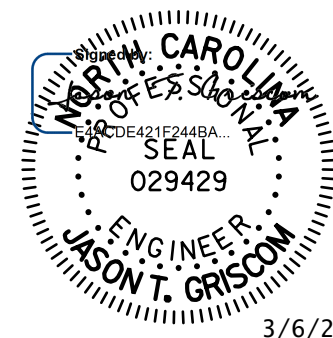
SHEET 11 OF 12

STATE OF NORTH CAROLINA
DEPARTMENT OF TRANSPORTATION
RALEIGH

WINGS 2 & 3 FOR
CONCRETE BOX CULVERT

H = 10'-0" SLOPE = 3:1
45° SKEW

REVISIONS				SHEET NO.
NO.	BY:	DATE:	NO.	DATE:
1			3	
2			4	
				TOTAL SHEETS 12



3/6/2025

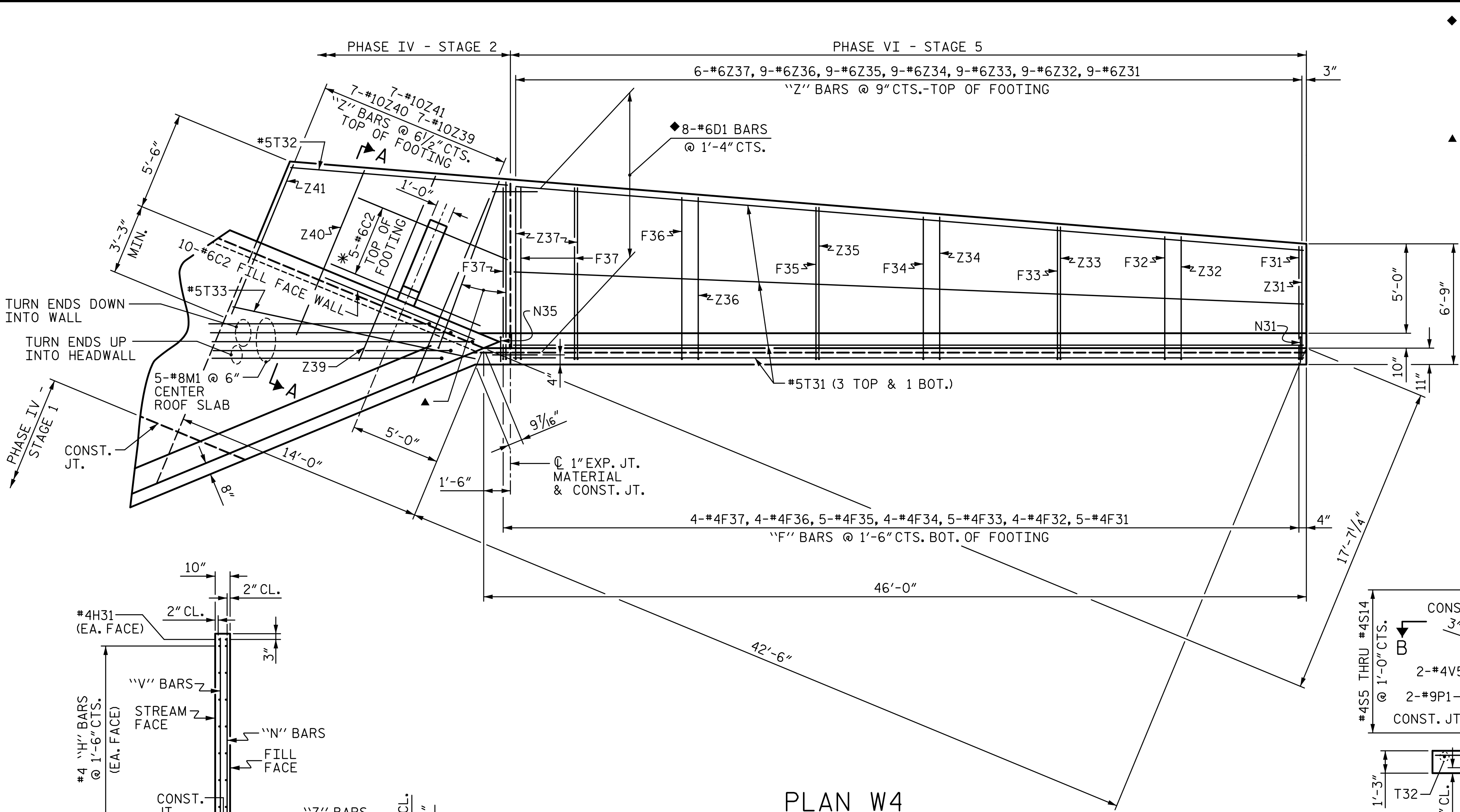


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Charlotte, NC 28203
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CHECKED BY : MLO DATE : 11-24
DESIGN ENGINEER OF RECORD : J. GRISCOM DATE : 11-24

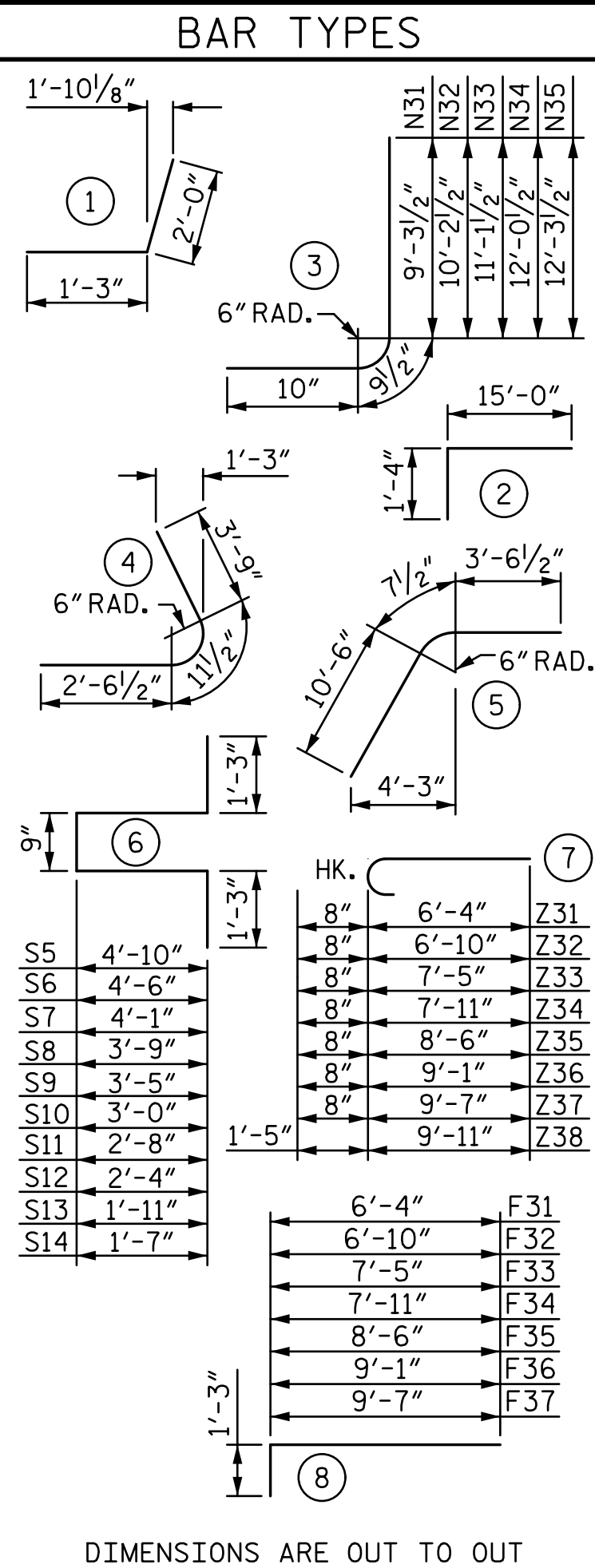
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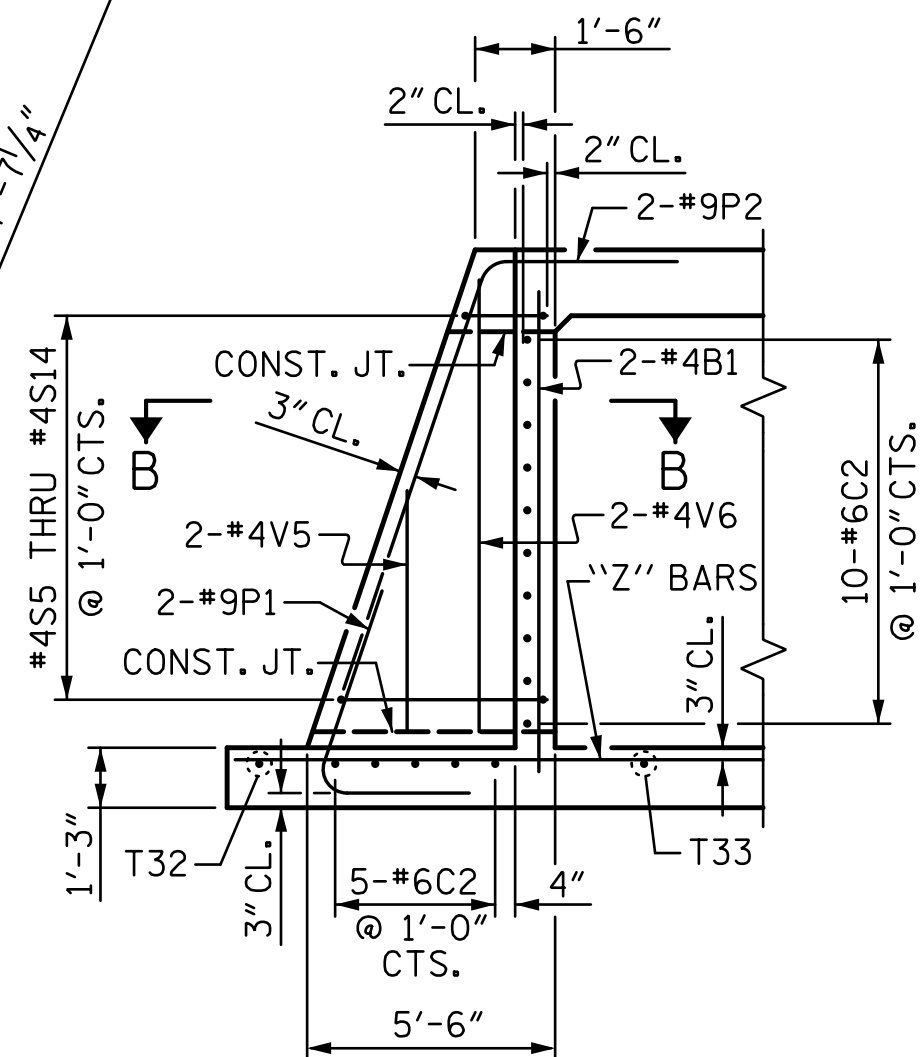
PLAN W4

* CENTER ALL #6C2 BARS ON C/C COUNTERFORT

◆ #6D2 BARS SHALL BE USED TO CONNECT THE PHASE VI-STAGE 5 PORTION OF THE FOOTING TO THE PHASE IV-STAGE 2 PORTION OF THE FOOTING. NO FIELD TESTING IS REQUIRED. FOR PLACEMENT REQUIREMENTS, SEE "STANDARD NOTES" SHEET.
▲ 4-#10Z38 EQUALLY SPLAYED TOP OF FOOTING

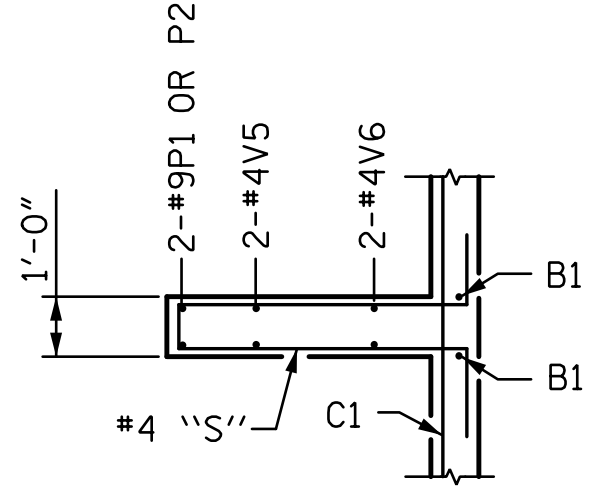


BILL OF MATERIAL FOR WING 4 (PHASE IV - STAGE 2)									
BAR	NO.	SIZE	TYPE	LENGTH	WEIGHT	BAR	NO.	SIZE	TYPE
B1	2	#4	STR	11'-0"	15				
C2	15	#6	STR	8'-8"	195				
F37	1	#4	8	10'-10"	7				
H34	18	#4	1	3'-3"	39				
M1	5	#8	2	16'-4"	218				
N35	2	#6	3	13'-11"	42				
P1	2	#9	4	7'-3"	49				
P2	2	#9	5	14'-8"	100				
S5	1	#4	6	12'-11"	9				
S6	1	#4	6	12'-3"	8				
S7	1	#4	6	11'-5"	8				
S8	1	#4	6	10'-9"	7				
S9	1	#4	6	10'-1"	7				
S10	1	#4	6	9'-3"	6				
S11	1	#4	6	8'-7"	6				
S12	1	#4	6	7'-11"	5				
S13	1	#4	6	7'-1"	5				
S14	1	#4	6	6'-5"	4				
T32	1	#5	STR	12'-0"	13				
T33	1	#5	STR	14'-0"	15				
V5	2	#4	STR	5'-0"	7				
V6	2	#4	STR	9'-6"	13				
V35	2	#4	STR	11'-3"	15				
Z38	4	#10	7	11'-4"	195				
Z39	7	#10	STR	11'-1"	334				
Z40	7	#10	STR	9'-11"	299				
Z41	7	#10	STR	8'-10"	266				
REINFORCING STEEL FOR WING 4 (PHASE IV - STAGE 2)					1,887 LBS.				



SECTION A-A

STANDARD REINFORCING STEEL IN BARREL NOT SHOWN

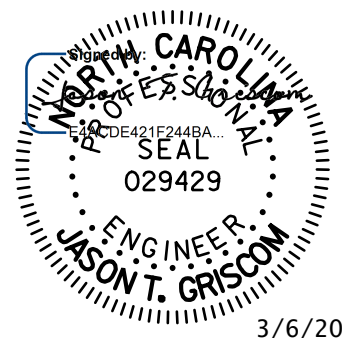


SECTION B-B

SPlice LENGTHS		
BAR	SIZE	LENGTH
H31	#4	1'-10"

PROJECT NO. U-5797
ROBESON COUNTY
STATION: 20+80.00 -Y2-

SHEET 12 OF 12

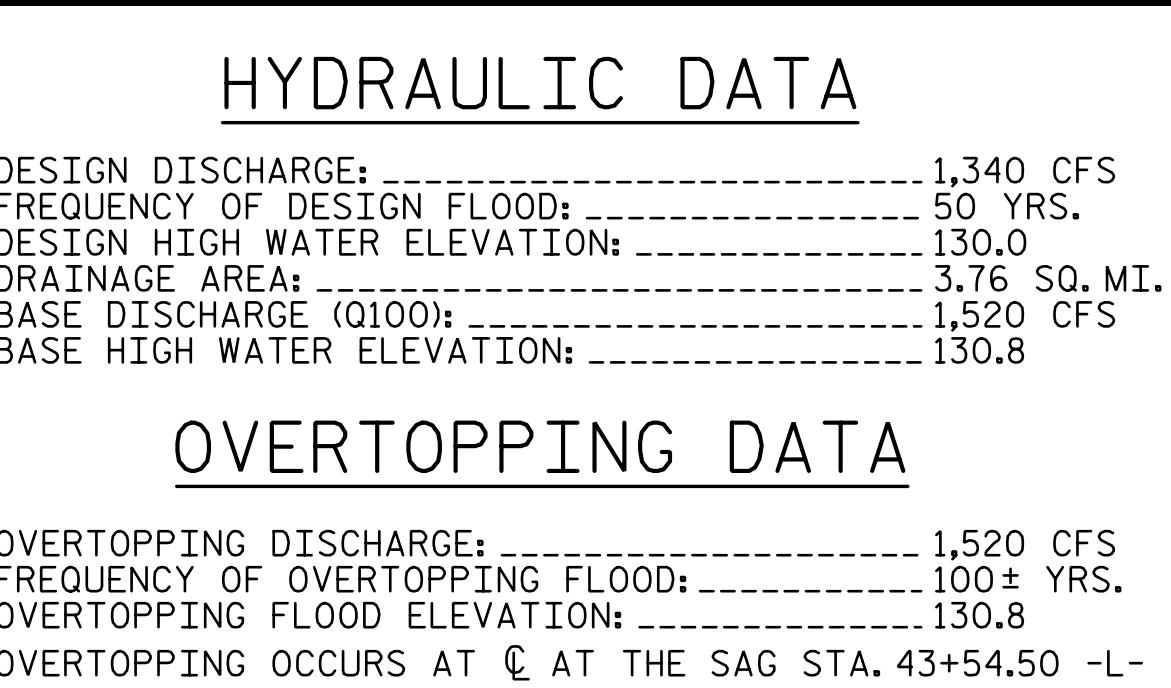
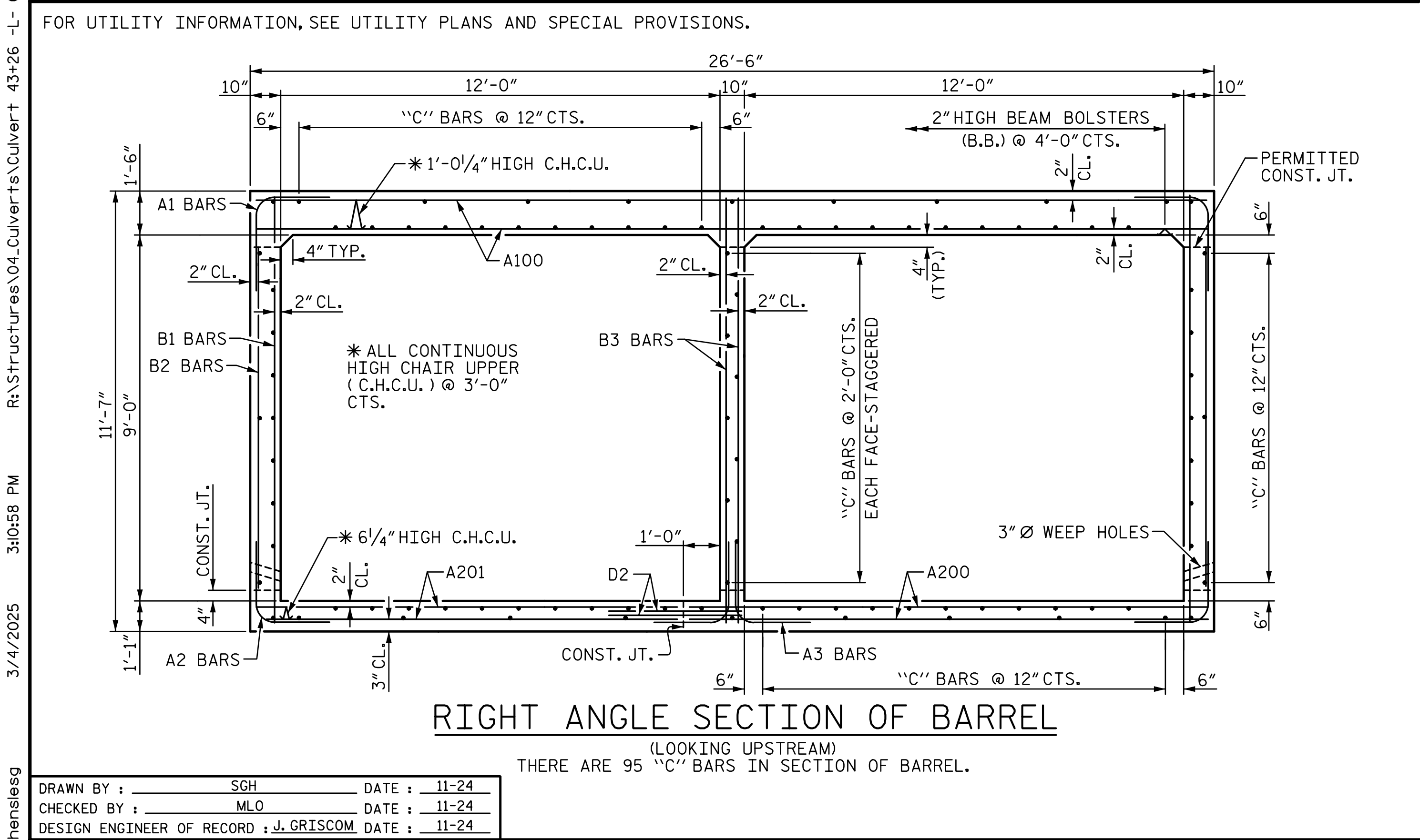
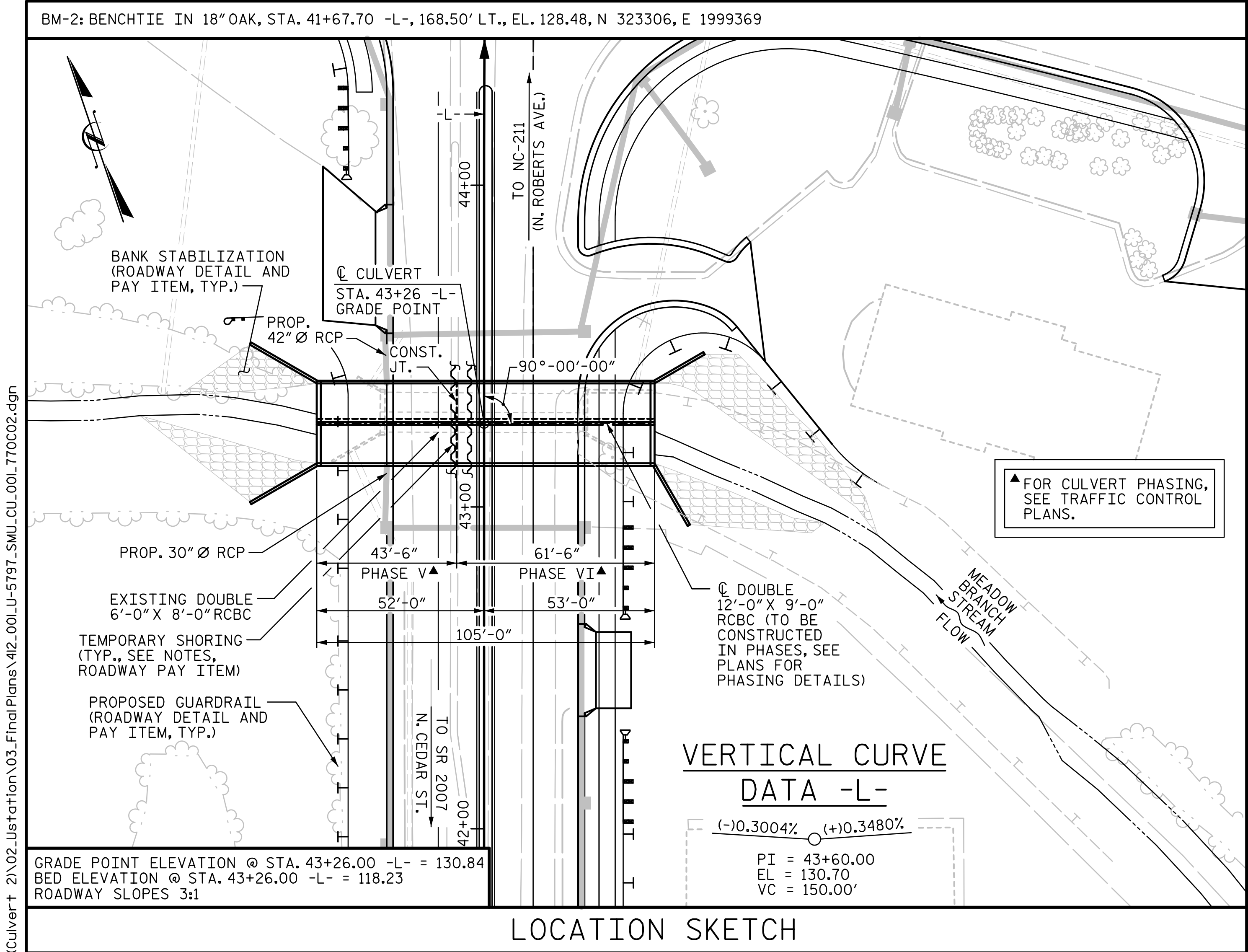


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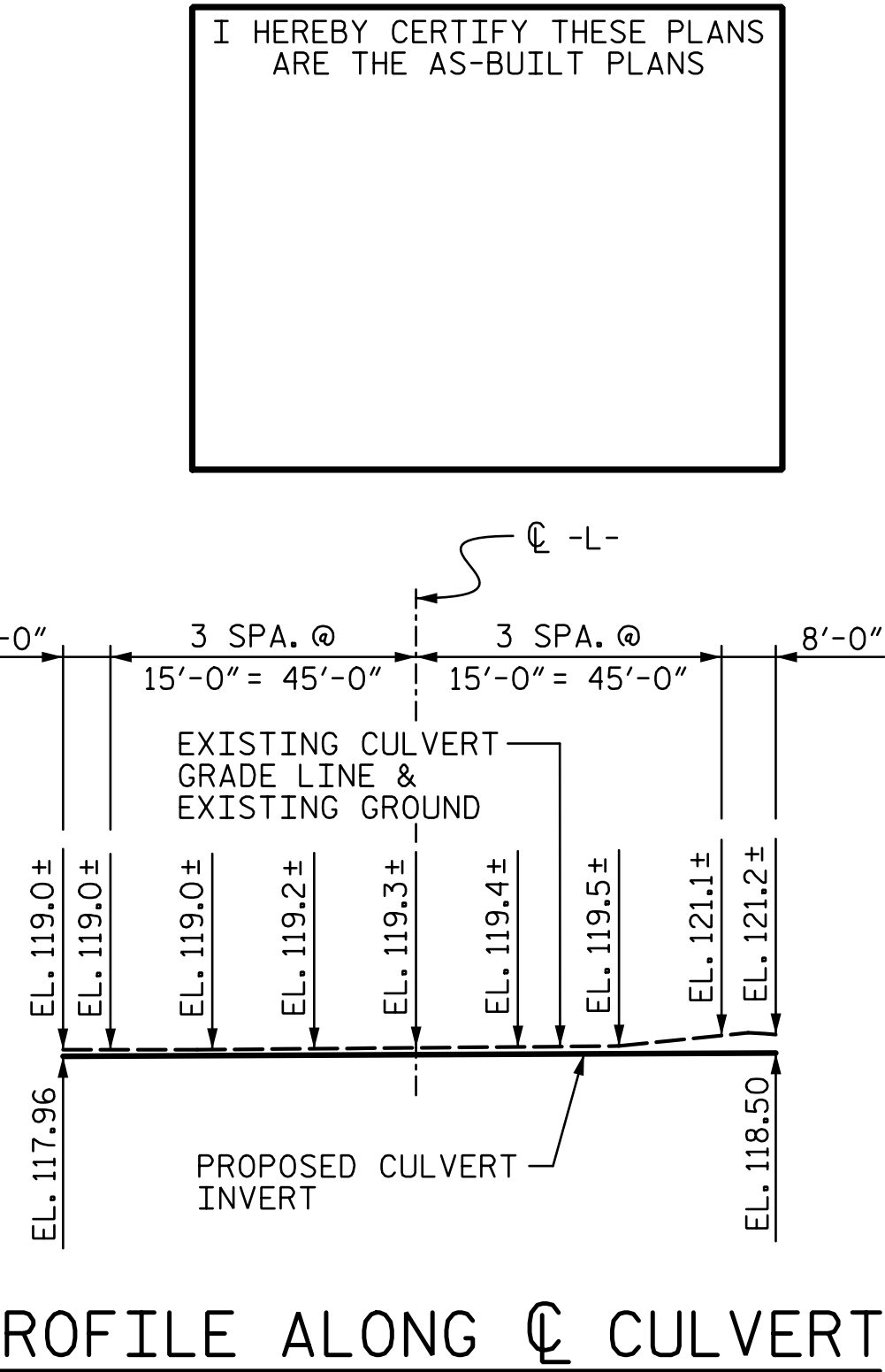
DOCUMENT NOT CONSIDERED FINAL UNLESS ALL SIGNATURES COMPLETED

STATE OF NORTH CAROLINA DEPARTMENT OF TRANSPORTATION RALEIGH									
WING 4 FOR CONCRETE BOX CULVERT									
H = 10'-0" SLOPE = 3:1 45° SKEW									
REVISIONS									
NO.	BY:	DATE:	NO.	BY:	DATE:	SHEET NO.			
1			3			C1-12			
2			4			TOTAL SHEETS 12			

DRAWN BY :	SGH	DATE :	11-24
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DESIGN ENGINEER OF RECORD :	J. GRISCOM	DATE :	11-24



TOTAL STRUCTURE QUANTITIES	
CLASS A CONCRETE	
PHASE V - STAGE 1	68.8 C.Y.
PHASE V - STAGE 2	121.0 C.Y.
PHASE VI - STAGE 3	88.5 C.Y.
PHASE VI - STAGE 4	158.1 C.Y.
TOTAL	436.4 C.Y.
REINFORCING STEEL	
PHASE V - STAGE 1	9,212 LBS.
PHASE V - STAGE 2	16,680 LBS.
PHASE VI - STAGE 3	12,033 LBS.
PHASE VI - STAGE 4	21,996 LBS.
TOTAL	59,921 LBS.
CULVERT EXCAVATION	LUMP SUM
FOUNDATION CONDITIONING MATERIAL	
PHASE V - STAGE 1	51 TONS
PHASE V - STAGE 2	42 TONS
PHASE VI - STAGE 3	72 TONS
PHASE VI - STAGE 4	60 TONS
TOTAL	225 TONS
REMOVAL OF EXISTING STRUCTURE STA. 43+26.00 -L- LUMP SUM	



NOTES:

ASSUMED LIVE LOAD = HL-93 OR ALTERNATE LOADING.

DESIGN FILL----- 2.29' MAX. AND 1.06' MIN.

FOR OTHER DESIGN DATA AND NOTES SEE STANDARD NOTE SHEET.

3"Ø WEEP HOLES INDICATED TO BE IN ACCORDANCE WITH THE SPECIFICATIONS.

CONCRETE IN EACH STAGE IN CULVERTS TO BE POURED IN THE FOLLOWING ORDER:

1. WING FOOTINGS AND FLOOR SLAB INCLUDING 4" OF ALL VERTICAL WALLS.
2. THE REMAINING PORTIONS OF THE WALLS AND WINGS FULL HEIGHT, ROOF SLAB AND HEADWALLS FOLLOWED BY THE SILLS.

THE RESIDENT ENGINEER SHALL CHECK THE LENGTH OF CULVERT BEFORE STAKING IT OUT TO MAKE CERTAIN THAT IT WILL PROPERLY TAKE CARE OF THE FILL.

DIMENSIONS FOR WING LAYOUTS AS WELL AS ADDITIONAL REINFORCING STEEL EMBEDDED IN BARREL ARE SHOWN ON WING SHEETS.

AT THE CONTRACTOR'S OPTION, HE MAY SPLICE THE VERTICAL REINFORCING STEEL IN THE INTERIOR FACE OF EXTERIOR WALL ABOVE LOWER WALL AND BOTH FACES OF INTERIOR WALLS ABOVE LOWER WALL CONSTRUCTION JOINT. THE SPLICE LENGTH SHALL BE AS PROVIDED IN THE SPLICE LENGTH CHART SHOWN ON THE PLANS. EXTRA WEIGHT OF STEEL DUE TO THE SPLICES SHALL BE PAID FOR BY THE CONTRACTOR.

FOR CULVERT DIVERSION DETAILS AND PAY ITEM, SEE EROSION CONTROL PLANS.

A 3 FOOT STRIP OF FILTER FABRIC SHALL BE ATTACHED TO THE FILL FACE OF THE WING COVERING THE ENTIRE LENGTH OF THE EXPANSION JOINT.

FOR SUBMITTAL OF WORKING DRAWINGS, SEE SPECIAL PROVISIONS.

FOR FALSEWORK AND FORMWORK, SEE SPECIAL PROVISIONS.

FOR CRANE SAFETY, SEE SPECIAL PROVISIONS.

FOR GROUT FOR STRUCTURES, SEE SPECIAL PROVISIONS.

NATIVE BED MATERIAL SHALL BE PLACED BETWEEN THE SILLS OF THE CULVERT TO PROVIDE A CONTINUOUS LOW FLOW CHANNEL. NATIVE MATERIAL CONSISTS OF MATERIAL THAT IS EXCAVATED FROM THE STREAM BED OR FLOODPLAIN AT THE PROJECT SITE DURING CONSTRUCTION. ONLY MATERIAL THAT IS EXCAVATED FROM THE STREAM BED MAY BE USED TO LINE THE LOW FLOW CULVERT BARREL. RIP RAP MAY BE USED TO SUPPLEMENT THE NATIVE BED MATERIAL IN THE HIGH FLOW CULVERT BARREL. IF RIP RAP IS USED TO LINE THE HIGH FLOW BARREL, NATIVE MATERIAL SHOULD BE PLACED ON TOP TO FILL VOIDS AND PROVIDE A FLAT SURFACE FOR ANIMAL PASSAGE. NATIVE MATERIAL IS SUBJECT TO APPROVAL BY THE ENGINEER AND MAY BE SUBJECT TO PERMIT CONDITIONS.

FOR LIMITS OF TEMPORARY SHORING FOR MAINTENANCE OF TRAFFIC, SEE TRAFFIC CONTROL PLANS. FOR PAY ITEM FOR TEMPORARY SHORING FOR MAINTENANCE OF TRAFFIC, SEE ROADWAY PLANS.

AFTER SERVING AS A TEMPORARY STRUCTURE THE EXISTING DOUBLE 6'-0" X 8'-0" RCBC LOCATED AT THE SAME LOCATION AS THE PROPOSED CULVERT SHALL BE REMOVED. THE EXISTING STRUCTURE IS PRESENTLY NOT POSTED FOR LOAD LIMIT. SHOULD THE STRUCTURAL INTEGRITY OF THE STRUCTURE DETERIORATE DURING CONSTRUCTION OF THE PROPOSED CULVERT, A LOAD LIMIT MAY BE POSTED AND MAY BE REDUCED AS FOUND NECESSARY DURING THE LIFE OF THE PROJECT.

TRAFFIC ON SR 1997 (FAYETTEVILLE RD.) SHALL BE MAINTAINED. IN ORDER TO MAINTAIN TRAFFIC THE CULVERT SHALL BE CONSTRUCTED IN PHASES AS SHOWN IN THE PLANS, SEE TRAFFIC CONTROL PLANS.

EXCAVATE FOUNDATION A MINIMUM OF 12" BELOW CULVERT BEARING ELEVATION. PLACE 12" OF CLASS VI FOUNDATION CONDITIONING MATERIAL IN ACCORDANCE WITH SECTION 414 OF THE STANDARD SPECIFICATIONS.

OVEREXCAVATE ADDITIONAL LOOSE/SOFT OR ORGANIC MATERIAL IF PRESENT TO SUITABLE BEARING MATERIALS AND REPLACE WITH ADDITIONAL CLASS VI FOUNDATION CONDITIONING MATERIAL.

ENCAPSULATE ALL FOUNDATION CONDITIONING MATERIAL IN TYPE 4 GEOTEXTILE. FOR FOUNDATION CONDITIONING GEOTEXTILE, SEE BOX CULVERT EXCAVATION (SPECIAL) PROVISION.

NO PRECAST REINFORCED BOX CULVERT OPTION WILL BE ALLOWED.

PROJECT NO. _____			U-5797		
_____ ROBESON _____			COUNTY		
STATION: _____			43+26.00 -L-		
SHEET 1 OF 11			STRUCTURE 770581		
STATE OF NORTH CAROLINA DEPARTMENT OF TRANSPORTATION RALEIGH					
DOUBLE 12'-0" X 9'-0" CONCRETE BOX CULVERT 90°-00'-00" SKEW					
REVISIONS					SHEET NO.
NO.	BY:	DATE:	NO.	BY:	DATE:
1			3		
2			4		
					TOTAL SHEETS
					11

LOAD FACTORS:

DESIGN LOAD RATING FACTORS

LOAD TYPE	MAX FACTOR	MIN FACTOR
DC	1.25	0.90
DW	1.50	0.65
EV	1.30	0.90
EH	1.35	0.90
ES	1.35	0.90
LS	1.75	--
WA	1.00	--

NOTE:

RATING FACTORS ARE BASED ON THE STRENGTH I LIMIT STATE.

1. FOR WALL ELEMENTS THE LEFT END OF ELEMENT IS LOCATED FROM THE BOTTOM OF THE BOTTOM SLAB.

COMMENTS:

CONTROLLING LOAD RATING

1 DESIGN LOAD RATING (HL-93)

2 DESIGN LOAD RATING (HS-20)

3 LEGAL LOAD RATING **

4 EMERGENCY VEHICLE LAOD RATING **

** SEE CHART FOR VEHICLE TYPE

PROJECT NO. U-5797

ROBESON COUNTY

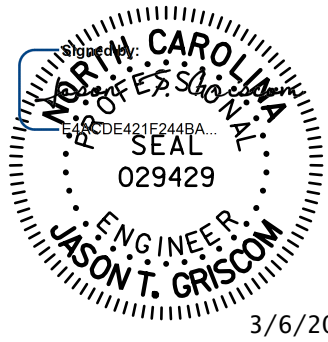
STATION: 43+26.00 -L-

SHEET 2 OF 11

STATE OF NORTH CAROLINA
DEPARTMENT OF TRANSPORTATION
RALEIGH

LRFR SUMMARY FOR
REINFORCED CONCRETE
BOX CULVERTS
(NON-INTERSTATE TRAFFIC)

REVISIONS					SHEET NO.
NO.	BY:	DATE:	NO.	BY:	DATE:
1			3		
2			4		
					TOTAL SHEETS 11



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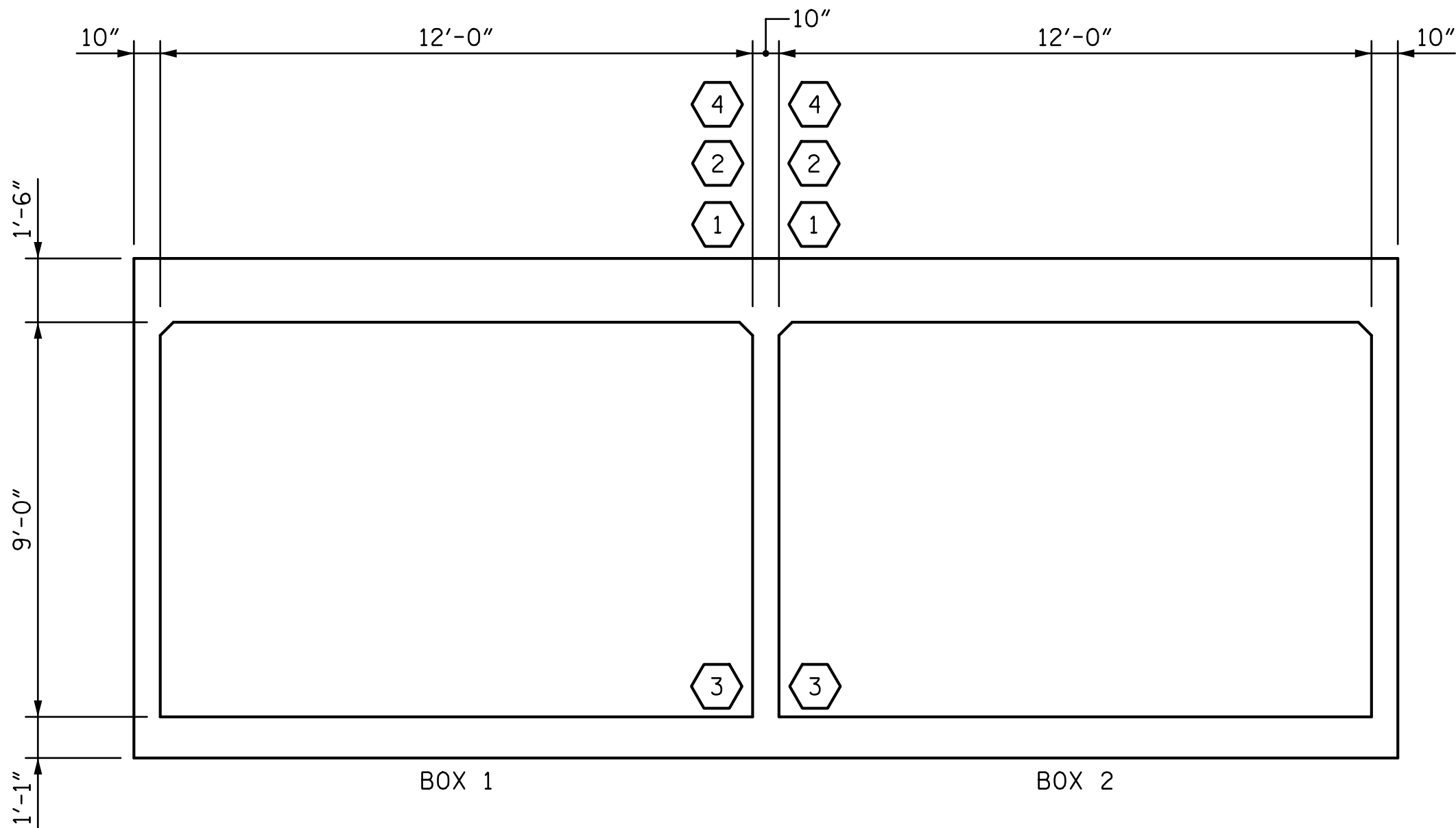


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LOAD AND RESISTANCE FACTOR RATING (LRFR)
SUMMARY FOR REINFORCED CONCRETE BOX CULVERTS

LOAD AND RESISTANCE FACTOR RATING (LRFR) SUMMARY FOR REINFORCED CONCRETE BOX CULVERTS																
LOAD TYPE	VEHICLE	WEIGHT (W) (TONS)	⬡	MINIMUM RATING FACTORS (RF)	TONS = W × RF	STRENGTH I LIMIT STATE									COMMENT NUMBER	
						LIVE-LOAD FACTORS (LL)	MOMENT				SHEAR					
							RATING FACTOR	BOX NO.	ELEMENT TYPE	DISTANCE FROM LEFT END OF ELEMENT (ft)	RATING FACTOR	BOX NO.	ELEMENT TYPE	DISTANCE FROM LEFT END OF ELEMENT (ft)		
DESIGN LOAD RATING	HL-93 (INVENTORY)	N/A	1	1.04	--	1.75	2.28	1 & 2	TOP SLAB	5.55'/20.95'	1.04	1 & 2	TOP SLAB	12.83'/13.67'		
	HL-93 (OPERATING)	N/A		1.35	--	1.35	2.95	1 & 2	TOP SLAB	5.55'/20.95'	1.35	1 & 2	TOP SLAB	12.83'/13.67'		
	HS-20 (INVENTORY)	36.000	2	1.52	54.720	1.75	2.70	1 & 2	BOTTOM SLAB	12.83'/13.67'	1.52	1 & 2	TOP SLAB	12.83'/13.67'		
	HS-20 (OPERATING)	36.000		1.97	70.920	1.35	3.51	1 & 2	BOTTOM SLAB	12.83'/13.67'	1.97	1 & 2	TOP SLAB	12.83'/13.67'		
LEGAL LOAD RATING	SINGLE VEHICLE (SV)	SNSH		3.40	45.900	1.40	4.72	1 & 2	EXTERIOR WALL	10.08'	3.40	1 & 2	TOP SLAB	12.83'/13.67'		
		SNGARBS2	20.000		2.96	59.200	1.40	4.59	1 & 2	EXTERIOR WALL	10.08'	2.96	1 & 2	TOP SLAB	12.83'/13.67'	
		SNAGRIS2	22.000		3.11	68.420	1.40	4.47	1 & 2	EXTERIOR WALL	1.08'	3.11	1 & 2	TOP SLAB	12.83'/13.67'	
		SNCOTTS3	27.250		1.47	40.058	1.40	3.05	1 & 2	TOP SLAB	5.55'/20.95'	1.47	1 & 2	TOP SLAB	12.83'/13.67'	
		SNAGGRS4	34.925		1.70	59.373	1.40	3.00	1 & 2	BOTTOM SLAB	12.83'/13.67'	1.70	1 & 2	TOP SLAB	12.83'/13.67'	
		SNS5A	35.550		1.62	57.591	1.40	2.93	1 & 2	BOTTOM SLAB	12.83'/13.67'	1.62	1 & 2	TOP SLAB	12.83'/13.67'	
		SNS6A	39.950		1.60	63.920	1.40	2.85	1 & 2	BOTTOM SLAB	12.83'/13.67'	1.60	1 & 2	TOP SLAB	12.83'/13.67'	
		SNS7B	42.000		1.59	66.780	1.40	2.86	1 & 2	BOTTOM SLAB	12.83'/13.67'	1.59	1 & 2	BOTTOM SLAB	12.83'/13.67'	
	TRUCK TRACTOR SEMI-TRAILER (TTST)	TNAGRIT3	33.000		1.93	63.690	1.40	3.32	1 & 2	BOTTOM SLAB	12.83'/13.67'	1.93	1 & 2	BOTTOM SLAB	12.83'/13.67'	
		TNT4A	33.075		1.74	57.551	1.40	3.18	1 & 2	BOTTOM SLAB	12.83'/13.67'	1.74	1 & 2	TOP SLAB	12.83'/13.67'	
		TNT6A	41.600		1.57	65.312	1.40	2.96	1 & 2	BOTTOM SLAB	12.83'/13.67'	1.57	1 & 2	TOP SLAB	12.83'/13.67'	
		TNT7A	42.000		1.55	65.100	1.40	2.79	1 & 2	BOTTOM SLAB	12.83'/13.67'	1.55	1 & 2	BOTTOM SLAB	12.83'/13.67'	
		TNT7B	42.000		1.68	70.560	1.40	2.92	1 & 2	BOTTOM SLAB	12.83'/13.67'	1.68	1 & 2	TOP SLAB	12.83'/13.67'	
		TNAGRIT4	43.000	3	1.33	57.190	1.40	2.51	1 & 2	BOTTOM SLAB	12.83'/13.67'	1.33	1 & 2	BOTTOM SLAB	12.83'/13.67'	
		TNAGT5A	45.000		1.58	71.100	1.40	2.86	1 & 2	BOTTOM SLAB	12.83'/13.67'	1.58	1 & 2	BOTTOM SLAB	12.83'/13.67'	
		TNAGT5B	45.000		1.37	61.650	1.40	2.59	1 & 2	BOTTOM SLAB	12.83'/13.67'	1.37	1 & 2	BOTTOM SLAB	12.83'/13.67'	
EMERGENCY VEHICLE (EV)	EV2	28.750		2.04	58.650	1.30	3.54	1 & 2	TOP SLAB	5.55'/20.95'	2.04	1 & 2	TOP SLAB	12.83'/13.67'		
	EV3	43.000	4	1.16	49.880	1.30	2.58	1 & 2	BOTTOM SLAB	12.83'/13.67'	1.16	1 & 2	TOP SLAB	12.83'/13.67'		



LRFR SUMMARY

(LOOKING DOWNSTREAM)

DRAWN BY : SGH DATE : 11-24
CHECKED BY : MLO DATE : 11-24
DESIGN ENGINEER OF RECORD : J. GRISCOM DATE : 11-24



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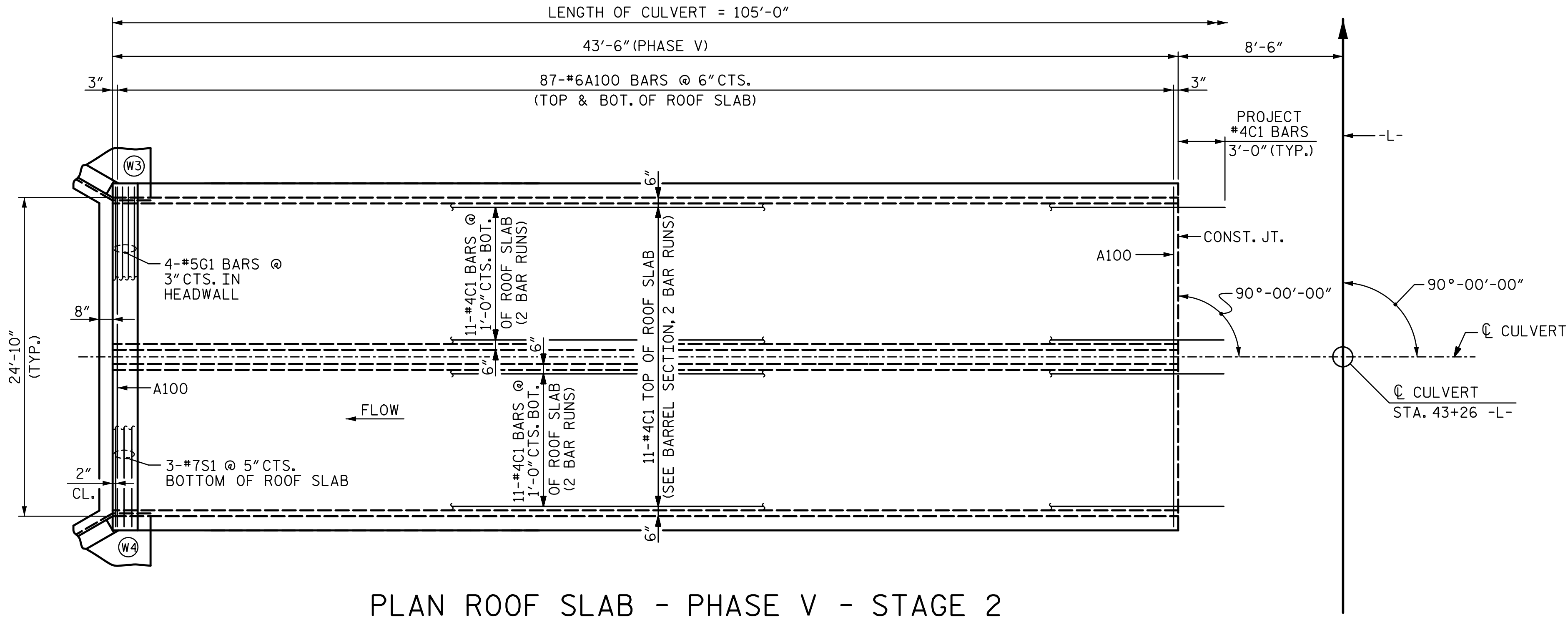
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NO.	BY:	DATE:	NO.	BY:	DATE:	
1			3			TOTAL SHEETS 11
2			4			

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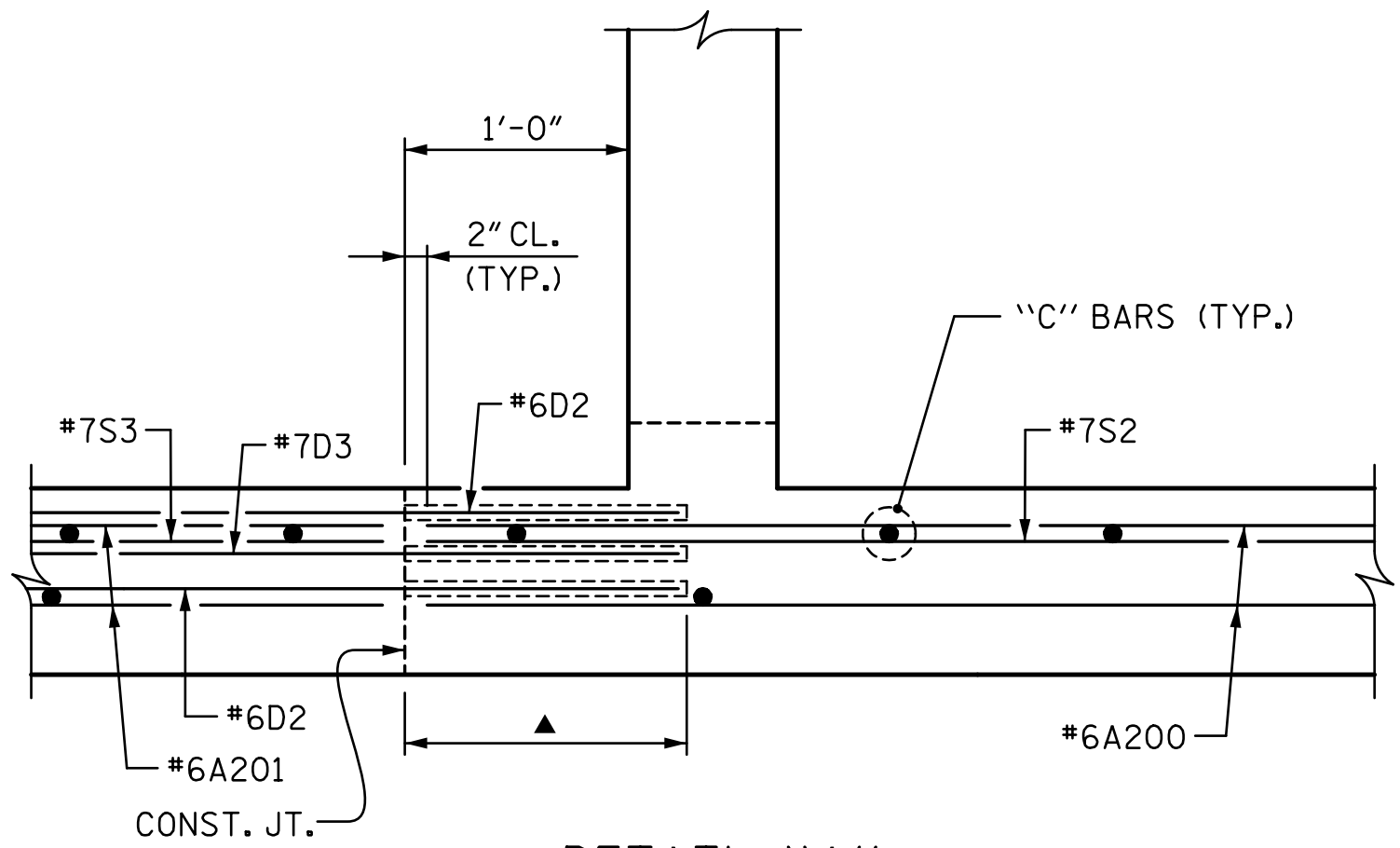
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PLAN ROOF SLAB - PHASE V - STAGE 2



DETAIL "A"

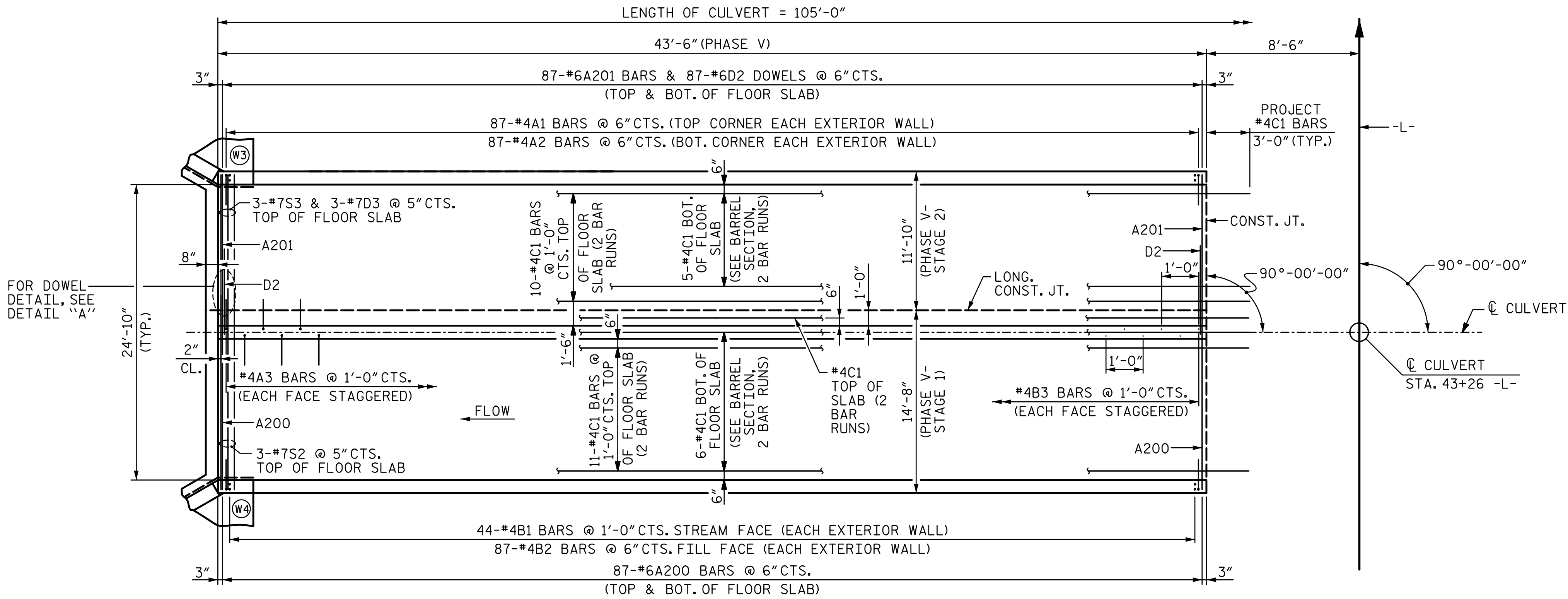
(A3 AND B3 BARS IN INTERIOR WALL NOT SHOWN FOR CLARITY.)

▲ EMBEDMENT SHALL BE PER MANUFACTURER'S RECOMMENDATION AND THE STANDARD SPECIFICATIONS.

THE CONTRACTOR SHALL USE ADHESIVELY ANCHORED DOWELS TO CONNECT EACH STAGE. LEVEL ONE FIELD TESTING IS REQUIRED. THE YIELD LOAD OF THE #6D2 DOWELS IS 26.4 KIPS AND THE YIELD LOAD OF THE #7D3 DOWELS IS 36.0 KIPS. FOR ADHESIVELY ANCHORED DOWELS, SEE STANDARD SPECIFICATIONS.

#6D2 DOWELS SHALL BE PLACED IN THE SAME HORIZONTAL PLANE AS THE TOP AND BOTTOM FLOOR SLAB REINFORCING.

#7D3 DOWELS SHALL BE PLACED IN THE SAME HORIZONTAL PLANE AS THE #7 S2 & S3 BARS.



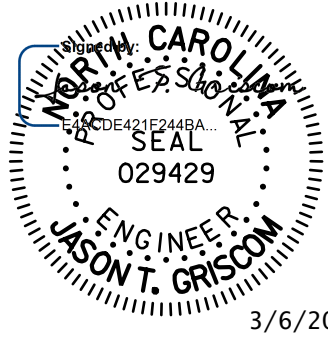
PLAN FLOOR SLAB AND WALLS - PHASE V - STAGES 1 & 2

PROJECT NO. U-5797

ROBESON COUNTY

STATION: 43+26.00 -L-

SHEET 4 OF 11



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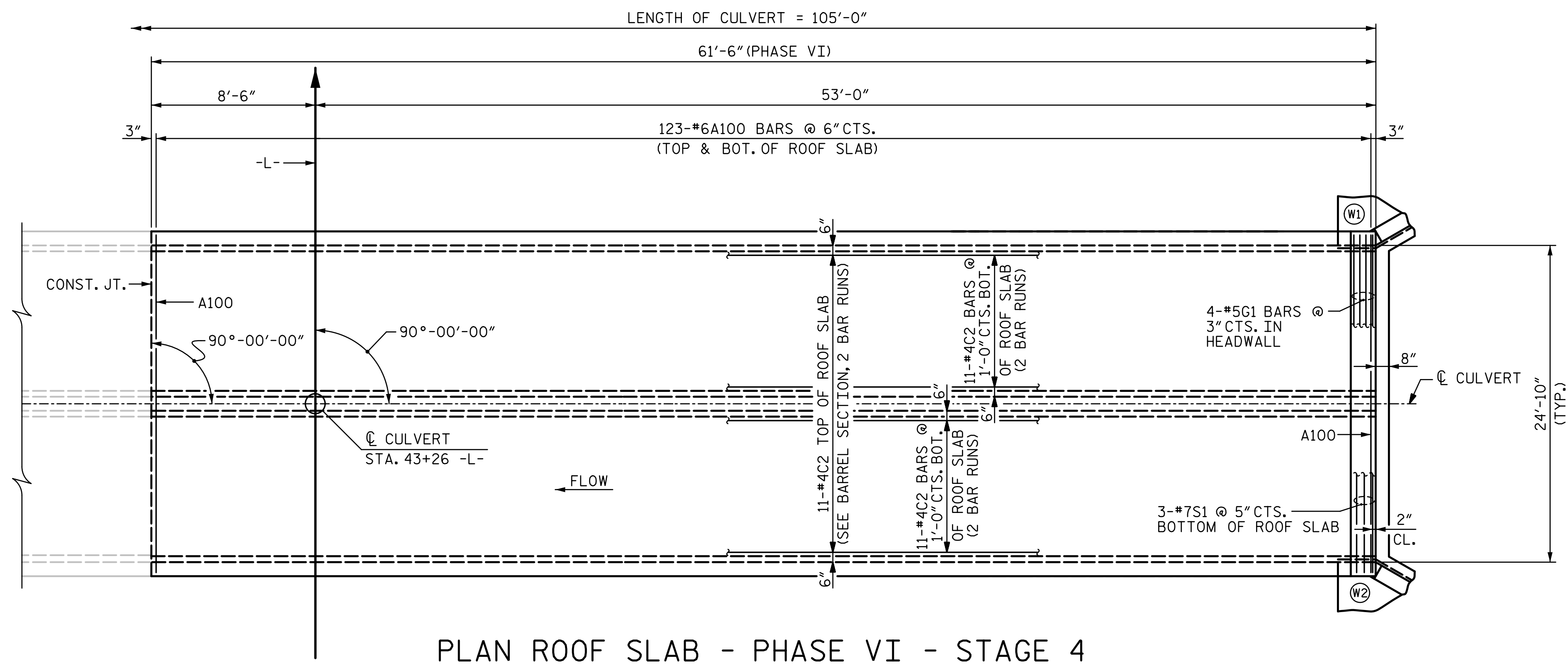
DOCUMENT NOT CONSIDERED
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STATE OF NORTH CAROLINA
DEPARTMENT OF TRANSPORTATION
RALEIGH

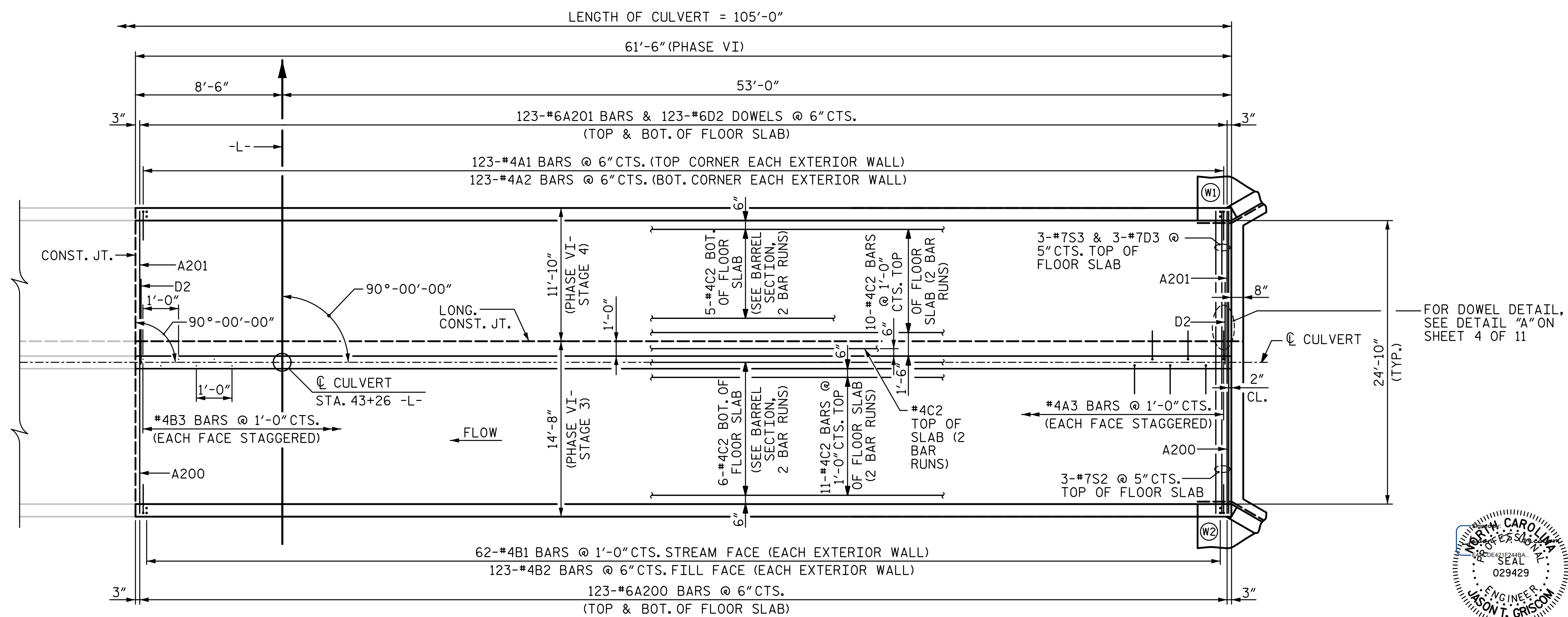
DOUBLE 12'-0" X 9'-0"
CONCRETE BOX CULVERT
90°-00'-00" SKEW

DRAWN BY : SGH DATE : 11-24
CHECKED BY : MLO DATE : 11-24
DESIGN ENGINEER OF RECORD : J. GRISCOM DATE : 11-24

REVISIONS					SHEET NO. C2-4
NO.	BY:	DATE:	NO.	BY:	
1			3		TOTAL SHEETS
2			4		11



PLAN ROOF SLAB - PHASE VI - STAGE 4



PLAN FLOOR SLAB AND WALLS - PHASE VI - STAGES 3 & 4

NOTES:

SPLICE #4C2 BARS 2'-5" MIN. TO #4C1 BARS
IN PHASE V - STAGES 1 & 2

PROJECT NO. U-5797

ROBESON COUNTY

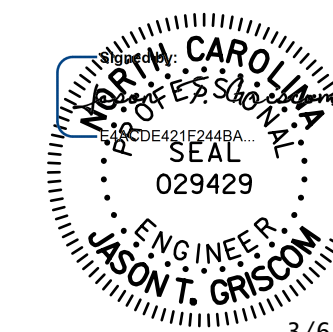
STATION: 43+26.00 -L-

SHEET 5 OF 11

STATE OF NORTH CAROLINA
DEPARTMENT OF TRANSPORTATION
 RAILEIGH

DOUBLE 12'-0" X 9'-0"
 CONCRETE BOX CULVERT
 90°-00'-00" SKEW

REVISIONS						SHEET NO.
NO.	BY:	DATE:	NO.	BY:	DATE:	C2-5 TOTAL SHEETS 11
1			3			
2			4			



3/6/2025

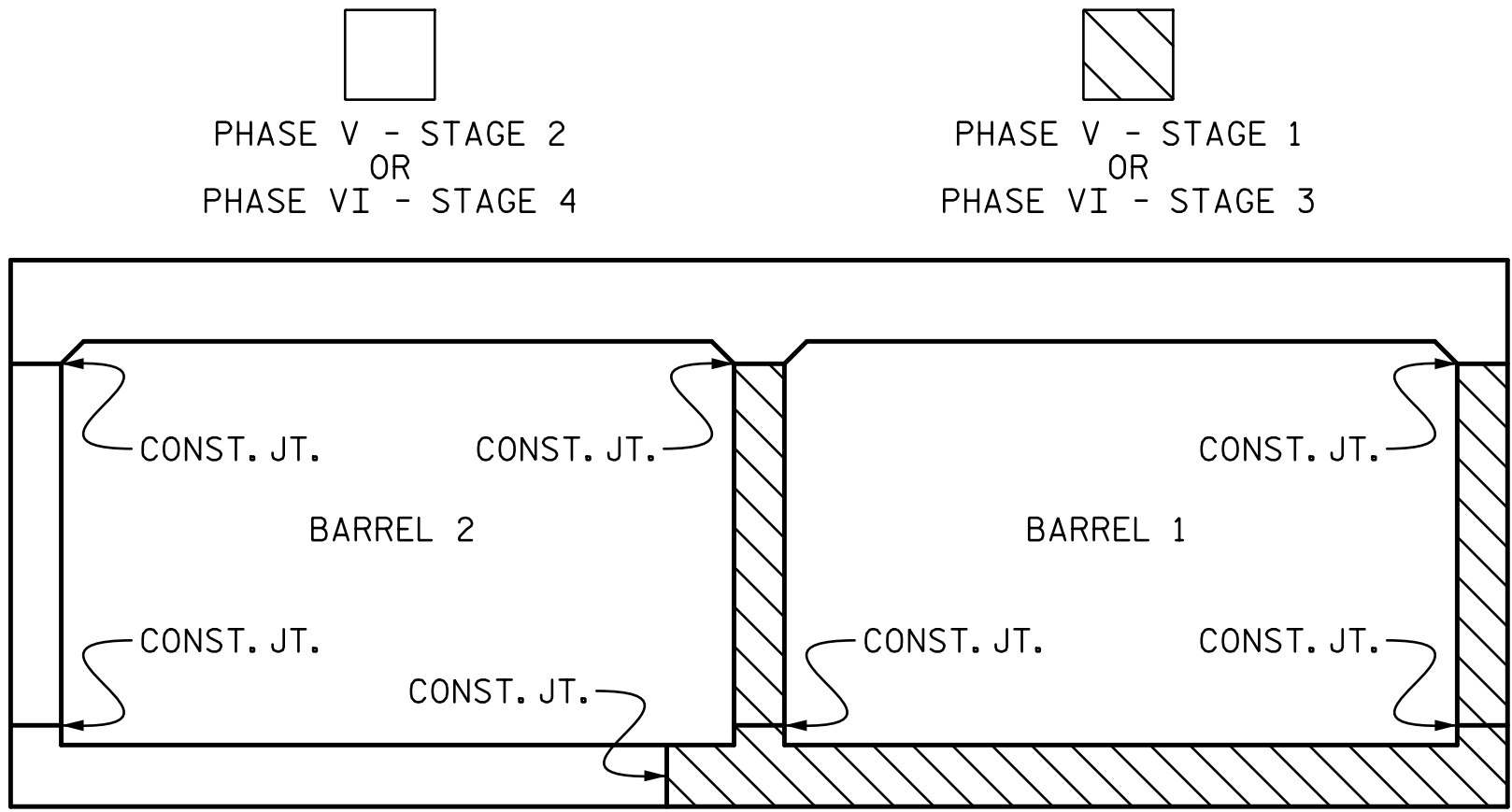


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 DESIGN ENGINEER OF RECORD : J. GRISCOM DATE : 11-24

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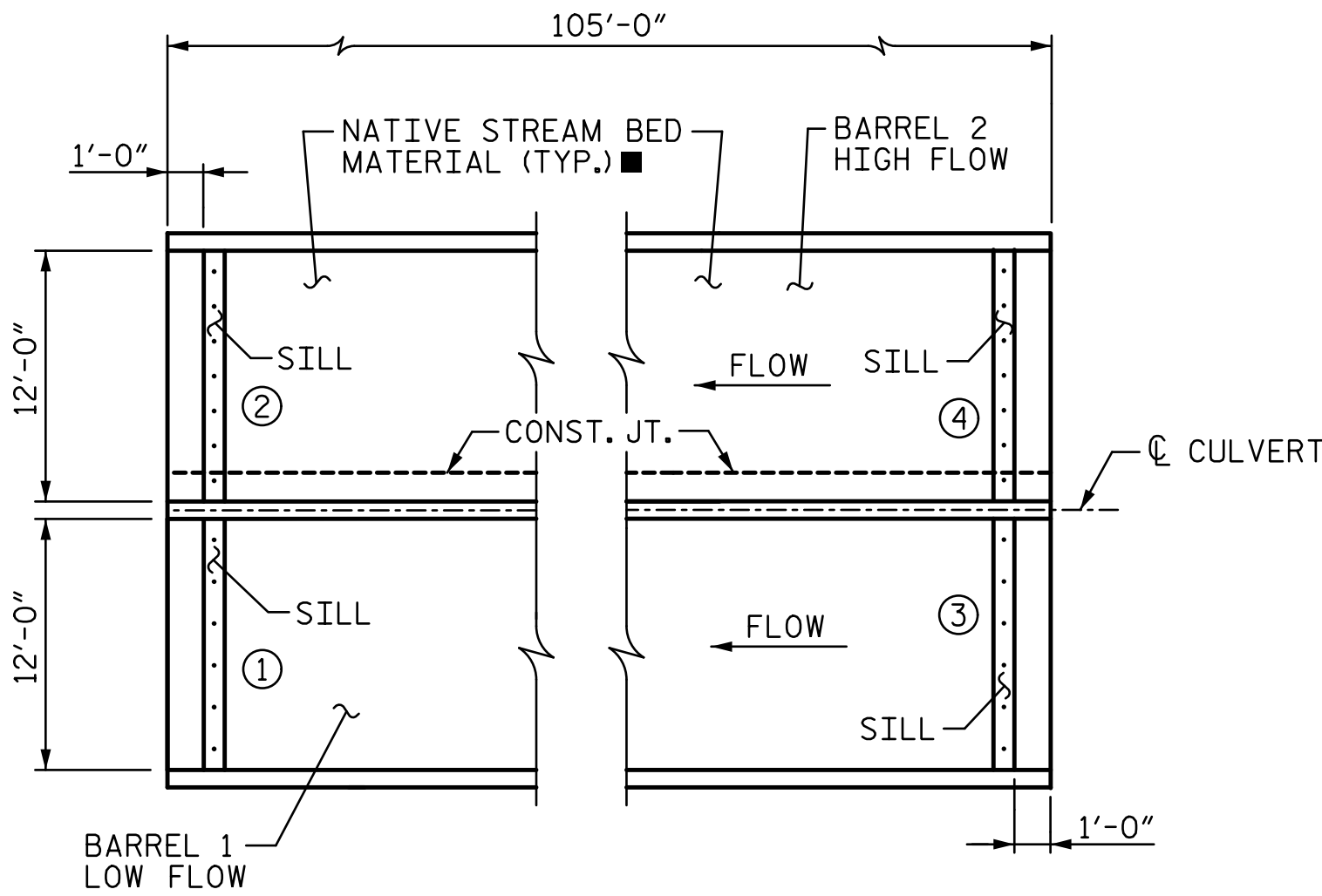


PHASE V - STAGE 1 AND PHASE V - STAGE 2
POUR 1: PHASE V - STAGE 1 - FLOOR SLAB AND 4" OF VERTICAL WALLS/WING
POUR 2: PHASE V - STAGE 1 - REMAINING PORTIONS OF EXTERIOR WALL, WING 4 AND INTERIOR WALL
POUR 3: PHASE V - STAGE 2 - FLOOR SLAB AND 4" OF VERTICAL WALL/WING
POUR 4: PHASE V - STAGE 2 - REMAINING PORTION OF EXTERIOR WALL AND WING 3
POUR 5: PHASE V - STAGE 2 - ENTIRE ROOF SLAB AND OUTLET HEADWALL

PHASE VI - STAGE 3 AND PHASE VI - STAGE 4
POUR 1: PHASE VI - STAGE 3 - FLOOR SLAB AND 4" OF VERTICAL WALLS/WING
POUR 2: PHASE VI - STAGE 3 - REMAINING PORTIONS OF EXTERIOR WALL, WING 2 AND INTERIOR WALL
POUR 3: PHASE VI - STAGE 4 - FLOOR SLAB AND 4" OF VERTICAL WALL/WING
POUR 4: PHASE VI - STAGE 4 - REMAINING PORTION OF EXTERIOR WALL AND WING 1
POUR 5: PHASE VI - STAGE 4 - ENTIRE ROOF SLAB AND INLET HEADWALL

CULVERT CONSTRUCTION SEQUENCE

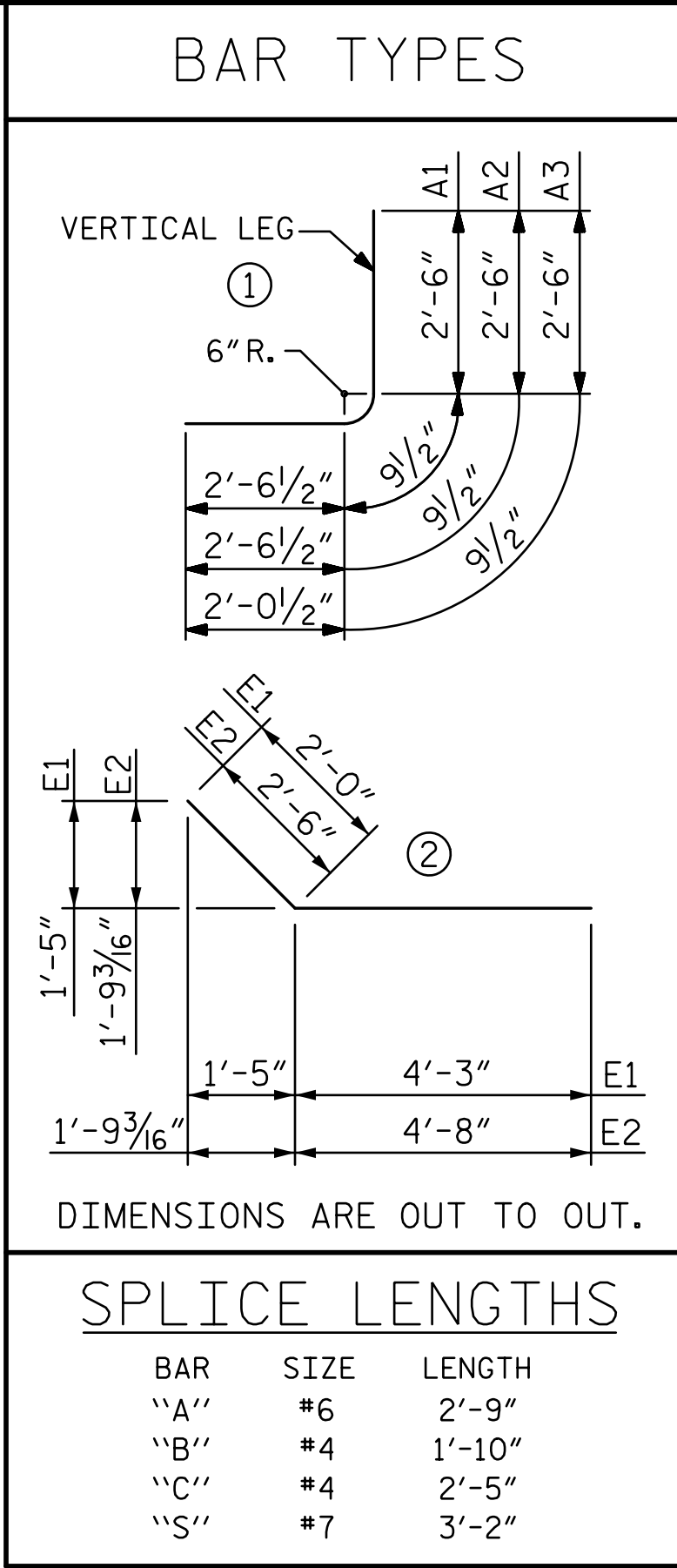
LOOKING UPSTREAM



PLAN VIEW - LOCATION OF SILLS

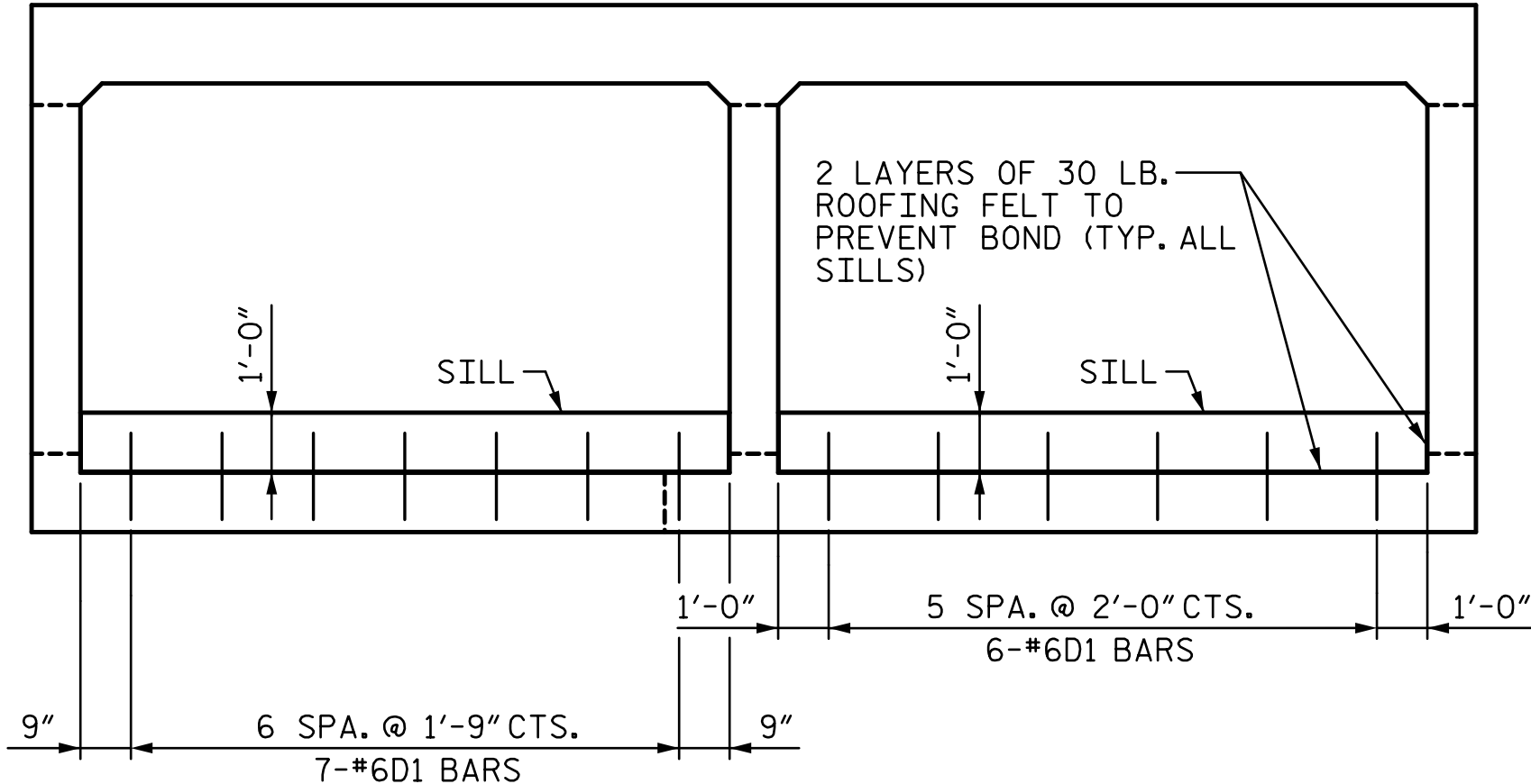
■ NATIVE STREAM BED MATERIAL SHALL BE USED TO BACKFILL THE CULVERT TO SILL HEIGHT BETWEEN SILLS. (SEE NOTE ON SHEET 1 OF 11)

- ① SILL TO BE CONSTRUCTED IN PHASE V - STAGE 1
- ② SILL TO BE CONSTRUCTED IN PHASE V - STAGE 2
- ③ SILL TO BE CONSTRUCTED IN PHASE VI - STAGE 3
- ④ SILL TO BE CONSTRUCTED IN PHASE VI - STAGE 4



SPLICE LENGTHS

BAR	SIZE	LENGTH
``A``	#6	2'-9"
``B``	#4	1'-10"
``C``	#4	2'-5"
``S``	#7	3'-2"



ELEVATION

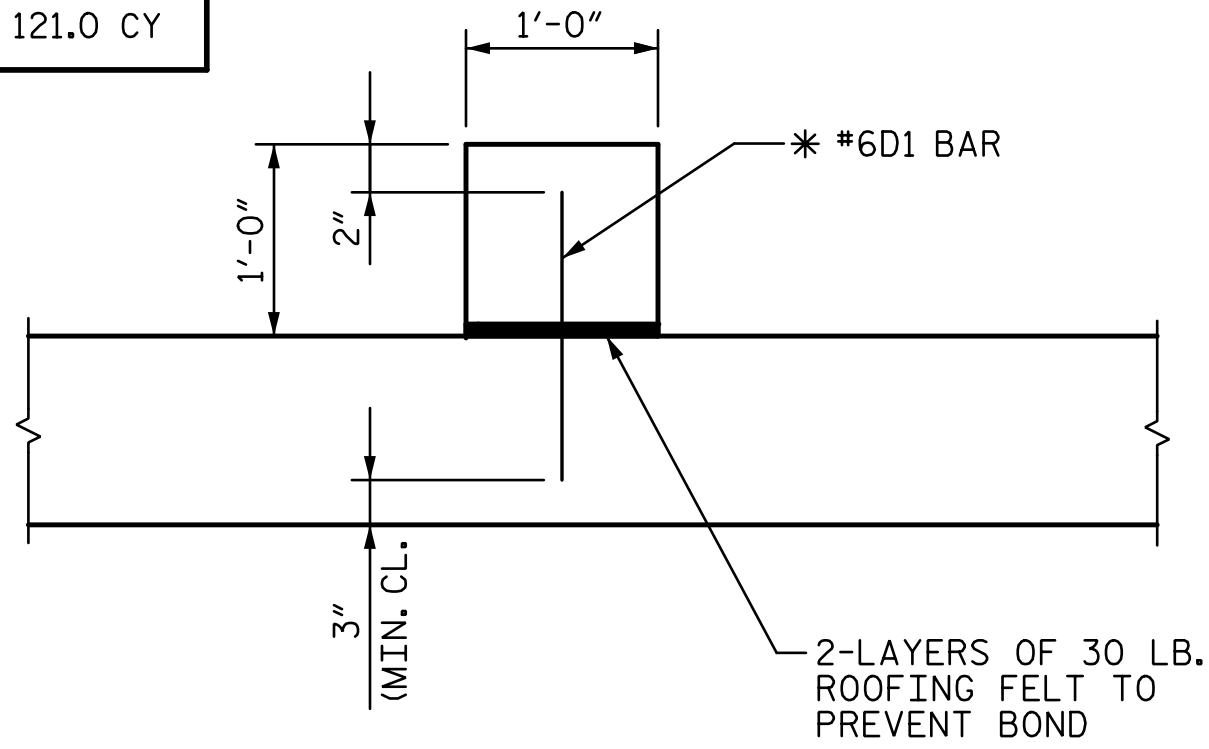
LOOKING UPSTREAM
ELEVATION AT OUTLET SHOWN, ELEVATION AT INLET SIMILAR.

BILL OF MATERIAL PHASE V - STAGE 1					
BAR	NO.	SIZE	TYPE	LENGTH	WEIGHT
A1	87	#4	1	5'-10"	339
A2	87	#4	1	5'-10"	339
A3	87	#4	1	5'-4"	310
A200	174	#6	STR	14'-4"	3,746
B1	44	#4	STR	11'-2"	328
B2	87	#4	STR	8'-4"	484
B3	87	#4	STR	11'-2"	649
C1	74	#4	STR	24'-5"	1,207
D1	6	#6	STR	1'-8"	15
E1	16	#5	2	6'-3"	104
S2	3	#7	STR	14'-4"	88
PHASE V - STAGE 1 REINFORCING STEEL 7,609 LBS.					
PHASE V - STAGE 1 QUANTITIES					
REINFORCING STEEL				7,609 LBS.	
BARREL				1,603 LBS.	
WING 4				9,212 LBS.	
TOTAL				9,212 LBS.	
CLASS A CONCRETE				48.9 CY	
BARREL @ 1.123 CY/FT				0.4 CY	
SILL				2.3 CY	
END CURTAIN WALL				17.2 CY	
WING 4				68.8 CY	
TOTAL				68.8 CY	

BILL OF MATERIAL PHASE V - STAGE 2					
BAR	NO.	SIZE	TYPE	LENGTH	WEIGHT
A1	87	#4	1	5'-10"	339
A2	87	#4	1	5'-10"	339
A3	87	#4	1	5'-4"	310
A100	174	#6	STR	26'-2"	6,839
A201	174	#6	STR	11'-6"	3,006
B1	44	#4	STR	11'-2"	328
B2	87	#4	STR	8'-4"	484
C1	116	#4	STR	24'-5"	1,892
D1	7	#6	STR	1'-8"	18
D2	174	#6	STR	4'-6"	1,176
D3	3	#7	STR	4'-10"	30
E2	16	#5	2	7'-2"	120
G1	4	#5	STR	26'-2"	109
S1	3	#7	STR	26'-2"	160
S3	3	#7	STR	11'-6"	71
PHASE V - STAGE 2 REINFORCING STEEL 14,911 LBS.					
PHASE V - STAGE 2 QUANTITIES					
REINFORCING STEEL				14,911 LBS.	
BARREL				1,769 LBS.	
WING 3				16,680 LBS.	
TOTAL				16,680 LBS.	
CLASS A CONCRETE				98.0 CY	
BARREL @ 2.254 CY/FT				0.4 CY	
SILL				2.1 CY	
END CURTAIN WALL				19.3 CY	
WING 3				1.2 CY	
HEADWALL				121.0 CY	
TOTAL				121.0 CY	

BILL OF MATERIAL PHASE VI - STAGE 3					
BAR	NO.	SIZE	TYPE	LENGTH	WEIGHT
A1	123	#4	1	5'-10"	479
A2	123	#4	1	5'-10"	479
A3	123	#4	1	5'-4"	438
A200	246	#6	STR	14'-4"	5,296
B1	62	#4	STR	11'-2"	462
B2	123	#4	STR	8'-4"	685
B3	123	#4	STR	11'-2"	917
C2	74	#4	STR	31'-10"	1,574
D1	6	#6	STR	1'-8"	15
D2	246	#6	STR	4'-6"	1,663
D3	3	#7	STR	4'-10"	30
S2	3	#7	STR	14'-4"	88
PHASE VI - STAGE 3 REINFORCING STEEL 10,433 LBS.					
PHASE VI - STAGE 3 QUANTITIES					
REINFORCING STEEL				10,433 LBS.	
BARREL				1,600 LBS.	
WING 2				12,033 LBS.	
TOTAL				12,033 LBS.	
CLASS A CONCRETE				69.1 CY	
BARREL @ 1.123 CY/FT				0.4 CY	
SILL				2.2 CY	
END CURTAIN WALL				16.8 CY	
WING 2				88.5 CY	
TOTAL				88.5 CY	

BILL OF MATERIAL PHASE VI - STAGE 4					
BAR	NO.	SIZE	TYPE	LENGTH	WEIGHT
A1	123	#4	1	5'-10"	479
A2	123	#4	1	5'-10"	479
A3	123	#4	1	5'-4"	438
A100	246	#6	STR	26'-2"	9,668
A201	246	#6	STR	11'-6"	4,249
B1	62	#4	STR	11'-2"	462
B2	123	#4	STR	8'-4"	685
C2	116	#4	STR	31'-10"	2,467
D1	7	#6	STR	1'-8"	18
D2	246	#6	STR	4'-6"	1,663
D3	3	#7	STR	4'-10"	30
G1	4	#5	STR	26'-2"	109
S1	3	#7	STR	26'-2"	160
S3	3	#7	STR	11'-6"	71
PHASE VI - STAGE 4 REINFORCING STEEL 20,540 LBS.					
PHASE VI - STAGE 4 QUANTITIES					
REINFORCING STEEL				20,540 LBS.	
BARREL				1,456 LBS.	
WING 1				21,996 LBS.	
TOTAL				21,996 LBS.	
CLASS A CONCRETE				138.6 CY	
BARREL @ 2.254 CY/FT				0.4 CY	
SILL				1.9 CY	
END CURTAIN WALL				16.0 CY	
WING 1				1.2 CY	
HEADWALL				158.1 CY	
TOTAL				158.1 CY	



SECTION THROUGH SILL

* DOWELS MAY BE PUSHED INTO GREEN CONCRETE AFTER SLAB HAS BEEN FLOATED FINISHED.

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ROBESON COUNTY

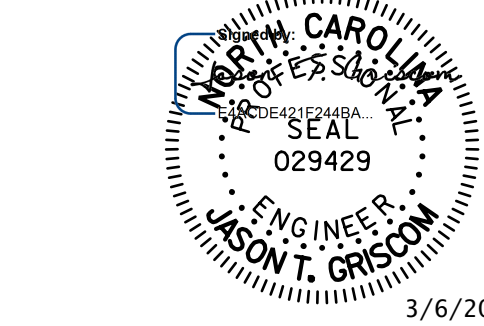
STATION: 43+26.00 -L-

SHEET 6 OF 11

STATE OF NORTH CAROLINA
DEPARTMENT OF TRANSPORTATION
RALEIGH

DOUBLE 12'-0" X 9'-0"
CONCRETE BOX CULVERT
90°-00'-00" SKEW

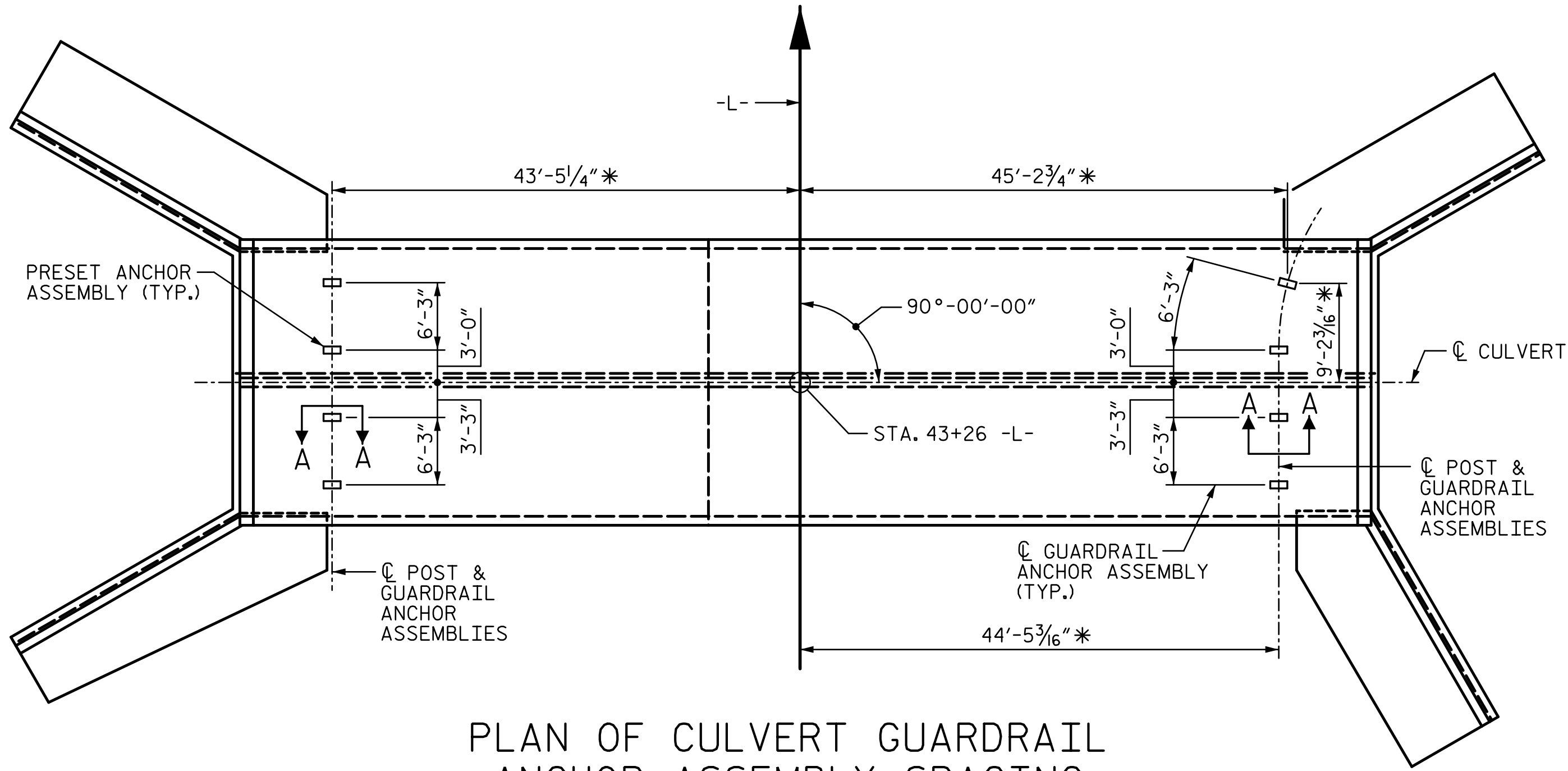
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2			4		
TOTAL SHEETS					11



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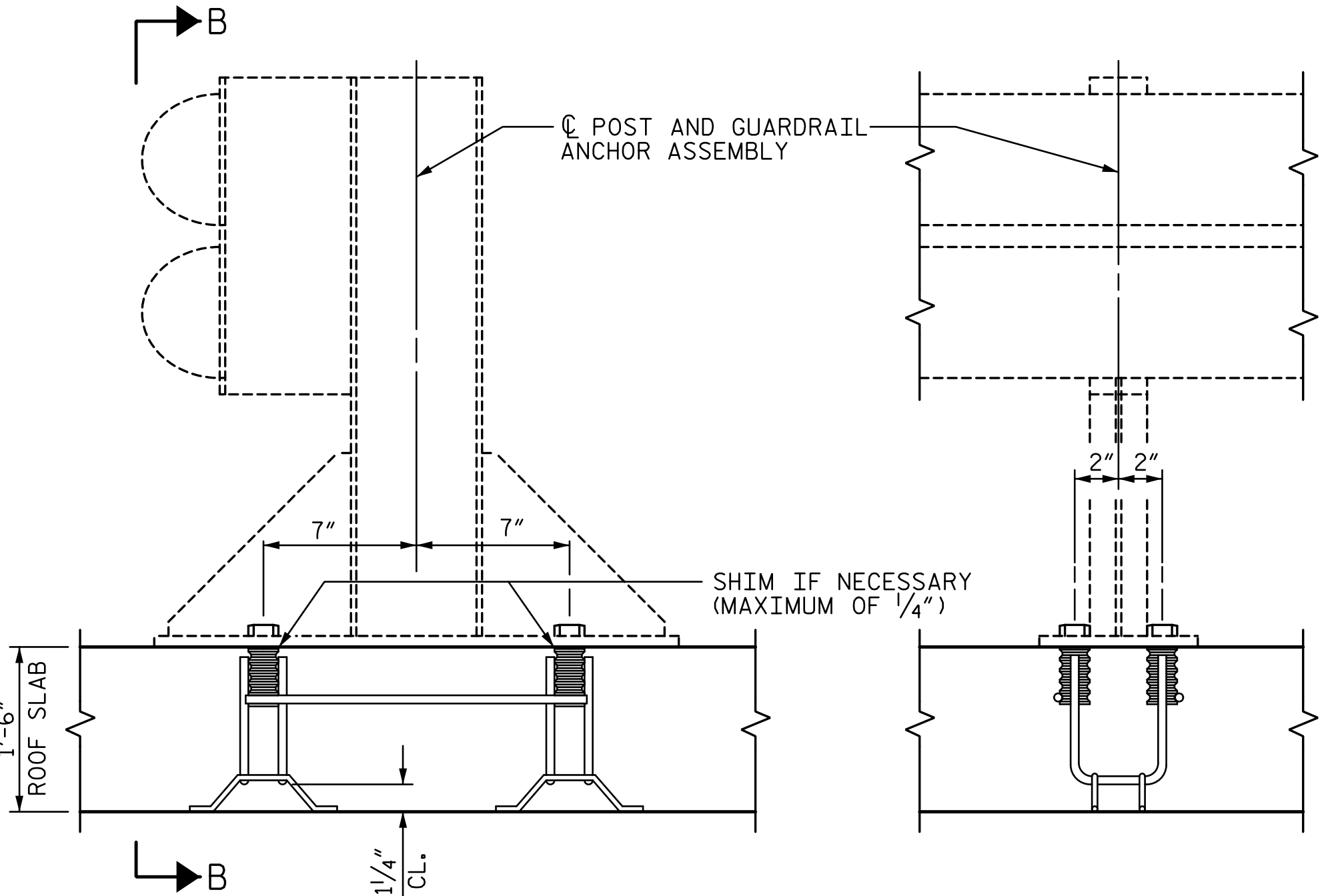
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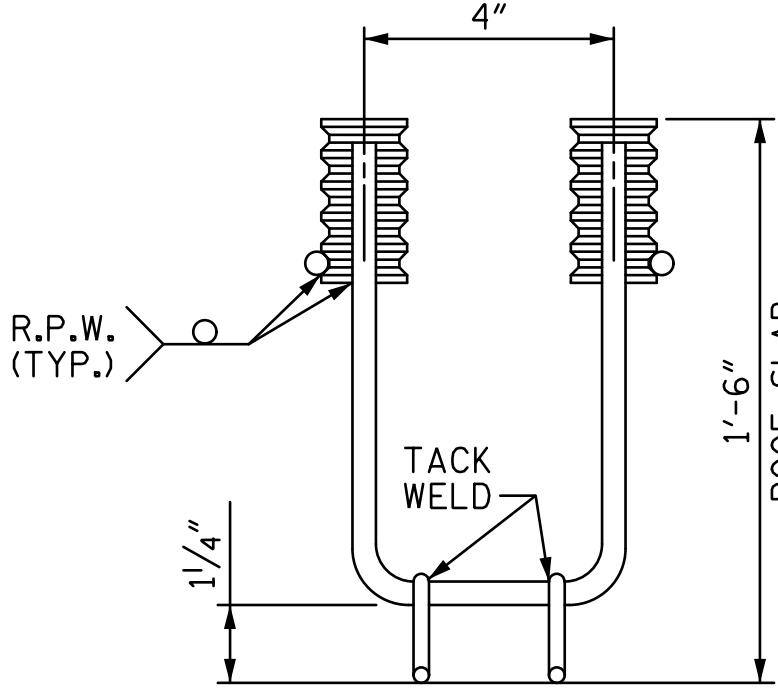
PLAN OF CULVERT GUARDRAIL ANCHOR ASSEMBLY SPACING

* THIS DIMENSION TO BE CONFIRMED BY THE ENGINEER IN THE FIELD.



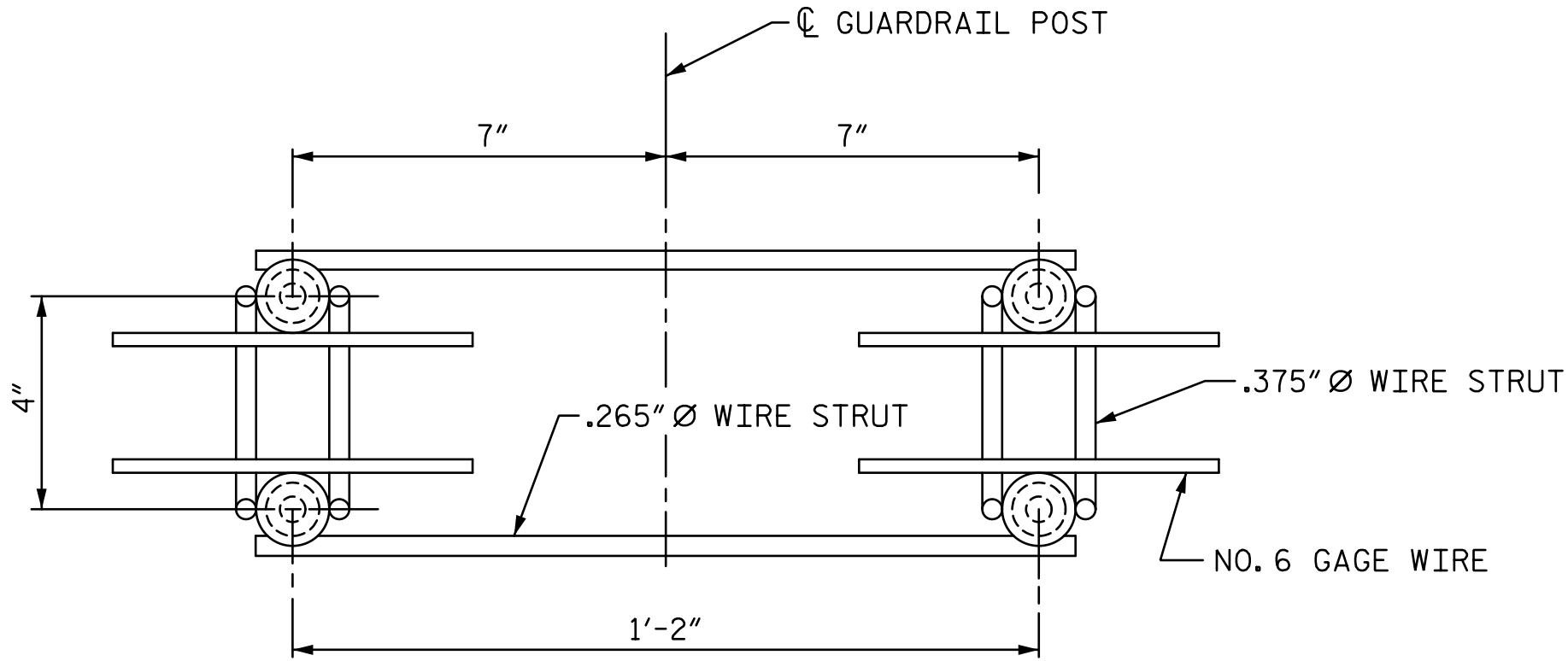
SECTION A-A

SECTION B-B

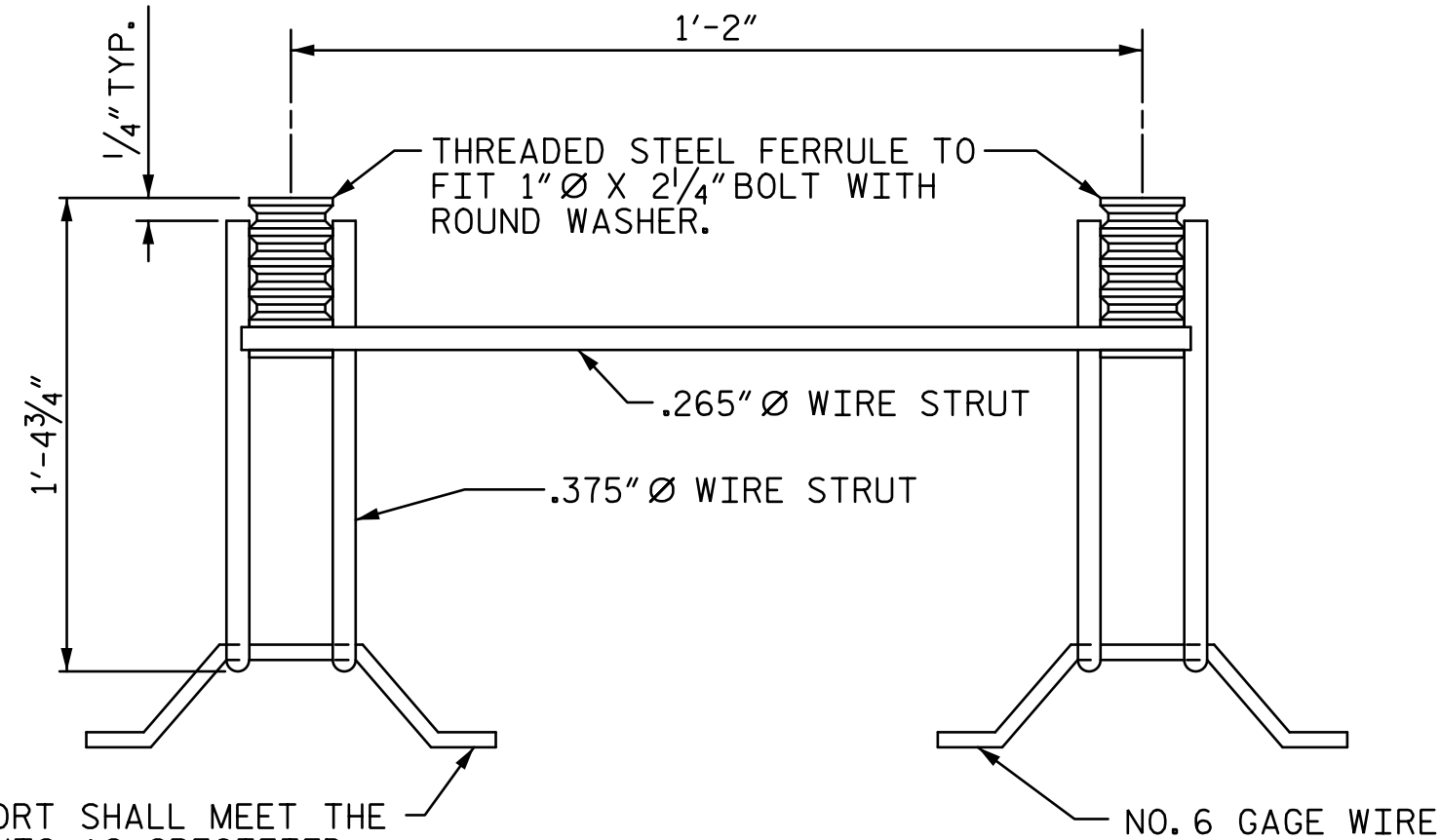


ELEVATION

THIS SUPPORT SHALL MEET THE REQUIREMENTS AS SPECIFIED FOR SUPPORTS FOR REINFORCING STEEL. SEE SPECIFICATIONS.



PLAN



SIDE VIEW

GUARDRAIL ANCHOR ASSEMBLY FOR CULVERTS

NOTES

THE GUARDRAIL ANCHOR ASSEMBLY FOR CULVERTS SHALL CONSIST OF THE FOLLOWING COMPONENTS :

- A. FERRULES SHALL BE MADE FROM STEEL MEETING THE REQUIREMENTS OF AASHTO M169, GRADE 12L14 AND SHALL HAVE A MINIMUM LENGTH OF THREADS OF 2 1/2\"/>
- B. 4 - 1\"/>
- C. WIRE STRUTS SHOWN IN THE GUARDRAIL ANCHOR ASSEMBLY FOR CULVERTS DETAIL ARE MINIMUM ALLOWABLE SIZE AND SHALL HAVE A MINIMUM TENSILE STRENGTH OF 100,000 P.S.I. AS AN OPTION, A 7/16\"/>

GUARDRAIL ANCHOR ASSEMBLY WITH BOLTS SHALL BE ASSEMBLED IN THE SHOP. BOLT THREADS MAY BE RECUT AS NECESSARY TO INSURE FIT.

THE COST OF THE GUARDRAIL ANCHOR ASSEMBLY FOR CULVERTS COMPLETE IN PLACE, SHALL BE INCLUDED IN THE UNIT CONTRACT PRICE BID FOR CLASS "A" CONCRETE.

FERRULES TO BE PLUGGED DURING POURING OF SLAB AS RECOMMENDED BY THE MANUFACTURER.

AT THE CONTRACTOR'S OPTION, FERRULES WITH OPEN OR CLOSED ENDS MAY BE USED.

PAYMENT FOR GUARDRAIL, POSTS, AND POST BASE PLATES IS INCLUDED IN ROADWAY PAY ITEMS.

SLAB REINFORCING STEEL MAY BE SHIFTED AS NECESSARY TO CLEAR GUARDRAIL ANCHOR ASSEMBLY. CARE SHOULD BE TAKEN TO KEEP THE SHIFTING OF REINFORCING STEEL TO A MINIMUM.

THE CONTRACTOR MAY USE ADHESIVELY ANCHORED ANCHOR BOLTS IN PLACE OF GUARDRAIL ANCHOR ASSEMBLY. LEVEL TWO FIELD TESTING IS REQUIRED, AND THE YIELD LOAD OF THE 1\"/>

PROJECT NO. U-5797

ROBESON COUNTY

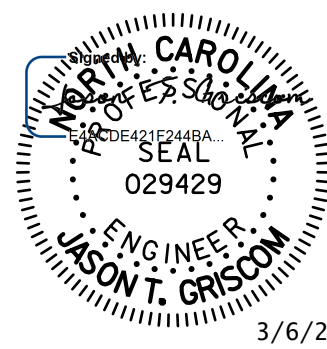
STATION: 43+26.00 -L-

SHEET 7 OF 11

STATE OF NORTH CAROLINA
DEPARTMENT OF TRANSPORTATION
RALEIGH

ANCHORAGE DETAILS FOR
GUARDRAIL ANCHOR ASSEMBLY
FOR CULVERTS

REVISIONS					SHEET NO.
NO.	BY:	DATE:	NO.	BY:	DATE:
1			3		
2			4		
					TOTAL SHEETS
					11



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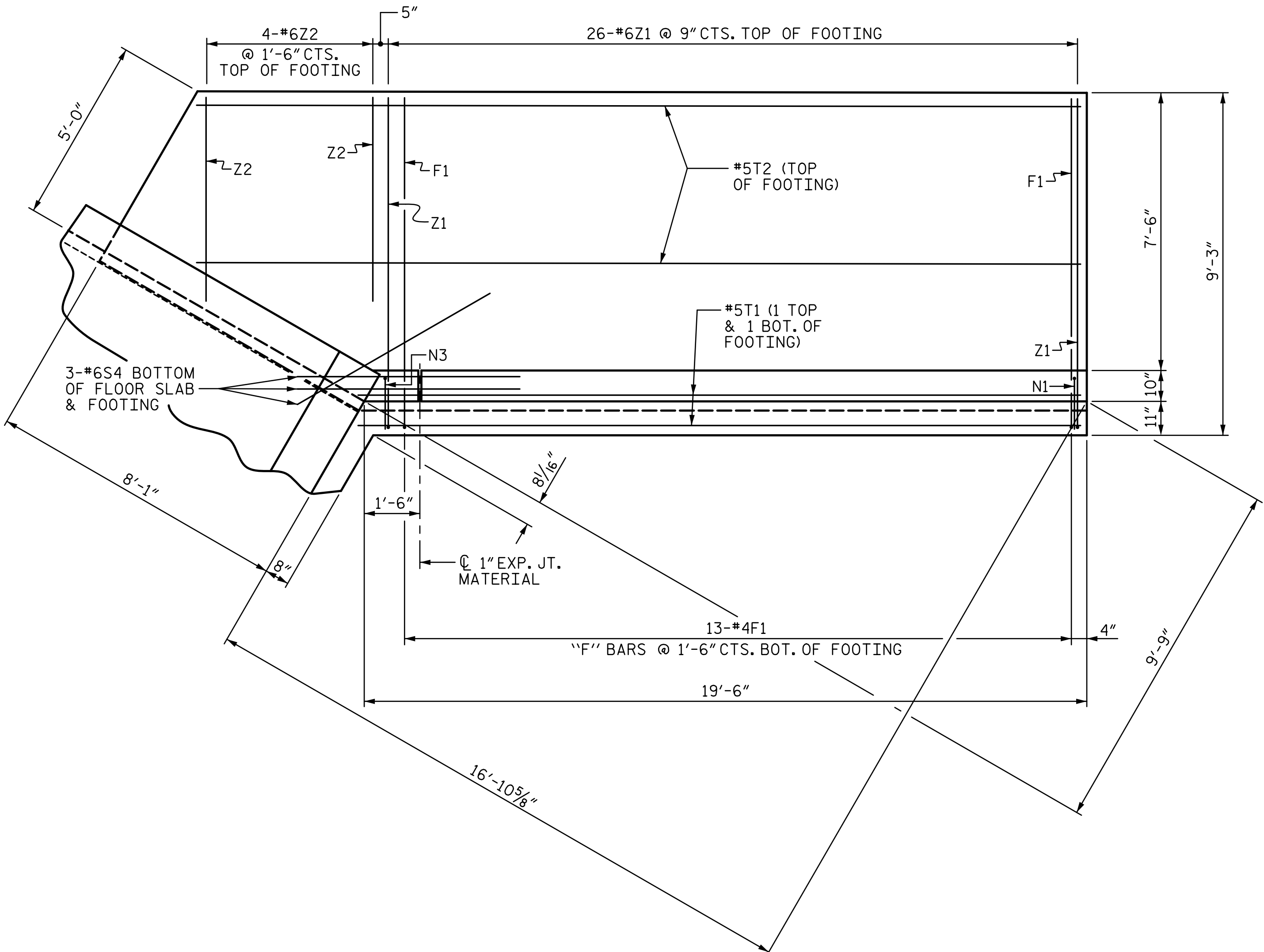
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DESIGN ENGINEER OF RECORD : J. GRISCOM DATE : 11-24

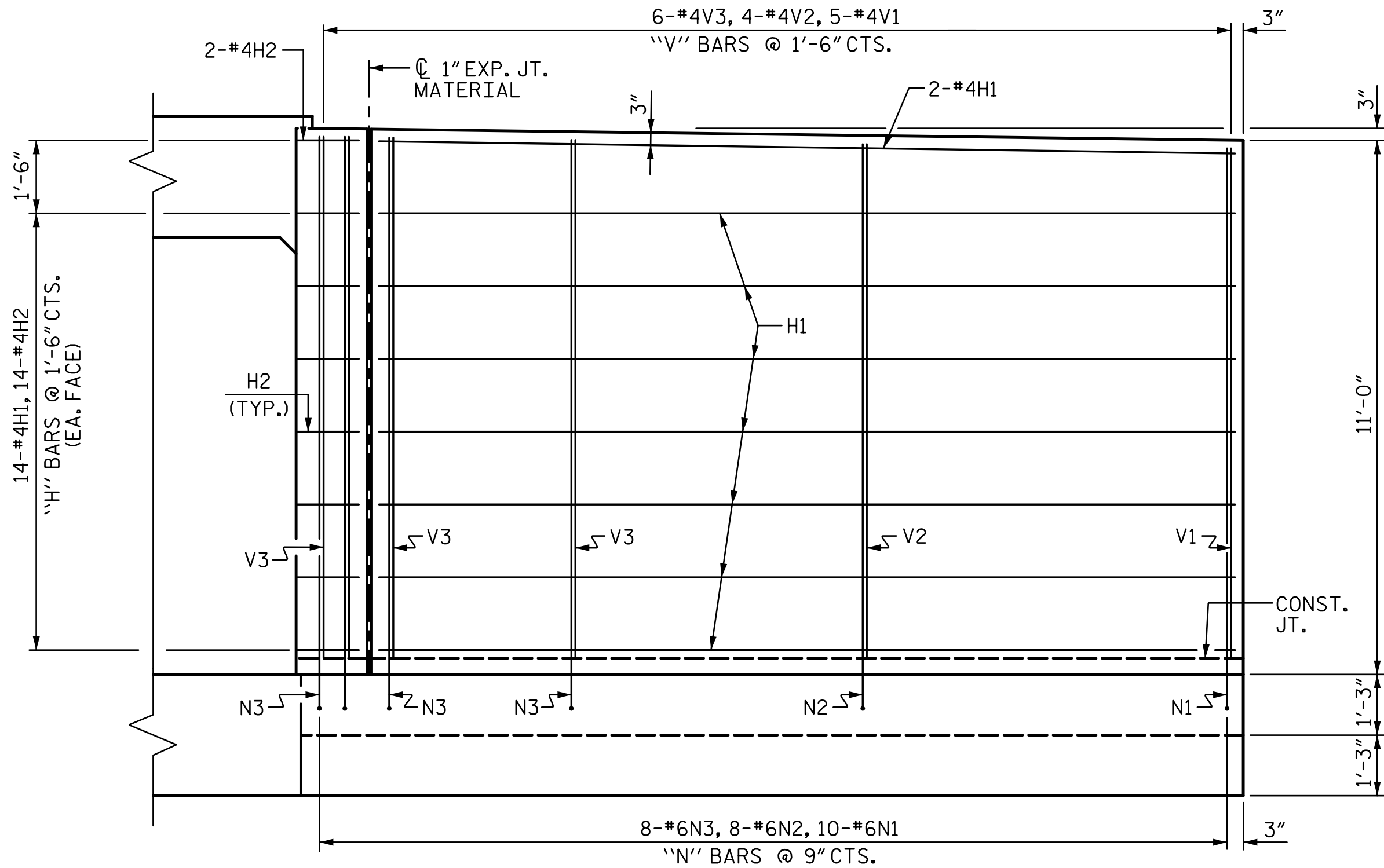
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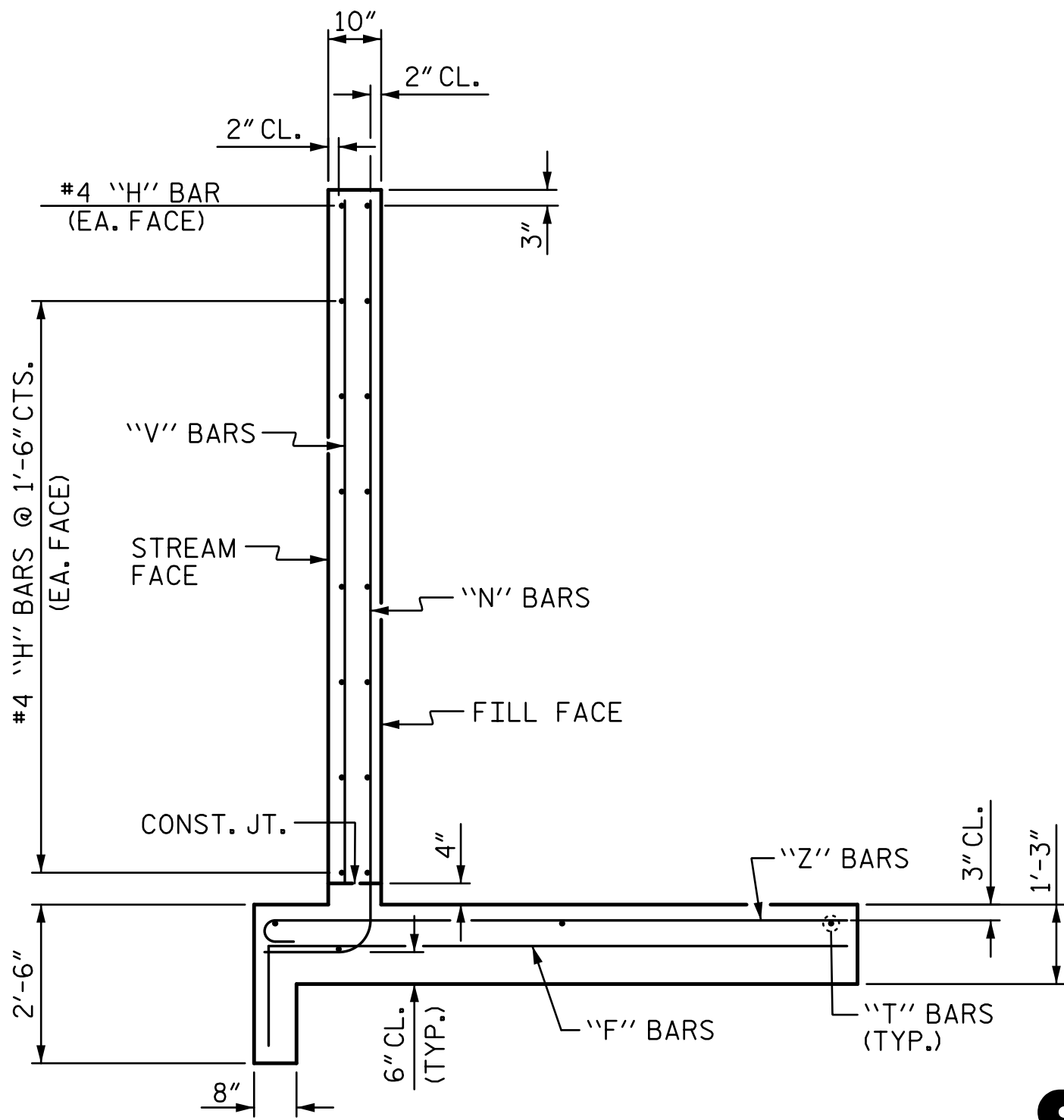
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PLAN

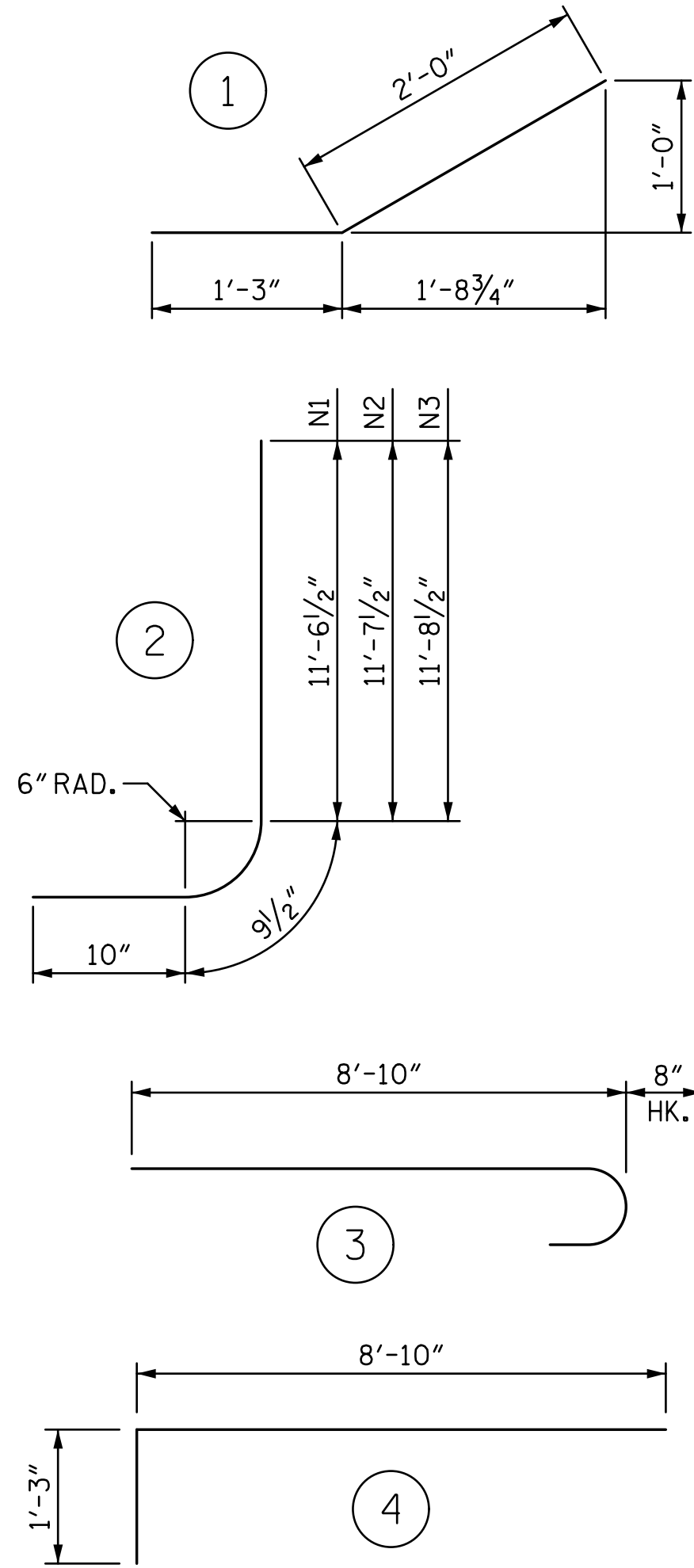


ELEVATION



TYPICAL WING SECTION

BAR TYPES



ALL BAR DIMENSIONS ARE OUT TO OUT.

BILL OF MATERIAL

BAR	NO.	SIZE	TYPE	LENGTH	WEIGHT
F1	13	#4	4	10'-1"	88
H1	16	#4	STR	17'-7"	188
H2	16	#4	1	3'-3"	35
N1	10	#6	2	13'-2"	198
N2	8	#6	2	13'-3"	159
N3	8	#6	2	13'-4"	160
S4	3	#6	STR	6'-0"	27
T1	2	#5	STR	19'-6"	41
T2	2	#5	STR	23'-10"	50
V1	5	#4	STR	10'-6"	35
V2	4	#4	STR	10'-7"	28
V3	6	#4	STR	10'-8"	43
Z1	26	#6	3	9'-6"	371
Z2	4	#6	STR	5'-6"	33
REINFORCING STEEL				1,456 LBS.	

PROJECT NO. U-5797

ROBESON COUNTY

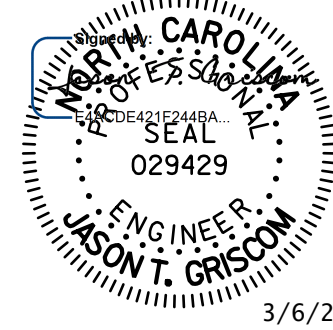
STATION: 43+26.00 -L-

SHEET 8 OF 11

STATE OF NORTH CAROLINA
DEPARTMENT OF TRANSPORTATION
RALEIGH

WING 1 FOR
CONCRETE BOX CULVERT
H = 9'-0" SLOPE = 3:1
90° SKEW

REVISIONS					SHEET NO.
NO.	BY:	DATE:	NO.	BY:	DATE:
1			3		
2			4		
					TOTAL SHEETS 11



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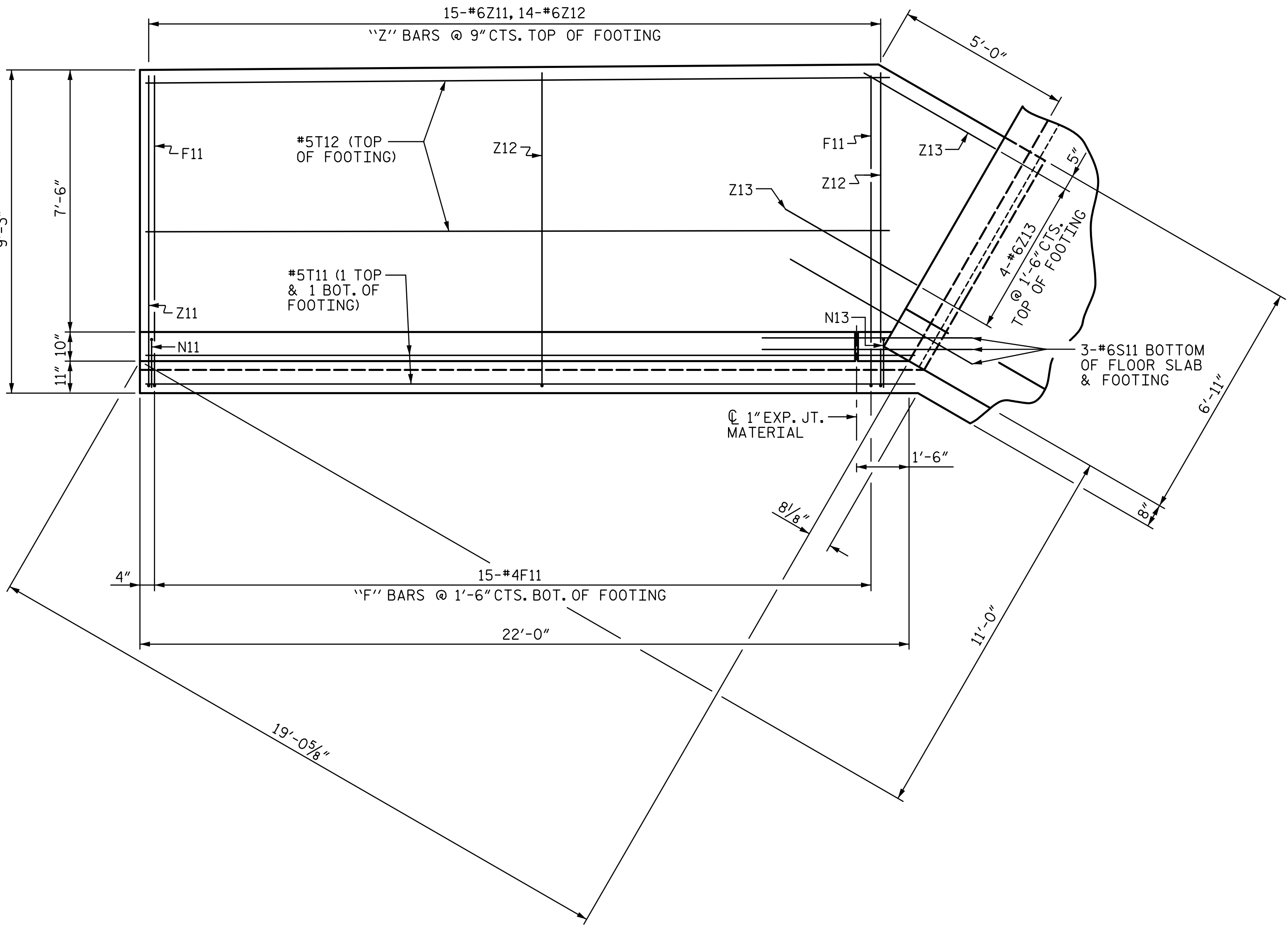
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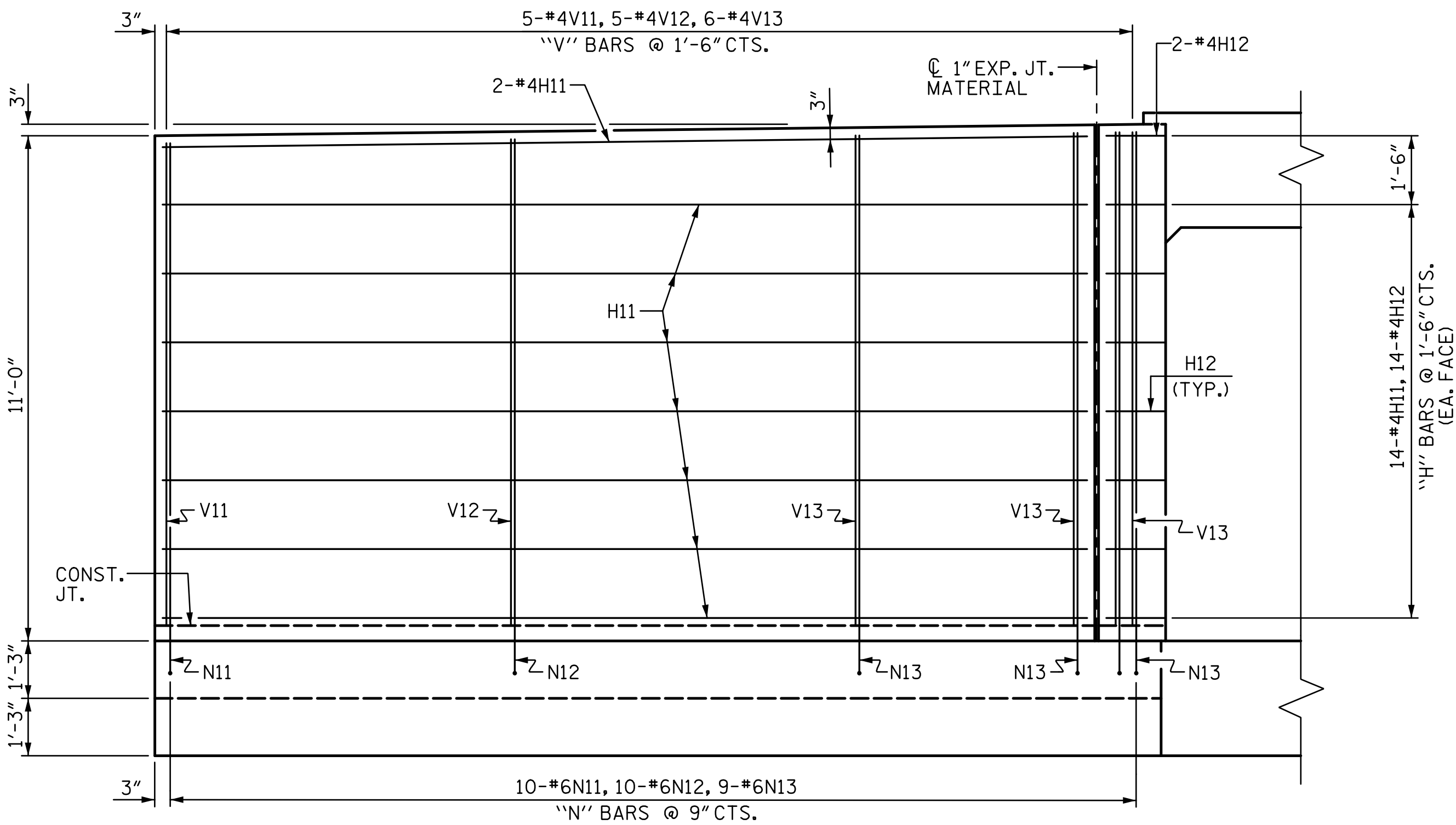
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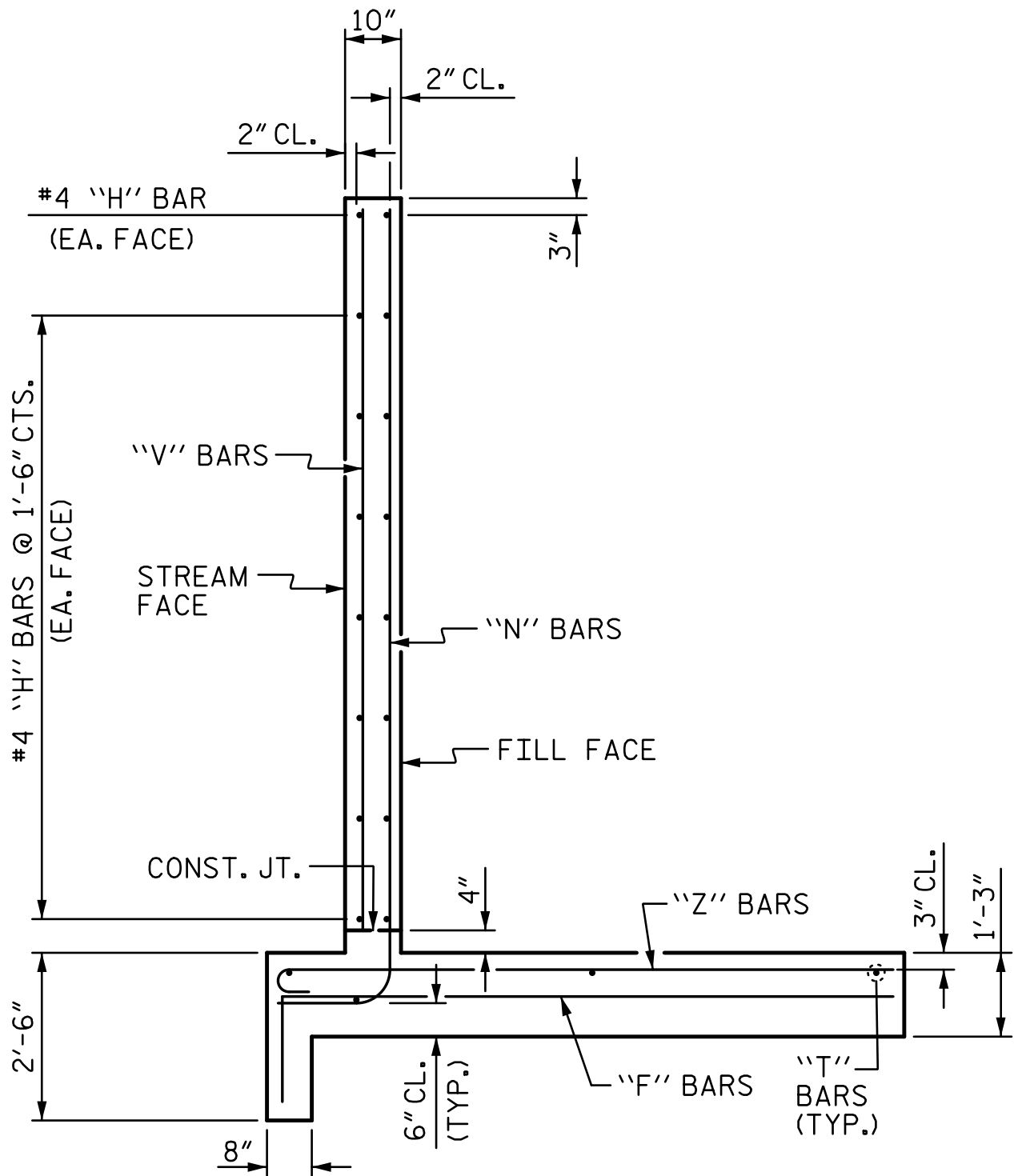
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PLAN

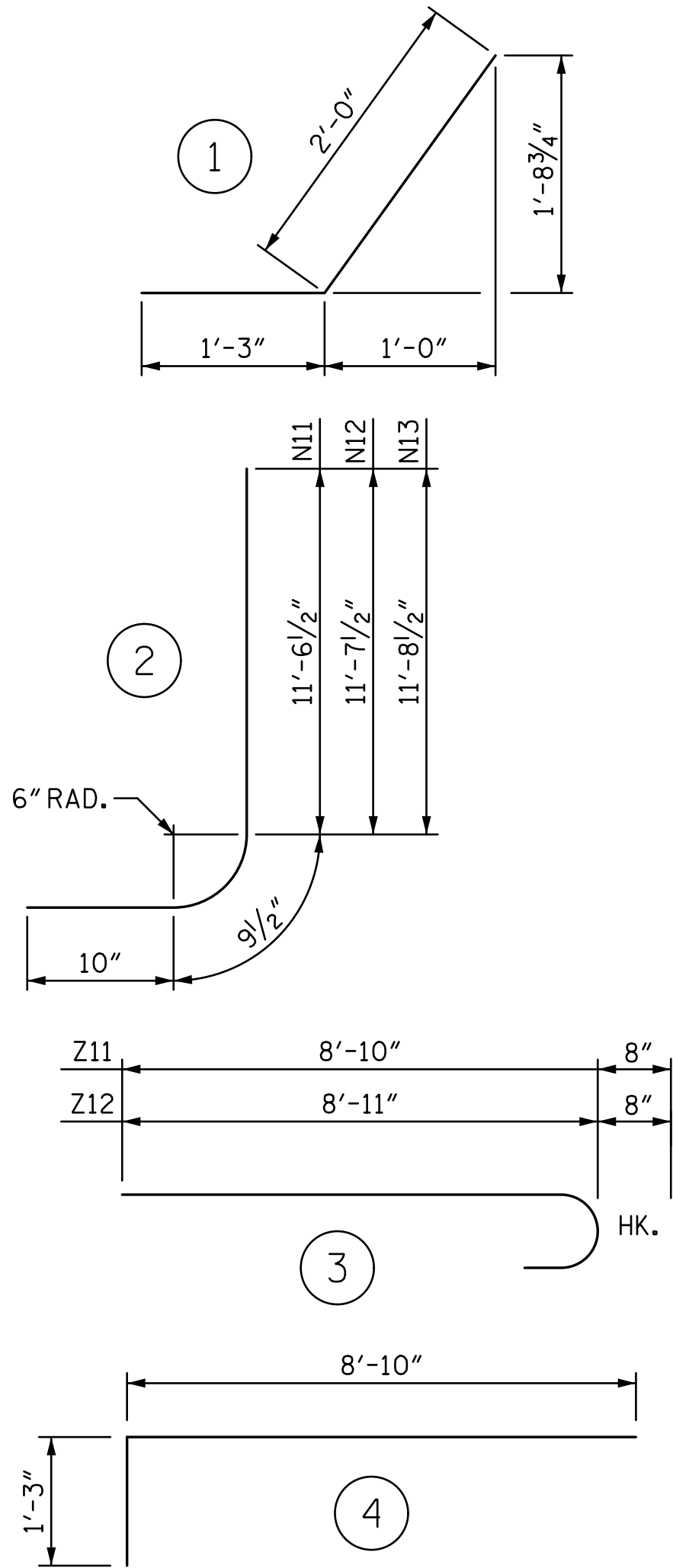


ELEVATION



TYPICAL WING SECTION

BAR TYPES



ALL BAR DIMENSIONS ARE OUT TO OUT.

BILL OF MATERIAL

BAR	NO.	SIZE	TYPE	LENGTH	WEIGHT
F11	15	#4	4	10'-1"	101
H11	16	#4	STR	20'-1"	215
H12	16	#4	1	3'-3"	35
N11	10	#6	2	13'-2"	198
N12	10	#6	2	13'-3"	199
N13	9	#6	2	13'-4"	180
S11	3	#6	STR	6'-0"	27
T11	2	#5	STR	22'-0"	46
T12	2	#5	STR	21'-3"	44
V11	5	#4	STR	10'-6"	35
V12	5	#4	STR	10'-7"	35
V13	6	#4	STR	10'-8"	43
Z11	15	#6	3	9'-6"	214
Z12	14	#6	3	9'-7"	202
Z13	4	#6	STR	4'-4"	26

REINFORCING STEEL 1,600 LBS.

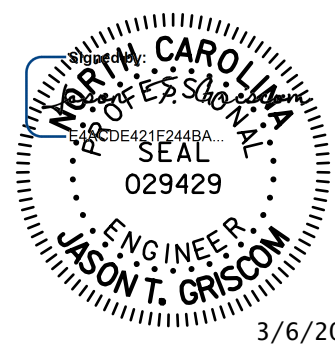
PROJECT NO. U-5797
ROBESON COUNTY
STATION: 43+26.00 -L-

SHEET 9 OF 11

STATE OF NORTH CAROLINA
DEPARTMENT OF TRANSPORTATION
RALEIGH

WING 2 FOR
CONCRETE BOX CULVERT
H = 9'-0" SLOPE = 3:1
90° SKEW

REVISIONS					SHEET NO.
NO.	BY:	DATE:	NO.	BY:	DATE:
1			3		
2			4		
					TOTAL SHEETS 11



3/6/2025



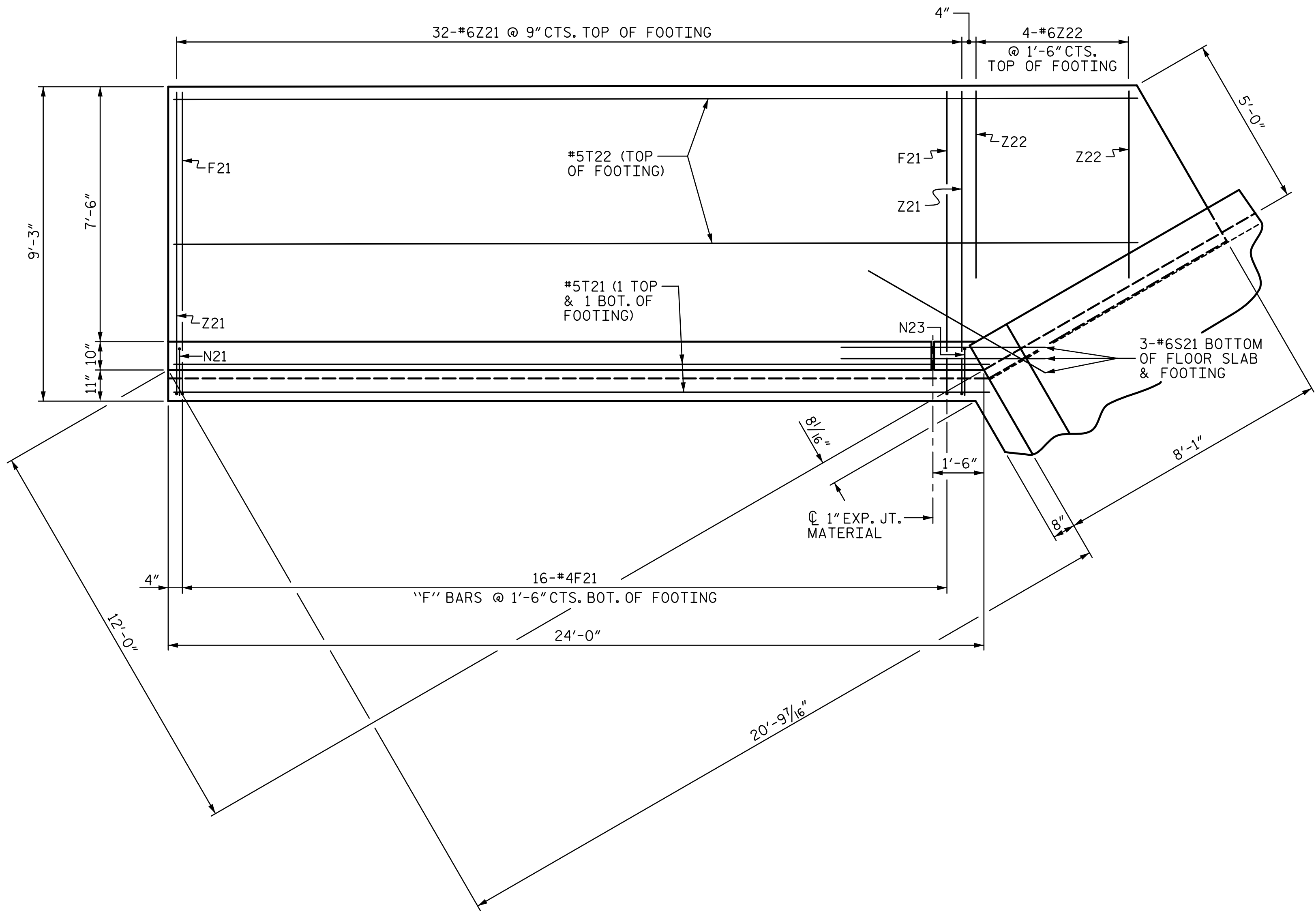
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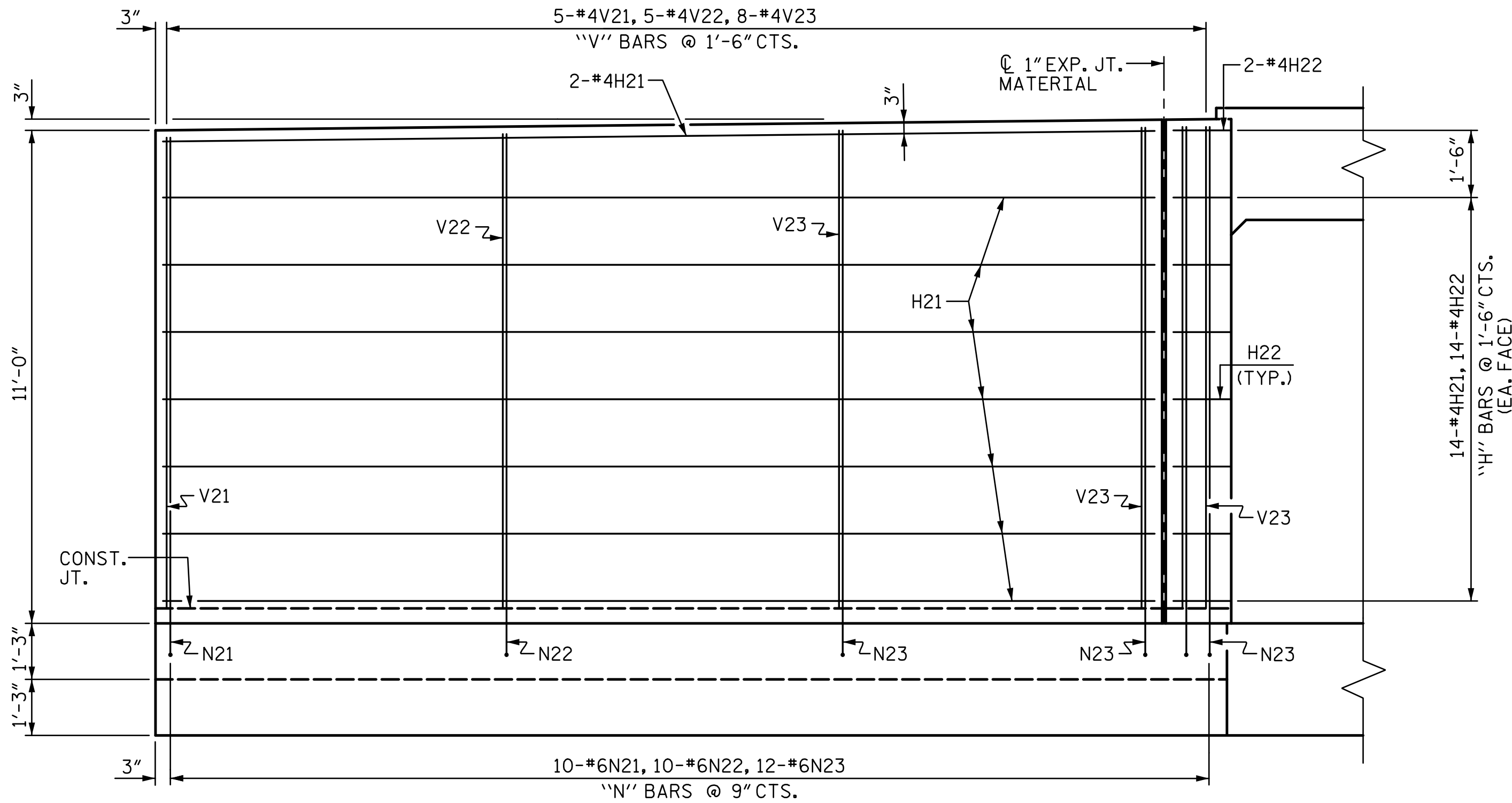
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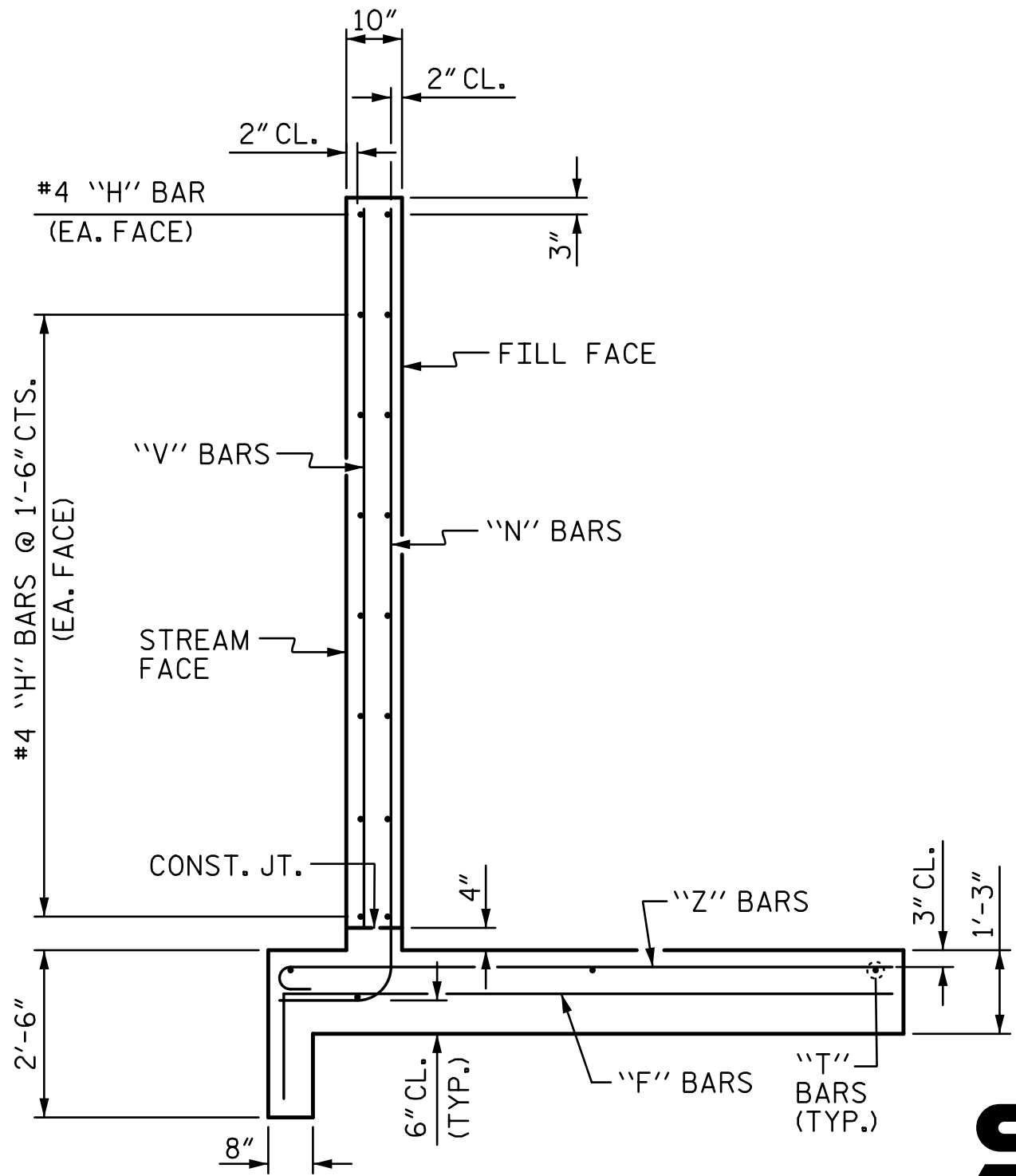
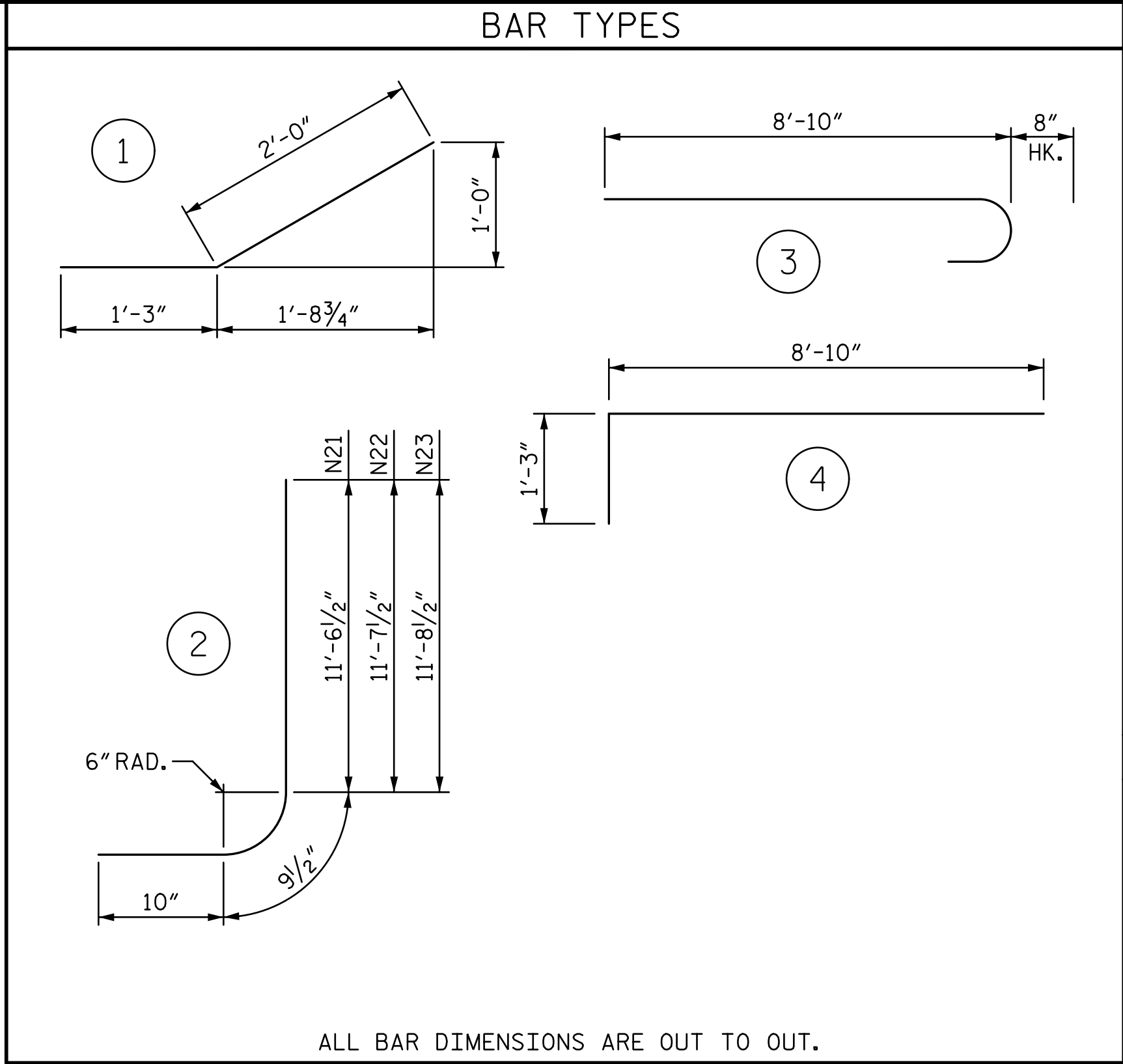
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PLAN



ELEVATION



TYPICAL WING SECTION

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SIGNATURES COMPLETED

BILL OF MATERIAL					
BAR	NO.	SIZE	TYPE	LENGTH	WEIGHT
F21	16	#4	4	10'-1"	108
H21	16	#4	STR	22'-1"	236
H22	16	#4	1	3'-3"	35
N21	10	#6	2	13'-2"	198
N22	10	#6	2	13'-3"	199
N23	12	#6	2	13'-4"	240
S21	3	#6	STR	6'-0"	27
T21	2	#5	STR	24'-0"	50
T22	2	#5	STR	28'-4"	59
V21	5	#4	STR	10'-6"	35
V22	5	#4	STR	10'-7"	35
V23	8	#4	STR	10'-8"	57
Z21	32	#6	3	9'-6"	457
Z22	4	#6	STR	5'-6"	33
REINFORCING STEEL					1,769 LBS.

PROJECT NO. U-5797

ROBESON COUNTY

STATION: 43+26.00 -L-

SHEET 10 OF 11

STATE OF NORTH CAROLINA
DEPARTMENT OF TRANSPORTATION
RALEIGH

WING 3 FOR
CONCRETE BOX CULVERT

H = 9'-0" SLOPE = 3:1
90° SKEW

REVISIONS					SHEET NO. C2-10 TOTAL SHEETS 11
NO.	BY:	DATE:	NO.	BY:	
1			3		
2			4		

PLAN

ELEVATION

TYPICAL WING SECTION

PROJECT NO. _____	U-5797
ROBESON COUNTY	
STATION: 43+26.00 -L-	
SHEET 11 OF 11	

STATE OF NORTH CAROLINA

DEPARTMENT OF TRANSPORTATION

RALEIGH

WING 4 FOR

CONCRETE BOX CULVERT

H = 9'-0"
SLOPE = 3:1

90° SKEW

REVISIONS						SHEET NO. C2-11
NO.	BY:	DATE:	NO.	BY:	DATE:	TOTAL SHEETS 11
1			3			
2			4			

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SIGNATURES COMPLETED

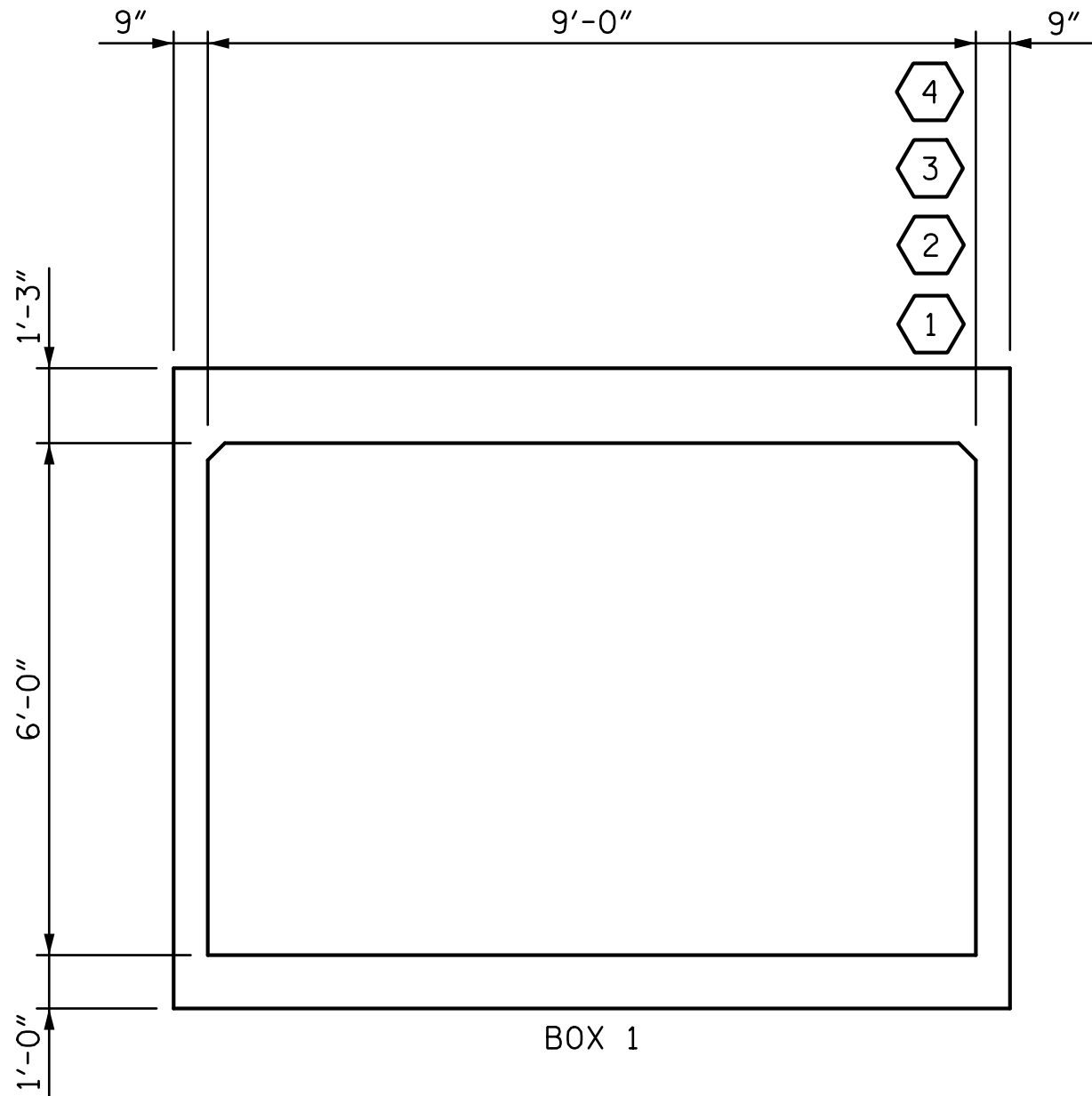
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SIGNATURES COMPLETED

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hensleg

LOAD AND RESISTANCE FACTOR RATING (LRFR) SUMMARY FOR REINFORCED CONCRETE BOX CULVERTS															
LOAD TYPE	VEHICLE	WEIGHT (W) (TONS)	CONTROLLING LOAD RATING ⬡#	MINIMUM RATING FACTORS (RF)	TONS = W × RF	STRENGTH I LIMIT STATE									COMMENT NUMBER
						LIVE-LOAD FACTORS (γ _{LL})	MOMENT				SHEAR				
							RATING FACTOR	BOX NO.	ELEMENT TYPE	DISTANCE FROM LEFT END OF ELEMENT (ft)	RATING FACTOR	BOX NO.	ELEMENT TYPE	DISTANCE FROM LEFT END OF ELEMENT (ft)	
DESIGN LOAD	HL-93 (INVENTORY)	N/A	⬡1	1.19	--	1.75	2.13	1	TOP SLAB	5.25'	1.19	1	TOP SLAB	9.75'	
	HL-93 (OPERATING)	N/A		1.55	--	1.35	2.76	1	TOP SLAB	5.25'	1.55	1	TOP SLAB	9.75'	
	HS-20 (INVENTORY)	36.000	⬡2	1.51	54.360	1.75	2.28	1	TOP SLAB	5.25'	1.51	1	TOP SLAB	9.75'	
	HS-20 (OPERATING)	36.000		1.96	70.560	1.35	2.95	1	TOP SLAB	5.25'	1.96	1	TOP SLAB	9.75'	
LEGAL LOAD	SINGLE VEHICLE (SV)	SNSH		3.06	41.310	1.40	4.15	1	TOP SLAB	5.25'	3.06	1	TOP SLAB	9.75'	
		SNGARBS2		2.79	55.800	1.40	3.88	1	TOP SLAB	5.25'	2.79	1	TOP SLAB	9.75'	
		SNAGRIS2		3.06	67.320	1.40	4.15	1	TOP SLAB	5.25'	3.06	1	TOP SLAB	9.75'	
		SNCOTTS3	⬡3	1.57	42.783	1.40	2.85	1	TOP SLAB	5.25'	1.57	1	TOP SLAB	9.75'	
		SNAGGRS4		1.97	68.802	1.40	3.33	1	EXTERIOR WALL	1.00'	1.97	1	TOP SLAB	9.75'	
		SNS5A		1.83	65.057	1.40	3.31	1	TOP SLAB	5.25'	1.83	1	TOP SLAB	9.75'	
		SNS6A		1.83	73.109	1.40	3.30	1	TOP SLAB	5.25'	1.83	1	TOP SLAB	9.75'	
		SNS7B		1.83	76.860	1.40	3.30	1	TOP SLAB	5.25'	1.83	1	TOP SLAB	9.75'	
	TRUCK TRACTOR SEMI-TRAILER (TTST)	TNAGRIT3		3.00	99.000	1.40	4.14	1	TOP SLAB	5.25'	3.00	1	TOP SLAB	9.75'	
		TNT4A		1.87	61.850	1.40	3.39	1	TOP SLAB	5.25'	1.87	1	TOP SLAB	9.75'	
		TNT6A		1.83	76.128	1.40	3.31	1	TOP SLAB	5.25'	1.83	1	TOP SLAB	9.75'	
		TNT7A		1.85	77.700	1.40	3.38	1	TOP SLAB	5.25'	1.85	1	TOP SLAB	9.75'	
		TNT7B		1.83	76.860	1.40	3.31	1	TOP SLAB	5.25'	1.83	1	TOP SLAB	9.75'	
		TNAGRIT4		1.87	80.410	1.40	3.39	1	TOP SLAB	5.25'	1.87	1	TOP SLAB	9.75'	
		TNAGT5A		1.87	84.150	1.40	3.39	1	TOP SLAB	5.25'	1.87	1	TOP SLAB	9.75'	
		TNAGT5B		1.87	84.150	1.40	3.39	1	TOP SLAB	5.25'	1.87	1	TOP SLAB	9.75'	
EMERGENCY VEHICLE (EV)	EV2		1.94	55.775	1.30	2.93	1	TOP SLAB	5.25'	1.94	1	TOP SLAB	9.75'		
	EV3	⬡4	1.36	58.480	1.30	2.47	1	TOP SLAB	5.25'	1.36	1	TOP SLAB	9.75'		



LRFR SUMMARY
(LOOKING DOWNSTREAM)

DRAWN BY :	SGH	DATE :	11-24
CHECKED BY :	JAD	DATE :	11-24
DESIGN ENGINEER OF RECORD :	J. GRISCOM	DATE :	11-24

LOAD FACTORS:

DESIGN LOAD RATING FACTORS

LOAD TYPE	MAX FACTOR	MIN FACTOR
DC	1.25	0.90
DW	1.50	0.65
EV	1.30	0.90
EH	1.35	0.90
ES	1.35	0.90
LS	1.75	--
WA	1.00	--

NOTE:

RATING FACTORS ARE BASED ON THE STRENGTH I LIMIT STATE.

FOR WALL ELEMENTS, THE LEFT END OF ELEMENT IS LOCATED AT THE BOTTOM OF THE BOTTOM SLAB.

COMMENTS:

⬡# CONTROLLING LOAD RATING

⬡1 DESIGN LOAD RATING (HL-93)

⬡2 DESIGN LOAD RATING (HS-20)

⬡3 LEGAL LOAD RATING **

⬡4 EMERGENCY VEHICLE LAOD RATING **

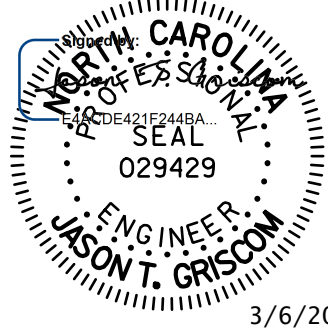
** SEE CHART FOR VEHICLE TYPE

PROJECT NO. U-5797

ROBESON COUNTY

STATION: 68+62.00 -L-

SHEET 2 OF 15



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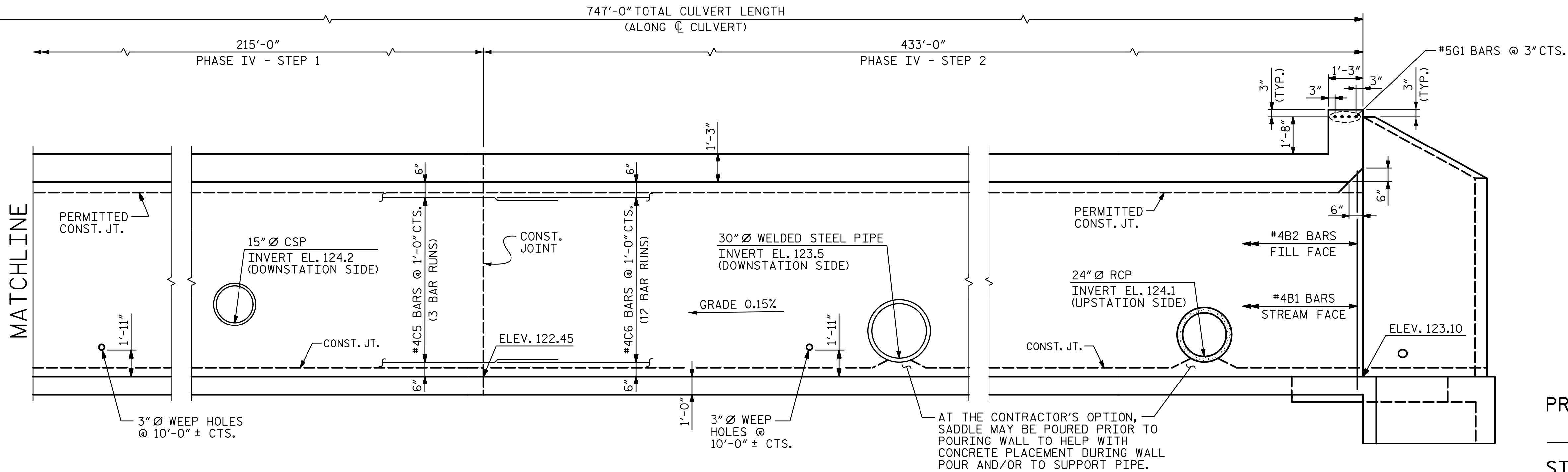
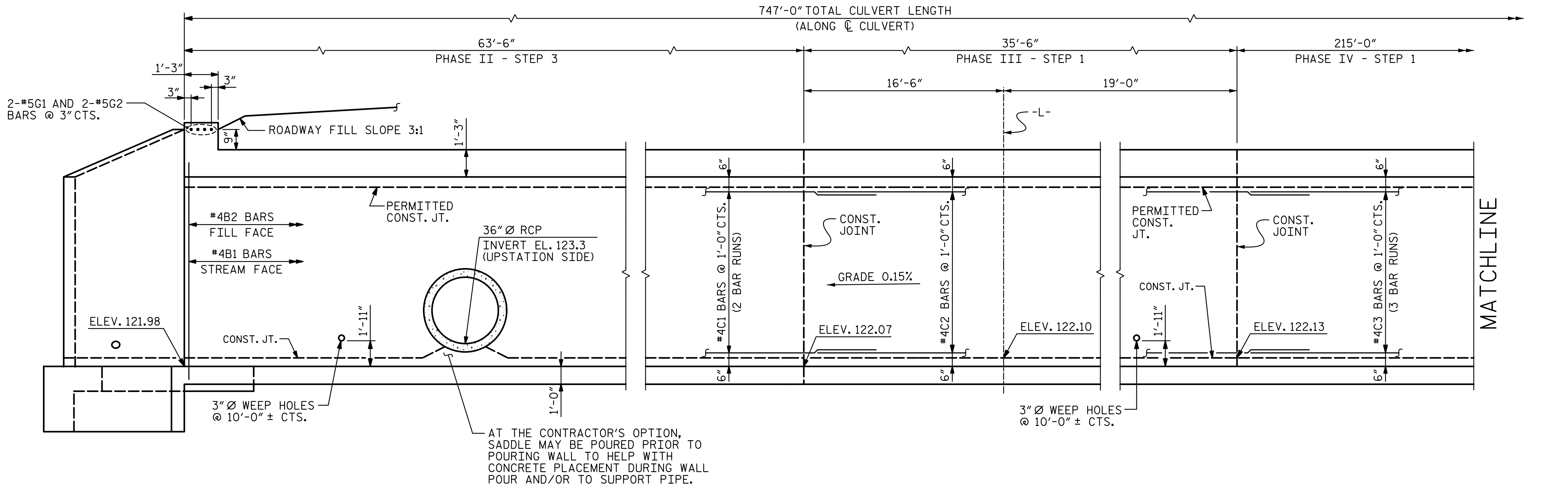
STATE OF NORTH CAROLINA
DEPARTMENT OF TRANSPORTATION
RALEIGH
STANDARD
LRFR SUMMARY FOR
REINFORCED CONCRETE
BOX CULVERTS
(NON-INTERSTATE TRAFFIC)

REVISIONS					SHEET NO.
NO.	BY:	DATE:	NO.	BY:	DATE:
1			3		
2			4		
					C3-2
					TOTAL SHEETS 15

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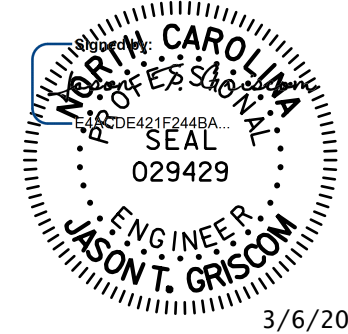
henslesg



CULVERT SECTION NORMAL TO -L-

FOR APPROXIMATE PLAN VIEW LOCATION OF PIPES, SEE LOCATION SKETCH ON SHEET 1 OF 15

DRAWN BY : SGH DATE : 11-24
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DESIGN ENGINEER OF RECORD : J. GRISCOM DATE : 11-24



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ROBESON COUNTY

STATION: 68+62.00 -L-

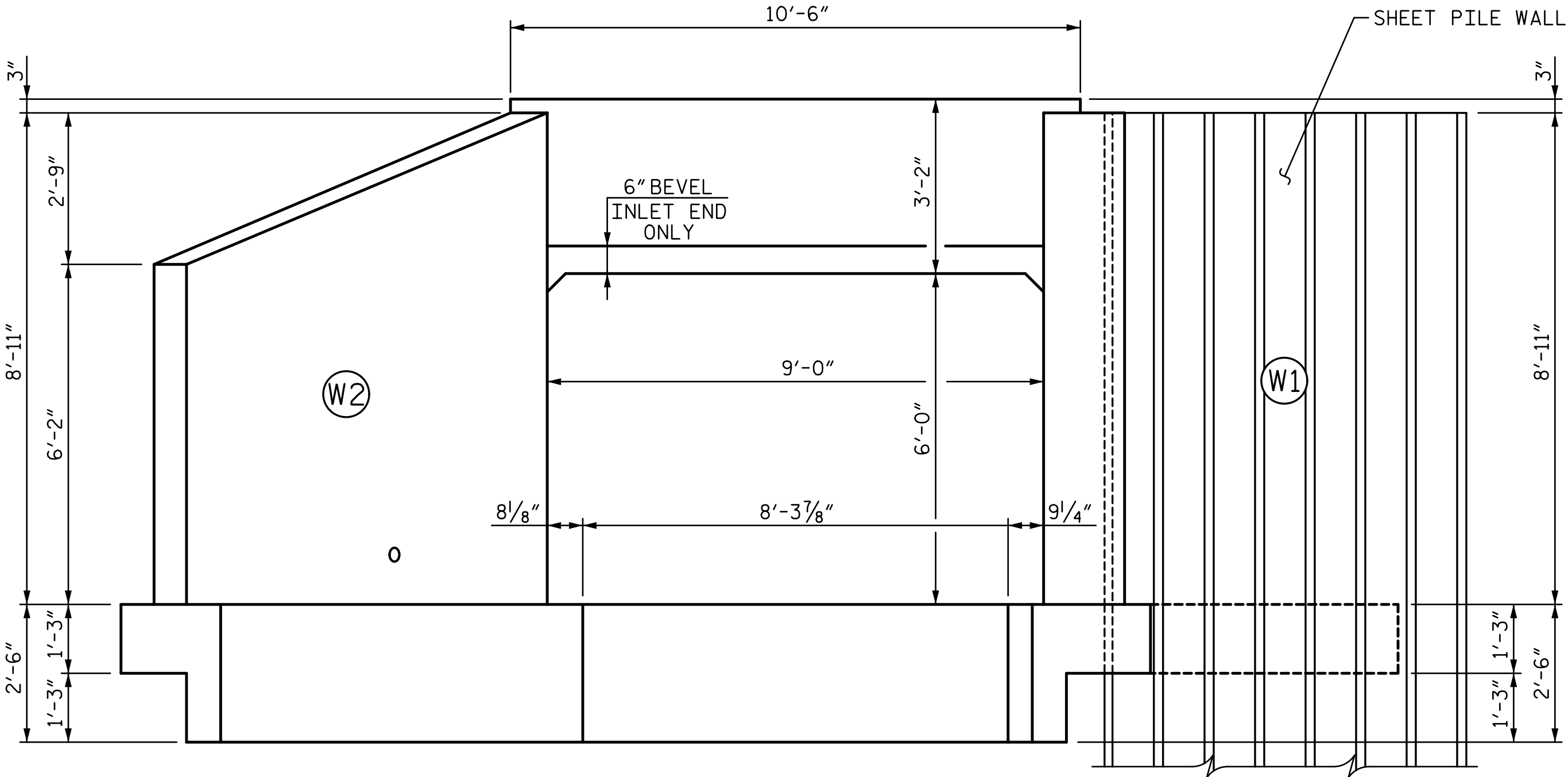
SHEET 3 OF 15

STATE OF NORTH CAROLINA
DEPARTMENT OF TRANSPORTATION
RALEIGH

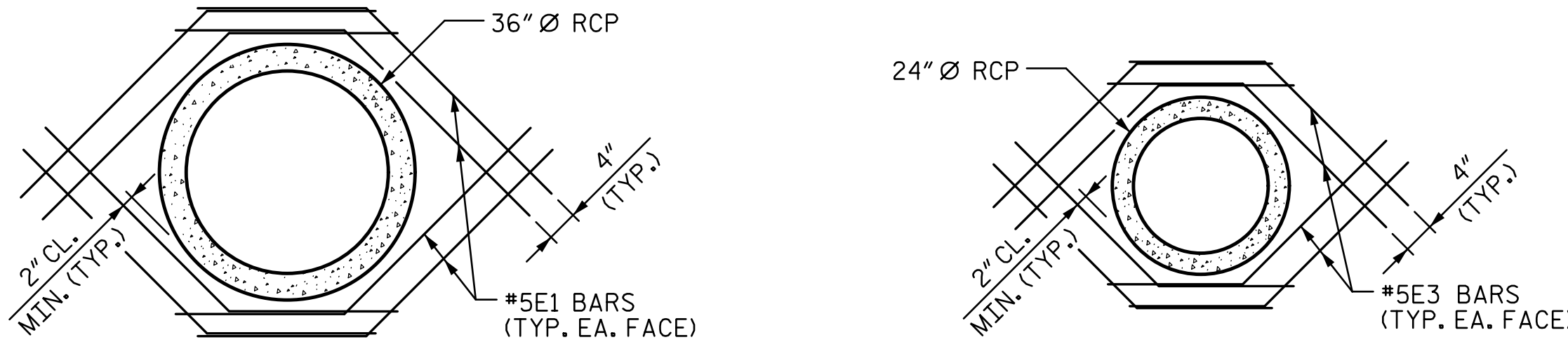
SINGLE 9'-0" X 6'-0"
CONCRETE BOX CULVERT
102°-30'-00" SKEW

REVISIONS					SHEET NO.
NO.	BY:	DATE:	NO.	BY:	DATE:
1			3		
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					TOTAL SHEETS
					15
					C3-3

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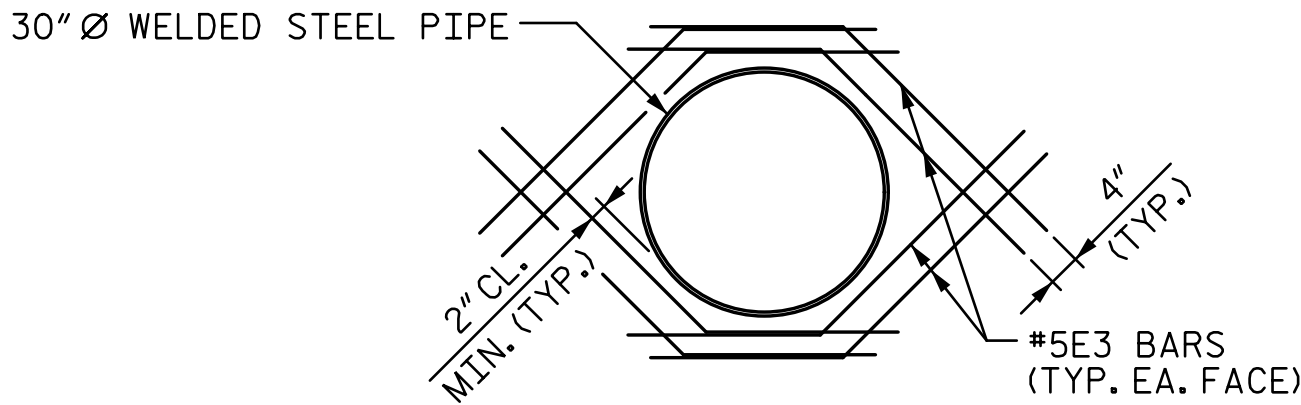


INLET END ELEVATION NORMAL TO SKEW



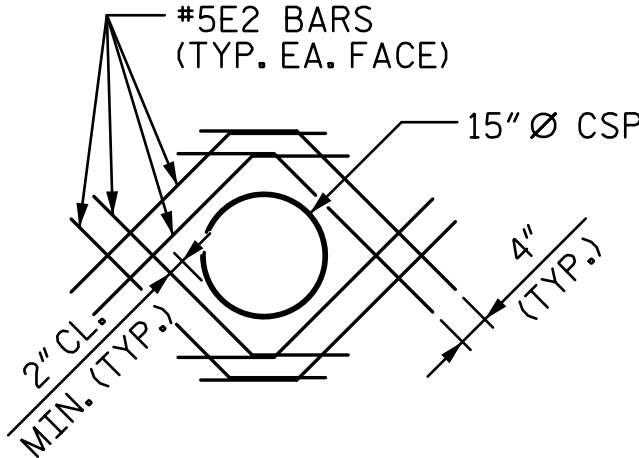
DETAIL OF REINFORCING AROUND RCP

"E" BARS MAY BE SHIFTED, AS NECESSARY TO MAINTAIN 2" MINIMUM CLEARANCE TO PIPE. PLACE "E" BARS BELOW THE PIPE WITH FLOOR SLAB REINFORCING IF NECESSARY.



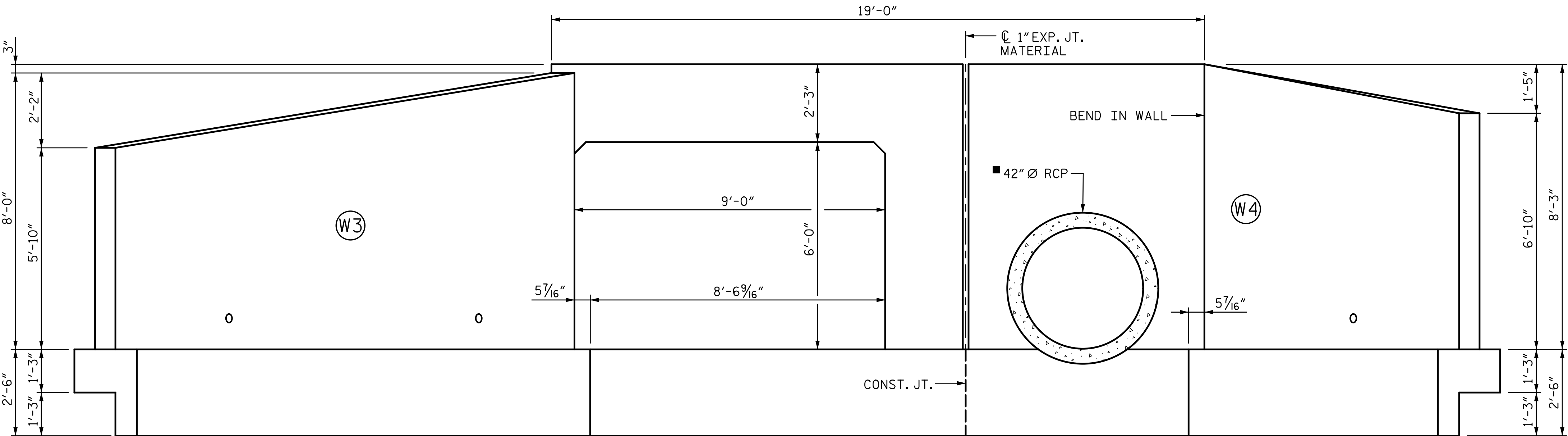
DETAIL OF REINFORCING AROUND WELDED STEEL PIPE

E3 BARS MAY BE SHIFTED, AS NECESSARY TO MAINTAIN 2" MINIMUM CLEARANCE TO PIPE. PLACE E3 BARS BELOW THE PIPE WITH FLOOR SLAB REINFORCING IF NECESSARY.



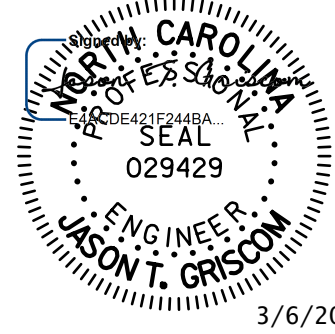
DETAIL OF REINFORCING AROUND CSP

E2 BARS MAY BE SHIFTED, AS NECESSARY TO MAINTAIN 2" MINIMUM CLEARANCE



OUTLET END ELEVATION NORMAL TO SKEW

■ FOR DETAIL OF REINFORCING AROUND 42" Ø RCP
SEE "WING 4 FOR CONCRETE BOX CULVERT" SHEET.



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ROBESON COUNTY

STATION: 68+62.00 -L-

SHEET 4 OF 15

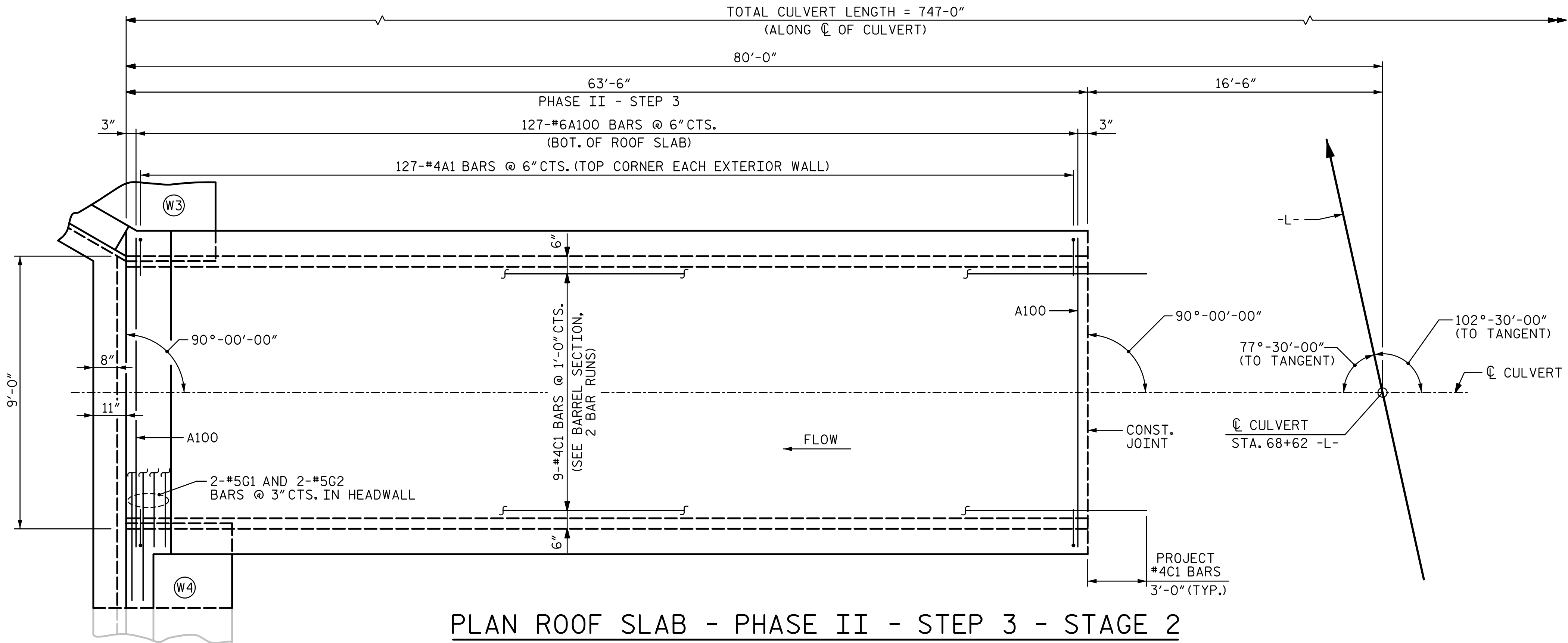
STATE OF NORTH CAROLINA
DEPARTMENT OF TRANSPORTATION
RALEIGH

DOUBLE 12'-0" X 9'-0"
CONCRETE BOX CULVERT
102°-30'-00" SKEW

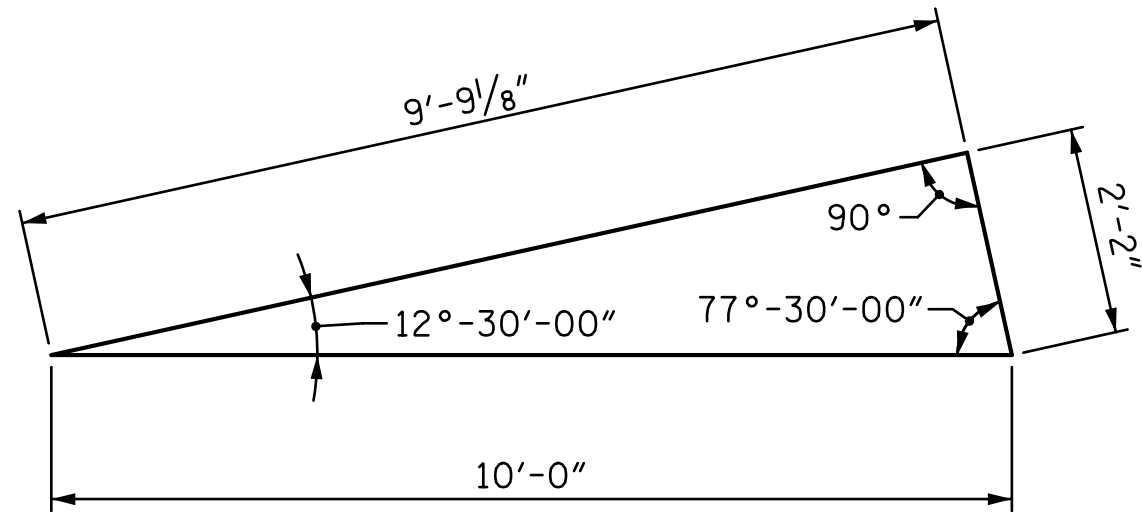
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CHECKED BY : JAD DATE : 11-24
DESIGN ENGINEER OF RECORD : J. GRISCOM DATE : 11-24

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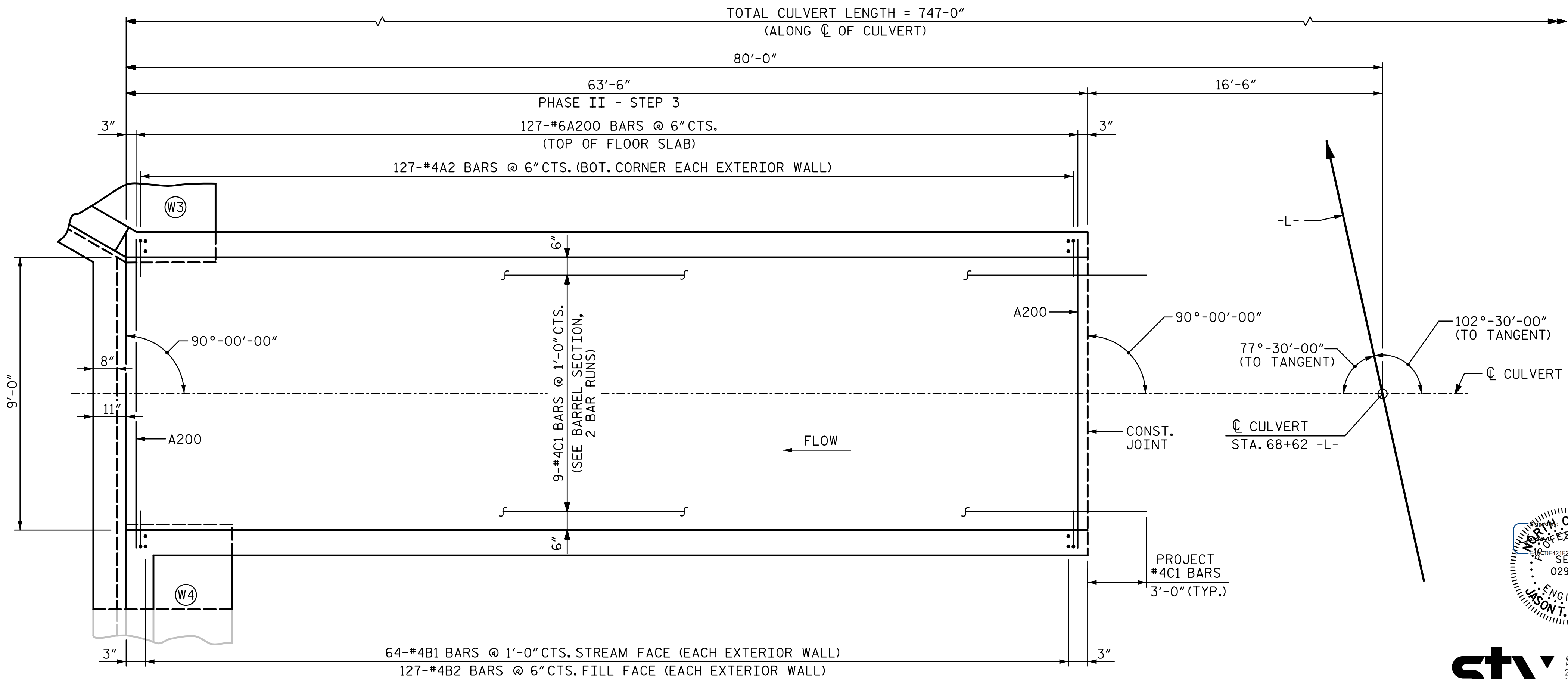
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PLAN ROOF SLAB - PHASE II - STEP 3 - STAGE 2



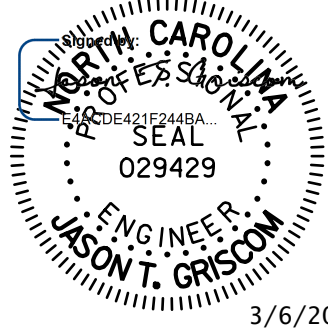
SKEW TRIANGLE



PLAN FLOOR SLAB - PHASE II - STEP 3 - STAGE 2

PROJECT NO. U-5797
ROBESON COUNTY
STATION: 68+62.00 -L-

SHEET 5 OF 15



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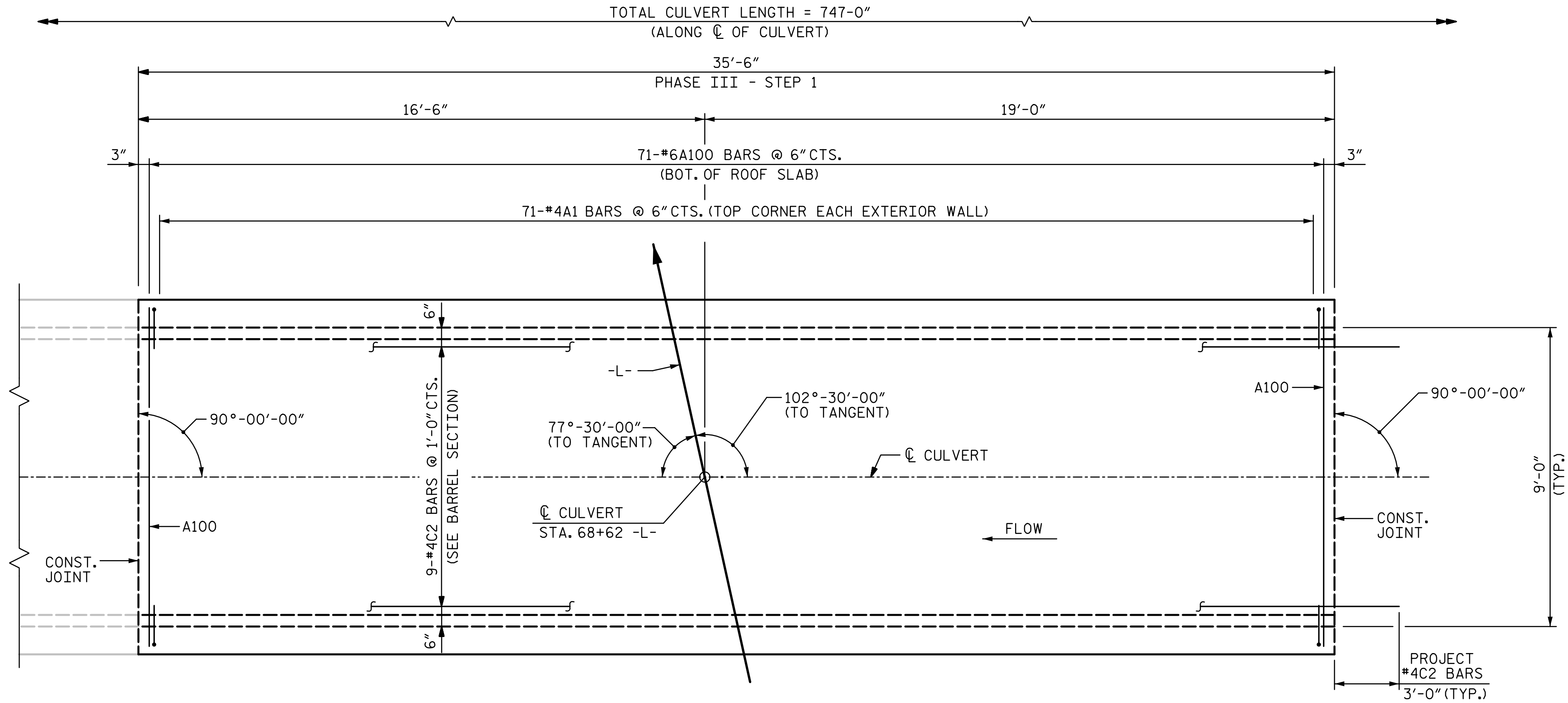
STATE OF NORTH CAROLINA
DEPARTMENT OF TRANSPORTATION
RALEIGH

SINGLE 9'-0" X 6'-0"
CONCRETE BOX CULVERT
102°-30'-00" SKEW

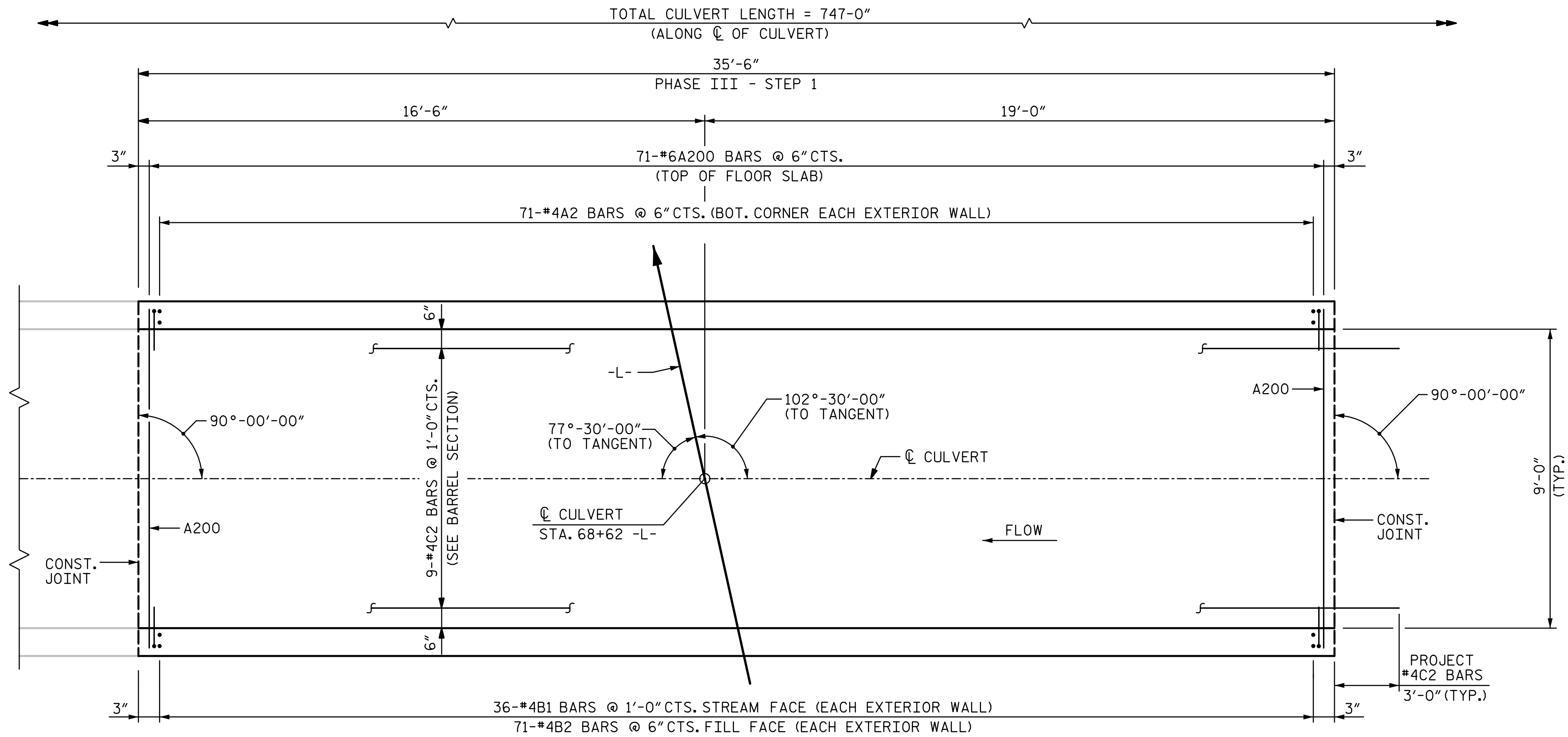
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CHECKED BY : JAD DATE : 11-24
DESIGN ENGINEER OF RECORD : J. GRISCOM DATE : 11-24

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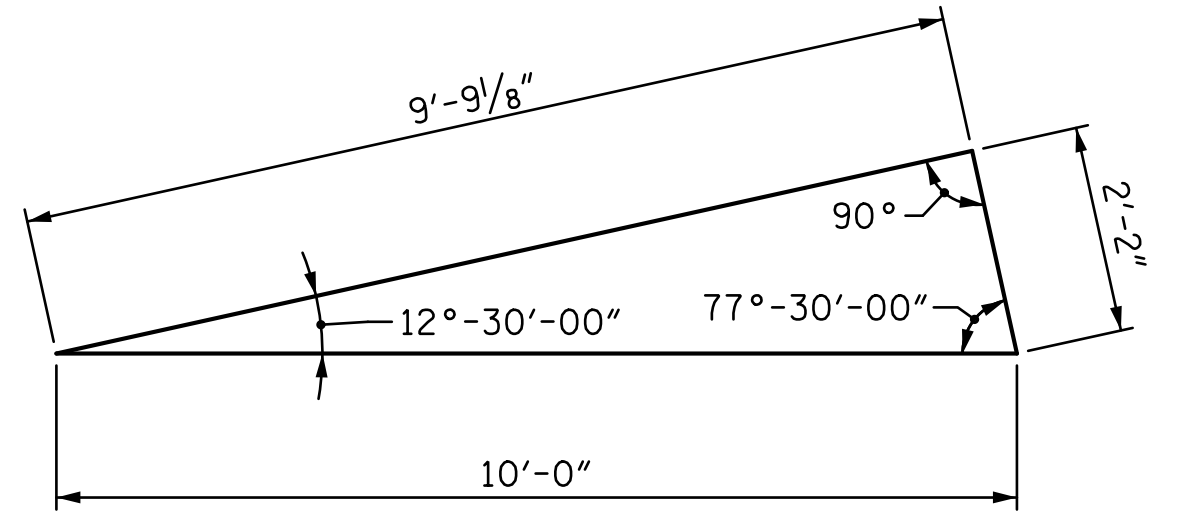
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PLAN ROOF SLAB - PHASE III - STEP 1



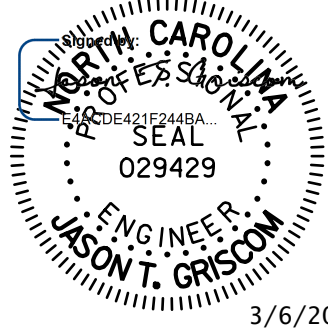
PLAN FLOOR SLAB - PHASE III - STEP 1



SKEW TRIANGLE

PROJECT NO. U-5797
ROBESON COUNTY
STATION: 68+62.00 -L-

SHEET 6 OF 15

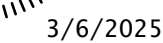


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DESIGN ENGINEER OF RECORD : J. GRISCOM DATE : 11-24

STATE OF NORTH CAROLINA DEPARTMENT OF TRANSPORTATION RALEIGH					
SINGLE 9'-0" X 6'-0" CONCRETE BOX CULVERT 102°-30'-00" SKEW					
REVISIONS					SHEET NO.
NO.	BY:	DATE:	NO.	BY:	DATE:
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2			4		
					C3-6
					TOTAL SHEETS 15

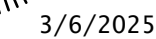
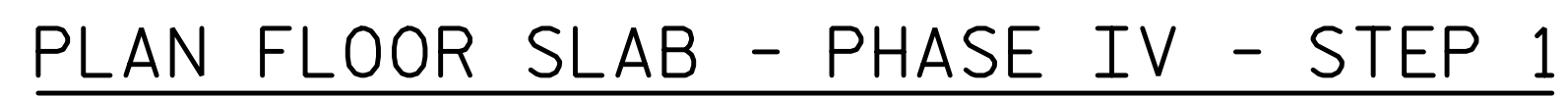


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STATE OF NORTH CAROLINA
 DEPARTMENT OF TRANSPORTATION
 RALEIGH

SINGLE 9'-0" X 6'-0"
 CONCRETE BOX CULVERT
 102°-30'-00" SKEW

REVISIONS						SHEET NO. C3-7
NO.	BY:	DATE:	NO.	BY:	DATE:	
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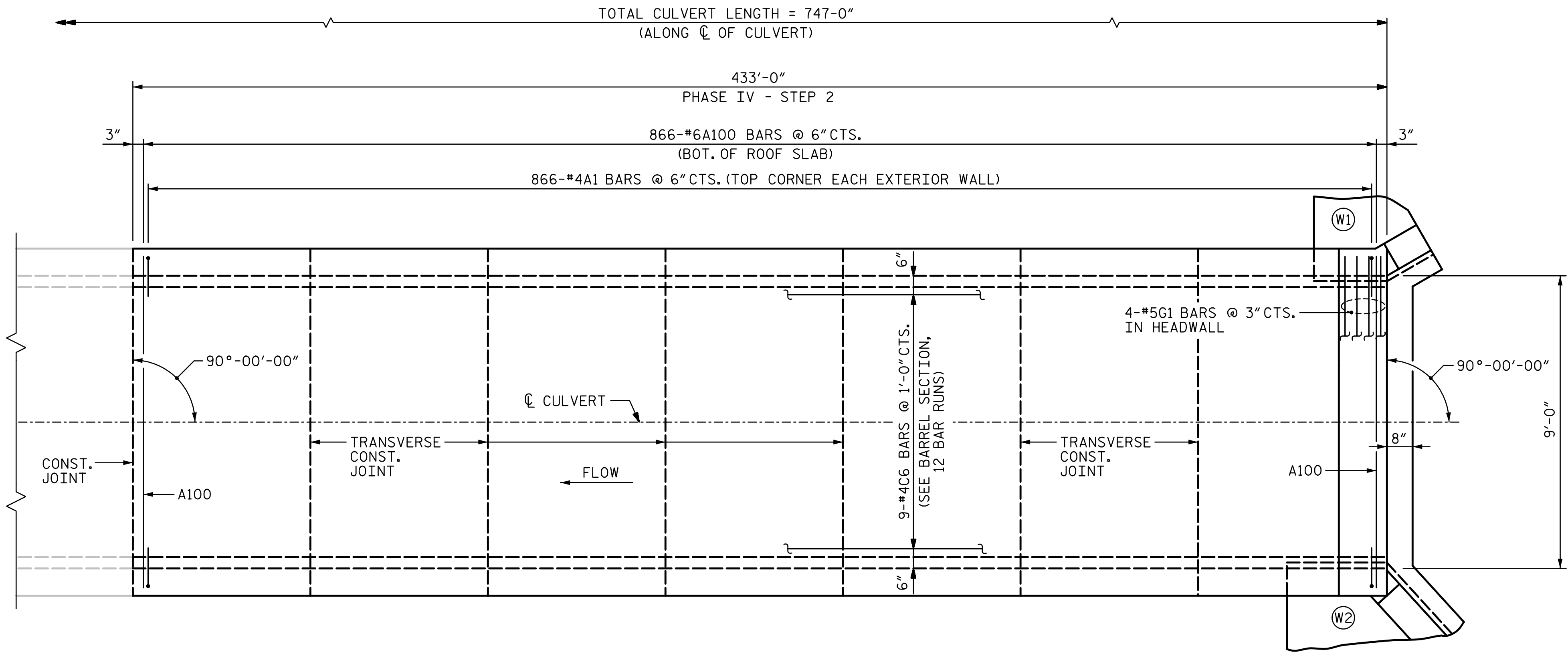
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STATE OF NORTH CAROLINA
DEPARTMENT OF TRANSPORTATION
RALEIGH

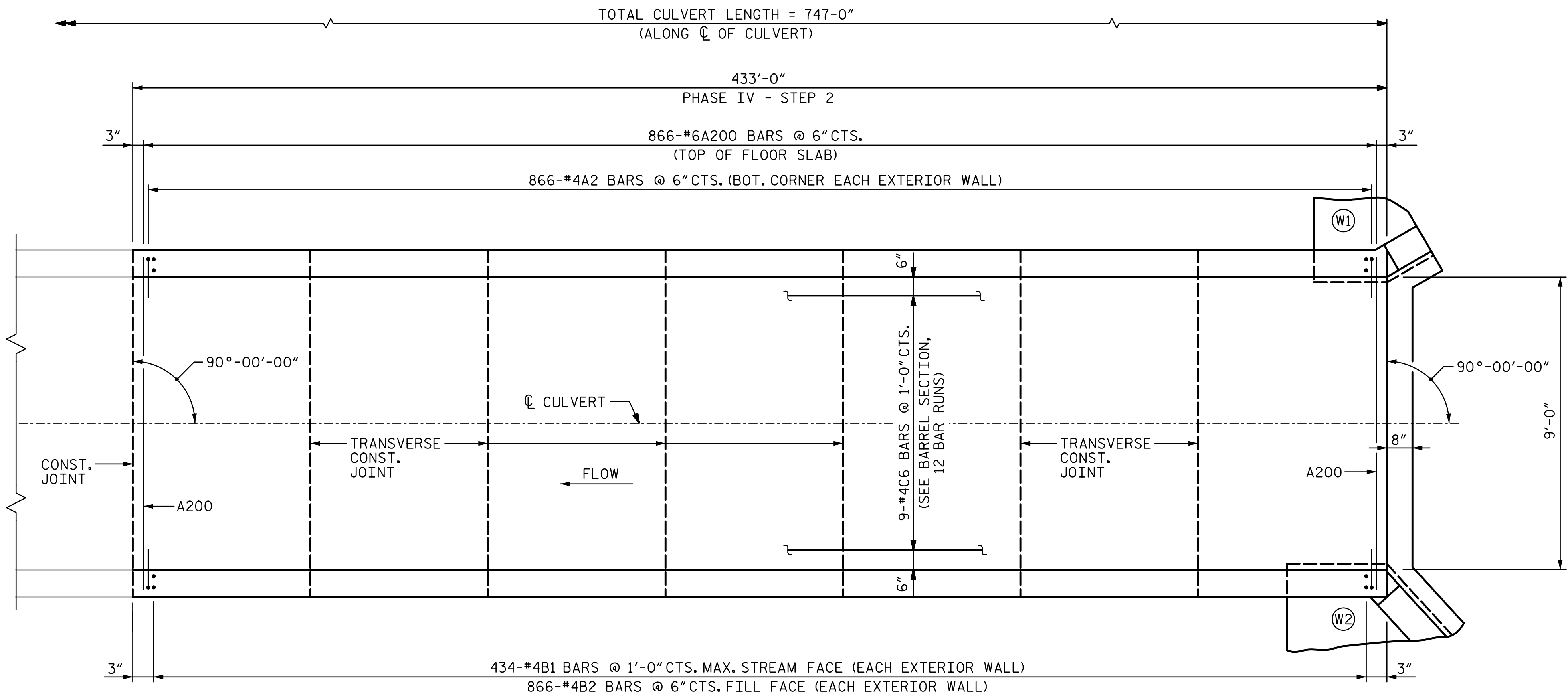
SINGLE 9'-0" X 6'-0"
CONCRETE BOX CULVERT
102°-30'-00" SKEW

REVISONS						SHEET NO. C3-8
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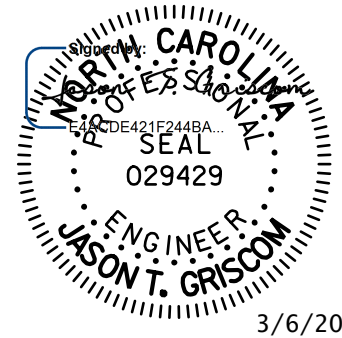
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PLAN ROOF SLAB - PHASE IV - STEP 2



PLAN FLOOR SLAB - PHASE IV - STEP 2



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ROBESON COUNTY
STATION: 68+62.00 -L-

SHEET 9 OF 15

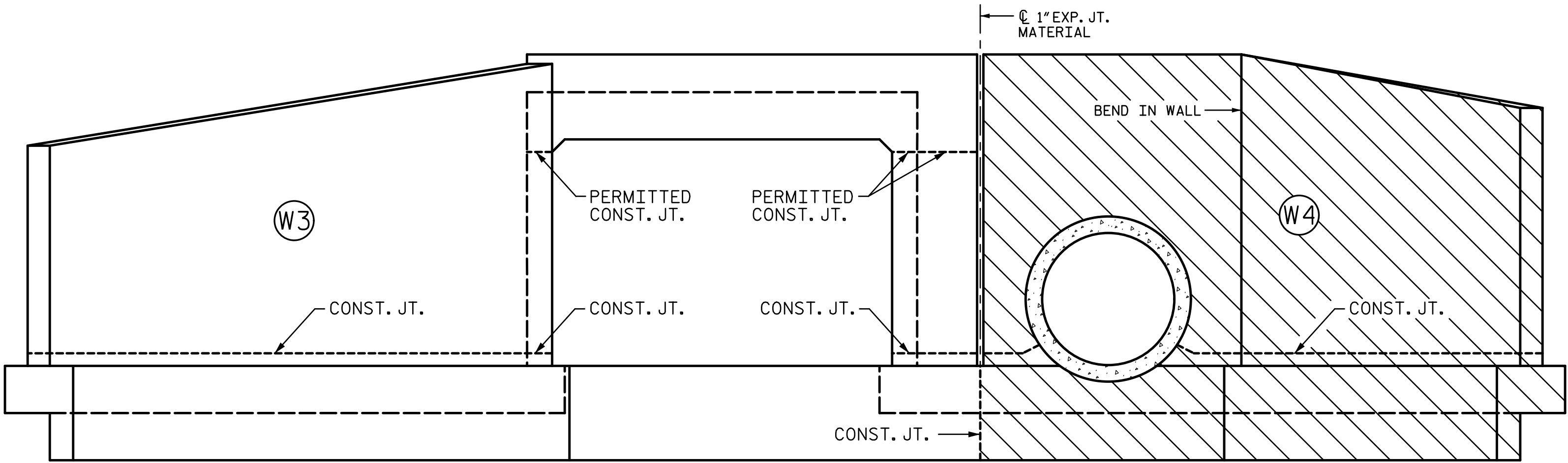
STATE OF NORTH CAROLINA
DEPARTMENT OF TRANSPORTATION
RALEIGH

SINGLE 9'-0" X 6'-0"
CONCRETE BOX CULVERT
102°-30'-00" SKEW

REVISIONS					SHEET NO.
NO.	BY:	DATE:	NO.	BY:	DATE:
1			3		
2			4		
					TOTAL SHEETS 15

DRAWN BY : SGH DATE : 11-24
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DESIGN ENGINEER OF RECORD : J. GRISCOM DATE : 11-24

BAR TYPES						BILL OF MATERIAL PHASE II - STEP 3 - STAGE 2						BILL OF MATERIAL PHASE III - STEP 1						BILL OF MATERIAL PHASE IV - STEP 1						BILL OF MATERIAL PHASE IV - STEP 2					
VERTICAL LEG ① 6" R. 3'-11½" 2'-8½" 2'-10" 2'-6" 9½" 9½" 4'-7⅞" 4'-5" 4'-8" 9¾" ③ E3 E2 E1 1'-5" 8½" 1'-9¼" 2'-0" 1'-0" 2'-6" ② E1 E2 E3 1'-9¼" 8½" 1'-5" 3'-9" 2'-4" 3'-0" DIMENSIONS ARE OUT TO OUT.						BAR	NO.	SIZE	TYPE	LENGTH	WEIGHT	BAR	NO.	SIZE	TYPE	LENGTH	WEIGHT	BAR	NO.	SIZE	TYPE	LENGTH	WEIGHT	BAR	NO.	SIZE	TYPE	LENGTH	WEIGHT
						A1	254	#4	1	7'-7"	1,287	A1	142	#4	1	7'-7"	719	A1	861	#4	1	7'-7"	4,362	A1	1,732	#4	1	7'-7"	8,774
						A2	254	#4	1	6'-0"	1,018	A2	142	#4	1	6'-0"	569	A2	861	#4	1	6'-0"	3,451	A2	1,732	#4	1	6'-0"	6,942
						A100	127	#6	STR	10'-2"	1,939	A100	71	#6	STR	10'-2"	1,084	A100	430	#6	STR	10'-2"	6,566	A100	866	#6	STR	10'-2"	13,224
						A200	127	#6	STR	10'-2"	1,939	A200	71	#6	STR	10'-2"	1,084	A101	3	#6	STR	6'-11"	31	A200	866	#6	STR	10'-2"	13,224
																		A200	430	#6	STR	10'-2"	6,566						
						B1	128	#4	STR	7'-10"	670	B1	72	#4	STR	7'-10"	377	A201	3	#6	STR	6'-11"	31	B1	868	#4	STR	7'-10"	4,542
						B2	254	#4	STR	5'-0"	848	B2	142	#4	STR	5'-0"	474							B2	1,732	#4	STR	5'-0"	5,785
																		B1	432	#4	STR	7'-10"	2,261						
						C1	72	#4	STR	34'-5"	1,655	C2	36	#4	STR	37'-11"	912	B2	861	#4	STR	5'-0"	2,876	C6	432	#4	STR	38'-3"	11,038
						E1	8	#5	2	6'-3"	52	PHASE III - STEP 1 REINFORCING STEEL 5,219 LBS.						C3	108	#4	STR	34'-8"	2,501	E3	16	#5	2	5'-0"	83
						G1	2	#5	STR	10'-2"	21	PHASE III - STEP 1 QUANTITIES						C4	36	#4	3	9'-1"	218						
						G2	2	#5	STR	11'-8"	24							C5	108	#4	STR	40'-0"	2,886	G1	4	#5	STR	10'-2"	42
						PHASE II - STEP 3 - STAGE 2 REINFORCING STEEL 9,505 LBS.						REINFORCING STEEL BARREL 5,219 LBS. TOTAL 5,219 LBS.						PHASE IV - STEP 1 REINFORCING STEEL 31,805 LBS.						PHASE IV - STEP 2 REINFORCING STEEL 63,738 LBS.					
						PHASE II - STEP 3 - STAGE 2 QUANTITIES						CLASS A CONCRETE BARREL @ 1.213 CY/FT 43.1 CY TOTAL 43.1 CY						PHASE IV - STEP 1 QUANTITIES						REINFORCING STEEL BARREL 63,738 LBS. WINGS 1 & 2 539 LBS. TOTAL 64,277 LBS.					
						REINFORCING STEEL BARREL 9,505 LBS. WING 3 & PORTION OF WING 4 1,025 LBS. TOTAL 10,530 LBS.												REINFORCING STEEL BARREL 31,805 LBS. TOTAL 31,805 LBS.						CLASS A CONCRETE BARREL @ 1.213 CY/FT 525.0 CY END CURTAIN WALL 1.1 CY WINGS 1 & 2 4.6 CY HEADWALL 0.9 CY TOTAL 531.6 CY					
						CLASS A CONCRETE BARREL @ 1.213 CY/FT 76.7 CY END CURTAIN WALL 2.0 CY WING 3 & PORTION OF WING 4 9.1 CY HEADWALL 0.5 CY TOTAL 88.3 CY												CLASS A CONCRETE BARREL @ 1.213 CY/FT 260.8 CY TOTAL 260.8 CY											
BAR						SIZE				LENGTH																			
"B"						#4				1'-10"																			
"C"						#4				2'-5"																			



- PHASE II - STEP 3 - STAGE 1
- PHASE II - STEP 3 - STAGE 2
OR
PHASE III - STEP 1
OR
PHASE IV - STEP 1
OR
PHASE IV - STEP 2

BILL OF MATERIAL PHASE II - STEP 3 - STAGE 1			
REINFORCING STEEL PORTION OF WING 4	1,003 LBS.		
TOTAL	1,003 LBS.		
CLASS A CONCRETE			
END CURTAIN WALL	1.1 CY		
PORTION OF WING 4	8.7 CY		
TOTAL	9.8 CY		

CULVERT CONSTRUCTION SEQUENCE

LOOKING UPSTREAM/NORMAL TO OUTLET END

PHASE II - STEP 3 - STAGE 1
POUR 1: PART OF W4 FOOTING & 4"OF W4 VERTICAL WALL
POUR 2: REMAINING PORTIONS OF W4 VERTICAL WALL

PHASE II - STEP 3 - STAGE 2
POUR 1: BARREL FLOOR SLAB, 4"OF VERTICAL WALLS,
W3 FOOTING AND REMAINING PORTION OF W4
FOOTING, 4"OF W3 VERTICAL WALL & 4"OF
REMAINING PORTION OF W4 VERTICAL WALL

POUR 2: REMAINING PORTIONS OF BARREL, W3 & W4
VERTICAL WALLS

POUR 3: ROOF SLAB, HEADWALL & REMIANING PORTION OF
W4 VERTICAL WALL

PHASE III - STEP 1
POUR 1: FLOOR SLAB & 4"OF VERTICAL WALLS

POUR 2: REMAINING PORTIONS OF VERTICAL WALLS

POUR 3: ROOF SLAB

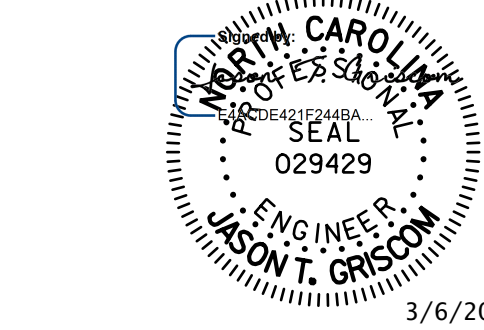
PHASE IV - STEP 1
POUR 1: FLOOR SLAB & 4"OF VERTICAL WALLS
POUR 2: REMAINING PORTIONS OF VERTICAL WALLS
POUR 3: ROOF SLAB

PHASE IV - STEP 2
POUR 1: W1 & W2 FOOTINGS, FLOOR SLAB, 4"BARREL, W1 &
W2 VERTICAL WALLS

POUR 2: REMAINING PORTIONS OF BARREL, W1 & W2
VERTICAL WALLS

POUR 3: ROOF SLAB & HEADWALL

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DESIGN ENGINEER OF RECORD :	J. GRISCOM	DATE :	11-24



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PROJECT NO. U-5797

ROBESON COUNTY

STATION: 68+62.00 -L-

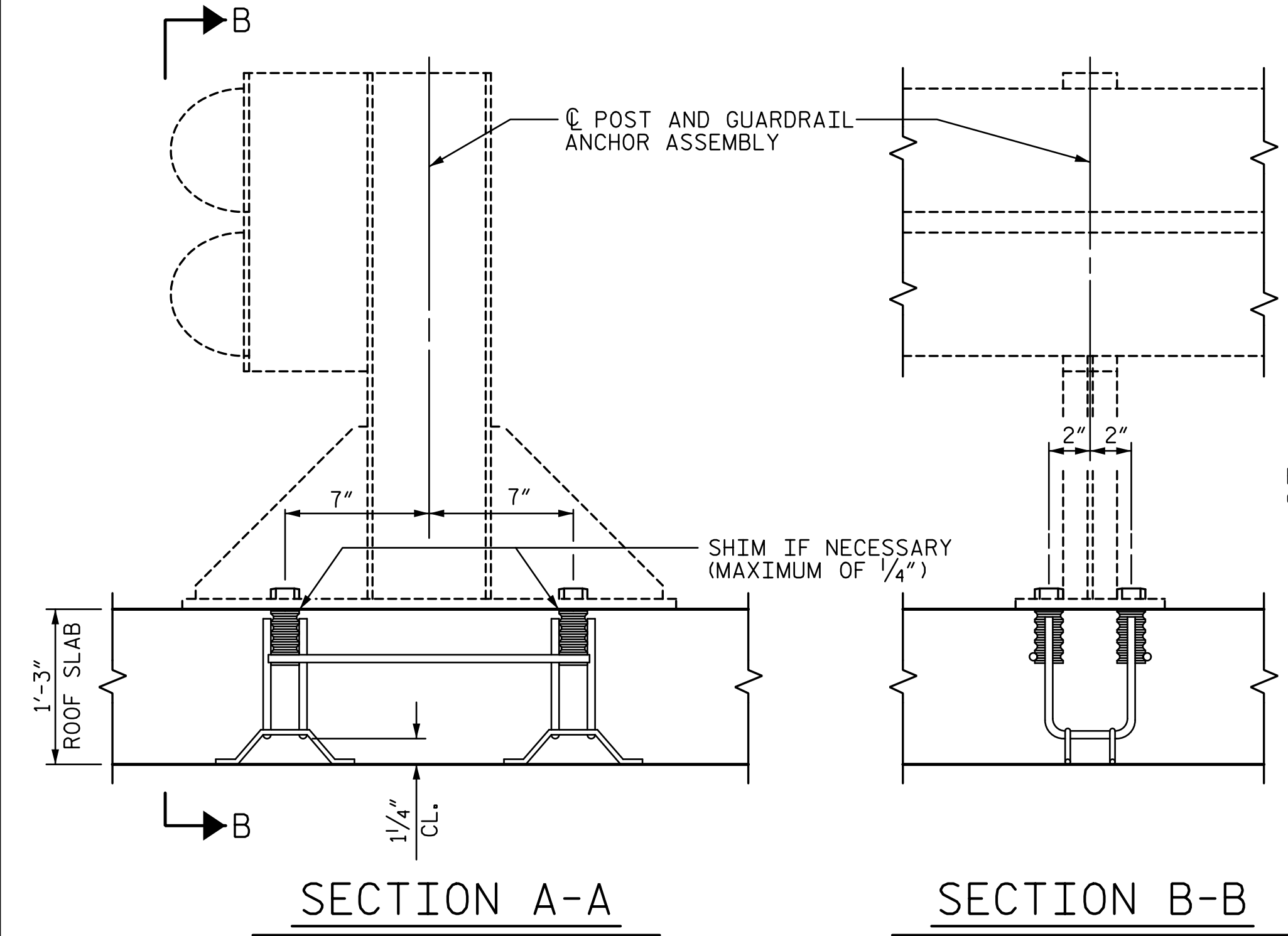
SHEET 10 OF 15

STATE OF NORTH CAROLINA DEPARTMENT OF TRANSPORTATION RALEIGH					
SINGLE 9'-0" X 6'-0" CONCRETE BOX CULVERT 102°-30'-00" SKEW					
REVISIONS					SHEET NO.
NO.	BY:	DATE:	NO.	BY:	DATE:
1			3		
2			4		
					C3-10
					TOTAL SHEETS 15

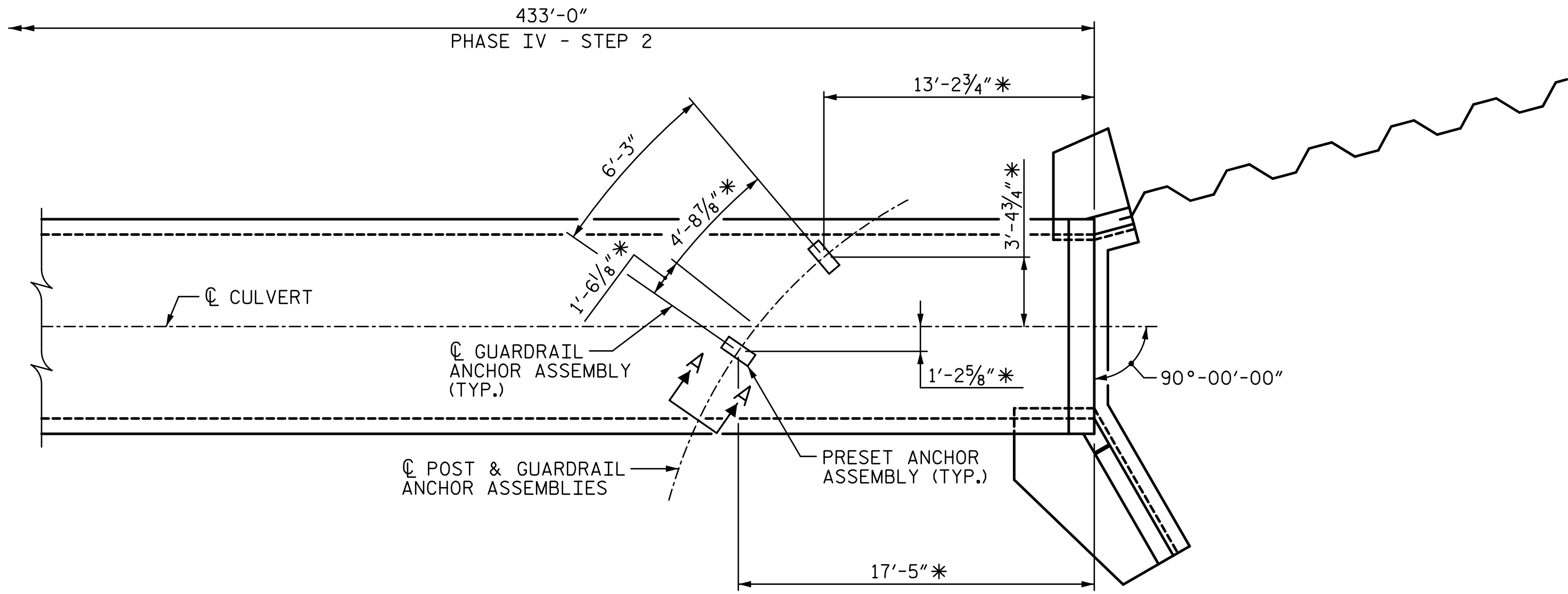
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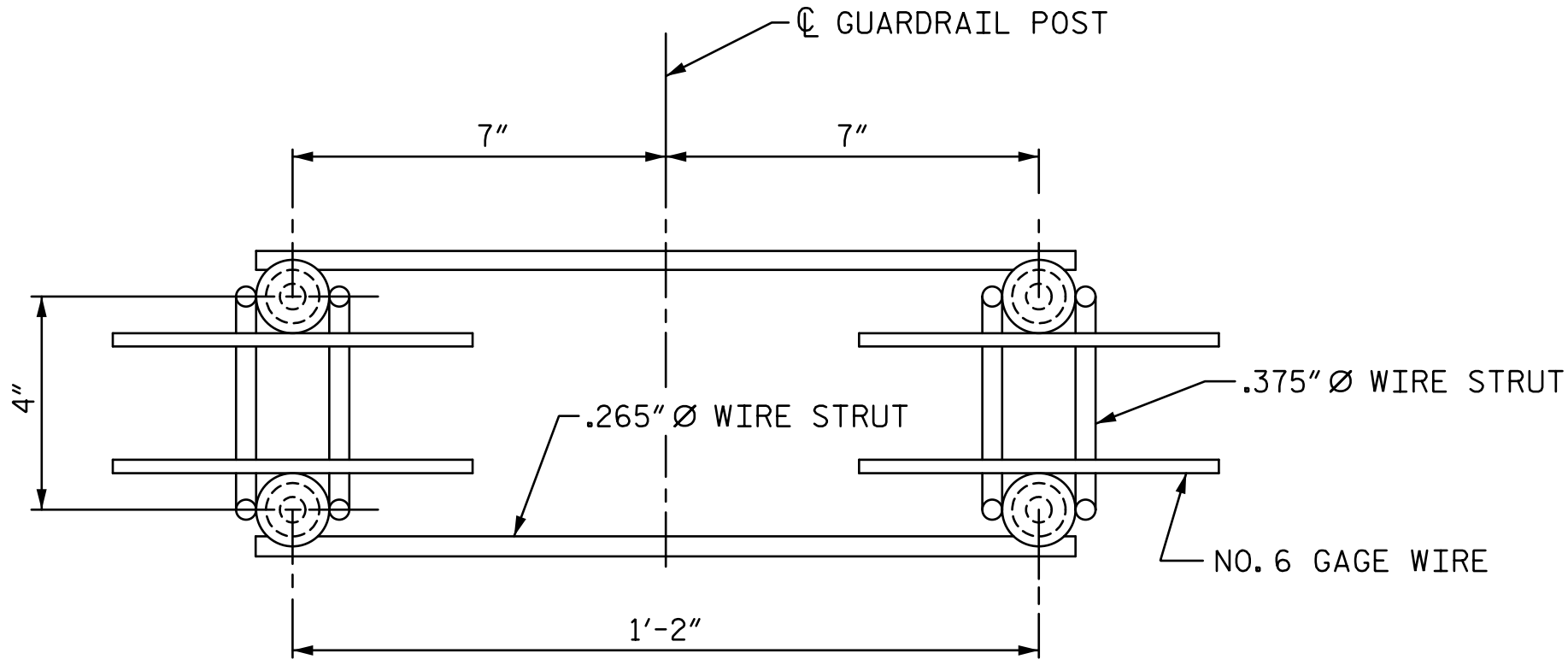


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DESIGN ENGINEER OF RECORD : J. GRISCOM DATE : 11-24

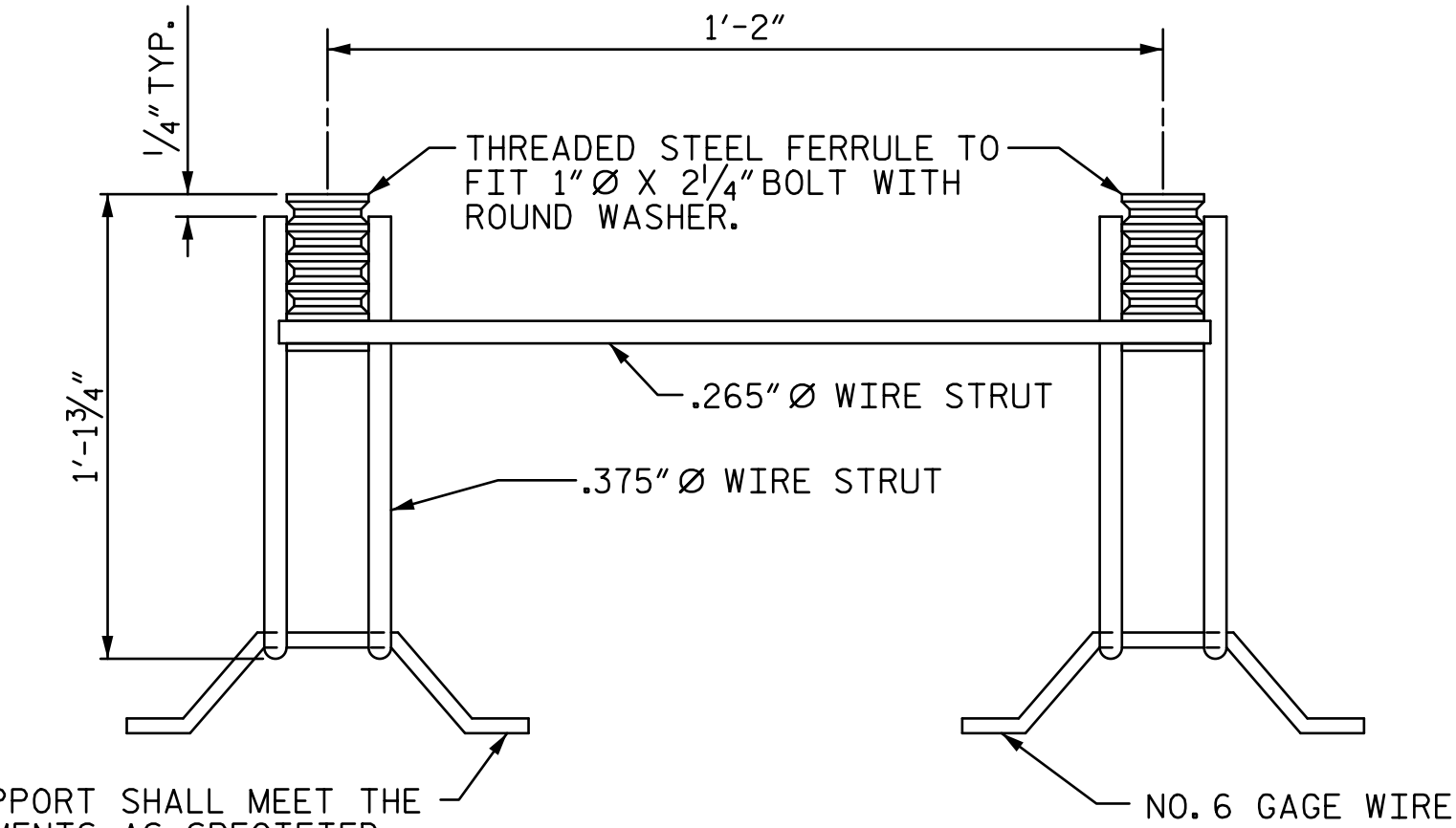


PLAN OF CULVERT GUARDRAIL ANCHOR ASSEMBLY SPACING AT INLET END

* THIS DIMENSION SHALL BE CONFIRMED BY THE ENGINEER IN THE FIELD.



PLAN



SIDE VIEW

ELEVATION

THIS SUPPORT SHALL MEET THE REQUIREMENTS AS SPECIFIED FOR SUPPORTS FOR REINFORCING STEEL. SEE SPECIFICATIONS.

GUARDRAIL ANCHOR ASSEMBLY FOR CULVERTS

NOTES

THE GUARDRAIL ANCHOR ASSEMBLY FOR CULVERTS SHALL CONSIST OF THE FOLLOWING COMPONENTS :

- FERRULES SHALL BE MADE FROM STEEL MEETING THE REQUIREMENTS OF AASHTO M169, GRADE 12L14 AND SHALL HAVE A MINIMUM LENGTH OF THREADS OF 2 1/2".
- 4 - 1" Ø X 2 1/4" BOLTS WITH WASHERS, BOLTS SHALL CONFORM TO THE REQUIREMENTS OF ASTM A307. BOLTS AND WASHERS SHALL BE GALVANIZED. (AT THE CONTRACTOR'S OPTION, STAINLESS STEEL BOLTS AND WASHERS MAY BE USED AS AN ALTERNATE FOR THE 1" Ø X 2 1/4" GALVANIZED BOLTS AND WASHERS. THEY SHALL CONFORM TO OR EXCEED THE MECHANICAL REQUIREMENTS OF ASTM A307. THE USE OF THIS ALTERNATE SHALL BE APPROVED BY THE ENGINEER.)
- WIRE STRUTS SHOWN IN THE GUARDRAIL ANCHOR ASSEMBLY FOR CULVERTS DETAIL ARE MINIMUM ALLOWABLE SIZE AND SHALL HAVE A MINIMUM TENSILE STRENGTH OF 100,000 P.S.I. AS AN OPTION, A 1/6" Ø WIRE STRUT WITH A MINIMUM TENSILE STRENGTH OF 90,000 PSI IS ACCEPTABLE.

GUARDRAIL ANCHOR ASSEMBLY WITH BOLTS SHALL BE ASSEMBLED IN THE SHOP. BOLT THREADS MAY BE RECUT AS NECESSARY TO INSURE FIT.

THE COST OF THE GUARDRAIL ANCHOR ASSEMBLY FOR CULVERTS COMPLETE IN PLACE, SHALL BE INCLUDED IN THE UNIT CONTRACT PRICE BID FOR CLASS "A" CONCRETE.

FERRULES TO BE PLUGGED DURING POURING OF SLAB AS RECOMMENDED BY THE MANUFACTURER.

AT THE CONTRACTOR'S OPTION, FERRULES WITH OPEN OR CLOSED ENDS MAY BE USED.

PAYMENT FOR GUARDRAIL, POSTS, AND POST BASE PLATES IS INCLUDED IN ROADWAY PAY ITEMS.

SLAB REINFORCING STEEL MAY BE SHIFTED AS NECESSARY TO CLEAR GUARDRAIL ANCHOR ASSEMBLY. CARE SHOULD BE TAKEN TO KEEP THE SHIFTING OF REINFORCING STEEL TO A MINIMUM.

THE CONTRACTOR MAY USE ADHESIVELY ANCHORED ANCHOR BOLTS IN PLACE OF GUARDRAIL ANCHOR ASSEMBLY. LEVEL TWO FIELD TESTING IS REQUIRED, AND THE YIELD LOAD OF THE 1" Ø BOLT IS 21.8 KIPS. FOR ADHESIVELY ANCHORED ANCHOR BOLTS OR DOWELS, SEE STANDARD SPECIFICATIONS.

PROJECT NO. U-5797

ROBESON COUNTY

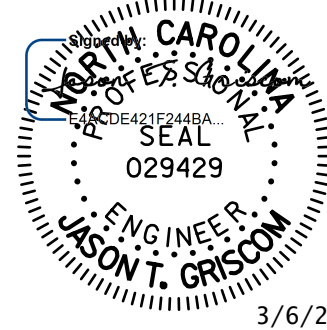
STATION: 68+62.00 -L-

SHEET 11 OF 15

STATE OF NORTH CAROLINA
DEPARTMENT OF TRANSPORTATION
RALEIGH

ANCHORAGE DETAILS FOR
GUARDRAIL ANCHOR ASSEMBLY
FOR CULVERTS

REVISIONS					SHEET NO.
NO.	BY:	DATE:	NO.	BY:	DATE:
1			3		
2			4		
					TOTAL SHEETS 15



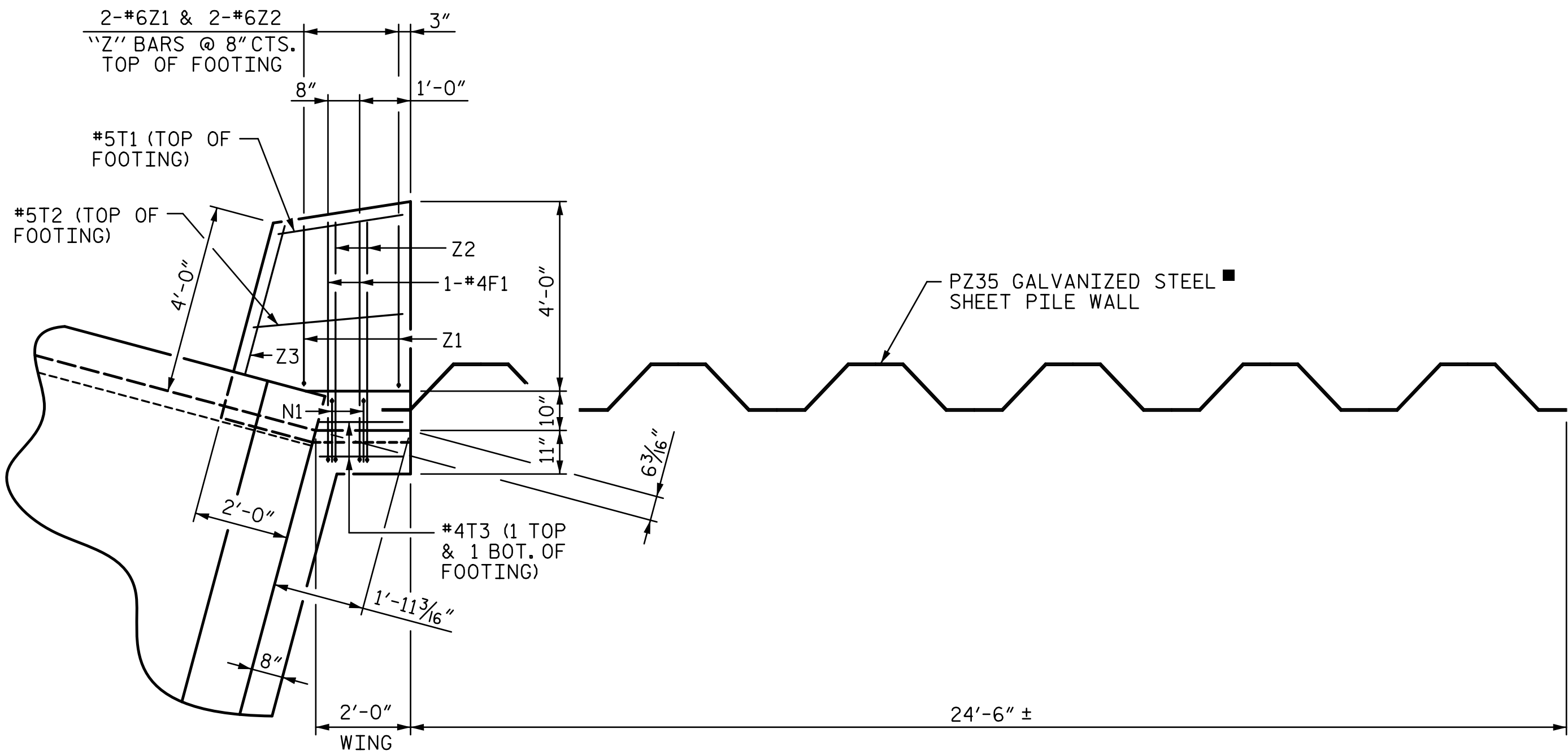
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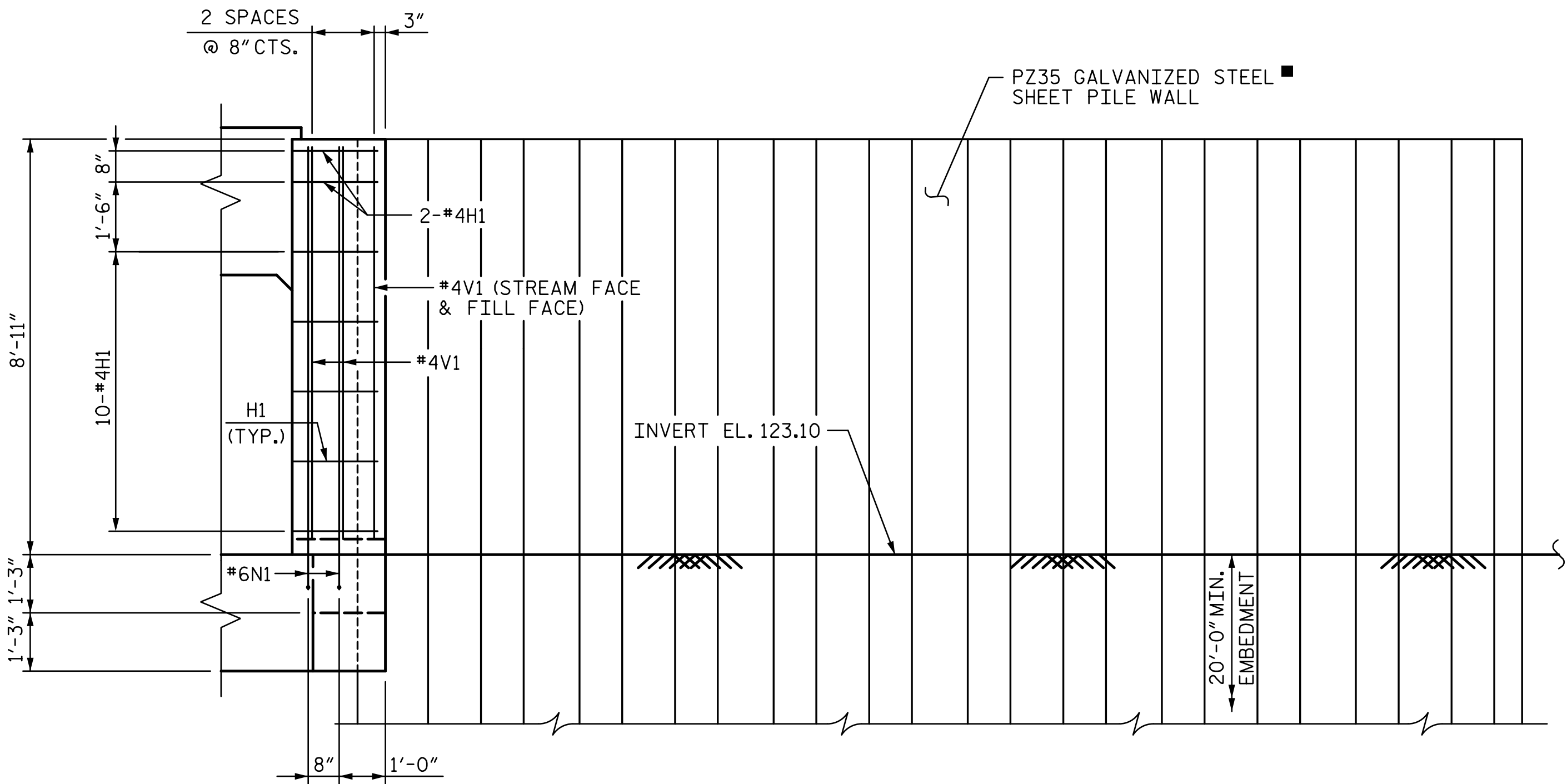
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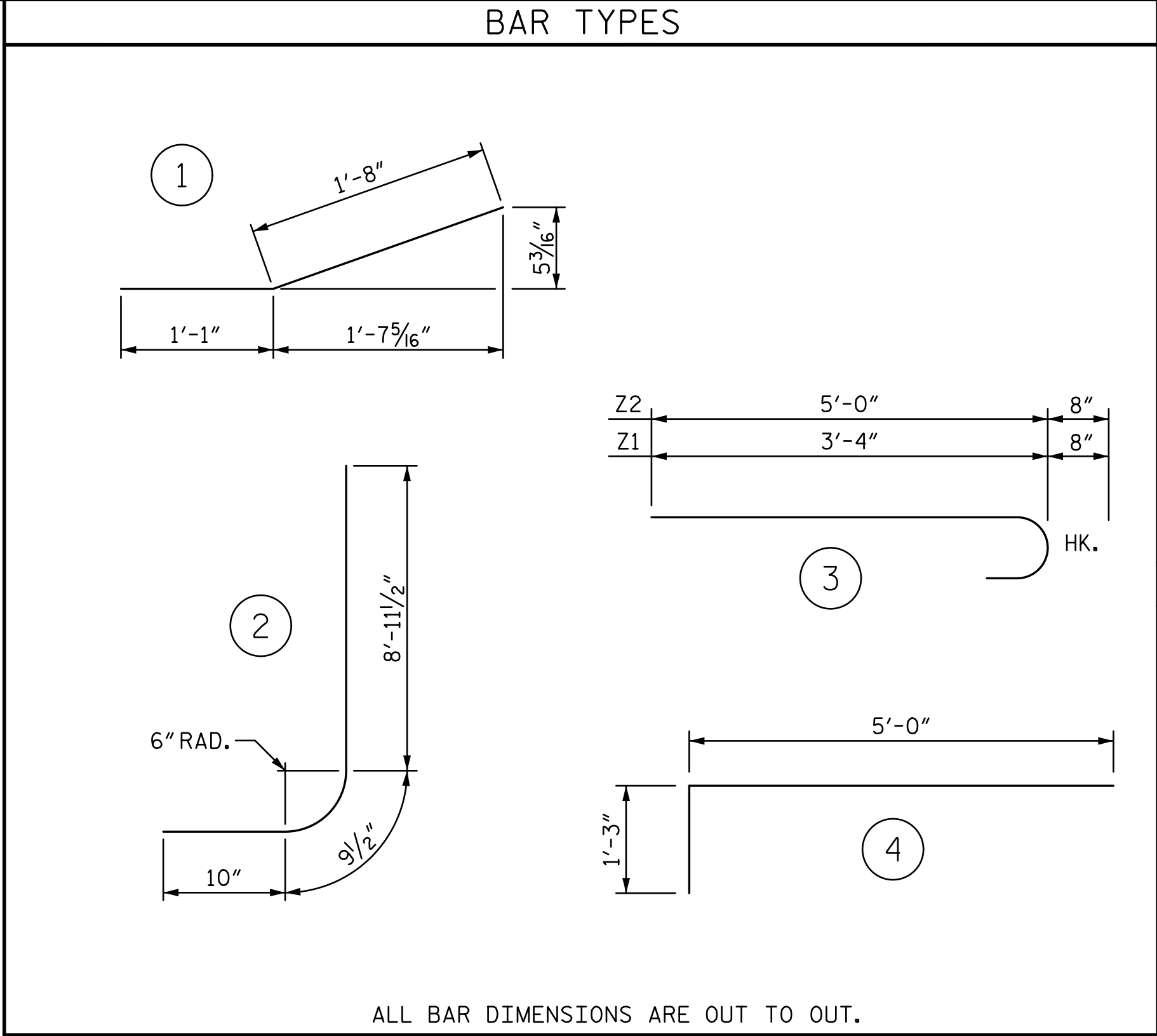


PLAN (W1)

■ GALVANIZED STEEL SHEET PILE SHALL BE EMBEDDED INTO CONCRETE WING A MIN. OF 6"

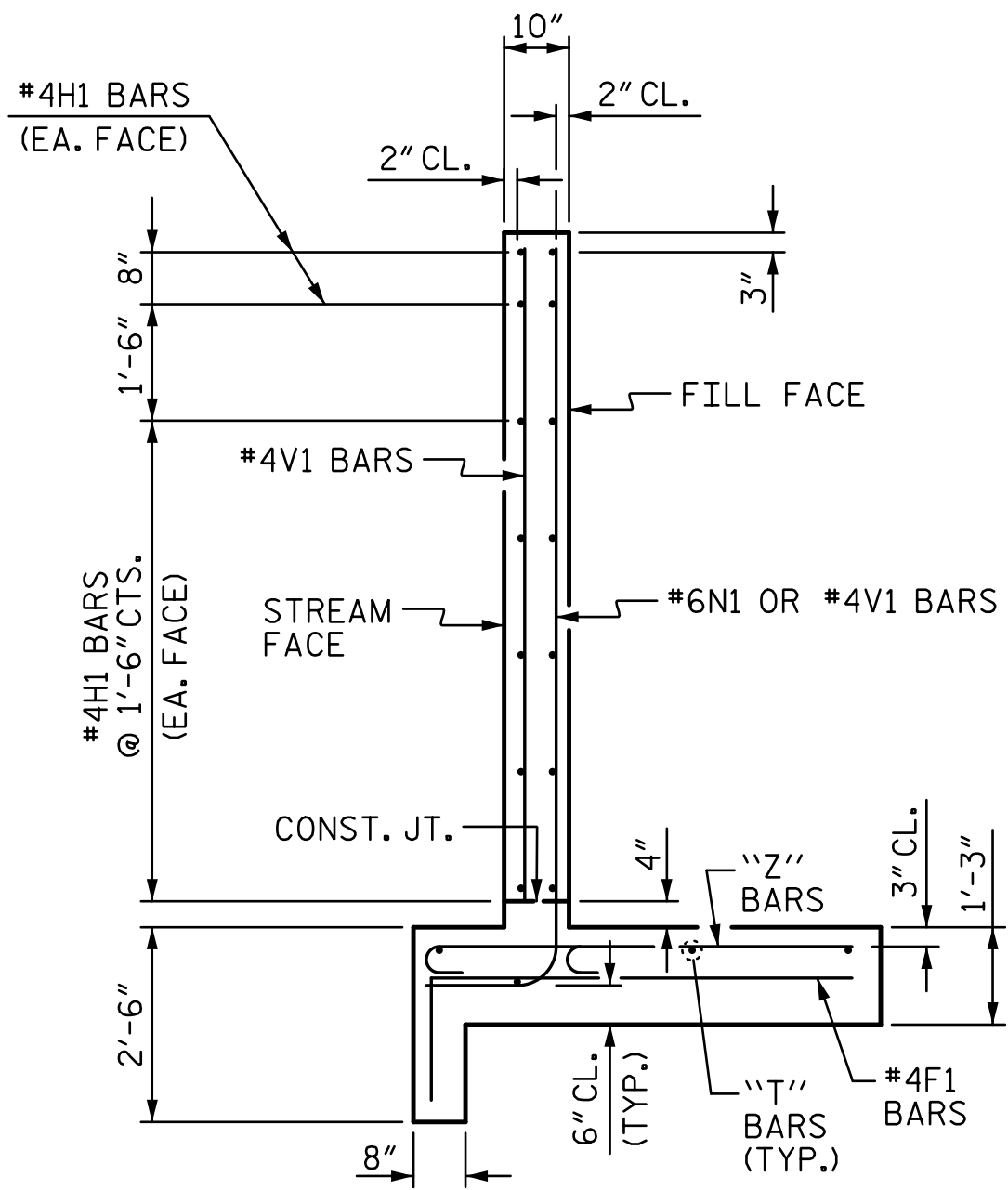


ELEVATION (W1)



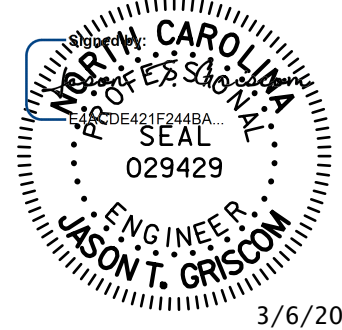
BILL OF MATERIAL					
BAR	NO.	SIZE	TYPE	LENGTH	WEIGHT
F1	2	#4	4	6'-3"	8
H1	14	#4	1	2'-9"	26
N1	2	#6	2	10'-7"	32
T1	1	#5	STR	2'-7"	3
T2	1	#5	STR	3'-2"	3
T3	2	#4	STR	1'-9"	2
V1	4	#4	STR	8'-5"	23
Z1	2	#6	3	4'-0"	12
Z2	2	#6	3	5'-8"	17
Z3	1	#6	STR	3'-3"	5

REINFORCING STEEL (W1) 131 LBS.



WING SECTION

SHEET PILE WALL NOT SHOWN FOR CLARITY



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ROBESON COUNTY
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SHEET 12 OF 15

STATE OF NORTH CAROLINA
DEPARTMENT OF TRANSPORTATION
RALEIGH

WING 1 FOR
CONCRETE BOX CULVERT

REVISIONS					SHEET NO.
NO.	BY:	DATE:	NO.	BY:	DATE:
1			3		
2			4		
TOTAL SHEETS					15

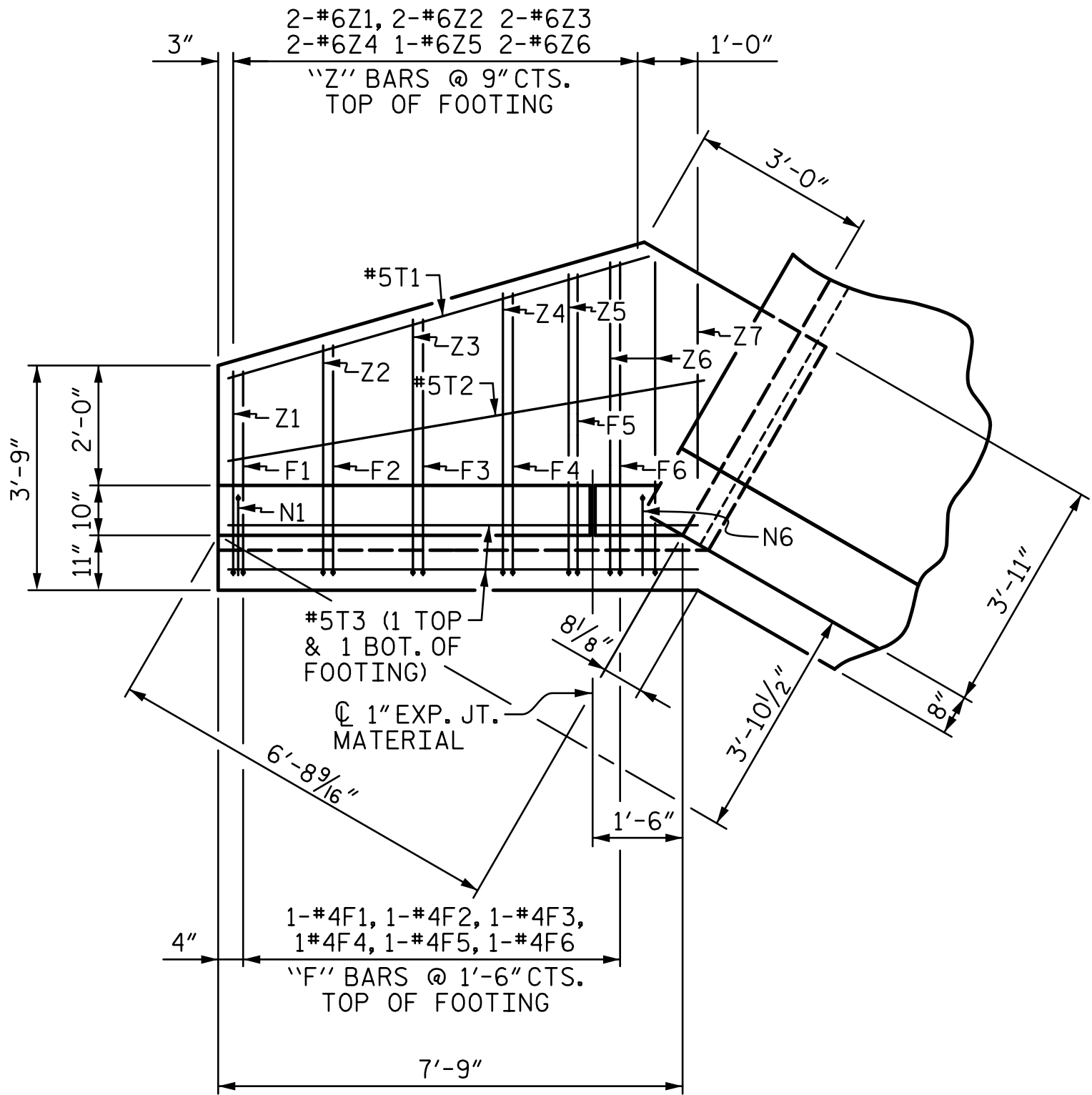
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CHECKED BY : JTG DATE : 11-24
DESIGN ENGINEER OF RECORD : J. GRISCOM DATE : 11-24

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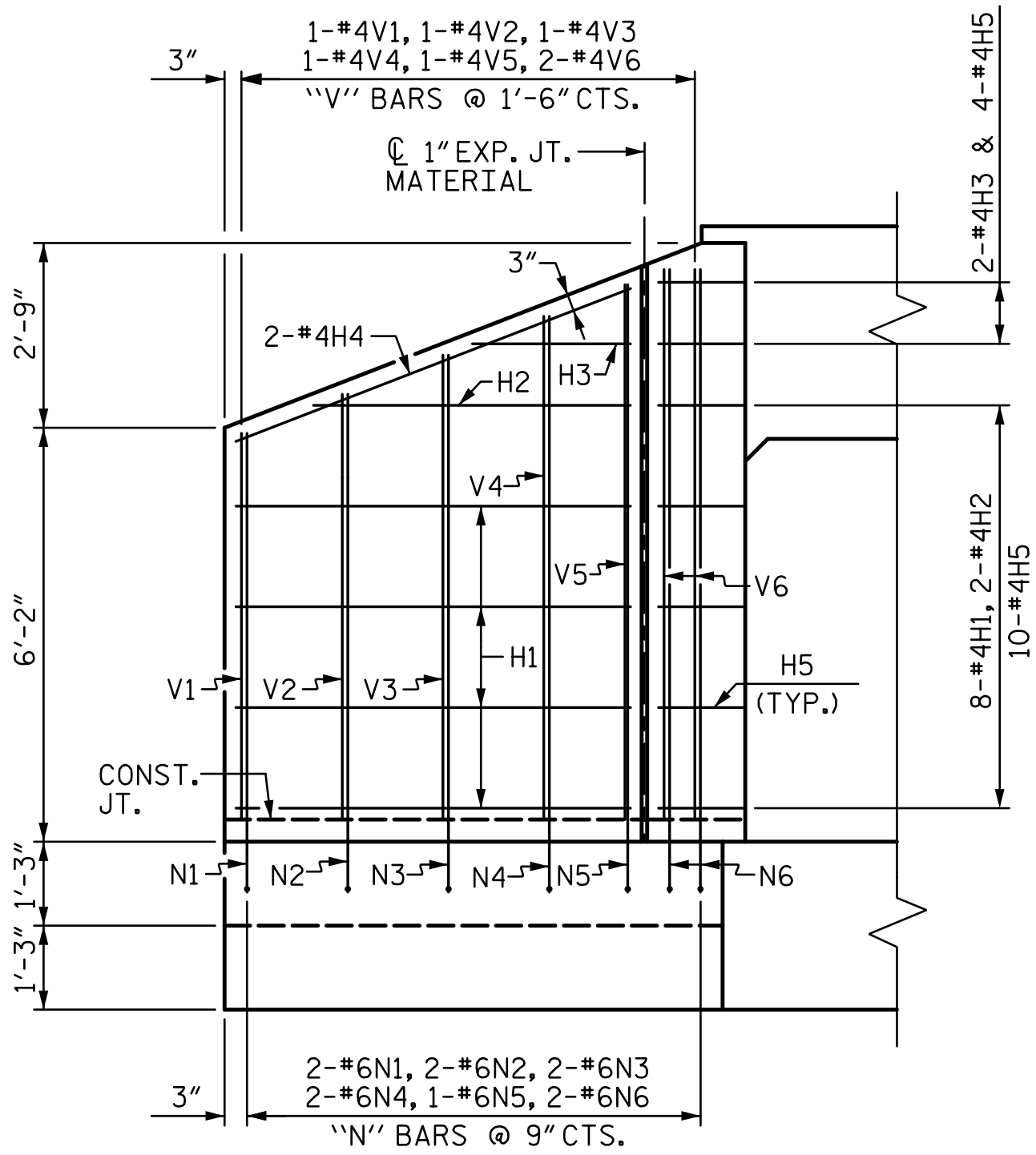
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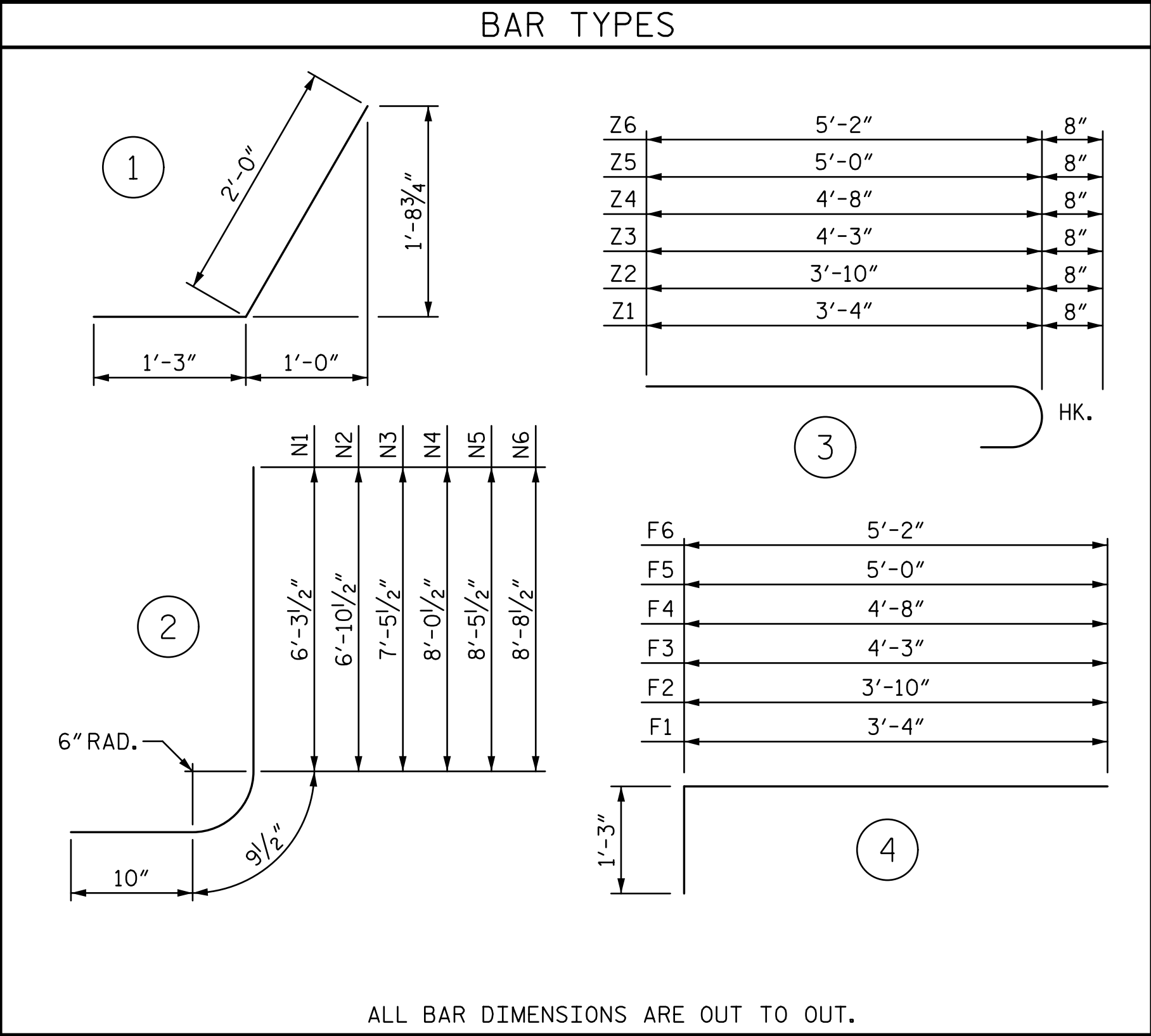
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PLAN (W2)

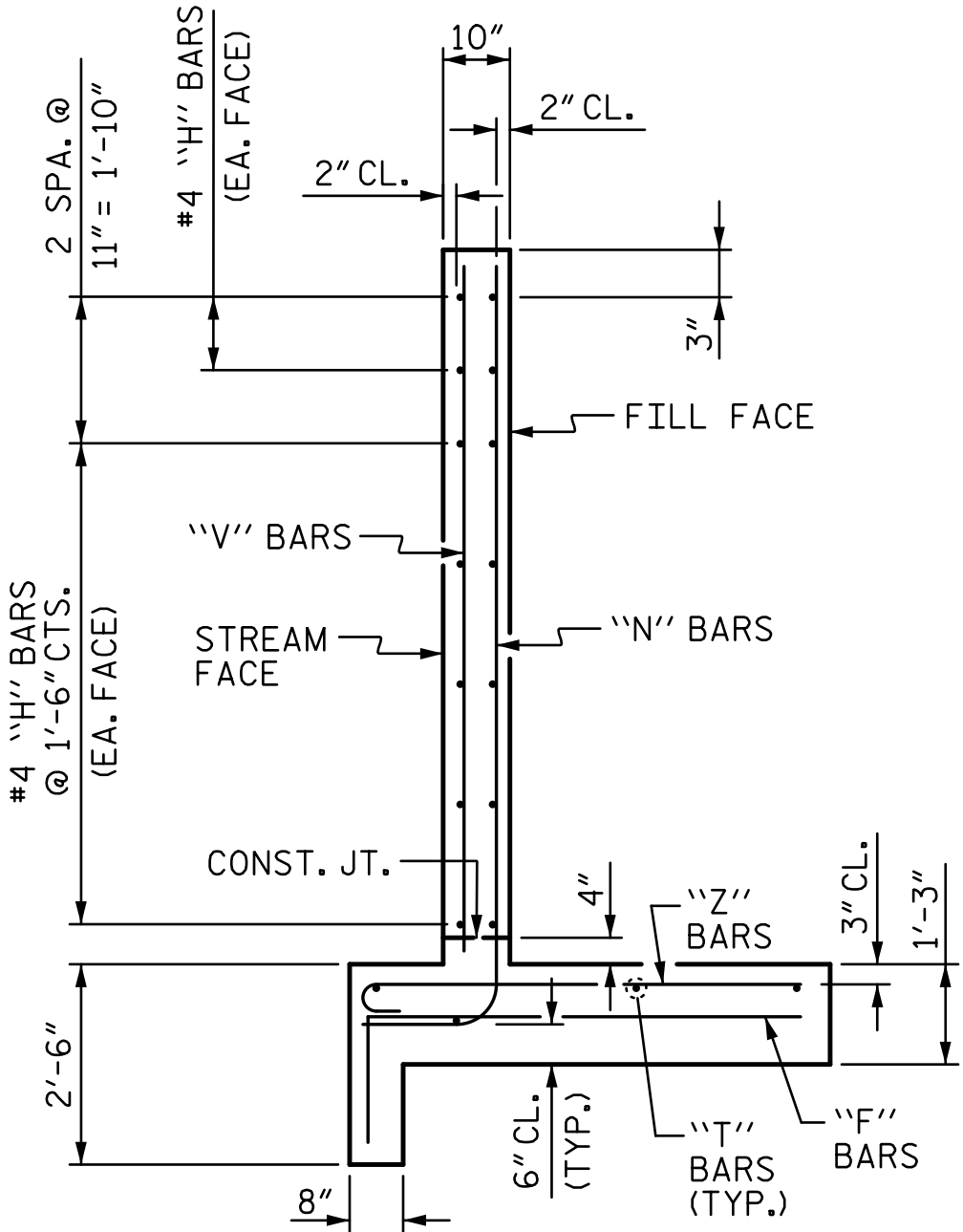


ELEVATION (W2)

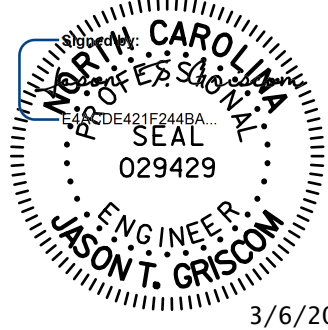


ALL BAR DIMENSIONS ARE OUT TO OUT.

BILL OF MATERIAL					
BAR	NO.	SIZE	TYPE	LENGTH	WEIGHT
F1	1	#4	4	4'-7"	3
F2	1	#4	4	5'-1"	3
F3	1	#4	4	5'-6"	4
F4	1	#4	4	5'-11"	4
F5	1	#4	4	6'-3"	4
F6	1	#4	4	6'-5"	4
H1	8	#4	STR	5'-10"	31
H2	2	#4	STR	4'-8"	6
H3	2	#4	STR	2'-4"	3
H4	2	#4	STR	6'-3"	8
H5	14	#4	1	3'-3"	30
N1	2	#6	2	7'-11"	24
N2	2	#6	2	8'-6"	26
N3	2	#6	2	9'-1"	27
N4	2	#6	2	9'-8"	29
N5	1	#6	2	10'-1"	15
N6	2	#6	2	10'-4"	31
T1	1	#5	STR	7'-3"	8
T2	1	#5	STR	8'-0"	8
T3	2	#5	STR	7'-10"	16
V1	1	#4	STR	5'-9"	4
V2	1	#4	STR	6'-4"	4
V3	1	#4	STR	6'-11"	5
V4	1	#4	STR	7'-6"	5
V5	1	#4	STR	7'-11"	5
V6	2	#4	STR	8'-2"	11
Z1	2	#6	3	4'-0"	12
Z2	2	#6	3	4'-6"	14
Z3	2	#6	3	4'-11"	15
Z4	2	#6	3	5'-4"	16
Z5	1	#6	3	5'-8"	9
Z6	2	#6	3	5'-10"	18
Z7	1	#6	STR	3'-9"	6
REINFORCING STEEL FOR 1 WING (W2)					408 LBS.



TYPICAL WING SECTION



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ROBESON COUNTY
STATION: 68+62.00 -L-

SHEET 13 OF 15

STATE OF NORTH CAROLINA DEPARTMENT OF TRANSPORTATION RALEIGH				
WING 2 FOR CONCRETE BOX CULVERT				
H = 6'-0" 90° SKEW				
REVISIONS				SHEET NO.
NO.	BY:	DATE:	NO.	DATE:
1			3	
2			4	
				C3-13
				TOTAL SHEETS 15

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REINFORCING STEEL FOR 1 WING (W3)	909 LBS.
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SHEET 14 OF 15

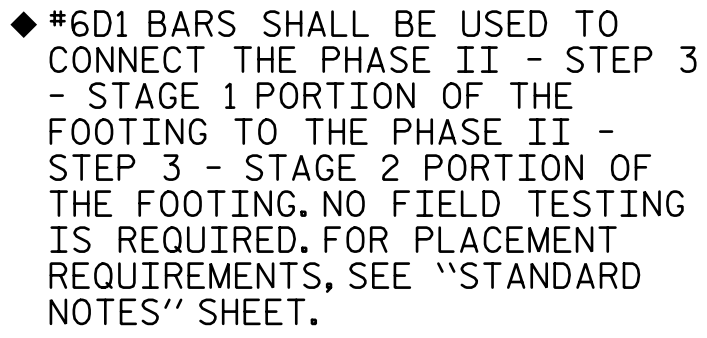
WING 3 FOR
CONCRETE BOX CULVERT

REVISIONS						SHEET NO. C3-14
NO.	BY:	DATE:	NO.	BY:	DATE:	
1			3			TOTAL SHEETS 15
2			4			

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■ E1 BARS TYP. EA. FACE.
BARS MAY BE SHIFTED,
AS NECESSARY TO
MAINTAIN 2" MINIMUM
CLEARANCE TO PIPE.



2'-0"

1

1'-6" H40

1'-10" H41

Z36 5'-4" 8"

Z34 4'-11" 8"

Z33 4'-7" 8"

Z32 4'-2" 8"

Z31 3'-10" 8"

HK.

2

6" RAD.

3

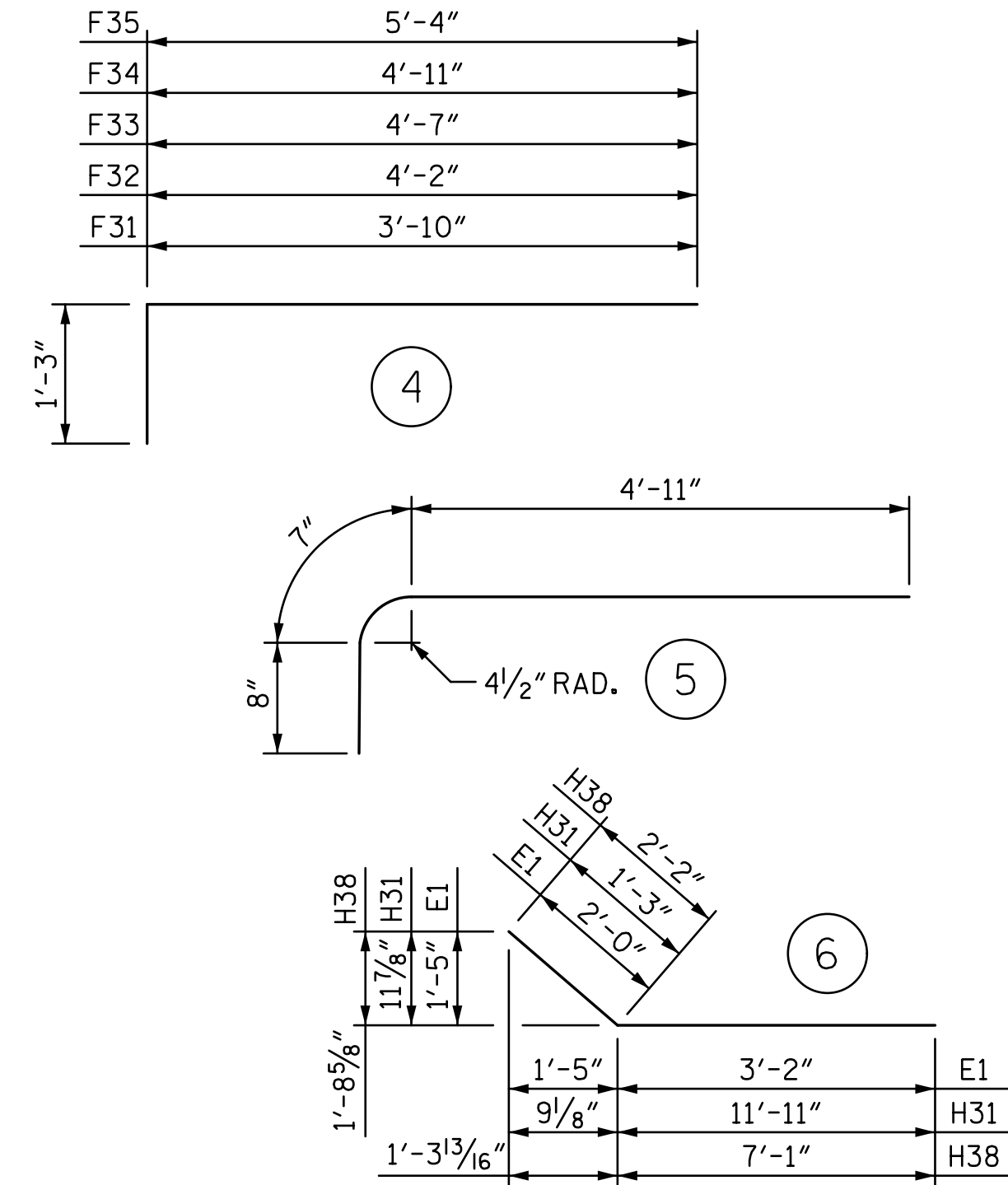
N31 6'-10 1/2"

N32 7'-3 1/2"

N33 7'-7 1/2"

N34 7'-11 1/2"

N35 8'-3 1/2"



SPLICE LENGTHS

BAR	SIZE	LENGTH
"H"	#4	1'-10"

BAR	NO.	SIZE	TYPE	LENGTH	WEIGHT	BAR	NO.	SIZE	TYPE	LENGTH	WEIGHT
D1	5	#6	STR	2'-8"	20	N35	2	#6	2	9'-11"	30
F35	2	#4	4	6'-7"	9	V35	2	#4	STR	7'-9"	10
H40	6	#4	1	3'-6"	14	Z36	2	#6	3	6'-0"	18
H41	6	#4	1	3'-10"	15						

REINFORCING STEEL (W4)	116 LBS.
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WING 4 FOR CONCRETE BOX CULVERT

REVISIONS						SHEET NO. C3-15
NO.	BY:	DATE:	NO.	BY:	DATE:	
1			3			TOTAL SHEETS 15
2			4			

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STANDARD NOTES

DESIGN DATA:	
SPECIFICATIONS.....	AASHTO (CURRENT)
LIVE LOAD	SEE PLANS
IMPACT ALLOWANCE.....	SEE AASHTO
STRESS IN EXTREME FIBER OF STRUCTURAL STEEL - AASHTO M270 GRADE 36	---- 20,000 LBS. PER SQ. IN.
- AASHTO M270 GRADE 50W	--- 27,000 LBS. PER SQ. IN.
- AASHTO M270 GRADE 50	----- 27,000 LBS. PER SQ. IN.
REINFORCING STEEL IN TENSION - GRADE 60	----- 24,000 LBS. PER SQ. IN.
CONCRETE IN COMPRESSION	1,200 LBS. PER SQ. IN.
CONCRETE IN SHEAR	SEE AASHTO
STRUCTURAL TIMBER - TREATED OR UNTREATED EXTREME FIBER STRESS	---- 1,800 LBS. PER SQ. IN.
COMPRESSION PERPENDICULAR TO GRAIN OF TIMBER	375 LBS. PER SQ. IN.
EQUIVALENT FLUID PRESSURE OF EARTH	30 LBS. PER CU. FT. (MINIMUM)

MATERIAL AND WORKMANSHIP:

EXCEPT AS MAY OTHERWISE BE SPECIFIED ON PLANS OR IN THE SPECIAL PROVISIONS, ALL MATERIAL AND WORKMANSHIP SHALL BE IN ACCORDANCE WITH THE 2024 "STANDARD SPECIFICATIONS FOR ROADS AND STRUCTURES" OF THE N. C. DEPARTMENT OF TRANSPORTATION.

STEEL SHEET PILING FOR PERMANENT OR TEMPORARY APPLICATIONS SHALL BE HOT ROLLED.

CONCRETE:

UNLESS OTHERWISE REQUIRED ON PLANS, CLASS A CONCRETE SHALL BE USED FOR ALL PORTIONS OF ALL STRUCTURES WITH THE EXCEPTION THAT: CLASS AA CONCRETE SHALL BE USED IN BRIDGE SUPERSTRUCTURES, ABUTMENT BACKWALLS, AND APPROACH SLABS; AND CLASS B CONCRETE SHALL BE USED FOR SLOPE PROTECTION AND RIP RAP.

CONCRETE CHAMFERS:

UNLESS OTHERWISE NOTED ON THE PLANS, ALL EXPOSED CORNERS ON STRUCTURES SHALL BE CHAMFERED ¾" WITH THE FOLLOWING EXCEPTIONS: TOP CORNERS OF CURBS MAY BE ROUNDED TO 1½" RADIUS WHICH IS BUILT INTO CURB FORMS; CORNERS OF TRANSVERSE FLOOR EXPANSION JOINTS SHALL BE ROUNDED WITH A ¼" FINISHING TOOL UNLESS OTHERWISE REQUIRED ON PLANS; AND CORNERS OF EXPANSION JOINTS IN THE ROADWAY FACES AND TOPS OF CURBS AND SIDEWALKS SHALL BE ROUNDED TO A ¼" RADIUS WITH A FINISHING STONE OR TOOL UNLESS OTHERWISE REQUIRED ON PLANS.

DOWELS:

DOWELS WHEN INDICATED ON PLANS AS FOR CULVERT EXTENSIONS, SHALL BE EMBEDDED AT LEAST 12" INTO THE OLD CONCRETE AND GROUTED INTO PLACE WITH 1:2 CEMENT MORTAR.

ALLOWANCE FOR DEAD LOAD DEFLECTION, SETTLEMENT, ETC. IN CASTING SUPERSTRUCTURES:

BRIDGES SHALL BE BUILT ON THE GRADE OR VERTICAL CURVE SHOWN ON PLANS. SLABS, CURBS AND PARAPETS SHALL CONFORM TO THE GRADE OR CURVE.

ALL DIMENSIONS WHICH ARE GIVEN IN SECTION AND ARE AFFECTED BY DEAD LOAD DEFLECTIONS ARE DIMENSIONS AT CENTER LINE OF BEARING UNLESS OTHERWISE NOTED ON PLANS. IN SETTING FORMS FOR STEEL BEAM BRIDGES AND PRESTRESSED CONCRETE GIRDER BRIDGES, ADJUSTMENTS SHALL BE MADE DUE TO THE DEAD LOAD DEFLECTIONS FOR THE ELEVATIONS SHOWN. WHERE BLOCKS ARE SHOWN OVER BEAMS FOR BUILDING UP TO THE SLAB, THE VERTICAL DIMENSIONS OF THE BLOCKS SHALL BE ADJUSTED BETWEEN BEARINGS TO COMPENSATE FOR DEAD LOAD DEFLECTIONS, VERTICAL CURVE ORDINATE, AND ACTUAL BEAM CAMBER. WHERE BOTTOM OF SLAB IS IN LINE WITH BOTTOM OF TOP FLANGES, DEPTH OF SLAB BETWEEN BEARINGS SHALL BE ADJUSTED TO COMPENSATE FOR DEAD LOAD DEFLECTION, VERTICAL CURVE ORDINATE, AND ACTUAL BEAM CAMBER.

IN SETTING FALSEWORK AND FORMS FOR REINFORCED CONCRETE SPANS, AN ALLOWANCE SHALL BE MADE FOR DEAD LOAD DEFLECTIONS, SETTLEMENT OF FALSEWORK, AND PERMANENT CAMBER WHICH SHALL BE PROVIDED FOR IN ADDITION TO THE ELEVATIONS SHOWN. AFTER REMOVAL OF THE FALSEWORK, THE FINISHED STRUCTURES SHALL CONFORM TO THE PROFILE AND ELEVATIONS SHOWN ON THE PLANS AND CONSTRUCTION ELEVATIONS FURNISHED BY THE ENGINEER.

DETAILED DRAWINGS FOR FALSEWORK OR FORMS FOR BRIDGE SUPERSTRUCTURE AND ANY STRUCTURE OR PARTS OF A STRUCTURE AS NOTED ON THE PLANS SHALL BE SUBMITTED TO THE ENGINEER FOR APPROVAL BEFORE CONSTRUCTION OF THE FALSEWORK OR FORMS IS STARTED.

REINFORCING STEEL:

ALL REINFORCING STEEL SHALL BE DEFORMED. DIMENSIONS RELATIVE TO PLACEMENT OF REINFORCING ARE TO CENTERS OF BARS UNLESS OTHERWISE INDICATED IN THE PLANS. DIMENSIONS ON BAR DETAILS ARE TO CENTERS OF BARS OR ARE OUT TO OUT AS INDICATED ON PLANS.

WIRE BAR SUPPORTS SHALL BE PROVIDED FOR REINFORCING STEEL WHERE INDICATED ON THE PLANS. WHEN BAR SUPPORT PIECES ARE PLACED IN CONTINUOUS LINES, THEY SHALL BE SO PLACED THAT THE ENDS OF THE SUPPORTING WIRES SHALL BE LAPPED TO LOCK LEGS ON ADJOINING PIECES.

STRUCTURAL STEEL:

AT THE CONTRACTOR'S OPTION, HE MAY SUBSTITUTE ⅞" Ø SHEAR STUDS FOR THE ¾" Ø STUDS SPECIFIED ON THE PLANS. THIS SUBSTITUTION SHALL BE MADE AT THE RATE OF 3 - ⅞" Ø STUDS FOR 4 - ¾" Ø STUDS, AND STUD SPACING CHANGES SHALL BE MADE AS NECESSARY TO PROVIDE THE SAME EQUIVALENT NUMBER OF ⅞" Ø STUDS ALONG THE BEAM AS SHOWN FOR ¾" Ø STUDS BASED ON THE RATIO OF 3 - ⅞"Ø STUDS FOR 4 - ¾" Ø STUDS. STUDS OF THE LENGTH SPECIFIED ON THE PLANS MUST BE PROVIDED. THE MAXIMUM SPACING SHALL BE 2'-0".

EXCEPT AT THE INTERIOR SUPPORTS OF CONTINUOUS BEAMS WHERE THE COVER PLATE IS IN CONTACT WITH BEARING PLATE, THE CONTRACTOR MAY, AT HIS OPTION, SUBSTITUTE FOR THE COVER PLATES DESIGNATED ON THE PLANS COVER PLATES OF THE EQUIVALENT AREA PROVIDED THESE PLATES ARE AT LEAST ⅝₁₆" IN THICKNESS AND DO NOT EXCEED A WIDTH EQUAL TO THE FLANGE WIDTH LESS 2" OR A THICKNESS EQUAL TO 2 TIMES THE FLANGE THICKNESS. THE SIZE OF FILLET WELDS SHALL CONFORM TO THE REQUIREMENTS OF THE CURRENT ANSI/AASHTO/AWS "BRIDGE WELDING CODE". ELECTROSLAG WELDING WILL NOT BE PERMITTED.

WITH THE SOLE EXCEPTION OF EDGES AT SURFACES WHICH BEAR ON OTHER SURFACES, ALL SHARP EDGES AND ENDS OF SHAPES AND PLATES SHALL BE SLIGHTLY ROUNDED BY SUITABLE MEANS TO A RADIUS OF APPROXIMATELY ⅛₁₆" OR EQUIVALENT FLAT SURFACE AT A SUITABLE ANGLE PRIOR TO PAINTING, GALVANIZING, OR METALLIZING.

HANDRAILS AND POSTS:

METAL STANDARDS AND FACES OF THE CONCRETE END POSTS FOR THE METAL RAIL SHALL BE SET NORMAL TO THE GRADE OF THE CURB, UNLESS OTHERWISE SHOWN ON PLANS. THE METAL RAIL AND TOPS OF CONCRETE POSTS USED WITH THE ALUMINUM RAIL SHALL BE BUILT PARALLEL TO THE GRADE OF THE CURB.

METAL HANDRAILS SHALL BE IN ACCORDANCE WITH THE PLANS. RAILS SHALL BE AS MANUFACTURED FOR BRIDGE RAILING. CASTINGS SHALL BE OF A UNIFORM APPEARANCE. FINS AND OTHER DEFORMATIONS RESULTING FROM CASTING OR OTHERWISE SHALL BE REMOVED IN A MANNER SO THAT A UNIFORM COLORING OF THE COMPLETED CASTING SHALL BE OBTAINED. CASTINGS WITH DISCOLORATIONS OR OF NON-UNIFORM COLORING WILL NOT BE ACCEPTED. CERTIFIED MILL REPORTS ARE REQUIRED FOR METAL RAILS AND POSTS.

SPECIAL NOTES:

GENERALLY, IN CASE OF DISCREPANCY, THIS STANDARD SHEET OF NOTES SHALL GOVERN OVER THE SPECIFICATIONS, BUT THE REMAINDER OF THE PLANS SHALL GOVERN OVER NOTES HEREON, AND SPECIAL PROVISIONS SHALL GOVERN OVER ALL. SEE SPECIFICATIONS ARTICLE 105-4.

STANDARD NOTES

DESIGN DATA:	
SPECIFICATIONS	AASHTO (CURRENT)
LIVE LOAD	SEE PLANS
IMPACT ALLOWANCE	SEE AASHTO
STRESS IN EXTREME FIBER OF STRUCTURAL STEEL - AASHTO M270 GRADE 36	20,000 LBS. PER SQ. IN.
- AASHTO M270 GRADE 50W	27,000 LBS. PER SQ. IN.
- AASHTO M270 GRADE 50	27,000 LBS. PER SQ. IN.
REINFORCING STEEL IN TENSION - GRADE 60	24,000 LBS. PER SQ. IN.
CONCRETE IN COMPRESSION	1,200 LBS. PER SQ. IN.
CONCRETE IN SHEAR	SEE AASHTO
STRUCTURAL TIMBER - TREATED OR UNTREATED EXTREME FIBER STRESS	1,800 LBS. PER SQ. IN.
COMPRESSION PERPENDICULAR TO GRAIN OF TIMBER	375 LBS. PER SQ. IN.
EQUIVALENT FLUID PRESSURE OF EARTH	30 LBS. PER CU. FT. (MINIMUM)

MATERIAL AND WORKMANSHIP:

EXCEPT AS MAY OTHERWISE BE SPECIFIED ON PLANS OR IN THE SPECIAL PROVISIONS, ALL MATERIAL AND WORKMANSHIP SHALL BE IN ACCORDANCE WITH THE 2024 "STANDARD SPECIFICATIONS FOR ROADS AND STRUCTURES" OF THE N. C. DEPARTMENT OF TRANSPORTATION.

STEEL SHEET PILING FOR PERMANENT OR TEMPORARY APPLICATIONS SHALL BE HOT ROLLED.

CONCRETE:

UNLESS OTHERWISE REQUIRED ON PLANS, CLASS A CONCRETE SHALL BE USED FOR ALL PORTIONS OF ALL STRUCTURES WITH THE EXCEPTION THAT: CLASS AA CONCRETE SHALL BE USED IN BRIDGE SUPERSTRUCTURES, ABUTMENT BACKWALLS, AND APPROACH SLABS; AND CLASS B CONCRETE SHALL BE USED FOR SLOPE PROTECTION AND RIP RAP.

CONCRETE CHAMFERS:

UNLESS OTHERWISE NOTED ON THE PLANS, ALL EXPOSED CORNERS ON STRUCTURES SHALL BE CHAMFERED 3/4" WITH THE FOLLOWING EXCEPTIONS: TOP CORNERS OF CURBS MAY BE ROUNDED TO 1 1/2" RADIUS WHICH IS BUILT INTO CURB FORMS; CORNERS OF TRANSVERSE FLOOR EXPANSION JOINTS SHALL BE ROUNDED WITH A 1/4" FINISHING TOOL UNLESS OTHERWISE REQUIRED ON PLANS; AND CORNERS OF EXPANSION JOINTS IN THE ROADWAY FACES AND TOPS OF CURBS AND SIDEWALKS SHALL BE ROUNDED TO A 1/4" RADIUS WITH A FINISHING STONE OR TOOL UNLESS OTHERWISE REQUIRED ON PLANS.

DOWELS:

DOWELS WHEN INDICATED ON PLANS AS FOR CULVERT EXTENSIONS, SHALL BE EMBEDDED AT LEAST 12" INTO THE OLD CONCRETE AND GROUTED INTO PLACE WITH 1:2 CEMENT MORTAR.

ALLOWANCE FOR DEAD LOAD DEFLECTION, SETTLEMENT, ETC. IN CASTING SUPERSTRUCTURES:

BRIDGES SHALL BE BUILT ON THE GRADE OR VERTICAL CURVE SHOWN ON PLANS. SLABS, CURBS AND PARAPETS SHALL CONFORM TO THE GRADE OR CURVE.

ALL DIMENSIONS WHICH ARE GIVEN IN SECTION AND ARE AFFECTED BY DEAD LOAD DEFLECTIONS ARE DIMENSIONS AT CENTER LINE OF BEARING UNLESS OTHERWISE NOTED ON PLANS. IN SETTING FORMS FOR STEEL BEAM BRIDGES AND PRESTRESSED CONCRETE GIRDER BRIDGES, ADJUSTMENTS SHALL BE MADE DUE TO THE DEAD LOAD DEFLECTIONS FOR THE ELEVATIONS SHOWN. WHERE BLOCKS ARE SHOWN OVER BEAMS FOR BUILDING UP TO THE SLAB, THE VERTICAL DIMENSIONS OF THE BLOCKS SHALL BE ADJUSTED BETWEEN BEARINGS TO COMPENSATE FOR DEAD LOAD DEFLECTIONS, VERTICAL CURVE ORDINATE, AND ACTUAL BEAM CAMBER. WHERE BOTTOM OF SLAB IS IN LINE WITH BOTTOM OF TOP FLANGES, DEPTH OF SLAB BETWEEN BEARINGS SHALL BE ADJUSTED TO COMPENSATE FOR DEAD LOAD DEFLECTION, VERTICAL CURVE ORDINATE, AND ACTUAL BEAM CAMBER.

IN SETTING FALSEWORK AND FORMS FOR REINFORCED CONCRETE SPANS, AN ALLOWANCE SHALL BE MADE FOR DEAD LOAD DEFLECTIONS, SETTLEMENT OF FALSEWORK, AND PERMANENT CAMBER WHICH SHALL BE PROVIDED FOR IN ADDITION TO THE ELEVATIONS SHOWN. AFTER REMOVAL OF THE FALSEWORK, THE FINISHED STRUCTURES SHALL CONFORM TO THE PROFILE AND ELEVATIONS SHOWN ON THE PLANS AND CONSTRUCTION ELEVATIONS FURNISHED BY THE ENGINEER.

DETAILED DRAWINGS FOR FALSEWORK OR FORMS FOR BRIDGE SUPERSTRUCTURE AND ANY STRUCTURE OR PARTS OF A STRUCTURE AS NOTED ON THE PLANS SHALL BE SUBMITTED TO THE ENGINEER FOR APPROVAL BEFORE CONSTRUCTION OF THE FALSEWORK OR FORMS IS STARTED.

REINFORCING STEEL:

ALL REINFORCING STEEL SHALL BE DEFORMED. DIMENSIONS RELATIVE TO PLACEMENT OF REINFORCING ARE TO CENTERS OF BARS UNLESS OTHERWISE INDICATED IN THE PLANS. DIMENSIONS ON BAR DETAILS ARE TO CENTERS OF BARS OR ARE OUT TO OUT AS INDICATED ON PLANS.

WIRE BAR SUPPORTS SHALL BE PROVIDED FOR REINFORCING STEEL WHERE INDICATED ON THE PLANS. WHEN BAR SUPPORT PIECES ARE PLACED IN CONTINUOUS LINES, THEY SHALL BE SO PLACED THAT THE ENDS OF THE SUPPORTING WIRES SHALL BE LAPPED TO LOCK LEGS ON ADJOINING PIECES.

STRUCTURAL STEEL:

AT THE CONTRACTOR'S OPTION, HE MAY SUBSTITUTE 7/8" Ø SHEAR STUDS FOR THE 3/4" Ø STUDS SPECIFIED ON THE PLANS. THIS SUBSTITUTION SHALL BE MADE AT THE RATE OF 3 - 7/8" Ø STUDS FOR 4 - 3/4" Ø STUDS, AND STUD SPACING CHANGES SHALL BE MADE AS NECESSARY TO PROVIDE THE SAME EQUIVALENT NUMBER OF 7/8" Ø STUDS ALONG THE BEAM AS SHOWN FOR 3/4" Ø STUDS BASED ON THE RATIO OF 3 - 7/8"Ø STUDS FOR 4 - 3/4" Ø STUDS. STUDS OF THE LENGTH SPECIFIED ON THE PLANS MUST BE PROVIDED. THE MAXIMUM SPACING SHALL BE 2'-0".

EXCEPT AT THE INTERIOR SUPPORTS OF CONTINUOUS BEAMS WHERE THE COVER PLATE IS IN CONTACT WITH BEARING PLATE, THE CONTRACTOR MAY, AT HIS OPTION, SUBSTITUTE FOR THE COVER PLATES DESIGNATED ON THE PLANS COVER PLATES OF THE EQUIVALENT AREA PROVIDED THESE PLATES ARE AT LEAST 5/16" IN THICKNESS AND DO NOT EXCEED A WIDTH EQUAL TO THE FLANGE WIDTH LESS 2" OR A THICKNESS EQUAL TO 2 TIMES THE FLANGE THICKNESS. THE SIZE OF FILLET WELDS SHALL CONFORM TO THE REQUIREMENTS OF THE CURRENT ANSI/AASHTO/AWS "BRIDGE WELDING CODE". ELECTROSLAG WELDING WILL NOT BE PERMITTED.

WITH THE SOLE EXCEPTION OF EDGES AT SURFACES WHICH BEAR ON OTHER SURFACES, ALL SHARP EDGES AND ENDS OF SHAPES AND PLATES SHALL BE SLIGHTLY ROUNDED BY SUITABLE MEANS TO A RADIUS OF APPROXIMATELY 1/16" OR EQUIVALENT FLAT SURFACE AT A SUITABLE ANGLE PRIOR TO PAINTING, GALVANIZING, OR METALLIZING.

HANDRAILS AND POSTS:

METAL STANDARDS AND FACES OF THE CONCRETE END POSTS FOR THE METAL RAIL SHALL BE SET NORMAL TO THE GRADE OF THE CURB, UNLESS OTHERWISE SHOWN ON PLANS. THE METAL RAIL AND TOPS OF CONCRETE POSTS USED WITH THE ALUMINUM RAIL SHALL BE BUILT PARALLEL TO THE GRADE OF THE CURB.

METAL HANDRAILS SHALL BE IN ACCORDANCE WITH THE PLANS. RAILS SHALL BE AS MANUFACTURED FOR BRIDGE RAILING. CASTINGS SHALL BE OF A UNIFORM APPEARANCE. FINIS AND OTHER DEFORMATIONS RESULTING FROM CASTING OR OTHERWISE SHALL BE REMOVED IN A MANNER SO THAT A UNIFORM COLORING OF THE COMPLETED CASTING SHALL BE OBTAINED. CASTINGS WITH DISCOLORATIONS OR OF NON-UNIFORM COLORING WILL NOT BE ACCEPTED. CERTIFIED MILL REPORTS ARE REQUIRED FOR METAL RAILS AND POSTS.

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