



	SEAL 042642	Signed by: Robert C. Krol	05/12/2025		
RTH CAROLINA		GEOTECHI ENGINE	NICAL ER	ENGINE	ER

				CHATHAM	COUI	NTY
RETAINING	WALL #1:	-W1- STA. 1	10+6	65.00 TO 13+52	2.09	
	SHEET 2 C	0F 3				
ORTH CAROLINA ENT OF TRANSPORTATION SION OF HIGHWAYS	SOI	RETAINI L NAIL R	NC ET	g wall #1 'Aining w	ALL	
		RE	EVIS	IONS		SHEET
NEEKING UNII	NO. BY	DATE	NO.	BY	DATE	NO.
	2		3			W-2

## NOTES:

FOR SOIL NAIL RETAINING WALLS, SEE SOIL NAIL RETAINING WALLS PROVISION. A FORM LINER ARCHITECTURAL FINISH IS REQUIRED FOR THE CIP REINFORCED CONCRETE FACE FOR RETAINING WALL #1. THE CONTRACTOR SHALL PROVIDE THE REQUESTED FINISH BEFORE BEGINNING CIP REINFORCED CONCRETE FACE CONSTRUCTION. THE APPEARANCE (STONE SIZE AND SHAPE, STONE COLOR, AND STONE TEXTURE, PATTERN, AND RELIEF) SHOULD MATCH NATURAL STONE AND ROCK. FOR FORM LINER ARCHITECHTURAL FINISH, SEE THE SIMULATED STONE FORM LINER FINISH SPECIAL PROVISION. BEFORE BEGINNING SOIL NAIL WALL DESIGN FOR RETAINING WALL #1, SURVEY WALL LOCATION AND SUBMIT A REVISED WALL PROFILE VIEW (WALL ENVELOPE) FOR REVIEW. DO NOT START WALL DESIGN OR CONSTRUCTION UNTIL THE REVISED WALL ENVELOPE IS ACCEPTED. DESIGN RETAINING WALL **#1** FOR THE FOLLOWING: 1) DESIGN HEIGHT (H) = WALL HEIGHT + WALL EMBEDMENT 2) DESIGN LIFE = 75 YEARS 3) MINIMUM WALL EMBEDMENT ELEVATION = VARIES (MIN.1 FT BELOW PROPOSED FINISHED GRADE ELEVATION) 4) IN-SITU ASSUMED PARAMETERS: -W1- STATION 11+50 TO 12+00 -W1- STATION 10+10+65 TO 10+87 ABOVE EL.469 FT. ABOVE EL. 471 FT. UNIT WEIGHT,  $\gamma$  = 120 PCF UNIT WEIGHT,  $\gamma$  = 120 PCF FRICTION ANGLE,  $\phi = 26$  degrees FRICTION ANGLE,  $\phi$  = 26 DEGREES COHESION, c = 0 PSF COHESION, c = 0 PSF EL.468 - 471 FT. BELOW EL.469 FT. UNIT WEIGHT,  $\gamma$  = 120 PCF UNIT WEIGHT,  $\gamma = 170$  PCF FRICTION ANGLE,  $\phi$ = 34 DEGREES FRICTION ANGLE,  $\phi$  = 34 DEGREES COHESION, c = 0 PSF COHESION, c = 1,000 PSF -W1- STATION 12+00 TO 12+50 BELOW EL.468 FT. ABOVE EL.464 FT. UNIT WEIGHT,γ= 120 PCF UNIT WEIGHT,  $\gamma$  = 135 PCF FRICTION ANGLE,  $\phi$  = 32 DEGREES COHESION, c = 500 PSF FRICTION ANGLE,  $\phi$  = 26 DEGREES -W1- STATION 10+87 TO 11+21 COHESION, c = 0 PSF ABOVE EL.468 FT. BELOW EL.464 FT. UNIT WEIGHT,  $\gamma = 120$  PCF UNIT WEIGHT, $\gamma$  = 135 PCF FRICTION ANGLE,  $\phi$  = 26 DEGREES FRICTION ANGLE,  $\phi$  = 32 DEGREES -W1- STATION 12+50 TO 13+52 ABOVE EL. 473 FT. COHESION, c = 0 PSF BELOW EL. 468 FT. UNIT WEIGHT,γ = 135 PCF FRICTION ANGLE,  $\phi$  = 32 DEGREES UNIT WEIGHT,γ = 120 PCF FRICTION ANGLE,  $\phi$  = 26 DEGREES COHESION, c = 500 PSF -W1- STATION 11+21 TO 11+50 COHESION, c = 0 PSF ABOVE EL.492 BELOW EL.473 FT. UNIT WEIGHT,  $\gamma$  = 120 PCF UNIT WEIGHT,γ = 135 PCF FRICTION ANGLE,  $\phi$  = 26 DEGREES FRICTION ANGLE,  $\phi$  = 32 DEGREES COHESION, c = 0 PSF EL. 483 - 492 FT. COHESION, c = 500 PSF UNIT WEIGHT,  $\gamma$  = 135 PCF FRICTION ANGLE,  $\phi$  = 32 DEGREES COHESION, c = 500 PSF EL.467 - 483 FT. UNIT WEIGHT,  $\gamma$  = 120 PCF FRICTION ANGLE,  $\phi$  = 26 DEGREES COHESION, c = 0 PSF BELOW EL.467 UNIT WEIGHT,  $\gamma = 135$  PCF

FRICTION ANGLE,  $\phi$  = 32 DEGREES

COHESION, c = 500 PSF

PREPARED BY: K. DE MONTBRUN	DATE: 05/12/25	
REVIEWED BY: R. KRAL	DATE: 05/12/25	

WHERE ROCK IS ENCOUNTERED IN THE WALL ENVELOPE, DESIGNERS SHOULD REFER TO THE FHWA PRESUMPTIVE STRENGTH PARAMETERS OR OTHER REPRESENTATIVE AND REPEATABLE VALUES AND PROVIDE SOURCE REFERENCES IN THEIR DESIGN SUBMITTAL.

WHEN ANALYZING FOR INFINITE SLOPE CONDITIONS,DESIGNERS SHOULD ANALYZE UP TO TWO (2) TIMES THE WALL HEIGHT BEHIND THE WALL FACE FOR FAILURE PLANE SEARCHES.THIS INFORMATION SHOULD BE INCLUDED WITH THE DESIGN SUBMITTAL.

EXISTING OR FUTURE OBSTRUCTIONS SUCH AS FOUNDATIONS, GUARDRAIL, FENCE OR HANDRAIL POSTS, PAVEMENTS, PIPES, INLETS OR UTILITIES MAY INTERFERE WITH SOIL NAILS FOR RETAINING WALL #1.

THE PROPOSED RIGHT OF WAY (ROW) BOUNDARY IS 16.1 FT FROM THE FACE OF RETAINING WALL **#1** AT ITS NEAREST POINT.SOIL NAILS MAY NOT BE INSTALLED BEYOND THE ROW BOUNDARY.SEE "SOIL NAIL WALL - TYPICAL SECTION" DETAIL.

WHERE ROCK IS PRESENT IN THE WALL ENVELOPE, CONTROLLED BLASTING IS RECOMMENDED, BUT NOT REQUIRED, TO MAINTAIN THE NEAT EXCAVATION LINE.VOIDS, RESULTING FROM BLASTING OR EXCAVATING, THAT EXTEND BEYOND THE NEAT LINES ARE TO BE FILLED WITH A COMBINATION OF SHORT SOIL NAILS, WELDED WIRE, AND SHOTCRETE, AT THE DISCRETION OF THE ENGINEER. THE COSTS ASSOCIATED WITH THIS WORK WILL BE CONSIDERED INCIDENTAL TO WALL CONSTRUCTION AND NO ADDITIONAL COMPENSATION WILL BE MADE.FOR BLASTING, SEE THE BLASTING PROVISION.

WHERE CONSTRUCTION VOIDS EXIST ALONG THE TOP OF RETAINING WALL #1, THE CONTRACTOR SHOULD BE PREPARED TO FORM THE CANTILEVERED SECTION OF THE CIP REINFORCED CONCRETE FACE TO THE TOP OF WALL ELEVATION. THE CONSTRUCTION VOID SHOULD BE FILLED WITH CONCRETE OR SHOTCRETE PRIOR TO CONSTRUCTION OF THE CONCRETE DITCH. ADDITIONAL WALL FACE REINFORCEMENT OR SOIL NAILS MAY BE REQUIRED FOR TALLER THAN TYPICAL CANTILEVER FACE HEIGHTS.



ENGINEER

DATE



### DOCUMENT NOT CONSIDERED FINAL UNLESS ALL SIGNATURES COMPLETED

SIGNATURE

PROJECT NO.: R-5963A

_						CHATHAM	COU	NTY
	RETAINING	WALL #1:	-W1- S	STA. <sup>-</sup>	10+0	65.00 TO 13+5	52.09	
		SHEET 3 C	)F 3					
NORTH CAROL EPARTMENT OF TRAN DIVISION OF HIG	INA SPORTATION HWAYS	SO	RET. IL NA	AINI IL R	N ET	G WALL # FAINING V	1 VALL	
GEOTECHNI	CAL							<del></del>
FNGINFFRING			,			SIONS		SHEET
		NO. BY	·	DATE	INO.	BY	DATE	

		RE	EVIS	SIONS		SHEFT
NO.	BY	DATE	NO.	BY	DATE	NO.
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2			4			VV-3

# **RETAINING WALL #2:**





REPARED BY: K. DE MONTBRUN	DATE: 05/12/25	RETAINING WALL NO. 2 ENVELOPE AND
REVIEWED BY: R. KRAL	DATE: 05/12/25	

ADT MATCHLINE (EOT)	OF WALL WALL FACE WALL WALL	V2- EXIST	ING GROUN	ND		
IL NAIL WALL ( TAINING WALLS E FEET) 20 21 LINER FINISH	QUAN AIL N TESTS	TITIE Soil NA Proof te 4 1,720 S	S IL STS F			
RETAINING	PROJE WALL # SHEE	<b>ECT NO</b> .: <b>#2</b> : <u>-W2-</u> T 1 OF 3	R-5963 STA. 10-	A CHATHAM +65.00 TO 13+{	_ COUI 52.09	NTY
ORTH CAROLINA ENT OF TRANSPORTATION SION OF HIGHWAYS OTECHNICAL NEERING UNIT	NO. 1 2	RET SOIL NA	AININ IL RE REVI DATE NO 3 4	G WALL #2 TAINING V ZSIONS	2 VALL DATE	SHEET NO. W-4

GEOTECHNICAL ENGINEER

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05/12/2025 DATE

DOCUMENT NOT CONSIDERED FINAL UNLESS ALL SIGNATURES COMPLETED

Robert E. Krol

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ENGINEER

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DATE



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SIGNATURE DATE	- SIGNATURE DATE
Signed by: Robert C. Kisl AAF88FF73C5E4B1 05/12/2025	
SEAL 042642	
GEOTECHNICAL ENGINEER	ENGINEER

PROJECT	NO.:	R-5963A

				CHATHAM	COUI	NTY
RETAINING	WALL #2:	-W2- STA. 1	0+0	65.00 TO 13+52	2.09	
	SHEET 2 O	F 3				
ORTH CAROLINA ENT OF TRANSPORTATION SION OF HIGHWAYS	SOI	RETAINI L NAIL R	N( E1	G WALL #2 AINING W	ALL	
		RE	VIS	SIONS		SHEET
NEERING UNII	NO. BY	DATE	NO.	BY	DATE	NO.
	1		3			W-5
	<b>4</b>		4			

NOTES:

FOR SOIL NAIL RETAINING WALLS, SEE SOIL NAIL RETAINING WALLS PROVISION.

A FORM LINER ARCHITECTURAL FINISH IS REQUIRED FOR THE CIP REINFORCED CONCRETE FACE FOR RETAINING WALL #2. THE CONTRACTOR SHALL PROVIDE THE REQUESTED FINISH BEFORE BEGINNING CIP REINFORCED CONCRETE FACE CONSTRUCTION. THE APPEARANCE (STONE SIZE AND SHAPE, STONE COLOR, AND STONE TEXTURE, PATTERN, AND RELIEF) SHOULD MATCH NATURAL STONE AND ROCK. FOR FORM LINER ARCHITECHTURAL FINISH, SEE THE SIMULATED STONE FORM LINER FINISH SPECIAL PROVISION.

BEFORE BEGINNING SOIL NAIL WALL DESIGN FOR RETAINING WALL **#2,** SURVEY WALL LOCATION AND SUBMIT A REVISED WALL PROFILE VIEW (WALL ENVELOPE) FOR REVIEW. DO NOT START WALL DESIGN OR CONSTRUCTION UNTIL THE REVISED WALL ENVELOPE IS ACCEPTED.

DESIGN RETAINING WALL #2 FOR THE FOLLOWING: 1) DESIGN HEIGHT (H) = WALL HEIGHT + WALL EMBEDMENT 2) DESIGN LIFE = 75 YEARS 3) MINIMUM WALL EMBEDMENT ELEVATION = VARIES (MIN.1 FT BELOW PROPOSED FINISHED GRADE ELEVATION) 4) IN-SITU ASSUMED SOIL PARAMETERS: -W2- STATION 10+65 TO 10+75 -W2- STATION 12+15 TO 13+25

ABOVE EL.471 FT. UNIT WEIGHT,γ = 120 PCF FRICTION ANGLE,φ = 26 DEGRE COHESION,c = 0 PSF BELOW EL.471 FT.

UNIT WEIGHT, γ = 135 PCF FRICTION ANGLE, φ = 32 DEGREE COHESION, c = 500 PSF -W2- STATION 10+75 TO 11+65

ABOVE EL. 475 FT. UNIT WEIGHT,  $\gamma$  = 120 PCF FRICTION ANGLE,  $\phi$  = 26 DEGRE

COHESION,c = Ο PSF El.472 - 475 FT. UNIT WEIGHT,γ=135 PCF

FRICTION ANGLE,  $\phi$  = 32 DEGREI

COHESION, c = 500 PSF

BELOW EL.472 FT. UNIT WEIGHT,γ = 170 PCF

FRICTION ANGLE,  $\phi = 34$  DEGREI

COHESION, c = 1,000 PSF -W4- STATION 11+65 TO 12+15 ABOVE EL.493 FT. UNIT WEIGHT,γ= 120 PCF

FRICTION ANGLE, \$\$\overline{4} = 26 DEGREE Cohesion, c = 0 PSF

EL. 481 - 483 FT.

UNIT WEIGHT,  $\gamma = 135$  PCF

FRICTION ANGLE,  $\phi$  = 32 DEGREES COHESION, c = 500 PSF

BELOW EL. 481 FT.

UNIT WEIGHT,γ = 170 PCF FRICTION ANGLE,φ = 34 DEGREES COHESION,c = 1,000 PSF

PREPARED BY: K. DE MONTBRUN	DATE: 05/12/25
REVIEWED BY: R. KRAL	DATE: 05/12/25

	-W2- STATION 12+15 TO 13+25 Above el.468 ft.
EES	UNIT WEIGHT,γ = 120 PCF Friction Angle,φ = 26 degrees Cohesion,c = 0 PSF el.458 - 468 ft.
ES	UNIT WEIGHT,γ = 120 PCF FRICTION ANGLE,φ = 34 DEGREES COHESION,c = 0 PSF BELOW EL.458 FT. UNIT WEIGHT,γ = 135 PCF
EES	FRICTION ANGLE, $\phi$ = 32 DEGREES COHESION, c = 500 PSF -W2- STATION 13+25 TO 13+52 ABOVE EL. 480 FT.
ES	UNIT WEIGHT, γ = 120 PCF FRICTION ANGLE, φ = 26 DEGREES COHESION, c = 0 PSF EL. 465 - 480 FT.
ES	UNIT WEIGHT, Y = 120 PCF FRICTION ANGLE, $\phi$ = 34 DEGREES COHESION, c = 0 PSF BELOW EL. 465 FT. UNIT WEIGHT Y = 120 PCF
ES	FRICTION ANGLE, $\phi$ = 26 DEGREES COHESION, c = 0 PSF

WHERE ROCK IS ENCOUNTERED IN THE WALL ENVELOPE, DESIGNERS SHOULD REFER TO THE FHWA PRESUMPTIVE STRENGTH PARAMETERS OR OTHER REPRESENTATIVE AND REPEATABLE VALUES AND PROVIDE SOURCE REFERENCES IN THEIR DESIGN SUBMITTAL.

WHEN ANALYZING FOR INFINITE SLOPE CONDITIONS, DESIGNERS SHOULD ANALYZE UP TO TWO (2) TIMES THE WALL HEIGHT BEHIND THE WALL FACE FOR FAILURE PLANE SEARCHES. THIS INFORMATION SHOULD BE INCLUDED WITH THE DESIGN SUBMITTAL.

EXISTING OR FUTURE OBSTRUCTIONS SUCH AS FOUNDATIONS,GUARDRAIL,FENCE OR HANDRAIL POSTS, PAVEMENTS,PIPES,INLETS OR UTILITIES MAY INTERFERE WITH SOIL NAILS FOR RETAINING WALL **#2**.

THE PROPOSED RIGHT OF WAY (ROW) BOUNDARY IS 15 FT FROM THE FACE OF RETAINING WALL #2 AT ITS NEAREST POINT.SOIL NAILS MAY NOT BE INSTALLED BEYOND THE ROW BOUNDARY.SEE "SOIL NAIL WALL - TYPICAL SECTION" DETAIL.

WHERE ROCK IS PRESENT IN THE WALL ENVELOPE, CONTROLLED BLASTING IS RECOMMENDED, BUT NOT REQUIRED, TO MAINTAIN THE NEAT EXCAVATION LINE. VOIDS, RESULTING FROM BLASTING OR EXCAVATING, THAT EXTEND BEYOND THE NEAT LINES ARE TO BE FILLED WITH A COMBINATION OF SHORT SOIL NAILS, WELDED WIRE, AND SHOTCRETE, AT THE DISCRETION OF THE ENGINEER. THE COSTS ASSOCIATED WITH THIS WORK WILL BE CONSIDERED INCIDENTAL TO WALL CONSTRUCTION AND NO ADDITIONAL COMPENSATION WILL BE MADE. FOR BLASTING, SEE THE BLASTING PROVISION.

WHERE CONSTRUCTION VOIDS EXIST ALONG THE TOP OF RETAINING WALL **#2,** THE CONTRACTOR SHOULD BE PREPARED TO FORM THE CANTILEVERED SECTION OF THE CIP REINFORCED CONCRETE FACE TO THE TOP OF WALL ELEVATION. THE CONSTRUCTION VOID SHOULD BE FILLED WITH CONCRETE OR SHOTCRETE PRIOR TO CONSTRUCTION OF THE CONCRETE DITCH. ADDITIONAL WALL FACE REINFORCEMENT OR SOIL NAILS MAY BE REQUIRED FOR TALLER THAN TYPICAL CANTILEVER FACE HEIGHTS.



ENGINEER

DATE



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SIGNATURE

PROJECT NO.: R-5963A

						CHATHAM	COU	NTY
RETAINING	WA	LL #2:	-W2- 3	STA. 1	10+	65.00 TO 13+52	2.09	
SHEET 3 OF 3								
ORTH CAROLINA INT OF TRANSPORTATION INT OF HIGHWAYS		SOI	RET L NA	AINI IL R	N( E1	g wall #2 Faining w	ALL	
	REVISIONS						SHEET	
NEEKING UNII	NO.	BY		DATE	NO.	BY	DATE	NO.

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W-6

# RETAINING WALL $\#3\cdot$

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-180+00 == = 282 :81 = E E E E 	
¥	
<u>END TIP PROJECT R-59</u> -L- POT Sta. 177+51.05 -Y14- POT Sta. 21+59.34	<u>963A&amp;B</u>
440	
430	
420	<u>– END RETAINING V</u> –W3– Sta. 176+9 –L– Sta. 176+92.87 (67 ELEV =
410	
400	
390	
380	
370	
360	
RET	AINING WALL NOT TO (LOOKING AT F
	END TIP PROJECT R-53 -L- POT Sta. ITT+5J.05 -YI4- POT Sta. 21+59.34 440 430 420 410 400 390 380 370 360 RET

REPARED BY: K. DE MONTBRUN	DATE:05/09/25	KE KI
EVIEWED BY: R. KRAL	DATE:05/09/25	

KETAINING WALL #3 ENVELOPE AND WALL LAYOUT PROVIDED BY (IMLEY-HORN MAY 8, 2025.









## NOTES:

FOR NON-STANDARD CIP GRAVITY RETAINING WALLS, REFER TO THE NON-STANDARD CIP GRAVITY RETAINING WALLS SPECIAL PROVISION.

FOR SINGLE FACED PRECAST CONCRETE BARRIER, SEE ROADWAY PLANS AND SECTION 857 OF THE STANDARD SPECIFICATIONS.

FOR SUBSURFACE DRAINAGE AT WEEP HOLES, SEE ARTICLE 414-8 OF THE STANDARD SPECIFICATIONS.

NON-STANDARD CIP GRAVITY WALLS ARE BASED ON THE FOLLOWING:

IN-SITU ASSUMED RETAINED SOIL PARAMETERS: UNIT WEIGHT,  $\gamma = 120$  PCF FRICTION ANGLE,  $\phi = 30$  DEGREES COHESION, c = 0 PSF

IN-SITU ASSUMED FOUNDATION SOIL PARAMETERS: UNIT WEIGHT,  $\gamma = 120$  PCF FRICTION ANGLE,  $\phi = 26$  DEGREES COHESION, c = 0 PSF

UNDERCUTTING OF SOFT AND/OR WET SOILS IN THE VICINITY OF THE WALL FOUNDATION MAY BE REQUIRED TO IMPROVE BEARING RESISTANCE AFTER THE WALL FOOTING IS EXCAVATED TO BEARING GRADE. IF REQUIRED BY THE ENGINEER, USE UNDERCUT EXCAVATION TO REMOVE SOFT AND/OR WET SOILS, UNDERCUT TO SUITABLE FOUNDATION SOILS OR TO DEPTH NO GREATER THAN 3 FEET BELOW THE BOTTOM OF FOOTING ELEVATION, WHICHEVER OCCURS FIRST. PLACE GEOTEXTILE FOR SOIL STABILIZATION IN THE BOTTOM OF THE EXCAVATION AND BACKFILL WITH SELECT GRANULAR MATERIAL. FOR UNDERCUT AND EXCAVATION AND SELECT GRANULAR MATERIAL, SEE STANDARD SPECIFICATION. UNDERCUT EXCAVATION, SELECT GRANULAR MATERIAL, AND GEOTEXTILE FOR SOIL STABILIZATION WILL BE PAID AS SEPARATE ADDITIONAL QUANTITIES.

BEFORE BEGINNING NON-STANDARD CIP GRAVITY WALL CONSTRUCTION, SURVEY WALL LOCATIONS AND SUBMIT WALL PROFILE VIEWS (WALL ENVELOPES) FOR REVIEW. FOR WALL ENVELOPES, INCLUDE BOTTOM OF WALL, EXISTING GROUND, AND GRADE ELEVATIONS. DO NOT START WALL CONSTRUCTION UNTIL WALL ENVELOPES ARE ACCEPTED.

DO NOT PLACE CONCRETE FOR FOOTINGS UNTIL EXCAVATION DIMENSIONS AND FOUNDATION MATERIAL ARE APPROVED.

A FORM LINER ARCHITECTURAL FINISH IS REQUIRED FOR THE CIP CONCRETE FACE FOR RETAINING WALL #3, MOMENT SLAB FACE, AND CONCRETE BARRIER RAIL FACE. THE CONTRACTOR SHALL PROVIDE THE REQUESTED FINISH BEFORE BEGINNING CIP CONCRETE FACE CONSTRUCTION. THE APPEARANCE (STONE SIZE AND SHAPE, STONE COLOR, TEXTURE, PATTERN, AND RELIEF) SHOULD MATCH NATURAL STONE AND ROCK IN THE PROJECT VICINITY. FOR FORM LINER ARCHITECTURAL FINISH, SEE THE SIMULATED STONE FORM LINER FINISH SPECIAL PROVISION.



ENGINEER

DATE

ENGINEER SEAL 042642 Robert E. Krol AF88FF73C5E4B1. 05/10/2025 DATE SIGNATURE

GEOTECHNICAL

DOCUMENT NOT CONSIDERED FINAL UNLESS ALL SIGNATURES COMPLETED

SIGNATURE

	PROJECT	NO.:	R-596	63A	CHATHAM	COU	NTY
RETAINING	WALL #3: SHEET 2 (	-W3- 3	STA. ´	174+	+00.00 TO 17	6+98.(	00
the Office of: CAROLINAS GEOTECHNICAL GROUP	CIP	RET N GRA WIT	AINI ON-S VITY H M	NG STA RE OM	WALL #3 NDARD TAINING V ENT SLAB	VALL	
100	REVISIONS						SHEET
NC 28270	NO. B`	(	DATE	NO.	BY	DATE	NO.

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W-8

