

REFERENCE: U-5813

PROJECT: 44385

**SEE SHEET 3 FOR PLAN SHEET LAYOUT
AT TIME OF INVESTIGATION**

CONTENTS

<u>LINE</u>	<u>STATION</u>	<u>PLAN</u>	<u>PROFILE</u>
-L-	10+00.00 - 129+34.14	4-13	N/A
-Y-	10+00.00 - 50+78.21	11-12, 14-15	16-17
-Y1-	10+00.00 - 14+90.83	6	N/A
-Y2-	10+00.00 - 14+11.81	6	N/A
-Y3-	10+00.00 - 13+13.41	6	N/A
-Y4_REV-	10+00.00 - 14+05.83	7	N/A
-Y5-	10+00.00 - 14+34.89	7	N/A
-Y6-	10+00.00 - 12+33.03	8	N/A
-Y7-	10+00.00 - 13+70.16	8	N/A
-Y8-	10+00.00 - 13+00.00	9	N/A
-Y9-	10+00.00 - 11+74.86	9	N/A
-Y10-	10+00.00 - 12+14.95	9	N/A
-Y11-	10+00.00 - 12+05.40	10	N/A
-Y12-	10+00.00 - 20+45.03	10, 14	N/A
-Y13-	10+00.00 - 15+16.93	11	N/A
-Y14-	10+00.00 - 11+08.53	11	N/A
-Y15-	10+00.00 - 12+99.22	14	N/A
-Y_SLIP-	10+00.00 - 21+26.05	11	N/A
-Y17-	10+00.00 - 20+65.00	12	N/A
-Y17A-	10+00.00 - 15+23.52	12	18
-Y18-	10+00.00 - 15+16.53	15	N/A
-Y19-	10+00.00 - 13+64.53	11-12	N/A
-RPBC-	10+00.00 - 16+69.66	12	N/A
-RPDA-	10+00.00 - 17+00.61	12	19
-RPA-	10+00.00 - 19+17.77	12	N/A
-RPB-	10+00.00 - 18+15.72	12	N/A
-RPC-	10+00.00 - 21+44.17	12	N/A
-RPD-	10+00.00 - 21+49.98	12	N/A
-LPA-	10+00.00 - 21+76.89	12	N/A
-LPC-	10+00.00 - 20+63.82	12	N/A
-SRV-	10+00.00 - 21+16.28	11, 14	N/A
-DRW01-	10+00.00 - 11+12.19	11	N/A

CROSS SECTIONS

<u>LINE</u>	<u>STATION</u>	<u>SHEETS</u>
-L-	26+00.00-128+50.00	20-69
-Y-	10+00.00-50+50.00	70-89
-Y1-	11+00.00-12+75.00	90-91
-Y2-	12+00.00-13+50.00	92-93
-Y3-	10+00.00-12+25.00	94-95
-Y4_REV-	11+00.00	96
-Y5-	11+00.00	97
-Y6-	11+00.00	98
-Y7-	11+25.00-13+00.00	99-100
-Y8-	10+75.00	101
-Y11-	11+00.00	102
-Y12-	19+00.00	103
-Y13-	10+75.00-14+00.00	104-107
-Y14-	11+00.00	108
-Y_SLIP-	11+75.00-17+50.00	109-112
-Y17-	11+00.00-21+00.00	113-120
-Y17A-	11+75.00-14+25.00	121-124
-Y18-	10+75.00	125
-Y19-	10+00.00-12+00.00	126-127
-RPBC-	11+00.00-15+50.00	128-134
-RPDA-	11+50.00-15+50.00	135-138
-RPA-	13+00.00-18+25.00	139-143
-RPB-	12+50.00-16+50.00	144-147
-RPC-	12+00.00-18+00.00	148-151
-RPD-	12+00.00-19+00.00	152-156
-LPA-	12+00.00-19+00.00	157-160
-LPC-	12+25.00-17+00.00	161-164
-SRV-	11+00.00-15+00.00	165-167

APPENDICES

<u>APPENDIX</u>	<u>TITLE</u>	<u>SHEETS</u>
A	LABORATORY RESULTS	168-198

STATE OF NORTH CAROLINA DEPARTMENT OF TRANSPORTATION DIVISION OF HIGHWAYS GEOTECHNICAL ENGINEERING UNIT

ROADWAY SUBSURFACE INVESTIGATION

COUNTY RANDOLPH
PROJECT DESCRIPTION US-64 FROM THE ASHEBORO
BYPASS TO EAST OF I-73/I-74/US-220

INVENTORY

STATE	STATE PROJECT REFERENCE NO.	SHEET NO.	TOTAL SHEETS
N.C.	U-5813	1	201

CAUTION NOTICE

THE SUBSURFACE INFORMATION AND THE SUBSURFACE INVESTIGATION ON WHICH IT IS BASED WERE MADE FOR THE PURPOSE OF STUDY, PLANNING AND DESIGN, AND NOT FOR CONSTRUCTION OR PAY PURPOSES. THE VARIOUS FIELD BORING LOGS, ROCK CORES AND SOIL TEST DATA AVAILABLE MAY BE REVIEWED OR INSPECTED IN RALEIGH BY CONTACTING THE N.C. DEPARTMENT OF TRANSPORTATION, GEOTECHNICAL ENGINEERING UNIT AT (919) 707-6850. THE SUBSURFACE PLANS AND REPORTS, FIELD BORING LOGS, ROCK CORES AND SOIL TEST DATA ARE NOT PART OF THE CONTRACT.

GENERAL SOIL AND ROCK STRATA DESCRIPTIONS AND INDICATED BOUNDARIES ARE BASED ON A GEOTECHNICAL INTERPRETATION OF ALL AVAILABLE SUBSURFACE DATA AND MAY NOT NECESSARILY REFLECT THE ACTUAL SUBSURFACE CONDITIONS BETWEEN BORINGS OR BETWEEN SAMPLED STRATA WITHIN THE BOREHOLE. THE LABORATORY SAMPLE DATA AND THE IN SITU (IN-PLACE) TEST DATA CAN BE RELIED ON ONLY TO THE DEGREE OF RELIABILITY INHERENT IN THE STANDARD TEST METHOD. THE OBSERVED WATER LEVELS OR SOIL MOISTURE CONDITIONS INDICATED IN THE SUBSURFACE INVESTIGATIONS ARE AS RECORDED AT THE TIME OF THE INVESTIGATION. THESE WATER LEVELS OR SOIL MOISTURE CONDITIONS MAY VARY CONSIDERABLY WITH TIME ACCORDING TO CLIMATIC CONDITIONS INCLUDING TEMPERATURES, PRECIPITATION AND WIND, AS WELL AS OTHER NON-CLIMATIC FACTORS.

THE BIDDER OR CONTRACTOR IS CAUTIONED THAT DETAILS SHOWN ON THE SUBSURFACE PLANS ARE PRELIMINARY ONLY AND IN MANY CASES THE FINAL DESIGN DETAILS ARE DIFFERENT. FOR BIDDING AND CONSTRUCTION PURPOSES, REFER TO THE CONSTRUCTION PLANS AND DOCUMENTS FOR FINAL DESIGN INFORMATION ON THIS PROJECT. THE DEPARTMENT DOES NOT WARRANT OR GUARANTEE THE SUFFICIENCY OR ACCURACY OF THE INVESTIGATION MADE, NOR THE INTERPRETATIONS MADE, OR OPINION OF THE DEPARTMENT AS TO THE TYPE OF MATERIALS AND CONDITIONS TO BE ENCOUNTERED. THE BIDDER OR CONTRACTOR IS CAUTIONED TO MAKE SUCH INDEPENDENT SUBSURFACE INVESTIGATIONS AS HE DEEMS NECESSARY TO SATISFY HIMSELF AS TO CONDITIONS TO BE ENCOUNTERED ON THE PROJECT. THE CONTRACTOR SHALL HAVE NO CLAIM FOR ADDITIONAL COMPENSATION OR FOR AN EXTENSION OF TIME FOR ANY REASON RESULTING FROM THE ACTUAL CONDITIONS ENCOUNTERED AT THE SITE DIFFERING FROM THOSE INDICATED IN THE SUBSURFACE INFORMATION.

- NOTES:
1. THE INFORMATION CONTAINED HEREIN IS NOT IMPLIED OR GUARANTEED BY THE N.C. DEPARTMENT OF TRANSPORTATION AS ACCURATE NOR IS IT CONSIDERED PART OF THE PLANS, SPECIFICATIONS OR CONTRACT FOR THE PROJECT.
 2. BY HAVING REQUESTED THIS INFORMATION, THE CONTRACTOR SPECIFICALLY WAIVES ANY CLAIMS FOR INCREASED COMPENSATION OR EXTENSION OF TIME BASED ON DIFFERENCES BETWEEN THE CONDITIONS INDICATED HEREIN AND THE ACTUAL CONDITIONS AT THE PROJECT SITE.

PERSONNEL

S. PAPKE

TRIGON EXPLORATION

C DRISCOLL

HPC LAND SERVICES

INVESTIGATED BY S. PAPKE

DRAWN BY S. PAPKE

CHECKED BY T. WELLS

SUBMITTED BY KLEINFELDER, INC.

DATE NOVEMBER 2019

Prepared in the Office of:



DocuSigned by:
Thomas R. Wells 11/5/2019

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**DOCUMENT NOT CONSIDERED FINAL
UNLESS ALL SIGNATURES COMPLETED**

NORTH CAROLINA DEPARTMENT OF TRANSPORTATION
DIVISION OF HIGHWAYS
GEOTECHNICAL ENGINEERING UNIT
SUBSURFACE INVESTIGATION
SOIL AND ROCK LEGEND, TERMS, SYMBOLS, AND ABBREVIATIONS

SOIL DESCRIPTION										GRADATION										ROCK DESCRIPTION										TERMS AND DEFINITIONS									
<p>SOIL IS CONSIDERED UNCONSOLIDATED, SEMI-CONSOLIDATED, OR WEATHERED EARTH MATERIALS THAT CAN BE PENETRATED WITH A CONTINUOUS FLIGHT POWER AUGER AND YIELD LESS THAN 100 BLOWS PER FOOT ACCORDING TO THE STANDARD PENETRATION TEST (AASHTO T 206, ASTM D1586). SOIL CLASSIFICATION IS BASED ON THE AASHTO SYSTEM. BASIC DESCRIPTIONS GENERALLY INCLUDE THE FOLLOWING: CONSISTENCY, COLOR, TEXTURE, MOISTURE, AASHTO CLASSIFICATION, AND OTHER PERTINENT FACTORS SUCH AS MINERALOGICAL COMPOSITION, ANGULARITY, STRUCTURE, PLASTICITY, ETC. FOR EXAMPLE, <i>VERY STIFF, GRAY, SILTY CLAY, MOIST WITH INTERBEDDED FINE SAND LAYERS, HIGHLY PLASTIC, A-7-6</i></p>										<p>WELL GRADED - INDICATES A GOOD REPRESENTATION OF PARTICLE SIZES FROM FINE TO COARSE. UNIFORMLY GRADED - INDICATES THAT SOIL PARTICLES ARE ALL APPROXIMATELY THE SAME SIZE. GAP-GRADED - INDICATES A MIXTURE OF UNIFORM PARTICLE SIZES OF TWO OR MORE SIZES.</p>										<p>HARD ROCK IS NON-COASTAL PLAIN MATERIAL THAT WOULD YIELD SPT REFUSAL IF TESTED, AN INFERRED ROCK LINE INDICATES THE LEVEL AT WHICH NON-COASTAL PLAIN MATERIAL WOULD YIELD SPT REFUSAL. SPT REFUSAL IS PENETRATION BY A SPLIT SPOON SAMPLER EQUAL TO OR LESS THAN 0.1 FOOT PER 60 BLOWS IN NON-COASTAL PLAIN MATERIAL. THE TRANSITION BETWEEN SOIL AND ROCK IS OFTEN REPRESENTED BY A ZONE OF WEATHERED ROCK. ROCK MATERIALS ARE TYPICALLY DIVIDED AS FOLLOWS:</p>										<p>ALLUVIUM (ALLUV.) - SOILS THAT HAVE BEEN TRANSPORTED BY WATER. AQUIFER - A WATER BEARING FORMATION OR STRATA. ARENACEOUS - APPLIED TO ROCKS THAT HAVE BEEN DERIVED FROM SAND OR THAT CONTAIN SAND. ARGILLACEOUS - APPLIED TO ALL ROCKS OR SUBSTANCES COMPOSED OF CLAY MINERALS, OR HAVING A NOTABLE PROPORTION OF CLAY IN THEIR COMPOSITION, SUCH AS SHALE, SLATE, ETC. ARTESIAN - GROUND WATER THAT IS UNDER SUFFICIENT PRESSURE TO RISE ABOVE THE LEVEL AT WHICH IT IS ENCOUNTERED, BUT WHICH DOES NOT NECESSARILY RISE TO OR ABOVE THE GROUND SURFACE. CALCAREOUS (CALC.) - SOILS THAT CONTAIN APPRECIABLE AMOUNTS OF CALCIUM CARBONATE. COLLUVIUM - ROCK FRAGMENTS MIXED WITH SOIL DEPOSITED BY GRAVITY ON SLOPE OR AT BOTTOM OF SLOPE. CORE RECOVERY (REC.) - TOTAL LENGTH OF ALL MATERIAL RECOVERED IN THE CORE BARREL DIVIDED BY TOTAL LENGTH OF CORE RUN AND EXPRESSED AS A PERCENTAGE. DIKE - A TABULAR BODY OF IGNEOUS ROCK THAT CUTS ACROSS THE STRUCTURE OF ADJACENT ROCKS OR CUTS MASSIVE ROCK. DIP - THE ANGLE AT WHICH A STRATUM OR ANY PLANAR FEATURE IS INCLINED FROM THE HORIZONTAL. DIP DIRECTION (DIP AZIMUTH) - THE DIRECTION OR BEARING OF THE HORIZONTAL TRACE OF THE LINE OF DIP, MEASURED CLOCKWISE FROM NORTH. FAULT - A FRACTURE OR FRACTURE ZONE ALONG WHICH THERE HAS BEEN DISPLACEMENT OF THE SIDES RELATIVE TO ONE ANOTHER PARALLEL TO THE FRACTURE. FISSILE - A PROPERTY OF SPLITTING ALONG CLOSELY SPACED PARALLEL PLANES. FLOAT - ROCK FRAGMENTS ON SURFACE NEAR THEIR ORIGINAL POSITION AND DISLOADED FROM PARENT MATERIAL. FLOOD PLAIN (FP) - LAND BORDERING A STREAM, BUILT OF SEDIMENTS DEPOSITED BY THE STREAM. FORMATION (FM) - A MAPPABLE GEOLOGIC UNIT THAT CAN BE RECOGNIZED AND TRACED IN THE FIELD. JOINT - FRACTURE IN ROCK ALONG WHICH NO APPRECIABLE MOVEMENT HAS OCCURRED. LEDGE - A SHELF-LIKE RIDGE OR PROJECTION OF ROCK WHOSE THICKNESS IS SMALL COMPARED TO ITS LATERAL EXTENT. LENS - A BODY OF SOIL OR ROCK THAT THINS OUT IN ONE OR MORE DIRECTIONS. MOTTLED (MOT.) - IRREGULARLY MARKED WITH SPOTS OF DIFFERENT COLORS. MOTTLING IN SOILS USUALLY INDICATES POOR AERATION AND LACK OF GOOD DRAINAGE. PERCHED WATER - WATER MAINTAINED ABOVE THE NORMAL GROUND WATER LEVEL BY THE PRESENCE OF AN INTERVENING IMPERVIOUS STRATUM. RESIDUAL (RES.) SOIL - SOIL FORMED IN PLACE BY THE WEATHERING OF ROCK. ROCK QUALITY DESIGNATION (ROQ) - A MEASURE OF ROCK QUALITY DESCRIBED BY TOTAL LENGTH OF ROCK SEGMENTS EQUAL TO OR GREATER THAN 4 INCHES DIVIDED BY THE TOTAL LENGTH OF CORE RUN AND EXPRESSED AS A PERCENTAGE. SAPROLITE (SAP.) - RESIDUAL SOIL THAT RETAINS THE RELIC STRUCTURE OR FABRIC OF THE PARENT ROCK. SILL - AN INTRUSIVE BODY OF IGNEOUS ROCK OF APPROXIMATELY UNIFORM THICKNESS AND RELATIVELY THIN COMPARED WITH ITS LATERAL EXTENT, THAT HAS BEEN EMPLACED PARALLEL TO THE BEDDING OR SCHISTOSITY OF THE INTRUDED ROCKS. SLICKENSIDE - POLISHED AND STRIATED SURFACE THAT RESULTS FROM FRICTION ALONG A FAULT OR SLIP PLANE. STANDARD PENETRATION TEST (PENETRATION RESISTANCE) (SPT) - NUMBER OF BLOWS (N OR BPF) OF A 140 LB. HAMMER FALLING 30 INCHES REQUIRED TO PRODUCE A PENETRATION OF 1 FOOT INTO SOIL WITH A 2 INCH OUTSIDE DIAMETER SPLIT SPOON SAMPLER. SPT REFUSAL IS PENETRATION EQUAL TO OR LESS THAN 0.1 FOOT PER 60 BLOWS. STRATA CORE RECOVERY (SREC.) - TOTAL LENGTH OF STRATA MATERIAL RECOVERED DIVIDED BY TOTAL LENGTH OF STRATUM AND EXPRESSED AS A PERCENTAGE. STRATA ROCK QUALITY DESIGNATION (SROQ) - A MEASURE OF ROCK QUALITY DESCRIBED BY TOTAL LENGTH OF ROCK SEGMENTS WITHIN A STRATUM EQUAL TO OR GREATER THAN 4 INCHES DIVIDED BY THE TOTAL LENGTH OF STRATA AND EXPRESSED AS A PERCENTAGE. TOPSOIL (TS.) - SURFACE SOILS USUALLY CONTAINING ORGANIC MATTER.</p>									
SOIL LEGEND AND AASHTO CLASSIFICATION										MINERALOGICAL COMPOSITION										WEATHERING										MISCELLANEOUS SYMBOLS									
<p>GENERAL CLASS. GRANULAR MATERIALS (<= 35% PASSING #200) SILT-CLAY MATERIALS (> 35% PASSING #200) ORGANIC MATERIALS</p> <p>GROUP CLASS. A-1, A-3, A-2, A-4, A-5, A-6, A-7, A-1, A-2, A-3, A-4, A-5, A-6, A-7</p> <p>SYMBOL [Grid patterns for soil classification]</p> <p>% PASSING #10, #40, #200</p> <p>MATERIAL PASSING #40 LL, PI</p> <p>GROUP INDEX</p> <p>USUAL TYPES OF MAJOR MATERIALS</p> <p>GEN. RATING AS SUBGRADE</p>										<p>MINERAL NAMES SUCH AS QUARTZ, FELDSPAR, MICA, TALC, KAOLIN, ETC. ARE USED IN DESCRIPTIONS WHEN THEY ARE CONSIDERED OF SIGNIFICANCE.</p> <p>COMPRESSIBILITY</p> <p>PERCENTAGE OF MATERIAL</p> <p>ORGANIC MATERIAL</p> <p>TRACE OF ORGANIC MATTER 2 - 3% LITTLE ORGANIC MATTER 3 - 5% MODERATELY ORGANIC 5 - 10% HIGHLY ORGANIC > 10%</p> <p>GRANULAR SOILS 2 - 3% SILT - CLAY SOILS 3 - 5% OTHER MATERIAL 1 - 10% LITTLE 10 - 20% SOME 20 - 35% HIGHLY 35% AND ABOVE</p>										<p>FRESH ROCK FRESH, CRYSTALS BRIGHT, FEW JOINTS MAY SHOW SLIGHT STAINING. ROCK RINGS UNDER HAMMER IF CRYSTALLINE.</p> <p>VERY SLIGHT (V SLI.) ROCK GENERALLY FRESH, JOINTS STAINED, SOME JOINTS MAY SHOW THIN CLAY COATINGS IF OPEN, CRYSTALS ON A BROKEN SPECIMEN FACE SHINE BRIGHTLY. ROCK RINGS UNDER HAMMER BLOWS IF OF A CRYSTALLINE NATURE.</p> <p>SLIGHT (SLI.) ROCK GENERALLY FRESH, JOINTS STAINED AND DISCOLORATION EXTENDS INTO ROCK UP TO 1 INCH. OPEN JOINTS MAY CONTAIN CLAY. IN GRANITOID ROCKS SOME OCCASIONAL FELDSPAR CRYSTALS ARE DULL AND DISCOLORED. CRYSTALLINE ROCKS RING UNDER HAMMER BLOWS.</p> <p>MODERATE (MOD.) SIGNIFICANT PORTIONS OF ROCK SHOW DISCOLORATION AND WEATHERING EFFECTS. IN GRANITOID ROCKS, MOST FELDSPARS ARE DULL AND DISCOLORED, SOME SHOW CLAY. ROCK HAS DULL SOUND UNDER HAMMER BLOWS AND SHOWS SIGNIFICANT LOSS OF STRENGTH AS COMPARED WITH FRESH ROCK.</p> <p>MODERATELY SEVERE (MOD. SEV.) ALL ROCK EXCEPT QUARTZ DISCOLORED OR STAINED. IN GRANITOID ROCKS, ALL FELDSPARS DULL AND DISCOLORED AND A MAJORITY SHOW KAOLINIZATION. ROCK SHOWS SEVERE LOSS OF STRENGTH AND CAN BE EXCAVATED WITH A GEOLOGIST'S PICK. ROCK GIVES 'CLUNK' SOUND WHEN STRUCK. <i>IF TESTED, WOULD YIELD SPT REFUSAL</i></p> <p>SEVERE (SEV.) ALL ROCK EXCEPT QUARTZ DISCOLORED OR STAINED. ROCK FABRIC CLEAR AND EVIDENT BUT REDUCED IN STRENGTH TO STRONG SOIL. IN GRANITOID ROCKS ALL FELDSPARS ARE KAOLINIZED TO SOME EXTENT. SOME FRAGMENTS OF STRONG ROCK USUALLY REMAIN. <i>IF TESTED, WOULD YIELD SPT N VALUES < 100 BPF</i></p> <p>VERY SEVERE (V SEV.) ALL ROCK EXCEPT QUARTZ DISCOLORED OR STAINED. ROCK FABRIC ELEMENTS ARE DISCERNIBLE BUT MASS IS EFFECTIVELY REDUCED TO SOIL STATUS, WITH ONLY FRAGMENTS OF STRONG ROCK REMAINING. SAPROLITE IS AN EXAMPLE OF ROCK WEATHERED TO A DEGREE THAT ONLY MINOR VESTIGES OF ORIGINAL ROCK FABRIC REMAIN. <i>IF TESTED, WOULD YIELD SPT N VALUES < 100 BPF</i></p> <p>COMPLETE ROCK REDUCED TO SOIL. ROCK FABRIC NOT DISCERNIBLE, OR DISCERNIBLE ONLY IN SMALL AND SCATTERED CONCENTRATIONS. QUARTZ MAY BE PRESENT AS DIKES OR STRINGERS. SAPROLITE IS ALSO AN EXAMPLE.</p>										<p>ROADWAY EMBANKMENT (RE) WITH SOIL DESCRIPTION</p> <p>SOIL SYMBOL</p> <p>ARTIFICIAL FILL (AF) OTHER THAN ROADWAY EMBANKMENT</p> <p>INFERRED SOIL BOUNDARY</p> <p>INFERRED ROCK LINE</p> <p>ALLUVIAL SOIL BOUNDARY</p> <p>DIP & DIP DIRECTION OF ROCK STRUCTURES</p> <p>SPT DMT YST PMT TEST BORING</p> <p>AUGER BORING</p> <p>CORE BORING</p> <p>MONITORING WELL</p> <p>PIEZOMETER INSTALLATION</p> <p>SLOPE INDICATOR INSTALLATION</p> <p>CONE PENETROMETER TEST</p> <p>SOUNDING ROD</p> <p>TEST BORING WITH CORE</p> <p>SPT N-VALUE</p>									
TEXTURE OR GRAIN SIZE										RECOMMENDATION SYMBOLS										ROCK HARDNESS										ABBREVIATIONS									
<p>U.S. STD. SIEVE SIZE OPENING (MM)</p> <p>4 10 40 60 200 270</p> <p>4.76 2.00 0.42 0.25 0.075 0.053</p> <p>BOULDER (BLDR.) COBBLE (COB.) GRAVEL (GR.) COARSE SAND (CSE, SD.) FINE SAND (F SD.) SILT (SL.) CLAY (CL.)</p> <p>GRAIN SIZE MM 305 75 2.0 0.25 0.05 0.005</p> <p>GRAIN SIZE IN. 12 3</p>										<p>UNDERCUT</p> <p>SHALLOW UNDERCUT</p> <p>UNCLASSIFIED EXCAVATION - UNSUITABLE WASTE</p> <p>UNCLASSIFIED EXCAVATION - ACCEPTABLE DEGRADABLE ROCK</p> <p>UNCLASSIFIED EXCAVATION - ACCEPTABLE, BUT NOT TO BE USED IN THE TOP 3 FEET OF EMBANKMENT OR BACKFILL</p>										<p>VERY HARD CANNOT BE SCRATCHED BY KNIFE OR SHARP PICK. BREAKING OF HAND SPECIMENS REQUIRES SEVERAL HARD BLOWS OF THE GEOLOGIST'S PICK.</p> <p>HARD CAN BE SCRATCHED BY KNIFE OR PICK ONLY WITH DIFFICULTY. HARD HAMMER BLOWS REQUIRED TO DETACH HAND SPECIMEN.</p> <p>MODERATELY HARD CAN BE SCRATCHED BY KNIFE OR PICK. GOUGES OR GROOVES TO 0.25 INCHES DEEP CAN BE EXCAVATED BY HARD BLOW OF A GEOLOGIST'S PICK. HAND SPECIMENS CAN BE DETACHED BY MODERATE BLOWS.</p> <p>MEDIUM HARD CAN BE GROOVED OR GOUGED 0.05 INCHES DEEP BY FIRM PRESSURE OF KNIFE OR PICK POINT. CAN BE EXCAVATED IN SMALL CHIPS TO PEICES 1 INCH MAXIMUM SIZE BY HARD BLOWS OF THE POINT OF A GEOLOGIST'S PICK.</p> <p>SOFT CAN BE GROOVED OR GOUGED READILY BY KNIFE OR PICK. CAN BE EXCAVATED IN FRAGMENTS FROM CHIPS TO SEVERAL INCHES IN SIZE BY MODERATE BLOWS OF A PICK POINT. SMALL, THIN PIECES CAN BE BROKEN BY FINGER PRESSURE.</p> <p>VERY SOFT CAN BE CARVED WITH KNIFE. CAN BE EXCAVATED READILY WITH POINT OF PICK. PIECES 1 INCH OR MORE IN THICKNESS CAN BE BROKEN BY FINGER PRESSURE. CAN BE SCRATCHED READILY BY FINGERNAIL.</p>										<p>AR - AUGER REFUSAL BT - BORING TERMINATED CL - CLAY CPT - CONE PENETRATION TEST CSE - COARSE DMT - DILATOMETER TEST DPT - DYNAMIC PENETRATION TEST e - VOID RATIO f - FINE FOSS - FOSSILIFEROUS FRAC. - FRACTURED, FRACTURES FRAGS. - FRAGMENTS HI. - HIGHLY</p> <p>MED. - MEDIUM MICA - MICACEOUS MOD. - MODERATELY NP - NON PLASTIC ORG. - ORGANIC PMT - PRESSUREMETER TEST SAP. - SAPROLITIC SD. - SAND, SANDY SL. - SILT, SILTY SLI. - SLIGHTLY TCR - TRICONE REFUSAL w - MOISTURE CONTENT v - VERY</p> <p>VST - VANE SHEAR TEST WEA. - WEATHERED γ_u - UNIT WEIGHT γ_d - DRY UNIT WEIGHT</p> <p>SAMPLE ABBREVIATIONS S - BULK SS - SPLIT SPOON ST - SHELBY TUBE RS - ROCK RT - RECOMPACTED TRIAXIAL CBR - CALIFORNIA BEARING RATIO</p>									
SOIL MOISTURE - CORRELATION OF TERMS										EQUIPMENT USED ON SUBJECT PROJECT										FRACTURE SPACING										BEDDING									
<p>SOIL MOISTURE SCALE (ATTERBERG LIMITS)</p> <p>FIELD MOISTURE DESCRIPTION</p> <p>GUIDE FOR FIELD MOISTURE DESCRIPTION</p> <p>LL - LIQUID LIMIT</p> <p>PL - PLASTIC LIMIT</p> <p>OM - OPTIMUM MOISTURE SHRINKAGE LIMIT</p> <p>SL - SHRINKAGE LIMIT</p> <p>- SATURATED - (SAT.) USUALLY LIQUID; VERY WET, USUALLY FROM BELOW THE GROUND WATER TABLE</p> <p>- WET - (W) SEMISOLID; REQUIRES DRYING TO ATTAIN OPTIMUM MOISTURE</p> <p>- MOIST - (M) SOLID; AT OR NEAR OPTIMUM MOISTURE</p> <p>- DRY - (D) REQUIRES ADDITIONAL WATER TO ATTAIN OPTIMUM MOISTURE</p>										<p>DRILL UNITS:</p> <p><input type="checkbox"/> CME-45C</p> <p><input checked="" type="checkbox"/> CME-55</p> <p><input type="checkbox"/> CME-550</p> <p><input type="checkbox"/> VANE SHEAR TEST</p> <p><input type="checkbox"/> PORTABLE HOIST</p> <p><input checked="" type="checkbox"/> MOBILE B-52</p> <p>ADVANCING TOOLS:</p> <p><input type="checkbox"/> CLAY BITS</p> <p><input type="checkbox"/> 6" CONTINUOUS FLIGHT AUGER</p> <p><input checked="" type="checkbox"/> 8" HOLLOW AUGERS</p> <p><input type="checkbox"/> HARD FACED FINGER BITS</p> <p><input type="checkbox"/> TUNG-CARBIDE INSERTS</p> <p><input type="checkbox"/> CASING <input type="checkbox"/> W/ ADVANCER</p> <p><input checked="" type="checkbox"/> TRICONE 2 1/8" STEEL TEETH</p> <p><input type="checkbox"/> TRICONE " TUNG-CARB.</p> <p><input type="checkbox"/> CORE BIT</p> <p>HAMMER TYPE:</p> <p><input checked="" type="checkbox"/> AUTOMATIC <input type="checkbox"/> MANUAL</p> <p>CORE SIZE:</p> <p><input type="checkbox"/> -B <input type="checkbox"/> -H <input type="checkbox"/> -N</p> <p>HAND TOOLS:</p> <p><input type="checkbox"/> POST HOLE DIGGER</p> <p><input checked="" type="checkbox"/> HAND AUGER</p> <p><input checked="" type="checkbox"/> SOUNDING ROD</p> <p><input type="checkbox"/> VANE SHEAR TEST</p>										<p>VERY WIDE MORE THAN 10 FEET</p> <p>WIDE 3 TO 10 FEET</p> <p>MODERATELY CLOSE 1 TO 3 FEET</p> <p>CLOSE 0.16 TO 1 FOOT</p> <p>VERY CLOSE LESS THAN 0.16 FEET</p>										<p>TERM SPACING THICKNESS</p> <p>VERY THICKLY BEDDED 4 FEET</p> <p>THICKLY BEDDED 1.5 - 4 FEET</p> <p>THINLY BEDDED 0.16 - 1.5 FEET</p> <p>VERY THINLY BEDDED 0.03 - 0.16 FEET</p> <p>THICKLY LAMINATED 0.008 - 0.03 FEET</p> <p>THINLY LAMINATED < 0.008 FEET</p>									
PLASTICITY										INDURATION										BENCH MARK										NOTES									
<p>PLASTICITY INDEX (PI)</p> <p>DRY STRENGTH</p> <p>NON PLASTIC 0-5 VERY LOW</p> <p>SLIGHTLY PLASTIC 6-15 SLIGHT</p> <p>MODERATELY PLASTIC 16-25 MEDIUM</p> <p>HIGHLY PLASTIC 26 OR MORE HIGH</p>										<p>FOR SEDIMENTARY ROCKS, INDURATION IS THE HARDENING OF MATERIAL BY CEMENTING, HEAT, PRESSURE, ETC.</p> <p>FRIABLE RUBBING WITH FINGER FREES NUMEROUS GRAINS; GENTLE BLOW BY HAMMER DISINTEGRATES SAMPLE.</p> <p>MODERATELY INDURATED GRAINS CAN BE SEPARATED FROM SAMPLE WITH STEEL PROBE; BREAKS EASILY WHEN HIT WITH HAMMER.</p> <p>INDURATED GRAINS ARE DIFFICULT TO SEPARATE WITH STEEL PROBE; DIFFICULT TO BREAK WITH HAMMER.</p> <p>EXTREMELY INDURATED SHARP HAMMER BLOWS REQUIRED TO BREAK SAMPLE; SAMPLE BREAKS ACROSS GRAINS.</p>										<p>BENCH MARK: N/A</p> <p>ELEVATION: N/A FEET</p>										<p>NOTES:</p> <p>BORING ELEVATIONS OBTAINED FROM PROJECT TIN FILE U5813.LS.TIN.TIN, RECEIVED ON AUGUST 2, 2019.</p>									
COLOR										EQUIPMENT USED ON SUBJECT PROJECT										BENCH MARK										NOTES									
<p>DESCRIPTIONS MAY INCLUDE COLOR OR COLOR COMBINATIONS (TAN, RED, YELLOW-BROWN, BLUE-GRAY). MODIFIERS SUCH AS LIGHT, DARK, STREAKED, ETC. ARE USED TO DESCRIBE APPEARANCE.</p>										<p>DRILL UNITS:</p> <p><input type="checkbox"/> CME-45C</p> <p><input checked="" type="checkbox"/> CME-55</p> <p><input type="checkbox"/> CME-550</p> <p><input type="checkbox"/> VANE SHEAR TEST</p> <p><input type="checkbox"/> PORTABLE HOIST</p> <p><input checked="" type="checkbox"/> MOBILE B-52</p> <p>ADVANCING TOOLS:</p> <p><input type="checkbox"/> CLAY BITS</p> <p><input type="checkbox"/> 6" CONTINUOUS FLIGHT AUGER</p> <p><input checked="" type="checkbox"/> 8" HOLLOW AUGERS</p> <p><input type="checkbox"/> HARD FACED FINGER BITS</p> <p><input type="checkbox"/> TUNG-CARBIDE INSERTS</p> <p><input type="checkbox"/> CASING <input type="checkbox"/> W/ ADVANCER</p> <p><input checked="" type="checkbox"/> TRICONE 2 1/8" STEEL TEETH</p> <p><input type="checkbox"/> TRICONE " TUNG-CARB.</p> <p><input type="checkbox"/> CORE BIT</p> <p>HAMMER TYPE:</p> <p><input checked="" type="checkbox"/> AUTOMATIC <input type="checkbox"/> MANUAL</p> <p>CORE SIZE:</p> <p><input type="checkbox"/> -B <input type="checkbox"/> -H <input type="checkbox"/> -N</p> <p>HAND TOOLS:</p> <p><input type="checkbox"/> POST HOLE DIGGER</p> <p><input checked="" type="checkbox"/> HAND AUGER</p> <p><input checked="" type="checkbox"/> SOUNDING ROD</p> <p><input type="checkbox"/> VANE SHEAR TEST</p>										<p>BENCH MARK: N/A</p> <p>ELEVATION: N/A FEET</p>										<p>NOTES:</p> <p>BORING ELEVATIONS OBTAINED FROM PROJECT TIN FILE U5813.LS.TIN.TIN, RECEIVED ON AUGUST 2, 2019.</p>									

03-NOV-2019 13:51 W:\share\GEO\TECHNICAL\Projects\Active Projects\20190942.009A U-5813 Roadway\U5813_GEO_RDWY\CADD_GEO\TECH\PlanProf\U-5813_rdy_tsh.dgn AT KAZI1387

TIP PROJECT: U-5813

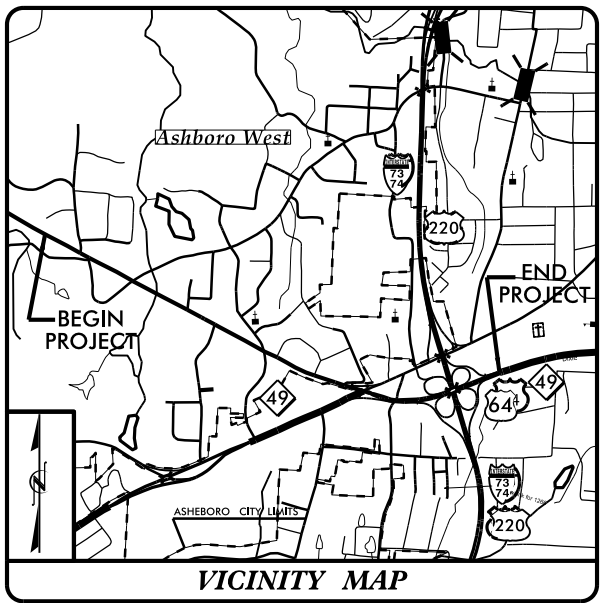
STATE OF NORTH CAROLINA
DIVISION OF HIGHWAYS

RANDOLPH COUNTY

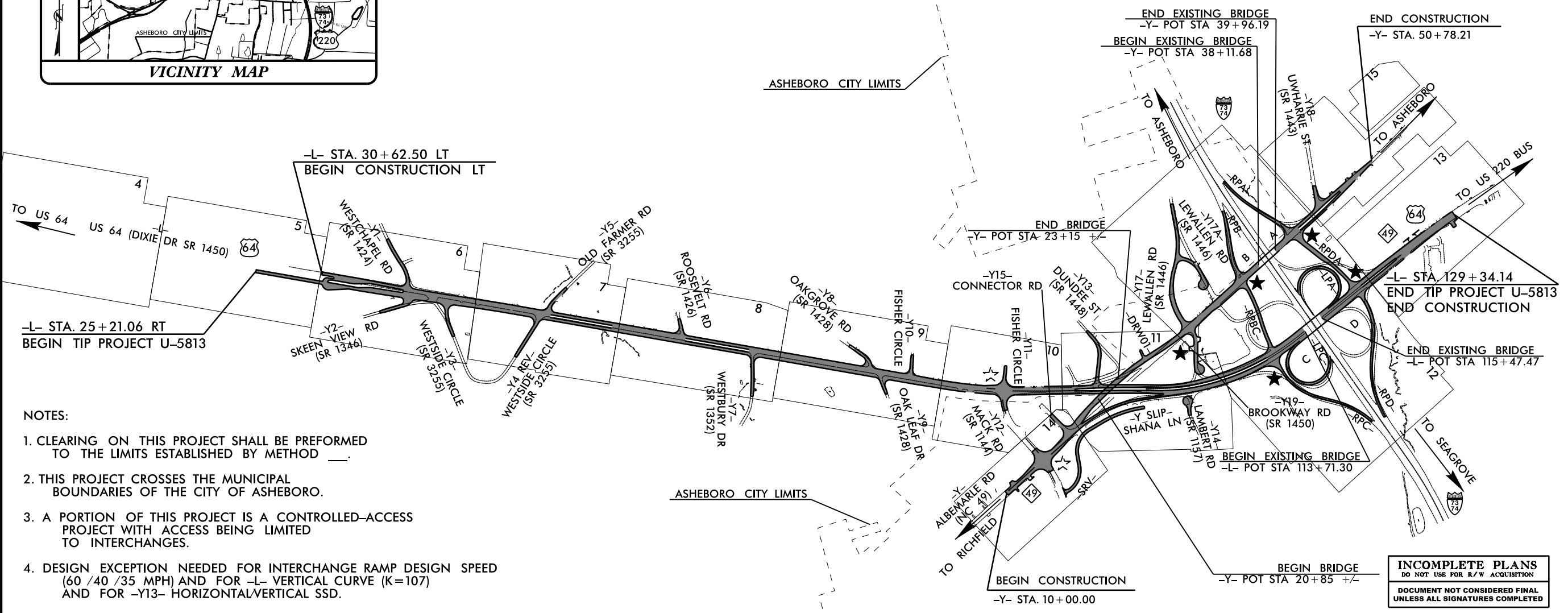
LOCATION: US 64 FROM ASHEBORO BYPASS TO EAST OF I-73/I-74/US 220 TO ASHEBORO

TYPE OF WORK: GRADING, DRAINAGE, PAVING, STRUCTURES, CULVERT & SIGNALS

STATE	STATE PROJECT REFERENCE NO.	SHEET NO.	TOTAL SHEETS
N.C.	U-5813	1	201
STATE PROJ. NO.	F.A. PROJ. NO.	DESCRIPTION	
44385.1.3		PE	
44385.2.3		RW /UTILITIES	

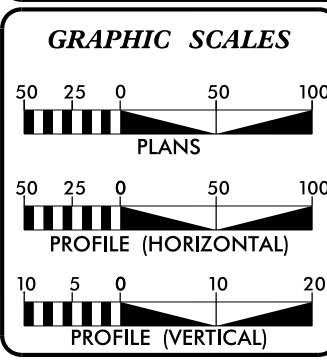


- ☆ EXISTING SIGNAL
- ★ PROPOSED SIGNAL



- NOTES:
- CLEARING ON THIS PROJECT SHALL BE PERFORMED TO THE LIMITS ESTABLISHED BY METHOD ____.
 - THIS PROJECT CROSSES THE MUNICIPAL BOUNDARIES OF THE CITY OF ASHEBORO.
 - A PORTION OF THIS PROJECT IS A CONTROLLED-ACCESS PROJECT WITH ACCESS BEING LIMITED TO INTERCHANGES.
 - DESIGN EXCEPTION NEEDED FOR INTERCHANGE RAMP DESIGN SPEED (60 /40 /35 MPH) AND FOR -L- VERTICAL CURVE (K=107) AND FOR -Y13- HORIZONTAL/VERTICAL SSD.

INCOMPLETE PLANS
DO NOT USE FOR R/W ACQUISITION
DOCUMENT NOT CONSIDERED FINAL
UNLESS ALL SIGNATURES COMPLETED



DESIGN DATA

ADT 2020 =	32,200
ADT 2040 =	35,700
K =	8%
D =	55%
T =	4 % *
V =	60 MPH
* TTST = 3% DUAL 1%	
FUNC CLASS =	
PRINCIPAL ARTERIAL	
STATEWIDE TIER	

PROJECT LENGTH

LENGTH ROADWAY TIP PROJECT U-5813	=	1.939 MILES
TOTAL LENGTH TIP PROJECT U-5813	=	1.939 MILES
TOTAL LENGTH BASED ON -L- CENTERLINE AND RIGHT SIDE BEGIN CONSTRUCTION		

PREPARED IN THE OFFICE OF:

HNTB
HNTB NORTH CAROLINA, P.C.
343 E. Six Forks Road, Suite 200
Raleigh, North Carolina 27609
NC License No: C-1554
FOR NCDOT

2018 STANDARD SPECIFICATIONS

RIGHT OF WAY DATE:
AUGUST 15, 2019

LETTING DATE:
SEPTEMBER 15, 2020

CORY GRELL, PE PROJECT ENGINEER
TATYANA GIBBS, EI PROJECT DESIGN ENGINEER
ALLISON WHITE NCDOT CONTACT

HYDRAULICS ENGINEER

SIGNATURE: _____ P.E.

ROADWAY DESIGN ENGINEER

SIGNATURE: _____ P.E.





November 3, 2019

STATE PROJECT: 44385.1.3 (U-5813)
 COUNTY: Randolph
 DESCRIPTION: US 64 from the Asheboro Bypass to East of I-73/I-74/US 220

SUBJECT: GEOTECHNICAL REPORT - INVENTORY

PROJECT DESCRIPTION

This project consists of a widening and realignment of existing US-64 (-L-) to multiple lanes. This project will also include a widening of West Chapel Road (-Y1-), Skeet View Road (-Y2-), Westside Circle (-Y3-, -Y4_REV-), Old Farmer Road (-Y5-), Roosevelt Road (-Y6-), Westbury Drive (-Y7-), Oak Grove Road (-Y8-), Oak Leaf Road (-Y9-), Fisher Circle (-Y10-, -Y11-), Mack Road (-Y12-), Connector Road (-Y15-), Driveway (-DRW01-), Shana Lane (-Y_SLIP-), and ramps (-RPA-, RPB-). This project will also include the realignment of Dundee Street (-Y13-), Lambert Road (-Y14-), Lewallen Road (-Y17-, -Y17A-), Brookway Road (-Y19-), several ramps (-RPBC-, -RPC-, -RPD-, -RPDA-) and the creation of a new alignment (-SRV-).

The geotechnical investigation was conducted between November 2018 and April 2019. Standard Penetration Test borings were advanced with a CME-55 and Mobile B-57 drill rig with an automatic hammer. Hand Augers and Rod Soundings were also performed in areas where the use of a drill rig was restricted or underground and overhead utility conflicts were observed. Representative soil samples were collected for visual classification in the field and selected samples were submitted for laboratory analysis by Froehling & Robertson and Geotechnics, Inc.

The following alignments, totaling 6.44 miles, were investigated. Plan sheets, profile sheets, and cross sections of these alignments are included in this report.

<u>LINE</u>	<u>STATIONS</u>
-L-	25+21 to 129+34
-Y-	10+00 to 50+78
-Y1-	10+00 to 14+90
-Y2-	10+80 to 14+11
-Y3-	10+00 to 13+13
-Y4_REV-	10+00 to 14+05
-Y5-	10+00 to 13+50
-Y6-	10+00 to 11+70
-Y7-	10+00 to 13+70
-Y8-	10+00 to 13+00
-Y9-	10+00 to 11+74
-Y10-	10+00 to 12+00
-Y11-	10+00 to 12+05
-Y12-	10+00 to 20+45
-Y13-	10+00 to 15+16
-Y14-	10+00 to 11+08
-Y15-	10+00 to 12+10
-Y_SLIP-	10+00 to 17+21
-Y17-	10+00 to 20+65

-Y17A-	10+00 to 15+23
-Y18-	10+00 to 15+16
-Y19-	10+00 to 13+64
-RPBC-	10+00 to 16+69
-RPDA-	10+00 to 11+83
-RPA-	10+00 to 18+82
-RPB-	10+00 to 18+15
-RPC-	10+00 to 21+44
-RPD-	10+00 to 21+50
-LPA-	10+00 to 21+76
-LPC-	10+00 to 20+63
-DRW01-	10+00 to 10+80
-SRV-	10+00 to 21+37

PHYSIOGRAPHY AND GEOLOGY

The project is located in the Piedmont Physiographic Province. The project corridor is comprised primarily of residential, rural, commercial properties as well as undeveloped wooded areas. The general topography along the project is generally flat to steeply sloping.

Geologically, the project is located within the Carolina Slate Belt. Soils are derived from the underlying bedrock which consists of metamorphosed Volcanic Rock and metamorphosed Mudstone.

Surface water is drained from the corridor by the existing roadway ditches. Surface water is also drained to Cedar Fork Creek and Cable Creek, generally running South to North and East to West, respectively.

SOIL PROPERTIES

Soils encountered during this investigation are separated into four categories based on origin. They consist of roadway embankment, artificial fill, alluvial soil, and residual soils.

Roadway Embankment soils are present along the existing roadways on the project. The roadway embankment encountered generally consist of moist to wet, coarse to fine sandy silt and clayey silt (A-4, A-5), moist, medium stiff to stiff, silty coarse to fine sand (A-2-4) with trace gravel; dry to wet, medium stiff to stiff, silty clay and sandy clay (A-7, A-6) with trace to little gravel; and moist, loose, coarse to fine sandy gravel (A-1-a). The plasticity index of the road embankment clays tested ranged from 11 to 16. The plasticity index of the roadway embankment silt tested was 3.

Soil identified as artificial fill are present adjacent to the existing roadway. The artificial fill encountered generally consists of moist to wet, soft to very stiff, sandy silts and clayey silts (A-4, A-5) with trace gravel; moist to wet, and soft to medium stiff, sandy clays and silty clays (A-6, A-7) with trace gravel. The plasticity index of the artificial fill silts tested ranged from 8 to 9. The plasticity index of the artificial fill clays tested ranged from 12 to 29.

Alluvial soils are present along the existing streams and creeks on the project. The alluvial soils encountered generally consist of moist to wet, very soft to stiff, silty clays and sandy clays (A-7, A-6) with a trace of organic matter; and moist, medium stiff, sandy silt (A-4). The plasticity index of the alluvial silt tested was 8. The plasticity index of the alluvial clay tested was 11.

Residual soils are derived from the weathering of underlying metavolcanics and metamudstone. The majority of the residual soils encountered consist of dry to wet, soft to hard, sandy silts, and clayey silts (A-4, A-5)

with little rock fragments and trace manganese; moist, soft to stiff, sandy clays and silty clays (A-6, A-7); and dry to moist, medium dense to very dense, silty sands and sands (A-2-4, A-1) with trace to little rock fragments. The plasticity index of the residual silts tested ranged from 9 to 13. The plasticity index of the clays tested ranged from 11 to 47.

ROCK PROPERTIES

Weathered rock was encountered along the existing roadways (-L-) at elevations ranging from 719.5 to 850.9 feet (MSL) and typically ranges from 8 to 78 feet below the existing ground surface; (-Y-) at elevations ranging from 816.2 to 877.3 and typically ranges from 6 to 24 feet below the existing ground surface, (-Y_SLIP-) at elevation 860.6 feet, 5 feet below the existing ground surface. (-Y11-) at elevation 845.4 feet, 10 feet below the existing ground surface; (-RPBC-) at elevation 847.5 feet, 4 feet below the existing ground surface, (-Y4_REV-) at elevation 782.6 feet, 8 feet below the existing ground surface; (-Y7-) at elevation 788.4 feet, 7 feet below the existing ground surface; (-Y13-) at elevation 851.4 feet, 7 feet below the existing ground surface; (-Y17-) at elevations ranging from 851.3 to 871.7 feet and typically ranges from 3 to 17 feet below the existing ground surface and (-Y17A-) at elevation 873.0 feet, 8 feet below the existing ground surface. The weathered rock consists of metavolcanic rock and metamudstone.

Crystalline rock was encountered along the existing roadways (-L-) at elevations ranging from 720.2 to 814.0 feet (MSL) and typically ranges from 2 to 24 feet below the existing ground surface; (-Y-) at elevations ranging from 823.1 to 875.4 feet and typically ranges from 7 to 36 feet below the existing ground surface; (-Y_SLIP-) at elevation 853.5 feet, 14 feet below the existing ground surface; (-Y11-) at elevation 844.6 feet, 10 feet below the existing ground surface, (-RPBC-) at elevation 847.1 feet, 4 feet below the existing ground surface; (-RPA-) at elevation 840.4 feet, 7 feet below the existing ground surface; (-RPB-) at elevation 850.7 feet, 10 feet below the existing ground surface, (-RPC-) at elevation 824.6 feet, 9 feet below the existing ground surface; (-LPC-) at elevation 833.7 feet, 8 feet below the existing ground surface; (-Y13-) at elevation 859.5 feet, 5 feet below the existing ground surface; (-Y17-) at elevation 850.0 feet, 4 feet below the existing ground surface; and (-Y19-) at elevation 864.9 feet, 12 feet below the existing ground surface. The crystalline rock consists of metavolcanic rock and metamudstone.

GROUNDWATER

Groundwater was encountered at elevations ranging from 727.5 to 888.7 feet and typically ranges from 0 to 23 feet below the existing ground surface.

AREAS OF SPECIAL GEOTECHNICAL INTEREST

1) Highly Plastic Clays: Highly plastic clays (PI > 25) were encountered on the project at the following locations:

<u>LINE</u>	<u>STATIONS</u>	<u>OFFSETS</u>
-L-	33+75 to 35+25	LT
-L-	40+25 to 41+25	LT
-L-	43+25 to 44+25	LT, RT
-L-	49+75 to 52+75	LT, RT
-L-	51+75 to 52+75	LT, RT
-L-	62+25 to 63+25	LT
-L-	64+25 to 65+25	LT
-L-	70+25 to 71+75	LT
-Y-	46+25 to 50+75	LT, RT
-Y3-	10+00 to 10+38	LT, RT

-RPB-	13+63 to 14+88	RT
-RPDA-	12+88 to 14+13	LT, RT

Other highly plastic clays were found that will not impact construction.

2) Groundwater: The following areas exhibit a high water table, seasonal high groundwater or the potential for groundwater related construction problems:

<u>LINE</u>	<u>STATIONS</u>	<u>OFFSETS</u>
-L-	92+00 to 93+50	LT, RT
-Y13-	10+00 to 13+50	LT, RT
-Y14-	10+00 to 11+08	LT, RT
-Y17-	14+00 to 19+00	LT, RT

2) Alluvial Soil: Alluvial Soil was encountered on the project at the following locations:

<u>LINE</u>	<u>STATIONS</u>	<u>OFFSETS</u>
-L-	25+00 to 27+00	RT
-L-	54+00 to 57+00	RT
-L-	71+50 to 75+50	RT
-L-	84+50 to 85+50	LT
-L-	88+50 to 89+50	RT
-L-	92+00 to 94+50	RT
-L-	105+00 to 107+50	LT, RT
-L-	117+50 to 118+50	RT
-Y-	18+50 to 21+50	LT
-Y5-	10+50 to 13+75	LT
-Y7-	10+00 to 11+50	LT
-Y17-	19+50 to 20+50	LT
-Y19-	12+00 to 12+50	LT
-RPB-	12+00 to 13+00	LT
-RPD-	18+00 to 19+00	LT, RT

3) Artificial Fill: Artificial Fill was encountered on the project at the following locations:

<u>LINE</u>	<u>STATIONS</u>	<u>OFFSETS</u>
-L-	26+50 to 33+50	RT
-L-	45+00 to 48+00	LT
-L-	52+00 to 60+50	LT
-L-	74+00 to 76+00	RT
-L-	81+50 to 87+50	RT
-L-	88+00 to 91+00	LT, RT
-L-	104+00 to 110+00	LT
-L-	103+00 to 108+00	RT

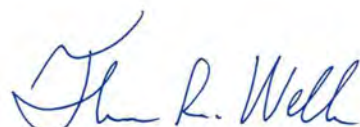
Artificial Fill (Continued)

<u>LINE</u>	<u>STATIONS</u>	<u>OFFSETS</u>
-L-	121+25 to 129+92	LT
-L-	120+50 to 129+92	RT
-Y8-	10+50 to 13+00	LT
-Y11-	10+50 to 12+00	RT
-Y12-	10+00 to 13+50	RT
-Y12-	14+00 to 20+00	LT, RT
-Y14-	10+00 to 11+50	LT, RT
-Y15-	10+00 to 13+00	LT
-Y_SLIP-	10+00 to 17+25	RT
-Y18-	10+50 to 12+00	RT
-RPDA-	10+50 to 13+25	LT, RT
-RPDA-	14+00 to 17+00	LT, RT
-RPC-	12+00 to 18+00	RT
-RPD-	14+75 to 16+75	LT
-LPA-	16+50 to 20+00	LT, RT
-LPC-	12+00 to 15+50	LT
-SRV-	10+00 to 21+37	LT, RT

Prepared by,
KLEINFELDER, INC.
NC License No. F-1312



Stefanie Papke
Staff Professional



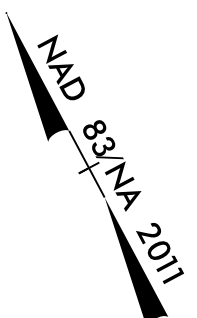
Thomas R. Wells, PE
Senior Professional

SAP/TRW:cas

Bulk Sample
Shelby Tube

<u>SAMPLE NO.</u>	<u>ALIGNMENT</u>	<u>STA.</u>	<u>OFFSET</u>	<u>DEPTH (FT)</u>	<u>TESTS PERFORMED</u>
ST-1	-L-	48+00	50' RT	0.0 – 2.0	Consolidation
ST-2	-L-	72+50	90' RT	0.0 – 2.0	Consolidation
BS-1	-Y17-	10+93	103' LT	0.0 – 10.0'	Standard Proctor, CBR
BS-2	-RPDA-	11+50	40' LT	0.0 – 10.0'	Standard Proctor, CBR

PROJECT REFERENCE NO. U-5813	SHEET NO. 4
RW SHEET NO.	
ROADWAY DESIGN ENGINEER	HYDRAULICS ENGINEER
INCOMPLETE PLANS DO NOT USE FOR R/W ACQUISITION	
DOCUMENT NOT CONSIDERED FINAL UNLESS ALL SIGNATURES COMPLETED	



R-2536 DB
 TO BE CONSTRUCTED
 BY OTHERS

-L- POT Sta. 10+00.00 =
 R-2536 -LFUTURE-
 POT Sta. 9+99.95

R-2536 DB
 TO BE CONSTRUCTED
 BY OTHERS

-L- 10

-L- 15

S 61°59'43.8" E -L-

MATCH LINE -L- 17 + 00
 SEE SHEET 5

REVISIONS

I5-OCT-2019_08:49 W:\sherpa\GEO\TECHNICAL\Projects\Active Projects\2011-12-20-11-0-D015 Roadway\05813-U-5813-Roadway\05813-U-5813-Roadway\05813-U-5813-Roadway.dwg \$\$\$\$\$\$

8/17/99

HNTB HNTB NORTH CAROLINA, P.C.
343 E. Six Forks Road, Suite 200
Raleigh, North Carolina 27609
NC License No: C-1554

PROJECT REFERENCE NO.	SHEET NO.
U-5813	5
RW SHEET NO.	
ROADWAY DESIGN ENGINEER	HYDRAULICS ENGINEER
INCOMPLETE PLANS DO NOT USE FOR R/W ACQUISITION	
DOCUMENT NOT CONSIDERED FINAL UNLESS ALL SIGNATURES COMPLETED	

15-OCT-2019 09:03
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 \$\$\$\$STANDARD\$\$\$\$

REVISIONS

MATCH LINE -L- 17+00
SEE SHEET 4

KELLY CONTRACTORS, LLC
DB 2173 PG 821
(REF DB 1278 PG 830)

-L- 20

R-2536 DB
TO BE CONSTRUCTED
BY OTHERS

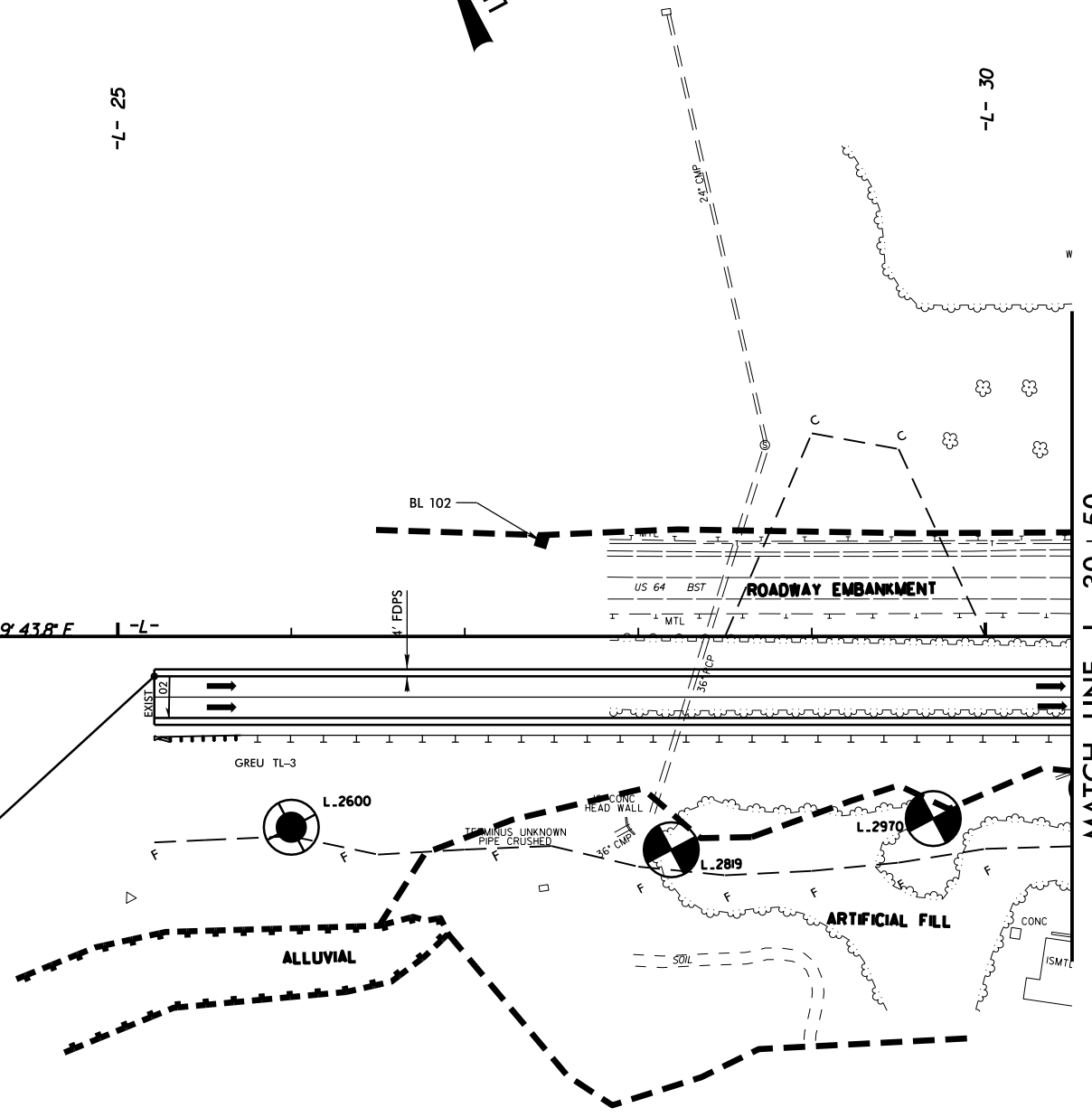
-L- 25

NAD 83/NA 2011

-L- 30

S 61°59'43.8" E -L-

BEGIN CONSTRUCTION
-L- Sta. 25+21.06 RT

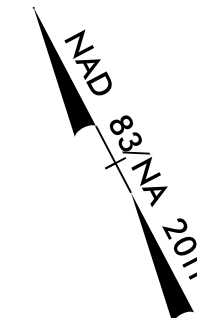
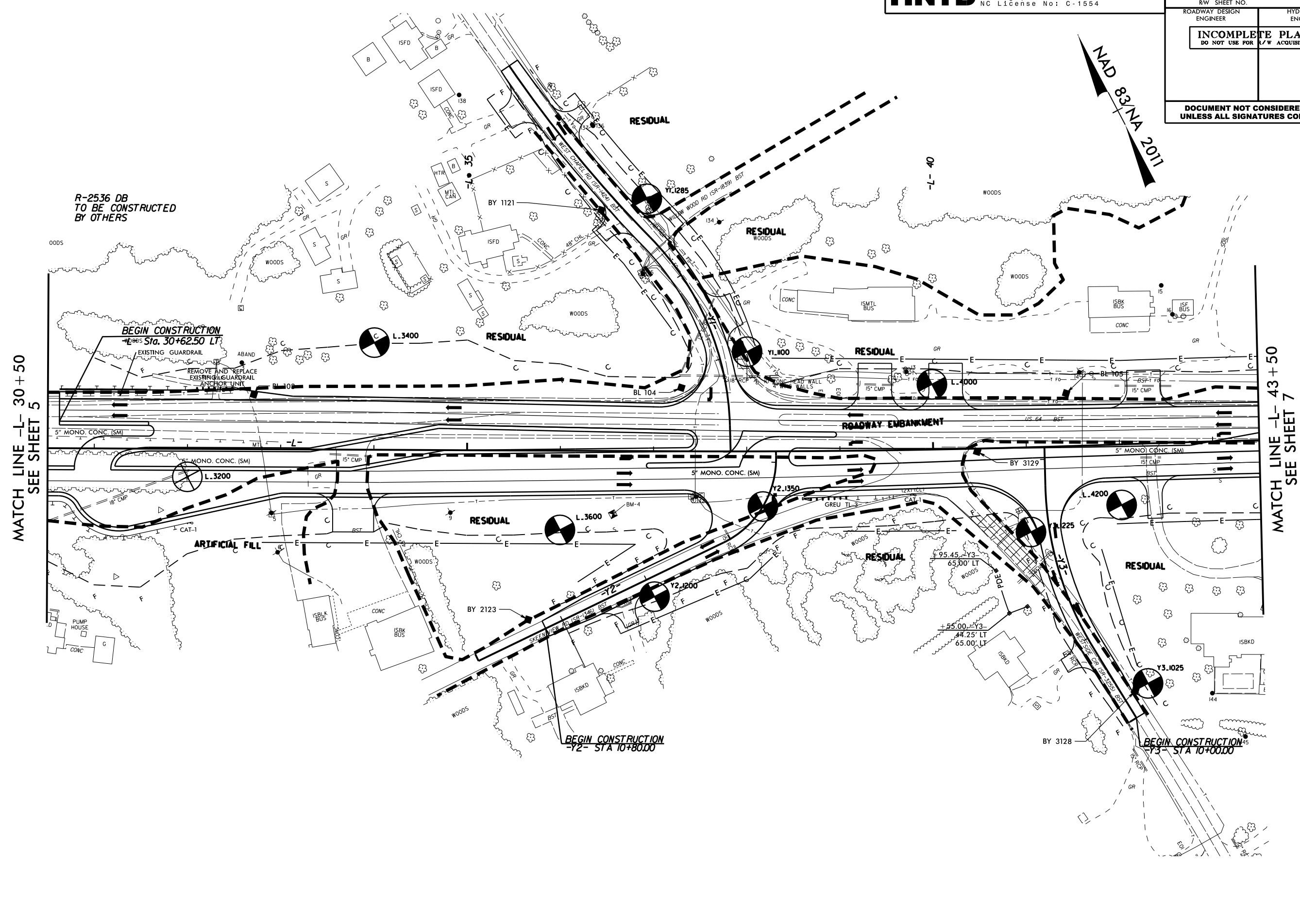


MATCH LINE -L- 30+50
SEE SHEET 6

14-OCT-2019 09:48 W:\shere\GEO\TECHNICAL\Projects\Active Projects\2019\10\14\2019-09-04\10\14\1371\10\14\1371.dwg
 8/17/99
 REVISIONS
 PROJECT REFERENCE NO. U-5813
 SHEET NO. 6
 ROADWAY DESIGN ENGINEER
 HYDRAULICS ENGINEER
 INCOMPLETE PLANS
 DO NOT USE FOR R/W ACQUISITION
 DOCUMENT NOT CONSIDERED FINAL
 UNLESS ALL SIGNATURES COMPLETED
 MATCH LINE -L- 30 + 50
 SEE SHEET 5
 MATCH LINE -L- 43 + 50
 SEE SHEET 7
 HNTB
 HNTB NORTH CAROLINA, P.C.
 343 E. Six Forks Road, Suite 200
 Raleigh, North Carolina 27609
 NC License No: C-1554
 R-2536 DB
 TO BE CONSTRUCTED
 BY OTHERS
 BEGIN CONSTRUCTION
 -Y2- STA 10+80.00
 BEGIN CONSTRUCTION
 -Y3- STA 10+00.00
 BEGIN CONSTRUCTION
 -Y1- STA 30+62.50 LT
 BEGIN CONSTRUCTION
 -Y3- STA 10+00.00

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 Raleigh, North Carolina 27609
 NC License No: C-1554

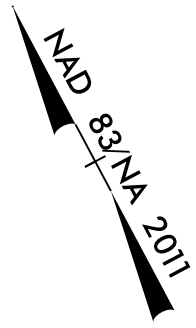
PROJECT REFERENCE NO. U-5813	SHEET NO. 6
ROADWAY DESIGN ENGINEER	HYDRAULICS ENGINEER
INCOMPLETE PLANS DO NOT USE FOR R/W ACQUISITION	
DOCUMENT NOT CONSIDERED FINAL UNLESS ALL SIGNATURES COMPLETED	



MATCH LINE -L- 30 + 50
SEE SHEET 5

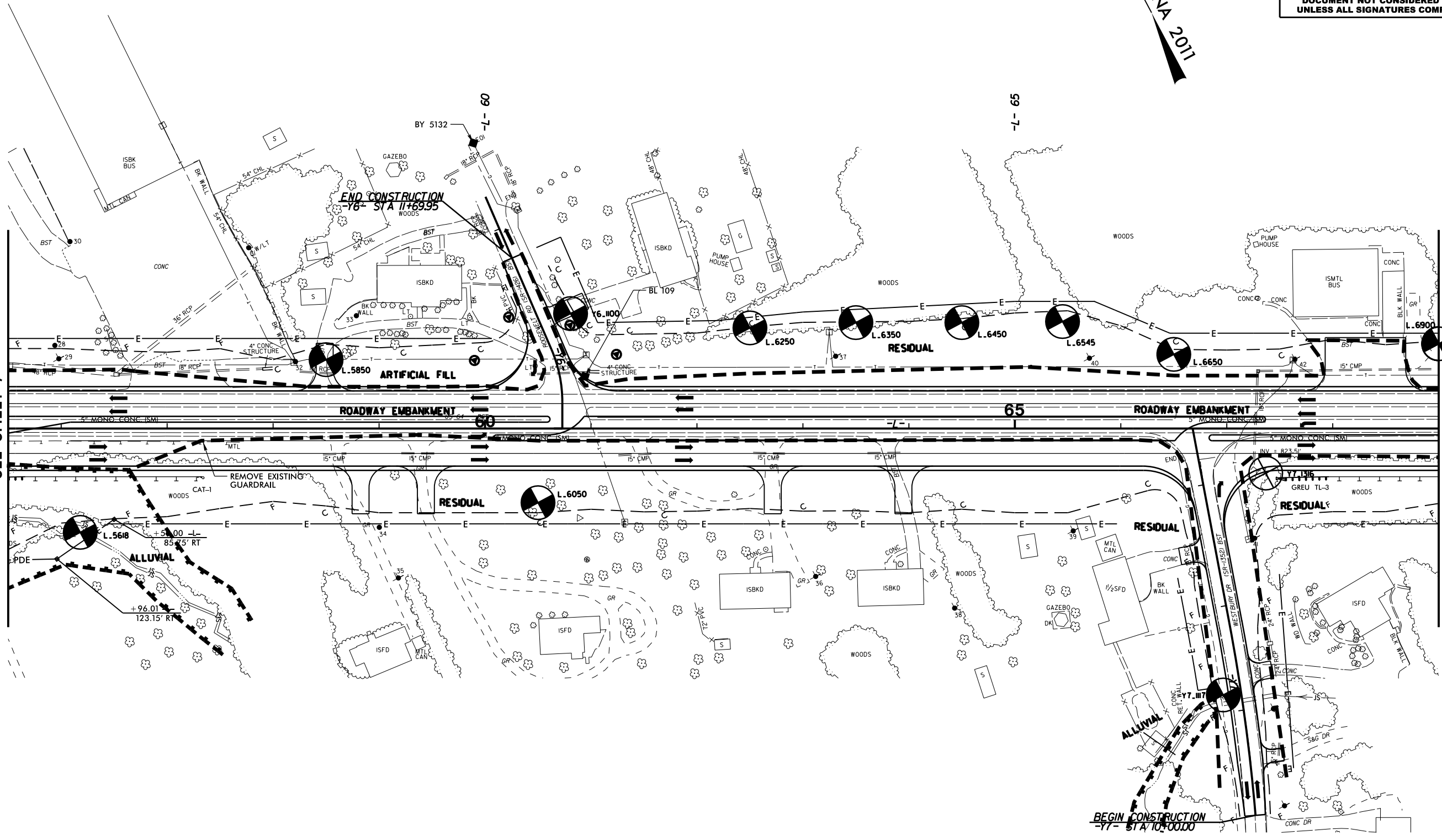
MATCH LINE -L- 43 + 50
SEE SHEET 7

PROJECT REFERENCE NO. U-5813	SHEET NO. 8
ROADWAY DESIGN ENGINEER	HYDRAULICS ENGINEER
INCOMPLETE PLANS DO NOT USE FOR R/W ACQUISITION	
DOCUMENT NOT CONSIDERED FINAL UNLESS ALL SIGNATURES COMPLETED	



MATCH LINE -L- 55 + 50
SEE SHEET 7

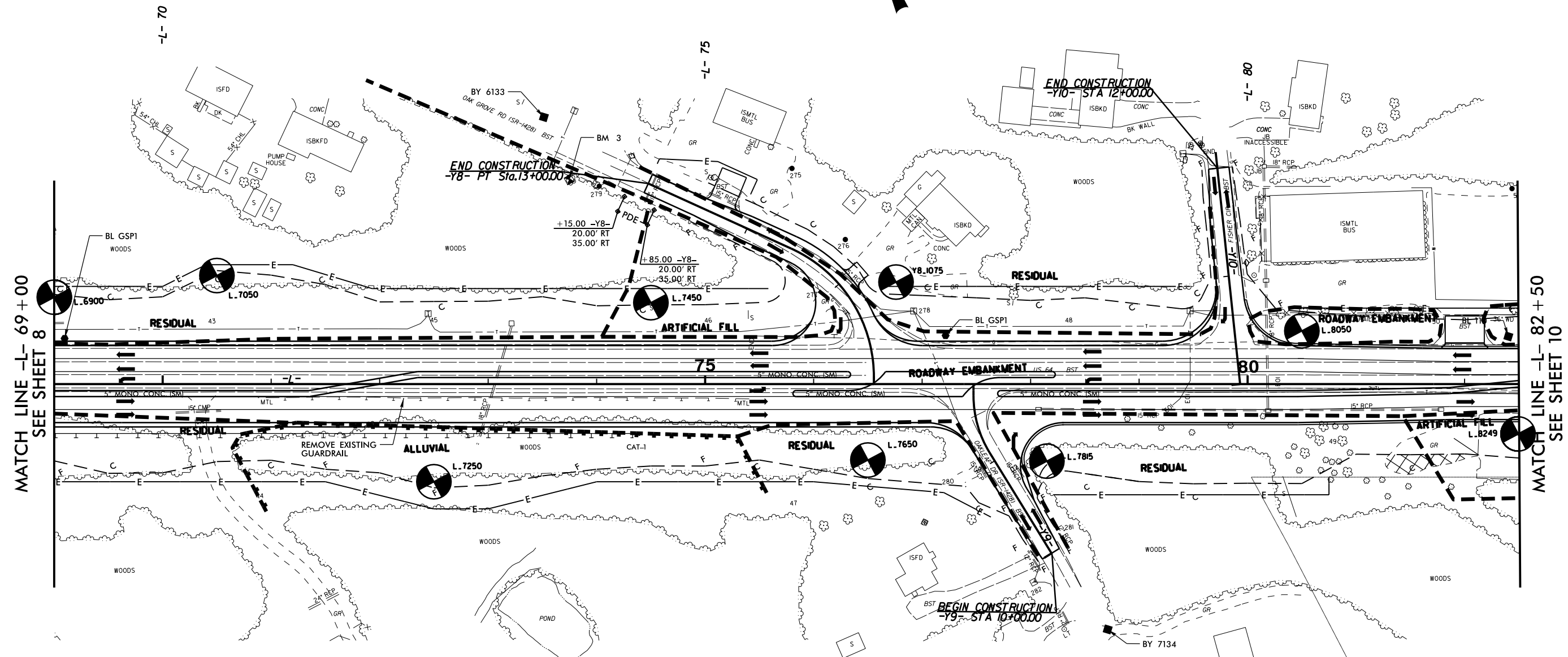
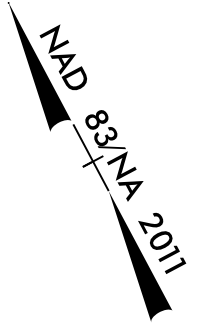
MATCH LINE -L- 69 + 00
SEE SHEET 9



REVISIONS

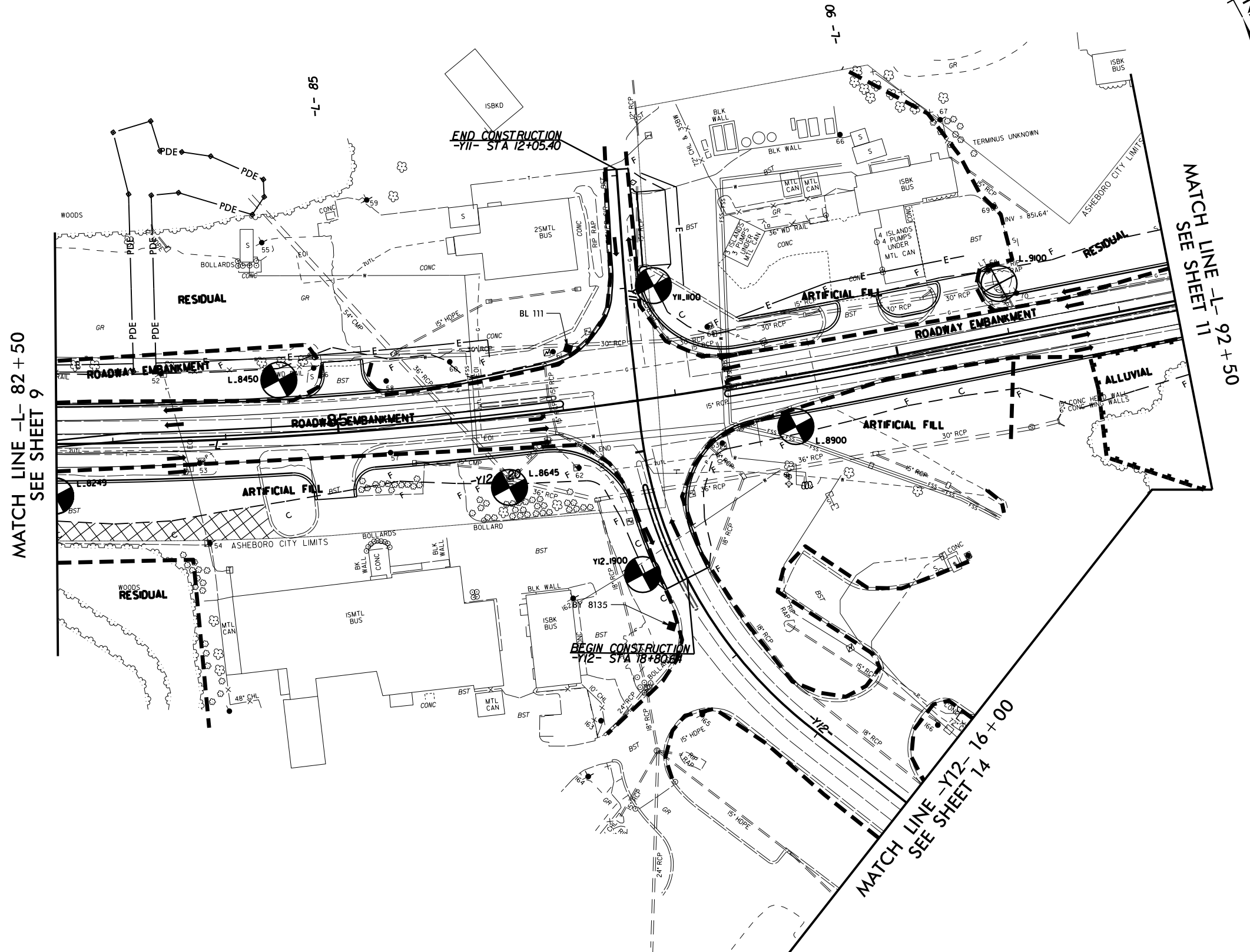
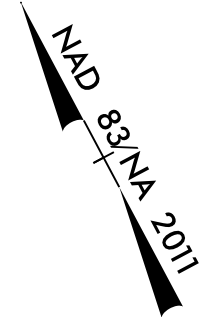
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 14-001-001-001-001-001.dwg 8/17/99

PROJECT REFERENCE NO.	SHEET NO.
U-5813	9
RW SHEET NO.	
ROADWAY DESIGN ENGINEER	HYDRAULICS ENGINEER
INCOMPLETE PLANS DO NOT USE FOR R/W ACQUISITION	
DOCUMENT NOT CONSIDERED FINAL UNLESS ALL SIGNATURES COMPLETED	



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 REVISIONS

PROJECT REFERENCE NO.	SHEET NO.
U-5813	10
RW SHEET NO.	
ROADWAY DESIGN ENGINEER	HYDRAULICS ENGINEER
INCOMPLETE PLANS DO NOT USE FOR R/W ACQUISITION	
DOCUMENT NOT CONSIDERED FINAL UNLESS ALL SIGNATURES COMPLETED	



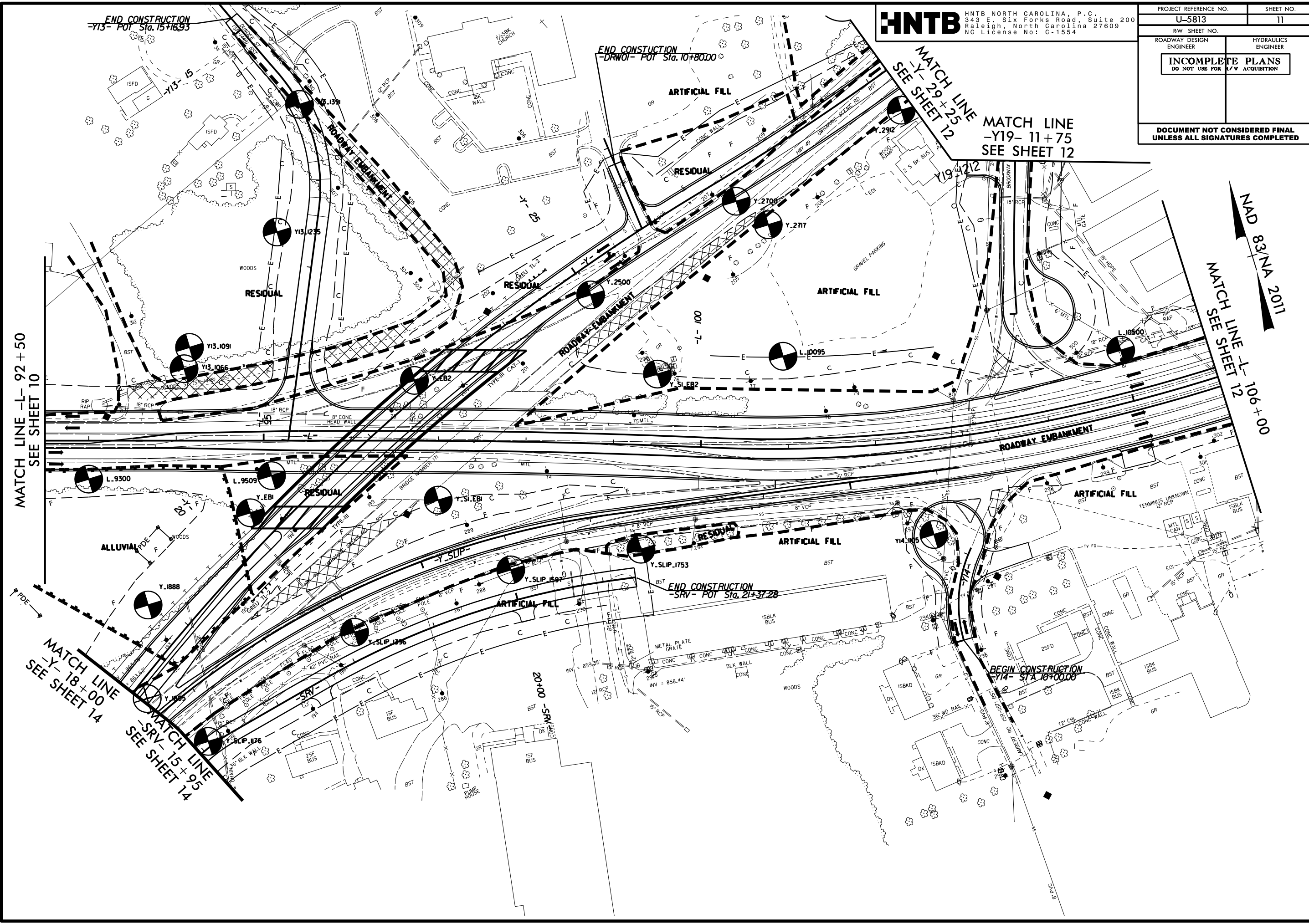
REVISIONS

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8/17/99
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PROJECT REFERENCE NO.	SHEET NO.
U-5813	11
ROW SHEET NO.	
ROADWAY DESIGN ENGINEER	HYDRAULICS ENGINEER
INCOMPLETE PLANS DO NOT USE FOR R/W ACQUISITION	
DOCUMENT NOT CONSIDERED FINAL UNLESS ALL SIGNATURES COMPLETED	



MATCH LINE -L- 92+50
SEE SHEET 10

MATCH LINE -Y- 18+00
SEE SHEET 14

MATCH LINE -SRV- 15+95
SEE SHEET 14

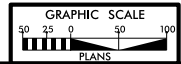
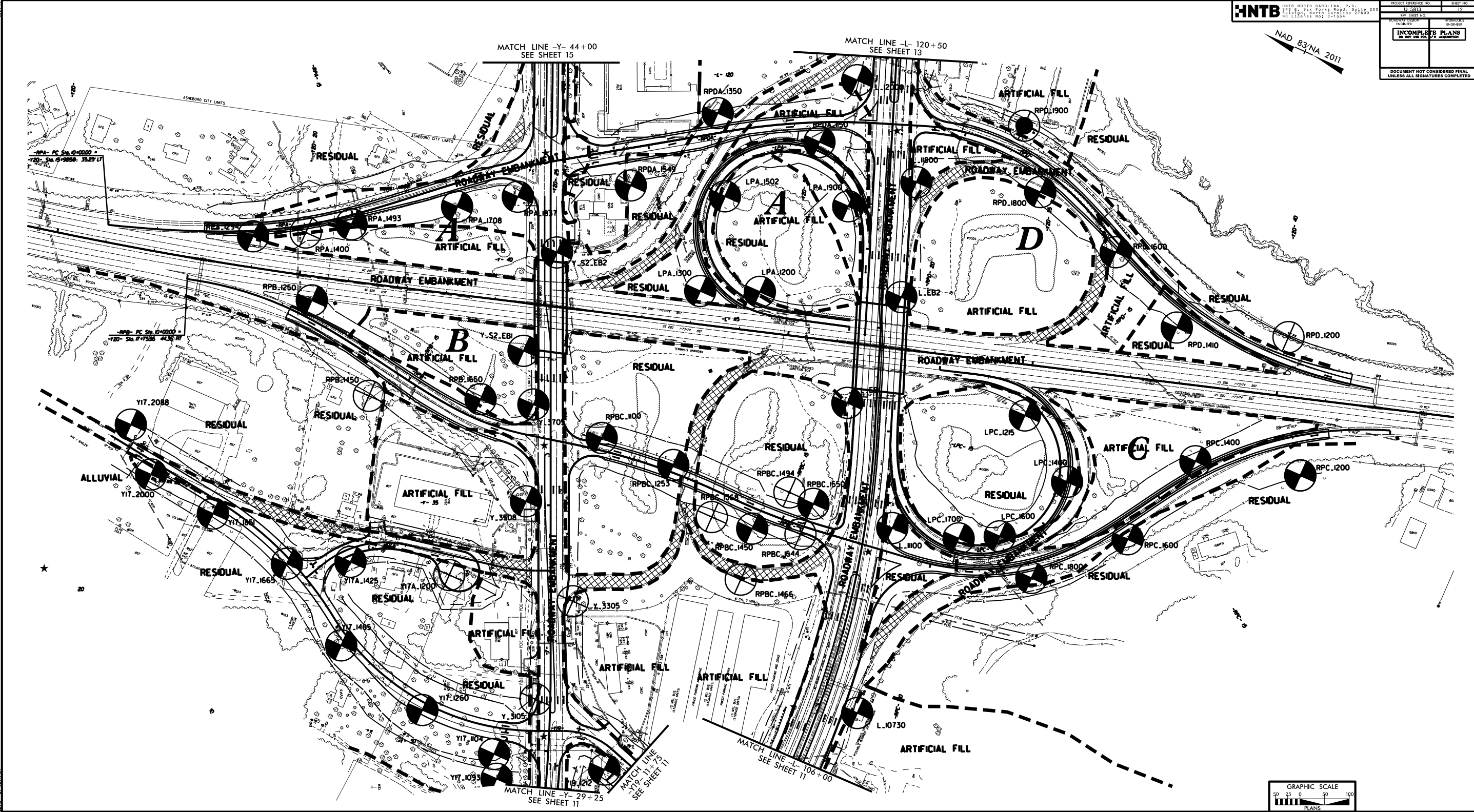
MATCH LINE -Y- 29+25
SEE SHEET 12

MATCH LINE -Y19- 11+75
SEE SHEET 12

MATCH LINE -L- 106+00
SEE SHEET 12

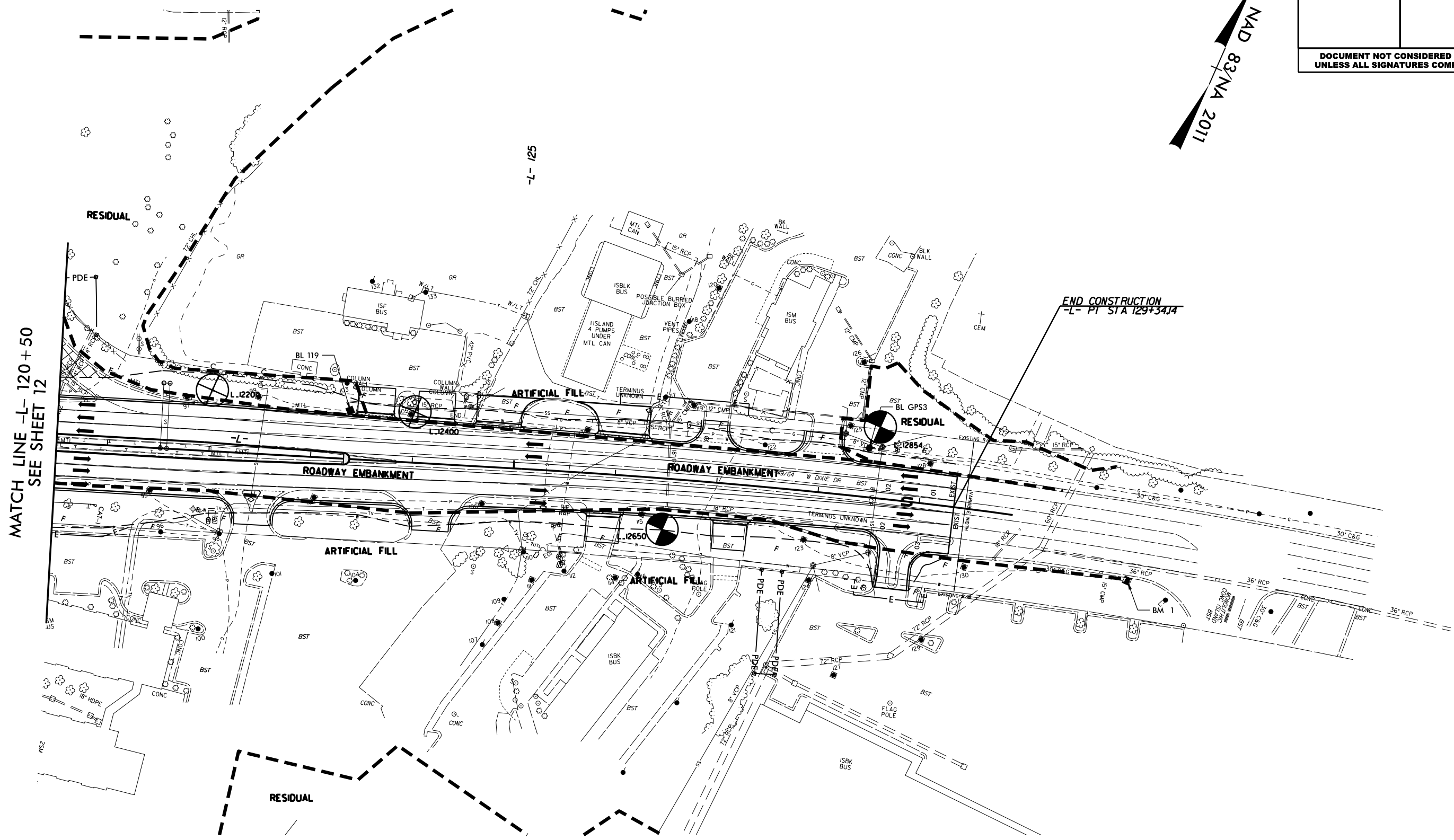
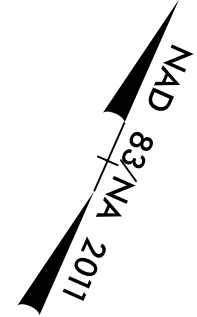
NAD 83/NA 2011

REVISIONS



Vertical text along the left edge of the drawing, likely a file path or revision history.

PROJECT REFERENCE NO.	SHEET NO.
U-5813	13
R/W SHEET NO.	ROADWAY DESIGN ENGINEER
	HYDRAULICS ENGINEER
INCOMPLETE PLANS DO NOT USE FOR R/W ACQUISITION	
DOCUMENT NOT CONSIDERED FINAL UNLESS ALL SIGNATURES COMPLETED	



MATCH LINE -L- 120+50
SEE SHEET 12

-L- 125

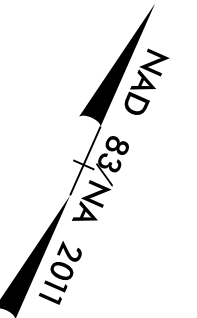
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-L- PT STA 129+34.14

REVISIONS

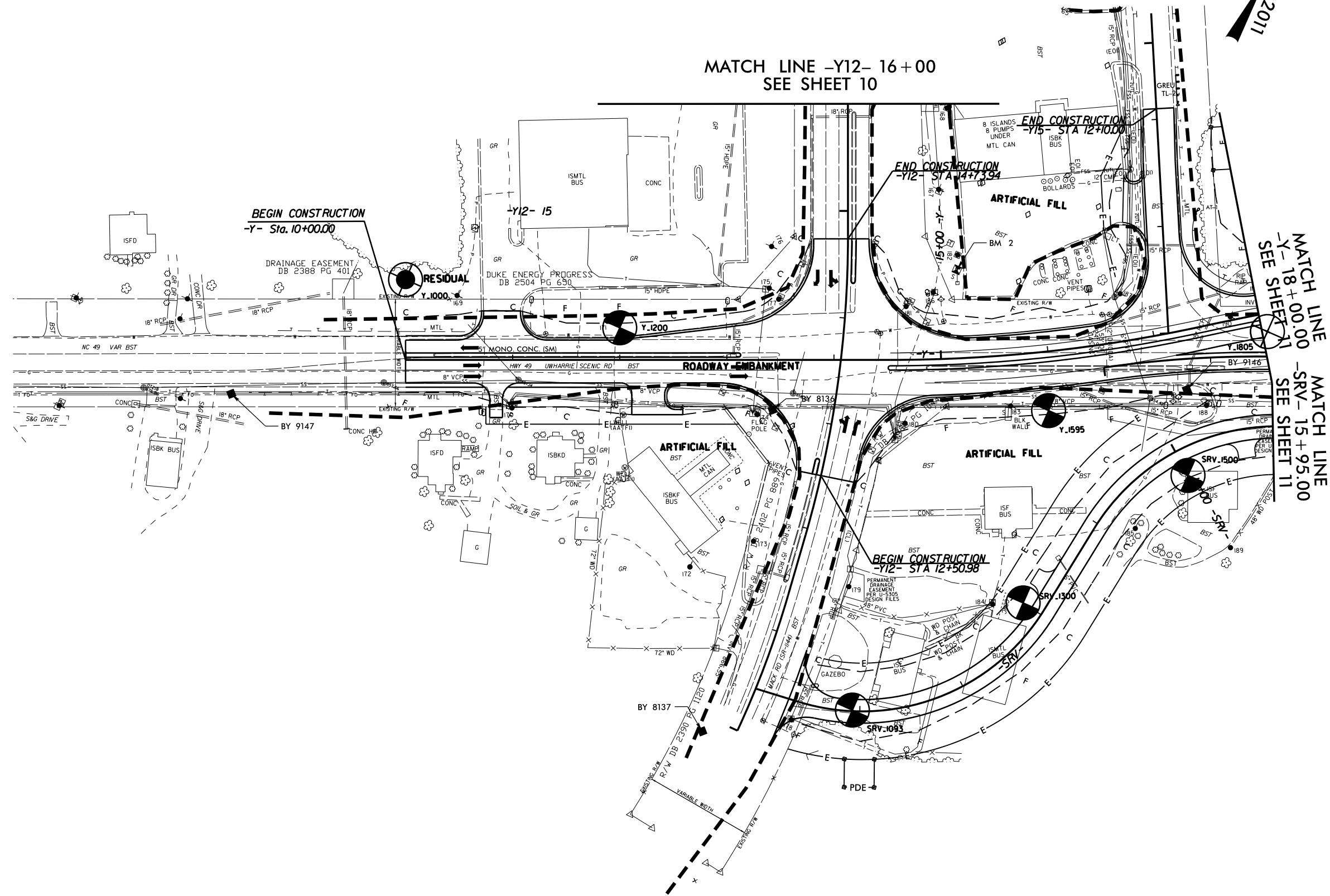
8/17/99
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PROJECT REFERENCE NO.	SHEET NO.
U-5813	14
R/W SHEET NO.	
ROADWAY DESIGN ENGINEER	HYDRAULICS ENGINEER
INCOMPLETE PLANS DO NOT USE FOR R/W ACQUISITION	
DOCUMENT NOT CONSIDERED FINAL UNLESS ALL SIGNATURES COMPLETED	

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MATCH LINE -Y12- 16+00
SEE SHEET 10

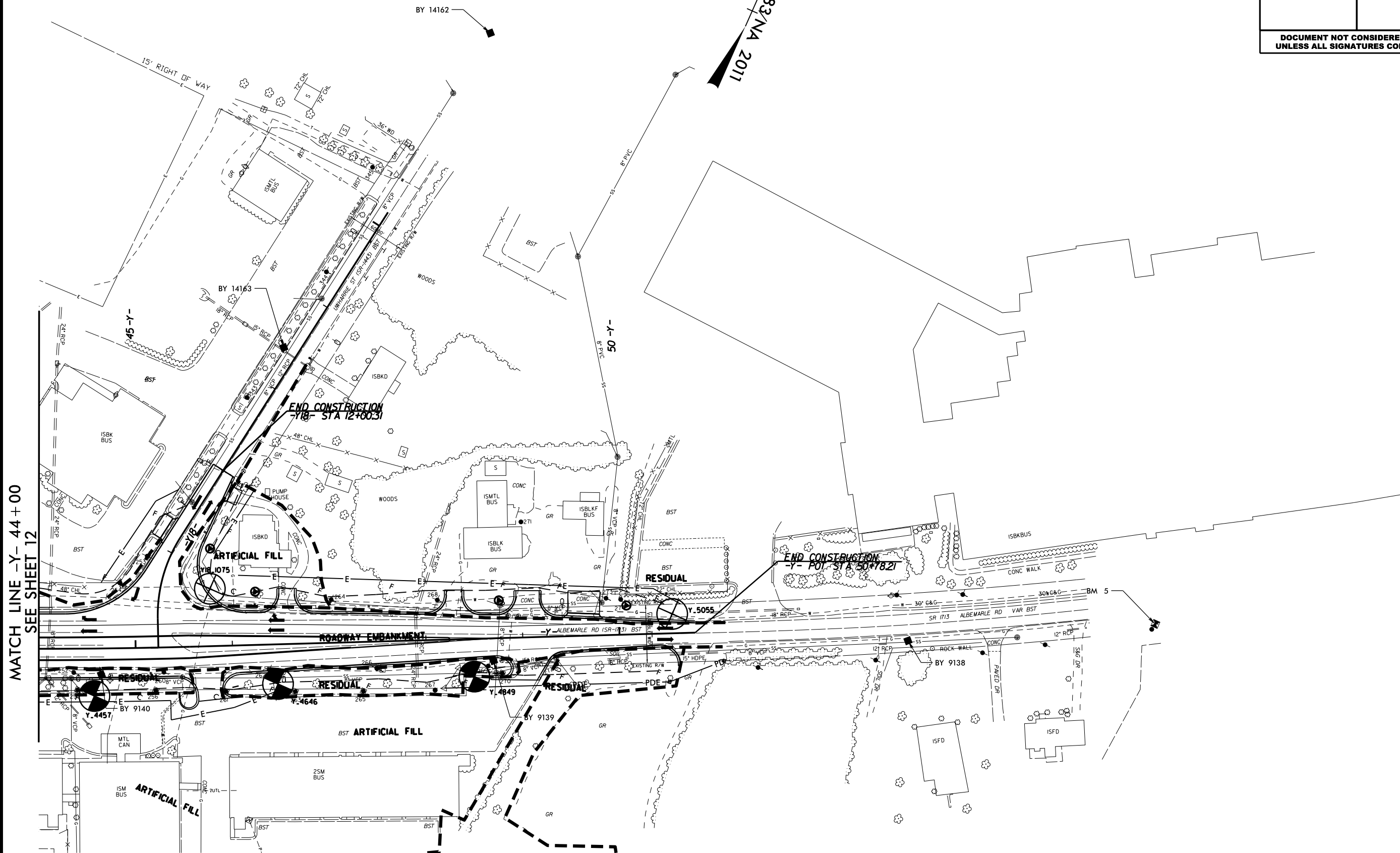


REVISIONS

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Checked: AT K2020545

PROJECT REFERENCE NO.	SHEET NO.
U-5813	15
R/W SHEET NO.	
ROADWAY DESIGN ENGINEER	HYDRAULICS ENGINEER
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DOCUMENT NOT CONSIDERED FINAL UNLESS ALL SIGNATURES COMPLETED	

MAD 83/NA 2011



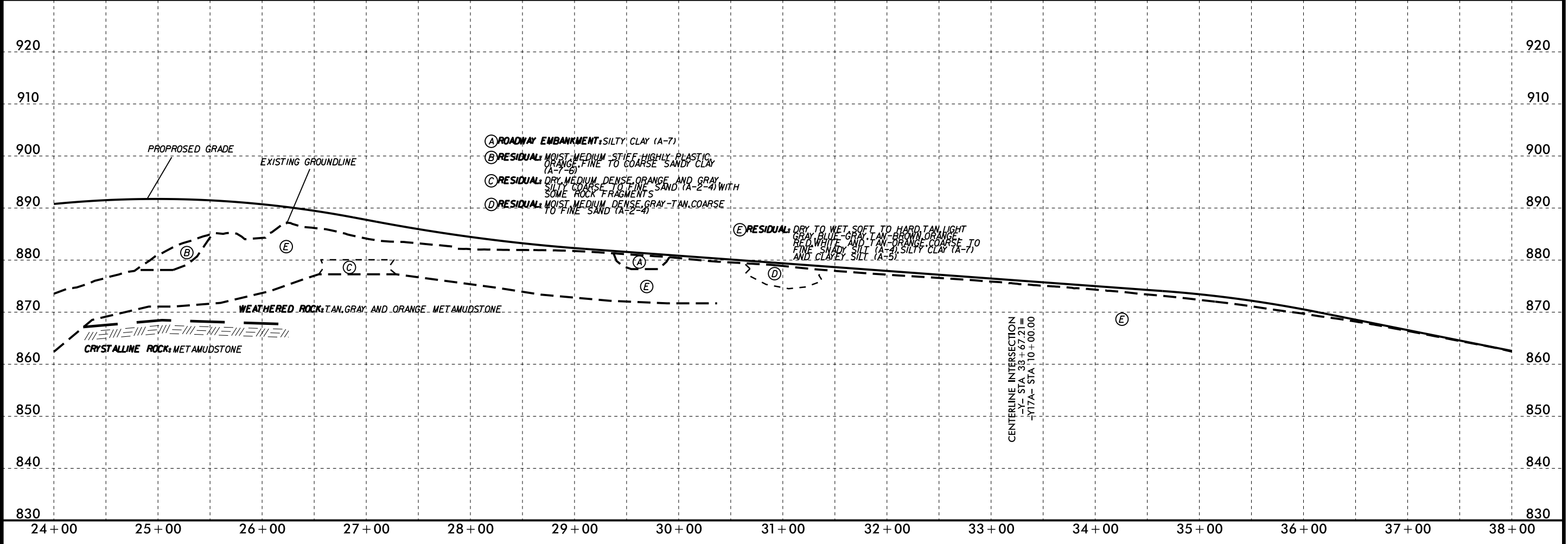
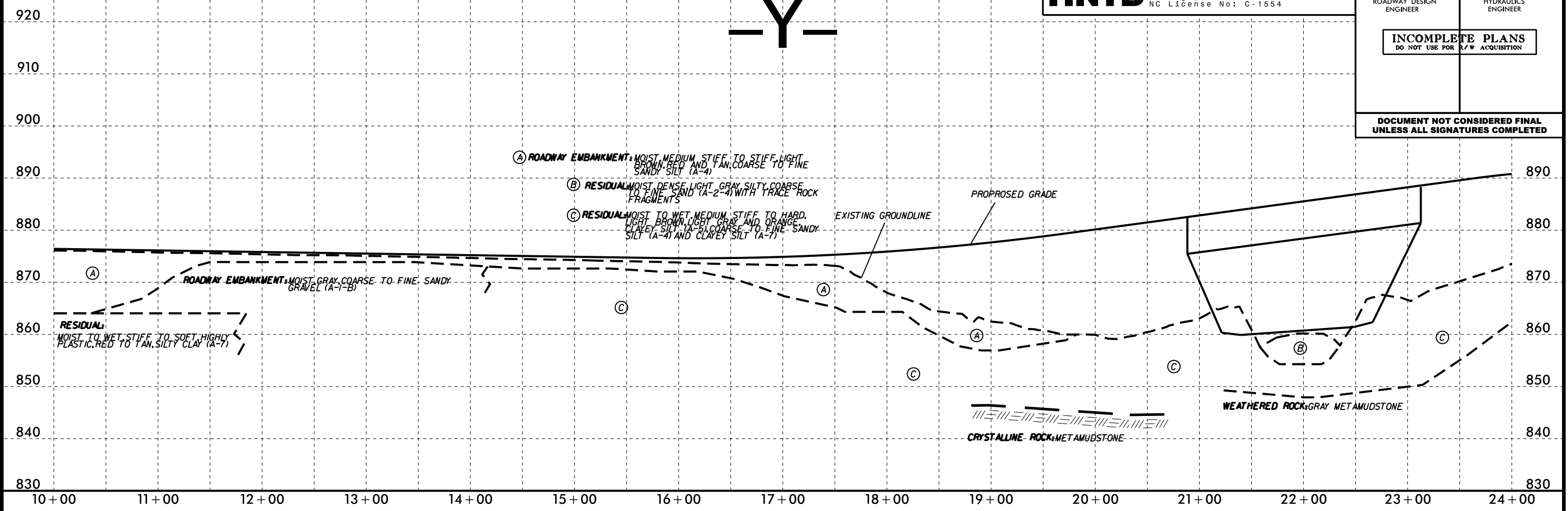
REVISIONS

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5/28/99

PROJECT REFERENCE NO. U-5813	SHEET NO. 16
ROADWAY DESIGN ENGINEER	HYDRAULICS ENGINEER
INCOMPLETE PLANS DO NOT USE FOR R/W ACQUISITION	
DOCUMENT NOT CONSIDERED FINAL UNLESS ALL SIGNATURES COMPLETED	

-Y-

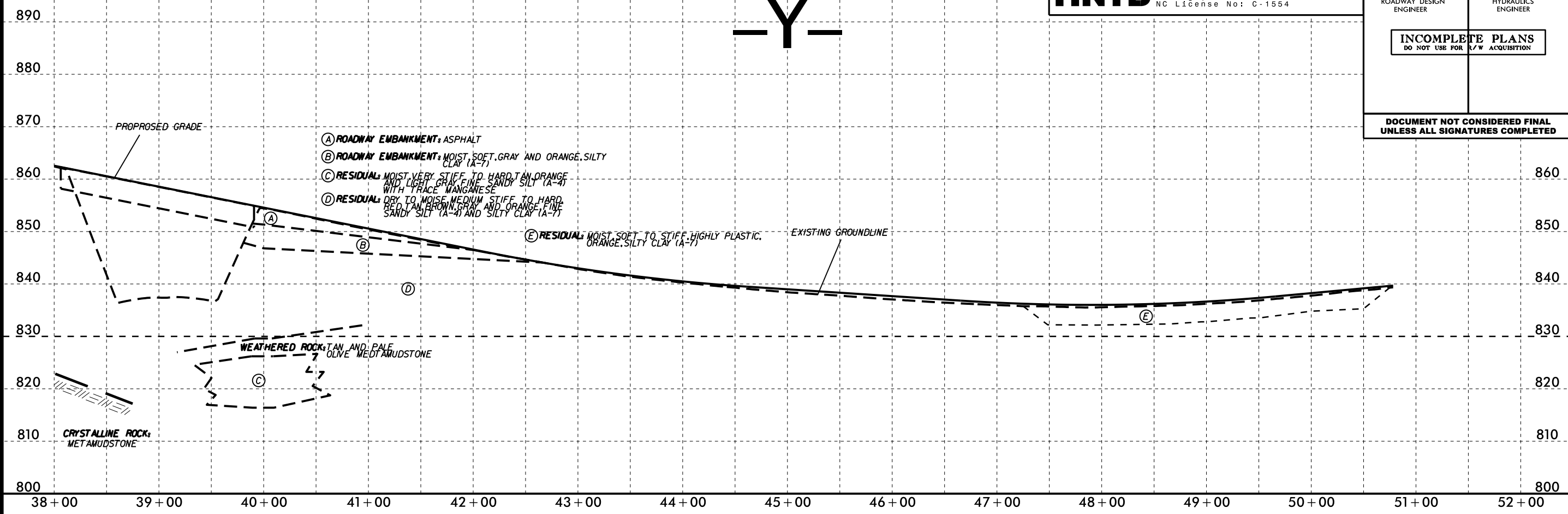


15-OCT-2019 11:29 AM
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 \$\$\$\$STRANDM\$\$\$\$

5/28/99

PROJECT REFERENCE NO. U-5813	SHEET NO. 17
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DOCUMENT NOT CONSIDERED FINAL UNLESS ALL SIGNATURES COMPLETED	

-Y-



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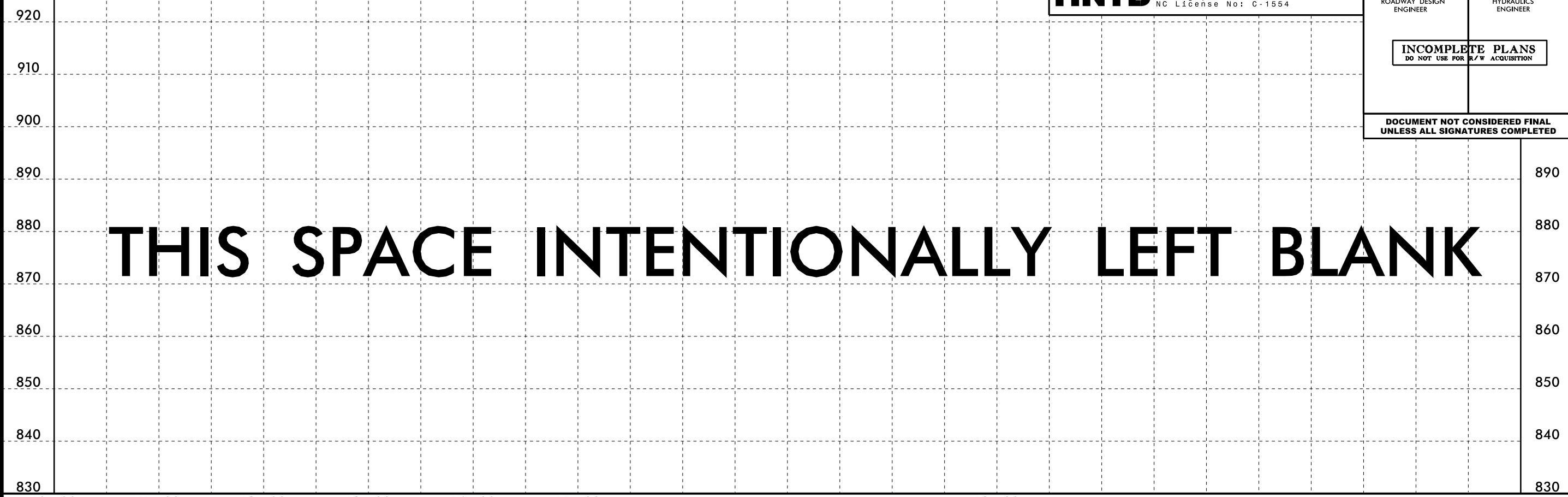
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5/28/99

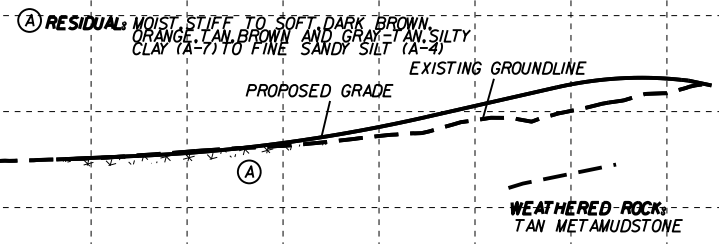
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PROJECT REFERENCE NO.	SHEET NO.
U-5813	18
ROADWAY DESIGN ENGINEER	HYDRAULICS ENGINEER
INCOMPLETE PLANS DO NOT USE FOR R/W ACQUISITION	
DOCUMENT NOT CONSIDERED FINAL UNLESS ALL SIGNATURES COMPLETED	

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-Y17A-



920	890	890	915	915
910	880	880	905	905
900	870	870	895	895
890	860	860	885	885
880	850	850	875	875
870	840	840	865	865
860	830	830	855	855
850	820	820	845	845
840	810	810	835	835
830	800	800	825	825

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10+00 11+00 12+00 13+00 14+00 15+00 16+00 10+00 11+00 12+00 10+00 11+00 12+00 13+00

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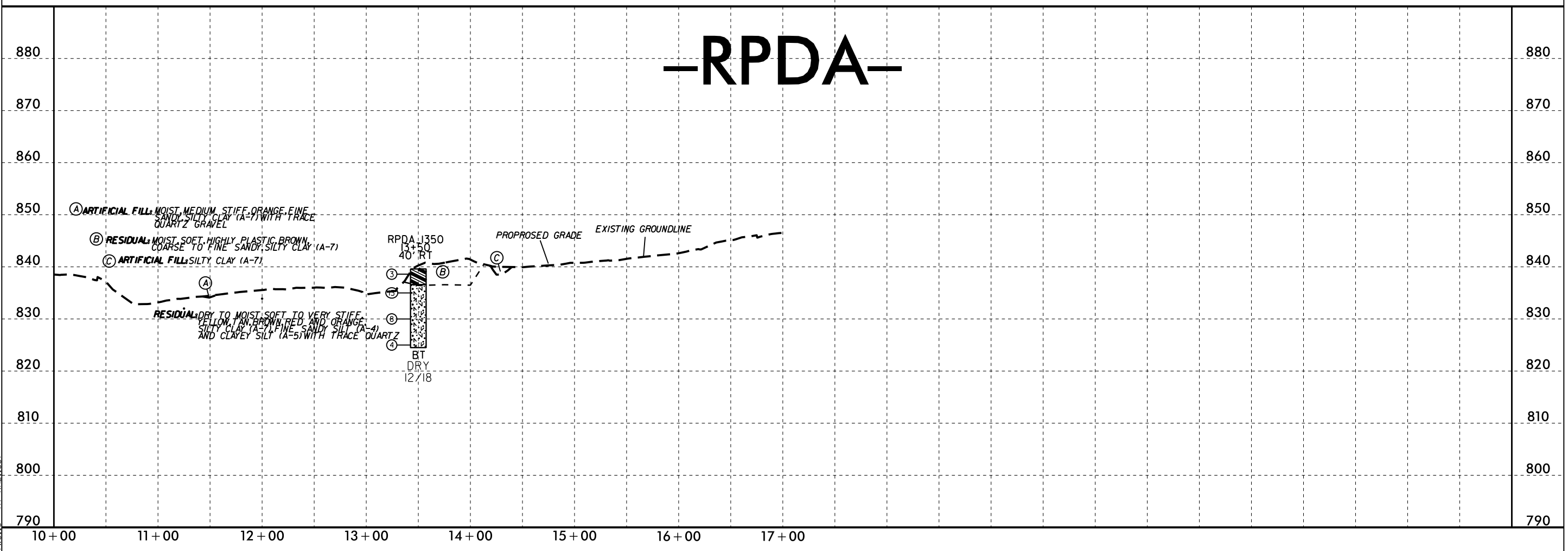
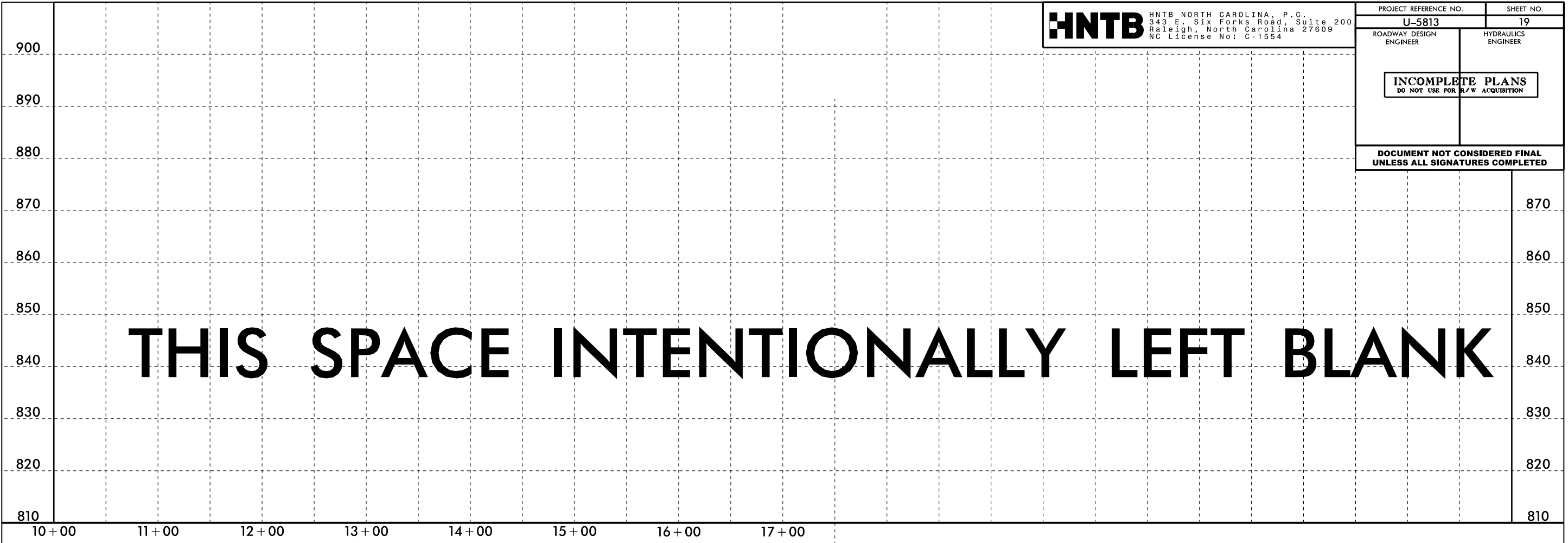
5/28/99

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PROJECT REFERENCE NO.	SHEET NO.
U-5813	19
ROADWAY DESIGN ENGINEER	HYDRAULICS ENGINEER
INCOMPLETE PLANS DO NOT USE FOR R/W ACQUISITION	
DOCUMENT NOT CONSIDERED FINAL UNLESS ALL SIGNATURES COMPLETED	

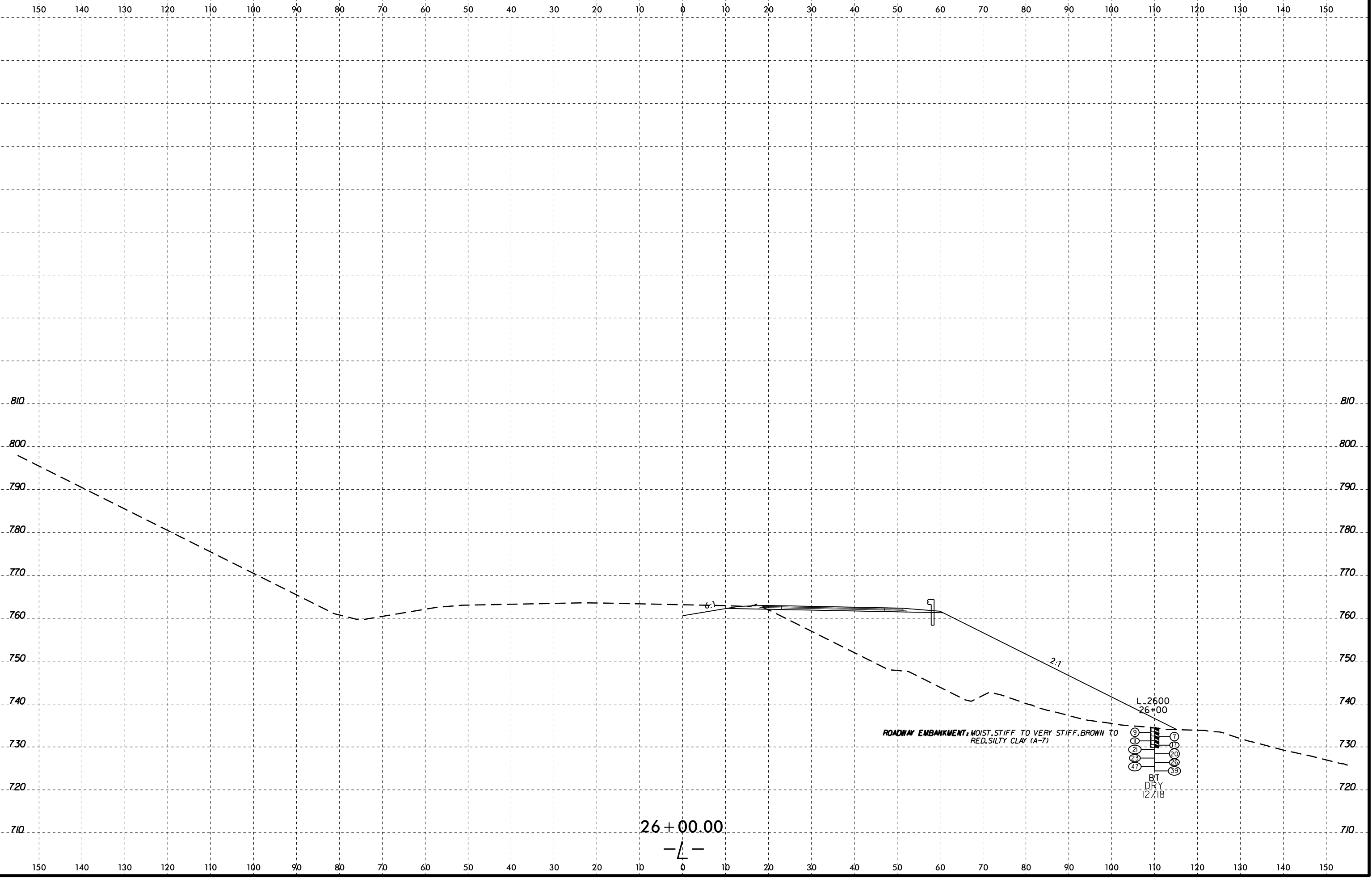
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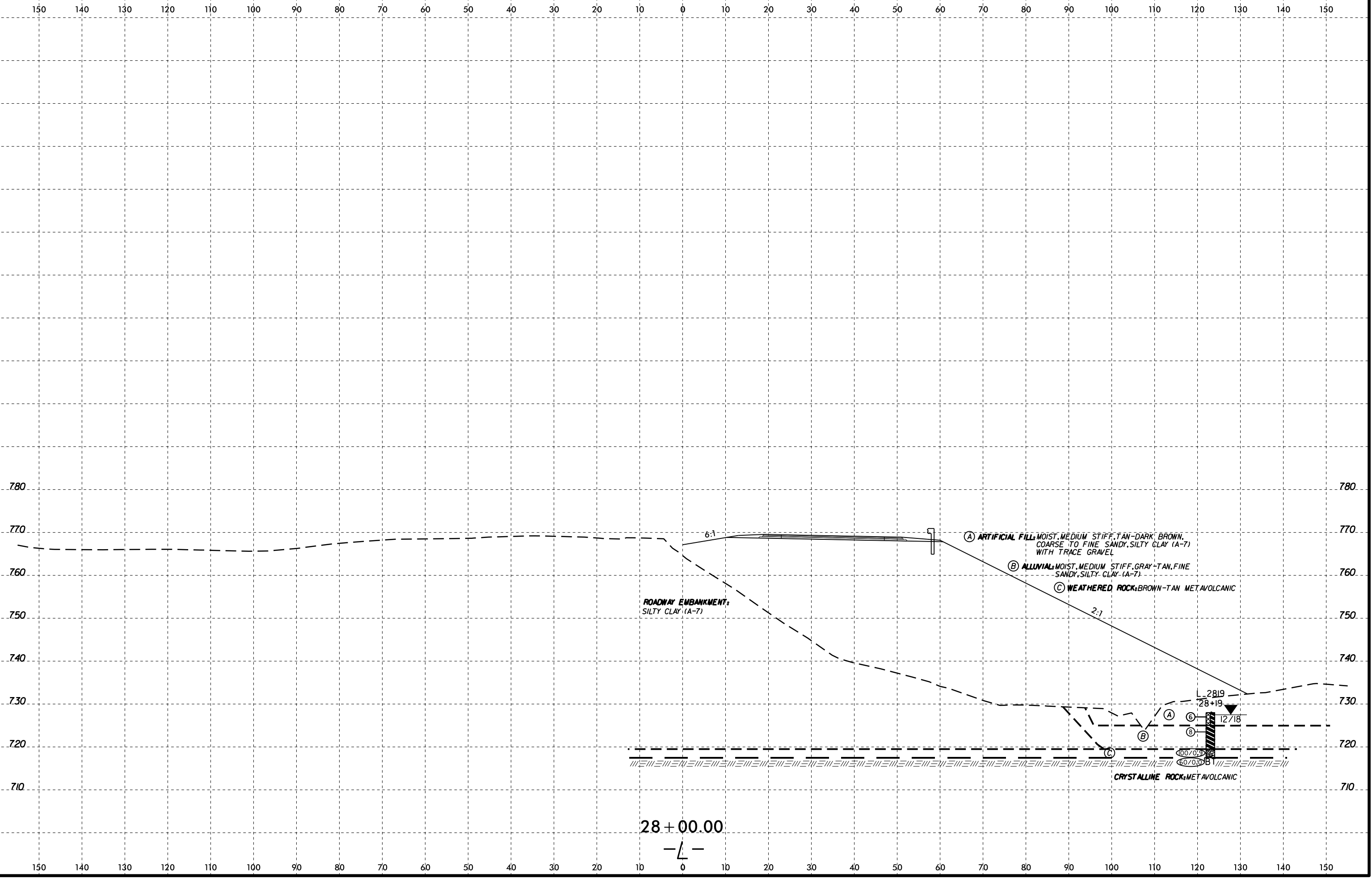
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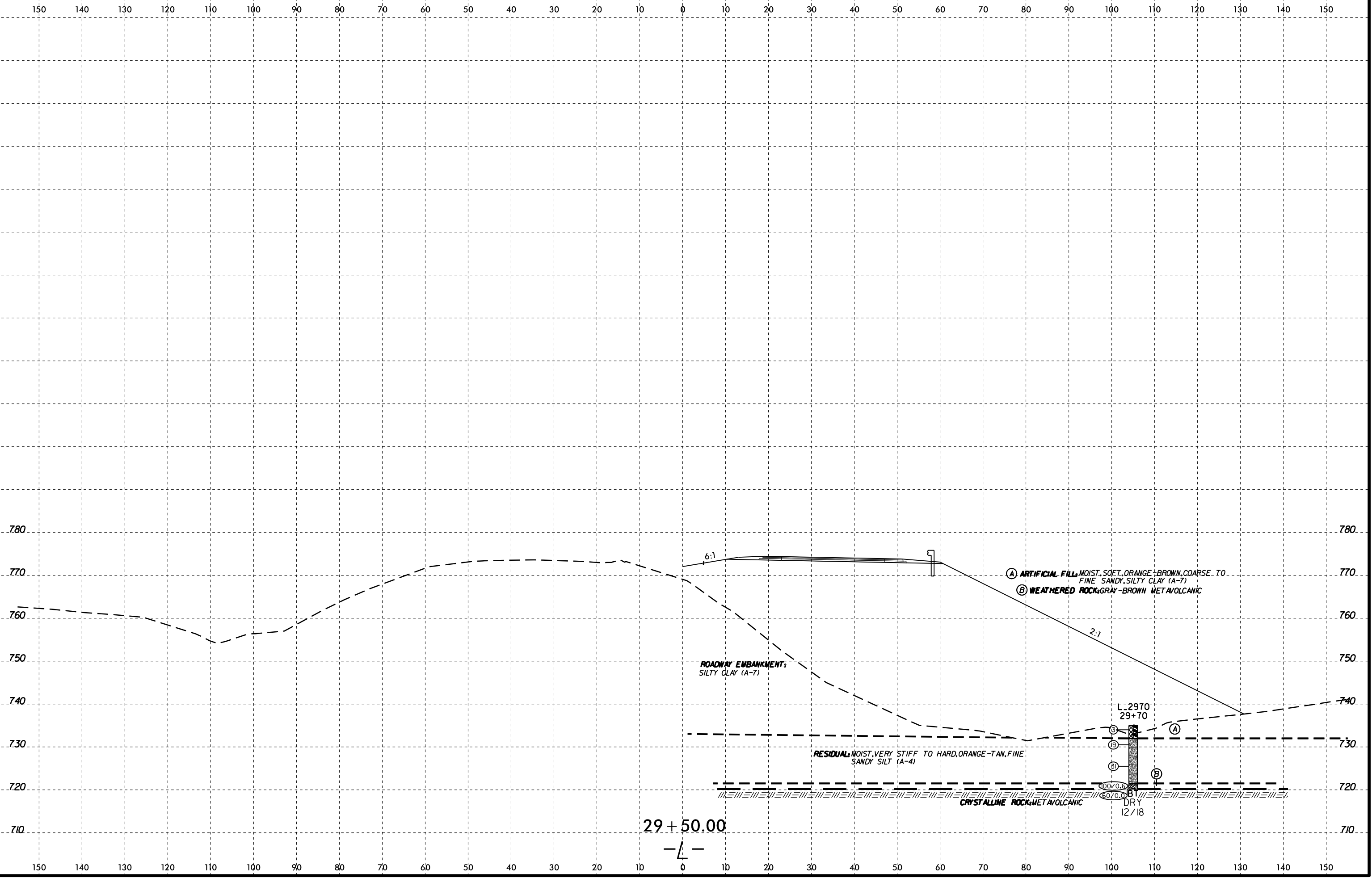
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6/23/16



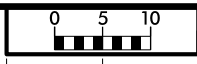


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14-OCT-2019 17:39
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6/23/16
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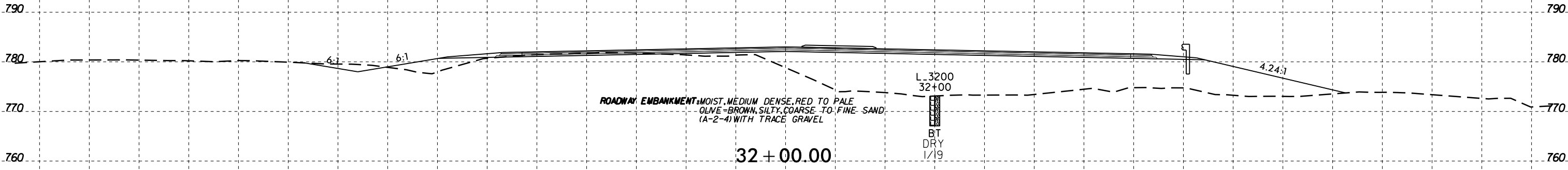


6/23/16



PROJ. REFERENCE NO.	SHEET NO.
U-5813	23

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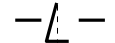


ROADWAY EMBANKMENT, MOIST, MEDIUM DENSE, RED TO PALE
OLIVE-BROWN, SILTY, COARSE-TO-FINE SAND
(A-2-4) WITH TRACE GRAVEL

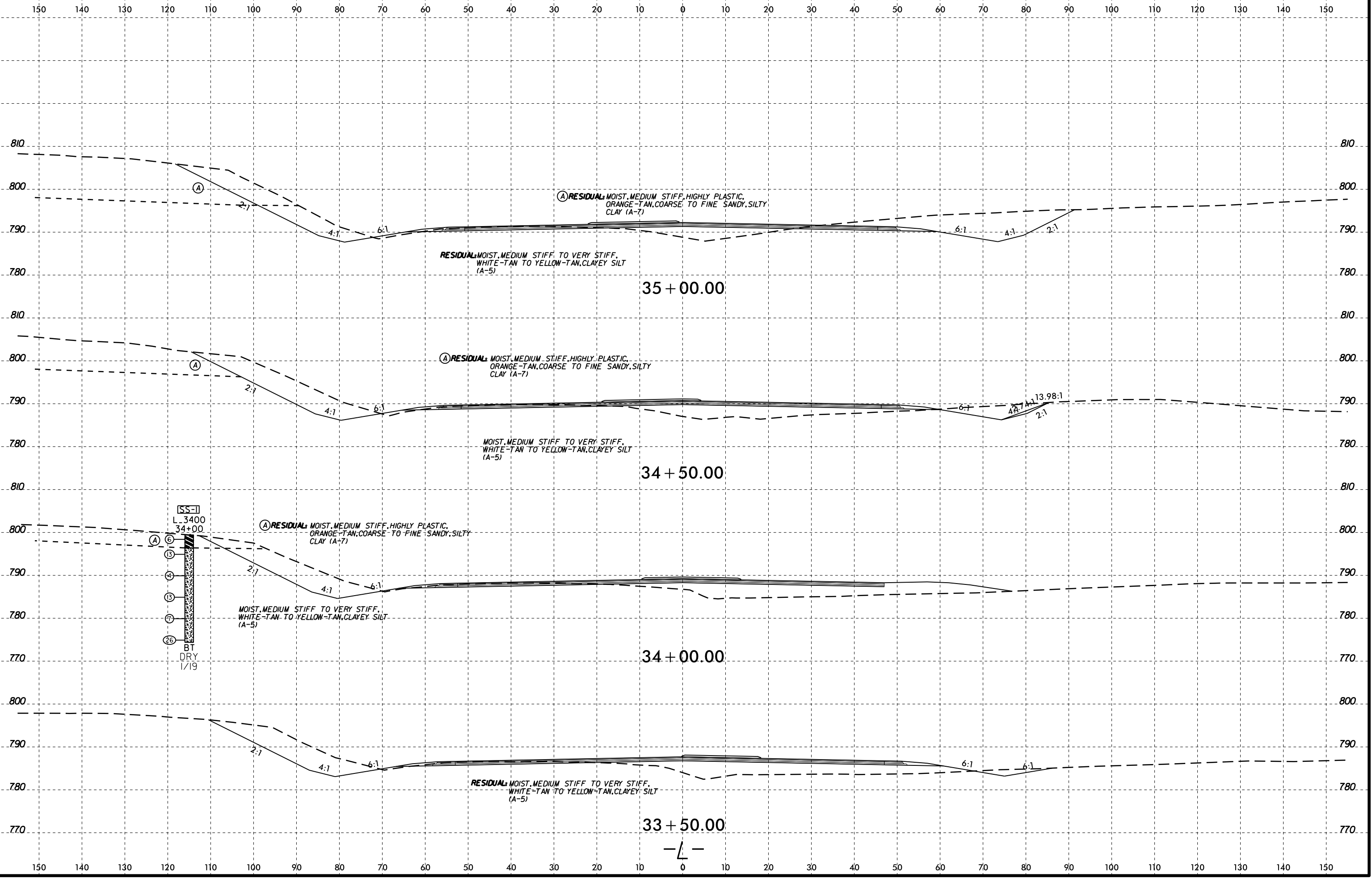
L 3200
32+00

BT
DRY
1/19

32 + 00.00

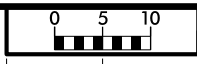


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SPopke AT K&20516



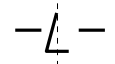
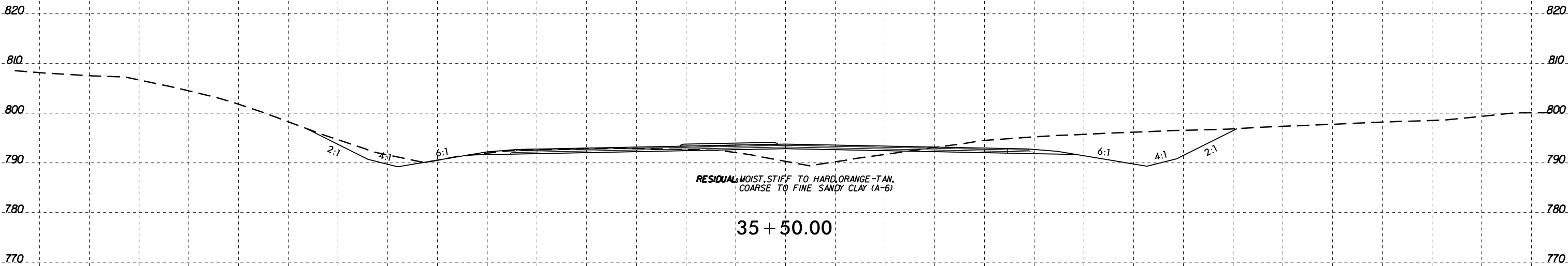
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 Spoke AT K620516

6/23/16



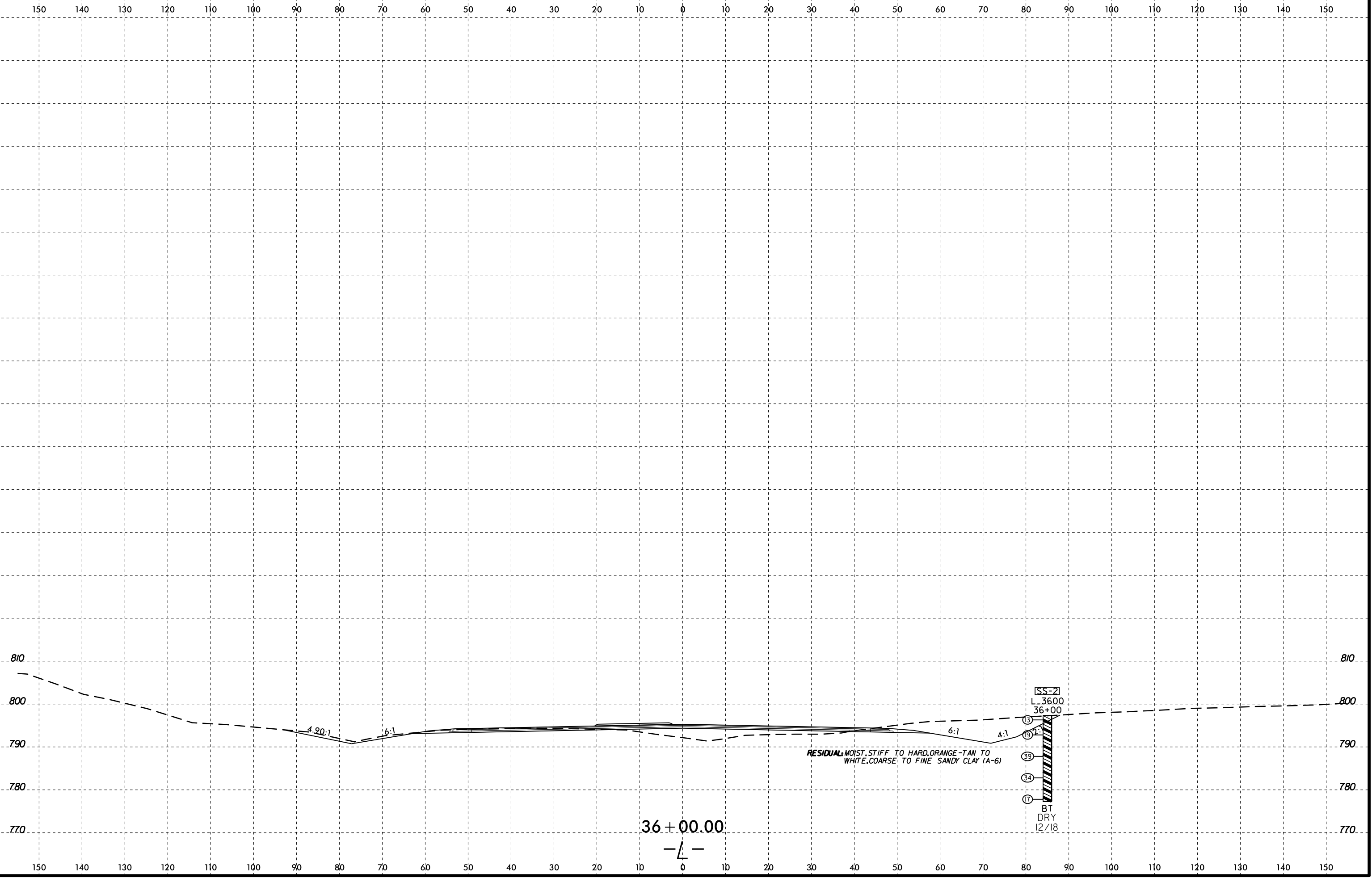
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U-5813	25

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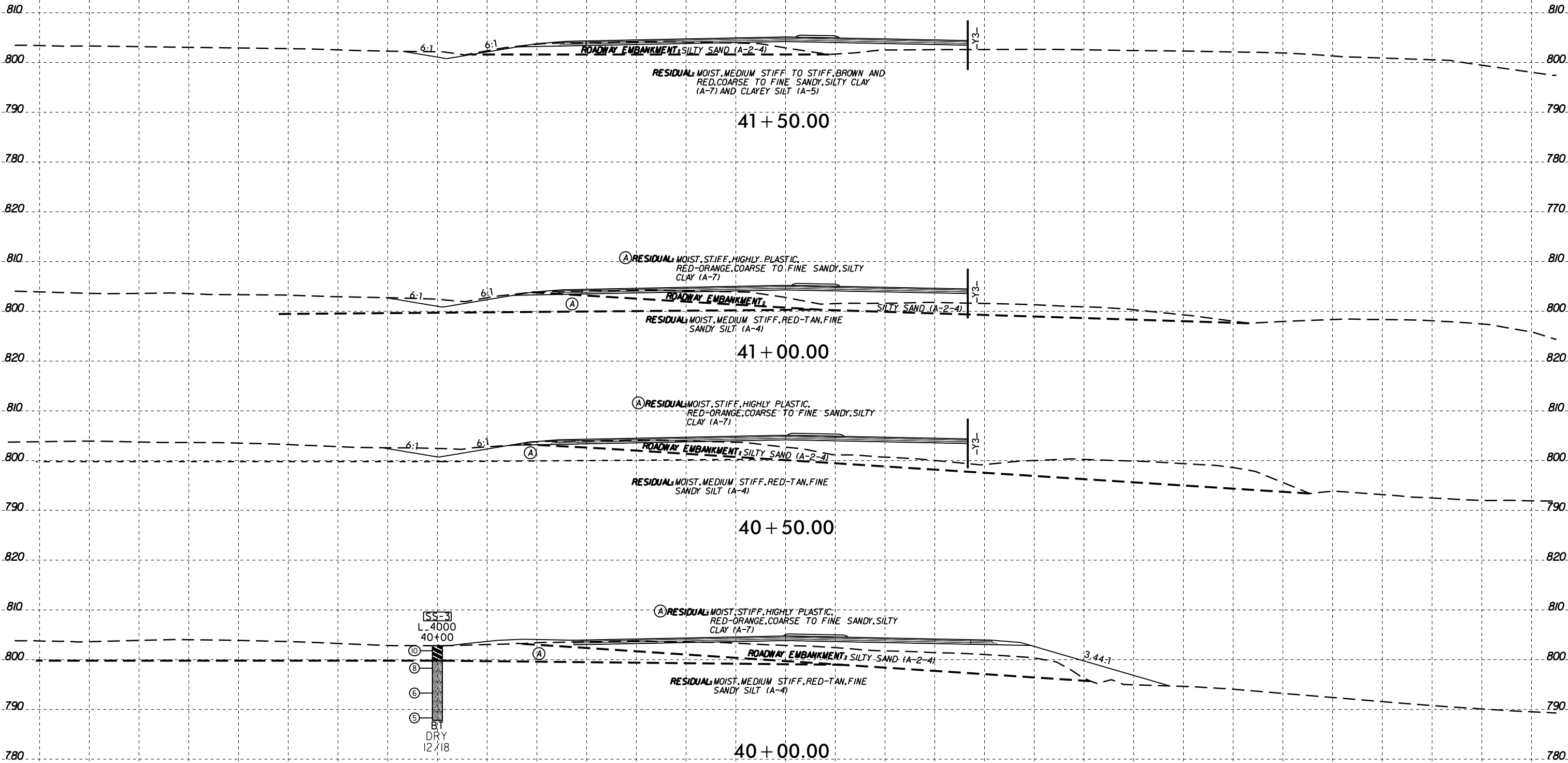


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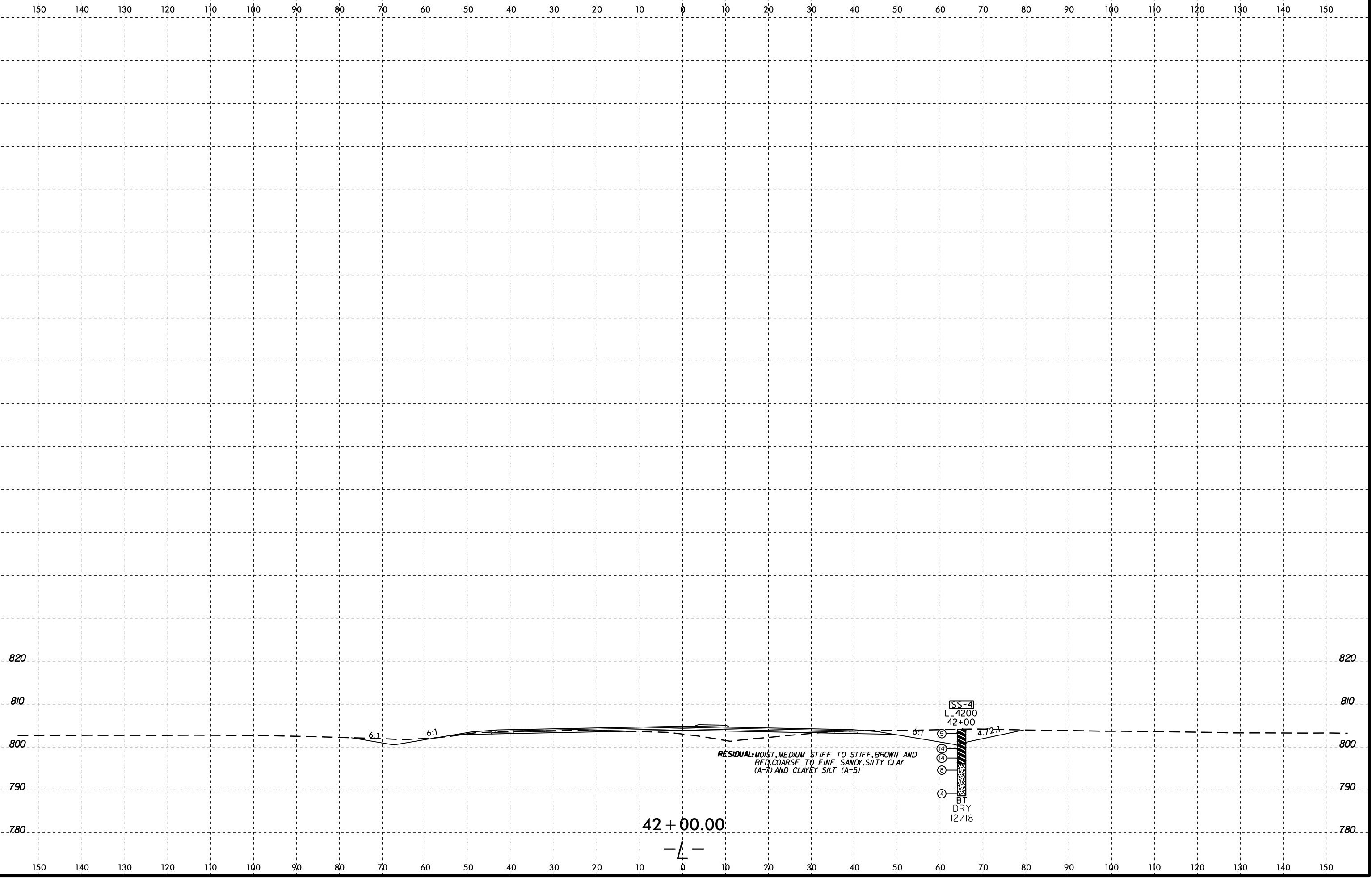
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6/23/16

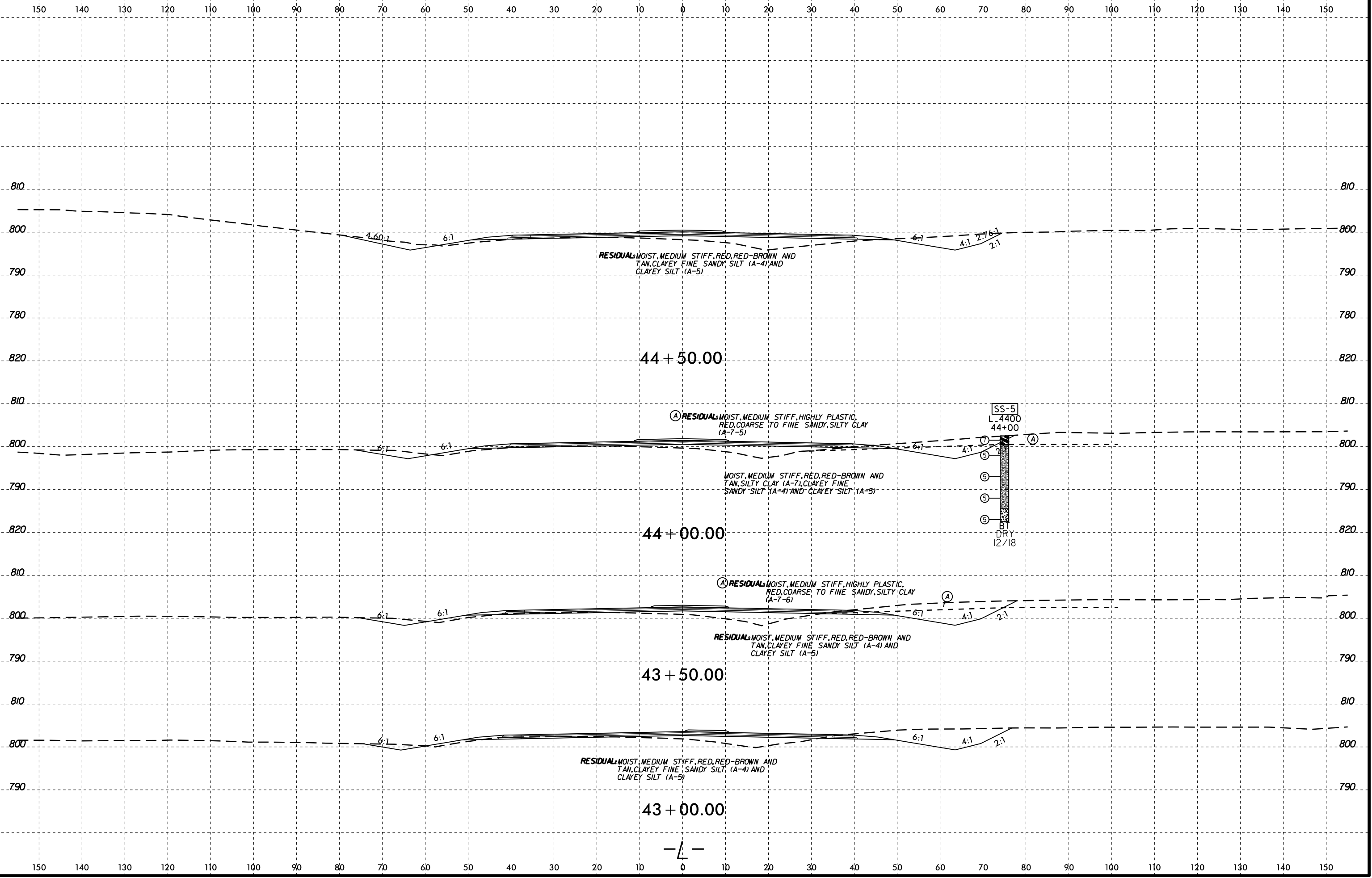


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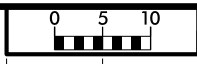
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SPopke AT K&B



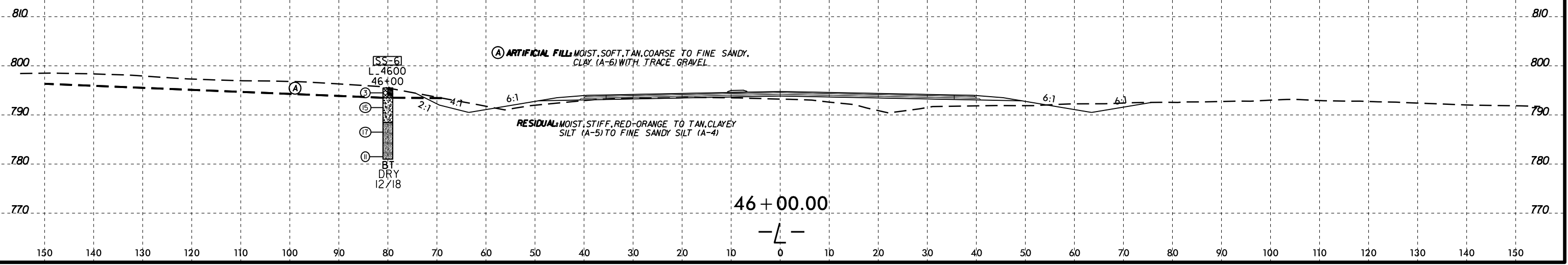


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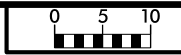
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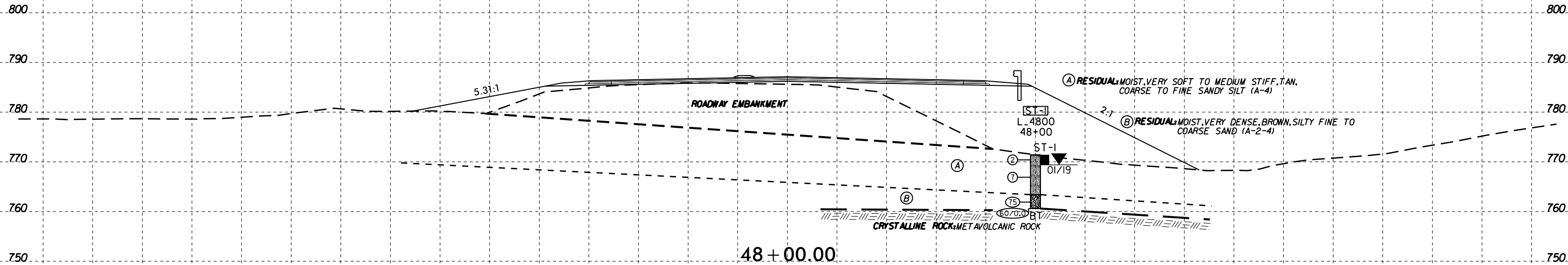
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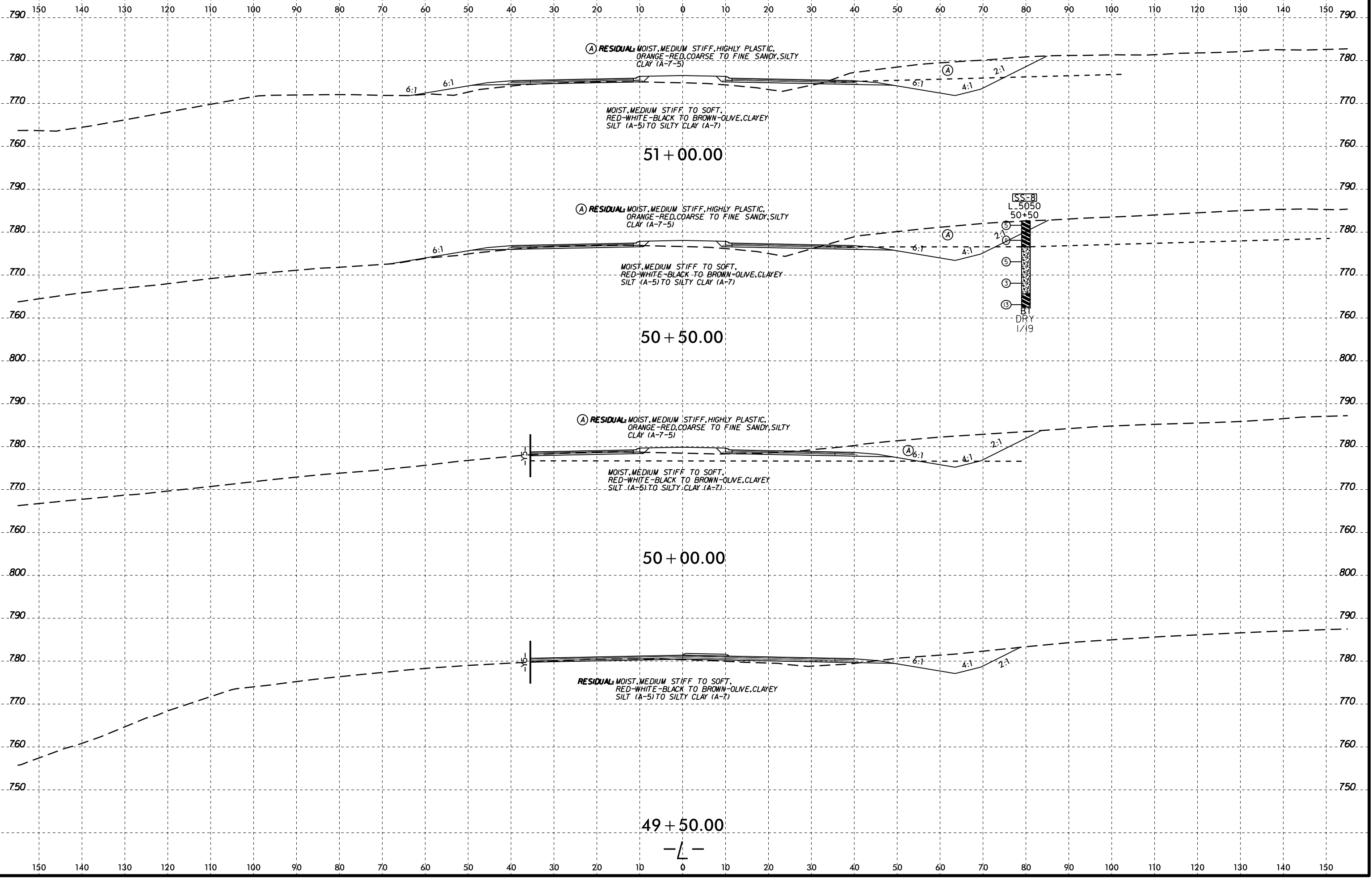
PROJ. REFERENCE NO.
U-5813

SHEET NO.
31

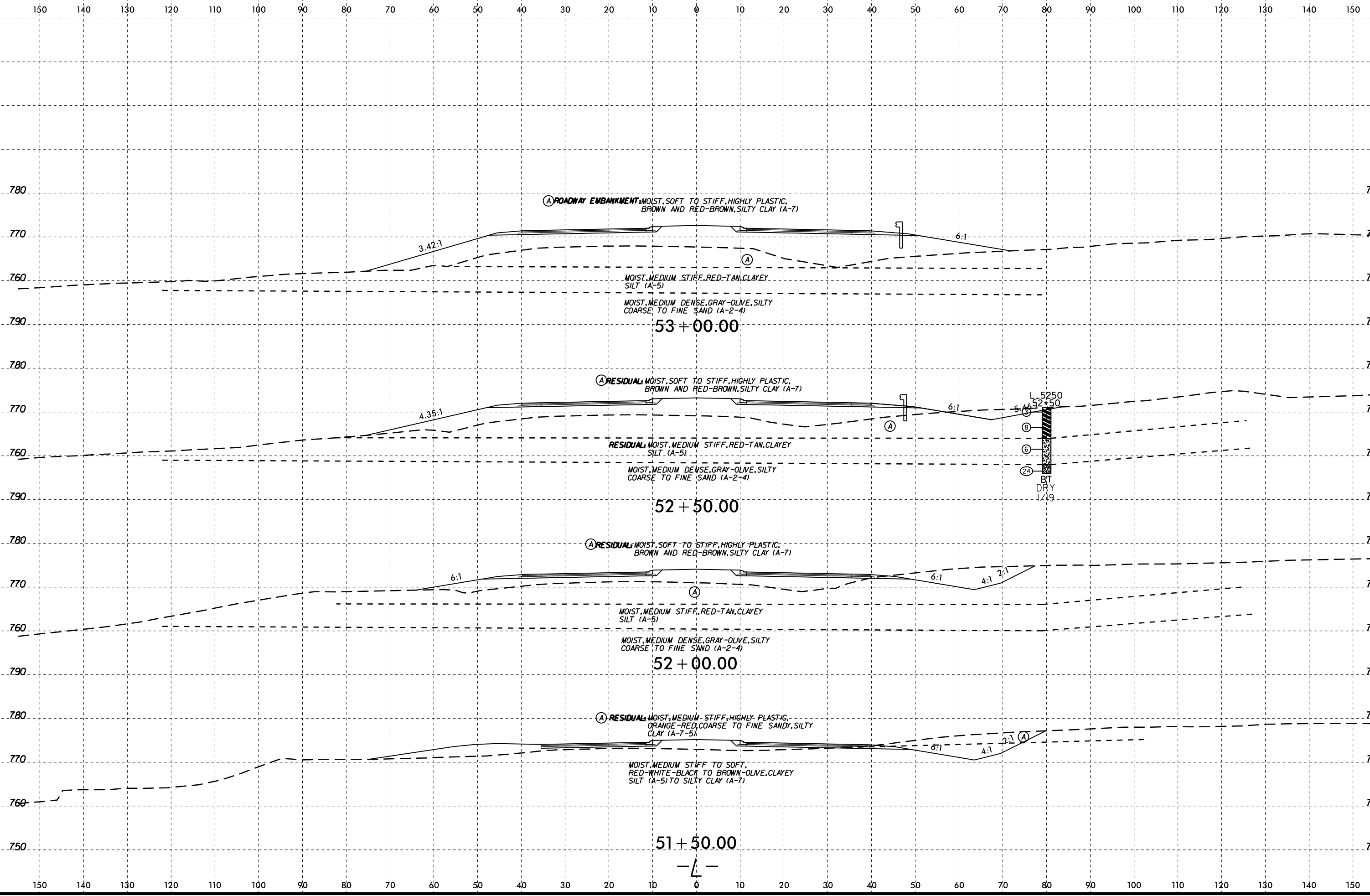
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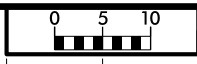
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SPopke AT K209546



15-OCT-2019 12:46
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 Spoke AT K208516



6/23/16

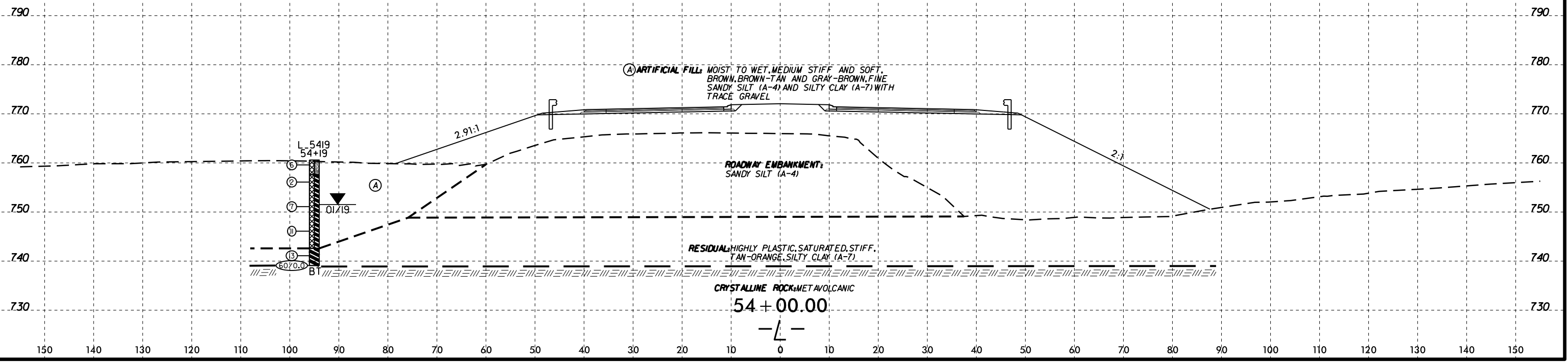


PROJ. REFERENCE NO.
U-5813

SHEET NO.
34

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SPopke AT K620516

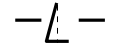


(A) ARTIFICIAL FILL: MOIST TO WET, MEDIUM STIFF AND SOFT BROWN, BROWN-TAN AND GRAY-BROWN, FINE SANDY SILT (A-4) AND SILTY CLAY (A-7) WITH TRACE GRAVEL

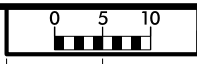
ROADWAY EMBANKMENT SANDY SILT (A-4)

RESIDUAL: HIGHLY PLASTIC, SATURATED, STIFF, TAN-ORANGE SILTY CLAY (A-7)

CRYSTALLINE ROCK METAVOLCANIC
54+00.00



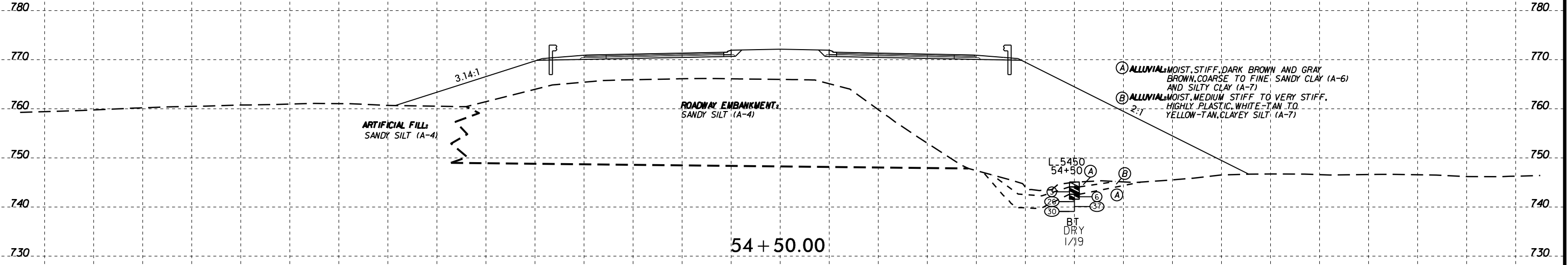
6/23/16



PROJ. REFERENCE NO.
U-5813

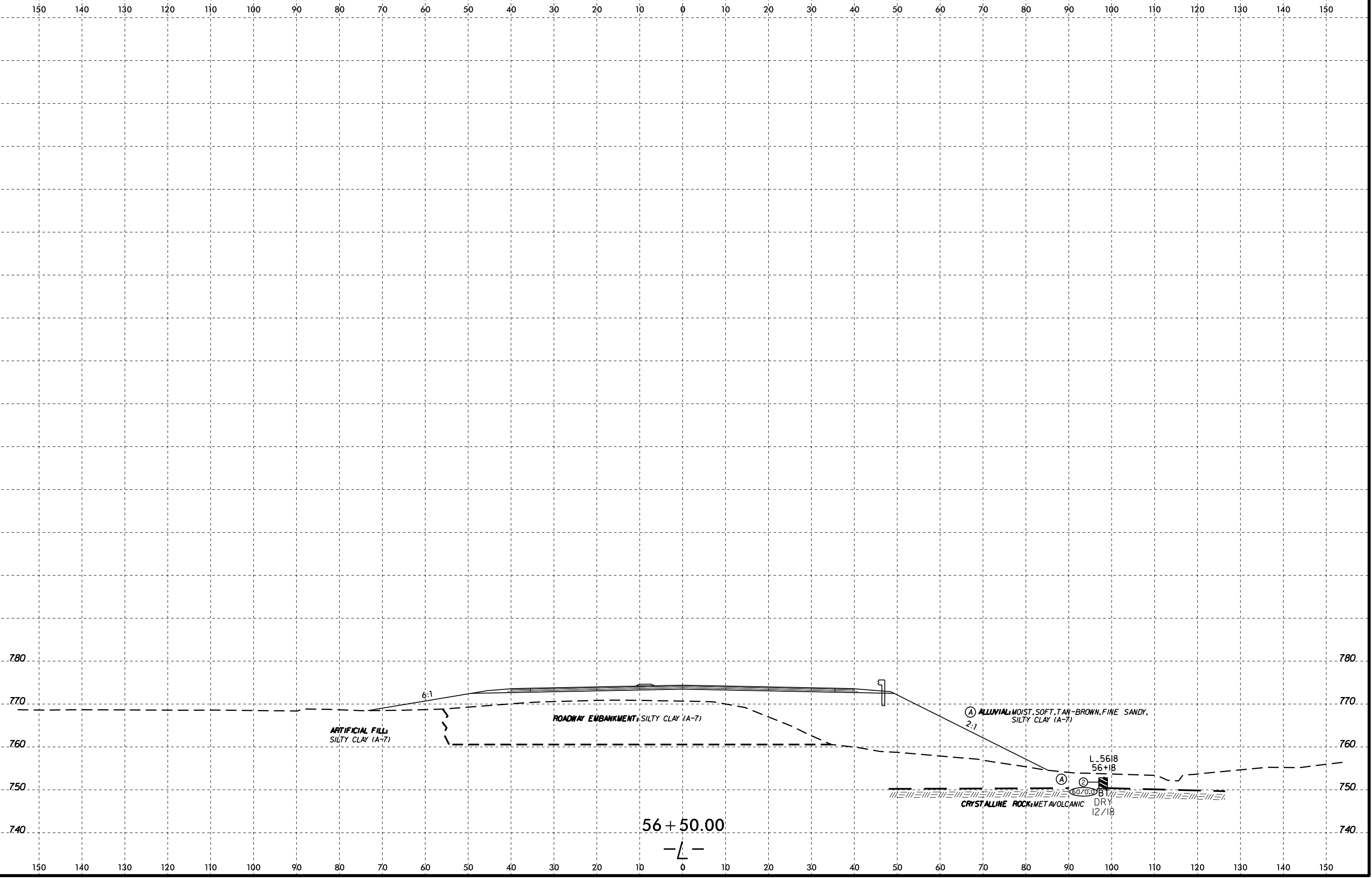
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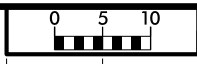


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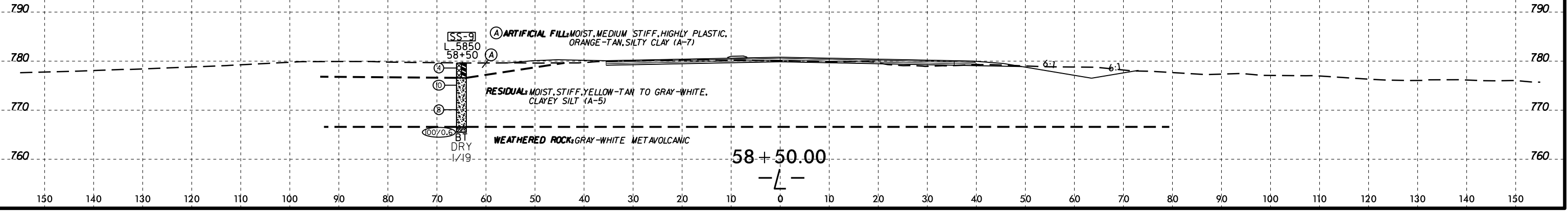
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PROJ. REFERENCE NO.
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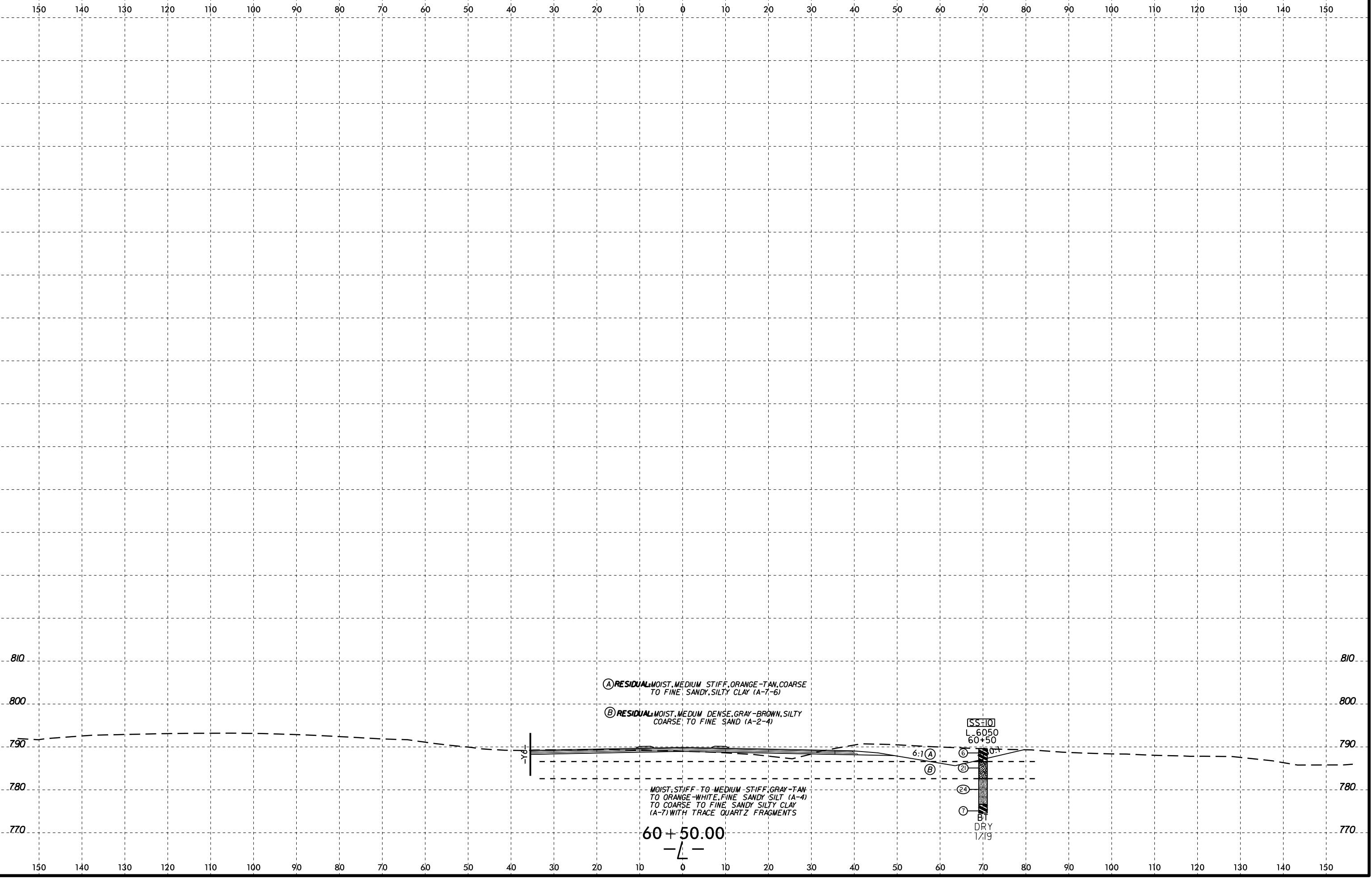
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Spoke AT K6209516

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14-OCT-2019 17:42
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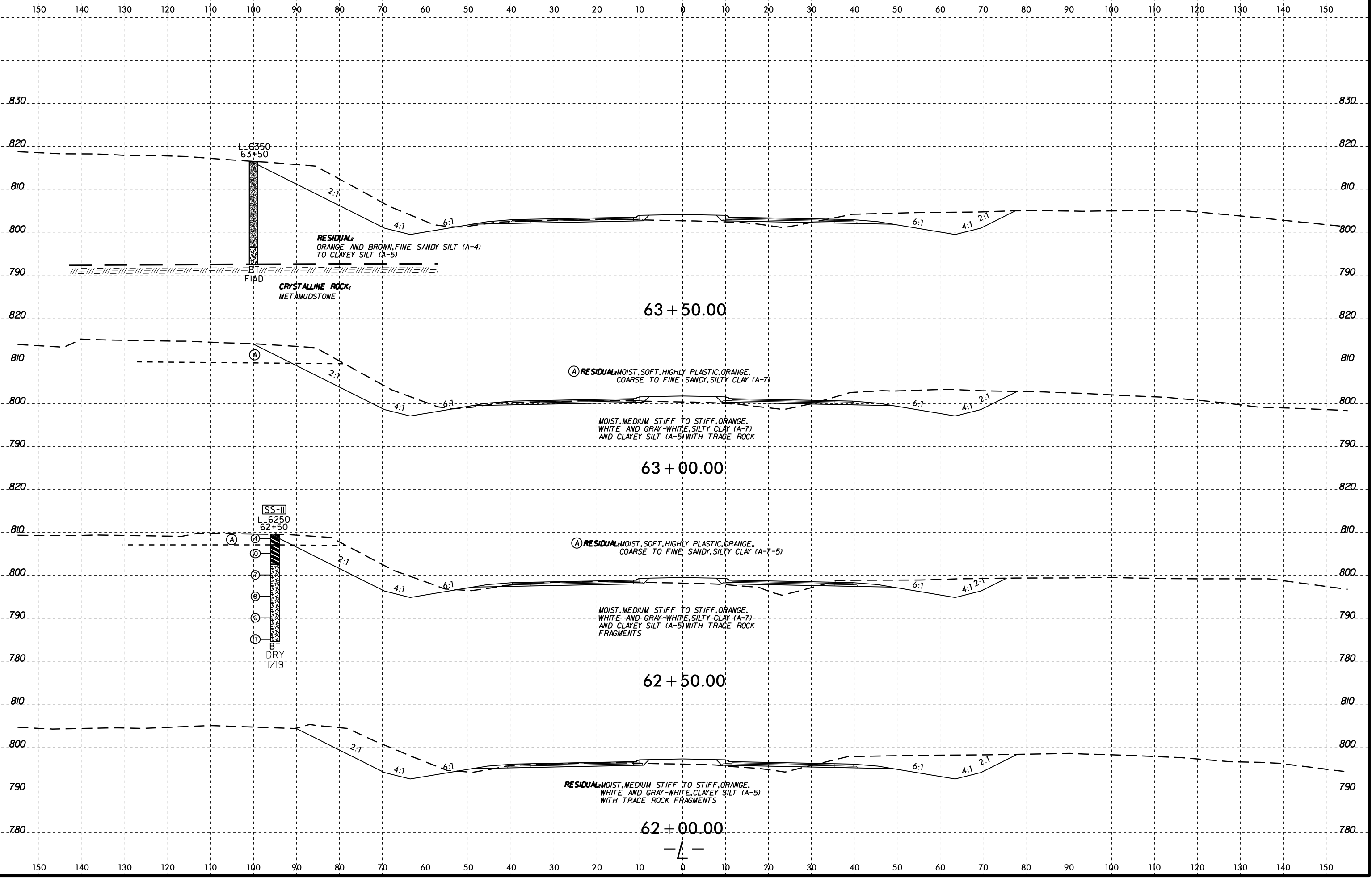


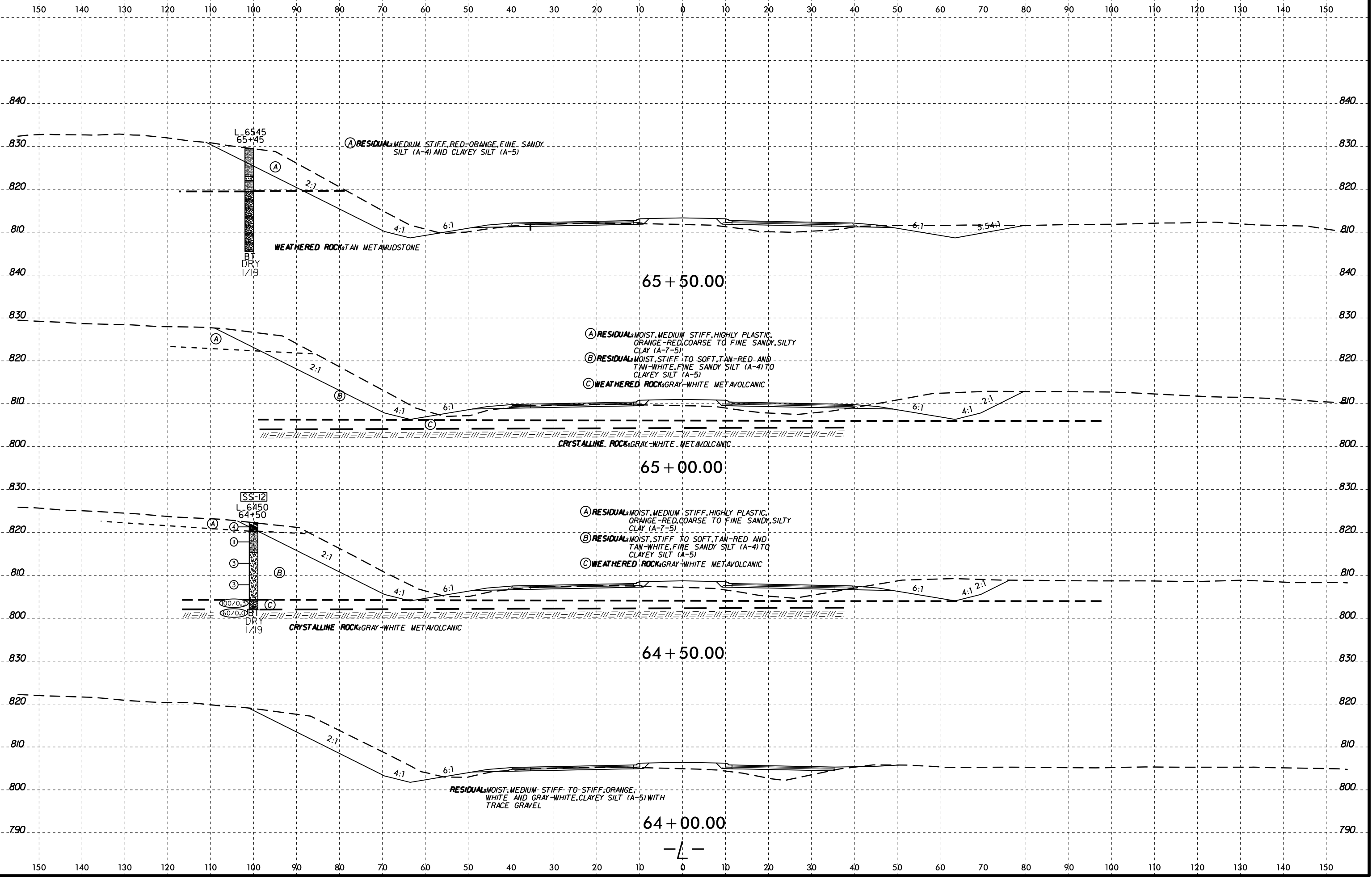
(A) RESIDUAL MOIST, MEDIUM STIFF, ORANGE-TAN, COARSE TO FINE SANDY, SILTY CLAY (A-7-6)
(B) RESIDUAL MOIST, MEDIUM DENSE, GRAY-BROWN, SILTY COARSE TO FINE SAND (A-2-4)

MOIST, STIFF TO MEDIUM STIFF, GRAY-TAN TO ORANGE-WHITE, FINE SANDY SILT (A-4) TO COARSE TO FINE SANDY SILTY CLAY (A-7) WITH TRACE QUARTZ FRAGMENTS

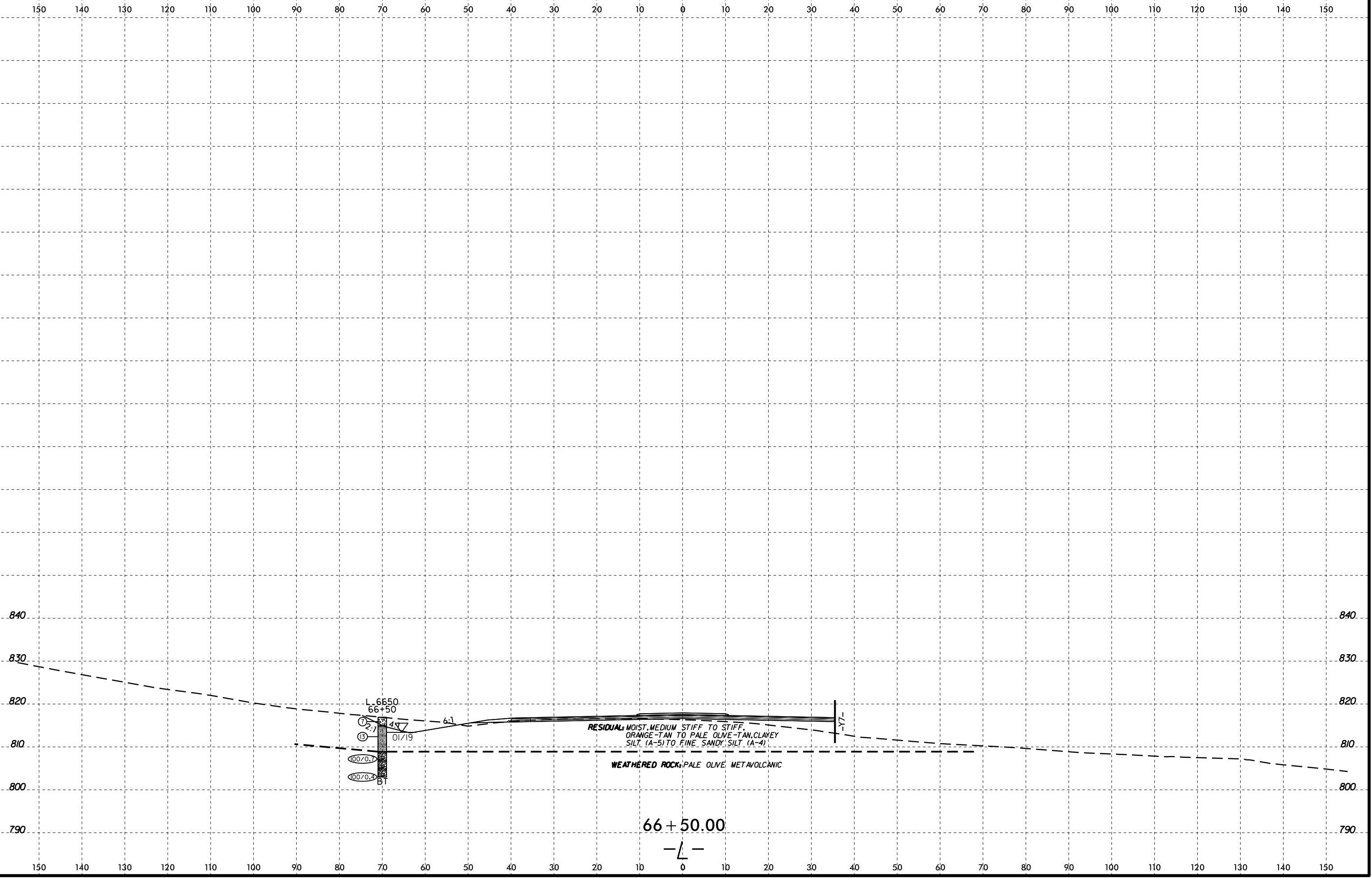
60+50.00
—L—

SS=10
L 6050
60+50
6:1 (A)
(B)
B1
DRY
1/19





6/23/16
14-OCT-2019 17:43
W:\share\GEO\TECHNICAL\Projects\Active Projects\20190942.009A U-5813 Roadway\U5813.GEO.ROWY\CADD.GEOTECH\sec\U5813.GEO.L.XSI.dgn
S:\pape AT K&209516

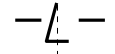


L 6650
66+50
① 2.7
② 01/19
③ 100/0.3
④ 100/0.3
BT

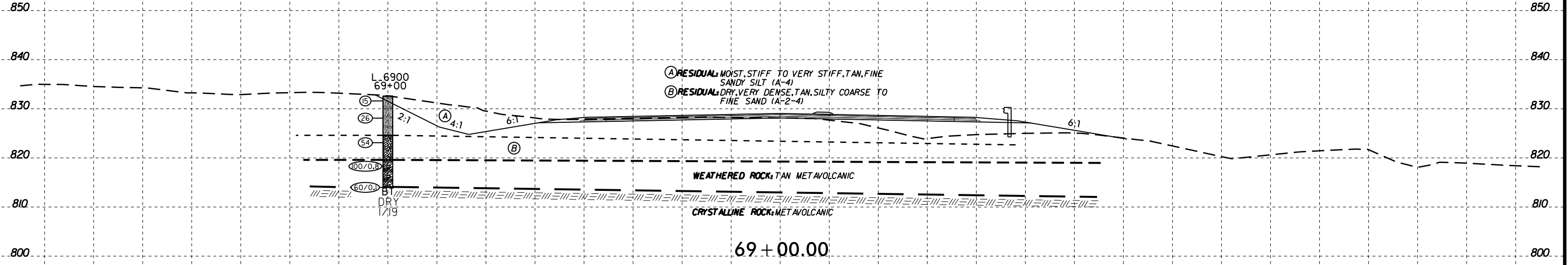
RESIDUAL MOIST. MEDIUM STIFF TO STIFF,
ORANGE-TAN TO PALE OLIVE-TAN, CLAYEY
SILT (A-5) TO FINE SANDY SILT (A-4)

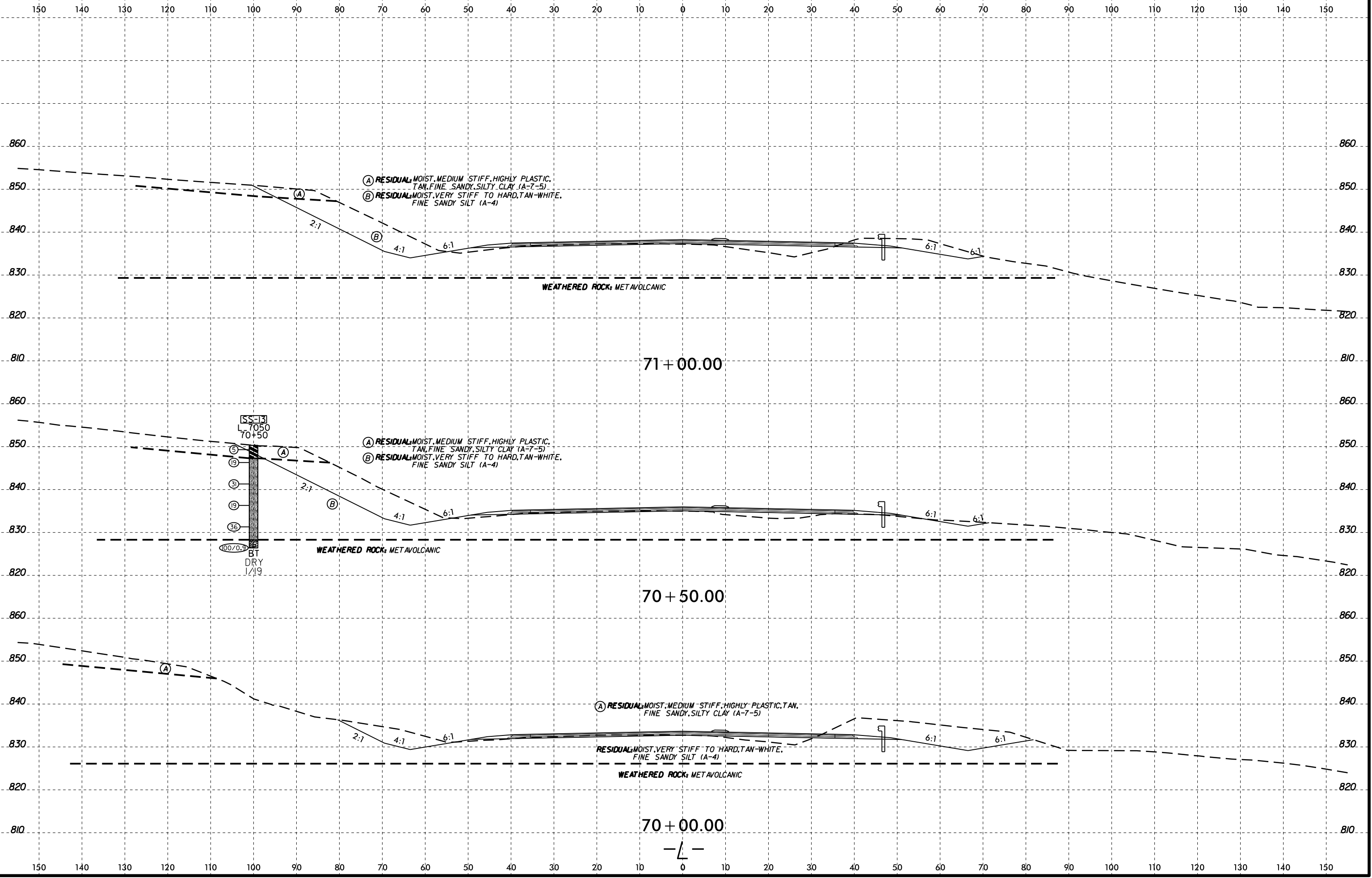
WEATHERED ROCK, PALE OLIVE METAVOLCANIC

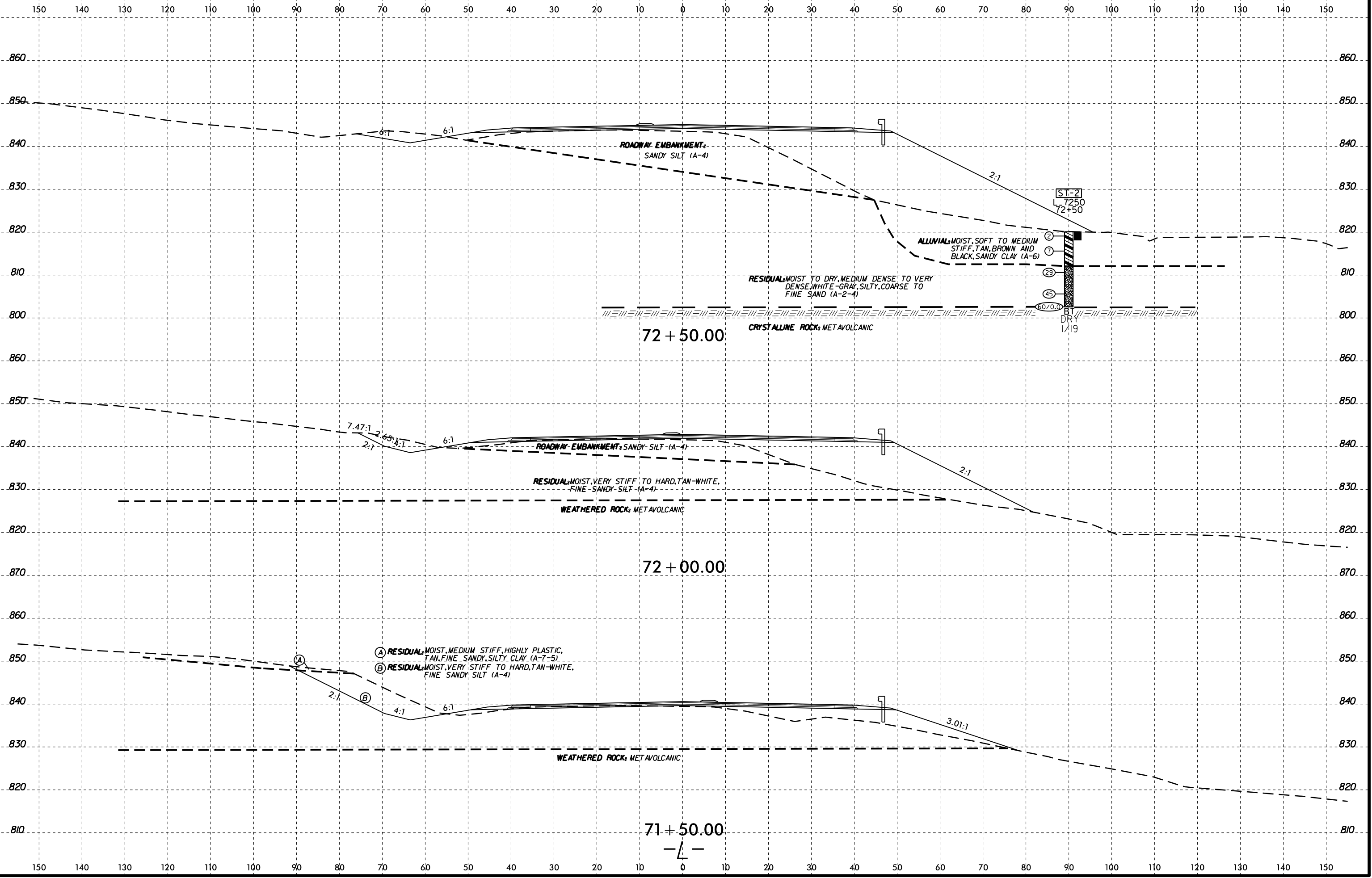
66 + 50.00



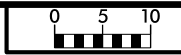
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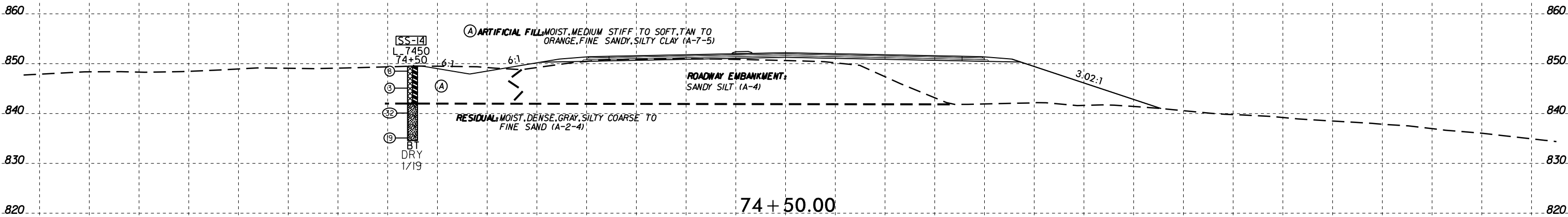
6/23/16



PROJ. REFERENCE NO.
U-5813

SHEET NO.
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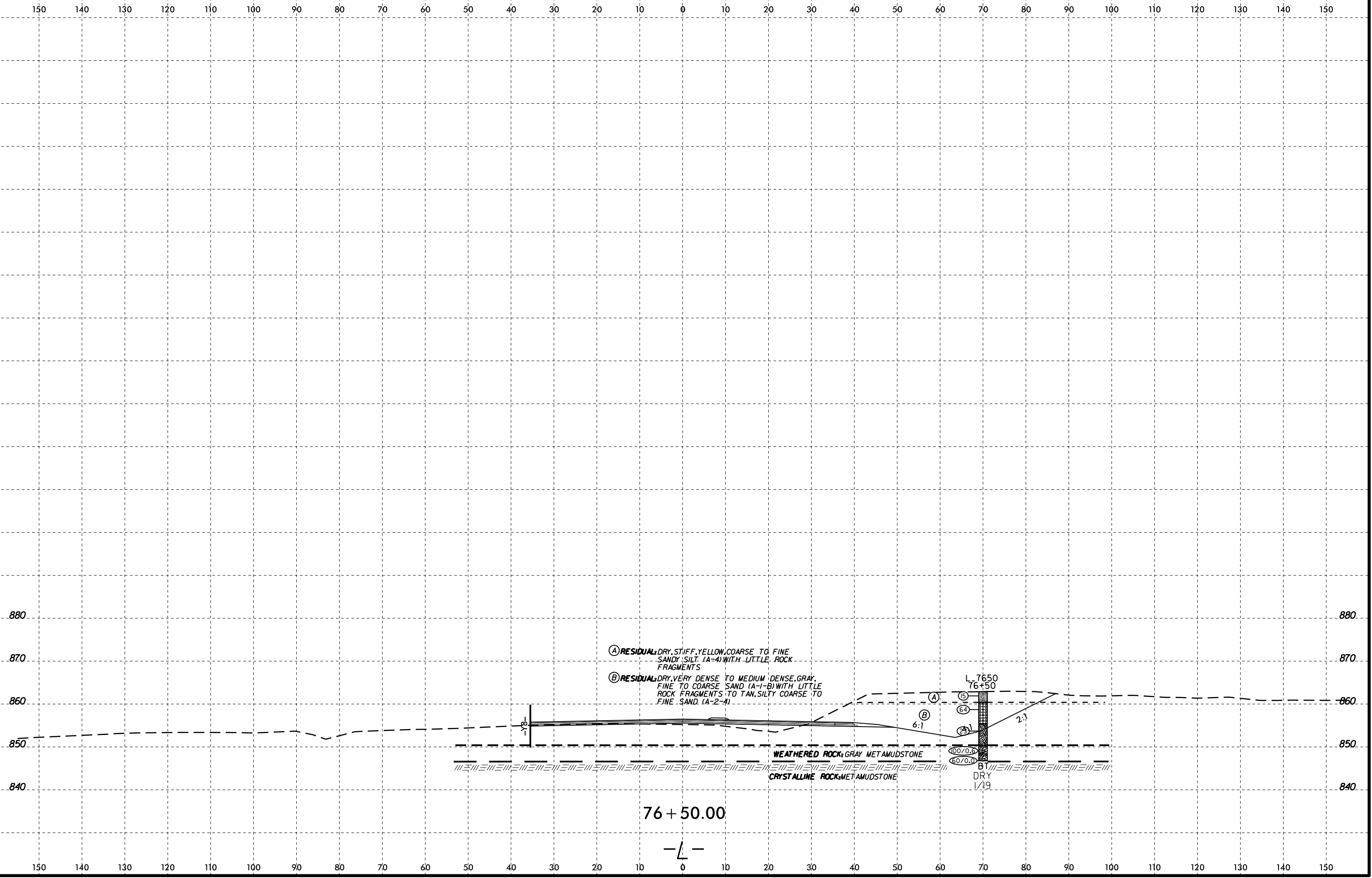
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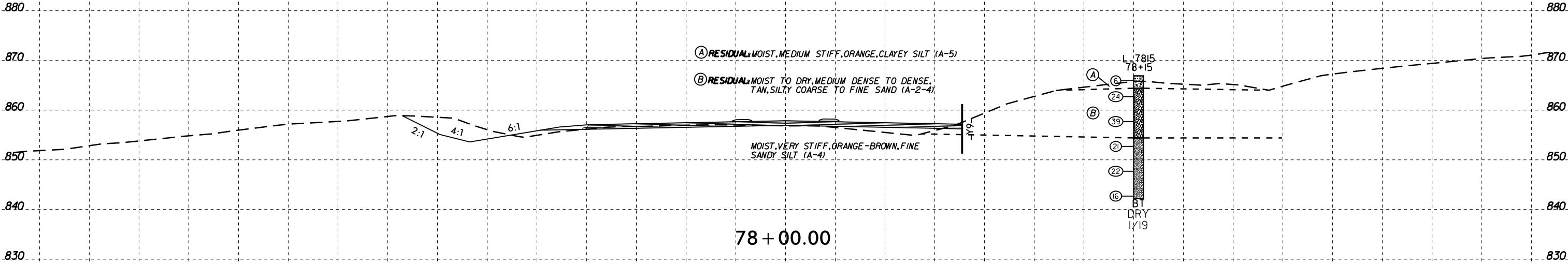
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SPopke AT K6209516

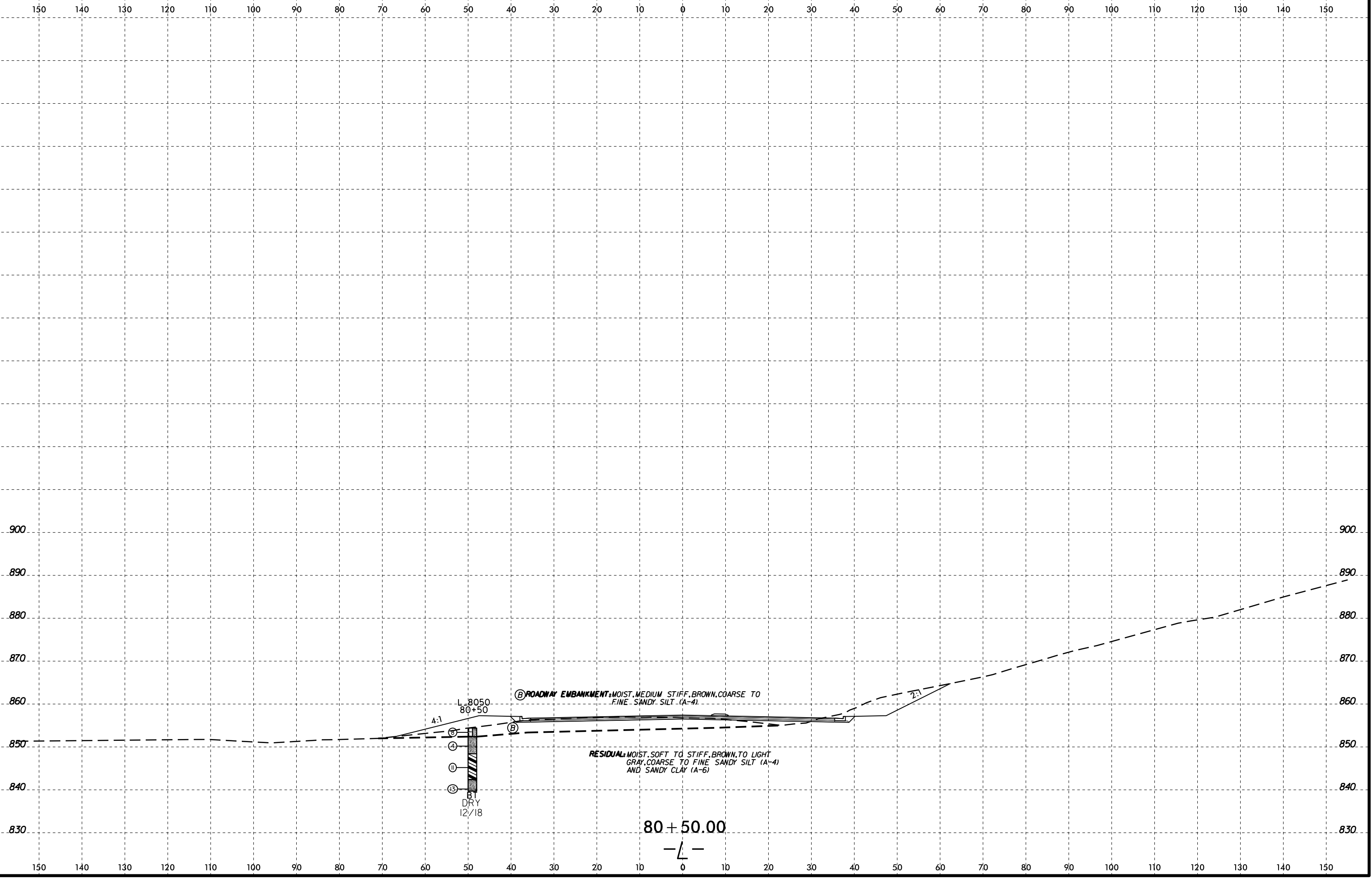
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6/23/16



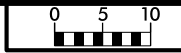
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14-OCT-2019 17:44
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SPopke AT K209516



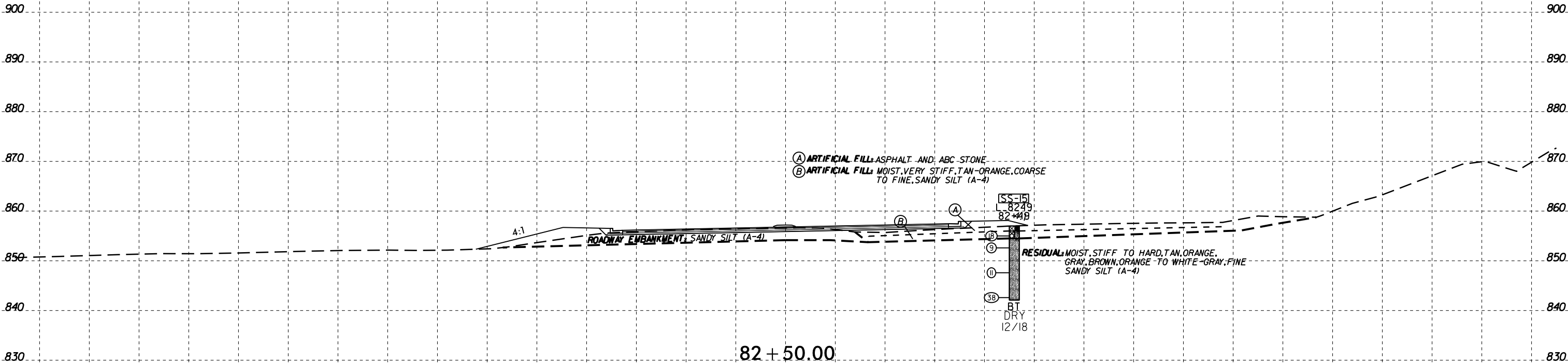
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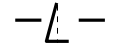
PROJ. REFERENCE NO.
U-5813

SHEET NO.
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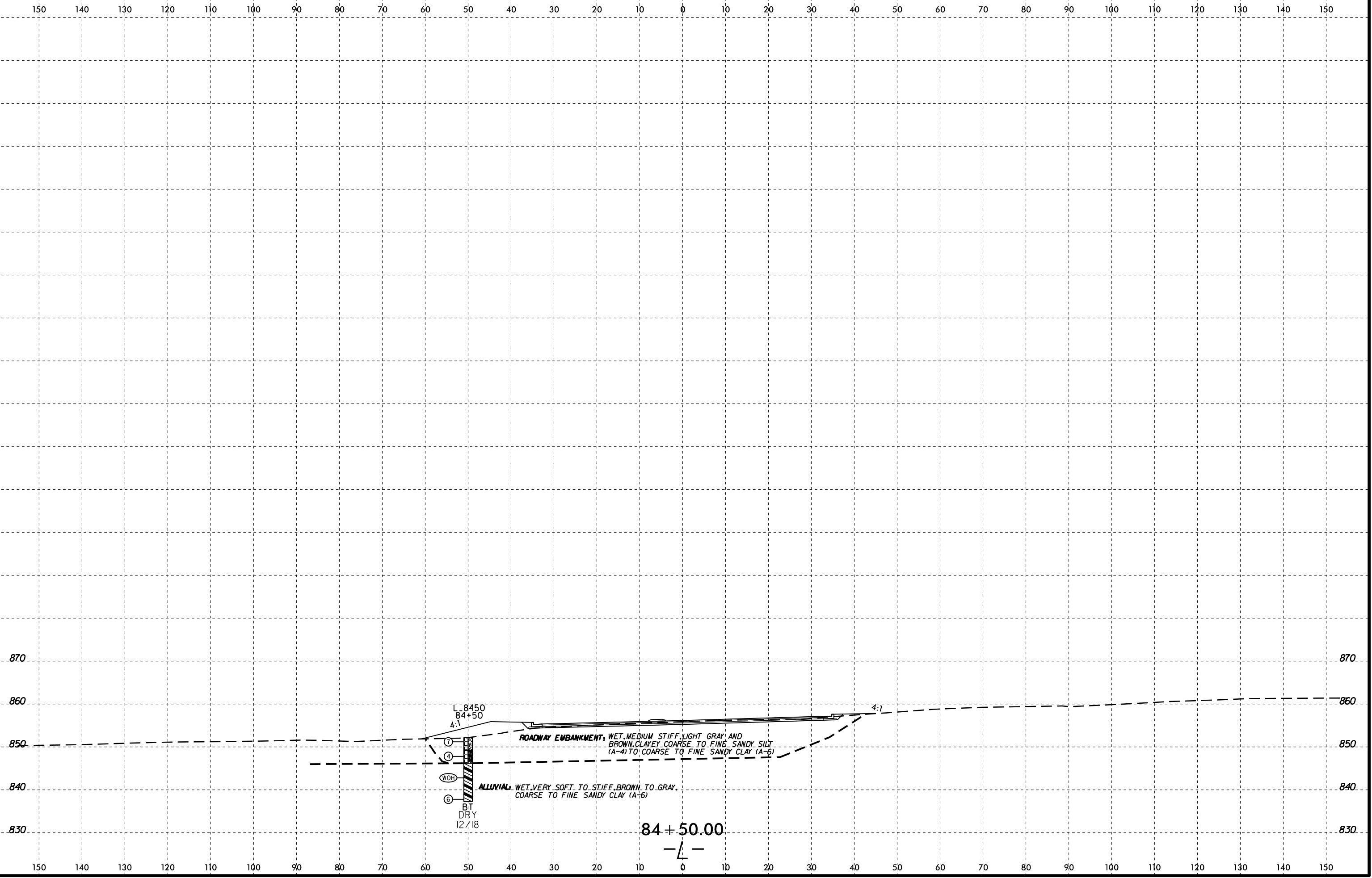


82 + 50.00

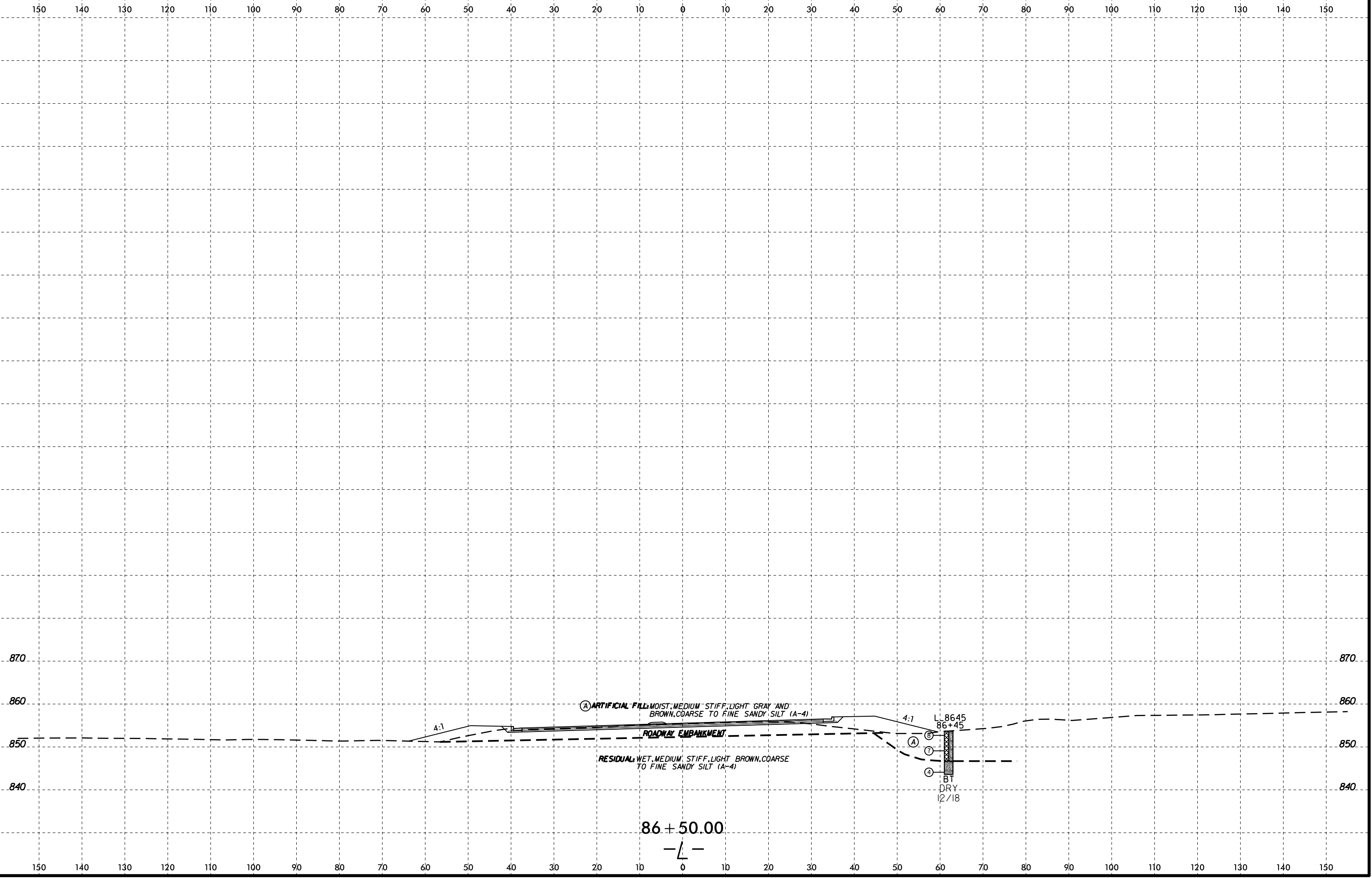


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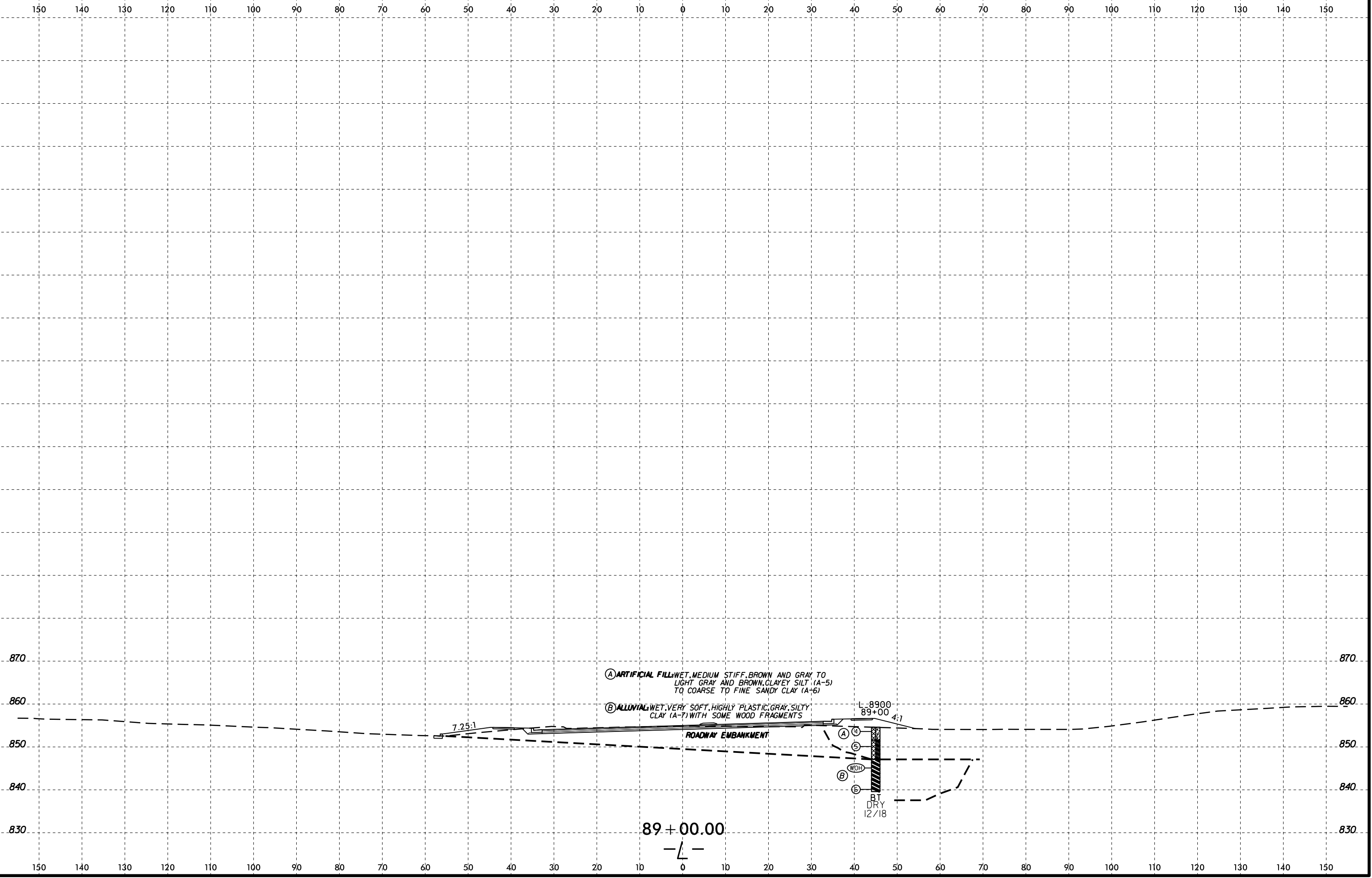
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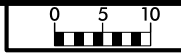


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SPopke AT K&20516

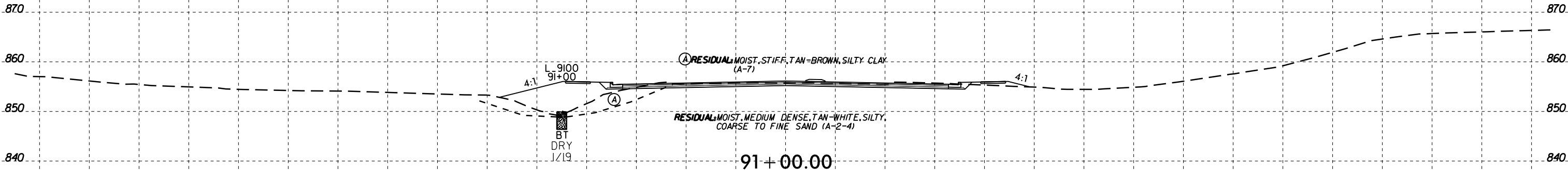


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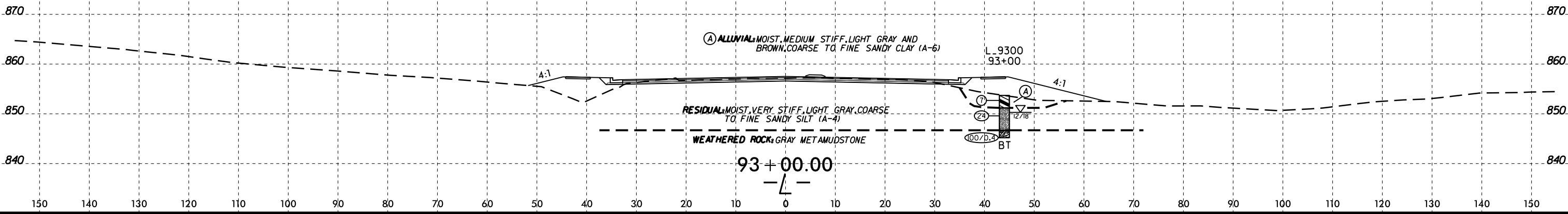
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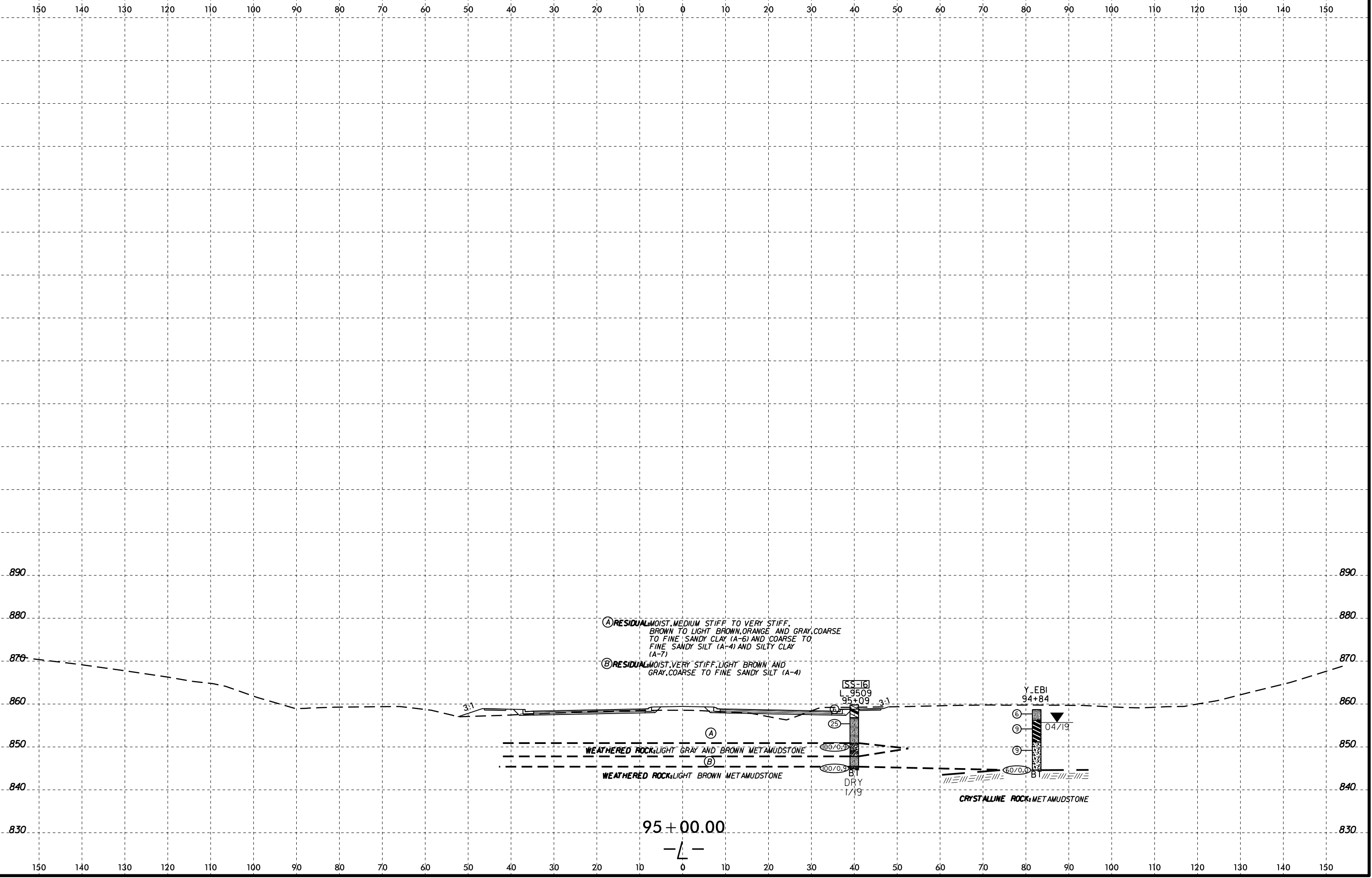


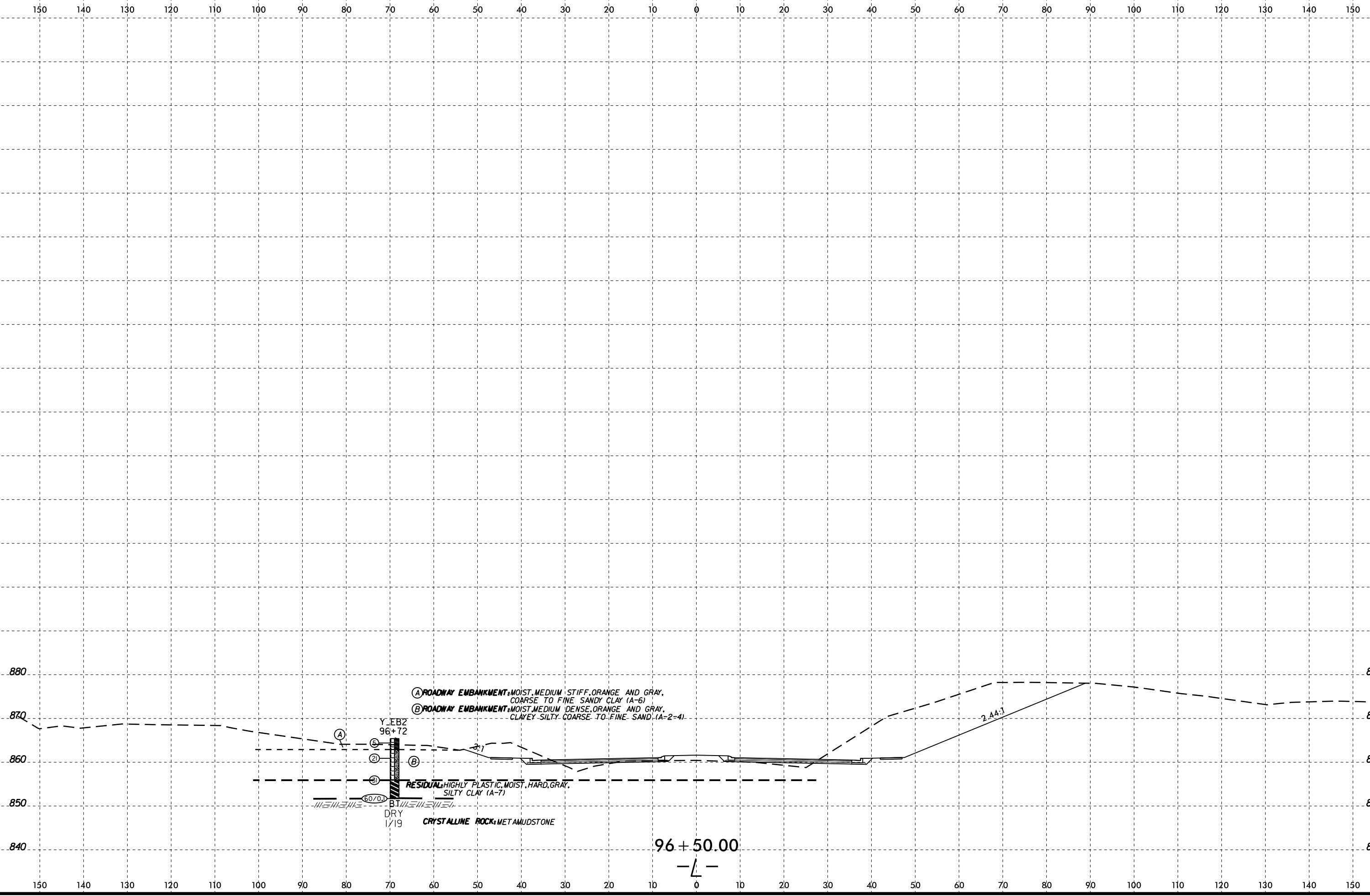
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150 140 130 120 110 100 90 80 70 60 50 40 30 20 10 0 10 20 30 40 50 60 70 80 90 100 110 120 130 140 150







(A) ROADWAY EMBANKMENT: MOIST, MEDIUM STIFF, ORANGE AND GRAY, COARSE TO FINE SANDY CLAY (A-6)
 (B) ROADWAY EMBANKMENT: MOIST, MEDIUM DENSE, ORANGE AND GRAY, CLAYEY SILTY COARSE TO FINE SAND (A-2=4)

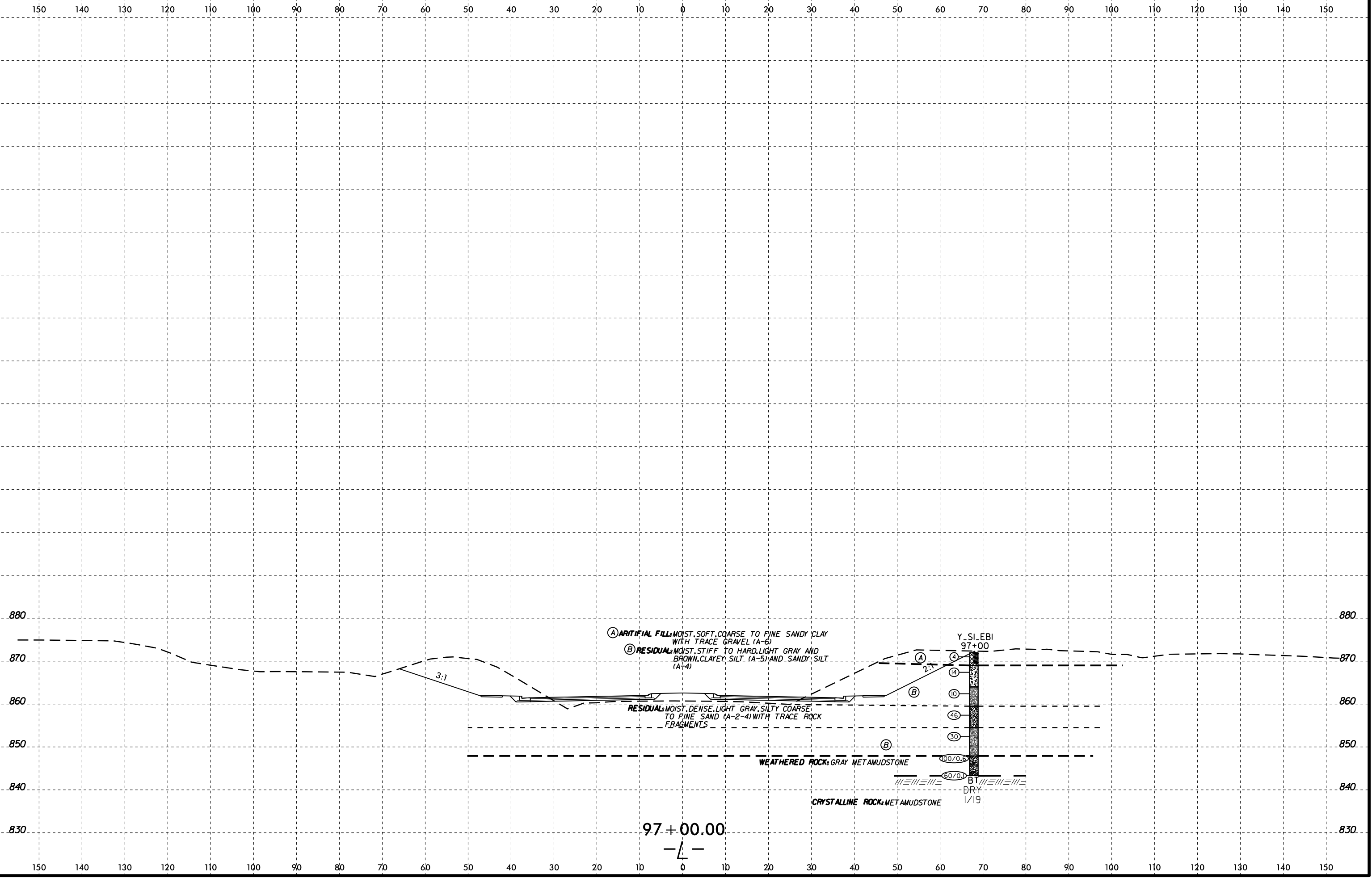
Y.EB2
96+72

RESIDUAL HIGHLY PLASTIC, MOIST, HARD, GRAY, SILTY CLAY (A-7)

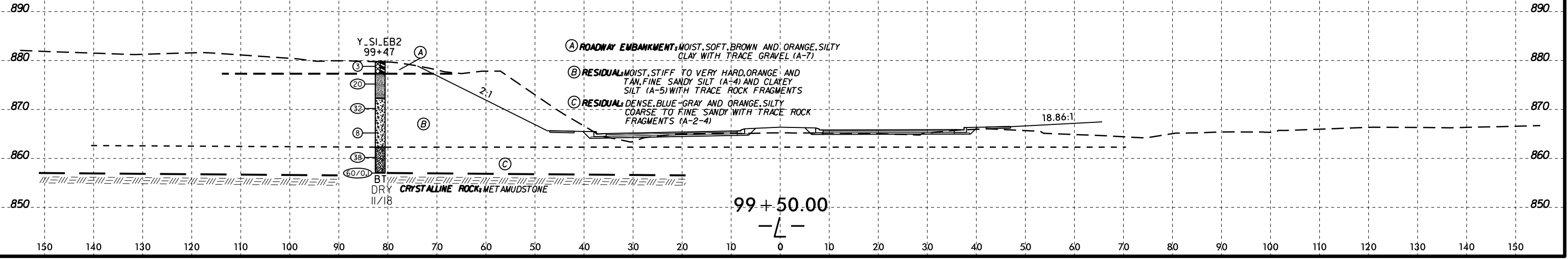
CRYSTALLINE ROCK METAMUDSTONE

96+50.00

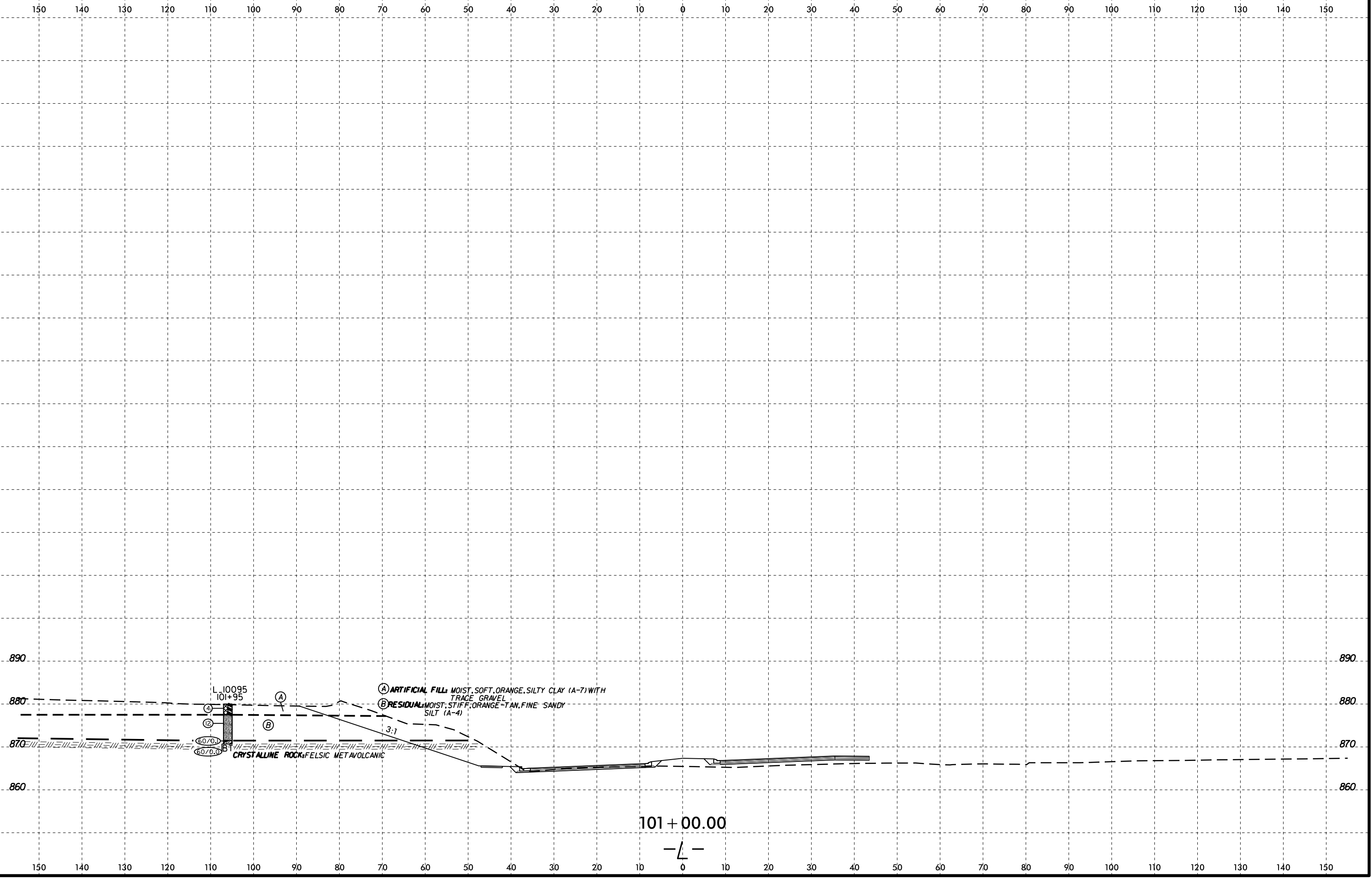
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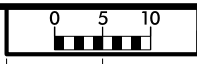


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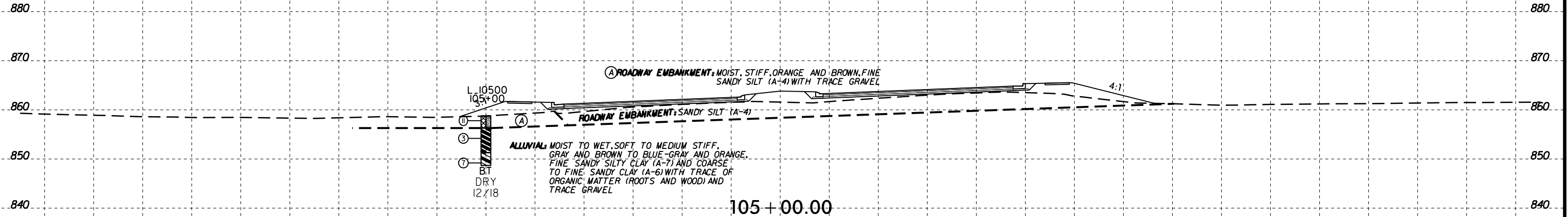


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Sheet AT K209516





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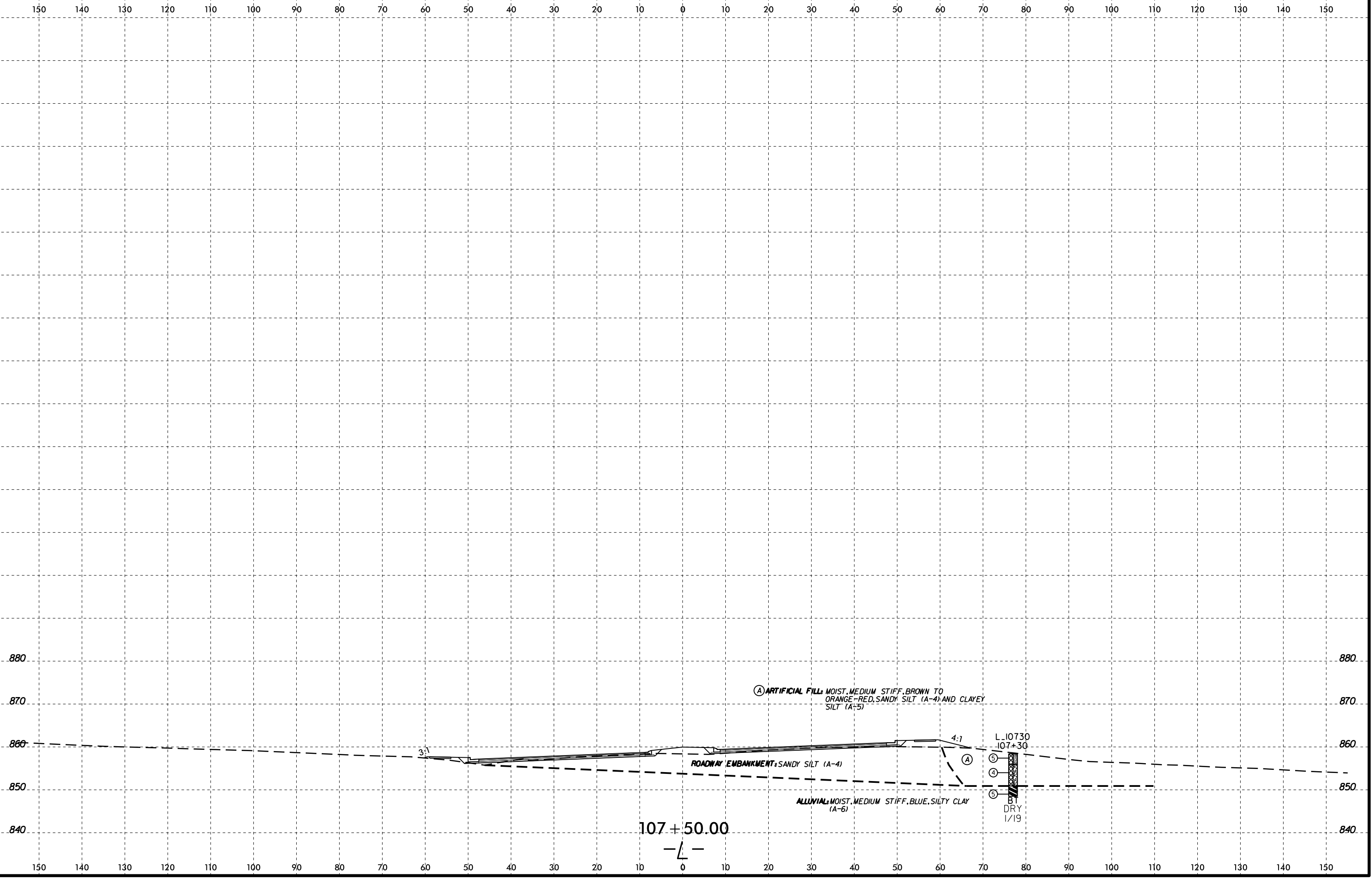
L 10500
105+00
BT
DRY
12/18

ROADWAY EMBANKMENT: MOIST, STIFF, ORANGE AND BROWN, FINE SANDY SILT (A-4) WITH TRACE GRAVEL.
ROADWAY EMBANKMENT: SANDY SILT (A-4)
ALLUVIAL: MOIST TO WET, SOFT TO MEDIUM STIFF, GRAY AND BROWN TO BLUE-GRAY AND ORANGE, FINE SANDY SILTY CLAY (A-7) AND COARSE TO FINE SANDY CLAY (A-6) WITH TRACE OF ORGANIC MATTER (ROOTS AND WOOD) AND TRACE GRAVEL.

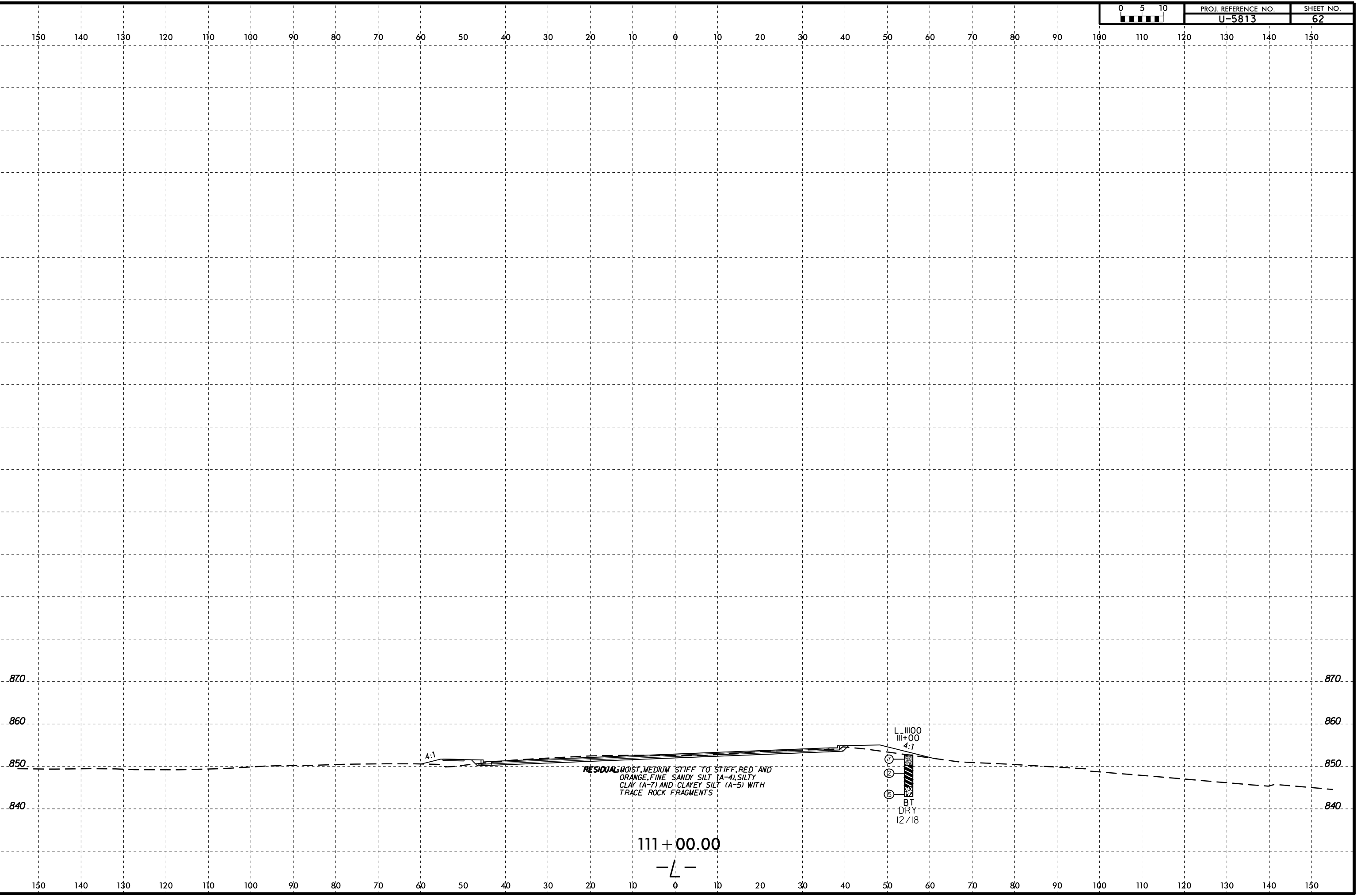
105 + 00.00

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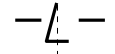
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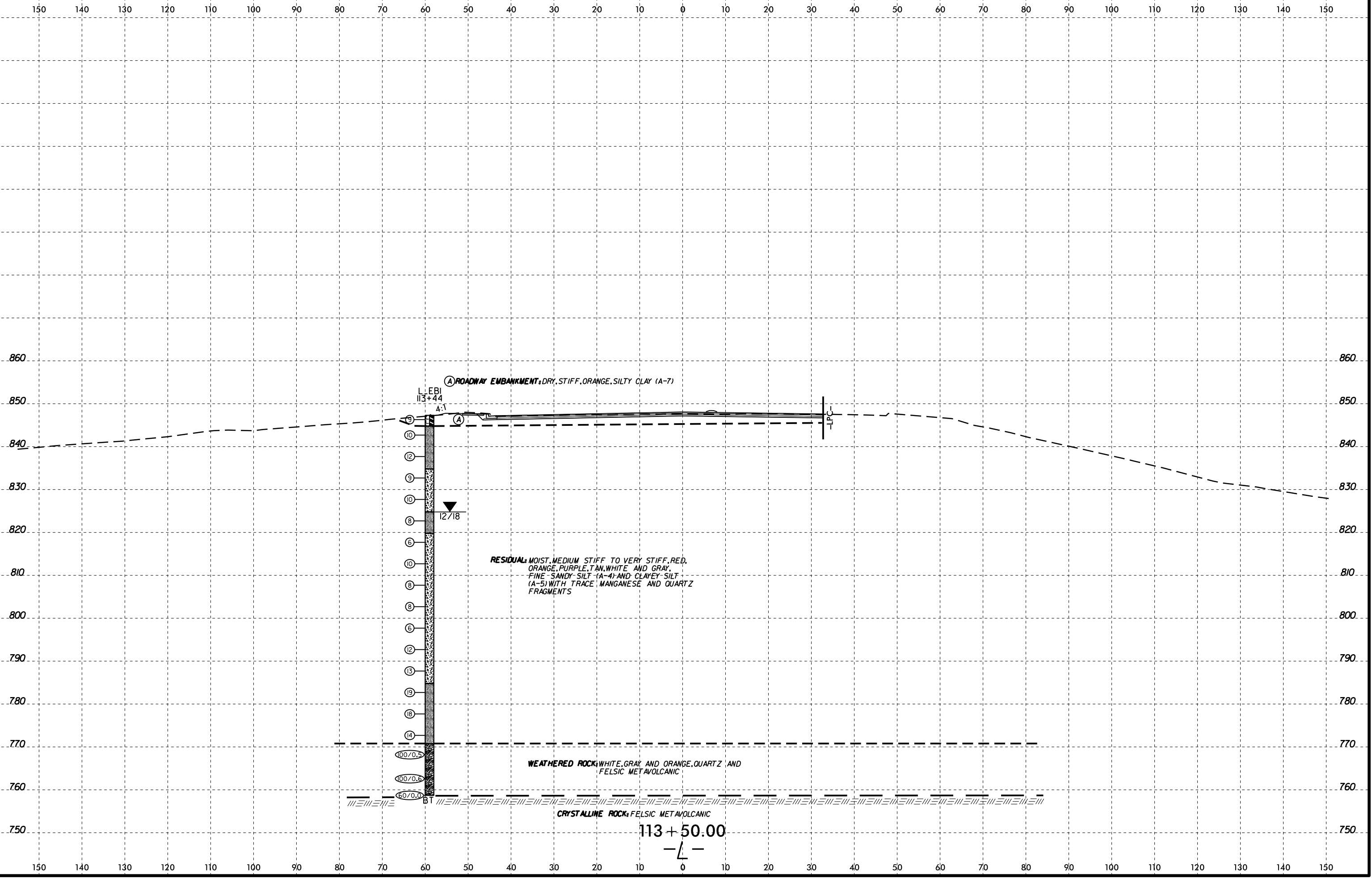
RESIDUAL MOIST MEDIUM STIFF TO STIFF RED AND ORANGE FINE SANDY SILT (A-4), SILTY CLAY (A-7) AND CLAYEY SILT (A-5) WITH TRACE ROCK FRAGMENTS

L 111+00
111+00
4:1
7
2
5
BT
DRY
12/18

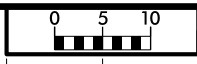
111+00.00



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 Spoke AT K420516

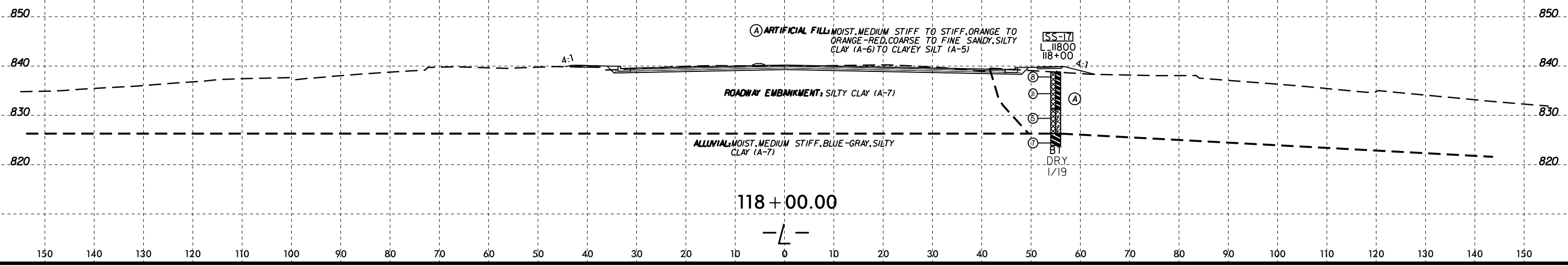


6/23/16



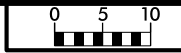
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 Spoke AT K209516

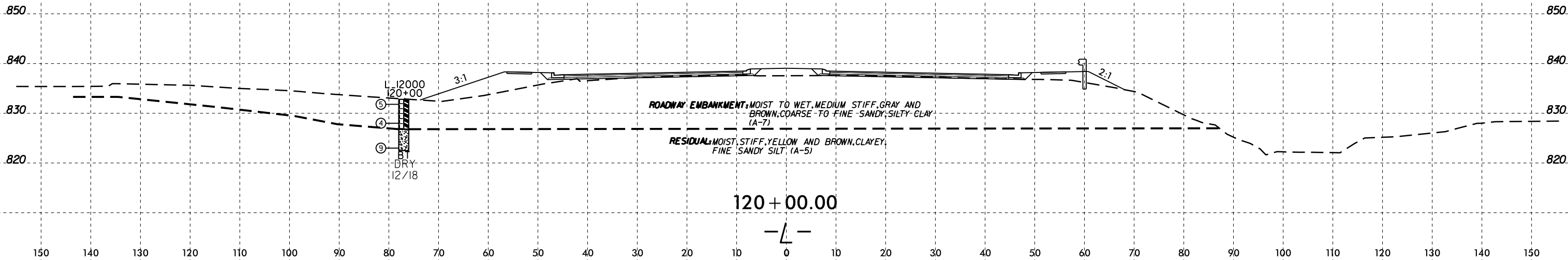
6/23/16



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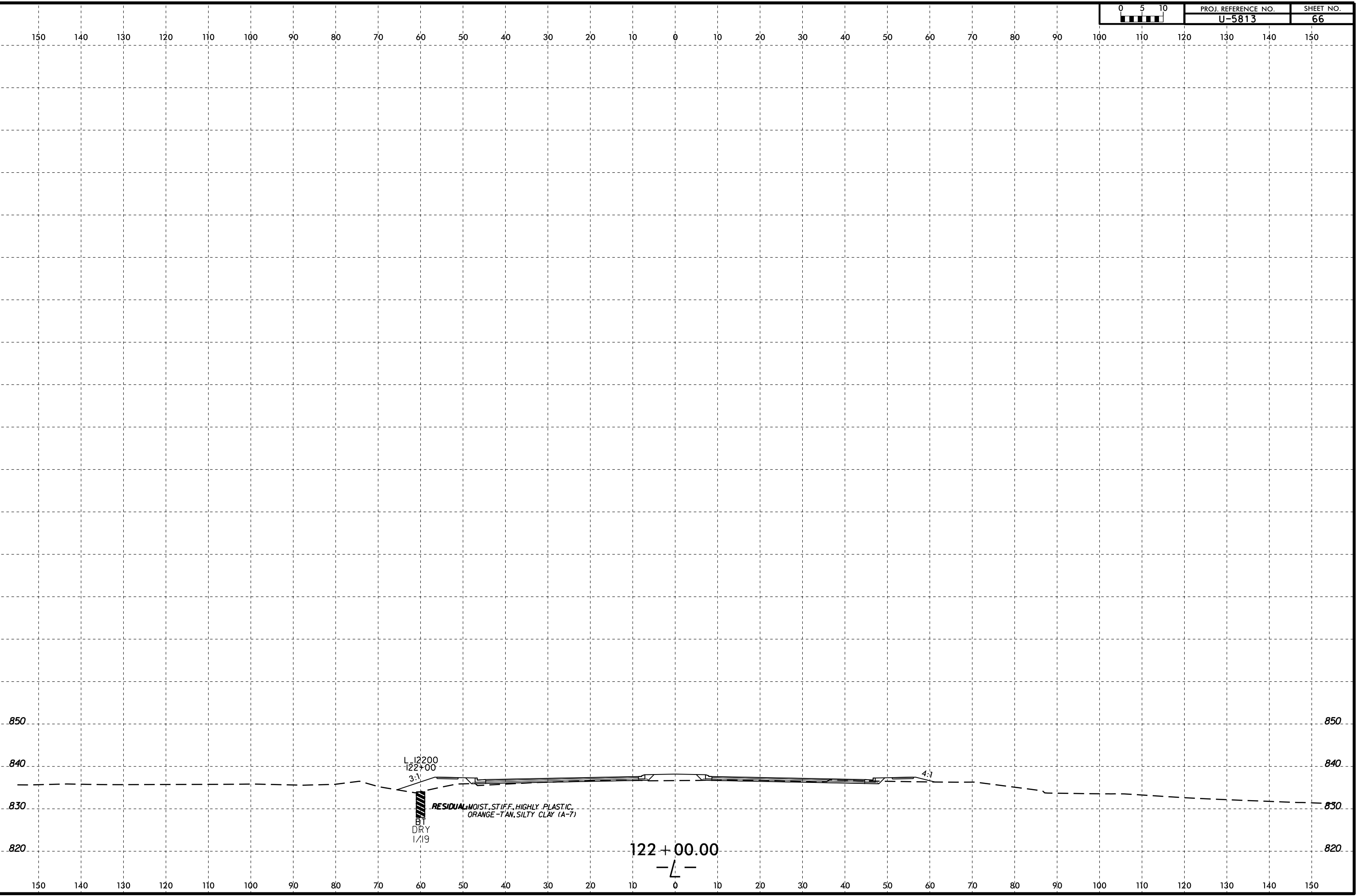
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SPopke AT K6209516

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850 850

840 840

830 830

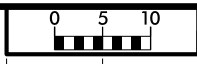
820 820

150 140 130 120 110 100 90 80 70 60 50 40 30 20 10 0 10 20 30 40 50 60 70 80 90 100 110 120 130 140 150

L 12200
122+00
3:1
BT
DRY
1/19
RESIDUAL MOIST, STIFF, HIGHLY PLASTIC,
ORANGE-TAN, SILTY CLAY (A-7)

122 + 00.00
— L —

6/23/16

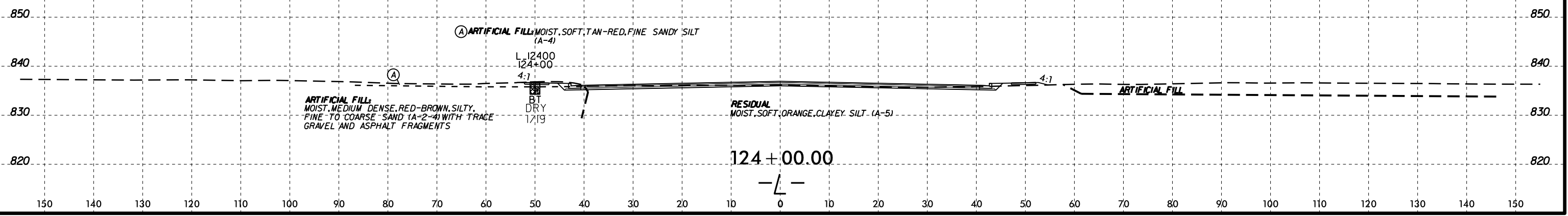


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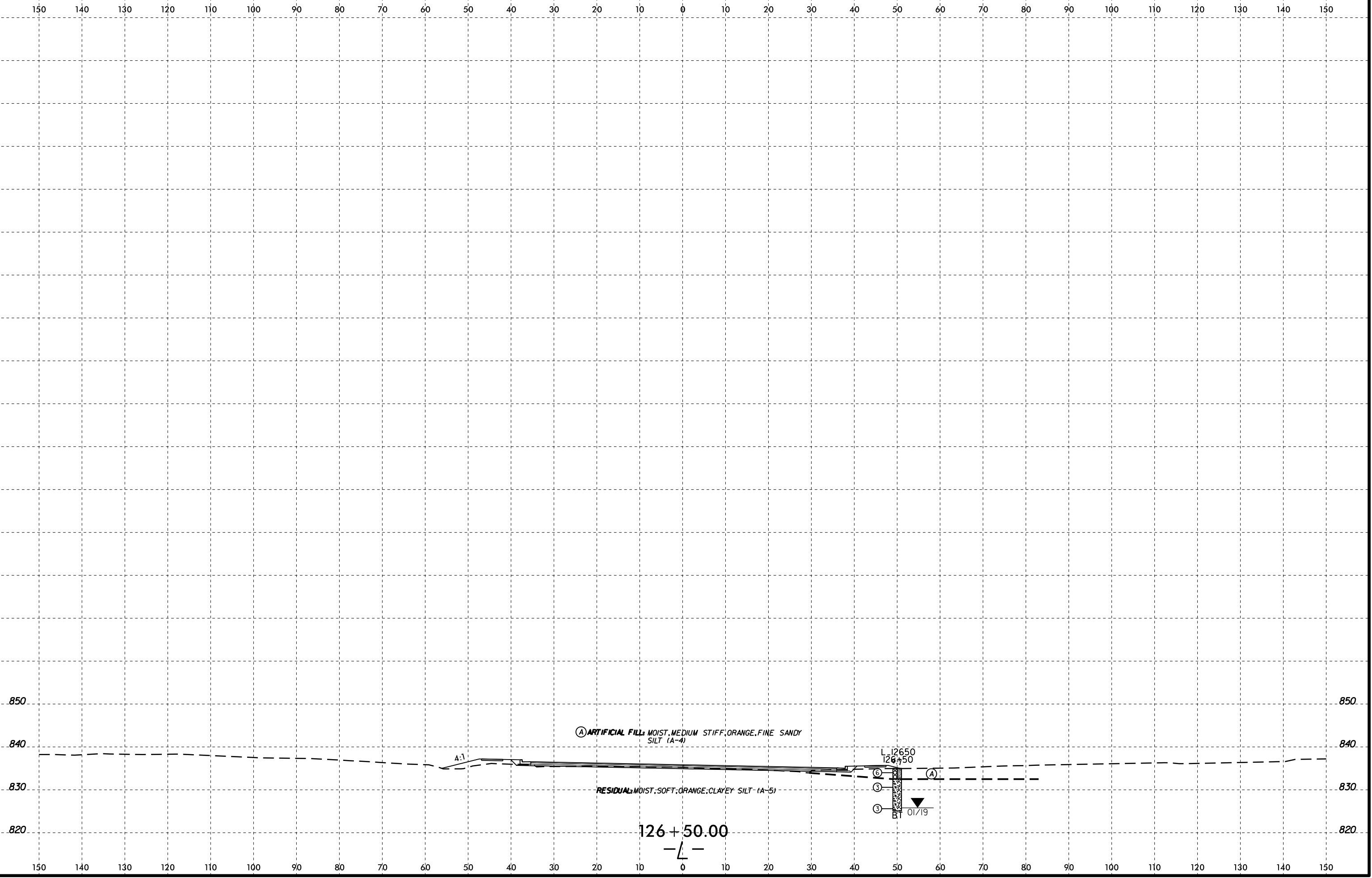
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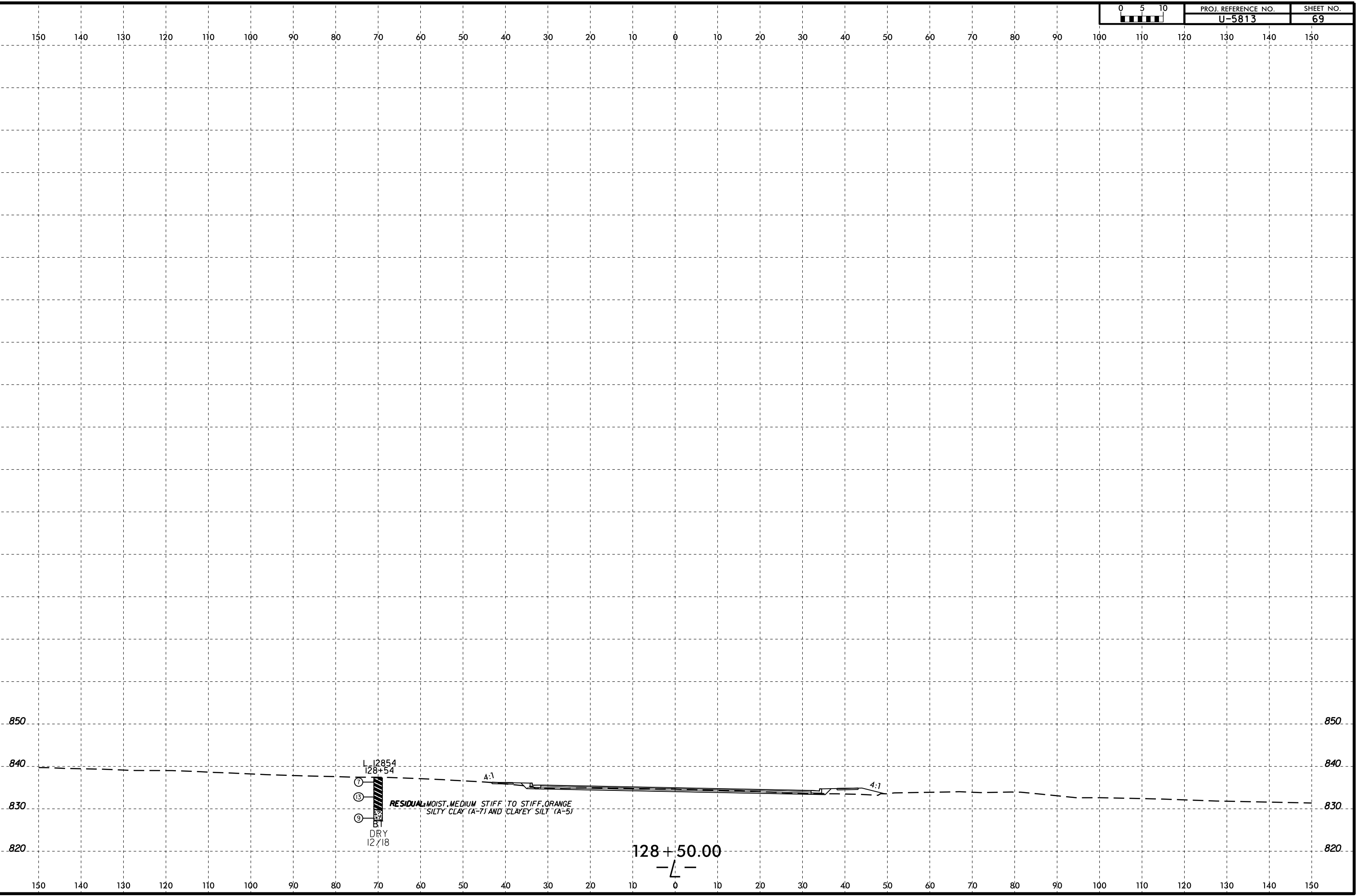
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SPopke AT K620516



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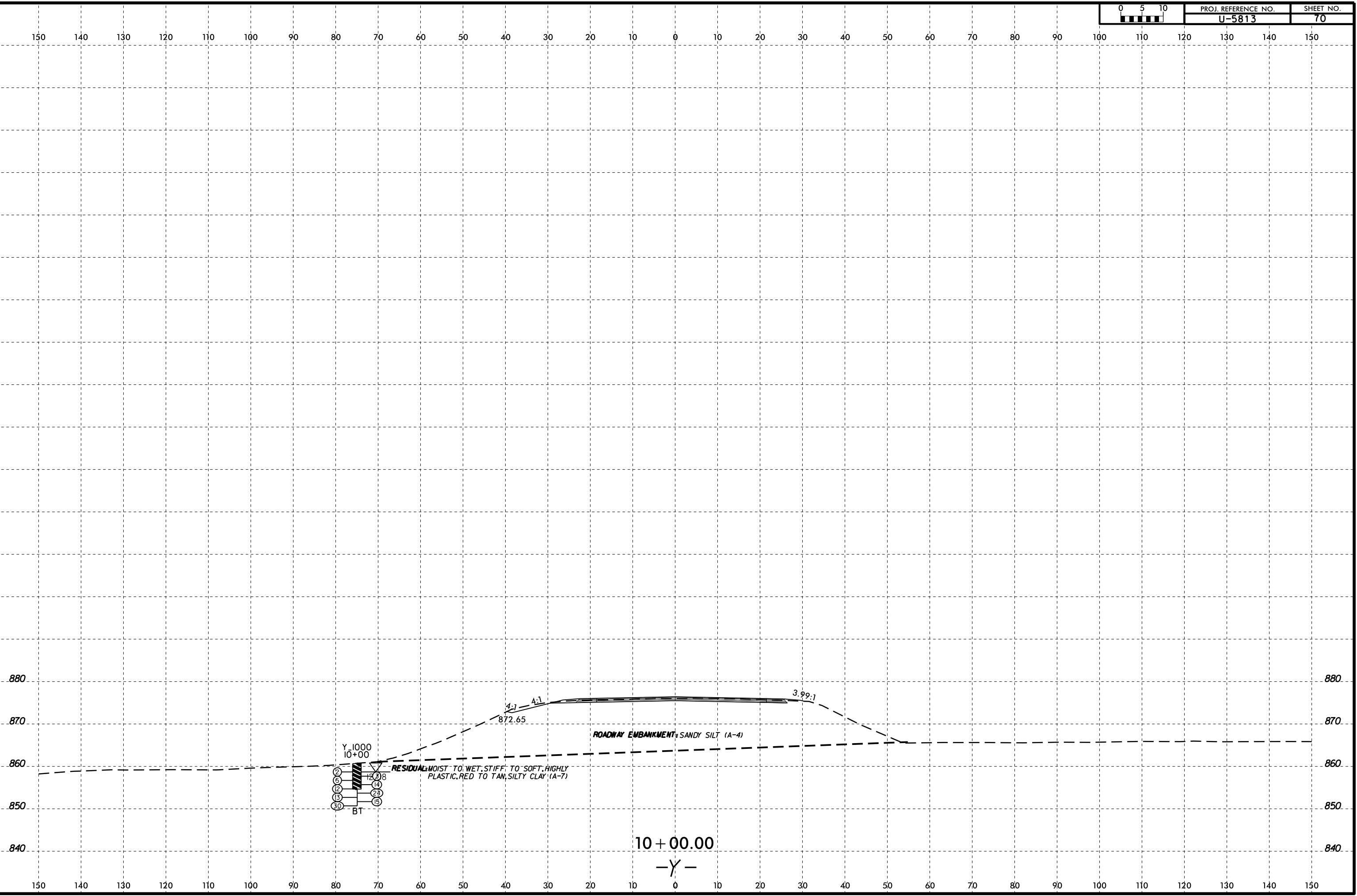


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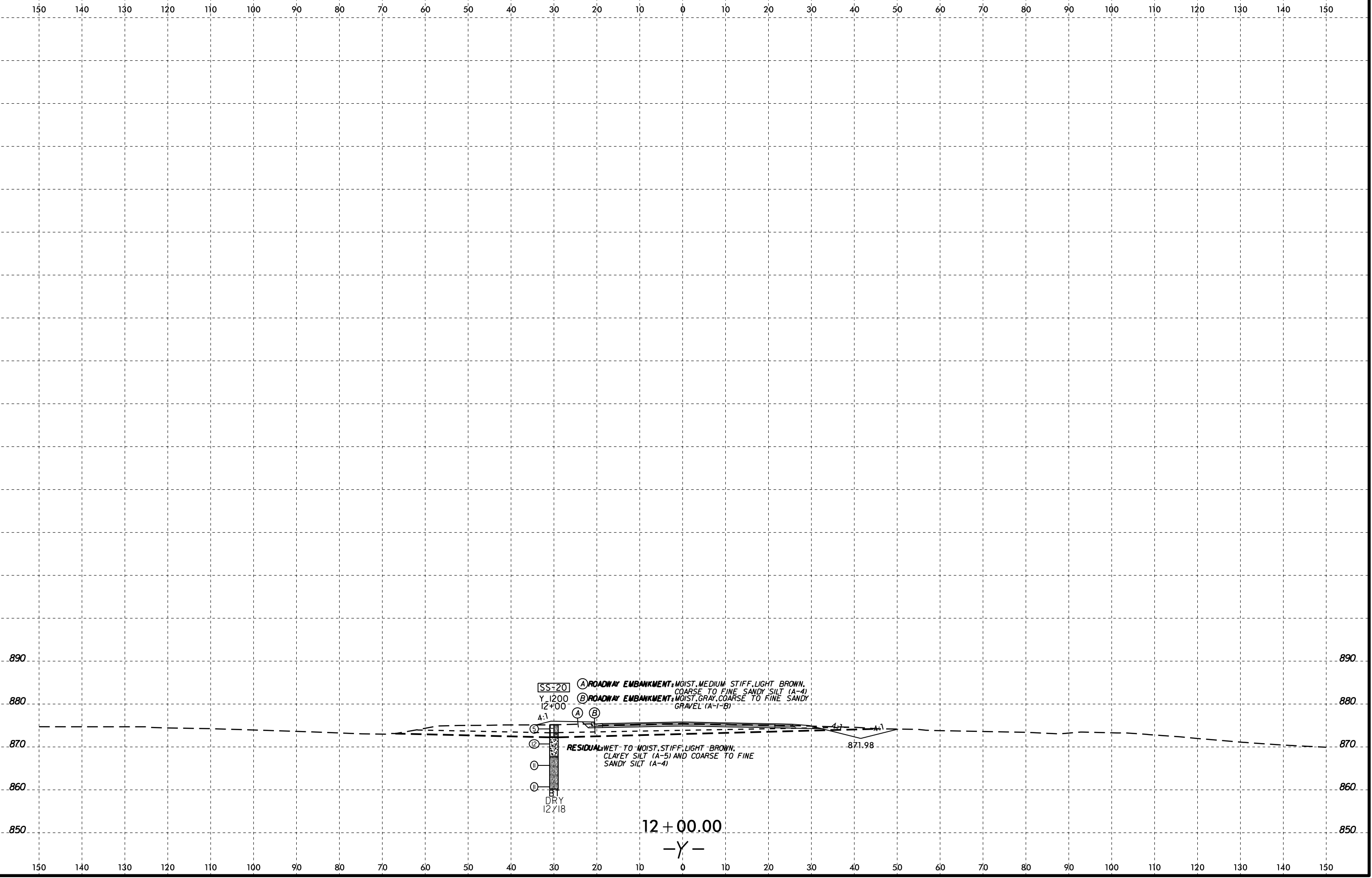


128+50.00
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SPopke AT K&209516



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S:\pape AT K6209516



SS+20
Y 1200
12+00
A ROADWAY EMBANKMENT: MOIST, MEDIUM STIFF, LIGHT BROWN, COARSE TO FINE SANDY SILT (A-4)
B ROADWAY EMBANKMENT: MOIST, GRAY, COARSE TO FINE SANDY GRAVEL (A-1-B)

5
12
11
11
BT
DRY
12/18

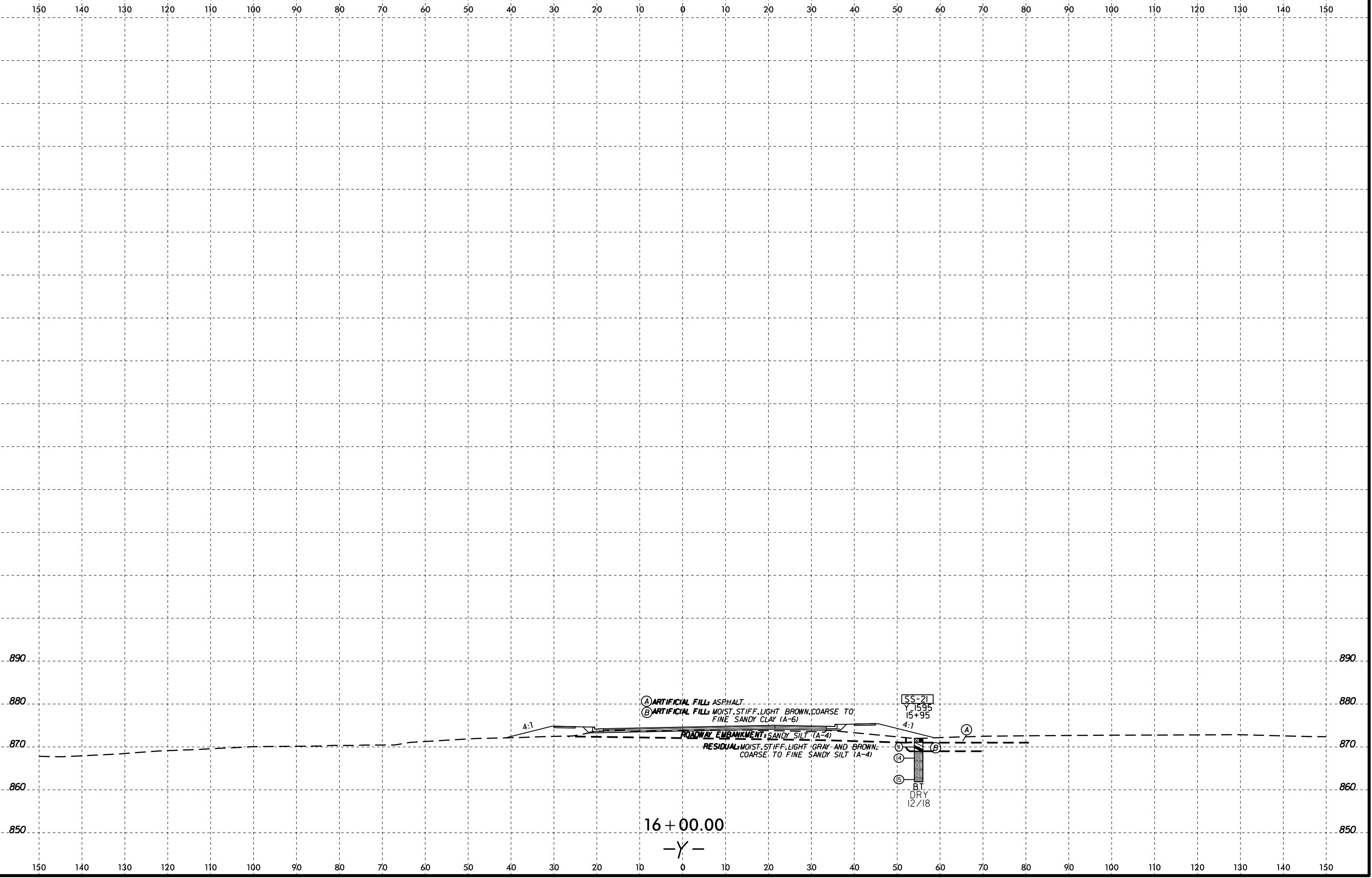
RESIDUAL: WET TO MOIST, STIFF, LIGHT BROWN, CLAYEY SILT (A-5) AND COARSE TO FINE SANDY SILT (A-4)

871.98

12 + 00.00

-Y-

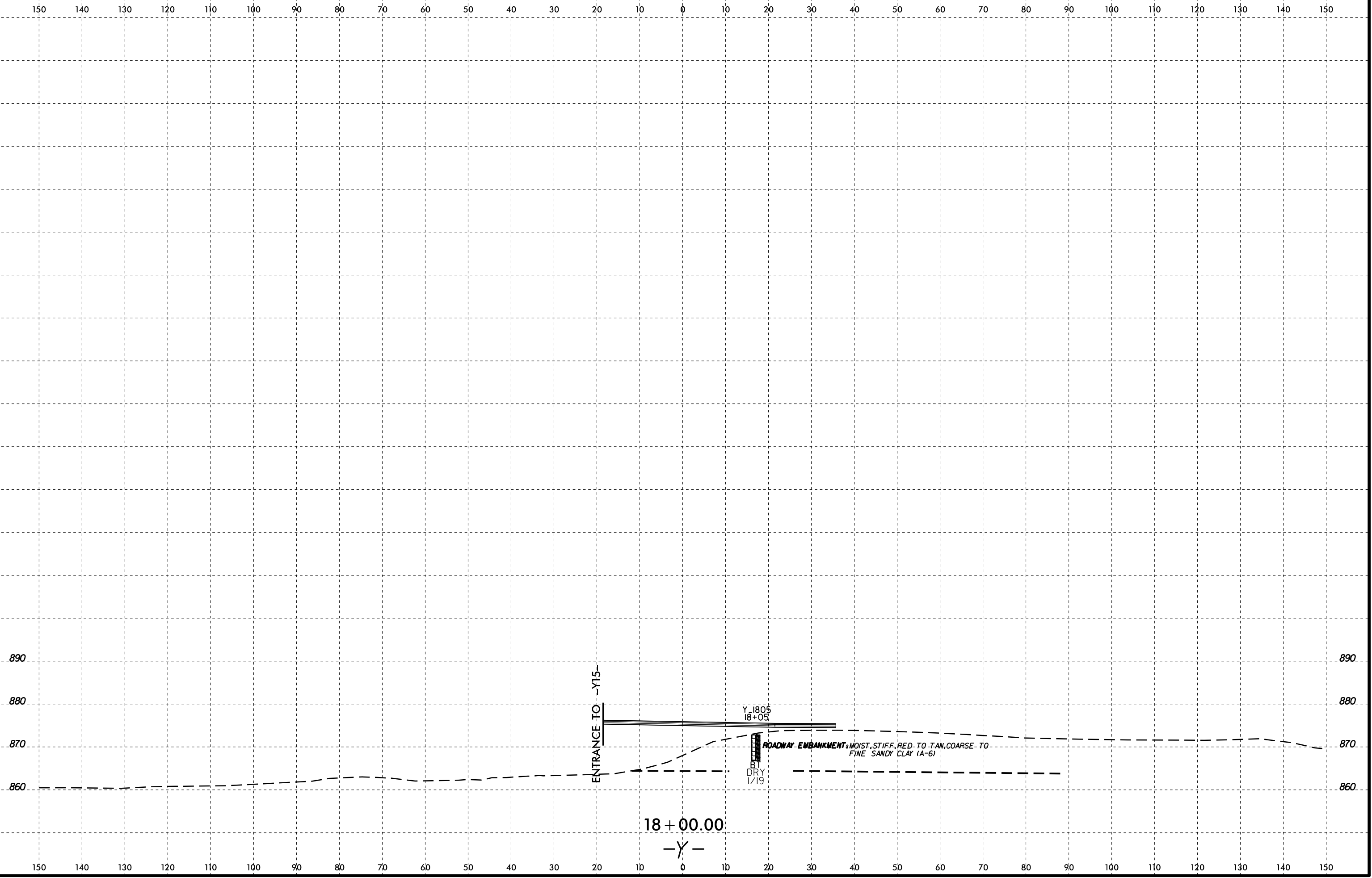
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Sheet AT K6209516

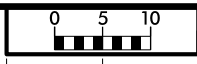


16 + 00.00
-Y-

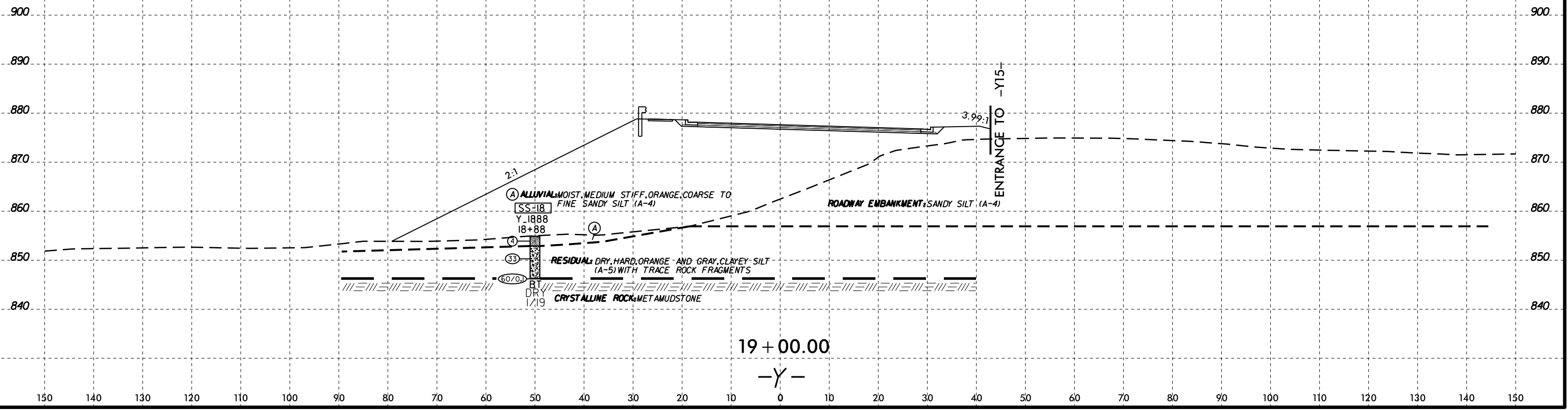
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0 5 10	PROJ. REFERENCE NO.	SHEET NO.
	U-5813	73





150 140 130 120 110 100 90 80 70 60 50 40 30 20 10 0 10 20 30 40 50 60 70 80 90 100 110 120 130 140 150



(A) ALLUVIAL MOIST MEDIUM STIFF ORANGE COARSE TO FINE SANDY SILT (A-4)

SS-18
Y-1888
18+88

(4)

(33)

(60/0)

BT
DRY
1/19

RESIDUAL DRY HARD ORANGE AND GRAY CLAYEY SILT (A-5) WITH TRACE ROCK FRAGMENTS

CRYSTALLINE ROCK METAMUDSTONE

ROADWAY EMBANKMENT SANDY SILT (A-4)

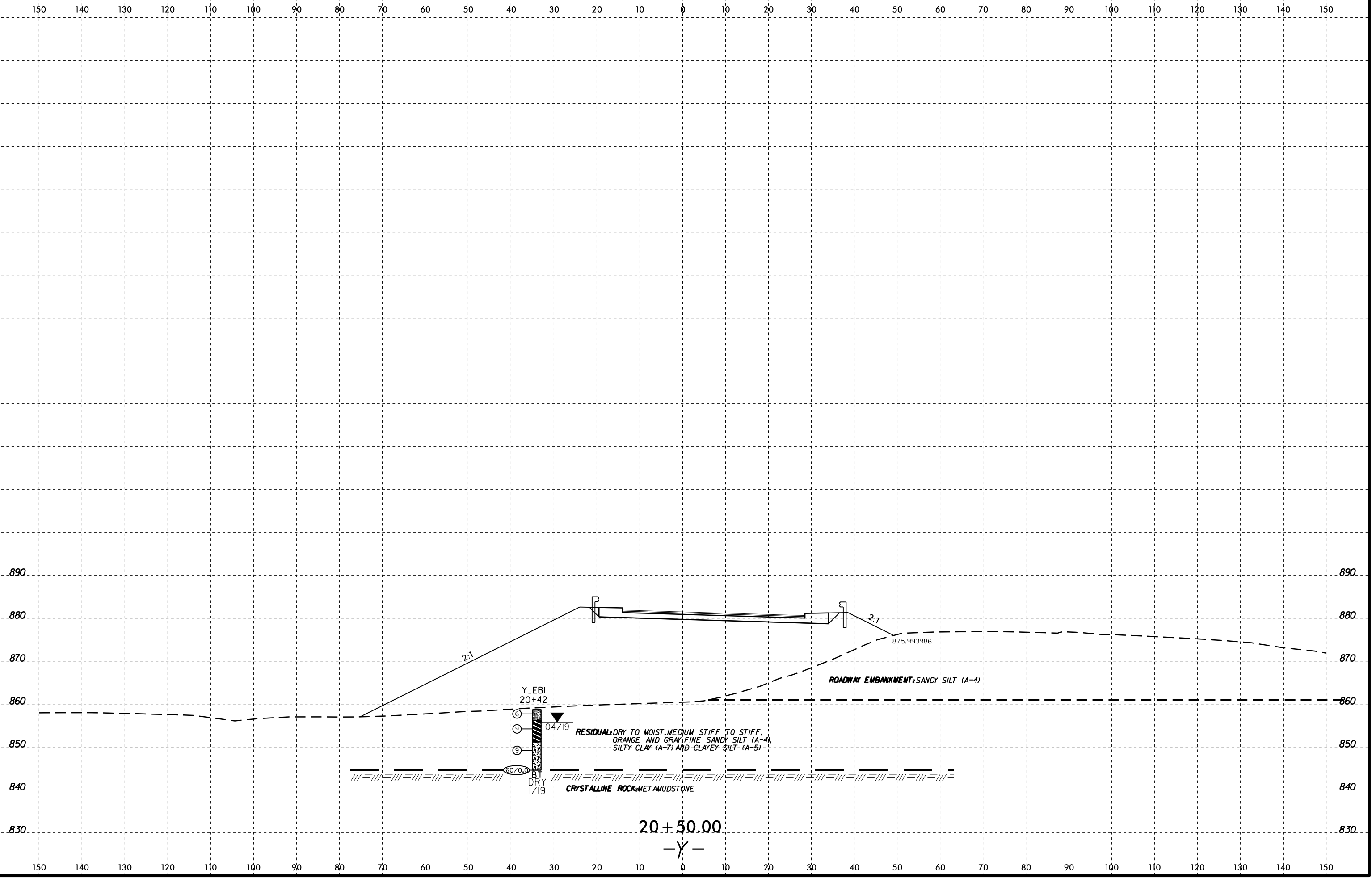
3.9%

ENTRANCE TO -Y15-

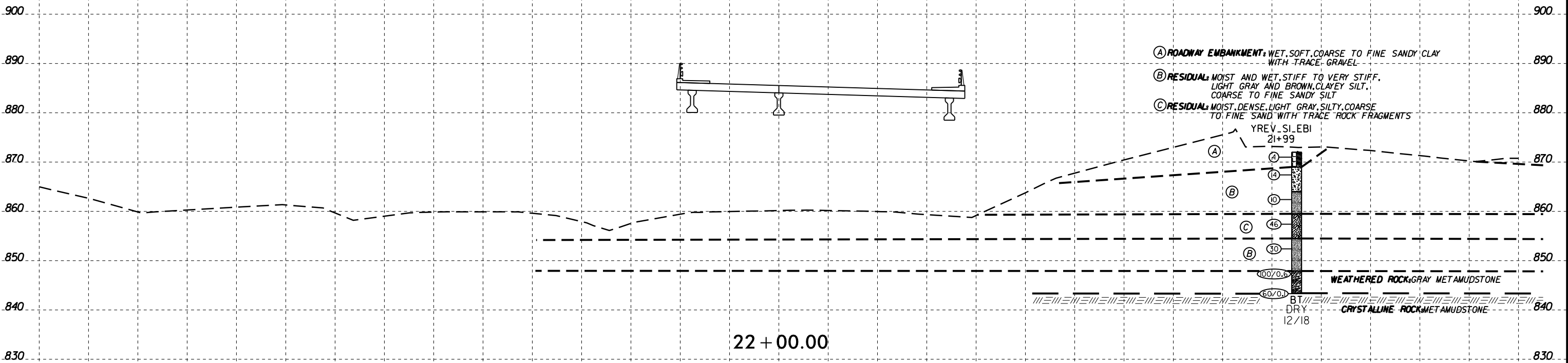
19 + 00.00

-Y-

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SPopke AT K&209516



150 140 130 120 110 100 90 80 70 60 50 40 30 20 10 0 10 20 30 40 50 60 70 80 90 100 110 120 130 140 150



- (A) ROADWAY EMBANKMENT: WET, SOFT, COARSE TO FINE SANDY CLAY WITH TRACE GRAVEL
- (B) RESIDUAL: MOIST AND WET, STIFF TO VERY STIFF, LIGHT GRAY AND BROWN, CLAYEY SILT, COARSE TO FINE SANDY SILT
- (C) RESIDUAL: MOIST, DENSE, LIGHT GRAY, SILTY, COARSE TO FINE SAND WITH TRACE ROCK FRAGMENTS

YREV_SI_EBI
21+99

(A) 4
(B) 14
(C) 10
(B) 46
(B) 30
(100/0.6)
(60/0.1)
BT
DRY
12/18

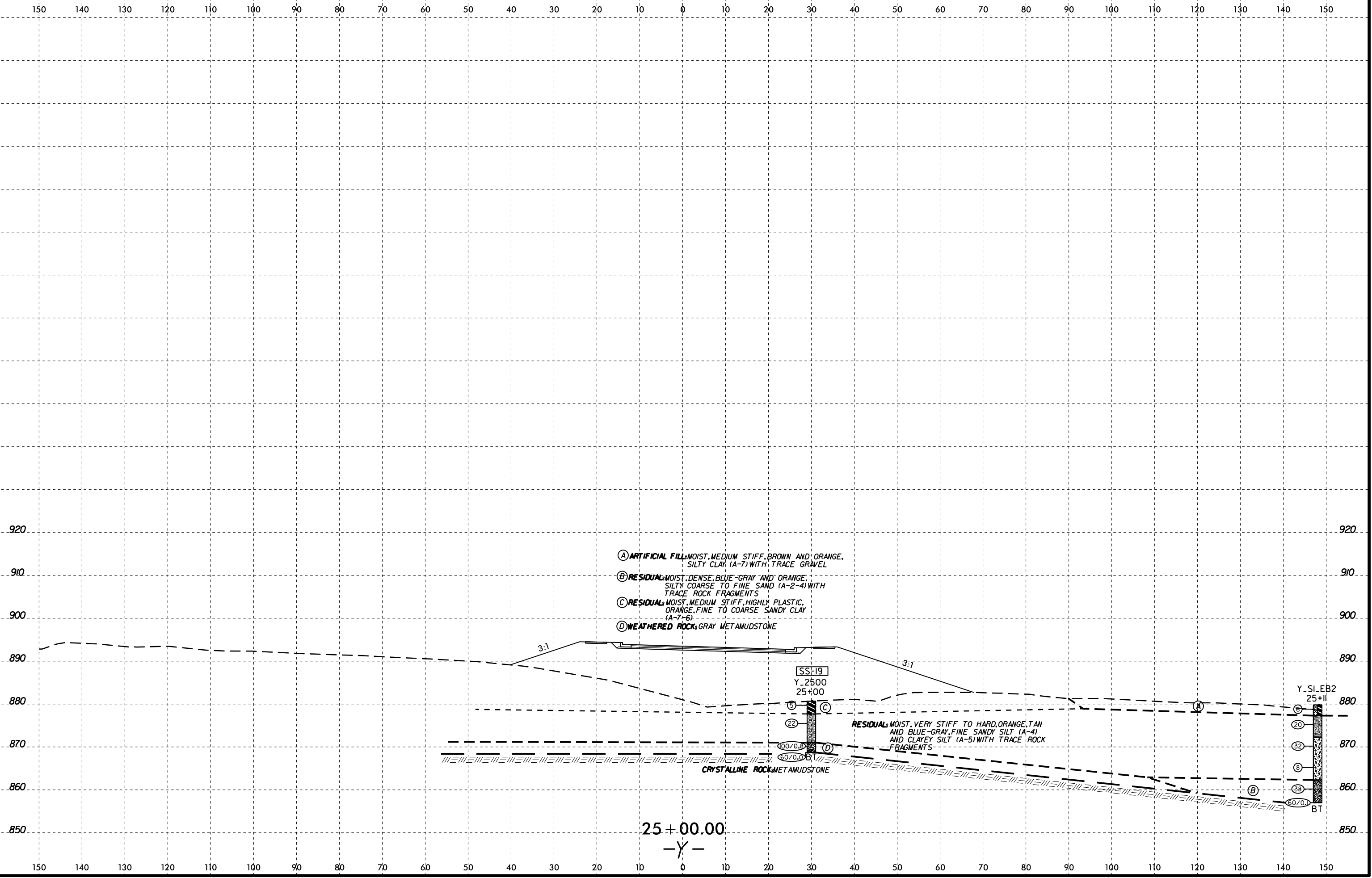
WEATHERED ROCK: GRAY METAMUDSTONE

CRYSTALLINE ROCK: METAMUDSTONE

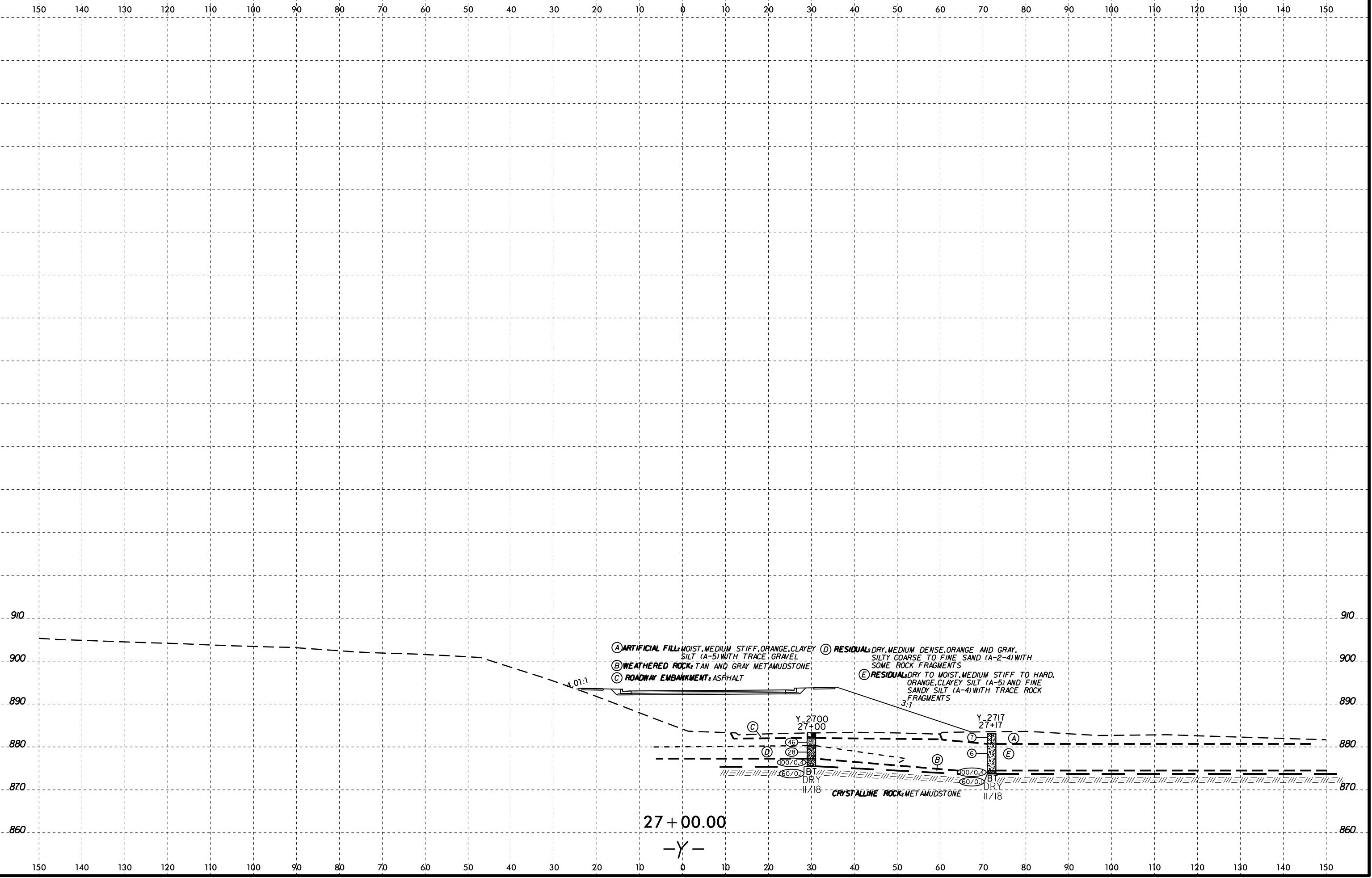
22 + 00.00

-Y-

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SPopke AT K208546



14-OCT-2019 17:47 W:\share\GEO\TECHNICAL\Projects\Active Projects\201909142.009A U-5813 Roadway\U5813_GEO_ROWY\CADD_GEO\TECH\sec\U5813_GEO_Y_XSI.dgn 6/23/16



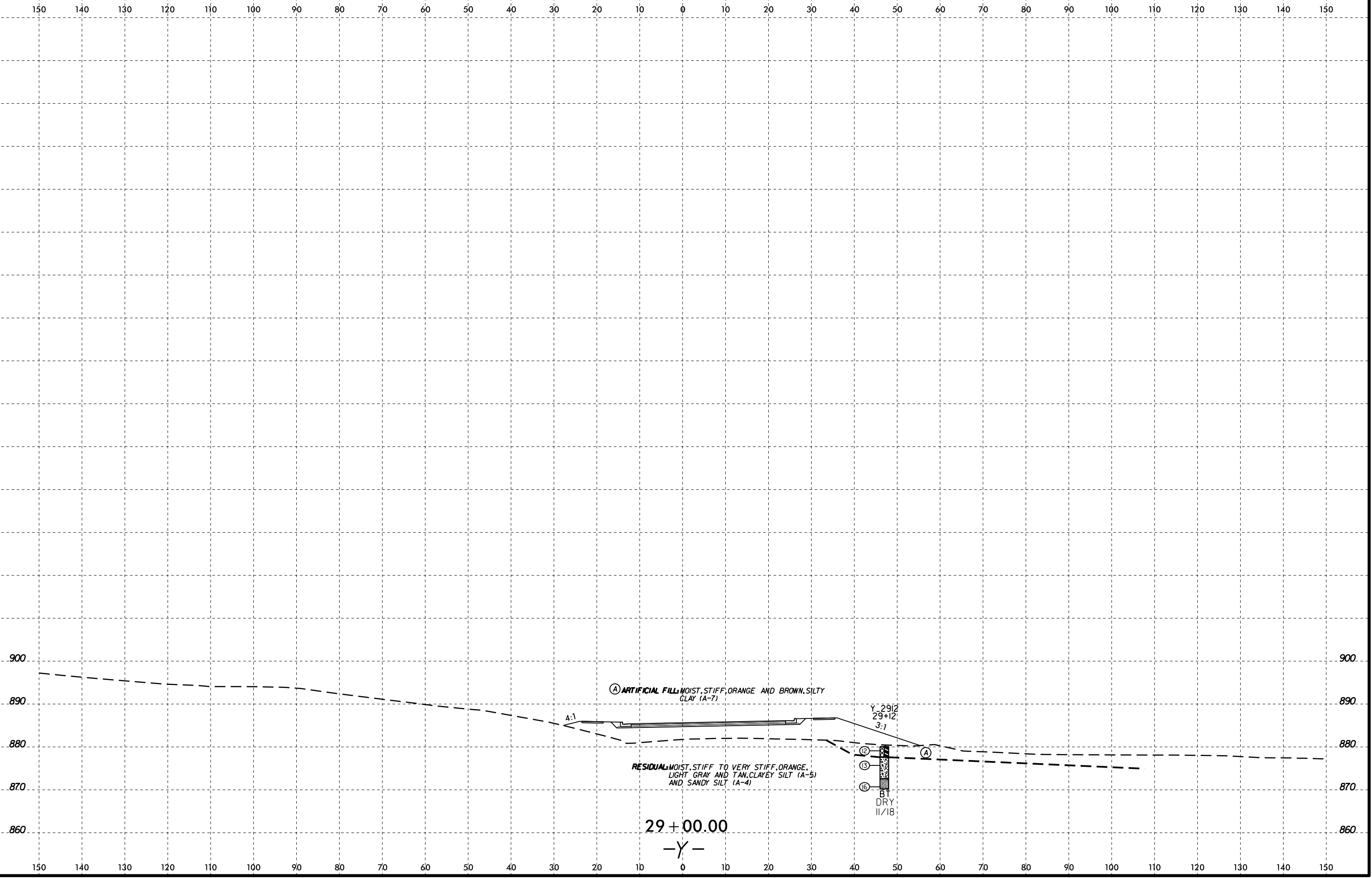
(A) ARTIFICIAL FILL: MOIST, MEDIUM STIFF, ORANGE, CLAYEY SILT (A-5) WITH TRACE GRAVEL
 (B) WEATHERED ROCK: TAN AND GRAY METAMUDSTONE
 (C) ROADWAY EMBANKMENT: ASPHALT

(D) RESIDUAL: DRY, MEDIUM DENSE, ORANGE AND GRAY, SILTY COARSE TO FINE SAND (A-2-4) WITH SOME ROCK FRAGMENTS
 (E) RESIDUAL: DRY TO MOIST, MEDIUM STIFF TO HARD, ORANGE, CLAYEY SILT (A-5) AND FINE SANDY SILT (A-4) WITH TRACE ROCK FRAGMENTS

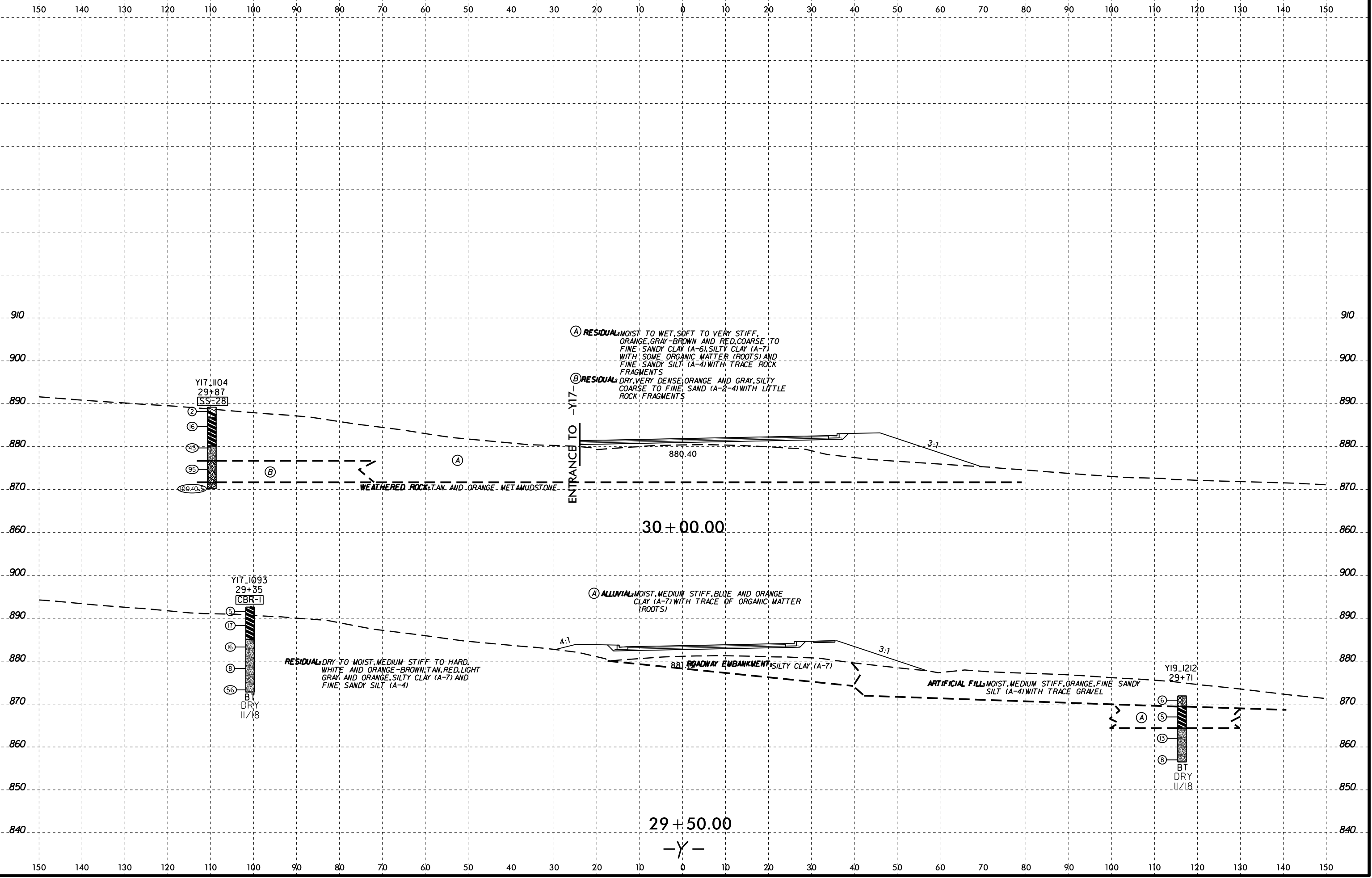
Y. 2700
 27+00
 Y. 2717
 27+17
 (46)
 (28)
 (100/0.5)
 (60/0.0)
 (60/0.0)
 (60/0.0)
 (60/0.0)
 DRY
 11/18
 DRY
 11/18
 CRYSTALLINE ROCK: METAMUDSTONE

27 + 00.00
 -Y-

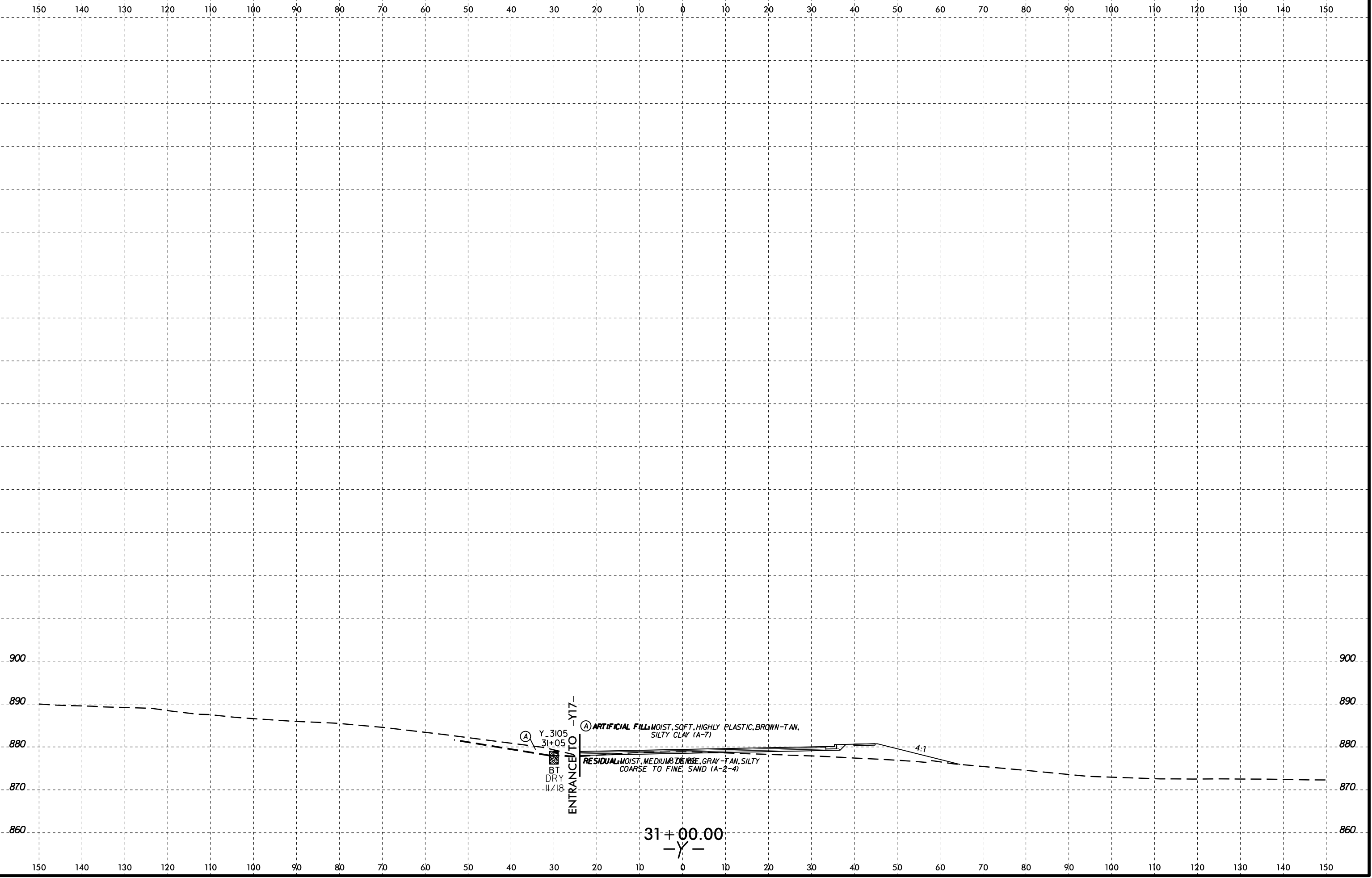
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6/23/16



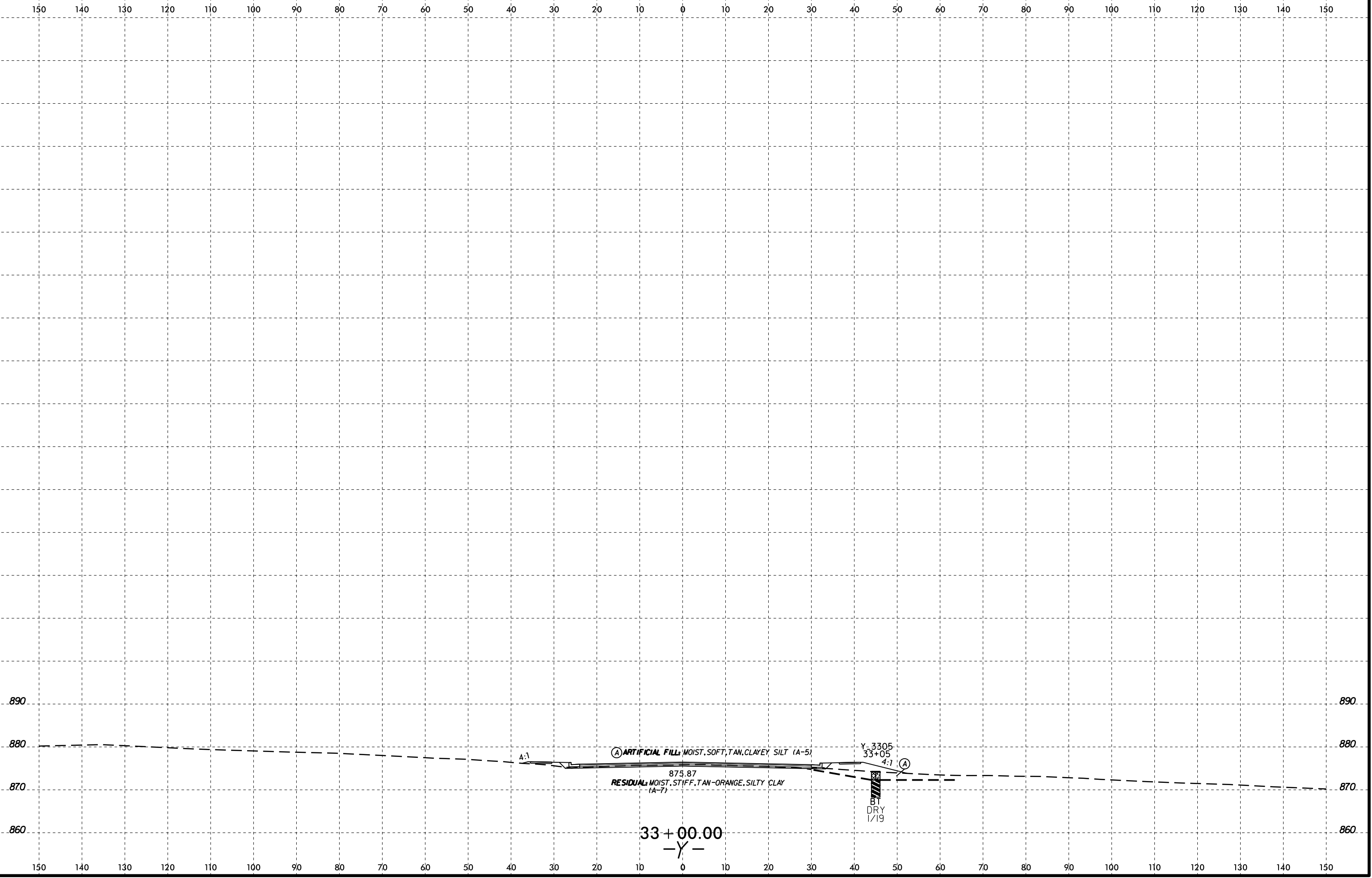
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6/23/16



6/23/16
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SPopke AT K&20916



6/23/16
14-OCT-2019 17:48
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SPopke AT K&209546



890
880
870
860

150 140 130 120 110 100 90 80 70 60 50 40 30 20 10 0 10 20 30 40 50 60 70 80 90 100 110 120 130 140 150

4:1

(A) ARTIFICIAL FILL; MOIST, SOFT, TAN, CLAYEY SILT (A-5)

875.87

RESIDUAL MOIST, STIFF, TAN-ORANGE, SILTY CLAY (A-7)

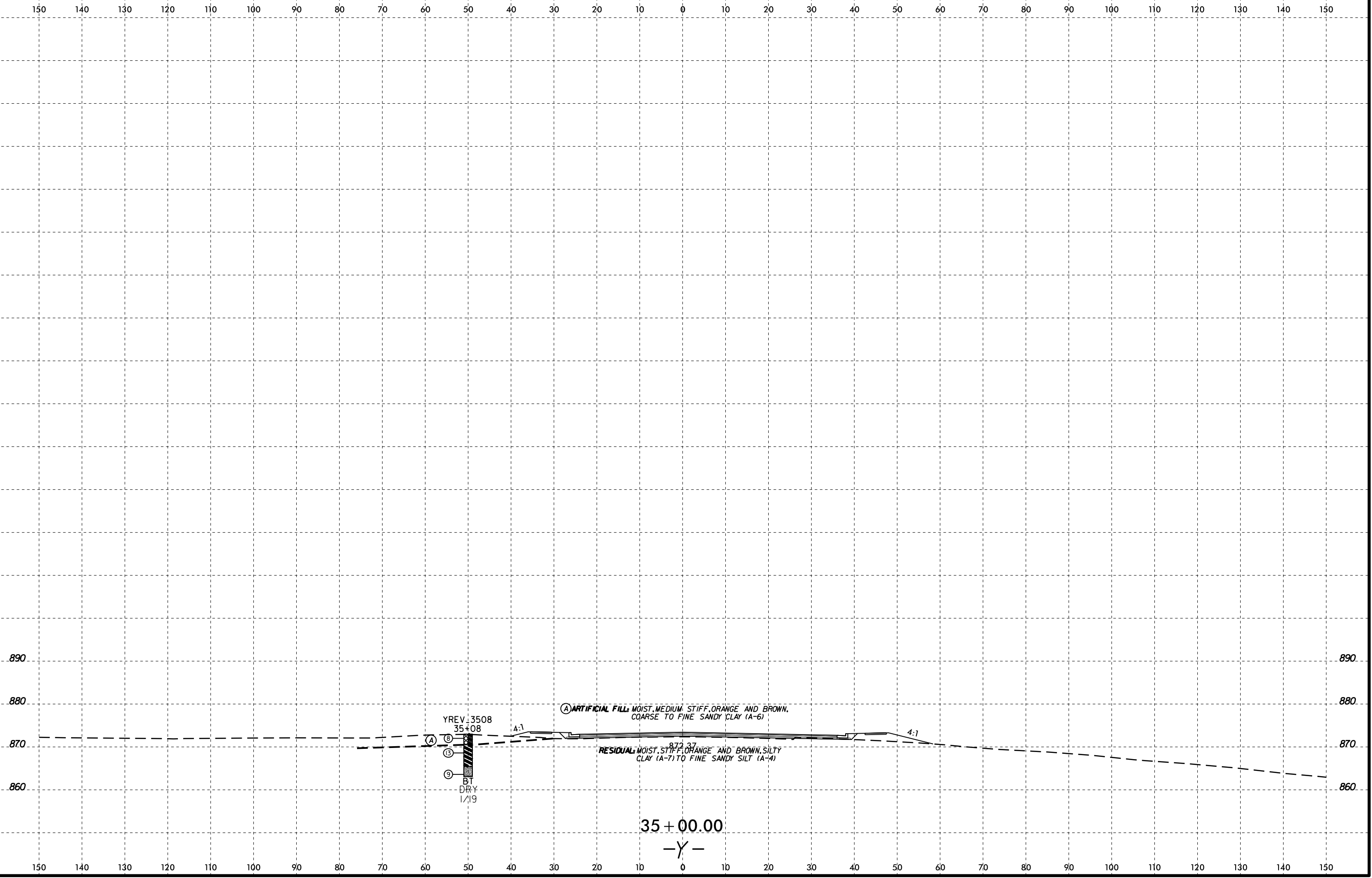
33+00.00

33+05.00

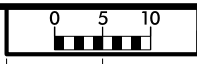
4:1

BT DRY 1/19

6/23/16
14-OCT-2019 17:48
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SPopke AT K&20516

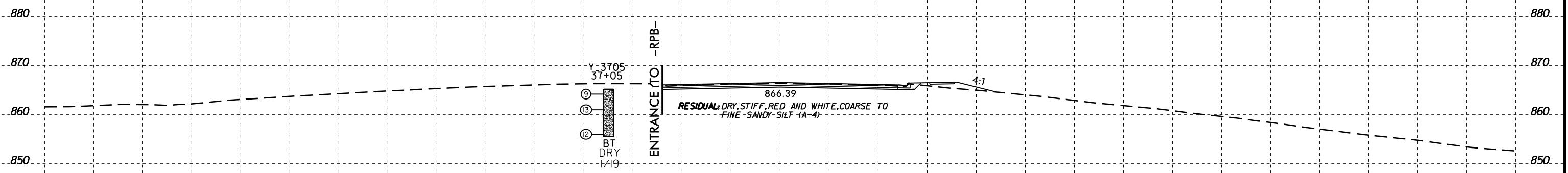


6/23/16



PROJ. REFERENCE NO.	SHEET NO.
U-5813	84

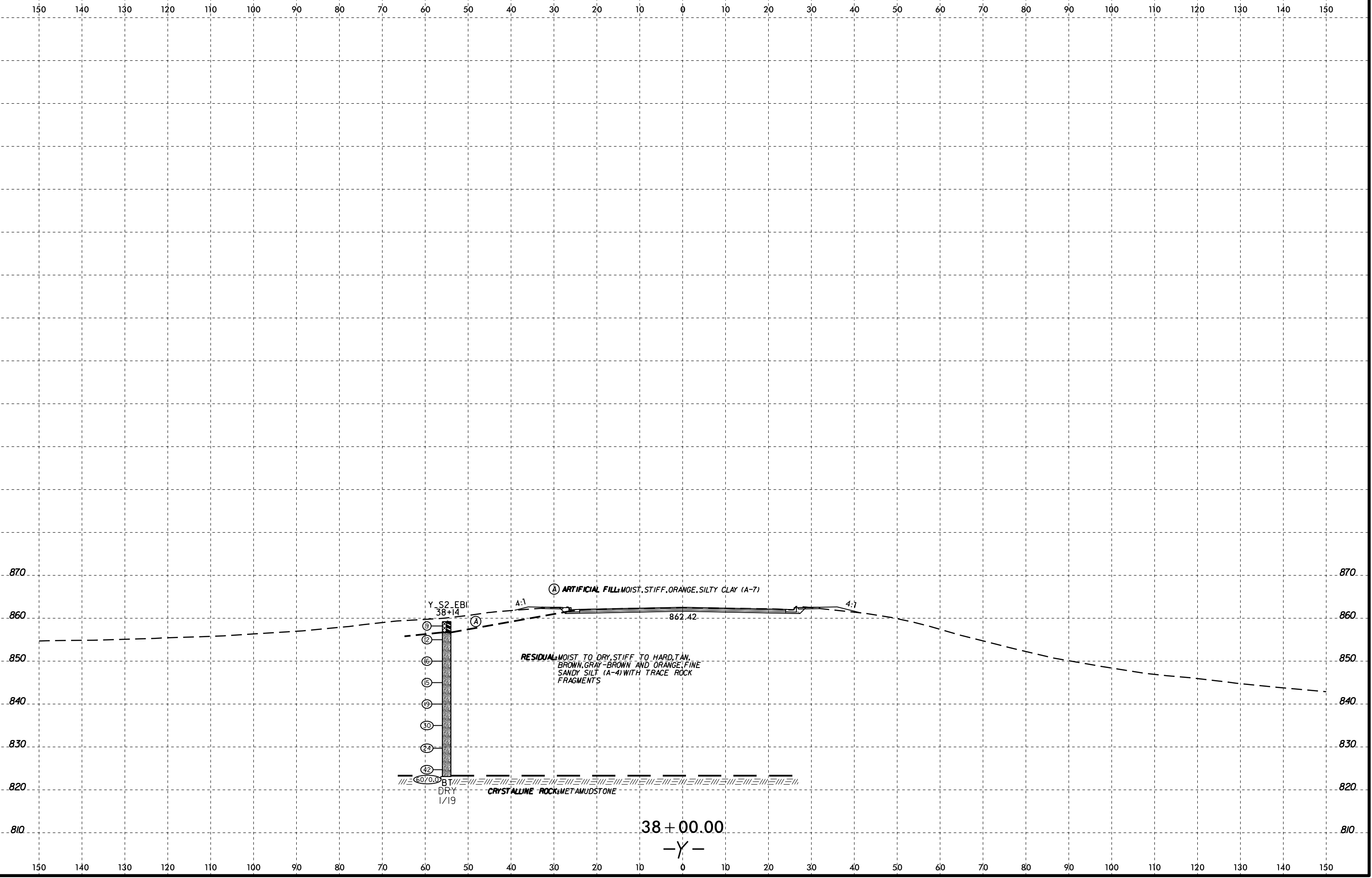
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37 + 00.00

-Y-

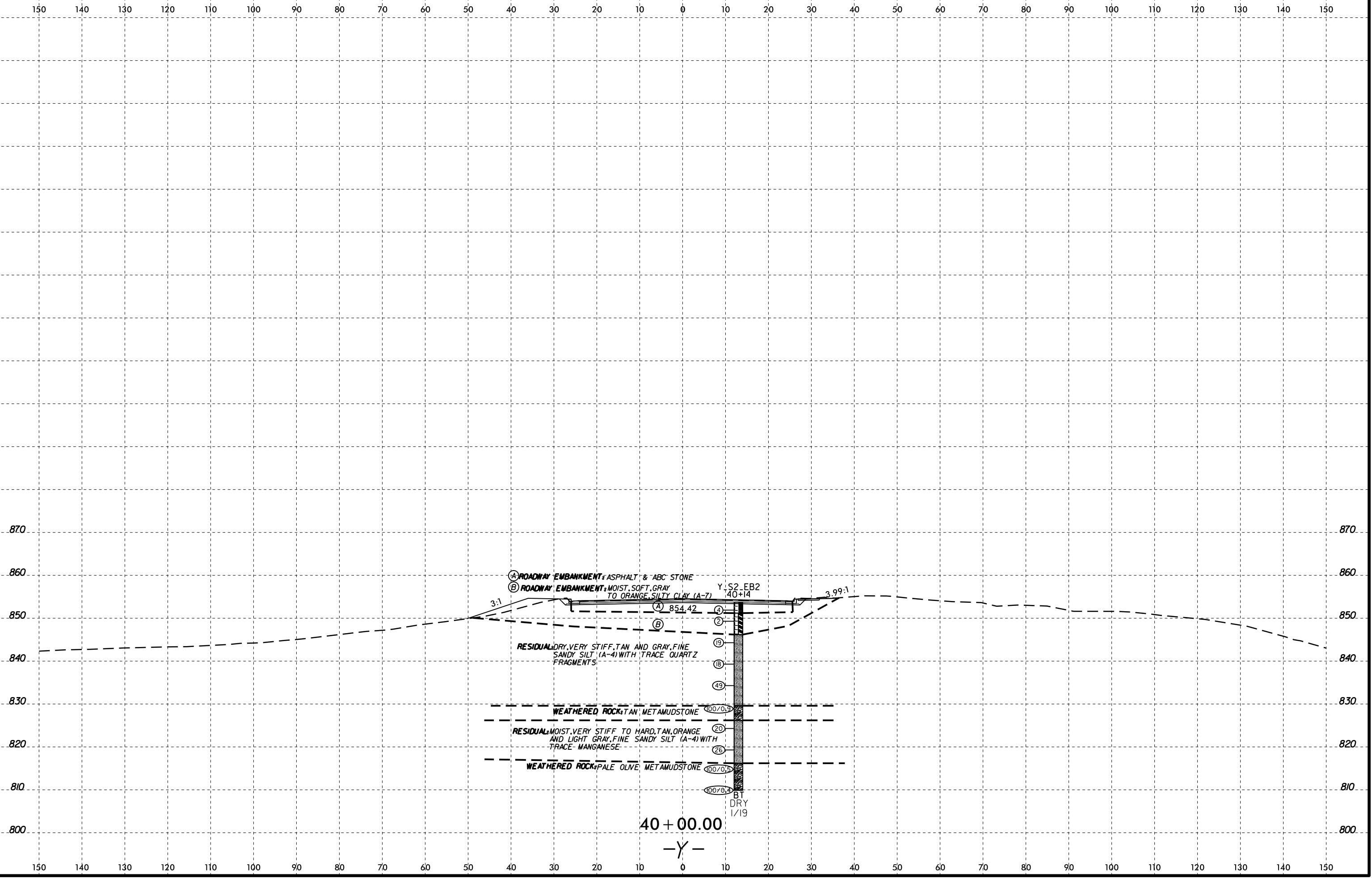
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SPopke AT K6209516



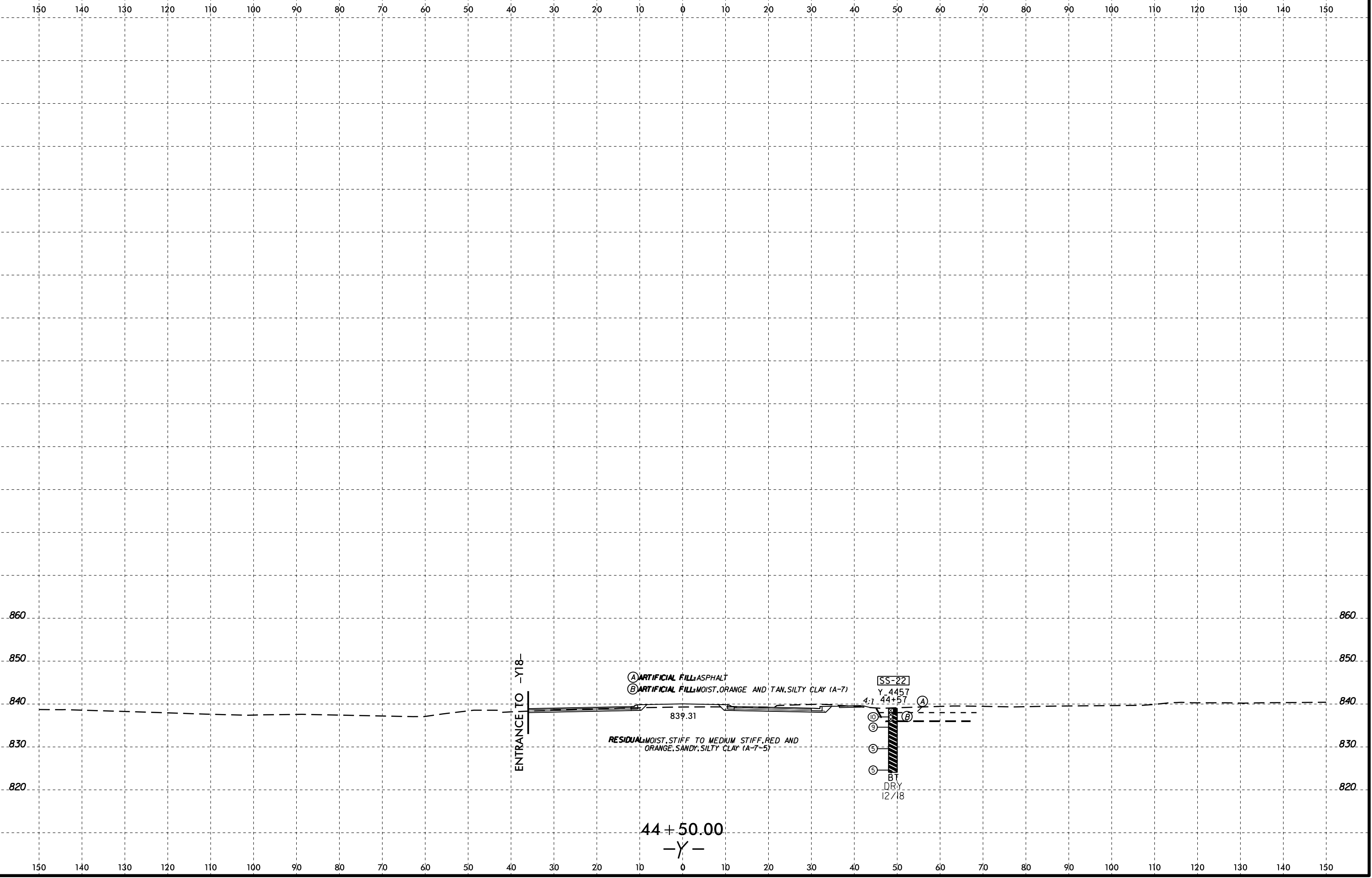
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 Spoke AT K620946

38 + 00.00
-Y-

6/23/16
14-OCT-2019 17:48
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S:\pape AT K620916



14-OCT-2019 17:48
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6/23/16



(A) ARTIFICIAL FILL ASPHALT
(B) ARTIFICIAL FILL MOIST, ORANGE AND TAN, SILTY CLAY (A-7)

RESIDUAL MOIST, STIFF TO MEDIUM STIFF, RED AND ORANGE, SANDY, SILTY CLAY (A-7-5)

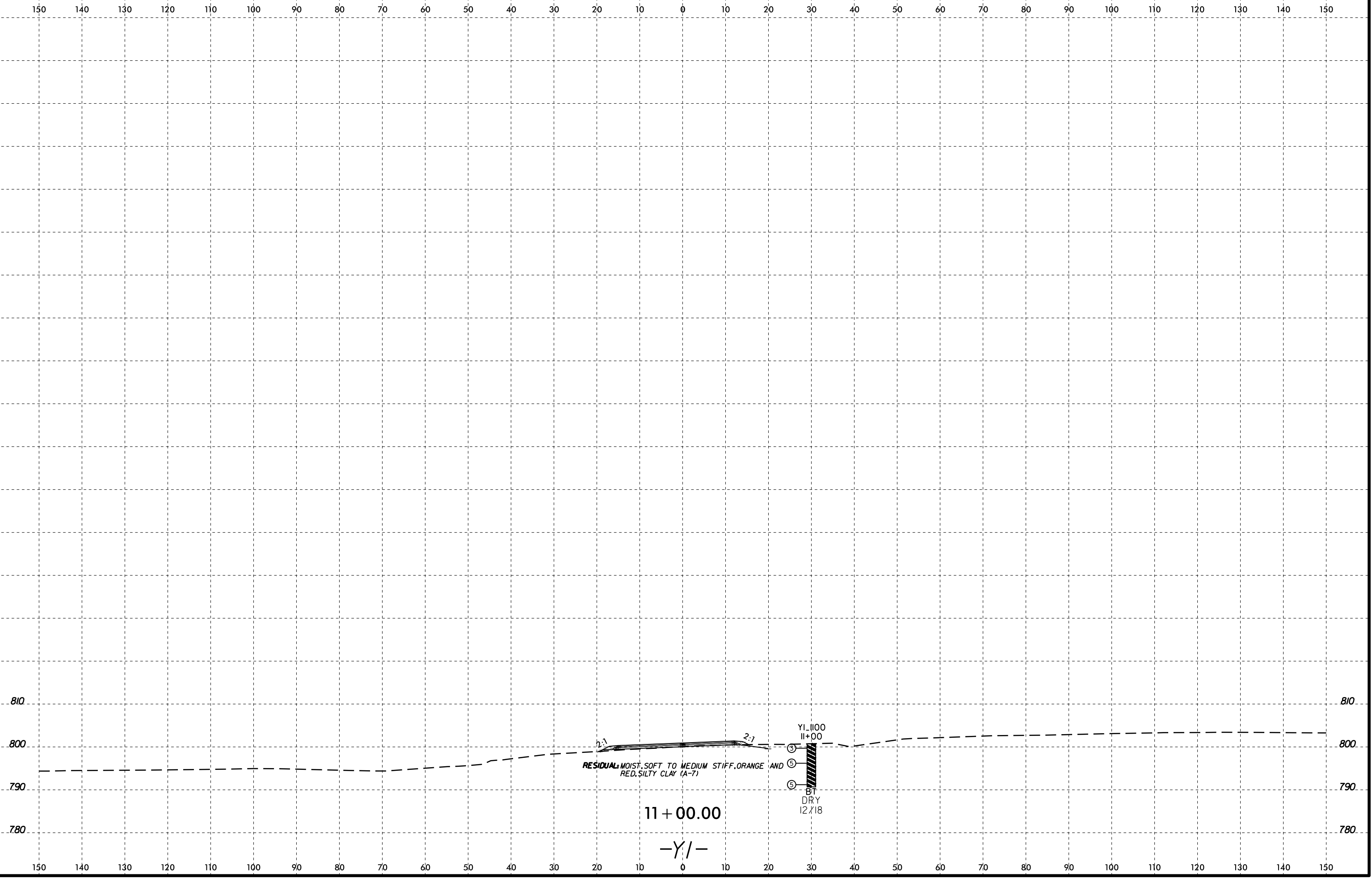
SS-22
Y 4457
4:1 44+51
BT
DRY
12/18

ENTRANCE TO -Y18-

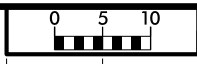
44 + 50.00
-Y-

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Spoke AT K6209516

0 5 10	PROJ. REFERENCE NO.	SHEET NO.
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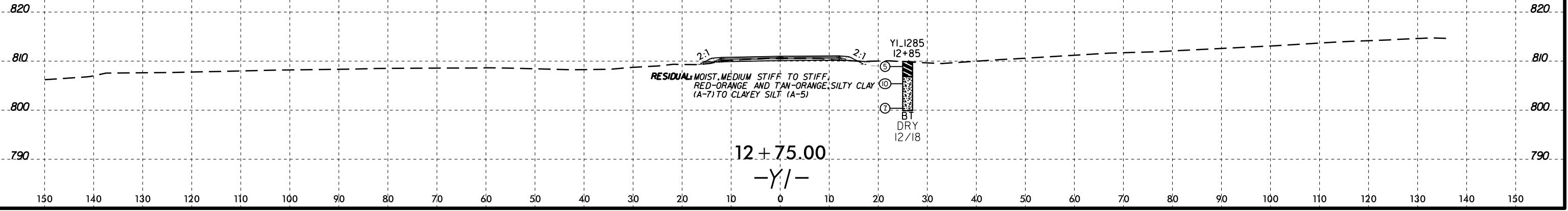
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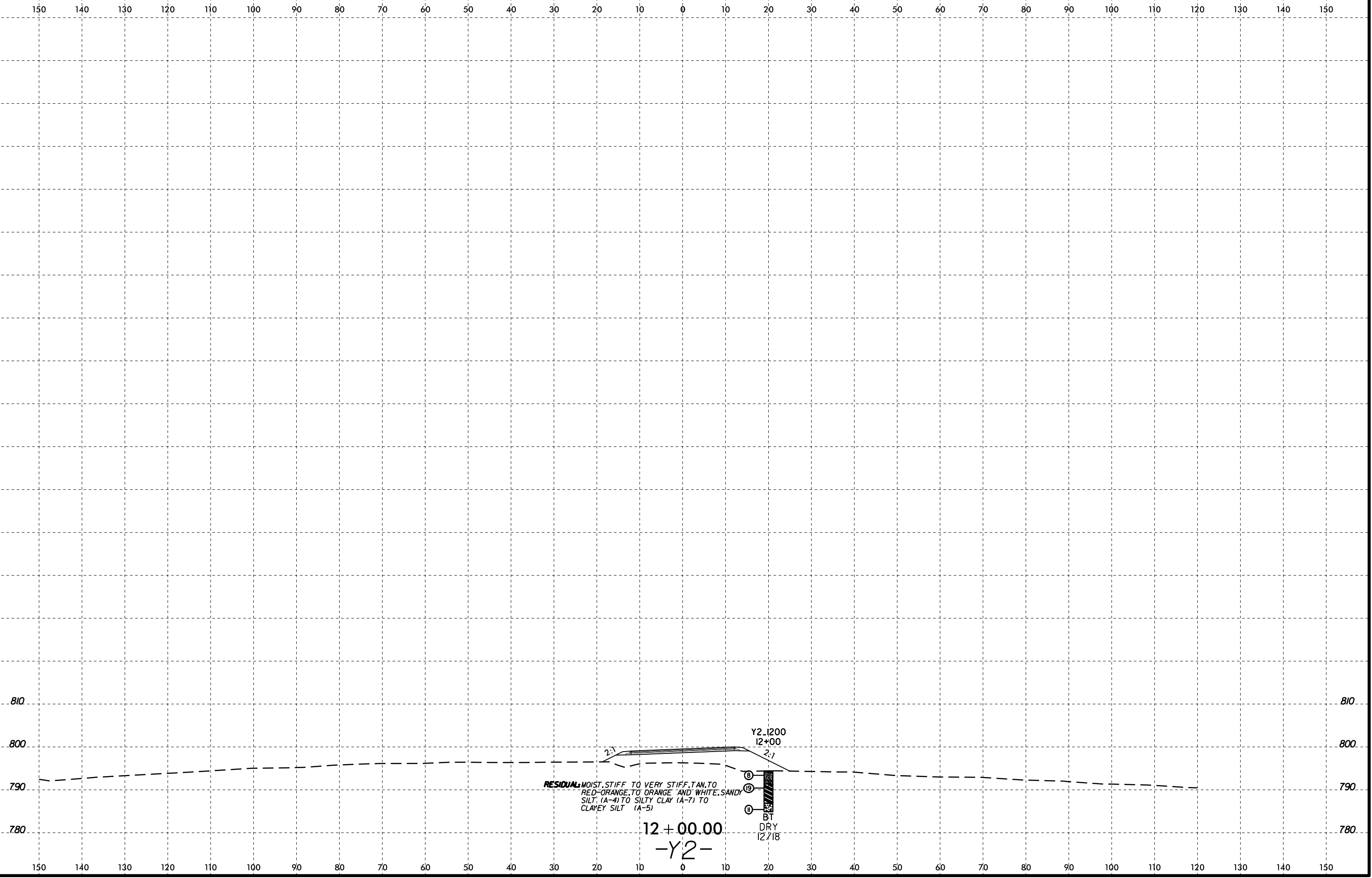
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 Spoke AT K6209516



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SPopke AT K&20516

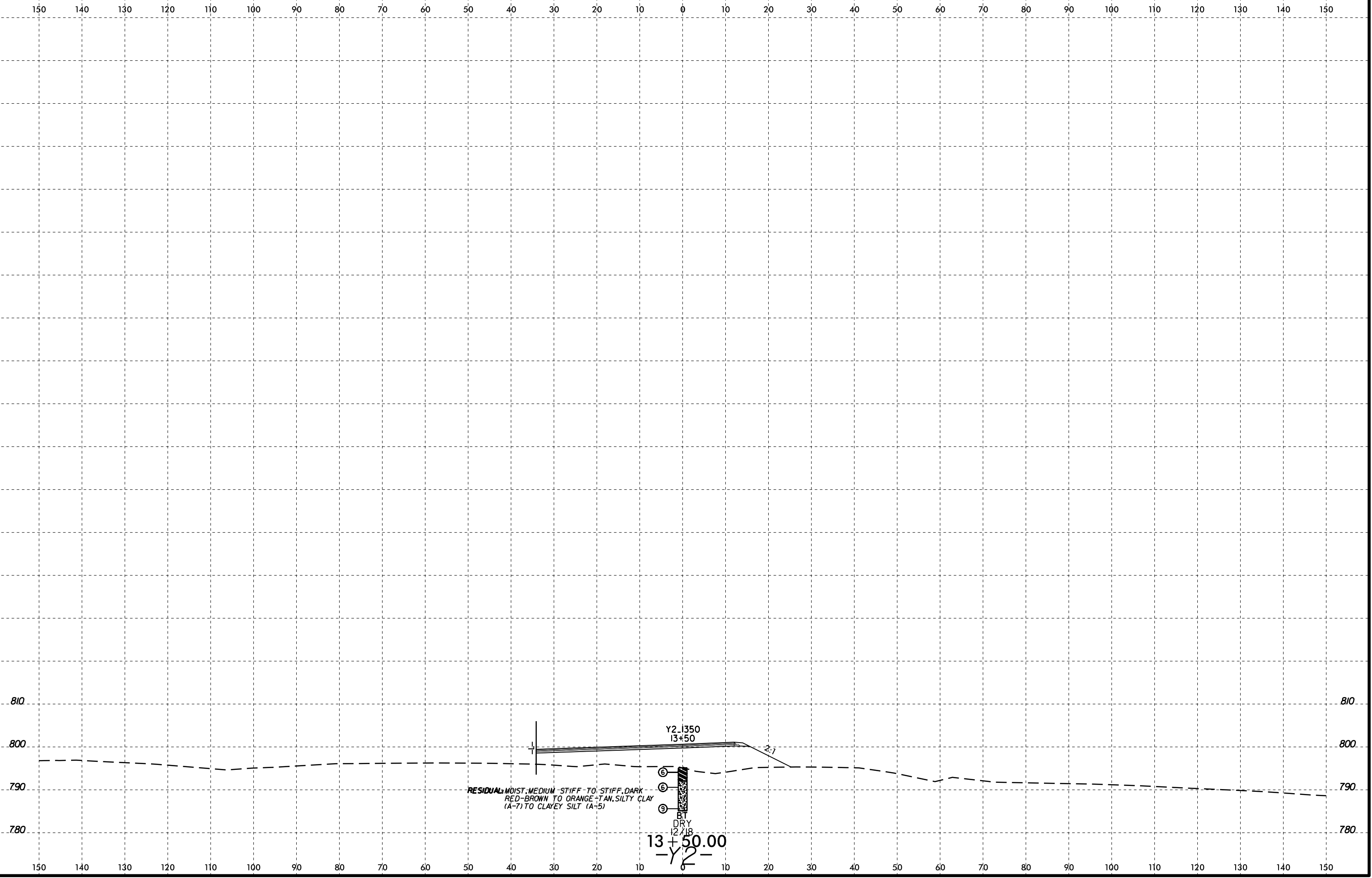


RESIDUAL MOIST, STIFF TO VERY STIFF, TAN TO RED-ORANGE TO ORANGE AND WHITE, SANDY SILT (A-4) TO SILTY CLAY (A-7) TO CLAYEY SILT (A-5)

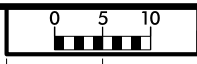
12 + 00.00
-Y2-

Y2.1200
12+00
BT
DRY
12.718

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SPopke AT K&20516



6/23/16



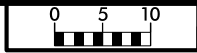
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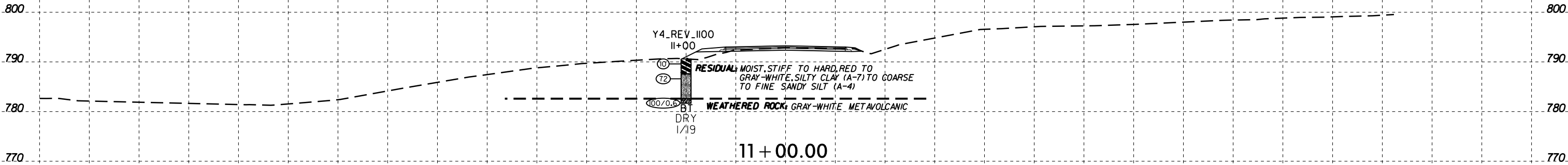


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Spoke AT K209516

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150 140 130 120 110 100 90 80 70 60 50 40 30 20 10 0 10 20 30 40 50 60 70 80 90 100 110 120 130 140 150



Y4_REV_1100
11+00

(10)

(72)

100/0.6/24

BT

DRY

1/19

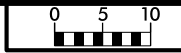
RESIDUAL MOIST STIFF TO HARD, RED TO GRAY-WHITE, SILTY CLAY (A-7) TO COARSE TO FINE SANDY SILT (A-4)

WEATHERED ROCK, GRAY-WHITE, METAVOLCANIC

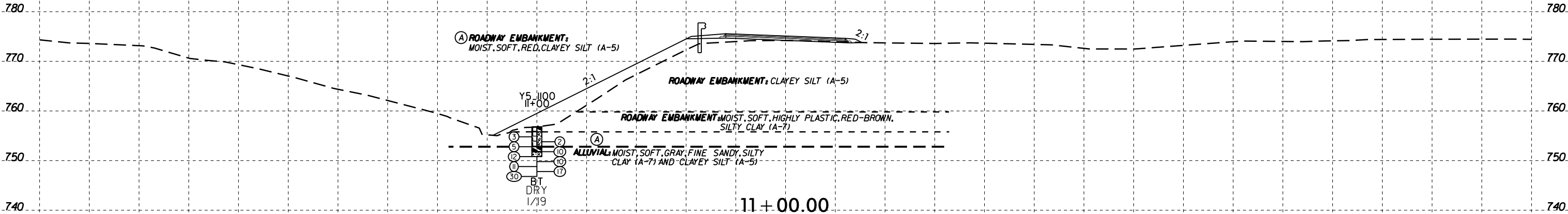
11 + 00.00

-Y4_REV-

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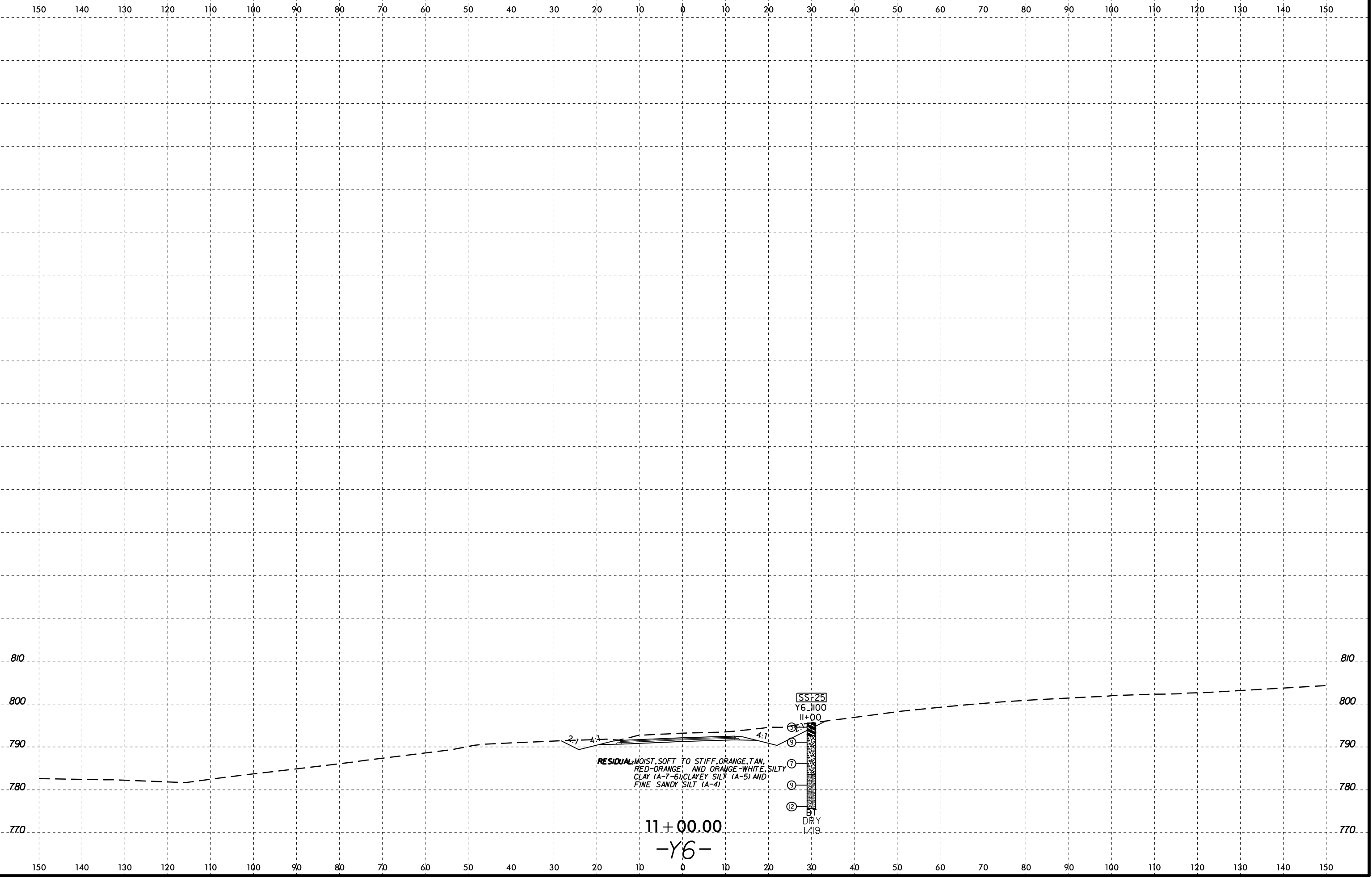


150 140 130 120 110 100 90 80 70 60 50 40 30 20 10 0 10 20 30 40 50 60 70 80 90 100 110 120 130 140 150



11+00.00
-Y5-

6/23/16
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SPopke AT K&209516

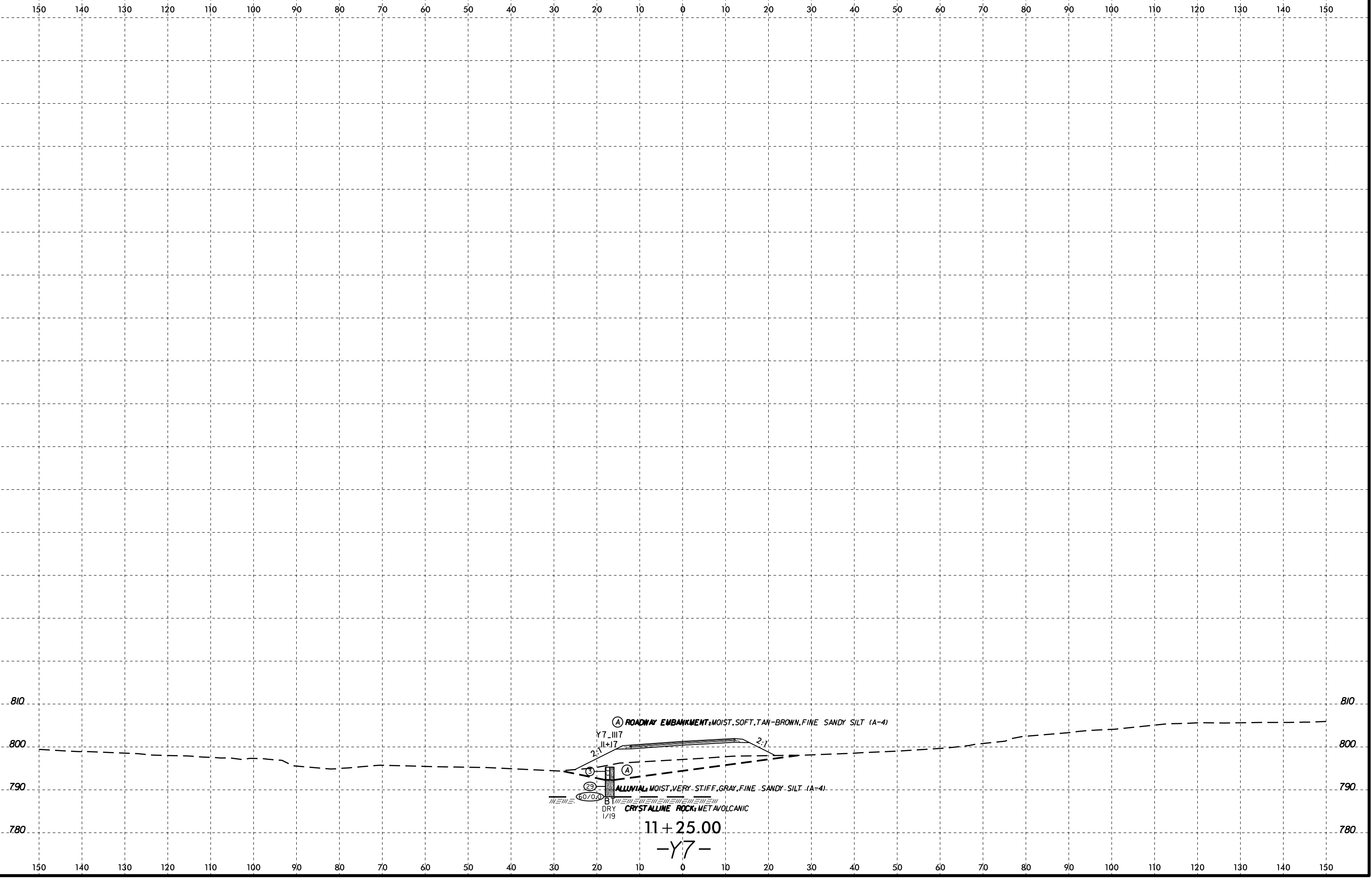


RESIDUAL MOIST, SOFT TO STIFF, ORANGE, TAN,
RED-ORANGE, AND ORANGE-WHITE, SILTY
CLAY (A-7-6), CLAYEY SILT (A-5) AND
FINE SANDY SILT (A-4)

11 + 00.00
-Y6-

SS+25
Y6.1100
11+00
BT
DRY
L/19

6/23/16
14-OCT-2019 17:49
W:\share\GEO\TECHNICAL\Projects\Active Projects\20190942.009A U-5813 Roadway\U5813.GEO.ROWY\CADD.GEOTECH\sec\U5813.GEO_Y7_XSI.dgn
SPopke AT K208516



(A) ROADWAY EMBANKMENT; MOIST, SOFT, TAN-BROWN, FINE SANDY SILT (A-4)

Y7-III7
IL+I7

2:1

(A)

ALLUVIAL MOIST, VERY STIFF, GRAY, FINE SANDY SILT (A-4)

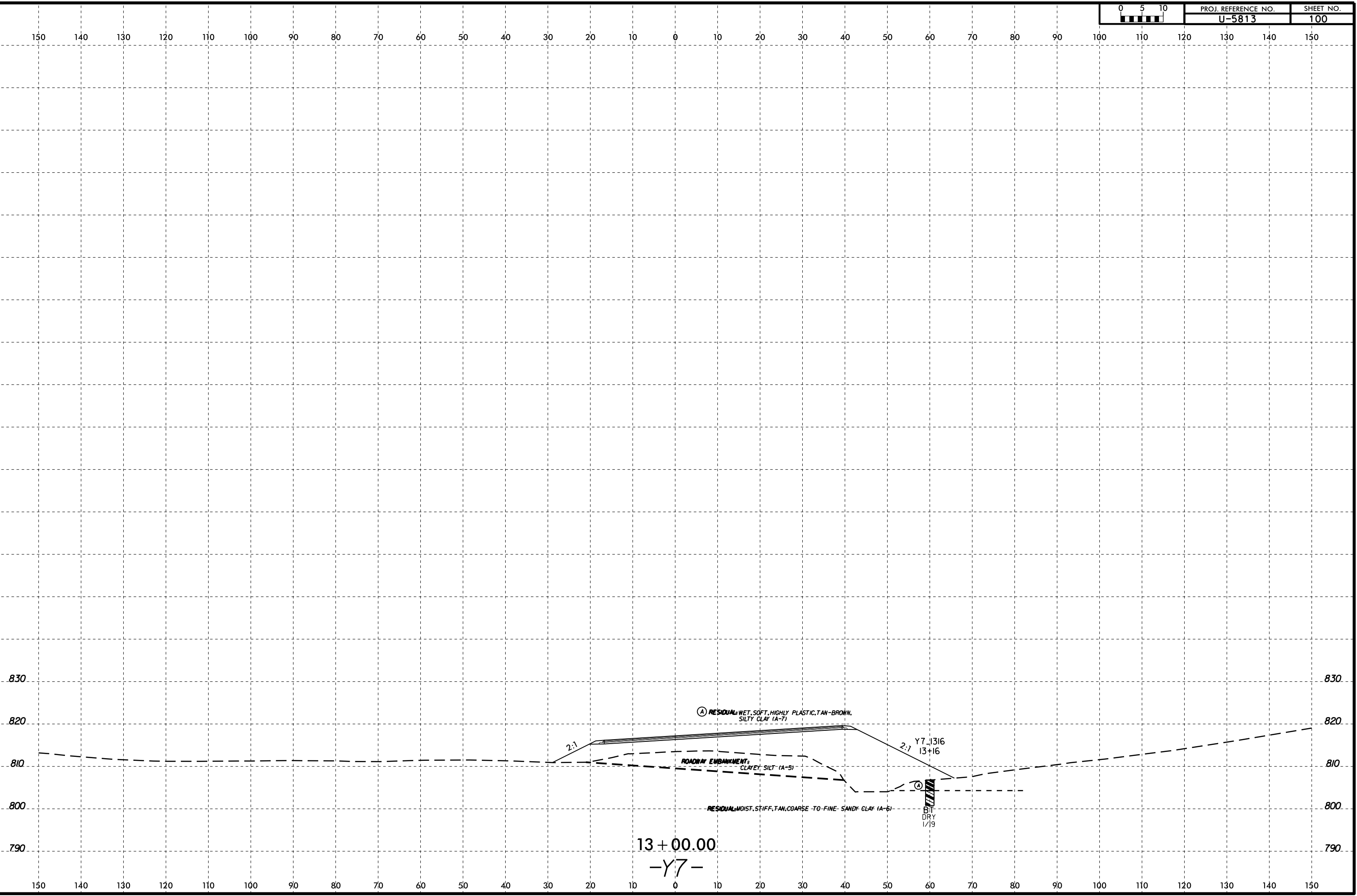
CRYSTALLINE ROCK, METAVOLCANIC
DRY 1/19

11+25.00

-Y7-

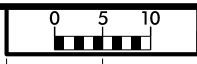
6070.0

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SPopke AT K209516



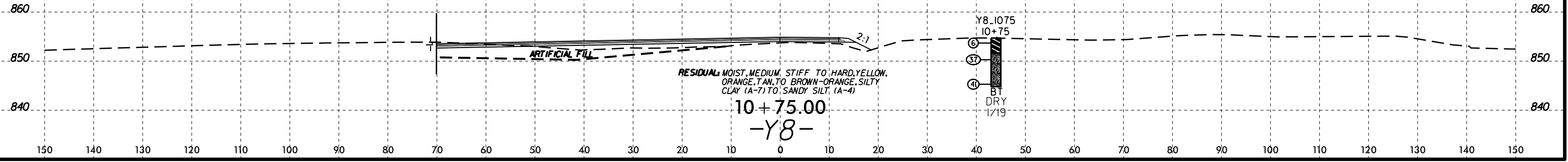
13+00.00
-Y7-

6/23/16



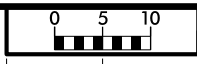
PROJ. REFERENCE NO.	SHEET NO.
U-5813	101

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SPopke AT K208516

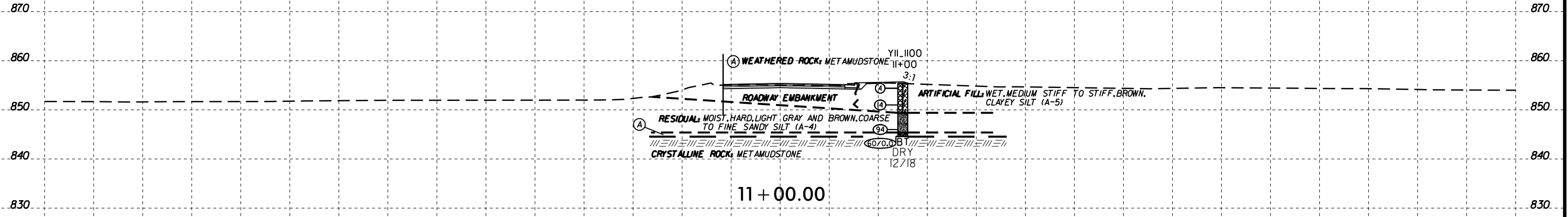
6/23/16



PROJ. REFERENCE NO.
U-5813

SHEET NO.
102

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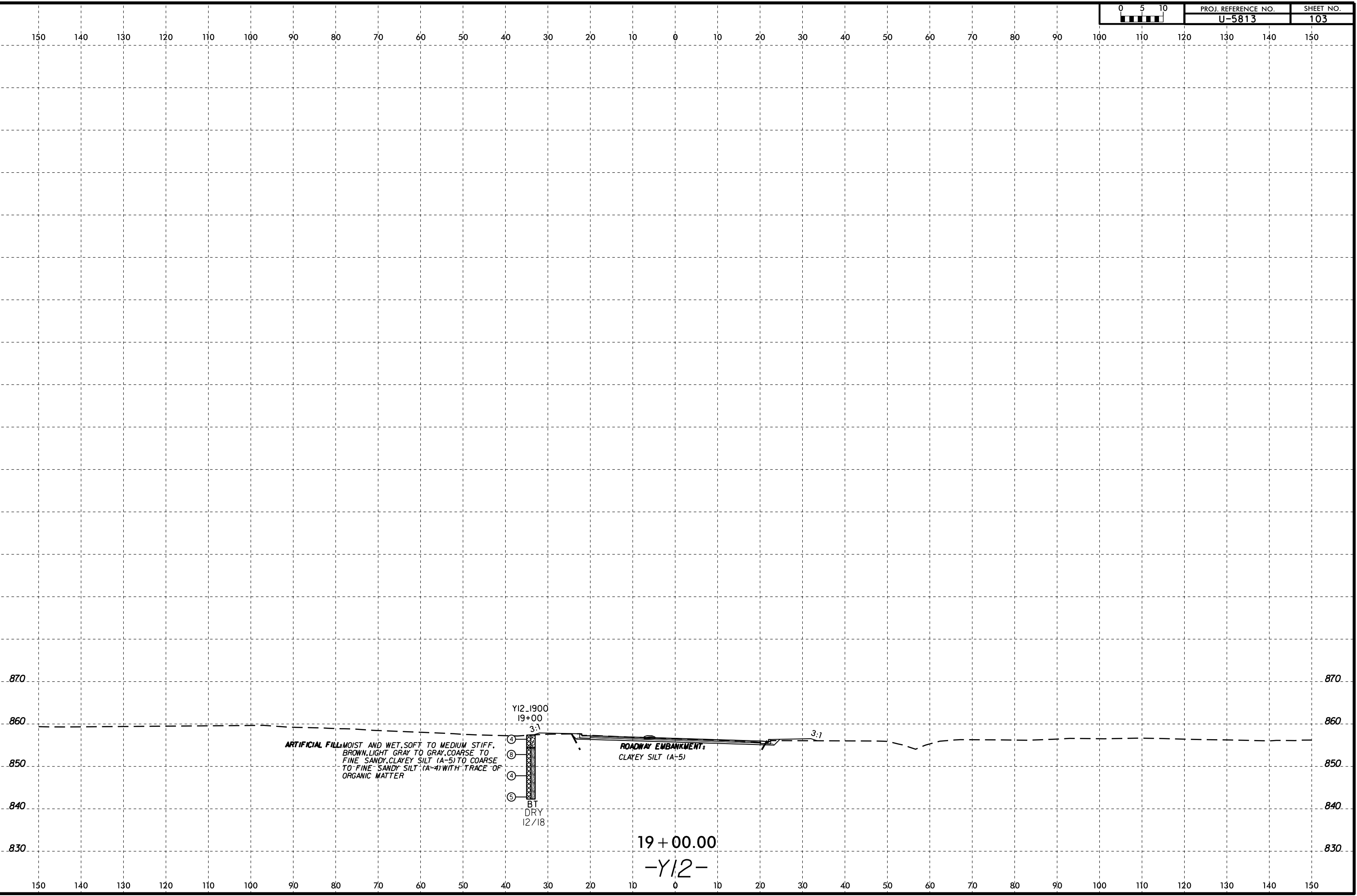


11 + 00.00

-Y//-

14-OCT-2019 17:50
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Spoke AT K6209516

6/23/16
14-OCT-2019 17:50
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Sheet AT K6209516



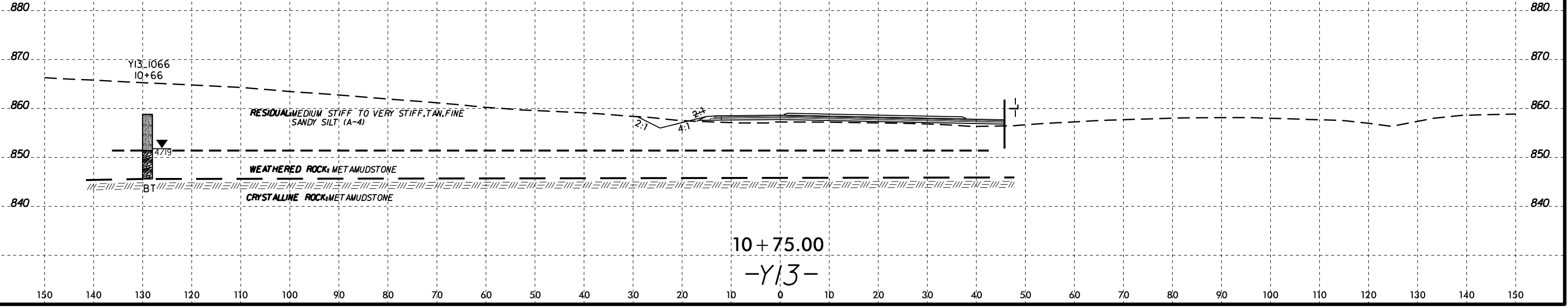
ARTIFICIAL FILL: MOIST AND WET, SOFT TO MEDIUM STIFF,
BROWN, LIGHT GRAY TO GRAY, COARSE TO
FINE SANDY CLAYEY SILT (A-5) TO COARSE
TO FINE SANDY SILT (A-4) WITH TRACE OF
ORGANIC MATTER

ROADWAY EMBANKMENT:
CLAYEY SILT (A-5)

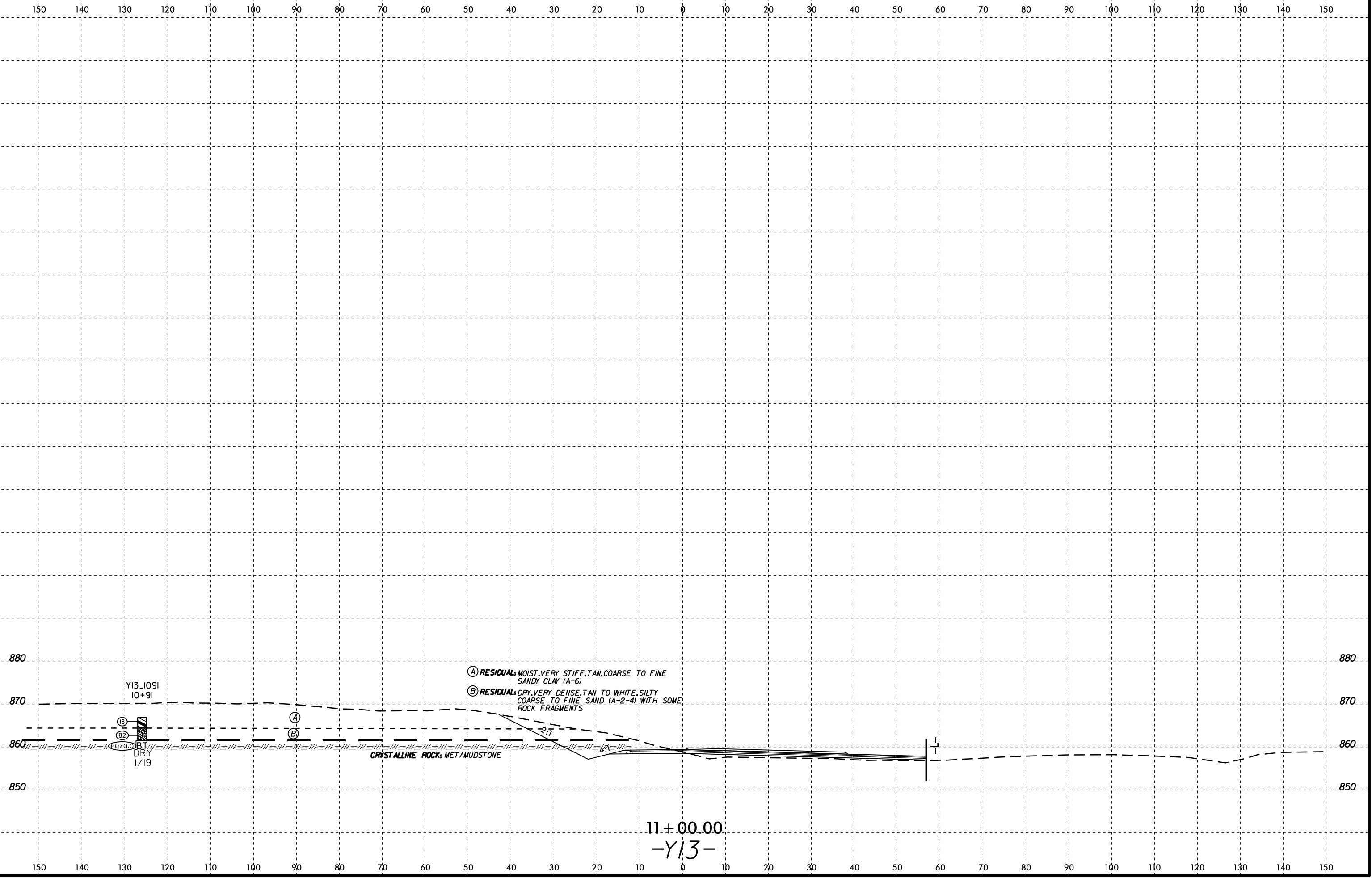
Y12_1900
19+00
3:1
④
③
④
⑤
BT
DRY
12/18

19 + 00.00
-Y12-

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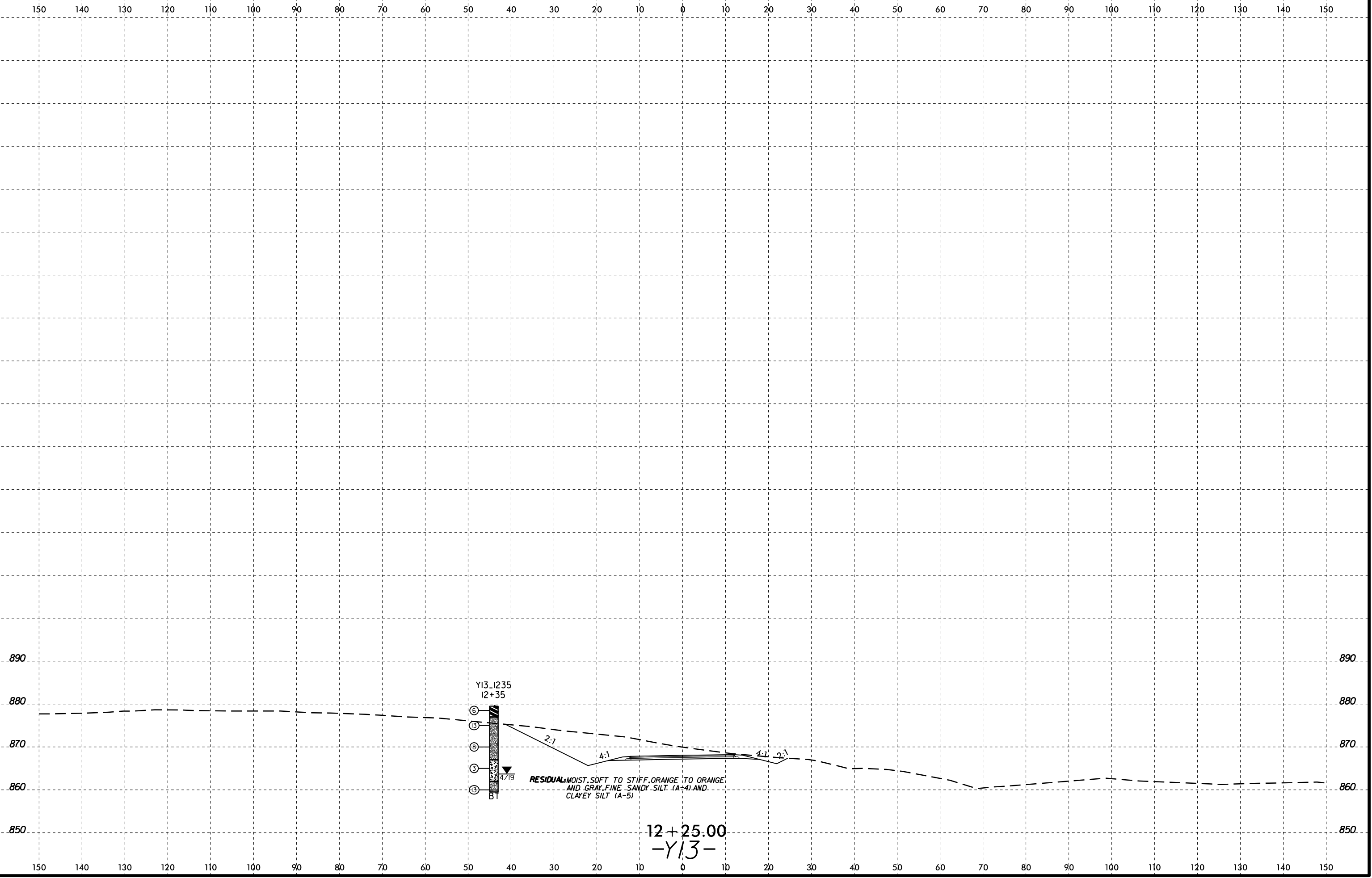


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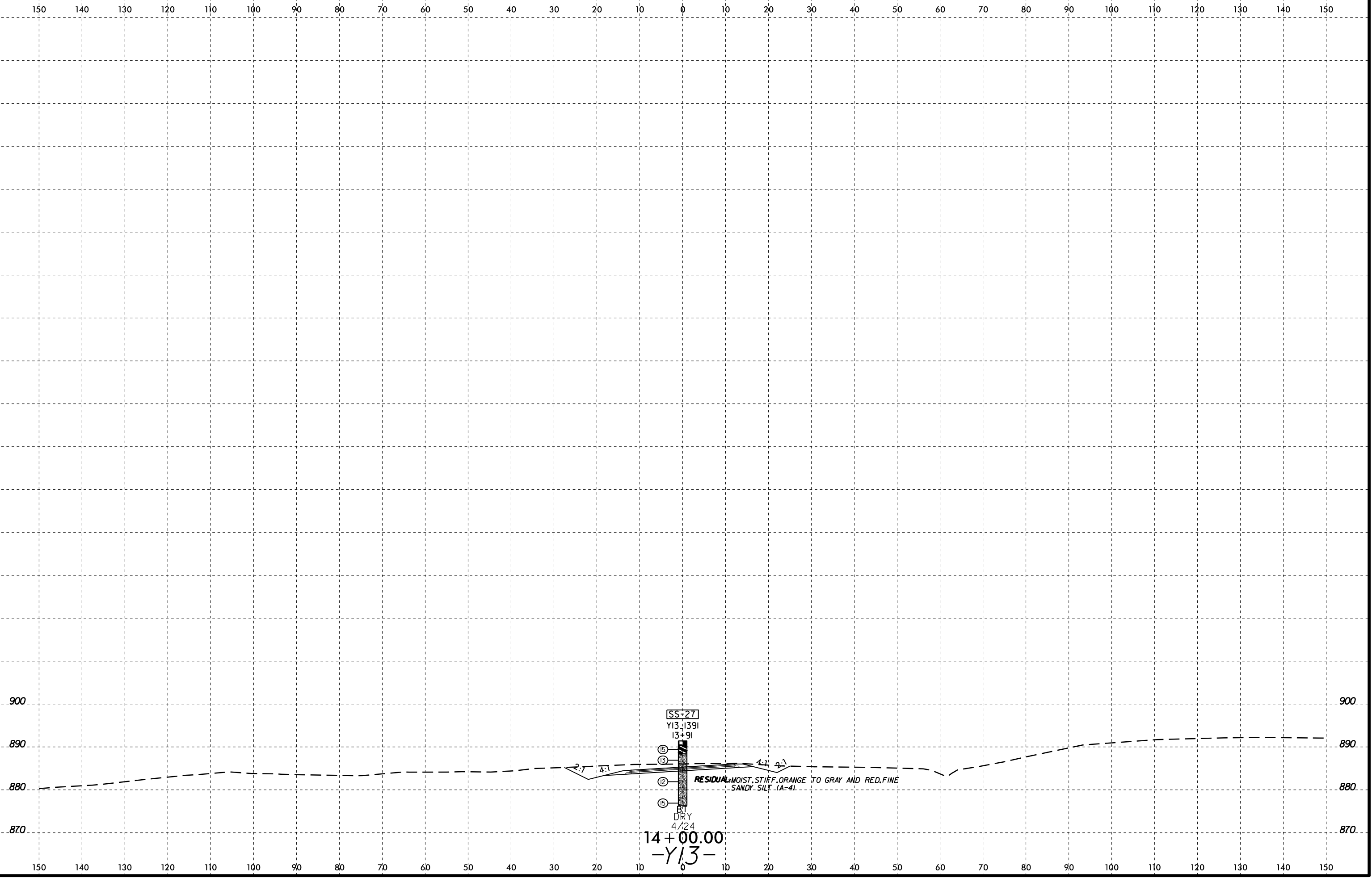


11+00.00
-Y13-

6/23/16
14-OCT-2019 17:50
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SPopke AT K&208516

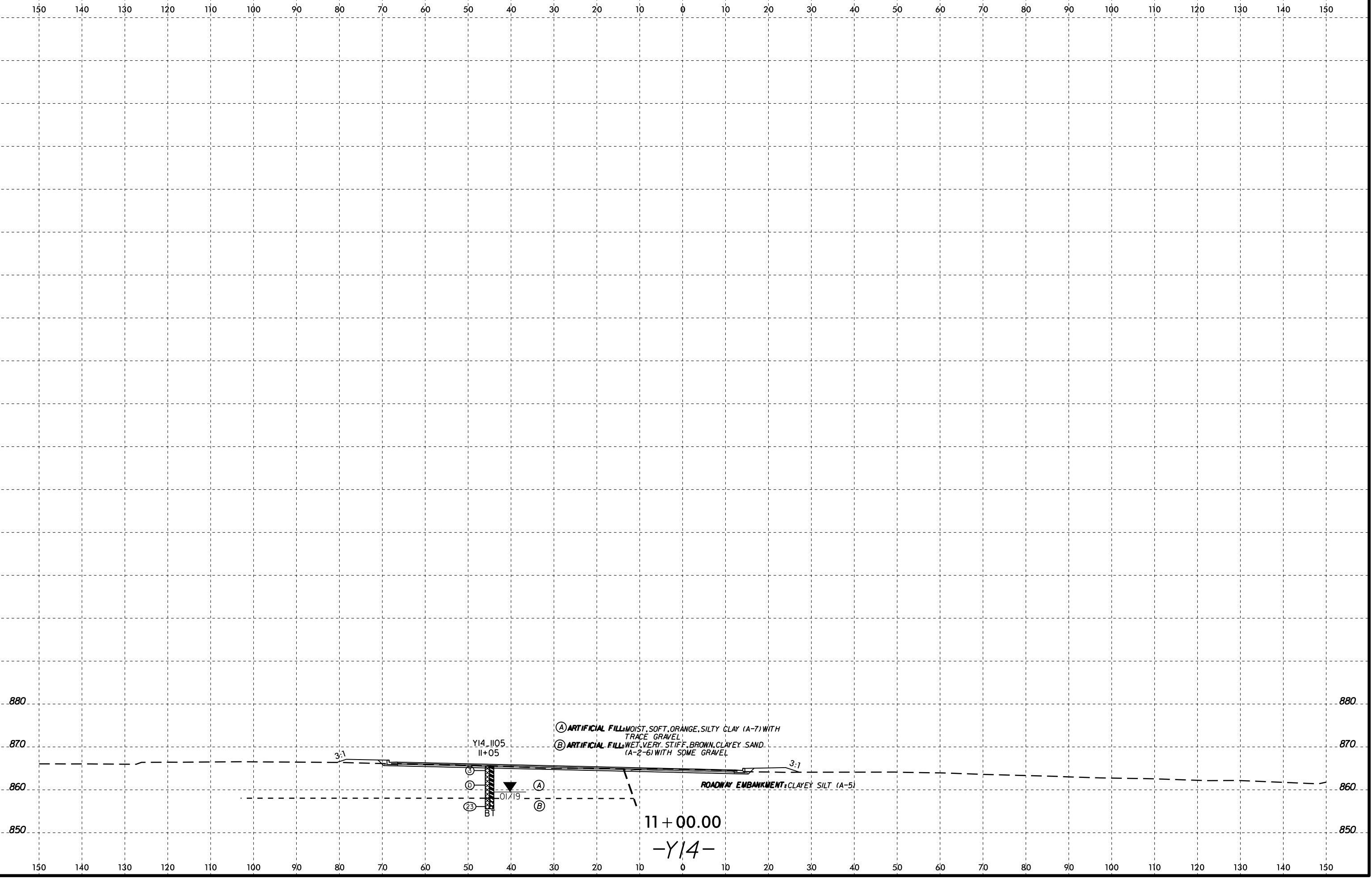


6/23/16
14-OCT-2019 17:50
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S:\pape AT K209516

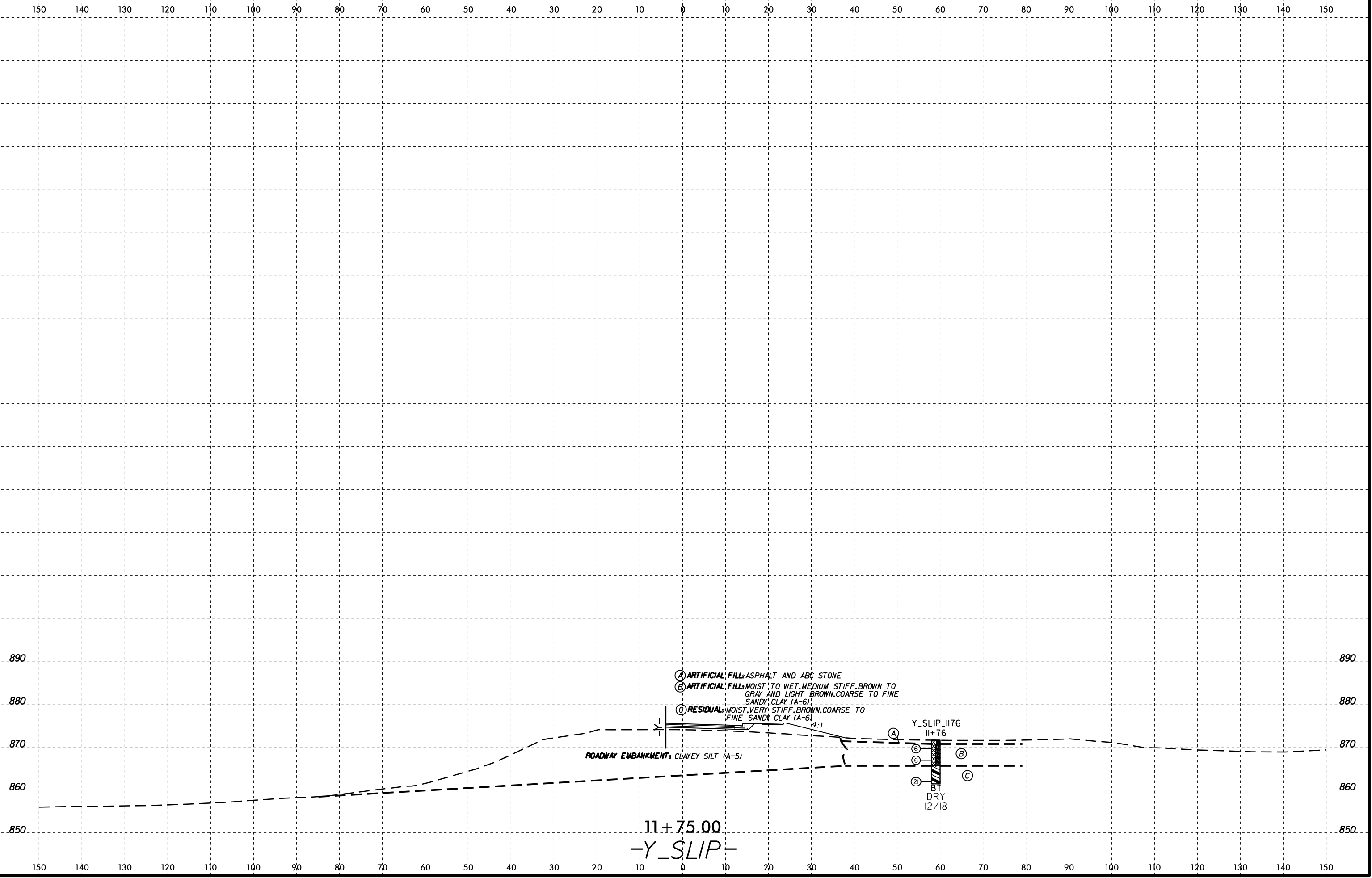


SS-27
Y13.1391
13+91
15
13
12
15
RESIDUAL MOIST, STIFF, ORANGE TO GRAY AND RED, FINE SANDY SILT (A-4)
DRY
4/24
14+00.00
-Y13-

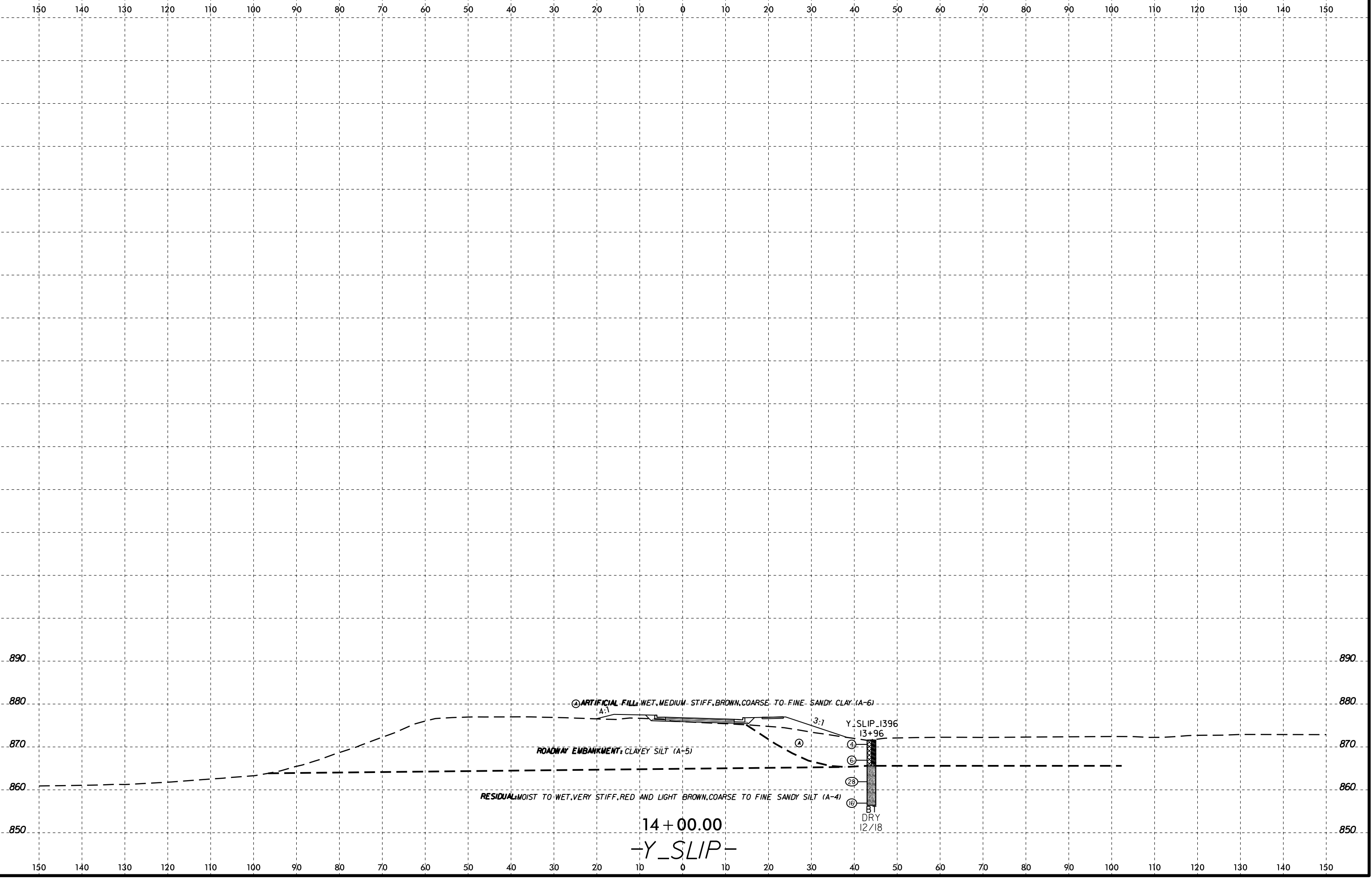
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SPopke AT K&B20516



14-OCT-2019 17:50
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6/23/16



6/23/16
14-OCT-2019 17:50
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Spoke AT K4209516



ARTIFICIAL FILL WET, MEDIUM STIFF, BROWN, COARSE TO FINE SANDY CLAY (A-6)

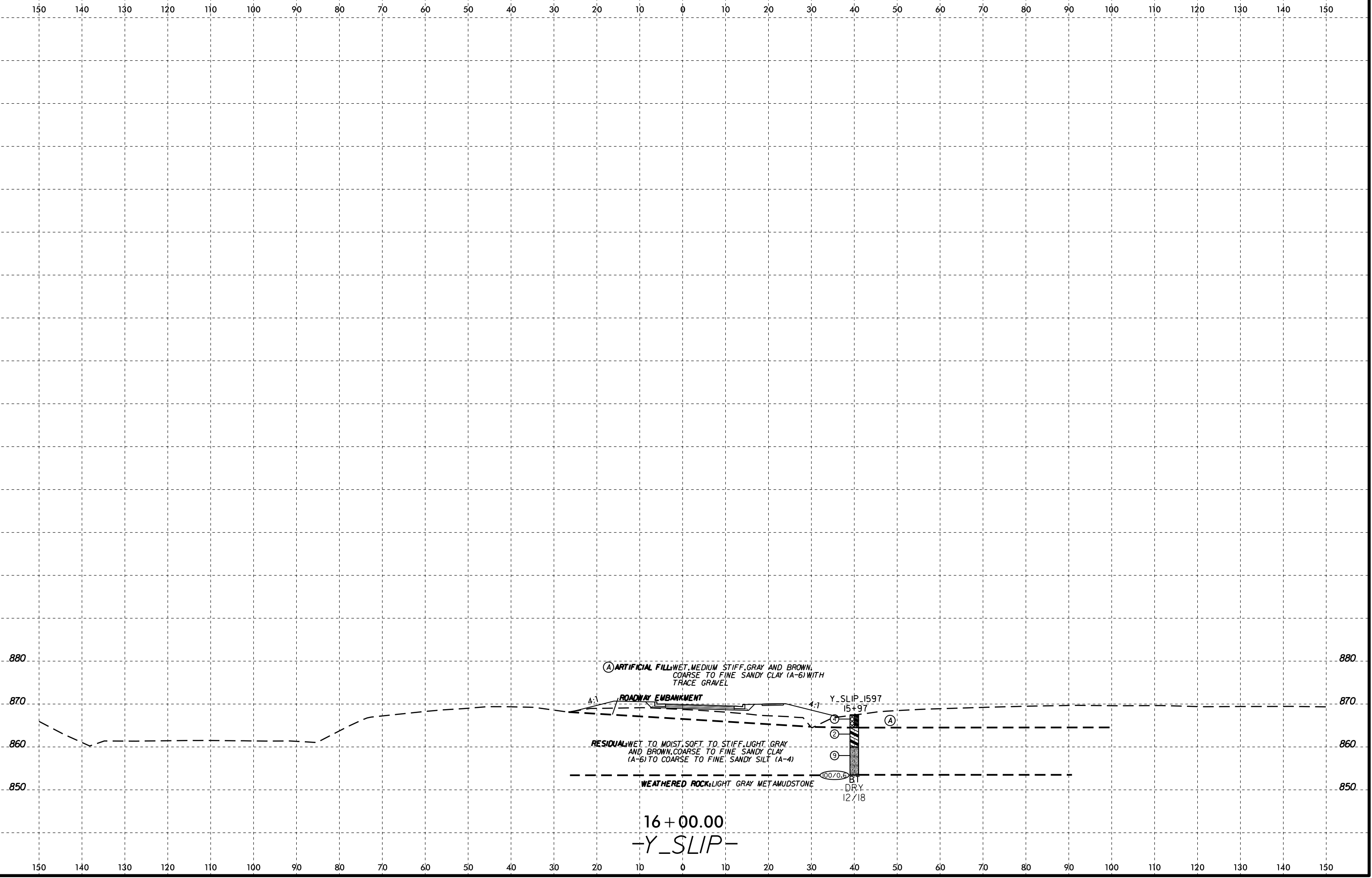
ROADWAY EMBANKMENT CLAYEY SILT (A-5)

RESIDUAL MOIST TO WET, VERY STIFF, RED AND LIGHT BROWN, COARSE TO FINE SANDY SILT (A-4)

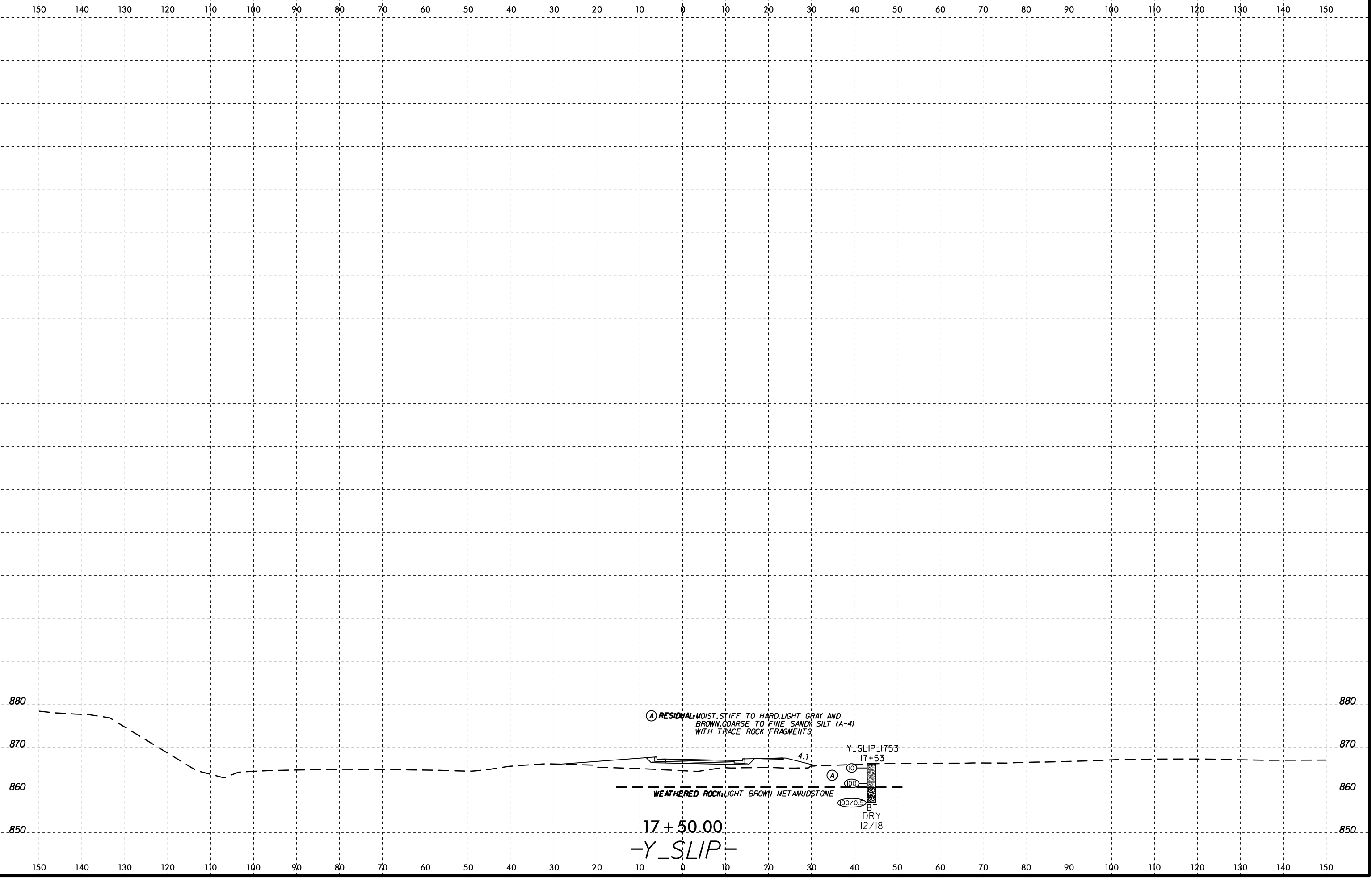
Y_SLIP_1396
13+96
4
6
28
16
BT
DRY
12/18

14+00.00
-Y_SLIP-

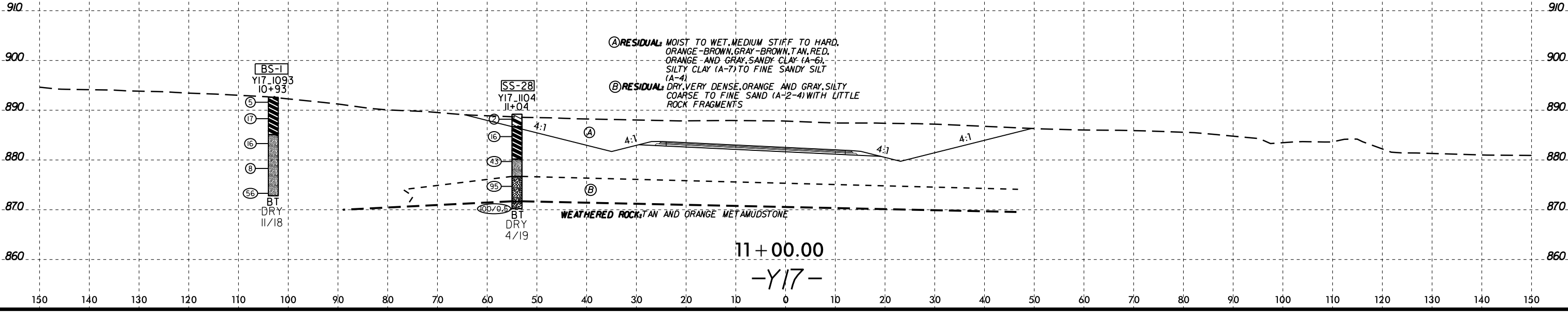
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14-OCT-2019 17:50
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Spoke AT K209516



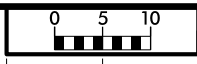
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Spoke AT K4209516



150 140 130 120 110 100 90 80 70 60 50 40 30 20 10 0 10 20 30 40 50 60 70 80 90 100 110 120 130 140 150

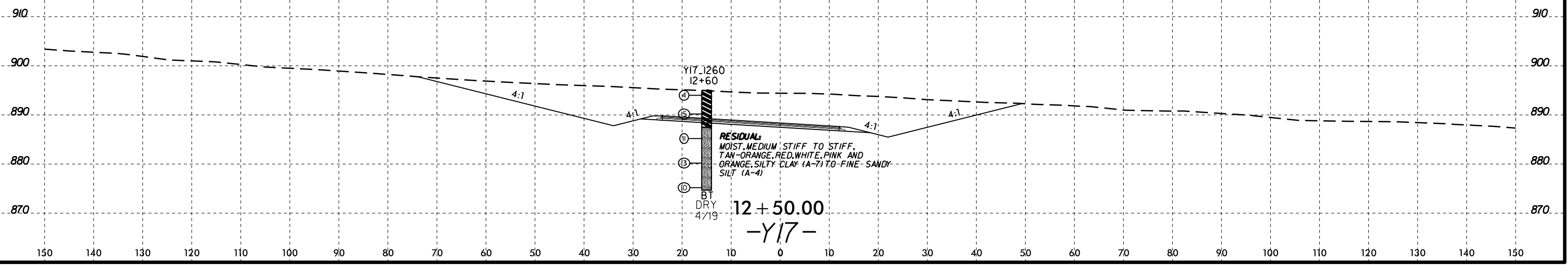


6/23/16



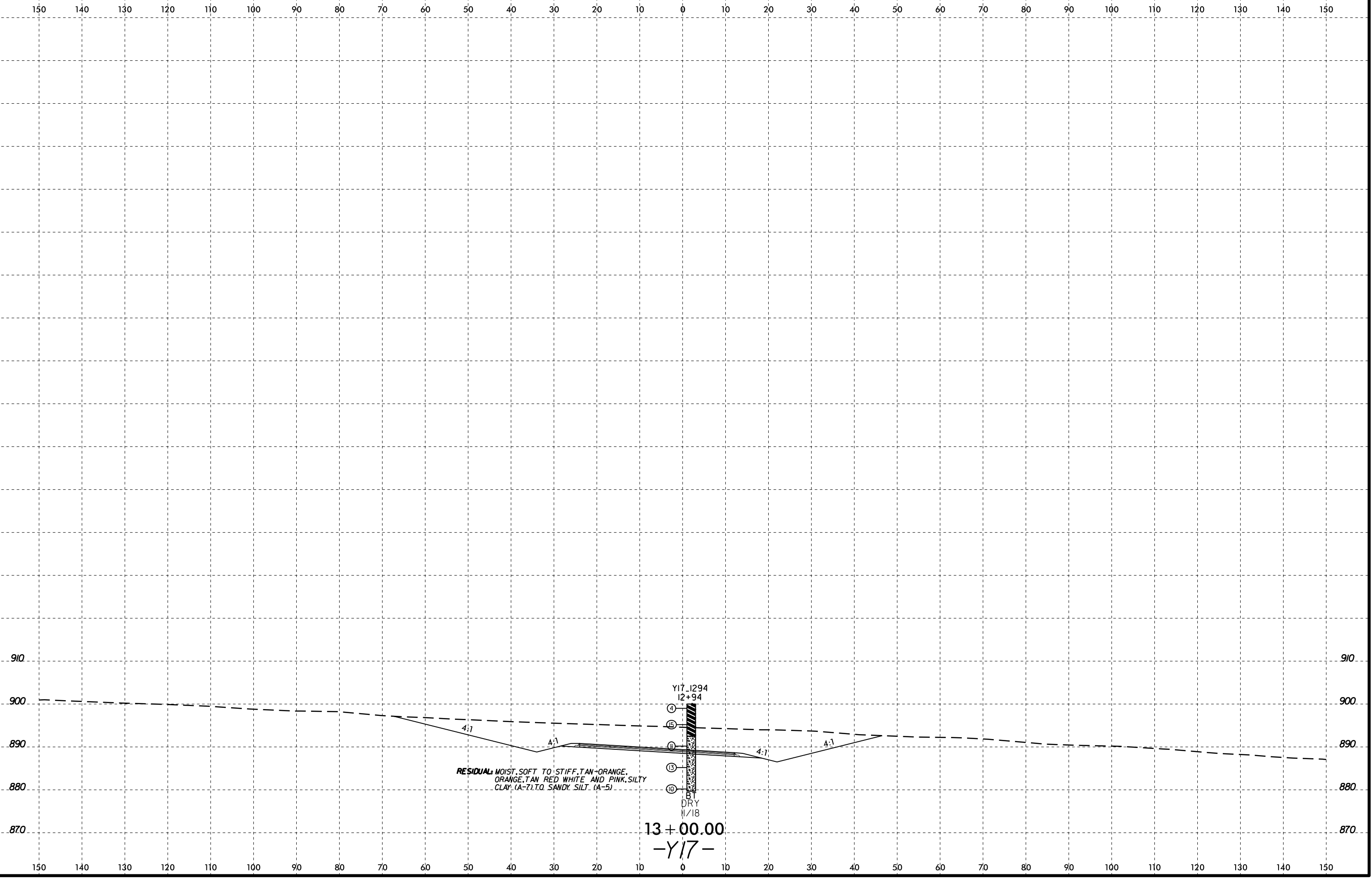
PROJ. REFERENCE NO.	SHEET NO.
U-5813	114

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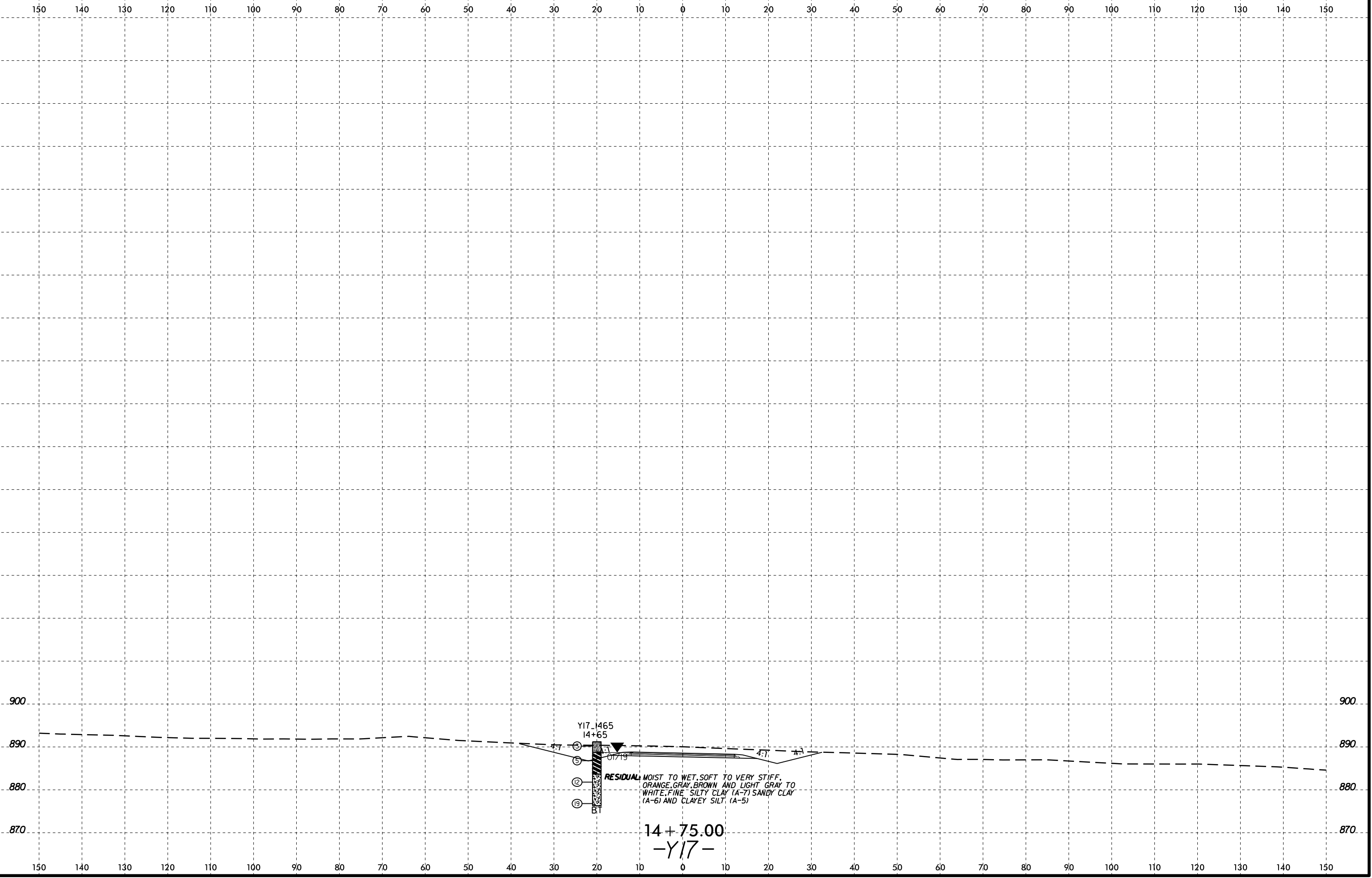


14-OCT-2019 17:51
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SPopke AT K6208546

6/23/16
14-OCT-2019 17:51
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SPopke AT K&209516



6/23/16
14-OCT-2019 17:51
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SPopke AT K&20516

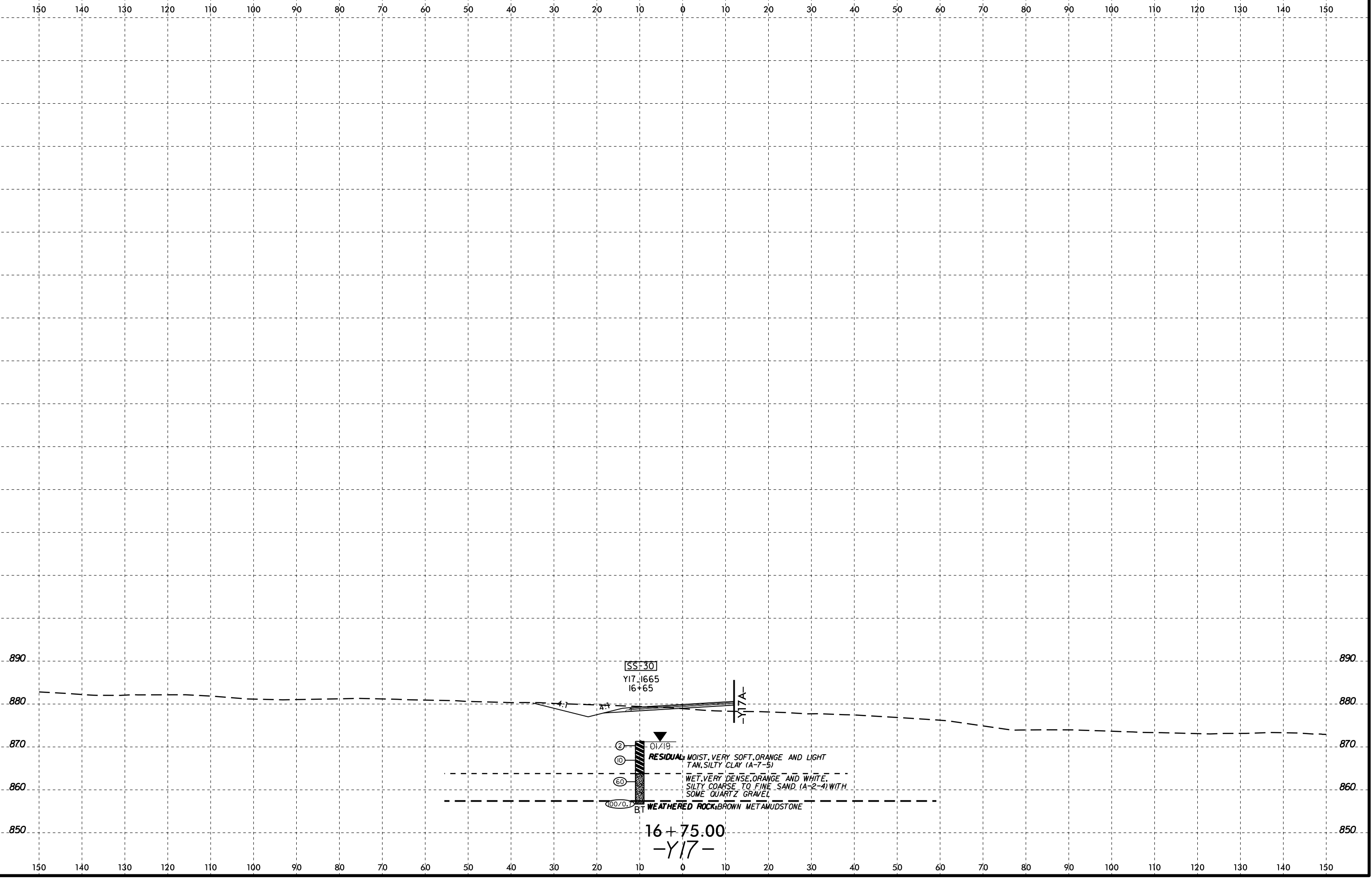


Y17_1465
14+65
①
②
③
BT

RESIDUAL: MOIST TO WET, SOFT TO VERY STIFF,
ORANGE, GRAY, BROWN AND LIGHT GRAY TO
WHITE, FINE SILTY CLAY (A-7) SANDY CLAY
(A-6) AND CLAYEY SILT. (A-5)

14+75.00
-Y17-

6/23/16
14-OCT-2019 17:51
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SPopke AT K&209516



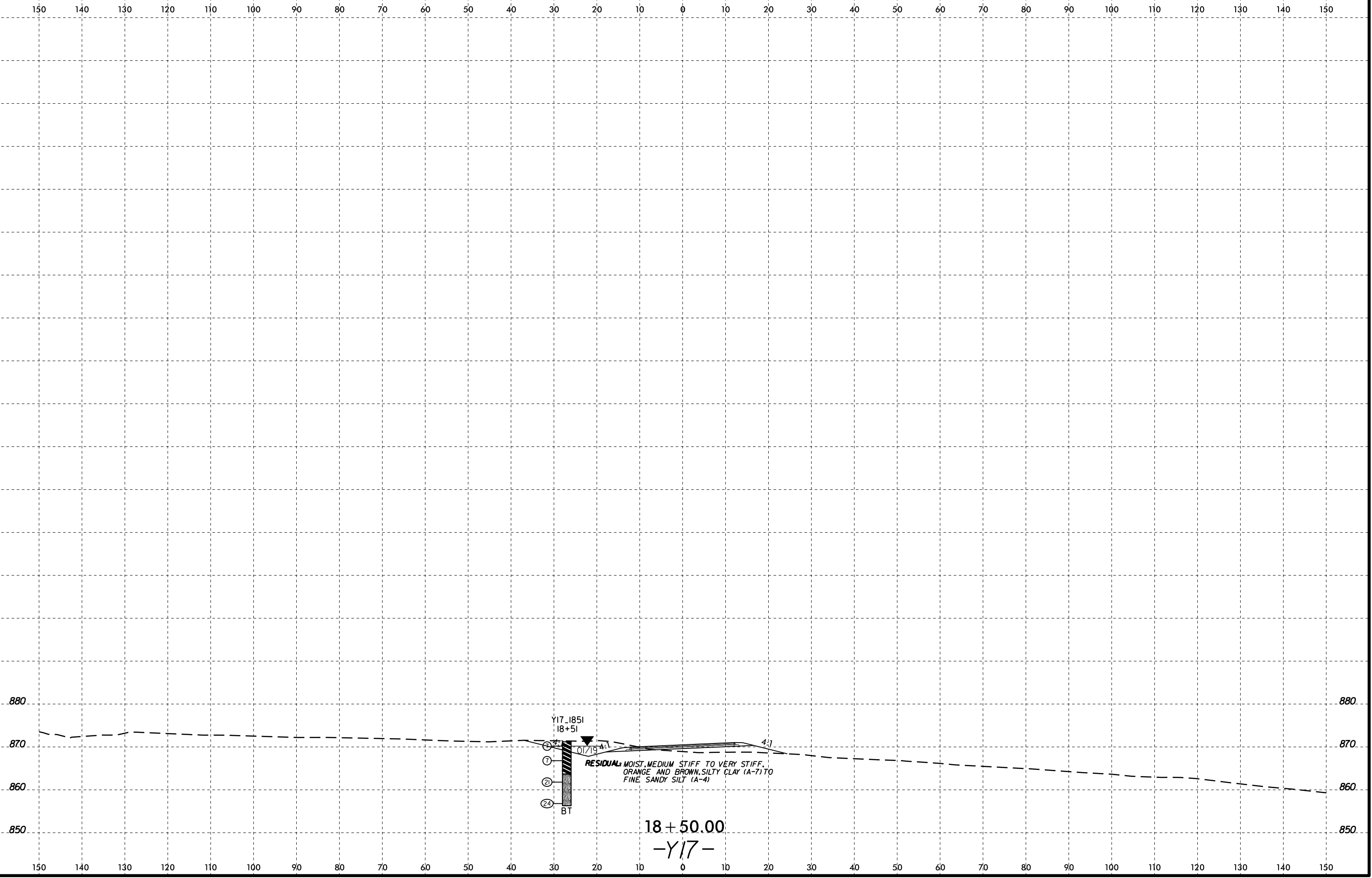
SS-30
Y17_1665
16+65

01/19
2
10
60
100/0.0
BT

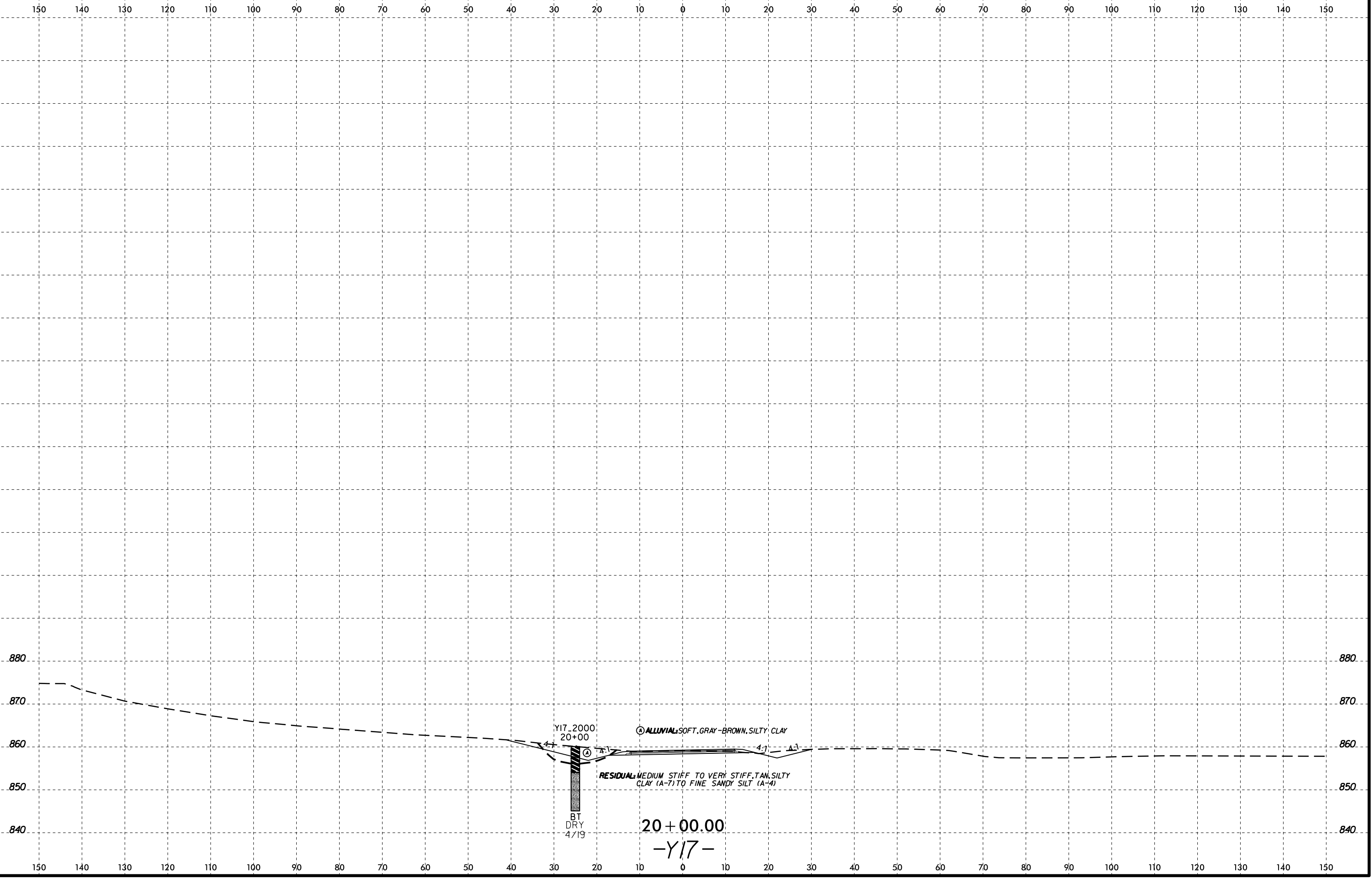
RESIDUAL, MOIST, VERY SOFT, ORANGE AND LIGHT TAN, SILTY CLAY (A-7-5)
WET, VERY DENSE, ORANGE AND WHITE, SILTY COARSE TO FINE SAND (A-2-4) WITH SOME QUARTZ GRAVEL
WEATHERED ROCK, BROWN METAMUDSTONE

16+75.00
-Y17-

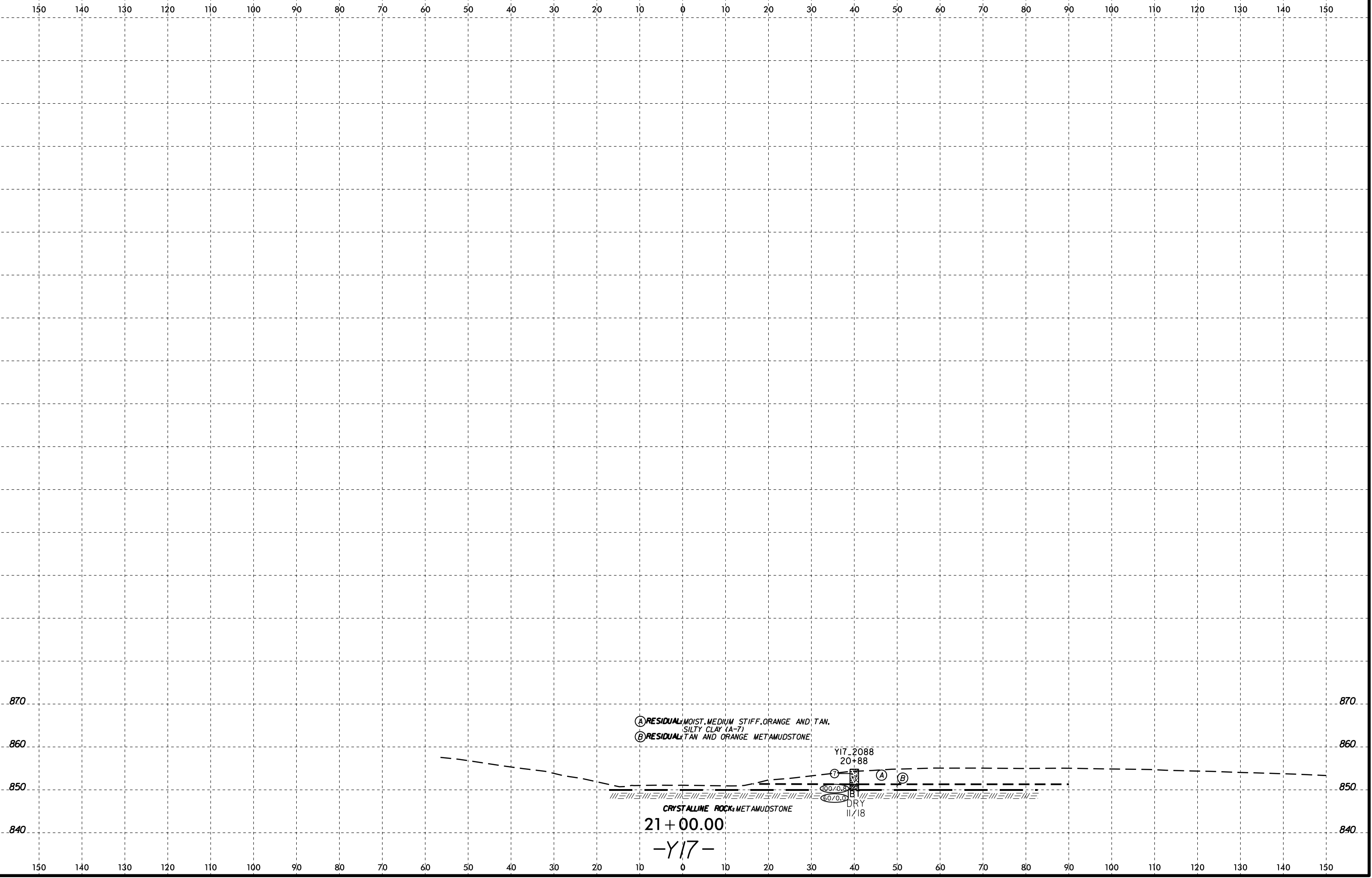
6/23/16
14-OCT-2019 17:51
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Sheet AT K6209516



6/23/16
14-OCT-2019 17:51
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SPopke AT K209516



6/23/16
14-OCT-2019 17:51
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SPopke AT K620546



(A) RESIDUAL MOIST, MEDIUM STIFF, ORANGE AND TAN, SILTY CLAY (A-7)
(B) RESIDUAL TAN AND ORANGE METAMUDSTONE

Y17_2088
20+88

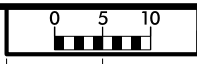
100/0.8
60/0.0
DRY
11/18

CRYSTALLINE ROCK METAMUDSTONE

21 + 00.00

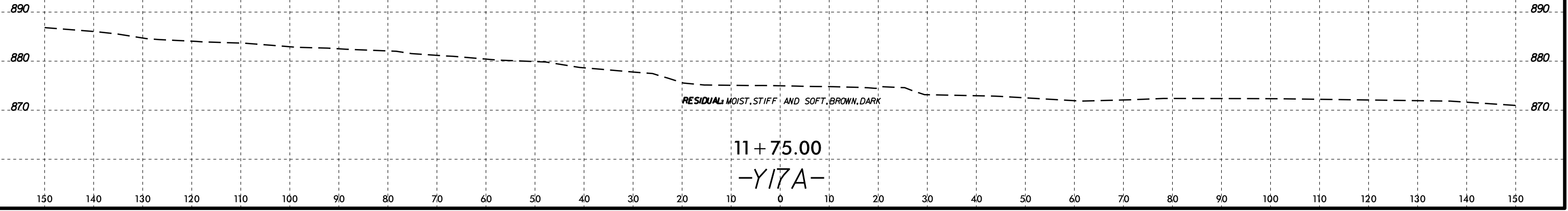
-Y17-

6/23/16



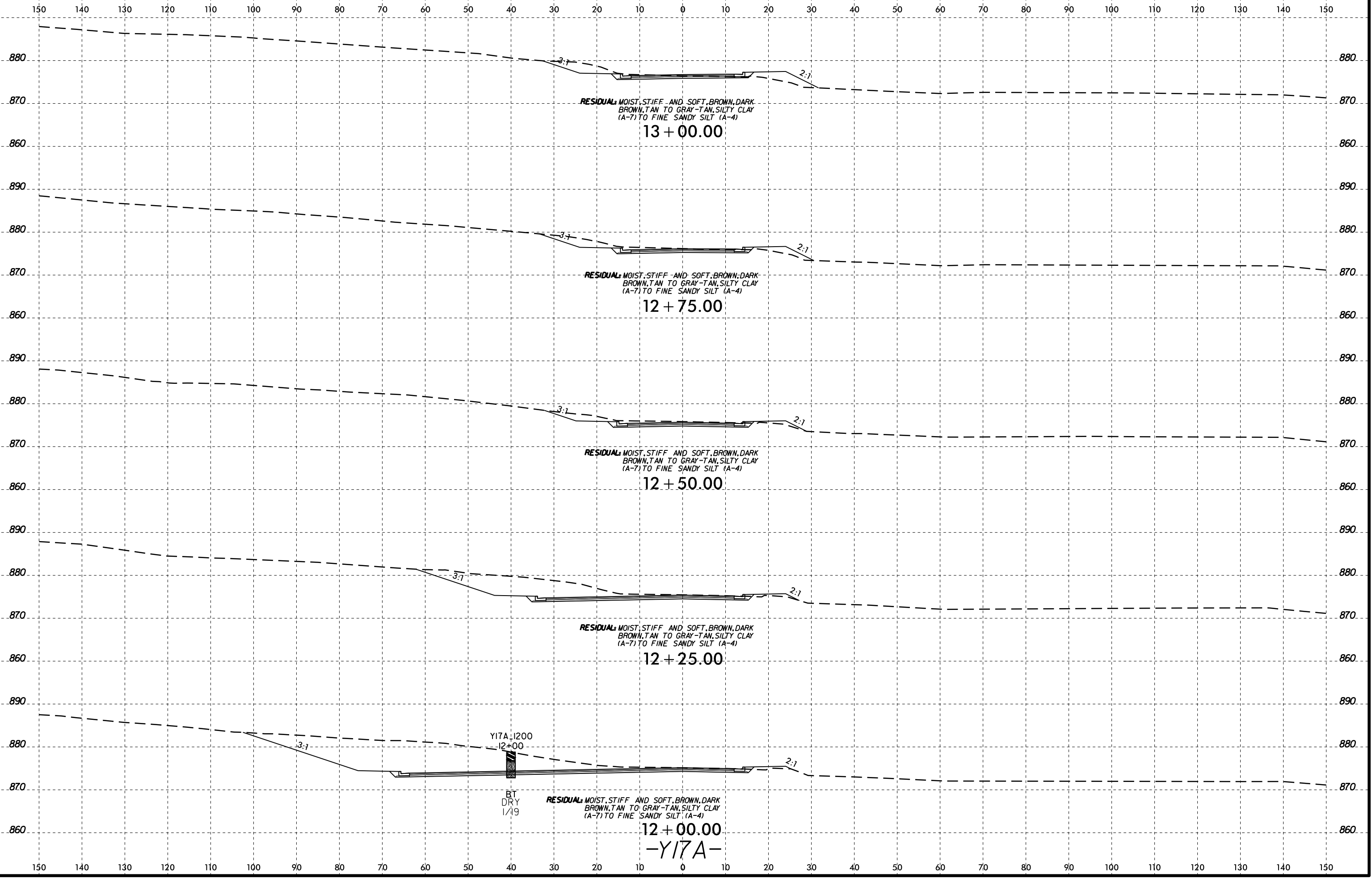
PROJ. REFERENCE NO.	SHEET NO.
U-5813	121

150 140 130 120 110 100 90 80 70 60 50 40 30 20 10 0 10 20 30 40 50 60 70 80 90 100 110 120 130 140 150



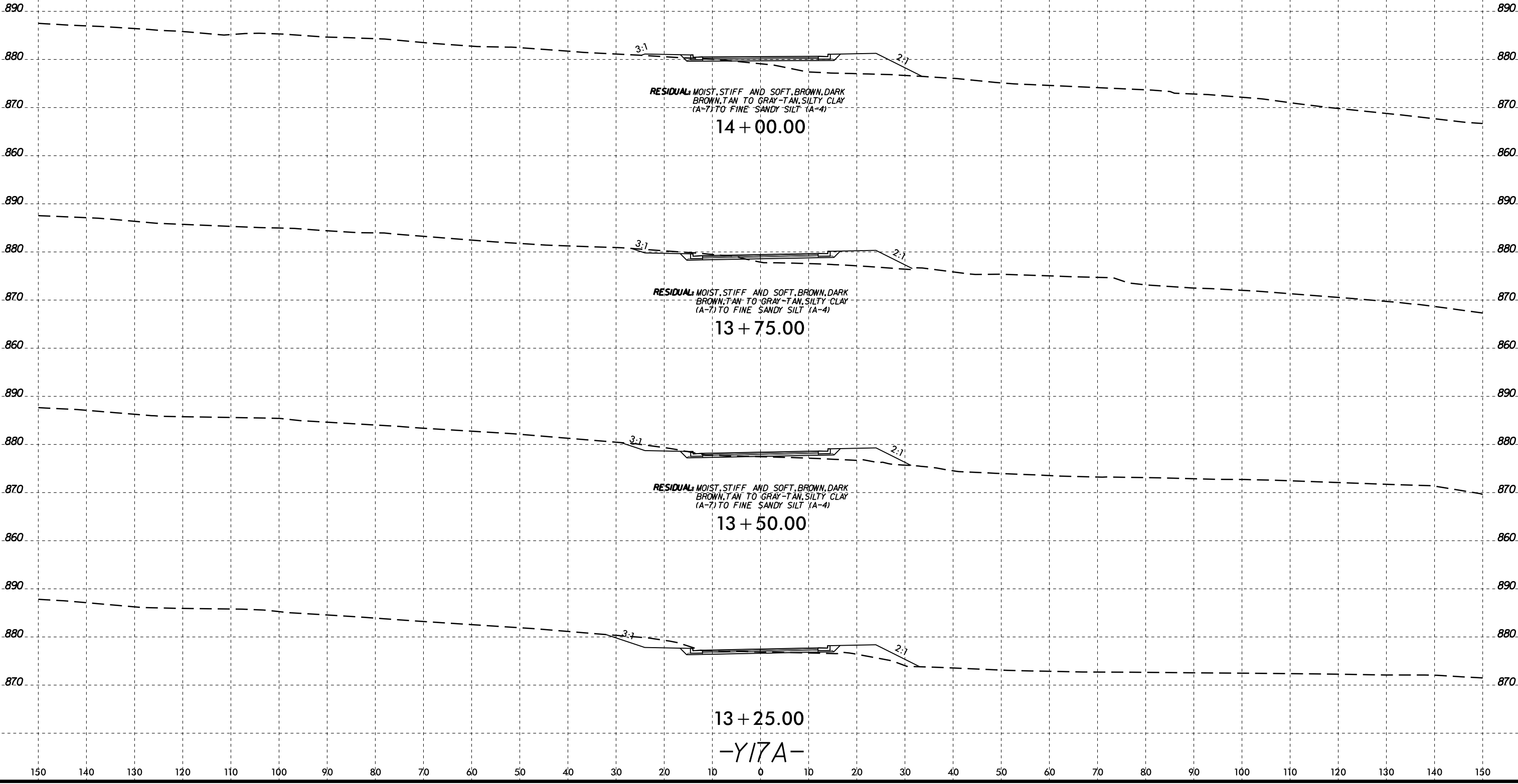
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SPopke AT K&209516

6/23/16
14-OCT-2019 17:52
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SPopke AT K208516

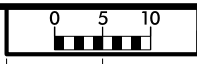




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6/23/16

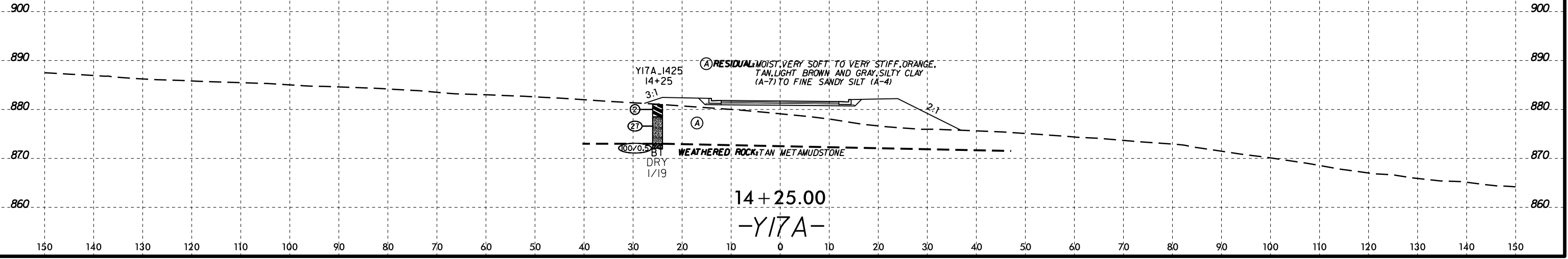


PROJ. REFERENCE NO.
U-5813

SHEET NO.
124

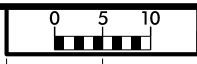
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14-OCT-2019 17:52
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SPopke AT K6209516



150 140 130 120 110 100 90 80 70 60 50 40 30 20 10 0 10 20 30 40 50 60 70 80 90 100 110 120 130 140 150

6/23/16

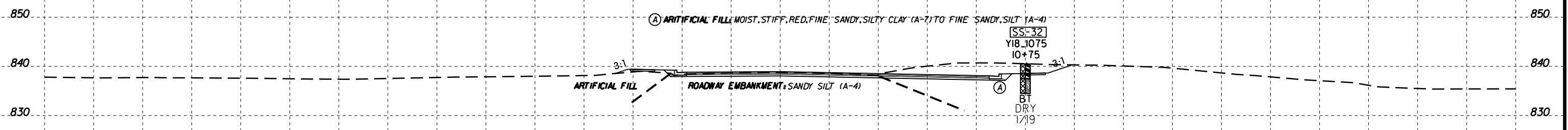


PROJ. REFERENCE NO.
U-5813

SHEET NO.
125

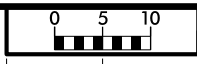
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14-OCT-2019 17:52
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SPopke AT K6209516

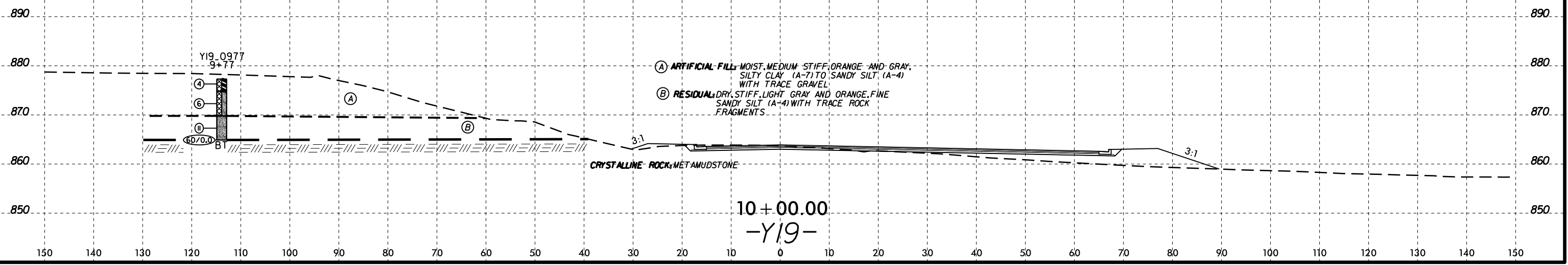


10+75.00
-Y18-

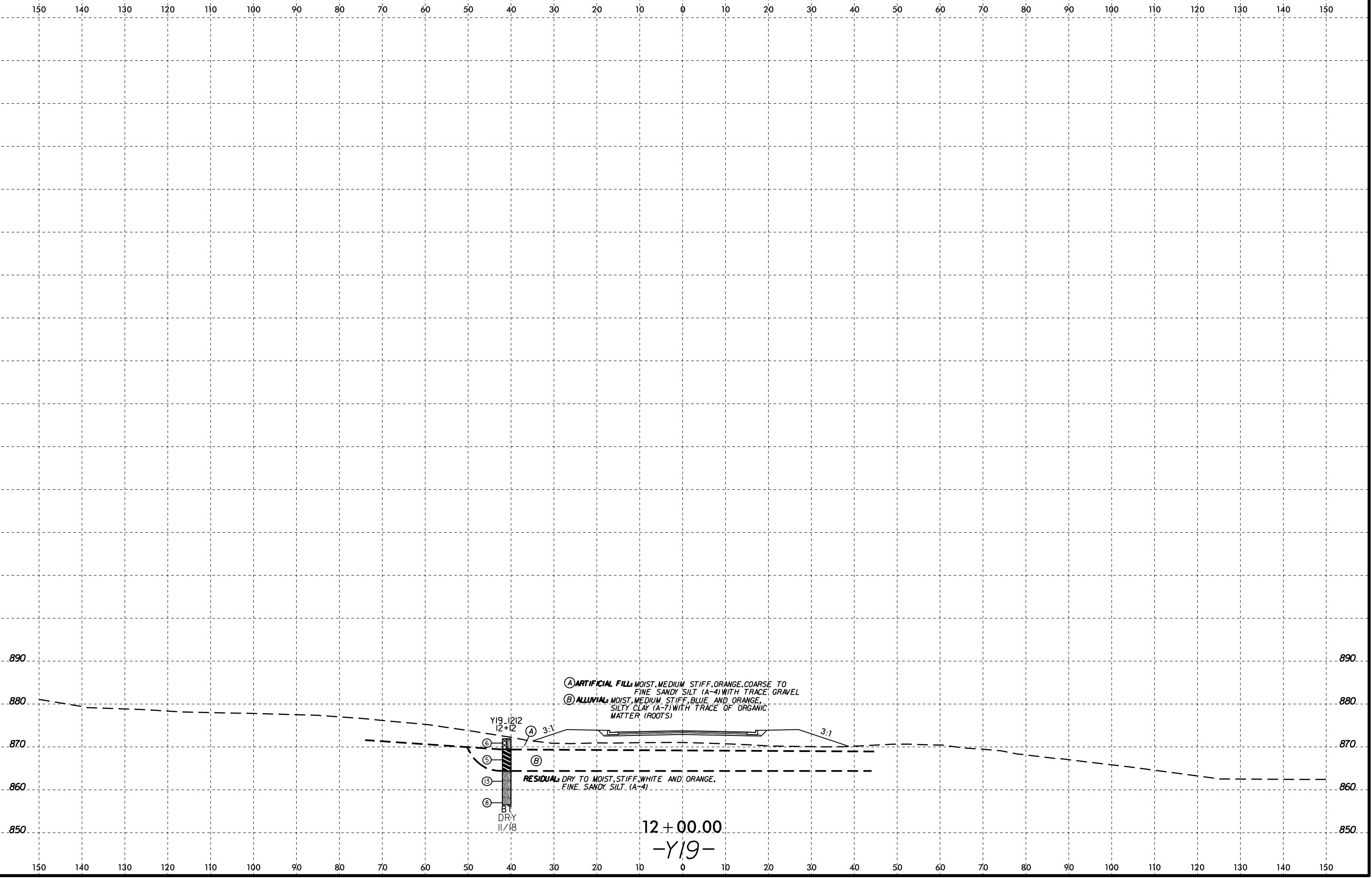
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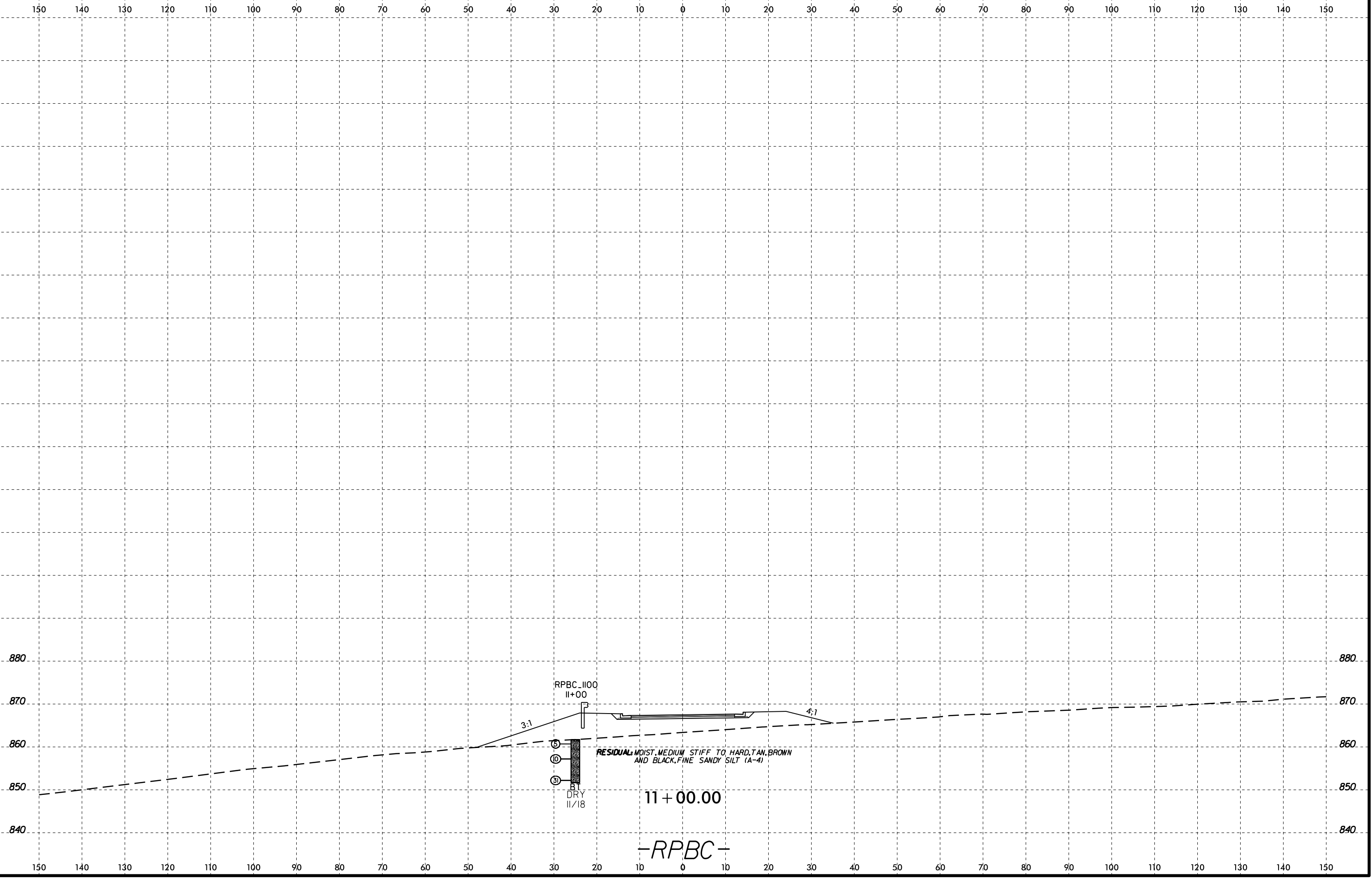
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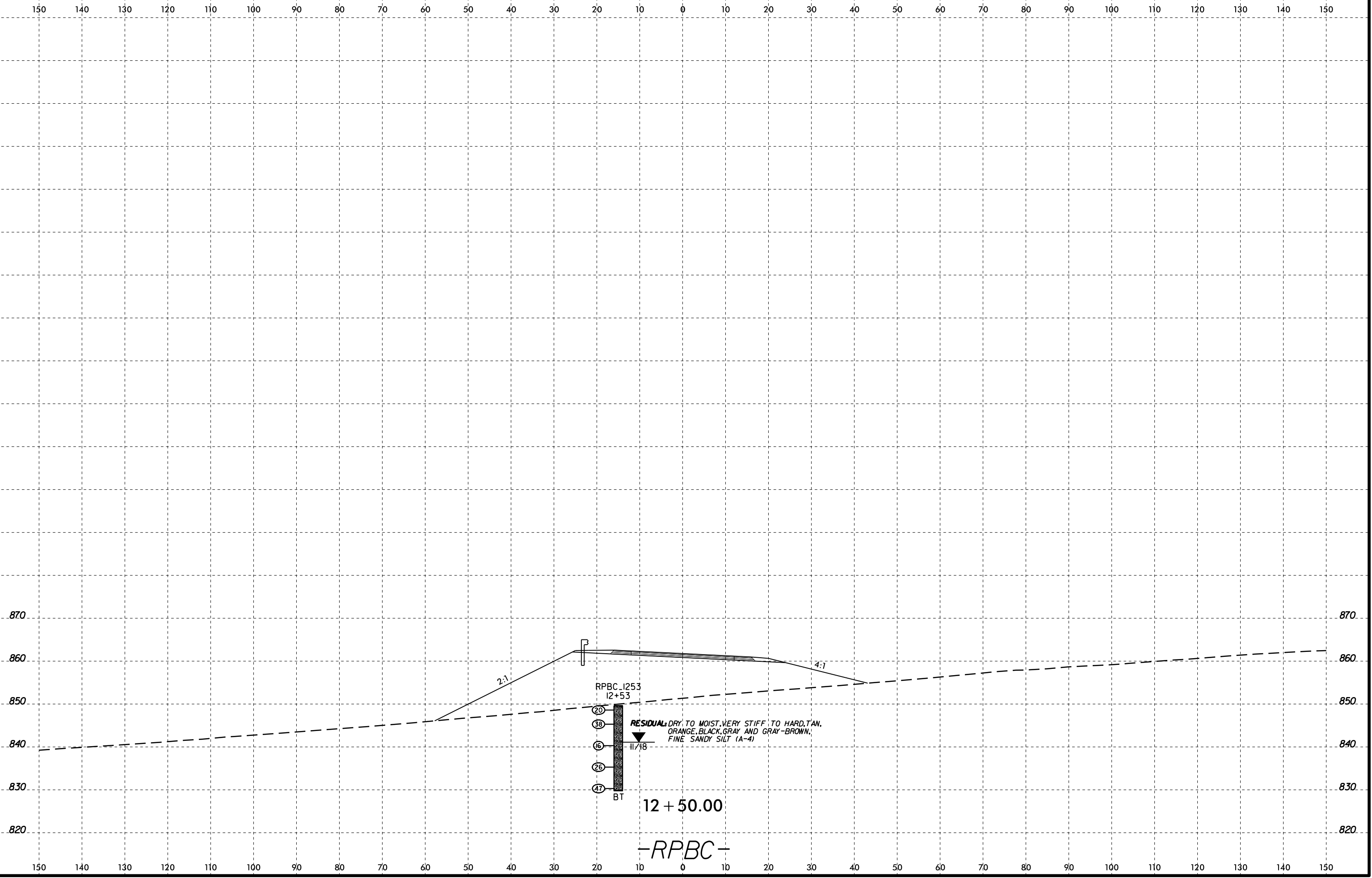
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6/23/16



6/23/16
14-OCT-2019 17:52
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SPopke AT K6209516

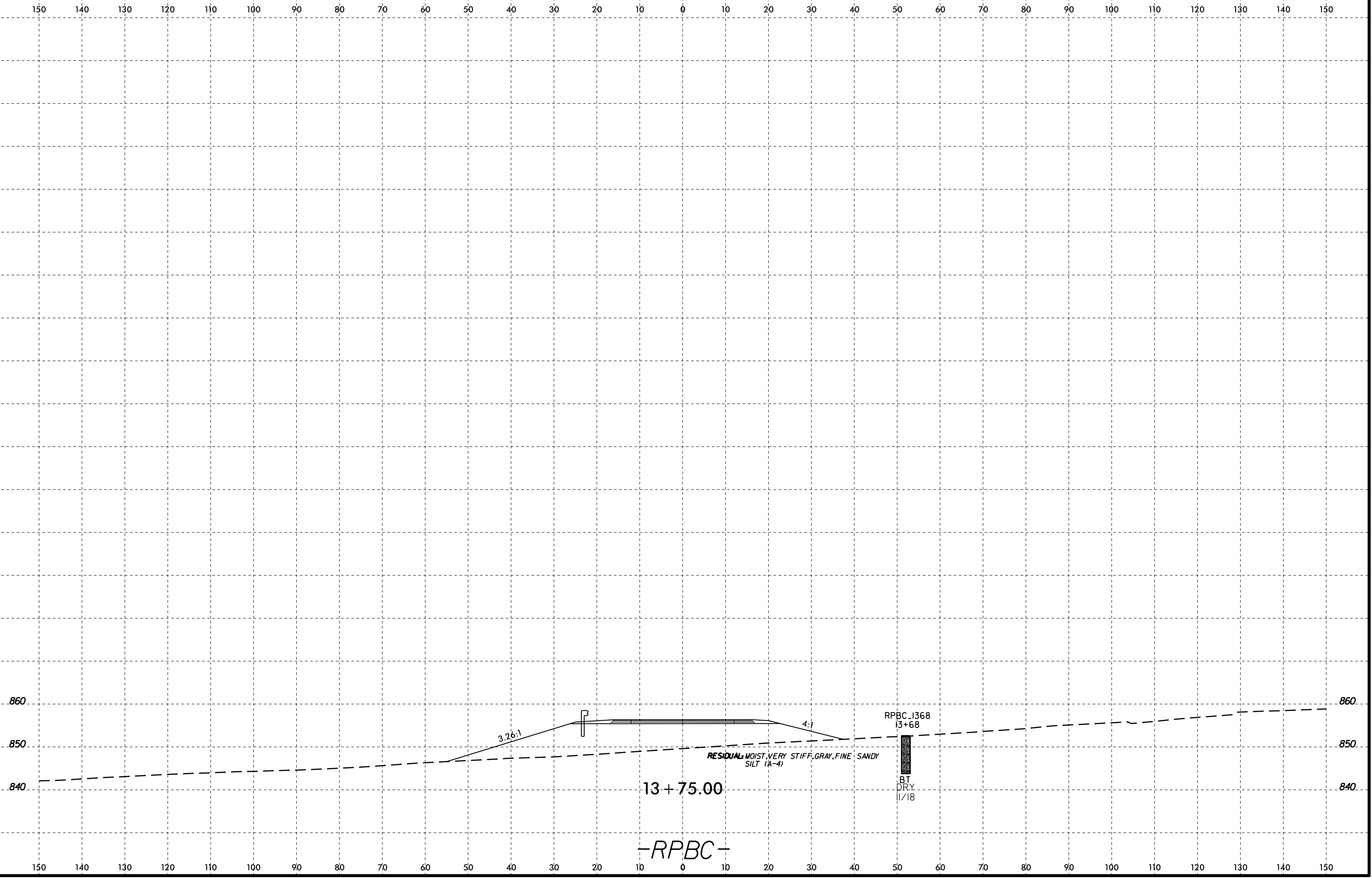


6/23/16
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SPopke AT K6209516



6/23/16
14-OCT-2019 17:53
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SPopke AT K6209516

0 5 10	PROJ. REFERENCE NO.	SHEET NO.
	U-5813	130



860
850
840

13+75.00

RPBC_1368
13+68

BT
DRY
11/18

RESIDUAL MOIST, VERY STIFF, GRAY, FINE SANDY SILT (A-4)

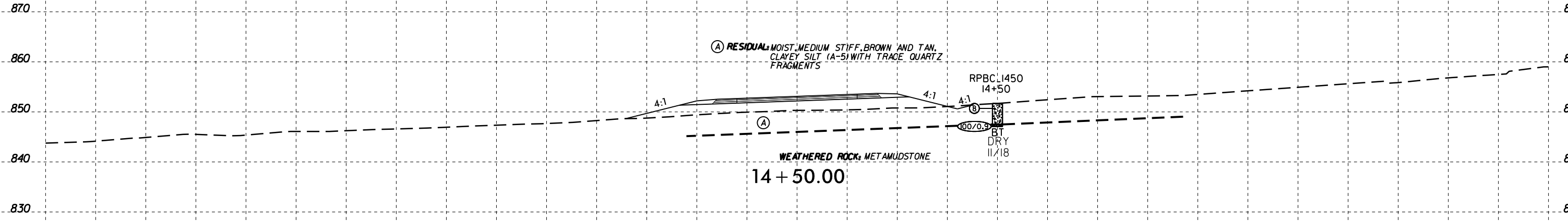
3.26:1

4:1

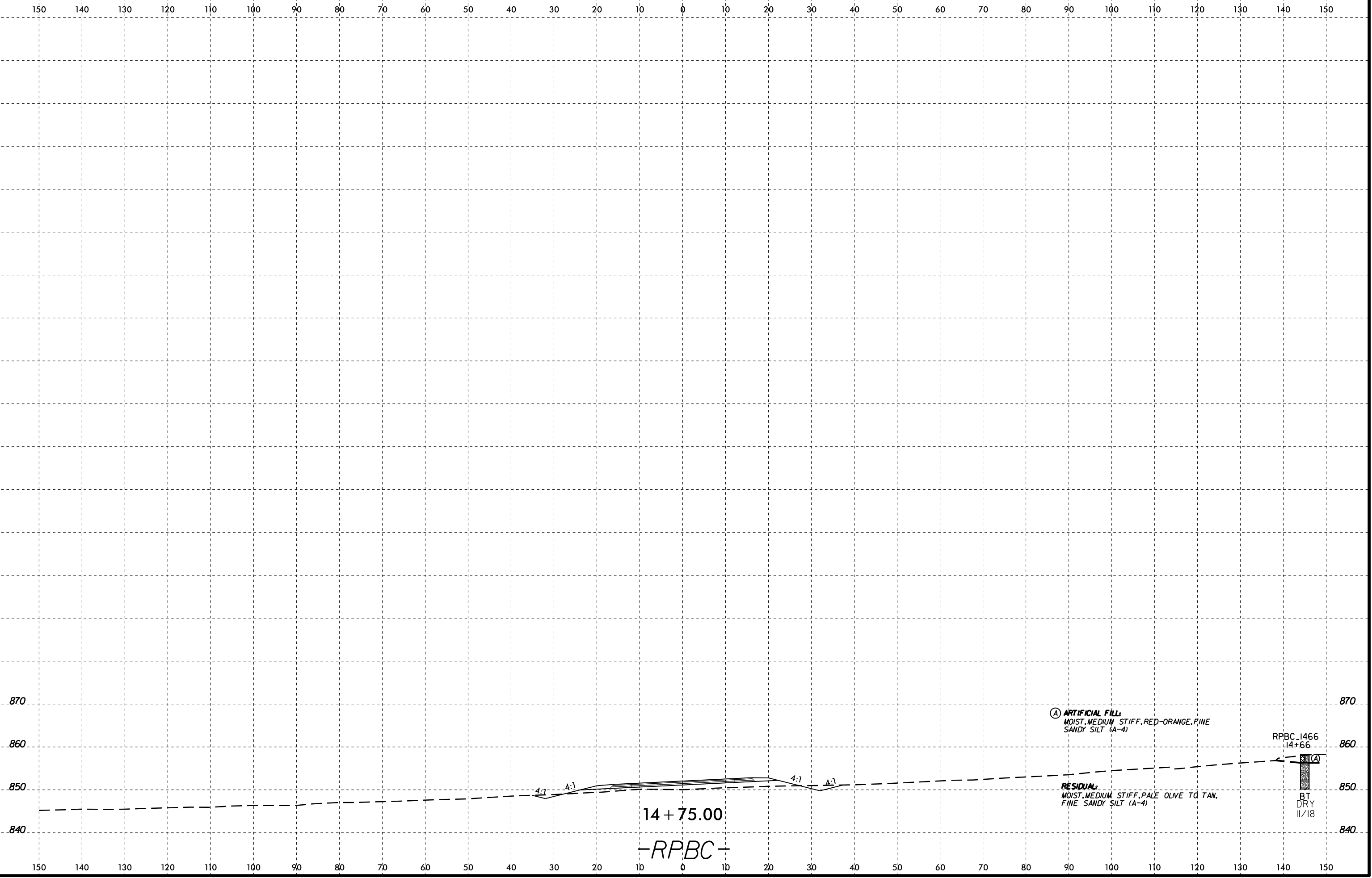
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-RPBC-

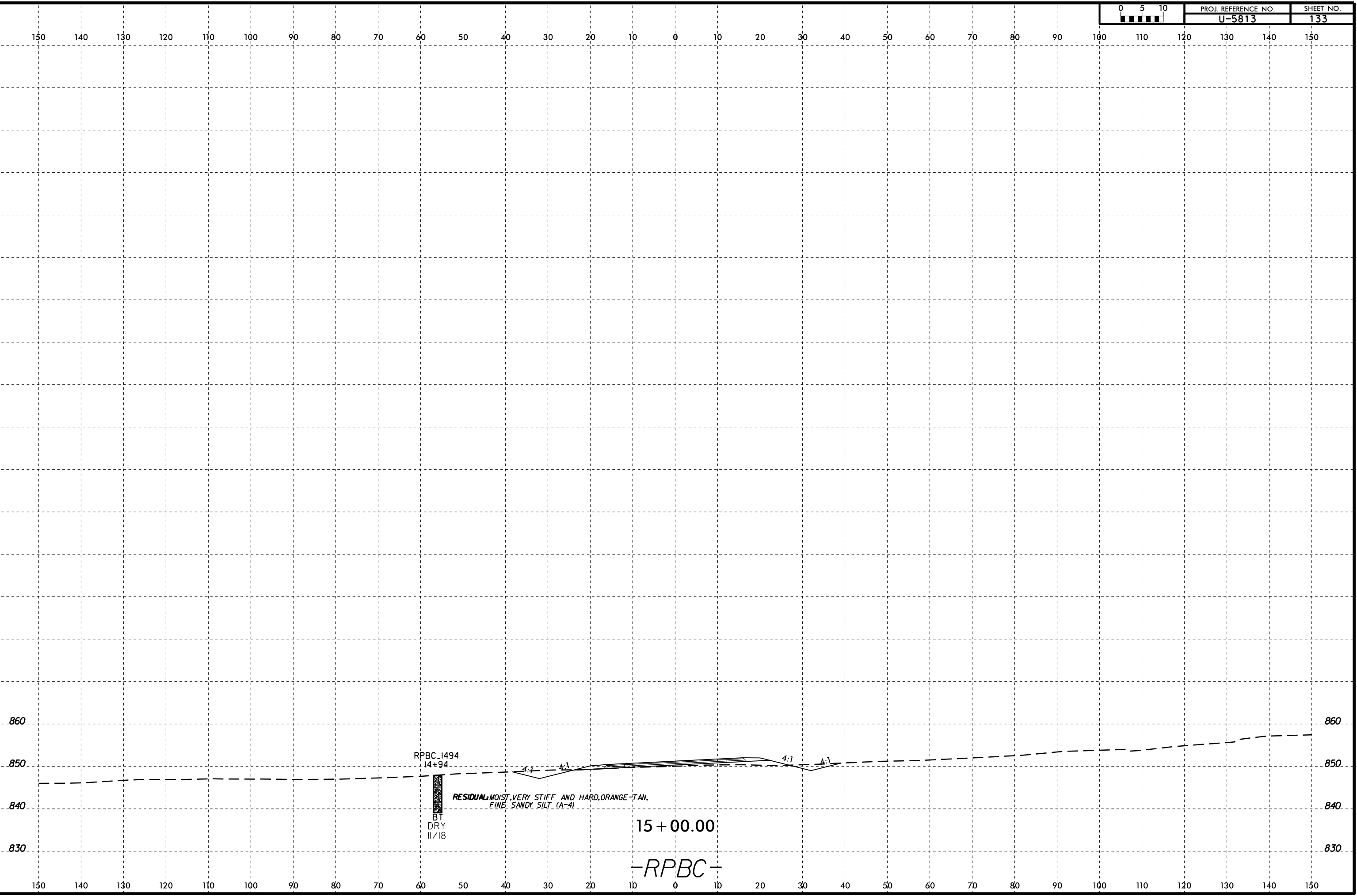
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6/23/16
14-OCT-2019 17:53
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SPopke AT K620516



6/23/16
14-OCT-2019 17:53
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SPopke AT K6209516



150 140 130 120 110 100 90 80 70 60 50 40 30 20 10 0 10 20 30 40 50 60 70 80 90 100 110 120 130 140 150

860 860

850 850

840 840

830 830

150 140 130 120 110 100 90 80 70 60 50 40 30 20 10 0 10 20 30 40 50 60 70 80 90 100 110 120 130 140 150

RPBC_1494
14+94

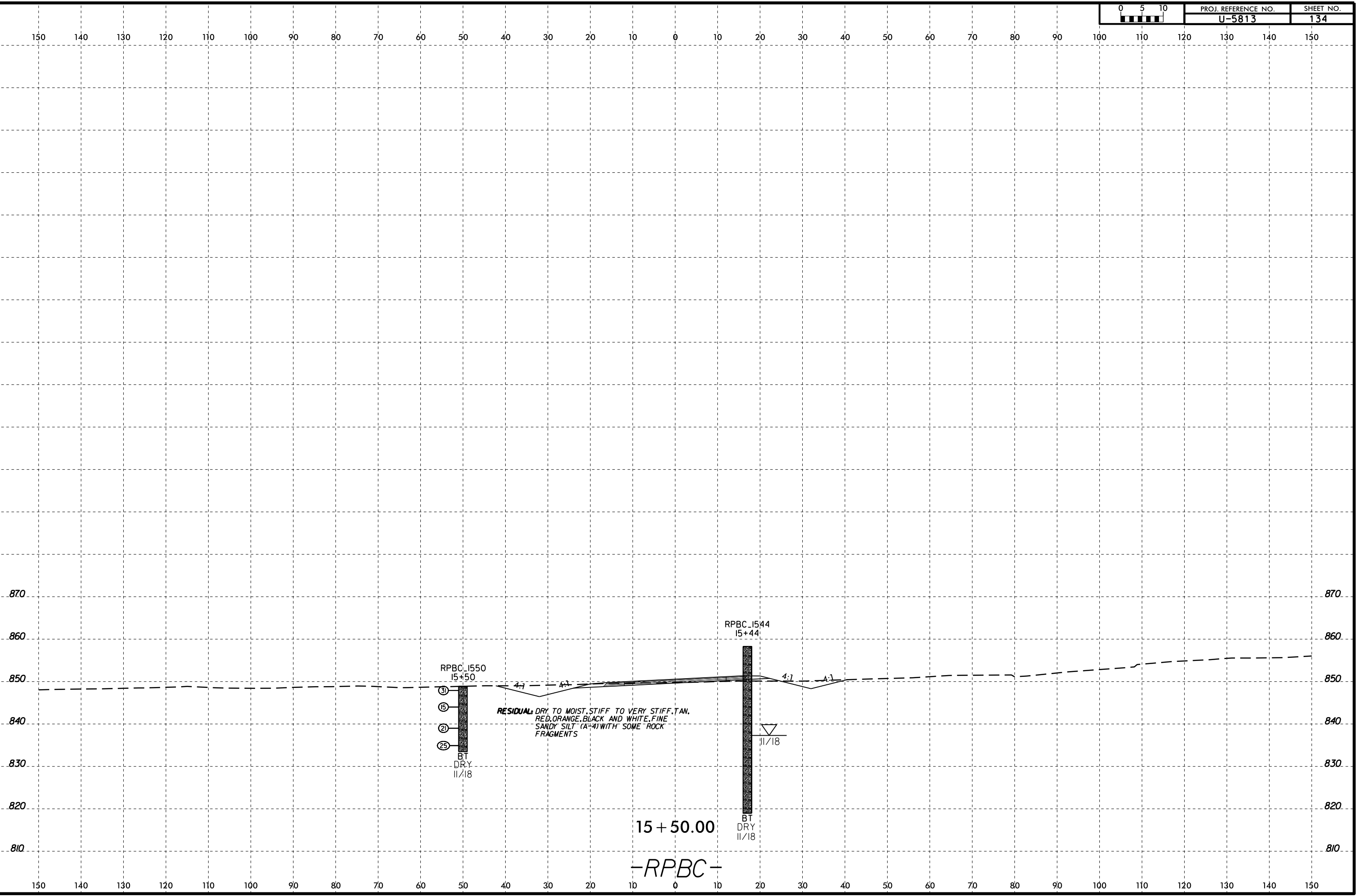
BT
DRY
11/18

RESIDUAL MOIST, VERY STIFF AND HARD, ORANGE-TAN,
FINE SANDY SILT (A-4)

15 + 00.00

-RPBC-

6/23/16
14-OCT-2019 17:53
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SPopke AT K6209516



RPBC_1550
15+50

- 31
- 15
- 21
- 25

BT
DRY
11/18

RESIDUAL DRY TO MOIST, STIFF TO VERY STIFF, TAN,
RED, ORANGE, BLACK AND WHITE, FINE
SANDY SILT (A=4) WITH SOME ROCK
FRAGMENTS

RPBC_1544
15+44

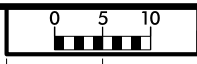
11/18

BT
DRY
11/18

15 + 50.00

-RPBC-

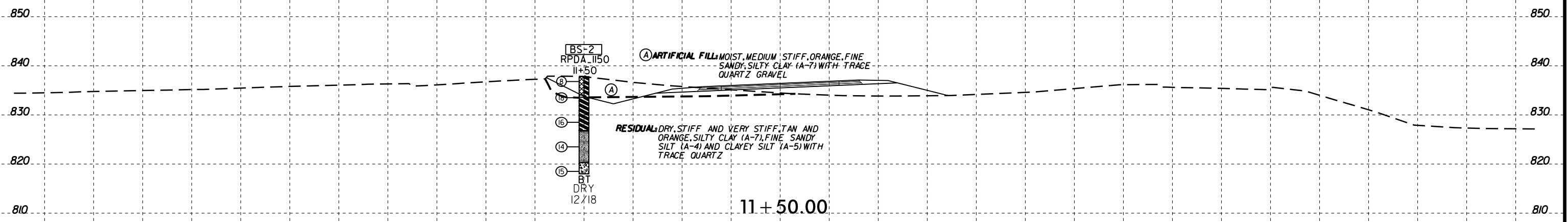
6/23/16



PROJ. REFERENCE NO.
U-5813

SHEET NO.
135

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BS-2
RPDA 1150
11+50
12x18
DRY

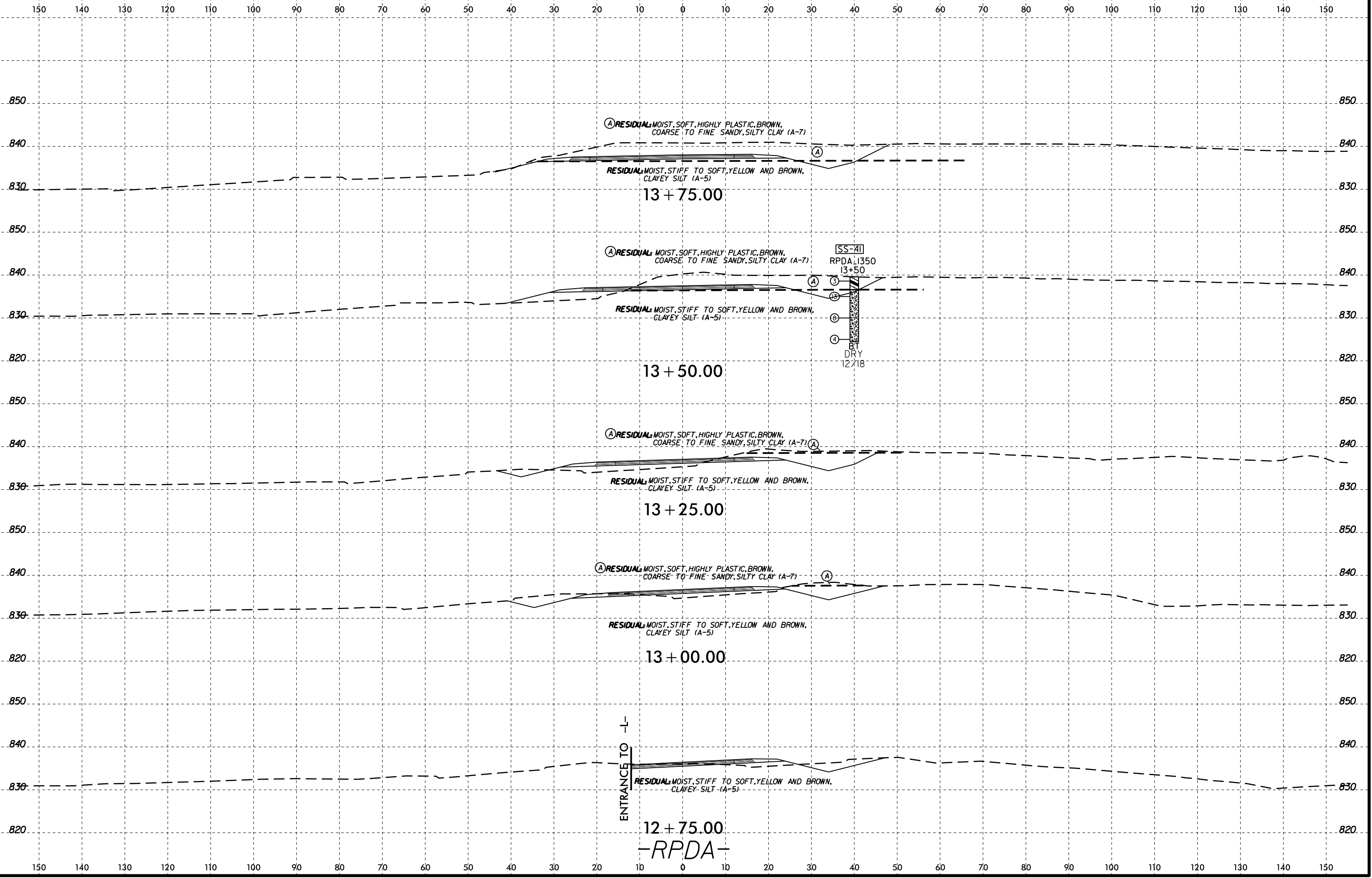
(A) ARTIFICIAL FILL: MOIST, MEDIUM STIFF, ORANGE, FINE SANDY SILTY CLAY (A-7) WITH TRACE QUARTZ GRAVEL

RESIDUAL: DRY, STIFF AND VERY STIFF, TAN AND ORANGE, SILTY CLAY (A-7), FINE SANDY SILT (A-4) AND CLAYEY SILT (A-5) WITH TRACE QUARTZ

11+50.00
-RPDA-

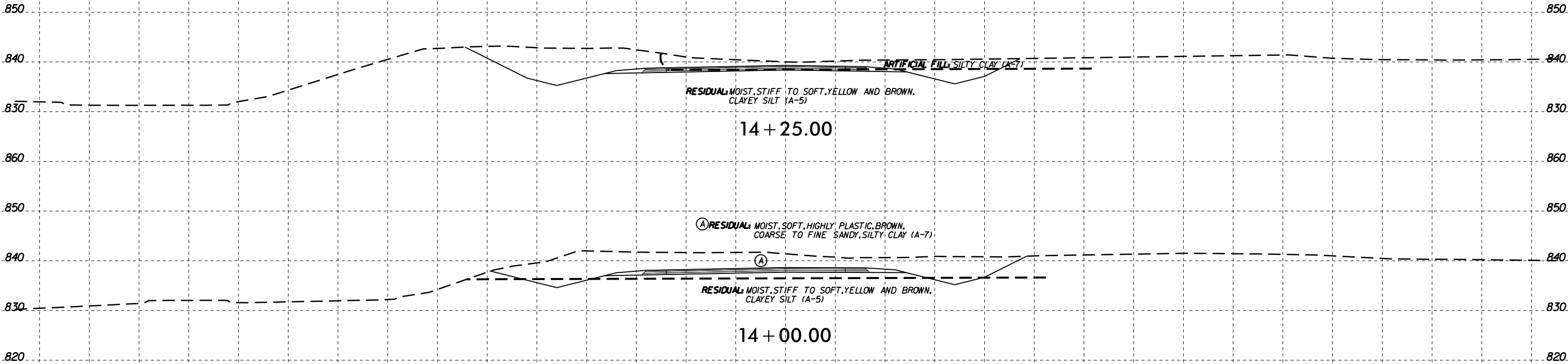
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SPopke AT K209516

14-OCT-2019 17:53
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6/23/16

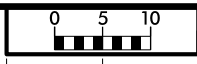




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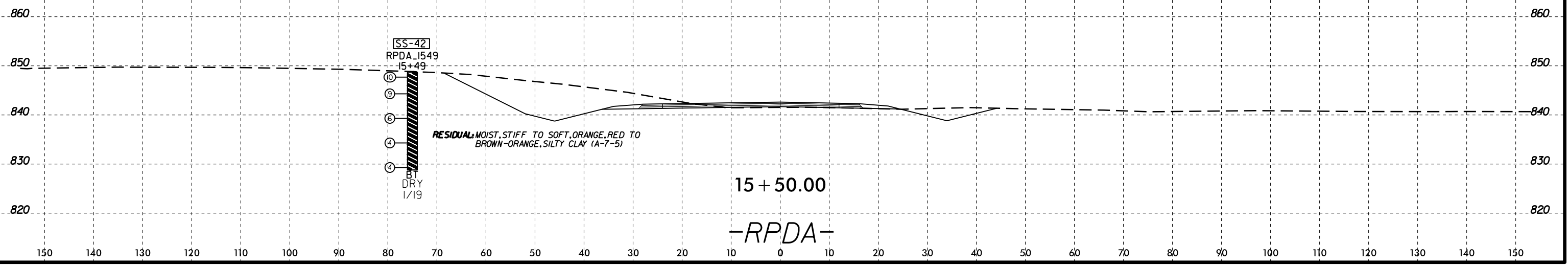


-RPDA-



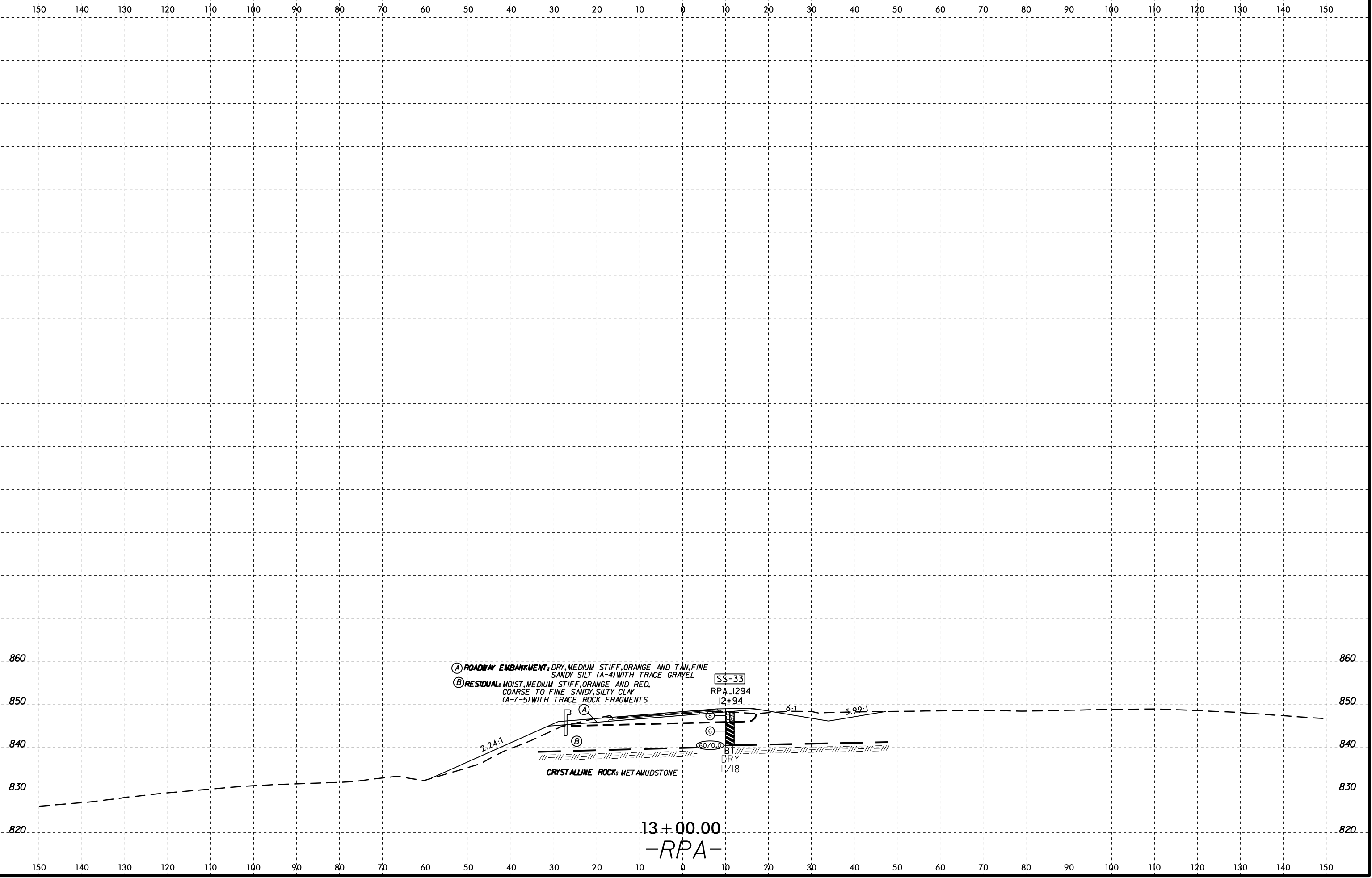
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14-OCT-2019 17:53
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SSopke AT K6209516



150 140 130 120 110 100 90 80 70 60 50 40 30 20 10 0 10 20 30 40 50 60 70 80 90 100 110 120 130 140 150

6/23/16
14-OCT-2019 17:53
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Sheet AT K420516





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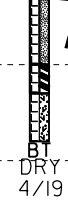
860
850
840
830
820
810

860
850
840
830
820
810

RESIDUAL
MOIST MEDIUM STIFF ORANGE
AND GRAY SILTY CLAY (A-7)

2:1

RPA 1400'
14+00'



24"
BT
DRY
4/19

ROADWAY EMBANKMENT: MEDIUM STIFF RED FINE SANDY SILT (A-4) SILTY CLAY (A-7) AND CLAYEY SILT (A-5)

6:1

5.99:1

14 + 00.00
-RPA-

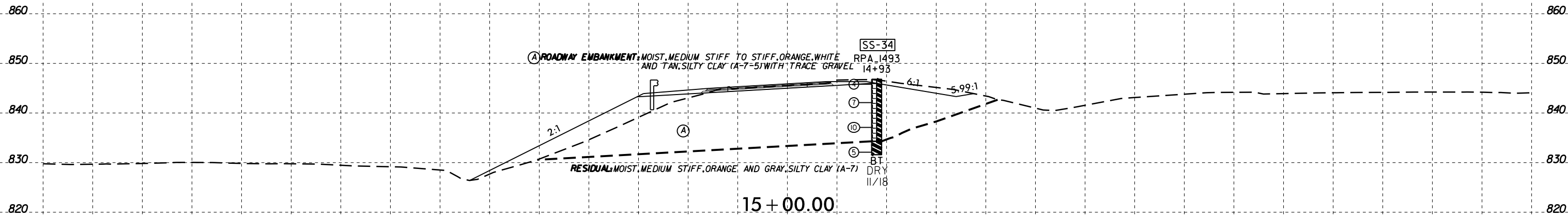
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6/23/16



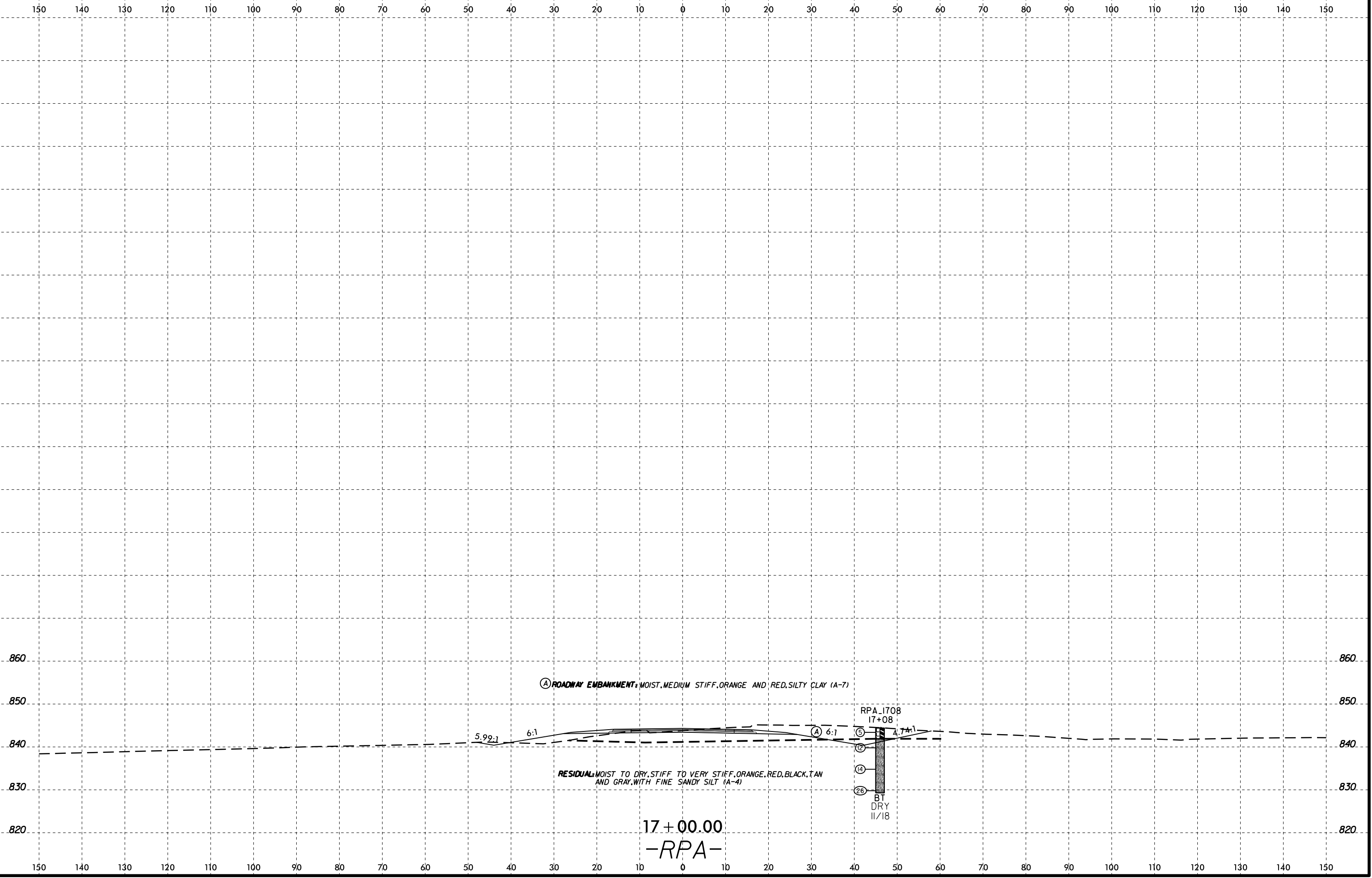
PROJ. REFERENCE NO.	SHEET NO.
U-5813	141

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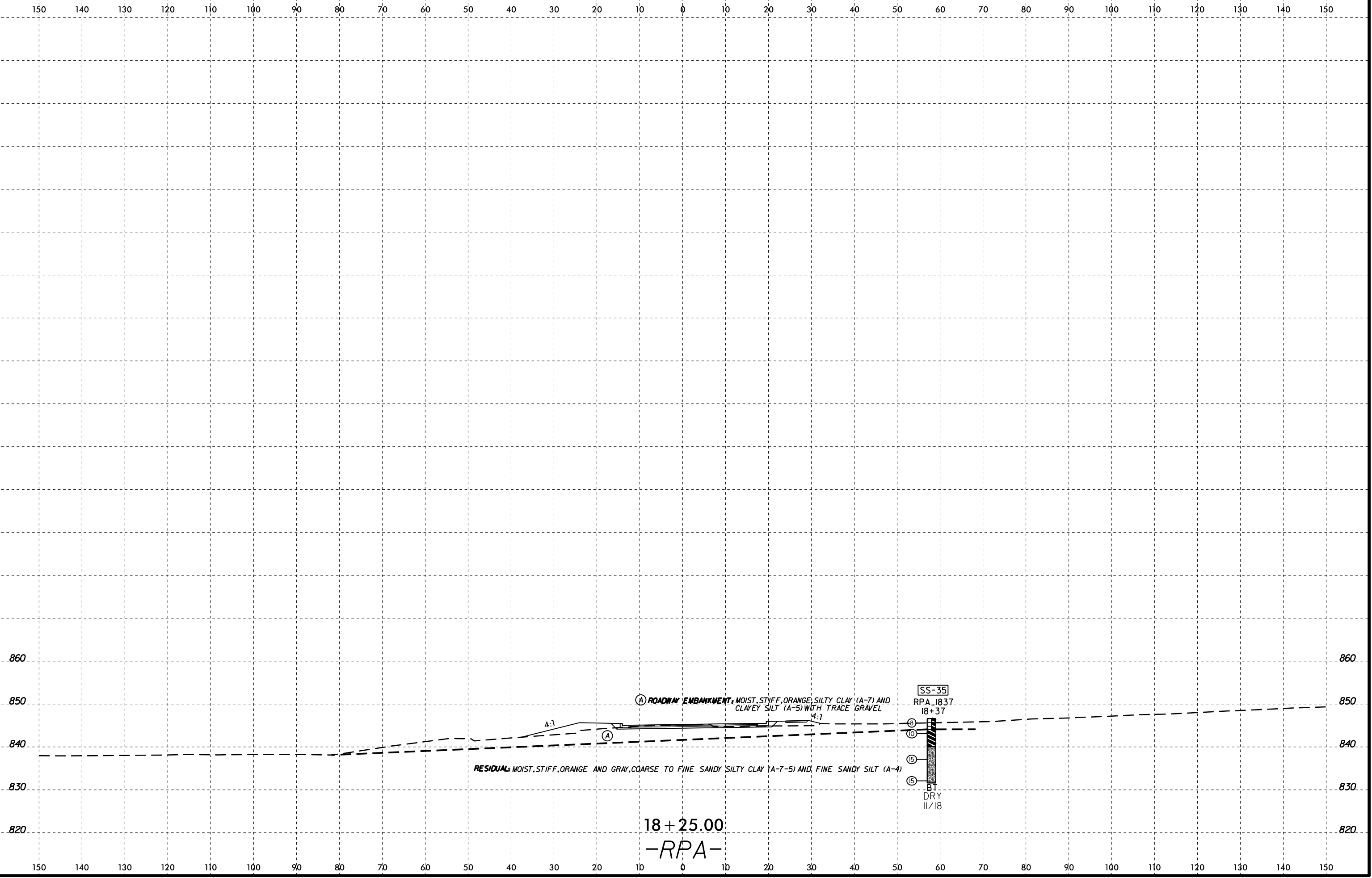


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 Spoke AT K620516

6/23/16
14-OCT-2019 17:53
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Speke AT K209516

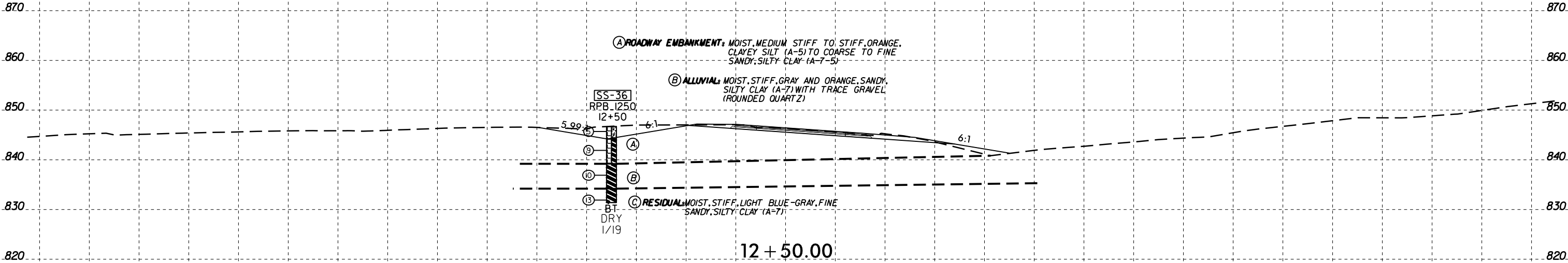


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6/23/16



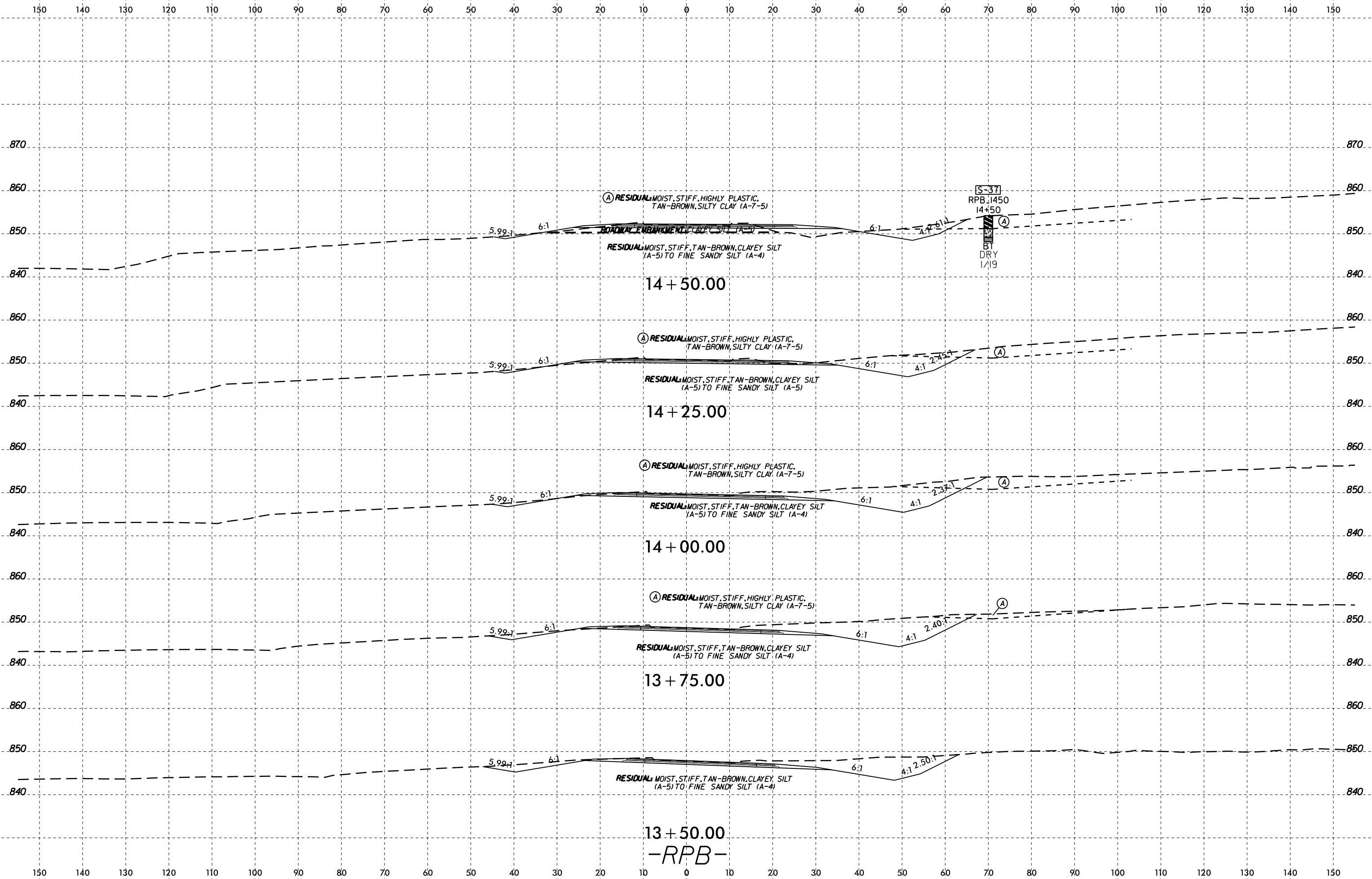


150 140 130 120 110 100 90 80 70 60 50 40 30 20 10 0 10 20 30 40 50 60 70 80 90 100 110 120 130 140 150



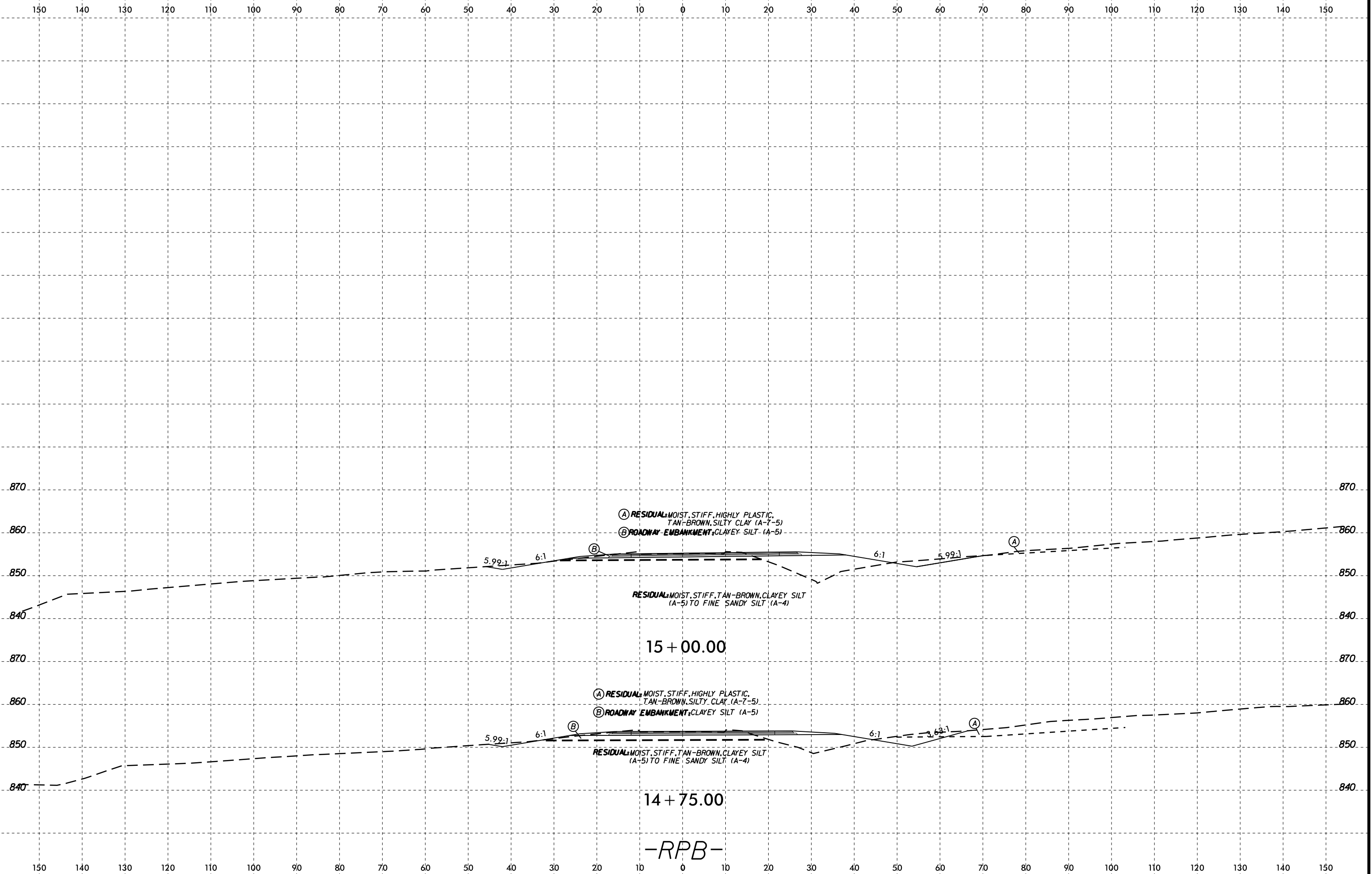
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-RPB-

15-OCT-2018 14:40
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S:\spoke



13 + 50.00
-RPB-

6/23/16
I:\OCT-2018\440\Washburne\GEO\TECHNICAL\Projects\Active Projects\20190942.009A U-5813 Roadway\U5813_GEO\RDWY\CADD\GEO\TECH\XSC\U5813_GEO_RPB_XSI.dgn
S:\pape

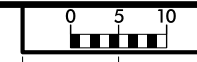


15 + 00.00

14 + 75.00

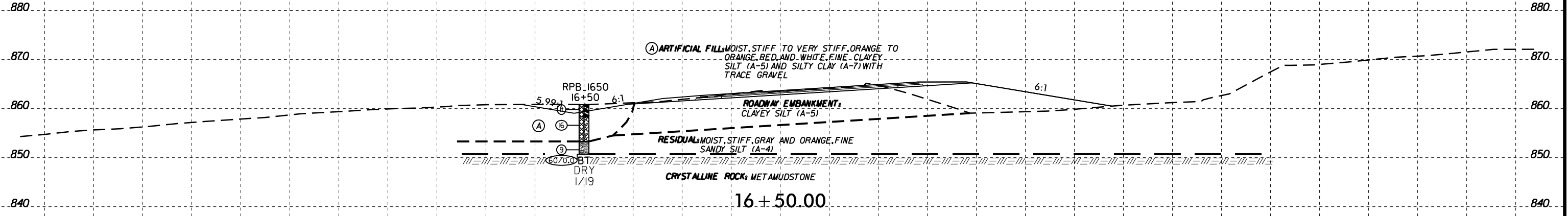
-RPB-

6/23/16



PROJ. REFERENCE NO.	SHEET NO.
U-5813	147

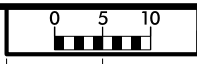
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-RPB-

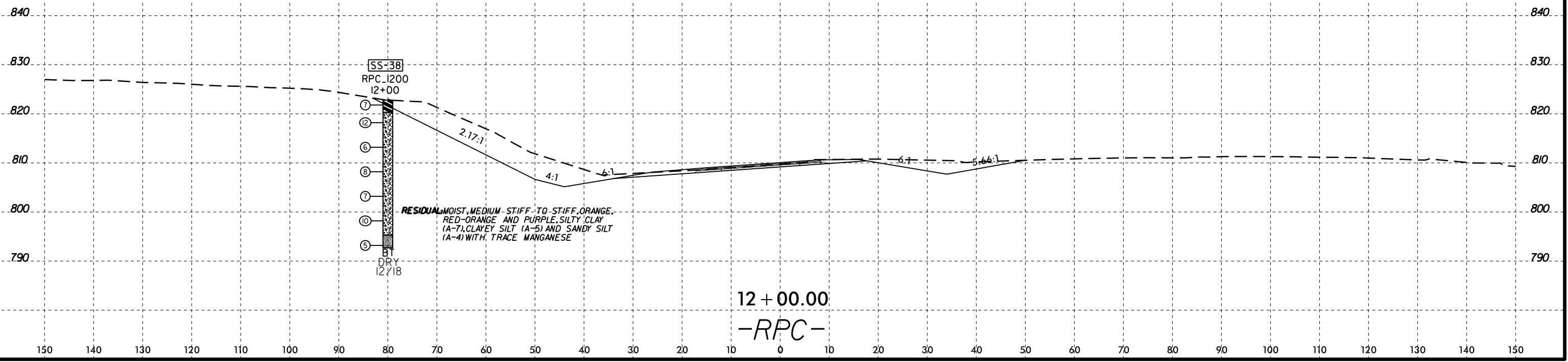
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6/23/16



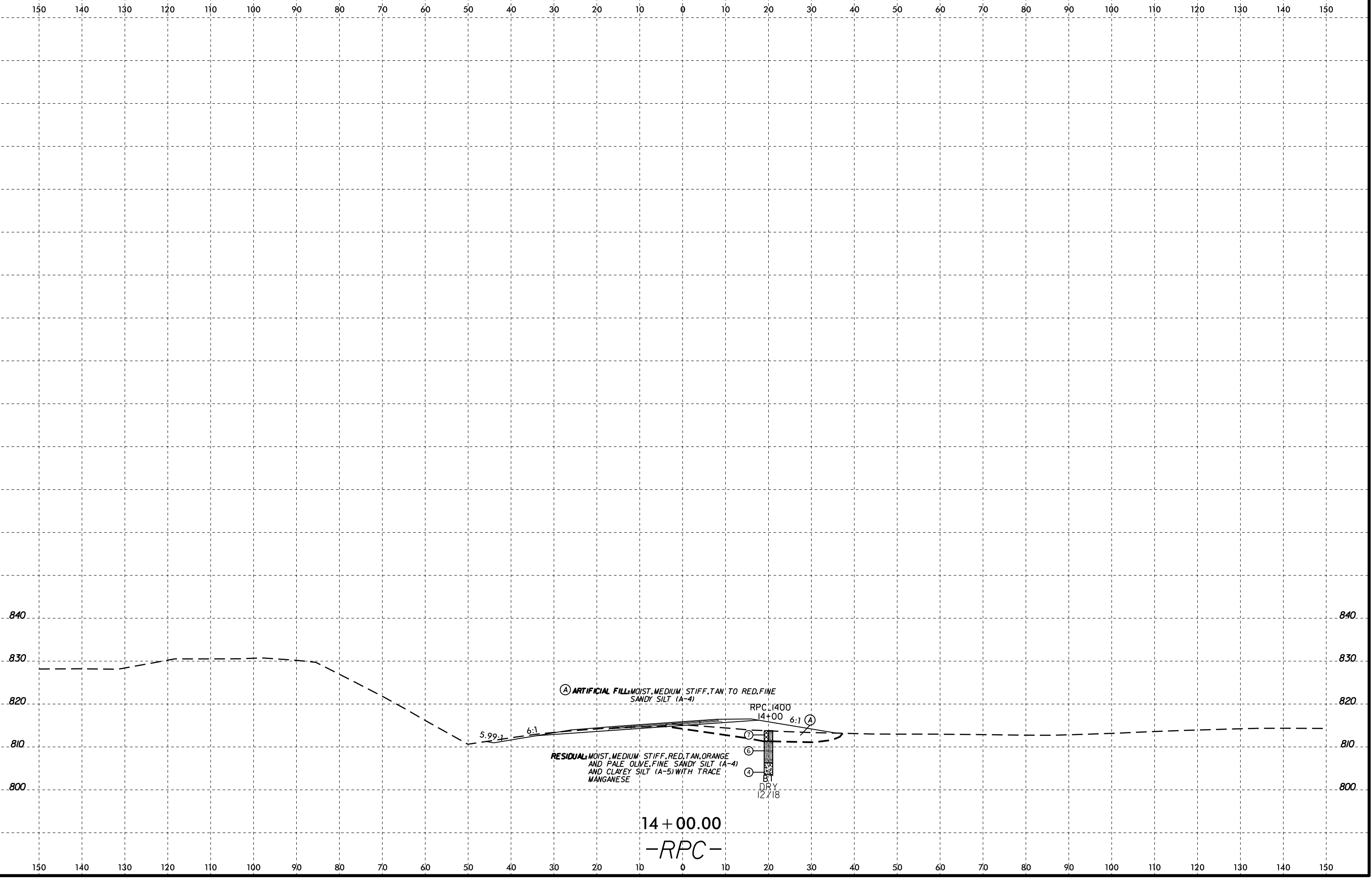
PROJ. REFERENCE NO.	SHEET NO.
U-5813	148

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 \$P\$pe AT K6209516

6/23/16
14-OCT-2019 17:54
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SPopke AT K&209516



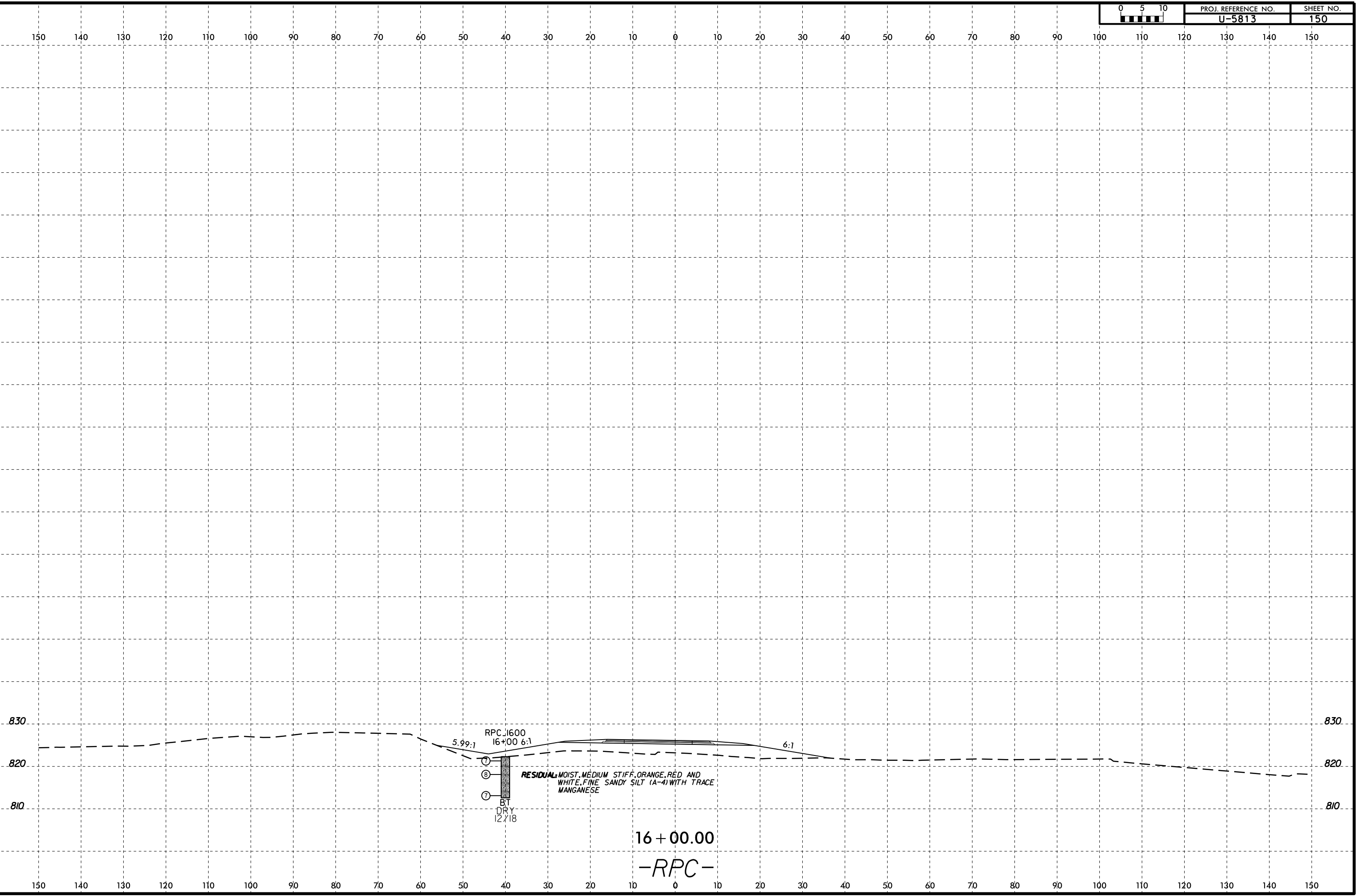
(A) ARTIFICIAL FILL, MOIST, MEDIUM STIFF, TAN TO RED, FINE SANDY SILT (A-4)

RESIDUAL, MOIST, MEDIUM STIFF, RED, TAN, ORANGE AND PALE OLIVE, FINE SANDY SILT (A-4) AND CLAYEY SILT (A-5) WITH TRACE MANGANESE

RPC 1400
14+00
BT
12/18

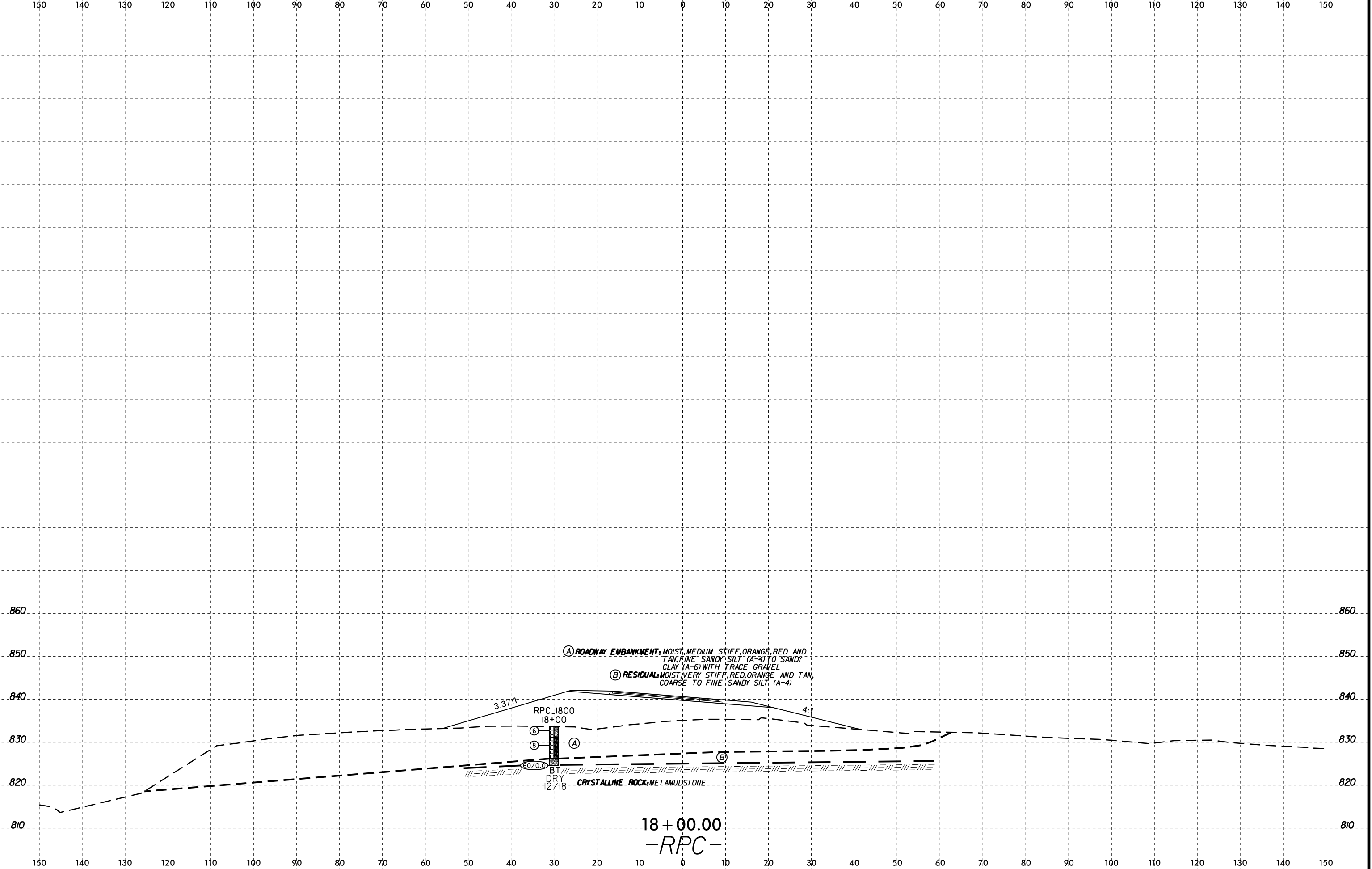
14 + 00.00
-RPC-

6/23/16
14-OCT-2019 17:54
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SPopke AT K&20916

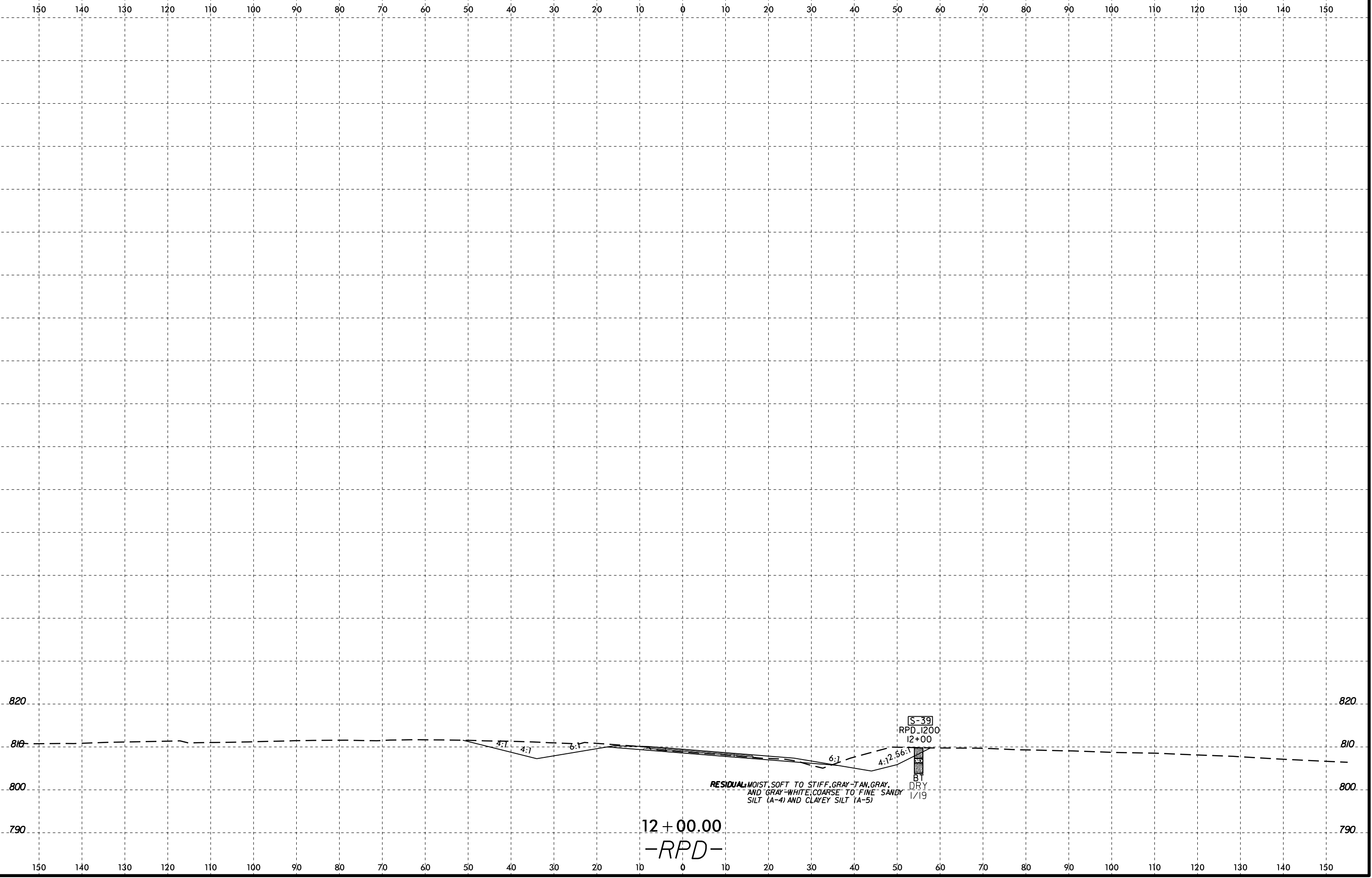


16 + 00.00
-RPC-

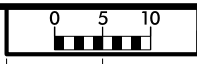
15-OCT-2018 14:41
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S:\spoke



6/23/16
14-OCT-2019 17:54
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SPopke AT K6209516

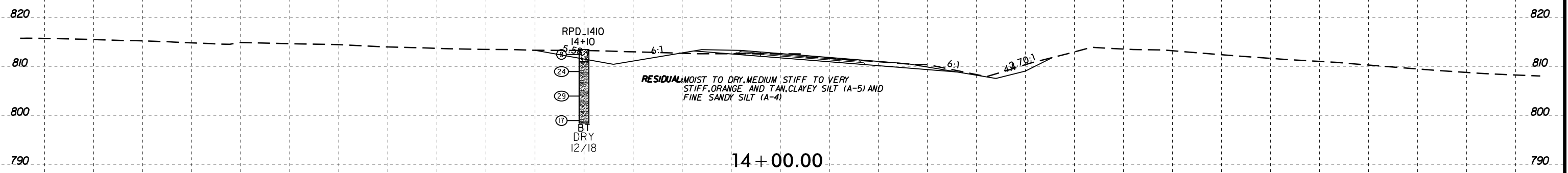


6/23/16



PROJ. REFERENCE NO.	SHEET NO.
U-5813	153

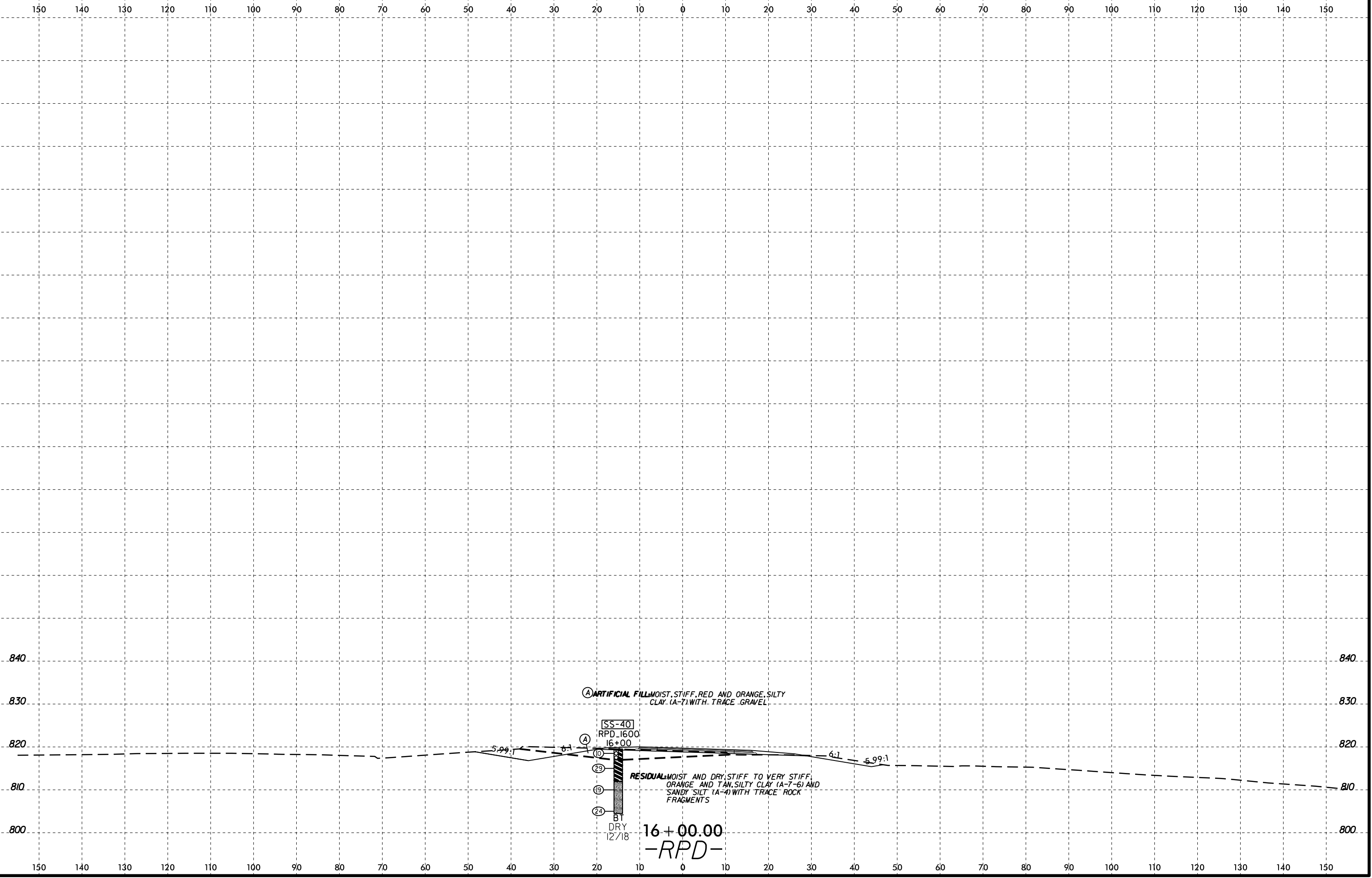
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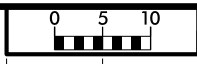
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S:\pape AT K209516

150 140 130 120 110 100 90 80 70 60 50 40 30 20 10 0 10 20 30 40 50 60 70 80 90 100 110 120 130 140 150

6/23/16
14-OCT-2019 17:54
W:\share\GEO\TECHNICAL\Projects\Active Projects\20190942\09A U-5813 Roadway\U5813.GEO.ROWY\CADD.GEOTECH\sec\U5813.GEO.RPD.XSI.dgn
SPopke AT K6208516

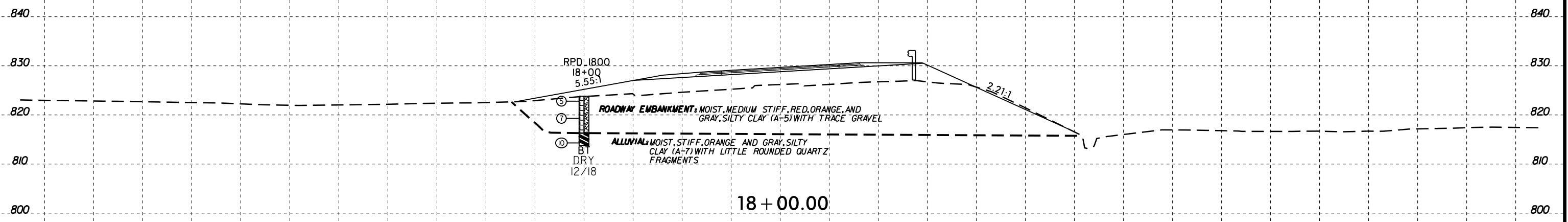


6/23/16



PROJ. REFERENCE NO.	SHEET NO.
U-5813	155

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RPD: 1800
18+00
5.55:1

⑤
①
⑩
BT
DRY
12'18"

ROADWAY EMBANKMENT: MOIST, MEDIUM STIFF, RED, ORANGE AND GRAY SILTY CLAY (A-5) WITH TRACE GRAVEL

ALLUVIAL: MOIST, STIFF, ORANGE AND GRAY SILTY CLAY (A-7) WITH LITTLE ROUNDED QUARTZ FRAGMENTS

2.21:1

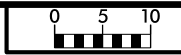
18 + 00.00

-RPD-

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14-OCT-2019 17:54
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SPopke AT K&B 2016

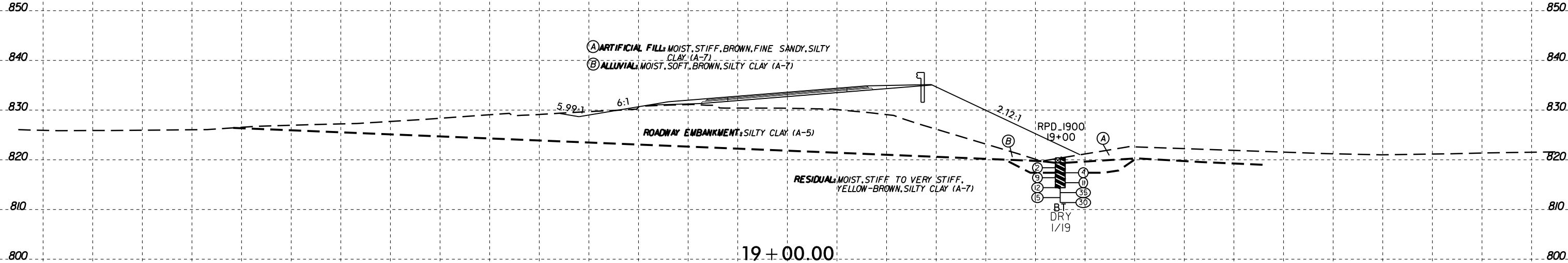
6/23/16



PROJ. REFERENCE NO.
U-5813

SHEET NO.
156

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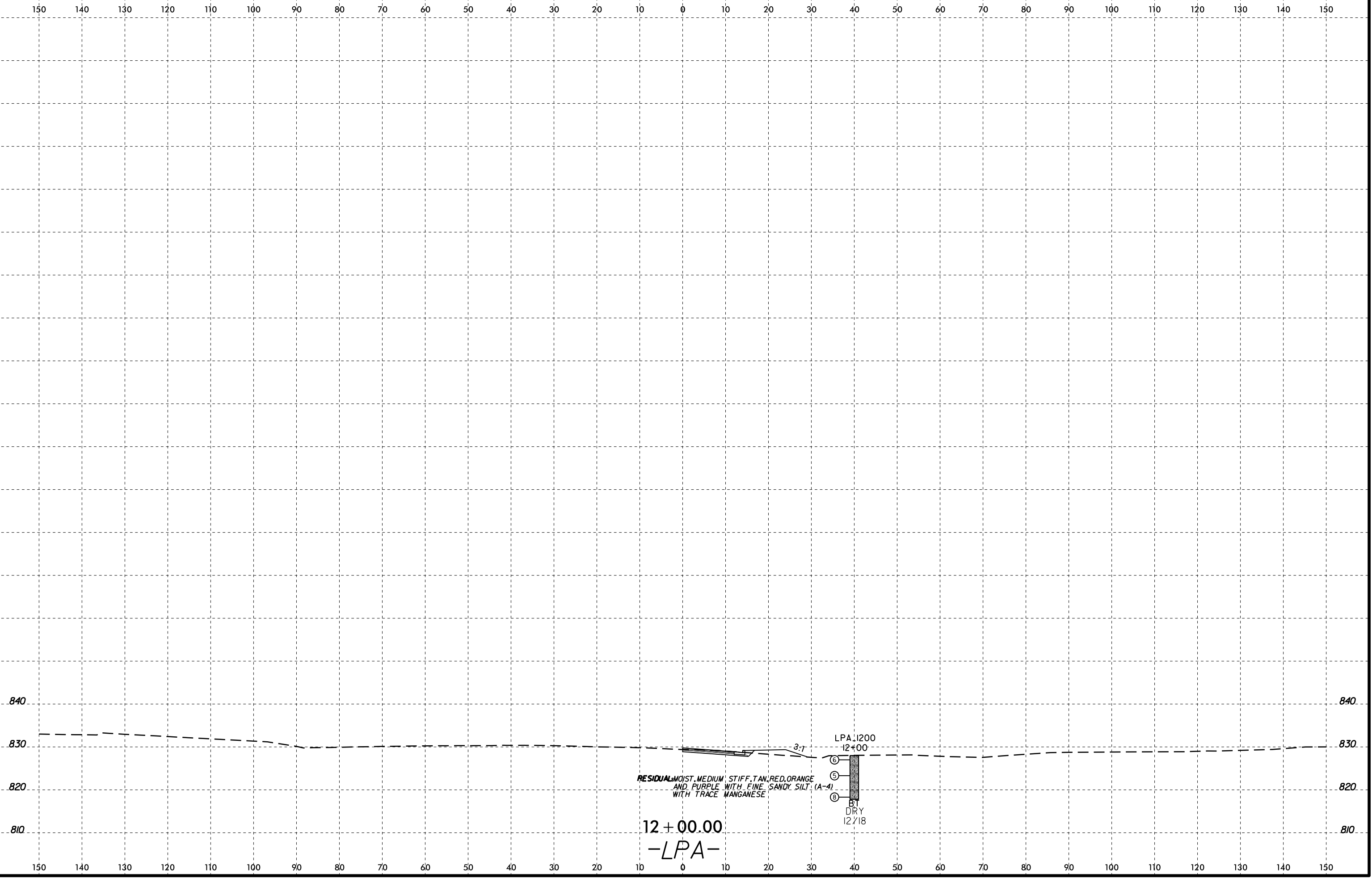
(A) ARTIFICIAL FILL, MOIST, STIFF, BROWN, FINE SANDY, SILTY CLAY (A-7)

(B) ALLUVIAL, MOIST, SOFT, BROWN, SILTY CLAY (A-7)

2
9
2
5
7
10
35
30

14-OCT-2019 17:55
W:\share\GEO\TECHNICAL\Projects\Active Projects\2019\042,009A U-5813 Roadway\U5813_GEO_ROWY\CADD\GEO\TECH\sec\U5813_GEO_RPD_XSI.dgn
SPopke AT K6209516

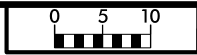
6/23/16
14-OCT-2019 17:55
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Speke AT K209516



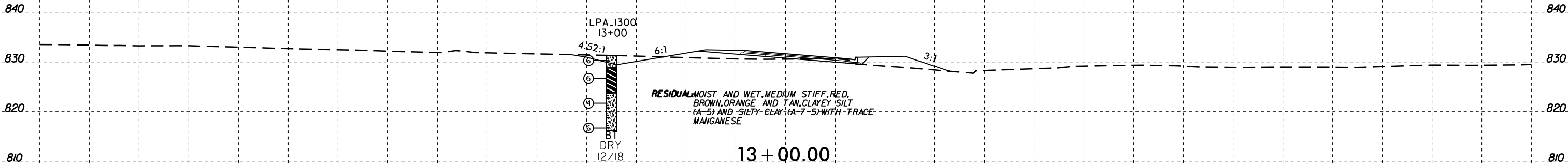
RESIDUAL MOIST, MEDIUM STIFF, TAN, RED, ORANGE
AND PURPLE WITH FINE SANDY SILT (A-4)
WITH TRACE MANGANESE

LPA 1200
12+00
BT
DRY
12/18

12 + 00.00
-LPA-



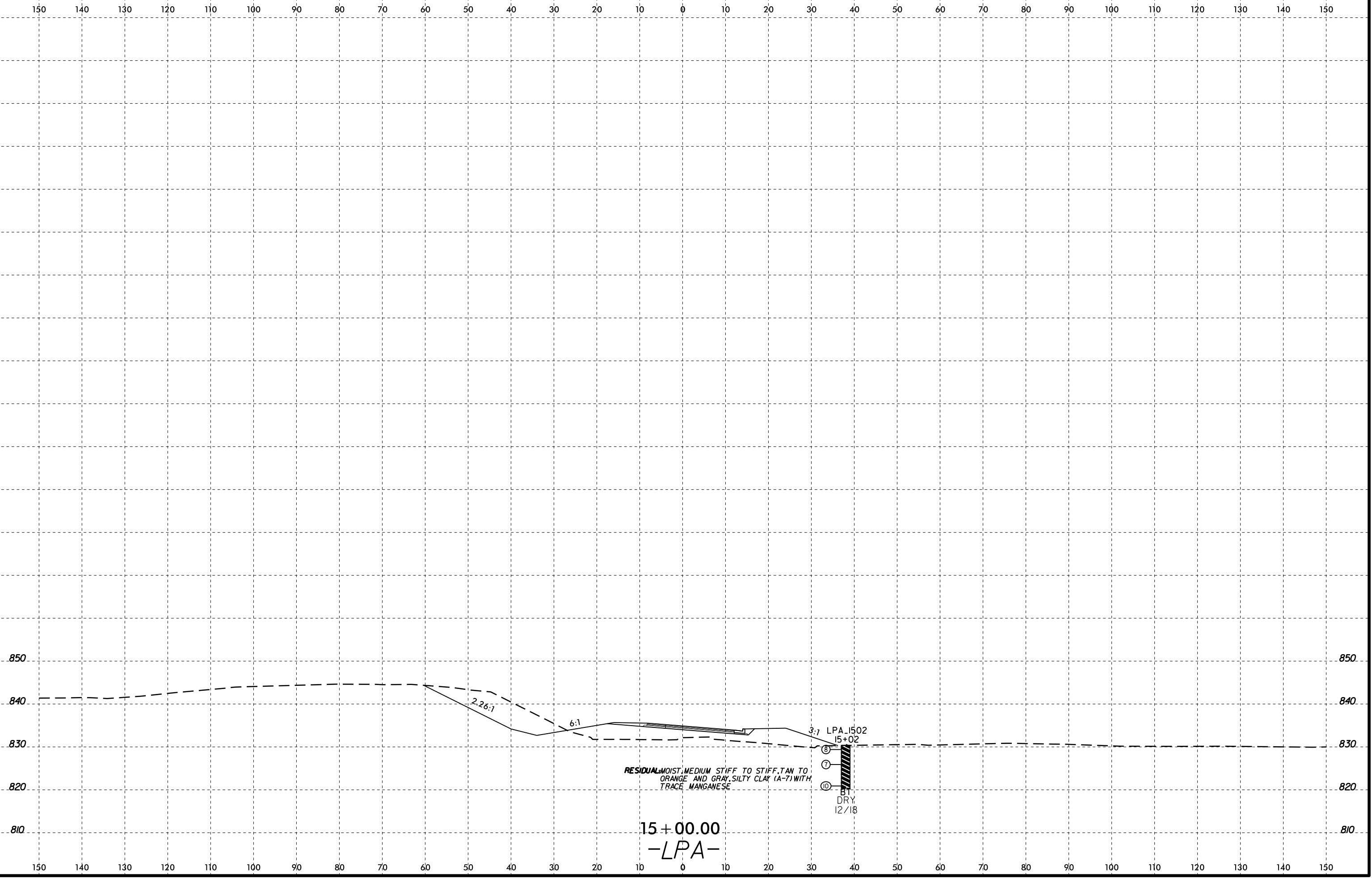
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150 140 130 120 110 100 90 80 70 60 50 40 30 20 10 0 10 20 30 40 50 60 70 80 90 100 110 120 130 140 150

13 + 00.00
-LPA-

6/23/16
14-OCT-2019 17:55
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SPopke AT K209516



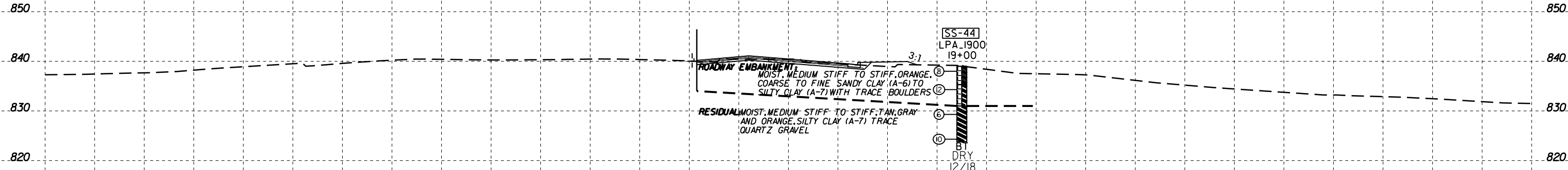
6/23/16



PROJ. REFERENCE NO.
U-5813

SHEET NO.
160

150 140 130 120 110 100 90 80 70 60 50 40 30 20 10 0 10 20 30 40 50 60 70 80 90 100 110 120 130 140 150



ROADWAY EMBANKMENT
MOIST, MEDIUM STIFF TO STIFF, ORANGE,
COARSE TO FINE SANDY CLAY (A-6) TO
SILTY CLAY (A-7) WITH TRACE BOULDERS

RESIDUAL MOIST, MEDIUM STIFF TO STIFF, TAN, GRAY
AND ORANGE, SILTY CLAY (A-7) TRACE
QUARTZ GRAVEL

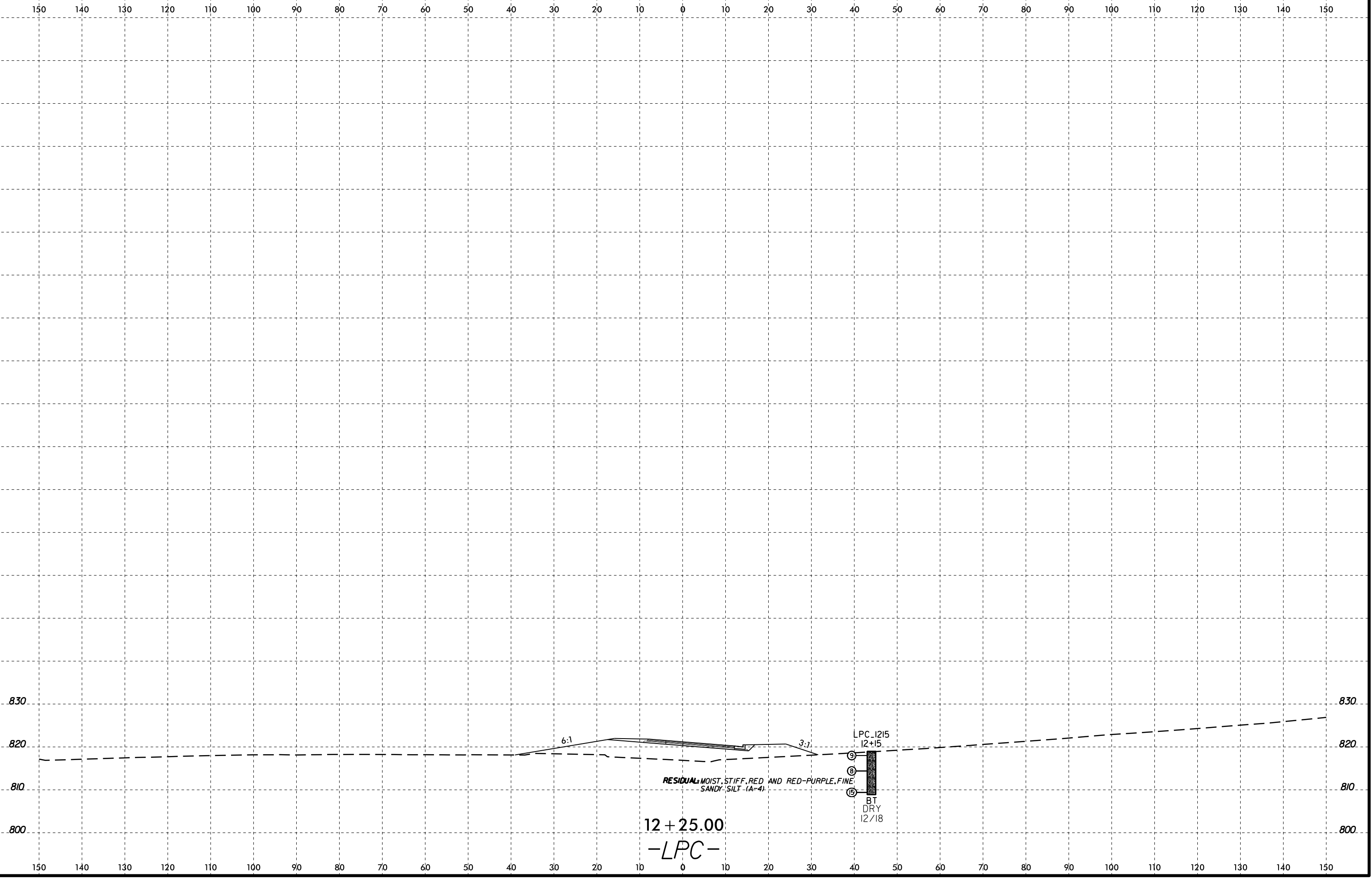
SS-44
LPA.1900
19+00
B1
DRY
12/18

19+00.00
-LPA-

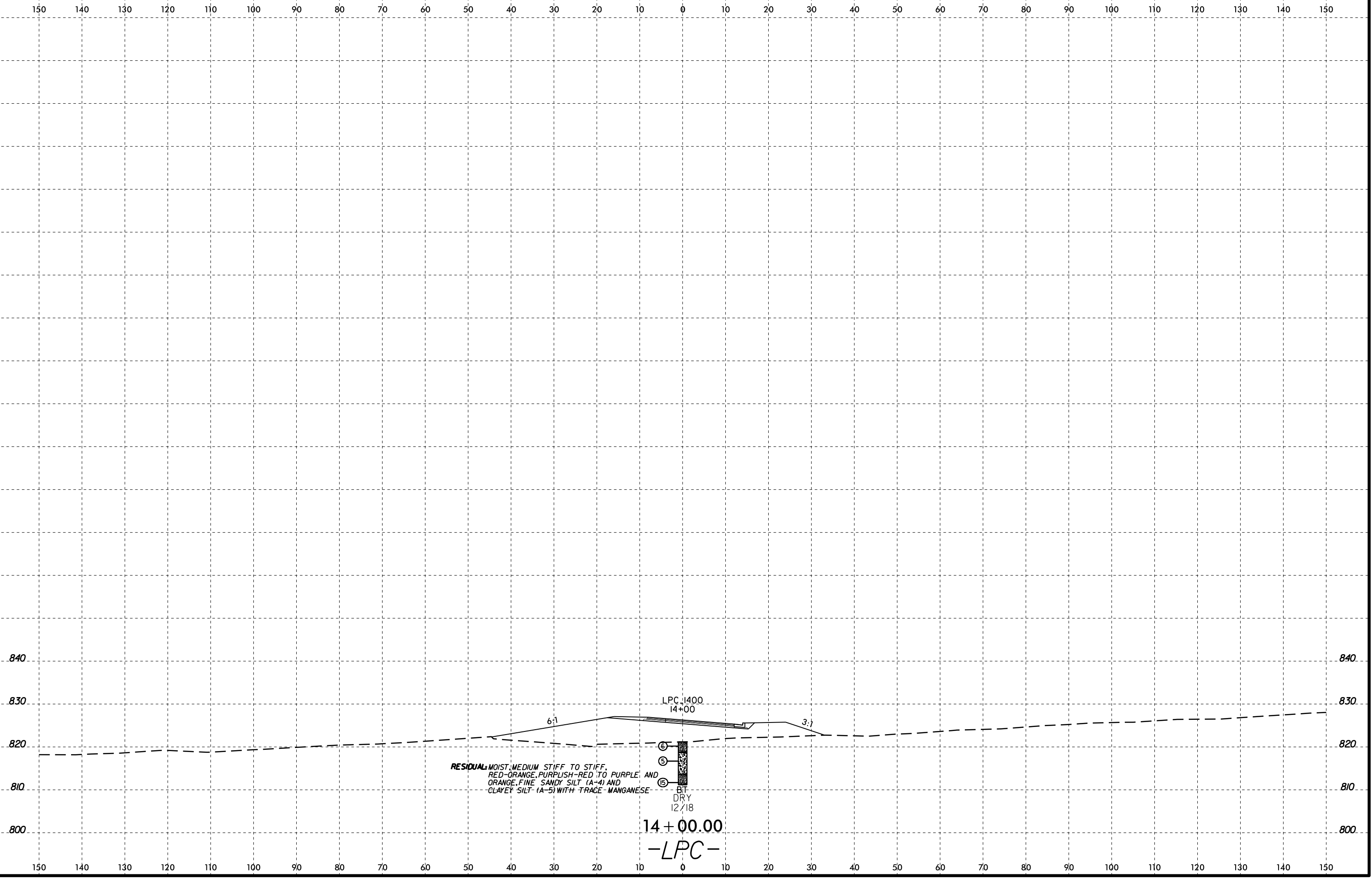
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14-OCT-2019 17:55
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SPopke AT K&209516

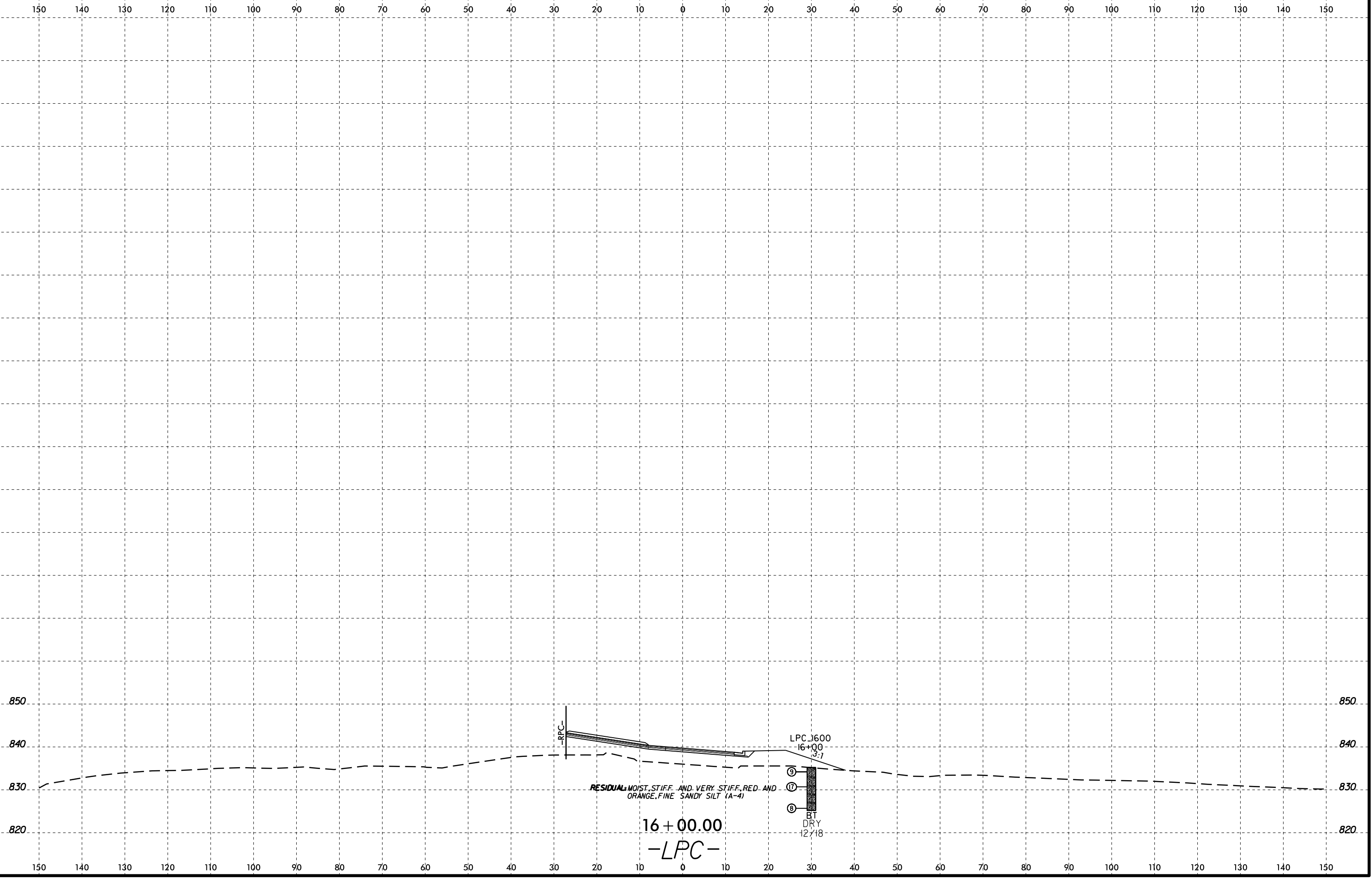
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SPopke AT K&20916



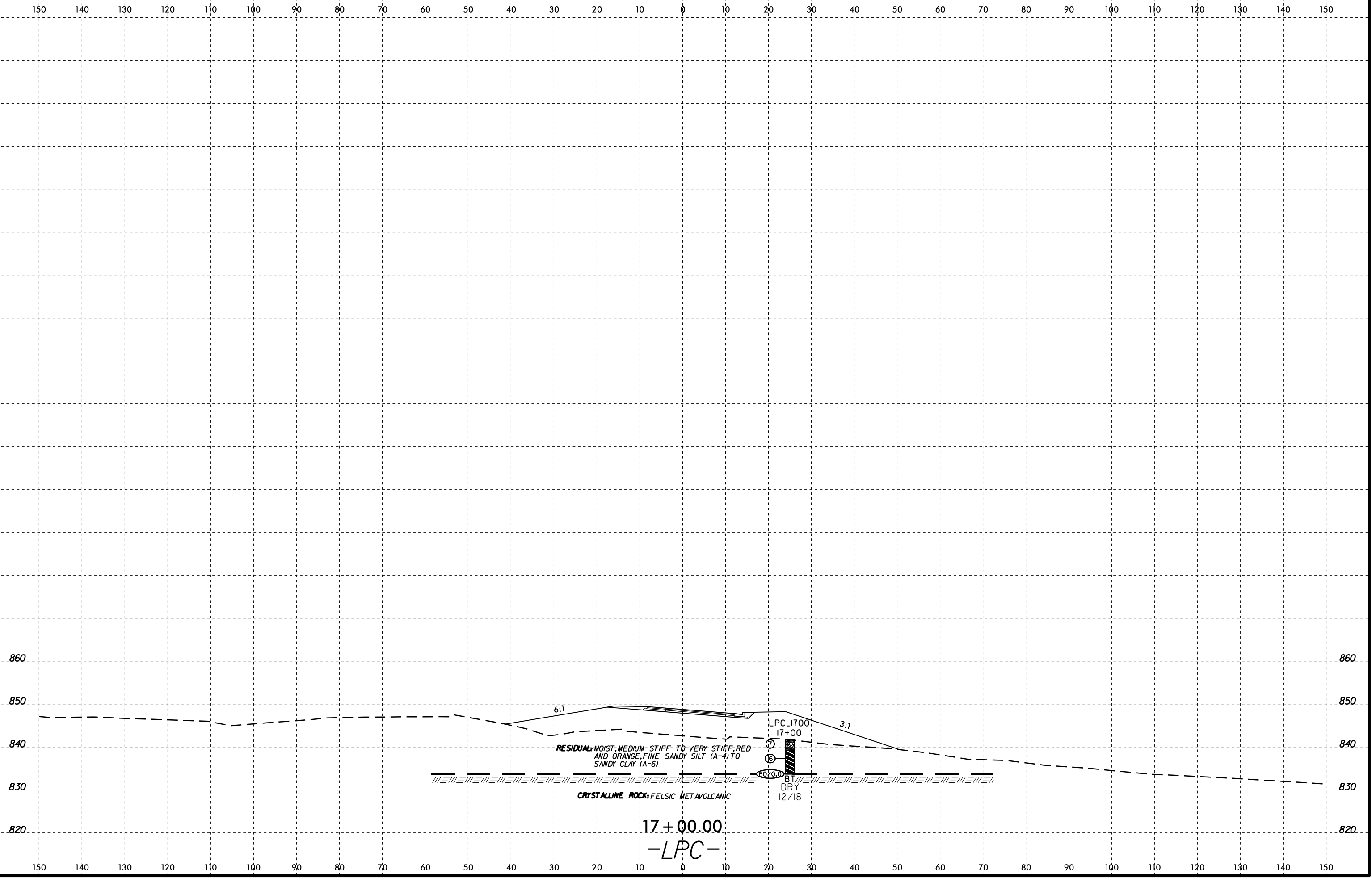
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14-OCT-2019 17:55
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S:\pape AT K&209516



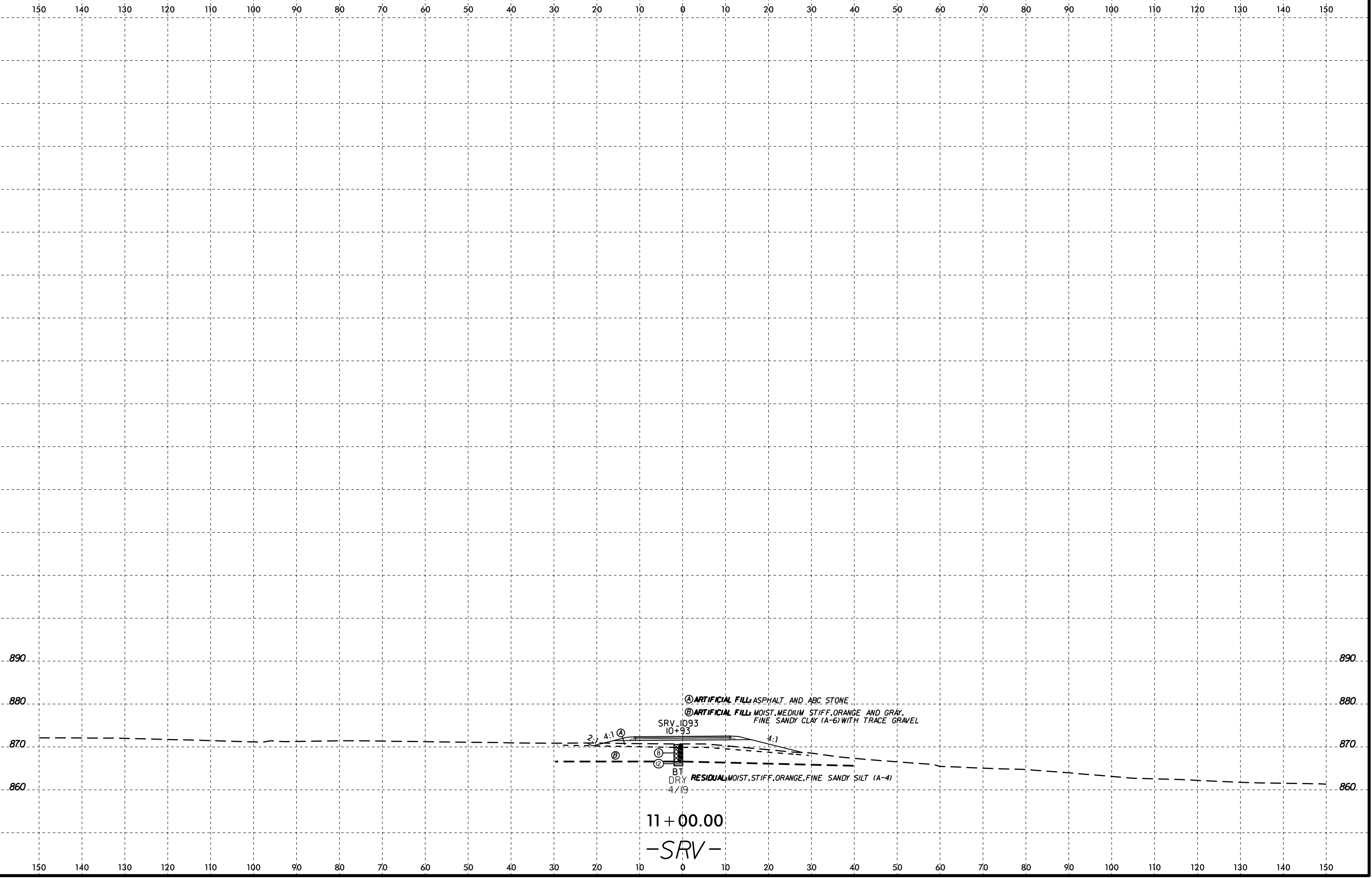
6/23/16
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S:\pape AT K&20916



6/23/16
14-OCT-2019 17:55
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SPopke AT K&B20916



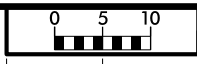
6/23/16
14-OCT-2019 17:55
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SRV AT K6209516



④ ARTIFICIAL FILL ASPHALT AND ABC STONE
③ ARTIFICIAL FILL MOIST, MEDIUM STIFF, ORANGE AND GRAY,
FINE SANDY CLAY (A-6) WITH TRACE GRAVEL
SRV_1093
10+93
BT
DRY
4/19
RESIDUAL MOIST, STIFF, ORANGE, FINE SANDY SILT (A-4)

11+00.00
-SRV-

6/23/16

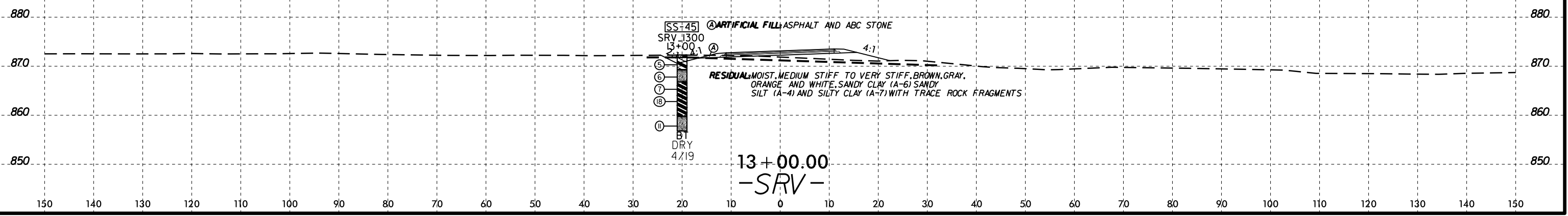


PROJ. REFERENCE NO.
U-5813

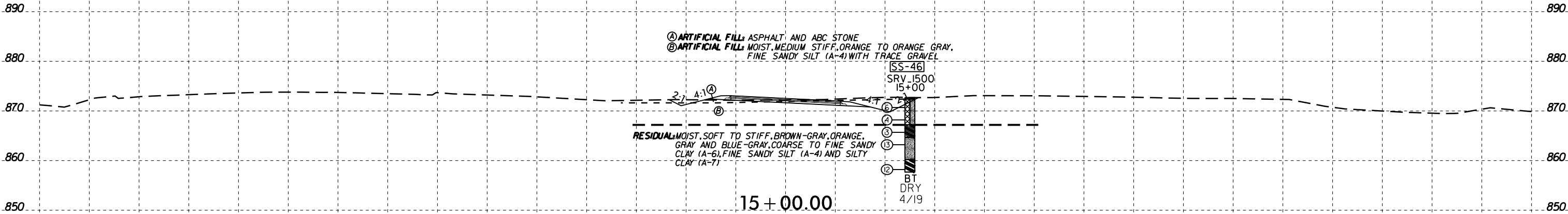
SHEET NO.
166

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SPopke AT K&209516



150 140 130 120 110 100 90 80 70 60 50 40 30 20 10 0 10 20 30 40 50 60 70 80 90 100 110 120 130 140 150



15 + 00.00
-SRV-

NORTH CAROLINA DEPARTMENT OF TRANSPORTATION
DIVISION OF HIGHWAYS
GEOTECHNICAL ENGINEERING UNIT
SUBSURFACE INVESTIGATION
APPENDIX A
LABORATORY RESULTS

REFERENCE: U-5813

PROJECT: 44385

DS
JRW

Prepared in the Office of:



LABORATORY SUMMARY SHEET FOR SOIL SAMPLES

SHEET 169

PROJECT NO.: 44385.1.3 (U-5813)

COUNTY: RANDOLPH

US 64 FROM ASHEBORO BYPASS TO EAST OF I-73/I-74/US 220

Sample No.	New Boring Number	Alignment	Station	Offset	Sample Depth (ft.)	Natural Moisture Content (%)	AASHTO Class.	Atterberg Limits			Gradation Results							
								L.L.	P.L.	P.I.	Retained #4 Sieve	Pass #10 Sieve	Pass #40 Sieve	Pass #200 Sieve	Coarse Sand (%)	Fine Sand (%)	Silt (%)	Clay (%)
SS-1	L_3400	-L-	34+00	115' LT	0.0-1.5	30.3	A-7-5	72	34	38	2.4	97.2	96.5	89.9	1.2	12.3	16.8	69.7
SS-2	L_3600	-L-	36+00	85' RT	0.0-1.5	20.3	A-6	37	26	11	1.6	96.9	95.8	90.8	1.7	10.3	49.6	38.4
SS-3	L_4000	-L-	40+00	70' LT	0.0-1.5	31.6	A-7-5	71	44	27	0.2	99.4	98.4	92.3	1.4	10.7	23.2	64.7
SS-4	L_4200	-L-	42+00	65' RT	0.0-1.5	27.3	A-7-6	42	24	18	4.7	94.0	92.1	84.0	2.9	14.6	35.1	47.4
SS-5	L_4400	-L-	44+00	75' RT	0.0-1.5	35.0	A-7-5	77	34	43	0.0	99.9	99.5	97.4	0.7	3.1	22.4	73.8
SS-6	L_4600	-L-	46+00	80' LT	0.0-1.5	19.1	A-6	38	22	16	5.7	93.3	91.5	86.2	2.6	9.1	44.9	43.4
ST-1	L_4800	-L-	48+00	50' RT	0.0-2.0	19.2	A-4	26	18	8	0.1	99.5	98.0	90.5	2.5	12.3	57.3	27.9
SS-8	L_5050	-L-	50+50	80' RT	0.0-1.5	25.9	A-7-5	59	33	26	0.0	99.9	99.2	95.7	1.2	5.5	31.5	61.8
SS-9	L_5850	-L-	58+50	65' LT	0.0-1.5	26.4	A-7-5	62	33	29	0.0	99.9	99.4	98.0	0.8	2.1	33.9	63.2
SS-10	L_6050	-L-	60+50	70' RT	0.0-1.5	20.5	A-7-6	49	27	22	0.7	97.4	94.1	88.7	4.8	6.2	37.9	51.1
SS-11	L_6250	-L-	62+50	95' LT	0.0-1.5	27.9	A-7-5	64	31	33	0.0	99.9	99.3	97.5	0.9	3.7	33.8	61.6
SS-12	L_6450	-L-	64+50	100' LT	0.0-1.5	28.5	A-7-5	69	33	36	0.0	100.0	99.6	99.0	0.4	1.2	29.9	68.5
SS-13	L_7050	-L-	70+50	100' LT	0.0-1.5	24.6	A-7-5	78	31	47	0.1	99.3	98.0	90.4	0.9	7.1	33.9	58.1
ST-2	L_7250	-L-	72+50	90' RT	0.0-2.0	--	A-6	31	20	11	3.8	84.1	75.6	62.3	15.4	15.2	45.5	23.9
SS-14	L_7450	-L-	74+50	75' LT	3.4-4.9	39.1	A-7-5	55	39	13	1.4	98.5	97.5	93.1	1.4	6.9	42.3	49.4
SS-15	L_8249	-L-	82+49	46' RT	1.0-2.5	18.6	A-4	35	27	8	1.3	98.2	97.0	85.2	2.1	19.9	45.5	32.5
SS-16	L_9509	-L-	95+09	40' RT	0.0-1.5	17.4	A-6	30	19	11	5.3	92.5	89.0	78.9	6.4	13.7	43.5	36.4
SS-17	L_11800	-L-	118+00	55' RT	3.4-4.9	25.2	A-7-5	48	34	14	3.4	95.5	93.0	87.2	3.6	9.6	42.5	44.3
SS-18	Y_1888	-Y-	18+88	50' LT	0.0-1.5	--	A-4	28	20	8	0.0	99.7	98.1	93.6	2.5	5.8	72.0	19.6
SS-19	Y_2500	-Y-	25+00	30' RT	0.0-1.5	--	A-7-6	46	16	30	8.5	91.0	81.9	71.4	13.1	12.3	44.1	30.5
SS-20	Y_1200	-Y-	12+00	30' LT	0.0-1.5	30.3	A-4	36	33	3	1.5	97.8	97.2	92.8	1.2	8.8	63.3	26.7
SS-21	Y_1595	-Y-	15+95	55' RT	1.0-2.5	17.2	A-6	36	24	12	4.5	91.7	89.3	84.8	3.6	7.5	52.6	36.3
SS-22	Y_4457	-Y-	44+57	49' RT	3.4-4.9	35.7	A-7-5	54	41	13	0.0	100.0	99.7	97.3	0.7	3.6	37.4	58.3
SS-23	Y_4646	-Y-	46+46	41' LT	3.6-5.1	30.4	A-7-5	73	42	31	0.0	99.9	99.7	97.1	0.4	4.3	31.1	64.2
SS-24	Y3_1025	-Y3-	10+25	30' RT	0.0-1.5	29.2	A-7-5	74	34	40	0.0	99.9	99.4	95.9	0.7	7.2	30.9	61.2
SS-25	Y6_1100	-Y6-	11+00	30' RT	0.0-1.5	26.7	A-7-6	43	27	16	0.0	100.0	99.7	97.2	0.5	5.1	41.6	52.8
SS-26	Y12_1900	-Y12-	19+00	34' LT	0.0-1.5	28.9	A-5	41	32	9	21.3	77.7	76.1	70.9	3.4	8.5	51.6	36.5
SS-27	Y13_1391	-Y13-	13+91	CL	0.0-1.5	25.4	A-7-6	45	26	19	0.0	100.0	99.8	96.8	0.3	6.0	62.9	30.8
BS-1	Y17_1093	-Y17-	10+93	103' LT	0.0-10.0	26.5	A-7-5	48	31	17	2.7	96.1	95.4	93.1	1.0	4.4	42.2	52.4
SS-28	Y17_1104	-Y17-	11+04	54' LT	0.0-1.5	23.9	A-6	32	20	12	10.6	88.3	85.5	79.0	4.2	10.3	57.6	27.9
SS-29	Y17_1465	-Y17-	14+65	20' LT	3.4-4.9	16.6	A-6	34	23	11	9.0	89.8	88.0	84.2	2.7	6.8	58.6	31.9
SS-30	Y17_1665	-Y17-	16+65	10' LT	3.4-4.9	29.4	A-7-5	52	31	21	0.0	100.0	99.5	95.3	0.9	7.4	41.4	50.3
S-31	Y17A_1200	-Y17A-	12+00	40' LT	2.0-4.0	19.0	A-4	22	20	2	0.2	99.6	98.5	93.8	1.8	7.3	62.0	28.9
S-32	Y18_1075	-Y18-	10+75	50' RT	0.0-3.0	32.1	A-7-5	58	33	25	0.5	98.9	97.7	93.1	2.1	5.6	34.1	58.2
SS-33	RPA_1294	-RPA-	12+94	11' RT	3.6-5.1	36.3	A-7-5	57	33	24	1.2	98.1	98.7	95.5	1.9	4.4	36.4	57.3
SS-34	RPA_1493	-RPA-	14+93	18' RT	0.0-1.5	29.5	A-7-5	47	31	16	5.5	93.7	98.4	95.3	2.1	4.4	41.5	52.0
SS-35	RPA_1837	-RPA-	18+37	58' RT	3.5-5.0	29.4	A-7-5	50	34	16	0.0	100.0	99.9	96.3	0.2	7.3	39.2	53.3
SS-36	RPB_1250	-RPB-	12+50	35' LT	3.8-5.3	26.8	A-7-5	47	36	11	0.6	98.6	99.0	94.3	1.8	6.5	42.7	49.0
S-37	RPB_1450	-RPB-	14+50	40' RT	0.0-3.0	32.1	A-7-5	63	30	33	0.1	99.8	99.1	95.1	1.3	6.0	41.4	51.3

LABORATORY SUMMARY SHEET FOR SOIL SAMPLES

SHEET 170

PROJECT NO.: 44385.1.3 (U-5813)

COUNTY: RANDOLPH

US 64 FROM ASHEBORO BYPASS TO EAST OF I-73/I-74/US 220

Sample No.	New Boring Number	Alignment	Station	Offset	Sample Depth (ft.)	Natural Moisture Content (%)	AASHTO Class.	Atterberg Limits			Gradation Results							
								L.L.	P.L.	P.I.	Retained #4 Sieve	Pass #10 Sieve	Pass #40 Sieve	Pass #200 Sieve	Coarse Sand (%)	Fine Sand (%)	Silt (%)	Clay (%)
SS-38	RPC_1200	-RPC-	12+00	80' LT	18.6-20.1	37.3	A-5	46	36	10	0.0	100.0	99.4	91.2	1.3	12.7	44.6	41.4
S-39	RPD_1200	-RPD-	12+00	55' LT	0.0-2.5	23.1	A-4	29	21	8	13.2	86.8	82.7	75.6	6.5	9.8	54.2	29.5
SS-40	RPD_1600	-RPD-	16+00	15' LT	3.5-5.0	17.8	A-7-6	41	24	17	17.9	77.1	64.6	51.7	20.6	17.2	24.5	37.7
BS-2	RPDA_1150	-RPDA-	11+50	40' LT	0.0-10.0	20.6	A-7-5	47	30	17	8.7	88.4	84.9	79.8	5.1	7.8	38.4	48.7
SS-41	RPDA_1350	-RPDA-	13+50	40' RT	0.0-1.5	31.0	A-7-5	60	33	27	0.0	99.4	96.4	89.6	5.0	6.9	28.1	60.0
SS-42	RPDA_1549	-RPDA-	15+49	75' LT	13.4-14.9	44.4	A-7-5	55	44	11	0.0	100.0	99.8	95.0	0.5	8.7	49.8	41.0
SS-43	LPA_1300	-LPA-	13+00	35' LT	3.6-5.1	52.3	A-7-5	56	41	15	0.0	99.7	98.2	89.2	2.8	11.8	36.6	48.8
SS-44	LPA_1900	-LPA-	19+00	35' RT	3.7-5.2	27.5	A-7-5	54	33	21	0.8	98.0	95.9	90.3	2.9	9.3	29.0	58.8
SS-45	SRV_1300	-SRV-	13+00	20' LT	1.0-2.5	21.8	A-6	35	22	13	8.1	91.3	84.9	80.6	8.7	5.4	53.1	32.8
SS-46	SRV_1500	-SRV-	15+00	25' RT	6.0-7.5	19.4	A-6	30	17	13	0.2	99.6	95.5	87.0	6.3	9.8	58.0	26.0



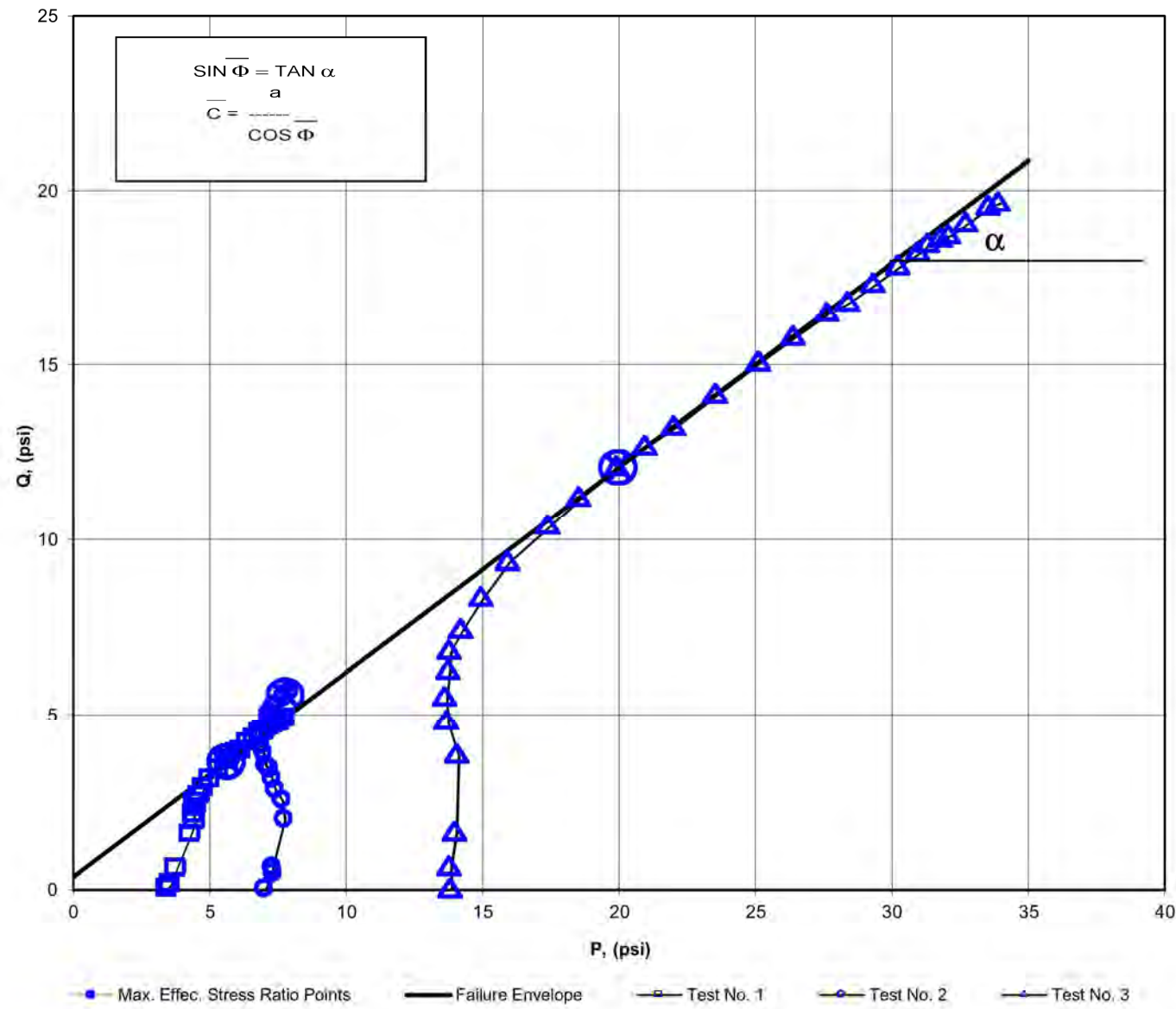
**CONSOLIDATED UNDRAINED TRIAXIAL TEST
WITH PORE PRESSURE READINGS
AASHTO T-297**

**MOHR TOTAL STRENGTH ENVELOPE
AASHTO T-297**

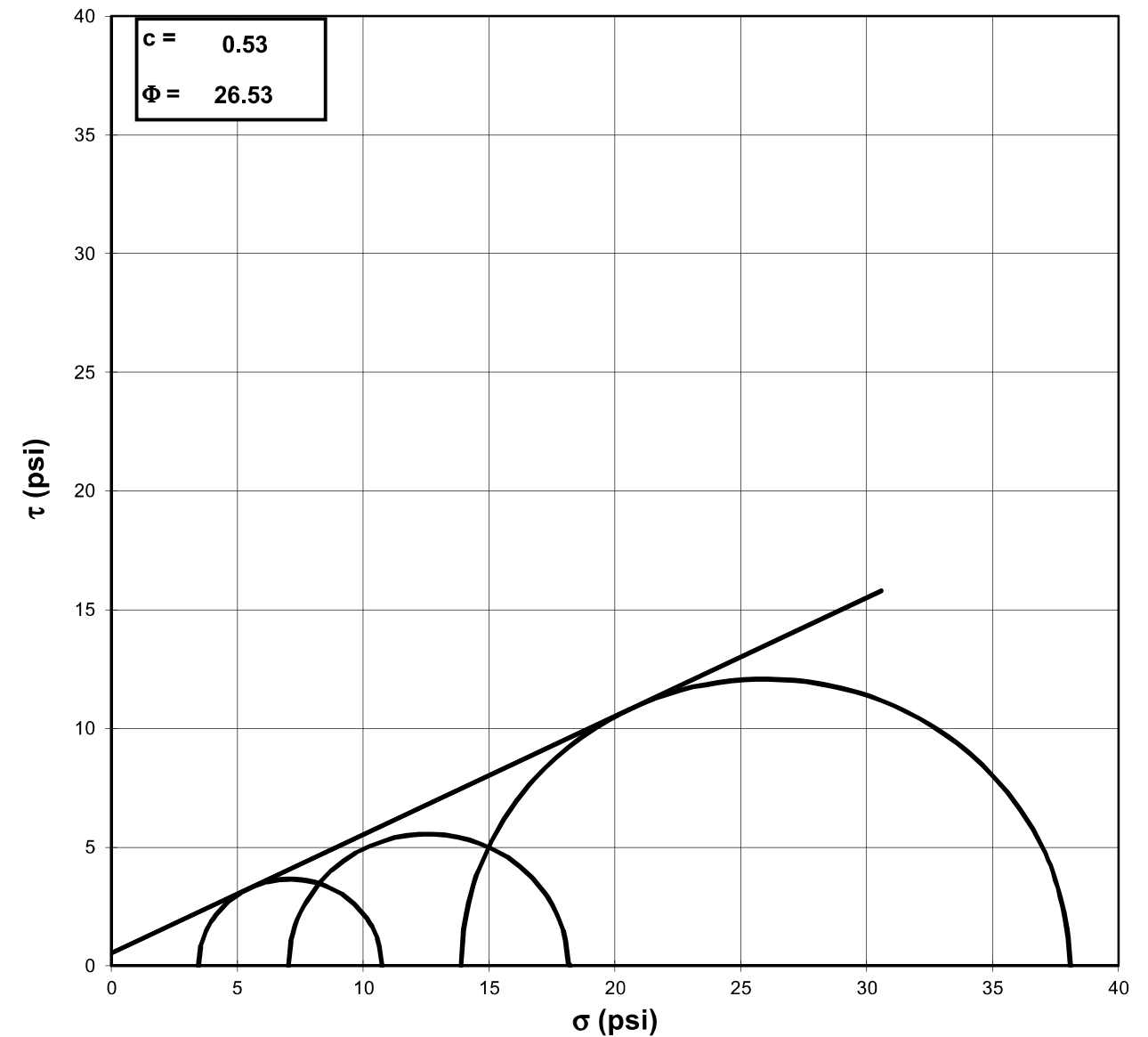
Client: Kleinfelder Boring No.: L_4800
 Client Reference: U-5813 Depth (ft): 0-2.0
 Project No.: R-2019-011-001 Sample No.: ST-1
 Lab ID: R-2019-011-001-001

Client: Kleinfelder Boring No.: L_4800
 Client Reference: U-5813 Depth (ft): 0-2.0
 Project No.: R-2019-011-001 Sample No.: ST-1
 Lab ID: R-2019-011-001-001
 Visual Description: TAN ORANGE CLAY WITH ORGANICS (UNDISTURBED)

Consolidated Undrained Triaxial Test with Pore Pressure



a	=	0.37	C̄	=	0.46
α	=	30.3	Φ̄	=	35.83



Failure Based on Maximum Effective Principal Stress Ratio

NOTE: GRAPH NOT TO SCALE

Tested By: 129-04-0411 Date: 1/21/19 Approved By: MPS Date: 1/29/19

Tested By: 129-04-0411 Date: 1/21/19 Approved By: MPS Date: 1/29/19



**CONSOLIDATED UNDRAINED TRIAXIAL TEST
WITH PORE PRESSURE READINGS**
AASHTO T-297

Client: Kleinfelder Boring No.: L_4800
 Client Reference: U-5813 Depth (ft): 0-2.0
 Project No.: R-2019-011-001 Sample No.: ST-1
 Lab ID: R-2019-011-001-001

Visual Description: TAN ORANGE CLAY WITH ORGANICS (UNDISTURBED)

Stage No.	1	INITIAL SAMPLE DIMENSIONS (in)			
Test No.	1	Length 1:	5.753	Diameter 1:	2.862
		Length 2:	5.751	Diameter 2:	2.814
		Length 3:	5.776	Diameter 3:	2.815
		Length 4:	5.745	Diameter 4:	2.834
		Avg. Length:	5.756	Avg. Diam.:	2.831
PRESSURES (psi)					
Cell Pressure (psi)	53.5				
Back Pressure (psi)	50.0				
Eff. Conf. Pressure (psi)	3.5				
Pore Pressure Response (%)	100				
VOLUME CHANGE					
		Initial Burette Reading (ml)	24.0		
		Final Burette Reading (ml)	13.5		
		Final Change (ml)	10.5		
		Initial Dial Reading (mil)	238		
		Dial Reading After Saturation (mil)	231		
		Dial Reading After Consolidation (mil)	246		
MAXIMUM OBLIQUITY POINTS					
\bar{P}	=	5.60			
Q	=	3.65			

LOAD (LB)	DEFORMATION (IN)	PORE PRESSURE (PSI)
5.8	0.000	50.0
7.4	0.002	50.0
13.1	0.003	50.3
25.2	0.008	50.7
29.9	0.014	50.9
32.2	0.019	51.2
34.9	0.028	51.3
36.9	0.037	51.5
39.1	0.048	51.5
42.0	0.068	51.6
45.3	0.097	51.6
49.3	0.130	51.5
52.5	0.165	51.5
53.7	0.205	51.5
56.4	0.234	51.4
57.6	0.273	51.3
60.7	0.329	51.2
63.1	0.387	51.1
65.9	0.430	51.1
66.9	0.486	51.0
67.7	0.528	51.0
70.2	0.572	50.9
71.3	0.616	50.9
71.9	0.644	50.9
73.7	0.673	50.8
72.9	0.700	50.7
74.4	0.729	50.7
74.5	0.772	50.7
76.6	0.815	50.6
77.3	0.858	50.6
79.8	0.901	50.6

Tested By: 129-04-0411 Date: 1/21/19 Input Checked By: GEM Date: 1/29/19



**CONSOLIDATED UNDRAINED TRIAXIAL TEST
WITH PORE PRESSURE READINGS**
AASHTO T-297

Client: Kleinfelder Boring No.: L_4800
 Client Reference: U-5813 Depth (ft): 0-2.0
 Project No.: R-2019-011-001 Sample No.: ST-1
 Lab ID: R-2019-011-001-001

Visual Description: TAN ORANGE CLAY WITH ORGANICS (UNDISTURBED)

Effective Confining Pressure (psi)	3.5	Stage No.	1
		Test No.	1

INITIAL DIMENSIONS		VOLUME CHANGE	
Initial Sample Length (in)	5.76	Volume After Consolidation (in ³)	35.73
Initial Sample Diameter (in)	2.83	Length After Consolidation (in)	5.75
Initial Sample Area (in ²)	6.30	Area After Consolidation (in ²)	6.216
Initial Sample Volume (in ³)	36.24		

Strain (%)	Deviation Stress	ΔU	$\bar{\sigma}_1$	$\bar{\sigma}_3$	Effective Principle Stress Ratio	\bar{A}	\bar{P}	Q
0.03	0.25	0.00	3.70	3.5	1.071	0.00	3.58	0.12
0.05	1.17	0.23	4.40	3.2	1.362	0.19	3.81	0.58
0.14	3.11	0.68	5.89	2.8	2.122	0.22	4.33	1.56
0.24	3.85	0.89	6.42	2.6	2.505	0.23	4.49	1.93
0.33	4.23	1.14	6.55	2.3	2.824	0.27	4.43	2.11
0.48	4.65	1.24	6.86	2.2	3.103	0.27	4.53	2.32
0.64	4.96	1.43	6.98	2.0	3.451	0.29	4.50	2.48
0.84	5.31	1.45	7.31	2.0	3.651	0.27	4.66	2.65
1.18	5.75	1.54	7.67	1.9	4.003	0.27	4.79	2.88
1.68	6.24	1.53	8.16	1.9	4.244	0.25	5.04	3.12
2.27	6.84	1.50	8.79	2.0	4.495	0.22	5.37	3.42
2.87	7.29	1.50	9.25	2.0	4.738	0.21	5.60	3.65
3.57	7.43	1.42	9.46	2.0	4.653	0.19	5.75	3.71
4.07	7.80	1.33	9.92	2.1	4.672	0.17	6.02	3.90
4.75	7.93	1.26	10.13	2.2	4.606	0.16	6.16	3.96
5.72	8.32	1.19	10.58	2.3	4.683	0.14	6.42	4.16
6.73	8.59	1.07	10.98	2.4	4.598	0.12	6.68	4.29
7.48	8.93	1.03	11.36	2.4	4.689	0.12	6.89	4.47
8.46	8.99	0.94	11.51	2.5	4.578	0.10	7.01	4.50
9.18	9.04	0.93	11.56	2.5	4.575	0.10	7.05	4.52
9.94	9.32	0.86	11.91	2.6	4.598	0.09	7.25	4.66
10.71	9.41	0.83	12.03	2.6	4.585	0.09	7.33	4.70
11.21	9.43	0.82	12.06	2.6	4.582	0.09	7.35	4.72
11.70	9.64	0.74	12.36	2.7	4.557	0.08	7.53	4.82
12.19	9.47	0.71	12.22	2.7	4.453	0.08	7.48	4.74
12.68	9.63	0.66	12.42	2.8	4.445	0.07	7.61	4.81
13.42	9.56	0.63	12.39	2.8	4.382	0.07	7.61	4.78
14.18	9.77	0.60	12.62	2.9	4.421	0.06	7.74	4.88
14.93	9.78	0.55	12.69	2.9	4.366	0.06	7.80	4.89
15.67	10.04	0.58	12.91	2.9	4.496	0.06	7.89	5.02

**CONSOLIDATED UNDRAINED TRIAXIAL TEST
WITH PORE PRESSURE READINGS**
AASHTO T-297



Client: Kleinfelder Boring No.: L_4800
 Client Reference: U-5813 Depth (ft): 0-2.0
 Project No.: R-2019-011-001 Sample No.: ST-1
 Lab ID: R-2019-011-001-001

Visual Description: TAN ORANGE CLAY WITH ORGANICS (UNDISTURBED)

Stage No.	1	INITIAL SAMPLE DIMENSIONS (in)			
Test No.	2	*Sample did not meet L:D ratio			
		Length 1:	5.128	Diameter 1:	2.771
		Length 2:	5.125	Diameter 2:	2.734
		Length 3:	5.020	Diameter 3:	2.752
		Length 4:	4.998	Diameter 4:	2.778
		Avg. Length:	5.068	Avg. Diam.:	2.759
PRESSURES (psi)		VOLUME CHANGE			
Cell Pressure (psi)	57.0	Initial Burette Reading (ml)	48.0		
Back Pressure (psi)	50.0	Final Burette Reading (ml)	24.7		
Eff. Conf. Pressure (psi)	7.0	Final Change (ml)	23.3		
Pore Pressure Response (%)	99				
MAXIMUM OBLIQUITY POINTS		Initial Dial Reading (mil)	261		
\bar{P}	= 7.78	Dial Reading After Saturation (mil)	297		
Q	= 5.55	Dial Reading After Consolidation (mil)	465		

LOAD (LB)	DEFORMATION (IN)	PORE PRESSURE (PSI)
7.8	0.000	50.0
12.9	0.001	50.1
14.7	0.003	50.3
30.7	0.010	51.2
37.2	0.017	51.9
40.6	0.024	52.4
44.2	0.034	52.8
47.7	0.046	53.2
49.0	0.060	53.5
53.5	0.085	53.9
56.2	0.120	54.2
60.0	0.161	54.4
64.3	0.204	54.6
67.5	0.254	54.7
69.0	0.289	54.7
70.1	0.338	54.8
74.0	0.423	54.9
76.2	0.519	54.9
79.7	0.588	54.8
82.8	0.684	54.8
85.2	0.754	54.7

**CONSOLIDATED UNDRAINED TRIAXIAL TEST
WITH PORE PRESSURE READINGS**
AASHTO T-297



Client: Kleinfelder Boring No.: L_4800
 Client Reference: U-5813 Depth (ft): 0-2.0
 Project No.: R-2019-011-001 Sample No.: ST-1
 Lab ID: R-2019-011-001-001

Visual Description: TAN ORANGE CLAY WITH ORGANICS (UNDISTURBED)

Effective Confining Pressure (psi)	7.0	Stage No.	1
		Test No.	2

INITIAL DIMENSIONS		VOLUME CHANGE	
Initial Sample Length (in)	5.07	Volume After Consolidation (in ³)	28.22
Initial Sample Diameter (in)	2.76	Length After Consolidation (in)	4.86
Initial Sample Area (in ²)	5.98	Area After Consolidation (in ²)	5.803
Initial Sample Volume (in ³)	30.29		

Strain (%)	Deviation Stress	ΔU	$\bar{\sigma}_1$	$\bar{\sigma}_3$	Effective Principle Stress Ratio	\bar{A}	\bar{P}	Q
0.02	0.87	0.10	7.81	6.9	1.126	0.12	7.37	0.44
0.05	1.20	0.30	7.93	6.7	1.178	0.26	7.33	0.60
0.20	3.95	1.25	9.74	5.8	1.682	0.32	7.76	1.97
0.34	5.04	1.87	10.21	5.2	1.976	0.37	7.69	2.52
0.48	5.63	2.39	10.27	4.6	2.212	0.43	7.45	2.81
0.71	6.23	2.82	10.45	4.2	2.480	0.46	7.33	3.12
0.94	6.81	3.22	10.63	3.8	2.783	0.48	7.23	3.41
1.23	7.02	3.46	10.59	3.6	2.964	0.50	7.08	3.51
1.75	7.74	3.90	10.87	3.1	3.471	0.51	7.00	3.87
2.46	8.14	4.19	10.99	2.8	3.858	0.52	6.92	4.07
3.32	8.70	4.42	11.31	2.6	4.323	0.51	6.96	4.35
4.20	9.33	4.56	11.80	2.5	4.766	0.49	7.14	4.66
5.22	9.76	4.71	12.09	2.3	5.189	0.49	7.21	4.88
5.94	9.92	4.74	12.22	2.3	5.316	0.48	7.26	4.96
6.96	9.99	4.84	12.18	2.2	5.554	0.49	7.19	5.00
8.70	10.42	4.87	12.58	2.2	5.818	0.47	7.37	5.21
10.66	10.53	4.85	12.71	2.2	5.819	0.47	7.45	5.26
12.08	10.89	4.78	13.15	2.3	5.834	0.44	7.70	5.45
14.07	11.10	4.81	13.33	2.2	5.988	0.44	7.78	5.55
15.49	11.27	4.70	13.60	2.3	5.827	0.42	7.97	5.63

**CONSOLIDATED UNDRAINED TRIAXIAL TEST
WITH PORE PRESSURE READINGS**
AASHTO T-297



Client: Kleinfelder Boring No.: L_4800
 Client Reference: U-5813 Depth (ft): 0-2.0
 Project No.: R-2019-011-001 Sample No.: ST-1
 Lab ID: R-2019-011-001-001

Visual Description: TAN ORANGE CLAY WITH ORGANICS (UNDISTURBED)

Stage No.	1
Test No.	3

INITIAL SAMPLE DIMENSIONS (in)

Length 1:	6.133	Diameter 1:	2.819
Length 2:	6.152	Diameter 2:	2.825
Length 3:	6.168	Diameter 3:	2.834
Length 4:	6.118	Diameter 4:	2.859
Avg. Length:	6.143	Avg. Diam.:	2.834

PRESSURES (psi)

Cell Pressure (psi)	63.9
Back Pressure (psi)	50.0
Eff. Conf. Pressure (psi)	13.9
Pore Pressure Response (%)	98

VOLUME CHANGE

Initial Burette Reading (ml)	24.0
Final Burette Reading (ml)	9.6
Final Change (ml)	14.4

MAXIMUM OBLIQUITY POINTS

\bar{P}	=	20.00
Q	=	12.08

Initial Dial Reading (mil)	288
Dial Reading After Saturation (mil)	275
Dial Reading After Consolidation (mil)	290

LOAD (LB)	DEFORMATION (IN)	PORE PRESSURE (PSI)
5.4	0.000	50.0
12.9	0.001	50.6
25.4	0.003	51.4
52.8	0.008	53.6
64.8	0.014	54.9
73.5	0.020	55.7
83.1	0.029	56.3
90.4	0.038	56.9
98.0	0.051	57.0
109.7	0.073	57.2
123.3	0.103	57.2
137.0	0.141	56.8
148.4	0.178	56.5
160.9	0.221	56.0
169.2	0.252	55.5
177.6	0.295	55.0
191.7	0.355	54.4
205.8	0.416	53.7
217.3	0.463	53.2
228.7	0.524	52.7
235.3	0.571	52.2
244.4	0.618	51.8
253.6	0.664	51.4
260.3	0.695	51.1
265.1	0.726	50.9
269.0	0.757	50.6
271.7	0.788	50.4
279.0	0.835	50.1
288.5	0.882	49.8
291.6	0.912	49.5
297.5	0.943	49.3

Tested By: 129-04-0411 Date: 1/21/19 Input Checked By: GEM Date: 1/29/19

**CONSOLIDATED UNDRAINED TRIAXIAL TEST
WITH PORE PRESSURE READINGS**
AASHTO T-297



Client: Kleinfelder Boring No.: L_4800
 Client Reference: U-5813 Depth (ft): 0-2.0
 Project No.: R-2019-011-001 Sample No.: ST-1
 Lab ID: R-2019-011-001-001

Visual Description: TAN ORANGE CLAY WITH ORGANICS (UNDISTURBED)

Effective Confining Pressure (psi)	13.9	Stage No.	1
		Test No.	3

INITIAL DIMENSIONS

Initial Sample Length (in)	6.14
Initial Sample Diameter (in)	2.83
Initial Sample Area (in ²)	6.31
Initial Sample Volume (in ³)	38.76

VOLUME CHANGE

Volume After Consolidation (in ³)	38.12
Length After Consolidation (in)	6.14
Area After Consolidation (in ²)	6.208

Strain (%)	Deviation Stress	ΔU	$\bar{\sigma}_1$	$\bar{\sigma}_3$	Effective Principle Stress Ratio	\bar{A}	\bar{P}	Q
------------	------------------	------------	------------------	------------------	----------------------------------	-----------	-----------	---

0.02	1.22	0.65	14.46	13.2	1.092	0.55	13.85	0.61
0.05	3.22	1.45	15.67	12.4	1.259	0.46	14.06	1.61
0.12	7.64	3.58	17.95	10.3	1.740	0.48	14.14	3.82
0.22	9.56	4.94	18.52	9.0	2.067	0.53	13.74	4.78
0.32	10.95	5.67	19.18	8.2	2.331	0.53	13.70	5.47
0.46	12.46	6.31	20.05	7.6	2.642	0.52	13.82	6.23
0.62	13.61	6.86	20.65	7.0	2.934	0.51	13.85	6.81
0.82	14.80	7.02	21.68	6.9	3.153	0.48	14.28	7.40
1.19	16.62	7.19	23.33	6.7	3.477	0.44	15.02	8.31
1.68	18.68	7.23	25.35	6.7	3.802	0.40	16.01	9.34
2.29	20.72	6.79	27.83	7.1	3.915	0.33	17.47	10.36
2.89	22.38	6.48	29.80	7.4	4.016	0.30	18.61	11.19
3.59	24.16	5.98	32.08	7.9	4.049	0.25	20.00	12.08
4.10	25.30	5.53	33.67	8.4	4.023	0.22	21.02	12.65
4.81	26.41	5.02	35.29	8.9	3.974	0.19	22.09	13.21
5.77	28.28	4.42	37.76	9.5	3.982	0.16	23.62	14.14
6.78	30.10	3.75	40.25	10.2	3.965	0.13	25.20	15.05
7.53	31.57	3.22	42.25	10.7	3.955	0.10	26.47	15.79
8.54	32.91	2.66	44.15	11.2	3.929	0.08	27.69	16.46
9.30	33.59	2.23	45.27	11.7	3.878	0.07	28.47	16.80
10.07	34.62	1.82	46.70	12.1	3.867	0.05	29.39	17.31
10.82	35.67	1.42	48.14	12.5	3.858	0.04	30.31	17.83
11.32	36.42	1.07	49.24	12.8	3.840	0.03	31.03	18.21
11.82	36.90	0.95	49.84	12.9	3.850	0.03	31.39	18.45
12.33	37.23	0.64	50.48	13.3	3.809	0.02	31.87	18.61
12.83	37.39	0.43	50.86	13.5	3.776	0.01	32.17	18.70
13.60	38.08	0.13	51.84	13.8	3.766	0.00	32.80	19.04
14.36	39.07	-0.19	53.16	14.1	3.773	0.00	33.62	19.53
14.85	39.26	-0.45	53.61	14.3	3.736	-0.01	33.98	19.63
15.36	39.83	-0.74	54.47	14.6	3.721	-0.02	34.55	19.91



**CONSOLIDATED UNDRAINED TRIAXIAL TEST
WITH PORE PRESSURE READINGS
AASHTO T-297**

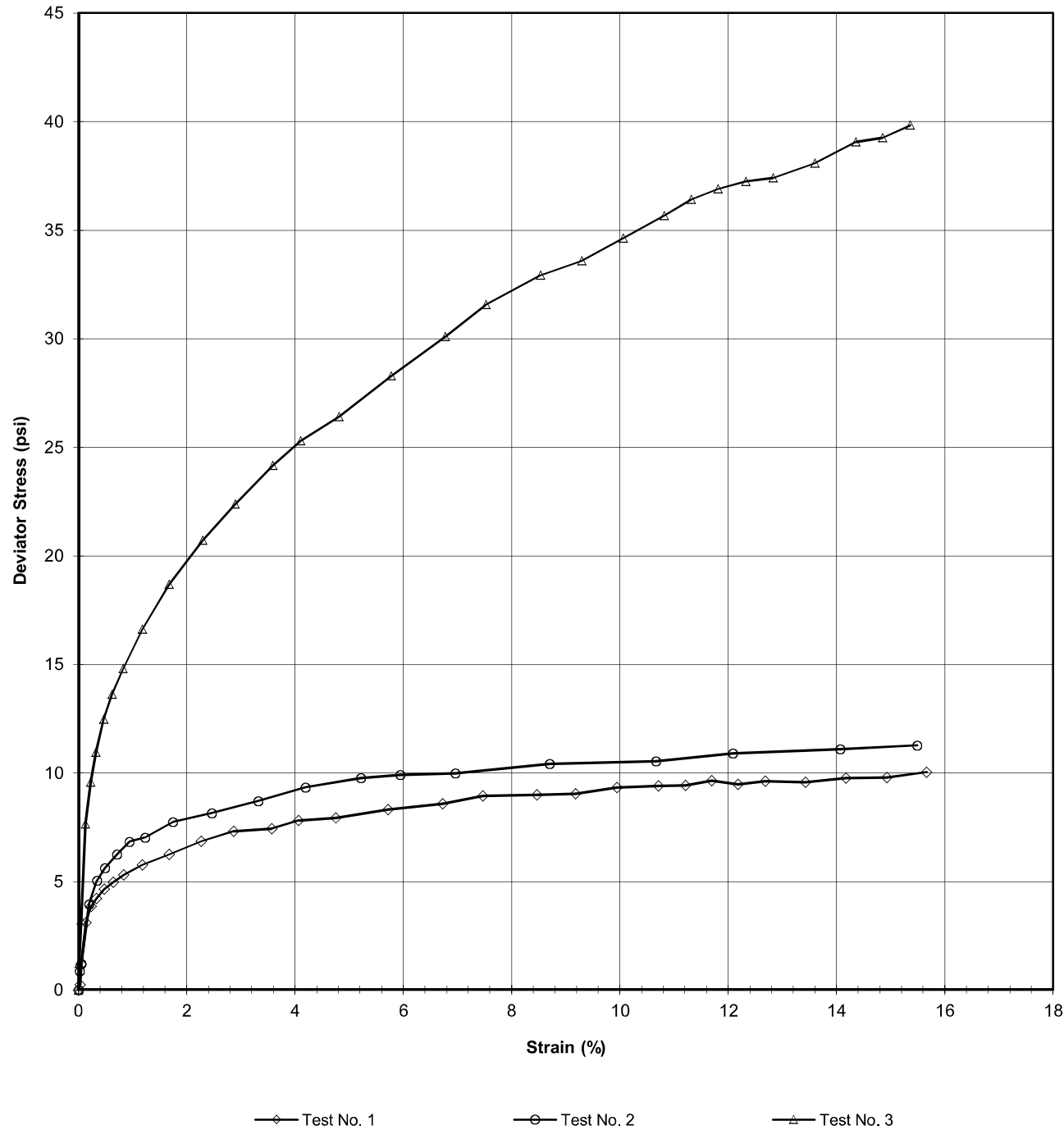
Client: Kleinfelder Boring No.: L_4800
 Client Reference: U-5813 Depth (ft): 0-2.0
 Project No.: R-2019-011-001 Sample No.: ST-1
 Lab ID: R-2019-011-001-001
 Visual Description: TAN ORANGE CLAY WITH ORGANICS (UNDISTURBED)

**CONSOLIDATED UNDRAINED TRIAXIAL TEST
WITH PORE PRESSURE READINGS
AASHTO T-297**

Client: Kleinfelder
 Client Reference: U-5813
 Project No.: R-2019-011-001
 Lab ID: R-2019-011-001-001 Specific Gravity (Measured) 2.62
 Visual Description: TAN ORANGE CLAY WITH ORGANICS (UNDISTURBED)

SAMPLE CONDITION SUMMARY

Boring No.:	L_4800	L_4800	L_4800
Depth (ft):	0-2.0	0-2.0	0-2.0
Sample No.:	ST-1001	ST-1001	ST-1001
Test No.	T1	T2	T3
Deformation Rate (in/min)	0.0012	0.0012	0.0012
Back Pressure (psi)	50.0	50.0	50.0
Consolidation Time (days)	1	1	1
Moisture Content (%) (INITIAL)	19.2	19.2	19.2
Total Unit Weight (pcf)	115.0	103.9	127.0
Dry Unit Weight (pcf)	96.5	87.2	106.6
Moisture Content (%) (FINAL)	31.1	45.0	20.5
Initial State Void Ratio, e	0.695	0.876	0.535
Void Ratio at Shear, e	0.672	0.748	0.510



Tested By: 129-04-0411 Date: 1/21/19 Approved By: MPS Date: 1/29/19

Tested By: 129-04-0411 Date: 1/21/19 Input Checked By: GEM Date: 1/29/19

**CONSOLIDATED UNDRAINED TRIAXIAL TEST
WITH PORE PRESSURE READINGS
AASHTO T-297**



Client: Kleinfelder
 Client Reference: U-5813
 Project No.: R-2019-011-001
 Lab ID: R-2019-011-001-001

Boring No.: L_4800
 Depth (ft): 0-2.0
 Sample No.: ST-1

TEST 1 INITIAL



TEST 1 FINAL



TEST 2 INITIAL



TEST 2 FINAL



TEST 3 INITIAL



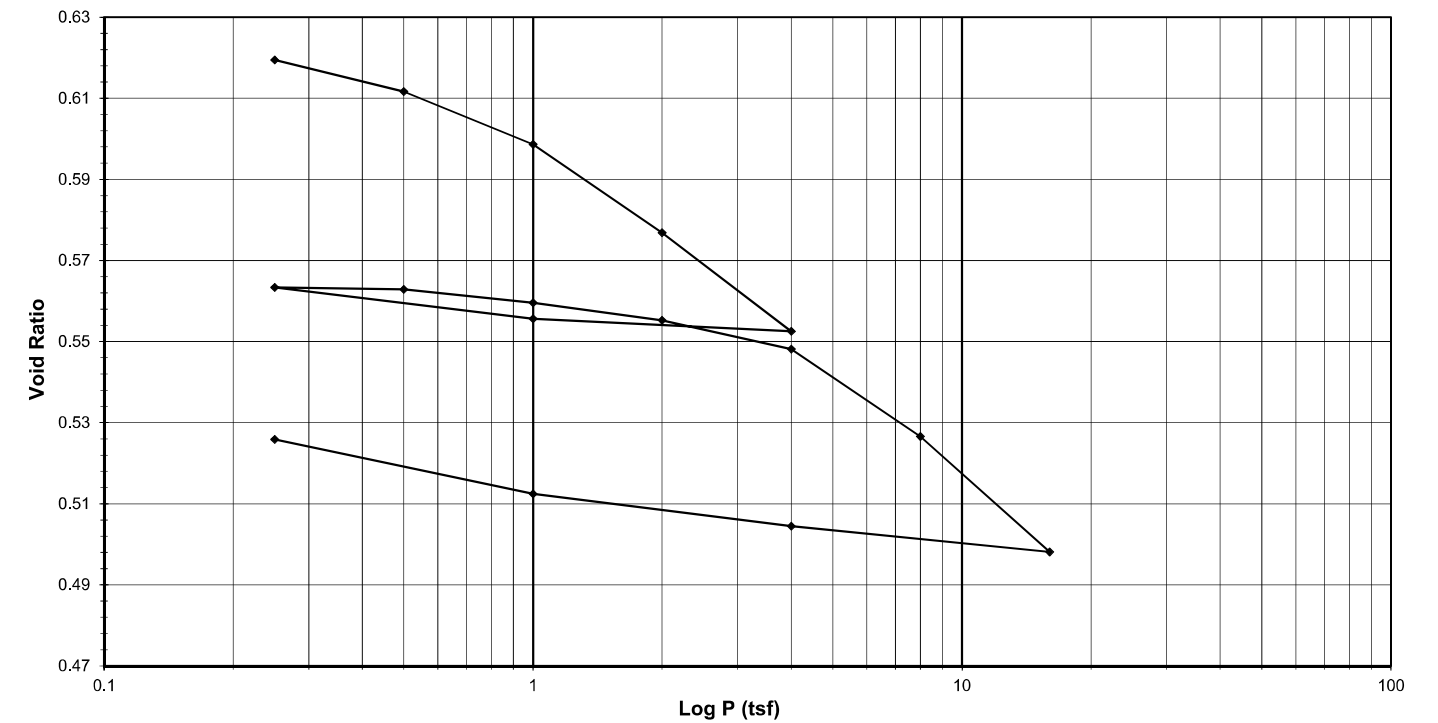
TEST 3 FINAL



**ONE DIMENSIONAL CONSOLIDATION
AASHTO T-216**

Client: Kleinfelder Boring No.: L_4800
 Client Reference: U-5813 Depth (ft): 0-2.0
 Project No.: R-2019-011-001 Sample No.: ST-1
 Lab ID: R-2019-011-001-001 Visual Description: TAN SILTY CLAY

Sample Conditions: UNDISTURBED, INUNDATED AND DOUBLE DRAINED



Tested By 129-04-0411 Date 1/16/19 Approved By MPS Date 1/29/19



ONE DIMENSIONAL CONSOLIDATION
AASHTO T-216

Client Kleinfelder Boring No. L_4800
 Client Reference U-5813 Depth (ft) 0-2.0
 Project No. R-2019-011-001 Sample No. ST-1
 Lab ID R-2019-011-001-001 Visual Description TAN SILTY CLAY

Sample Conditions: UNDISTURBED, INUNDATED AND DOUBLE DRAINED
 Consolidometer No. R409
 1 Division = 0.0001 (in.)

Sample Properties	Initial	Final	Test Data Summary							
			Applied Pressure (tsf)	Final Dial Reading (div)	Machine Deflection (div)	Corrected Reading (div)	Height of Sample (mm)	Volume (cc)	Dry Density (g/cc)	Void Ratio
<i>Water Content</i>										
Tare Number	861	812								
Wt. Tare & WS (g)	239.90	261.30								
Wt. Tare & DS (g)	223.70	237.67								
Wt. Water (g)	16.20	23.63	Seating	0	0	0	25.400	80.440	1.64961	0.63675
Wt. Tare (g)	139.22	104.57	0.25	137.5	31.8	105.7	25.132	79.589	1.66723	0.61945
Wt. DS (g)	84.48	133.10	0.5	200.3	47.0	153.3	25.011	79.207	1.67529	0.61166
Water Content (%)	19.18	17.75	1	302.8	70.0	232.9	24.809	78.567	1.68894	0.59864
			2	461.8	95.6	366.1	24.470	77.495	1.71231	0.57682
			4	641.6	126.9	514.8	24.092	76.299	1.73914	0.55249
<i>Sample Parameters</i>										
Sample Diameter (in)	2.5	2.5	1	588.5	92.9	495.6	24.141	76.453	1.73563	0.55563
Sample Height (in)	1.0000	0.9323	0.25	513.8	65.2	448.6	24.261	76.832	1.72708	0.56333
Sample Volume (cc)	80.44	74.99	0.5	523.4	72.1	451.3	24.254	76.809	1.72758	0.56288
Wt. Wet Sample + Ring (g)	372.34	370.45	1	555.8	84.4	471.4	24.203	76.648	1.73121	0.55960
Wt. of Ring (g)	214.20	214.20	2	598.7	100.9	497.8	24.136	76.436	1.73602	0.55528
Wt. of Wet Sample (g)	158.14	156.25	4	670.4	128.9	541.5	24.025	76.084	1.74404	0.54813
Wet Density (pcf)	122.67	130.02	8	839.6	166.5	673.1	23.690	75.025	1.76866	0.52658
Wet Density (g/cc)	1.97	2.08	16	1068.6	221.8	846.8	23.249	73.628	1.80222	0.49815
Water Content (%)	19.18	17.75	4	965.2	156.9	808.3	23.347	73.938	1.79468	0.50445
Wt. of Dry Sample (g)	132.69	132.69	1	868.6	109.0	759.6	23.471	74.329	1.78522	0.51242
Dry Density (pcf)	102.94	110.41	0.25	754.5	77.2	677.3	23.680	74.992	1.76945	0.52589
Dry Density (g/cc)	1.65	1.77								
Void Ratio	0.6368	0.5259								
Saturation (%)	81.31	91.15								
Specific Gravity	2.70	Assumed								

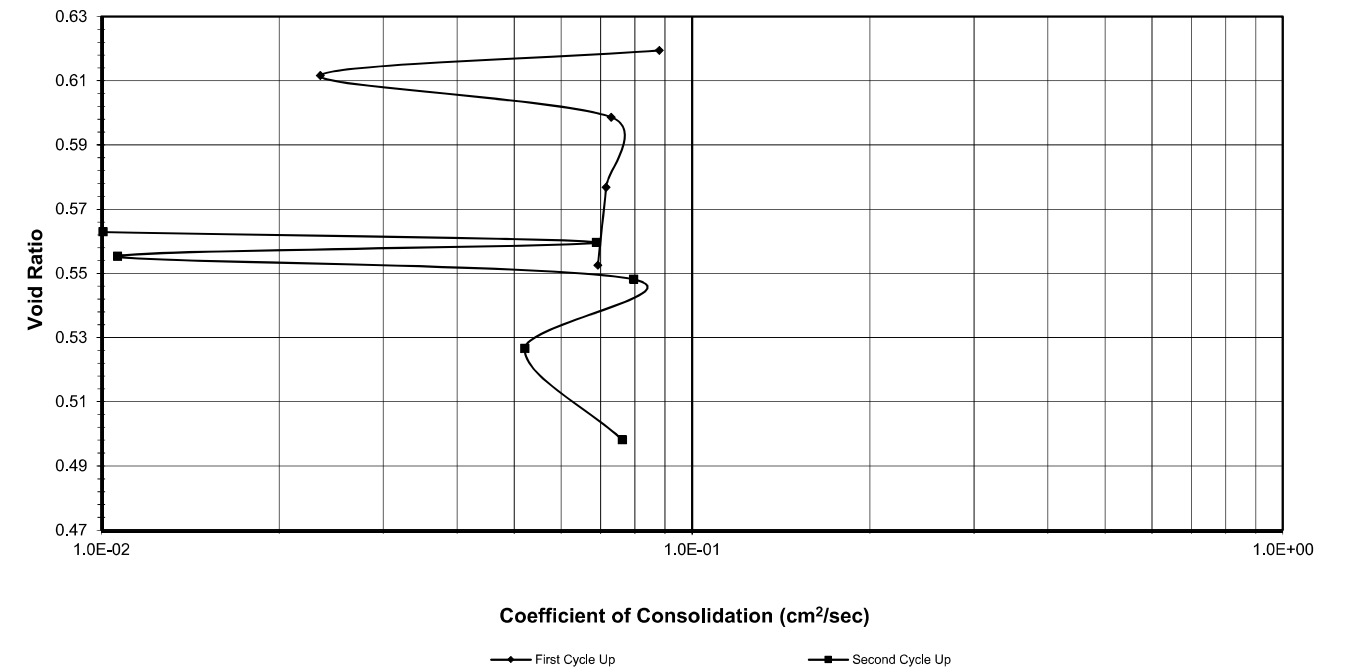
Tested By 129-04-0411 Date 1/16/19 Input Checked By GEM Date 1/29/19



ONE DIMENSIONAL CONSOLIDATION
AASHTO T-216

Client Kleinfelder Boring No. L_4800
 Client Reference U-5813 Depth (ft) 0-2.0
 Project No. R-2019-011-001 Sample No. ST-1
 Lab ID R-2019-011-001-001 Visual Description TAN SILTY CLAY

Sample Conditions: UNDISTURBED, INUNDATED AND DOUBLE DRAINED



Tested By 129-04-0411 Date 1/16/19 Input Checked By GEM Date 1/29/19



ONE DIMENSIONAL CONSOLIDATION
AASHTO T-216

Client	Kleinfelder	Boring No.	L_4800
Client Project	U-5813	Depth (ft)	0-2.0
Project No.	R-2019-011-001	Sample No.	ST-1
Lab ID	R-2019-011-001-001	Visual Description	TAN SILTY CLAY



ONE DIMENSIONAL CONSOLIDATION
AASHTO T-216

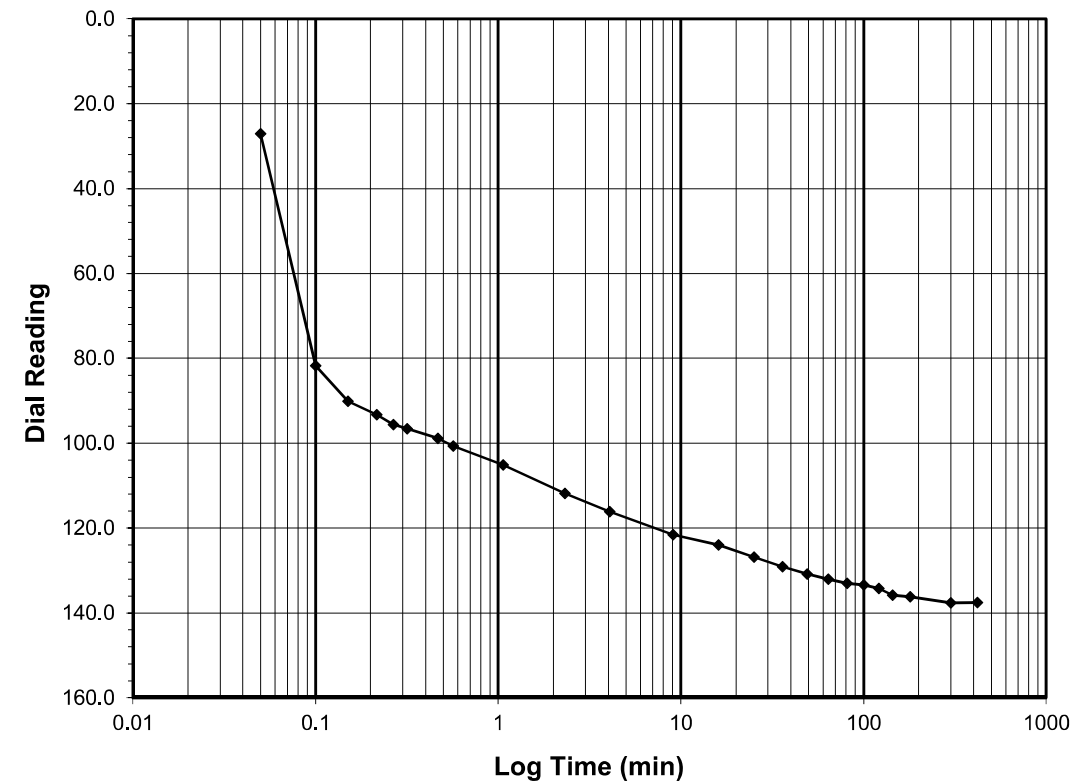
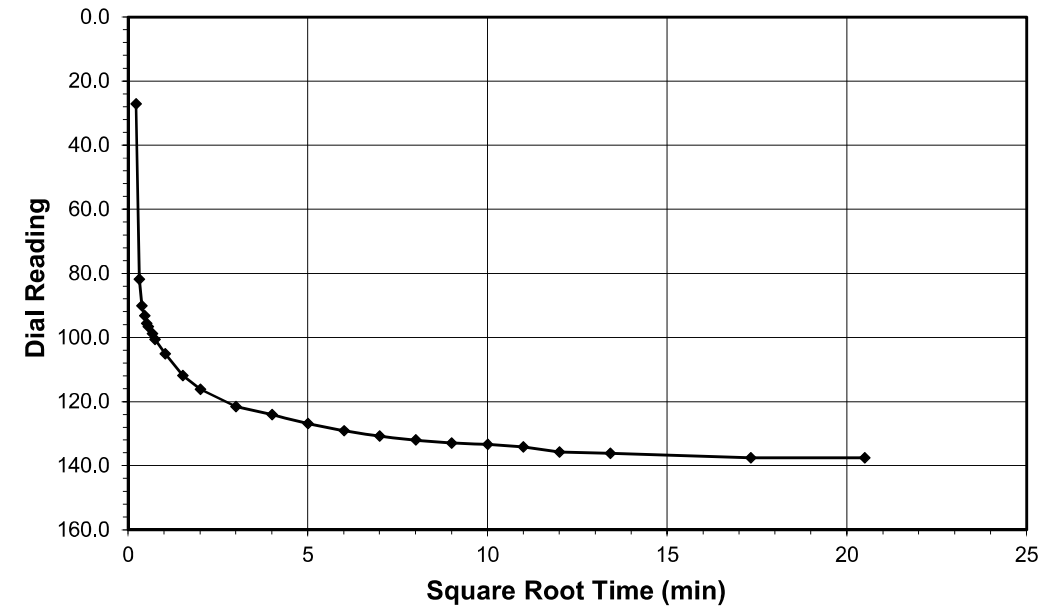
Client	Kleinfelder	Boring No.	L_4800
Client Reference	U-5813	Depth (ft)	0-2.0
Project No.	R-2019-011-001	Sample No.	ST-1
Lab ID	R-2019-011-001-001	Visual Description	TAN SILTY CLAY

Sample Conditions: UNDISTURBED, INUNDATED AND DOUBLE DRAINED
Consolidometer No. R409
1 Division = 0.0001 (in.)

Sample Properties	Initial	Final	C _v Test Data Summary					
			Load Increment (tsf)	Dial Reading @ t ₅₀ (div)	Machine Deflection (div)	Corrected Dial Reading @ t ₅₀ (div)	Sample Height @ t ₅₀ (cm)	Time t ₅₀ (min.)
Water Content								
Tare Number	861	812						
Wt. Tare & WS (g)	239.90	261.30						
Wt. Tare & DS (g)	223.70	237.67						
Wt. Water (g)	16.20	23.63	0 - 0.25	45.5	31.8	13.7	2.537	0.06
Wt. Tare (g)	139.22	104.57	0.25 - 0.5	171.9	47.0	124.9	2.508	0.22
Wt. DS (g)	84.48	133.10	0.5 - 1.0	248.8	70.0	178.8	2.495	0.07
Water Content (%)	19.18	17.75	1.0 - 2.0	373.9	95.6	278.3	2.469	0.07
			2.0 - 4.0	557.7	126.9	430.8	2.431	0.07
			4.0 - 1.0	NA	92.9	NA	NA	NA
			1.0 - 0.25	NA	65.2	NA	NA	NA
Sample Parameters			0.25 - 0.5	521.1	72.1	449.0	2.426	0.48
Sample Diameter (in)	2.5	2.5	0.5 - 1.0	545.8	84.4	461.3	2.423	0.07
Sample Height (in)	1.000	0.932	1.0 - 2.0	585.5	100.9	484.6	2.417	0.45
Sample Volume (cc)	80.44	74.99	2.0 - 4.0	632.7	128.9	503.8	2.412	0.06
Wt. Wet Sample + Ring (g)	372.34	370.45	4.0 - 8.0	758.4	166.5	591.8	2.390	0.09
Wt. of Ring (g)	214.20	214.20	8.0 - 16.0	931.2	221.8	709.3	2.360	0.06
Wt. of Wet Sample (g)	158.14	156.25	16.0 - 4.0	NA	156.9	NA	NA	NA
Wet Density (pcf)	122.67	130.02	4.0 - 1.0	NA	109.0	NA	NA	NA
Wet Density (g/cc)	1.97	2.08	1.0 - 0.25	NA	77.2	NA	NA	NA
Water Content (%)	19.18	17.75						
Wt. of Dry Sample (g)	132.69	132.69						
Dry Density (pcf)	102.94	110.41						
Dry Density (g/cc)	1.65	1.77						
Void Ratio	0.6368	0.5259						
Saturation (%)	81.31	91.15						
Specific Gravity	2.7	Assumed						

Tested By 129-04-0411 Date 1/16/19 Input Checked By GEM Date 1/29/19

Sample Conditions: UNDISTURBED, INUNDATED AND DOUBLE DRAINED



Test Load (tsf) 0.0-0.25
Final Reading (div) 137.5
Consolidometer No. R409
1 Division (in) 0.0001
Start Date 1/16/19
Start Time 11:32:10

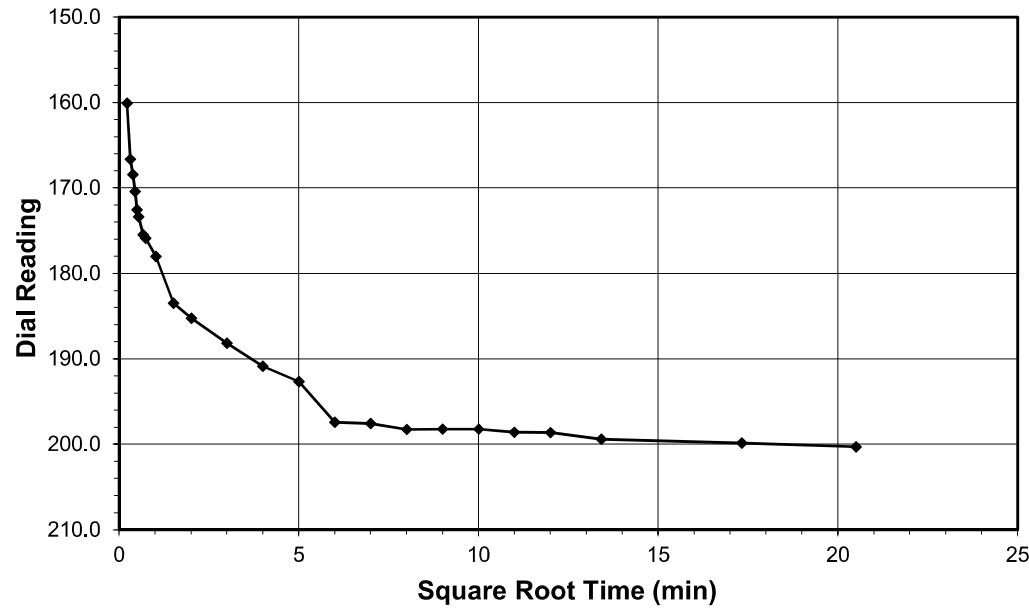
Elapsed Time (min)	Dial Reading (div)
Initial	0.0
0.05	27.1
0.10	81.8
0.15	90.1
0.22	93.3
0.27	95.6
0.32	96.6
0.47	98.8
0.57	100.7
1.07	105.1
2.32	111.9
4.07	116.1
9.07	121.5
16.07	124.0
25.07	126.8
36.07	129.1
49.07	130.8
64.07	132.0
81.07	133.0
100.07	133.4
121.07	134.2
144.07	135.8
180.07	136.2
300.08	137.6
420.10	137.5

Tested By 129-04-0411 Date 1/16/19 Checked By GEM Date 1/29/19

ONE DIMENSIONAL CONSOLIDATION
AASHTO T-216

Client Kleinfelder Boring No. L_4800
 Client Project U-5813 Depth (ft) 0-2.0
 Project No. R-2019-011-001 Sample No. ST-1
 Lab ID R-2019-011-001-001 Visual Description TAN SILTY CLAY

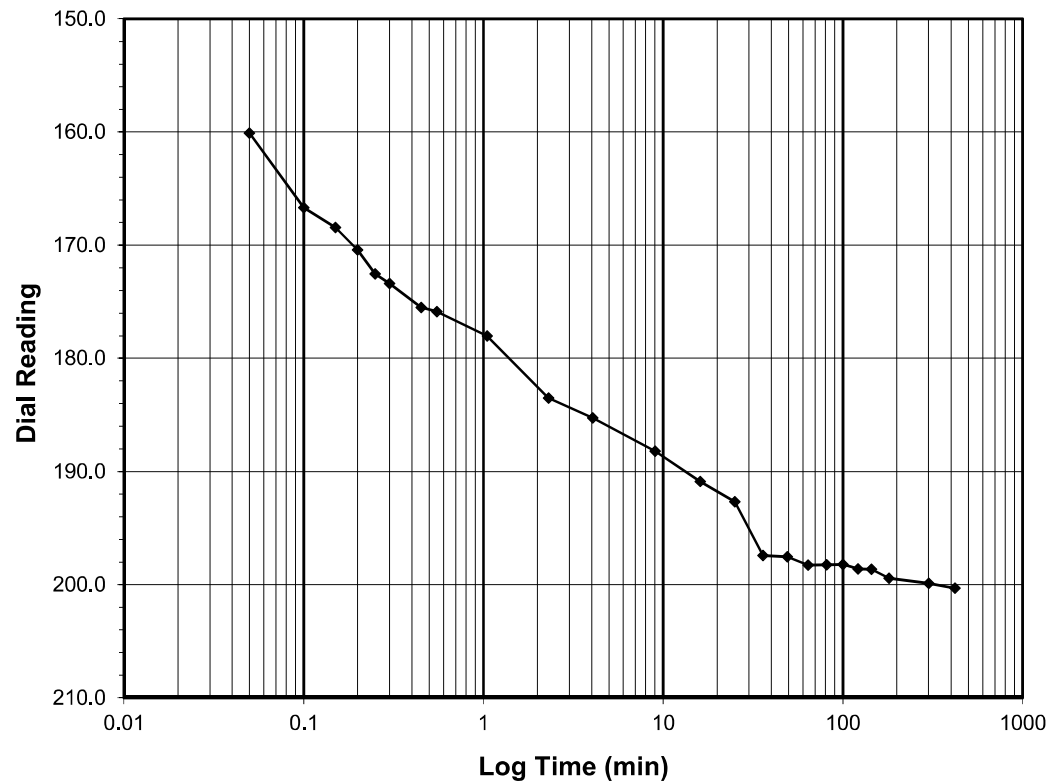
Sample Conditions: UNDISTURBED, INUNDATED AND DOUBLE DRAINED



Test Load (tsf) 0.25-0.5
 Final Reading (div) 200.3
 Consolidometer No. R409
 1 Division (in) 0.0001

Start Date 1/16/19
 Start Time 18:32:17

Elapsed Time (min)	Dial Reading (div)
Initial	137.5
0.05	160.1
0.10	166.7
0.15	168.5
0.20	170.4
0.25	172.6
0.30	173.4
0.45	175.5
0.55	175.9
1.05	178.0
2.30	183.5
4.05	185.3
9.05	188.2
16.05	190.9
25.05	192.7
36.05	197.4
49.07	197.5
64.07	198.3
81.07	198.2
100.07	198.2
121.07	198.6
144.07	198.6
180.07	199.4
300.07	199.9
420.40	200.3

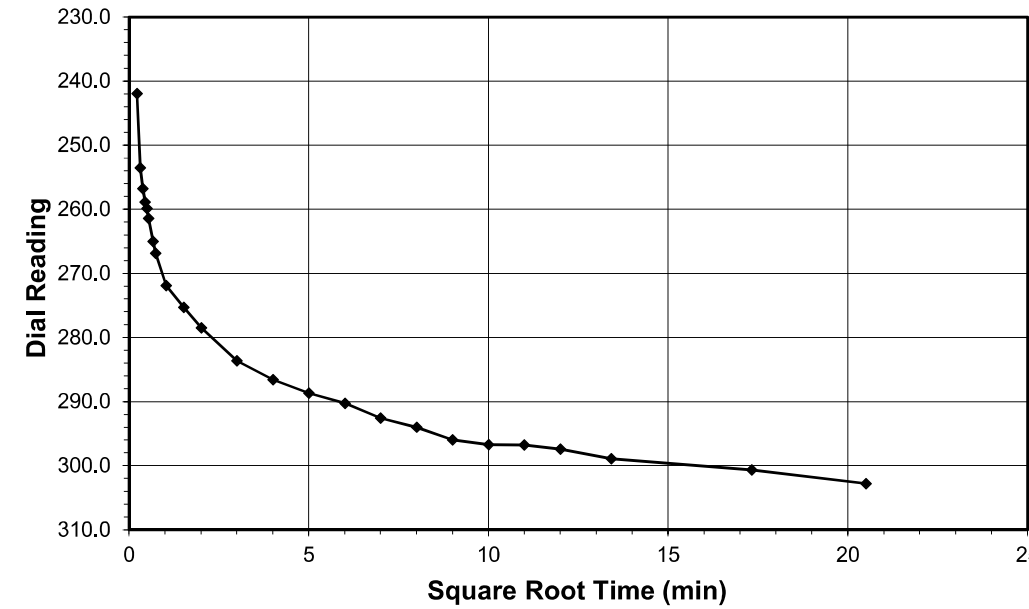


Tested By 129-04-0411 Date 1/16/19 Checked By GEM Date 1/29/19

ONE DIMENSIONAL CONSOLIDATION
AASHTO T-216

Client Kleinfelder Boring No. L_4800
 Client Project U-5813 Depth (ft) 0-2.0
 Project No. R-2019-011-001 Sample No. ST-1
 Lab ID R-2019-011-001-001 Visual Description TAN SILTY CLAY

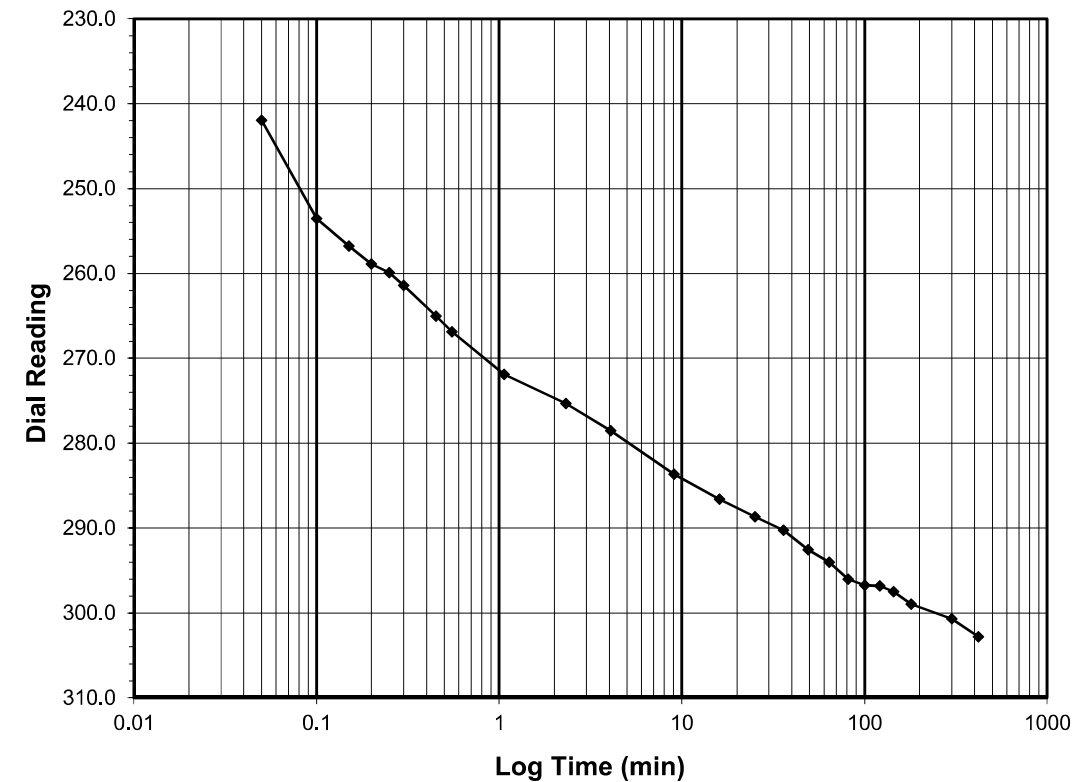
Sample Conditions: UNDISTURBED, INUNDATED AND DOUBLE DRAINED



Test Load (tsf) 0.5-1.0
 Final Reading (div) 302.8
 Consolidometer No. R409
 1 Division (in) 0.0001

Start Date 1/17/19
 Start Time 1:32:41

Elapsed Time (min)	Dial Reading (div)
Initial	200.3
0.05	242.0
0.10	253.6
0.15	256.8
0.20	258.9
0.25	259.9
0.30	261.4
0.45	265.0
0.55	266.9
1.07	271.9
2.32	275.3
4.07	278.5
9.07	283.6
16.07	286.6
25.07	288.7
36.07	290.3
49.07	292.5
64.07	294.0
81.07	296.0
100.07	296.7
121.07	296.8
144.07	297.5
180.07	299.0
300.07	300.7
420.33	302.8



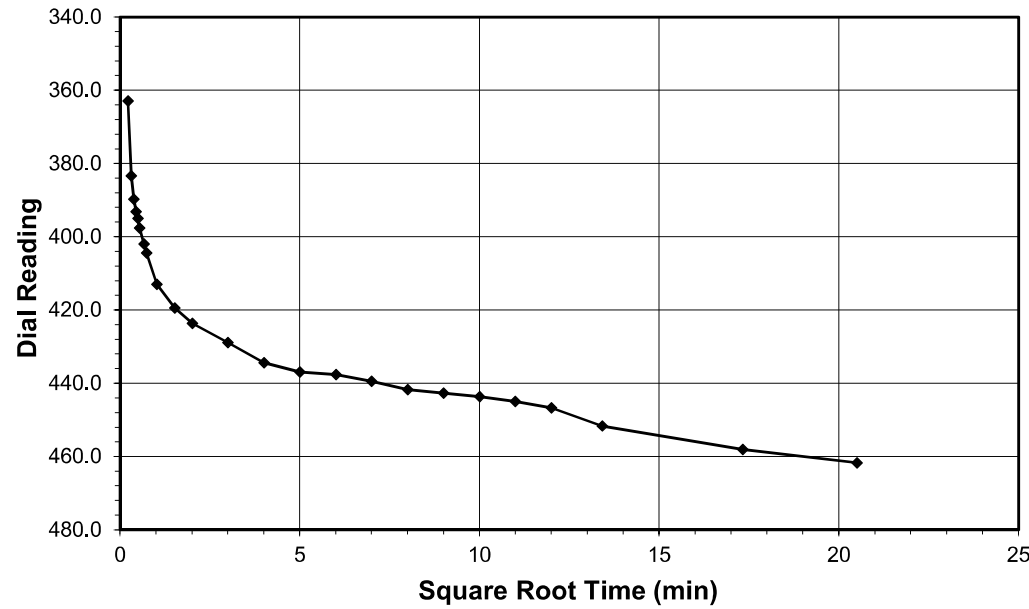
Tested By 129-04-0411 Date 1/17/19 Checked By GEM Date 1/29/19



ONE DIMENSIONAL CONSOLIDATION
AASHTO T-216

Client Kleinfelder Boring No. L_4800
 Client Project U-5813 Depth (ft) 0-2.0
 Project No. R-2019-011-001 Sample No. ST-1
 Lab ID R-2019-011-001-001 Visual Description TAN SILTY CLAY

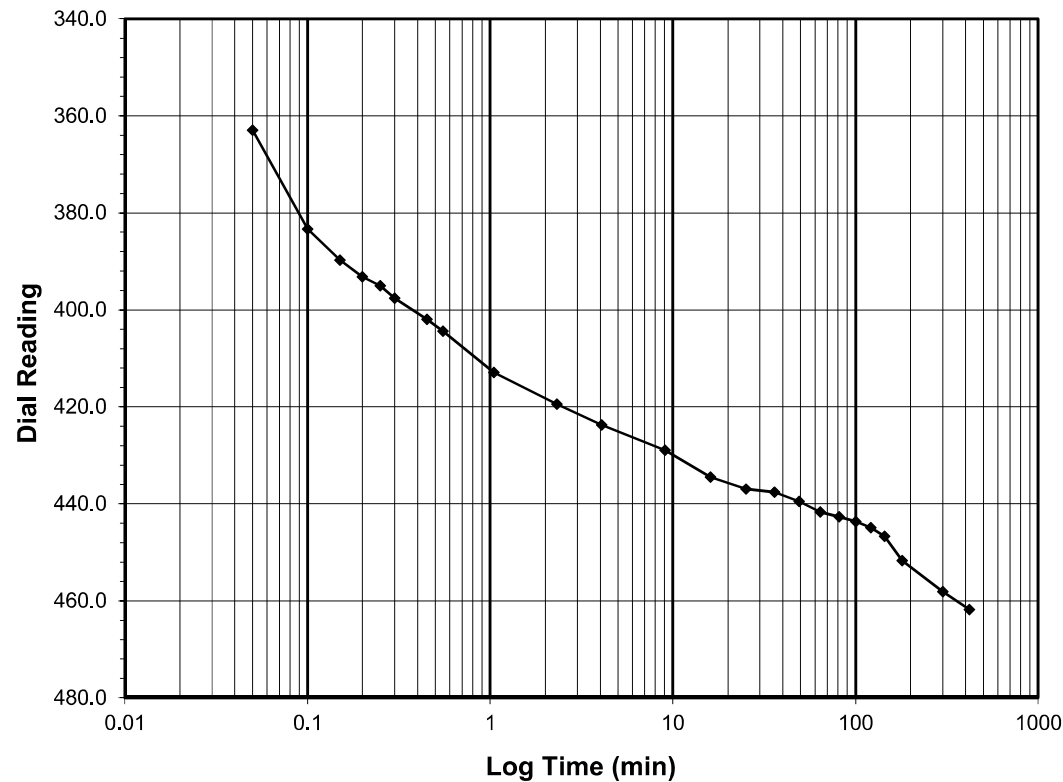
Sample Conditions: UNDISTURBED, INUNDATED AND DOUBLE DRAINED



Test Load (tsf) 1.0-2.0
Final Reading (div) 461.8
 Consolidometer No. **R409**
 1 Division (in) 0.0001

Start Date 1/17/19
 Start Time 8:33:01

Elapsed Time (min)	Dial Reading (div)
Initial	302.8
0.05	363.0
0.10	383.4
0.15	389.8
0.20	393.2
0.25	395.0
0.30	397.6
0.45	401.9
0.55	404.4
1.05	412.9
2.32	419.5
4.07	423.7
9.07	428.9
16.07	434.4
25.07	436.9
36.07	437.6
49.07	439.5
64.07	441.7
81.07	442.7
100.07	443.7
121.07	444.9
144.07	446.7
180.07	451.7
300.07	458.1
420.38	461.8



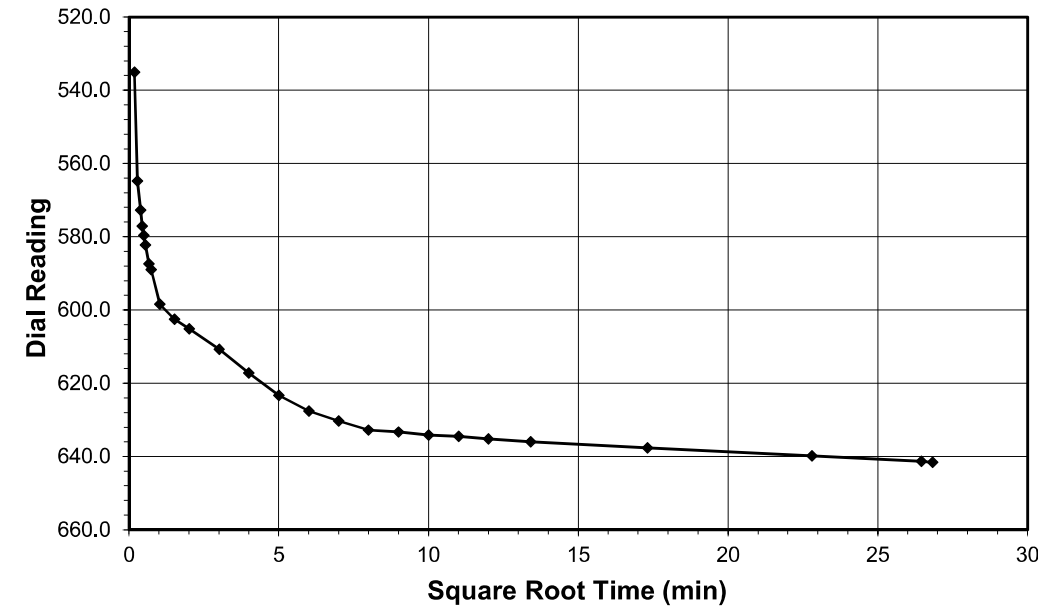
Tested By 129-04-0411 Date 1/17/19 Checked By GEM Date 1/29/19



ONE DIMENSIONAL CONSOLIDATION
AASHTO T-216

Client Kleinfelder Boring No. L_4800
 Client Project U-5813 Depth (ft) 0-2.0
 Project No. R-2019-011-001 Sample No. ST-1
 Lab ID R-2019-011-001-001 Visual Description TAN SILTY CLAY

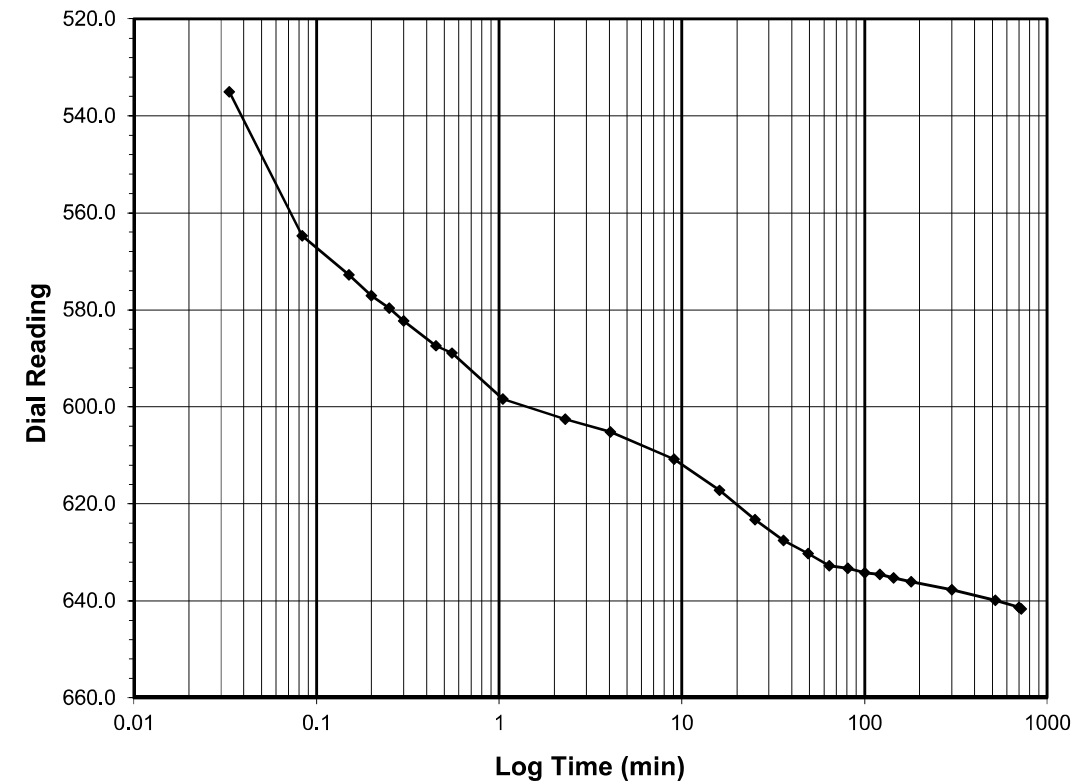
Sample Conditions: UNDISTURBED, INUNDATED AND DOUBLE DRAINED



Test Load (tsf) 2.0-4.0
Final Reading (div) 641.6
 Consolidometer No. **R409**
 1 Division (in) 0.0001

Start Date 1/17/19
 Start Time 15:33:24

Elapsed Time (min)	Dial Reading (div)
Initial	461.8
0.03	535.1
0.08	564.8
0.15	572.8
0.20	577.1
0.25	579.6
0.30	582.3
0.45	587.4
0.55	588.9
1.05	598.4
2.30	602.5
4.05	605.2
9.05	610.7
16.05	617.2
25.05	623.3
36.05	627.5
49.05	630.2
64.05	632.8
81.05	633.3
100.05	634.2
121.05	634.5
144.05	635.2
180.05	636.0
300.05	637.7
520.05	639.9
700.05	641.3
720.05	641.6



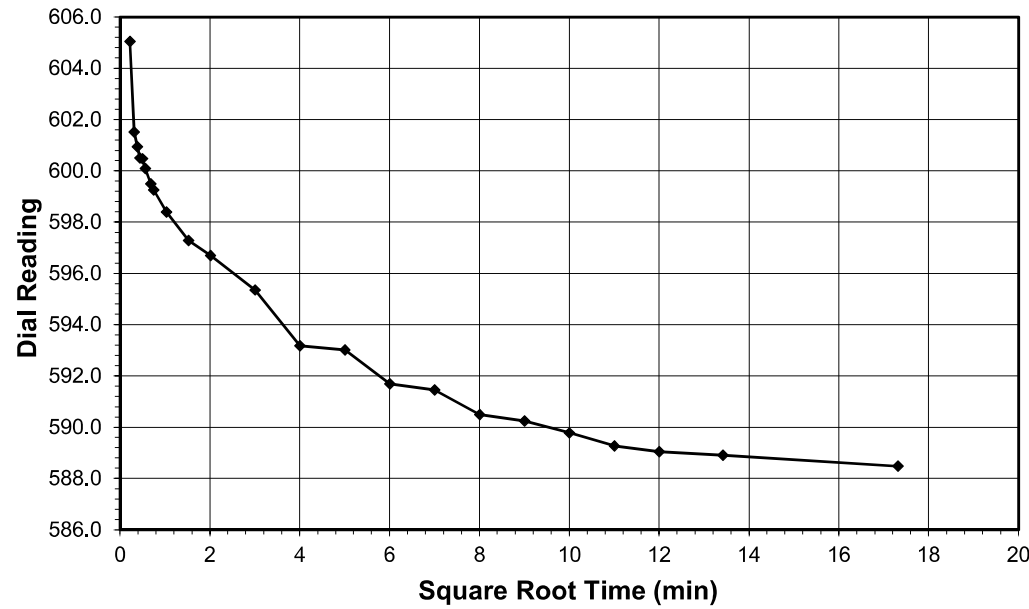
Tested By 129-04-0411 Date 1/17/19 Checked By GEM Date 1/29/19



ONE DIMENSIONAL CONSOLIDATION
AASHTO T-216

Client Kleinfelder Boring No. L_4800
 Client Project U-5813 Depth (ft) 0-2.0
 Project No. R-2019-011-001 Sample No. ST-1
 Lab ID R-2019-011-001-001 Visual Description TAN SILTY CLAY

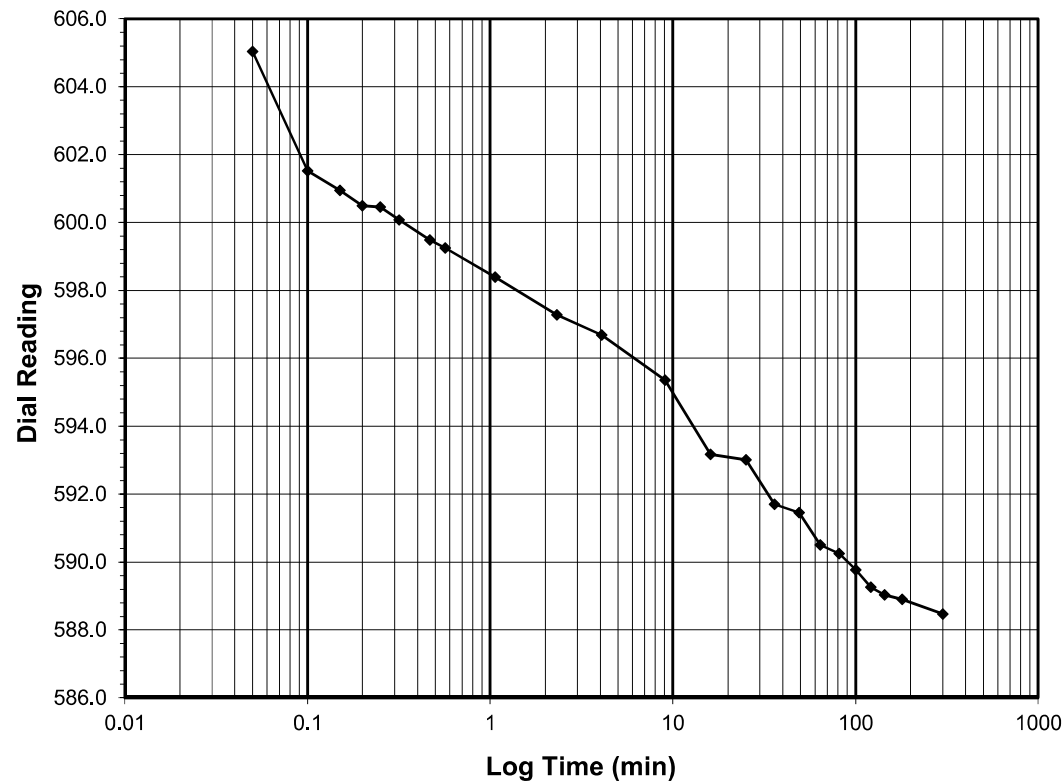
Sample Conditions: UNDISTURBED, INUNDATED AND DOUBLE DRAINED



Test Load (tsf) 4.0-1.0
Final Reading (div) 588.5
 Consolidometer No. **R409**
 1 Division (in) 0.0001

Start Date 1/18/19
 Start Time 3:33:28

Elapsed Time (min)	Dial Reading (div)
Initial	641.6
0.05	605.0
0.10	601.5
0.15	600.9
0.20	600.5
0.25	600.5
0.32	600.1
0.47	599.5
0.57	599.3
1.07	598.4
2.32	597.3
4.07	596.7
9.07	595.4
16.07	593.2
25.07	593.0
36.07	591.7
49.07	591.5
64.08	590.5
81.08	590.2
100.08	589.8
121.08	589.3
144.08	589.0
180.08	588.9
300.08	588.5



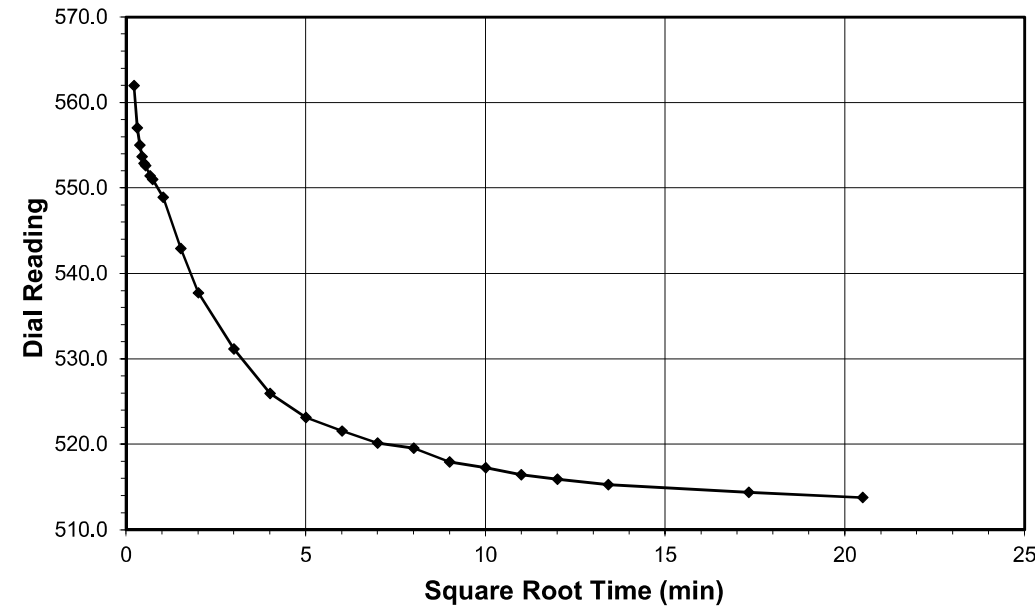
Tested By 129-04-0411 Date 1/18/19 Checked By GEM Date 1/29/19



ONE DIMENSIONAL CONSOLIDATION
AASHTO T-216

Client Kleinfelder Boring No. L_4800
 Client Project U-5813 Depth (ft) 0-2.0
 Project No. R-2019-011-001 Sample No. ST-1
 Lab ID R-2019-011-001-001 Visual Description TAN SILTY CLAY

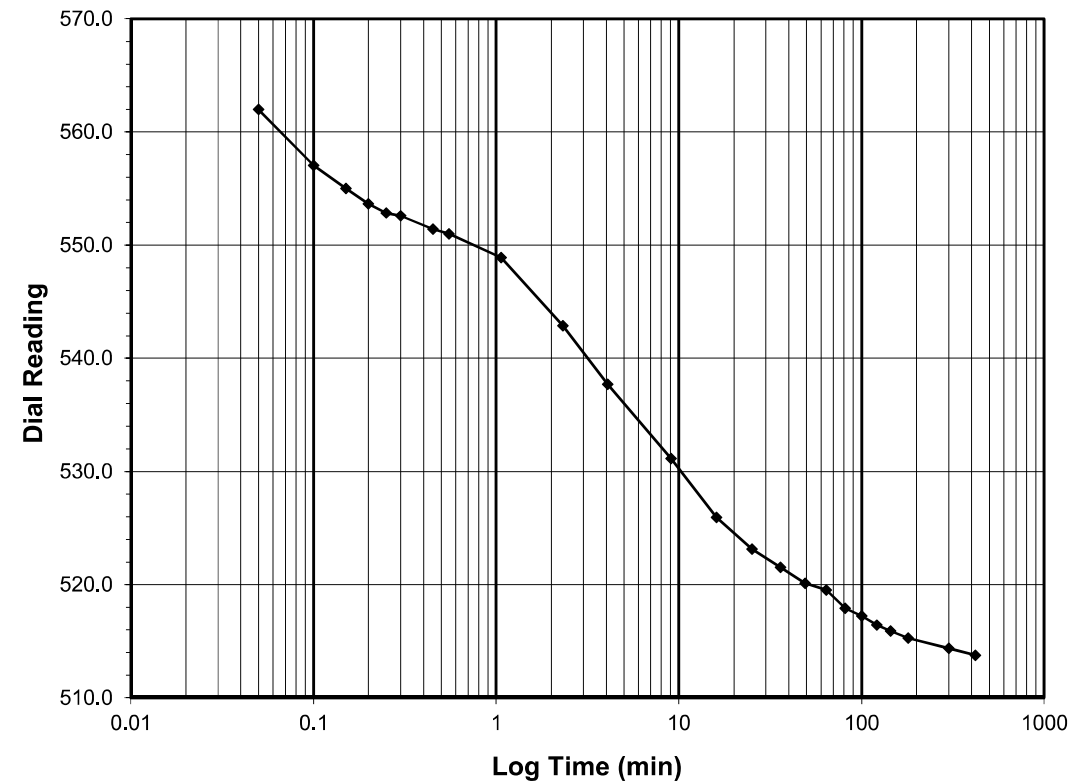
Sample Conditions: UNDISTURBED, INUNDATED AND DOUBLE DRAINED



Test Load (tsf) 1.0-0.25
Final Reading (div) 513.8
 Consolidometer No. **R409**
 1 Division (in) 0.0001

Start Date 1/18/19
 Start Time 10:33:49

Elapsed Time (min)	Dial Reading (div)
Initial	588.5
0.05	562.0
0.10	557.0
0.15	555.0
0.20	553.7
0.25	552.9
0.30	552.6
0.45	551.4
0.55	551.0
1.07	548.9
2.32	542.9
4.07	537.7
9.07	531.2
16.07	525.9
25.07	523.2
36.07	521.5
49.07	520.1
64.07	519.5
81.07	517.9
100.07	517.2
121.07	516.4
144.07	515.9
180.07	515.3
300.07	514.4
420.32	513.8



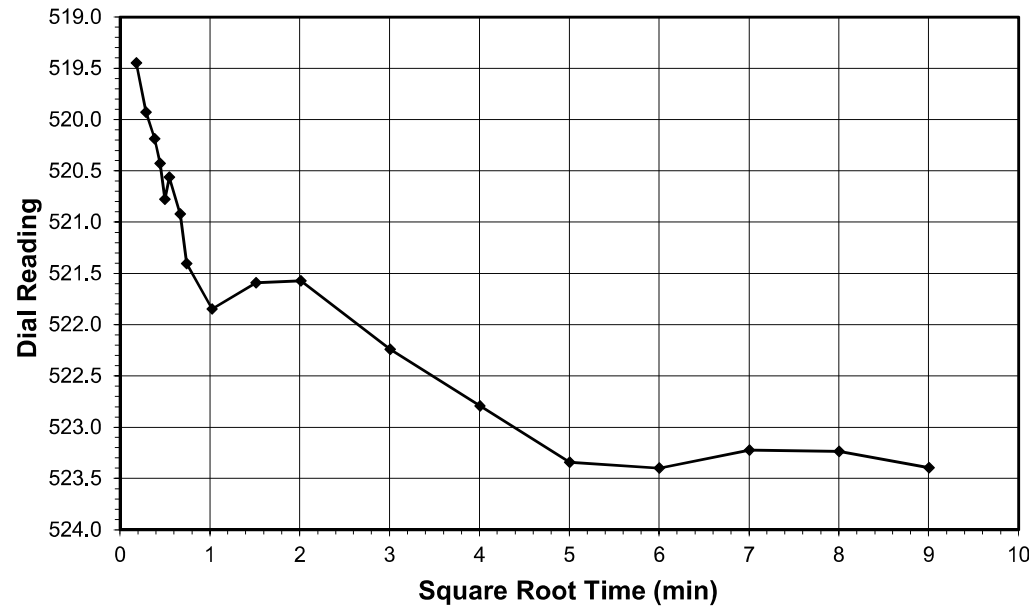
Tested By 129-04-0411 Date 1/18/19 Checked By GEM Date 1/29/19



ONE DIMENSIONAL CONSOLIDATION
AASHTO T-216

Client Kleinfelder Boring No. L_4800
 Client Project U-5813 Depth (ft) 0-2.0
 Project No. R-2019-011-001 Sample No. ST-1
 Lab ID R-2019-011-001-001 Visual Description TAN SILTY CLAY

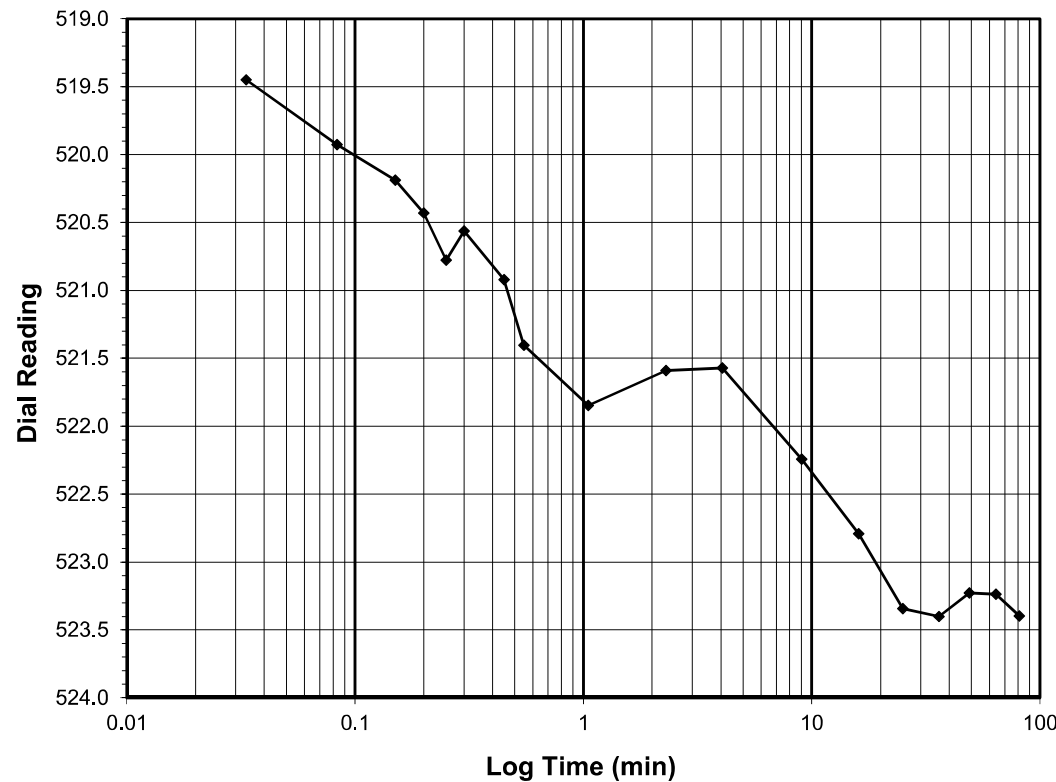
Sample Conditions: UNDISTURBED, INUNDATED AND DOUBLE DRAINED



Test Load (tsf) 0.25-0.5
Final Reading (div) 523.4
 Consolidometer No. **R409**
 1 Division (in) 0.0001

Start Date 1/18/19
 Start Time 17:34:08

Elapsed Time (min)	Dial Reading (div)
Initial	513.8
0.03	519.4
0.08	519.9
0.15	520.2
0.20	520.4
0.25	520.8
0.30	520.6
0.45	520.9
0.55	521.4
1.05	521.8
2.30	521.6
4.05	521.6
9.05	522.2
16.05	522.8
25.05	523.3
36.05	523.4
49.05	523.2
64.05	523.2
81.07	523.4



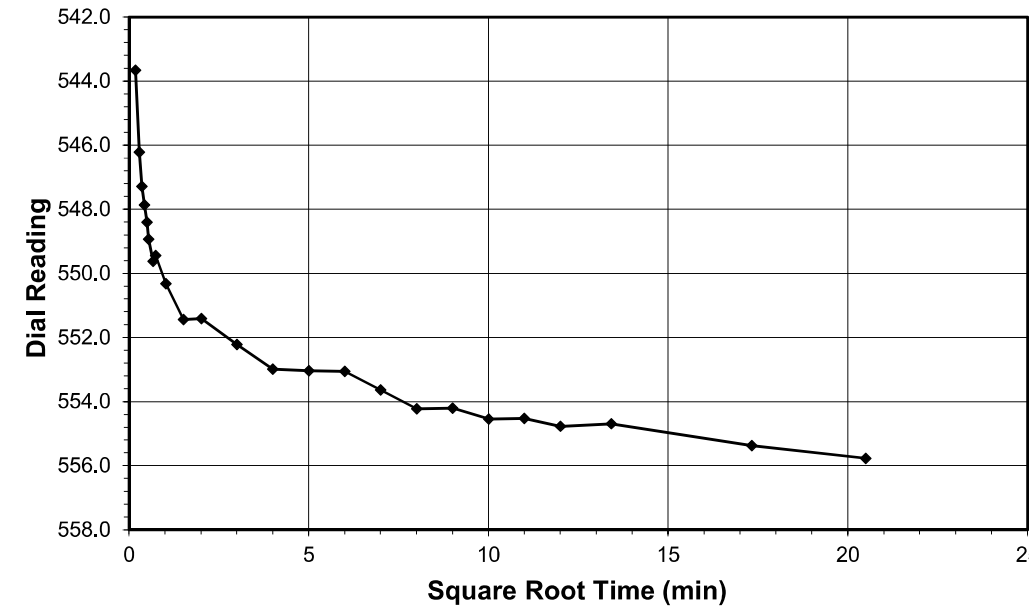
Tested By 129-04-0411 Date 1/18/19 Checked By GEM Date 1/29/19



ONE DIMENSIONAL CONSOLIDATION
AASHTO T-216

Client Kleinfelder Boring No. L_4800
 Client Project U-5813 Depth (ft) 0-2.0
 Project No. R-2019-011-001 Sample No. ST-1
 Lab ID R-2019-011-001-001 Visual Description TAN SILTY CLAY

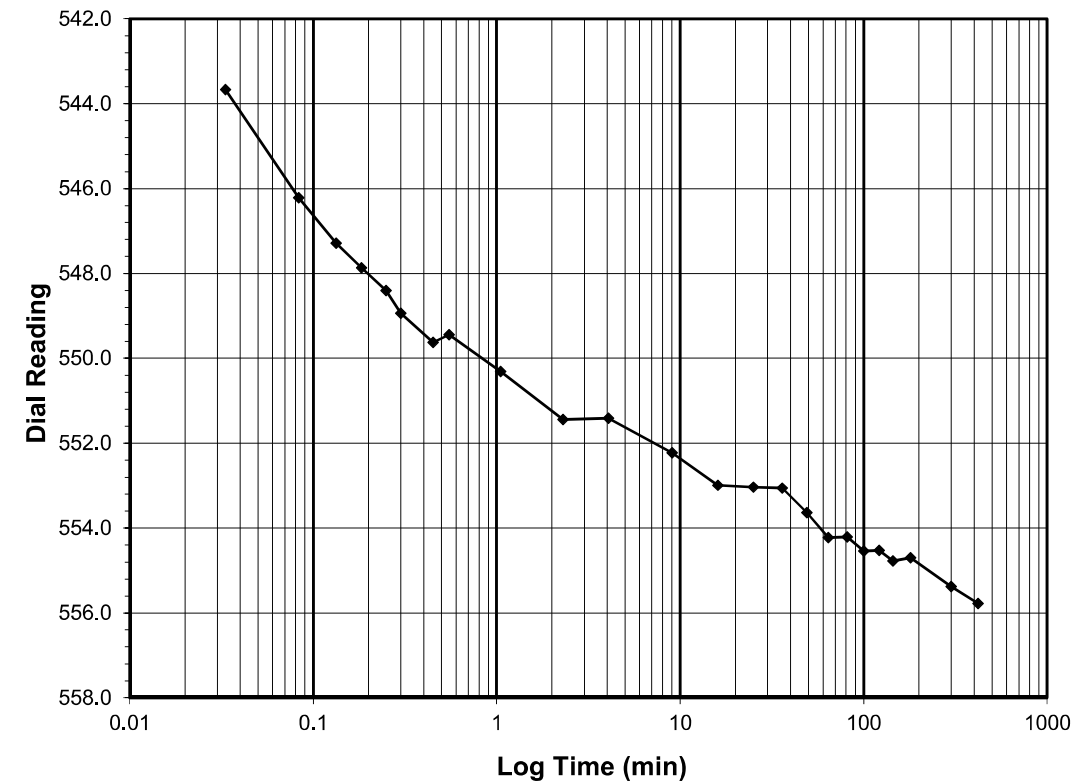
Sample Conditions: UNDISTURBED, INUNDATED AND DOUBLE DRAINED



Test Load (tsf) 0.5-1.0
Final Reading (div) 555.8
 Consolidometer No. **R409**
 1 Division (in) 0.0001

Start Date 1/19/19
 Start Time 0:34:28

Elapsed Time (min)	Dial Reading (div)
Initial	523.4
0.03	543.7
0.08	546.2
0.13	547.3
0.18	547.9
0.25	548.4
0.30	548.9
0.45	549.6
0.55	549.4
1.05	550.3
2.30	551.4
4.05	551.4
9.05	552.2
16.05	553.0
25.05	553.0
36.05	553.1
49.07	553.6
64.07	554.2
81.07	554.2
100.07	554.5
121.07	554.5
144.07	554.8
180.07	554.7
300.07	555.4
420.28	555.8



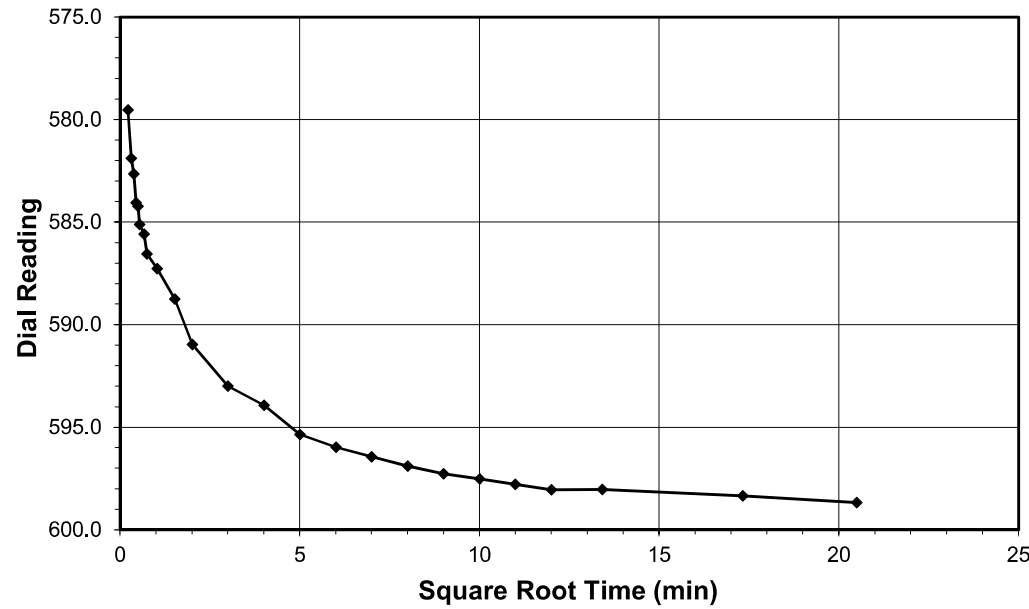
Tested By 129-04-0411 Date 1/19/19 Checked By GEM Date 1/29/19



ONE DIMENSIONAL CONSOLIDATION
AASHTO T-216

Client Kleinfelder Boring No. L_4800
 Client Project U-5813 Depth (ft) 0-2.0
 Project No. R-2019-011-001 Sample No. ST-1
 Lab ID R-2019-011-001-001 Visual Description TAN SILTY CLAY

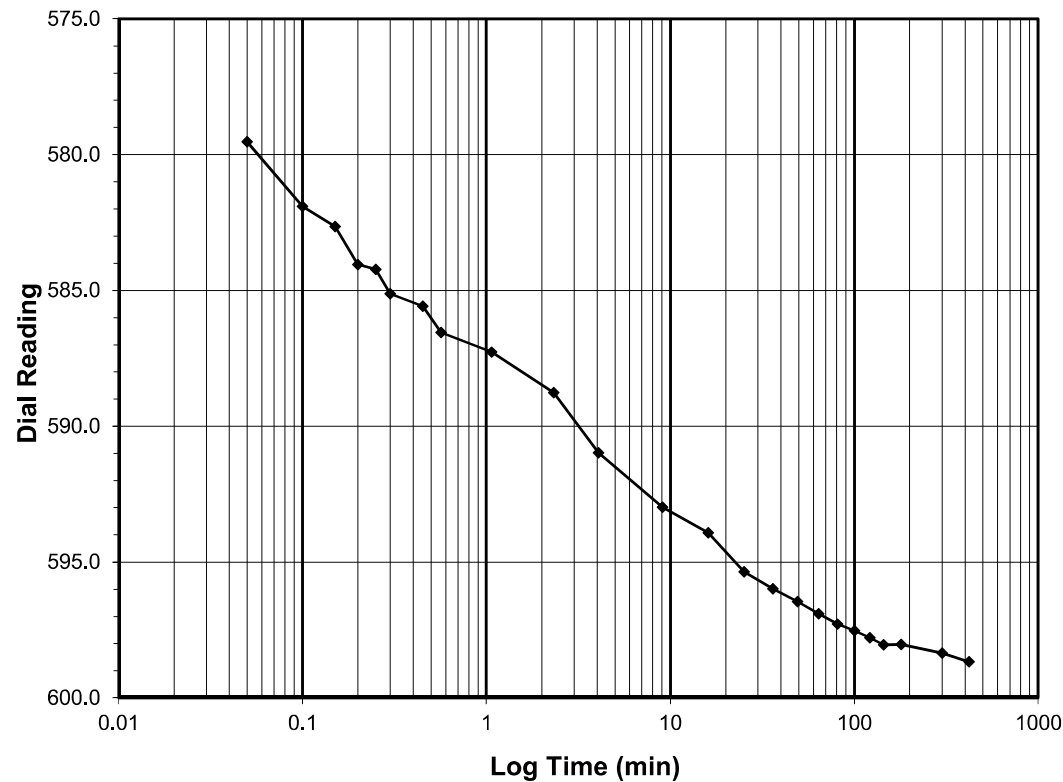
Sample Conditions: UNDISTURBED, INUNDATED AND DOUBLE DRAINED



Test Load (tsf) 1.0-2.0
Final Reading (div) 598.7
 Consolidometer No. **R409**
 1 Division (in) 0.0001

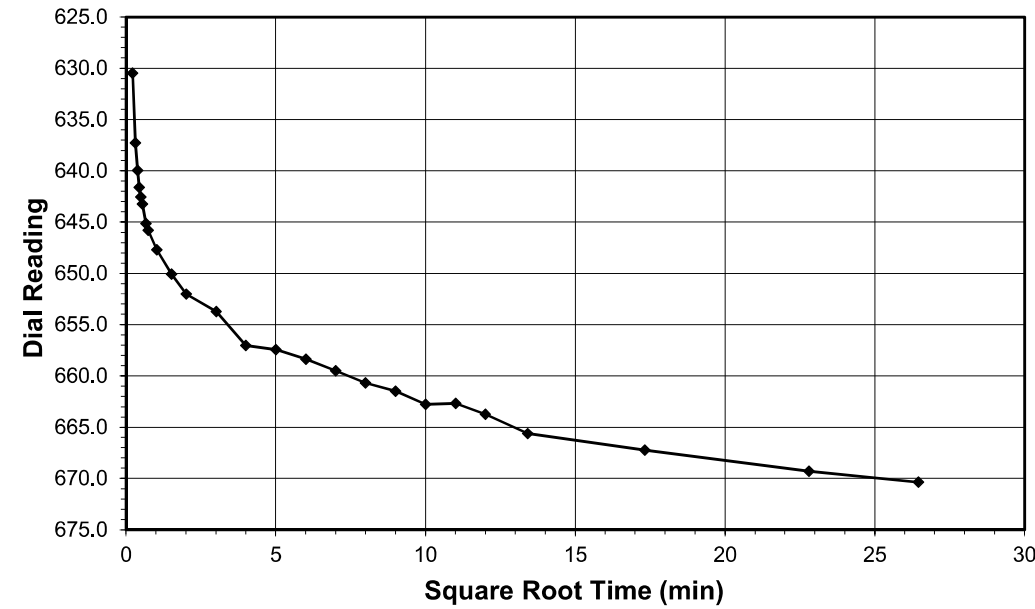
Start Date 1/19/19
 Start Time 7:34:46

Elapsed Time (min)	Dial Reading (div)
Initial	555.8
0.05	579.5
0.10	581.9
0.15	582.7
0.20	584.1
0.25	584.2
0.30	585.1
0.45	585.6
0.57	586.5
1.07	587.3
2.32	588.8
4.07	591.0
9.07	593.0
16.07	593.9
25.07	595.4
36.07	596.0
49.07	596.4
64.07	596.9
81.07	597.3
100.07	597.5
121.07	597.8
144.07	598.1
180.07	598.0
300.08	598.4
420.23	598.7



Client Kleinfelder Boring No. L_4800
 Client Project U-5813 Depth (ft) 0-2.0
 Project No. R-2019-011-001 Sample No. ST-1
 Lab ID R-2019-011-001-001 Visual Description TAN SILTY CLAY

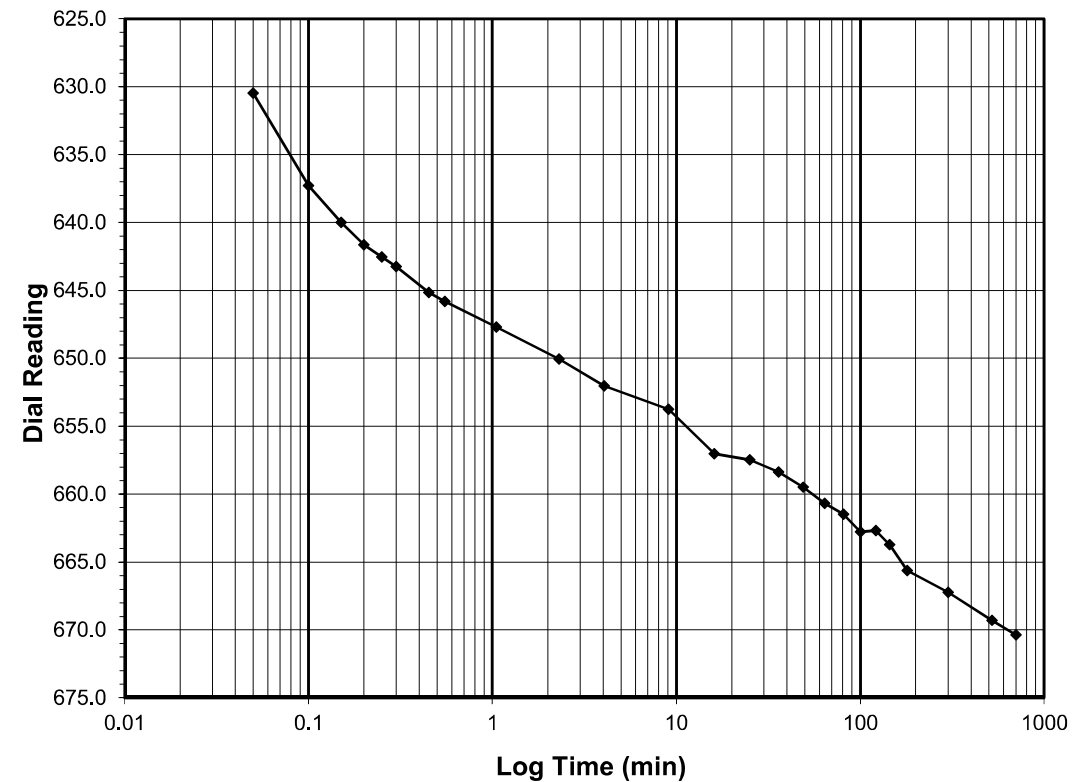
Sample Conditions: UNDISTURBED, INUNDATED AND DOUBLE DRAINED



Test Load (tsf) 2.0-4.0
Final Reading (div) 670.4
 Consolidometer No. **R409**
 1 Division (in) 0.0001

Start Date 1/19/19
 Start Time 14:35:00

Elapsed Time (min)	Dial Reading (div)
Initial	598.7
0.05	630.5
0.10	637.3
0.15	640.0
0.20	641.6
0.25	642.5
0.30	643.2
0.45	645.2
0.55	645.8
1.05	647.7
2.30	650.1
4.05	652.0
9.05	653.7
16.05	657.0
25.05	657.5
36.05	658.4
49.05	659.5
64.05	660.7
81.07	661.5
100.07	662.8
121.07	662.7
144.07	663.7
180.07	665.6
300.07	667.2
520.07	669.3
700.07	670.4



Tested By 129-04-0411 Date 1/19/19 Checked By GEM Date 1/29/19

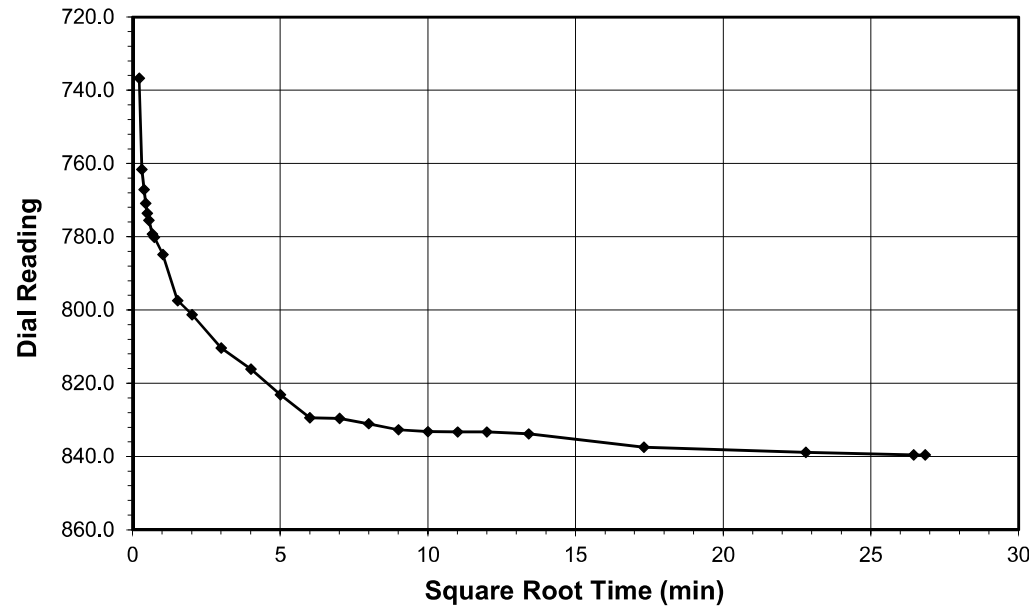
Tested By 129-04-0411 Date 1/19/19 Checked By GEM Date 1/29/19



ONE DIMENSIONAL CONSOLIDATION
AASHTO T-216

Client Kleinfelder Boring No. L_4800
 Client Project U-5813 Depth (ft) 0-2.0
 Project No. R-2019-011-001 Sample No. ST-1
 Lab ID R-2019-011-001-001 Visual Description TAN SILTY CLAY

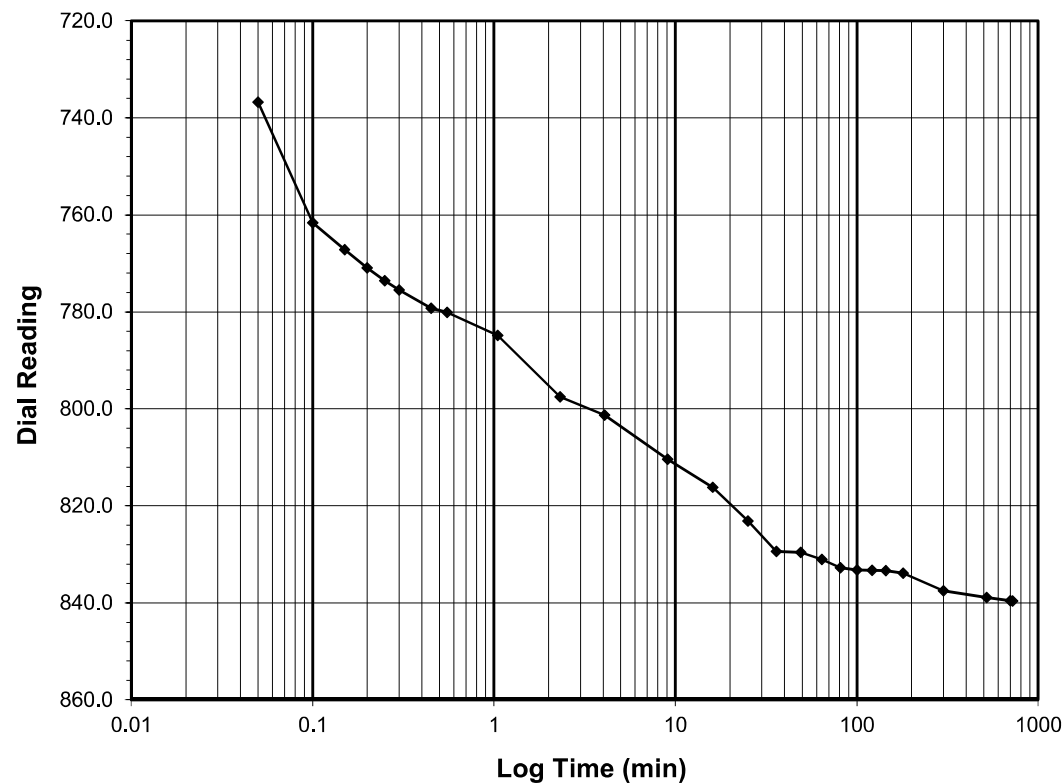
Sample Conditions: UNDISTURBED, INUNDATED AND DOUBLE DRAINED



Test Load (tsf) 4.0-8.0
Final Reading (div) 839.6
 Consolidometer No. **R409**
 1 Division (in) 0.0001

Start Date 1/20/19
 Start Time 2:35:20

Elapsed Time (min)	Dial Reading (div)
Initial	670.4
0.05	736.8
0.10	761.7
0.15	767.2
0.20	770.9
0.25	773.6
0.30	775.5
0.45	779.2
0.55	780.1
1.05	784.9
2.32	797.5
4.07	801.3
9.07	810.4
16.07	816.1
25.07	823.1
36.07	829.4
49.07	829.6
64.07	831.0
81.07	832.7
100.07	833.2
121.07	833.3
144.07	833.3
180.07	833.9
300.07	837.5
520.07	838.9
700.07	839.6
720.33	839.6



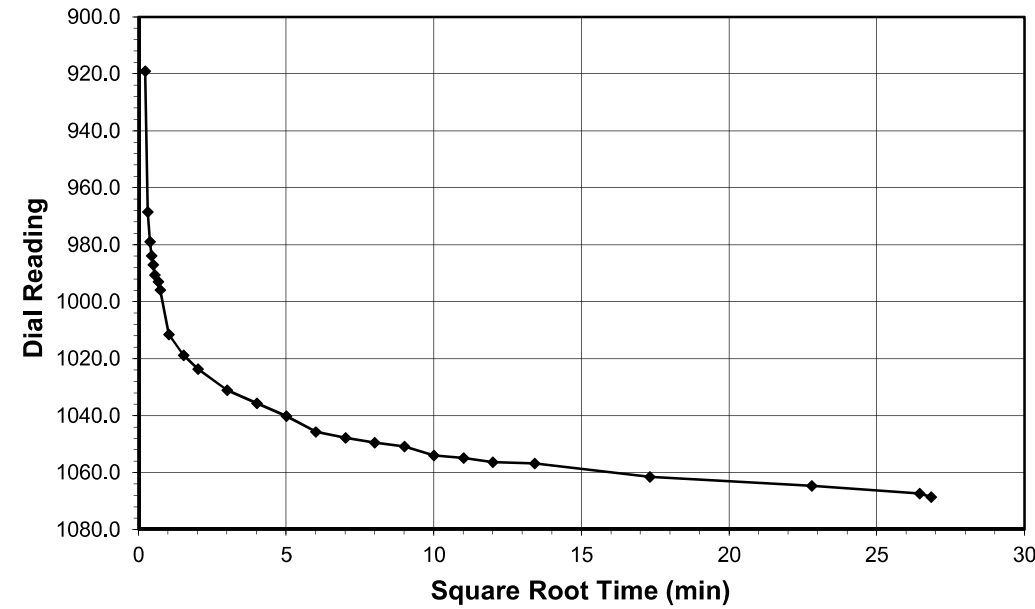
Tested By 129-04-0411 Date 1/20/19 Checked By GEM Date 1/29/19



ONE DIMENSIONAL CONSOLIDATION
AASHTO T-216

Client Kleinfelder Boring No. L_4800
 Client Project U-5813 Depth (ft) 0-2.0
 Project No. R-2019-011-001 Sample No. ST-1
 Lab ID R-2019-011-001-001 Visual Description TAN SILTY CLAY

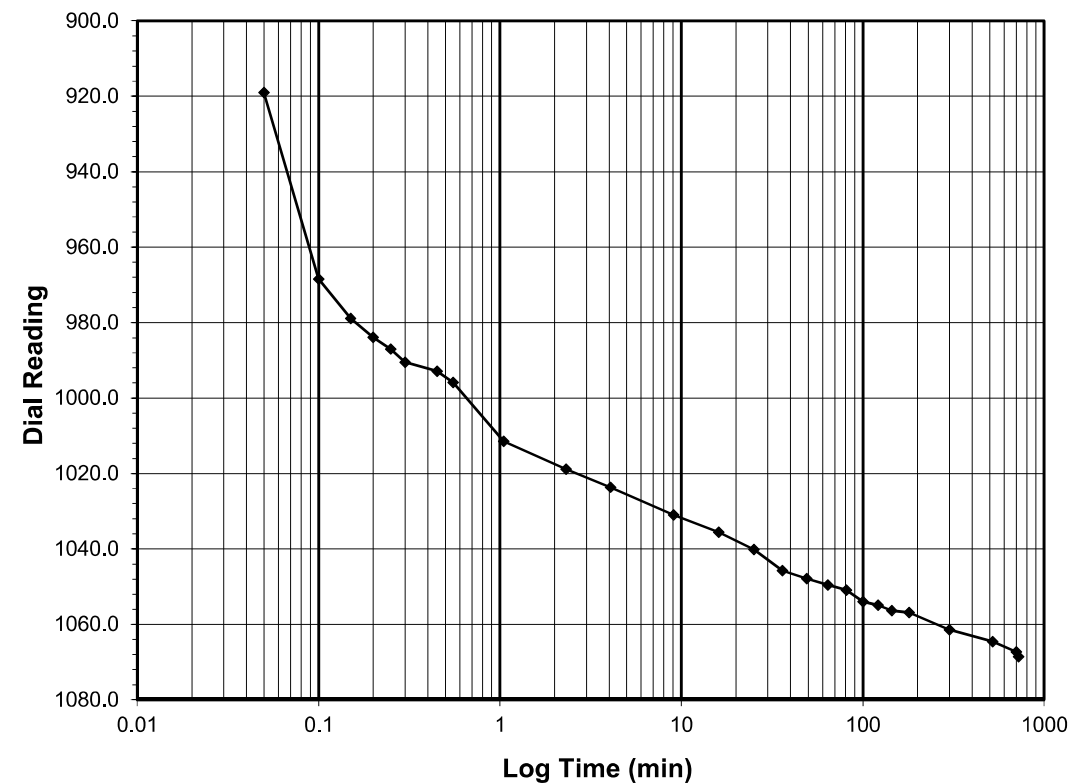
Sample Conditions: UNDISTURBED, INUNDATED AND DOUBLE DRAINED



Test Load (tsf) 8.0-16.0
Final Reading (div) 1068.6
 Consolidometer No. **R409**
 1 Division (in) 0.0001

Start Date 1/20/19
 Start Time 14:35:40

Elapsed Time (min)	Dial Reading (div)
Initial	839.6
0.05	919.1
0.10	968.4
0.15	978.9
0.20	983.9
0.25	987.0
0.30	990.6
0.45	992.9
0.55	995.9
1.05	1011.5
2.32	1018.8
4.07	1023.6
9.07	1031.0
16.07	1035.6
25.07	1040.1
36.07	1045.7
49.07	1047.8
64.07	1049.6
81.07	1050.9
100.07	1054.0
121.07	1054.9
144.07	1056.3
180.07	1056.8
300.07	1061.5
520.08	1064.6
700.08	1067.4
720.40	1068.6



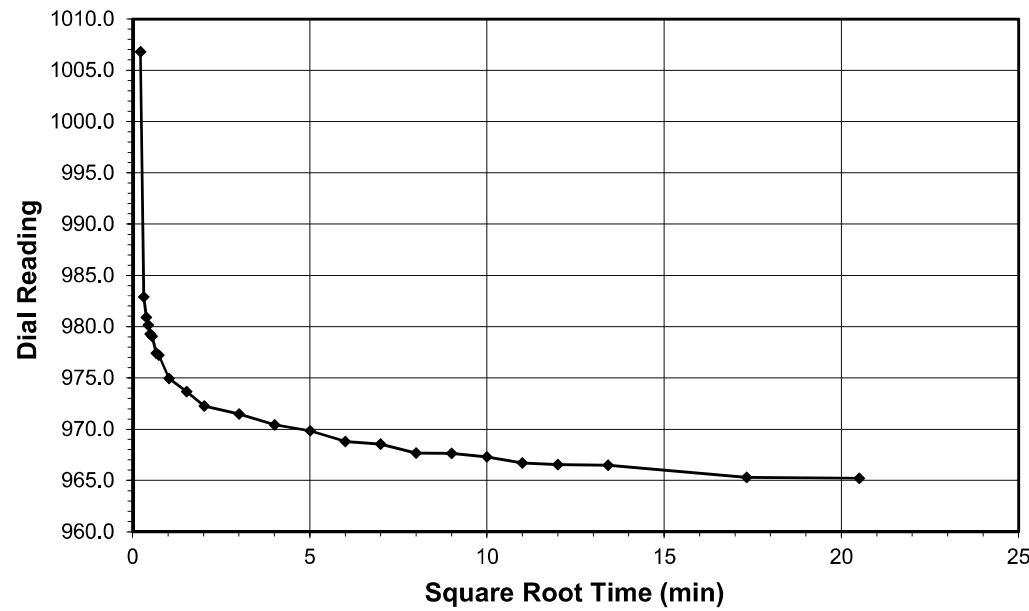
Tested By 129-04-0411 Date 1/20/19 Checked By GEM Date 1/29/19



ONE DIMENSIONAL CONSOLIDATION
AASHTO T-216

Client Kleinfelder Boring No. L_4800
 Client Project U-5813 Depth (ft) 0-2.0
 Project No. R-2019-011-001 Sample No. ST-1
 Lab ID R-2019-011-001-001 Visual Description TAN SILTY CLAY

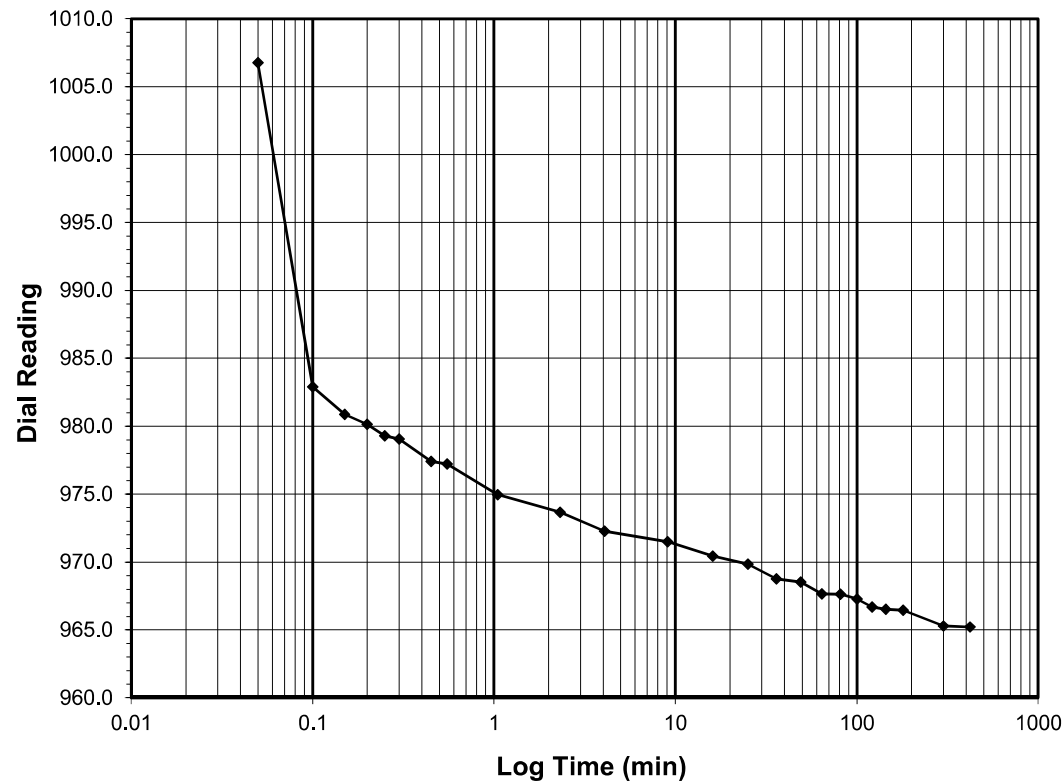
Sample Conditions: UNDISTURBED, INUNDATED AND DOUBLE DRAINED



Test Load (tsf) 16.0-4.0
Final Reading (div) 965.2
 Consolidometer No. **R409**
 1 Division (in) 0.0001

Start Date 1/21/19
 Start Time 2:36:04

Elapsed Time (min)	Dial Reading (div)
Initial	1068.6
0.05	1006.8
0.10	982.9
0.15	980.9
0.20	980.2
0.25	979.3
0.30	979.1
0.45	977.4
0.55	977.2
1.05	974.9
2.32	973.7
4.07	972.3
9.07	971.5
16.07	970.4
25.07	969.9
36.07	968.8
49.07	968.5
64.07	967.7
81.07	967.6
100.07	967.3
121.07	966.7
144.07	966.5
180.07	966.5
300.07	965.3
420.37	965.2



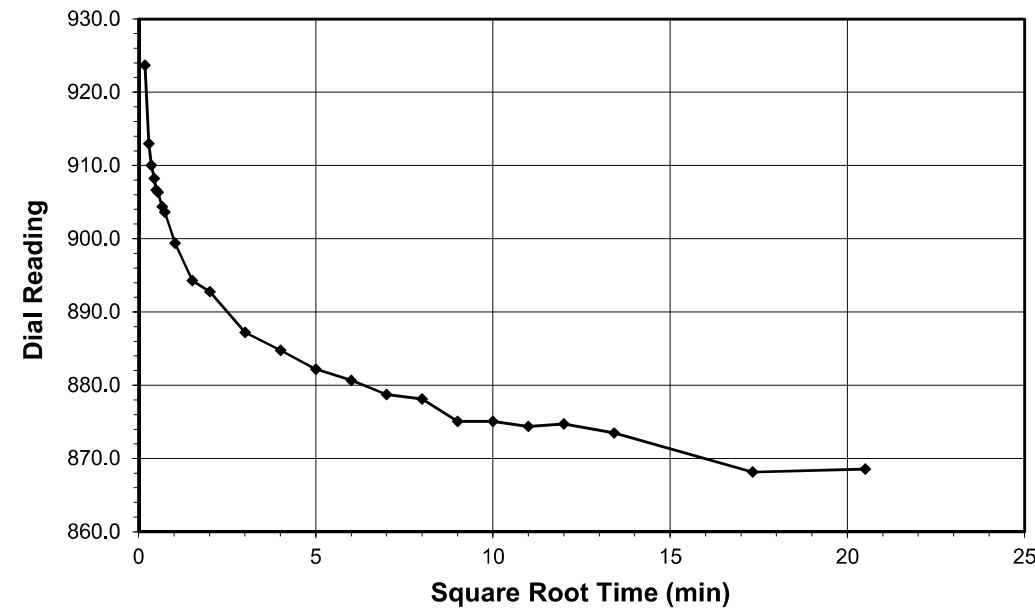
Tested By 129-04-0411 Date 1/21/19 Checked By GEM Date 1/29/19



ONE DIMENSIONAL CONSOLIDATION
AASHTO T-216

Client Kleinfelder Boring No. L_4800
 Client Project U-5813 Depth (ft) 0-2.0
 Project No. R-2019-011-001 Sample No. ST-1
 Lab ID R-2019-011-001-001 Visual Description TAN SILTY CLAY

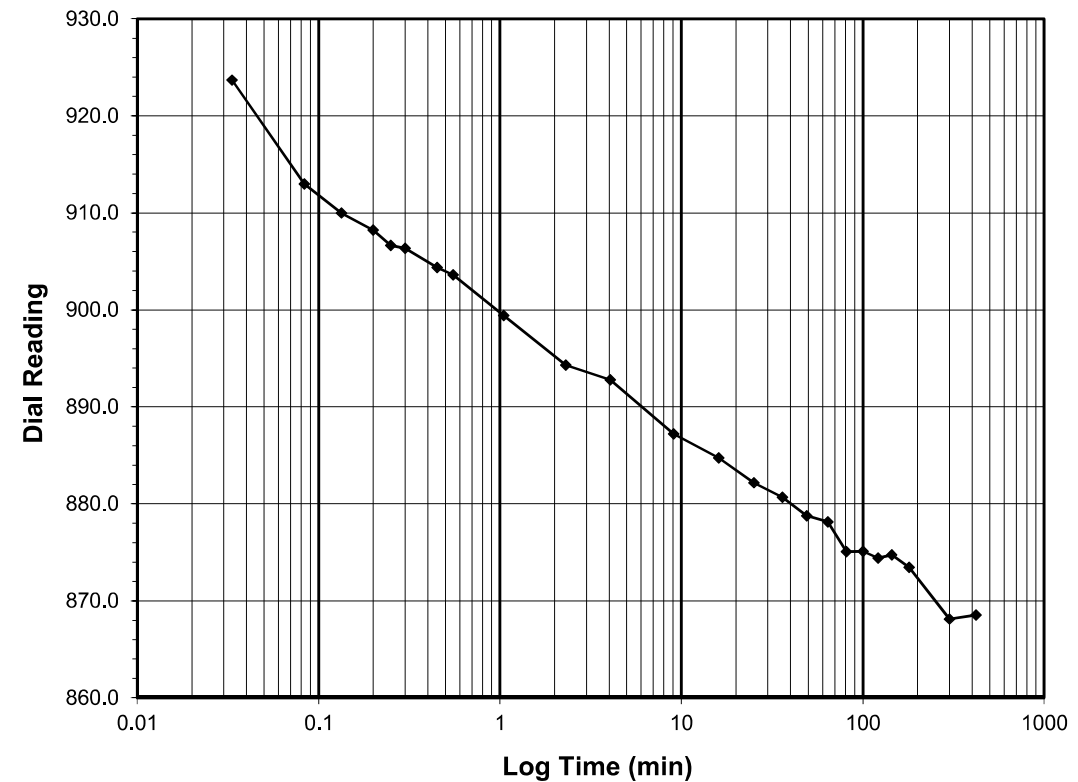
Sample Conditions: UNDISTURBED, INUNDATED AND DOUBLE DRAINED



Test Load (tsf) 4.0-1.0
Final Reading (div) 868.6
 Consolidometer No. **R409**
 1 Division (in) 0.0001

Start Date 1/21/19
 Start Time 9:36:26

Elapsed Time (min)	Dial Reading (div)
Initial	965.2
0.03	923.7
0.08	913.0
0.13	910.0
0.20	908.2
0.25	906.6
0.30	906.3
0.45	904.4
0.55	903.6
1.05	899.4
2.30	894.3
4.05	892.8
9.05	887.2
16.05	884.8
25.05	882.2
36.05	880.7
49.05	878.8
64.05	878.1
81.05	875.1
100.05	875.1
121.05	874.4
144.05	874.7
180.07	873.5
300.07	868.1
420.42	868.6



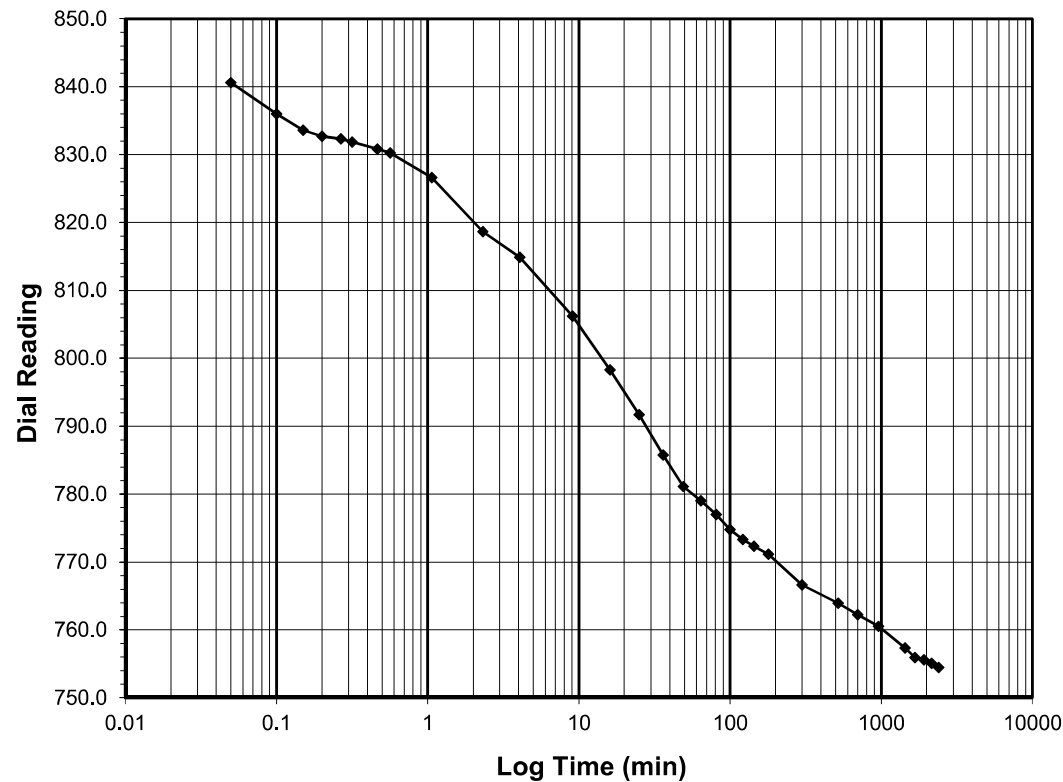
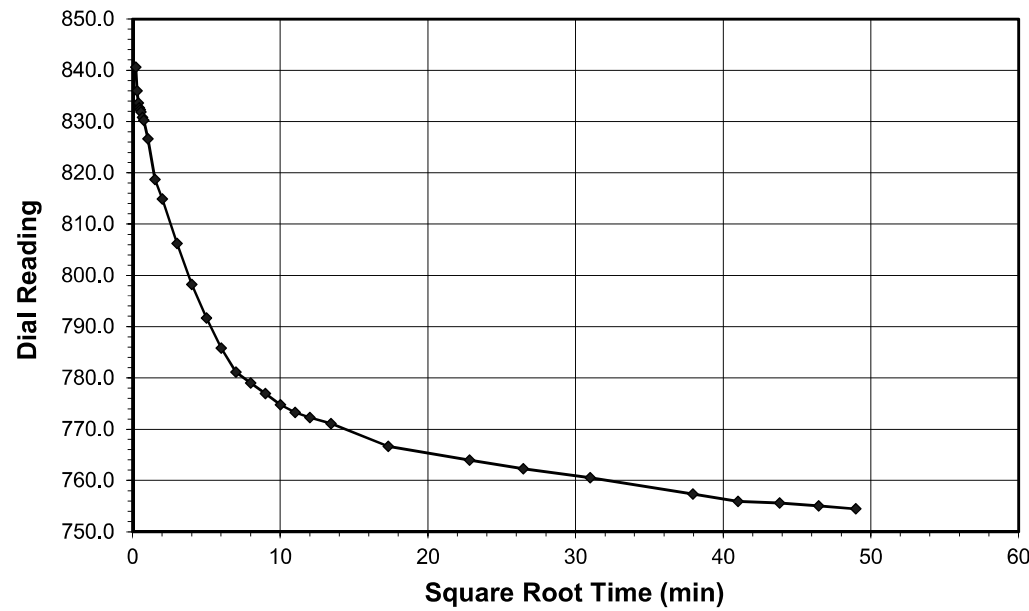
Tested By 129-04-0411 Date 1/21/19 Checked By GEM Date 1/29/19



ONE DIMENSIONAL CONSOLIDATION
AASHTO T-216

Client: Kleinfelder Boring No.: L_4800
 Client Project: U-5813 Depth (ft): 0-2.0
 Project No.: R-2019-011-001 Sample No.: ST-1
 Lab ID: R-2019-011-001-001 Visual Description: TAN SILTY CLAY

Sample Conditions: UNDISTURBED, INUNDATED AND DOUBLE DRAINED



Test Load (tsf) 1.0-0.25
 Final Reading (div) 754.5
 Consolidometer No. R409
 1 Division (in) 0.0001

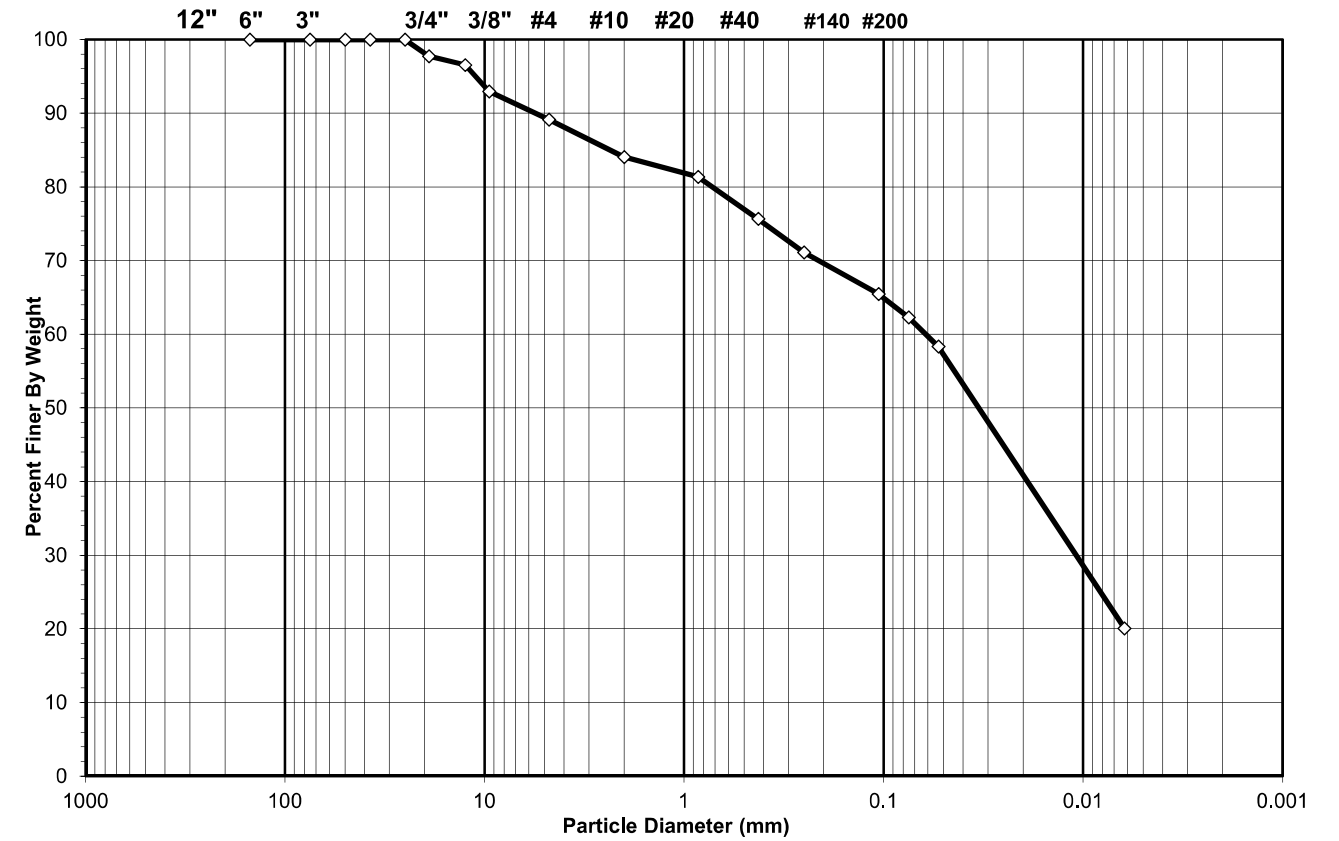
Start Date 1/21/19
 Start Time 16:36:52

Elapsed Time (min)	Dial Reading (div)
Initial	868.6
0.05	840.6
0.10	836.0
0.15	833.6
0.20	832.7
0.27	832.3
0.32	831.8
0.47	830.8
0.57	830.3
1.07	826.6
2.32	818.7
4.07	814.9
9.07	806.2
16.07	798.3
25.07	791.7
36.08	785.8
49.08	781.1
64.08	779.0
81.08	777.0
100.08	774.8
121.08	773.3
144.08	772.3
180.08	771.1
300.08	766.6
520.08	764.0
700.08	762.3
960.10	760.5
1440.10	757.3
1680.10	755.9
1920.10	755.6
2160.10	755.1
2397.53	754.5

SIEVE AND HYDROMETER ANALYSIS
NCDOT MOD. AASHTO T-88,

Client: Kleinfelder Boring No.: L_7250
 Client Reference: U-5813 Depth (ft): 0-2.0
 Project No.: R-2019-011-001 Sample No.: ST-2
 Lab ID: R-2019-011-001-003 Soil Color: Tan

USCS	SIEVE ANALYSIS			HYDROMETER
	cobbles	gravel	sand	silt and clay fraction
AASHTO	cobbles	gravel	sand	silt and clay fraction



Sieve Size (mm)	Percent Finer	USCS %	AASHTO %	NCDOT SOIL MORTAR %
100	100.00	Gravel 10.87	Gravel 15.93	Coarse Sand Ret. #60 15.44
2	84.07	Sand 26.83	Coarse Sand 8.45	Fine Sand Ret. #270 15.22
0.075	62.30	Silt&Clay 62.30	Fine Sand 13.32	Silt 0.05-0.005mm 45.46
			Silt & Clay 62.30	Clay <0.005mm 23.87

AASHTO (GI) A-6 (5)

Tested By 129-04-0411 Date 1/21/19 Checked By GEM Date 1/29/19



WASH SIEVE ANALYSIS

NCDOT MOD. AASHTO T-88,

HYDROMETER ANALYSIS

NCDOT MOD. AASHTO T-88,

Client Kleinfelder Boring No. L_7250
 Client Reference U-5813 Depth (ft) 0-2.0
 Project No. R-2019-011-001 Sample No. ST-2
 Lab ID R-2019-011-001-003 Soil Color Tan

Client Kleinfelder Boring No. L_7250
 Client Reference U-5813 Depth (ft) 0-2.0
 Project No. R-2019-011-001 Sample No. ST-2
 Lab ID R-2019-011-001-003 Soil Color Tan

Minus #10 for Hygroscopic (10-15gm)		Hydrometer Specimen 50 or 100gms	
Tare No.	C-27	Air Dried Hydrometer Material (gm)	53.74
Wgt. Tare + Wet Specimen (gm)	36.35	Corrected Dry Wt. of Hydro Mtrl. (gm)	49.47
Wgt. Tare + Dry Specimen (gm)	35.20		
Weight of Tare (gm)	21.86	Weight of -#270 Material	34.30
Weight of Water (gm)	1.15	Weight of -#10; +#270 Material	15.17
Weight of Dry Soil (gm)	13.34		
Moisture Content (%)	8.6		

Tare No.	921		
Wgt. Tare + Air Dry Soil (gm)	747.22	Dry Weight of Material Ret. #10 (gm)	77.66
Weight of Tare (gm)	224.37	Corrected Dry Sample Wt - #10 (gm)	409.86
Air Dried Wgt. Total Sample (gm)	522.85		
Total Dry Weight Sample (gm)	487.5	J - Factor (Percent Finer than #10)	0.8407

Sieve Size	Sieve Opening (mm)	Wgt. of Soil Retained (gm)	Percent Retained (%)	Accumulated Percent Retained (%)	Percent Finer (%)	Accumulated Percent Finer (%)
12"	300	0.00	0.00	0.00	100.00	100.00
6"	150	0.00	0.00	0.00	100.00	100.00
3"	75	0.00	0.00	0.00	100.00	100.00
2"	50	0.00	0.00	0.00	100.00	100.00
1 1/2"	37.5	0.00	0.00	0.00	100.00	100.00
1"	25.0	0.00	0.00	0.00	100.00	100.00
3/4"	19.0	11.04	2.26	2.26	97.74	97.74
1/2"	12.5	5.90	1.21	3.47	96.53	96.53
3/8"	9.5	17.57	3.60	7.08	92.92	92.92
#4	4.75	18.48	3.79	10.87	89.13	89.13
#10	2.00	24.67	5.06	15.93	84.07	84.07
#20	0.85	1.60	3.23	3.23	96.77	81.35
#40	0.425	3.37	6.81	10.05	89.95	75.63
#60	0.25	2.67	5.40	15.44	84.56	71.09
#140	0.106	3.30	6.67	22.11	77.89	65.48
#200	0.075	1.87	3.78	25.89	74.11	62.30
#270	0.053	2.36	4.77	30.66	69.34	58.29
Pan	-	34.30	69.34	100.00	-	-

Elapsed Time (min)	R Measured	Temp. (°C)	Composite Correction	R Corrected	N (%)	K Factor	Diameter (mm)	N' (%)
11:28:00	0	NA	NA	NA	NA	NA	NA	NA
11:28:33	0.55	37.0	-2.21	33.2	66.7	0.01307	0.0563	56.1
12:28:00	60.00	15.5	-2.38	11.9	23.9	0.01294	0.0062	20.1

Corrections		
a - Factor		0.994
Percent Finer than # 10		84.07
Specific Gravity		2.68 Assumed

Note: Hydrometer test is performed on - #10 sieve material.

LL = 31
 PL = 20
 PI = 11

Tested By 129-05-0411 Date 1/21/19 Checked By GEM Date 1/21/19

Tested By 129-05-0411 Date 1/18/19 Checked By GEM Date 1/21/19



ATTERBERG LIMITS
AASHTO T-89, T-90 (DOT Modified)

Client	Kleinfelder	Boring No.	L_7250
Client Reference	U-5813	Depth (ft)	0-2.0
Project No.	R-2019-011-001	Sample No.	ST-2
Lab ID	R-2019-011-001-003	Soil Description	TAN LEAN CLAY

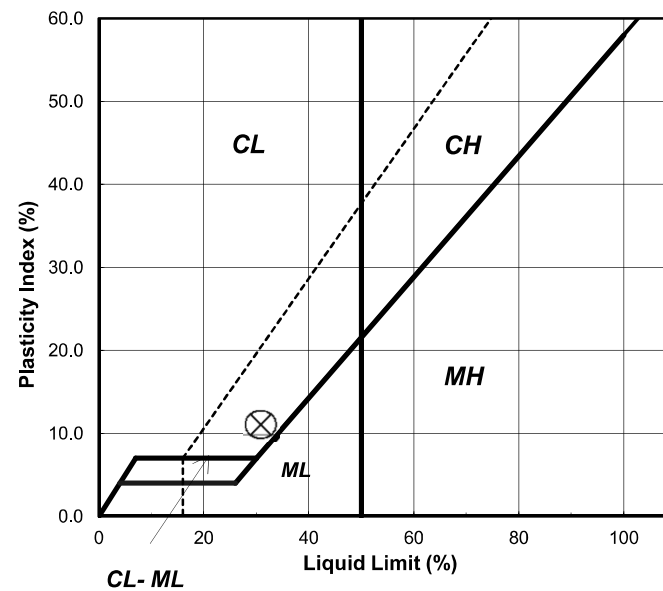
Note: The USCS symbol used with this test refers only to the minus No. 40 (Minus No. 40 sieve material, Airdried)
sieve material. See the "Sieve and Hydrometer Analysis" graph page for the complete material description.

Liquid Limit Test		1
Tare Number		A-I
Wt. of Tare & WS (gm)		27.96
Wt. of Tare & DS (gm)		25.02
Wt. of Tare (gm)		15.39
Wt. of Water (gm)		2.9
Wt. of DS (gm)		9.6
Moisture Content (%)		30.5
Number of Blows		25

Plastic Limit Test	1	2	Range	Test Results
Tare Number	R	A-L		Liquid Limit (%) 31
Wt. of Tare & WS (gm)	22.99	22.55		Plastic Limit (%) 20
Wt. of Tare & DS (gm)	21.66	21.39		Plasticity Index (%) 11
Wt. of Tare (gm)	15.09	15.45		USCS Symbol CL
Wt. of Water (gm)	1.3	1.2		
Wt. of DS (gm)	6.6	5.9		
Moisture Content (%)	20.2	19.5	0.7	

Note: The acceptable range of the two Moisture contents is ± 2.6

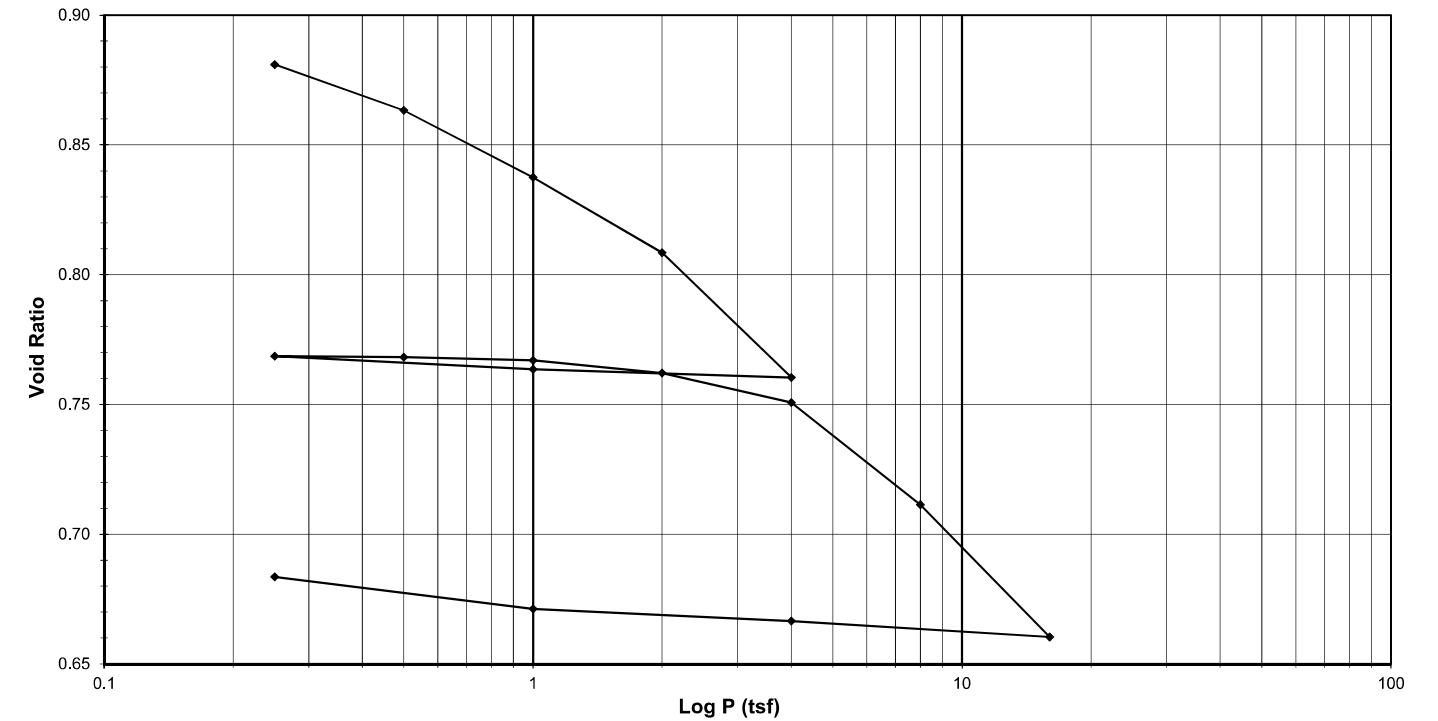
Plasticity Chart



ONE DIMENSIONAL CONSOLIDATION
AASHTO T-216

Client	Kleinfelder	Boring No.	L_7250
Client Reference	U-5813	Depth (ft)	0-2.0
Project No.	R-2019-011-001	Sample No.	ST-2
Lab ID	R-2019-011-001-003	Visual Description	TAN SILTY CLAY

Sample Conditions: UNDISTURBED, INUNDATED AND DOUBLE DRAINED



Tested By 129-04-0411 Date 1/16/19 Approved By MPS Date 1/29/19

page 1 of 4

DCN: CT-24E Date: 5/3/12 Revision: 6

Z:\2019 PROJECTS\KLEINFELDER\2019-011 KLEINFELDER - U-5813\2019-011-001-003 DOT GEOJAC-16TSF1 Cv.xlsm\FINAL PLOT

2200 Westinghouse Blvd., Suite 103 • Raleigh, NC 27604 • Phone (919) 876-0405 • Fax (919) 876-0460 • www.geotechnics.net

Tested By 129-05-0411 Date 1/19/19 Checked By GEM Date 1/21/19

page 1 of 1 DCN: CT-S4B DATE: 10/8/01 REVISION: 2



ONE DIMENSIONAL CONSOLIDATION
AASHTO T-216

Client Kleinfelder Boring No. L_7250
 Client Reference U-5813 Depth (ft) 0-2.0
 Project No. R-2019-011-001 Sample No. ST-2
 Lab ID R-2019-011-001-003 Visual Description TAN SILTY CLAY

Sample Conditions: UNDISTURBED, INUNDATED AND DOUBLE DRAINED
 Consolidometer No. R553
 1 Division = 0.0001 (in.)

Sample Properties	Initial	Final	Test Data Summary								
			Applied Pressure (tsf)	Final Dial Reading (div)	Machine Deflection (div)	Corrected Reading (div)	Height of Sample (mm)	Volume (cc)	Dry Density (g/cc)	Void Ratio	
<i>Water Content</i>											
Tare Number	860	TB-04									
Wt. Tare & WS (g)	240.39	278.49									
Wt. Tare & DS (g)	215.86	253.12									
Wt. Water (g)	24.53	25.37	Seating	0	0	0	25.400	80.440	1.41298	0.91086	
Wt. Tare (g)	135.07	134.77	0.25	169.2	12.8	156.4	25.003	79.181	1.43544	0.88096	
Wt. DS (g)	80.79	118.35	0.5	269.5	20.7	248.8	24.768	78.438	1.44903	0.86331	
Water Content (%)	30.36	21.44	1	420.8	36.9	383.8	24.425	77.352	1.46938	0.83751	
			2	623.7	87.8	535.8	24.039	76.130	1.49298	0.80846	
			4	875.4	87.8	787.6	23.399	74.104	1.53378	0.76035	
<i>Sample Parameters</i>			1	822.3	51.5	770.8	23.442	74.240	1.53099	0.76357	
Sample Diameter (in)	2.5	2.5	0.25	772.4	27.9	744.5	23.509	74.451	1.52664	0.76859	
Sample Height (in)	1.0000	0.8811	0.5	776.6	30.2	746.4	23.504	74.436	1.52695	0.76824	
Sample Volume (cc)	80.44	70.87	1	795.4	42.2	753.1	23.487	74.382	1.52807	0.76694	
Wt. Wet Sample + Ring (g)	252.49	242.34	2	835.3	56.7	778.6	23.422	74.177	1.53229	0.76207	
Wt. of Ring (g)	104.32	104.32	4	926.1	88.0	838.1	23.271	73.698	1.54223	0.75071	
Wt. of Wet Sample (g)	148.17	138.02	8	1178.1	133.9	1044.2	22.748	72.040	1.57773	0.71132	
Wet Density (pcf)	114.94	121.52	16	1500.4	189.8	1310.6	22.071	69.897	1.62610	0.66042	
Wet Density (g/cc)	1.84	1.95	4	1387.4	109.0	1278.4	22.153	70.156	1.62009	0.66657	
Water Content (%)	30.36	21.44	1	1314.3	60.6	1253.8	22.215	70.355	1.61553	0.67128	
Wt. of Dry Sample (g)	113.66	113.66	0.25	1226.8	37.6	1189.3	22.379	70.873	1.60370	0.68361	
Dry Density (pcf)	88.17	100.07									
Dry Density (g/cc)	1.41	1.60									
Void Ratio	0.9109	0.6836									
Saturation (%)	90.00	84.67									
Specific Gravity	2.70	Assumed									

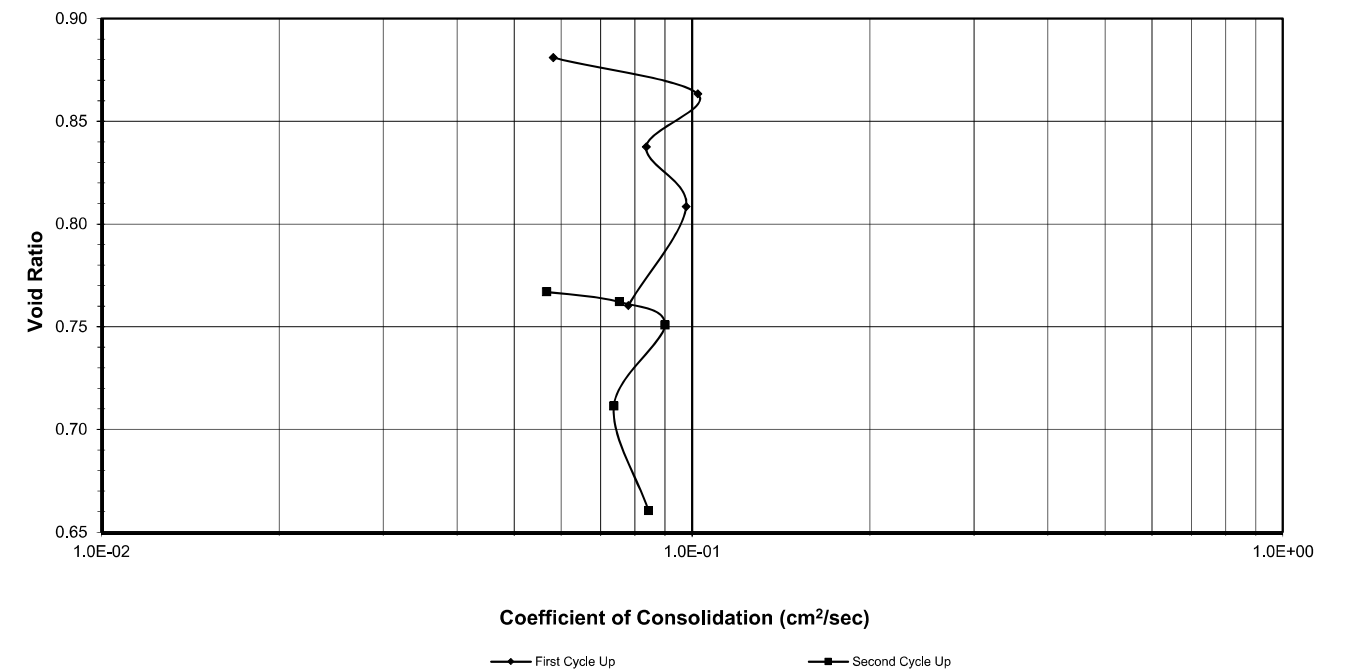
Tested By 129-04-0411 Date 1/16/19 Input Checked By GEM Date 1/29/19



ONE DIMENSIONAL CONSOLIDATION
AASHTO T-216

Client Kleinfelder Boring No. L_7250
 Client Reference U-5813 Depth (ft) 0-2.0
 Project No. R-2019-011-001 Sample No. ST-2
 Lab ID R-2019-011-001-003 Visual Description TAN SILTY CLAY

Sample Conditions: UNDISTURBED, INUNDATED AND DOUBLE DRAINED



Tested By 129-04-0411 Date 1/16/19 Input Checked By GEM Date 1/29/19



ONE DIMENSIONAL CONSOLIDATION

AASHTO T-216

Client	Kleinfelder	Boring No.	L_7250
Client Project	U-5813	Depth (ft)	0-2.0
Project No.	R-2019-011-001	Sample No.	ST-2
Lab ID	R-2019-011-001-003	Visual Description	TAN SILTY CLAY



ONE DIMENSIONAL CONSOLIDATION
AASHTO T-216

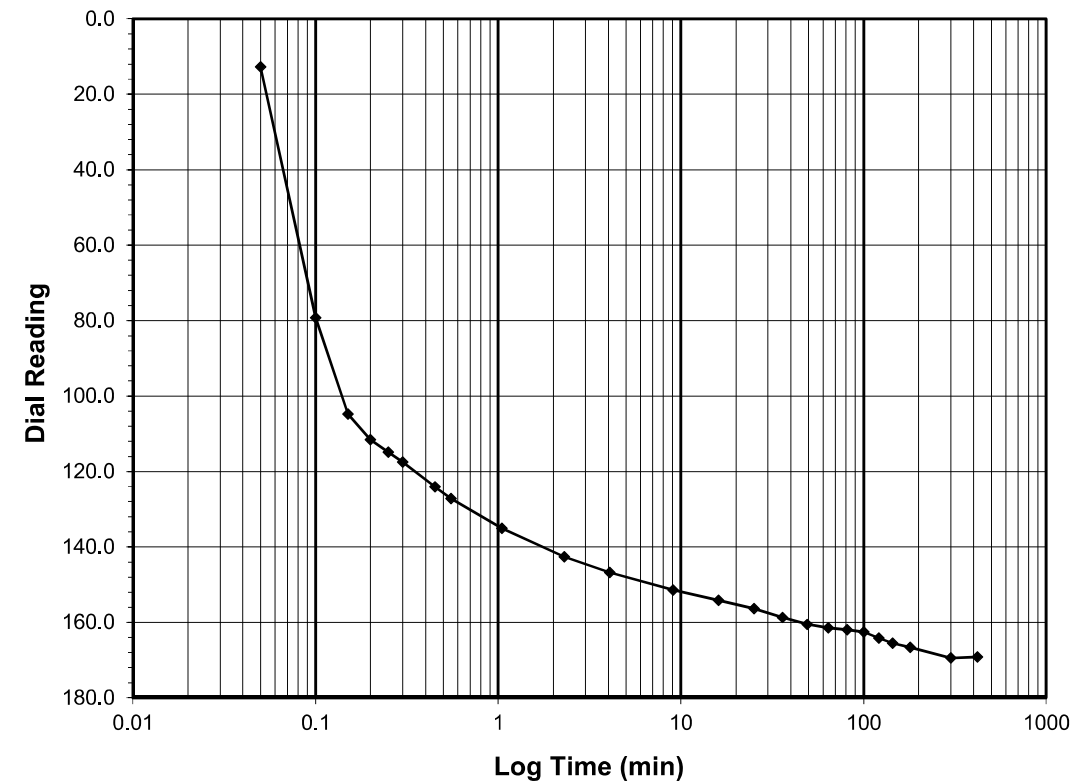
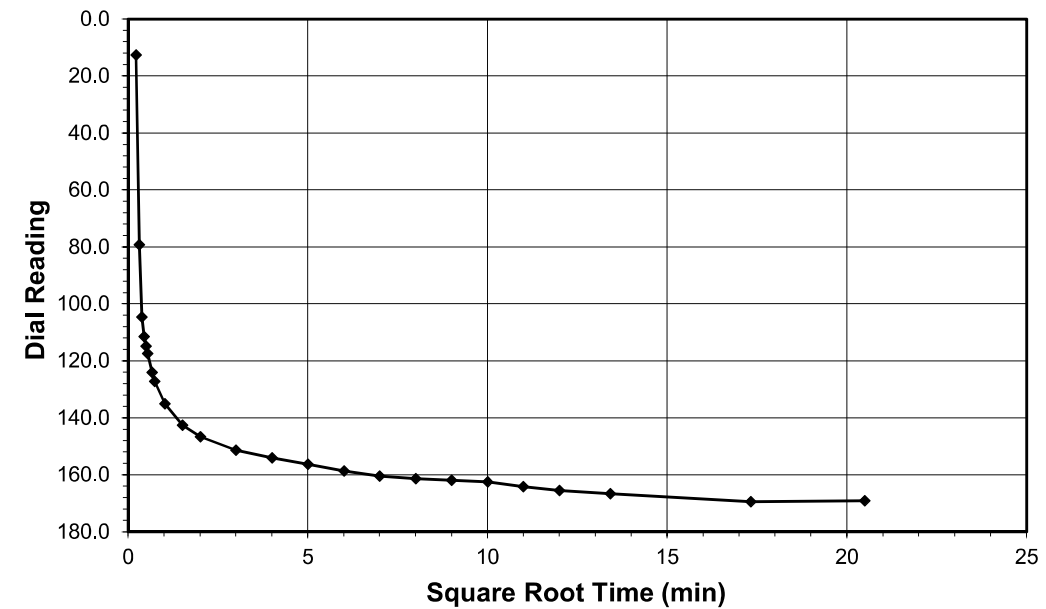
Client	Kleinfelder	Boring No.	L_7250
Client Reference	U-5813	Depth (ft)	0-2.0
Project No.	R-2019-011-001	Sample No.	ST-2
Lab ID	R-2019-011-001-003	Visual Description	TAN SILTY CLAY

Sample Conditions: UNDISTURBED, INUNDATED AND DOUBLE DRAINED
 Consolidometer No. R553
 1 Division = 0.0001 (in.)

Sample Properties	Initial	Final	C _v Test Data Summary					
			Load Increment (tsf)	Dial Reading @ t ₅₀ (div)	Machine Deflection (div)	Corrected Dial Reading @ t ₅₀ (div)	Sample Height @ t ₅₀ (cm)	Time t ₅₀ (min.)
Water Content								
Tare Number	860	TB-04						
Wt. Tare & WS (g)	240.39	278.49						
Wt. Tare & DS (g)	215.86	253.12						
Wt. Water (g)	24.53	25.37	0 - 0.25	65.5	12.8	52.7	2.527	0.09
Wt. Tare (g)	135.07	134.77	0.25 - 0.5	198.2	20.7	177.5	2.495	0.05
Wt. DS (g)	80.79	118.35	0.5 - 1.0	302.3	36.9	265.3	2.473	0.06
Water Content (%)	30.36	21.44	1.0 - 2.0	480.7	87.8	392.9	2.440	0.05
			2.0 - 4.0	688.0	87.8	600.1	2.388	0.06
			4.0 - 1.0	NA	51.5	NA	NA	NA
			1.0 - 0.25	NA	27.9	NA	NA	NA
			0.25 - 0.5	NA	30.2	NA	NA	NA
			0.5 - 1.0	786.9	42.2	744.7	2.351	0.08
			1.0 - 2.0	820.1	56.7	763.4	2.346	0.06
			2.0 - 4.0	873.1	88.0	785.1	2.341	0.05
			4.0 - 8.0	996.9	133.9	863.0	2.321	0.06
			8.0 - 16.0	1265.7	189.8	1075.8	2.267	0.05
			16.0 - 4.0	NA	109.0	NA	NA	NA
			4.0 - 1.0	NA	60.6	NA	NA	NA
			1.0 - 0.25	NA	37.6	NA	NA	NA

Tested By 129-04-0411 Date 1/16/19 Input Checked By GEM Date 1/29/19

Sample Conditions: UNDISTURBED, INUNDATED AND DOUBLE DRAINED



Test Load (tsf) 0.0-0.25
 Final Reading (div) 169.2
 Consolidometer No. R553
 1 Division (in) 0.0001
 Start Date 1/16/19
 Start Time 12:07:05

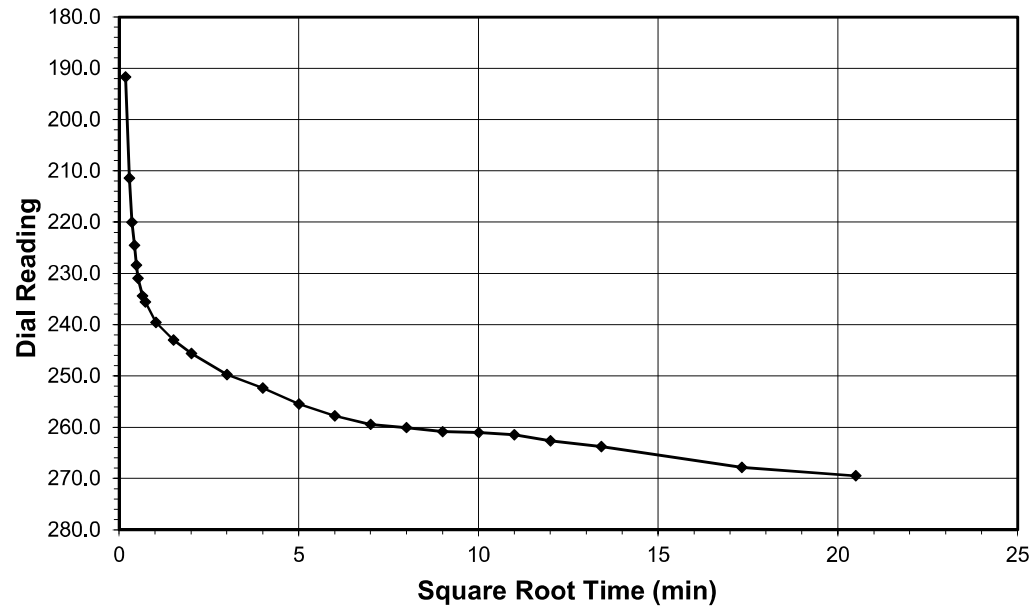
Elapsed Time (min)	Dial Reading (div)
Initial	0.0
0.05	12.7
0.10	79.2
0.15	104.7
0.20	111.5
0.25	114.9
0.30	117.5
0.45	124.1
0.55	127.2
1.05	135.1
2.30	142.6
4.07	146.7
9.07	151.4
16.07	154.1
25.07	156.3
36.07	158.7
49.07	160.5
64.07	161.4
81.07	162.0
100.07	162.5
121.07	164.2
144.07	165.6
180.07	166.6
300.07	169.5
420.02	169.2

Tested By 129-04-0411 Date 1/16/19 Checked By GEM Date 1/29/19

ONE DIMENSIONAL CONSOLIDATION
AASHTO T-216

Client Kleinfelder Boring No. L_7250
 Client Project U-5813 Depth (ft) 0-2.0
 Project No. R-2019-011-001 Sample No. ST-2
 Lab ID R-2019-011-001-003 Visual Description TAN SILTY CLAY

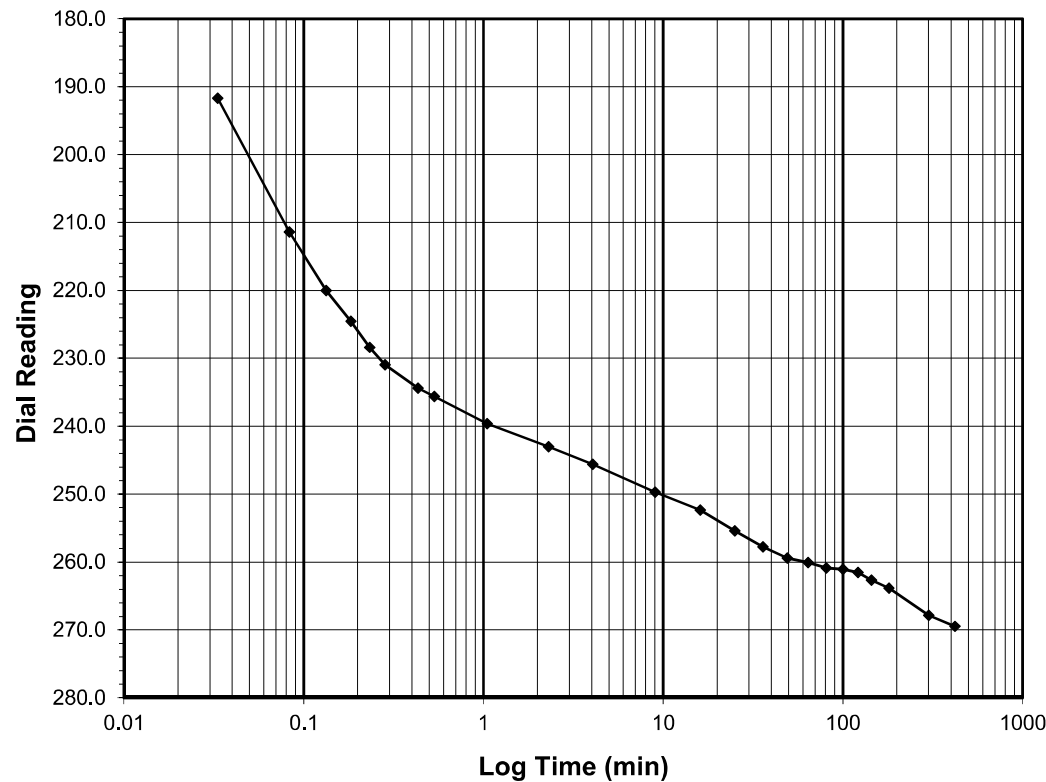
Sample Conditions: UNDISTURBED, INUNDATED AND DOUBLE DRAINED



Test Load (tsf) 0.25-0.5
 Final Reading (div) 269.5
 Consolidometer No. R553
 1 Division (in) 0.0001

Start Date 1/16/19
 Start Time 19:07:06

Elapsed Time (min)	Dial Reading (div)
Initial	169.2
0.03	191.7
0.08	211.4
0.13	220.0
0.18	224.5
0.23	228.4
0.28	231.0
0.43	234.4
0.53	235.6
1.05	239.6
2.30	243.0
4.05	245.6
9.05	249.7
16.05	252.4
25.05	255.4
36.05	257.8
49.05	259.4
64.05	260.1
81.05	260.9
100.07	261.1
121.07	261.5
144.07	262.7
180.07	263.8
300.07	267.8
420.13	269.5

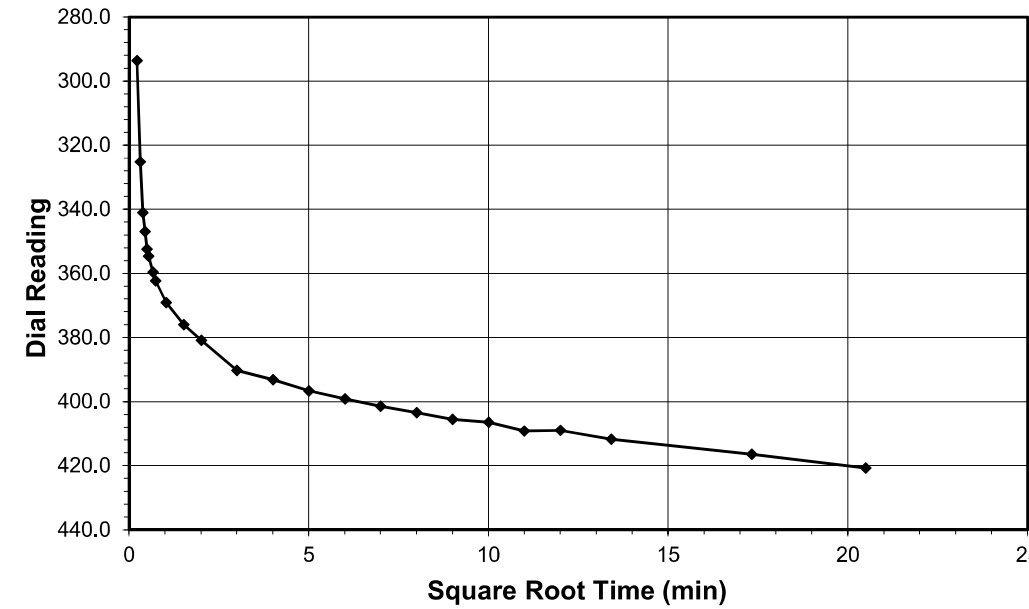


Tested By 129-04-0411 Date 1/16/19 Checked By GEM Date 1/29/19

ONE DIMENSIONAL CONSOLIDATION
AASHTO T-216

Client Kleinfelder Boring No. L_7250
 Client Project U-5813 Depth (ft) 0-2.0
 Project No. R-2019-011-001 Sample No. ST-2
 Lab ID R-2019-011-001-003 Visual Description TAN SILTY CLAY

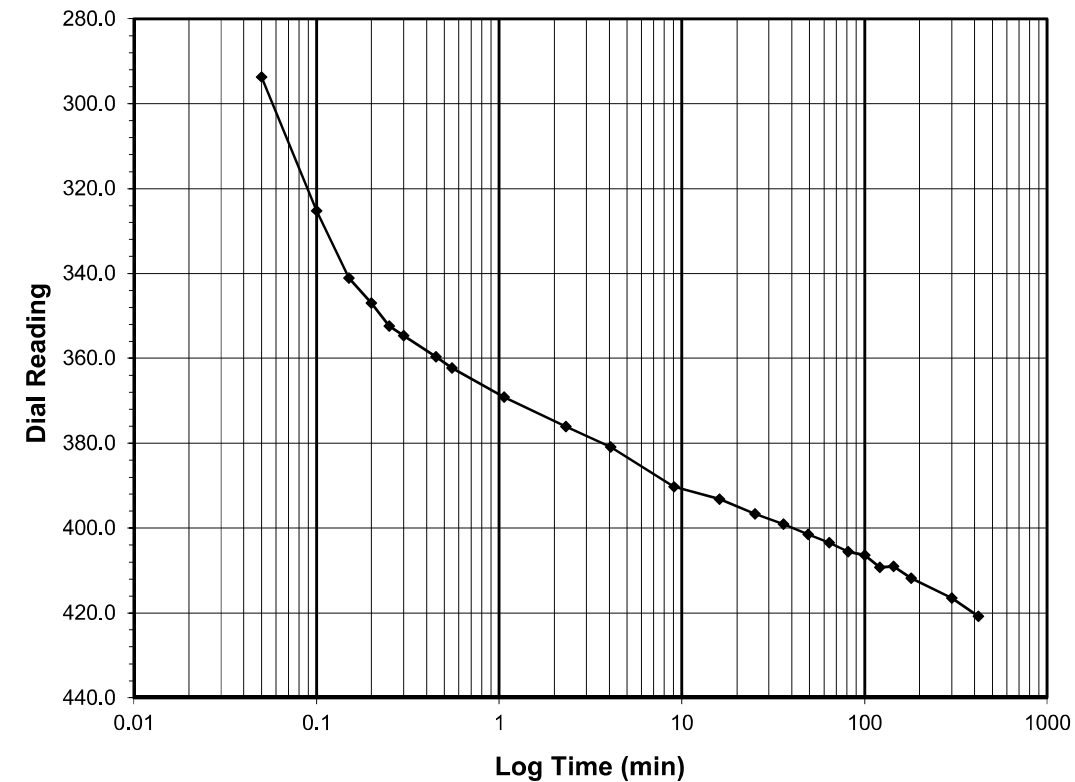
Sample Conditions: UNDISTURBED, INUNDATED AND DOUBLE DRAINED



Test Load (tsf) 0.5-1.0
 Final Reading (div) 420.8
 Consolidometer No. R553
 1 Division (in) 0.0001

Start Date 1/17/19
 Start Time 2:07:15

Elapsed Time (min)	Dial Reading (div)
Initial	269.5
0.05	293.7
0.10	325.2
0.15	341.1
0.20	346.9
0.25	352.4
0.30	354.6
0.45	359.6
0.55	362.3
1.07	369.2
2.32	376.1
4.07	380.9
9.07	390.3
16.07	393.2
25.07	396.7
36.07	399.1
49.07	401.5
64.07	403.4
81.07	405.5
100.07	406.4
121.07	409.2
144.07	409.0
180.08	411.8
300.08	416.4
420.15	420.8



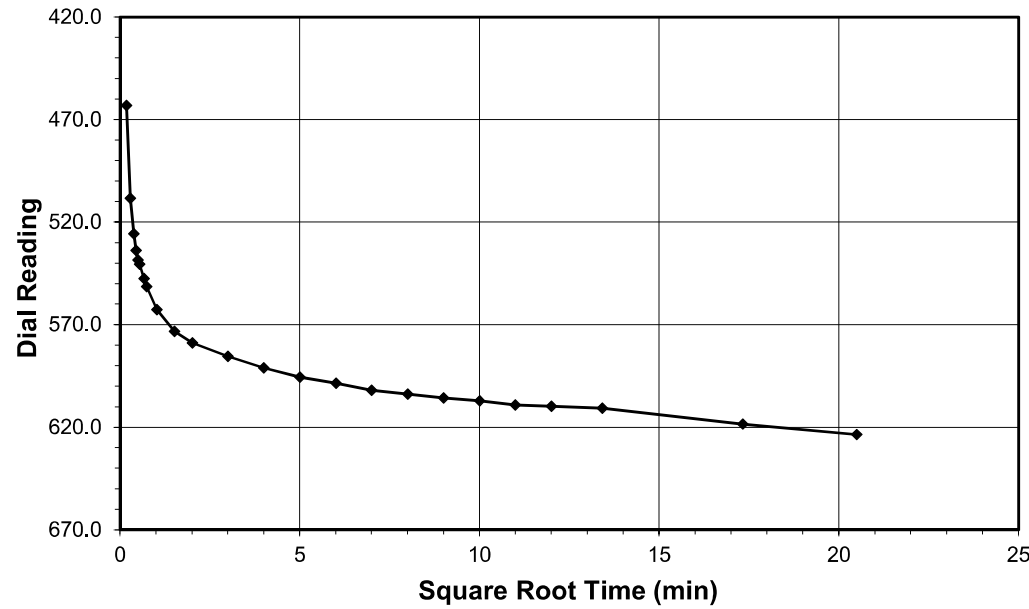
Tested By 129-04-0411 Date 1/17/19 Checked By GEM Date 1/29/19



ONE DIMENSIONAL CONSOLIDATION
AASHTO T-216

Client Kleinfelder Boring No. L_7250
 Client Project U-5813 Depth (ft) 0-2.0
 Project No. R-2019-011-001 Sample No. ST-2
 Lab ID R-2019-011-001-003 Visual Description TAN SILTY CLAY

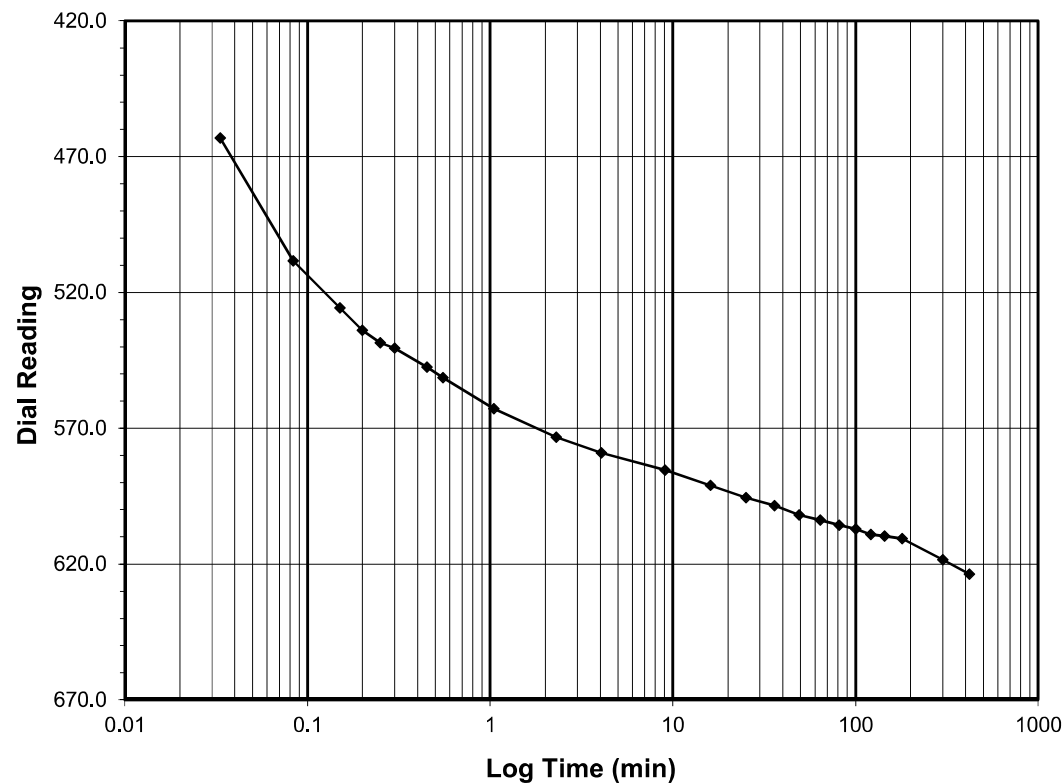
Sample Conditions: UNDISTURBED, INUNDATED AND DOUBLE DRAINED



Test Load (tsf) 1.0-2.0
 Final Reading (div) 623.7
 Consolidometer No. R553
 1 Division (in) 0.0001

Start Date 1/17/19
 Start Time 9:07:24

Elapsed Time (min)	Dial Reading (div)
Initial	420.8
0.03	463.2
0.08	508.4
0.15	525.7
0.20	533.9
0.25	538.5
0.30	540.5
0.45	547.5
0.55	551.4
1.05	562.8
2.30	573.3
4.05	579.0
9.05	585.4
16.05	591.0
25.07	595.6
36.07	598.6
49.07	601.9
64.07	603.8
81.07	605.7
100.07	607.2
121.07	609.2
144.07	609.7
180.07	610.7
300.07	618.4
420.22	623.7



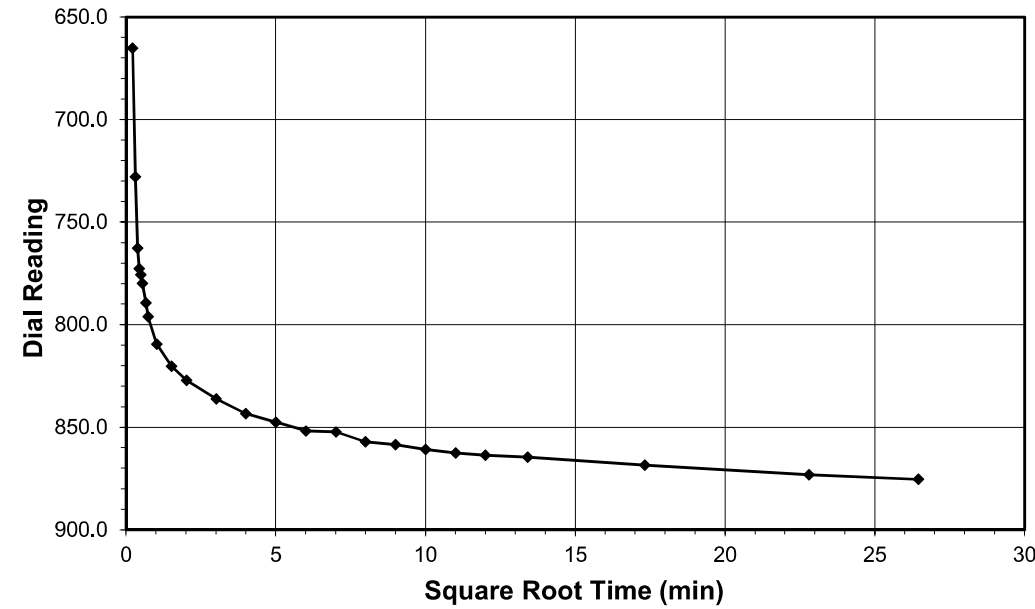
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ONE DIMENSIONAL CONSOLIDATION
AASHTO T-216

Client Kleinfelder Boring No. L_7250
 Client Project U-5813 Depth (ft) 0-2.0
 Project No. R-2019-011-001 Sample No. ST-2
 Lab ID R-2019-011-001-003 Visual Description TAN SILTY CLAY

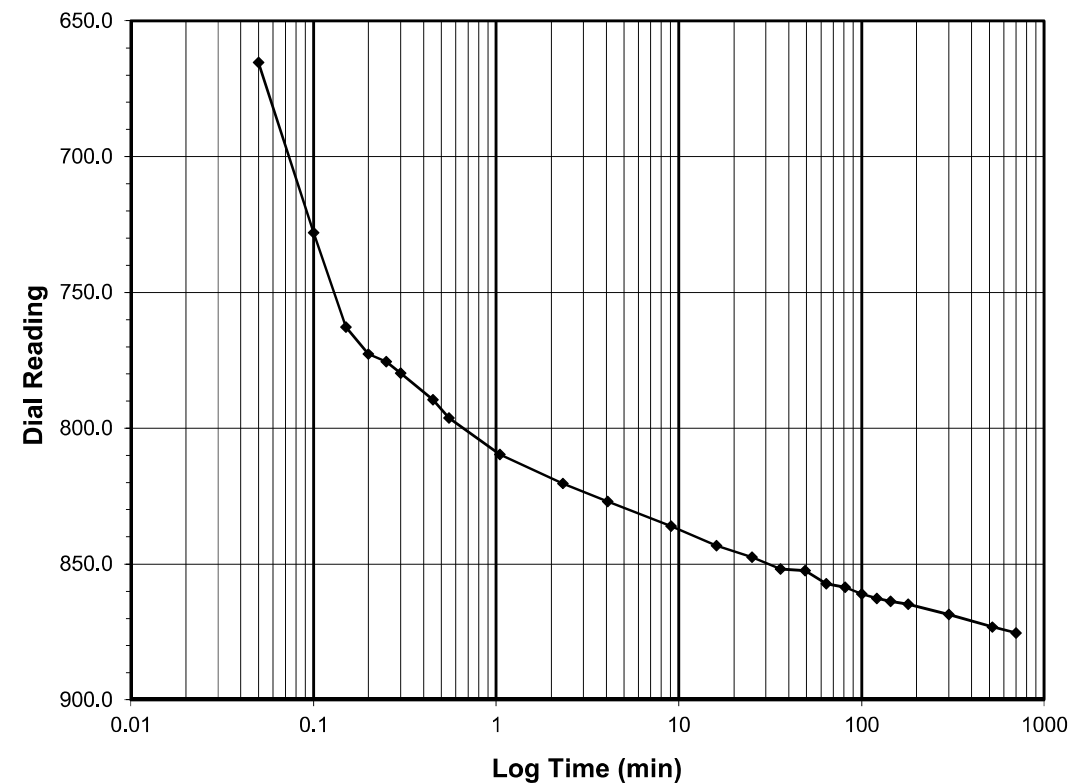
Sample Conditions: UNDISTURBED, INUNDATED AND DOUBLE DRAINED



Test Load (tsf) 2.0-4.0
 Final Reading (div) 875.4
 Consolidometer No. R553
 1 Division (in) 0.0001

Start Date 1/17/19
 Start Time 16:07:38

Elapsed Time (min)	Dial Reading (div)
Initial	623.7
0.05	665.3
0.10	728.0
0.15	762.7
0.20	772.7
0.25	775.6
0.30	779.8
0.45	789.5
0.55	796.2
1.05	809.6
2.32	820.3
4.07	827.1
9.07	836.1
16.07	843.3
25.07	847.5
36.07	851.9
49.07	852.4
64.07	857.2
81.07	858.6
100.07	861.0
121.07	862.7
144.07	863.7
180.08	864.7
300.08	868.5
520.08	873.2
700.08	875.4



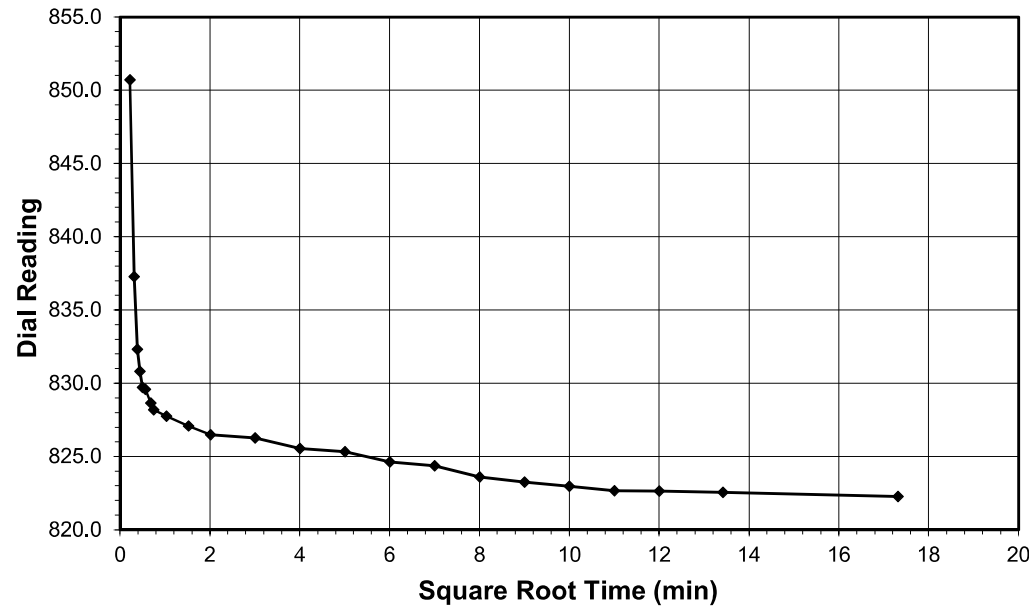
Tested By 129-04-0411 Date 1/17/19 Checked By GEM Date 1/29/19



ONE DIMENSIONAL CONSOLIDATION
AASHTO T-216

Client Kleinfelder Boring No. L_7250
 Client Project U-5813 Depth (ft) 0-2.0
 Project No. R-2019-011-001 Sample No. ST-2
 Lab ID R-2019-011-001-003 Visual Description TAN SILTY CLAY

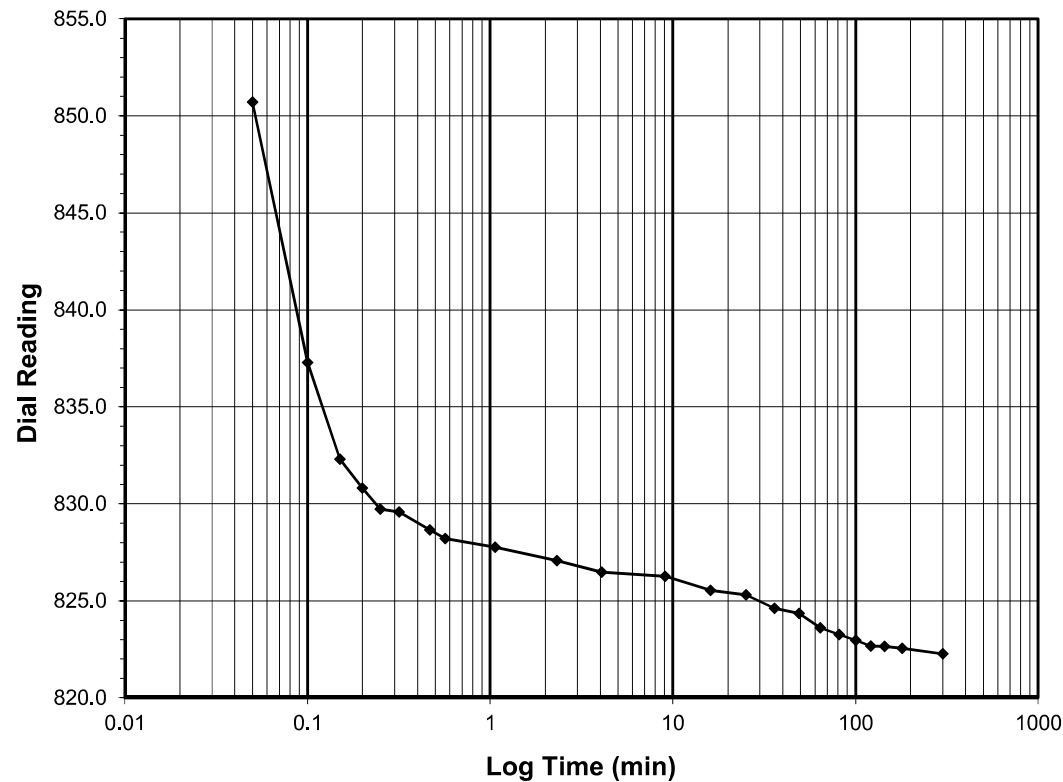
Sample Conditions: UNDISTURBED, INUNDATED AND DOUBLE DRAINED



Test Load (tsf) 4.0-1.0
Final Reading (div) 822.3
 Consolidometer No. **R553**
 1 Division (in) 0.0001

Start Date 1/18/19
 Start Time 4:08:03

Elapsed Time (min)	Dial Reading (div)
Initial	875.4
0.05	850.7
0.10	837.3
0.15	832.3
0.20	830.8
0.25	829.7
0.32	829.6
0.47	828.6
0.57	828.2
1.07	827.8
2.32	827.1
4.07	826.5
9.07	826.3
16.07	825.5
25.07	825.3
36.07	824.6
49.07	824.4
64.08	823.6
81.08	823.3
100.08	823.0
121.08	822.7
144.08	822.6
180.08	822.6
300.08	822.3



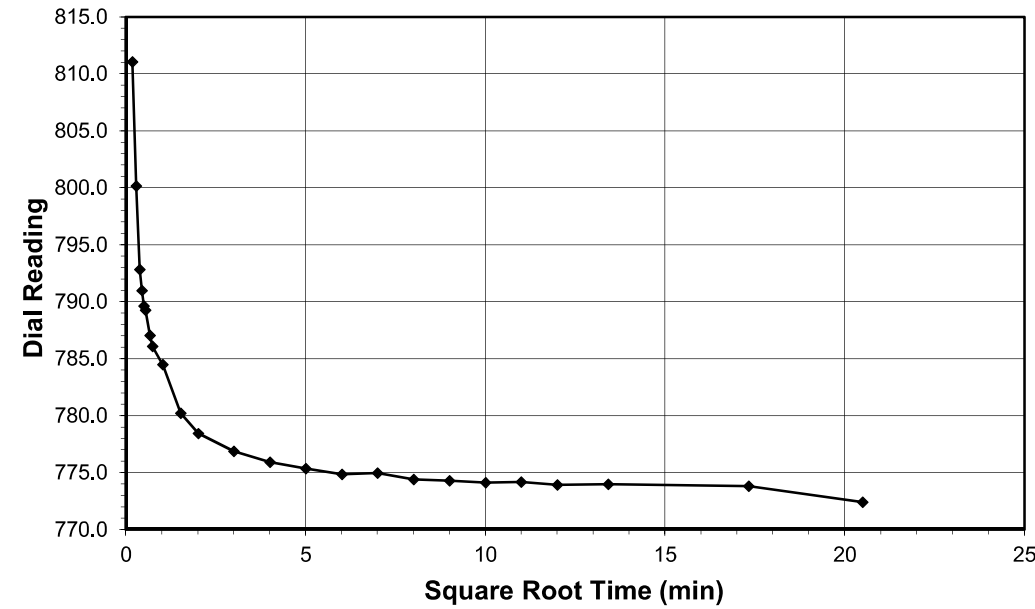
Tested By 129-04-0411 Date 1/18/19 Checked By GEM Date 1/29/19



ONE DIMENSIONAL CONSOLIDATION
AASHTO T-216

Client Kleinfelder Boring No. L_7250
 Client Project U-5813 Depth (ft) 0-2.0
 Project No. R-2019-011-001 Sample No. ST-2
 Lab ID R-2019-011-001-003 Visual Description TAN SILTY CLAY

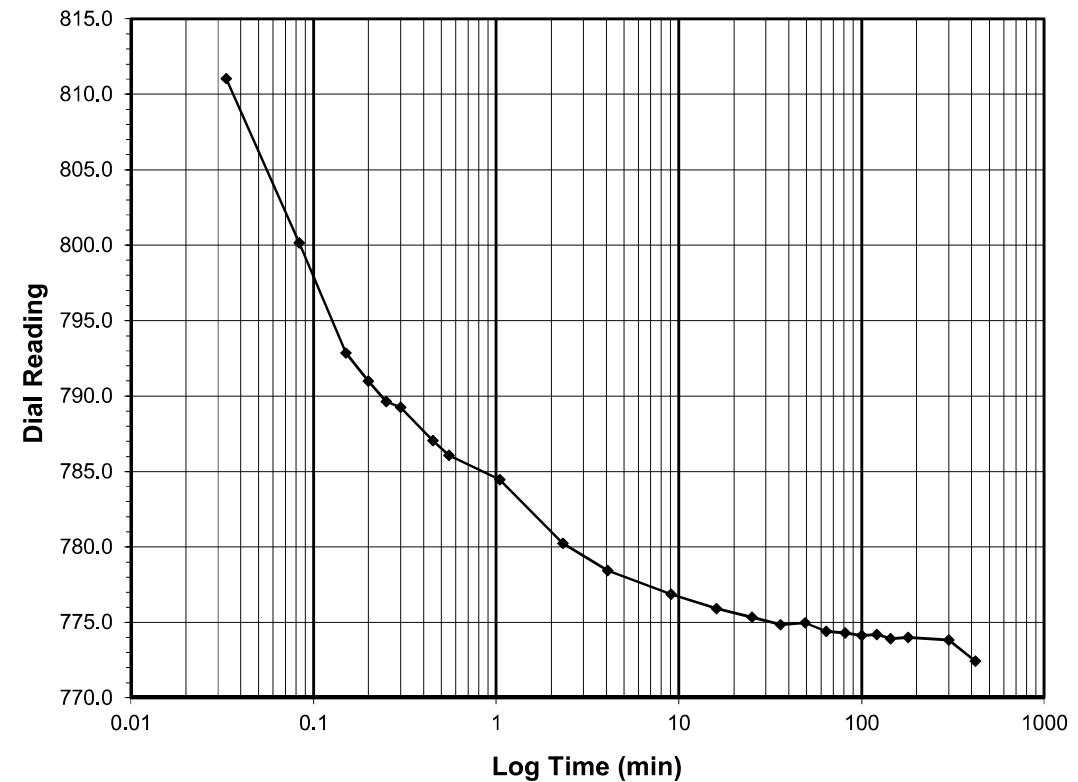
Sample Conditions: UNDISTURBED, INUNDATED AND DOUBLE DRAINED



Test Load (tsf) 1.0-0.25
Final Reading (div) 772.4
 Consolidometer No. **R553**
 1 Division (in) 0.0001

Start Date 1/18/19
 Start Time 11:08:15

Elapsed Time (min)	Dial Reading (div)
Initial	822.3
0.03	811.0
0.08	800.1
0.15	792.8
0.20	791.0
0.25	789.6
0.30	789.2
0.45	787.0
0.55	786.1
1.05	784.5
2.32	780.2
4.07	778.4
9.07	776.9
16.07	775.9
25.07	775.3
36.07	774.9
49.07	775.0
64.07	774.4
81.07	774.3
100.07	774.1
121.07	774.2
144.07	773.9
180.07	774.0
300.07	773.8
420.22	772.4



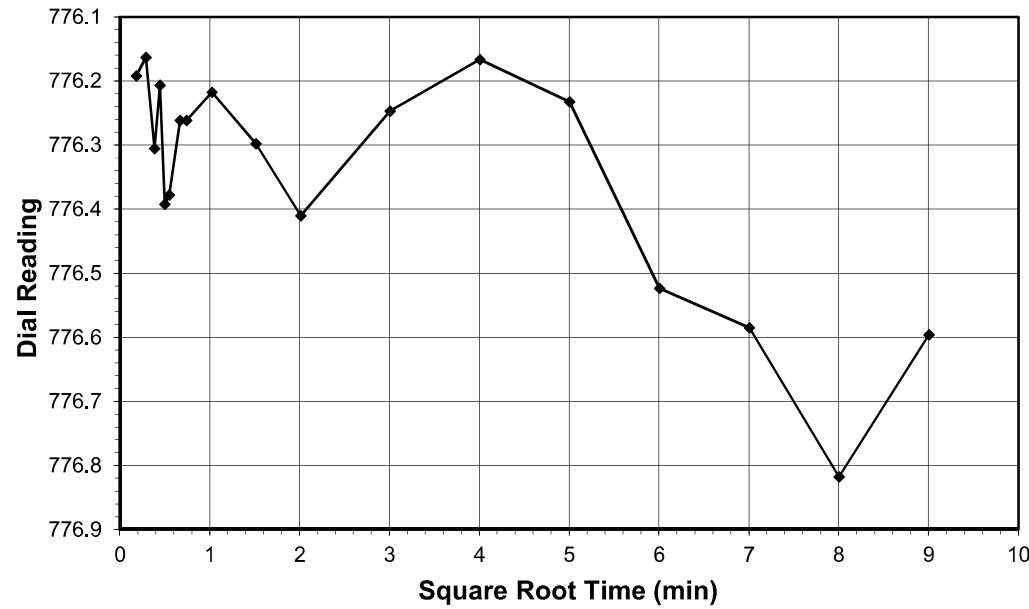
Tested By 129-04-0411 Date 1/18/19 Checked By GEM Date 1/29/19



ONE DIMENSIONAL CONSOLIDATION
AASHTO T-216

Client Kleinfelder Boring No. L_7250
 Client Project U-5813 Depth (ft) 0-2.0
 Project No. R-2019-011-001 Sample No. ST-2
 Lab ID R-2019-011-001-003 Visual Description TAN SILTY CLAY

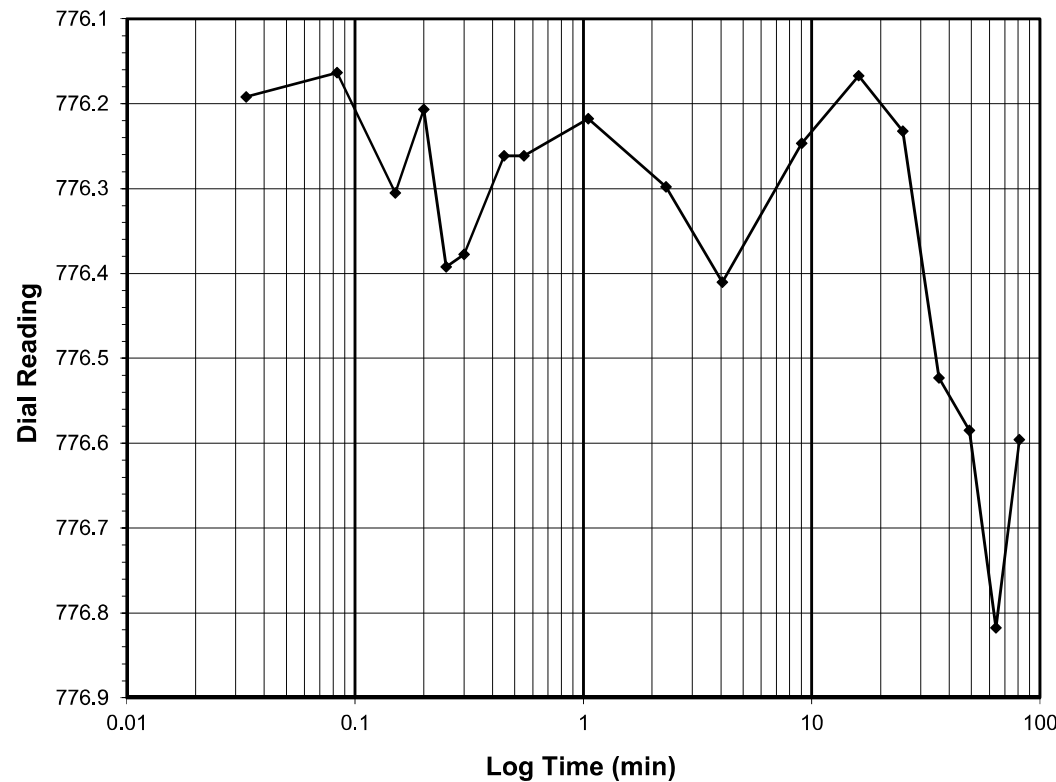
Sample Conditions: UNDISTURBED, INUNDATED AND DOUBLE DRAINED



Test Load (tsf) 0.25-0.5
 Final Reading (div) 776.6
 Consolidometer No. R553
 1 Division (in) 0.0001

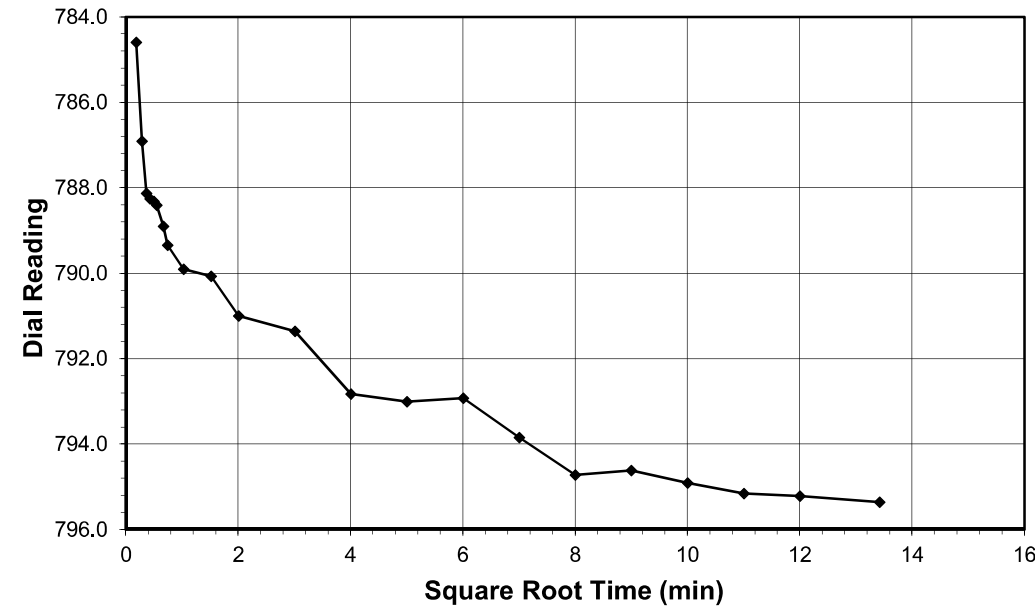
Start Date 1/18/19
 Start Time 18:08:29

Elapsed Time (min)	Dial Reading (div)
Initial	772.4
0.03	776.2
0.08	776.2
0.15	776.3
0.20	776.2
0.25	776.4
0.30	776.4
0.45	776.3
0.55	776.3
1.05	776.2
2.30	776.3
4.05	776.4
9.05	776.2
16.05	776.2
25.05	776.2
36.07	776.5
49.07	776.6
64.07	776.8
81.07	776.6



Client Kleinfelder Boring No. L_7250
 Client Project U-5813 Depth (ft) 0-2.0
 Project No. R-2019-011-001 Sample No. ST-2
 Lab ID R-2019-011-001-003 Visual Description TAN SILTY CLAY

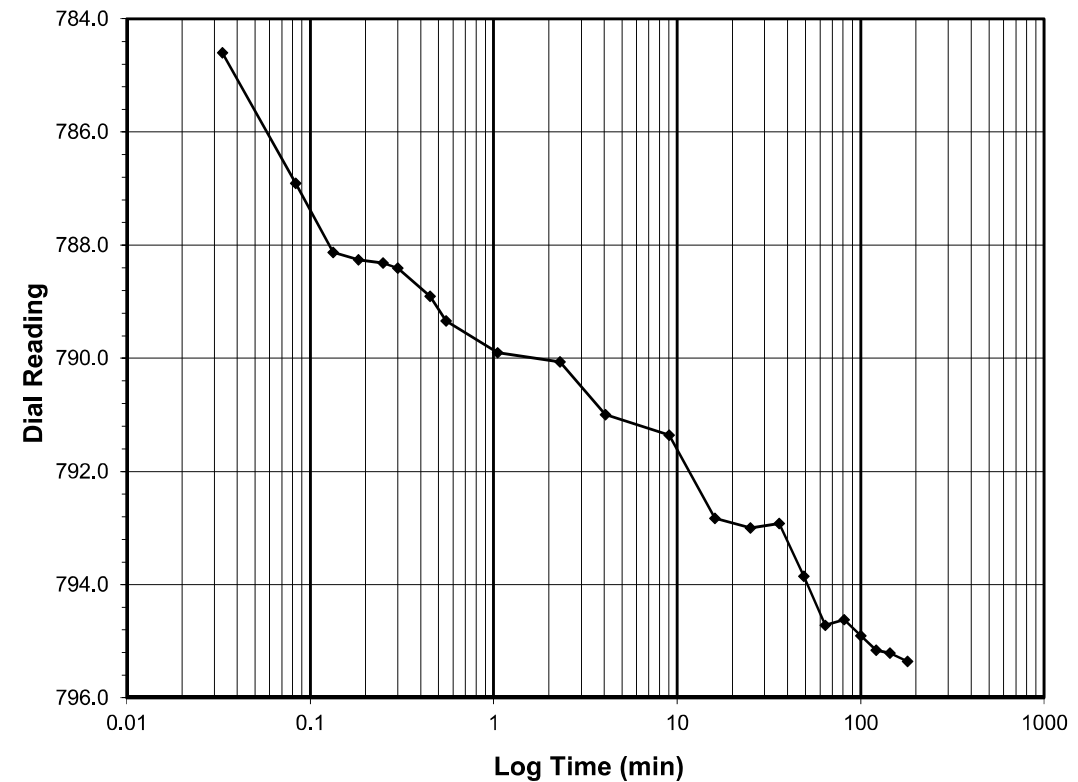
Sample Conditions: UNDISTURBED, INUNDATED AND DOUBLE DRAINED



Test Load (tsf) 0.5-1.0
 Final Reading (div) 795.4
 Consolidometer No. R553
 1 Division (in) 0.0001

Start Date 1/19/19
 Start Time 1:08:43

Elapsed Time (min)	Dial Reading (div)
Initial	776.6
0.03	784.6
0.08	786.9
0.13	788.1
0.18	788.3
0.25	788.3
0.30	788.4
0.45	788.9
0.55	789.3
1.05	789.9
2.30	790.1
4.05	791.0
9.05	791.4
16.05	792.8
25.05	793.0
36.07	792.9
49.07	793.8
64.07	794.7
81.07	794.6
100.07	794.9
121.07	795.2
144.07	795.2
180.07	795.4



Tested By 129-04-0411 Date 1/18/19 Checked By GEM Date 1/29/19

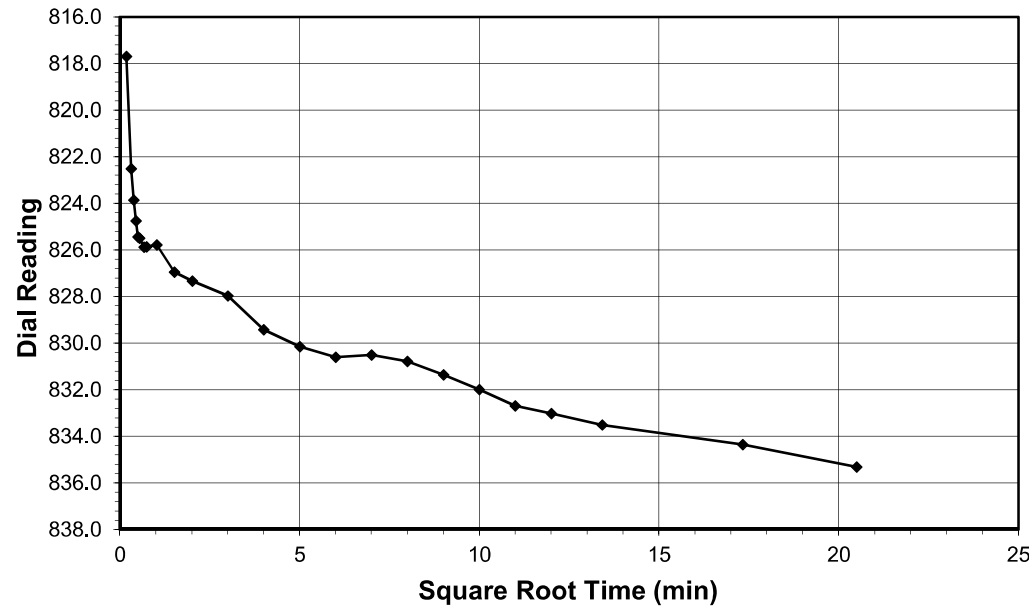
Tested By 129-04-0411 Date 1/19/19 Checked By GEM Date 1/29/19



ONE DIMENSIONAL CONSOLIDATION
AASHTO T-216

Client Kleinfelder Boring No. L_7250
 Client Project U-5813 Depth (ft) 0-2.0
 Project No. R-2019-011-001 Sample No. ST-2
 Lab ID R-2019-011-001-003 Visual Description TAN SILTY CLAY

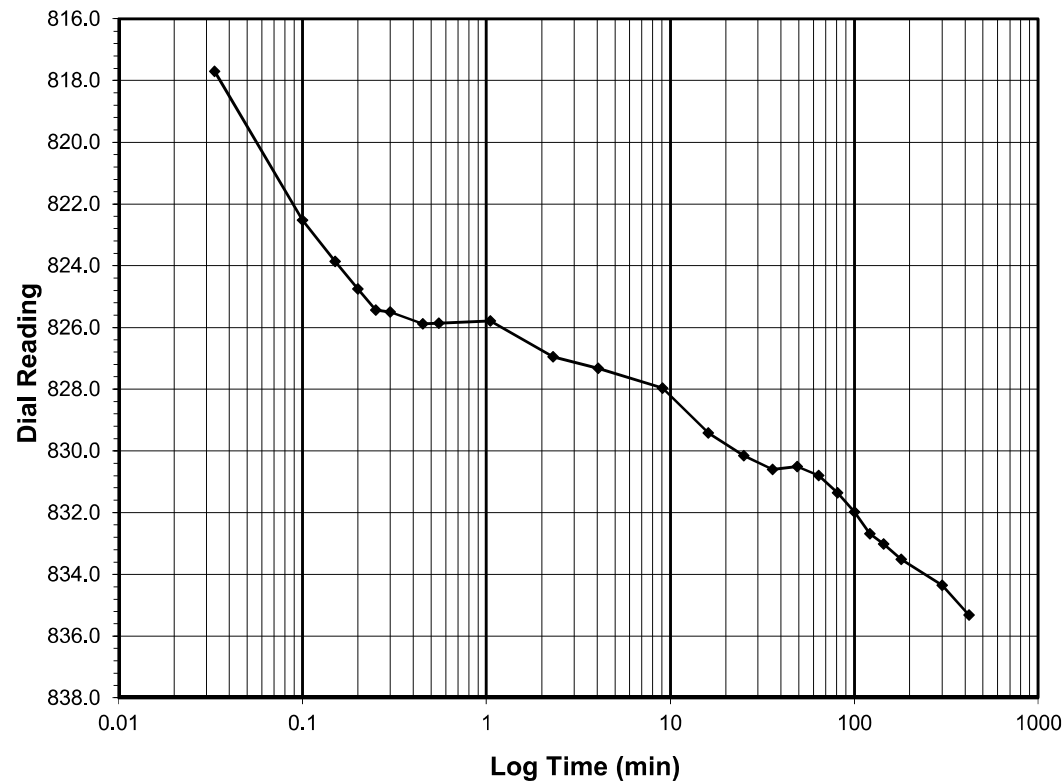
Sample Conditions: UNDISTURBED, INUNDATED AND DOUBLE DRAINED



Test Load (tsf) 1.0-2.0
Final Reading (div) 835.3
 Consolidometer No. **R553**
 1 Division (in) 0.0001

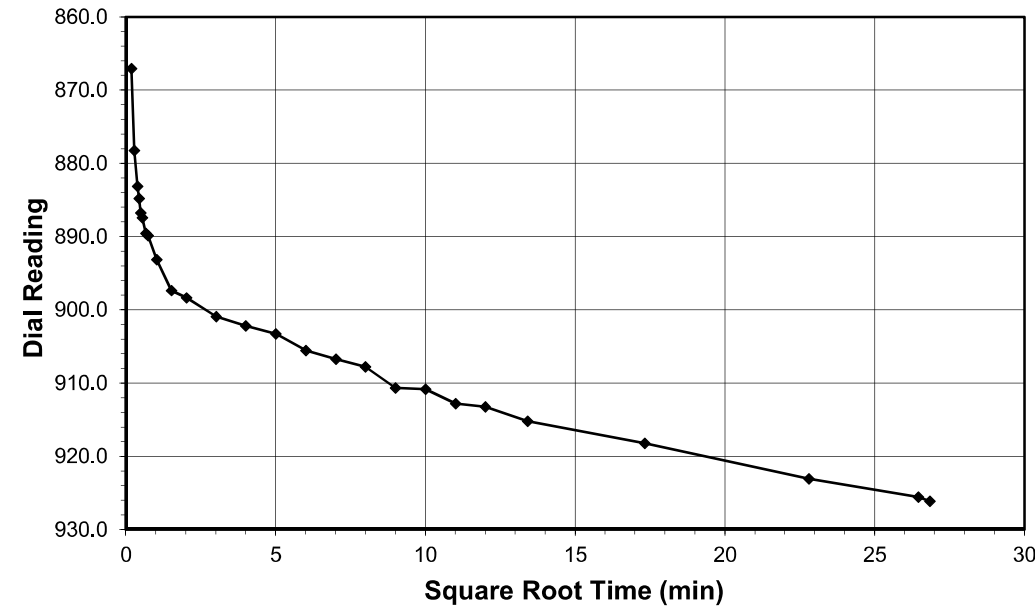
Start Date 1/19/19
 Start Time 8:08:52

Elapsed Time (min)	Dial Reading (div)
Initial	795.4
0.03	817.7
0.10	822.5
0.15	823.9
0.20	824.8
0.25	825.4
0.30	825.5
0.45	825.9
0.55	825.9
1.05	825.8
2.30	827.0
4.05	827.3
9.05	828.0
16.05	829.4
25.05	830.1
36.05	830.6
49.05	830.5
64.05	830.8
81.05	831.4
100.05	832.0
121.07	832.7
144.07	833.0
180.07	833.5
300.07	834.4
420.18	835.3



Client Kleinfelder Boring No. L_7250
 Client Project U-5813 Depth (ft) 0-2.0
 Project No. R-2019-011-001 Sample No. ST-2
 Lab ID R-2019-011-001-003 Visual Description TAN SILTY CLAY

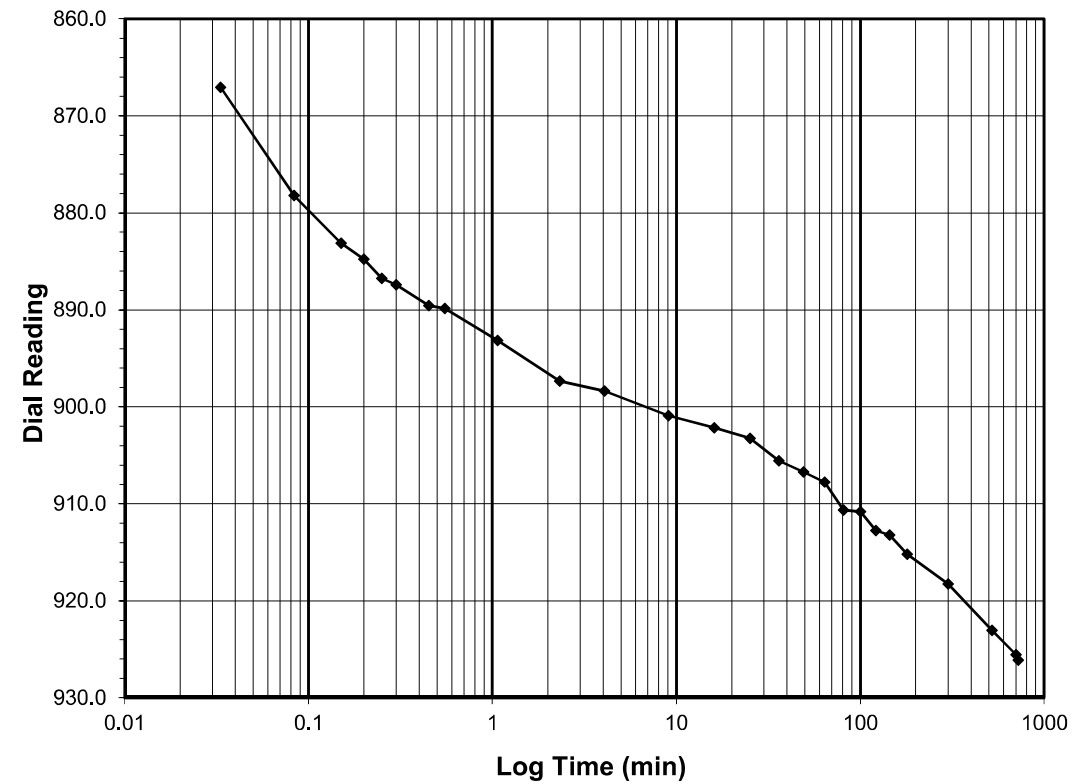
Sample Conditions: UNDISTURBED, INUNDATED AND DOUBLE DRAINED



Test Load (tsf) 2.0-4.0
Final Reading (div) 926.1
 Consolidometer No. **R553**
 1 Division (in) 0.0001

Start Date 1/19/19
 Start Time 15:09:03

Elapsed Time (min)	Dial Reading (div)
Initial	835.3
0.03	867.1
0.08	878.2
0.15	883.1
0.20	884.8
0.25	886.8
0.30	887.4
0.45	889.6
0.55	889.9
1.07	893.1
2.32	897.3
4.07	898.4
9.07	900.9
16.07	902.2
25.07	903.3
36.07	905.6
49.07	906.7
64.07	907.8
81.07	910.7
100.07	910.8
121.07	912.8
144.07	913.2
180.07	915.2
300.07	918.2
520.07	923.1
700.07	925.6
720.42	926.1



Tested By 129-04-0411 Date 1/19/19 Checked By GEM Date 1/29/19

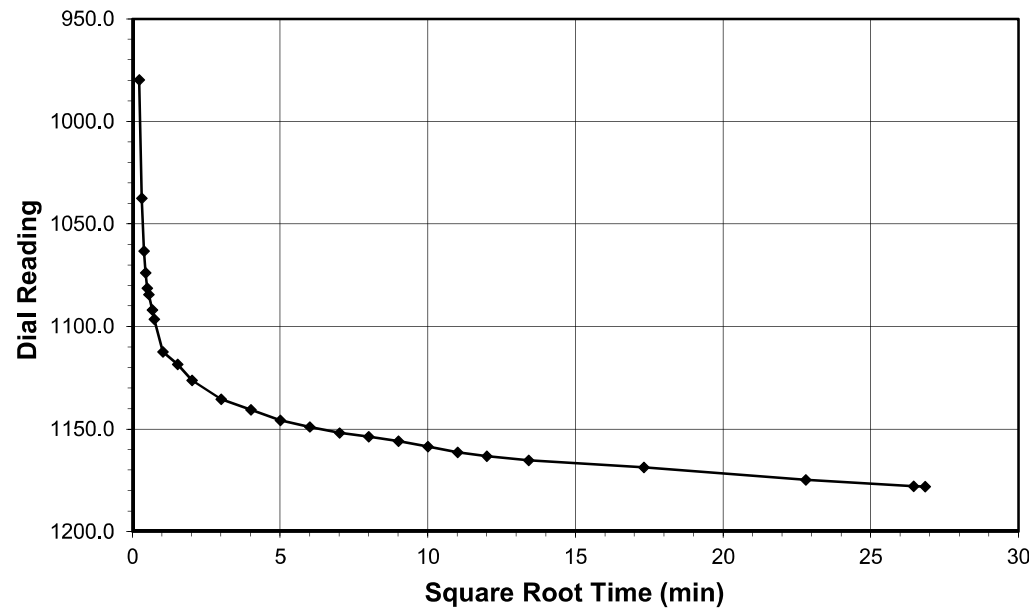
Tested By 129-04-0411 Date 1/19/19 Checked By GEM Date 1/29/19



ONE DIMENSIONAL CONSOLIDATION
AASHTO T-216

Client Kleinfelder Boring No. L_7250
 Client Project U-5813 Depth (ft) 0-2.0
 Project No. R-2019-011-001 Sample No. ST-2
 Lab ID R-2019-011-001-003 Visual Description TAN SILTY CLAY

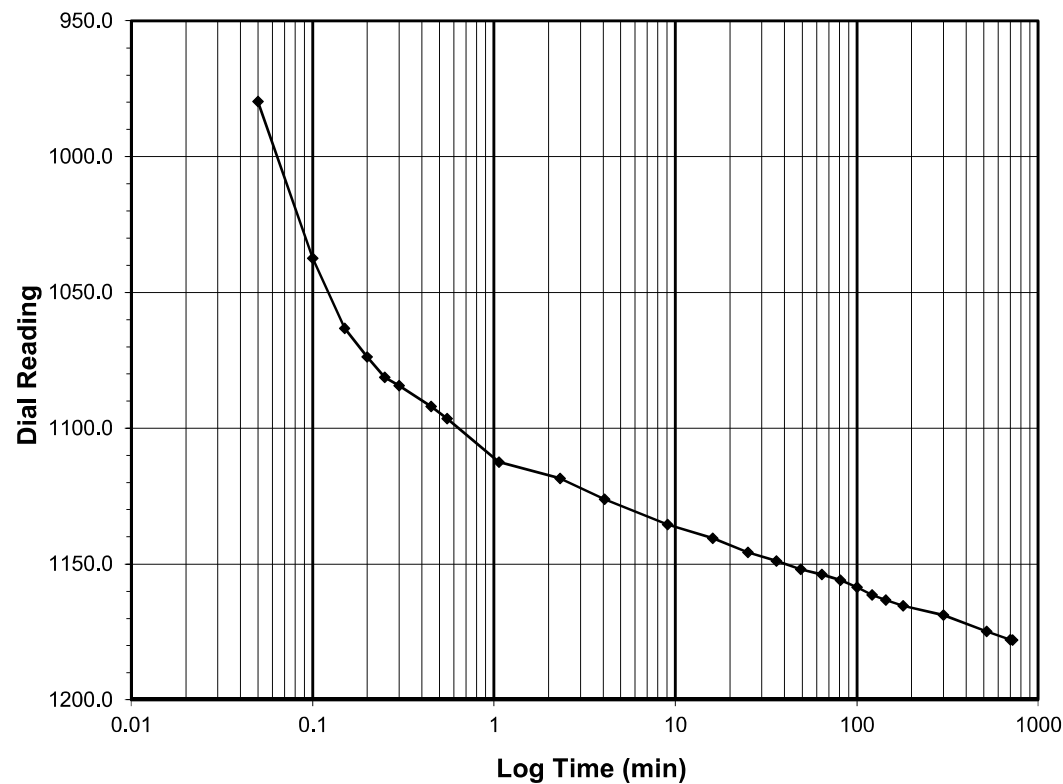
Sample Conditions: UNDISTURBED, INUNDATED AND DOUBLE DRAINED



Test Load (tsf) 4.0-8.0
Final Reading (div) 1178.1
 Consolidometer No. **R553**
 1 Division (in) 0.0001

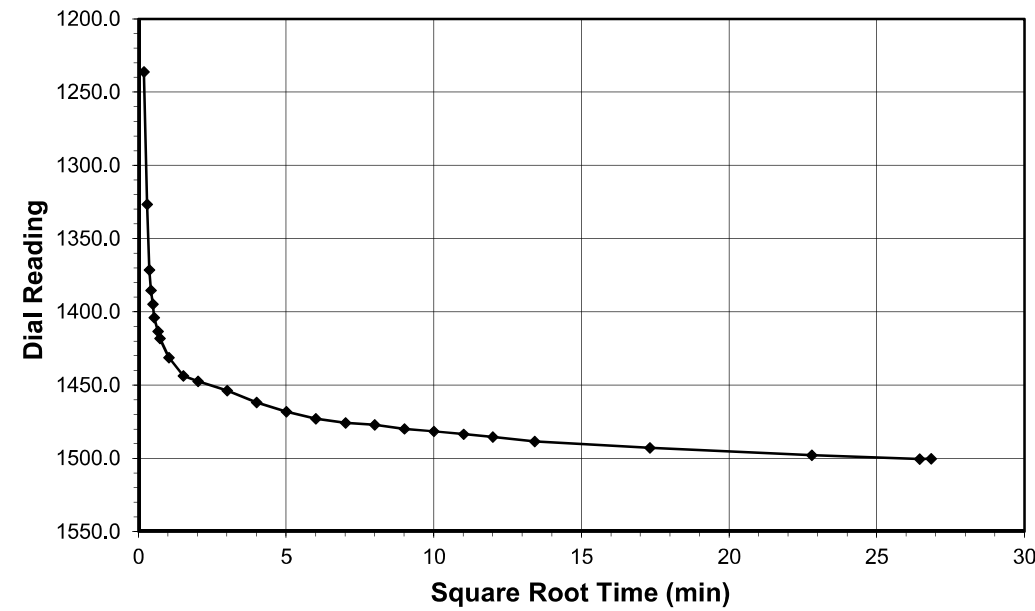
Start Date 1/20/19
 Start Time 3:09:29

Elapsed Time (min)	Dial Reading (div)
Initial	926.1
0.05	979.7
0.10	1037.4
0.15	1063.2
0.20	1073.7
0.25	1081.3
0.30	1084.4
0.45	1092.0
0.55	1096.4
1.07	1112.4
2.32	1118.4
4.07	1126.2
9.07	1135.5
16.07	1140.5
25.07	1145.7
36.07	1148.9
49.07	1152.0
64.07	1153.8
81.08	1155.9
100.08	1158.5
121.08	1161.3
144.08	1163.2
180.08	1165.3
300.08	1168.7
520.08	1174.8
700.08	1177.9
720.43	1178.1



Client Kleinfelder Boring No. L_7250
 Client Project U-5813 Depth (ft) 0-2.0
 Project No. R-2019-011-001 Sample No. ST-2
 Lab ID R-2019-011-001-003 Visual Description TAN SILTY CLAY

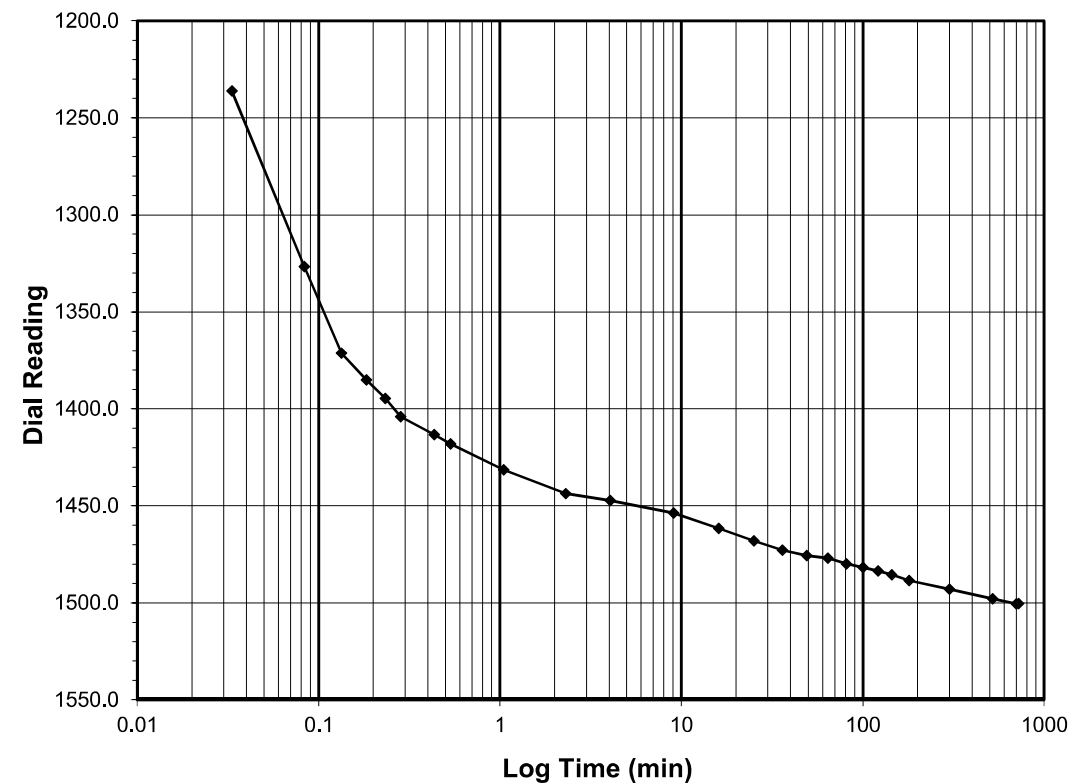
Sample Conditions: UNDISTURBED, INUNDATED AND DOUBLE DRAINED



Test Load (tsf) 8.0-16.0
Final Reading (div) 1500.4
 Consolidometer No. **R553**
 1 Division (in) 0.0001

Start Date 1/20/19
 Start Time 15:09:55

Elapsed Time (min)	Dial Reading (div)
Initial	1178.1
0.03	1236.3
0.08	1326.7
0.13	1371.3
0.18	1385.2
0.23	1394.7
0.28	1404.1
0.43	1413.3
0.53	1418.1
1.05	1431.3
2.30	1443.8
4.05	1447.3
9.05	1453.7
16.05	1461.7
25.05	1468.0
36.05	1472.9
49.05	1475.7
64.05	1477.0
81.05	1479.9
100.05	1481.7
121.05	1483.6
144.05	1485.5
180.05	1488.5
300.05	1492.9
520.05	1497.9
700.05	1500.4
720.35	1500.4



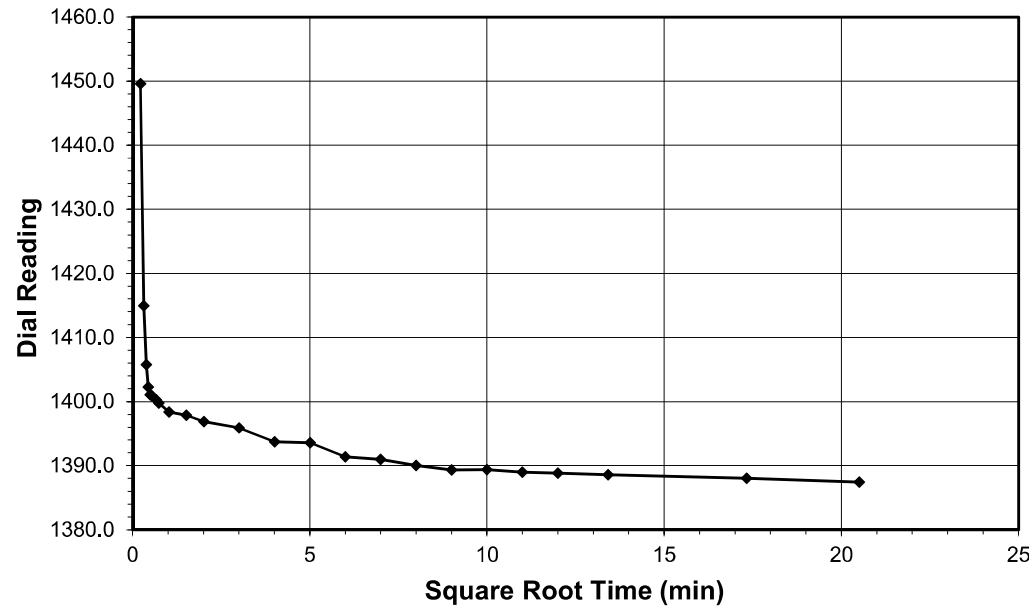
Tested By 129-04-0411 Date 1/20/19 Checked By GEM Date 1/29/19

Tested By 129-04-0411 Date 1/20/19 Checked By GEM Date 1/29/19

ONE DIMENSIONAL CONSOLIDATION
AASHTO T-216

Client Kleinfelder Boring No. L_7250
 Client Project U-5813 Depth (ft) 0-2.0
 Project No. R-2019-011-001 Sample No. ST-2
 Lab ID R-2019-011-001-003 Visual Description TAN SILTY CLAY

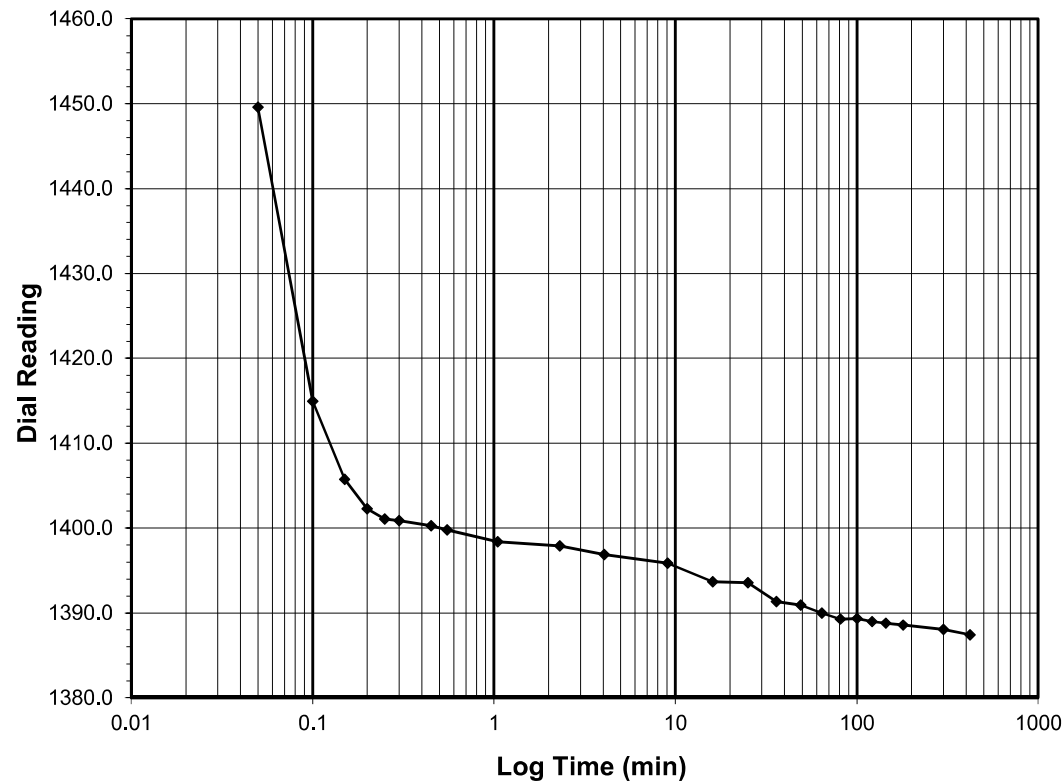
Sample Conditions: UNDISTURBED, INUNDATED AND DOUBLE DRAINED



Test Load (tsf) 16.0-4.0
 Final Reading (div) 1387.4
 Consolidometer No. R553
 1 Division (in) 0.0001

Start Date 1/21/19
 Start Time 3:10:17

Elapsed Time (min)	Dial Reading (div)
Initial	1500.4
0.05	1449.6
0.10	1414.9
0.15	1405.8
0.20	1402.3
0.25	1401.1
0.30	1400.9
0.45	1400.3
0.55	1399.8
1.05	1398.4
2.30	1397.9
4.05	1396.9
9.07	1395.9
16.07	1393.7
25.07	1393.6
36.07	1391.3
49.07	1390.9
64.07	1390.0
81.07	1389.3
100.07	1389.4
121.07	1389.0
144.07	1388.8
180.07	1388.6
300.07	1388.0
420.33	1387.4

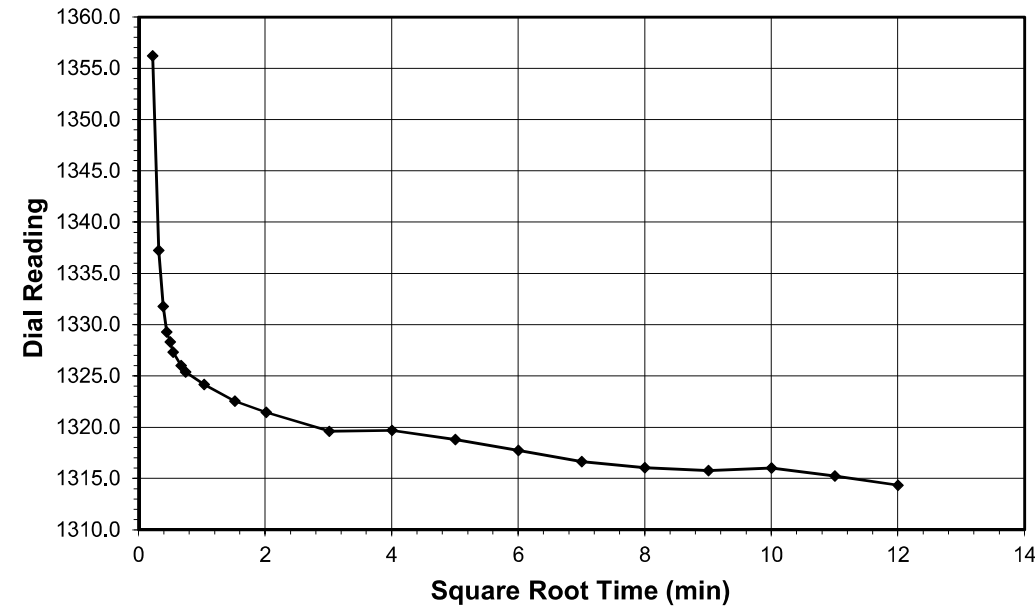


Tested By 129-04-0411 Date 1/21/19 Checked By GEM Date 1/29/19

ONE DIMENSIONAL CONSOLIDATION
AASHTO T-216

Client Kleinfelder Boring No. L_7250
 Client Project U-5813 Depth (ft) 0-2.0
 Project No. R-2019-011-001 Sample No. ST-2
 Lab ID R-2019-011-001-003 Visual Description TAN SILTY CLAY

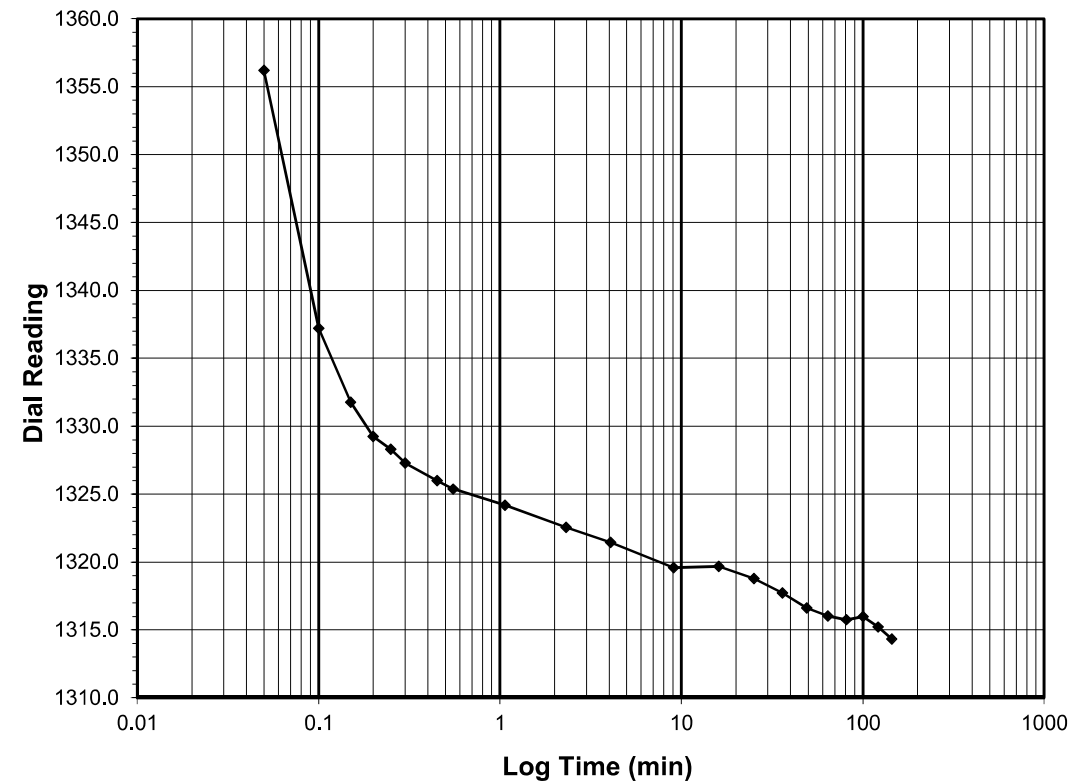
Sample Conditions: UNDISTURBED, INUNDATED AND DOUBLE DRAINED



Test Load (tsf) 4.0-1.0
 Final Reading (div) 1314.3
 Consolidometer No. R553
 1 Division (in) 0.0001

Start Date 1/21/19
 Start Time 10:10:37

Elapsed Time (min)	Dial Reading (div)
Initial	1387.4
0.05	1356.2
0.10	1337.2
0.15	1331.8
0.20	1329.3
0.25	1328.3
0.30	1327.3
0.45	1326.0
0.55	1325.4
1.07	1324.2
2.32	1322.6
4.07	1321.4
9.07	1319.6
16.07	1319.7
25.07	1318.8
36.07	1317.7
49.07	1316.6
64.07	1316.1
81.07	1315.8
100.07	1316.0
121.07	1315.2
144.08	1314.3

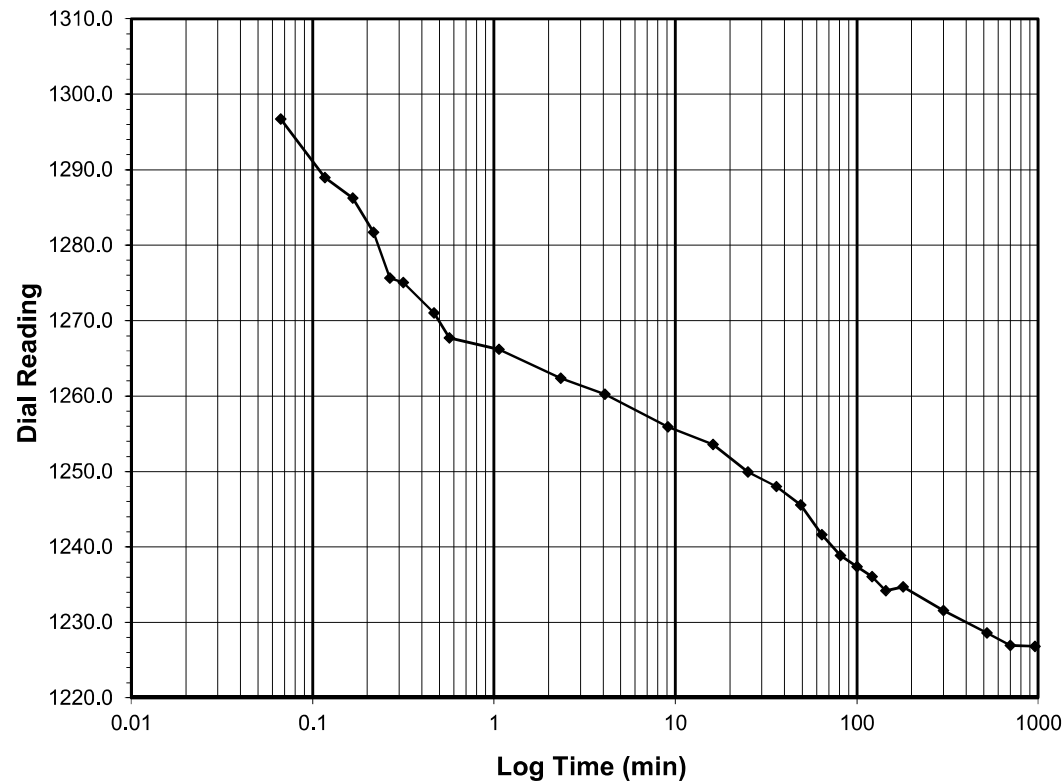
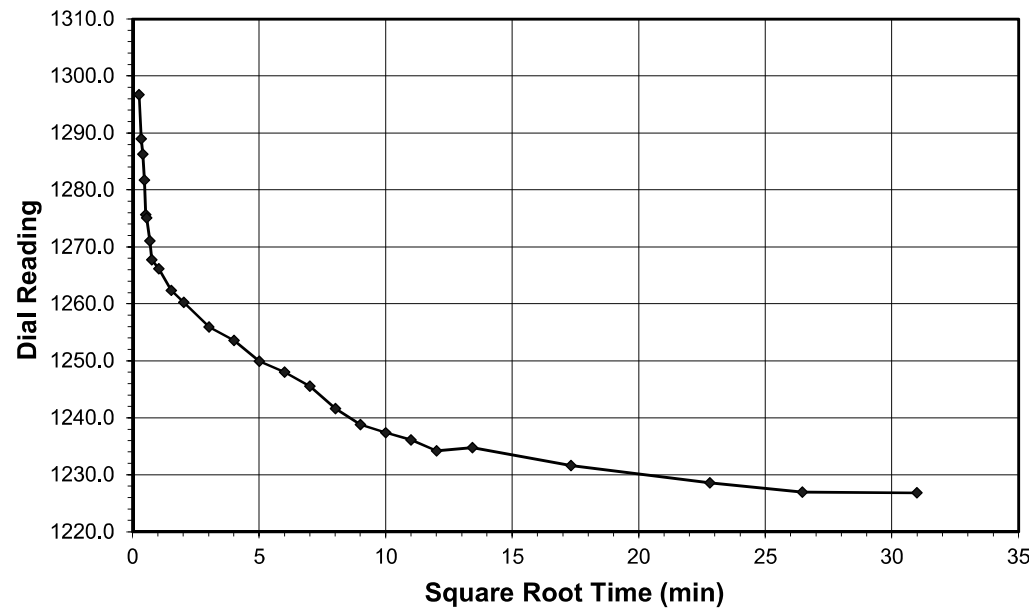


Tested By 129-04-0411 Date 1/21/19 Checked By GEM Date 1/29/19

ONE DIMENSIONAL CONSOLIDATION
AASHTO T-216

Client: Kleinfelder Boring No.: L_7250
 Client Project: U-5813 Depth (ft): 0-2.0
 Project No.: R-2019-011-001 Sample No.: ST-2
 Lab ID: R-2019-011-001-003 Visual Description: TAN SILTY CLAY

Sample Conditions: UNDISTURBED, INUNDATED AND DOUBLE DRAINED



Test Load (tsf): 1.0-0.25
Final Reading (div): 1226.8
 Consolidometer No.: **R553**
 1 Division (in): 0.0001

Start Date: 1/21/19
 Start Time: 12:42:26

Elapsed Time (min)	Dial Reading (div)
Initial	1314.3
0.07	1296.7
0.12	1289.0
0.17	1286.3
0.22	1281.7
0.27	1275.6
0.32	1275.1
0.47	1271.0
0.57	1267.7
1.07	1266.2
2.33	1262.4
4.08	1260.2
9.08	1255.9
16.08	1253.6
25.08	1249.9
36.08	1248.0
49.08	1245.5
64.08	1241.6
81.08	1238.9
100.08	1237.4
121.08	1236.1
144.10	1234.2
180.10	1234.7
300.10	1231.6
520.10	1228.6
700.10	1226.9
960.10	1226.8

California Bearing Ratio Test Report - AASHTO T-193

Load Penetration Curve

SOAKED CBR TEST RESULTS				
Results	CBR (A)	CBR (B)		
0.1" Penetration	5.7	5.8		
0.2" Penetration	6.1	6.1		
Initial Moisture (%)	18.9	18.9		
Initial Density (pcf)	99.6	103.1		
Final Moisture (%)	21.6	21.9		
Final Density (pcf)	100.3	103.9		
Project Information		Natural Moisture (%)		
Project Name:	U-5813	BS-2	20.6	
Client Name:	Kleinfelder			
Project Number:	20190942.009A	Percent Swell	A	B
Date Received:	1/13/2019	BS-2	1.490	1.470
Project Location:	N/A			
Sample Information		Proctor Value (AASHTO T-193)		
Sample Number	Classification	Max. Dry Density (pcf)	Optimum Moisture %	
BS-2	A-7-5	104.2	18.9	
Sample Number	Sample Location	AASHTO	% - #270	
BS-2	RPDA_1150	A-7-5	77.0	

Tested By 129-04-0411 Date 1/21/19 Checked By GEM Date 1/29/19