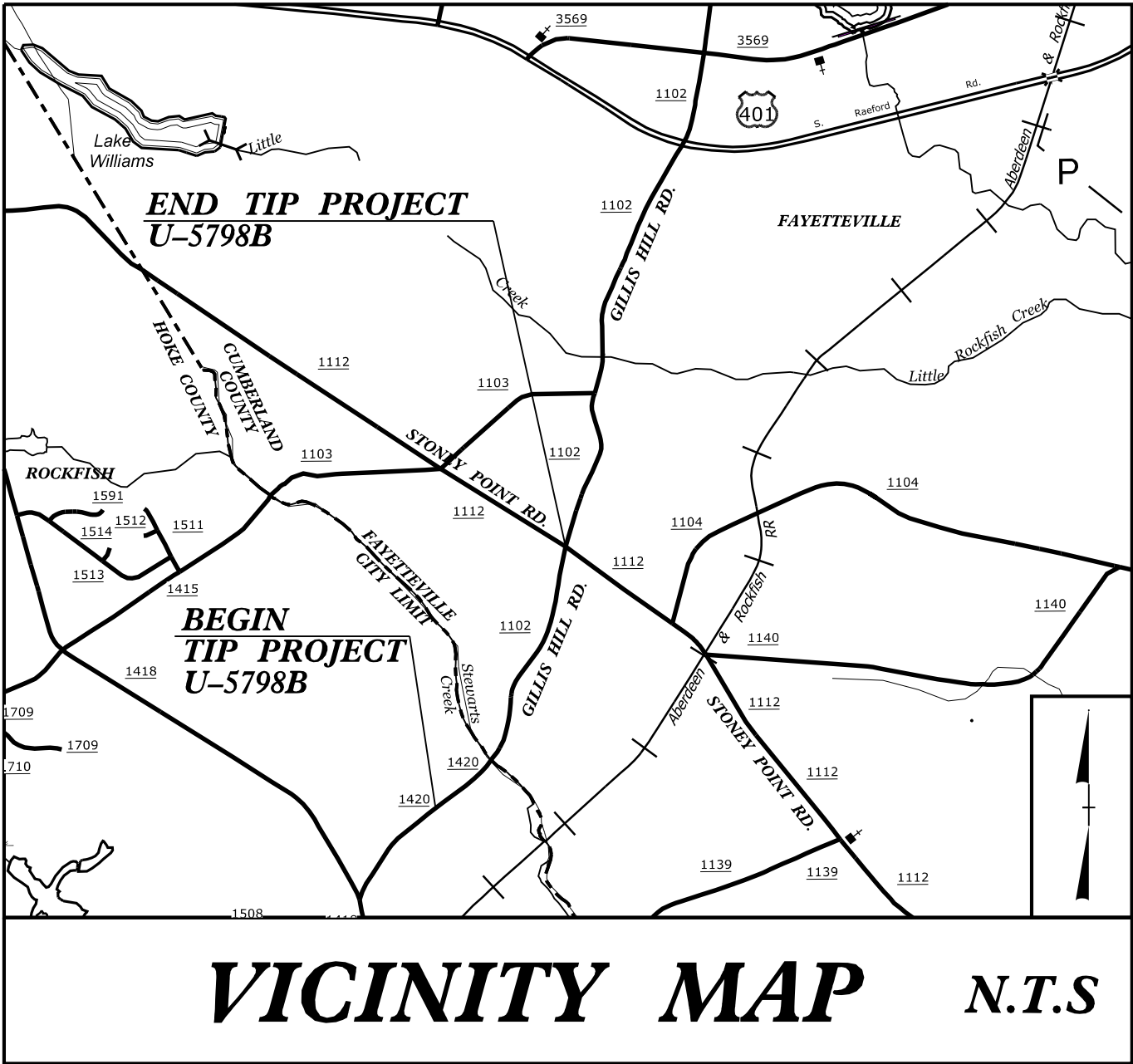


09.08/25

05-MAR-2025 14:33  
R:\Roadway\Proj\U-5798B\U-5798B\_Rdy.tsh.dgn  
\$\$\$\$\$USERNAME\$\$\$\$\$

TIP PROJECT: U-5798B

CONTRACT: C205076



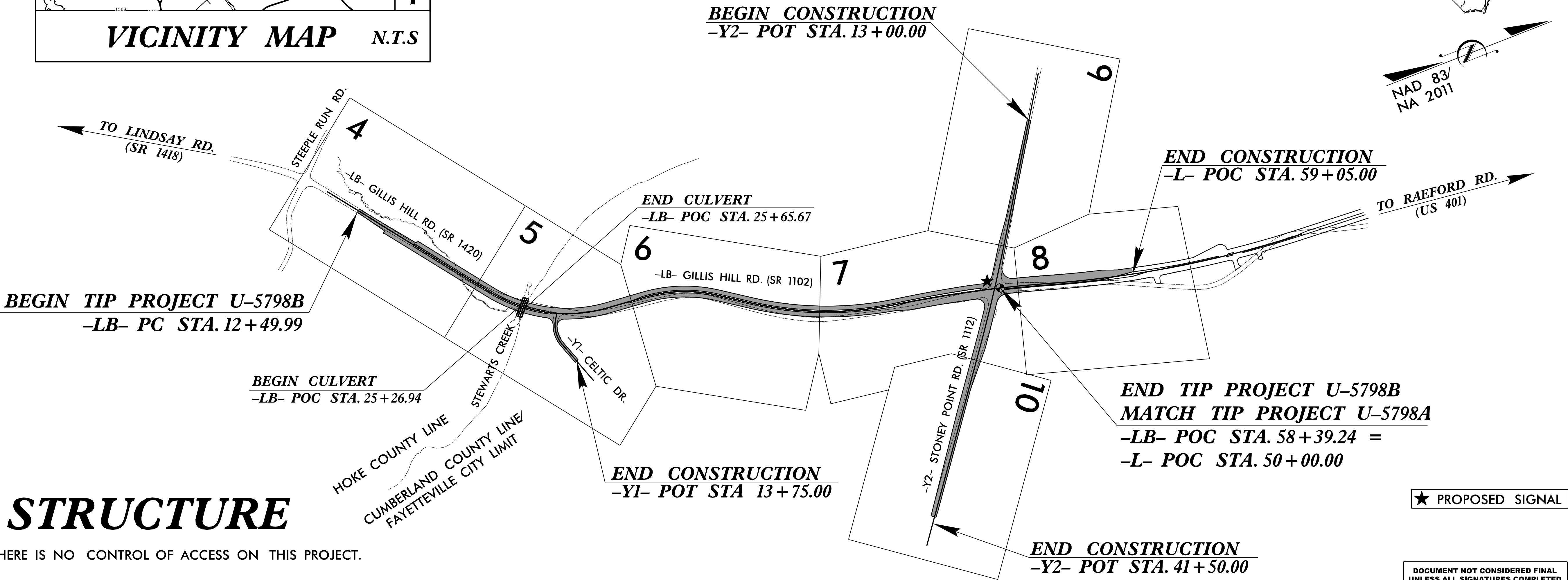
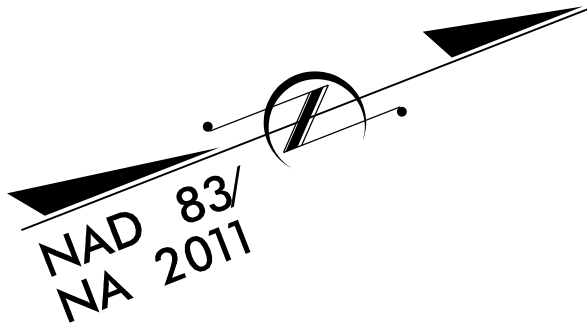
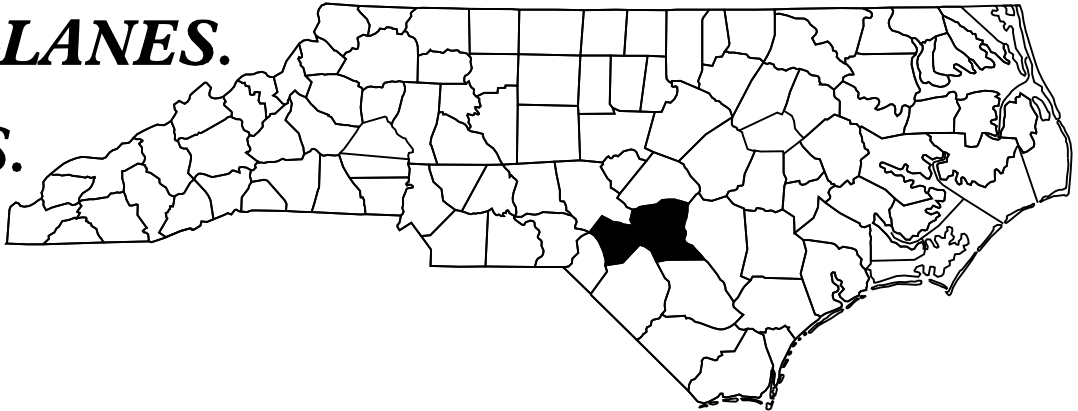
STATE OF NORTH CAROLINA  
DIVISION OF HIGHWAYS

HOKE AND CUMBERLAND  
COUNTIES

LOCATION: SR 1102 (GILLIS HILL ROAD) FROM SR 1418 (LINDSAY ROAD)  
TO NORTH OF SR 1112 (STONEY POINT ROAD). WIDEN TO MULTI-LANES.

TYPE OF WORK: PAVING, GRADING, DRAINAGE, CULVERT, WALLS, AND SIGNALS.

| STATE           | STATE PROJECT REFERENCE NO. | SHEET NO.   | TOTAL SHEETS |
|-----------------|-----------------------------|-------------|--------------|
| N.C.            | U-5798B                     | 1           |              |
| STATE PROJ. NO. | F.A. PROJ. NO.              | DESCRIPTION |              |
| 44369.1.3       | N/A                         | PE          |              |
| 44369.2.3       | N/A                         | ROW         |              |
| 44369.2.4       | N/A                         | UTL.        |              |
| 44369.3.3       | N/A                         | CONST.      |              |
|                 |                             |             |              |
|                 |                             |             |              |
|                 |                             |             |              |



STRUCTURE

THERE IS NO CONTROL OF ACCESS ON THIS PROJECT.

DESIGN DATA

ADT 2025 = 10,800  
ADT 2045 = 17,500  
K = 7 %  
D = 55 %  
T = 3 % \*  
V = 50 MPH  
(TTST=1 + DUAL=2)  
FUNC CLASS =  
MINOR COLLECTOR  
REGIONAL TIER

PROJECT LENGTH

LENGTH ROADWAY TIP PROJECT U-5798B = 0.862 MILES  
LENGTH CULVERT TIP PROJECT U-5798B = 0.007 MILES  
TOTAL LENGTH TIP PROJECT U-5798B = 0.869 MILES

-LB- USED TO CALCULATE PROJECT LENGTH.

PREPARED IN THE OFFICE OF:

**RS&H**

8521 SIX FORKS ROAD, SUITE 400  
RALEIGH, NC 27615  
NC FIRM LICENSE No: F-0493

FOR THE NORTH CAROLINA DEPARTMENT OF TRANSPORTATION

2024 STANDARD SPECIFICATIONS

RIGHT OF WAY DATE:  
JUNE 13, 2023

LETTING DATE:  
AUGUST 19, 2025

SEAN KORTOVICH, PE  
PROJECT ENGINEER

DANA M. PACZEK, PE  
PROJECT DESIGN ENGINEER

KHALED A. ALAKHDAR  
NCDOT CONTACT

STRUCTURAL ENGINEER

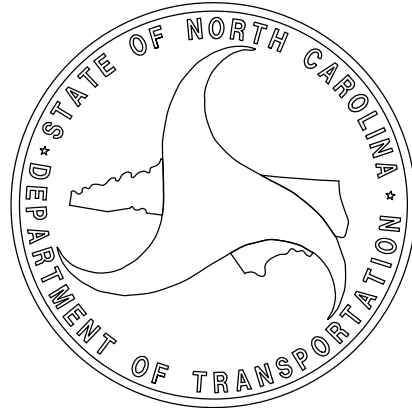


Signed by:  
Matthew R. Acosta  
SIGNATURE:

6/19/2025  
P.E.

★ PROPOSED SIGNAL

DOCUMENT NOT CONSIDERED FINAL  
UNLESS ALL SIGNATURES COMPLETED



8/4/2025  
\\rsandh.com\files\Transportation\PI\1031782004\_U-5798 (Gillis Hill Road).P&D\Design\Structures\CAD\Culvert\410\_001\_U-5798B\_SMU\_CU\_001\_250001.dgn  
Acostam

LOAD FACTORS:

DESIGN LOAD RATING FACTORS

| LOAD TYPE | MAX FACTOR | MIN FACTOR |
|-----------|------------|------------|
| DC        | 1.25       | 0.90       |
| DW        | 1.50       | 0.65       |
| EV        | 1.30       | 0.90       |
| EH        | 1.35       | 0.90       |
| ES        | 1.35       | 0.90       |
| LS        | 1.75       | --         |
| WA        | 1.00       | --         |

NOTE:

RATING FACTORS ARE BASED ON THE STRENGTH I LIMIT STATE.

LOAD RATINGS FOR MAXIMUM AND MINIMUM FILL CONDITIONS HAVE BEEN EVALUATED. MINIMUM FILL CONDITION CONTROL LOAD RATINGS.

COMMENTS:

1.
2.
3.
4.

|                               |                                   |
|-------------------------------|-----------------------------------|
| #                             | CONTROLLING LOAD RATING           |
| 1                             | DESIGN LOAD RATING (HL-93)        |
| 2                             | DESIGN LOAD RATING (HS-20)        |
| 3                             | LEGAL LOAD RATING **              |
| 4                             | EMERGENCY VERHICLE LOAD RATING ** |
| ** SEE CHART FOR VEHICLE TYPE |                                   |

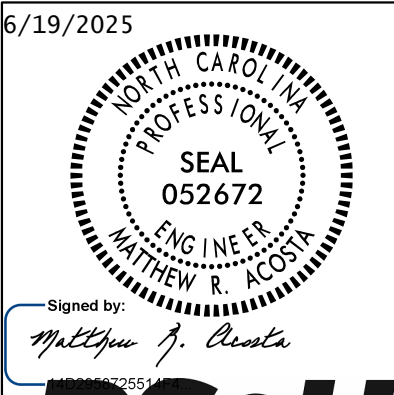
PROJECT NO. U-5798B

CUMBERLAND COUNTY

STATION: 25+46.30 -LB-

SHEET 2 OF 8

6/19/2025



Signed by:  
*Matthew J. Acosta*  
Matthew J. Acosta  
Professional Engineer  
License No. 052672

**RS&H**  
RS&H Architects-Engineers-Planners, Inc.  
8521 Six Forks Road, Suite 400  
Raleigh, NC 27615  
919-926-4100 FAX 919-846-9080  
www.rsandh.com  
North Carolina License Nos. 50073-F-6403 • C-28

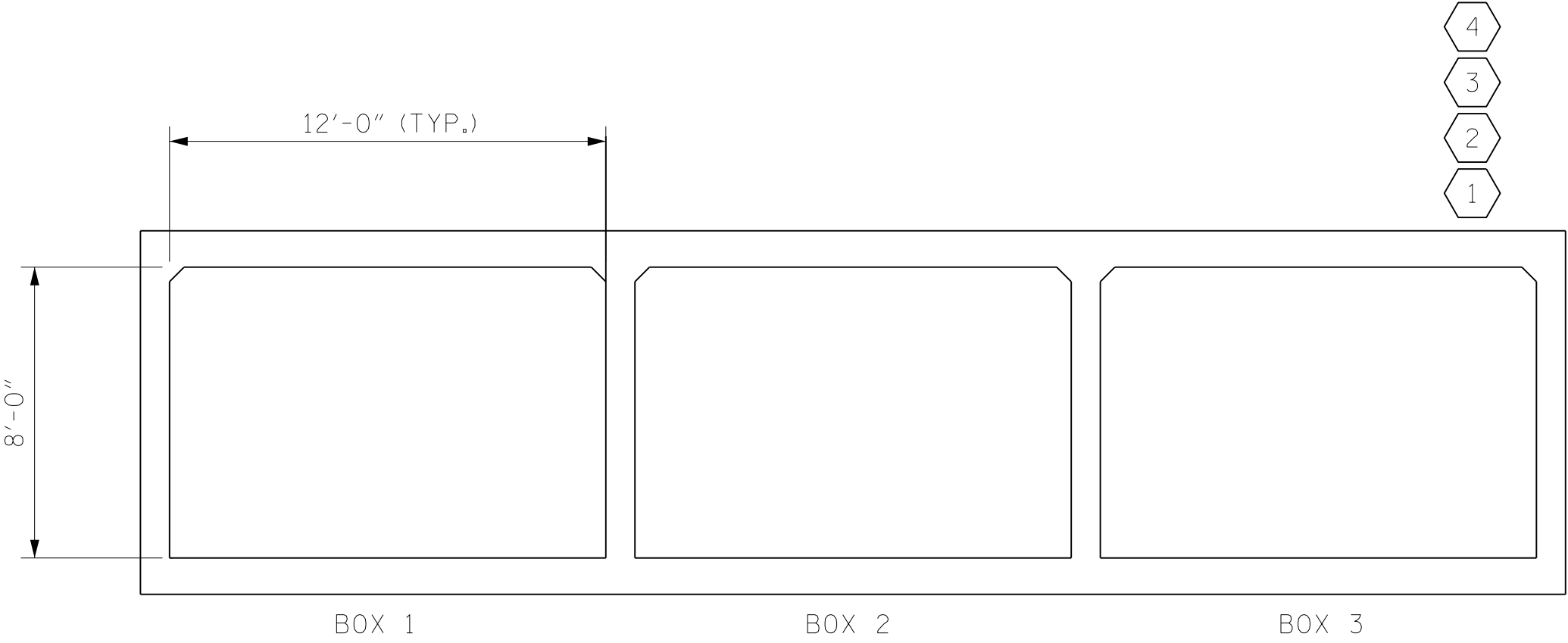
STATE OF NORTH CAROLINA  
DEPARTMENT OF TRANSPORTATION  
RALEIGH

LRFR SUMMARY FOR  
REINFORCED CONCRETE  
BOX CULVERTS  
(NON-INTERSTATE TRAFFIC)

| REVISIONS |     |       |     |     |       | SHEET NO.    |
|-----------|-----|-------|-----|-----|-------|--------------|
| NO.       | BY: | DATE: | NO. | BY: | DATE: | C-2          |
| 1         |     |       | 3   |     |       | TOTAL SHEETS |
| 2         |     |       | 4   |     |       | 8            |

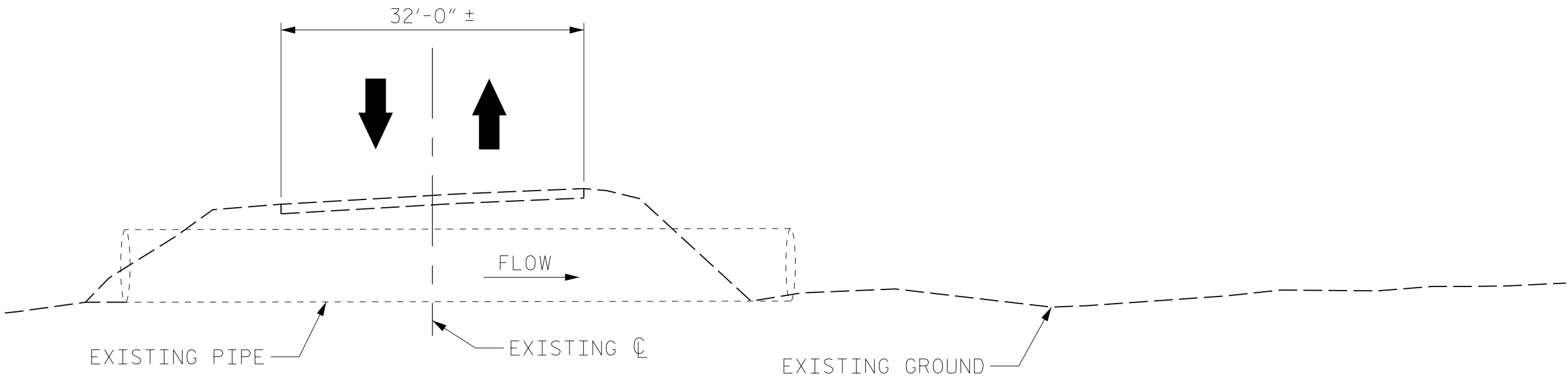
DOCUMENT NOT CONSIDERED  
FINAL UNLESS ALL  
SIGNATURES COMPLETED

| LOAD AND RESISTANCE FACTOR RATING (LRFR)<br>SUMMARY FOR REINFORCED CONCRETE BOX CULVERTS |                                      |                      |                            |                                   |               |                                         |               |         |                 |                                              |               |         |                 |         |        |                                              |
|------------------------------------------------------------------------------------------|--------------------------------------|----------------------|----------------------------|-----------------------------------|---------------|-----------------------------------------|---------------|---------|-----------------|----------------------------------------------|---------------|---------|-----------------|---------|--------|----------------------------------------------|
| LEVEL                                                                                    | VEHICLE                              | WEIGHT (W)<br>(TONS) | CONTROLLING<br>LOAD RATING | MINIMUM<br>RATING FACTORS<br>(RF) | TONS = W × RF | STRENGTH I LIMIT STATE                  |               |         |                 |                                              |               |         |                 | COMMENT | NUMBER |                                              |
|                                                                                          |                                      |                      |                            |                                   |               | LIVE-LOAD<br>FACTORS (γ <sub>LL</sub> ) | MOMENT        |         |                 |                                              | SHEAR         |         |                 |         |        |                                              |
|                                                                                          |                                      |                      |                            |                                   |               |                                         | RATING FACTOR | BOX NO. | ELEMENT<br>TYPE | DISTANCE FROM<br>LEFT END OF<br>ELEMENT (ft) | RATING FACTOR | BOX NO. | ELEMENT<br>TYPE |         |        | DISTANCE FROM<br>LEFT END OF<br>ELEMENT (ft) |
| DESIGN<br>LOAD<br>RATING                                                                 | HL-93 (INVENTORY)                    | N/A                  | 1                          | 1.05                              | --            | 1.75                                    | 1.82          | 3       | TOP SLAB        | 4.73                                         | 1.05          | 3       | TOP SLAB        | 11.39   |        |                                              |
|                                                                                          | HL-93 (OPERATING)                    | N/A                  |                            | 1.36                              | --            | 1.35                                    | 2.37          | 3       | TOP SLAB        | 4.73                                         | 1.36          | 3       | TOP SLAB        | 11.39   |        |                                              |
|                                                                                          | HS-20 (INVENTORY)                    | 36,000               | 2                          | 1.21                              | 43,560        | 1.75                                    | 2.04          | 3       | TOP SLAB        | 4.73                                         | 1.21          | 3       | TOP SLAB        | 11.39   |        |                                              |
|                                                                                          | HS-20 (OPERATING)                    | 36,000               |                            | 1.56                              | 56,160        | 1.35                                    | 2.64          | 3       | TOP SLAB        | 4.73                                         | 1.56          | 3       | TOP SLAB        | 11.39   |        |                                              |
| LEGAL<br>LOAD<br>RATING                                                                  | SINGLE VEHICLE<br>(SV)               | SN5H                 |                            | 3.36                              | 45,360        | 1.40                                    | 3.74          | 3       | EXTERIOR WALL   | 8.63                                         | 3.36          | 3       | TOP SLAB        | 11.39   |        |                                              |
|                                                                                          |                                      | SNGARBS2             |                            | 2.83                              | 56,600        | 1.40                                    | 3.60          | 3       | EXTERIOR WALL   | 8.63                                         | 2.83          | 3       | TOP SLAB        | 11.39   |        |                                              |
|                                                                                          |                                      | SNAGRIS2             |                            | 2.98                              | 65,560        | 1.40                                    | 3.72          | 3       | EXTERIOR WALL   | 8.63                                         | 2.98          | 3       | TOP SLAB        | 11.39   |        |                                              |
|                                                                                          |                                      | SNCOTTS3             | 3                          | 1.45                              | 39,513        | 1.40                                    | 2.44          | 3       | TOP SLAB        | 4.73                                         | 1.45          | 3       | TOP SLAB        | 11.39   |        |                                              |
|                                                                                          |                                      | SNAGGRS4             |                            | 1.74                              | 60,770        | 1.40                                    | 2.77          | 3       | EXTERIOR WALL   | 8.63                                         | 1.74          | 3       | TOP SLAB        | 11.39   |        |                                              |
|                                                                                          |                                      | SNS5A                |                            | 1.64                              | 58,302        | 1.40                                    | 2.72          | 3       | EXTERIOR WALL   | 8.63                                         | 1.64          | 3       | TOP SLAB        | 11.39   |        |                                              |
|                                                                                          |                                      | SNS6A                |                            | 1.62                              | 64,719        | 1.40                                    | 2.72          | 3       | EXTERIOR WALL   | 8.63                                         | 1.62          | 3       | TOP SLAB        | 11.39   |        |                                              |
|                                                                                          |                                      | SNS7B                |                            | 1.61                              | 67,620        | 1.40                                    | 2.84          | 3       | EXTERIOR WALL   | 8.63                                         | 1.61          | 3       | TOP SLAB        | 11.39   |        |                                              |
|                                                                                          | TRUCK TRACTOR SEMI-TRAILER<br>(TTST) | TNAGRIT3             |                            | 2.49                              | 82,170        | 1.40                                    | 3.78          | 3       | EXTERIOR WALL   | 8.63                                         | 2.49          | 3       | TOP SLAB        | 11.39   |        |                                              |
|                                                                                          |                                      | TNT4A                |                            | 1.73                              | 57,220        | 1.40                                    | 2.80          | 3       | EXTERIOR WALL   | 8.63                                         | 1.73          | 3       | TOP SLAB        | 11.39   |        |                                              |
|                                                                                          |                                      | TNT6A                |                            | 1.59                              | 66,144        | 1.40                                    | 2.89          | 3       | EXTERIOR WALL   | 8.63                                         | 1.59          | 3       | TOP SLAB        | 11.39   |        |                                              |
|                                                                                          |                                      | TNT7A                |                            | 1.62                              | 68,040        | 1.40                                    | 2.82          | 3       | EXTERIOR WALL   | 8.63                                         | 1.62          | 3       | TOP SLAB        | 11.39   |        |                                              |
|                                                                                          |                                      | TNT7B                |                            | 1.71                              | 71,820        | 1.40                                    | 2.78          | 3       | EXTERIOR WALL   | 8.63                                         | 1.71          | 3       | TOP SLAB        | 11.39   |        |                                              |
|                                                                                          |                                      | TNAGRIT4             |                            | 1.64                              | 70,520        | 1.40                                    | 2.80          | 3       | EXTERIOR WALL   | 8.63                                         | 1.64          | 3       | TOP SLAB        | 11.39   |        |                                              |
|                                                                                          |                                      | TNAGT5A              |                            | 1.62                              | 72,900        | 1.40                                    | 3.06          | 3       | TOP SLAB        | 12.33                                        | 1.62          | 3       | TOP SLAB        | 11.39   |        |                                              |
|                                                                                          |                                      | TNAGT5B              |                            | 1.55                              | 69,750        | 1.40                                    | 2.60          | 3       | TOP SLAB        | 12.33                                        | 1.55          | 3       | TOP SLAB        | 11.39   |        |                                              |
| EMERGENCY<br>VEHICLE (EV)                                                                | EV2                                  | 28,750               |                            | 1.95                              | 56,063        | 1.30                                    | 2.87          | 3       | TOP SLAB        | 4.73                                         | 1.95          | 3       | TOP SLAB        | 11.39   |        |                                              |
|                                                                                          | EV3                                  | 43,000               | 4                          | 1.20                              | 51,600        | 1.30                                    | 2.12          | 3       | TOP SLAB        | 4.73                                         | 1.20          | 3       | TOP SLAB        | 11.39   |        |                                              |



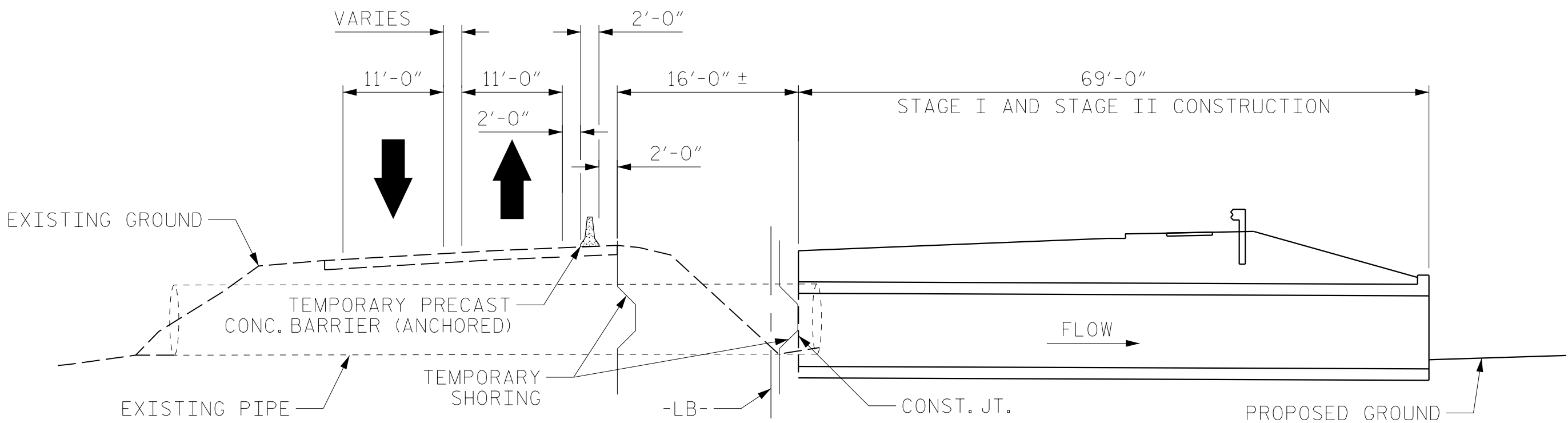
LRFR SUMMARY  
(LOOKING DOWNSTREAM)

|                            |     |        |         |
|----------------------------|-----|--------|---------|
| DRAWN BY :                 | NSC | DATE : | 12/2023 |
| CHECKED BY :               | MRA | DATE : | 12/2023 |
| DESIGN ENGINEER OF RECORD: | MRA | DATE : | 03/2025 |



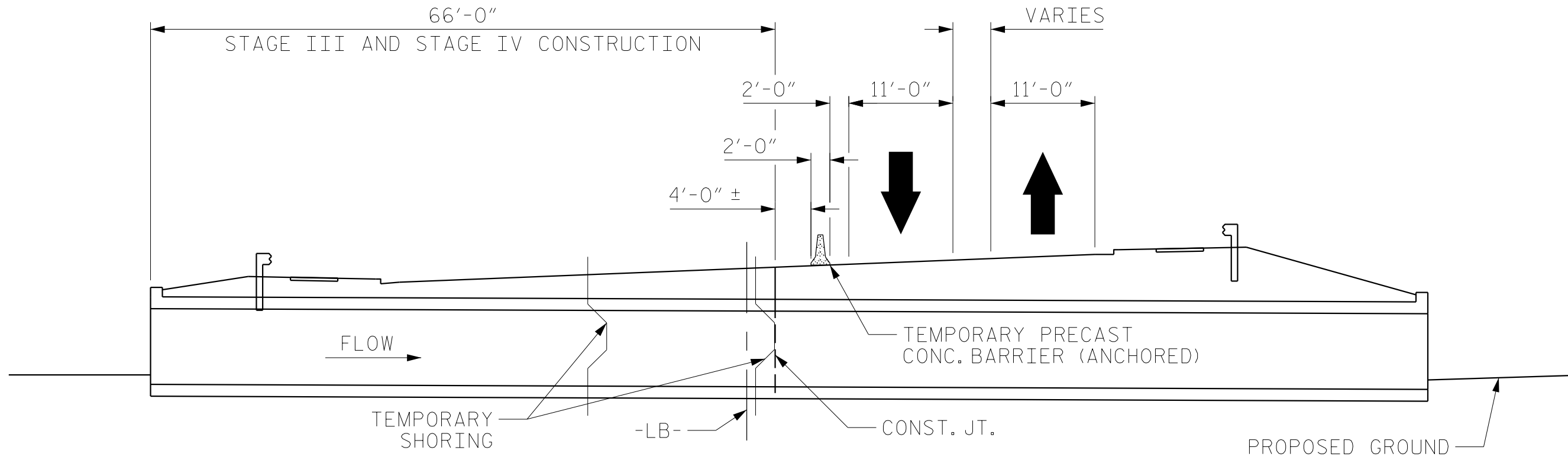
INITIAL CONDITION

(LOOKING UPSTATION)  
(SECTION ALONG C/L CULVERT)



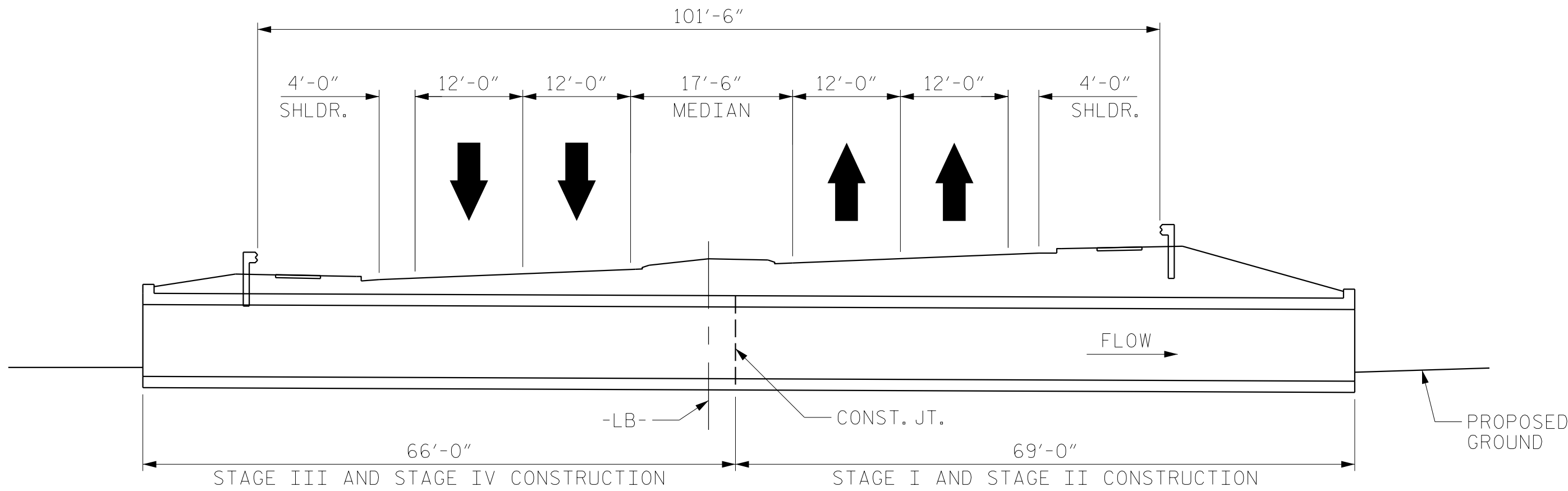
STAGE I AND STAGE II CONSTRUCTION

(LOOKING UPSTATION)  
(SECTION ALONG C/L CULVERT)



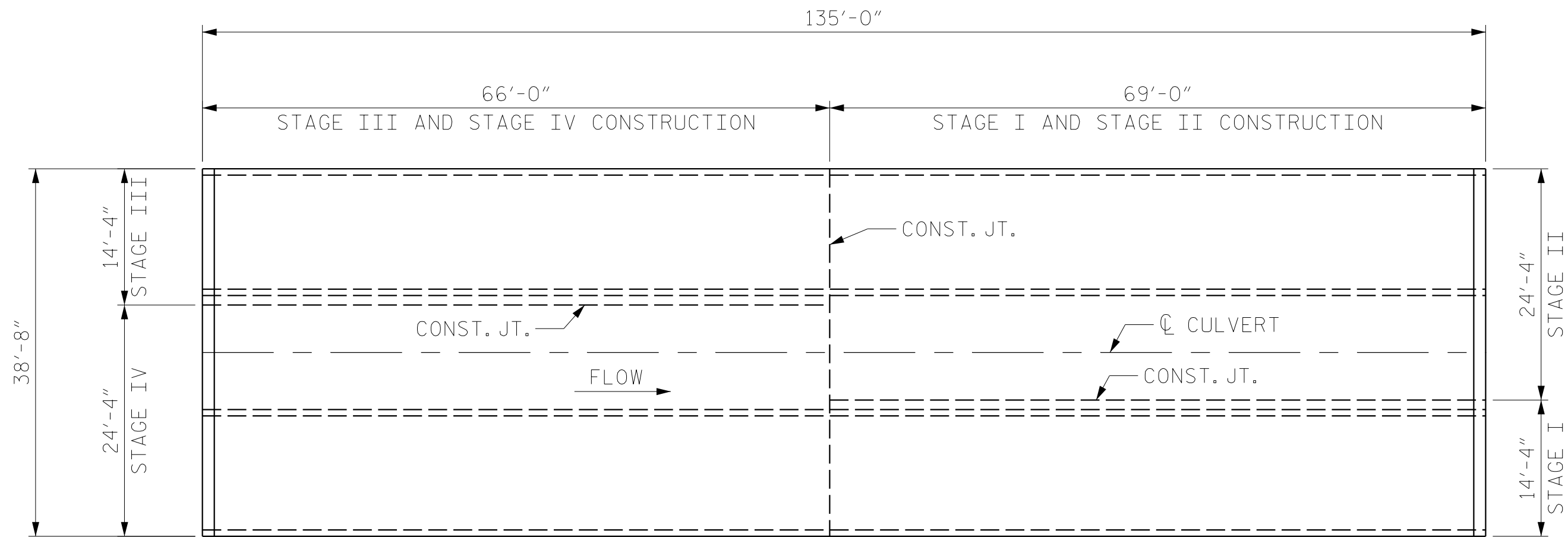
STAGE III AND STAGE IV CONSTRUCTION

(LOOKING UPSTATION)  
(SECTION ALONG C/L CULVERT)



FINAL CONDITION

(LOOKING UPSTATION)  
(SECTION ALONG C/L CULVERT)



PLAN VIEW

(WINGS NOT SHOWN FOR CLARITY)

DRAWN BY : NSC DATE : 12/2023  
CHECKED BY : MRA DATE : 12/2023  
DESIGN ENGINEER OF RECORD: MRA DATE : 03/2025

6/19/2025  
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AcostoM

NOTES:

FOR MAINTENANCE OF TRAFFIC, SEE TRANSPORTATION MANAGEMENT PLAN.

FOR LIMITS OF TEMPORARY SHORING FOR MAINTENANCE OF TRAFFIC, SEE TRAFFIC CONTROL PLANS.

FOR PAY ITEM FOR TEMPORARY SHORING FOR MAINTENANCE OF TRAFFIC, SEE ROADWAY PLANS.

DOCUMENT NOT CONSIDERED  
FINAL UNLESS ALL  
SIGNATURES COMPLETED

PROJECT NO. U-5798B  
CUMBERLAND COUNTY  
STATION: 25+46.30 -LB-

SHEET 3 OF 8

6/19/2025

STATE OF NORTH CAROLINA

DEPARTMENT OF TRANSPORTATION

RALEIGH

TRIPLE 12'-0" X 8'-0"  
CONCRETE BOX CULVERT  
STAGING DETAILS

90° SKEW

NO. BY: DATE:

1 2

NO. BY: DATE:

3 4

NO. BY: DATE:

5 6

REVISIONS

SHEET NO.

C-3

TOTAL SHEETS

8

Seal

Professional Engineer

052672

ANTHONY R. ACCO

Signed by

Matthew J. Resnik

6/19/2025

RS&H

RS&H Architects-Engineers-Planners, Inc.

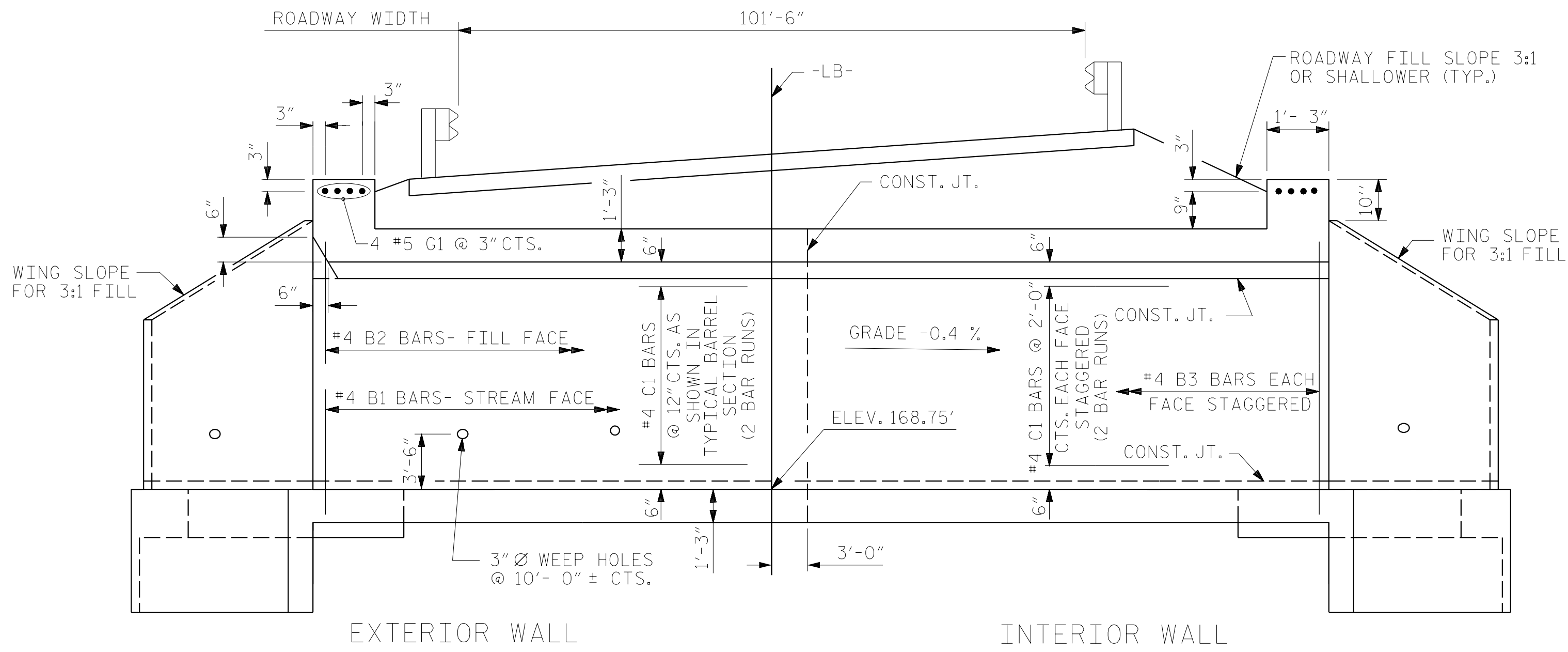
8521 Six Forks Road, Suite 400

Raleigh, NC 27615

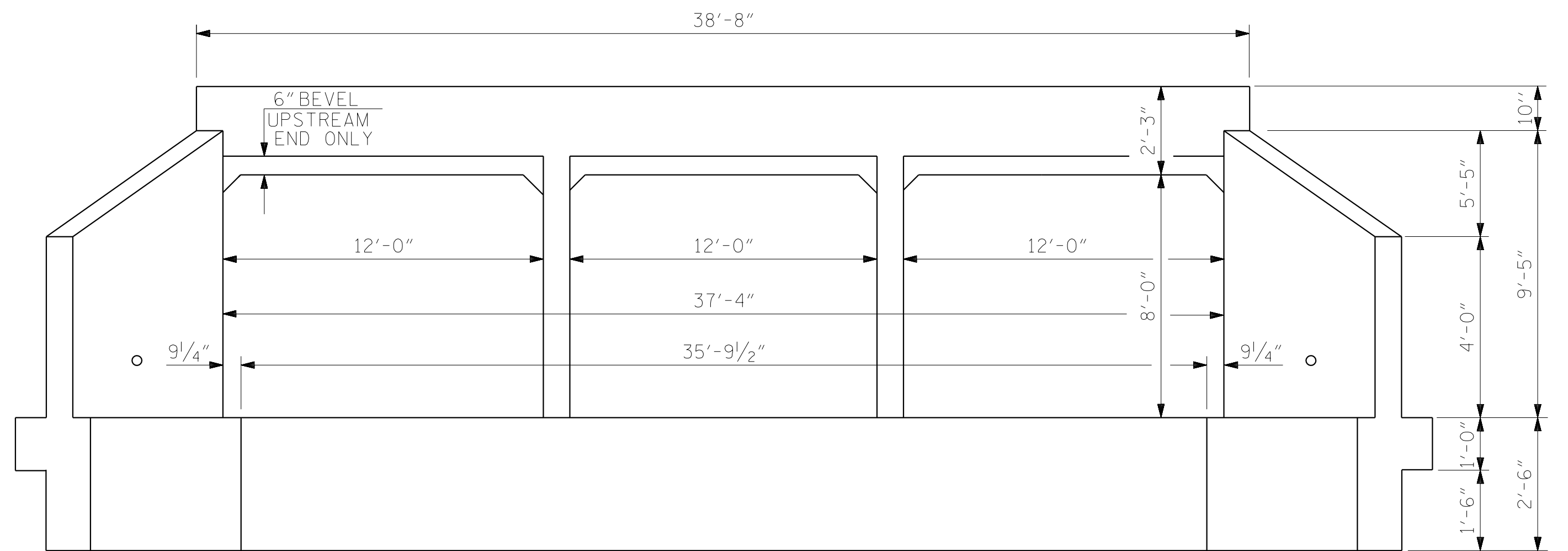
919-926-4100 FAX 919-846-9080

www.rsandh.com

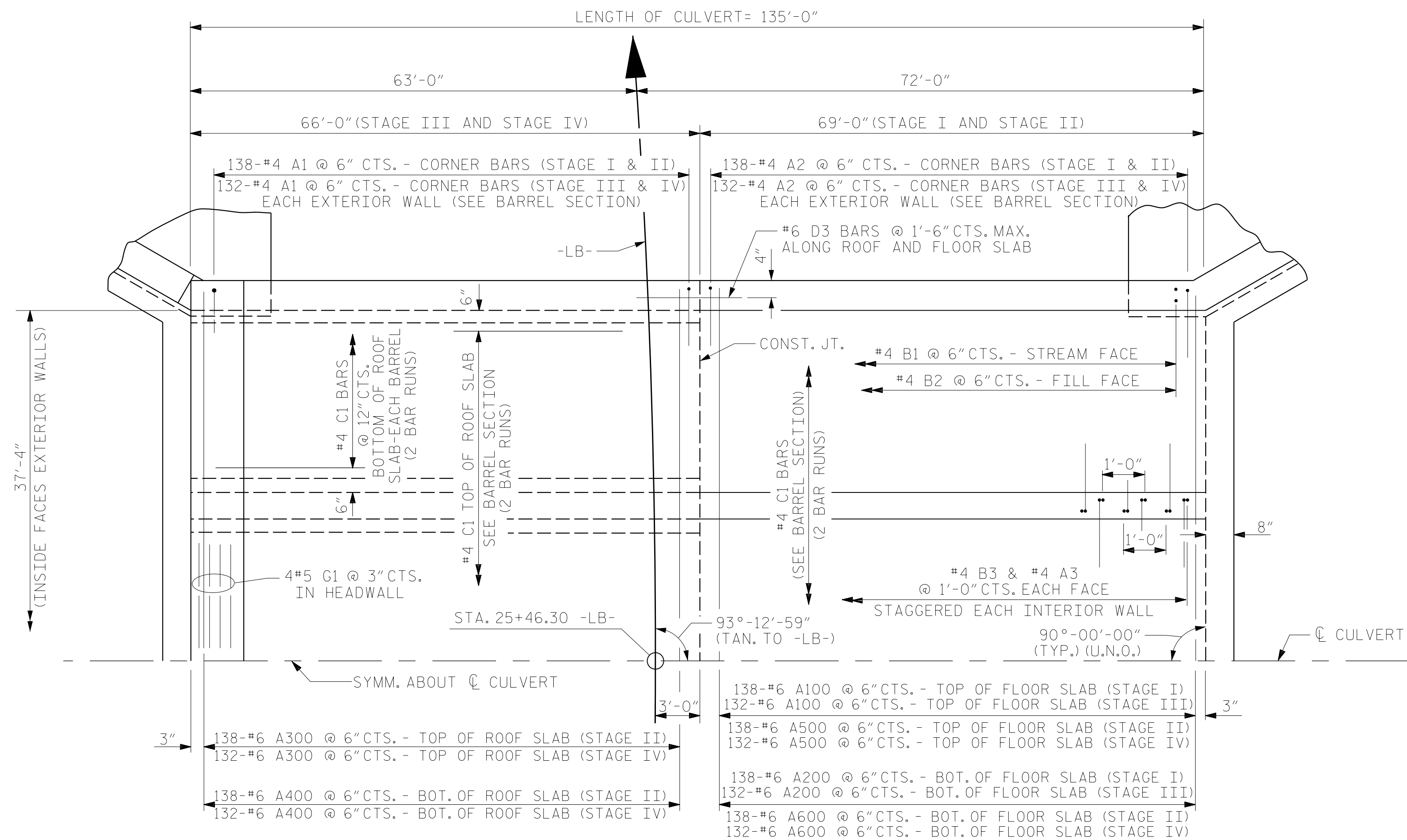
North Carolina License Nos. 50073-F-5493-C-28



CULVERT SECTION NORMAL TO ROADWAY

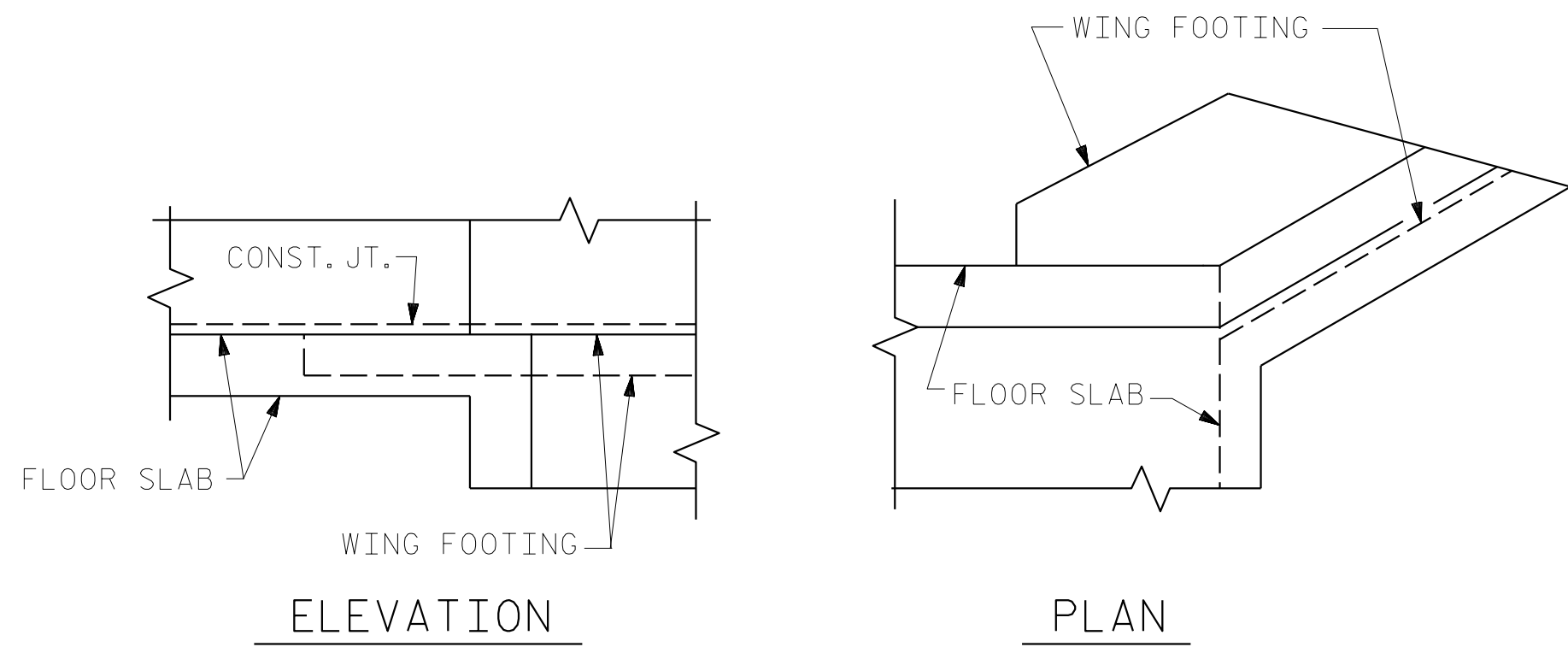


END ELEVATION



PART PLAN - ROOF SLAB

PART PLAN - FLOOR SLAB



CONNECTION OF WING FOOTING AND FLOOR SLAB WHEN SLAB IS THICKER THAN FOOTING

PROJECT NO. U-5798B  
CUMBERLAND COUNTY  
STATION: 25+46.30 -LB-

SHEET 4 OF 8

|                |     |        |         |
|----------------|-----|--------|---------|
| ASSEMBLED BY : | NSC | DATE : | 12/2023 |
| CHECKED BY :   | MRA | DATE : | 12/2023 |
| DRAWN BY :     | TSS | REV.   | 6/19    |
| CHECKED BY :   | ARB |        | MAA/THC |

NOTE: ROOF AND FLOOR SLAB REINFORCING IS SHOWN SCHEMATICALLY PER STAGE IN 'PART PLAN - ROOF SLAB' AND 'PART PLAN - FLOOR SLAB'. FOR ADDITIONAL INFORMATION AND DETAILING OF CULVERT STAGING, SEE PLAN VIEW ON SHEET 3 OF 8.

DOCUMENT NOT CONSIDERED  
FINAL UNLESS ALL  
SIGNATURES COMPLETED

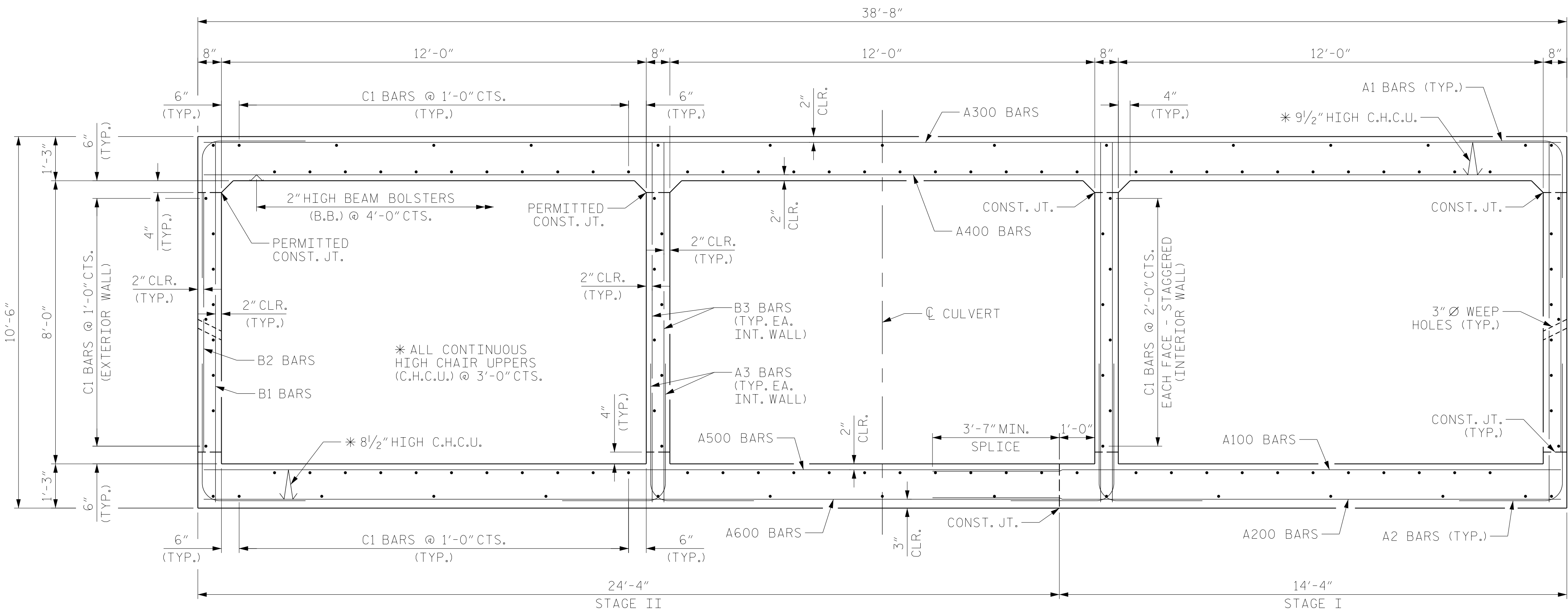
6/19/2025

STATE OF NORTH CAROLINA  
DEPARTMENT OF TRANSPORTATION  
RALEIGH

BARREL STANDARD  
TRIPLE 12 FT. X 8 FT.  
CONCRETE BOX CULVERT  
90° SKEW

RS&H Architects-Engineers-Planners, Inc.  
8521 Six Forks Road, Suite 400  
Raleigh, NC 27615  
919-926-4100 FAX 919-846-9080  
www.rsandh.com  
North Carolina License Nos. 50073-F-5403-C-28

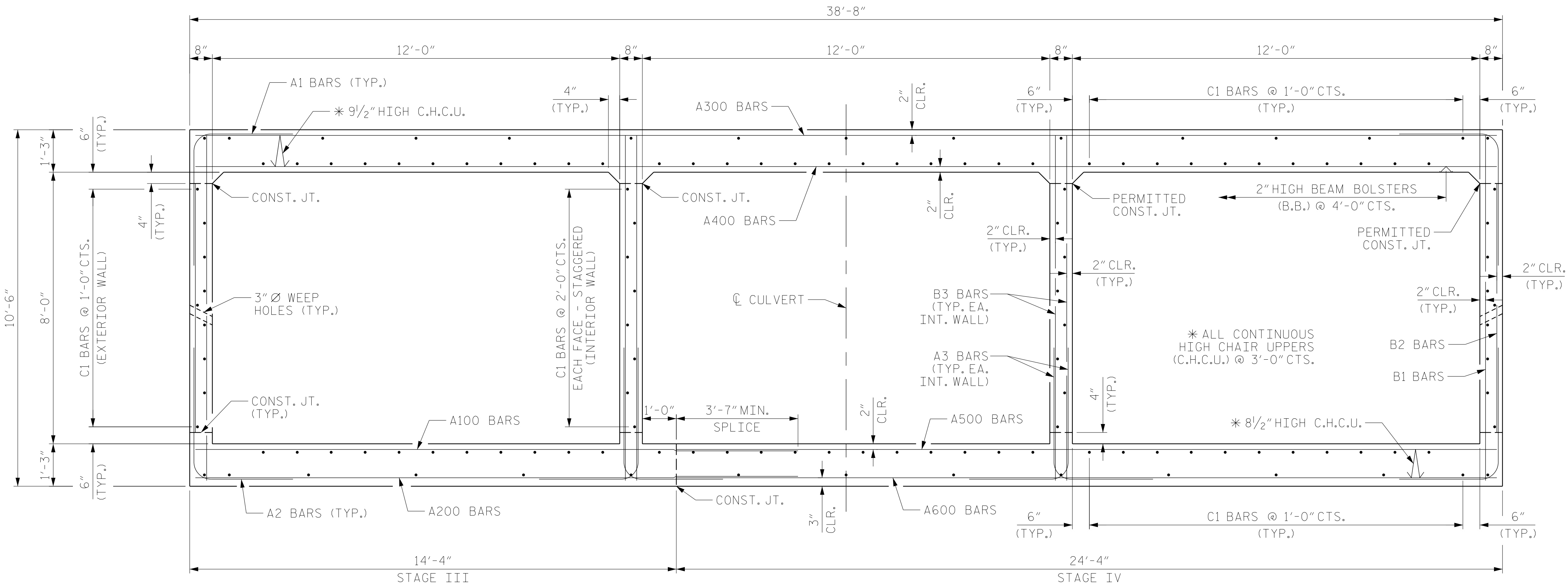
| REVISIONS |     |       |     |     |       | SHEET NO.    |
|-----------|-----|-------|-----|-----|-------|--------------|
| NO.       | BY: | DATE: | NO. | BY: | DATE: | C-4          |
| 1         |     |       | 3   |     |       | TOTAL SHEETS |
| 2         |     |       | 4   |     |       | 8            |



NOTES:

THERE ARE 35 "C" BARS IN STAGE I AND STAGE III SECTION OF BARREL.

THERE ARE 97 "C" BARS IN STAGE II AND STAGE IV SECTION OF BARREL.



PROJECT NO. U-5798B  
CUMBERLAND COUNTY  
STATION: 25+46.30 -LB-

SHEET 5 OF 8

DRAWN BY : NSC DATE : 12/2023  
CHECKED BY : MRA DATE : 12/2023  
DESIGN ENGINEER OF RECORD: MRA DATE : 03/2025

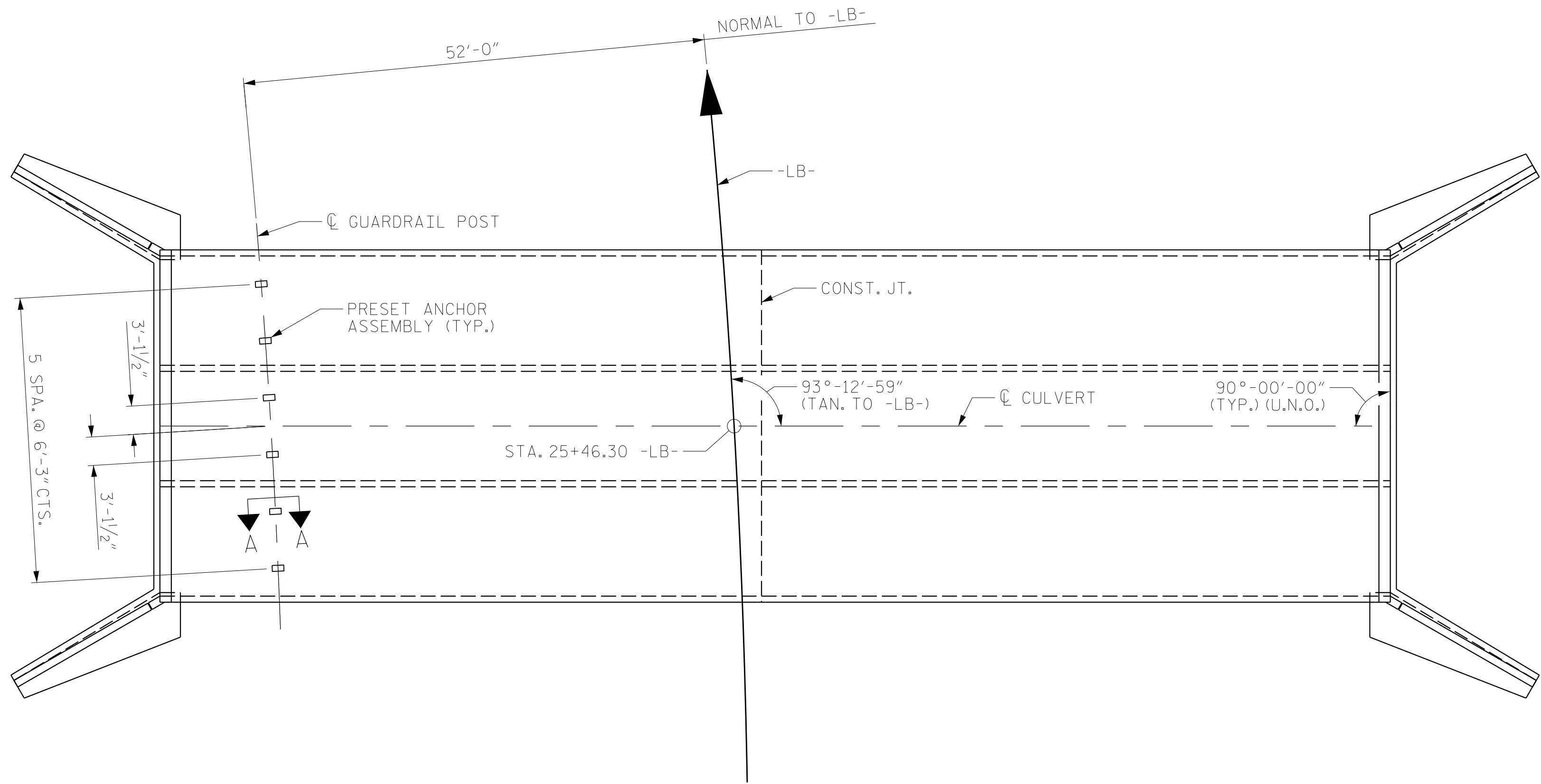
6/19/2025  
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DOCUMENT NOT CONSIDERED  
FINAL UNLESS ALL  
SIGNATURES COMPLETED

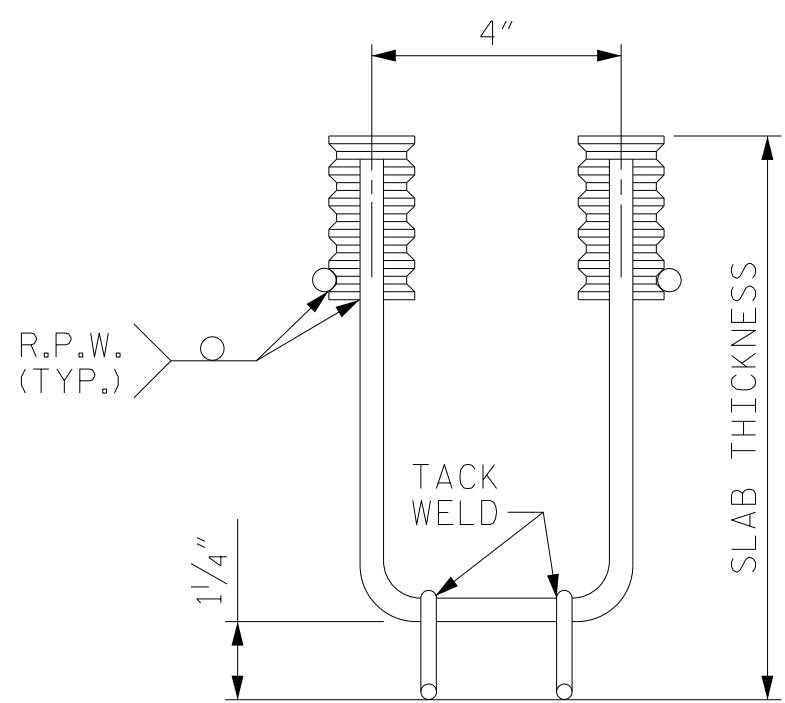
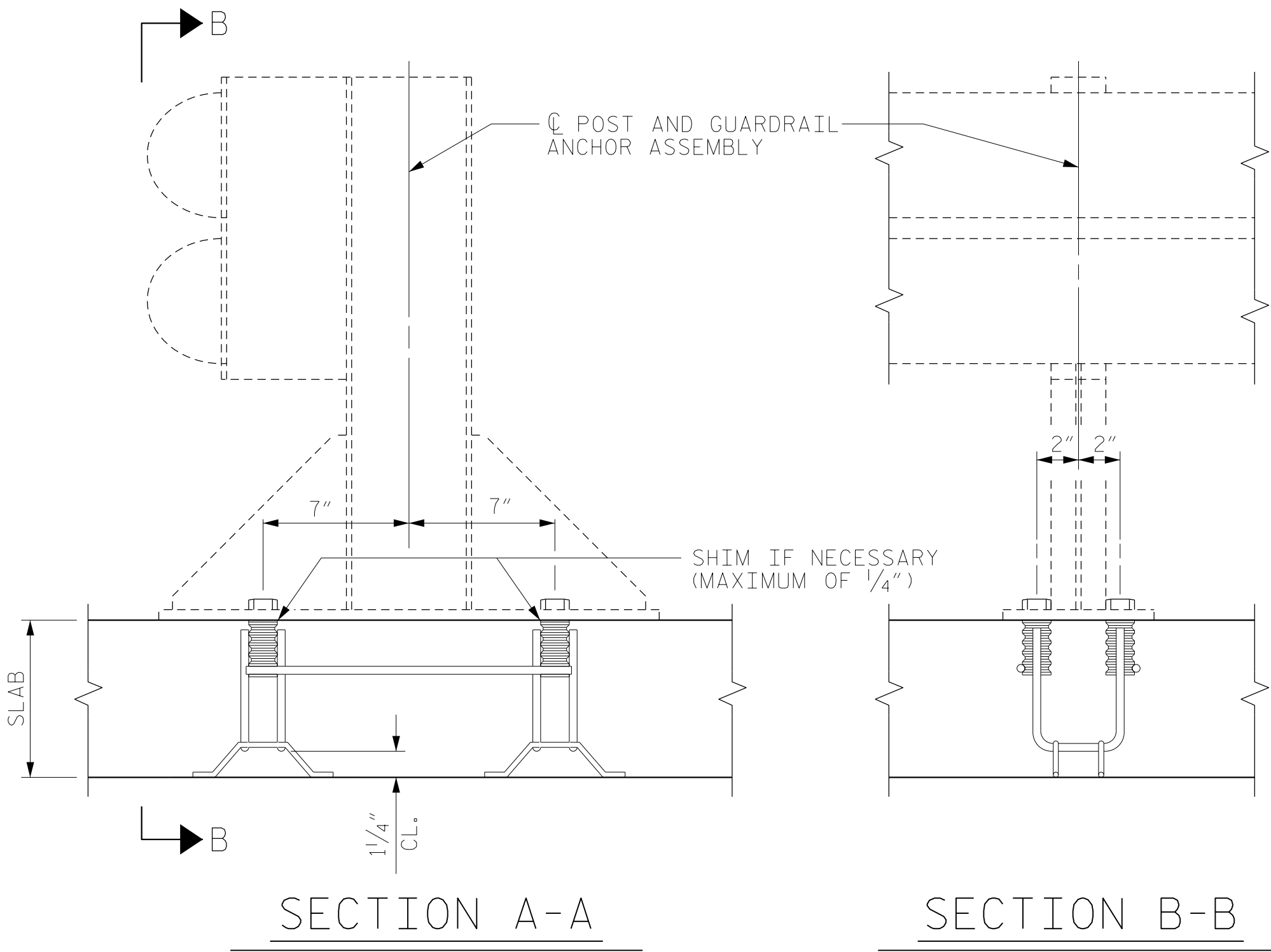


| REVISIONS |     |       |     |     |       | SHEET NO.         |  |
|-----------|-----|-------|-----|-----|-------|-------------------|--|
| NO.       | BY: | DATE: | NO. | BY: | DATE: | C-5               |  |
| 1         |     |       | 3   |     |       | TOTAL SHEETS<br>8 |  |
| 2         |     |       | 4   |     |       |                   |  |

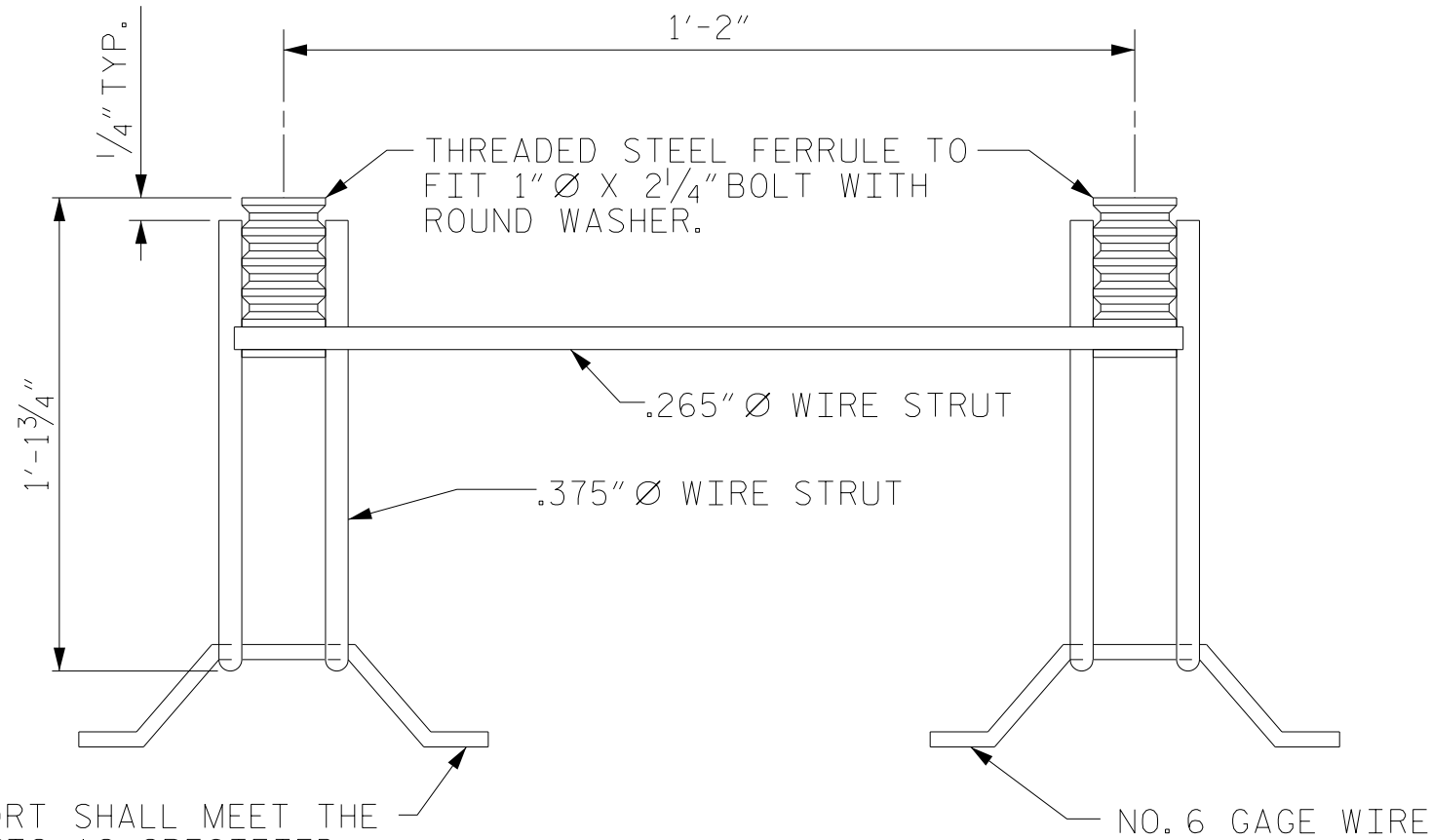
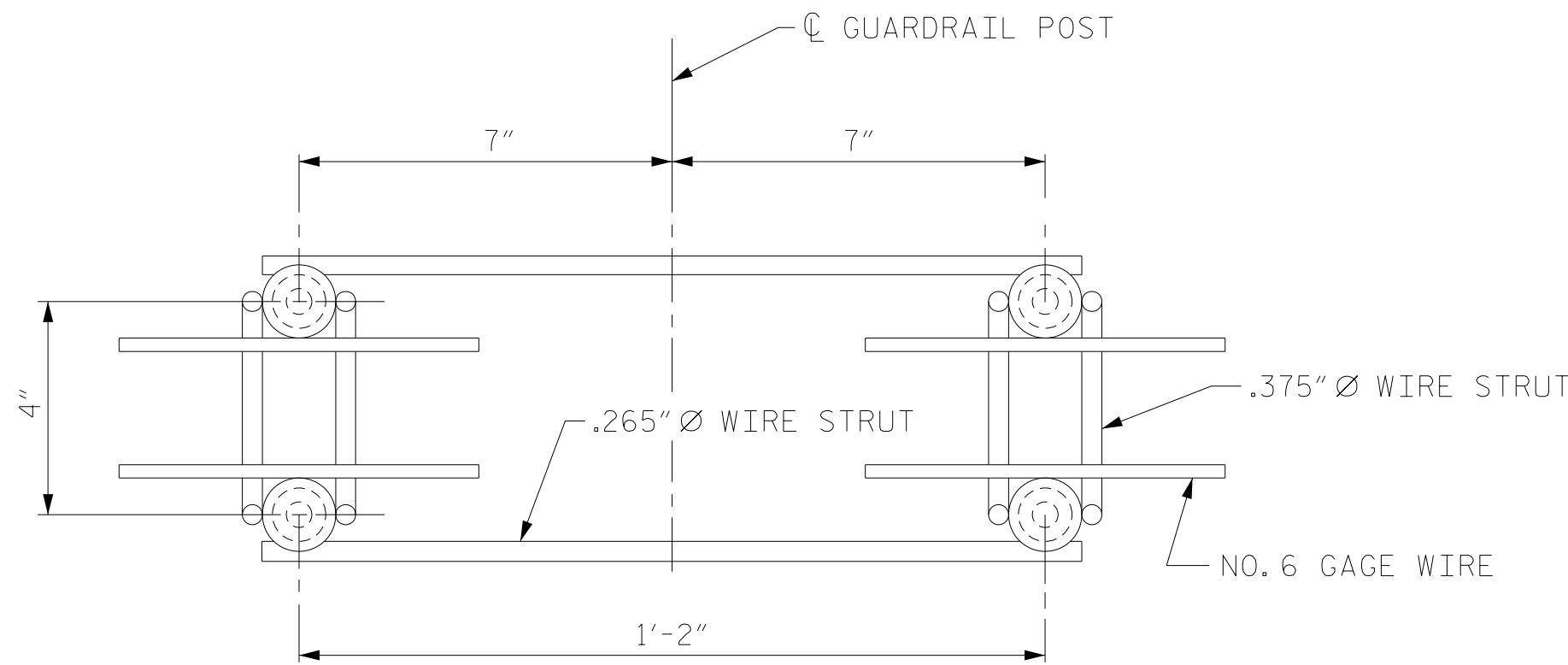




PLAN OF CULVERT GUARDRAIL ANCHOR ASSEMBLY SPACING



THIS SUPPORT SHALL MEET THE REQUIREMENTS AS SPECIFIED FOR SUPPORTS FOR REINFORCING STEEL. SEE SPECIFICATIONS.



GUARDRAIL ANCHOR ASSEMBLY FOR CULVERTS

NOTES

- THE GUARDRAIL ANCHOR ASSEMBLY FOR CULVERTS SHALL CONSIST OF THE FOLLOWING COMPONENTS :
- A. FERRULES SHALL BE MADE FROM STEEL MEETING THE REQUIREMENTS OF AASHTO M169, GRADE 12L14 AND SHALL HAVE A MINIMUM LENGTH OF THREADS OF 2 1/2".
  - B. 4 - 1" Ø X 2 1/4" BOLTS WITH WASHERS, BOLTS SHALL CONFORM TO THE REQUIREMENTS OF ASTM A307. BOLTS AND WASHERS SHALL BE GALVANIZED. (AT THE CONTRACTOR'S OPTION, STAINLESS STEEL BOLTS AND WASHERS MAY BE USED AS AN ALTERNATE FOR THE 1" Ø X 2 1/4" GALVANIZED BOLTS AND WASHERS. THEY SHALL CONFORM TO OR EXCEED THE MECHANICAL REQUIREMENTS OF ASTM A307. THE USE OF THIS ALTERNATE SHALL BE APPROVED BY THE ENGINEER.)
  - C. WIRE STRUTS SHOWN IN THE GUARDRAIL ANCHOR ASSEMBLY FOR CULVERTS DETAIL ARE MINIMUM ALLOWABLE SIZE AND SHALL HAVE A MINIMUM TENSILE STRENGTH OF 100,000 P.S.I. AS AN OPTION, A 7/16" Ø WIRE STRUT WITH A MINIMUM TENSILE STRENGTH OF 90,000 PSI IS ACCEPTABLE.
- GUARDRAIL ANCHOR ASSEMBLY WITH BOLTS SHALL BE ASSEMBLED IN THE SHOP. BOLT THREADS MAY BE RECUT AS NECESSARY TO INSURE FIT.
- THE COST OF THE GUARDRAIL ANCHOR ASSEMBLY FOR CULVERTS COMPLETE IN PLACE, SHALL BE INCLUDED IN THE UNIT CONTRACT PRICE BID FOR CLASS "A" CONCRETE.
- FERRULES TO BE PLUGGED DURING POURING OF SLAB AS RECOMMENDED BY THE MANUFACTURER.
- AT THE CONTRACTOR'S OPTION, FERRULES WITH OPEN OR CLOSED ENDS MAY BE USED.
- PAYMENT FOR GUARDRAIL, POSTS, AND POST BASE PLATES IS INCLUDED IN ROADWAY PAY ITEMS.
- SLAB REINFORCING STEEL MAY BE SHIFTED AS NECESSARY TO CLEAR GUARDRAIL ANCHOR ASSEMBLY. CARE SHOULD BE TAKEN TO KEEP THE SHIFTING OF REINFORCING STEEL TO A MINIMUM.
- THE CONTRACTOR MAY USE ADHESIVELY ANCHORED ANCHOR BOLTS IN PLACE OF GUARDRAIL ANCHOR ASSEMBLY. LEVEL TWO FIELD TESTING IS REQUIRED, AND THE YIELD LOAD OF THE 1" Ø BOLT IS 21.8 KIPS. FOR ADHESIVELY ANCHORED ANCHOR BOLTS OR DOWELS, SEE STANDARD SPECIFICATIONS.

PROJECT NO. U-5798B  
CUMBERLAND COUNTY  
STATION: 25+46.30 -LB-

SHEET 7 OF 8

|                |     |              |         |
|----------------|-----|--------------|---------|
| ASSEMBLED BY : | NSC | DATE :       | 12/2023 |
| CHECKED BY :   | MRA | DATE :       | 12/2023 |
| DRAWN BY :     | FCJ | REV. 10/1/11 | MAA/CM  |
| CHECKED BY :   | ARB | REV. 12/17   | MAA/THC |
|                |     | REV. 6/19    | MAA/THC |

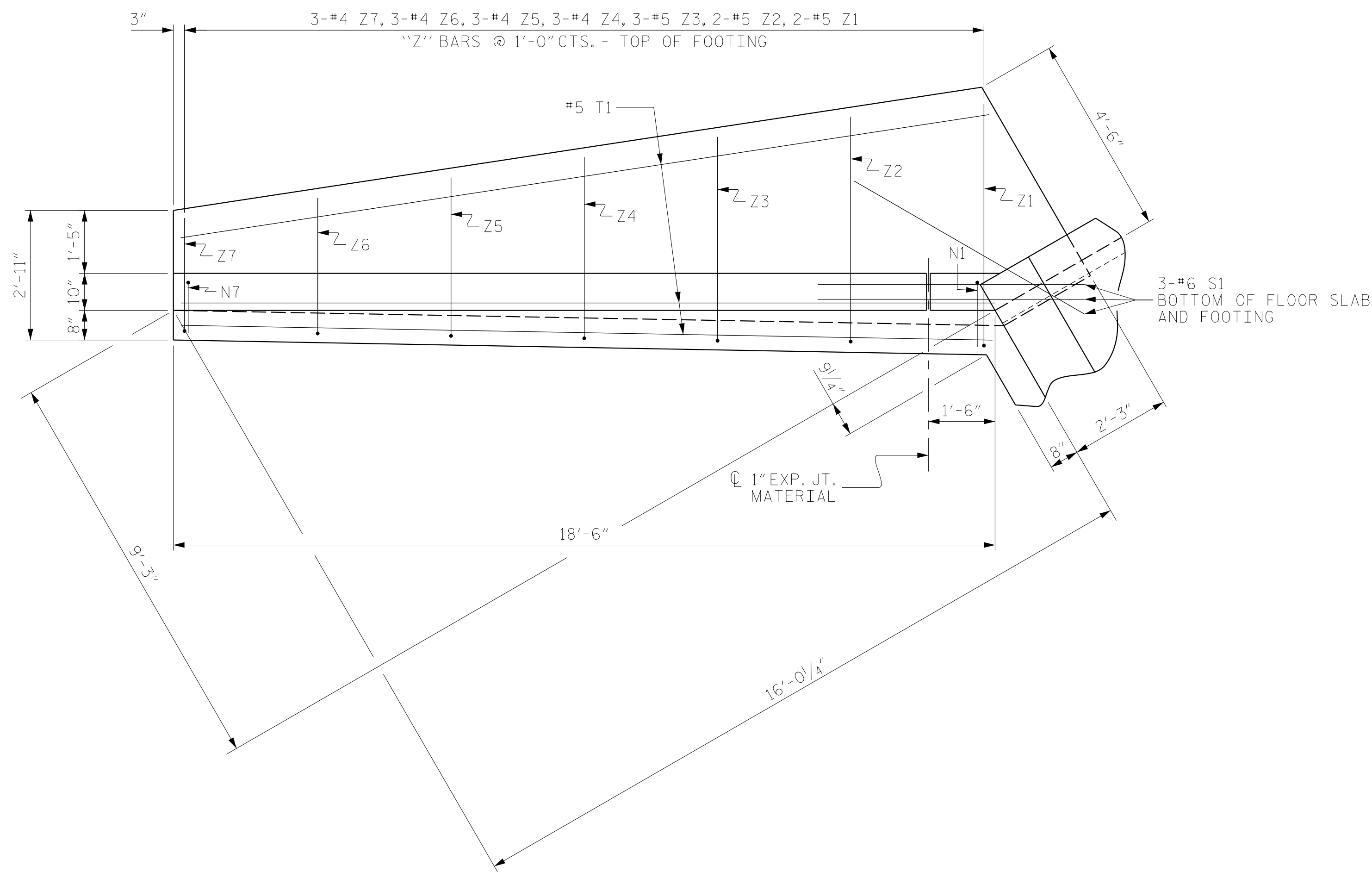
6/19/2025  
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DOCUMENT NOT CONSIDERED  
FINAL UNLESS ALL  
SIGNATURES COMPLETED

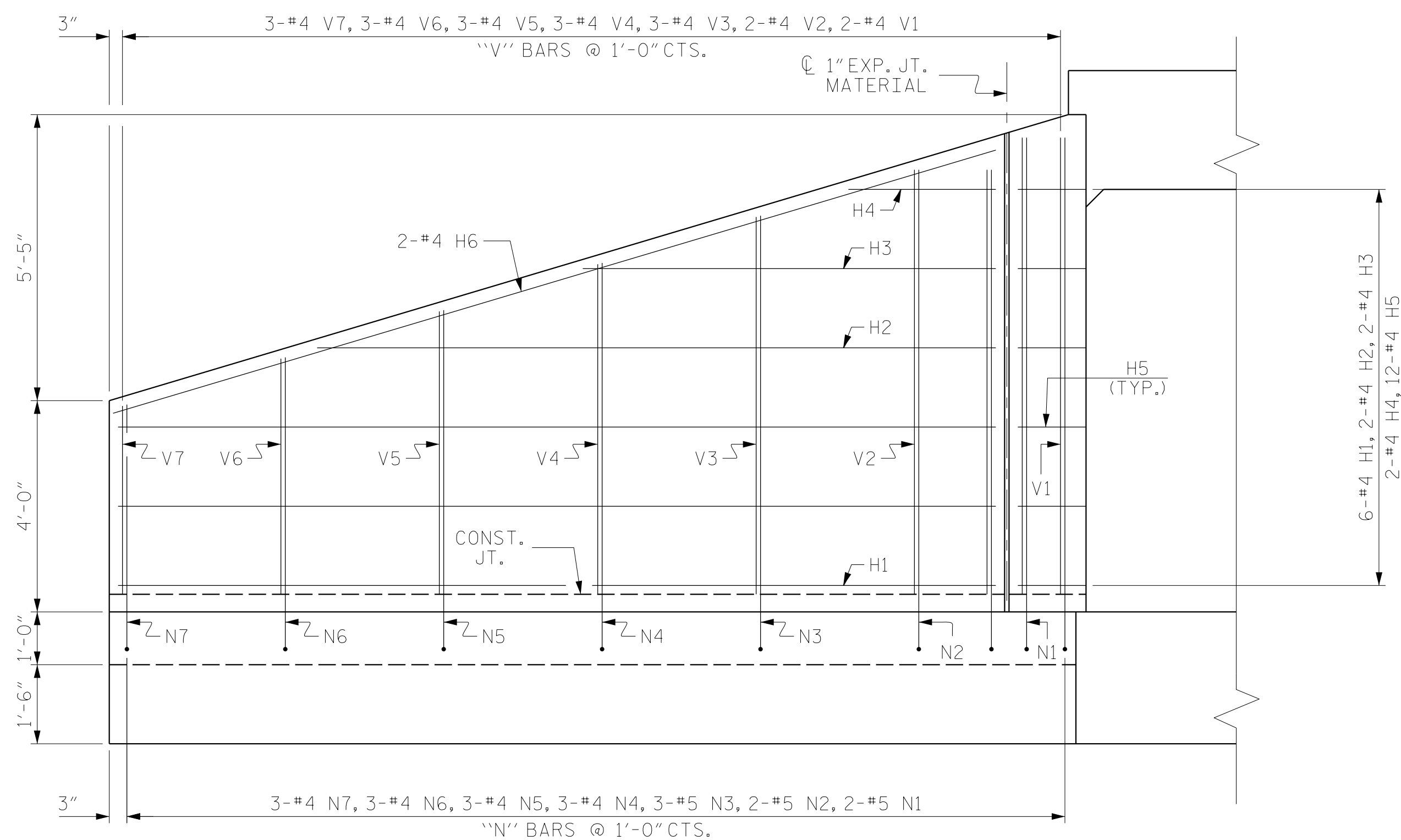
6/19/2025  
NORTH CAROLINA  
PROFESSIONAL  
SEAL  
052672  
ENGINEER  
ANTHONY R. ACCO  
Signed by  
Matthew J. Acosta  
6/19/2025  
RS&H  
RS&H Architects-Engineers-Planners, Inc.  
8521 Six Forks Road, Suite 400  
Raleigh, NC 27615  
919-926-4100 FAX 919-846-9080  
www.rsandh.com  
North Carolina License Nos. 50073-F-5403-C-28

STATE OF NORTH CAROLINA  
DEPARTMENT OF TRANSPORTATION  
RALEIGH  
STANDARD  
ANCHORAGE DETAILS FOR  
GUARDRAIL ANCHOR ASSEMBLY  
FOR CULVERTS

| REVISIONS |     |       |     |     |       | SHEET NO.         |
|-----------|-----|-------|-----|-----|-------|-------------------|
| NO.       | BY: | DATE: | NO. | BY: | DATE: |                   |
| 1         |     |       | 3   |     |       | C-7               |
| 2         |     |       | 4   |     |       | TOTAL SHEETS<br>8 |

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## PLAN

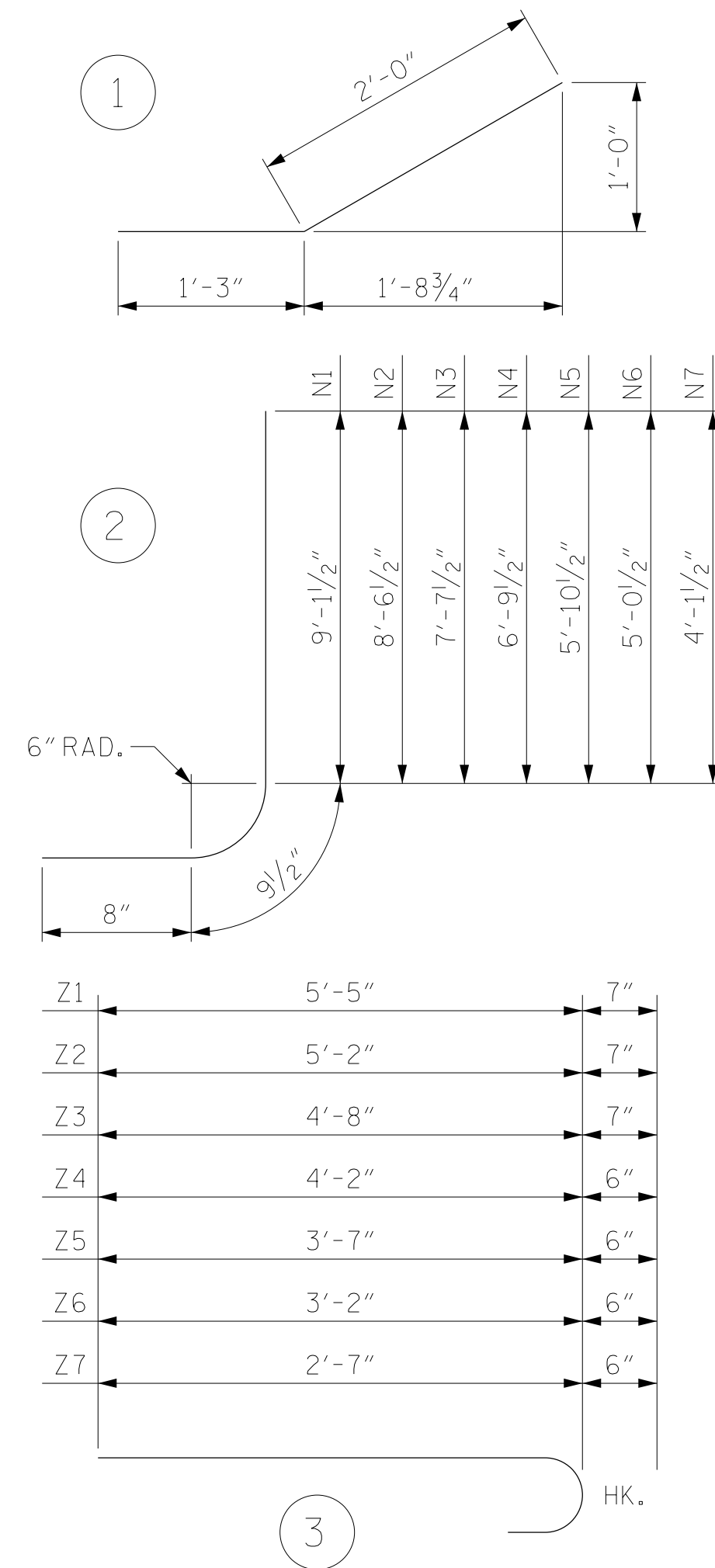


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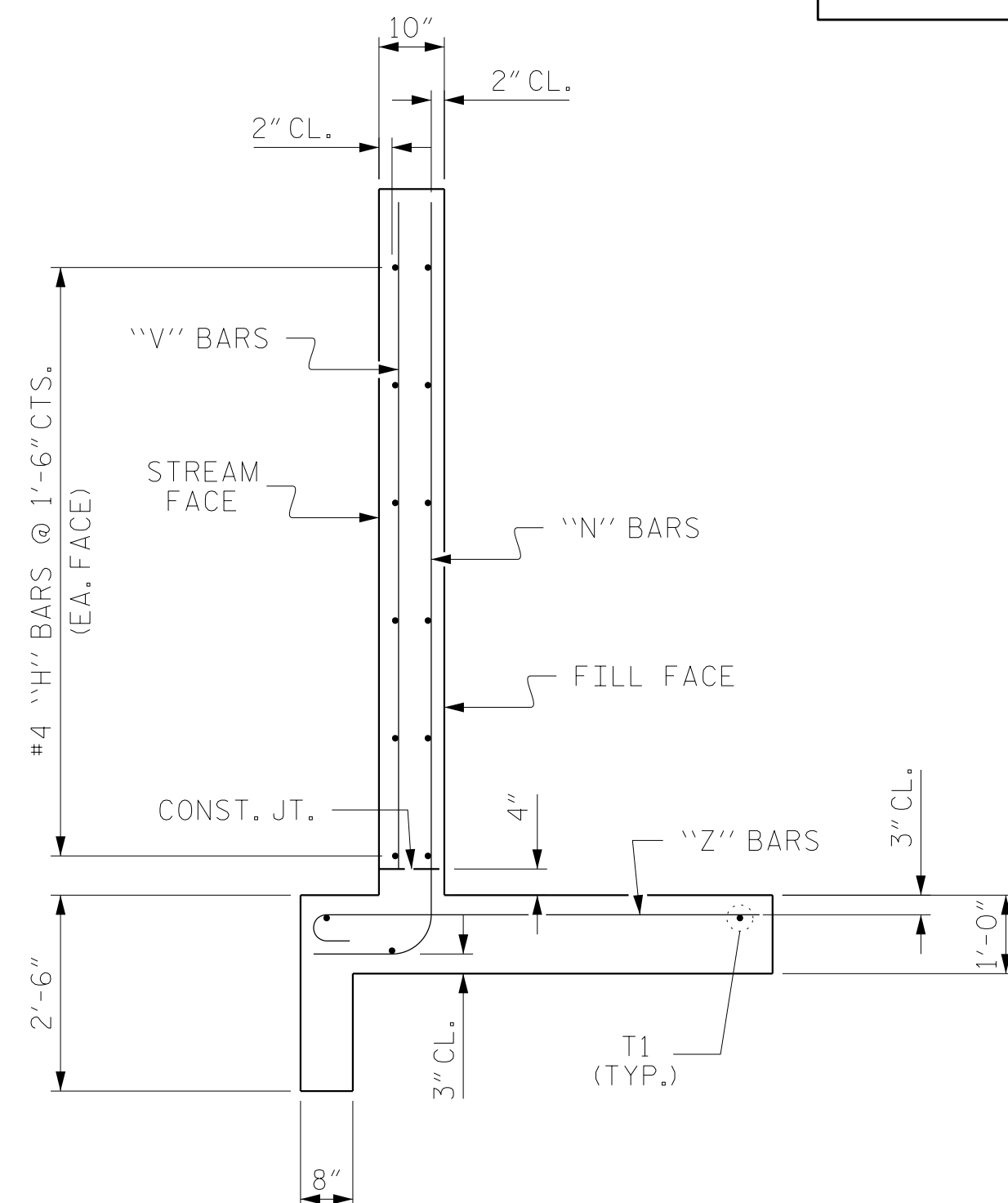
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 CHECKED BY : \_\_\_\_\_ MRA \_\_\_\_\_ DATE : 12/2023  
 DESIGN ENGINEER OF RECORD: \_\_\_\_\_ MRA \_\_\_\_\_ DATE : 03/2025

6/19/2025  
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## BAR TYPES



ALL BAR DIMENSIONS ARE OUT TO OUT.



## TYPICAL WING SECTION

DOCUMENT NOT CONSIDERED  
FINAL UNLESS ALL  
SIGNATURES COMPLETED

## BILL OF MATERIAL

STAGE I AND STAGE III  
(EACH STAGE)

| BAR | NO. | SIZE | TYPE | LENGTH  | WEIGHT |
|-----|-----|------|------|---------|--------|
| H1  | 6   | #4   | STR  | 16'-7"  | 66     |
| H2  | 2   | #4   | STR  | 12'-10" | 17     |
| H3  | 2   | #4   | STR  | 7'-9"   | 10     |
| H4  | 2   | #4   | STR  | 2'-9"   | 4      |
| H5  | 12  | #4   | 1    | 3'-3"   | 26     |
| H6  | 2   | #4   | STR  | 17'-4"  | 23     |

|    |   |    |   |        |    |
|----|---|----|---|--------|----|
| N1 | 2 | #5 | 2 | 10'-7" | 22 |
| N2 | 2 | #5 | 2 | 10'-0" | 21 |
| N3 | 3 | #5 | 2 | 9'-1"  | 28 |
| N4 | 3 | #4 | 2 | 8'-3"  | 17 |
| N5 | 3 | #4 | 2 | 7'-4"  | 15 |
| N6 | 3 | #4 | 2 | 6'-6"  | 13 |
| N7 | 3 | #4 | 2 | 5'-7"  | 11 |

|    |   |    |     |       |    |
|----|---|----|-----|-------|----|
| S1 | 3 | #6 | STR | 6'-0" | 27 |
|----|---|----|-----|-------|----|

|    |   |    |     |        |    |
|----|---|----|-----|--------|----|
| T1 | 3 | #5 | STR | 18'-1" | 57 |
|----|---|----|-----|--------|----|

|    |   |    |     |        |    |
|----|---|----|-----|--------|----|
| V1 | 2 | #4 | STR | 8'-6"  | 11 |
| V2 | 2 | #4 | STR | 7'-11" | 11 |
| V3 | 3 | #4 | STR | 7'-0"  | 14 |
| V4 | 3 | #4 | STR | 6'-2"  | 12 |
| V5 | 3 | #4 | STR | 5'-3"  | 11 |
| V6 | 3 | #4 | STR | 4'-5"  | 9  |
| V7 | 3 | #4 | STR | 3'-6"  | 7  |

|    |   |    |   |       |    |
|----|---|----|---|-------|----|
| Z1 | 2 | #5 | 3 | 6'-0" | 13 |
| Z2 | 2 | #5 | 3 | 5'-9" | 12 |
| Z3 | 3 | #5 | 3 | 5'-3" | 16 |
| Z4 | 3 | #4 | 3 | 4'-8" | 9  |
| Z5 | 3 | #4 | 3 | 4'-1" | 8  |
| Z6 | 3 | #4 | 3 | 3'-8" | 7  |
| Z7 | 3 | #4 | 3 | 3'-1" | 6  |

|                                 |          |
|---------------------------------|----------|
| REINFORCING STEEL<br>FOR 1 WING | 503 LBS. |
| CLASS A CONCRETE                |          |
| 1 WING                          | 7.1 CY   |
| 1 HEADWALL                      | 0.0 CY   |
| 1 END CURTAIN WALL              | 1.2 CY   |
| TOTAL                           | 8.3 CY   |

## BILL OF MATERIAL

STAGE II AND STAGE IV  
(EACH STAGE)

| BAR | NO. | SIZE | TYPE | LENGTH  | WEIGHT |
|-----|-----|------|------|---------|--------|
| H1  | 6   | #4   | STR  | 16'-7"  | 66     |
| H2  | 2   | #4   | STR  | 12'-10" | 17     |
| H3  | 2   | #4   | STR  | 7'-9"   | 10     |
| H4  | 2   | #4   | STR  | 2'-9"   | 4      |
| H5  | 12  | #4   | 1    | 3'-3"   | 26     |
| H6  | 2   | #4   | STR  | 17'-4"  | 23     |

|    |   |    |   |        |    |
|----|---|----|---|--------|----|
| N1 | 2 | #5 | 2 | 10'-7" | 22 |
| N2 | 2 | #5 | 2 | 10'-0" | 21 |
| N3 | 3 | #5 | 2 | 9'-1"  | 28 |
| N4 | 3 | #4 | 2 | 8'-3"  | 17 |
| N5 | 3 | #4 | 2 | 7'-4"  | 15 |
| N6 | 3 | #4 | 2 | 6'-6"  | 13 |
| N7 | 3 | #4 | 2 | 5'-7"  | 11 |

|    |   |    |     |       |    |
|----|---|----|-----|-------|----|
| S1 | 3 | #6 | STR | 6'-0" | 27 |
|----|---|----|-----|-------|----|

|    |   |    |     |        |    |
|----|---|----|-----|--------|----|
| T1 | 3 | #5 | STR | 18'-1" | 57 |
|----|---|----|-----|--------|----|

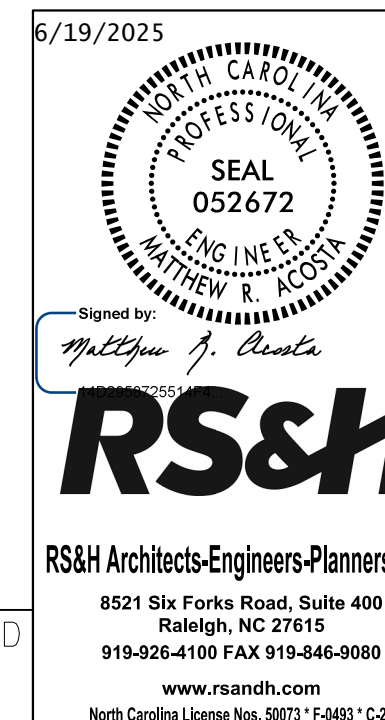
|    |   |    |     |        |    |
|----|---|----|-----|--------|----|
| V1 | 2 | #4 | STR | 8'-6"  | 11 |
| V2 | 2 | #4 | STR | 7'-11" | 11 |
| V3 | 3 | #4 | STR | 7'-0"  | 14 |
| V4 | 3 | #4 | STR | 6'-2"  | 12 |
| V5 | 3 | #4 | STR | 5'-3"  | 11 |
| V6 | 3 | #4 | STR | 4'-5"  | 9  |
| V7 | 3 | #4 | STR | 3'-6"  | 7  |

|    |   |    |   |       |    |
|----|---|----|---|-------|----|
| Z1 | 2 | #5 | 3 | 6'-0" | 13 |
| Z2 | 2 | #5 | 3 | 5'-9" | 12 |
| Z3 | 3 | #5 | 3 | 5'-3" | 16 |
| Z4 | 3 | #4 | 3 | 4'-8" | 9  |
| Z5 | 3 | #4 | 3 | 4'-1" | 8  |
| Z6 | 3 | #4 | 3 | 3'-8" | 7  |
| Z7 | 3 | #4 | 3 | 3'-1" | 6  |

|                                 |          |
|---------------------------------|----------|
| REINFORCING STEEL<br>FOR 1 WING | 503 LBS. |
| CLASS A CONCRETE                |          |
| 1 WING                          | 7.1 CY   |
| 1 HEADWALL                      | 1.8 CY   |
| 1 END CURTAIN WALL              | 1.5 CY   |
| TOTAL                           | 10.4 CY  |

PROJECT NO. U-5798B  
CUMBERLAND COUNTY  
 STATION: 25+46.30 -LB-

SHEET 8 OF 8



STATE OF NORTH CAROLINA  
DEPARTMENT OF TRANSPORTATION  
RALEIGH

# WINGS FOR CONCRETE BOX CULVERT

H = 8'-0" SLOPE = 3:1  
90° SKEW

| REVISIONS |     |       |     |     |       | SHEET NO.<br><br>C-8 |
|-----------|-----|-------|-----|-----|-------|----------------------|
| NO.       | BY: | DATE: | NO. | BY: | DATE: |                      |
| 1         |     |       | 3   |     |       | TOTAL<br>SHEETS<br>8 |
| 2         |     |       | 4   |     |       |                      |

STANDARD NOTES

DESIGN DATA:

|                                                                       |           |                                  |
|-----------------------------------------------------------------------|-----------|----------------------------------|
| SPECIFICATIONS                                                        | - - - - - | A.A.S.H.T.O. (CURRENT)           |
| LIVE LOAD                                                             | - - - - - | SEE PLANS                        |
| IMPACT ALLOWANCE                                                      | - - - - - | SEE A.A.S.H.T.O.                 |
| STRESS IN EXTREME FIBER OF<br>STRUCTURAL STEEL - AASHTO M270 GRADE 36 | - -       | 20,000 LBS. PER SQ. IN.          |
| - AASHTO M270 GRADE 50W                                               | - -       | 27,000 LBS. PER SQ. IN.          |
| - AASHTO M270 GRADE 50                                                | - -       | 27,000 LBS. PER SQ. IN.          |
| REINFORCING STEEL IN TENSION - GRADE 60                               | - - -     | 24,000 LBS. PER SQ. IN.          |
| CONCRETE IN COMPRESSION                                               | - - - - - | 1,200 LBS. PER SQ. IN.           |
| CONCRETE IN SHEAR                                                     | - - - - - | SEE A.A.S.H.T.O.                 |
| STRUCTURAL TIMBER - TREATED OR UNTREATED<br>EXTREME FIBER STRESS      | - - -     | 1,800 LBS. PER SQ. IN.           |
| COMPRESSION PERPENDICULAR TO GRAIN<br>OF TIMBER                       | - - - -   | 375 LBS. PER SQ. IN.             |
| EQUIVALENT FLUID PRESSURE OF EARTH                                    | - - - -   | 30 LBS. PER CU. FT.<br>(MINIMUM) |

MATERIAL AND WORKMANSHIP:

EXCEPT AS MAY OTHERWISE BE SPECIFIED ON PLANS OR IN THE SPECIAL PROVISIONS, ALL MATERIAL AND WORKMANSHIP SHALL BE IN ACCORDANCE WITH THE 2024 "STANDARD SPECIFICATIONS FOR ROADS AND STRUCTURES" OF THE N.C. DEPARTMENT OF TRANSPORTATION.

STEEL SHEET PILING FOR PERMANENT OR TEMPORARY APPLICATIONS SHALL BE HOT ROLLED.

CONCRETE:

UNLESS OTHERWISE REQUIRED ON PLANS, CLASS A CONCRETE SHALL BE USED FOR ALL PORTIONS OF ALL STRUCTURES WITH THE EXCEPTION THAT: CLASS AA CONCRETE SHALL BE USED IN BRIDGE SUPERSTRUCTURES, ABUTMENT BACKWALLS, AND APPROACH SLABS; AND CLASS B CONCRETE SHALL BE USED FOR SLOPE PROTECTION AND RIP RAP.

CONCRETE CHAMFERS:

UNLESS OTHERWISE NOTED ON THE PLANS, ALL EXPOSED CORNERS ON STRUCTURES SHALL BE CHAMFERED 3/4" WITH THE FOLLOWING EXCEPTIONS: TOP CORNERS OF CURBS MAY BE ROUNDED TO 1 1/2" RADIUS WHICH IS BUILT INTO CURB FORMS; CORNERS OF TRANSVERSE FLOOR EXPANSION JOINTS SHALL BE ROUNDED WITH A 1/4" FINISHING TOOL UNLESS OTHERWISE REQUIRED ON PLANS; AND CORNERS OF EXPANSION JOINTS IN THE ROADWAY FACES AND TOPS OF CURBS AND SIDEWALKS SHALL BE ROUNDED TO A 1/4" RADIUS WITH A FINISHING STONE OR TOOL UNLESS OTHERWISE REQUIRED ON PLANS.

DOWELS:

DOWELS WHEN INDICATED ON PLANS AS FOR CULVERT EXTENSIONS, SHALL BE EMBEDDED AT LEAST 12" INTO THE OLD CONCRETE AND GROUTED INTO PLACE WITH 1:2 CEMENT MORTAR.

ALLOWANCE FOR DEAD LOAD DEFLECTION, SETTLEMENT, ETC. IN CASTING SUPERSTRUCTURES:

BRIDGES SHALL BE BUILT ON THE GRADE OR VERTICAL CURVE SHOWN ON PLANS. SLABS, CURBS AND PARAPETS SHALL CONFORM TO THE GRADE OR CURVE.

ALL DIMENSIONS WHICH ARE GIVEN IN SECTION AND ARE AFFECTED BY DEAD LOAD DEFLECTIONS ARE DIMENSIONS AT CENTER LINE OF BEARING UNLESS OTHERWISE NOTED ON PLANS. IN SETTING FORMS FOR STEEL BEAM BRIDGES AND PRESTRESSED CONCRETE GIRDER BRIDGES, ADJUSTMENTS SHALL BE MADE DUE TO THE DEAD LOAD DEFLECTIONS FOR THE ELEVATIONS SHOWN. WHERE BLOCKS ARE SHOWN OVER BEAMS FOR BUILDING UP TO THE SLAB, THE VERTICAL DIMENSIONS OF THE BLOCKS SHALL BE ADJUSTED BETWEEN BEARINGS TO COMPENSATE FOR DEAD LOAD DEFLECTIONS, VERTICAL CURVE ORDINATE, AND ACTUAL BEAM CAMBER. WHERE BOTTOM OF SLAB IS IN LINE WITH BOTTOM OF TOP FLANGES, DEPTH OF SLAB BETWEEN BEARINGS SHALL BE ADJUSTED TO COMPENSATE FOR DEAD LOAD DEFLECTION, VERTICAL CURVE ORDINATE, AND ACTUAL BEAM CAMBER.

IN SETTING FALSEWORK AND FORMS FOR REINFORCED CONCRETE SPANS, AN ALLOWANCE SHALL BE MADE FOR DEAD LOAD DEFLECTIONS, SETTLEMENT OF FALSEWORK, AND PERMANENT CAMBER WHICH SHALL BE PROVIDED FOR IN ADDITION TO THE ELEVATIONS SHOWN. AFTER REMOVAL OF THE FALSEWORK, THE FINISHED STRUCTURES SHALL CONFORM TO THE PROFILE AND ELEVATIONS SHOWN ON THE PLANS AND CONSTRUCTION ELEVATIONS FURNISHED BY THE ENGINEER.

DETAILED DRAWINGS FOR FALSEWORK OR FORMS FOR BRIDGE SUPERSTRUCTURE AND ANY STRUCTURE OR PARTS OF A STRUCTURE AS NOTED ON THE PLANS SHALL BE SUBMITTED TO THE ENGINEER FOR APPROVAL BEFORE CONSTRUCTION OF THE FALSEWORK OR FORMS IS STARTED.

REINFORCING STEEL:

ALL REINFORCING STEEL SHALL BE DEFORMED. DIMENSIONS RELATIVE TO PLACEMENT OF REINFORCING ARE TO CENTERS OF BARS UNLESS OTHERWISE INDICATED IN THE PLANS. DIMENSIONS ON BAR DETAILS ARE TO CENTERS OF BARS OR ARE OUT TO OUT AS INDICATED ON PLANS.

WIRE BAR SUPPORTS SHALL BE PROVIDED FOR REINFORCING STEEL WHERE INDICATED ON THE PLANS. WHEN BAR SUPPORT PIECES ARE PLACED IN CONTINUOUS LINES, THEY SHALL BE SO PLACED THAT THE ENDS OF THE SUPPORTING WIRES SHALL BE LAPPED TO LOCK LEGS ON ADJOINING PIECES.

STRUCTURAL STEEL:

AT THE CONTRACTOR'S OPTION, HE MAY SUBSTITUTE 7/8" Ø SHEAR STUDS FOR THE 3/4" Ø STUDS SPECIFIED ON THE PLANS. THIS SUBSTITUTION SHALL BE MADE AT THE RATE OF 3 - 7/8" Ø STUDS FOR 4 - 3/4" Ø STUDS, AND STUD SPACING CHANGES SHALL BE MADE AS NECESSARY TO PROVIDE THE SAME EQUIVALENT NUMBER OF 7/8" Ø STUDS ALONG THE BEAM AS SHOWN FOR 3/4" Ø STUDS BASED ON THE RATIO OF 3 - 7/8" Ø STUDS FOR 4 - 3/4" Ø STUDS. STUDS OF THE LENGTH SPECIFIED ON THE PLANS MUST BE PROVIDED. THE MAXIMUM SPACING SHALL BE 2'-0".

EXCEPT AT THE INTERIOR SUPPORTS OF CONTINUOUS BEAMS WHERE THE COVER PLATE IS IN CONTACT WITH BEARING PLATE, THE CONTRACTOR MAY, AT HIS OPTION, SUBSTITUTE FOR THE COVER PLATES DESIGNATED ON THE PLANS COVER PLATES OF THE EQUIVALENT AREA PROVIDED THESE PLATES ARE AT LEAST 3/16" IN THICKNESS AND DO NOT EXCEED A WIDTH EQUAL TO THE FLANGE WIDTH LESS 2" OR A THICKNESS EQUAL TO 2 TIMES THE FLANGE THICKNESS. THE SIZE OF FILLET WELDS SHALL CONFORM TO THE REQUIREMENTS OF THE CURRENT ANSI/AASHTO/AWS "BRIDGE WELDING CODE". ELECTROSLAG WELDING WILL NOT BE PERMITTED.

WITH THE SOLE EXCEPTION OF EDGES AT SURFACES WHICH BEAR ON OTHER SURFACES, ALL SHARP EDGES AND ENDS OF SHAPES AND PLATES SHALL BE SLIGHTLY ROUNDED BY SUITABLE MEANS TO A RADIUS OF APPROXIMATELY 1/16" INCH OR EQUIVALENT FLAT SURFACE AT A SUITABLE ANGLE PRIOR TO PAINTING, GALVANIZING, OR METALLIZING.

HANDRAILS AND POSTS:

METAL STANDARDS AND FACES OF THE CONCRETE END POSTS FOR THE METAL RAIL SHALL BE SET NORMAL TO THE GRADE OF THE CURB, UNLESS OTHERWISE SHOWN ON PLANS. THE METAL RAIL AND TOPS OF CONCRETE POSTS USED WITH THE ALUMINUM RAIL SHALL BE BUILT PARALLEL TO THE GRADE OF THE CURB.

METAL HANDRAILS SHALL BE IN ACCORDANCE WITH THE PLANS. RAILS SHALL BE AS MANUFACTURED FOR BRIDGE RAILING. CASTINGS SHALL BE OF A UNIFORM APPEARANCE. FINIS AND OTHER DEFORMATIONS RESULTING FROM CASTING OR OTHERWISE SHALL BE REMOVED IN A MANNER SO THAT A UNIFORM COLORING OF THE COMPLETED CASTING SHALL BE OBTAINED. CASTINGS WITH DISCOLORATIONS OR OF NON-UNIFORM COLORING WILL NOT BE ACCEPTED. CERTIFIED MILL REPORTS ARE REQUIRED FOR METAL RAILS AND POSTS.

SPECIAL NOTES:

GENERALLY, IN CASE OF DISCREPANCY, THIS STANDARD SHEET OF NOTES SHALL GOVERN OVER THE SPECIFICATIONS, BUT THE REMAINDER OF THE PLANS SHALL GOVERN OVER NOTES HEREON, AND SPECIAL PROVISIONS SHALL GOVERN OVER ALL. SEE SPECIFICATIONS ARTICLE 105-4.

ENGLISH

JANUARY, 1990

STD. NO. SN