

REFERENCE: B-5777

PROJECT: 45733

STATE OF NORTH CAROLINA
DEPARTMENT OF TRANSPORTATION
DIVISION OF HIGHWAYS
GEOTECHNICAL ENGINEERING UNIT

STRUCTURE
SUBSURFACE INVESTIGATION

COUNTY DAVIDSON
PROJECT DESCRIPTION REPLACE BRIDGE NO. 58
ON NC 109 OVER US 64

SITE DESCRIPTION -L- STA. 20+64

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NORTH CAROLINA DEPARTMENT OF TRANSPORTATION
DIVISION OF HIGHWAYS
GEOTECHNICAL ENGINEERING UNIT
SUBSURFACE INVESTIGATION
SOIL AND ROCK LEGEND, TERMS, SYMBOLS, AND ABBREVIATIONS

SOIL DESCRIPTION

SOIL IS CONSIDERED UNCONSOLIDATED, SEMI-CONSOLIDATED, OR WEATHERED EARTH MATERIALS THAT CAN BE PENETRATED WITH A CONTINUOUS FLIGHT POWER AUGER AND YIELD LESS THAN 100 BLOWS PER FOOT ACCORDING TO THE STANDARD PENETRATION TEST (AASHTO T 209, ASTM D1586). SOIL CLASSIFICATION IS BASED ON THE AASHTO SYSTEM. BASIC DESCRIPTIONS GENERALLY INCLUDE THE FOLLOWING: CONSISTENCY, COLOR, TEXTURE, MOISTURE, AASHTO CLASSIFICATION, AND OTHER PERTINENT FACTORS SUCH AS MINERALOGICAL COMPOSITION, ANGULARITY, STRUCTURE, PLASTICITY, ETC. FOR EXAMPLE, *VERY STIFF, GRAY, SILTY CLAY, MOIST WITH INTERBEDDED FINE SAND LAYERS, HIGHLY PLASTIC, A-7-6*

SOIL LEGEND AND AASHTO CLASSIFICATION

| GENERAL CLASS. | GRANULAR MATERIALS (≤ 35% PASSING #200) | | | | | | | SILT-CLAY MATERIALS (> 35% PASSING #200) | | | | | | | ORGANIC MATERIALS | | | | |
|--------------------------------|---|-------------|---------------------------------|-------------|--------------|-------------|-------------|--|-------------|-------------|--------------|----------|-------------|----------|-------------------|--|--|--|--|
| | A-1 | A-1-b | A-2 | A-2-4 | A-2-5 | A-2-6 | A-2-7 | A-4 | A-5 | A-6 | A-7 | A-1, A-2 | A-3 | A-4, A-5 | A-6, A-7 | | | | |
| GROUP CLASS. | A-1-a | A-1-b | A-2 | A-2-4 | A-2-5 | A-2-6 | A-2-7 | A-4 | A-5 | A-6 | A-7 | A-1, A-2 | A-3 | A-4, A-5 | A-6, A-7 | | | | |
| SYMBOL | | | | | | | | | | | | | | | | | | | |
| % PASSING #10 #40 #200 | 50 MX 30 MX 15 MX | 50 MX 25 MX | 51 MN 35 MX | 35 MX | 35 MX | 35 MX | 35 MX | 36 MN | 36 MN | 36 MN | 36 MN | 36 MN | 36 MN | | | | | | |
| MATERIAL PASSING #40 LL PI | - | 6 MX | NP | 40 MX 10 MX | 41 MN 10 MX | 40 MX 11 MN | 41 MN 11 MN | 40 MX 10 MX | 41 MN 10 MX | 40 MX 11 MN | 41 MN 11 MN | | | | | | | | |
| GROUP INDEX | 0 | 0 | 0 | 4 MX | 8 MX | 12 MX | 16 MX | NO MX | | | | | | | | | | | |
| USUAL TYPES OF MAJOR MATERIALS | STONE FRAGS. GRAVEL, AND SAND | FINE SAND | SILTY OR CLAYEY GRAVEL AND SAND | SILTY SOILS | CLAYEY SOILS | | | | | | | | | | | | | | |
| GEN. RATING AS SUBGRADE | EXCELLENT TO GOOD | | | | | | | FAIR TO POOR | | | FAIR TO POOR | POOR | UNSATURABLE | | | | | | |

PI OF A-7-5 SUBGROUP IS ≤ LL - 30; PI OF A-7-6 SUBGROUP IS > LL - 30

CONSISTENCY OR DENSENESS

| PRIMARY SOIL TYPE | COMPACTNESS OR CONSISTENCY | RANGE OF STANDARD PENETRATION RESISTANCE (N-VALUE) | RANGE OF UNCONFINED COMPRESSIVE STRENGTH (TONS/FT ²) |
|--|--|--|--|
| GENERALLY GRANULAR MATERIAL (NON-COHESIVE) | VERY LOOSE LOOSE MEDIUM DENSE DENSE VERY DENSE | < 4 4 TO 10 10 TO 30 30 TO 50 > 50 | N/A |
| GENERALLY SILT-CLAY MATERIAL (COHESIVE) | VERY SOFT SOFT MEDIUM STIFF STIFF VERY STIFF HARD | < 2 2 TO 4 4 TO 8 8 TO 15 15 TO 30 > 30 | < 0.25 0.25 TO 0.5 0.5 TO 1.0 1 TO 2 2 TO 4 > 4 |

TEXTURE OR GRAIN SIZE

| U.S. STD. SIEVE SIZE OPENING (MM) | 4 | 10 | 40 | 60 | 200 | 270 |
|-----------------------------------|------|------|------|------|-------|-------|
| | 4.76 | 2.00 | 0.42 | 0.25 | 0.075 | 0.053 |
| BOULDER (BLDR.) | | | | | | |
| COBBLE (COB.) | | | | | | |
| GRAVEL (GR.) | | | | | | |
| COARSE SAND (CS, SD.) | | | | | | |
| FINE SAND (F SD.) | | | | | | |
| SILT (SL.) | | | | | | |
| CLAY (CL.) | | | | | | |
| GRAIN SIZE | 305 | 75 | 2.0 | 0.25 | 0.05 | 0.005 |
| MM | | | | | | |
| IN. | 12 | 3 | | | | |

SOIL MOISTURE - CORRELATION OF TERMS

| SOIL MOISTURE SCALE (ATTERBERG LIMITS) | FIELD MOISTURE DESCRIPTION | GUIDE FOR FIELD MOISTURE DESCRIPTION |
|--|----------------------------|---|
| LL - LIQUID LIMIT | - SATURATED - (SAT.) | USUALLY LIQUID; VERY WET, USUALLY FROM BELOW THE GROUND WATER TABLE |
| PLASTIC RANGE (PI) | - WET - (W) | SEMISOLID; REQUIRES DRYING TO ATTAIN OPTIMUM MOISTURE |
| OM - OPTIMUM MOISTURE SHRINKAGE LIMIT | - MOIST - (M) | SOLID; AT OR NEAR OPTIMUM MOISTURE |
| SL - SHRINKAGE LIMIT | - DRY - (D) | REQUIRES ADDITIONAL WATER TO ATTAIN OPTIMUM MOISTURE |

PLASTICITY

| | PLASTICITY INDEX (PI) | DRY STRENGTH |
|--------------------|-----------------------|--------------|
| NON PLASTIC | 0-5 | VERY LOW |
| SLIGHTLY PLASTIC | 6-15 | SLIGHT |
| MODERATELY PLASTIC | 16-25 | MEDIUM |
| HIGHLY PLASTIC | 26 OR MORE | HIGH |

COLOR

DESCRIPTIONS MAY INCLUDE COLOR OR COLOR COMBINATIONS (TAN, RED, YELLOW-BROWN, BLUE-GRAY). MODIFIERS SUCH AS LIGHT, DARK, STREAKED, ETC. ARE USED TO DESCRIBE APPEARANCE.

GRADATION

WELL GRADED - INDICATES A GOOD REPRESENTATION OF PARTICLE SIZES FROM FINE TO COARSE.
UNIFORMLY GRADED - INDICATES THAT SOIL PARTICLES ARE ALL APPROXIMATELY THE SAME SIZE.
GAP-GRADED - INDICATES A MIXTURE OF UNIFORM PARTICLE SIZES OF TWO OR MORE SIZES.

ANGULARITY OF GRAINS

THE ANGULARITY OR ROUNDNESS OF SOIL GRAINS IS DESIGNATED BY THE TERMS: ANGULAR, SUBANGULAR, SUBROUNDED, OR ROUNDED.

MINERALOGICAL COMPOSITION

MINERAL NAMES SUCH AS QUARTZ, FELDSPAR, MICA, TALC, KAOLIN, ETC. ARE USED IN DESCRIPTIONS WHEN THEY ARE CONSIDERED OF SIGNIFICANCE.

COMPRESSIBILITY

SLIGHTLY COMPRESSIBLE LL < 31
MODERATELY COMPRESSIBLE LL = 31 - 50
HIGHLY COMPRESSIBLE LL > 50

PERCENTAGE OF MATERIAL

| | ORGANIC MATERIAL | GRANULAR SOILS | SILT - CLAY SOILS | OTHER MATERIAL |
|-------------------------|------------------|----------------|-------------------|----------------|
| TRACE OF ORGANIC MATTER | 2 - 3% | 3 - 5% | TRACE | 1 - 10% |
| LITTLE ORGANIC MATTER | 3 - 5% | 5 - 12% | LITTLE | 10 - 20% |
| MODERATELY ORGANIC | 5 - 10% | 12 - 20% | SOME | 20 - 35% |
| HIGHLY ORGANIC | > 10% | > 20% | HIGHLY | 35% AND ABOVE |

GROUND WATER

- WATER LEVEL IN BORE HOLE IMMEDIATELY AFTER DRILLING
- STATIC WATER LEVEL AFTER 24 HOURS
- PERCHED WATER, SATURATED ZONE, OR WATER BEARING STRATA
- SPRING OR SEEP

MISCELLANEOUS SYMBOLS

- ROADWAY EMBANKMENT (RE) WITH SOIL DESCRIPTION
- SOIL SYMBOL
- ARTIFICIAL FILL (AF) OTHER THAN ROADWAY EMBANKMENT
- INFERRED SOIL BOUNDARY
- INFERRED ROCK LINE
- ALLUVIAL SOIL BOUNDARY
- DIP & DIP DIRECTION OF ROCK STRUCTURES
- SPT TEST BORING
- AUGER BORING
- CORE BORING
- MONITORING WELL
- PIEZOMETER INSTALLATION
- SLOPE INDICATOR INSTALLATION
- CONE PENETROMETER TEST
- SOUNDING ROD
- TEST BORING WITH CORE
- SPT N-VALUE

RECOMMENDATION SYMBOLS

- UNDERCUT
- SHALLOW UNDERCUT
- UNCLASSIFIED EXCAVATION - UNSUITABLE WASTE
- UNCLASSIFIED EXCAVATION - ACCEPTABLE DEGRADABLE ROCK
- UNCLASSIFIED EXCAVATION - ACCEPTABLE, BUT NOT TO BE USED IN THE TOP 3 FEET OF EMBANKMENT OR BACKFILL

ABBREVIATIONS

- AR - AUGER REFUSAL
- BT - BORING TERMINATED
- CL - CLAY
- CPT - COARSE PENETRATION TEST
- CSE - COARSE
- DMT - DILATOMETER TEST
- DPT - DYNAMIC PENETRATION TEST
- e - VOID RATIO
- F - FINE
- FOSS. - FOSSILIFEROUS
- FRAC. - FRACTURED, FRACTURES
- FRAGS. - FRAGMENTS
- HI. - HIGHLY
- MED. - MEDIUM
- MICA - MICACEOUS
- MOD. - MODERATELY
- NP - NON PLASTIC
- ORG. - ORGANIC
- PMT - PRESSUREMETER TEST
- SAP. - SAPROLITIC
- SD. - SAND, SANDY
- SL. - SILT, SILTY
- SLI. - SLIGHTLY
- TCR - TRICONE REFUSAL
- w - MOISTURE CONTENT
- V - VERY
- VST - VANE SHEAR TEST
- WEA. - WEATHERED
- W - UNIT WEIGHT
- W_d - DRY UNIT WEIGHT
- S - BULK
- SS - SPLIT SPOON
- ST - SHELBY TUBE
- RS - ROCK
- RT - RECOMPACTED TRIAXIAL
- CBR - CALIFORNIA BEARING RATIO

EQUIPMENT USED ON SUBJECT PROJECT

- DRILL UNITS:
 - CME-45C
 - CME-55
 - CME-750X
 - VANE SHEAR TEST
 - PORTABLE HOIST
 - DIEDRICH D-70
- ADVANCING TOOLS:
 - CLAY BITS
 - 6" CONTINUOUS FLIGHT AUGER
 - 8" HOLLOW AUGERS
 - HARD FACED FINGER BITS
 - TUNG-CARBIDE INSERTS
 - CASING
 - W/ ADVANCER
 - TRICONE * STEEL TEETH
 - TRICONE * TUNG-CARB.
 - CORE BIT
- HAMMER TYPE:
 - AUTOMATIC
 - MANUAL
- CORE SIZE:
 - B
 - H
 - N 02
- HAND TOOLS:
 - POST HOLE DIGGER
 - HAND AUGER
 - SOUNDING ROD
 - VANE SHEAR TEST

ROCK DESCRIPTION

HARD ROCK IS NON-COASTAL PLAIN MATERIAL THAT WOULD YIELD SPT REFUSAL IF TESTED, AN INFERRED ROCK LINE INDICATES THE LEVEL AT WHICH NON-COASTAL PLAIN MATERIAL WOULD YIELD SPT REFUSAL. SPT REFUSAL IS PENETRATION BY A SPLIT SPOON SAMPLER EQUAL TO OR LESS THAN 0.1 FOOT PER 60 BLOWS IN NON-COASTAL PLAIN MATERIAL. THE TRANSITION BETWEEN SOIL AND ROCK IS OFTEN REPRESENTED BY A ZONE OF WEATHERED ROCK. ROCK MATERIALS ARE TYPICALLY DIVIDED AS FOLLOWS:

- WEATHERED ROCK (WR) - NON-COASTAL PLAIN MATERIAL THAT WOULD YIELD SPT N VALUES > 100 BLOWS PER FOOT IF TESTED.
- CRYSTALLINE ROCK (CR) - FINE TO COARSE GRAIN IGNEOUS AND METAMORPHIC ROCK THAT WOULD YIELD SPT REFUSAL IF TESTED. ROCK TYPE INCLUDES GRANITE, GNEISS, GABBRO, SCHIST, ETC.
- NON-CRYSTALLINE ROCK (NCR) - FINE TO COARSE GRAIN METAMORPHIC AND NON-COASTAL PLAIN SEDIMENTARY ROCK THAT WOULD YIELD SPT REFUSAL IF TESTED. ROCK TYPE INCLUDES PHYLLITE, SLATE, SANDSTONE, ETC.
- COASTAL PLAIN SEDIMENTARY ROCK (CP) - COASTAL PLAIN SEDIMENTS CEMENTED INTO ROCK, BUT MAY NOT YIELD SPT REFUSAL. ROCK TYPE INCLUDES LIMESTONE, SANDSTONE, CEMENTED SHELL BEDS, ETC.

WEATHERING

- FRESH** - ROCK FRESH, CRYSTALS BRIGHT, FEW JOINTS MAY SHOW SLIGHT STAINING. ROCK RINGS UNDER HAMMER IF CRYSTALLINE.
- VERY SLIGHT (V SL.)** - ROCK GENERALLY FRESH, JOINTS STAINED, SOME JOINTS MAY SHOW THIN CLAY COATINGS IF OPEN. CRYSTALS ON A BROKEN SPECIMEN FACE SHINE BRIGHTLY. ROCK RINGS UNDER HAMMER BLOWS IF OF A CRYSTALLINE NATURE.
- SLIGHT (SL.)** - ROCK GENERALLY FRESH, JOINTS STAINED AND DISCOLORATION EXTENDS INTO ROCK UP TO 1 INCH. OPEN JOINTS MAY CONTAIN CLAY. IN GRANITOID ROCKS SOME OCCASIONAL FELDSPAR CRYSTALS ARE DULL AND DISCOLORED. CRYSTALLINE ROCKS RING UNDER HAMMER BLOWS.
- MODERATE (MOD.)** - SIGNIFICANT PORTIONS OF ROCK SHOW DISCOLORATION AND WEATHERING EFFECTS. IN GRANITOID ROCKS, MOST FELDSPARS ARE DULL AND DISCOLORED, SOME SHOW CLAY. ROCK HAS DULL SOUND UNDER HAMMER BLOWS AND SHOWS SIGNIFICANT LOSS OF STRENGTH AS COMPARED WITH FRESH ROCK.
- MODERATELY SEVERE (MOD. SEV.)** - ALL ROCK EXCEPT QUARTZ DISCOLORED OR STAINED. IN GRANITOID ROCKS, ALL FELDSPARS DULL AND DISCOLORED AND A MAJORITY SHOW KAOLINIZATION. ROCK SHOWS SEVERE LOSS OF STRENGTH AND CAN BE EXCAVATED WITH A GEOLOGIST'S PICK. ROCK GIVES "CLUNK" SOUND WHEN STRUCK. *IF TESTED, WOULD YIELD SPT REFUSAL*
- SEVERE (SEV.)** - ALL ROCK EXCEPT QUARTZ DISCOLORED OR STAINED. ROCK FABRIC CLEAR AND EVIDENT BUT REDUCED IN STRENGTH TO STRONG SOIL. IN GRANITOID ROCKS ALL FELDSPARS ARE KAOLINIZED TO SOME EXTENT. SOME FRAGMENTS OF STRONG ROCK USUALLY REMAIN. *IF TESTED, WOULD YIELD SPT N VALUES > 100 BPF*
- VERY SEVERE (V SEV.)** - ALL ROCK EXCEPT QUARTZ DISCOLORED OR STAINED. ROCK FABRIC ELEMENTS ARE DISCERNIBLE BUT MASS IS EFFECTIVELY REDUCED TO SOIL STATUS, WITH ONLY FRAGMENTS OF STRONG ROCK REMAINING. SAPROLITE IS AN EXAMPLE OF ROCK WEATHERED TO A DEGREE THAT ONLY MINOR VESTIGES OF ORIGINAL ROCK FABRIC REMAIN. *IF TESTED, WOULD YIELD SPT N VALUES < 100 BPF*
- COMPLETE** - ROCK REDUCED TO SOIL. ROCK FABRIC NOT DISCERNIBLE, OR DISCERNIBLE ONLY IN SMALL AND SCATTERED CONCENTRATIONS. FABRIC MAY BE PRESENT AS DIKES OR STRINGERS. SAPROLITE IS ALSO AN EXAMPLE.

ROCK HARDNESS

- VERY HARD** - CANNOT BE SCRATCHED BY KNIFE OR SHARP PICK. BREAKING OF HAND SPECIMENS REQUIRES SEVERAL HARD BLOWS OF THE GEOLOGIST'S PICK.
- HARD** - CAN BE SCRATCHED BY KNIFE OR PICK ONLY WITH DIFFICULTY. HARD HAMMER BLOWS REQUIRED TO DETACH HAND SPECIMEN.
- MODERATELY HARD** - CAN BE SCRATCHED BY KNIFE OR PICK, GOUGES OR GROOVES TO 0.25 INCHES DEEP CAN BE EXCAVATED BY HARD BLOW OF A GEOLOGIST'S PICK. HAND SPECIMENS CAN BE DETACHED BY MODERATE BLOWS.
- MEDIUM HARD** - CAN BE GROOVED OR GOUGED 0.05 INCHES DEEP BY FIRM PRESSURE OF KNIFE OR PICK POINT. CAN BE EXCAVATED IN SMALL CHIPS TO PIECES 1 INCH MAXIMUM SIZE BY HARD BLOWS OF THE POINT OF A GEOLOGIST'S PICK.
- SOFT** - CAN BE GROOVED OR GOUGED READILY BY KNIFE OR PICK. CAN BE EXCAVATED IN FRAGMENTS FROM CHIPS TO SEVERAL INCHES IN SIZE BY MODERATE BLOWS OF A PICK POINT. SMALL, THIN PIECES CAN BE BROKEN BY FINGER PRESSURE.
- VERY SOFT** - CAN BE CARVED WITH KNIFE. CAN BE EXCAVATED READILY WITH POINT OF PICK. PIECES 1 INCH OR MORE IN THICKNESS CAN BE BROKEN BY FINGER PRESSURE. CAN BE SCRATCHED READILY BY FINGER NAIL.

FRACTURE SPACING

| TERM | SPACING | TERM | THICKNESS |
|------------------|---------------------|---------------------|-------------------|
| VERY WIDE | MORE THAN 10 FEET | VERY THICKLY BEDDED | 4 FEET |
| WIDE | 3 TO 10 FEET | THICKLY BEDDED | 1.5 - 4 FEET |
| MODERATELY CLOSE | 1 TO 3 FEET | THINLY BEDDED | 0.16 - 1.5 FEET |
| CLOSE | 0.16 TO 1 FOOT | VERY THINLY BEDDED | 0.03 - 0.16 FEET |
| VERY CLOSE | LESS THAN 0.16 FEET | THICKLY LAMINATED | 0.008 - 0.03 FEET |
| | | THINLY LAMINATED | < 0.008 FEET |

INDURATION

- FRIABLE** - RUBBING WITH FINGER FREES NUMEROUS GRAINS; GENTLE BLOW BY HAMMER DISINTEGRATES SAMPLE.
- MODERATELY INDURATED** - GRAINS CAN BE SEPARATED FROM SAMPLE WITH STEEL PROBE; BREAKS EASILY WHEN HIT WITH HAMMER.
- INDURATED** - GRAINS ARE DIFFICULT TO SEPARATE WITH STEEL PROBE; DIFFICULT TO BREAK WITH HAMMER.
- EXTREMELY INDURATED** - SHARP HAMMER BLOWS REQUIRED TO BREAK SAMPLE; SAMPLE BREAKS ACROSS GRAINS.

TERMS AND DEFINITIONS

- ALLUVIUM (ALLUV.)** - SOILS THAT HAVE BEEN TRANSPORTED BY WATER.
 - AQUIFER** - A WATER BEARING FORMATION OR STRATA.
 - ARENACEOUS** - APPLIED TO ROCKS THAT HAVE BEEN DERIVED FROM SAND OR THAT CONTAIN SAND.
 - ARGILLACEOUS** - APPLIED TO ALL ROCKS OR SUBSTANCES COMPOSED OF CLAY MINERALS, OR HAVING A NOTABLE PROPORTION OF CLAY IN THEIR COMPOSITION, SUCH AS SHALE, SLATE, ETC.
 - ARTESIAN** - GROUND WATER THAT IS UNDER SUFFICIENT PRESSURE TO RISE ABOVE THE LEVEL AT WHICH IT IS ENCOUNTERED, BUT WHICH DOES NOT NECESSARILY RISE TO OR ABOVE THE GROUND SURFACE.
 - CALCAREOUS (CALC.)** - SOILS THAT CONTAIN APPRECIABLE AMOUNTS OF CALCIUM CARBONATE.
 - COLLUVIUM** - ROCK FRAGMENTS MIXED WITH SOIL DEPOSITED BY GRAVITY ON SLOPE OR AT BOTTOM OF SLOPE.
 - CORE RECOVERY (REC.)** - TOTAL LENGTH OF ALL MATERIAL RECOVERED IN THE CORE BARREL DIVIDED BY TOTAL LENGTH OF CORE RUN AND EXPRESSED AS A PERCENTAGE.
 - DIKE** - A TABULAR BODY OF IGNEOUS ROCK THAT CUTS ACROSS THE STRUCTURE OF ADJACENT ROCKS OR CUTS MASSIVE ROCK.
 - DIP** - THE ANGLE AT WHICH A STRATUM OR ANY PLANAR FEATURE IS INCLINED FROM THE HORIZONTAL.
 - DIP DIRECTION (DIP AZIMUTH)** - THE DIRECTION OR BEARING OF THE HORIZONTAL TRACE OF THE LINE OF DIP, MEASURED CLOCKWISE FROM NORTH.
 - FAULT** - A FRACTURE OR FRACTURE ZONE ALONG WHICH THERE HAS BEEN DISPLACEMENT OF THE SIDES RELATIVE TO ONE ANOTHER PARALLEL TO THE FRACTURE.
 - FISSILE** - A PROPERTY OF SPLITTING ALONG CLOSELY SPACED PARALLEL PLANES.
 - FLOAT** - ROCK FRAGMENTS ON SURFACE NEAR THEIR ORIGINAL POSITION AND DISLOADED FROM PARENT MATERIAL.
 - FLOOD PLAIN (FP)** - LAND BORDERING A STREAM, BUILT OF SEDIMENTS DEPOSITED BY THE STREAM.
 - FORMATION (FM.)** - A MAPPABLE GEOLOGIC UNIT THAT CAN BE RECOGNIZED AND TRACED IN THE FIELD.
 - JOINT** - FRACTURE IN ROCK ALONG WHICH NO APPRECIABLE MOVEMENT HAS OCCURRED.
 - LEDGE** - A SHELF-LIKE RIDGE OR PROJECTION OF ROCK WHOSE THICKNESS IS SMALL COMPARED TO ITS LATERAL EXTENT.
 - LENS** - A BODY OF SOIL OR ROCK THAT THINS OUT IN ONE OR MORE DIRECTIONS.
 - MOTTLED (MOT.)** - IRREGULARLY MARKED WITH SPOTS OF DIFFERENT COLORS. MOTTLING IN SOILS USUALLY INDICATES POOR AERATION AND LACK OF GOOD DRAINAGE.
 - PERCHED WATER** - WATER MAINTAINED ABOVE THE NORMAL GROUND WATER LEVEL BY THE PRESENCE OF AN INTERVENING IMPERVIOUS STRATUM.
 - RESIDUAL (RES.) SOIL** - SOIL FORMED IN PLACE BY THE WEATHERING OF ROCK.
 - ROCK QUALITY DESIGNATION (RQD)** - A MEASURE OF ROCK QUALITY DESCRIBED BY TOTAL LENGTH OF ROCK SEGMENTS EQUAL TO OR GREATER THAN 4 INCHES DIVIDED BY THE TOTAL LENGTH OF CORE RUN AND EXPRESSED AS A PERCENTAGE.
 - SAPROLITE (SAP.)** - RESIDUAL SOIL THAT RETAINS THE RELIC STRUCTURE OR FABRIC OF THE PARENT ROCK.
 - SILL** - AN INTRUSIVE BODY OF IGNEOUS ROCK OF APPROXIMATELY UNIFORM THICKNESS AND RELATIVELY THIN COMPARED WITH ITS LATERAL EXTENT, THAT HAS BEEN EMPLACED PARALLEL TO THE BEDDING OR SCHISTOSITY OF THE INTRUDED ROCKS.
 - SLICKENSIDE** - POLISHED AND STRIATED SURFACE THAT RESULTS FROM FRICTION ALONG A FAULT OR SLIP PLANE.
 - STANDARD PENETRATION TEST (PENETRATION RESISTANCE) (SPT)** - NUMBER OF BLOWS (N OR BPF) OF A 140 LB. HAMMER FALLING 30 INCHES REQUIRED TO PRODUCE A PENETRATION OF 1 FOOT INTO SOIL WITH A 2 INCH OUTSIDE DIAMETER SPLIT SPOON SAMPLER. SPT REFUSAL IS PENETRATION EQUAL TO OR LESS THAN 0.1 FOOT PER 60 BLOWS.
 - STRATA CORE RECOVERY (SREC.)** - TOTAL LENGTH OF STRATA MATERIAL RECOVERED DIVIDED BY TOTAL LENGTH OF STRATUM AND EXPRESSED AS A PERCENTAGE.
 - STRATA ROCK QUALITY DESIGNATION (SROD)** - A MEASURE OF ROCK QUALITY DESCRIBED BY TOTAL LENGTH OF ROCK SEGMENTS WITHIN A STRATUM EQUAL TO OR GREATER THAN 4 INCHES DIVIDED BY THE TOTAL LENGTH OF STRATA AND EXPRESSED AS A PERCENTAGE.
 - TOPSOIL (TS.)** - SURFACE SOILS USUALLY CONTAINING ORGANIC MATTER.
- BENCH MARK:**
BM-1 (N737693 E1673323)
STA. 23+07, I21' RT ELEVATION: 779.6 FEET
- NOTES:**
ROADWAY DESIGN FILES, .DTM FILE PROVIDED BY NCDOT.
NORTHING AND EASTINGS OBTAINED USING A TRIMBLE GEO7X.
ELEVATIONS FOR BRIDGE BORINGS OBTAINED USING BENCHMARK BM-1 (N737693 E1673323)
FIAD = FILLED IMMEDIATELY AFTER DRILLING

**NORTH CAROLINA DEPARTMENT OF TRANSPORTATION
DIVISION OF HIGHWAYS
GEOTECHNICAL ENGINEERING UNIT**

SUBSURFACE INVESTIGATION

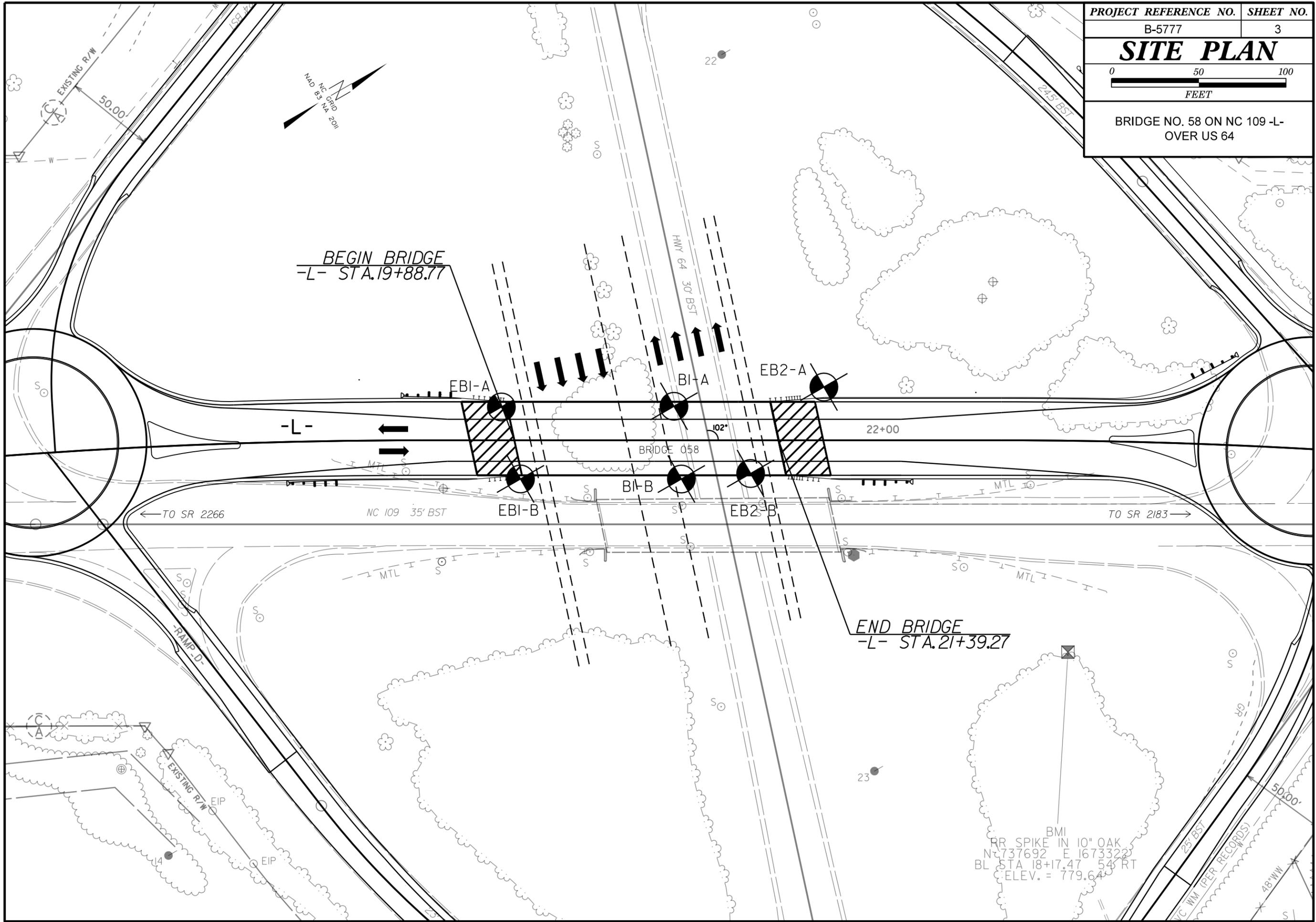
**SUPPLEMENTAL LEGEND, GEOLOGICAL STRENGTH INDEX (GSI) TABLES
FROM AASHTO LRFD BRIDGE DESIGN SPECIFICATIONS**

AASHTO LRFD Figure 10.4.6.4-1 — Determination of GSI for Jointed Rock Mass (Marinos and Hoek, 2000)

AASHTO LRFD Figure 10.4.6.4-2 — Determination of GSI for Tectonically Deformed Heterogeneous Rock Masses (Marinos and Hoek, 2000)

| <p>GEOLOGICAL STRENGTH INDEX (GSI) FOR JOINTED ROCKS (Hoek and Marinos, 2000)</p> <p>From the lithology, structure and surface conditions of the discontinuities, estimate the average value of GSI. Do not try to be too precise. Quoting a range from 33 to 37 is more realistic than stating that GSI = 35. Note that the table does not apply to structurally controlled failures. Where weak planar structural planes are present in an unfavorable orientation with respect to the excavation face, these will dominate the rock mass behaviour. The shear strength of surfaces in rocks that are prone to deterioration as a result of changes in moisture content will be reduced if water is present. When working with rocks in the fair to very poor categories, a shift to the right may be made for wet conditions. Water pressure is dealt with by effective stress analysis.</p> <p>STRUCTURE</p> | <p>SURFACE CONDITIONS</p> | <p>VERY GOOD Very rough, fresh unweathered surfaces</p> | <p>GOOD Rough, slightly weathered, iron stained surfaces</p> | <p>FAIR Smooth, moderately weathered and altered surfaces</p> | <p>POOR Slickensided, highly weathered surfaces with compact coatings or fillings or angular fragments</p> | <p>VERY POOR Slickensided, highly weathered surfaces with soft clay coatings or fillings</p> |
|---|--|--|---|--|---|--|
| <p>STRUCTURE</p> | <p>SURFACE CONDITIONS OF DISCONTINUITIES (Predominantly bedding planes)</p> | <p>VERY GOOD - Very Rough, fresh unweathered surfaces</p> | <p>GOOD - Rough, slightly weathered surfaces</p> | <p>FAIR - Smooth, moderately weathered and altered surfaces</p> | <p>POOR - Very smooth, occasionally slickensided surfaces with compact coatings or fillings with angular fragments</p> | <p>VERY POOR - Very smooth, slickensided or highly weathered surfaces with soft clay coatings or fillings</p> |
| <p>INTACT OR MASSIVE - intact rock specimens or massive in situ rock with few widely spaced discontinuities</p> <p>BLOCKY - well interlocked undisturbed rock mass consisting of cubical blocks formed by three intersecting discontinuity sets</p> <p>VERY BLOCKY - interlocked, partially disturbed mass with multi-faceted angular blocks formed by 4 or more joint sets</p> <p>BLOCKY/DISTURBED/SEAMY - folded with angular blocks formed by many intersecting discontinuity sets. Persistence of bedding planes or schistosity</p> <p>DISINTEGRATED - poorly interlocked, heavily broken rock mass with mixture of angular and rounded rock pieces</p> <p>LAMINATED/SHEARED - Lack of blockiness due to close spacing of weak schistosity or shear planes</p> | <p>DECREASING INTERLOCKING OF ROCK PIECES</p> | <p>DECREASING SURFACE QUALITY →</p> | | | | |
| <p>COMPOSITION AND STRUCTURE</p> <p>A. Thick bedded, very blocky sandstone The effect of pelitic coatings on the bedding planes is minimized by the confinement of the rock mass. In shallow tunnels or slopes these bedding planes may cause structurally controlled instability.</p> <p>B. Sandstone with thin inter-layers of siltstone</p> <p>C. Sandstone and siltstone in similar amounts</p> <p>D. Siltstone or silty shale with sandstone layers</p> <p>E. Weak siltstone or clayey shale with sandstone layers</p> <p>F. Tectonically deformed, intensively folded/faulted, sheared clayey shale or siltstone with broken and deformed sandstone layers forming an almost chaotic structure</p> <p>G. Undisturbed silty or clayey shale with or without a few very thin sandstone layers</p> <p>H. Tectonically deformed silty or clayey shale forming a chaotic structure with pockets of clay. Thin layers of sandstone are transformed into small rock pieces.</p> <p>→ Means deformation after tectonic disturbance</p> | <p>COMPOSITION AND STRUCTURE</p> | <p>VERY GOOD - Very Rough, fresh unweathered surfaces</p> | <p>GOOD - Rough, slightly weathered surfaces</p> | <p>FAIR - Smooth, moderately weathered and altered surfaces</p> | <p>POOR - Very smooth, occasionally slickensided surfaces with compact coatings or fillings with angular fragments</p> | <p>VERY POOR - Very smooth, slickensided or highly weathered surfaces with soft clay coatings or fillings</p> |
| | | <p>90</p> | <p>80</p> | <p>70</p> | <p>60</p> | <p>50</p> |
| | | <p>N/A</p> | <p>N/A</p> | <p>N/A</p> | <p>N/A</p> | <p>N/A</p> |
| | | <p>70</p> | <p>60</p> | <p>50</p> | <p>40</p> | <p>30</p> |
| | | <p>60</p> | <p>50</p> | <p>40</p> | <p>30</p> | <p>20</p> |
| | | <p>50</p> | <p>40</p> | <p>30</p> | <p>20</p> | <p>10</p> |
| | | <p>N/A</p> | <p>N/A</p> | <p>N/A</p> | <p>N/A</p> | <p>N/A</p> |

| | |
|---|-----------|
| PROJECT REFERENCE NO. | SHEET NO. |
| B-5777 | 3 |
| SITE PLAN | |
| | |
| BRIDGE NO. 58 ON NC 109 -L- OVER US 64 | |



GEOTECHNICAL BORING REPORT

BORE LOG

| WBS 45733.1.1 | | TIP B-5777 | | COUNTY DAVIDSON | | GEOLOGIST A. Suttle | | | | | | | | | | |
|--|-----------------|---------------------|------------|--------------------------|--------|-------------------------|-----------------|----|----|-----|-----------|-----|---------------------------|------------|------|---|
| SITE DESCRIPTION Replace Bridge No. 58 on NC 109 over US 64 | | | | | | | GROUND WTR (ft) | | | | | | | | | |
| BORING NO. EB1-A | | STATION 19+81 | | OFFSET 19 ft LT | | ALIGNMENT -L- | 0 HR. 36.3 | | | | | | | | | |
| COLLAR ELEV. 781.9 ft | | TOTAL DEPTH 38.7 ft | | NORTHING 737,481 | | EASTING 1,673,040 | 24 HR. Caved | | | | | | | | | |
| DRILL RIG/HAMMER EFF./DATE ECS049 Diedrich D-70 94% 04/02/2024 | | | | DRILL METHOD H.S. Augers | | HAMMER TYPE Automatic | | | | | | | | | | |
| DRILLER C. Osborne | | START DATE 04/24/24 | | COMP. DATE 04/24/24 | | SURFACE WATER DEPTH N/A | | | | | | | | | | |
| ELEV (ft) | DRIVE ELEV (ft) | DEPTH (ft) | BLOW COUNT | | | BLOWS PER FOOT | | | | | SAMP. NO. | LOG | SOIL AND ROCK DESCRIPTION | | | |
| | | | 0.5ft | 0.5ft | 0.5ft | 0 | 25 | 50 | 75 | 100 | | | ELEV. (ft) | DEPTH (ft) | | |
| 785 | | | | | | | | | | | | | | | | |
| | 780.9 | 1.0 | | 4 | 4 | 4 | | | | | | | M | 781.9 | 0.0 | GROUND SURFACE |
| 780 | 778.4 | 3.5 | | 3 | 4 | 6 | | | | | | | M | | | ROADWAY EMBANKMENT Medium Stiff to Very Stiff, Tan-Red-Orange-Gray, Silty CLAY (A-7-5/A-7-6), with trace gravel |
| | 775.9 | 6.0 | | 9 | 8 | 9 | | | | | | | M | | | |
| 775 | 773.4 | 8.5 | | 18 | 40 | 43 | | | | | | | M | 773.9 | 8.0 | RESIDUAL Hard, Tan-Gray, Fine to Coarse Sandy SILT (A-4), with some rock fragments, clay seams |
| | 768.4 | 13.5 | | 19 | 20 | 19 | | | | | | | M | | | |
| 770 | 763.4 | 18.5 | | 18 | 82/0.3 | | | | | | | | M | 763.4 | 18.5 | WEATHERED ROCK Tan-Gray (META-ARGILLITE) |
| | 758.4 | 23.5 | | 25 | 39 | 61/0.2 | | | | | | | M | | | |
| 765 | 753.4 | 28.5 | | 17 | 45 | 39 | | | | | | | M | 753.9 | 28.0 | RESIDUAL Hard, Tan-Gray, Fine to Coarse Sandy SILT (A-4), with some rock fragments |
| | 748.4 | 33.5 | | 11 | 51 | 49/0.4 | | | | | | | M | 747.9 | 34.0 | WEATHERED ROCK Gray-Tan (META-ARGILLITE) |
| 750 | 743.3 | 38.6 | | | | | | | | | | | | 743.3 | 38.6 | NON-CRYSTALLINE ROCK Gray (META-ARGILLITE) Boring Terminated with Standard Penetration Test Refusal at Elevation 743.2 ft In Non-Crystalline Rock (META-ARGILLITE) Surficial Organic Soil (0.0' to 0.3') |
| 745 | | | | | | | | | | | | | | 743.2 | 38.7 | |

NCDOT BORE DOUBLE B5777_GEO_GTM.GPJ NC_DOT.GDT 5/11/24

GEOTECHNICAL BORING REPORT

BORE LOG

| WBS 45733.1.1 | | TIP B-5777 | | COUNTY DAVIDSON | | GEOLOGIST A. Suttle | | | | | | | | | | | |
|--|-----------------|------------------------------|------------|-----------------------|-------|-------------------------|-----------------|----|----|-----|-----------|-----|---------------------------|------------|--|------|--|
| SITE DESCRIPTION Replace Bridge No. 58 on NC 109 over US 64 | | | | | | | GROUND WTR (ft) | | | | | | | | | | |
| BORING NO. EB1-B | | STATION 19+92 | | OFFSET 22 ft RT | | ALIGNMENT -L- | | | | | | | | | | | |
| COLLAR ELEV. 784.8 ft | | TOTAL DEPTH 35.9 ft | | NORTHING 737,470 | | EASTING 1,673,081 | | | | | | | | | | | |
| DRILL RIG/HAMMER EFF./DATE ECS049 Diedrich D-70 94% 04/02/2024 | | DRILL METHOD SPT Core Boring | | HAMMER TYPE Automatic | | | | | | | | | | | | | |
| DRILLER C. Osborne | | START DATE 04/24/24 | | COMP. DATE 04/24/24 | | SURFACE WATER DEPTH N/A | | | | | | | | | | | |
| ELEV (ft) | DRIVE ELEV (ft) | DEPTH (ft) | BLOW COUNT | | | BLOWS PER FOOT | | | | | SAMP. NO. | LOG | SOIL AND ROCK DESCRIPTION | DEPTH (ft) | | | |
| | | | 0.5ft | 0.5ft | 0.5ft | 0 | 25 | 50 | 75 | 100 | | | | | | | |
| 785 | | | | | | | | | | | | | | 784.8 | GROUND SURFACE | 0.0 | |
| | 783.8 | 1.0 | 2 | 2 | 3 | | | | | | | M | | | ROADWAY EMBANKMENT | | |
| | 781.3 | 3.5 | 4 | 5 | 3 | | | | | | | M | | | Medium Stiff to Stiff, Red-Brown-Tan-Gray, Silty CLAY (A-7-5), with trace to little gravel | | |
| 780 | 778.8 | 6.0 | 4 | 8 | 6 | | | | | | | M | | | | | |
| | 776.3 | 8.5 | 4 | 7 | 18 | | | | | | | M | | | | | |
| 775 | 771.3 | 13.5 | 8 | 36 | 49 | | | | | | | M | | | RESIDUAL | 8.0 | |
| | 766.3 | 18.5 | 20 | 80/0.5 | | | | | | | | M | | | Very Stiff to Hard, Tan-Gray-Orange, Fine to Coarse Sandy SILT (A-4), with little to some rock fragments | | |
| 770 | 761.3 | 23.5 | 100/0.5 | | | | | | | | | M | | | | | |
| | 759.0 | 25.8 | 60/0.1 | | | | | | | | | M | | | | | |
| 765 | 758.9 | 25.9 | | | | | | | | | | | | | WEATHERED ROCK | 18.5 | |
| | | | | | | | | | | | | | | | Tan-Gray (META-ARGILLITE) | | |
| 760 | | | | | | | | | | | | | | | | | |
| | | | | | | | | | | | | | | | | | |
| 755 | | | | | | | | | | | | | | | | | |
| | | | | | | | | | | | | | | | | | |
| 750 | | | | | | | | | | | | | | | | | |
| | | | | | | | | | | | | | | | | | |

| WBS 45733.1.1 | | TIP B-5777 | | COUNTY DAVIDSON | | GEOLOGIST A. Suttle | | | | | | |
|--|---------------|------------------------------|----------|--|---------------|-------------------------|-----------------|--------------|--------------|-----|---|--------------|
| SITE DESCRIPTION Replace Bridge No. 58 on NC 109 over US 64 | | | | | | | GROUND WTR (ft) | | | | | |
| BORING NO. EB1-B | | STATION 19+92 | | OFFSET 22 ft RT | | ALIGNMENT -L- | | | | | | |
| COLLAR ELEV. 784.8 ft | | TOTAL DEPTH 35.9 ft | | NORTHING 737,470 | | EASTING 1,673,081 | | | | | | |
| DRILL RIG/HAMMER EFF./DATE ECS049 Diedrich D-70 94% 04/02/2024 | | DRILL METHOD SPT Core Boring | | HAMMER TYPE Automatic | | | | | | | | |
| DRILLER C. Osborne | | START DATE 04/24/24 | | COMP. DATE 04/24/24 | | SURFACE WATER DEPTH N/A | | | | | | |
| ELEV (ft) | RUN ELEV (ft) | DEPTH (ft) | RUN (ft) | DRILL RATE (Min/ft) | TOTAL RUN | | SAMP. NO. | STRATA | | LOG | DESCRIPTION AND REMARKS | DEPTH (ft) |
| | | | | | REC. (ft) % | RQD (ft) % | | REC. (ft) % | RQD (ft) % | | | |
| 759 | 759.0 | 25.8 | 3.6 | 2:56/1.0 N=60/0.1 | (3.3) 92% | (2.2) 61% | | (9.8) 98% | (6.1) 61% | | Begin Coring @ 25.8 ft | |
| | 758.9 | 25.9 | | | | | | | | | NON-CRYSTALLINE ROCK | 25.8 25.9 |
| 755 | 755.4 | 29.4 | 5.0 | 2:56/1.0 2:36/1.0 4:14/1.0 2:58/0.6 | (5.0) 100% | (3.2) 64% | | | | | Slight to Very Slightly Weathered, Hard to Very Hard, Brown-Gray META-ARGILLITE, with Very Close to Close Fracture Spacing, Indurated to Extremely Indurated, with clay seams present between fractures | |
| | | | | | | | | | | | GSI = 65-70 | |
| 750 | 750.4 | 34.4 | 1.5 | 2:42/1.0 3:08/1.0 3:11/1.0 2:49/1.0 2:32/1.0 | (1.5) 100% | (0.7) 47% | | | | | | |
| | 748.9 | 35.9 | | 2:52/1.0 1:39/0.5 | | | | | | | Boring Terminated at Elevation 748.9 ft In Non-Crystalline Rock (META-ARGILLITE) | 35.9 |
| | | | | | | | | | | | Surficial Organic Soil (0.0' to 0.3') | |

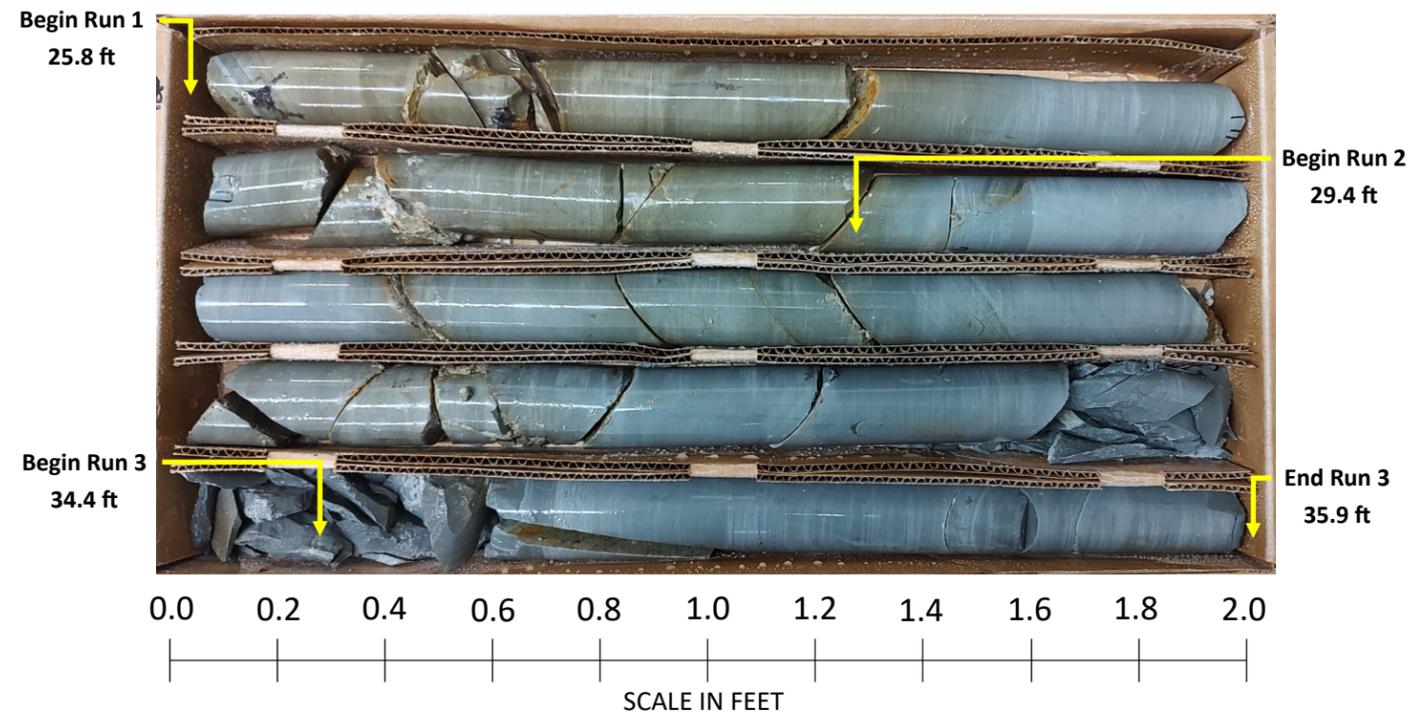


Replace Bridge No. 58 on NC 109 over US 64

WBS - 45733.1.1 TIP No. B-5777

Rock Core Photographs: Boring - EB1-B

Station: 19+92 Offset: 22' RT



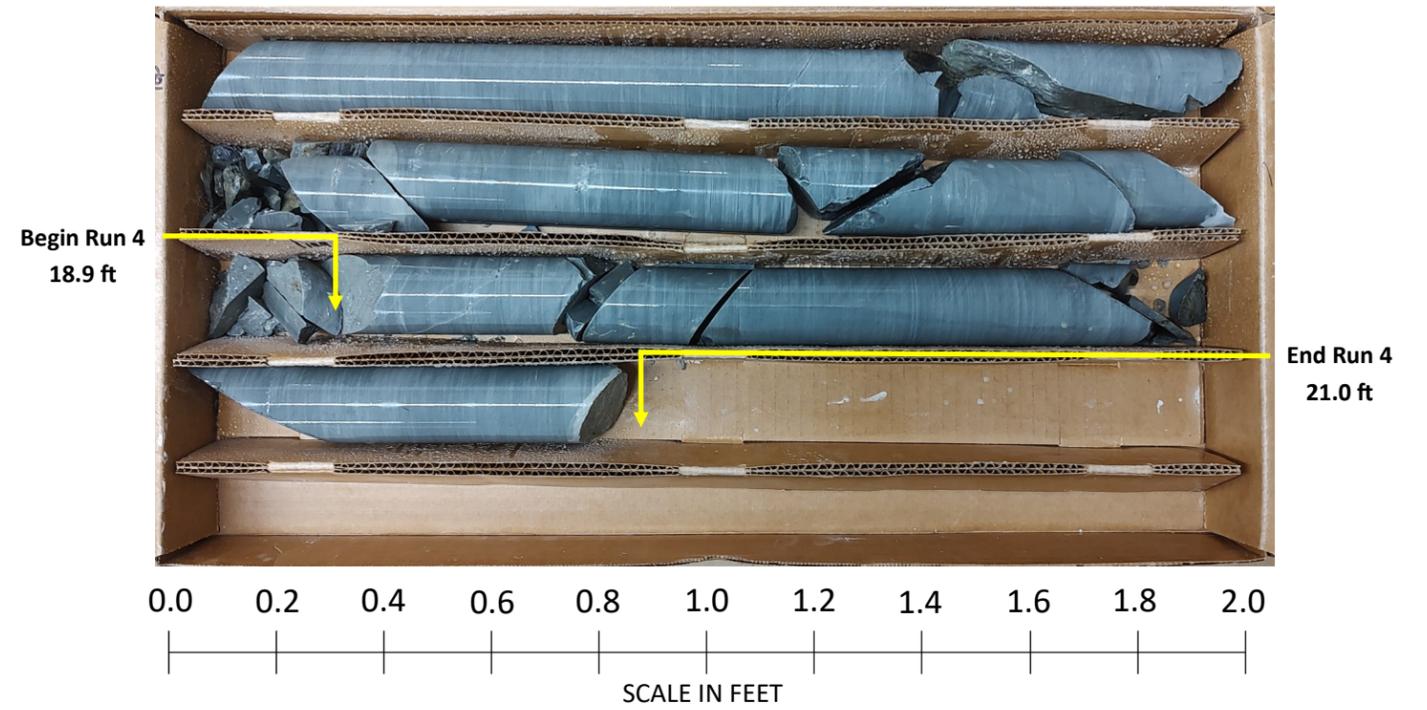
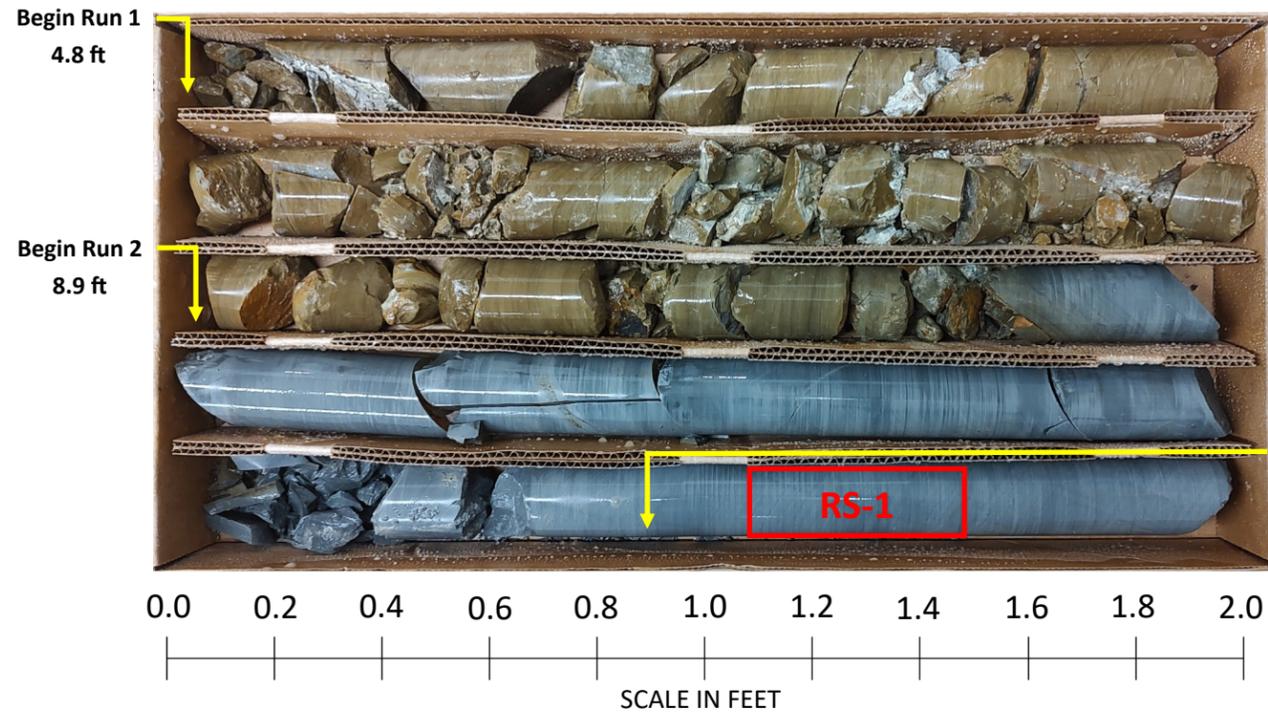


Replace Bridge No. 58 on NC 109 over US 64

WBS - 45733.1.1 TIP No. B-5777

Rock Core Photographs: Boring - B1-A

Station: 20+80 Offset: 19' LT



*See log for note

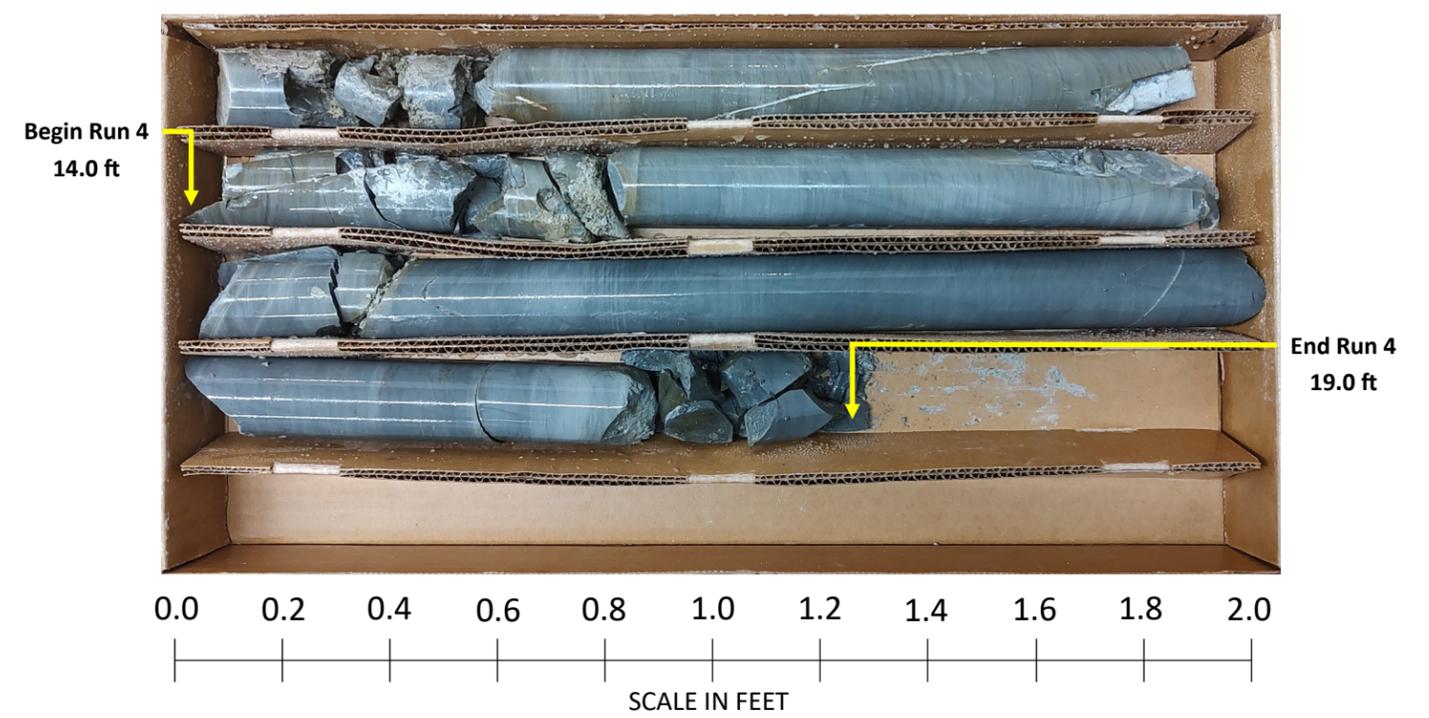
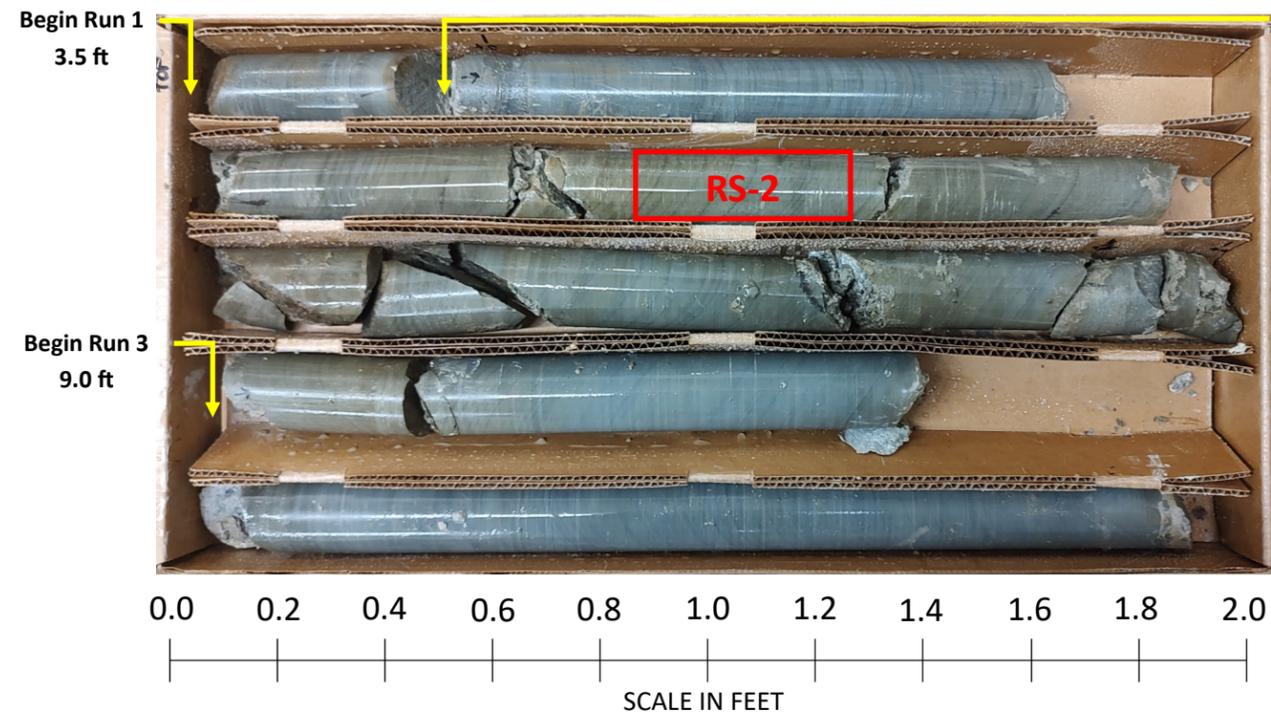


Replace Bridge No. 58 on NC 109 over US 64

WBS - 45733.1.1 TIP No. B-5777

Rock Core Photographs: Boring - B1-B

Station: 20+84 Offset: 22' RT



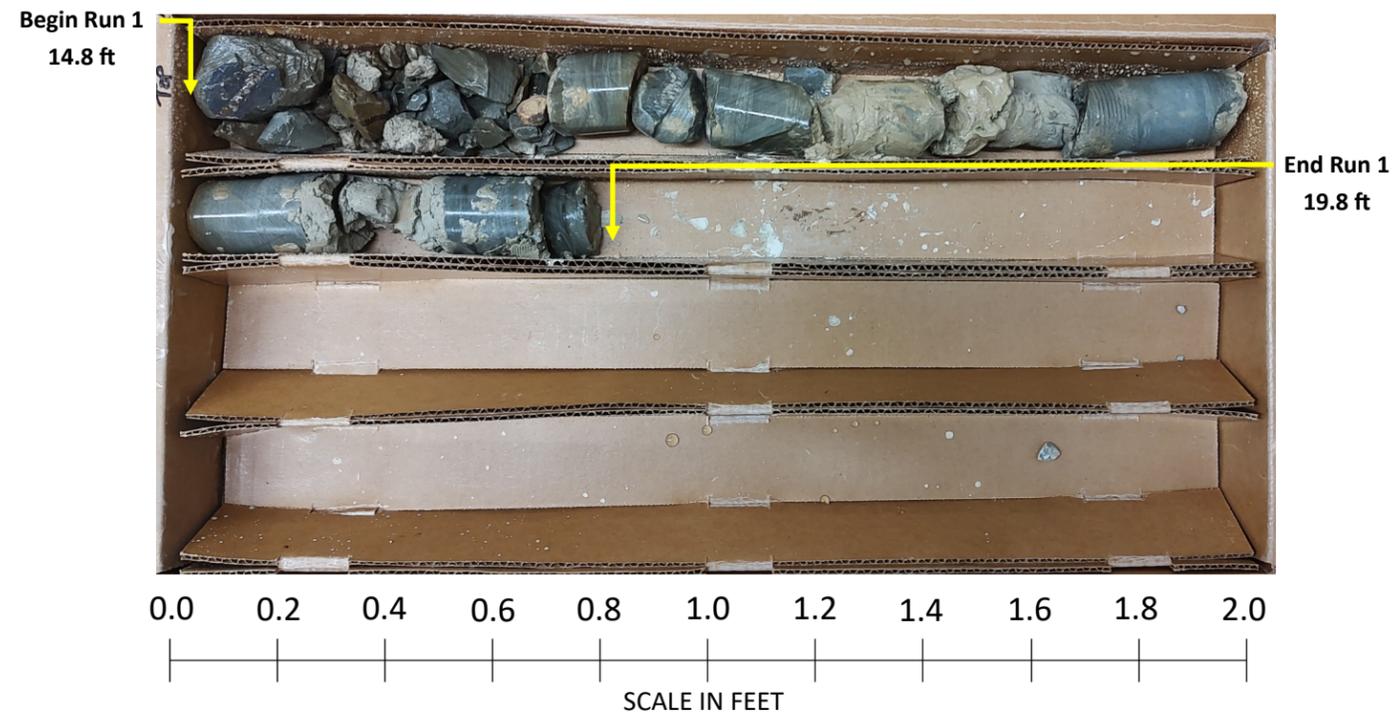


Replace Bridge No. 58 on NC 109 over US 64

WBS - 45733.1.1 TIP No. B-5777

Rock Core Photographs: Boring - EB2-A

Station: 21+66 Offset: 30' LT



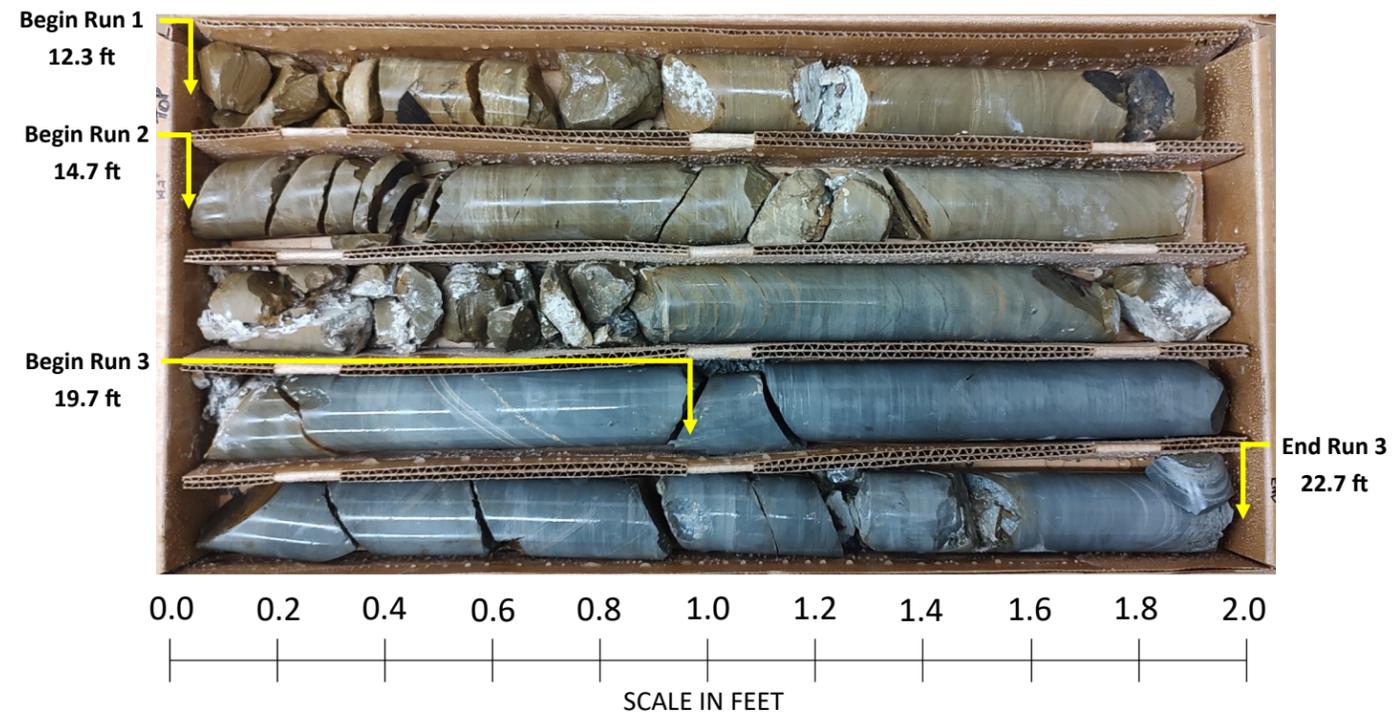


Replace Bridge No. 58 on NC 109 over US 64

WBS - 45733.1.1 TIP No. B-5777

Rock Core Photographs: Boring - EB2-A (2)

Station: 21+70 Offset: 33' LT



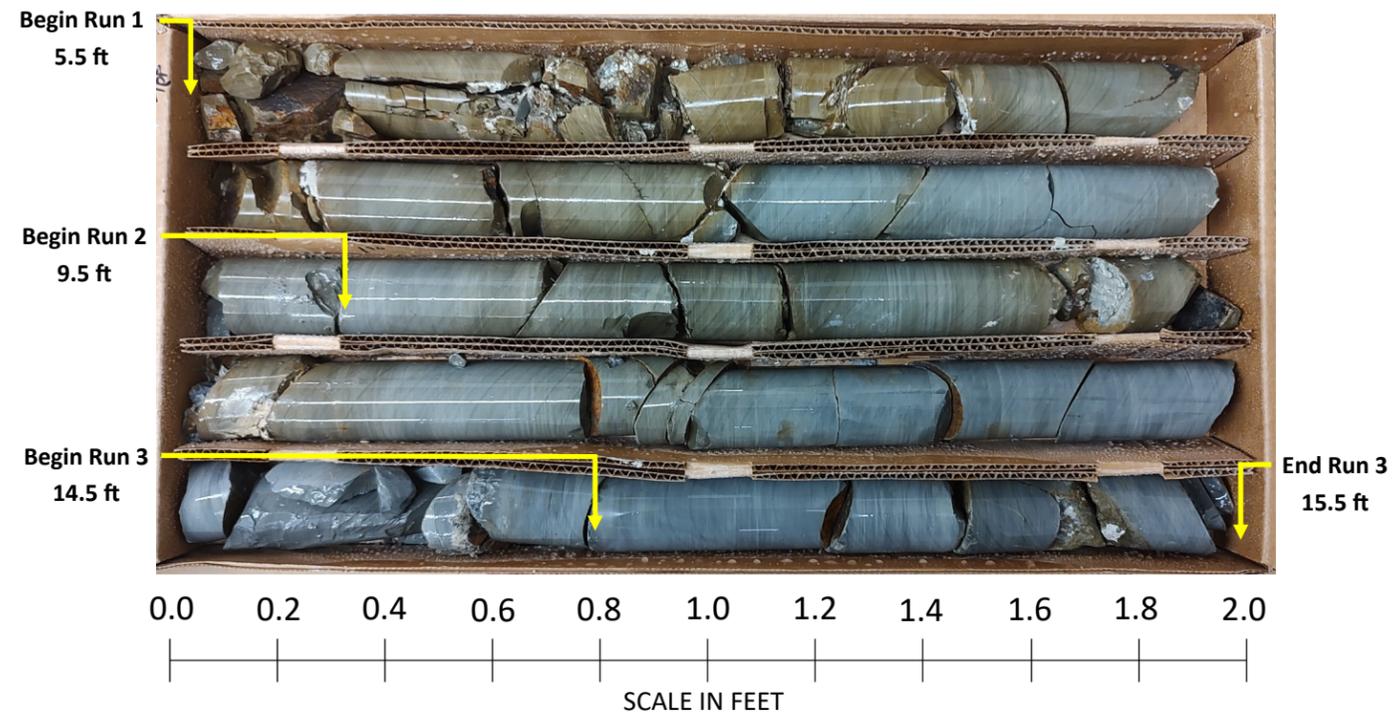


Replace Bridge No. 58 on NC 109 over US 64

WBS - 45733.1.1 TIP No. B-5777 Rock

Core Photographs: Boring - EB2-B

Station: 21+24 Offset: 19' RT



ROCK TEST RESULTS

| SAMPLE NO. | BORING | STATION -L- | OFFSET | DEPTH INTERVAL | RUN REC (%) | RUN RQD (%) | Rock Type | Unit Weight LB/FT ³ | Unconfined Compressive Strength (PSI/KSF) |
|------------|--------|----------------|--------|-------------------|----------------|----------------|----------------|-----------------------------------|--|
| RS-1 | B1-A | 20+80 | 19' LT | 14.1-14.5 | 100 | 76 | Meta-Argillite | 172.6 | 10,770 psi / 1,551 ksf |
| RS-2 | B1-B | 20+84 | 22' RT | 6.2-6.6 | 100 | 74 | Meta-Argillite | 169.3 | 4,590 psi / 661 ksf |

RS = NQ2 Rock Core Barrel Sample (ASTM D-2113)



PHOTO 1: VIEW FROM -L- REALIGNMENT OF PROPOSED BRIDGE APPROACH AT END BENT 1, LOOKING UPSTATION.



PHOTO 2: VIEW FROM -L- REALIGNMENT OF PROPOSED BRIDGE APPROACH AT END BENT 2, LOOKING DOWNSTATION.

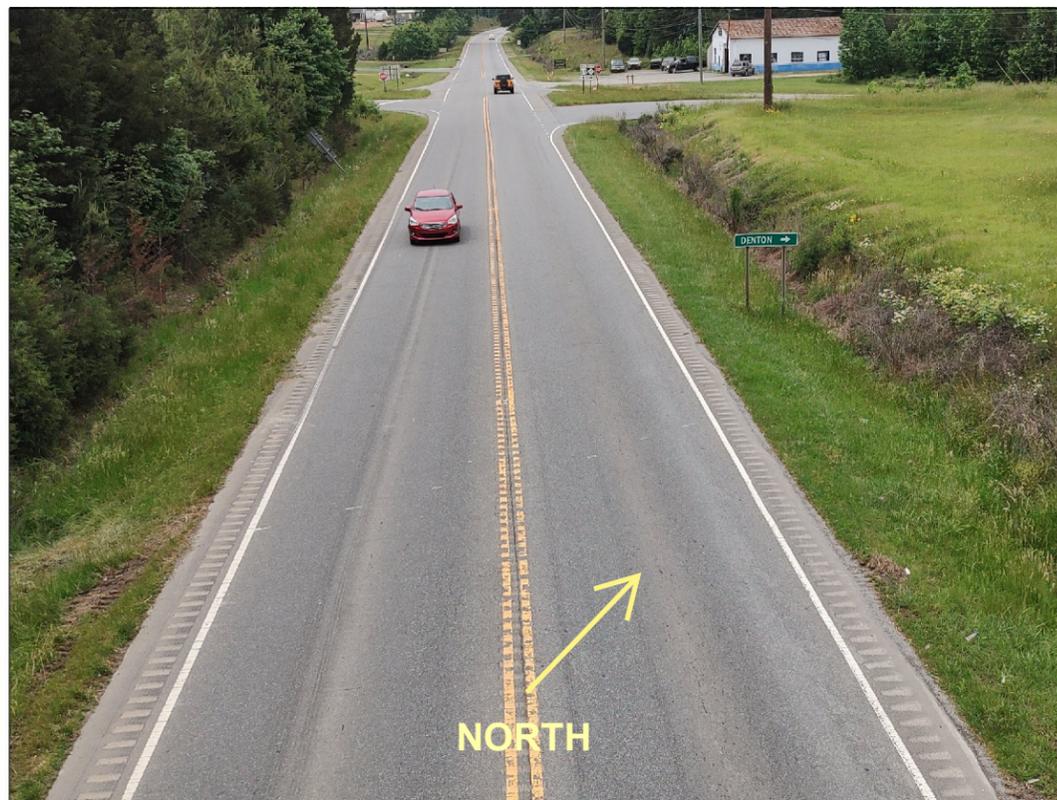


PHOTO 3: VIEW LOOKING NORTHWEST ALONG US 64 FROM EXISTING BRIDGE DECK.

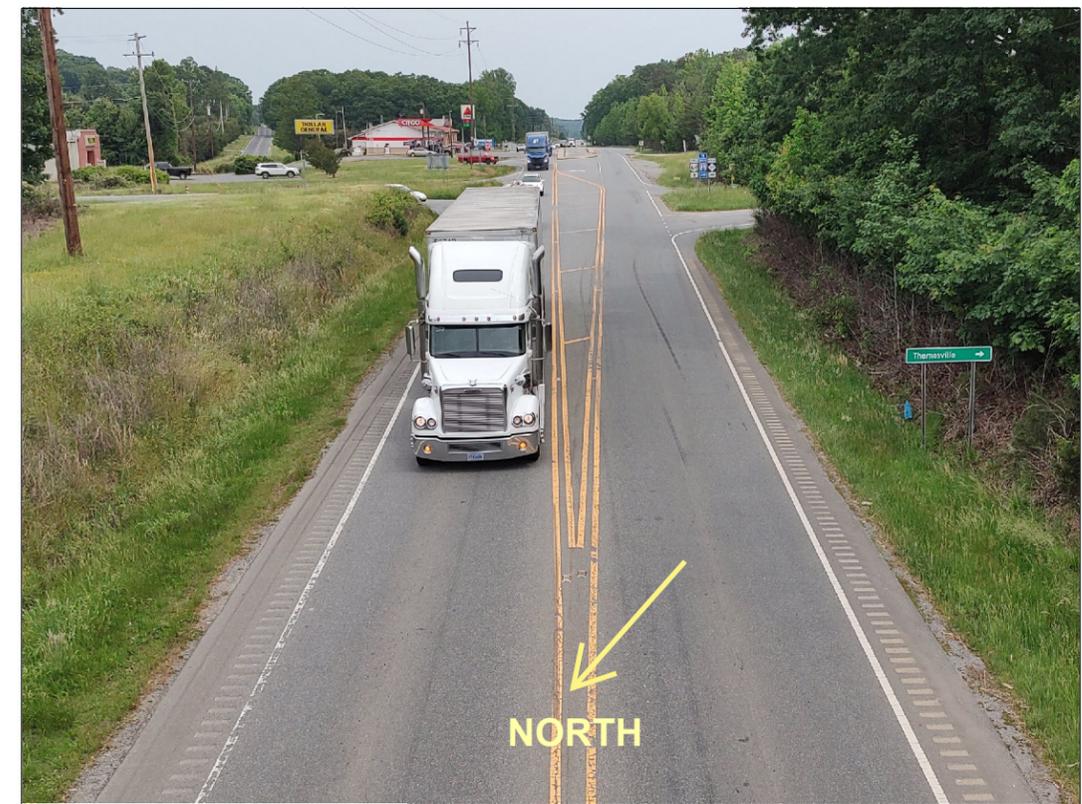


PHOTO 4: VIEW LOOKING SOUTHEAST ALONG US 64 FROM EXISTING BRIDGE DECK.